## JULIE D. KOHLER

PLEASE REPLY TO: Bridgeport
WRITER'S DIRECT DIAL: (203) 337-4157
E-Mail Address: jkohler@cohenandwolf.com
February 26, 2015
Attorney Melanie Bachman
Acting Executive Director
Connecticut Siting Council
Ten Franklin Square
New Britain, CT 06051

## Re: Notice of Exempt Modification <br> AT\&T Towers/T-Mobile equipment upgrade Site ID CT11349A <br> 453 Loon Meadow Road, Norfolk Connecticut

Dear Attorney Bachman:
This office represents T-Mobile Northeast LLC ("T-Mobile") and has been retained to file exempt modification filings with the Connecticut Siting Council on its behalf.

In this case, AT\&T Towers owns the existing guyed tower and related facility located at 453 Loon Meadow Road, Norfolk, Connecticut (Latitude: 42.009073; Longitude: -73.180934). T-Mobile intends to remove three (3) antennas and add three (3) antennas and related equipment at this existing telecommunications facility in Norfolk ("Norfolk Facility"). Please accept this letter as notification, pursuant to R.C.S.A. § 16-50j-73, of construction which constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, copies of this letter are being sent to the First Selectman, Susan M. Dyer, and the property owner, AT\&T Capital Services, Inc.

The existing Norfolk Facility consists of a 160 foot tall guyed tower, approved as a replacement tower by the Council in Petition No. 106. ${ }^{1}$ T-Mobile plans to remove three (3) antennas and add three (3) antennas at a centerline of 120 feet. T-Mobile will replace an equipment cabinet and mount three (3) RRU's (remote radio units) inside the equipment shelter. T-Mobile will also add coax cable to follow the existing coax cable inside the cable tray. (See the plans revised to September 15, 2014 attached hereto as Exhibit A). The existing Norfolk Facility is structurally capable of supporting T-Mobile's proposed modifications, as indicated in the structural analysis dated January 23, 2015 and attached hereto as Exhibit B.

The planned modifications to the Norfolk Facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modification will not increase the height of the tower. T-Mobile's proposed modifications will be installed at a centerline of 120 feet, merely modifying existing
[^0]February 26, 2015
Site ID CT11349A
Page 2
antennas located at the same 120 foot elevation. The enclosed tower drawing confirms that the proposed modification will not increase the height of the tower.
2. The proposed modifications will not require an extension of the site boundaries. T-Mobile's modifications are all within the existing compound area as shown on Sheets A-1 and A-2.
3. The proposed modification to the Norfolk Facility will not increase the noise levels at the existing facility by six decibels or more.
4. The operation of the replacement antennas will not increase the total radio frequency (RF) power density, measured at the base of the tower, to a level at or above the applicable standard. According to a Radio Frequency Emissions Analysis Report prepared by EBI dated February 24, 2015, T-Mobile's operations would add $7.02 \%$ of the FCC Standard. Therefore, the calculated "worst case" power density for the planned combined operation at the site including all of the proposed antennas would be $29.96 \%$ of the FCC Standard as calculated for a mixed frequency site as evidenced by the engineering exhibit attached hereto as Exhibit C.

For the foregoing reasons, T-Mobile respectfully submits that the proposed replacement/additional antennas and equipment at the Norfolk Facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Upon acknowledgement by the Council of this proposed exempt modification, T-Mobile shall commence construction approximately sixty days from the date of the Council's notice of acknowledgement.

Sincerely,


Julie D. Kohler, Esq.
cc: Town of Norfolk, First Selectman Susan M. Dyer
AT\&T Towers
AT\&T Capital Services, Inc.
Elizabeth Jamieson, Transcend Wireless

EXHIBIT A








AT\&T Towers
2300 Northlake Center Dr. Ste 405
Tucker, GA 30084-4032
(770) 708-6130

GPD GROUP.

Kevin Clements
520 South Main Street, Suite 2531
Akron, OH 44311
(330) 572-3546
kclements@gpdgroup.com

GPD\# 2015723.01.SNET020.03
January 23, 2015

## STRUCTURAL ANALYSIS REPORT

| AT\&T DESIGNATION: | Site USID: <br> Site FA: <br> Site Name: <br> AT\&T Project: | SNET020 <br> 10137487 <br> NORFOLK |
| :--- | :--- | :--- |
| 1) Wireline T-Mobile Modification 09.10 .2014 |  |  |

Ms. Julie Overman,

GPD is pleased to submit this Structural Analysis Report to determine the structural integrity of the aforementioned tower. The purpose of the analysis is to determine the suitability of the tower with the existing and proposed loading configuration detailed in the analysis report.

## Analysis Results

| Tower Stress Level with Proposed Equipment: | $55.5 \%$ | Pass |
| :--- | :--- | :--- |
| Foundation Ratio with Proposed Equipment: | $98.3 \%$ | Pass |

We at GPD appreciate the opportunity of providing our continuing professional services to you and AT\&T Towers. If you have any questions or need further assistance on this or any other projects please do not hesitate to call.


## SUMMARY \& RESULTS

The purpose of this analysis was to verify whether the existing structure is capable of carrying the proposed loading configuration as specified by AT\&T Mobility to AT\&T Towers. This report was commissioned by Ms. Julie Overman of AT\&T Towers.

The proposed coax shall be installed in a 3 on 4 configuration along Face $C$ for the results of this analysis to be valid. See Appendix C for the proposed coax layout.

## TOWER SUMMARY AND RESULTS

| Member | Capacity | Results |
| :--- | :---: | :---: |
| Leg | $43.2 \%$ | Pass |
| Diagonal | $55.5 \%$ | Pass |
| Horizontal | $33.9 \%$ | Pass |
| Top Girt | $4.6 \%$ | Pass |
| Guy Wires | $54.6 \%$ | Pass |
| Top Guy Pull-Off | $28.2 \%$ | Pass |
| Bottom Guy Pull-Off | $19.1 \%$ | Pass |
| Torque Arm Top | $17.5 \%$ | Pass |
| Torque Arm Bottom | $12.4 \%$ | Pass |
| Bolt Checks | $52.0 \%$ | Pass |
|  |  |  |
| Guy Anchor Foundation | $95.5 \%$ | Pass |
| Tower Foundation | $98.3 \%$ | Pass |

## ANALYSIS METHOD

tnxTower (Version 6.1.4.1), a commercially available software program, was used to create a three-dimensional model of the tower and calculate primary member stresses for various dead, live, wind, and ice load cases. Selected output from the analysis is included in Appendix B. The following table details the information provided to complete this structural analysis. This analysis is solely based on this information and is being completed without the benefit of a detailed site visit.

## DOCUMENTS PROVIDED

| Document | Remarks | Source |
| :--- | :--- | :---: |
| Notice of Co-lo Form (Part 2) | AT\&T Loading Document, uploaded 8/10/2014 | Siterra |
| Site Lease Application | AT\&T Application, uploaded 8/11/2014 | Siterra |
| Tower Design | Not Provided | N/A |
| Foundation Design | Not Provided | N/A |
| Geotechnical Report | WEI Project \# 2010-1212, dated 9/15/2010 | Siterra |
| Previous Structural Analysis | GPD Job \# 2013723.01.SNET020.02, dated 9/27/2013 | Siterra |
| Tower Mapping | GPD \& MTSI Northeast, dated 7/21/2010 | Siterra |
| Foundation Mapping | WEI Project \# 2010-1212, dated 9/15/2010 | Siterra |

## ASSUMPTIONS

This structural analysis is based on the theoretical capacity of the members and is not a condition assessment of the tower. This analysis is from information supplied, and therefore, its results are based on and are as accurate as that supplied data. GPD has made no independent determination, nor is it required to, of its accuracy. The following assumptions were made for this structural analysis.

1. The tower member sizes and shapes are considered accurate as supplied. The material grade is as per data supplied and/or as assumed and as stated in the materials section.
2. The antenna configuration is as supplied and/or as modeled in the analysis. It is assumed to be complete and accurate. All antennas, mounts, coax and waveguides are assumed to be properly installed and supported as per manufacturer requirements.
3. Some assumptions are made regarding antennas and mount sizes and their projected areas based on best interpretation of data supplied and of best knowledge of antenna type and industry practice.
4. All mounts, if applicable, are considered adequate to support the loading. No actual analysis of the mount(s) is performed. This analysis is limited to analyzing the tower only.
5. The soil parameters are as per data supplied or as assumed and stated in the calculations.
6. Foundations are properly designed and constructed to resist the original design loads indicated in the documents provided.
7. The tower and structures have been properly maintained in accordance with TIA Standards and/or with manufacturer's specifications.
8. All welds and connections are assumed to develop at least the member capacity unless determined otherwise and explicitly stated in this report.
9. All prior structural modifications are assumed to be as per data supplied/available and to have been properly installed.
10. Loading interpreted from photos is accurate to $\pm 5^{\prime} \mathrm{AGL}$, antenna size accurate to $\pm 3.3 \mathrm{sf}$, and coax equal to the number of existing antennas without reserve.
11. The existing loading was obtained from the previous structural analysis by GPD (Job \# 2013723.01.SNET020.02, dated 9/27/2013), the provided Site Lease Application, the provided Notice of CoLocation Form and site photos and is assumed to be accurate.
12. The proposed coax shall be installed in a 3 on 4 configuration along Face $C$ for the results of this analysis to be valid. See Appendix C for the proposed coax layout.
13. The azimuth orientation of Leg A was assumed to be at 340 degrees based on the tower mapping performed by GPD \& MTSI Northeast (dated 7/21/2010).
14. Foundation steel was not able to be determined through testing for the tower base. Therefore it was assumed that the foundation steel in place is equal to the minimum required steel per code specifications.

If any of these assumptions are not valid or have been made in error, this analysis may be affected, and GPD Group should be allowed to review any new information to determine its effect on the structural integrity of the tower.

## DISCLAIMER OF WARRANTIES

GPD GROUP has not performed a site visit to the tower to verify the member sizes or antenna/coax loading. If the existing conditions are not as represented on the tower elevation contained in this report, we should be contacted immediately to evaluate the significance of the discrepancy. This is not a condition assessment of the tower or foundation. This report does not replace a full tower inspection. The tower and foundations are assumed to have been properly fabricated, erected, maintained, in good condition, twist free, and plumb.

The engineering services rendered by GPD GROUP in connection with this Structural Analysis are limited to a computer analysis of the tower structure and theoretical capacity of its main structural members. No allowance was made for any damaged, bent, missing, loose, or rusted members (above and below ground). No allowance was made for loose bolts or cracked welds.

This analysis is limited to the designated maximum wind and seismic conditions per the governing tower standards and code. Wind forces resulting in tower vibrations near the structure's resonant frequencies were not considered in this analysis and are outside the scope of this analysis. Lateral loading from any dynamic response was not evaluated under a time-domain based fatigue analysis.

GPD GROUP does not analyze the fabrication of the structure (including welding). It is not possible to have all the very detailed information needed to perform a thorough analysis of every structural sub-component and connection of an existing tower. GPD GROUP provides a limited scope of service in that we cannot verify the adequacy of every weld, plate connection detail, etc. The purpose of this report is to assess the capability of adding appurtenances usually accompanied by transmission lines to the structure.

It is the owner's responsibility to determine the amount of ice accumulation in excess of the code specified amount, if any, that should be considered in the structural analysis.

The attached sketches are a schematic representation of the analyzed tower. If any material is fabricated from these sketches, the contractor shall be responsible for field verifying the existing conditions, proper fit, and clearance in the field. Any mentions of structural modifications are reasonable estimates and should not be used as a precise construction document. Precise modification drawings are obtainable from GPD GROUP, but are beyond the scope of this report.

Miscellaneous items such as antenna mounts, etc., have not been designed or detailed as a part of our work. We recommend that material of adequate size and strength be purchased from a reputable tower manufacturer.

Towers are designed to carry gravity, wind, and ice loads. All members, legs, diagonals, struts, and redundant members provide structural stability to the tower with little redundancy. Absence or removal of a member can trigger catastrophic failure unless a substitute is provided before any removal. Legs carry axial loads and derive their strength from shorter unbraced lengths by the presence of redundant members and their connection to the diagonals with bolts or welds. If the bolts or welds are removed without providing any substitute to the frame, the leg is subjected to a higher unbraced length that immediately reduces its load carrying capacity. If a diagonal is also removed in addition to the connection, the unbraced length of the leg is greatly increased, jeopardizing its load carrying capacity. Failure of one leg can result in a tower collapse because there is no redundancy. Redundant members and diagonals are critical to the stability of the tower.

GPD GROUP makes no warranties, expressed and/or implied, in connection with this report and disclaims any liability arising from material, fabrication, and erection of this tower. GPD GROUP will not be responsible whatsoever for, or on account of, consequential or incidental damages sustained by any person, firm, or organization as a result of any data or conclusions contained in this report. The maximum liability of GPD GROUP pursuant to this report will be limited to the total fee received for preparation of this report.

## APPENDIX A

Tower Analysis Summary Form
Tower Analysis Summary Form



| Guy Wires | EHS |
| :--- | :--- |
| Nota: Stael grades assumed based on past experience with similar towera. |  |



## APPENDIX B

tnxTower Output File

| tnxTTOWeF | Job | Page |  |
| :---: | :--- | :---: | :--- |
|  | Project | SNET020 NORFOLK | 1 of 9 |
|  | Client | 2015723.01.SNET020.03 | Date <br> $14: 20: 23$ 01/28/15 |

## Tower Input Data

The main tower is a 3 x guyed tower with an overall height of 160.00 ft above the ground line.
The base of the tower is set at an elevation of 0.00 ft above the ground line.
The face width of the tower is 4.00 ft at the top and tapered at the base.
This tower is designed using the TIA/EIA-222-F standard.
The following design criteria apply:
Tower is located in Litchfield County, Connecticut.
Basic wind speed of 80 mph .
Nominal ice thickness of 0.7500 in .
Ice thickness is considered to increase with height.
Ice density of 56 pcf.
A wind speed of 28 mph is used in combination with ice.
Temperature drop of $50^{\circ} \mathrm{F}$.
Deflections calculated using a wind speed of 50 mph .
Pressures are calculated at each section.
Safety factor used in guy design is 2 .
Stress ratio used in tower member design is 1.333 .
Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.
Feed Line/Linear Appurtenances - Entered As Round Or Flat

| Description | $\begin{gathered} \text { Face } \\ \text { or } \\ \text { Leg } \end{gathered}$ | Allow Shield | Component Type | Placement $\qquad$ $f t$ | Face Offset in | Lateral Offset <br> (Frac FW) | \# | \# Per Row | Clear Spacing in | Width or Diameter in | Perimeter <br> in | Weight plf |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Climbing Ladder | B | Yes | Af (CfAe) | 160.00-8.00 | $-2.0000$ | 0 | 1 | 1 | 3.8400 | 3.8400 | 15.3600 | 4.81 |
| $\begin{gathered} \text { LDF5-50A } \\ (7 / 8 \text { FOAM) } \end{gathered}$ | A | Yes | Ar (CfAe) | 160.00-8.00 | $-2.0000$ | -0.35 | 1 | 1 | 1.0000 | 1.0900 |  | 0.33 |
| $\begin{gathered} \text { LDF7-50A } \\ (1-5 / 8 \text { FOAM }) \end{gathered}$ | A | Yes | Ar (CfAe) | 158.00-8.00 | 0.0000 | 0.1 | 6 | 6 | 0.5000 | 1.9800 |  | 0.82 |
| Hybriflex (1-1/4") | A | Yes | Ar (CfAe) | 158.00-8.00 | 0.0000 | -0.15 | 3 | 3 | 0.5000 | 1.5400 |  | 1.30 |
| $\begin{aligned} & \text { LDF4-50A } \\ & (1 / 2 \text { FOAM }) \end{aligned}$ | A | Yes | Ar (CfAe) | 148.00-8.00 | $-2.0000$ | 0.1 | I | 1 | 0.0000 | 0.0000 |  | 0.15 |
| $\begin{aligned} & \text { LDF6-50A } \\ & (1-1 / 4 \text { FOAM }) \end{aligned}$ | A | Yes | At (CfAe) | 120.00-8.00 | -3.0000 | 0 | 6 | 5 | 1.0000 | 0.0000 |  | 0.66 |
| LDF1-50A <br> (1/4 FOAM) | A | Yes | Ar (CfAe) | $75.00-8.00$ | 0.0000 | -0.075 | 1 | 1 | 0.3500 | 0.3500 |  | 0.06 |
| Coax Bracket $5 / 20^{\prime} \times 12^{\prime \prime}$ | A | Yes | Af(CfAe) | 160.00-8.00 | 0.0000 | 0 | 1 | 1 | 0.5000 | 0.5000 | 1.5000 | 0.43 |
| $\begin{aligned} & \text { LDF4-50A } \\ & \text { (1/2 FOAM) } \end{aligned}$ | B | Yes | Ar (CfAe) | $13.00-8.00$ | 0.0000 | -0.25 | 1 | 1 | 0.6300 | 0.6300 |  | 0.15 |
| $\begin{gathered} \text { LDF7-50A } \\ (1-5 / 8 \text { FOAM) } \end{gathered}$ | B | Yes | $\operatorname{Ar}(\mathrm{CfAe})$ | 142.00-8.00 | 0.0000 | 0.4 | 6 | 3 | 1.0000 | 1.9800 |  | 0.82 |
| 1.5" DC/Fiber Bundle | B | Yes | Ar (CfAe) | 142.00-8.00 | 0.0000 | 0.3 | 1 | 1 | 1.5000 | 1.5000 |  | 0.80 |
| $\begin{gathered} \text { LDF7-50A } \\ (1-5 / 8 \text { FOAM) } \end{gathered}$ | B | Yes | Ar (CfAe) | 142.00-8.00 | 0.0000 | -0.4 | 6 | 3 | 1.0000 | 1.9800 |  | 0.82 |
| Coax Bracket $5 / 20^{\prime} \times 12^{\prime \prime}$ | C | Yes | Af(CfAe) | 120.00-8.00 | 0.0000 | 0 | 1 | 1 | 0.5000 | 0.5000 | 1.5000 | 0.43 |
| $\begin{gathered} 3 / 8^{\prime \prime} \mathrm{RET} \\ \text { Cable } \end{gathered}$ | C | Yes | Ar (CfAe) | 120.00-8.00 | 0.0000 | 0.1 | 1 | 1 | 0.3750 | 0.3750 |  | 0.10 |
| $\begin{gathered} \text { LDF7-50A } \\ (1-5 / 8 \text { FOAM }) \end{gathered}$ | C | Yes | Ar (CfAe) | 120.00-8.00 | 0.0000 | 0 | 6 | 3 | 1.0000 | 1.9800 |  | 0.82 |


| tnxTOwer | Job | SNET020 NORFOLK | $\text { Page } 2 \text { of } 9$ |
| :---: | :---: | :---: | :---: |
| GPD Group 520 S. Main Street | Project | 2015723.01.SNET020.03 | $\begin{array}{\|l\|} \hline \text { Date } \\ \text { 14:20:23 01/28/15 } \end{array}$ |
| Akron, OH 44311 <br> Phone: 330.572.2201 <br> FAX: | Client | AT\&T Towers | Designed by jdischinger |

## Discrete Tower Loads

\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline Description \& \[
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
\] \& \[
\begin{aligned}
\& \text { Offset } \\
\& \text { Type }
\end{aligned}
\] \& \begin{tabular}{l}
Offsets: \\
Horz \\
Lateral Vert \\
\(f t\) \\
\(f t\)
\end{tabular} \& \begin{tabular}{l}
Azimuth Adjustment \\
0
\end{tabular} \& Placement

$f t$ \& \& | $C_{A} A_{A}$ |
| :--- |
| Front |
| $f^{2}$ | \& | $C_{A} A_{A}$ |
| :--- |
| Side |
| $f t^{2}$ | \& Weight <br>

\hline \multirow[t]{5}{*}{$10^{\prime}$ Dipole} \& \multirow[t]{5}{*}{A} \& \multirow[t]{5}{*}{From Leg} \& 0.50 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{160.00} \& No Ice \& 2.00 \& 2.00 \& 0.02 <br>
\hline \& \& \& 0.00 \& \& \& $1 / 2^{1 \prime}$ Ice \& 3.02 \& 3.02 \& 0.04 <br>
\hline \& \& \& \multirow[t]{3}{*}{8.00} \& \& \& $1{ }^{\prime \prime}$ Ice \& 4.07 \& 4.07 \& 0.06 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 5.70 \& 5.70 \& 0.12 <br>
\hline \& \& \& \& \& \& $4{ }^{\prime \prime}$ Ice \& 8.26 \& 8.26 \& 0.33 <br>
\hline \multirow[t]{5}{*}{Pipe Mount 3'x2.375'} \& \multirow[t]{5}{*}{A} \& \multirow[t]{5}{*}{From Leg} \& 0.25 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{160.00} \& No Ice \& 0.58 \& 0.58 \& 0.01 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 0.77 \& 0.77 \& 0.02 <br>
\hline \& \& \& 1.50 \& \& \& 1" Ice \& 0.97 \& 0.97 \& 0.02 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 1.42 \& 1.42 \& 0.05 <br>
\hline \& \& \& \& \& \& $4^{\prime \prime}$ Ice \& 2.54 \& 2.54 \& 0.13 <br>
\hline \multirow[t]{5}{*}{$3{ }^{\prime}$ Omni} \& \multirow[t]{5}{*}{B} \& \multirow[t]{5}{*}{From Face} \& 0.50 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{160.00} \& No Ice \& 0.52 \& 0.52 \& 0.02 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 0.71 \& 0.71 \& 0.02 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& $1^{\prime \prime}$ Ice \& 0.90 \& 0.90 \& 0.03 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 1.33 \& 1.33 \& 0.05 <br>
\hline \& \& \& \& \& \& 4 I' Ice \& 2.44 \& 2.44 \& 0.12 <br>
\hline \multirow[t]{5}{*}{Rohn 12' Boom Gate} \& \multirow[t]{5}{*}{A} \& \multirow[t]{5}{*}{From Leg} \& 1.29 \& \multirow[t]{5}{*}{50.0000} \& \multirow[t]{5}{*}{158.00} \& No. Ice \& 15.35 \& 14.00 \& 0.56 <br>
\hline \& \& \& 1.53 \& \& \& 1/2" Ice \& 21.29 \& 20.81 \& 0.74 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& 1" Ice \& 27.23 \& 27.62 \& 0.92 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 39.11 \& 41.24 \& 1.29 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 62.87 \& 68.48 \& 2.03 <br>
\hline \multirow[t]{5}{*}{Rohn 12' Boom Gate} \& \multirow[t]{5}{*}{B} \& \multirow[t]{5}{*}{From Leg} \& 1.29 \& \multirow[t]{5}{*}{50.0000} \& \multirow[t]{5}{*}{158.00} \& No Ice \& 15.35 \& 14.00 \& 0.56 <br>
\hline \& \& \& 1.53 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 21.29 \& 20.81 \& 0.74 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& $1^{\prime \prime}$ Ice \& 27.23 \& 27.62 \& 0.92 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 39.11 \& 41.24 \& 1.29 <br>
\hline \& \& \& \& \& \& $4^{\prime \prime}$ Ice \& 62.87 \& 68.48 \& 2.03 <br>

\hline \multirow[t]{5}{*}{Rohn 12' Boom Gate} \& \multirow[t]{5}{*}{C} \& \multirow[t]{5}{*}{From Leg} \& $$
1.29
$$ \& \multirow[t]{5}{*}{50.0000} \& \multirow[t]{5}{*}{158.00} \& No Ice \& 15.35 \& 14.00 \& 0.56 <br>

\hline \& \& \& 1.53 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 21.29 \& 20.81 \& 0.74 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& $1^{\prime \prime}$ Ice \& 27.23 \& 27.62 \& 0.92 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 39.11 \& 41.24 \& 1.29 <br>
\hline \& \& \& \& \& \& $4^{\prime \prime}$ Ice \& 62.87 \& 68.48 \& 2.03 <br>
\hline \multirow[t]{5}{*}{(2) DB980H90E-M w/ Mount Pipe} \& \multirow[t]{5}{*}{A} \& \multirow[t]{5}{*}{From Leg} \& \& \multirow[t]{5}{*}{50.0000} \& \multirow[t]{5}{*}{158.00} \& No Ice \& 4.04 \& 3.62 \& 0.03 <br>
\hline \& \& \& 3.06 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 4.50 \& 4.48 \& 0.07 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& $1{ }^{1 \prime}$ Ice \& 4.95 \& 5.22 \& 0.11 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 5.87 \& 6.74 \& 0.22 <br>
\hline \& \& \& \& \& \& 4" Ice \& 8.05 \& 10.00 \& 0.55 <br>
\hline \multirow[t]{5}{*}{(2) DB980H90E-M w/ Mount Pipe} \& \multirow[t]{5}{*}{B} \& \multirow[t]{5}{*}{From Leg} \& \& \multirow[t]{5}{*}{50.0000} \& \multirow[t]{5}{*}{158.00} \& No Ice \& 4.04 \& 3.62 \& 0.03 <br>
\hline \& \& \& 3.06 \& \& \& $1 / 2^{\text {" }}$ Ice \& 4.50 \& 4.48 \& 0.07 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& $1^{\prime \prime}$ Ice \& 4.95 \& 5.22 \& 0.11 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 5.87 \& 6.74 \& 0.22 <br>
\hline \& \& \& \& \& \& $4{ }^{\prime \prime}$ Ice \& 8.05 \& 10.00 \& 0.55 <br>
\hline \multirow[t]{5}{*}{(2) DB980H90E-M w/ Mount Pipe} \& \multirow[t]{5}{*}{C} \& \multirow[t]{5}{*}{From Leg} \& 2.57 \& \multirow[t]{5}{*}{50.0000} \& \multirow[t]{5}{*}{158.00} \& No Ice \& 4.04 \& 3.62 \& 0.03 <br>
\hline \& \& \& 3.06 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 4.50 \& 4.48 \& 0.07 <br>
\hline \& \& \& 0.00 \& \& \& $1{ }^{11}$ Ice \& 4.95 \& 5.22 \& 0.11 <br>
\hline \& \& \& \& \& \& 2" Ice \& 5.87 \& 6.74 \& 0.22 <br>
\hline \& \& \& \& \& \& 4" Ice \& 8.05 \& 10.00 \& 0.55 <br>
\hline \multirow[t]{5}{*}{APXVSPP18-C-A20 w/ Mount Pipe} \& \multirow[t]{5}{*}{A} \& \multirow[t]{5}{*}{From Leg} \& 2.57 \& \multirow[t]{5}{*}{70.0000} \& \multirow[t]{5}{*}{158.00} \& No Ice \& 8.26 \& 6.71 \& 0.08 <br>
\hline \& \& \& 3.05 \& \& \& $1 / 2^{\text {" }}$ Ice \& 8.81 \& 7.66 \& 0.14 <br>
\hline \& \& \& 0.00 \& \& \& $1^{\prime \prime}$ Ice \& 9.36 \& 8.49 \& 0.22 <br>
\hline \& \& \& \& \& \& $2^{11}$ Ice \& 10.50 \& 10.20 \& 0.39 <br>
\hline \& \& \& \& \& \& 4"Ice \& 12.88 \& 13.98 \& 0.87 <br>
\hline \multirow[t]{4}{*}{APXVSPP18-C-A20 w/ Mount Pipe} \& \multirow[t]{4}{*}{B} \& \multirow[t]{4}{*}{From Leg} \& 2.57 \& \multirow[t]{4}{*}{70.0000} \& \multirow[t]{4}{*}{158.00} \& No Ice \& 8.26 \& 6,71 \& 0.08 <br>
\hline \& \& \& 3.05 \& \& \& $1 / 2^{\text {" }}$ Ice \& 8.81 \& 7.66 \& 0.14 <br>
\hline \& \& \& \multirow[t]{2}{*}{0.00} \& \& \& 1" Ice \& 9.36 \& 8.49 \& 0.22 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 10.50 \& 10.20 \& 0.39 <br>
\hline
\end{tabular}

| tnxTower | Job | SNET020 NORFOLK | $\text { Page } \quad 3 \text { of } 9$ |
| :---: | :---: | :---: | :---: |
| GPD Group <br> 520 S. Main Street | Project | 2015723.01.SNET020.03 | $\begin{array}{\|l\|} \hline \text { Date } \\ \text { 14:20:23 01/28/15 } \end{array}$ |
| Alkon, OH 44311 <br> Phone: 330.572.2201 <br> FAX: | Client | AT\&T Towers | Designed by jdischinger |



| tnxTower <br> GPD Group <br> 520 S. Main Street | Job | SNET020 NORFOLK | $\text { Page } 4 \text { of } 9$ |
| :---: | :---: | :---: | :---: |
|  | Project | 2015723.01.SNET020.03 | $\begin{array}{\|l\|} \hline \text { Date } \\ \text { 14:20:23 01/28/15 } \\ \hline \end{array}$ |
| Alkon, OH 44311 Phone: 330.572.2201 FAX: | Client | AT\&T Towers | Designed by jdischinger |

\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline Description \& \begin{tabular}{l}
Face \\
or \\
Leg
\end{tabular} \& \begin{tabular}{l}
Offset \\
Type
\end{tabular} \& \begin{tabular}{l}
Offsets: \\
Horz \\
Lateral \\
Vert \\
\(f t\) \\
\(f t\) \\
\(f\)
\end{tabular} \& \begin{tabular}{l}
Azinuth Adjustment \\
。
\end{tabular} \& Placement \& \& \(C_{4} A_{A}\) Front
\[
f t^{2}
\] \& \begin{tabular}{l}
\(C_{A} A_{A}\) \\
Side \\
\(f t^{2}\)
\end{tabular} \& Weight

K <br>
\hline \multirow{9}{*}{8 ' Frame} \& \multirow{8}{*}{A} \& \multirow{7}{*}{From Leg} \& 0.00 \& \multirow{7}{*}{50.0000} \& \multirow{7}{*}{142.00} \& 1/2" Ice \& 1.11 \& 1.11 \& 0.03 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& 1 Ice \& 1.36 \& 1.36 \& 0.04 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 1,90 \& 1.90 \& 0.07 <br>
\hline \& \& \& \& \& \& 4" Ice \& 3.23 \& 3.23 \& 0.17 <br>
\hline \& \& \& 0.48 \& \& \& No Ice \& 14.48 \& 3.61 \& 0.31 <br>
\hline \& \& \& 0.57 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 18.67 \& 4.62 \& 0.45 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& 1" Ice \& 22.86 \& 5.62 \& 0.60 <br>
\hline \& \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{50.0000} \& \multirow{5}{*}{142.00} \& $2^{\prime \prime}$ Ice \& 31.24 \& 7.63 \& 0.89 <br>
\hline \& \multirow{5}{*}{B} \& \& \& \& \& 4 " Ice \& 48.00 \& 11.65 \& 1.46 <br>
\hline \multirow[t]{5}{*}{8 Frame} \& \& \& 0.48 \& \& \& No Ice \& 14.48 \& 3.61 \& 0.31 <br>
\hline \& \& \& 0.57 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 18.67 \& 4.62 \& 0.45 <br>
\hline \& \& \& 0.00 \& \& \& $1^{\prime \prime}$ Ice \& 22.86 \& 5.62 \& 0.60 <br>
\hline \& \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{50.0000} \& \multirow{5}{*}{142.00} \& $2^{\prime \prime}$ Ice \& 31.24 \& 7.63 \& 0.89 <br>
\hline \& \multirow{4}{*}{C} \& \& \& \& \& 4 Ice \& 48.00 \& 11.65 \& 1.46 <br>
\hline \multirow[t]{5}{*}{8' Frame} \& \& \& 0.48 \& \& \& No Ice \& 14.48 \& 3.61 \& 0.31 <br>
\hline \& \& \& 0.57 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 18.67 \& 4.62 \& 0.45 <br>
\hline \& \& \& 0.00 \& \& \& $1^{11}$ Ice \& 22.86 \& 5.62 \& 0.60 <br>
\hline \& \multirow{5}{*}{A} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{50.0000} \& \multirow{5}{*}{142.00} \& $2^{11}$ Ice \& 31.24 \& 7.63 \& 0.89 <br>
\hline \& \& \& \& \& \& $4^{\prime \prime}$ Ice \& 48.00 \& 11.65 \& 1.46 <br>
\hline \multirow[t]{5}{*}{AM-X-CD-16-65-00T w/ Mount Pipe} \& \& \& 0.96 \& \& \& No Ice \& 8.55 \& 6.65 \& 0.09 <br>
\hline \& \& \& 1.15 \& \& \& 1/2" Ice \& 9.18 \& 7.68 \& 0.16 <br>
\hline \& \& \& 0.00 \& \& \& 1 " Ice \& 9.79 \& 8.56 \& 0.23 <br>
\hline \& \multirow{5}{*}{B} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{50.0000} \& \multirow{5}{*}{142.00} \& $2^{\prime \prime}$ Ice \& 11.06 \& 10.38 \& 0.41 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 13.71 \& 14.23 \& 0.91 <br>
\hline \multirow[t]{5}{*}{AM-X-CD-16-65-00T w/ Mount Pipe} \& \& \& 0.96 \& \& \& No Ice \& 8.55 \& 6.65 \& 0.09 <br>
\hline \& \& \& 1.15 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 9.18 \& 7.68 \& 0.16 <br>
\hline \& \& \& 0.00 \& \& \& $1^{\prime \prime}$ Ice \& 9.79 \& 8.56 \& 0.23 <br>
\hline \& \multirow{5}{*}{C} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{50.0000} \& \multirow{5}{*}{142.00} \& $2^{\prime \prime}$ Ice \& 11.06 \& 10.38 \& 0.41 <br>
\hline \& \& \& \& \& \& $4^{\prime \prime}$ Ice \& 13.71 \& 14.23 \& 0.91 <br>
\hline \multirow[t]{5}{*}{AM-X-CD-16-65-00T w/ Mount Pipe} \& \& \& 0.96 \& \& \& No Ice \& 8.55 \& 6.65 \& 0.09 <br>
\hline \& \& \& 1.15 \& \& \& $1 / 2^{11}$ Ice \& 9.18 \& 7.68 \& 0.16 <br>
\hline \& \& \& 0.00 \& \& \& $1{ }^{\prime \prime}$ Ice \& 9.79 \& 8.56 \& 0.23 <br>
\hline \& \multirow{5}{*}{A} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{50.0000} \& \multirow{5}{*}{142.00} \& $2^{\prime \prime}$ Ice \& 11.06 \& 10.38 \& 0.41 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 13.71 \& 14.23 \& 0.91 <br>
\hline \multirow[t]{5}{*}{(2) $7770.00 \mathrm{w} /$ Mount Pipe} \& \& \& 0.96 \& \& \& No Ice \& 5.88 \& 4.10 \& 0.06 <br>
\hline \& \& \& 1.15 \& \& \& $1 / 2^{\text {" }}$ Ice \& 6.31 \& 4.73 \& 0.11 <br>
\hline \& \& \& 0.00 \& \& \& 1 " Ice \& 6.75 \& 5.37 \& 0.16 <br>
\hline \& \multirow{6}{*}{B} \& \multirow{6}{*}{From Leg} \& \& \multirow{5}{*}{50.0000} \& \multirow{5}{*}{142.00} \& $2^{\prime \prime}$ Ice \& 7.66 \& 6.70 \& 0.29 <br>
\hline \& \& \& \& \& \& $4^{\prime \prime}$ Ice \& 9.58 \& 9.87 \& 0.65 <br>
\hline \multirow[t]{5}{*}{(2) $7770.00 \mathrm{w} /$ Mount Pipe} \& \& \& 0.96 \& \& \& No Ice \& 5.88 \& 4.10 \& 0.06 <br>
\hline \& \& \& 1.15 \& \& \& 1/2" Ice \& 6.31 \& 4.73 \& 0.11 <br>
\hline \& \& \& 0.00 \& \& \& 1" Ice \& 6.75 \& 5.37 \& 0.16 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 7.66 \& 6.70 \& 0.29 <br>
\hline \& \multirow{4}{*}{C} \& \multirow{4}{*}{From Leg} \& \& \multirow{4}{*}{50.0000} \& \multirow{4}{*}{142.00} \& $4^{\prime \prime}$ Ice \& 9.58 \& 9.87 \& 0.65 <br>
\hline \multirow[t]{5}{*}{(2) $7770.00 \mathrm{~W} / \mathrm{Mount}$ Pipe} \& \& \& 0.96 \& \& \& No Ice \& 5.88 \& 4.10 \& 0.06 <br>
\hline \& \& \& 1.15 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 6.31 \& 4.73 \& 0.11 <br>
\hline \& \& \& 0.00 \& \& \& 1 Ice \& 6.75 \& 5.37 \& 0.16 <br>
\hline \& \multirow{5}{*}{A} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{50.0000} \& \multirow{5}{*}{142.00} \& $2^{\prime \prime}$ Ice \& 7.66 \& 6.70 \& 0.29 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 9.58 \& 9.87 \& 0.65 <br>
\hline \multirow[t]{4}{*}{(2) RRU-11} \& \& \& 0.96 \& \& \& No Ice \& 1.91 \& 1.47 \& 0.04 <br>
\hline \& \& \& 1.15 \& \& \& 1/2" Ice \& 2.10 \& 1.65 \& 0.06 <br>
\hline \& \& \& 0.00 \& \& \& 1" Ice \& 2.30 \& 1.83 \& 0.08 <br>
\hline \& \multirow{5}{*}{B} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{50.0000} \& \multirow{5}{*}{142.00} \& $2^{\prime \prime}$ Ice \& 2.72 \& 2.22 \& 0.12 <br>
\hline \multirow{4}{*}{(2) RRU-11} \& \& \& \& \& \& 4 " Ice \& 3.68 \& 3.10 \& 0.25 <br>
\hline \& \& \& 0.96 \& \& \& No Ice \& 1.91 \& 1.47 \& 0.04 <br>
\hline \& \& \& 1.15 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 2.10 \& 1.65 \& 0.06 <br>
\hline \& \& \& 0.00 \& \& \& 1" Ice \& 2.30 \& 1.83 \& 0.08 <br>
\hline
\end{tabular}

| tnxTower | Job | SNET020 NORFOLK | Page 5 of 9 |
| :---: | :---: | :---: | :---: |
| GPD Group <br> 520 S. Main Street <br> Akron, OH 44311 <br> Phone: 330.572.2201 <br> FAX: | Project | 2015723.01.SNET020.03 | $\begin{array}{\|l\|} \hline \text { Date } \\ \text { 14:20:23 01/28/15 } \end{array}$ |
|  | Client | AT\&T Towers | Designed by jdischinger |



| tnxTower <br> GPD Group 520 S. Main Street Akron, OH 44311 Phone: 330.572.2201 FAX: | Job | SNET020 NORFOLK | $\text { Page } 6 \text { of } 9$ |
| :---: | :---: | :---: | :---: |
|  | Project | 2015723.01.SNET020.03 | $\begin{array}{\|l\|} \hline \text { Date } \\ \text { 14:20:23 01/28/15 } \end{array}$ |
|  | Client | AT\&T Towers | Designed by jdischinger |

\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline Description \& \[
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
\] \& \[
\begin{aligned}
\& \text { Offset } \\
\& \text { Type }
\end{aligned}
\] \& \begin{tabular}{l}
Offsets: \\
Horz \\
Lateral \\
Vert \\
\(f t\) \\
\(f\) \\
ft
\end{tabular} \& \begin{tabular}{l}
Azimuth Adjustment \\
。
\end{tabular} \& Placement \& \& \(C_{A} A_{A}\) Front
\[
f^{2}
\] \& \(C_{A} A_{A}\) Side \(f t^{2}\) \& Weight

$K$ <br>
\hline \multirow[t]{5}{*}{RRUS 11 B12} \& \multirow[t]{5}{*}{B} \& \multirow[t]{5}{*}{From Leg} \& 4.00 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{120.00} \& No Ice \& 3.31 \& 1.36 \& 0.05 <br>
\hline \& \& \& 0.00 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 3.55 \& 1.54 \& 0.07 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& $1^{1 \prime}$ Ice \& 3.80 \& 1.73 \& 0.10 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 4.33 \& 2.13 \& 0.15 <br>
\hline \& \& \& \& \& \& $4^{\prime \prime}$ Ice \& 5.50 \& 3.04 \& 0.31 <br>
\hline \multirow[t]{5}{*}{RRUS 11 B12} \& \multirow[t]{5}{*}{C} \& \multirow[t]{5}{*}{From Leg} \& 4.00 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{120.00} \& No Ice \& 3.31 \& 1.36 \& 0.05 <br>
\hline \& \& \& 0.00 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 3.55 \& 1.54 \& 0.07 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& $1^{\prime \prime}$ Ice \& 3.80 \& 1.73 \& 0.10 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 4.33 \& 2.13 \& 0.15 <br>
\hline \& \& \& \& \& \& $4^{\prime \prime}$ Ice \& 5.50 \& 3.04 \& 0.31 <br>
\hline \multirow[t]{5}{*}{(2) $1412 \mathrm{D}-1 \mathrm{~A} 20$} \& \multirow[t]{5}{*}{A} \& \multirow[t]{5}{*}{From Leg} \& 4.00 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{120.00} \& No Ice \& 0.00 \& 0.47 \& 0.01 <br>
\hline \& \& \& 0.00 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 0.00 \& 0.57 \& 0.02 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& $1^{\prime \prime}$ Ice \& 0.00 \& 0.69 \& 0.03 <br>
\hline \& \& \& \& \& \& $2{ }^{\prime \prime}$ Ice \& 0.00 \& 0.95 \& 0.06 <br>
\hline \& \& \& \& \& \& 4"Ice \& 0.00 \& 1.57 \& 0.14 <br>
\hline \multirow[t]{5}{*}{(2) 1412D-1A20} \& \multirow[t]{5}{*}{B} \& \multirow[t]{5}{*}{From Leg} \& 4.00 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{120.00} \& No Ice \& 0.00 \& 0.47 \& 0.01 <br>
\hline \& \& \& 0.00 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 0.00 \& 0.57 \& 0.02 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& 1" Ice \& 0.00 \& 0.69 \& 0.03 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 0.00 \& 0.95 \& 0.06 <br>
\hline \& \& \& \& \& \& $4^{\prime \prime}$ Ice \& 0.00 \& 1.57 \& 0.14 <br>
\hline \multirow[t]{5}{*}{(2) $1412 \mathrm{D}-1 \mathrm{~A} 20$} \& \multirow[t]{5}{*}{C} \& \multirow[t]{5}{*}{From Leg} \& 4.00 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{120.00} \& No Ice \& 0.00 \& 0.47 \& 0.01 <br>
\hline \& \& \& 0.00 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 0.00 \& 0.57 \& 0.02 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& $1^{\prime \prime}$ Ice \& 0.00 \& 0.69 \& 0.03 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 0.00 \& 0.95 \& 0.06 <br>
\hline \& \& \& \& \& \& 4 I' Ice \& 0.00 \& 1.57 \& 0.14 <br>
\hline \multirow[t]{5}{*}{4' Standoff - Flat (GPD)} \& \multirow[t]{5}{*}{A} \& \multirow[t]{5}{*}{From Leg} \& 2.00 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{120.00} \& No Ice \& 1.96 \& 6.13 \& 0.07 <br>
\hline \& \& \& 0.00 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 3.08 \& 8.58 \& 0.11 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& 1 I' Ice \& 4.20 \& 11.03 \& 0.14 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 6.44 \& 15.93 \& 0.20 <br>
\hline \& \& \& \& \& \& $4{ }^{\prime \prime}$ Ice \& 10.92 \& 25.73 \& 0.33 <br>
\hline \multirow[t]{5}{*}{4' Standoff - Flat (GPD)} \& \multirow[t]{5}{*}{B} \& \multirow[t]{5}{*}{From Leg} \& \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{120.00} \& No Ice \& 1.96 \& 6.13 \& 0.07 <br>
\hline \& \& \& 0.00 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 3.08 \& 8.58 \& 0.11 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& $1^{1 \prime}$ Ice \& 4.20 \& 11.03 \& 0.14 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 6.44 \& 15.93 \& 0.20 <br>
\hline \& \& \& \& \& \& $4{ }^{\prime \prime}$ Ice \& 10.92 \& 25.73 \& 0.33 <br>
\hline \multirow[t]{5}{*}{4' Standoff - Flat (GPD)} \& \multirow[t]{5}{*}{C} \& \multirow[t]{5}{*}{From Leg} \& \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{120.00} \& \& 1.96 \& 6.13 \& 0.07 <br>
\hline \& \& \& 0.00 \& \& \& $1 / 2^{\text {" }}$ Ice \& 3.08 \& 8.58 \& 0.11 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& 1" Ice \& 4.20 \& 11.03 \& 0.14 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 6.44 \& 15.93 \& 0.20 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 10.92 \& 25.73 \& 0.33 <br>
\hline \multirow[t]{5}{*}{GPS-TMG-HR-26NCM} \& \multirow[t]{5}{*}{C} \& \multirow[t]{5}{*}{From Leg} \& \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{75.00} \& \& 0.80 \& 0.93 \& 0.03 <br>
\hline \& \& \& 0.00 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 1.05 \& 1.17 \& 0.03 <br>
\hline \& \& \& 0.00 \& \& \& $1^{\prime \prime}$ Ice \& 1.30 \& 1.41 \& 0.04 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 1.80 \& 1.89 \& 0.05 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 2.80 \& 2.85 \& 0.08 <br>
\hline \multirow[t]{5}{*}{2' Sidearm - Round (GPD)} \& \multirow[t]{5}{*}{C} \& \multirow[t]{5}{*}{From Leg} \& \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{75.00} \& \& 0.80 \& 0.93 \& 0.03 <br>
\hline \& \& \& 0.00 \& \& \& $1 / 2^{\text {" }}$ Ice \& 1.05 \& 1.17 \& 0.03 <br>
\hline \& \& \& 0.00 \& \& \& $1{ }^{1 \prime}$ Ice \& 1.30 \& 1.41 \& 0.04 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 1.80 \& 1.89 \& 0.05 <br>
\hline \& \& \& \& \& \& 4'Ice \& 2.80 \& 2.85 \& 0.08 <br>
\hline \multirow[t]{5}{*}{GPS} \& \multirow[t]{5}{*}{B} \& \multirow[t]{5}{*}{From Leg} \& 1.00 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{13.00} \& No Ice \& 0.17 \& 0.17 \& 0.00 <br>
\hline \& \& \& 0.00 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 0.24 \& 0.24 \& 0.00 <br>
\hline \& \& \& 0.00 \& \& \& $1{ }^{\prime \prime}$ Ice \& 0.32 \& 0.32 \& 0.01 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 0.51 \& 0.51 \& 0.02 <br>
\hline \& \& \& \& \& \& 4"Ice \& 1.02 \& 1.02 \& 0.06 <br>
\hline \multirow[t]{2}{*}{1 1' Sidearm - Flat (GPD)} \& \multirow[t]{2}{*}{B} \& \multirow[t]{2}{*}{From Leg} \& 0.50 \& \multirow[t]{2}{*}{0.0000} \& \multirow[t]{2}{*}{13.00} \& No Ice \& 0.80 \& 0.80 \& 0.02 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 1.05 \& 1.00 \& 0.02 <br>
\hline
\end{tabular}

| tnxTower <br> GPD Group 520 S. Main Street Akron, OH 44311 <br> Phone: 330.572.2201 FAX: | Job | SNET020 NORFOLK | $\text { Page } 7 \text { of } 9$ |
| :---: | :---: | :---: | :---: |
|  | Project | 2015723.01.SNET020.03 | $\begin{array}{\|l\|} \hline \text { Date } \\ \text { 14:20:23 01/28/15 } \end{array}$ |
|  | Client | AT\&T Towers | Designed by jdischinger |

\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline Description \& \[
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
\] \& \begin{tabular}{l}
Offset \\
Type
\end{tabular} \& \begin{tabular}{l}
Offsets: \\
Horz \\
Lateral \\
Vert \\
\(f t\) \\
\(f t\) \\
\(t\)
\end{tabular} \& \begin{tabular}{l}
Azimuth Adjustment \\
0
\end{tabular} \& Placement \& \& \(C_{A} A_{A}\) Front
\[
f^{2}
\] \& \begin{tabular}{l}
\(C_{A} A_{A}\) Side \\
\(f t^{2}\)
\end{tabular} \& Weight

$K$ <br>
\hline \& \& \& 0.00 \& \& \& $1^{11}$ Ice \& 1.30 \& 1.20 \& 0.03 <br>
\hline \& \& \& \& \& \& $2^{\prime \prime}$ Ice \& 1.80 \& 1.60 \& 0.04 <br>
\hline \& \& \& \& \& \& $4^{\prime \prime}$ Ice \& 2.80 \& 2.40 \& 0.06 <br>
\hline
\end{tabular}

## Critical Deflections and Radius of Curvature - Service Wind

| Elevation ft | Appurtenance | Gov. <br> Load <br> Comb. | Deflection in | Tilt | Twist | Radius of Curvature $f t$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 160.00 | 10' Dipole | 27 | 1.364 | 0.0260 | 0.0213 | 302519 |
| 158.00 | Rohn 12' Boom Gate | 27 | 1.374 | 0.0277 | 0.0239 | 302519 |
| 154.00 | $800 \mathrm{MHz} 2 \times 50 \mathrm{w}$ | 27 | 1.395 | 0.0308 | 0.0292 | 252100 |
| 148.00 | 4' Yagi | 27 | 1.428 | 0.0339 | 0.0368 | 126050 |
| 145.75 | Guy | 27 | 1.441 | 0.0344 | 0.0396 | 106151 |
| 142.00 | 8 ' Frame | 27 | 1.464 | 0.0339 | 0.0442 | 95290 |
| 137.00 | $4^{\prime}$ Sidearm - Flat (GPD) | 27 | 1.496 | 0.0306 | 0.0498 | 498885 |
| 120.00 | RR90-17-02DP w/ Mount Pipe | 27 | 1.579 | 0.0135 | 0.0623 | 23428 |
| 86.00 | Guy | 27 | 1.437 | 0.0256 | 0.0964 | 30777 |
| 75.00 | GPS-TMG-HR-26NCM | 27 | 1.397 | 0.0246 | 0.1066 | 148739 |
| 13.00 | GPS | 27 | 0.394 | 0.1338 | 0.1176 | 53218 |

Bolt Design Data

| Section No. | Elevation <br> $f t$ | Component Type | Bolt Grade | Bolt Size in | Number Of Bolts | Maximum <br> Load per Bolt K | Allowable Load K | Ratio <br> Load <br> Allowable | Allowable Ratio | Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 | 160 | Leg | A325N | 0.6250 | 12 | 2.49 | 12.89 | 0.193 | $1$ | Bolt DS |
|  |  | Diagonal | A325N | 0.6250 | 2 | 2.86 | 9.58 | 0.299 | 1.333 | Member Block Shear |
|  |  | Horizontal | A 325 N | 0.6250 | 2 | 0.47 | 4.79 | 0.098 | 1.333 | Member Block Shear |
| T2 | 141 | Leg | A325N | 0.6250 | 12 | 4.22 | 12.89 | 0.328 | 1.333 | Bolt DS |
|  |  | Diagonal | A325N | 0.6250 | 1 | 4.27 | 6.44 | 0.662 | 1.333 | Bolt Shear |
|  | 121 | Horizontal | A325N | 0.6250 | 1 | 1.23 | 4.55 | 0.270 | 1.333 | Member Block Shear |
| T3 |  | Leg | A 325 N | 0.6250 | 12 | 3.11 | 12.89 | 0.241 | 1 | Bolt DS |
|  |  | Diagonal | A325N | 0.6250 | 1 | 2.48 | 5.10 | 0.486 | 1.333 | Member Bearing |
|  |  | Horizontal | A 325 N | 0.6250 | 1 | 1.23 | 4.55 | 0.269 | 1.333 | Member Block Shear |
| T4 | 101 | Leg | A325N | 0.6250 | 12 | 3.27 | 12.89 | 0.254 | 1 | Bolt DS |
|  |  | Diagonal | A 325 N | 0.6250 | 1 | 4.46 | 6.44 | 0.693 | 1.333 | Bolt Shear |
|  |  | Horizontal | A325N | 0.6250 | 1 | 2.01 | 4.55 | 0.441 | 1.333 | Member Block Shear |
| T5 | 86 | Leg | A325N | 0.6250 | 12 | 4.02 | 12.89 | 0.312 | 1 | Bolt DS |
|  |  | Diagonal | A 325 N | 0.6250 | 1 | 4.23 | 6.44 | 0.657 | 1.333 | Bolt Shear |
|  |  | Horizontal | A325N | 0.6250 | 1 | 0.63 | 4.55 | 0.139 | 1 | Member Block Shear |


| tnxTower <br> GPD Group 520 S. Main Street Akron, OH 44311 Phone: 330.572.2201 | Job | SNET020 NORFOLK | $\begin{aligned} & \text { Page } 8 \text { of } 9 \end{aligned}$ |
| :---: | :---: | :---: | :---: |
|  | Project | 2015723.01.SNET020.03 | Date 14:20:23 01/28/15 |
|  | Client | AT\&T Towers | Designed by jdischinger |


| Section No. | Elevation <br> $f t$ | Component Type | Bolt Grade | Bolt Size in | Number Of Bolts | Maximum <br> Load per Bolt K | Allowable <br> Load <br> K | Ratio <br> Load <br> Allowable | Allowable Ratio | Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T6 | 66 | Leg | A325N | 0.6250 | 12 | 5.73 | 12.89 | 0.445 | 1.333 | Bolt DS |
|  |  | Diagonal | A 325 N | 0.6250 | 1 | 2.62 | 6.44 | 0.407 | 1.333 | Bolt Shear |
|  |  | Horizontal | A 325 N | 0.6250 | 1 | 0.67 | 4.55 | 0.147 | 1 | Member Block Shear |
| T7 | 46 | Leg | A 325 N | 0.6250 | 12 | 4.44 | 12.89 | 0.345 | 1 | Bolt DS |
|  |  | Diagonal | A 325 N | 0.6250 | 1 | 2.09 | 6,44 | 0.325 | 1.333 | Bolt Shear |
|  |  | Horizontal | A 325 N | 0.6250 | 1 | 0.70 | 4.55 | 0.155 | 1 | Member Block Shear |
| T8 | 26 | Leg | A325N | 0.7500 | 3 | 0.00 | 19.44 | 0.000 | 1.333 | Bolt Tension |
|  |  | Diagonal | A325N | 0.6250 | 1 | 2.55 | 5.10 | 0.501 | 1.333 | Member Bearing |
|  |  | Horizontal | A325N | 0.6250 | 1 | 0.71 | 4.55 | 0.157 | 1 | Member Block Shear |

## Section Capacity Table

| Section No. | $\begin{gathered} \text { Elevation } \\ f t \end{gathered}$ | Component Type | Size | Critical Element | $\begin{aligned} & P \\ & K \end{aligned}$ | $\begin{gathered} S F^{*} P_{\text {allow }} \\ K \end{gathered}$ | \% <br> Capacity | $\begin{aligned} & \text { Pass } \\ & \text { Fail } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 | 160-141 | Leg | V5x5x5/16 | 2 | -15.63 | 92.01 | 17.0 | Pass |
| T2 | 141-121 | Leg | V $5 \times 5 \times 5 / 16$ | 29 | -25.32 | 90.67 | 27.9 | Pass |
| T3 | 121-101 | Leg | V5x5x5/16 | 57 | -26.10 | 90.67 | 28.8 | Pass |
| T4 | 101-86 | Leg | V $5 \times 5 \times 5 / 16$ | 84 | -19.65 | 68.02 | 28.9 | Pass |
| T5 | 86-66 | Leg | V5x5x5/16 | 105 | -24.11 | 68.02 | 35.5 | Pass |
| T6 | 66-46 | Leg | V5x5x5/16 | 132 | -34.39 | 90.67 | 37.9 | Pass |
| T7 | 46-26 | Leg. | V5 $\times 5 \times 5 / 16$ | 159 | -34.84 | 90.67 | 38.4 | Pass |
| T8 | 26-6 | Leg | V $5 \times 5 \times 5 / 16$ | 186 | -28.40 | 68.02 | 41.7 | Pass |
| T9 | 6-0 | Leg | V5x5x5/16 | 213 | -31.06 | 71.83 | 43.2 | Pass |
| T1 | 160-141 | Diagonal | 2L2 1/2x2 1/2×3/16 | 9 | -6.60 | 29.65 | 22.3 | Pass |
| T2 | 141-121 | Diagonal | L2 1/2×2 1/2×3/16 | 54 | -4.27 | 8.04 | 53.1 | Pass |
| T3 | 121-101 | Diagonal | L2 1/2×2 1/2×3/16 | 61 | -3.01 | 8.04 | 37.4 | Pass |
| T4 | 101-86 | Diagonal | L2 $1 / 2 \times 21 / 2 \times 3 / 16$ | 88 | -4.46 | 8.04 | 55.5 | Pass |
| T5 | 86-66 | Diagonal | L2 1/2×2 1/2×3/16 | 129 | -4.23 | 8.04 | 52.6 | Pass |
| T6 | 66-46 | Diagonal | L2 1/2x2 1/2×3/16 | 156 | -2.62 | 8.04 | 32.6 | Pass |
| T7 | 46-26 | Diagonal | L2 1/2x2 1/2×3/16 | 163 | -2.09 | 8.04 | 26.0 | Pass |
| T8 | 26-6 | Diagonal | L2 1/2x2 1/2x3/16 | 198 | -3.16 | 8.04 | 39.3 | Pass |
| T9 | 6-0 | Diagonal | L2 1/2×2 1/2×3/16 | 222 | -0.60 | 10.30 | 5.8 | Pass |
| T1 | 160-141 | Horizontal | L2 1/2x2 1/2x3/16 | 24 | -0.91 | 15.04 | 6.0 | Pass |
| T2 | 141-121 | Horizontal | L2 $2 \times 3 \times 16$ | 31 | 1.23 | 16.65 | 7.4 | Pass |
| T3 | 121-101 | Horizontal | L $2 \times 2 \times 3 / 16$ | 60 | 1.23 | 16.65 | 7.4 | Pass |
| T4 | 101-86 | Horizontal | L2 $\times 2 \times 3 / 16$ | 85 | -1.67 | 10.17 | 16.5 | Pass |
| T5 | 86-66 | Horizontal | L2 $2 \times 2 \times 3 / 16$ | 119 | -0.42 | 7.63 | 5.5 | Pass |
| T6 | 66-46 | Horizontal | L2 $2 \times 3 \times 3 / 16$ | 134 | -0.60 | 10.17 | 5.9 | Pass |
| T7 | 46-26 | Horizontal | L $2 \times 2 \times 3 / 16$ | 161 | -0.46 | 7.63 | 6.1 | Pass |
| T8 | 26-6 | Horizontal | L2 $2 \times 2 \times 3 / 16$ | 188 | -0.49 | 7.63 | 6.4 | Pass |
| T9 | 6-0 | Horizontal | L2 1/2×2 1/2x3/16 | 215 | 6.61 | 19.48 | 33.9 | Pass |
| T1 | 160-141 | Top Girt | L2 1/2×2 1/2x3/16 | 5 | -0.66 | 14.28 | 4.6 | Pass |
| T1 | 160-141 | Guy A@145.75 | 3/4 | 236 | 12.34 | 29.15 | 42.3 | Pass |
| T5 | 86-66 | Guy A@86 | 5/8 | 243 | 10.79 | 21.20 | 50.9 | Pass |
| T1 | 160-141 | Guy B@145.75 | 3/4 | 230 | 12.95 | 29.15 | 44.4 | Pass |
| T5 | 86-66 | Guy B@86 | 5/8 | 242 | 11.58 | 21.20 | 54.6 | Pass |
| T1 | 160-141 | Guy C@145.75 | 3/4 | 223 | 12.82 | 29.15 | 44.0 | Pass |
| T5 | 86-66 | Guy C@86 | 5/8 | 241 | 11.31 | 21.20 | 53.4 | Pass |
| T1 | 160-141 | Top Guy <br> Pull-Off@145.75 | L2 1/2x2 1/2x3/16 | 17 | -2.54 | 16.06 | 15.8 | Pass |
| T5 | 86-66 | Top Guy | L $2 \times 2 \times 3 / 16$ | 106 | 5.81 | 20.59 | 28.2 | Pass |


| tnxTower <br> GPD Group 520 S. Main Street Akron, OH 44311 Phone: 330.572.2201 FAX: | Job | SNET020 NORFOLK | $\begin{aligned} & \text { Page } \quad 9 \text { of } 9 \end{aligned}$ |
| :---: | :---: | :---: | :---: |
|  | Project | 2015723.01.SNET020.03 | $\begin{array}{\|l\|} \hline \text { Date } \\ \text { 14:20:23 01/28/15 } \end{array}$ |
|  | Client | AT\&T Towers | Designed by jdischinger |


| Section No. | $\begin{gathered} \text { Elevation } \\ f t \end{gathered}$ | Component Type | Size | Critical Element | $\begin{aligned} & P \\ & K \end{aligned}$ | $\begin{gathered} S F^{*} P_{\text {allow }} \\ K \end{gathered}$ | Capacity | Pass <br> Fail |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 | 160-141 | Pull-Off@86 Bottom Guy Pull-Off@145.75 | L2 1/2x2 1/2x3/16 | 10 | 4.96 | 25.97 | 19.1 | Pass |
| T1 | 160-141 | Torque Arm Тор@145.75 | 2L2 $1 / 2 \times 2 \times 1 / 4$ | 237 | 8.07 | 46.01 | 17.5 | Pass |
| T1 | 160-141 | Torque Arm Bottom@145.75 | 2L3x2 1/2x1/4 | 239 | -7.98 | 64.09 | 12.4 | Pass |
|  |  | * |  |  |  | Summary | ELC: | Existing/Pro posed |
|  |  |  |  |  |  | Leg (T9) | 43.2 | Pass |
|  |  |  |  |  |  | Diagonal (T4) | 55.5 | Pass |
|  |  |  |  |  |  | Horizontal (T9) | 33.9 | Pass |
|  |  |  |  |  |  | Top Girt (T1) | 4.6 | Pass |
|  |  |  |  |  |  | Guy A (T5) | 50.9 | Pass |
|  |  |  |  |  |  | Guy B (T5) | 54.6 | Pass |
|  |  |  |  |  |  | Guy C (T5) | 53.4 | Pass |
|  |  |  |  |  |  | Top Guy Pull-Off (T5) | 28.2 | Pass |
|  |  |  |  |  |  | Bottom Guy Pull-Off (T1) | 19.1 | Pass |
|  |  |  |  |  |  | Torque Arm Top (T1) | 17.5 | Pass |
|  |  |  |  |  |  | Torque Arm Bottom (T1) | 12.4 | Pass |
|  |  |  |  |  |  | Bolt Checks | 52.0 | Pass |
|  |  |  |  |  |  | Rating $=$ | 55.5 | Pass |

## APPENDIX C

Tower Elevation Drawing


Feed Line Distribution Chart
$\qquad$ ound $\qquad$ Flat $\qquad$ 6' $-160^{\prime}$
App In Face $\qquad$ App Out Face $\qquad$ Truss Leg

| GPD Group 520 S. Main Street Akron, OH 44311 <br> Phone: 330.572.2201 FAX: | ${ }^{\text {pob: }}$ SNET020 NORFOLK |  |  |
| :---: | :---: | :---: | :---: |
|  | Project 2015723.01.SNET020.03 |  |  |
|  | Client: AT\&T Towers | Drawn by.jdischinger | Appd: |
|  | Code: TIAVEIA-222-F | Date: 01/28/15 | TS |
|  |  |  |  |

## Feed Line Plan

$\qquad$ Round $\qquad$ Flat App in Face $\qquad$ App Out Face


|  | Pob: SNET020 NORFOLK |  |  |
| :---: | :---: | :---: | :---: |
|  |  |  |  |
|  | Client: AT\&T Towers | Drawn by. jdischinger | App'd: |
|  | Code: TIA/EIA-222-F | Date: 01/28/15 | le: NTS |
|  | Path: ${ }_{\text {Nupgi liatand SNETO2 }}$ |  | g No. E-7 |

## APPENDIX D

Foundation Analysis


| Pad \& Pier Geometry |  |  |
| :---: | :---: | :---: |
| Pier Width, $\varnothing$ | 0 | ft |
| Pad Length, L | 4 | ft |
| Pad Width, W | 4 | ft |
| Pad Thickness, t | 4 | ft |
| Depth, D | 3.5 | ft |
| Height Above Grade, HG | 0.5 | ft |


| Pad \& Pier Reinforcing |  |  |
| :---: | :---: | :--- |
| Rebar Fy | 60 | ksi |
| Concrete Fc' | 3 | ksi |
| Clear Cover | 3 | in |
| Reinforced Top \& Bottom? | Yes |  |
| Pad Reinforcing Size | $\# 8$ |  |
| Pad Quantity Per Layer | 6 |  |
| Pier Rebar Size |  |  |
| Pier Quantity of Rebar |  |  |


| Soil Properties |  |
| :---: | :---: |
| Soil Type | Cohesive |
| Soil Unit Weight | 135 pcf |
| Cohesion, Cu | 2.5 ksf |
| Bearing Type | Net |
| Ultimate Bearing | 15 ksf |
| Water Table Depth | 99 ft |
| Frost Depth | 3.33 ft |

[^1]| Tower Reactions |  |
| :---: | :---: |
| Moment | $0 \mathrm{k}-\mathrm{ft}$ |
| Axial | 87.42 k |
| Shear | 3.05 k |


| Base Foundation Reinforcement Check | Code |
| :--- | :---: |
| SNET020 / NORFOLK | TIA/EIA-222-F |
| 2015723.1.SNET020.03 |  |


| Overall Capacities |  |  |
| :---: | :---: | :---: |
| Reinforcement Capacity | $15.4 \%$ | OK |
| As Min Met? | Yes |  |
| Controlling Capacity | $\mathbf{1 5 . 4 \%}$ | OK |


| Pad \& Pier Geometry |  |
| :---: | :---: |
| Height | 3.5 ft |
| Height above Grade | 0.5 ft |
| Pad Length, L | 4 ft |
| Pad Width, W | 4 ft |
| Pad Thickness | 4 ft |
| Pier Shape | Square |
| Square Pier Width | 0 ft |


| Pad \& Pier Reinforcing |  |
| :---: | :---: |
| Reinforcing Known | Yes |
| $f_{c}^{\prime}$ | 3 ksi |
| Clear Cover | 3 in |
| Rebar Fy | 60 ksi |
| Pad Rebar Size | $\# 8$ |
| Pad Rebar Quantity | 6 |
| Pier Rebar Size |  |
| Pier Rebar Quantity |  |


| Unit Weights |  |
| :---: | :---: |
| Concrete Unit Weight | 150 pcf |
| Soil Unit Weight | 135 pcf |
| Orthogonal Bearing |  |
| $\mathrm{Q}_{\max }$ | 7.14 ksf |
| $\mathrm{Q}_{\min }$ | 4.86 ksf |


| Pad Moment Capacity |  |  |
| :---: | :---: | :--- |
| 中 (bending) $=$ | 0.90 |  |
| $\mathrm{M}_{\mathrm{u}}=$ | $12.14 \mathrm{k}-\mathrm{ft}$ |  |
| $\phi \mathrm{M}_{\mathrm{n}}=$ | $225.77 \mathrm{k}-\mathrm{ft}$ |  |
| Moment Capacity | $5.4 \%$ | OK |
| One-Way (Wide-Beam) | Shear |  |
| $\mathrm{V}_{\mathrm{u}}=$ | 2.30 psi |  |
| $\phi \mathrm{V}_{\mathrm{n}}=$ | 82.16 psi |  |
| Shear Capacity | $2.8 \%$ | OK |
| Two-Way (Punching) | Shear |  |
| $\mathrm{V}_{\mathrm{u}}=$ | 25.25 psi |  |
| $\phi \mathrm{V}_{\mathrm{n}}=$ | 164.32 psi |  |
| Shear Capacity | $15.4 \%$ | OK |



| Soil Capacity Calcualtions |  |  |
| :---: | :---: | :--- |
| $\mathrm{W}_{\mathrm{s}}$ | -3.45 k |  |
| $\mathrm{W}_{\mathrm{c}}$ | 42.19 k |  |
| $\left(\mathrm{W}_{\mathrm{s}}+\mathrm{W}_{\mathrm{I}} / 1.5\right.$ | 25.82 k |  |
| $\left(\mathrm{W}_{\mathrm{s}} / 2\right)+\left(\mathrm{W}_{\mathrm{c}} / 1.25\right)$ | 32.02 k |  |
| Uplift Resistance | 25.82 k |  |
| Horizontal Resistance | 61.75 k |  |
| Uplift Capacity= | $95.5 \%$ | OK |
| Horizontal Capacity $=$ | $44.2 \%$ | OK |


| Anchor Block Reinformement |  |
| :---: | :---: |
| is Reinforcement Known? | yes |
| Fc' | 4 ksi |
| Fy | 60 ksi |
| Clear Cover | 3 in |
| Top Bar Size | $\# 8$ |
| Top Bar Quantity | 5 |
| Front Bar Size | $\# 8$ |
| Front Bar Quantity | 4 |
| Back \& Bottom Bar Size | $\# 8$ |
| Back \& Bottom Bar Quantity | 0 |


| Reinforcement Capacity Calculations |  |  |
| :--- | :---: | :--- |
| Moment Check |  |  |
| $\mathrm{M}_{\mathrm{u}}=$ | $55.47 \mathrm{k}-\mathrm{ft}$ |  |
| $\phi \mathrm{M}_{\mathrm{n}}=$ | $782.60 \mathrm{k}-\mathrm{ft}$ |  |
| Moment Capacity | $7.1 \% \quad$ OK |  |
| Minimum Reinforcment |  |  |
| $\mathrm{A}_{\text {smin }}$ Requirements Met? | Yes |  |



GPD Guyed Tower Anchor Foundation Analysis (Rev F) - V1.03

environmental | engineering | due diligence

# RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS 

T-Mobile Existing Facility
Site ID: CT11349A

Norfolk SNET
453 Loon Meadow Road
Norfolk, CT 06058
February 24, 2015
EBI Project Number: 6215001280

| Site Compliance Summary |  |
| :---: | :---: |
| Compliance Status: | COMPLIANT |
| Site total MPE\% of <br> FCC general public <br> allowable limit: | $29.96 \%$ |

February 24, 2015

T-Mobile USA
Attn: Jason Overbey, RF Manager
35 Griffin Road South
Bloomfield, CT 06002

Emissions Analysis for Site: CT11349A - Norfolk SNET

EBI Consulting was directed to analyze the proposed T-Mobile facility located at 453 Loon Meadow
Road, Norfolk, CT, for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (\% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu \mathrm{W} / \mathrm{cm} 2$ ). The number of $\mu \mathrm{W} / \mathrm{cm}^{2}$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307 (b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter $\left(\mu \mathrm{W} / \mathrm{cm}^{2}\right)$. The general population exposure limit for the 700 MHz Band is $467 \mu \mathrm{~W} / \mathrm{cm}^{2}$, and the general population exposure limit for the PCS and AWS bands is $1000 \mu \mathrm{~W} / \mathrm{cm}^{2}$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

## CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at $\mathbf{4 5 3}$ Loon Meadow Road, Norfolk, CT, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB , was focused at the base of the tower. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

1) 2 GSM channels (PCS Band -1900 MHz ) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel
2) 2 UMTS channels (PCS Band -1900 MHz ) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
3) 2 LTE channels (PCS Band - 1900 MHz ) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
4) 1 LTE channel ( 700 MHz Band) was considered for each sector of the proposed installation. This channel has a transmit power of 30 Watts.
5) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
environmental | engineering | due diligence
6) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
7) The antennas used in this modeling are the EMS RR90_17_02DP for 1900 MHz (PCS) channels and the Commscope LNX-6515DS-VTM for 700 MHz channels. This is based on feedback from the carrier with regards to anticipated antenna selection. The EMS
RR90_17_02DP has a maximum gain of $\mathbf{1 4 . 4} \mathbf{d B d}$ at its main lobe. The Commscope LNX-6515DS-VTM has a maximum gain of 14.6 dBd at its main lobe. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB , was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
8) The antenna mounting height centerline of the proposed antennas is $\mathbf{1 2 0}$ feet above ground level (AGL).
9) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general public threshold limits.

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## T-Mobile Site Inventory and Power Data

| Sector: | A | Sector: | B | Sector: | C |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Antenna \#: | 1 | Antenna \#: | 1 | Anterna \#: | 1 |
| Make / Model: | $\begin{gathered} \text { EMS } \\ \text { RR90_17_02DP } \end{gathered}$ | Make / Model: | EMS RR90_17_02DP | Make / Model: | $\begin{gathered} \text { EMS } \\ \text { RR90_17_02DP } \end{gathered}$ |
| Gain: | 14.4 dBd | Gain: | 14.4 dBd | Gain: | 14.4 dBd |
| Height (AGL): | 120 | Height (AGL): | 120 | Height (AGL): | 120 |
| Frequency Bands | $\begin{aligned} & 1900 \mathrm{MHz}(\mathrm{PCS}) / \\ & 2100 \mathrm{MHz} \text { (AWS) } \end{aligned}$ | Frequency Bands | $\begin{aligned} & 1900 \mathrm{MHz}(\mathrm{PCS}) / \\ & 2100 \mathrm{MHz} \text { (AWS) } \end{aligned}$ | Frequency Bands | $\begin{aligned} & 1900 \mathrm{MHz}(\mathrm{PCS}) / \\ & 2100 \mathrm{MHz} \text { (AWS) } \end{aligned}$ |
| Channel Count | 6 | Chamnel Count | 6 | \# PCS Channels: | 6 |
| Total TX Power: | 240 | Total TX Power: | 240 | \# AWS Channels: | 240 |
| ERP (W): | 6,610.15 | ERP (W): | 6,610.15 | ERP (W): | 6,610.15 |
| Antenna A1 MPE\% | 1.83 | Antenna B1 MPE\% | 1.83 | Antenna C1 MPE\% | 1.83 |
| Antenna \#: | 2 | Antenna \#: | 2 | Antenna \#: | 2 |
| Make / Model: | Commscope LNX- 6515DS-VTM | Make / Model: | Commscope LNX-6515DS-VTM | Make / Model: | Commscope LNX-6515DS-VTM |
| Gain: | 14.6 dBd | Gain: | 14.6 dBd | Gain: | 14.6 dBd |
| Height (AGL): | 120 | Height (AGL): | 120 | Height (AGL): | 120 |
| Frequency Bands | 700 Mhz | Frequency Bands | 700 Mhz | Frequency Bands | 700 Mhz |
| Channel Count | 1 | Channel Count | 1 | Channel Count | 1 |
| Total TX Power: | 30 | Total TX Power: | 30 | Total TX Power: | 30 |
| ERP (W): | 865.21 | ERP (W): | 865.21 | ERP (W): | 865.21 |
| Antenna A2 MPE\% | 0.51 | Antenna B2 MPE\% | 0.51 | Antenna C2 MPE\% | 0.51 |


| Site Composite MPE \% |  |
| :---: | :---: |
| Carrier | MPE \% |
| T-Mobile | $\mathbf{7 . 0 2}$ |
| AT\&T | $15.95 \%$ |
| PageNet | $3.38 \%$ |
| Sprint | $3.61 \%$ |
| Site Total MPE \%: | $\mathbf{2 9 . 9 6 \%}$ |


| T-Mobile Sector 1 Total: | $2.34 \%$ |
| :---: | :---: |
| T-Mobile Sector 2 Total: | $2.34 \%$ |
| T-Mobile Sector 3 Total: | $2.34 \%$ |
| Site Total: |  |

## Summary

All calculations performed for this analysis yielded results that were within the allowable limits for general public exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general public exposure to RF Emissions are shown here:

| T-Mobile Sector | Power Density Value (\%) |
| ---: | :--- |
| Sector 1: | $2.34 \%$ |
| Sector 2: | $2.34 \%$ |
| Sector 3: | $2.34 \%$ |
| T-Mobile Total: | $7.02 \%$ |
|  |  |
| Site Total: | $29.96 \%$ |
| Site Compliance Status: | COMPLIANT |

The anticipated composite MPE value for this site assuming all carriers present is $\mathbf{2 9 . 9 6 \%}$ of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a $5 \%$ contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable $100 \%$ threshold standard per the federal government.


## Scott Heffernan

RF Engineering Director

## EBI Consulting

21 B Street
Burlington, MA 01803


[^0]:    ${ }^{1}$ The Staff Report for this Petition does not contain any relevant limitations on the configuration of the Norfoik Facility.

[^1]:    GPD Mat Foundation Analysis - V1.02

