

EM-CING-097-110602



Cingular Wireless PCS, LLC
500 Enterprise Drive
Rocky Hill, Connecticut 06067-3900
Phone: (860) 463-5511
Fax: (860) 513-7190

Douglas L. Culp
Real Estate Consultant

HAND DELIVERED

June 2, 2011

Ms. Linda Roberts
Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, Connecticut 06051

RECEIVED
JUN - 2 2011
CONNECTICUT
SITING COUNCIL

Re: New Cingular Wireless PCS, LLC notice of intent to modify an existing telecommunications facility located at 8 Ferris Road Newtown, CT (owner TowerCo)

Dear Ms. Roberts:

In order to accommodate technological changes, implement Uniform Mobile Telecommunications System (“UMTS”) and/or Long Term Evolution (“LTE”) capabilities, and enhance system performance in the State of Connecticut, New Cingular Wireless PCS, LLC (“AT&T”) plans to modify the equipment configurations at many of its existing cell sites. Please accept this letter and attachments as notification, pursuant to R.C.S.A. Section 16-50j-73, of construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2). In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter and attachments is being sent to the chief elected official of the municipality in which the affected cell site is located.

UMTS technology offers services to mobile computer and phone users anywhere in the world. Based on the Global System for Mobile (“GSM”) communication standard, UMTS is the planned worldwide standard for mobile users. UMTS, fully implemented, gives computer and phone users high-speed access to the Internet as they travel. They have the same capabilities even when they roam, through both terrestrial wireless and satellite transmissions.

LTE is a new high-performance air interface for cellular mobile communications. It is designed to increase the capacity and speed of mobile telephone networks.

Attached is a summary of the planned modifications, including power density calculations reflecting the change in AT&T’s operations at the site. Also included is documentation of the structural sufficiency of the tower to accommodate the revised antenna configuration.

The changes to the facility do not constitute modifications as defined in Connecticut General Statutes (“C.G.S.”) Section 16-50i(d) because the general physical characteristics of the facility will not be significantly changed or altered. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in R.C.S.A. Section 16-50j-72(b)(2).

1. The height of the overall structure will be unaffected.
2. The proposed changes will not extend the site boundaries. There will be no effect on the site compound other than some enlarged equipment pads as may be noted in the attachments.
3. The proposed changes will not increase the noise level at the existing facility by six decibels or more.
4. Radio frequency power density may increase due to use of one or more GSM channel for UMTS transmissions. Moreover, LTE will utilize additional radio frequencies newly-licensed by the FCC for cellular mobile communications. However, the changes will not increase the calculated “worst case” power density for the combined operations at the site to a level at or above the applicable standard for uncontrolled environments as calculated for a mixed frequency site.

For the foregoing reasons, New Cingular Wireless respectfully submits that the proposed changes at the referenced site constitute exempt modifications under R.C.S.A. Section 16-50j-72(b)(2).

Please feel free to call me at (860) 463-5511 with questions concerning this matter. Thank you for your consideration.

Sincerely,



Douglas L. Culp
Real Estate Consultant

Attachments

NEW CINGULAR WIRELESS PCS, LLC
Equipment Modification

8 Ferris Road Newtown, CT
Site Number CT5511
Exempt Mods: 08/02, 05/11 and 12/06

Tower Owner/Manager: Tower Co

Equipment configuration: Monopole

Current and/or approved: Three PowerWave P7770 antennas @ 88 ft
Six PowerWave TMA's @ 88 ft
Six runs 7/8 inch coax to 88 ft
Equipment on Concrete Pad

Planned Modifications: Retain existing PowerWave P7770 Antenna's and TMA's @ 88 ft
Retain all Coax Cabling
Install three PowerWave P65-16 antennas or equivalent @ 88 ft
Install six remote radio heads Ericsson RRUS-11 @ 88 ft
Install one Raycap Fiber Power Connector/ Surge Suppressor – DC6-48-60-18-8F @ 88 ft
Install one fiber and two DC power cables @ 88 ft

Power Density:

Worst-case calculations for existing wireless operations at the site, using standard parameters for other carriers, indicate a radio frequency electromagnetic radiation power density, measured at ground level beside the Tower, of approximately 85.1% of the standard adopted by the FCC. As depicted in the second table below, the total radio frequency electromagnetic radiation power density following proposed modifications would be approximately 89.8% of the standard.

Existing

Other Users							56.45
AT&T UMTS	88	1900 Band	1	500	0.0232	1.0000	2.32
AT&T UMTS	88	800 Band	1	500	0.0232	0.5867	3.96
AT&T GSM	88	800Band	7	296	0.0962	0.5867	16.40
AT&T GSM	88	1900 Band	3	427	0.0595	1.0000	5.95
Total							85.1%

* Data for other users are from Siting Council records.

Proposed

Company	Centerline Ht (feet)	Frequency (MHz)	Number of Channels	Power Per Channel (Watts)	Power Density (mW/cm ²)	Standard Limits (mW/cm ²)	Percent of Limit
Other Users							56.45
AT&T UMTS	88	800 Band	1	500	0.0232	0.5867	3.96
AT&T UMTS	88	1900 Band	1	500	0.0232	1.0000	2.32
AT&T GSM	88	1900 Band	3	427	0.0595	1.0000	5.95
AT&T GSM	88	880 - 894	7	296	0.0962	0.5867	16.40
AT&T LTE	88	740 - 746	1	500	0.0232	0.4933	4.71
Total							89.8%

* Data for other users are from Siting Council records.

Structural information:

The attached structural analysis demonstrates that the monopole and foundation have adequate structural capacity to accommodate the proposed modifications. (Vertical Solutions, Inc. dated 5-23-11).

NEW CINGULAR WIRELESS PCS, LLC

WIRELESS COMMUNICATIONS FACILITY CT5511

8 FERRIS ROAD NEWTOWN, CONNECTICUT



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NEW CINGULAR WIRELESS PCS, LLC
500 ENTERPRISE DRIVE
ROCKY HILL, CT 06067

CHA
Engineering & Architecture

200 Main Street, Suite 210, Rocky Hill, CT 06067-2457
www.cha.com

Drawn by: [Signature] © 2011

Printed by: [Signature]

Job No.: 2457 - 4000

Date: 2/26/11

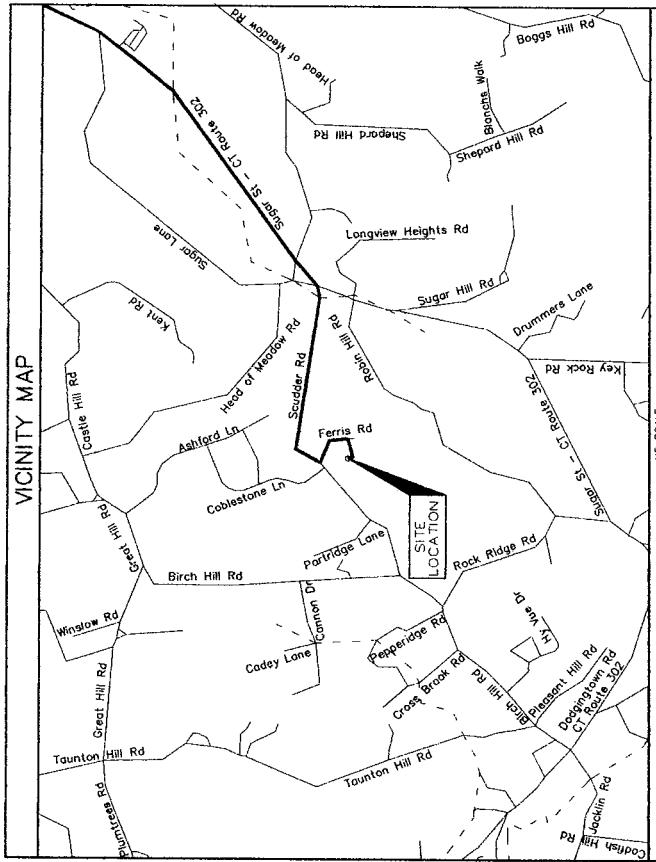
PROJECT SUMMARY

SITE NUMBER: CT5511
SITE NAME: NEWTOWN - SR302
SITE ADDRESS: 8 FERRIS ROAD
STRUCTURE OWNER: TOMEKO ASSETS LLC
APPLICANT: NEW CINGULAR WIRELESS PCS, LLC
CONTACT: 213 S. DEANE HIGHWAY
SUITE 212
ROCKY HILL, CT 06067
MICHAEL D. FOLEY
(203) 414-1844
COORDINATES: 41° 23' 23.57" N
73° 20' 18.47" W
HORIZONTAL DATUM: NAD 83
ENGINEER: CHA, INC.
213 S. DEANE HIGHWAY
SUITE 212
ROCKY HILL, CT 06067
PAUL JUSTINI
(860) 257-4557

DRIVING DIRECTIONS

FROM HARTFORD:
 1. TAKE I-91N
 2. TAKE EXIT 10 FOR US-6W TOWARD
 NEWTOWN/SANDY HOOK
 3. TURN RIGHT ONTO CT-34W/US-6W/CHURCH HILL
 ROAD.
 4. TURN LEFT ONTO MAIN STREET.
 5. TAKE FIRST RIGHT ONTO CT-302W/SUGAR STREET.
 6. TURN LEFT ONTO FERRIS ROAD.
 7. TURN RIGHT ONTO ACCESS DRIVE AND FOLLOW TO
 TOWER.

VICINITY MAP



HEET INDEX

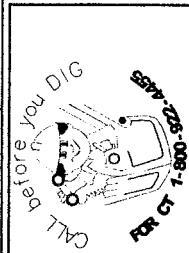
SHEET NO.	SHEET INDEX		REVISION HISTORY
	NO.	DATE	
T01	01	05 / 16 / 11	01/27/2011 ISSUED FOR REVIEW
C01	01	05 / 16 / 11	01/27/2011 ISSUED FOR CONSTRUCTION
C02	01	05 / 16 / 11	01/27/2011 CDE PNL APPROVED
C03	01	05 / 16 / 11	01/27/2011 ELEVATION AND ANTENNA PLAN
C04	01	05 / 16 / 11	01/27/2011 STRUCTURAL DETAILS
C05	01	05 / 16 / 11	01/27/2011 GROUNDS DETAILS & PLUMBING DIAGRAM
ED1	01	05 / 16 / 11	01/27/2011 GENERAL NOTES
GNO1	01	05 / 16 / 11	01/27/2011 GENERAL NOTES
GNO2	01	05 / 16 / 11	01/27/2011 GENERAL NOTES

DO NOT SCALE DRAWINGS

CONTRACTOR SHALL VERIFY ALL PLANS & CONDITIONS ON DIMENSIONS & CONDITIONS ON THE JOB SITE. S/he SHALL NOT MAKE ANY CHANGES OR ALTERATIONS OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

SHEET TITLE
TITLE SHEET

SHEET NUMBER
T01



MAY 16, 2011



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NEW CINGULAR WIRELESS PCS, LLC
500 ENTERPRISE DRIVE
ROCKY HILL, CT 06067

CHA

Drawing Change 0201

222702 Rev. 0000000204

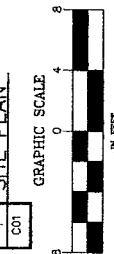
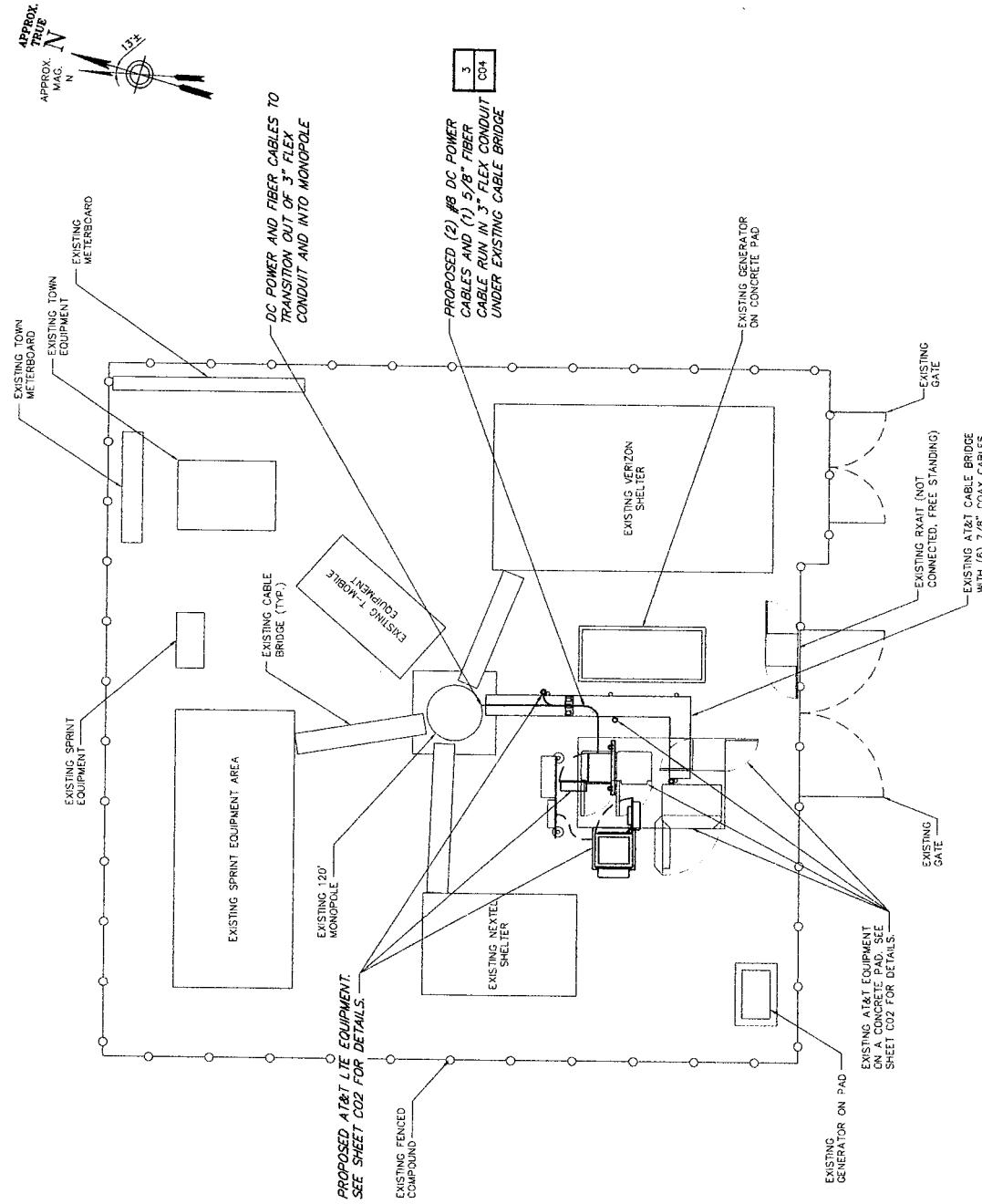
222702 Rev. 0000000204

CMA PROJECT NO. 10000

NO.	SUBMITTED
0	03/20/11 ISSUED FOR REVIEW
1	03/16/11 CDA, PHM APPROVED
2	03/16/11 ISSUED FOR CONSTRUCTION
3	03/16/11 CDA, PHM APPROVED

STATE OF CONNECTICUT
PROFESSIONAL ENGINEER
LICENSING BOARD
No. 17827
SUBJECT
TO THE
RECORD
IT IS A VIOLATION OF LAW FOR ANY PERSON,
UNLESS AUTHORIZED BY THE PROFESSIONAL
ENGINEER, TO ALTER THIS DOCUMENT.

SITE ID:
C15511
SITE NAME:
NEWTOWN - SR302
SITE ADDRESS:
8 FERRIS ROAD
NEWTOWN, CT
06470
FAIRFIELD COUNTY

SITE TITLE
SITE PLANSHEET NUMBER
C01

NOTE:
1. PLANS BASED ON A SITE VISIT BY CHA ON MARCH 25, 2011, AND DRAWINGS
PREPARED BY HUDSON DESIGN GROUP LLC, LAST DATED 08/05/08.



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NEW CIRCULAR WIRELESS POS, LLC
500 ENTERPRISE DRIVE
ROCKY HILL, CT 06067



2010 Main Avenue, Wethersfield, CT 06519-2200
Tel: (860) 529-4467 • www.cha.com

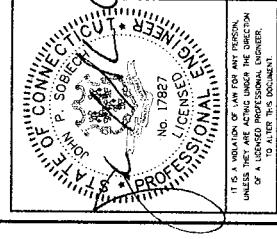
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Rev: 0

Sheet No.: 008/008

Scale: 1:100

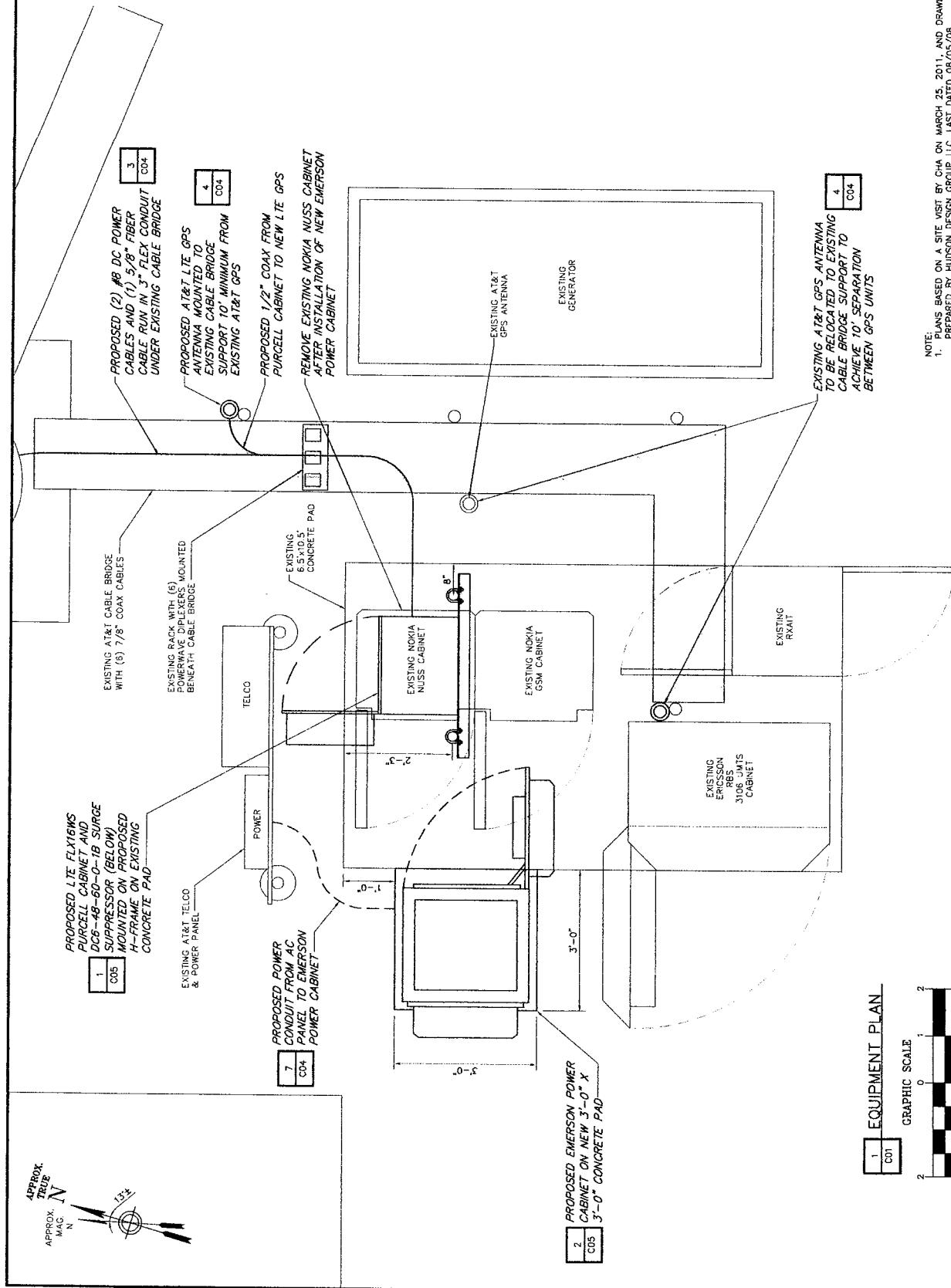
Project No.: 20702 - 1037 - A5000



SITE NAME:
C15511

SITE ADDRESS:
NEWTOWN — SR302
8 FERRIS ROAD
NEWTOWN, CT
06470
FAIRFIELD COUNTY

SHEET TITLE:
EQUIPMENT PLAN
SHEET NUMBER:
CO2

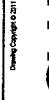


NOTE:
1. PLANS BASED ON A SITE VISIT BY CHA ON MARCH 25, 2011, AND DRAWINGS
PREPARED BY HUDSON DESIGN GROUP LLC, LAST DATED 08/05/08.



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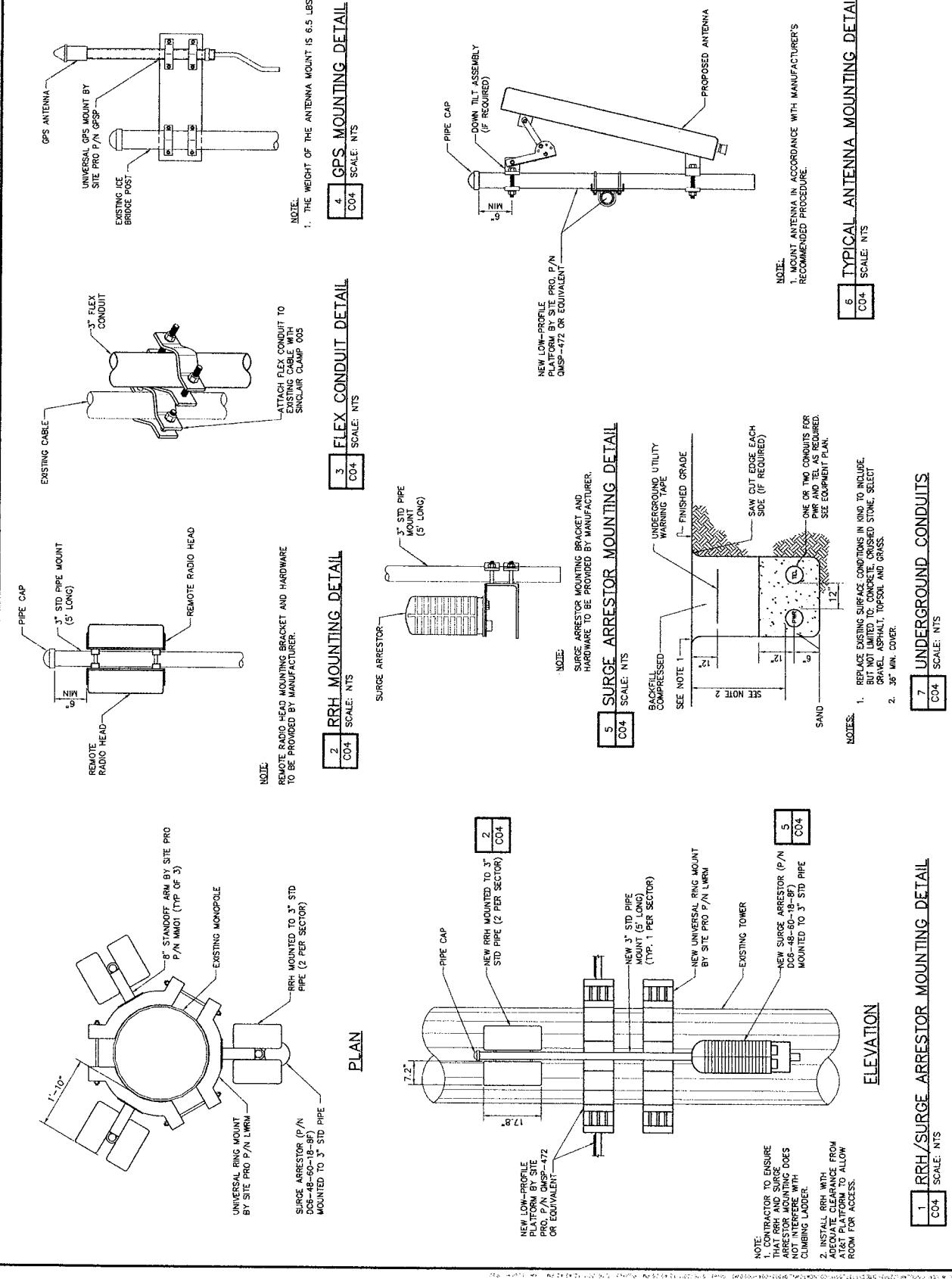
NEW Cingular Wireless POS, LLC
500 Enterprise Drive
Rocky Hill, CT 06067



Drawing Control 01/21

CWA PROJECT NO.

22702 - 1037 - 45000





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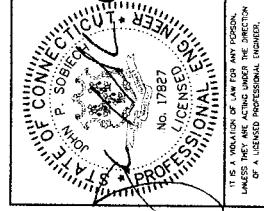
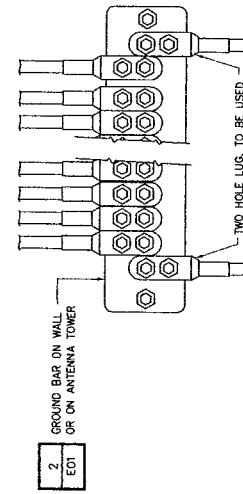
NEW CIRCULAR WIRELESS PCS, LLC
500 ENTERPRISE DRIVE
ROCKY HILL, CT 06067**CHA**2425 Main Street, Newton, MA 02461 | 781-255-0600 | www.chatele.com

Proj. No. 22722 - Date: 05/20/11

Rev. No. 1

Draf. No. 000

Proj. No.	22722
Sheet No.	000
Draft No.	1
Rev. No.	05/20/11
Date	2011

Site No.: C15511
Site Name: NEWTOWN - SR302
Site Address: 8 FERRIS ROAD
 NEWTOWN, CT 06470
Fairfield CountySHEET TITLE: GROUNDING DETAILS & PLUMBING DIAGRAM
SHEET NUMBER: E01

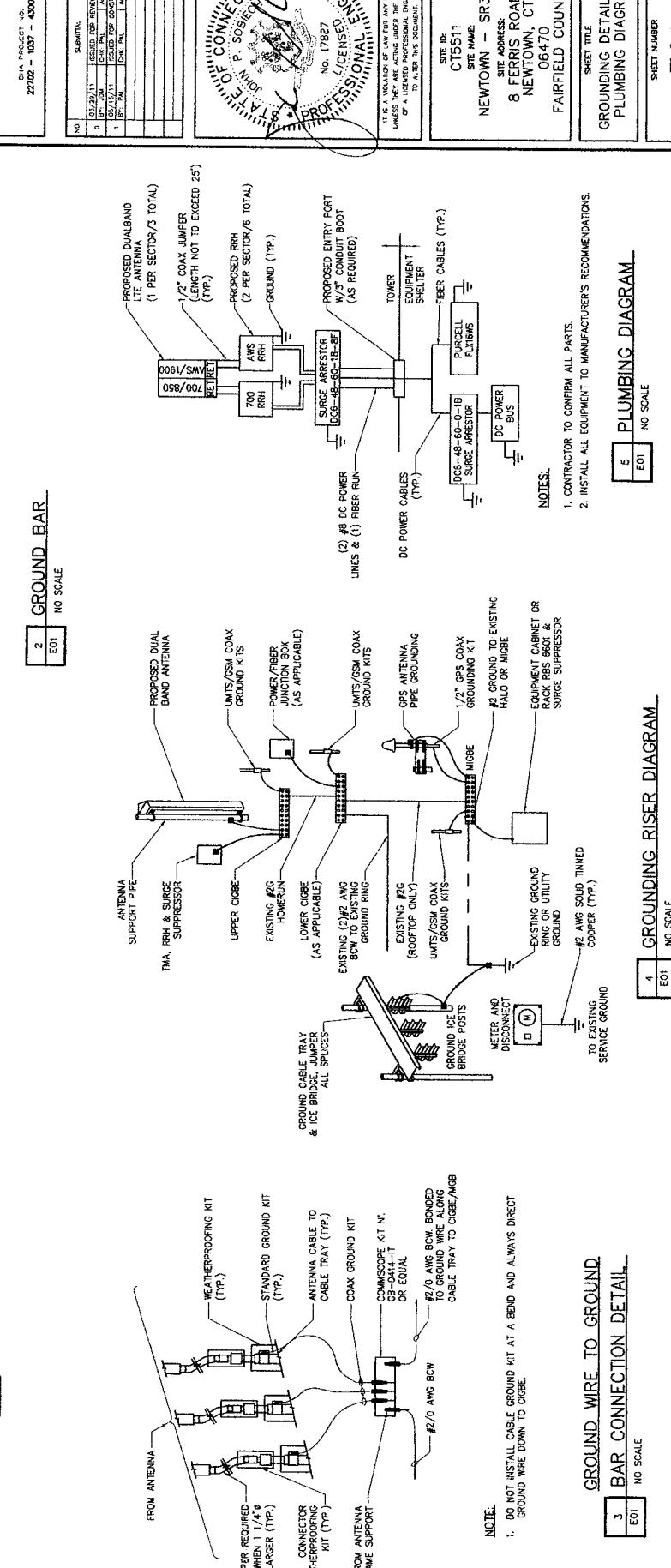
1 GROUND WIRE INSTALLATION TO GROUND BAR

E01 NO SCALE

2 GROUND BAR

E01 NO SCALE

- LEGEND**
- 1 - COPPER GROUND BAR, HOLE CENTERS TO MATCH NEMA DOUBLE LUG
 - 2 - INSULATORS, NEWTON INSTRUMENT CAT. NO. 3061-4. (NOT TO BE USED ON BARS PHYSICALLY ATTACHED TO TOWER)
 - 3 - 5/8" LOCKWASHER, NEWTON INSTRUMENT CO. CAT. NO. 3015-8
 - 4 - WALL MOUNTING BRACKET, NEWTON INSTRUMENT CO. CAT. NO. A-6056
 - 5 - 5/8"-11 X 1" H-HC SCREWS, NEWTON INSTRUMENT CO. CAT. NO. 3012-1



GROUNDING SYSTEM NOTES:

SECTION 16120 CONDUCTORS

1. CONDUCTOR USED FOR CELLULAR GROUNDING SYSTEM
EGR - #2 AWG ANNEALED SOLID TINNED BARE COPPER
INTER-BUS EXTENSION (FROM IGR TO EGR) - #2 AWG ANNEALED SOLID TINNED BARE COPPER.
EXTERNAL BOND CONNECTIONS TO EGR - #2 AWG ANNEALED SOLID TINNED BARE COPPER.
TOWER BOND CONNECTION TO EGR - #2 AWG SOLID COPPER.
2. MINIMUM BENDING RADIUS - 8" NOMINAL AND 8" MINIMUM. CELLULAR GROUNDING CONDUCTOR SHALL BE AS STRAIGHT AS POSSIBLE WITH MINIMUM 8" RADIUS.
3. CONNECTIONS (MECHANICAL)
COMPRESSOR LUG CONNECTOR - 15 TON COMPRESSION, 2 HOLE, LONG BARREL ELECTRO TINNED LATED. HIGH CONDUCTIVITY COPPER, BODY RATED, USE 1/4" DIA. BOLT. 3/4" SPACING LUGS TO BOND OBJECTS FROM IGR.
EXOTHERMIC WELD LUG CONNECTOR - 2 HOLE OFFSET, ELECTRO TINNED PLATED. HIGH CONDUCTIVITY COPPER LUG, USE 1/4" DA BOLT, 1-3/4" SPACING LUGS.
CONNECTOR SHALL BE CALLED DECOUPLER CONNECTION. (ABLE TO SPAN SURFACE) TYPE "C". EXOTHERMIC WELD TO LUG AS REQUIRED.
- "C" TAB COMPRESSION CONNECTOR - HIGH CONDUCTIVITY COPPER FOR MAIN-BRANCH TAPPING. CONNECTOR SHALL BE BURNED "HTAP" SERIES OR EQUAL. USE MATCHING MANUFACTURER TOOL AND DIE FOR COMPRESSION CONNECTION. APPLY ANTI-oxidANT CONDUCTIVITY ENHANCER COMPUND ON SURFACES THAT ARE COMPRESSED. SURFACES INTENDED TO BE CONNECTED WITH MECHANICAL CONNECTORS SHALL BE BARE METAL TO BARE METAL. PRIME AND PAINT OVER BONDED AREA TO PREVENT CORROSION.
4. CONNECTIONS, GRADE (EXOTHERMIC) PROVIDED CADWELL CONNECTIONS - STYLE AND TYPE AS REQUIRED.
5. WHEEL BONDING 1/2" TO 42" - USE EXOTHERMIC WELD CONNECTION.
6. WHEN BONDING #2 TO FENCE POST
USE EXOTHERMIC WELD "C" WELD TYPE VS. CONNECTION TO FENCE POST STEEL SURFACE. TEST WELD FOR POSSIBLE BURN THROUGH, PATCH WELDED AREA WITH GALVANIZED COATING AS REQUIRED FOR PROPER WELDED PERMANENT BOND.
REFER TO MANUFACTURERS REQUIREMENTS FOR DETAILS.

SECTION 16050 GROUNDING

- 1.01 ALL NON-CURRENT CARRYING PARTS OF THE ELECTRICAL SYSTEM AND CONNECTED TO PROVIDE AN INDEPENDENT RETURN PATH TO THE EQUIPMENT GROUNDING SOURCES.
- 1.02 GROUNDING SYSTEM WILL BE IN ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE AND LOCAL INSPECTOR HAVING JURISDICTION.
- 1.03 ELECTRICAL AC SERVICE GROUNDED SYSTEM - GROUNDING AT MAIN SERVICE OVERCURRENT PROTECTION DEVICE
- A. THE GROUNDED CONDUCTOR (NEUTRAL) OF THE INCOMING SERVICE FEEDERS (LINE SIDE OF THE NEUTER SOCKET) SHALL TERMINATE INTO THE MAIN OVERCURRENT DEVICE ENCLOSEMENT. EQUIPMENT GROUND BAR KIT SHALL BE LOOCHED TO THE ENCLOSURE WITH THE BARE METAL TO BARE METAL, PROVIDED BONDING SURFACES BETWEEN THE BARE METAL EQUIPMENT GROUND BAR AND THE MAIN OVERCURRENT DEVICE ENCLOSEMENT.
- B. THE GROUNDED ELECTRODE CONDUCTOR SHALL EXTEND CONTINUOUSLY, WITHOUT SPLICES OR JOINTS FROM THE MAIN OVERCURRENT DEVICE ENCLOSEMENT TO THE MAIN SWITCHBOARD GROUND TERMINAL.
- C. THE MAIN SERVICE OVERCURRENT PROTECTION DEVICE ENCLOSURE EQUIPMENT GROUND BAR KIT SHALL BE LOOCHED TO THE ENCLOSURE WITH THE BARE METAL TO BARE METAL, PROVIDED BONDING SURFACES BETWEEN THE BARE METAL EQUIPMENT GROUND BAR AND THE MAIN OVERCURRENT DEVICE ENCLOSEMENT. THE BARE METAL LINE SIDE OF THE OVERCURRENT DEVICE ENCLOSEMENT SHALL HAVE GROUNDING TYPE BUSBUSES. THE BUSBUSES SHALL BE BONDED TOGETHER WITH #10 AWG BARE COPPER WHICH IN TURN IS TERMINATED INTO THE EQUIPMENT GROUND BAR KIT.

1.04 CELLULAR GROUNDING SYSTEM:

- PROVIDE THE CELLULAR GROUNDING SYSTEM AS SPECIFIED ON DRAWINGS.
INCLUDING BUT NOT LIMITED TO:
-GROUND BARS
-EXTERIOR GROUNDING RING
-ANTENNA GROUND CONNECTIONS AND PLATES
- 1.05 CONTRACTOR, AFTER COMPLETION OF THE COMPLETE GROUNDING SYSTEM BUT PRIOR TO CONCEALMENT / BURIAL OF SAME, SHALL NOTIFY THE AIRPORT AUTHORITY WHO WILL MAKE A VISUAL INSPECTION OF THE GROUNDING GRID, RODS AND CONNECTIONS OF THE EXTERIOR GROUNDING SYSTEMS.

STEEL NOTES:

- SECTION 16120 CONDUCTORS
- 1.01 ALL CONDUCTORS SHALL BE THE TYPE THINN (INTERIOR) AND XHHW (EXTERIOR), 75 DEGREES C, 600 VOLT INSULATION, SOFT ANNEALED STRANDED COPPER. #10 AWG AND SMALLER SHALL BE SPLICED USING SOLDERLESS PRESSURE CONNECTORS. ACCEPTABLE, #12 AWG SHALL BE MINIMUM SIZE CONDUCTOR OR LINE VOLTAGE BRANCH CIRCUITS. REFER TO PANEL SCHEDULE FOR BRANCH CIRCUIT CONDUCTOR SIZES. CONDUCTORS SHALL BE COLOR CODED FOR CONSISTENT PHASE IDENTIFICATION:

120 / 240 VAC - 1 PHASE, 3 WIRE SYSTEM
PHASE:

A
B
N
C

CONTINUOUS WHITE
CONTINUOUS GREEN

- 1.02 MINIMUM BENDING RADIUS FOR CONDUCTORS SHALL BE 12 TIMES THE LARGEST DIAMETER OF BRANCH CIRCUIT CONDUCTOR.

- SECTION 16130 RACEWAY
1.01 CONDUIT MATERIAL SHALL BE AS FOLLOWS:

- (1) GALVANIZED RIGID CONDUIT (GRC) - FEEDERS EXPOSED TO EXTERIOR & UNDERGROUND CONDUIT SWEEPS.
(2) PVC CONDUIT - SERVICE CONDUITS AND WHERE SHOWN ON GROUNDING DETAILS.

GENERAL NOTES:

1. ALL DIMENSIONS TO, OF, AND IN EXISTING STRUCTURES SHALL BE VERIFIED IN FIELD BY CONTRACTOR WITH ALL DISCREPANCIES REPORTED TO THE ENGINEER.
2. DO NOT CHANGE THE SIZE NOR SPACING OF STRUCTURAL ELEMENTS WITHOUT THE APPROVAL OF THE ENGINEER.
3. DETAILS SHOWN ARE TYPICAL AND APPLY TO SIMILAR CONDITIONS UNLESS NOTED OTHERWISE.
4. THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.
5. BRACE STRUCTURES AS REQUIRED FOR CONSTRUCTION AND WIND LOADS UNTIL ALL STRUCTURAL ELEMENTS NEEDED FOR STABILITY ARE INSTALLED. THESE ELEMENTS ARE AS FOLLOWS: (LATERAL BRACING MEMBERS, ANCHOR BOLTS, ETC.)
6. THE DESIGN IS BASED ON THE 2005 CONNECTICUT STATE BUILDING CODE (IBC 2003), 2005 CONNECTICUT SUPPLEMENT AND THE 2009 AMENDMENT TO THE 2005 CONNECTICUT STATE BUILDING CODE AND ISEA-YELA-222-G.
7. CONTRACTOR SHALL DETERMINE EXACT LOCATION OF EXISTING UTILITIES BEFORE COMMENCING WORK. HE AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY HIS FAILURE TO EXACTLY LOCATE AND PRESERVE UNDERGROUND UTILITIES.
8. INCORRECTLY FABRICATED, DAMAGED, OR OTHERWISE MISFITTING OR NONCONFORMING MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE OWNER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER'S APPROVAL.
9. EACH CONTRACTOR SHALL COOPERATE WITH THE OWNER'S REPRESENTATIVE AND COORDINATE HIS WORK WITH THE WORK OF OTHERS.
10. DIMENSIONS NOT SHOWN SEE CIVIL DRAWINGS.
11. VERITY SIZE AND LOCATION OF EQUIPMENT PADS WITH MECHANICAL AND/OR ELECTRICAL CONTRACTOR AND EQUIPMENT MANUFACTURER.
12. CONTRACTOR TO FOLLOW ALL STATE, LOCAL AND NATIONAL CODES AS APPLICABLE.

APPURNEMENT SUPPORT BRACKET NOTES:

1. DESIGN RESPONSIBILITY OF APPURTENANCE MOUNTING BRACKETS AND POLES AND ALL COMPONENTS THEREOF AND ATTACHMENT THERETO SHALL BE THE RESPONSIBILITY OF THE MANUFACTURER. CONTRACTOR SHALL PROVIDE TO THE ENGINEER FOR APPROVAL, DETAILS OF APPURTENANCE MOUNTING BRACKETS AND POLES ASSEMBLY, INCLUDING CONNECTIONS DESIGN LOADS, AND ALL OTHER REQUIREMENT DATA. ALL SUBMISSIONS SHALL BE IN PRINT FORM AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF CONNECTICUT.
2. BRACKETS SHALL BE DESIGNED TO SUPPORT CURRENT AND FUTURE PANEL ANTENNAS, REMOTE RADIO HEADS, SURGE ARRESTORS, AND COAXIAL CABLES AS SHOWN.

1. STRUCTURAL STEEL FABRICATION AND ERECTION SHALL CONFORM TO THE LATEST EDITION OF THE AISI STEEL CONSTRUCTION MANUAL

2. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:
A. WIDE FLANGE AND CHANNEL SHAPES - A992 GR 50 (50 KS) UNLESS OTHERWISE NOTED
B. ANGLES AND PLATES - ASTM A36 (36 KS)
C. STEEL PIPE - ASTM A53, GRADE B A500 GRADE B (35 KS)

3. ALL EXTERIOR STEEL WORK SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM SPECIFICATION A123 UNLESS OTHERWISE NOTED. GALVANIZING SHALL BE PERFORMED AFTER SHOP FABRICATION TO THE GREATEST EXTENT POSSIBLE. ALL SPRINGS, SCREWS, NAILS AND WELDS IN THE GALVANIZED AREAS SHALL BE REPAVED BY FIELD TOUCHUP PRIOR TO COMPLETION OF THE WORK USING ZINC COAT GALVANIZING COMPOUND OR APPROVED EQUAL.

4. CONNECTIONS:
A. ALL BOLTS, NUTS AND WASHERS USED IN EXTERIOR APPLICATIONS SHALL BE GALVANIZED.

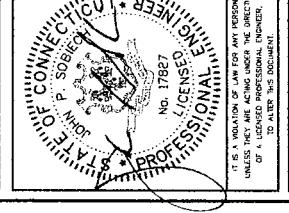
DESIGN LOADS:

- THE FOLLOWING DESIGN LOADS WERE USED FOR THIS BUILDING BASED ON THE 2005 CONNECTICUT STATE BUILDING CODE (IBC 2003), 2005 CONNECTICUT SUPPLEMENT AND THE 2009 AMENDMENT TO THE 2005 CONNECTICUT STATE BUILDING CODE AND ISEA-YELA-222-G.
- ICE LOAD: 3/4" RADIAL ON ALL COMPONENTS AND CABLE
- | |
|---|
| WIND DESIGN DATA: |
| BASIC WIND SPEED (3 SECOND GUST): 110 MPH |
| WIND EXPOSURE CATEGORY: B |

- EARTHQUAKE DESIGN DATA:
SEISMIC IMPACT FACTOR: 1.10
SEISMIC IMPACT RESPONSE ACCELERATIONS: SS=0.273 SITE CLASS: D
SEISMIC DESIGN CATEGORY: B

SECTION 16120 CONDUCTORS

1. STRUCTURAL STEEL FABRICATION AND ERECTION SHALL CONFORM TO THE LATEST EDITION OF THE AISI STEEL CONSTRUCTION MANUAL



CH-A
2/28/2011
213 Main Street, New Haven, Suite 210 • P.O. Box 1200 • CT 06513-1200
(203) 539-1740 • Fax (203) 539-1741 • www.ctpe.org

NO. 167627	ISSUED FOR ROBERT J.
02/29/2011	CHEMICAL, PETROLEUM, CHEMICAL, INDUSTRIAL PROCESS, PIPE LINE, PETROLEUM, REFINERY, PETROCHEMICAL, INDUSTRIAL, PETROCHEMICAL AND RELATED PROFESSIONAL PRACTICES
0	167627
1	167627
1	167627
1	167627

SITE ID: CT1551	SHEET TITLE: GENERAL NOTES
NEWTON - SR302 8 FERRIS ROAD NEWTON, CT 06470 FAIRFIELD COUNTY	SHEET NUMBER: GN01

FOUNDATION AND SOIL PREPARATION NOTES:

- BEAR ALL FOOTINGS ON COMPACTED STRUCTURAL FILL OR NATIVE UNDISTURBED SOIL AS APPROVED BY THE GEOTECHNICAL ENGINEER. SOIL BEARING SURFACES, PREVIOUSLY ACCEPTED BY OWNER'S REPRESENTATIVE, WHICH ARE ALLOWED TO BECOME SATURATED, FROZEN OR DISTURBED SHALL BE REMOVED TO SATISFACTION OF OWNER'S REPRESENTATIVE.
 - BEAR ALL CONCRETE PADS AND SLABS ON GRADE, ON A 6" LAYER OF ASTM #57 SOIL UNDISTURBED BELOW STONE LAYER SHALL BE COMPACTED STRUCTURAL FILL, OR NATIVE SOIL AS APPROVED BY THE GEOTECHNICAL ENGINEER. SOIL BEARING SURFACES, PREVIOUSLY ACCEPTED BY OWNER'S REPRESENTATIVE, WHICH ARE ALLOWED TO BECOME SATURATED, FROZEN OR DISTURBED SHALL BE REMOVED TO SATISFACTION OF OWNER'S REPRESENTATIVE.
 - DO NOT PLACE CONCRETE IN WATER OR ON FROZEN GROUND.
 - DO NOT ALLOW CONCRETE SUBBASE TO FREEZE DURING CONCRETE SETTING AND Curing PERIOD OR FOR A MINIMUM OF 14 DAYS AFTER PLACEMENT.
 - STRUCTURAL FILL AND BACKFILL: SOUND, DURABLE, SAND, GRAVEL, STONE, OR BLENDS OF THESE MATERIALS, FREE FROM ORGANIC, FROZEN OR OTHER DELETERIOUS MATERIALS, AND MEETING THE FOLLOWING GRADATION REQUIREMENTS:
- | SIZE | BEREGER PASSING |
|---------|-----------------|
| 4" | 100% |
| No. 40 | 0 - 10 |
| No. 200 | 0 |
- a. FINES PASSING NO. 200 SHALL BE NON-PLASTIC.
b. PARTICLE SIZE ANALYSIS SHALL SHOW NO GAP GRADING.
- THE SOIL beneath STRUCTURES AND PADS AND 5 FEET AROUND THEIR PERIMETER SHALL BE TREATED AS FOLLOWS:
 - STRIP THE AREA OF ALL VEGETATION AND REMOVE ALL ORGANICS.
 - PERFORATE ANY LUFT TUBE CONNECTIONS.
 - PROFESSOR ROLL THE SITE WITH A TANDEM AXLE LOADER OR DUMP TRUCK IN TWO DIRECTIONS AND 10' APART, AND BACKFILL WITH COMPACTED STRUCTURAL FILL ACCORDING TO THE COMPACTION REQUIREMENTS NOTED BELOW.
 - THE FILL REQUIRED TO RAISE THE SUBGRADE BENEATH THE FLOOR SLAB SHALL BE EITHER IMPORTED STRUCTURAL FILL OR ON SITE MATERIAL. MEETING THE PLASTICITY INDEX REQUIREMENT 4 AND 12 AND A LIQUID LIMIT LESS THAN 30. PLACE ALL FILL (ON SITE OR SELECT) FILL IN 8-INCH LISTS AND COMPACT TO AT LEAST 95% OF THE STANDARD PROCTOR TEST AT A MOISTURE CONTENT WITHIN -3 AND +3 PERCENTAGE POINTS OF OPTIMUM.
 - EACH LIST SHALL BE TESTED FOR MOISTURE CONTENT AND IN PLACE DENSITY AT A RATE OF ONE TEST PER 3000 SQUARE FEET (MIN OF THREE PER LIST).
 - ALL CONCRETE PADS AND SLABS-ON-GRADE SHALL BEAR ON A BASE COURSE OF CLEAN, COMPACTED #57 STONE, A MINIMUM OF 6" THICK.
 - CONTRACTOR SHALL FINISH GRADE SITE LEVEL WITH EXISTING, 5 FEET BEYOND PROPOSED PADS AND STRUCTURES, THEN TAPER TO EXISTING GRADE, IF REQUIRED, AT A MAXIMUM SLOPE OF 3%.

CAST-IN-PLACE CONCRETE NOTES:

- DESIGN AND CONSTRUCTION OF ALL CONCRETE ELEMENTS SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING APPENDIX CODES: ACI 301, SPECIFICATIONS FOR REINFORCED CONCRETE; ACI 318, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE".
- MIX DESIGN SHALL BE APPROVED BY THE OWNER'S REPRESENTATIVE, PRIOR TO PLACING CONCRETE, PREPARE AND SUBMIT MIX DESIGNS IN ACCORDANCE WITH ACI 211, "PROPORTIONING CONCRETE MIXTURES"; AND ACI 301, SPECIFICATIONS FOR STRUCTURAL CONCRETE.
- ALL CONCRETE SHALL HAVE A 28 DAY DESIGN COMPRESSIVE STRENGTH OF 4,000 PSI. A MINIMUM OF 500 CFS OF AIR, 100°F AIR, AND CEMENT FOR CUBIC YARD, 45° TO 60° AIR TEMP, AND 100°F AIR ENTERING THE CONCRETE PUMP IS REQUIRED. 6'-0" - 2'-0" SUMP prior to the addition of ACI 318A (TYPE F) OR WATER REDUCING AGENT 20% of CLASS F ETASH MAY BE USED WITH THE FIRM APPROVAL OF THE ENGINEER AND THE CONCRETE FINISHER/CONTRACTOR.
- CONCRETE AGGREGATE SHALL MEET ASTM C 33 SPECIFICATIONS AND SHALL HAVE A MAXIMUM SIZE OF 3".
- CONCRETE SHALL HAVE A MAXIMUM WATER TO CEMENT RATIO OF 0.45, ADDITION OF WATER AT THE DB SITE IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE ENGINEER.
- IF THE AIR TEMPERATURE IS GREATER THAN 90 DEGREES WITHIN 24 HOURS AFTER PLACEMENT, NOT WHETHER CONCRETE PROCEDURES PER ACI 301SF SHALL BE USED. THE CONTRACTOR SHALL SUBMIT A PROCEDURE TO THE ENGINEER FOR APPROVAL. THESE PROCEDURES MAY INCLUDE THE FOLLOWING:
 - PLACING THE CONCRETE IN THE EARLY MORNING HOURS
 - THE USE OF EVAPORATION REDUCER (SEE BELOW)
 - THE USE OF MISTING AS A CURING METHOD
 - THE USE OF WET BLANKETS AS A CURING METHOD
 - THE USE OF A RETARDING ADMIXTURE (NOT PREFERRED)

7. COLD WEATHER CONCRETING SHALL BE PERFORMED PER ACI 506 REQUIREMENTS.

- FOUR 4X8 CONCRETE CYLINDERS SHALL BE MADE FOR EVERY 15 CUBIC YARDS OR EACH 50' OF PIPING. THE CYLINDERS SHALL BE MEASURED EVERY TIME A SET OF CYLINDERS IS MADE.
- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF THE AMERICAN CONCRETE INSTITUTE STANDARDS "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318) AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 317). SPlices IN REINFORCEMENT SHALL MEET CLASS B TENSION LAP REQUIREMENTS, UNLESS NOTED OTHERWISE.
- Cover for all reinforcement shall meet the coverage requirements as shown in the latest ACI 318, as noted below or as shown on the details. Cover dimensions shown on the details control over the ACI 318 or those noted below.
 - AGAINST FORMED SURFACES: 1 1/2"
 - AGAINST EARTH: 3"
 - BETWEEN REBAR: 1 1/2"
 - TOP OF SLAB ON GRADE: 1 1/2"

- REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BULLETT STEEL CONFORMING TO ASTM A-615, GRADE #40, #40 REINFORCING BARS, AND SMALLER SHALL BE COLD BENT WHENEVER BENDING IS REQUIRED IN THE FIELD. REINFORCEMENT GREATER THAN A #4# BAR MAY BE BENT IN THE FIELD WITH HEAT, UNLESS NOTED ON THE PLANS OR DIRECTED BY THE ENGINEER OTHERWISE.
- REINFORCING DETAILS SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 315.
- REINFORCING SPlices SHALL BE CLASS 'B' AND ALL HOOKS SHALL BE STANDARD, UNQ DO NOT WELD REINFORCING.
- PROVE 3 4" x 3 4" CHAMFER AT ALL EXPOSED CORNERS UNLESS NOTED OTHERWISE.
- NO. HOLES OR OPENINGS ARE PERMITTED THROUGH CONCRETE SLABS OR WALLS EXCEPT AS NOTED BELOW:
 - WHERE SHOWN AND AS DETAILED ON DRAWINGS.
 - MISCELLANEOUS HOLES THROUGH SLABS OR WALLS WHICH DO NOT DISPLACE MORE THAN ONE BAR. THESE DO NOT REQUIRE ADDITIONAL REINFORCEMENT.
- LOCATE ADDITIONAL CONSTRUCTION JOINTS REQUIRED TO FACILITATE CONSTRUCTION AS ACCEPTABLE TO ENGINEER, LOCATE WALL CONSTRUCTION JOINTS AT MASONRY CONTROL JOINTS WHERE POSSIBLE. PLACE REINFORCEMENT CONTINUOUSLY THROUGH JOINT.
- CAST CONCRETE ON SLOPED SURFACES BEGINNING AT LOWEST ELEVATION AND CONTINUING MONOLITHICALLY TOWARD HIGHER ELEVATIONS UNTIL INTENDED POOL IS COMPLETED.
- PLACE CONCRETE IN A UNIFORM MANNER TO PREVENT THE FORMATION OF COLD BLOBS AND OTHER PLACES OF WEAKNESS. VIBRATE THE CONCRETE TO FULLY EMBED REINFORCING. DO NOT USE VIBRATORS TO TRANSPORT CONCRETE THROUGH CHUTES OR FORWARK.
- REINFORCING BARS, BAR SUPPORTS, AND SPACERS SHALL BE PROVIDED AND PROVIDED IN ACCORDANCE WITH THE LATEST ACI CONCRETE MANUAL. USE WIRE-BAR SUPPORTS UNLESS NOTED OTHERWISE IN THE SPECIFICATIONS. SUPPORTS SHALL NOT BE PLACED FURTHER THAN 4 FEET APART. DAYTON/RICHMOND PRODUCTS (BD-745-5703) OR EQUAL.
- A. SLABS-ON-GRADE: (SLAB THICKNESS MINUS 1 1/2 INCHES) HIGH, TYPE R21, OR TYPE BBP USE SPLATES OR HORIZONTAL RUNNERS WHERE BASE MATERIAL WILL NOT SUPPORT CHAIR LEGS. CONCRETE BLOCK OR CLAY MASONRY MAY NOT BE USED AS SUPPORTS. WHERE LEGS OF SUPPORTS ARE IN CONTACT WITH THE CONCRETE SURFACES, WHERE LEGS THAT ARE PLASTIC PROTECTED (GIPS, CLASS 1) OR STAINLESS STEEL PROTECTED (GIPS, CLASS 2).
- USE ONE OF THE FOLLOWING SEALERS ON ALL INTERIOR EXPOSED CONCRETE SURFACES:
 - SEAL HARD BY E&M
 - ECCO DIAMOND HARD BY EUCLID
- DECUSCA CONCRETE EVAPORATION REDUCERS SHALL BE USED AFTER EACH POUR. APPROVAL FROM THE ENGINEER HAS BEEN OBTAINED TO USE THIS PRODUCT.
- SANCTION IN CONCRETE SLABS SHALL BE MADE AS SOON AS THE CONCRETE IS OF SUFFICIENT STRENGTH TO SAW WITHOUT RAVELING THE AGGREGATE, ANY TIME LAPSE GREATER THAN 8 HOURS AFTER PLACING THE CONCRETE SHALL BE PERMITTED ONLY IF APPROVED BY THE ENGINEER. FILL ALL INTERIOR JOINTS WITH ARDEX MM-80 JOINT COMPOUND. FILL ALL EXTERIOR JOINTS WITH ARDEX ARDISEAL RAPID.

23. ADHESIVE ANCHORS WITH REBAR OR THREADED RODS, SHALL BE AS NOTED BELOW, INSTALLED ACCORDING TO THE MANUFACTURER'S RECOMMENDATIONS, WHICH INCLUDES CLEARANCE TO THE REBAR, THE AIR, AND USING A MANUFACTURER APPROVED DISPENSING TOOL WITH MATING NOZZLE.

- IF THIS PROJECT IS IN SEISMIC ZONE D SEE THE DESIGN LOAD NOTE SECTION).
 - INTO CONCRETE OR GROUTED CAULI HIT IT HIT 150 MAX OR SIMPSON SET HIGH
 - IF THE PROJECT IS IN THE MIDDLE OF THE SLAB AND NOT BE OVER ONE INCH IN DIAMETER.
- ALL DOWELS, ANCHOR BOLTS, EMBEDDED STEEL, ELECTRICAL CONDUITS, PIPE SLEEVES, PIPING, GROUNDS, AND ALL OTHER EMBEDDED ITEMS AND FORMED DETAILS SHALL BE IN PLACE BEFORE START OF CONCRETE PLACEMENT. VERIFY SIZE AND LOCATION OF ALL OPENINGS.
- ALL PIPING AND DUCT PENETRATIONS THROUGH NEW STRUCTURAL SLABS ARE TO BE SLEVED OR CHASED. NO CORTING OF SLAB IS PERMITTED. ALL PIPING THROUGH EXISTING STRUCTURAL SLABS MAY BE CORED IF APPROVED BY ENGINEER.



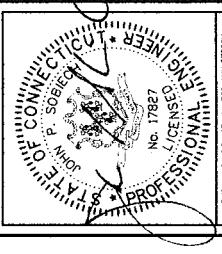
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NEW CIRCULAR WHEELS, PCS, LLC
500 ENTERPRISE DRIVE
ROCK HILL, CT 06867



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CHA PROJECT NO.
2702 - 1037 - 33000



Site ID:
CT5511

Site Name:
NEWTOWN - SR302

Site Address:
8 FERRIS ROAD
NEWTON, CT
06470

FAX:
1-800-254-2547
http://www.ctpe.org

Sheet ID:
GENERAL
NOTES

SHEET NUMBER:
GN02

May 23, 2010

Mr. Stephen Rambeau
TowerCo, LLC
5000 Valleystone Drive
Cary, NC 27519
(919) 653-5722

Vertical Solutions, Inc.
PO Box 579
Holly Springs, NC 27540
(888) 321-6167
operations@verticalsolutions-inc.com

Subject:

Rigorous Structural Analysis

Carrier Designation

AT&T, Reconfiguration
Site Number: 5511
Site Name: Newtown-Ferris Road

TowerCo Designation

Site Number: CT2008
Site Name: Newtown-Ferris Rd

Engineering Firm Designation

Vertical Solutions Project: 110591.01Rev 0

Site Data

8 Ferris Road, Newtown, Fairfield County, CT 06470
Latitude: N41° 23' 23.09"±; Longitude: W073° 20' 18.40"±
Elevation: 783 ft±,
118-ft Self Supporting Pole Structure (Monopole)

Dear Mr. Rambeau,

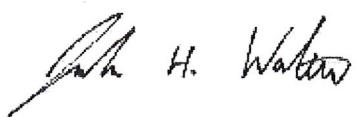
To your request, we present our structural analysis.

Our work indicates that with the proposed appurtenance configuration, the tower and foundation will satisfy the structural strength requirements of TIA/EIA-222-F-1996, *Structural Standards for Steel Antenna Towers and Supporting Structures* (industry standard) and the *2006 International Building Code* (local building code) for:

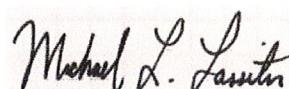
- 85-mph fastest mile basic wind speed
- 75-mph fastest mile basic wind speed with ½" radial ice

We trust you find our work satisfactory. Please do not hesitate to call should you have any questions.

Sincerely,



Joshua H. Walton, E.I.
Structural Engineer-In-Training



Michael L. Lassiter, S.E., P.E., C.W.I.
Structural Engineer, Civil Engineer, Certified Weld Inspector
& President
CT PE License: 25064

Table 1: Existing, Proposed and Reserved Appurtenance Configuration

Elevation (AGL, ft)	Carrier	Mount	Equipment	Coax	Location ¹
130	Town of Newtown	Pipe off of Platform at 118'	(1) Decibel DB222	(1) 7/8	Inside
118			(1) Telewave ANT150D	(1) 7/8	Inside
118.75	Sprint Nextel	Low Profile Platform	(9) Decibel DB844H90E-XY	(10) 7/8	Inside
105 ²	Sprint Nextel (Design)	Low Profile Platform	(12) DAPA 48000	(12) 1 5/8	Inside
	Sprint Nextel (Existing)		(6) Decibel DB980H90E-M	(6) 1 5/8	Inside
98	Verizon Wireless	Low Profile Platform	(3) Powerwave P65-16-XL-2 (4) Andrew DB846H80E-SX (2) RFS APL868013 (3) RYMSA Wireless MG D3-800TX	(12) 1 5/8	Inside
91	AT&T	Low Profile Platform	(3) Powerwave 7770 (6) LGP 21401 TMAs (3) Powerwave P65-16-XLH-RR (6) Ericsson RRUS-11 (1) Raycap Surge Suppressor	(12) 7/8 (1) 3/8" Fiber cable (2) 5/8" DC cables	Inside
81	T-Mobile	Low Profile Platform	(6) RFS APX16DWV-16DWV (6) S20057A1 TMAs	(24) 7/8	Inside
75	Sprint Nextel	Flush	(1) GPS	(1) 1/2	Outside

1 – See coax configuration plan, QP-P for coax locations.

2_ Existing Loading [EPA(A) = 24.4 sq ft]; Design Loading [EPA(A) = 76.8 sq ft]; Maximum Loading [EPA(A) = 92.4 sq ft]; Design loading used in analysis.

Table 2: Tower Structure Results, Percent Capacity Utilized

Elevation (ft)	Shaft	Result	Connections	Result
118 to 91.3	44	O. K.	--	--
91.3 to 45.13	83	O. K.	--	--
45.13 to 0	77	O. K.	98	O. K.

Table 3: Foundation Results, Percent Capacity Utilized

Component	Design Reactions	Analysis Reactions	Percent Utilized	Result
Moment (k-ft)	2006	1930	96	O. K.
Shear (kips)	22.0	22	100	O. K.

Attachments:

- Project History
- Coax configuration plan, Sheet QP-P
- Program input and output
- Base Plate and Anchor Rod

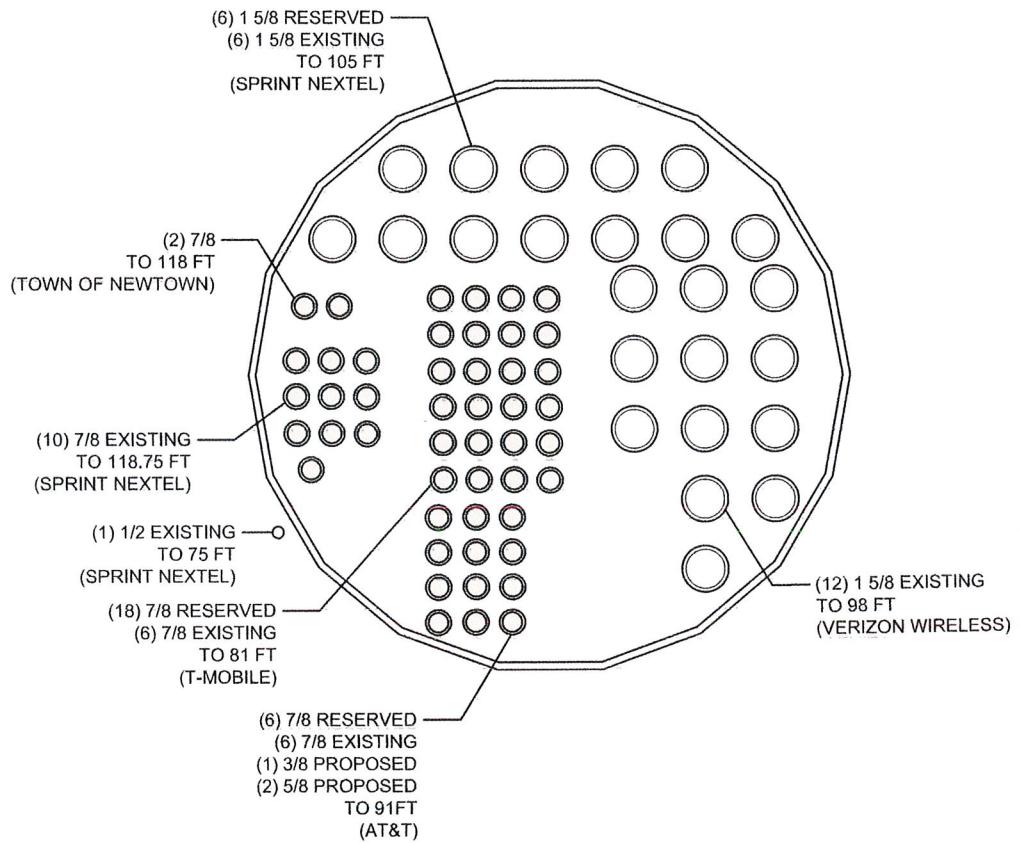
Project History, 110591.01, CT2008 Newtown-Ferris Rd

TowerCo Doc ID:	File	By: / For:	Description
242372	19990607_GEO_CT2008.pdf	Applied Earth Technologies / Herbst – Musciano	Geotechnical Investigation Report
242370	19990629_TFDC_CT2008.pdf	Engineered Endeavors Inc. / Nextel	Tower and Foundation Design Calculations
242396	20020308_SAR_CT2008.pdf	Engineered Endeavors Inc. / Sprint PCS	Structural Analysis
506182	20030731_PEL_CT2008.pdf	Engineered Endeavors Inc. / Town of Newtown	PE Letter
470864	20061025_SAR_CT2008.pdf	Semaan Engineering / Sprint USA	Structural Analysis
242404	20080708_SAR_CT2008.pdf	Semaan Engineering / Sprint USA	Structural Analysis
714947	20081102_TEPC_CT2008.pdf	SiteMaster / TowerCo	Tower Elevation Profile
719728	20081102_SIR_CT2008.pdf	SiteMaster / TowerCo	Site Inspection Report
	20100129_RTA_CT2008.doc	Verizon Wireless / TowerCo	Reconfiguration Application
---	20100201_COR_CT2008.mht	TowerCo / Vertical Solutions Inc	Correspondence, Email
	20100507_SAR_CT2008.pdf	Vertical Solutions / TowerCo	Structural Analysis Report
	20110512_RTA_CT2008.doc	TowerCo / Vertical Solutions	Reconfiguration Application

Table Note:
Files name format YYYYMMDD-XXX-ZZZZZZ.pdf

Where:

YYYY=year
MM=month
DD=day published/issued
XXX=file describer
ZZZZZZ=TowerCo Site ID



COAX CONFIGURATION PLAN AT 81-FT

SCALE: 1 1/2" = 1'-0"

PREPARED BY:	Vertical Solutions Inc.	PROJECT NAME:	Newtown-Ferris Rd
REVISION:	0	TOWERCO JOB #:	CT2008
DRAWN BY:	MLO	CHECKED BY:	MIL
SHEET NUMBER:	QP-P	DATE:	5/20/11
VSI #: 110591.OI		REV:	
<p>5000 Valley Stone Drive Cary, NC 27519 Office: (919) 469-5559 Fax: (919) 469-5530 www.towerco.com</p> <p>TowerCo</p>			

DESIGNED APPURTEINANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
DB222 (Newton)	130	RYMSA Wireless MG D3-800TX (Verizon)	98
(3) DB844H90E-XY w/Mount Pipe (Sprint)	118.75	(2) Powerwave LGP2140X (ATT)	91
(3) DB844H90E-XY w/Mount Pipe (Sprint)	118.75	(2) Powerwave LGP2140X (ATT)	91
(3) DB844H90E-XY w/Mount Pipe (Sprint)	118.75	7770.00 (ATT)	91
PiROD 13' Low Profile Platform (Sprint)	118	7770.00 (ATT)	91
Telewave ANT150D (Newton)	118	7770.00 (ATT)	91
PiROD 13' Low Profile Platform (Sprint)	105	Powerwave P65-16-XLH-RR w/ MP (ATT)	91
(4) 48000 w/Mount Pipe (Sprint)	105	Powerwave P65-16-XLH-RR w/ MP (ATT)	91
(4) 48000 w/Mount Pipe (Sprint)	105	Powerwave P65-16-XLH-RR w/ MP (ATT)	91
(4) 48000 w/Mount Pipe (Sprint)	105	Powerwave P65-16-XLH-RR w/ MP (ATT)	91
Powerwave P65-16-XL-2 with Mount Pipe (Verizon)	98	(2) Ericsson RRUS-11 (ATT)	91
Powerwave P65-16-XL-2 with Mount Pipe (Verizon)	98	(2) Ericsson RRUS-11 (ATT)	91
Powerwave P65-16-XL-2 with Mount Pipe (Verizon)	98	(2) Ericsson RRUS-11 (ATT)	91
Powerwave P65-16-XL-2 with Mount Pipe (Verizon)	98	Raycap Surge Arrestor 24" X 11" (ATT)	91
PiROD 13' Low Profile Platform (Verizon)	98	(2) 12" x 6" x 2" TMA (T-Mobile)	81
(2) DB846H80E-SX w/Mount Pipe (Verizon)	98	PiROD 13' Low Profile Platform (T-Mobile)	81
(2) DB846H80E-SX w/Mount Pipe (Verizon)	98	(2) APX16DWV-16DWV w/MP (T-Mobile)	81
(2) APL868013 w/Mount Pipe (Verizon)	98	(2) APX16DWV-16DWV w/MP (T-Mobile)	81
RYMSA Wireless MG D3-800TX (Verizon)	98	(2) 12" x 6" x 2" TMA (T-Mobile)	81
RYMSA Wireless MG D3-800TX (Verizon)	98	(2) 12" x 6" x 2" TMA (T-Mobile)	81
GPS (Sprint)	75		

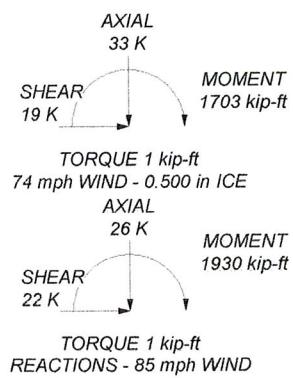
MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower is located in Fairfield County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 74 mph basic wind with 0.50 in ice.
4. Deflections are based upon a 50 mph wind.

Section	1	2	3
Length (ft)	118.0 ft	26.700	49.877
Number of Sides	18	18	18
Thickness (in)	0.188	0.188	0.313
Lap Splice (ft)	3.417	3.417	3.417
Top Dia (in)	4.750	4.750	4.750
Bot Dia (in)	31.760	31.760	31.760
Grade	A572-65	A572-65	A572-65
Weight (K)	7.7	4.6	4.6
Length (ft)	91.3 ft	45.1 ft	0.0 ft



RISA Tower TowerCo 5000 Valleystone Drive Cary, NC 27519 Phone: (919) 469-5559 FAX: (919) 469-5530	Job	CT2008	Page
	Project	110591	Date 10:44:47 05/24/11
	Client	TowerCo	Designed by jwalton

Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

- Tower is located in Fairfield County, Connecticut.
- Basic wind speed of 85 mph.
- Nominal ice thickness of 0.500 in.
- Ice density of 56 pcf.
- A wind speed of 74 mph is used in combination with ice.
- Temperature drop of 50 °F.
- Deflections calculated using a wind speed of 50 mph.
- A non-linear (P-delta) analysis was used.
- Pressures are calculated at each section.
- Stress ratio used in pole design is 1.333.
- Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Options

- | | | |
|--|--|---|
| Consider Moments - Legs
Consider Moments - Horizontals
Consider Moments - Diagonals
Use Moment Magnification
✓ Use Code Stress Ratios
✓ Use Code Safety Factors - Guys
Escalate Ice
Always Use Max Kz
Use Special Wind Profile
✓ Include Bolts In Member Capacity
✓ Leg Bolts Are At Top Of Section
✓ Secondary Horizontal Braces Leg
Use Diamond Inner Bracing (4 Sided)
Add IBC .6D+W Combination | Distribute Leg Loads As Uniform
Assume Legs Pinned
✓ Assume Rigid Index Plate
✓ Use Clear Spans For Wind Area
✓ Use Clear Spans For KL/r
✓ Retension Guys To Initial Tension
Bypass Mast Stability Checks
✓ Use Azimuth Dish Coefficients
✓ Project Wind Area of Appurt.
✓ Autocalc Torque Arm Areas
SR Members Have Cut Ends
Sort Capacity Reports By Component
✓ Triangulate Diamond Inner Bracing | Treat Feedline Bundles As Cylinder
Use ASCE 10 X-Brace Ly Rules
✓ Calculate Redundant Bracing Forces
Ignore Redundant Members in FEA
SR Leg Bolts Resist Compression
✓ All Leg Panels Have Same Allowable
Offset Girt At Foundation
Consider Feedline Torque
Include Angle Block Shear Check
Poles
✓ Include Shear-Torsion Interaction
Always Use Sub-Critical Flow
Use Top Mounted Sockets |
|--|--|---|

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	118.000-91.290	26.710	3.417	18	17.000	23.220	0.188	0.750	A572-65 (65 ksi)
L2	91.290-45.127	49.580	4.750	18	22.049	33.480	0.313	1.250	A572-65 (65 ksi)
L3	45.127-0.000	49.877		18	31.760	45.190	0.375	1.500	A572-65 (65 ksi)

<p>RISATower</p> <p>TowerCo 5000 Valleystone Drive Cary, NC 27519 Phone: (919) 469-5559 FAX: (919) 469-5530</p>	Job	CT2008	Page
	Project	110591	Date 10:44:47 05/24/11
	Client	TowerCo	Designed by jwalton

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	I/Q in ³	w in	w/t
L1	17.262	10.006	357.308	5.968	8.636	41.374	715.086	5.004	2.662	14.197
	23.578	13.707	918.690	8.177	11.796	77.883	1838.588	6.855	3.757	20.036
L2	23.189	21.560	1287.016	7.717	11.201	114.901	2575.726	10.782	3.331	10.658
	33.996	32.898	4572.278	11.774	17.008	268.834	9150.572	16.452	5.342	17.096
L3	33.549	37.356	4648.763	11.142	16.134	288.134	9303.644	18.681	4.930	13.146
	45.887	53.341	13534.613	15.909	22.957	589.576	27087.035	26.676	7.293	19.449

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor <i>A_f</i>	Adjust. Factor <i>A_r</i>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in
L1 118.000-91.29				1	1	1		
0								
L2 91.290-45.127				1	1	1		
L3 45.127-0.000				1	1	1		

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	<i>C_{A,A}</i>	Weight plf
LDF5-50A (7/8 FOAM) (Sprint / Nextel)	C	No	Inside Pole	118.000 - 0.000	10	No Ice 1/2" Ice	0.000 0.330
LDF7-50A (1-5/8 FOAM) (Sprint / Nextel)	C	No	Inside Pole	105.000 - 0.000	12	No Ice 1/2" Ice	0.000 0.820
LDF5-50A (7/8 FOAM) (Newton)	C	No	Inside Pole	118.000 - 0.000	2	No Ice 1/2" Ice	0.000 0.330
LDF5-50A (7/8 FOAM) (AT&T)	C	No	Inside Pole	91.000 - 0.000	12	No Ice 1/2" Ice	0.000 0.330
LDF7-50A (1-5/8 FOAM) (Verizon)	C	No	Inside Pole	98.000 - 0.000	12	No Ice 1/2" Ice	0.000 0.820
LDF5-50A (7/8 FOAM) (T-Mobile)	C	No	Inside Pole	81.000 - 0.000	24	No Ice 1/2" Ice	0.000 0.330
LDF4P-50A (1/2 FOAM) (Sprint / Nextel)	C	No	CaAa (Out Of Face)	75.000 - 0.000	1	No Ice 1/2" Ice	0.063 0.163
LDF2-50 (3/8 FOAM) (AT&T)	C	No	Inside Pole	91.000 - 0.000	1	No Ice 1/2" Ice	0.000 0.080
LDF4.5-50 (5/8 FOAM) (AT&T)	C	No	Inside Pole	91.000 - 0.000	2	No Ice 1/2" Ice	0.000 0.150

Feed Line/Linear Appurtenances Section Areas

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Tower Section	Tower Elevation ft	Face	A_R ft ²	A_F ft ²	$C_A A_A$ In Face ft ²	$C_A A_A$ Out Face ft ²	Weight K
L1	118.000-91.290	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000	0.307
L2	91.290-45.127	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	1.882	1.579
L3	45.127-0.000	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	2.843	1.627

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A_R ft ²	A_F ft ²	$C_A A_A$ In Face ft ²	$C_A A_A$ Out Face ft ²	Weight K
L1	118.000-91.290	A	0.500	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000	0.000	0.307
L2	91.290-45.127	A	0.500	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000	4.869	1.600
L3	45.127-0.000	A	0.500	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	7.356	1.658	

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Vert ft ft ft	Azimuth Adjustment °	Placement ft	$C_A A_A$ Front	$C_A A_A$ Side	Weight K	
(3) DB844H90E-XY w/Mount Pipe (Sprint)	A	From Leg	4.000 0.000 0.000	0.000	118.750	No Ice 1/2" Ice	3.579 4.201	5.396 6.491	0.036 0.077
(3) DB844H90E-XY w/Mount Pipe (Sprint)	B	From Leg	4.000 0.000 0.000	0.000	118.750	No Ice 1/2" Ice	3.579 4.201	5.396 6.491	0.036 0.077
(3) DB844H90E-XY w/Mount Pipe (Sprint)	C	From Leg	4.000 0.000 0.000	0.000	118.750	No Ice 1/2" Ice	3.579 4.201	5.396 6.491	0.036 0.077
PiROD 13' Low Profile Platform (Sprint) *** ***	C	None		0.000	118.000	No Ice 1/2" Ice	15.700 20.100	15.700 20.100	1.300 1.765
DB222 (Newton)	A	From Leg	4.000 0.000 0.000	0.000	130.000	No Ice 1/2" Ice	1.600 2.880	1.600 2.880	0.016 0.021
Telewave ANT150D (Newton)	C	From Leg	4.000 0.000 0.000	0.000	118.000	No Ice 1/2" Ice	9.064 9.837	9.064 9.837	0.032 0.118

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C4A4 Front		C4A4 Side	Weight K
						ft	ft²		

7770.00 (AT&T)	A	From Leg	4.000 0.000 0.000	0.000	91.000	No Ice 1/2" Ice	5.882 6.314	2.928 3.273	0.035 0.068
7770.00 (AT&T)	B	From Leg	4.000 0.000 0.000	0.000	91.000	No Ice 1/2" Ice	5.882 6.314	2.928 3.273	0.035 0.068
7770.00 (AT&T)	C	From Leg	4.000 0.000 0.000	0.000	91.000	No Ice 1/2" Ice	5.882 6.314	2.928 3.273	0.035 0.068
(2) Powerwave LGP2140X (AT&T)	A	From Leg	4.000 0.000 0.000	0.000	91.000	No Ice 1/2" Ice	1.288 1.445	0.364 0.479	0.014 0.021
(2) Powerwave LGP2140X (AT&T)	B	From Leg	4.000 0.000 0.000	0.000	91.000	No Ice 1/2" Ice	1.288 1.445	0.364 0.479	0.014 0.021
(2) Powerwave LGP2140X (AT&T)	C	From Leg	4.000 0.000 0.000	0.000	91.000	No Ice 1/2" Ice	1.288 1.445	0.364 0.479	0.014 0.021

(2) APX16DWV-16DWV w/MP (T-Mobile)	A	From Leg	4.000 0.000 0.000	0.000	81.000	No Ice 1/2" Ice	6.837 7.305	3.190 3.822	0.036 0.079
(2) APX16DWV-16DWV w/MP (T-Mobile)	B	From Leg	4.000 0.000 0.000	0.000	81.000	No Ice 1/2" Ice	6.837 7.305	3.190 3.822	0.036 0.079
(2) APX16DWV-16DWV w/MP (T-Mobile)	C	From Leg	4.000 0.000 0.000	0.000	81.000	No Ice 1/2" Ice	6.837 7.305	3.190 3.822	0.036 0.079
(2) 12" x 6" x 2" TMA (T-Mobile)	A	From Leg	4.000 0.000 0.000	0.000	81.000	No Ice 1/2" Ice	0.700 0.821	0.233 0.328	0.010 0.014
(2) 12" x 6" x 2" TMA (T-Mobile)	B	From Leg	4.000 0.000 0.000	0.000	81.000	No Ice 1/2" Ice	0.700 0.821	0.233 0.328	0.010 0.014
(2) 12" x 6" x 2" TMA (T-Mobile)	C	From Leg	4.000 0.000 0.000	0.000	81.000	No Ice 1/2" Ice	0.700 0.821	0.233 0.328	0.010 0.014
PiROD 13' Low Profile Platform (T-Mobile)	C	None		0.000	81.000	No Ice 1/2" Ice	15.700 20.100	15.700 20.100	1.300 1.765

Powerwave P65-16-XL-2 with Mount Pipe (Verizon)	A	From Leg	4.000 0.000 0.000	0.000	98.000	No Ice 1/2" Ice	8.637 9.290	5.779 6.949	0.059 0.119
Powerwave P65-16-XL-2 with Mount Pipe (Verizon)	B	From Leg	4.000 0.000 0.000	0.000	98.000	No Ice 1/2" Ice	8.637 9.290	5.779 6.949	0.059 0.119
Powerwave P65-16-XL-2 with Mount Pipe (Verizon)	C	From Leg	4.000 0.000 0.000	0.000	98.000	No Ice 1/2" Ice	8.637 9.290	5.779 6.949	0.059 0.119
PiROD 13' Low Profile Platform (Verizon)	C	None		0.000	98.000	No Ice 1/2" Ice	15.700 20.100	15.700 20.100	1.300 1.765

(2) DB846H80E-SX	A	From Leg	4.000	0.000	98.000	No Ice	5.321	7.725	0.041

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C.A.A Front	C.A.A Side	Weight K
w/Mount Pipe (Verizon)			0.000 0.000 0.000			1/2" Ice 5.874	8.915	0.096
(2) DB846H80E-SX	B	From Leg	4.000 0.000 0.000	0.000	98.000	No Ice 1/2" Ice 5.321 5.874	7.725 8.915	0.041 0.096
w/Mount Pipe (Verizon)			0.000 0.000 0.000					
(2) APL868013 w/Mount Pipe (Verizon)	C	From Leg	4.000 0.000 0.000	0.000	98.000	No Ice 1/2" Ice 3.579 4.201	5.396 6.491	0.032 0.073
RYMSA Wireless MG D3-800TX (Verizon)	A	From Leg	4.000 0.000 0.000	0.000	98.000	No Ice 1/2" Ice 4.176 4.591	3.994 4.773	0.035 0.070
RYMSA Wireless MG D3-800TX (Verizon)	B	From Leg	4.000 0.000 0.000	0.000	98.000	No Ice 1/2" Ice 4.176 4.591	3.994 4.773	0.035 0.070
RYMSA Wireless MG D3-800TX (Verizon)	C	From Leg	4.000 0.000 0.000	0.000	98.000	No Ice 1/2" Ice 4.176 4.591	3.994 4.773	0.035 0.070

PiROD 13' Low Profile Platform (Sprint)	C	None		0.000	105.000	No Ice 1/2" Ice 15.700 20.100	15.700 20.100	1.300 1.765
(4) 48000 w/Mount Pipe (Sprint)	A	From Leg	4.000 0.000 0.000	0.000	105.000	No Ice 1/2" Ice 5.116 5.788	3.486 4.537	0.044 0.082
(4) 48000 w/Mount Pipe (Sprint)	B	From Leg	4.000 0.000 0.000	0.000	105.000	No Ice 1/2" Ice 5.116 5.788	3.486 4.537	0.044 0.082
(4) 48000 w/Mount Pipe (Sprint)	C	From Leg	4.000 0.000 0.000	0.000	105.000	No Ice 1/2" Ice 5.116 5.788	3.486 4.537	0.044 0.082
GPS (Sprint)	C	None		0.000	75.000	No Ice 1/2" Ice 3.539 3.802	2.275 2.506	0.008 0.033

PiROD 13' Low Profile Platform (AT&T)	C	None		0.000	91.000	No Ice 1/2" Ice 15.700 20.100	15.700 20.100	1.300 1.765
Powerwave P65-16-XLH-RR w/ MP (AT&T)	A	From Leg	4.000 0.000 0.000	0.000	91.000	No Ice 1/2" Ice 8.637 9.290	6.362 7.538	0.090 0.152
Powerwave P65-16-XLH-RR w/ MP (AT&T)	B	From Leg	4.000 0.000 0.000	0.000	91.000	No Ice 1/2" Ice 8.637 9.290	6.362 7.538	0.090 0.152
Powerwave P65-16-XLH-RR w/ MP (AT&T)	C	From Leg	4.000 0.000 0.000	0.000	91.000	No Ice 1/2" Ice 8.637 9.290	6.362 7.538	0.090 0.152
(2) Ericsson RRUS-11 (AT&T)	A	From Leg	4.000 0.000 0.000	0.000	91.000	No Ice 1/2" Ice 2.942 3.172	1.190 1.351	0.055 0.075
(2) Ericsson RRUS-11 (AT&T)	B	From Leg	4.000 0.000 0.000	0.000	91.000	No Ice 1/2" Ice 2.942 3.172	1.190 1.351	0.055 0.075
(2) Ericsson RRUS-11 (AT&T)	C	From Leg	4.000 0.000 0.000	0.000	91.000	No Ice 1/2" Ice 2.942 3.172	1.190 1.351	0.055 0.075
Raycap Surge Arrestor 24" X 11" (AT&T)	C	None		0.000	91.000	No Ice 1/2" Ice 1.467 1.667	1.467 1.667	0.050 0.068

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Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
L1	118 - 91.29	Pole	TP23.22x17x0.188	1	-5.608	687.980	43.8	Pass
L2	91.29 - 45.1267	Pole	TP33.48x22.049x0.313	2	-15.131	1653.800	83.5	Pass
L3	45.1267 - 0	Pole	TP45.19x31.76x0.375	3	-22.720	2535.606	76.9	Pass
Summary								
Pole (L2)								Pass
RATING =								Pass

Program Version 5.3.1.0 - 10/3/2008 File:L:/2011/0591_Newtown-Ferris Rd_CT/Task 1/Models/RISA/CT2008.eri

FLANGE PLATE DESIGN, DEFORMATION METHOD (DIFFERENT AREAS)

Input - $M := 1930 \cdot \text{kip} \cdot \text{ft}$ = moment at top of flange plate
 $P := 26 \cdot \text{kip}$ = axial load (use zero if base plate is grouted)
 $F_y := 60 \cdot \text{ksi}$ = yield stress of flange plate
 $b_{\text{eff}} := 11.322 \cdot \text{in}$ = effective width of flange plate in flexure
 $t := 1.75 \cdot \text{in}$ = thickness of flange plate
 $\text{ASI} := 133\%$ = allowable stress increase

CONSTANTS:

$$\text{psi} \equiv \frac{\text{lb}}{\text{in}^2}$$

$$\text{ksi} \equiv 1000 \cdot \text{psi}$$

$$\text{kip} \equiv 1000 \cdot \text{lb}$$

$$Q := \begin{pmatrix} 2 \\ 4 \\ 4 \\ 2 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{pmatrix} \quad d := \begin{pmatrix} 2 \cdot 12 + 2 \\ 1 \cdot 12 + 10 + \frac{1}{2} \\ 1 \cdot 12 + 1 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{pmatrix} \cdot \text{in} \quad A_{\text{stiff}} := \begin{pmatrix} 3.98 \\ 3.98 \\ 3.98 \\ 3.98 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{pmatrix} \text{in}^2 \quad A_{\text{stress}} := \begin{pmatrix} 3.25 \\ 3.25 \\ 3.25 \\ 3.25 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{pmatrix} \text{in}^2 \quad F_t := \begin{pmatrix} 0.675 \\ 0.675 \\ 0.675 \\ 0.675 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{pmatrix} \cdot \text{ksi}$$

$$\sum \overrightarrow{(Q)} = 12 \quad \text{sumQAd} := \sum \overrightarrow{(Q \cdot d^2 \cdot A_{\text{stiff}})} \quad \text{sumQAd} = 16131 \cdot \text{in}^4$$

$$R := \frac{M \cdot \overrightarrow{(d \cdot A_{\text{stiff}})}}{\text{sumQAd}} + \frac{P \cdot A_{\text{stiff}}}{\sum \overrightarrow{(A_{\text{stiff}} \cdot Q)}}$$

$$f_t := \left(\frac{R}{A_{\text{stress}}} \right) \quad r := \left(\frac{f_t}{\text{ASI} \cdot F_t} \right)$$

$$R = \begin{pmatrix} 150.7 \\ 130.7 \\ 76.5 \\ 2.2 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \end{pmatrix} \cdot \text{kip} \quad f_t = \begin{pmatrix} 46.4 \\ 40.2 \\ 23.5 \\ 0.7 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \end{pmatrix} \cdot \text{ksi} \quad r = \begin{pmatrix} 77 \\ 67 \\ 39 \\ 1 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{pmatrix} \cdot \%$$

Q = quantity of fasteners

d = distance from center

A = area of fastener

Ft = allowable tension stress



VSi Job No.: 100188
 Date: 05/07/2010
 Calculated by: MLO

BASE PLATE DESIGN (with stiffener plates, no grout), TIA-222-F

<u>Input -</u>		CONSTANTS:
$P_{max} := 151\text{-kip}$	= ultimate tension in anchor rod	
$x := 3\text{-in}$	= distance from centerline of stiffener plate to center of anchor rod	$\text{psi} \equiv \frac{\text{lb}}{\text{in}^2}$
$y := 4.375\text{-in}$	= distance from shaft wall to center of anchor rod	$\text{ksi} \equiv 1000\text{-psi}$
$t := 1.75\text{-in}$	= thickness of base plate	$\text{kip} \equiv 1000\text{-lb}$
$F_y := 60\text{-ksi}$	= minimum specified yield strength	
$k := 3\text{-in}$	= distance from center of anchor rod to edge of base plate	

$$I_x := \frac{2 \cdot x \cdot t^3}{12} \quad k_x := \frac{3 \cdot I_x}{y^3} \quad S_x := \frac{I_x}{\left(\frac{t}{2}\right)}$$

$$I_y := \frac{(y + k) \cdot t^3}{12} \quad k_y := \frac{3 \cdot I_y}{x^3} \quad S_y := \frac{I_y}{\left(\frac{t}{2}\right)}$$

$$M_{xMax} := \frac{k_x}{k_x + 2k_y} \cdot P_{max}, M'_{xAll} := F_y \cdot S_x \quad r_x := \frac{M_{xMax}}{M'_{xAll}} \quad r_x = 42\%$$

$$M_{yMax} := \frac{k_y}{k_x + 2k_y} \cdot P_{max}, M'_{yAll} := F_y \cdot S_y \quad r_y := \frac{M_{yMax}}{M'_{yAll}} \quad r_y = 89\%$$

$$r := \sqrt{r_x^2 + r_y^2} \quad r = 98\%$$

$$P_{Shaft} := \frac{k_x}{k_x + 2k_y} \cdot P_{max} \quad P_{Shaft} = 17.5\text{-kip}$$

$$P_{Stiff} := \frac{k_y}{k_x + 2k_y} \cdot P_{max} \quad P_{Stiff} = 66.7\text{-kip}$$

P65-16-XLH-RR

Dual Broadband Antennas

POLARIZATION: Dual linear ±45°
 FREQUENCY (MHz): 698-894, 1710-2170
 HORIZONTAL BEAM WIDTH (°): 65, 65
 GAIN (dBi/dBd): 15.5/13.4 17.5/15.4
 TILT: 1-12, 0-8
 LENGTH: 72"

ELECTRICAL SPECIFICATIONS*

	698-894		1710-2170		
	698-806	806-894	1710-1880	1850-1990	1900-2170
Gain (dBi/dBd)	14.8/12.7	15.5/13.4	16.9/14.8	17.2/15.1	17.5/15.4
Polarization					
Nominal Impedance (Ω)	50		50		
VSWR	< 1.5:1		< 1.5:1		
Horizontal beam width, -3 dB (°)	66	65	60	63	63
Vertical beam width, -3 dB (°)	14.7	12.5	6.8	6.4	5.7
Electrical down tilt (°)	1 to 12		0 to 8		
Side lobe suppression, vertical 1st upper (dB)	> 16	> 16	> 16		
	> 16	> 16			
Isolation between inputs (dB)	> 30	> 30	> 30	> 30	
Inter band Isolation (dB)	> 40		> 40		
Tracking, horizontal plane ±60° (dB)	< 2		< 2	< 2	< 2
First null fill (dB)			>-20	>-20	>-20
Vertical beam squint (°)	< 0.8	< 0.8	< 0.5	< 0.5	< 0.5
Front to back ratio (dB) 180°±30° copolar	>24	>24	>30	>30	>28
Front to back ratio (dB) 180°±30° total power					
Cross polar discrimination (XPD) 0° (dB)	> 15	> 15	> 15	> 15	> 15
Cross polar discrimination (XPD) ±60° (dB)	> 10	> 10	> 10	> 10	> 10
Far field coupling					
IM3, 2xTx@43dBm (dBc)		<-153		<-153	
IM7, 2xTx@43dBm (dBc)					
Power handling, average per input (W)	500			250	
Power handling, average total (W)	1000			500	

MECHANICAL SPECIFICATIONS*

Connector	4 X 7/16 DIN Female, IP67
Connector position	Bottom
Dimensions, HxDxD, mm (ft)	72" x 12" x 6" (1829 x 305 x 152)
Mounting	Pre-mounted Tilt Brackets
Weight, with brackets, kg (lbs)	29 (64)
Weight, without brackets, kg (lbs)	24 (53)
Wind load, frontal/lateral/rear side 42 m/s Cd=1.6 (N)	1380
Maximum operational wind speed, m/s (mph)	100 (45)
Survival wind speed, m/s (mph)	150 (67)
Lightning protection	DC Ground
Operating Temperature	-40C to +60C
Radome material	PVC, IP55
Packet size, HxDxD, mm (ft)	87" x 16" x 10" (2225 x 400 x 225)
Radome colour	Light Grey
Shipping weight, kg (lbs)	34 (75)
RET	iRET AISGv1.1, MET and AISGv2.0
Brackets	7256.00, 7454.00



*All specifications subject to change without notice. Please contact your Powerwave representative for complete performance data.

ANTENNA PATTERNS*

For detailed patterns visit <http://www.powerwave.com/rpa/>.

RRUS 11 – Dual PA RRU.

Technical Data

RBS6000

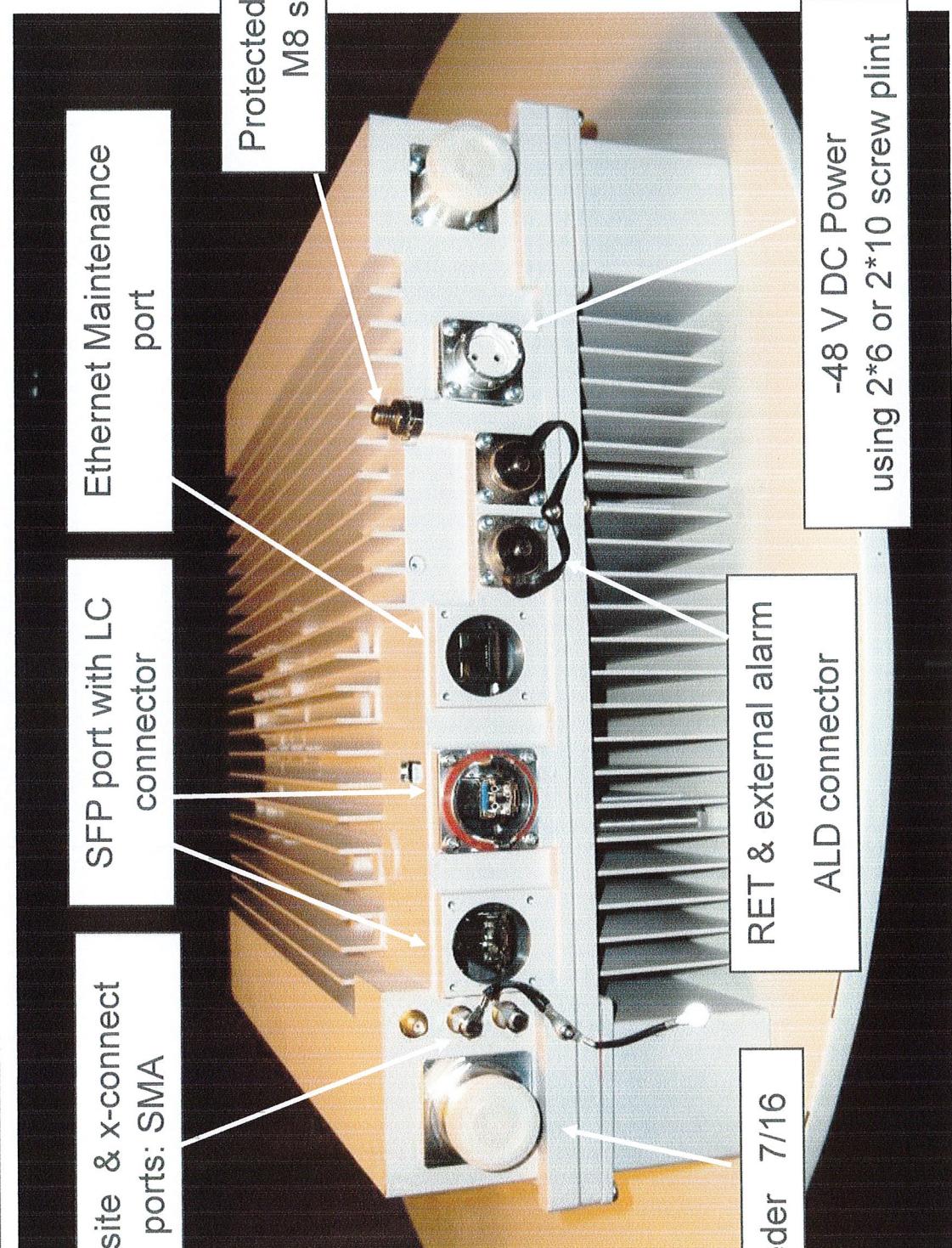


- > Multi standard
- > RF: 2x30 Watts
- > Carrier BW: 1.4 – 20 MHz
- > Alarms: 2
- > Dimensions (with sunshield):
 - Width: 17.0 in
 - Height: 17.8 in
 - Depth: 7.2 in
 - Weight: 55 lbs (Band 12)
 - Weight: 50 lbs (Band 4)
- > Temperature: -40 to +131 F
- > Cooling: Self convection
- > Power: -48 VDC
- > Rec. fuse size 20 Amp
 - Rec. DC cable:
 - > 6 mm² up to 60 meters
 - > 10 mm² over 60 meters
 - > Shielded
- > Power Cons: 200 Watts typ.

RRUS-11 I/F



RBS6000



POWER

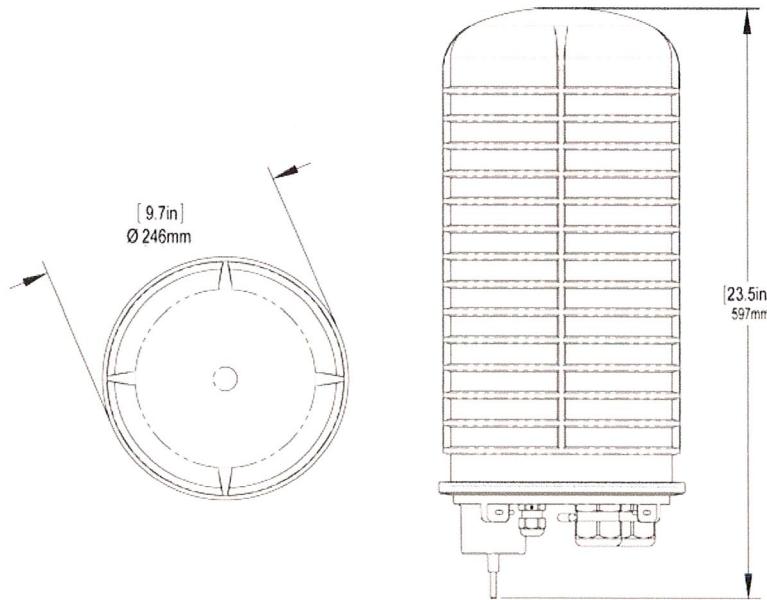
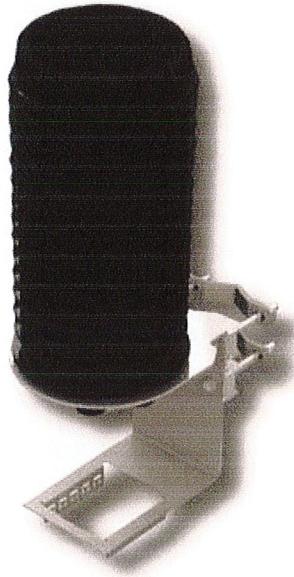
DC6-48-60-18-8F

DC Surge Suppression Solution

The DC6-48-60-18 is a dual chambered, DC surge suppression system for use in multi-circuit, Distributed Antenna Systems. The system will protect up to 6 Remote Radio Heads from voltage surges and lightning, and connect up to 18 fiber pairs. The system is enclosed in a NEMA 4 rated, waterproof enclosure.

FEATURES

- Protects up to 6 Remote Radio Heads, each with its own protection circuit.
- Flexible design allows for installation at the top of a tower for Remote Radio Head protection.
- Includes fiber connections for up to 18 pairs of fiber.
- LED indicators on individual circuits provide visual indication of suppressor status.
- Form 'C' relays allow for remote monitoring of the suppressor status.
- Patented Strikesorb technology provides over 60 kA of surge current capacity per circuit.
- Strikesorb suppression modules are fully recognized to UL 1449-3rd Edition Safety Standard, meeting all intermediate and high current fault requirements to facilitate use in OEM applications.
- Raycap recommends that DC protection system be installed within 2 meters or 6 feet of the radio.
- Dome design is lightweight and aerodynamic providing maximum flexibility for installation on top of towers.



Raycap

DC6-48-60-18-8F

DC Power Surge Protection

Electrical Specifications

Model Number	DC6-48-60-18-8F
Nominal Operating Voltage	48 VDC
Nominal Discharge Current (I_n)	20 kA 8/20 μ s
Maximum Discharge Current (I_{max}) per NEMA LS-1	60 kA 8/20 μ s
Maximum Continuous Operating Voltage (U_c)	75 VDC
Voltage Protection Rating	400 V

Mechanical Specifications

Suppression Connection Method	Compression lug, #2-#14 AWG Copper, #2-#12 Aluminum
Fiber Connection Method	LC-LC Single mode duplex
Environmental Rating	IP 68, 7m 72hrs
Operating Temperature	-40° C to + 80° C
Storage Temperature	-70° C to + 80° C
Cold Temperature Cycling	IEC 61300-2-22e -30° C to + 60° C 200 hrs @ 5 psi
Resistance to Aggressive Materials	CEI IEC 61073-2 including acids and bases
UV Protection	ISO 4892-2 Method A Xenon-Arc 2160 hrs
Weight	20 lbs without Mounting Bracket

STANDARDS

Strikesorb modules are compliant to the following Surge Protection Device (SPD) Standards:

- ANSI/UL 1449 – 3rd Edition
- IEEE C62.41
- NEMA LS-1, IEC 61643-1:2005 2nd Edition:2005
- IEC 61643-12
- EN 61643-11:2002 (including A11:2007)



Raycap

G02-00-068 REV 050610

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GS-07F-0435V



Certified to
ISO 9001:2000



TÜV Rheinland
of North America



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Fax: (860) 513-7190

Douglas L. Culp
Real Estate Consultant

June 2, 2011

Honorable E. Patricia Llodra
First Selectman Newtown
Newtown Town Hall
3 Primrose Street
Newtown, CT 06470

Re: Telecommunications Facility – 3 Primrose Street Newtown, CT

Dear First Selectman Llodra:

In order to accommodate technological changes, implement Uniform Mobile Telecommunications System (“UMTS”) and Long Term Evolution (“LTE”) capabilities, and enhance system performance in the State of Connecticut, New Cingular Wireless PCS, LLC (“AT&T”) will be changing its equipment configuration at certain cell sites.

As required by Regulations of Connecticut State Agencies (“R.C.S.A.”) Section 16-50j-73, the Connecticut Siting Council has been notified of the changes and will review AT&T’s proposal. Please accept this letter as notification under Section 16-50j-73 of construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2).

The accompanying letter to the Siting Council fully describes Cingular’s proposal for the referenced cell site. However, if you have any questions or require any further information on our plans or the Siting Council’s procedures; please call me at (860) 463-5511 or Ms. Linda Roberts, Executive Director, Connecticut Siting Council at (860) 827-2935.

Sincerely,

A handwritten signature in blue ink, appearing to read "DLC".

Douglas L. Culp
Real Estate Consultant

Enclosure