

Northeast Site Solutions Denise Sabo 199 Brickyard Rd Farmington, CT 06032 860-209-4690 denise@northeastsitesolutions.com

January 30, 2017

Members of the Siting Council Connecticut Siting Council Ten Franklin Square New Britain, CT 06051

RE: Notice of Exempt Modification

33 Boardman Road, New Milford CT 06776

Latitude: 41.59955439 Longitude: -73.43760920

T-Mobile Site#: CTNH362B_L700

Dear Ms. Bachman:

T-Mobile currently maintains three (3) antennas at the 140-foot level of the existing 154-foot monopine at 33 Boardman Road, New Milford CT 06776. The tower is owned by Quarry Stone and Gravel, LLC. The property is owned by Quarry Stone and Gravel, LLC. T-Mobile now intends to install three (3) new 700 MHz antenna. The new antennas would be installed at the 140-foot level of the tower. T-Mobile also intends to make the following modifications.

Planned Modifications:

Remove: None

Remove and Replace: None

Install New:

- (3) Commscope LNX-6515DS-A1M Antenna
- (3) Smart Bias Tees
- (6) 1-5/8" Coax

Existing to Remain:

- (3) RFS- APX18D-209014C Antenna
- (6) 1-5/8" Coax
- (3) TMA-1A-TwinPCS

This facility was approved by the Connecticut Siting Council. Docket No.285 – Approval was received for the installation of a telecommunication tower. Please see attached.



Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16- SOj-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-SOj-73, a copy of this letter is being sent to Mayor David Gronbach, Elected Official and Laura Regan, Zoning Enforcement Officer for the Town of New Milford, as well as the property owner and the tower owner.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S;A. § 16-50j-72(b)(2).

- 1. The proposed modifications will not result in an increase in the height of the existing structure.
- 2. The proposed modifications will not require the extension of the site boundary.
- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
- 4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard.
- 5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
- 6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above referenced telecommunications facility constitute an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

Denise Sabo

Mobile: 860-209-4690 Fax: 413-521-0558

Office: 199 Brickyard Rd, Farmington, CT 06032 Email: denise@northeastsitesolutions.com

Attachments

cc: David Gronbach- Mayor - as elected official Laura Regan- Zoning Enforcement Officer Quarry Stone and Gravel, LLC - as tower owner & property owner

Exhibit A



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Frequently Asked Questions



Decisions Printable Version

DOCKET NO. 285 - Sprint Spectrum, L.P. application for a Certificate of Environmental Compatibility and Public Need for the construction, maintenance and operation of a wireless telecommunications facility at 33 Boardman Road, New Milford, Connecticut.

Connecticut

Siting

Council

July 13, 2004

Decision and Order

Pursuant to the foregoing Findings of Fact and Opinion, the Connecticut Siting Council (Council) finds that the effects associated with the construction, operation, and maintenance of a telecommunications facility including effects on the natural environment; ecological integrity and balance; public health and safety; scenic, historic, and recreational values; forests and parks; air and water purity; and fish and wildlife are not disproportionate either alone or cumulatively with other effects when compared to need, are not in conflict with the policies of the State concerning such effects, and are not sufficient reason to deny the application and therefore directs that a Certificate of Environmental Compatibility and Public Need, as provided by General Statutes § 16-50k, be issued to Sprint Spectrum, L.P. for the construction, maintenance and operation of a wireless telecommunications facility at 33 Boardman Road, New Milford, Connecticut.

The facility shall be constructed, operated, and maintained substantially as specified in the Council's record in this matter, and subject to the following conditions:

1. The tower shall be constructed as a monopole, no taller than necessary to provide the proposed telecommunications services, sufficient to accommodate the antennas of Sprint Spectrum L.P., Nextel











Robert Stein Chairman

Melanie Bachman, Acting Executive Director

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Communications, Inc., and other entities, both public and private, but such tower shall not exceed a height of 150 feet above ground level. The height at the top of the antennas shall not exceed a height of 153 feet above ground level.

- 2. The Certificate Holder shall prepare a Development and Management (D&M) Plan for this site in compliance with Sections 16-50j-75 through 16-50j-77 of the Regulations of Connecticut State Agencies. The D&M Plan shall be served on the Town of New Milford, and all parties and intervenors as listed in the service list, and submitted to and approved by the Council prior to the commencement of facility construction. The D&M shall include:
 - a. a final site plan(s) of site development to include specifications for the tower, tower foundation, antennas, equipment building, access road, utility line, and landscaping; and
 - b. construction plans for site clearing, water drainage, and erosion and sedimentation control consistent with the <u>2002 Connecticut Guidelines for Soil Erosion and Sediment Control</u>, as amended.
- 3. Prior to submission of the D&M plan to the Council, the Certificate Holder shall discuss the appropriateness and feasibility of stealth tower designs for this site with the Town. The Town and Certificate Holder shall agree upon the final tower design.
- 4. The Certificate Holder shall, prior to the commencement of operation, provide the Council worst-case

modeling of electromagnetic radio frequency power density of all proposed entities' antennas at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin No. 65, August 1997. The Certificate Holder shall ensure a recalculated report of electromagnetic radio frequency power density is submitted to the Council when circumstances in operation cause a change in power density above the levels calculated and provided pursuant to this Decision and Order.

- 5. Upon the establishment of any new State or federal radio frequency standards applicable to frequencies of this facility, the facility granted herein shall be brought into compliance with such standards.
- 6. The Certificate Holder shall permit public or private entities to share space on the proposed tower for fair consideration, or shall provide any requesting entity with specific legal, technical, environmental, or economic reasons precluding such tower sharing.
- 7. The Certificate Holder shall provide reasonable space on the tower for no compensation for any municipal antennas, provided such antennas are compatible with the structural integrity of the tower.
- 8. If the facility does not initially provide wireless services within one year of completion of construction or ceases to provide wireless services for a period of one year, this Decision and Order shall be void, and the Certificate Holder shall dismantle the tower and remove all associated equipment or reapply for any continued or new use to the Council before any such use is made.

- 9. Any antenna that becomes obsolete and ceases to function shall be removed within 60 days after such antennas become obsolete and cease to function.
- 10. Unless otherwise approved by the Council, this Decision and Order shall be void if the facility authorized herein is not operational within one year of the effective date of this Decision and Order or within one year after all appeals to this Decision and Order have been resolved. Any request for extension of this period shall be filed with the Council no later than sixty days prior to expiration date of this Certificate and shall be served on all parties and intervenors, as listed in the service list. Any proposed modifications to this Decision and Order shall likewise be so served.

Pursuant to General Statutes § 16-50p, we hereby direct that a copy of the Findings of Fact, Opinion, and Decision and Order be served on each person listed below, and notice of issuance shall be published in the <u>Hartford Courant</u>, the <u>New Milford Spectrum</u>, and the <u>New Milford Times</u>.

By this Decision and Order, the Council disposes of the legal rights, duties, and privileges of each party named or admitted to the proceeding in accordance with Section 16-50j-17 of the Regulations of Connecticut State Agencies.

The parties and intervenors to this proceeding are:

Applicant	Its Representative
Sprint Spectrum, L.P.	Thomas J. Regan, Esquire Brown Rudnick Berlack Isreals LLP CityPlace I, 38 th Floor 185 Asylum Street Hartford, CT 06103-3402
<u>Intervenor</u>	Its Representative
Nextel Communications, Inc.	Julie Donaldson Kohler Hurwitz & Sagarin P.O. Box 112 Milford, CT 06460

Content Last Modified on 7/15/2004 12:58:33 PM

Ten Franklin Square New Britain, CT 06051 / 860-827-2935

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Exhibit B

33 BOARDMAN RD

Location 33 BOARDMAN RD

Mblu 47/ / 73/ /

Acct# 005304

Owner QUARRY STONE AND GRAVEL

LLC

Assessment \$2,871,680

Appraisal \$4,896,500

PID 8323

Building Count 1

Current Value

Appraisal						
Valuation Year	Improvements	Land	Total			
2015	\$1,205,300	\$3,691,200	\$4,896,500			
	Assessment					
Valuation Year	Improvements	Land	Total			
2015	\$843,710	\$2,027,970	\$2,871,680			

Owner of Record

Owner QUARRY STONE AND GRAVEL LLC

Co-Owner % O + G INDUSTRIES Certificate

Address 112 WALL ST

TORRINGTON, CT 06790 Sale Date 09/11/2003

Instrument 03

Book & Page 778/681

\$0

Sale Price

Ownership History

Ownership History					
Owner	Sale Price	Certificate	Book & Page	Instrument	Sale Date
QUARRY STONE AND GRAVEL LLC	\$0		778/ 681	03	09/11/2003
QUARRY STONE AND GRAVEL LLC	\$0		765/ 512	03	07/08/2003
KOVACS ROBERT G + KOVACS PAUL B + KOVACS	\$0		705/ 499	29	05/23/2002
QUARRY STONE AND GRAVEL LLC	\$0		690/ 804	03	01/09/2002
KOVACS ROGER P + PAUL B + ROBERT G	\$0		361/ 142		12/24/1986

Building Information

Building 1 : Section 1

Year Built: 1989 Living Area: 9,000

Replacement Cost: \$305,640

Building Percent 66

Good:

Replacement Cost

Less Depreciation: \$201,700

Building Attributes			
Field Description			
STYLE	Pre-Eng Whse		
MODEL	Ind/Comm		
Grade	С		
Stories:	1		

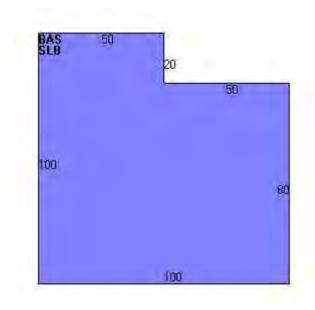
Occupancy	1
Exterior Wall 1	Pre-finsh Metl
Exterior Wall 2	Minimum
Roof Structure	Gable
Roof Cover	Enamel Metal
Interior Wall 1	Minim/Masonry
Interior Wall 2	
Interior Floor 1	Concr-Finished
Interior Floor 2	
Heating Fuel	Oil
Heating Type	Forced Air-Duc
AC Type	None
Bldg Use	Sand+Gravl
Total Rooms	
Total Bedrms	00
Total Baths	0
1st Floor Use:	410I
Heat/AC	NONE
Frame Type	STEEL
Baths/Plumbing	AVERAGE
Ceiling/Wall	NONE
Rooms/Prtns	AVERAGE
Wall Height	20
% Comn Wall	0

Building Photo



(http://images.vgsi.com/photos/NewMilfordCTPhotos//\00\01\5

Building Layout



Building Sub-Areas (sq ft)

Legend

Code	Description	Gross Area	Living Area
BAS	First Floor	9,000	9,000
SLB	Slab	9,000	0
		18,000	9,000

4

Extra Features

Extra Features	<u>Legend</u>
No Data for Extra Features	

Land

Land Use		Land Line Valuation	
Use Code	410	Size (Acres)	342.45
Description	Sand+Gravl	Frontage	0
Zone	I/R40	Depth	0
Neighborhood	C100	Assessed Value	\$2,027,970
Alt Land Appr	No	Appraised Value	\$3,691,200
Category			

Outbuildings

Outbuildings				<u>Legend</u>		
Code Description Sub Code Sub Description Size				Value	Bldg #	
SHD1	Shed	СВ	CindBk/Frame	200 S.F.	\$1,600	1

SHD1	Shed	FR	Frame	400 S.F.	\$3,200	1
LT1	Light (1)			100 Units	\$93,600	1
SCL1	Scale			60 TONS	\$21,600	1
SCL1	Scale			60 TONS	\$21,600	1
SITE	Cell Site Tenant	TW	Tower	4 Units	\$862,000	1

Valuation History

Appraisal				
Valuation Year	Improvements	Land	Total	
2014	\$1,086,200	\$3,691,200	\$4,777,400	
2009	\$219,200	\$9,275,200	\$9,494,400	
2009	\$219,200	\$9,275,200	\$9,494,400	

Assessment					
Valuation Year	Improvements	Land	Total		
2014	\$760,340	\$2,010,550	\$2,770,890		
2009	\$153,440	\$2,038,320	\$2,191,760		
2009	\$153,440	\$2,038,320	\$2,191,760		

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Property Information

Property 34/37.A ID

Location HOUSATONIC AVE

Owner HOUSATONIC REAL ESTATE LLC



MAP FOR REFERENCE ONLY NOT A LEGAL DOCUMENT

Town of New Milford, CT makes no claims and no warranties, expressed or implied, concerning the validity or accuracy of the GIS data presented on this map.

Parcels updated 10/1/2014 Properties updated 01/16/2017

Exhibit C

ANTENNA UPGRADES BY

T··Mobile·

T-MOBILE NORTHEAST LLC

SITE NUMBER: CTNH362B

SITE NAME: NH362/Boardman_MP

SITE ADDRESS: 33 Boardman Road, New Milford, CT 06776

(704G CONFIGURATION)

PROJECT SCOPE:

T-MOBILE, A WIRELESS TELECOMMUNICATIONS PROVIDER PROPOSES TO UPGRADE THEIR EXISTING FACILITY AS FOLLOWS:

ADD: (3) ANTENNAS, (1) BACKUP BATTERY UNIT (6) COAX CABLES AND (3) SMART BIAS TEE .

PROJECT NOTES:

 THIS IS AN UNMANNED TELECOMMUNICATION FACILITY AND NOT FOR HUMAN HABITATION: HANDICAPPED ACCESS IS NOT REQUIRED. POTABLE WATER OR SANITARY SERVICE IS NOT REQUIRED.

NO OUTDOOR STORAGE OR ANY SOLID WASTE RECEPTACLES REQUIRED.

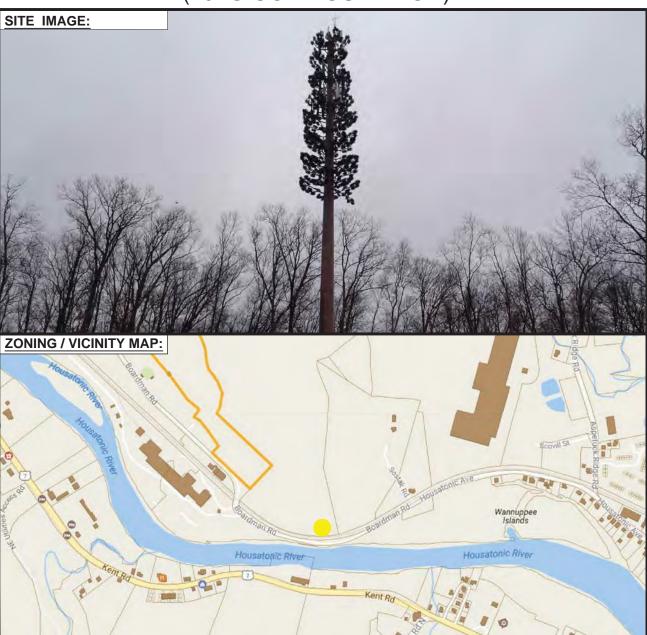
- 2. CONTRACTOR SHALL VERIFY ALL PLANS, EXISTING DIMENSIONS, AND CONDITIONS ON THE JOB SITE. CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT/ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK. FAILURE TO NOTIFY THE ARCHITECT/ENGINEER PLACES THE RESPONSIBILITY ON THE CONTRACTOR TO CORRECT THE DISCREPANCIES AT THE CONTRACTOR'S EXPENSE.
- DEVELOPMENT AND USE OF THE SITE WILL CONFORM TO ALL APPLICABLE CODES, ORDINANCES AND SPECIFICATIONS.

APPLICABLE STATE ADOPTION CODES:

2016 CONNECTICUT STATE BUILDING CODE (CSBC).

ANSI/TIA-222-G-2005 STRUCTURAL STANDARD FOR ANTENNA SUPPORTING STRUCTURES AND ANTENNAS.

2014 NATIONAL ELECTRICAL CODE (NFPA 70) FOR POWER AND GROUNDING REQUIREMENTS.



PROJECT INFORMATION:

ADDRESS:

33 BOARDMAN ROAD NEW MILFORD, CT 06776

STRUCTURE TYPE: MONOPINE

ZONING DISTRICT:

DODDINATES:

COORDINATES: N 41.59955439, W-73.43760920

STRUCTURE HEIGHT: 154' AGL

PROJECT TEAM:

APPLICANT:

T-MOBILE NORTHEAST, LLC. 35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002

860-692-7100

LANDLOARD:

SPRINT SITES USA 6550 SPRINT PARKWAY OVERLAND PARK, KS 66251

DEVELOPER:

NORTHEAST SITE SOLUTIONS 199 BRICKYARD RD FARMINGTON, CT 06032 SHELDON FREINCLE SHELDON@NORTHEASTSITE SOLUTIONS.COM

203-376-9186

FORESITE LLC

CONSULTANTS:

462 WALNUT ST NEWTON, MA 02460 SAEED MOSSAVAT SMOSSAVAT@FORESITELLC.COM

617-212-3123

SHEET INDEX:

T-1: TITLE SHEET

N-1: NOTES AND DISCLAIMERS

A-1: PLANS AND ELEVATIONS

A-2: ANTENNAS, EQUIPMENT AND INSTALLATION

-1: GROUNDING DETAILS

APPLICANT:

T - · Mobile · T-MOBILE NORTHEAST LLC

35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002 860-692-7100



CONSULTANT:



462 Walnut street Newton, MA 02460 617-212-3123



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REV	DESCRIPTION	DATE
Α	PRELIMINARY	01/17/17
0 FINAL ISSUED		01/25/17

SITE NUMBER: CTNH362B SITE NAME: NH362/Boardman_MP SITE ADDRESS: 33 Boardman Road New Milford, CT 06776

> SHEET TITLE: T-1: TITLE SHEET

NOTES AND DISCLAIMERS:

- 1. THE CONTRACTOR SHALL GIVE ALL NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY, MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS, AND LOCAL AND STATE JURISDICTIONAL CODES BEARING ON THE PERFORMANCE OF THE WORK. THE WORK PERFORMED ON THE PROJECT AND THE MATERIALS INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES.
- 2. THE ARCHITECT/ENGINEER HAS MADE EVERY EFFORT TO SET FORTH IN THE CONSTRUCTION AND CONTRACT DOCUMENTS THE COMPLETE SCOPE OF WORK. THE CONTRACTOR BIDDING THE JOB IS NEVERTHELESS CAUTIONED THAT MINOR OMISSIONS OR ERRORS IN THE DRAWINGS AND OR SPECIFICATIONS SHALL NOT EXCUSE SAID CONTRACTOR FROM COMPLETING THE PROJECT AND IMPROVEMENTS IN ACCORDANCE WITH THE INTENT OF THESE DOCUMENTS.
- 3. THE CONTRACTOR OR BIDDER SHALL BEAR THE RESPONSIBILITY OF NOTIFYING (IN WRITING) THE CLIENT'S REPRESENTATIVE OF ANY CONFLICTS, ERRORS, OR OMISSIONS PRIOR TO THE SUBMISSION OF CONTRACTOR'S PROPOSAL OR PERFORMANCE OF WORK.
- 5. THE CONTRACTOR SHALL VISIT THE JOB SITE PRIOR TO THE SUBMISSION OF BIDS OR PERFORMING WORK TO FAMILIARIZE HIMSELF WITH THE FIELD CONDITIONS AND TO VERIFY THAT THE PROJECT CAN BE CONSTRUCTED IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS.
- 6. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS ACCORDING TO THE MANUFACTURER'S / VENDOR'S SPECIFICATIONS UNLESS NOTED OTHERWISE OR WHERE LOCAL CODES OR ORDINANCES TAKE PRECEDENCE.
- 7. THE CONTRACTOR SHALL MAKE NECESSARY PROVISIONS TO PROTECT EXISTING IMPROVEMENTS DURING CONSTRUCTION.
- 8. THE CONTRACTOR SHALL COMPLY WITH ALL PERTINENT SECTIONS OF THE BASIC STATE BUILDING CODE, LATEST EDITION, AND ALL OSHA REQUIREMENTS AS THEY APPLY TO THIS PROJEC
- 9. THE CONTRACTOR SHALL NOTIFY THE CLIENT'S REPRESENTATIVE IN WRITING WHERE A CONFLICT OCCURS ON ANY OF THE CONTRACT DOCUMENTS. THE CONTRACTOR IS NOT TO ORDER MATERIAL OR CONSTRUCT ANY PORTION OF THE WORK THAT IS IN CONFLICT UNTIL CONFLICT IS RESOLVED BY THE CLIENT'S REPRESENTATIVE.
- 10. THE WORK SHALL CONFORM TO THE CODES AND STANDARDS OF THE FOLLOWING AGENCIES AS FURTHER CITED HEREIN:
- A. ASTM: AMERICAN SOCIETY FOR TESTING AND MATERIALS, AS PUBLISHED IN "COMPILATION OF ASTM STANDARDS BUILDING CODES" OR LATEST EDITION.
- B. AWS: AMERICAN WELDING SOCIETY INC. AS PUBLISHED IN "STANDARD D1.1-08, STRUCTURAL WELDING CODE" OR LATEST EDITION.
- C. AISC: AMERICAN INSTITUTE FOR STEEL CONSTRUCTION AS PUBLISHED IN "CODE FOR STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"; "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" (LATEST EDITION).
- 11. BOLTING:
- A. BOLTS SHALL BE CONFORMING TO ASTM A325 HIGH STRENGTH, HOT DIP GALVANIZED WITH ASTM A153 HEAVY HEX TYPE NUTS.
- B. BOLTS SHALL BE 3/4" MINIMUM (UNLESS OTHERWISE NOTED)
- C. ALL CONNECTIONS SHALL BE 2 BOLTS MINIMUM.
- 12. FABRICATION:
- A. FABRICATION OF STEEL SHALL CONFORM TO THE AISC AND AWS STANDARDS AND CODES (LATEST EDITION).
- B. ALL STRUCTURAL STEEL SHALL BE HOT—DIP GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 (LATEST EDITION), UNLESS OTHERWISE NOTED.
- 13. ERECTION OF STEEL:
- A. PROVIDE ALL ERECTION EQUIPMENT, BRACING, PLANKING, FIELD BOLTS, NUTS, WASHERS, DRIFT PINS, AND SIMILAR MATERIALS WHICH DO NOT FORM A PART OF THE COMPLETED CONSTRUCTION BUT ARE NECESSARY FOR ITS PROPER ERECTION.
- B. ERECT AND ANCHOR ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC REFERENCE STANDARDS. ALL WORK SHALL BE ACCURATELY SET TO ESTABLISHED LINES AND ELEVATIONS AND RIGIDLY FASTENED IN PLACE WITH SUITABLE ATTACHMENTS TO THE CONSTRUCTION OF THE BUILDING.
- C. TEMPORARY BRACING, GUYING AND SUPPORT SHALL BE PROVIDED TO KEEP THE STRUCTURE SAFE AND ALIGNED AT ALL TIMES DURING CONSTRUCTION, AND TO PREVENT DANGER TO PERSONS AND PROPERTY. CHECK ALL TEMPORARY LOADS AND STAY WITHIN SAFE CAPACITY OF ALL BUILDING COMPONENTS.

- 14. ANTENNA INSTALLATION:
- A. INSTALL ANTENNAS AS INDICATED ON DRAWINGS AND CLIENT'S REPRESENTATIVE SPECIFICATIONS.
- B. INSTALL GALVANIZED STEEL ANTENNA MOUNTS AS INDICATED ON DRAWINGS.
- C. INSTALL COAXIAL / FIBER CABLES AND TERMINATIONS BETWEEN ANTENNAS AND EQUIPMENT PER MANUFACTURER'S RECOMMENDATIONS. WEATHERPROOF ALL CONNECTORS BETWEEN THE ANTENNA AND EQUIPMENT PER MANUFACTURER'S REQUIREMENTS.
- 15. ANTENNA AND COAXIAL / FIBER CABLE GROUNDING:
- A. ALL EXTERIOR #6 GREEN GROUND WIRE "DAISY CHAIN" CONNECTIONS ARE TO BE WEATHER SEALED WITH ANDREWS CONNECTOR/SPLICE WEATHERPROOFING KIT TYPE #221213 OR EQUAL.
- B. ALL COAXIAL / FIBER CABLE GROUNDING KITS ARE TO BE INSTALLED ON STRAIGHT RUNS OF COAXIAL / FIBER CABLE (NOT WITHIN BENDS).
- 16. RELATED WORK, FURNISH THE FOLLOWING WORK AS SPECIFIED UNDER CONSTRUCTION DOCUMENTS, BUT COORDINATE WITH OTHER TRADES PRIOR TO BID:
- A. FLASHING OF OPENING INTO OUTSIDE WALLS
- B. SEALING AND CAULKING ALL OPENINGS
- C. PAINTING
- D. CUTTING AND PATCHING
- 17. REQUIREMENTS OF REGULATORY AGENCIES:
- A. FURNISH U.L. LISTED EQUIPMENT WHERE SUCH LABEL IS AVAILABLE. INSTALL IN CONFORMANCE WITH U.L. STANDARDS WHERE APPLICABLE.
- B. INSTALL ANTENNA, ANTENNA CABLES, GROUNDING SYSTEM IN ACCORDANCE WITH DRAWINGS AND SPECIFICATION IN EFFECT AT PROJECT LOCATION AND RECOMMENDATIONS OF STATE AND LOCAL BUILDING CODES, AND SPECIAL CODES HAVING JURISDICTION OVER SPECIFIC PORTIONS OF WORK. THIS WORK INCLUDES BUT IS NOT LIMITED TO THE FOLLOWING:
- C. TIA—EIA 222 (LATEST EDITION). STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND ANTENNA SUPPORTING STRUCTURES.
- D. FAA FEDERAL AVIATION ADMINISTRATION ADVISORY CIRCULAR AC 70/7460-IH, OBSTRUCTION MARKING AND LIGHTING.
- E. FCC FEDERAL COMMUNICATIONS COMMISSION RULES AND REGULATIONS FORM 715, OBSTRUCTION MARKING AND LIGHTING SPECIFICATION FOR ANTENNA STRUCTURES AND FORM 715A, HIGH INTENSITY OBSTRUCTION LIGHTING SPECIFICATIONS FOR ANTENNA STRUCTURES.
- F. AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 BOLTS (LATEST EDITION).
- G. NEC NATIONAL ELECTRICAL CODE ON TOWER LIGHTING KITS.
- H. UL UNDERWRITER'S LABORATORIES APPROVED ELECTRICAL PRODUCTS.
- I. IN ALL CASES, PART 77 OF THE FAA RULES AND PARTS 17 AND 22 OF THE FCC RULES ARE APPLICABLE AND IN THE EVENT OF CONFLICT, SUPERSEDE ANY OTHER STANDARDS OR SPECIFICATIONS.
- J. 2009 LIFE SAFETY CODE NFPA 101.

APPLICANT:

T · · Mobile · T-Mobile ·

35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002 860-692-7100



CONSULTANT:



462 Walnut street Newton, MA 02460 617-212-3123

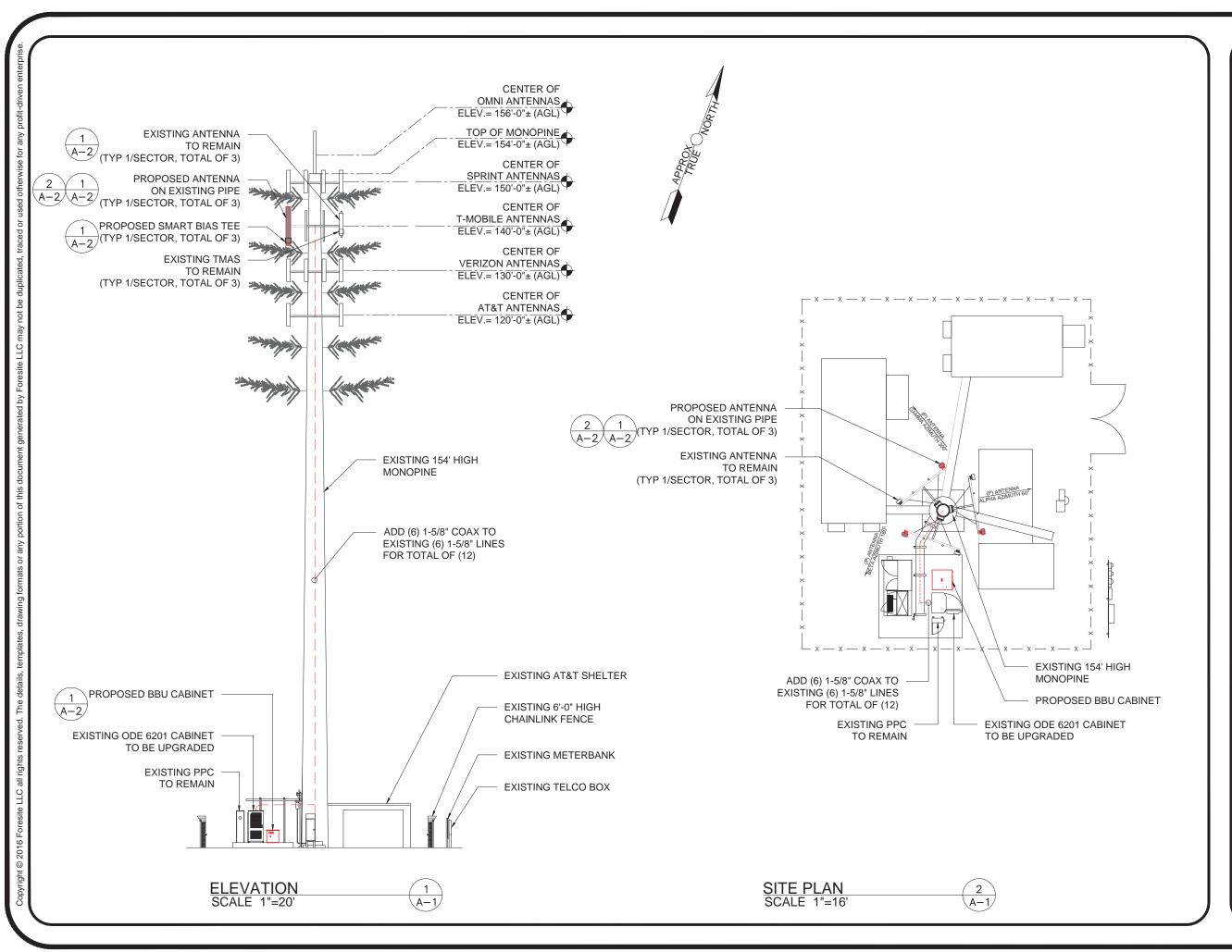


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REV	DESCRIPTION	DATE			
Α	PRELIMINARY	01/17/17			
0	FINAL ISSUED	01/25/17			
	-	A PRELIMINARY			

SITE NUMBER: CTNH362B SITE NAME: NH362/Boardman_MP SITE ADDRESS: 33 Boardman Road New Milford, CT 06776

SHEET TITLE:
N-1: NOTES AND DISCLAIMERS



APPLICANT:

T··Mobile·

T-MOBILE NORTHEAST LLC

35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002 860-692-7100



Turnkey Wireless Developmen 199 Brickyard road Farmington, CT 06032 203-275-6669

CONSULTANT:



462 Walnut street Newton, MA 02460 617-212-3123



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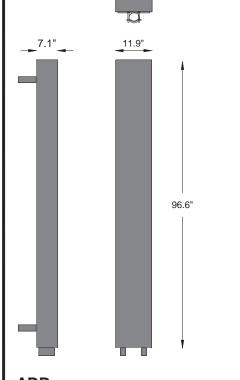
REV	DESCRIPTION	DATE
Α	PRELIMINARY	01/17/17
0	FINAL ISSUED	01/25/17

SITE NUMBER: CTNH362B SITE NAME: NH362/Boardman_MP SITE ADDRESS: 33 Boardman Road New Milford, CT 06776

SHEET TITLE: A-1: PLANS AND ELEVATIONS

ADD: (3) ANTENNAS

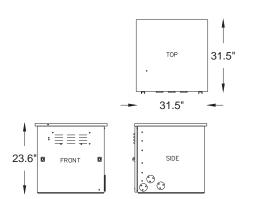
Manufacturer: Model Footprint: weight: Frequency band Antenna type Wind loading lateral Wind loading rear Wind loading maximum COMMSCOPE LNX-6515DS-A1M 96.6"Hx11.9"Wx7.1"D 43.7 lbs 698-896 MHZ Single Sector 150 km/h 150 km/h 241 km/h



ADD: (1) BATTERY BACKUP UNIT

Manufacturer: Footprint: weight:

ALCATEL LUCENT 30.55"Hx30.55"Wx29.67"D 100 lbs



3.2" -

(3) SMART BIAS TEES

AT ANTENNA LEVEL

ADD:

EXISTING PIPE MOUNT PROPOSED MOUNTING BRACKET (TYP) PROPOSED ANTENNA

ANTENNA MOUNTING DETAIL N.T.S

APPLICANT:

T··Mobile· T-MOBILE NORTHEAST LLC

35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002 860-692-7100



199 Brickyard road Farmington, CT 06032 203-275-6669

CONSULTANT:



Architects . Engineers . Surveyors

462 Walnut street Newton, MA 02460 617-212-3123



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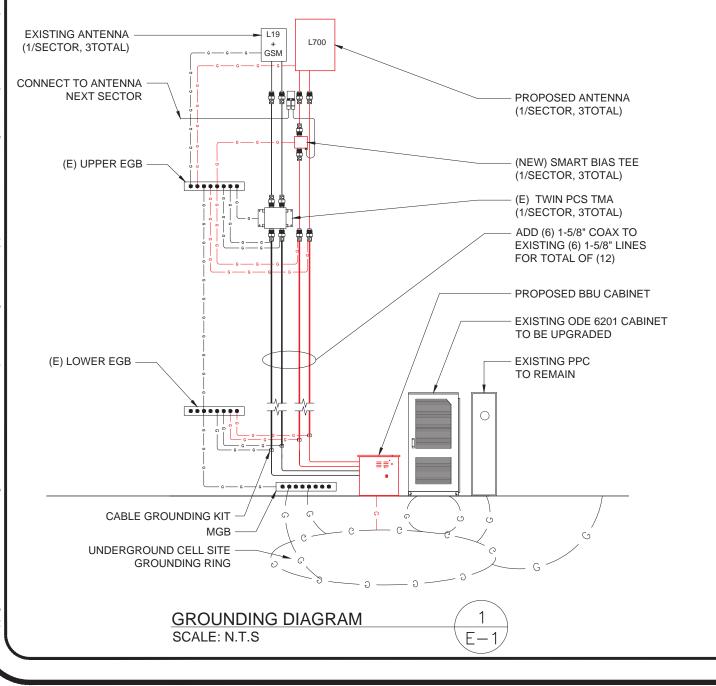
SITE NUMBER: CTNH362B SITE NAME: NH362/Boardman_MP SITE ADDRESS: 33 Boardman Road New Milford, CT 06776

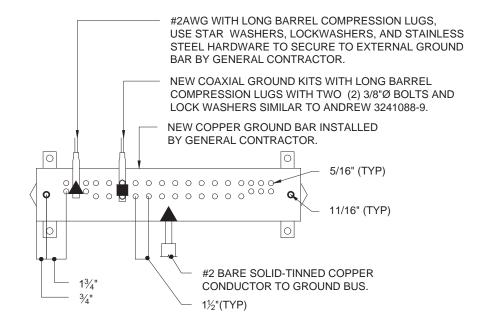
A-2: ANTENNAS, EQUIPMENT AND DETAILS

ANTENNA AND EQUIPMENT DETAILS 1 N.T.S

NOTES TO CONTRACTOR

- 1. THE ENTIRE ELECTRICAL INSTALLATION SHALL BE GROUNDED AS REQUIRED BY ALL APPLICABLE CODES.
- 2. ALL GROUNDING WORK SHALL BE IN ACCORDANCE WITH T-MOBILE STANDARD PRACTICE.
- 3. ALL BUS CONNECTORS SHALL BE TWO-HOLE, LONG-BARREL TYPE COMPRESSION LUGS, T&B OR EQUAL, UNLESS OTHERWISE NOTED ON DRAWINGS. ALL LUGS SHALL BE ATTACHED TO BUSSES USING BOLTS, NUTS, AND LOCK WASHERS. NO WASHERS ARE ALLOWED BETWEEN THE ITEMS BEING GROUNDED.
- 4. ALL CONNECTORS SHALL BE CRIMPED USING HYDRAULIC CRIMPING TOOLS, T&B #TBM 8 OR EQUIVALENT.
- 5. ALL CONNECTIONS SHALL BE MADE TO BARE METAL. ALL PAINTED SURFACES SHALL BE FILED TO ENSURE PROPER CONTACT. NO WASHERS ARE ALLOWED BETWEEN THE ITEMS BEING GROUNDED. ALL CONNECTIONS ARE TO HAVE A NON-OXIDIZING AGENT APPLIED PRIOR TO INSTALLATION.
- 6. ALL COPPER BUSSES SHALL BE CLEANED, POLISHED, AND A NON-OXIDIZING AGENT APPLIED. NO FINGERPRINTS OR DISCOLORED COPPER WILL BE PERMITTED.
- . ALL BENDS SHALL BE AS SHALLOW AS POSSIBLE, WITH NO TURN SHORTER THAN AN 8-INCH NOMINAL
- 3. GROUNDING CONDUCTORS SHALL BE SOLID TINNED COPPER AND ANNEALED #2. ALL GROUNDING CONDUCTORS SHALL RUN THROUGH PVC SLEEVES WHEREVER CONDUCTORS RUN THROUGH WALLS, FLOORS, OR CEILINGS. IF CONDUCTORS MUST RUN THROUGH EMT, BOTH ENDS OF CONDUIT SHALL BE GROUNDED. SEAL BOTH ENDS OF CONDUIT WITH SILICONE CAULK.
- GROUNDING SYSTEM RESISTANCE SHALL NOT EXCEED 10 OHMS. IF THE RESISTANCE VALUE IS EXCEEDED, NOTIFY THE PROJECT MANAGER FOR FURTHER INSTRUCTION ON METHODS FOR REDUCING THE RESISTANCE
- 0. ALL ROOF TOP ANTENNA MOUNTS SHALL BE GROUNDED WITH A #2 GROUND WIRE CONNECTED TO THE NEAREST GROUND BUS. ALL CONNECTIONS ARE TO BE CAD-WELDED IF POSSIBLE.
- 11. UPON COMPLETION OF WORK, CONDUCT CONTINUITY, SHORT CIRCUIT, AND FALL OF POTENTIAL GROUNDING TESTS FOR APPROVAL. SUBMIT TEST REPORTS TO THE PROJECT MANAGER.
- 12. GROUNDING CONNECTION TO TRAVEL IN A DOWNWARD DIRECTION.
- 13. ALL EXPOSED #2 WIRE MUST BE TINN NOT BTW.



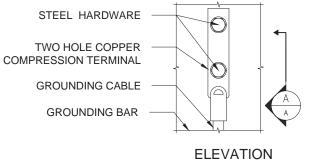


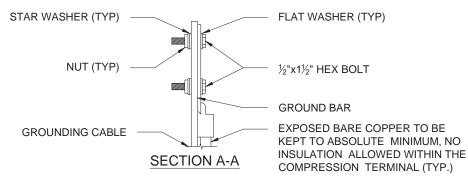
NOTE

- ALL HARDWARE STAINLESS STEEL COAT ALL SURFACES WITH KOPR-SHIELD BEFORE MATING.
- 2. FOR GROUND BOND TO STEEL ONLY: INSERT A TOOTH WASHER BETWEEN LUG AND STEEL, COAT ALL SURFACES WITH KOPR-SHIELD.
- 3. ALL HOLES ARE COUNTERSUNK 1/16".

GROUND BAR DETAILS







NOTES:

1. OXIDE INHIBITING COMPOUND TO BE USED AT ALL LOCATIONS.

TYPICAL GROUND BAR CONNECTIOS DETAIL SCALE: N.T.S

APPLICANT:

T - Mobile - T-MOBILE NORTHEAST LLC

35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002 860-692-7100



199 Brickyard road Farmington, CT 06032 203-275-6669

CONSULTANT:



Architects . Engineers . Surveyors

462 Walnut street Newton, MA 02460 617-212-3123



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REV	DESCRIPTION	DATE
Α	PRELIMINARY	01/17/17
0	FINAL ISSUED	01/25/17

SITE NUMBER: CTNH362B SITE NAME: NH362/Boardman_MP SITE ADDRESS: 33 Boardman Road New Milford, CT 06776

> SHEET TITLE: E-1: GROUNDING DETAILS

Exhibit D

Structural Analysis report

Date: January 6, 2017

Site Number: CTNH362B

Site Name: NH362/Boardman_MP

Site Address: 33 Boardman Road New Milford, CT 06776

PREPARED FOR:



35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002 860-692-7100

CONSULTANT:



462 Walnut street Newton, MA 02460 Contact: Saeed Mossavat email:smossavat@Foresitellc.com

617-527-3031

TURNKEY DEVELOPER:



199 Brickyard road Farmington, CT 06032 Contact: Sheldon Freincle sheldon@northeastsitesolution.com 203-275-6669

STRUCTURAL ANALYSIS REPORT – REV.1 MONOPOLE



Prepared For:



T-Mobile Northeast, LLC 35 Griffin Road South Bloomfield, CT 06002

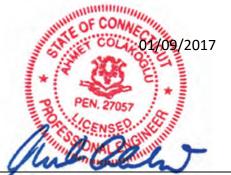


Structure Rating

Monopole: Pass (90.6%)

Foundation: Pass

Destek Engineering, LLC License No: PEC0001429



Ahmet Colakoglu, PE Connecticut Professional Engineer

License No: 27057

Site Name: NH362/Boardman_MP
Site ID: CTNH362B
33 Boardman Road
New Milford, CT 06776

Destek Job No: 1675009 January 09, 2017

CONTENTS

- 1.0 SUBJECT AND REFERENCES
- 1.1 STRUCTURE
- 2.0 EXISTING AND PROPOSED APPURTENANCES
- 3.0 CODES AND LOADING
- 4.0 STANDARD CONDITIONS FOR ENGINEERING SERVICES ON EXISTING STRUCTURES
- 5.0 ANALYSIS AND ASSUMPTIONS
- 6.0 CONCLUSION AND RESULTS

APPENDICES

A – SOFTWARE OUTPUT

1.0 SUBJECT AND REFERENCES

The purpose of this analysis is to evaluate the structural capacity of the existing 154' monopine located at 33 Boardman Road, New Milford, CT 06776 for the additions and alterations proposed by T-Mobile.

The structural analysis is based on the following documentation provided to Destek Engineering, LLC (Destek):

- RFDS provided by T-Mobile, dated 12/01/2016.
- Structural Analysis Report prepared by Centek, dated 12/01/2014.

1.1 STRUCTURE

The structure is a 154'-0" (18) sided monopine, which is attached to the foundation with anchor bolts and base plate. Please refer to the software output in Appendix A, for monopole geometry, member sizes, and other details.

Section Length (Feet)	Lap Splice (feet)	Shaft Thickness (Inches)	Top Diameter (Inches)	Bottom Diameter (Inches)	Yield Strength (ksi)
19.17	4.33	0.1875	25.250	30.03	65
50.00	5.67	0.3125	28.575	40.91	65
50.29	6.92	0.5000	38.886	51.28	65
50.49	0	0.5625	48.575	61.00	65

2.0 EXISTING AND PROPOSED APPURTENANCES

Existing Configuration of T-Mobile Appurtenances:

Rad Center (ft.)	Antennas & Equipment	Coax	Mount
140	(3) APXV18-209014-C (3) TMA – 1A - Twin PCS	(6) 1-5/8"	(3) Sector Mounts

Proposed and Final Configuration of T-Mobile Appurtenances:

Rad Center (ft.)	Antennas & Equipment	Coax	Mount
140	(3) APXV18-209014-C (3) LNX-6515DS-A1M (3) TMA – 1A - Twin PCS (3) Smart Bias Tee	(12) 1-5/8"	(3) Sector Mounts

Existing Configuration of Appurtenances by Others:

Rad Center (ft.) Carrier	Antennas & Equipment	Coax	Mount
156.0 Motorola 154.0 Motorola	(1) HP2-4.7 (1) HP3-4.7 (1) BA40-41	(3) 7/8"	(3) Pipe Mounts
150.0 Sprint	(6) 72"x6.5"x8" Panel (3) APXVSPP18-C-A20 (3) FD-RRH 2x50 800 (3) FD-RRH4x40 1900 (3) BXA-70063/6CF	(9) 1-5/8" (3) 1-1/4"	(3) Sector Mounts
130.0 Verizon	(3) LNX-8513DS (6) HBXX-6517DS (3) RRH2x60AWS (6) FD9R6004/2C-3L Diplexer (1) DB-T1-6Z-8AB-0Z	(12) 1-5/8"	(3) Sector Mounts
120.0 AT&T	(6) Powerwave 7770.00 (2) AM-X-CD-16-65-00T-RET (1) AM-X-CD-17-65-00T-RET (6) RRUS-11 (6) LGP21401 (3) LGP21901 (1) DC6-48-60-18-8F	(12) 1-5/8" (1)RG6-Fiber (2)#8 AWG Copper Wire	(3) Sector Mounts

3.0 CODES AND LOADING

The monopole was analyzed per *TIA/EIA-222-G* as referenced by the *2016 Connecticut State Building Code* with all of the adopted Addendums and Supplements. The following wind loading was used in compliance with the standard for New Milford, CT:

- Ultimate wind speed 115 mph converted to a Basic wind speed 89 mph without ice (W₀)
- Basic wind speed 50 mph with 1" escalating ice (W_i)
- Exposure Category C
- Topographic Category 1
- Structure Class II (I_w = 1.0)

The following load combinations were used with wind blowing at 0°, 30°, 45°, 60°, and 90° measured from a line normal to the face of the monopole.

- 1.2 D + 1.6 W₀
- $0.9 D + 1.6 W_0$
- 1.2 D + 1.0 D_i + 1.0 W_i

D: Dead Load of structure and appurtenances

W₀: Wind Load, without ice W_i: Wind Load, with ice

D_i: Weight of Ice

4.0 STANDARD CONDITIONS FOR ENGINEERING SERVICES ON EXISTING STRUCTURES

The analysis is based on the information provided and is assumed to be current and correct. Unless otherwise noted, the structure is assumed to be in good condition, free of defects and can achieve theoretical strength.

It is assumed that the structure has been maintained and shall be maintained during its service. The superstructure and the foundation system are assumed to be designed with proper engineering practice and fabricated, constructed and erected in accordance with the design documents. Destek will accept no liability which may arise due to any existing deficiency in design, material, fabrication, erection, construction, etc. or lack of maintenance.

The analysis does not include a qualification of the mounts attached on the structure or their connections. The analysis is performed to verify the capacity of the main structural members, which is the current practice in the tower industry.

The analysis results presented in this report are only applicable for the previously mentioned existing and proposed appurtenances. Any deviation of the appurtenances and placement, etc., will require Destek to generate an additional structural analysis.

5.0 ANALYSIS AND ASSUMPTIONS

The monopole was analyzed by utilizing tnxTower, a 3-Dimensional finite element software, a product of Tower Numerics, Inc. Software output for this analysis is provided in Appendix-A of this report.

6.0 CONCLUSION AND RESULTS

Based on an analysis per *TIA/EIA-222-G*, the existing monopole has **adequate** structural capacity for the proposed modifications by T-Mobile. For the code specified load combinations and as a maximum, the monopole shaft between 89.16 feet to 134.83 feet is stressed to **90.6%** of its capacity.

Based on the reactions comparison, the monopole foundation is found to have **adequate** capacity for the proposed loading by T-Mobile.

Reactions Comparison:

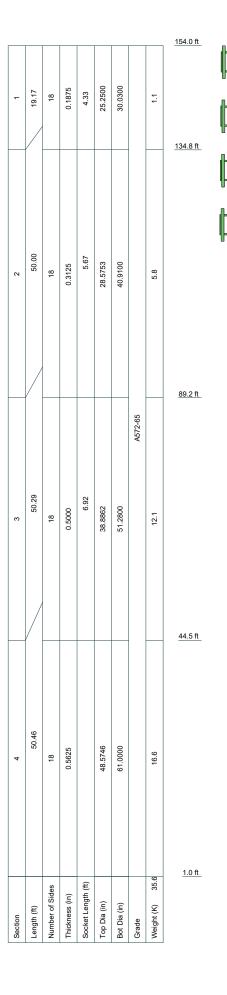
Maximums	Destek Analysis	Centek Reactions*
Moment (kip-ft)	8422	9033
Shear (kips)	75	78.3
Compression(kips)	72	83.7

^{*}Reactions have been multiplied by 1.35 as per Section 15.5.1 of ANSI/TIA-222-G.

Therefore, the proposed additions and alterations by T-Mobile **can** be implemented as intended with the conditions outlined in this report.

Should you have any questions about this report, please contact us at (770) 693-0835.

APPENDIX A SOFTWARE OUTPUT



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
BA4041	157	(2) HBXX-6517DS-A2M w/ Mount Pipe	131
HP2-4.7	157	(2) HBXX-6517DS-A2M w/ Mount Pipe	
HP3-4.7	157	(2) HBXX-6517DS-A2M w/ Mount Pipe	
Pipe Mount [PM 601-3]	155	RRH2X60-AWS BAND 4	131
(2) 72"x6.5"x8" Panel	151	RRH2X60-AWS BAND 4	131
(2) 72"x6.5"x8" Panel	151	RRH2X60-AWS BAND 4	131
(2) 72"x6.5"x8" Panel	151	(2) FD9R6004/2C-3L	131
APXVSPP18-C-A20 w/ Mount Pipe	151	(2) FD9R6004/2C-3L	131
APXVSPP18-C-A20 w/ Mount Pipe	151	(2) FD9R6004/2C-3L	131
APXVSPP18-C-A20 w/ Mount Pipe	151	DB-T1-6Z-8AB-0Z	131
FD-RRH-2x50-800	151	EEI 10' Universal T-Arm	131
FD-RRH-2x50-800	151	EEI 10' Universal T-Arm	131
FD-RRH-2x50-800	151	EEI 10' Universal T-Arm	131
FD-RRH-4x40-1900	151	EEI Pine Braches	131
FD-RRH-4x40-1900	151	(2) 7770.00 w/ Mount Pipe	121
FD-RRH-4x40-1900	151	(2) 7770.00 w/ Mount Pipe	121
EEI 10' Universal T-Arm	151	(2) 7770.00 w/ Mount Pipe	121
EEI 10' Universal T-Arm	151	AM-X-CD-16-65-00T-RET w/ Mount	121
EEI 10' Universal T-Arm	151	Pipe	
EEI Pine Braches	141	AM-X-CD-16-65-00T-RET w/ Mount	121
APXV18-209014-C w/ Mount Pipe	141	Pipe	
APXV18-209014-C w/ Mount Pipe	141	AM-X-CD-17-65-00T-RET w/ Mount Pipe	121
APXV18-209014-C w/ Mount Pipe	141	•	121
TMA-1A Twin AWS	141	(2) RRUS 11	
TMA-1A Twin AWS	141	(2) RRUS 11	121
TMA-1A Twin AWS	141	(2) RRUS 11 (2) LGP21401	121
EEI 10' Universal T-Arm	141	` '	
EEI 10' Universal T-Arm	141	(2) LGP21401	121
EEI 10' Universal T-Arm	141	(2) LGP21401	121
LNX-6515DS-A1M w/ Mount Pipe	141	(2) LGP21901 (2) LGP21901	121
LNX-6515DS-A1M w/ Mount Pipe	141	, ,	· · · ·
LNX-6515DS-A1M w/ Mount Pipe	141	(2) LGP21901 DC6-48-60-18-8F	121
Smart Bias Tee	141	EEI 10' Universal T-Arm	121
Smart Bias Tee	141		
Smart Bias Tee	141	EEI 10' Universal T-Arm	121
BXA-70063/6CF w/ Mount Pipe	131	EEI 10' Universal T-Arm	121
BXA-70063/6CF w/ Mount Pipe	131	EEI Pine Braches	121
BXA-70063/6CF w/ Mount Pipe	131	EEI Pine Braches	111
LNX-8513DS-A1M w/ Mount Pipe	131	EEI Pine Braches	101
LNX-8513DS-A1M w/ Mount Pipe	131	EEI Pine Braches	91
LNX-8513DS-A1M w/ Mount Pipe	131	EEI Pine Braches	81

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

- 1. Tower is located in Litchfield County, Connecticut.
 2. Tower designed for Exposure C to the TIA-222-G Standard.
 3. Tower designed for a 100 mph basic wind in accordance with the TIA-222-G Standard.
 4. Tower is also designed for a 40 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.

ALL REACTIOI5. Deflections are based upon a 60 mph wind.

ARE FACTORI6. Tower Structure Class II.

- 7. Topographic Category 1 with Crest Height of 0.00 ft
- 8. TOWER RATING: 90.6%

AXIAL 129 K SHEAR' MOMENT 18 K / 2095481 lb-ft

TORQUE 285 lb-ft 40 mph WIND - 1.0000 in ICE AXIAL 72 K

SHEAR' MOMENT 75 K 8422383 lb-ft

TORQUE 2012 lb-ft REACTIONS - 100 mph WIND

Destek Engineering, LLC **DESTEK** 1281 Kennestone Circle, Suite 100

Marietta, GA 30066 Phone: (770) 693 0835 FAX:

^{ob:} 1675009		
Project: CTNH362B		
Client: Foresite LLC	Drawn by: Ahmet Colakoglu	App'd:
Code: TIA-222-G	Date: 01/09/17	Scale: NTS
Path:		Dwg No. F

Destek Engineering, LLC 1281 Kennestone Circle, Suite 100 Marietta, GA 30066

> Phone: (770) 693 0835 FAX:

Job		Page
	1675009	1 of 23
Project		Date
	CTNH362B	11:28:43 01/09/17
Client	Foresite LLC	Designed by Ahmet Colakoglu

Tower Input Data

There is a pole section.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Litchfield County, Connecticut.

Basic wind speed of 100 mph.

Structure Class II.

Exposure Category C.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 40 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals

Use Moment Magnification Use Code Stress Ratios

Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz

Use Special Wind Profile

Include Bolts In Member Capacity Leg Bolts Are At Top Of Section

Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric

Distribute Leg Loads As Uniform Assume Legs Pinned

- Assume Rigid Index Plate
- Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension Bypass Mast Stability Checks
- Use Azimuth Dish Coefficients
- Project Wind Area of Appurt.
- Autocalc Torque Arm Areas Add IBC .6D+W Combination
- Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder

Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation

- Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-G Bracing Resist. Exemption Use TIA-222-G Tension Splice Exemption Poles
- Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets

Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft	Sides	in	in	in	in	
L1	154.00-134.83	19.17	4.33	18	25.2500	30.0300	0.1875	0.7500	A572-65 (65 ksi)
L2	134.83-89.16	50.00	5.67	18	28.5753	40.9100	0.3125	1.2500	À572-65 (65 ksi)

tnxTower	tn.	xI	0	W	er
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Destek Engineering, LLC 1281 Kennestone Circle, Suite 100 Marietta, GA 30066 Phone: (770) 693 0835 FAX:

Job	Page
1675009	2 of 23
Project	Date
CTNH362B	11:28:43 01/09/17
Client Foresite LLC	Designed by Ahmet Colakoglu

Section	Elevation	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft	Sides	in	in	in	in	
L3	89.16-44.54	50.29	6.92	18	38.8862	51.2800	0.5000	2.0000	A572-65 (65 ksi)
L4	44.54-1.00	50.46		18	48.5746	61.0000	0.5625	2.2500	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia.	Area	I	r	С	I/C	J	It/Q	w	w/t
	in	in^2	in^4	in	in	in^3	in^4	in^2	in	
L1	25.6395	14.9153	1183.6378	8.8972	12.8270	92.2771	2368.8330	7.4591	4.1140	21.941
	30.4933	17.7600	1998.2566	10.5941	15.2552	130.9882	3999.1425	8.8817	4.9553	26.428
L2	30.1008	28.0332	2829.0525	10.0333	14.5163	194.8885	5661.8276	14.0193	4.4793	14.334
	41.5411	40.2676	8384.7915	14.4121	20.7823	403.4587	16780.6158	20.1376	6.6502	21.281
L3	40.9050	60.9190	11340.7394	13.6271	19.7542	574.0922	22696.4013	30.4653	5.9640	11.928
	52.0711	80.5879	26253.8178	18.0269	26.0502	1007.8148	52542.1810	40.3016	8.1453	16.291
L4	51.0542	85.7196	24964.2715	17.0443	24.6759	1011.6866	49961.3916	42.8679	7.5591	13.438
	61.9410	107.9036	49795.0730	21.4553	30.9880	1606.9147	99655.6677	53.9621	9.7460	17.326

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade Ad	ljust. Factor A_f	$Adjust. \ Factor \ A_r$	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft ²	in					in	in	in
 L1	<i>y</i>			1	1	1			
154.00-134.83									
L2				1	1	1			
134.83-89.16									
L3 89.16-44.54				1	1	1			
L4 44.54-1.00				1	1	1			

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Component Type	Placement	Total Number		$C_A A_A$	Weight
	Leg		J1	ft			ft²/ft	plf
LDF5-50A(7/8")	В	No	Inside Pole	152.00 - 5.00	3	No Ice	0.00	0.33
						1/2" Ice	0.00	0.33
						1" Ice	0.00	0.33
LDF7-50A(1-5/8")	В	No	Inside Pole	152.00 - 5.00	9	No Ice	0.00	0.82
` ′						1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82
LDF7-50A(1-5/8")	В	No	Inside Pole	142.00 - 5.00	12	No Ice	0.00	0.82
· · · · · · · ·						1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82
LDF7-50A(1-5/8")	В	No	Inside Pole	132.00 - 5.00	12	No Ice	0.00	0.82
` ′						1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82
LDF7-50A(1-5/8")	В	No	Inside Pole	122.00 - 5.00	12	No Ice	0.00	0.82
` ′						1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82
RG6-Fiber	C	No	Inside Pole	122.00 - 5.00	1	No Ice	0.00	0.82
						1/2" Ice	0.00	0.82

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Description	Face or	Allow Shield	Component Type	Placement	Total Number		$C_A A_A$	Weight
	Leg		71	ft			ft²/ft	plf
						1" Ice	0.00	0.82
#8 AWG Copper Wire	C	No	Inside Pole	122.00 - 5.00	2	No Ice	0.00	0.82
						1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82
LDF6-50A(1-1/4")	C	No	Inside Pole	152.00 - 5.00	3	No Ice	0.00	0.66
						1/2" Ice	0.00	0.66
						1" Ice	0.00	0.66

Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation				In Face	Out Face	
	ft		ft^2	ft ²	ft ²	ft ²	K
L1	154.00-134.83	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.21
		C	0.000	0.000	0.000	0.000	0.03
L2	134.83-89.16	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	1.58
		C	0.000	0.000	0.000	0.000	0.17
L3	89.16-44.54	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	1.69
		C	0.000	0.000	0.000	0.000	0.20
L4	44.54-1.00	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	1.50
		C	0.000	0.000	0.000	0.000	0.18

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation	or	Thickness			In Face	Out Face	
	ft	Leg	in	ft ²	ft^2	ft ²	ft ²	K
L1	154.00-134.83	A	2.318	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.21
		C		0.000	0.000	0.000	0.000	0.03
L2	134.83-89.16	A	2.258	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	1.58
		C		0.000	0.000	0.000	0.000	0.17
L3	89.16-44.54	A	2.145	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	1.69
		C		0.000	0.000	0.000	0.000	0.20
L4	44.54-1.00	A	1.929	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	1.50
		C		0.000	0.000	0.000	0.000	0.18

Feed Line Center of Pressure

Section	Elevation	CP_X	CP_Z	CP_X	CP_Z
				Ice	Ice
	ft	in	in	in	in
L1	154.00-134.83	0.0000	0.0000	0.0000	0.0000
L.2.	134.83-89.16	0.0000	0.0000	0.0000	0.0000

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Section	Elevation	CP_X	CP_Z	CP_X	CP_Z
				Ice	Ice
	ft	in	in	in	in
L3	89.16-44.54	0.0000	0.0000	0.0000	0.0000
L4	44.54-1.00	0.0000	0.0000	0.0000	0.0000

Shielding Factor Ka

Tower	Feed Line	Description	Feed Line	K_a	K_a
Section	Record No.		Segment Elev.	No Ice	Ice

Discrete Tower Loads

EEI Pine Braches C None 0.0000 141.00 No Ice 90.00 90.00 1.50	Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weight
EEI Pine Braches C None O,0000 EEI Pine Braches EEI Pine Braches C None O,0000 EEI Pine Braches O,0000 EEI Pine Braches EEI Pine Braches O,0000 EEI Pine Braches EEI Pine Braches O,0000 EEI Pine Braches EEI Pine Braches EEI Pine Braches O,0000 EEI Pine Braches EEI Pine Braches O,0000 EEI Pine Braches O,0000 EEI Pine Braches O,0000 EEI Pine Braches O,0000 EI Ruches O				ft ft	0	ft		ft ²	ft²	K
EEI Pine Braches C None 0.0000 131.00 No Ice 90.00 90.00 1.50 1.50 1.50 1.50 1.50 1.50 1.50	EEI Pine Braches	С	None		0.0000	141.00	No Ice	90.00	90.00	1.50
EEI Pine Braches							1/2" Ice	130.00	130.00	1.90
EEI Pine Braches C None 0.0000 121.00 No Ice 90.00 130.00 1.90 1.50 1.70 1.70 1.70 1.70 1.70 1.70 1.70 1.7							1" Ice	170.00	170.00	2.30
EEI Pine Braches	EEI Pine Braches	C	None		0.0000	131.00	No Ice	90.00	90.00	1.50
EEI Pine Braches							1/2" Ice	130.00	130.00	1.90
EEI Pine Braches C None O.0000 EEI Pine Braches O.000 EEI Pine Braches O.0000 O.000 O.0000 O.000 O.000 O.000 O.000 O.000 O.000 O.000 O.000 O.0000 O.000 O.000 O.000 O.000 O.000 O.000 O.000 O.000 O.0000 O.000 O.000 O.000 O.000 O.000 O.000 O.000 O.000 O.0000 O.000 O.000 O.000 O.000 O.000 O.000 O.000 O.000 O.0000 O.000 O.0000 O.00000 O.0000 O.00000 O.								170.00		2.30
EEI Pine Braches C None	EEI Pine Braches	C	None		0.0000	121.00	No Ice	90.00	90.00	1.50
EEI Pine Braches							1/2" Ice	130.00	130.00	1.90
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$								170.00	170.00	2.30
EEI Pine Braches C None O.0000 D.0000 D.00000 D.00000 D.00000 D.00000 D.00000 D.00000 D.00000 D.000	EEI Pine Braches	C	None		0.0000	111.00				
EEI Pine Braches C None O.0000 D.0000 D.00000 D.								130.00		
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$										
EEI Pine Braches C None 0.0000 P1.00 No Ice 90.00 90.00 1.50 1/2" Ice 130.00 130.00 1.90 1" Ice 170.00 170.00 2.30 1.50 1/2" Ice 130.00 130.00 1.50 1/2" Ice 130.00 170.00 2.30 1/2" Ice 170.00 170.00 170.00 2.30 1/2" Ice 170.00 170.00 170.00 1/2" Ice 170.00 170.0	EEI Pine Braches	C	None		0.0000	101.00				
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$										
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$										2.30
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	EEI Pine Braches	C	None		0.0000	91.00				
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$										
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$										
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	EEI Pine Braches	C	None		0.0000	81.00				
BA4041 C None 0.0000 157.00 No Ice 0.22 0.22 0.00 1/2" Ice 0.58 0.58 0.00 1/2" Ice 0.94 0.94 0.01 Pipe Mount [PM 601-3] C None 0.0000 155.00 No Ice 4.39 4.39 0.20 1/2" Ice 5.48 5.48 0.24 1" Ice 6.57 6.57 0.28 ***150ft*** (2) 72"x6.5"x8" Panel A From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 0.00 1" Ice 6.00 6.99 0.10 (2) 72"x6.5"x8" Panel B From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 1" Ice 6.00 6.99 0.10 0.00 1" Ice 6.00 6.99 0.10 0.00 1/2" Ice 5.55 6.53 0.06										
Pipe Mount [PM 601-3] C None 0.0000 155.00 No Ice 0.0000 4.39 0.20 1/2" Ice 0.58 0.58 0.00 1" Ice 0.94 0.01 0.01 No Ice 0.94 0.01 1/2" Ice 0.94 0.01 1/2" Ice 0.94 0.01 1/2" Ice 0.94 0.02 1/2" Ice 0.94 0.02 1/2" Ice 0.94 0.02 1/2" Ice 0.94 0.02 1/2" Ice 0.94 0.03 1/2" Ice 0.94 0.04 1/2" Ice 0.94 0.05 1/2" Ice 0.94 0.06 1/2" Ice 0.94 0.06 1/2" Ice 0.94 0.07 1/2" Ice 0.94 0.07 1/2" Ice 0.94 0.07 1/2" Ice 0.94 0.08 1/2" Ice 0.94 0.09 1/2" Ice 0.94 0.09 1/2" Ice 0.94 0.00 1/2" Ice 0.94 0.00 1/2" Ice 0.94 0.00 1/2" Ice 0.94 0.01 1/2" I										
Pipe Mount [PM 601-3] C None 0.0000 155.00 No Ice 4.39 4.39 0.20 1/2" Ice 5.48 5.48 0.24 1" Ice 6.57 6.57 0.28 ***150ft*** (2) 72"x6.5"x8" Panel A From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 0.00 1" Ice 6.00 6.99 0.10 (2) 72"x6.5"x8" Panel B From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 1" Ice 6.00 6.99 0.10 0.00 1" Ice 6.00 6.99 0.10 0.00 1" Ice 6.00 6.99 0.10 0.00 1/2" Ice 5.55 6.53 0.06 0.00 1/2" Ice 5.55 6.53 0.06	BA4041	C	None		0.0000	157.00				
Pipe Mount [PM 601-3] C None 0.0000 155.00 No Ice 4.39 4.39 0.20 1/2" Ice 5.48 5.48 0.24 1" Ice 6.57 6.57 0.28 ***150ft*** (2) 72"x6.5"x8" Panel A From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 0.00 1/2" Ice 5.55 6.53 0.06 0.00 1" Ice 6.00 6.99 0.10 (2) 72"x6.5"x8" Panel B From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 0.00 1/2" Ice 5.55 6.53 0.06										
1/2" Ice 5.48 5.48 0.24										
150ft (2) 72"x6.5"x8" Panel A From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 0.00 1/2" Ice 5.55 6.53 0.06 0.00 1" Ice 6.00 6.99 0.10 (2) 72"x6.5"x8" Panel B From Face 3.00 0.000 151.00 No Ice 5.09 6.06 0.01 0.00 1/2" Ice 5.55 6.53 0.06 0.00 1/2" Ice 5.55 6.53 0.06	Pipe Mount [PM 601-3]	С	None		0.0000	155.00				
150fi (2) 72"x6.5"x8" Panel A From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 0.00 1/2" Ice 5.55 6.53 0.06 0.00 1" Ice 6.00 6.99 0.10 (2) 72"x6.5"x8" Panel B From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 0.00 1/2" Ice 5.55 6.53 0.06										
(2) 72"x6.5"x8" Panel A From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 0.00 1/2" Ice 5.55 6.53 0.06 0.00 1" Ice 6.00 6.99 0.10 (2) 72"x6.5"x8" Panel B From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 0.00 1/2" Ice 5.55 6.53 0.06							1" Ice	6.57	6.57	0.28
0.00 1/2" Ice 5.55 6.53 0.06 0.00 1" Ice 6.00 6.99 0.10 (2) 72"x6.5"x8" Panel B From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 0.00 1/2" Ice 5.55 6.53 0.06										
0.00 1" Ice 6.00 6.99 0.10 (2) 72"x6.5"x8" Panel B From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 0.00 1/2" Ice 5.55 6.53 0.06	(2) 72"x6.5"x8" Panel	Α	From Face		0.0000	151.00				
(2) 72"x6.5"x8" Panel B From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01 0.00 1/2" Ice 5.55 6.53 0.06										
0.00 1/2" Ice 5.55 6.53 0.06			_							
	(2) 72"x6.5"x8" Panel	В	From Face		0.0000	151.00				
0.00 1" Ice 6.00 6.99 0.10										
	(2) 524 (54 04 2				0.0000	1.51.00				
(2) 72"x6.5"x8" Panel C From Face 3.00 0.0000 151.00 No Ice 5.09 6.06 0.01	(2) 72"x6.5"x8" Panel	С	From Face		0.0000	151.00				
0.00 1/2" Ice 5.55 6.53 0.06										
0.00 1" Ice 6.00 6.99 0.10				0.00			I" Ice	6.00	6.99	0.10

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Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		$C_A A_A$ Front	$C_A A_A$ Side	Weigh
	Leg		Lateral						
			Vert ft	0	ft		ft ²	ft ²	K
			ft		Ji		Ji	Ji	Λ
APXVSPP18-C-A20 w/	A	From Face	3.00	0.0000	151.00	No Ice	8.26	6.95	0.08
Mount Pipe	А	1 Iom 1 acc	0.00	0.0000	131.00	1/2" Ice	8.82	8.13	0.05
Would I ipe			0.00			1" Ice	9.35	9.02	0.23
APXVSPP18-C-A20 w/	В	From Face	3.00	0.0000	151.00	No Ice	8.26	6.95	0.08
Mount Pipe			0.00			1/2" Ice	8.82	8.13	0.15
•			0.00			1" Ice	9.35	9.02	0.23
APXVSPP18-C-A20 w/	C	From Face	3.00	0.0000	151.00	No Ice	8.26	6.95	0.08
Mount Pipe			0.00			1/2" Ice	8.82	8.13	0.15
			0.00			1" Ice	9.35	9.02	0.23
FD-RRH-2x50-800	A	From Face	3.00	0.0000	151.00	No Ice	1.36	3.01	0.05
			0.00			1/2" Ice	1.52	3.22	0.08
FD DDV 4 50 000	-		0.00	0.0000	151.00	1" Ice	1.68	3.45	0.10
FD-RRH-2x50-800	В	From Face	3.00	0.0000	151.00	No Ice 1/2" Ice	1.36	3.01	0.05
			0.00			1/2" Ice	1.52	3.22	0.08
FD-RRH-2x50-800	С	From Face	0.00 3.00	0.0000	151.00	No Ice	1.68 1.36	3.45 3.01	0.10 0.05
FD-RRH-2X30-800	C	rioin race	0.00	0.0000	131.00	1/2" Ice	1.50	3.01	0.03
			0.00			1" Ice	1.68	3.45	0.08
FD-RRH-4x40-1900	Α	From Face	3.00	0.0000	151.00	No Ice	2.32	2.24	0.10
1 D-RRC11-4X40-1700	А	1 Iom 1 acc	0.00	0.0000	131.00	1/2" Ice	2.53	2.44	0.08
			0.00			1" Ice	2.74	2.65	0.11
FD-RRH-4x40-1900	В	From Face	3.00	0.0000	151.00	No Ice	2.32	2.24	0.06
			0.00			1/2" Ice	2.53	2.44	0.08
			0.00			1" Ice	2.74	2.65	0.11
FD-RRH-4x40-1900	C	From Face	3.00	0.0000	151.00	No Ice	2.32	2.24	0.06
			0.00			1/2" Ice	2.53	2.44	0.08
			0.00			1" Ice	2.74	2.65	0.11
EEI 10' Universal T-Arm	A	None		0.0000	151.00	No Ice	13.34	13.34	0.45
						1/2" Ice	16.80	16.80	0.60
						1" Ice	20.26	20.26	0.75
EEI 10' Universal T-Arm	В	None		0.0000	151.00	No Ice	13.34	13.34	0.45
						1/2" Ice	16.80	16.80	0.60
TEL 10111 : 175 4	-			0.0000	151.00	1" Ice	20.26	20.26	0.75
EEI 10' Universal T-Arm	C	None		0.0000	151.00	No Ice	13.34	13.34	0.45
						1/2" Ice	16.80	16.80	0.60
140ft						1" Ice	20.26	20.26	0.75
APXV18-209014-C w/	A	From Face	3.00	0.0000	141.00	No Ice	3.72	3.31	0.04
Mount Pipe	А	11011111acc	0.00	0.0000	141.00	1/2" Ice	4.13	4.02	0.04
Would I ipe			0.00			1" Ice	4.54	4.68	0.11
APXV18-209014-C w/	В	From Face	3.00	0.0000	141.00	No Ice	3.72	3.31	0.04
Mount Pipe	2	11011111111	0.00	0.0000	1.1.00	1/2" Ice	4.13	4.02	0.07
1			0.00			1" Ice	4.54	4.68	0.11
APXV18-209014-C w/	C	From Face	3.00	0.0000	141.00	No Ice	3.72	3.31	0.04
Mount Pipe			0.00			1/2" Ice	4.13	4.02	0.07
-			0.00			1" Ice	4.54	4.68	0.11
TMA-1A Twin AWS	A	From Face	3.00	0.0000	141.00	No Ice	0.67	0.31	0.01
			0.00			1/2" Ice	0.79	0.39	0.02
			0.00			1" Ice	0.91	0.49	0.02
TMA-1A Twin AWS	В	From Face	3.00	0.0000	141.00	No Ice	0.67	0.31	0.01
			0.00			1/2" Ice	0.79	0.39	0.02
m	_		0.00	0.0000	444.00	1" Ice	0.91	0.49	0.02
TMA-1A Twin AWS	C	From Face	3.00	0.0000	141.00	No Ice	0.67	0.31	0.01
			0.00			1/2" Ice	0.79	0.39	0.02
EEI 10' Universal T-Arm	A	None	0.00	0.0000	141.00	1" Ice No Ice	0.91 13.34	0.49 13.34	0.02 0.45

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		$C_A A_A$ Front	$C_A A_A$ Side	Weight
	208		Vert ft ft ft	o	ft		ft²	ft²	K
			Ji			1" Ice	20.26	20.26	0.75
EEI 10' Universal T-Arm	В	None		0.0000	141.00	No Ice 1/2" Ice	13.34 16.80	13.34 16.80	0.45 0.60
						1" Ice	20.26	20.26	0.75
EEI 10' Universal T-Arm	C	None		0.0000	141.00	No Ice	13.34	13.34	0.45
						1/2" Ice	16.80	16.80	0.60
LNX-6515DS-A1M w/	A	From Face	3.00	0.0000	141.00	1" Ice No Ice	20.26 11.68	20.26 9.84	0.75 0.08
Mount Pipe	А	FIOIII Face	0.00	0.0000	141.00	1/2" Ice	12.40	11.37	0.08
Would I ipe			0.00			1" Ice	13.14	12.91	0.27
LNX-6515DS-A1M w/	В	From Face	3.00	0.0000	141.00	No Ice	11.68	9.84	0.08
Mount Pipe			0.00			1/2" Ice	12.40	11.37	0.17
	_		0.00			1" Ice	13.14	12.91	0.27
LNX-6515DS-A1M w/	C	From Face	3.00	0.0000	141.00	No Ice	11.68	9.84	0.08
Mount Pipe			$0.00 \\ 0.00$			1/2" Ice 1" Ice	12.40 13.14	11.37 12.91	0.17 0.27
Smart Bias Tee	A	From Face	3.00	0.0000	141.00	No Ice	0.15	0.08	0.00
Smart Blas Tee	11	Trom ruce	0.00	0.0000	111.00	1/2" Ice	0.20	0.13	0.00
			0.00			1" Ice	0.26	0.18	0.01
Smart Bias Tee	В	From Face	3.00	0.0000	141.00	No Ice	0.15	0.08	0.00
			0.00			1/2" Ice	0.20	0.13	0.00
			0.00			1" Ice	0.26	0.18	0.01
Smart Bias Tee	C	From Face	3.00	0.0000	141.00	No Ice	0.15	0.08	0.00
			0.00			1/2" Ice	0.20	0.13	0.00
130ft			0.00			1" Ice	0.26	0.18	0.01
BXA-70063/6CF w/ Mount	Α	From Face	3.00	0.0000	131.00	No Ice	7.82	5.41	0.04
Pipe		1101111 1 4000	0.00	0.0000	101.00	1/2" Ice	8.37	6.56	0.10
1			0.00			1" Ice	8.89	7.42	0.17
BXA-70063/6CF w/ Mount	В	From Face	3.00	0.0000	131.00	No Ice	7.82	5.41	0.04
Pipe			0.00			1/2" Ice	8.37	6.56	0.10
DVA GOOGLIGE IN		Б Б	0.00	0.0000	121.00	1" Ice	8.89	7.42	0.17
BXA-70063/6CF w/ Mount	С	From Face	3.00	0.0000	131.00	No Ice	7.82	5.41	0.04
Pipe			$0.00 \\ 0.00$			1/2" Ice 1" Ice	8.37 8.89	6.56 7.42	0.10 0.17
LNX-8513DS-A1M w/	A	From Face	3.00	0.0000	131.00	No Ice	8.41	7.42	0.17
Mount Pipe	11	Trom ruce	0.00	0.0000	151.00	1/2" Ice	8.97	8.27	0.13
1			0.00			1" Ice	9.50	9.18	0.21
LNX-8513DS-A1M w/	В	From Face	3.00	0.0000	131.00	No Ice	8.41	7.08	0.06
Mount Pipe			0.00			1/2" Ice	8.97	8.27	0.13
1307.051205.4134.7		Б Б	0.00	0.0000	121.00	1" Ice	9.50	9.18	0.21
LNX-8513DS-A1M w/	С	From Face	3.00 0.00	0.0000	131.00	No Ice 1/2" Ice	8.41 8.97	7.08 8.27	0.06 0.13
Mount Pipe			0.00			1/2" Ice	8.97 9.50	8.27 9.18	0.13
(2) HBXX-6517DS-A2M w/	Α	From Face	3.00	0.0000	131.00	No Ice	8.77	6.96	0.21
Mount Pipe		1101111 1 4000	0.00	0.0000	101.00	1/2" Ice	9.34	8.18	0.14
•			0.00			1" Ice	9.89	9.14	0.21
(2) HBXX-6517DS-A2M w/	В	From Face	3.00	0.0000	131.00	No Ice	8.77	6.96	0.07
Mount Pipe			0.00			1/2" Ice	9.34	8.18	0.14
(2) HDVV (517DC +2) (-	F F	0.00	0.0000	121.00	1" Ice	9.89	9.14	0.21
(2) HBXX-6517DS-A2M w/	С	From Face	3.00	0.0000	131.00	No Ice	8.77	6.96	0.07
Mount Pipe			$0.00 \\ 0.00$			1/2" Ice 1" Ice	9.34 9.89	8.18 9.14	0.14 0.21
RRH2X60-AWS BAND 4	A	From Face	3.00	0.0000	131.00	No Ice	3.36	2.00	0.21
Tadibito Itiio Dilio T	. 1	1 10111 1 400	0.00	0.0000	151.00	1/2" Ice	3.61	2.24	0.08
			0.00			1" Ice	3.88	2.48	0.11
RRH2X60-AWS BAND 4	В	From Face	3.00	0.0000	131.00	No Ice	3.36	2.00	0.06

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		$C_A A_A$ Front	C_AA_A Side	Weigh
	Leg		Laierai Vert						
			ft ft	٥	ft		ft ²	ft ²	K
						1/2" Ice	3.61	2.24	0.08
			0.00			1" Ice	3.88	2.48	0.11
RRH2X60-AWS BAND 4	C	From Face	3.00	0.0000	131.00	No Ice	3.36	2.00	0.06
			0.00			1/2" Ice	3.61	2.24	0.08
			0.00			1" Ice	3.88	2.48	0.11
(2) FD9R6004/2C-3L	A	From Face	3.00	0.0000	131.00	No Ice	0.31	0.08	0.00
			0.00			1/2" Ice	0.39	0.12	0.01
			0.00			1" Ice	0.47	0.17	0.01
(2) FD9R6004/2C-3L	В	From Face	3.00	0.0000	131.00	No Ice	0.31	0.08	0.00
			0.00			1/2" Ice	0.39	0.12	0.01
			0.00			1" Ice	0.47	0.17	0.01
(2) FD9R6004/2C-3L	C	From Face	3.00	0.0000	131.00	No Ice	0.31	0.08	0.00
			0.00			1/2" Ice	0.39	0.12	0.01
DD 71 (7 0 1 D 0 7	-		0.00	0.0000	121.00	1" Ice	0.47	0.17	0.01
DB-T1-6Z-8AB-0Z	C	From Face	3.00	0.0000	131.00	No Ice	4.80	2.00	0.04
			0.00			1/2" Ice	5.07	2.19	0.08
EEL 101 II.		N	0.00	0.0000	121.00	1" Ice	5.35	2.39	0.12
EEI 10' Universal T-Arm	A	None		0.0000	131.00	No Ice 1/2" Ice	13.34	13.34	0.45
						1/2" Ice	16.80 20.26	16.80	0.60
EEI 10' Universal T-Arm	В	None		0.0000	131.00	No Ice	13.34	20.26 13.34	0.75 0.45
EEI 10 Olliveisai 1-Ailli	ь	None		0.0000	131.00	1/2" Ice	16.80	16.80	0.43
						1" Ice	20.26	20.26	0.00
EEI 10' Universal T-Arm	C	None		0.0000	131.00	No Ice	13.34	13.34	0.75
LLI 10 Chiversai 1-Aim	C	None		0.0000	131.00	1/2" Ice	16.80	16.80	0.60
						1" Ice	20.26	20.26	0.75
120ft									*****
(2) 7770.00 w/ Mount Pipe	Α	From Face	3.00	0.0000	121.00	No Ice	5.75	4.25	0.06
1			0.00			1/2" Ice	6.18	5.01	0.10
			0.00			1" Ice	6.61	5.71	0.16
(2) 7770.00 w/ Mount Pipe	В	From Face	3.00	0.0000	121.00	No Ice	5.75	4.25	0.06
-			0.00			1/2" Ice	6.18	5.01	0.10
			0.00			1" Ice	6.61	5.71	0.16
(2) 7770.00 w/ Mount Pipe	C	From Face	3.00	0.0000	121.00	No Ice	5.75	4.25	0.06
			0.00			1/2" Ice	6.18	5.01	0.10
			0.00			1" Ice	6.61	5.71	0.16
AM-X-CD-16-65-00T-RET	A	From Face	3.00	0.0000	121.00	No Ice	8.26	6.30	0.07
w/ Mount Pipe			0.00			1/2" Ice	8.82	7.48	0.14
	-		0.00	0.0000	121.00	1" Ice	9.35	8.37	0.21
AM-X-CD-16-65-00T-RET	В	From Face	3.00	0.0000	121.00	No Ice	8.26	6.30	0.07
w/ Mount Pipe			0.00			1/2" Ice	8.82	7.48	0.14
AM V CD 17 (5 00T DET		г г	0.00	0.0000	121.00	1" Ice	9.35	8.37	0.21
AM-X-CD-17-65-00T-RET	C	From Face	3.00	0.0000	121.00	No Ice	11.55	8.94	0.09
w/ Mount Pipe			0.00			1/2" Ice	12.27	10.45	0.18
(2) RRUS 11	A	From Face	0.00 3.00	0.0000	121.00	1" Ice No Ice	13.00 2.78	11.99 1.19	0.27 0.05
(2) KKUS 11	А	FIOIII Face	0.00	0.0000	121.00	1/2" Ice	2.78	1.19	0.03
			0.00			1" Ice	3.21	1.33	0.07
(2) RRUS 11	В	From Face	3.00	0.0000	121.00	No Ice	2.78	1.49	0.10
(2) KKOS 11	D	1 Tom Face	0.00	0.0000	121.00	1/2" Ice	2.78	1.19	0.03
			0.00			1" Ice	3.21	1.49	0.10
(2) RRUS 11	C	From Face	3.00	0.0000	121.00	No Ice	2.78	1.19	0.05
(-)	- Z		0.00	0.0000	121.00	1/2" Ice	2.99	1.33	0.07
			0.00			1" Ice	3.21	1.49	0.10
(2) LGP21401	A	From Face	3.00	0.0000	121.00	No Ice	1.10	0.21	0.01
		_	0.00			1/2" Ice	1.24	0.27	0.02
			0.00			1" Ice	1.38	0.35	0.03

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weight
			Vert ft ft	0	ft		ft²	ft²	K
			ft						
(2) LGP21401	В	From Face	3.00	0.0000	121.00	No Ice	1.10	0.21	0.01
			0.00			1/2" Ice	1.24	0.27	0.02
			0.00			1" Ice	1.38	0.35	0.03
(2) LGP21401	C	From Face	3.00	0.0000	121.00	No Ice	1.10	0.21	0.01
			0.00			1/2" Ice	1.24	0.27	0.02
			0.00			1" Ice	1.38	0.35	0.03
(2) LGP21901	Α	From Face	3.00	0.0000	121.00	No Ice	0.23	0.16	0.01
			0.00			1/2" Ice	0.29	0.21	0.01
			0.00			1" Ice	0.36	0.28	0.01
(2) LGP21901	В	From Face	3.00	0.0000	121.00	No Ice	0.23	0.16	0.01
			0.00			1/2" Ice	0.29	0.21	0.01
			0.00			1" Ice	0.36	0.28	0.01
(2) LGP21901	C	From Face	3.00	0.0000	121.00	No Ice	0.23	0.16	0.01
			0.00			1/2" Ice	0.29	0.21	0.01
			0.00			1" Ice	0.36	0.28	0.01
DC6-48-60-18-8F	C	From Face	3.00	0.0000	121.00	No Ice	2.20	2.20	0.02
			0.00			1/2" Ice	2.40	2.40	0.04
			0.00			1" Ice	2.60	2.60	0.07
EEI 10' Universal T-Arm	A	None		0.0000	121.00	No Ice	13.34	13.34	0.45
						1/2" Ice	16.80	16.80	0.60
						1" Ice	20.26	20.26	0.75
EEI 10' Universal T-Arm	В	None		0.0000	121.00	No Ice	13.34	13.34	0.45
						1/2" Ice	16.80	16.80	0.60
						1" Ice	20.26	20.26	0.75
EEI 10' Universal T-Arm	C	None		0.0000	121.00	No Ice	13.34	13.34	0.45
						1/2" Ice	16.80	16.80	0.60
						1" Ice	20.26	20.26	0.75

					Di	shes					
Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter		Aperture Area	Weight
				Vert ft	٥	0	ft	ft		ft ²	K
HP2-4.7	С	Paraboloid w/Shroud (HP)	From Leg	1.00 0.00	0.0000		157.00	2.00	No Ice 1/2" Ice	3.14 3.41	0.03 0.04
HP3-4.7	C	Paraboloid	From	0.00 1.00	0.0000		157.00	3.00	1" Ice No Ice	3.68 7.88	0.06 0.05
		w/Shroud (HP)	Leg	$0.00 \\ 0.00$					1/2" Ice 1" Ice	8.30 8.72	0.05 0.07

Tower Pressures - No Ice

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Section	z	K_Z	q_z	A_G	F	A_F	A_R	A_{leg}	Leg	$C_A A_A$	$C_A A_A$
Elevation					а			_	%	In	Out
					С					Face	Face
ft	ft		psf	ft^2	e	ft ²	ft ²	ft^2		ft ²	ft^2
L1	144.14	1.367	33	44.836	A	0.000	44.836	44.836	100.00	0.000	0.000
154.00-134.83					В	0.000	44.836		100.00	0.000	0.000
					C	0.000	44.836		100.00	0.000	0.000
L2	111.02	1.294	31	136.329	Α	0.000	136.329	136.329	100.00	0.000	0.000
134.83-89.16					В	0.000	136.329		100.00	0.000	0.000
					C	0.000	136.329		100.00	0.000	0.000
L3 89.16-44.54	66.35	1.161	28	172.858	Α	0.000	172.858	172.858	100.00	0.000	0.000
					В	0.000	172.858		100.00	0.000	0.000
					C	0.000	172.858		100.00	0.000	0.000
L4 44.54-1.00	22.98	0.929	22	204.992	Α	0.000	204.992	204.992	100.00	0.000	0.000
					В	0.000	204.992		100.00	0.000	0.000
					C	0.000	204.992		100.00	0.000	0.000

Tower Pressure - With Ice

 $G_H = 1.100$

Section	z	K_Z	q_z	t_Z	A_G	F	A_F	A_R	A_{leg}	Leg	$C_A A_A$	$C_A A_A$
Elevation						a				%	In	Out
						c					Face	Face
ft	ft		psf	in	ft^2	e	ft^2	ft ²	ft^2		ft^2	ft^2
L1	144.14	1.367	5	2.3177	52.241	A	0.000	52.241	52.241	100.00	0.000	0.000
154.00-134.83						В	0.000	52.241		100.00	0.000	0.000
						C	0.000	52.241		100.00	0.000	0.000
L2 134.83-89.16	111.02	1.294	5	2.2580	153.970	Α	0.000	153.970	153.970	100.00	0.000	0.000
						В	0.000	153.970		100.00	0.000	0.000
						C	0.000	153.970		100.00	0.000	0.000
L3 89.16-44.54	66.35	1.161	5	2.1447	189.650	A	0.000	189.650	189.650	100.00	0.000	0.000
						В	0.000	189.650		100.00	0.000	0.000
						C	0.000	189.650		100.00	0.000	0.000
L4 44.54-1.00	22.98	0.929	4	1.9289	220.555	Α	0.000	220.555	220.555	100.00	0.000	0.000
						В	0.000	220.555		100.00	0.000	0.000
						C	0.000	220.555		100.00	0.000	0.000

Tower Pressure - Service

 $G_H = 1.100$

Section	Z	K_Z	q_z	A_G	F	A_F	A_R	A_{leg}	Leg	$C_A A_A$	$C_A A_A$
Elevation					а				%	In	Out
					С					Face	Face
ft	ft		psf	ft^2	e	ft ²	ft ²	ft^2		ft^2	ft^2
L1	144.14	1.367	11	44.836	A	0.000	44.836	44.836	100.00	0.000	0.000
154.00-134.83					В	0.000	44.836		100.00	0.000	0.000
					C	0.000	44.836		100.00	0.000	0.000
L2	111.02	1.294	10	136.329	Α	0.000	136.329	136.329	100.00	0.000	0.000
134.83-89.16					В	0.000	136.329		100.00	0.000	0.000
					C	0.000	136.329		100.00	0.000	0.000
L3 89.16-44.54	66.35	1.161	9	172.858	Α	0.000	172.858	172.858	100.00	0.000	0.000
					В	0.000	172.858		100.00	0.000	0.000
					C	0.000	172.858		100.00	0.000	0.000
L4 44.54-1.00	22.98	0.929	7	204.992	Α	0.000	204.992	204.992	100.00	0.000	0.000

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Section	z	K_Z	q_z	A_G	F	A_F	A_R	A_{leg}	Leg	$C_A A_A$	$C_A A_A$
Elevation					а				%	In	Out
					С					Face	Face
ft	ft		psf	ft^2	e	ft^2	ft^2	ft^2		ft^2	ft^2
					В	0.000	204.992		100.00	0.000	0.000
					C	0.000	204.992		100.00	0.000	0.000

Tower Forces - No Ice - Wind Normal To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	K	K	e						ft ²	K	plf	
L1	0.25	1.07	Α	1	0.65	33	1	1	44.836	1.07	55.59	C
154.00-134.83			В	1	0.65		1	1	44.836			
			C	1	0.65		1	1	44.836			
L2	1.75	5.81	Α	1	0.65	31	1	1	136.329	3.06	67.07	C
134.83-89.16			В	1	0.65		1	1	136.329			
			C	1	0.65		1	1	136.329			
L3	1.89	12.11	Α	1	0.65	28	1	1	172.858	3.48	77.92	C
89.16-44.54			В	1	0.65		1	1	172.858			
			C	1	0.65		1	1	172.858			
L4 44.54-1.00	1.67	16.62	Α	1	0.65	22	1	1	204.992	3.29	75.63	C
			В	1	0.65		1	1	204.992			
			C	1	0.65		1	1	204.992			
Sum Weight:	5.56	35.61						OTM	789147.24	10.90		
									lb-ft			

Tower Forces - No Ice - Wind 60 To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			c			psf						
ft	K	K	e						ft ²	K	plf	
L1	0.25	1.07	Α	1	0.65	33	1	1	44.836	1.07	55.59	C
154.00-134.83			В	1	0.65		1	1	44.836			
			C	1	0.65		1	1	44.836			
L2	1.75	5.81	Α	1	0.65	31	1	1	136.329	3.06	67.07	C
134.83-89.16			В	1	0.65		1	1	136.329			
			C	1	0.65		1	1	136.329			
L3	1.89	12.11	Α	1	0.65	28	1	1	172.858	3.48	77.92	C
89.16-44.54			В	1	0.65		1	1	172.858			
			C	1	0.65		1	1	172.858			
L4 44.54-1.00	1.67	16.62	Α	1	0.65	22	1	1	204.992	3.29	75.63	C
			В	1	0.65		1	1	204.992			
			C	1	0.65		1	1	204.992			
Sum Weight:	5.56	35.61						OTM	789147.24	10.90		
									lb-ft			

Tower Forces - No Ice - Wind 90 To Face

tnxTower

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Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			С			psf						
ft	K	K	e						ft ²	K	plf	
L1	0.25	1.07	Α	1	0.65	33	1	1	44.836	1.07	55.59	C
154.00-134.83			В	1	0.65		1	1	44.836			
			C	1	0.65		1	1	44.836			
L2	1.75	5.81	Α	1	0.65	31	1	1	136.329	3.06	67.07	C
134.83-89.16			В	1	0.65		1	1	136.329			
			C	1	0.65		1	1	136.329			
L3	1.89	12.11	Α	1	0.65	28	1	1	172.858	3.48	77.92	C
89.16-44.54			В	1	0.65		1	1	172.858			
			C	1	0.65		1	1	172.858			
L4 44.54-1.00	1.67	16.62	Α	1	0.65	22	1	1	204.992	3.29	75.63	C
			В	1	0.65		1	1	204.992			
			C	1	0.65		1	1	204.992			
Sum Weight:	5.56	35.61						OTM	789147.24	10.90		
									lb-ft			

Tower Forces - With Ice - Wind Normal To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			c			psf						
ft	K	K	e						ft ²	K	plf	
L1	0.25	2.71	Α	1	1.2	5	1	1	52.241	0.37	19.13	C
154.00-134.83			В	1	1.2		1	1	52.241			
			C	1	1.2		1	1	52.241			
L2	1.75	10.59	Α	1	1.2	5	1	1	153.970	1.02	22.37	C
134.83-89.16			В	1	1.2		1	1	153.970			
			C	1	1.2		1	1	153.970			
L3	1.89	17.77	Α	1	1.2	5	1	1	189.650	1.13	25.25	C
89.16-44.54			В	1	1.2		1	1	189.650			
			C	1	1.2		1	1	189.650			
L4 44.54-1.00	1.67	22.59	Α	1	1.2	4	1	1	220.555	1.05	24.04	C
			В	1	1.2		1	1	220.555			
			C	1	1.2		1	1	220.555			
Sum Weight:	5.56	53.66						OTM	261568.45	3.56		
									lb-ft			

Tower Forces - With Ice - Wind 60 To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			c			psf						
ft	K	K	e						ft^2	K	plf	
L1	0.25	2.71	Α	1	1.2	5	1	1	52.241	0.37	19.13	C
154.00-134.83			В	1	1.2		1	1	52.241			
			C	1	1.2		1	1	52.241			
L2	1.75	10.59	Α	1	1.2	5	1	1	153.970	1.02	22.37	C
134.83-89.16			В	1	1.2		1	1	153.970			
			C	1	1.2		1	1	153.970			

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Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	K	K	e						ft^2	K	plf	
L3	1.89	17.77	Α	1	1.2	5	1	1	189.650	1.13	25.25	C
89.16-44.54			В	1	1.2		1	1	189.650			
			C	1	1.2		1	1	189.650			
L4 44.54-1.00	1.67	22.59	Α	1	1.2	4	1	1	220.555	1.05	24.04	C
			В	1	1.2		1	1	220.555			
			C	1	1.2		1	1	220.555			
Sum Weight:	5.56	53.66						OTM	261568.45	3.56		
									lb-ft			

Tower Forces - With Ice - Wind 90 To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			С			psf			_			
ft	K	K	e						ft ²	K	plf	
L1	0.25	2.71	Α	1	1.2	5	1	1	52.241	0.37	19.13	C
154.00-134.83			В	1	1.2		1	1	52.241			
			C	1	1.2		1	1	52.241			
L2	1.75	10.59	Α	1	1.2	5	1	1	153.970	1.02	22.37	C
134.83-89.16			В	1	1.2		1	1	153.970			
			C	1	1.2		1	1	153.970			
L3	1.89	17.77	Α	1	1.2	5	1	1	189.650	1.13	25.25	C
89.16-44.54			В	1	1.2		1	1	189.650			
			C	1	1.2		1	1	189.650			
L4 44.54-1.00	1.67	22.59	Α	1	1.2	4	1	1	220.555	1.05	24.04	C
			В	1	1.2		1	1	220.555			
			C	1	1.2		1	1	220.555			
Sum Weight:	5.56	53.66						OTM	261568.45	3.56		
8									lb-ft			

Tower Forces - Service - Wind Normal To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			c			psf						
ft	K	K	e						ft ²	K	plf	
L1	0.25	1.07	Α	1	0.65	11	1	1	44.836	0.34	17.91	C
154.00-134.83			В	1	0.65		1	1	44.836			
			C	1	0.65		1	1	44.836			
L2	1.75	5.81	Α	1	0.65	10	1	1	136.329	0.99	21.60	C
134.83-89.16			В	1	0.65		1	1	136.329			
			C	1	0.65		1	1	136.329			
L3	1.89	12.11	Α	1	0.65	9	1	1	172.858	1.12	25.10	C
89.16-44.54			В	1	0.65		1	1	172.858			
			C	1	0.65		1	1	172.858			
L4 44.54-1.00	1.67	16.62	Α	1	0.65	7	1	1	204.992	1.06	24.36	C
			В	1	0.65		1	1	204.992			
			C	1	0.65		1	1	204.992			
Sum Weight:	5.56	35.61						OTM	254188.48	3.51		

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Γ	Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
	Elevation	Weight	Weight	а									Face
				С			psf						
	ft	K	K	e						ft^2	K	plf	
L										lb-ft			

Tower Forces - Service - Wind 60 To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			c			psf						
ft	K	K	e						ft^2	K	plf	
L1	0.25	1.07	Α	1	0.65	11	1	1	44.836	0.34	17.91	C
154.00-134.83			В	1	0.65		1	1	44.836			
			C	1	0.65		1	1	44.836			
L2	1.75	5.81	Α	1	0.65	10	1	1	136.329	0.99	21.60	C
134.83-89.16			В	1	0.65		1	1	136.329			
			C	1	0.65		1	1	136.329			
L3	1.89	12.11	Α	1	0.65	9	1	1	172.858	1.12	25.10	C
89.16-44.54			В	1	0.65		1	1	172.858			
			C	1	0.65		1	1	172.858			
L4 44.54-1.00	1.67	16.62	Α	1	0.65	7	1	1	204.992	1.06	24.36	C
			В	1	0.65		1	1	204.992			
			C	1	0.65		1	1	204.992			
Sum Weight:	5.56	35.61						OTM	254188.48	3.51		
									lb-ft			

Tower Forces - Service - Wind 90 To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			c			psf			_			
ft	K	K	e						ft ²	K	plf	
L1	0.25	1.07	Α	1	0.65	11	1	1	44.836	0.34	17.91	C
154.00-134.83			В	1	0.65		1	1	44.836			
			C	1	0.65		1	1	44.836			
L2	1.75	5.81	Α	1	0.65	10	1	1	136.329	0.99	21.60	C
134.83-89.16			В	1	0.65		1	1	136.329			
			C	1	0.65		1	1	136.329			
L3	1.89	12.11	Α	1	0.65	9	1	1	172.858	1.12	25.10	C
89.16-44.54			В	1	0.65		1	1	172.858			
			C	1	0.65		1	1	172.858			
L4 44.54-1.00	1.67	16.62	Α	1	0.65	7	1	1	204.992	1.06	24.36	C
			В	1	0.65		1	1	204.992			
			C	1	0.65		1	1	204.992			
Sum Weight:	5.56	35.61						OTM	254188.48	3.51		
									lb-ft			

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Force Totals

Load	Vertical	Sum of	Sum of	Sum of	Sum of	Sum of Torques
Case	Forces	Forces	Forces	Overturning	Overturning	
		X	Z	Moments, M_x	Moments, M_z	
	K	K	K	lb-ft	lb-ft	lb-ft
Leg Weight	35.61					
Bracing Weight	0.00					
Total Member Self-Weight	35.61			428.16	142.17	
Total Weight	60.36			428.16	142.17	
Wind 0 deg - No Ice		0.26	-46.61	-5091080.13	-40504.93	-299.57
Wind 30 deg - No Ice		23.48	-40.42	-4416865.98	-2573606.99	281.36
Wind 60 deg - No Ice		40.45	-23.40	-2560401.14	-4424391.97	728.88
Wind 90 deg - No Ice		46.67	-0.17	-25333.09	-5102736.36	981.09
Wind 120 deg - No Ice		40.41	23.08	2510980.88	-4418604.59	1028.45
Wind 150 deg - No Ice		23.18	40.33	4404122.07	-2527924.96	1056.24
Wind 180 deg - No Ice		-0.16	46.59	5088333.33	25547.93	484.09
Wind 210 deg - No Ice		-23.40	40.34	4406348.98	2561692.65	-204.07
Wind 240 deg - No Ice		-40.36	23.35	2553381.96	4411035.55	-728.88
Wind 270 deg - No Ice		-46.56	0.13	21311.69	5087071.79	-1058.38
Wind 300 deg - No Ice		-40.34	-23.15	-2521522.40	4408147.87	-1212.97
Wind 330 deg - No Ice		-23.26	-40.28	-4396265.31	2540334.42	-1056.24
Member Ice	18.05					
Total Weight Ice	115.06			2519.93	380.83	
Wind 0 deg - Ice		0.05	-17.89	-1944677.55	-8019.58	
Wind 30 deg - Ice		8.98	-15.50	-1685439.07	-979437.67	73.53
Wind 60 deg - Ice		15.52	-8.96	-974194.34	-1689826.34	177.46
Wind 90 deg - Ice		17.91	-0.03	-2804.07	-1950034.73	233.84
Wind 120 deg - Ice		15.51	8.90	968843.70	-1688630.28	239.39
Wind 150 deg - Ice		8.92	15.49	1687668.22	-969996.70	233.58
Wind 180 deg - Ice		-0.03	17.88	1948972.77	5631.36	
Wind 210 deg - Ice		-8.97	15.49	1688128.44	977678.27	-57.79
Wind 240 deg - Ice		-15.50	8.95	977606.60	1687768.92	-177.46
Wind 270 deg - Ice		-17.89	0.03	6835.87	1947500.28	-249.59
Wind 300 deg - Ice		-15.49	-8.91	-966159.39	1687172.13	-277.30
Wind 330 deg - Ice		-8.94	-15.48	-1681181.59	973264.23	-233.58
Total Weight	60.36			428.16	142.17	
Wind 0 deg - Service		0.08	-15.01	-1639573.46	-12950.47	-96.49
Wind 30 deg - Service		7.56	-13.02	-1422405.53	-828875.98	90.63
Wind 60 deg - Service		13.03	-7.54	-824428.44	-1425023.56	234.78
Wind 90 deg - Service		15.03	-0.05	-7869.68	-1643521.86	316.01
Wind 120 deg - Service		13.02	7.43	809090.40	-1423159.42	331.27
Wind 150 deg - Service		7.47	12.99	1418881.14	-814161.56	
Wind 180 deg - Service		-0.05	15.01	1639269.19	8325.50	155.93
Wind 210 deg - Service		-7.54	13.00	1419598.44	825231.06	-65.73
Wind 240 deg - Service		-13.00	7.52	822748.01	1420914.15	-234.78
Wind 270 deg - Service		-15.00	0.04	7154.85	1638668.98	-340.91
Wind 300 deg - Service		-12.99	-7.46	-811905.39	1419984.01	-390.70
Wind 330 deg - Service		-7.49	-12.98	-1415769.95	818351.47	-340.22

Load Combinations

Comb.		Description	
No.			
1	Dead Only		
2	1.2 Dead+1.6 Wind 0 deg - No Ice		
3	0.9 Dead+1.6 Wind 0 deg - No Ice		
4	1.2 Dead+1.6 Wind 30 deg - No Ice		
5	0.9 Dead+1.6 Wind 30 deg - No Ice		

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Comb.	Description
No.	·
6	1.2 Dead+1.6 Wind 60 deg - No Ice
7	0.9 Dead+1.6 Wind 60 deg - No Ice
8	1.2 Dead+1.6 Wind 90 deg - No Ice
9	0.9 Dead+1.6 Wind 90 deg - No Ice
10	1.2 Dead+1.6 Wind 120 deg - No Ice
11	0.9 Dead+1.6 Wind 120 deg - No Ice
12	1.2 Dead+1.6 Wind 150 deg - No Ice
13	0.9 Dead+1.6 Wind 150 deg - No Ice
14	1.2 Dead+1.6 Wind 180 deg - No Ice
15	0.9 Dead+1.6 Wind 180 deg - No Ice
16	1.2 Dead+1.6 Wind 210 deg - No Ice
17	0.9 Dead+1.6 Wind 210 deg - No Ice
18	1.2 Dead+1.6 Wind 240 deg - No Ice
19	0.9 Dead+1.6 Wind 240 deg - No Ice
20	1.2 Dead+1.6 Wind 270 deg - No Ice
21	0.9 Dead+1.6 Wind 270 deg - No Ice
22	1.2 Dead+1.6 Wind 300 deg - No Ice
23	0.9 Dead+1.6 Wind 300 deg - No Ice
24	1.2 Dead+1.6 Wind 330 deg - No Ice
25	0.9 Dead+1.6 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31 32	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
33 34	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load	Axial	Major Axis Moment	Minor Axis Moment
IVO.	Ji	Туре		Comb.	K	lb-ft	lb-ft
L1	154 - 134.83	Pole	Max Tension	14	0.00	-4.26	0.02
			Max. Compression	26	-21.10	413.56	-252.97
			Max. Mx	8	-5.81	-118119.92	4665.66
			Max. My	2	-5.85	-7378.30	113796.58
			Max. Vy	8	18.13	-118119.92	4665.66
			Max. Vx	2	-17.88	-7378.30	113796.58
			Max. Torque	24			1056.95
L2	134.83 - 89.16	Pole	Max Tension	1	0.00	0.00	0.00

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Section	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
No.	ft	Type		Load		Moment	Moment
	·			Comb.	K	lb-ft	lb-ft
			Max. Compression	26	-68.31	425.65	-2694.33
			Max. Mx	8	-24.39	-1860734.1	16538.12
						2	
			Max. My	2	-24.41	-26673.98	1850253.40
			Max. Vy	8	55.91	-1860734.1	16538.12
			•			2	
			Max. Vx	2	-55.81	-26673.98	1850253.40
			Max. Torque	22			2029.66
L3	89.16 - 44.54	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-96.91	420.53	-2666.89
			Max. Mx	8	-44.88	-4730372.9	28531.19
						5	
			Max. My	2	-44.89	-45670.49	4715792.16
			Max. Vy	8	70.49	-4730372.9	28531.19
			•			5	
			Max. Vx	2	-70.40	-45670.49	4715792.16
			Max. Torque	22			2023.27
L4	44.54 - 1	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-129.25	413.74	-2630.48
			Max. Mx	8	-72.36	-8406715.8	42188.06
						7	
			Max. My	2	-72.36	-67227.50	8387524.79
			Max. Vy	8	74.73	-8406715.8	42188.06
			·			7	
			Max. Vx	2	-74.64	-67227.50	8387524.79
			Max. Torque	22			2015.30

Maximum Reactions

Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, Z
		Load	K	K	K
		Comb.			
Pole	Max. Vert	26	129.25	-0.00	0.00
	Max. H _x	21	54.32	74.50	-0.21
	Max. H _z	3	54.32	-0.42	74.58
	Max. M _x	2	8387524.78	-0.42	74.57
	Max. M _z	8	8406715.87	-74.66	0.26
	Max. Torsion	22	2012.14	64.55	37.04
	Min. Vert	9	54.32	-74.66	0.26
	Min. H _x	9	54.32	-74.66	0.26
	Min. H _z	15	54.32	0.26	-74.54
	Min. M _x	14	-8382658.31	0.26	-74.54
	Min. M _z	20	-8380766.76	74.50	-0.21
	Min. Torsion	12	-1707.28	-37.09	-64.53

Tower Mast Reaction Summary

Load Combination	Vertical	Shear _x	Shear _z	Overturning Moment, M _x	Overturning Moment, M_z	Torque
	K	K	K	lb-ft	lb-ft	lb-ft
Dead Only	60.36	-0.00	0.00	428.16	142.17	0.00
1.2 Dead+1.6 Wind 0 deg - No	72.43	0.42	-74.57	-8387524.78	-67227.61	-349.24
Ice						

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Load Combination	Vertical	$Shear_x$	$Shear_z$	Overturning Moment, M_x	Overturning Moment, M_z	Torque
	K	K	K	lb-ft	lb-ft	lb-ft
0.9 Dead+1.6 Wind 0 deg - No	54.32	0.42	-74.58	-8321828.53	-66620.80	-346.06
Ice 1.2 Dead+1.6 Wind 30 deg - No	72.43	37.56	-64.67	-7277042.60	-4240422.41	518.73
Ice 0.9 Dead+1.6 Wind 30 deg - No Ice	54.32	37.56	-64.67	-7219932.98	-4207025.46	521.41
1.2 Dead+1.6 Wind 60 deg - No Ice	72.43	64.72	-37.44	-4218593.80	-7289478.97	1142.78
0.9 Dead+1.6 Wind 60 deg - No Ice	54.32	64.72	-37.44	-4185510.73	-7232136.26	1144.41
1.2 Dead+1.6 Wind 90 deg - No Ice	72.43	74.66	-0.26	-42188.49	-8406715.87	1461.36
0.9 Dead+1.6 Wind 90 deg - No Ice	54.32	74.66	-0.26	-41913.01	-8340719.74	1461.45
1.2 Dead+1.6 Wind 120 deg - No Ice	72.43	64.66	36.93	4136409.20	-7280115.29	1493.64
0.9 Dead+1.6 Wind 120 deg - No Ice	54.32	64.66	36.93	4103852.99	-7222855.25	1492.05
1.2 Dead+1.6 Wind 150 deg - No Ice	72.43	37.09	64.53	7255818.17	-4164826.95	1707.28
0.9 Dead+1.6 Wind 150 deg - No Ice	54.32	37.09	64.53	7198638.29	-4132156.76	1702.52
1.2 Dead+1.6 Wind 180 deg - No Ice	72.43	-0.26	74.54	8382658.31	42305.43	869.33
0.9 Dead+1.6 Wind 180 deg - No Ice	54.32	-0.26	74.54	8316737.62	41851.46	863.59
1.2 Dead+1.6 Wind 210 deg - No Ice	72.43	-37.44	64.55	7259376.16	4220626.42	-235.96
0.9 Dead+1.6 Wind 210 deg - No Ice	54.32	-37.44	64.55	7202164.47	4187328.17	-240.49
1.2 Dead+1.6 Wind 240 deg - No Ice	72.43	-64.58	37.36	4206667.78	7267343.10	-1143.98
0.9 Dead+1.6 Wind 240 deg - No Ice	54.32	-64.58	37.36	4173430.57	7210119.69	-1145.56
1.2 Dead+1.6 Wind 270 deg - No Ice	72.43	-74.50	0.21	35160.50	8380766.76	-1745.08
0.9 Dead+1.6 Wind 270 deg - No Ice	54.32	-74.50	0.21	34686.70	8314927.51	-1743.32
1.2 Dead+1.6 Wind 300 deg - No Ice	72.43	-64.55	-37.04	-4154268.67	7262680.00	-2012.14
0.9 Dead+1.6 Wind 300 deg - No Ice	54.32	-64.55	-37.04	-4121804.83	7205500.29	-2008.04
1.2 Dead+1.6 Wind 330 deg - No Ice	72.43	-37.22	-64.46	-7243143.06	4185276.71	-1706.19
0.9 Dead+1.6 Wind 330 deg - No Ice	54.32	-37.22	-64.46	-7186354.19	4152322.05	-1701.42
1.2 Dead+1.0 Ice+1.0 Temp	129.25	0.00	-0.00	2630.48	413.74	0.05
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	129.25	0.05	-17.89	-2088174.65	-8722.27	-69.00
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	129.25	8.98	-15.50	-1809885.27	-1051986.75	65.85
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	129.25	15.52	-8.96	-1046042.74	-1814899.61	170.82
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	129.25	17.91	-0.03	-2820.65	-2094252.13	230.04
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	129.25	15.51	8.90	1040684.14	-1813591.47	239.88
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	129.25	8.92	15.48	1812827.47	-1041649.64	243.92
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	129.25	-0.03	17.88	2093376.69	6225.13	114.24

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Load	Vertical	$Shear_x$	$Shear_z$	Overturning	Overturning	Torque
Combination				Moment, M_x	Moment, M_z	
	K	K	K	lb-ft	lb-ft	lb-ft
1.2 Dead+1.0 Wind 210	129.25	-8.97	15.49	1813330.52	1050178.61	-44.95
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 240	129.25	-15.50	8.95	1050279.57	1812765.70	-170.79
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 270	129.25	-17.89	0.03	7734.87	2091596.61	-250.81
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 300	129.25	-15.49	-8.91	-1037245.60	1812113.66	-284.88
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 330	129.25	-8.94	-15.48	-1805225.65	1045345.84	-243.73
deg+1.0 Ice+1.0 Temp						
Dead+Wind 0 deg - Service	60.36	0.08	-15.01	-1682278.56	-13366.20	-94.08
Dead+Wind 30 deg - Service	60.36	7.56	-13.02	-1459467.72	-850532.01	91.77
Dead+Wind 60 deg - Service	60.36	13.03	-7.54	-845926.75	-1462190.98	233.80
Dead+Wind 90 deg - Service	60.36	15.03	-0.05	-8116.27	-1686376.88	313.22
Dead+Wind 120 deg - Service	60.36	13.02	7.43	830109.61	-1460268.58	327.93
Dead+Wind 150 deg - Service	60.36	7.47	12.99	1455847.31	-835343.46	344.30
Dead+Wind 180 deg - Service	60.36	-0.05	15.00	1681980.73	8597.46	162.60
Dead+Wind 210 deg - Service	60.36	-7.54	12.99	1456586.81	846775.88	-60.48
Dead+Wind 240 deg - Service	60.36	-13.00	7.52	844208.28	1457955.80	-233.84
Dead+Wind 270 deg - Service	60.36	-15.00	0.04	7393.90	1681374.19	-344.52
Dead+Wind 300 deg - Service	60.36	-12.99	-7.46	-833000.17	1456996.44	-396.38
Dead+Wind 330 deg - Service	60.36	-7.49	-12.97	-1452619.82	839674.26	-344.26

Solution Summary

	Sui	m of Applied Forces	·		Sum of Reaction	ıs	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
1	0.00	-60.36	0.00	0.00	60.36	-0.00	0.000%
2	0.42	-72.43	-74.58	-0.42	72.43	74.57	0.003%
3	0.42	-54.32	-74.58	-0.42	54.32	74.58	0.002%
4	37.56	-72.43	-64.67	-37.56	72.43	64.67	0.000%
5	37.56	-54.32	-64.67	-37.56	54.32	64.67	0.000%
6	64.72	-72.43	-37.44	-64.72	72.43	37.44	0.000%
7	64.72	-54.32	-37.44	-64.72	54.32	37.44	0.000%
8	74.67	-72.43	-0.26	-74.66	72.43	0.26	0.003%
9	74.67	-54.32	-0.26	-74.66	54.32	0.26	0.002%
10	64.66	-72.43	36.93	-64.66	72.43	-36.93	0.000%
11	64.66	-54.32	36.93	-64.66	54.32	-36.93	0.000%
12	37.09	-72.43	64.53	-37.09	72.43	-64.53	0.000%
13	37.09	-54.32	64.53	-37.09	54.32	-64.53	0.000%
14	-0.26	-72.43	74.54	0.26	72.43	-74.54	0.003%
15	-0.26	-54.32	74.54	0.26	54.32	-74.54	0.002%
16	-37.44	-72.43	64.55	37.44	72.43	-64.55	0.000%
17	-37.44	-54.32	64.55	37.44	54.32	-64.55	0.000%
18	-64.58	-72.43	37.36	64.58	72.43	-37.36	0.000%
19	-64.58	-54.32	37.36	64.58	54.32	-37.36	0.000%
20	-74.50	-72.43	0.21	74.50	72.43	-0.21	0.003%
21	-74.50	-54.32	0.21	74.50	54.32	-0.21	0.002%
22	-64.55	-72.43	-37.04	64.55	72.43	37.04	0.000%
23	-64.55	-54.32	-37.04	64.55	54.32	37.04	0.000%
24	-37.22	-72.43	-64.46	37.22	72.43	64.46	0.000%
25	-37.22	-54.32	-64.46	37.22	54.32	64.46	0.000%
26	0.00	-129.25	0.00	-0.00	129.25	0.00	0.001%
27	0.05	-129.25	-17.89	-0.05	129.25	17.89	0.000%
28	8.98	-129.25	-15.50	-8.98	129.25	15.50	0.000%
29	15.52	-129.25	-8.96	-15.52	129.25	8.96	0.000%
30	17.91	-129.25	-0.03	-17.91	129.25	0.03	0.001%

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	Sui	m of Applied Forces	ĭ		Sum of Reaction	S	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
31	15.51	-129.25	8.90	-15.51	129.25	-8.90	0.000%
32	8.92	-129.25	15.49	-8.92	129.25	-15.48	0.000%
33	-0.03	-129.25	17.88	0.03	129.25	-17.88	0.001%
34	-8.97	-129.25	15.49	8.97	129.25	-15.49	0.000%
35	-15.50	-129.25	8.95	15.50	129.25	-8.95	0.000%
36	-17.89	-129.25	0.03	17.89	129.25	-0.03	0.001%
37	-15.49	-129.25	-8.91	15.49	129.25	8.91	0.000%
38	-8.94	-129.25	-15.48	8.94	129.25	15.48	0.000%
39	0.08	-60.36	-15.01	-0.08	60.36	15.01	0.003%
40	7.56	-60.36	-13.02	-7.56	60.36	13.02	0.003%
41	13.03	-60.36	-7.54	-13.03	60.36	7.54	0.003%
42	15.03	-60.36	-0.05	-15.03	60.36	0.05	0.003%
43	13.02	-60.36	7.43	-13.02	60.36	-7.43	0.003%
44	7.47	-60.36	12.99	-7.47	60.36	-12.99	0.003%
45	-0.05	-60.36	15.01	0.05	60.36	-15.00	0.003%
46	-7.54	-60.36	13.00	7.54	60.36	-12.99	0.003%
47	-13.00	-60.36	7.52	13.00	60.36	-7.52	0.003%
48	-15.00	-60.36	0.04	15.00	60.36	-0.04	0.003%
49	-12.99	-60.36	-7.46	12.99	60.36	7.46	0.003%
50	-7.49	-60.36	-12.98	7.49	60.36	12.97	0.003%

Non-Linear Convergence Results

C 12			
Converged?	Number	Displacement	Force
	of Cycles	Tolerance	Tolerance
Yes	6	0.00000001	0.00000001
Yes	12	0.00003624	0.00009493
Yes	12	0.00002433	0.00007326
Yes	15	0.00000001	0.00013692
Yes	15	0.00000001	0.00009463
Yes	15	0.00000001	0.00013543
Yes	15	0.00000001	0.00009357
Yes	12	0.00003621	0.00006447
Yes	12	0.00002430	0.00005182
Yes	15	0.00000001	0.00013375
Yes	15	0.00000001	0.00009257
Yes	15	0.00000001	0.00013073
Yes	15	0.00000001	0.00009039
Yes	12	0.00003625	0.00013494
Yes	12	0.00002433	0.00010307
Yes	15	0.00000001	0.00013514
Yes	15	0.00000001	0.00009340
Yes	15	0.00000001	0.00013588
Yes	15	0.00000001	0.00009396
Yes	12	0.00003624	0.00014765
Yes	12	0.00002433	0.00011281
Yes	15	0.00000001	0.00013052
Yes	15	0.00000001	0.00009026
Yes	15	0.00000001	0.00013471
Yes	15	0.00000001	0.00009324
Yes	6	0.00000001	0.00000614
Yes	13	0.00000001	0.00010135
Yes	14	0.00000001	0.00005843
Yes	14	0.00000001	0.00005815
Yes	13	0.00000001	0.00010190
Yes	14	0.00000001	0.00005843
	Yes	of Cycles Yes 6 Yes 12 Yes 15 Yes 15 Yes 15 Yes 15 Yes 12 Yes 12 Yes 15 Yes 15 Yes 15 Yes 12 Yes 12 Yes 15 Yes 15 <tr< td=""><td>Yes 6 0.00000001 Yes 12 0.00003624 Yes 12 0.00002433 Yes 15 0.00000001 Yes 12 0.00003621 Yes 12 0.00002430 Yes 15 0.00000001 Yes 12 0.00003625 Yes 15 0.00000001 Yes 15 0.000000001 <td< td=""></td<></td></tr<>	Yes 6 0.00000001 Yes 12 0.00003624 Yes 12 0.00002433 Yes 15 0.00000001 Yes 12 0.00003621 Yes 12 0.00002430 Yes 15 0.00000001 Yes 12 0.00003625 Yes 15 0.00000001 Yes 15 0.000000001 <td< td=""></td<>

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32	Yes	14	0.00000001	0.00005790
33	Yes	13	0.00000001	0.00010199
34	Yes	14	0.00000001	0.00005873
35	Yes	14	0.00000001	0.00005889
36	Yes	13	0.00000001	0.00010184
37	Yes	14	0.00000001	0.00005743
38	Yes	14	0.00000001	0.00005815
39	Yes	11	0.00009827	0.00006571
40	Yes	11	0.00009809	0.00012767
41	Yes	11	0.00009809	0.00012184
42	Yes	11	0.00009827	0.00006668
43	Yes	11	0.00009810	0.00013060
44	Yes	11	0.00009810	0.00011375
45	Yes	11	0.00009828	0.00006670
46	Yes	11	0.00009810	0.00012484
47	Yes	11	0.00009810	0.00012984
48	Yes	11	0.00009828	0.00006750
49	Yes	11	0.00009810	0.00011309
50	Yes	11	0.00009810	0.00013211

Maximum Tower Deflections - Service Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	154 - 134.83	24.258	41	1.3358	0.0022
L2	139.16 - 89.16	20.129	41	1.3126	0.0014
L3	94.83 - 44.54	9.269	41	0.9317	0.0005
L4	51.46 - 1	2.679	41	0.4857	0.0002

Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	۰	۰	ft
157.00	HP2-4.7	41	24.258	1.3358	0.0022	43749
155.00	Pipe Mount [PM 601-3]	41	24.258	1.3358	0.0022	43749
151.00	(2) 72"x6.5"x8" Panel	41	23.417	1.3337	0.0020	43749
141.00	EEI Pine Braches	41	20.634	1.3182	0.0015	16930
131.00	EEI Pine Braches	41	17.921	1.2735	0.0012	10984
121.00	EEI Pine Braches	41	15.317	1.1994	0.0010	8369
111.00	EEI Pine Braches	41	12.857	1.1048	0.0008	6760
101.00	EEI Pine Braches	41	10.574	0.9988	0.0006	5670
91.00	EEI Pine Braches	41	8.502	0.8904	0.0005	5100
81.00	EEI Pine Braches	41	6.665	0.7846	0.0004	4956

Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0

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Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	154 - 134.83	120.726	6	6.6654	0.0105
L2	139.16 - 89.16	100.200	6	6.5481	0.0079
L3	94.83 - 44.54	46.187	6	4.6474	0.0024
L4	51.46 - 1	13.357	6	2.4224	0.0008

Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	0	ft
157.00	HP2-4.7	6	120.726	6.6654	0.0105	9022
155.00	Pipe Mount [PM 601-3]	6	120.726	6.6654	0.0105	9022
151.00	(2) 72"x6.5"x8" Panel	6	116.548	6.6547	0.0099	9022
141.00	EEI Pine Braches	6	102.715	6.5763	0.0082	3489
131.00	EEI Pine Braches	6	89.226	6.3527	0.0066	2259
121.00	EEI Pine Braches	6	76.278	5.9827	0.0052	1718
111.00	EEI Pine Braches	6	64.040	5.5109	0.0039	1384
101.00	EEI Pine Braches	6	52.682	4.9823	0.0029	1158
91.00	EEI Pine Braches	6	42.372	4.4414	0.0024	1038
81.00	EEI Pine Braches	6	33.223	3.9136	0.0020	1005

Compression Checks

			Po	le Des	sign [Data			
Section No.	Elevation	Size	L	L_u	Kl/r	A	P_u	ϕP_n	Ratio P _u
	ft		ft	ft		in^2	K	K	ϕP_n
L1	154 - 134.83 (1)	TP30.03x25.25x0.1875	19.17	153.00	179.8	17.1175	-5.80	119.61	0.048
L2	134.83 - 89.16	TP40.91x28.5753x0.3125	50.00	153.00	131.9	38.8803	-24.37	504.57	0.048
L3	89.16 - 44.54	TP51.28x38.8862x0.5	50.29	153.00	105.4	77.8814	-44.87	1584.16	0.028
L4	44.54 - 1 (4)	TP61x48.5746x0.5625	50.46	153.00	85.6	107.904 0	-72.36	3304.30	0.022

Pole Bending Design Data

Section	Elevation	Size	M_{ux}	ϕM_{nx}	Ratio	M_{uy}	ϕM_{ny}	Ratio
No.					M_{ux}			M_{uy}
	ft		lb-ft	lb-ft	ϕM_{nx}	lb-ft	lb-ft	ϕM_{ny}
L1	154 - 134.83	TP30.03x25.25x0.1875	119235.00	652442.50	0.183	0.00	652442.50	0.000
	(1)							
L2	134.83 - 89.16	TP40.91x28.5753x0.3125	1866016.67	2179983.33	0.856	0.00	2179983.33	0.000
	(2)							

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Section	Elevation	Size	M_{ux}	ϕM_{nx}	Ratio	M_{uy}	ϕM_{ny}	Ratio
No.					M_{ux}		•	M_{uy}
	ft		lb-ft	lb-ft	ϕM_{nx}	lb-ft	lb-ft	ϕM_{ny}
L3	89.16 - 44.54 (3)	TP51.28x38.8862x0.5	4740400.00	5825591.33	0.814	0.00	5825591.33	0.000
L4	44.54 - 1 (4)	TP61x48.5746x0.5625	8422416.67	9764666.67	0.863	0.00	9764666.67	0.000

	Pole Shear Design Data								
Section No.	Elevation	vation Size	Actual V _u	ϕV_n	Ratio V_u	Actual T _u	ϕT_n	Ratio T _u	
	ft		K	K	ϕV_n	lb-ft	lb-ft	ϕT_n	
L1	154 - 134.83 (1)	TP30.03x25.25x0.1875	18.20	550.82	0.033	0.27	1306483.33	0.000	
L2	134.83 - 89.16	TP40.91x28.5753x0.3125	56.02	1352.41	0.041	1150.09	4365300.00	0.000	
L3	89.16 - 44.54	TP51.28x38.8862x0.5	70.60	2893.10	0.024	1145.24	11665416.00	0.000	
L4	44.54 - 1 (4)	TP61x48.5746x0.5625	74.85	3934.15	0.019	518.65	19553166.67	0.000	

			F	Pole Int	teraction	on Des	ign Da	ta	
Section No.	Elevation	Ratio P _u	Ratio M _{ux}	Ratio M _{uy}	Ratio V _u	Ratio T _u	Comb. Stress	Allow. Stress	Criteria
	ft	ϕP_n	ϕM_{nx}	ϕM_{ny}	ϕV_n	ϕT_n	Ratio	Ratio	
L1	154 - 134.83 (1)	0.048	0.183	0.000	0.033	0.000	0.232	1.000	4.8.2
L2	134.83 - 89.16 (2)	0.048	0.856	0.000	0.041	0.000	0.906	1.000	4.8.2
L3	89.16 - 44.54 (3)	0.028	0.814	0.000	0.024	0.000	0.843	1.000	4.8.2
L4	44.54 - 1 (4)	0.022	0.863	0.000	0.019	0.000	0.885	1.000	4.8.2

Section Capacity Table								
Section No.	Elevation ft	Component Type	Size	Critical Element	P K	${^{\not{o}P_{allow}}_{K}}$	% Capacity	Pass Fail
L1	154 - 134.83	Pole	TP30.03x25.25x0.1875	1	-5.80	119.61	23.2	Pass
L2	134.83 - 89.16	Pole	TP40.91x28.5753x0.3125	2	-24.37	504.57	90.6	Pass
L3	89.16 - 44.54	Pole	TP51.28x38.8862x0.5	3	-44.87	1584.16	84.3	Pass
L4	44.54 - 1	Pole	TP61x48.5746x0.5625	4	-72.36	3304.30	88.5	Pass
							Summary	
						Pole (L2)	90.6	Pass
						RATING =	90.6	Pass

<i>tnxTower</i>

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Exhibit E



RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CTNH362B

NH362/Boardman_MP 33 Boardman Road New Milford, CT 06776

January 22, 2017

EBI Project Number: 6217000227

Site Compliance Summary					
Compliance Status:	COMPLIANT				
Site total MPE% of FCC general public allowable limit:	11.87 %				



January 22, 2017

T-Mobile USA Attn: Jason Overbey, RF Manager 35 Griffin Road South Bloomfield, CT 06002

Emissions Analysis for Site: CTNH362B – NH362/Boardman_MP

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **33 Boardman Road**, **New Milford**, **CT**, for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter (μ W/cm²). The number of μ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter (μ W/cm²). The general population exposure limit for the 700 MHz Band is approximately 467 μ W/cm², and the general population exposure limit for the 1900 MHz (PCS) and 2100 MHz (AWS) bands is 1000 μ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at **33 Boardman Road, New Milford, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 GSM channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) 2 LTE channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 3) 2 LTE channels (AWS Band 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel
- 4) 1 LTE channel (700 MHz Band) was considered for each sector of the proposed installation. This channel has a transmit power of 30 Watts.
- 5) Since all radios are ground mounted there are additional cabling losses accounted for. For each ground mounted RF path the following losses were calculated. 1.12 dB of additional cable loss for all ground mounted 700 MHz Channels and 1.95 dB of additional cable loss for all ground mounted 1900 MHz channels has been added to the calculations. This is based on manufacturers Specifications for 160 feet of 1-1/4" coax cable on each path.



- 6) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 7) For the following calculations the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antennas used in this modeling are the **RFS APXV18-209014-C** for 1900 MHz (PCS) channels and the **Commscope LNX-6515DS-VTM** for 700 MHz channels. This is based on feedback from the carrier with regards to anticipated antenna selection. The **RFS APXV18-209014-C** has a maximum gain of **14.4 dBd** at its main lobe at 1900 MHz. The **Commscope LNX-6515DS-VTM** has a maximum gain of **14.6 dBd** at its main lobe. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 9) The antenna mounting height centerline of the proposed antennas is **140 feet** above ground level (AGL).
- 10) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 11) All calculations were done with respect to uncontrolled / general public threshold limits.



T-Mobile Site Inventory and Power Data

Sector:	A	Sector:	В	Sector:	С
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	RFS APXV18- 209014-C	Make / Model:	RFS APXV18- 209014-C	Make / Model:	RFS APXV18- 209014-C
Gain:	14.4 dBd	Gain:	14.4 dBd	Gain:	14.4 dBd
Height (AGL):	140	Height (AGL):	140	Height (AGL):	140
Frequency Bands	1900 MHz (PCS)	Frequency Bands	1900 MHz (PCS)	Frequency Bands	1900 MHz (PCS)
Channel Count	4	Channel Count	4	Channel Count	4
Total TX Power(W):	180	Total TX Power(W):	180	Total TX Power(W):	180
ERP (W):	3,164.26	ERP (W):	3,164.26	ERP (W):	3,164.26
Antenna A1 MPE%	0.63	Antenna B1 MPE%	0.63	Antenna C1 MPE%	0.63
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	Commscope LNX- 6515DS-VTM	Make / Model:	Commscope LNX- 6515DS-VTM	Make / Model:	Commscope LNX- 6515DS-VTM
Gain:	14.6 dBd	Gain:	14.6 dBd	Gain:	14.6 dBd
Height (AGL):	140	Height (AGL):	140	Height (AGL):	140
Frequency Bands	700 MHz	Frequency Bands	700 MHz	Frequency Bands	700 MHz
Channel Count	1	Channel Count	1	Channel Count	1
Total TX Power(W):	30	Total TX Power(W):	30	Total TX Power(W):	30
ERP (W):	668.53	ERP (W):	668.53	ERP (W):	668.53
Antenna A2 MPE%	0.29	Antenna B2 MPE%	0.29	Antenna C2 MPE%	0.29

Site Composite MPE%					
Carrier	MPE%				
T-Mobile (Per Sector Max)	0.92 %				
Sprint	0.54 %				
AT&T	6.96 %				
Verizon Wireless	3.45 %				
Site Total MPE %:	11.87 %				

T-Mobile Sector A Total:	0.92 %
T-Mobile Sector B Total:	0.92 %
T-Mobile Sector C Total:	0.92 %
Site Total:	11.87 %

T-Mobile _Max Values per sector	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm²)	Frequency (MHz)	Allowable MPE (µW/cm²)	Calculated % MPE
T-Mobile PCS - 1950 MHz LTE	2	1,054.75	140	4.22	PCS - 1950 MHz	1000	0.42%
T-Mobile PCS - 1950 MHz GSM	2	527.38	140	2.11	PCS - 1950 MHz	1000	0.21%
T-Mobile 700 MHz LTE	1	668.53	140	1.34	700 MHz	467	0.29%
						Total:	0.92%



Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general public exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general public exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector A:	0.92 %
Sector B:	0.92 %
Sector C:	0.92 %
T-Mobile Per Sector	0.92 %
Maximum:	0.92 %
Site Total:	11.87 %
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **11.87%** of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.