T · · Mobile

EM-T-MOBILE-093-081218B

December 18, 2008

ORIGINAL

Via Federal Express

S. Derek Phelps, Executive Director Connecticut Siting Council Ten Franklin Square New Britain, CT 06051



CONNECTICUT SITING COUNCIL

Re: Notice of Exempt Modification

Laydon Construction Company Telecommunications Facility

69 Wheeler Street, New Haven, Connecticut

T-Mobile Site CTNH039A

Dear Mr. Phelps:

Omnipoint Communications, a subsidiary of T-Mobile USA, Inc. ("T-Mobile"), intends to install additional antennas and additional ground equipment at the existing 98-foot self-supporting monopole facility owned by Laydon Construction Company and located at 69 Wheeler Street, New Haven, Connecticut ("Facility"). T-Mobile is licensed by the Federal Communications Commission (FCC) to provide PCS wireless telecommunications service in the State of Connecticut, which includes the area to be served by the proposed installation. This installation constitutes an exempt modification pursuant to the Public Utility Environmental Standards Act, Connecticut General Statutes Section 16-50g et. seq. (PUESA), and Section 16-50j-72(b)(2) of the Regulations of the Connecticut State Agencies adopted pursuant to PUESA. In accordance with R.C.S.A. Section 16-50j-73, a copy of this notice has been sent to John DeStefano, Jr., Mayor, City of New Haven and April Capone Almon, Mayor of East Haven.

The existing Facility consists of a 98-foot self-supporting monopole tower capable of supporting multiple carriers within a fenced compound. The coordinates for the Facility are approximately Lat: 41°-17'-45" and Long: 72°-53-52". The tower is located in the eastern portion of New Haven, roughly 100 feet west of Wheeler Street and roughly 350 feet east of Interstate 95 (see Site Map, attached as Exhibit A). The tower currently supports AT&T antennas at the sixty eight foot (68') level centerline AGL (above ground level), Nextel antennas at the seventy eight foot level (78') AGL, and AT&T antennas at the eighty eight foot level (88') AGL. T-Mobile currently has antennas on the tower at the ninety eight foot (98') level centerline AGL (above ground level) The current T-Mobile antenna configuration is one per

¹ The Siting Council database and information provided by the tower owner indicate that this facility is located in the City of New Haven. However, the facility is in close proximity to the New Haven/East Haven line. Therefore, copies of the application are being sent to the chief elected officials of both municipalities.

per sector), for a total of six antennas at their current elevation on the tower. T-Mobile proposes to install three RFS APX16PV-16PVL-C antennas on existing pipe mounts at the same elevation, (98') level centerline AGL. T-Mobile also intends to add a UMTS 3106 equipment cabinet to its current configuration of one existing S12000 equipment cabinet on an existing concrete pad. The two cabinets will both be mounted on T-Mobile's existing equipment pad contained within T-Mobile's existing lease area. T-Mobile intends to run new coaxial cable on its existing ice bridge from its current equipment pad to the existing tower. Utilities will be run via a proposed underground conduit from existing utility sources at the Facility (See Design Drawings and Equipment Specifications, attached as Exhibits B and C respectively).

For the following reasons, the proposed modifications to the Wheeler Street Facility meet the exempt modification criteria set forth in R.C.S.A. Section 16-50j-72(b)(2):

- 1. The proposed modification will not increase the height of the tower as T-Mobile seeks to replace its existing antennas and install new antennas at a center line height of approximately 98 feet.
- 2. The installation and replacement of T-Mobile's antennas and ground equipment will not require an extension of the site boundaries.
- 3. The proposed modifications will not increase the noise levels at the existing Facility by six decibels or more.
- 4. The operation of the additional antennas will not increase the total radio frequency (RF) power density, measured at the site boundary, to a level at or above the standard adopted by the Connecticut Department of Environmental Protection as set forth in Section 22a-Communications Commission. The worst-case RF power density calculations for the proposed Pocket antennas would be 62.157% of the FCC standard (see general power density calculations table, attached as Exhibit D).

Also attached, Exhibit E, is a structural assessment confirming that the tower can support the existing and proposed antennas and associated equipment. Of note, because all antennas are internally mounted in the flagpole, the additional three antennas will not increase the ice and winding loading for the tower.

For the foregoing reasons, T-Mobile respectfully submits that the proposed antenna installation and equipment at the New Haven Facility constitutes an exempt modification under R.C.S.A. Section 16-50j-72(b)(2).

Respectfully Submitted,

Mark R. Richard UMTS Project Manager

Agent for T-Mobile

DeStefano, Jr., Mayor, City of New Haven April Capone Almon, Mayor of East Haven Laydon Construction Company cc:

Carrie Larson

Exhibit A

Site Map T-Mobile Site CTNH039A 69 Wheeler Street New Haven, Connecticut



Exhibit B

Design Drawings T-Mobile Site CTNH039A 69 Wheeler Street New Haven, Connecticut

Exhibit C

Equipment Specifications
T-Mobile Site CTNH039A
69 Wheeler Street
New Haven, Connecticut



Product Description

Gathering two X-Polarised antennas in a sigle radome this pair of variable tilt antenna provides exceptional suppression of all upper sidelobes at all downtilt angles. It also features a wide downtilt range with optional remote tilt.



Features/Benefits

- Variable electrical downtilt provides enhanced precision in controlling intercell interference. The tilt is infield adjustable 0-10 deg.
- High Suppression of all Upper Sidelobes (Typically <-20dB).
- Optional remote tilt can be retrofitted.
- Two X-Polarised panels in a single radome.
- Dual polarization.
- Low profile for low visual impact.
- ·Broadband design.

Technical Features	
Frequency Band	PCS 1000 (1950 1000 11)
Horizontal Pattern	PCS 1900 (1850-1990 MHz) Directional
Antenna Type	Panel Dual Polarized
Electrical Down Tilt Option	Variable Variable
Gain, dBi (dBd) Frequency Range, MHz	17.8 (15.8) , 17.8 (15.8)
. requeries realige, MHZ	1850-1990 , 1850-1990

DEC The OL		
RFS The Clear Choice ™	APX16PV-16PVL-C	_
loggo vielt	ALATON A-TOPAL-C	Print Date: 21.03.2005
lease visit us on the internet at http://www.rf	sworld com	11.00.2003



Connector Type	(4) 7-16 DIN Female
Connector Location	Bottom
Mount Type	Downtilt
Electrical Downtilt, deg	0-10 , 0-10
Horizontal Beamwidth, deg	66,66
Mounting Hardware	APM40-2
Rated Wind Speed, km/h (mph)	160 (100)
VSWR	< 1.5:1
Vertical Beamwidth, deg	6.6
1st Upper Sidelobe Suppression, dB	> 17 (typically > 20)
Upper Sidelobe Suppression, dB	> 18 all (typically > 20)
Polarization	Dual pol +/-45°
Front-To-Back Ratio, dB	> 25
Maximum Power Input, W	300
Isolation between Ports, dB	> 30
Lightning protection	Direct Ground
3rd Order IMP @ 2 x 38 dBm, dBc	> 160
Overall Length, m (ft)	1.35 (4.42)
Dimensions - HxWxD, mm (in)	1349 x 330 x 80 (53 x 12.9 x 3.1)
Weight w/o Mtg. Hardware, kg (lb)	18.0 (39.6)
Radiating Element Material	Brass
Radome Material	Fiberglass
Reflector Material	Aluminum
Max Wind Loading Area, m² (ft²)	0.64 (6.6)
Maximum Thrust @ Rated Wind, N (lbf)	787 (177)
Shipping Weight, kg (lb)	23.8 (52)
Packing Dimensions, HxWxD, mm (in)	1550 x 420 x 210 (61 x 16.5 x 8.3)
Survival Wind Speed, km/h (mph)	200 (125)

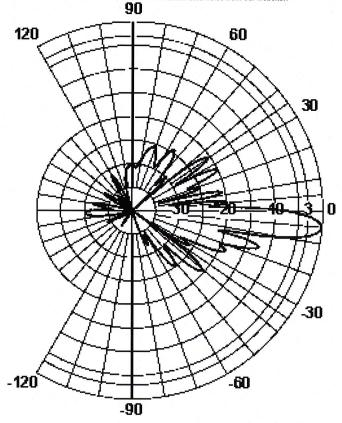
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Print Date: 21.03.2005



Vertical Pattern

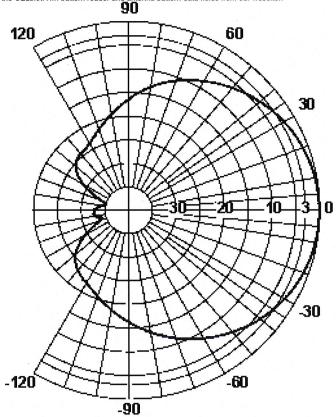
(This is a general representation of the antenna family pattern. For the latest detailed pattern contact Applications Engineering. You may also download the CELolot(TM) pattern reader and antenna pattern data fields from our website.)





Horizontal Pattern

(This is a general representation of the antenna family pattern. For the latest detailed pattern contact Applications Engineering. You may also download the CELolot(TM) pattern reader and antenna pattern data fields from our website.)



The Outdoor Cabinet

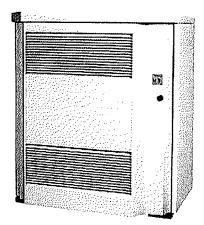


Figure 2 RBS 3106 Cabinet

- · Specified for outdoor environment
- Can be configured for both 6 and 9 radio units.
- · The cabinet fulfils seismic requirements
- · Vandal protected
- Forced convection and heat exchanger (Eco-Cooling)

The RBS 3106 cabinet has the same footprint as the GSM RBS 2106 and the WCDMA RBS 3101.

The RBS 3106 cabinet is a weatherproof outdoor cabinet for outdoor sites with primarily high capacity and high coverage requirements. The RBS 3106 houses integrated power supply with optional backup batteries, space for transmission equipment and a climate package for ensuring an indoor climate for all units inside, including the batteries.

2 Descriptions

The RBS HW is modularly structured into several subsystems for easy expansion and evolution purposes. From a physical viewpoint the subsystem are located together on shelves in the cabinet. The shelves are basically identical between the cabinets.

Nortel Networks

Univity GSM S12000 BTS Indoor & Outdoor versions

As the GSM industry moves into the world of data, pressure has increased on capacity and so network enhancement and development costs are rising. The Univity GSM S12000 BTS -Indoor and Outdoor versions - is a product that meets the needs of a mature GSM market by increasing site capacity and at the same time lowering the risks and the costs of introduction.

The Univity GSM S12000 BTS - Indoor and Outdoor versions is built on an existing field proven platform, the Univity GSM S8000 BTS, which is known for its quality and robustness. The reuse of a considerable amount of technology lowers the risk and cost for the operators when introducing this new product into their network.

The Univity GSM S12000 BTS - Indoor version is designed for protected sites while the Outdoor version is a fully integrated BTS site with AC power supply and extended temperature range.



In this document the term "the S12000 BTS" stands for "the Univity GSM S12000 BTS - Indoor and Outdoor version" except where mentioned.

The high capacity cell site

Nortel Networks addresses the growing needs of GSM capacity by introducing the S12000 BTS, which is an innovative development of the S8000 BTS. This innovative approach to network expansion and development is aimed at providing high capacity sites installed with low risk, reduced network impact and a lower cost of ownership.

The S12000 BTS is a key component to the delivery of more capacity within a GSM/GPRS network while driving down network costs. The S12000 BTS offers nearly double the capacity of the S8000 BTS, thereby offering a more compact site and improved operational efficiency.

Finally the S12000 BTS supports more users and offers higher speed data access and quality then increasing opportunities of revenues.

Lowering the cost of ownership and network introduction

It is not just the introduction of the evolution of a field proven and reliable technology that reduces the cost of ownership but also the reduced spares holding and training requirements. By the design of the S12000 BTS, Nortel Networks has aimed to reduce the cost of introducing the S12000 BTS into a GSM network. The S12000 BTS brings considerable savings in CAPEX and OPEX to the operator since main modules and skills are usable within both the S8000 BTS and S12000 BTS. The operator does not have to change the network Engineering and

Operational procedures on the existing S8000 BTS. Moreover, via the high capacity and the high RF performance of the S12000 BTS, fewer sites are required. Low introduction costs are invaluable when facing the financial pressures of network enhancements such as GPRS or new services such as UMTS. The use of the S12000 BTS puts the operator in a position to make efficient use of all resources and reduce network complexity relieving pressure on investment.



Modular and flexible

The S12000 BTS supports twelve TRX per cabinet and offers cost effective configurations from 1 to 16 TRX per cell in a tri-sector configuration. A dual band configuration of 6 + 6 TRX can be supported in a single cabinet for all coupling configurations.

The modular design of the S12000 BTS and the possibility to choose between multiple RF-combining options allows the operator to deploy the S12000 BTS solution in a number of different scenarios such as high-capacity solution in cities or alternatively enabling to provide wide coverage with a minimum number of sites in rural area.

High Performance

The Nortel Networks family of BTS holds a high market position for reliability, operability and service quality. The \$12000 BTS provides high data services and voice quality, high coverage and building penetration and smooth call handovers. It possesses many advanced RF features to improve spectral usage and optimisation and so increase available capacity. The AMR and EDGE solutions will further enhance spectrum efficiency. These high performance qualities are extremely important with the introduction of GPRS services.

In addition, as for the S8000 BTS, the S12000 BTS supports UMTS co-sitting thanks to specific combiners, allowing a smooth UMTS introduction.

Technical Specifications: Indoor Outdoor Frequency range 900 MHz GSM / 900 MHZ Extended GSM 1800 MHz GSM and Dual Band GSM 900 / 1800 850 MHZ GSM 1900 MHz GSM and Dual Band GSM 850 / 1900 Receive sensitivity w/o diversity -110 dBm garanteed (w/o TMA) with diversity -115 dBm garanteed (w/o TMA) **Dimensions** Height 1950 mm 1910 mm Width 910 mm 1350 mm Depth 450 mm 650 mm Weight 170 kg Empty cabinet 200 kg Fully equipped 415 kg 570 kg Capacity Standard 12 TRX per radio cabinet Up to 3 radio cabinets Future option Up to 4 radio cabinets Configuration Monoband Trisectorial Up to S16-16-16 (4 radio cabinets) **Dual Band Trisectorial** S222_222 (1 radio cabinet) Mono-BCCH dual band cells Cell Splitting Cell splitting across radio cabinets Amplifier output power Standard 30W (+/- 0.5 dB) GMSK 30W (+/- 0.5 dB) 8-PSK EDGE Optional 60W (+/- 0.5 dB) GMSK* 45W (+/- 0.5 dB) 8-PSK Transmission coupling All coupling configurations From Duplexers to 4 Ways Hybrid Coupling (H4D) Power control Static 6 steps of 2 dB Dynamic 15 steps of 2 dB Space for customer Equipment NA 6U Frequency Hopping RF Synthetised Supported vocoders Full Rate (FR) Enhanced Full Rate (EFR) Adaptive Multi-Rate - Full Rate (AMR FR) Adaptive Multi-Rate - Half Rate (AMR HR) Encryption algorithms A5/1 & A5/2 Power supply Nominal DC -48 V | Single, single-split or tri-phase 230V (50/60Hz) AC Integrated battery backup Optional ancillary battery cabinet Operational temperature range -5°C to +45°C -40°C to +50°C Max acoustic noise 65 dB(A) Backhaul Standard 6 E1 / T1 links Future option 8 E1 / T1 links

* Frequency dependant

The high radio performance and advanced digital processing of the S12000 BTS provide one of the highest receive sensitivity in the market today, offering –115 dBm guaranteed and without the need for masthead amplifiers (-117dBm typical). The high radio performance enhances the resistance to interference, improving voice quality, data throughput, cell coverage and service availability.

Nortel Networks experience in frequency hopping, fractional re-use, cell tiering and multi-layer management algorithms provide high spectrum efficiency which releases more capacity for a fixed allocation of spectrum.

Growing the business and ensuring success

The Univity GSM S12000 BTS is future ready. The high capacity and flexibility of the S12000 BTS, the introduction of AMR and EDGE, put the operator in a best position to meet the challenges and opportunities of GSM/GPRS. These advantages enable the operator to capture new revenues, improve profitability and gain a better return on investment as the network develops and moves forward.

In North america, the Caribbean, and Latin America Tel: 1-800-4-Nortel or 1-506-674-5470

In Europe, Middle East, and Africa: Tel:00-800-8008-9009' or+44 (0)20 8920 4618

In Asia : Tel : 65-287-2877

for more information contact your Nortel Networks account representative, or visit: www.nortelnetworks.com/contact 'callare not romall Furgean Countries

www.nortelnetworks.com

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NN101082-0702



Exhibit D

Power Density Calculations T-Mobile Site CTNH039A 69 Wheeler Street New Haven, Connecticut



T-Mobile USA Inc.

35 Griffin Rd South, Bloomfield, CT 06002-1853

Phone: (860) 692-7100 Fax: (860) 692-7159

Technical Memo

To: Maxton

From: Farid Marbouh - Radio Frequency Engineer

cc: Jason Overbey

Subject: Power Density Report for CTNH039A

Date: December 9, 2008

1. Introduction:

This report is the result of an Electromagnetic Field Intensities (EMF - Power Densities) study for the T-Mobile PCS antenna installation on a Monopole at 69 Wheeler Street, New Haven, CT. This study incorporates the most conservative consideration for determining the practical combined worst case power density levels that would be theoretically encountered from locations surrounding the transmitting location.

2. Discussion:

The following assumptions were used in the calculations:

- 1) The emissions from T-Mobile transmitters are in the (1935-1944.8), (1980.2-1984.8), (2140-2145), (2110-2120)MHz frequency Band.
- 2) The antenna array consists of three sectors, with 3 antennas per sector.
- 3) The model number for GSM antenna is APX16PV-16PVL.
- 3) The model number for UMTS antenna is APX16DWV-16DWV.
- 4) GSM antenna center line height is 98 ft.
- 4) UMTS antenna center line height is 98 ft.
- 5) The maximum transmit power from any GSM sector is 2157.35 Watts Effective Radiated Power (EiRP) assuming 8 channels per sector.
- 5) The maximum transmit power from any UMTS sector is 2525.17 Watts Effective Radiated Power (EiRP) assuming 2 channels per sector.
- 6) All the antennas are simultaneously transmitting and receiving, 24 hours a day.
- 7) Power levels emitting from the antennas are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 8) The average ground level of the studied area does not change significantly with respect to the transmitting location

Equations given in "FCC OET Bulletin 65, Edition 97-01" were then used with the above information to perform the calculations.

3. Conclusion:

Based on the above worst case assumptions, the power density calculation from the T-Mobile PCS antenna installation on a Monopole at 69 Wheeler Street, New Haven, CT, is 0.12137 mW/cm^2. This value represents 12.137% of the Maximum Permissible Exposure (MPE) standard of 1 milliwatt per square centimeter (mW/cm^2) set forth in the FCC/ANSI/IEEE C95.1-1991. Furthermore, the proposed antenna location for T-Mobile will not interfere with existing public safety communications, AM or FM radio broadcasts, TV, Police Communications, HAM Radio communications or any other signals in the area. The combined Power Density from other carriers is 50.02%. The combined Power Density for the site is 62.157% of the M.P.E. standard.

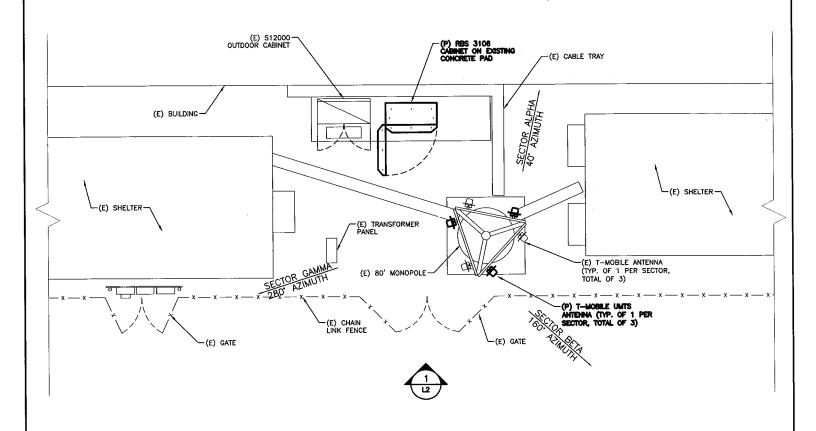
Connecticut Market T · · Mobile · **Worst Case Power Density** CTNH039A Site Address: **69 Wheeler Street** Town: **New Haven Tower Height:** 90 ft. **Tower Style:** Monopole **GSM Data UMTS Data Base Station TX output** 20 W **Base Station TX output** 40 W **Number of channels** 8 Number of channels Antenna Model APX16PV-16PVL Antenna Model APX16DWV-16DWV Cable Size 1 1/4 Cable Size 1 1/4 ~ Cable Length 130 ft. Cable Length 130 ft. Antenna Height 98.0 ft. Antenna Height 98.0 ft. **Ground Reflection** 1.6 **Ground Reflection** 1.6 Frequency 1945.0 MHz Frequency 2.1 GHz **Jumper & Connector loss** 4.50 dB Jumper & Connector loss 1.50 dB Antenna Gain 17.8 dBi Antenna Gain 18.0 dBi Cable Loss per foot 0.0154 dB Cable Loss per foot 0.0116 dB **Total Cable Loss** 2.0020 dB **Total Cable Loss** 1.5080 dB **Total Attenuation** 6.5020 dB **Total Attenuation** 3.0080 dB Total EIRP per Channel 54.31 dBm Total EIRP per Channel 61.01 dBm (In Watts) 269.67 W (In Watts) 1262.58 W Total EIRP per Sector 63.34 dBm Total EIRP per Sector 64.02 dBm (In Watts) 2157.35 W (In Watts) 2525.17 W nsg 11.2980 14.9920 Power Density (S) = 0.055920 mW/cm^2 Power Density (S) = 0.065454 mW/cm^2 T-Mobile Worst Case % MPE = 12.1373% Equation Used : (1000)(grf)2(Power)*10 (nsg10) $4\pi (R)^2$ Office of Engineering and Technology (OET) Bulletin 65, Edition 97-01, August 1997

Co-Location Total		
Carrier	% of Standard	
Verizon		
Cingular		
Sprint		
AT&T Wireless	5.8700 %	
Nextel	23.7700 %	
Other Antenna Systems	20.3800 %	
Total Excluding T-Mobile	50.0200 %	
T-Mobile	12.1373	
Total % MPE for Site	62.1573%	

Exhibit E

Structural Analysis T-Mobile Site CTNH039A 69 Wheeler Street New Haven, Connecticut





COMPOUND LAYOUT PLAN SCALE: NTS

T - Mobile
15 COMMERCE WAY
NORTON, MA 02766



50 Eastman St. South Easton, MA 02375 Phone: (508) 936–6393 Fax: (508) 936–6395

PROJECT LOCATION: NH039 / LAYDON CONSTRUCTION	PROJECT MANAGE KB	r: drawn by: DM	BSDA PROJ. #: 2889.124	
CTNH039A 69 WHEELER STREET.		COMPOUND	SHEET:	
NEW HAVEN, CT 06512	12/10/08 11/18/08	LAYOUT		
APPROVED BY:	11/10/08	PLAN	🗕 !	



December 15, 2008

Mr. Hans Fiedler UMTS Development Project Manager TMobile, USA 35 Griffin Road South Bloomfield, CT 06002

Ref:

TMobile Site ID CTNH039A

69 Wheeler Street, New Haven, CT

Dear Mr. Fiedler:

As requested a structural analysis was performed for the replacement of existing panel type antennas with a total of six new APX16 panel antennas at elevation 98' on the subject 98' high monopole structure. The analysis was conducted in accordance with Tia/EIA-222F and included an 85 mph wind speed and ½ radial ice loadings. Tower geometry and appurtenance loadings were obtained from a previous structural analysis conducted by All Points Technology Corporation, PC, dated March 10, 2006. The geometry of the monopole and existing appurtenances are assumed to be correct and in good condition. If the existing conditions differ from what was assumed in the analysis, contact the undersigned immediately.

Proposed Appurtenance Configuration (Bolt indicates new equipment):

- Three APX16PV-16PVL (one per sector) at elevation 98'
- Three APX16DWV-16DWVS-A20 (one per sector) at elevation 98'
- Nine DUO1417 panel antennas (three per sector) at elevation 90'
- One 2"x8' Omni antenna at elevation 90'
- Twelve DB844G45ZAXY panel antennas (four per sector) at elevation 80'
- Six RR90-17-02DPL2 panel antennas (two per sector) at elevation 70'
- Three ALP 7262 panel antennas (one per sector) at elevation 70'

The analysis shows that the existing foundation, anchor bolts and base plates are adequate to support the above load configuration in accordance with the TIA/EIA-222F specifications. Additionally, the main pole shaft has a calculated overstress of approximately 2% which is considered to be acceptable. The peak stress ratios were found to be as follows:

Component	Ratio	
Pole	1.02% OK	
Base Plate	81% OK	
Anchor Bolts	85.6% OK	

The proposed antenna configuration is therefore structurally acceptable.

Sincerely yours,

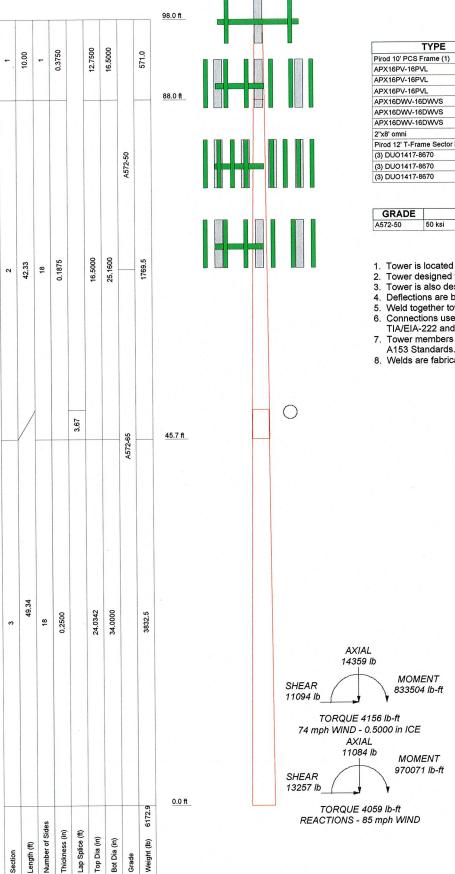
Bay State Design, Inc.

Gordon Govalet, P.E.

Vice President

Bay State Design, Inc.





DESIGNED APPURTENANCE LOADING

TYPE	TYPE ELEVATION		ELEVATION	
Pirod 10' PCS Frame (1)	98	Pirod 12' T-Frame Sector Mount (1)	80	
APX16PV-16PVL	98	(4) DB844G45ZAXY	80	
APX16PV-16PVL	98	(4) DB844G45ZAXY	80	
APX16PV-16PVL	98	(4) DB844G45ZAXY	80	
APX16DWV-16DWVS	98	Pirod 12' T-Frame Sector Mount (1)	70	
APX16DWV-16DWVS	98	(2) RR90-17-DP	70	
APX16DWV-16DWVS	98	7262.xx	70	
2"x8' omni	90	7262.xx	70	
Pirod 12' T-Frame Sector Mount (1)	90	(2) RR90-17-DP	70	
(3) DUO1417-8670	90	(2) RR90-17-DP	70	
(3) DUO1417-8670	90	7262.xx	70	
(3) DUO1417-8670	90			

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A572-65	65 ksi	80 ksi

TOWER DESIGN NOTES

- 1. Tower is located in New Haven County, Connecticut.
- 2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
- 3. Tower is also designed for a 74 mph basic wind with 0.50 in ice.
- 4. Deflections are based upon a 60 mph wind.
- 5. Weld together tower sections have flange connections.
- Connections use galvanized A325 bolts, nuts and locking devices. Installation per TIA/EIA-222 and AISC Specifications.
- 7. Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A133 Standards
- 8. Welds are fabricated with ER-70S-6 electrodes.

Spartan Engineering, LLC

50 Spruce Street
Framingham, MA 01701
Phone: (508) 532-0876
FAX:

Project: CTNH039A

Client: TMobile
Drawn by: Frank Lagodimos
Drawn by: Frank Lagodimos
Power TlA/EIA-222-F
Date: 12/15/08
Scale: NTS
Dwg No. E-1

Spartan Engineering, LLC 50 Spruce Street

Framingham, MA 01701 Phone: (508) 532-0876 FAX:

Job	69 Wheeler Street, NewHaven, CT	Page 1 of 5
Project	CTNH039A	Date 15:02:30 12/15/08
Client	TMobile	Designed by Frank Lagodimos

Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in New Haven County, Connecticut.

Basic wind speed of 85 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 74 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

Weld together tower sections have flange connections..

Connections use galvanized A325 bolts, nuts and locking devices. Installation per TIA/EIA-222 and AISC

Specifications..

Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards..

Welds are fabricated with ER-70S-6 electrodes..

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals

Use Moment Magnification
Use Code Stress Ratios

- ✓ Use Code Stress Ratios
 ✓ Use Code Safety Factors Guys Escalate Ice Always Use Max Kz
- Use Special Wind Profile

 √ Include Bolts In Member Capacity

 √ Leg Bolts Are At Top Of Section
- √ Secondary Horizontal Braces Leg
 Use Diamond Inner Bracing (4 Sided)
 Add IBC .6D+W Combination

Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
 √ Use Clear Spans For Wind Area
- √ Use Clear Spans For KL/r
- √ Retension Guys To Initial Tension Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt.
 √ Autocalc Torque Arm Areas
- SR Members Have Cut Ends Sort Capacity Reports By Component
- √ Triangulate Diamond Inner Bracing

Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules

- √ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA
- SR Leg Bolts Resist Compression

 √ All Leg Panels Have Same Allowable

 Offset Girt At Foundation

Consider Feedline Torque Include Angle Block Shear Check

Poles

Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets

Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
	JI	10.00	0.00			16.5000	0.3750		A572-50
LI	98.00-88.00	10.00	0.00	Round	12.7500	16.3000	0.3730		(50 ksi)
L2	88.00-45.67	42.33	3.67	18	16.5000	25.1600	0.1875	0.7500	A572-65
									(65 ksi)
L3	45.67-0.00	49.34		18	24.0342	34.0000	0.2500	1.0000	A572-65

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, -	TMobile	Frank Lagodimos

Section	Elevation	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft	Sides	in	in	in	in	
									(65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in²	I in⁴	r in	C in	I/C in³	J in⁴	It/Q in²	w in	w/t
L1	12.7500	14.5716	279.2926	4.3807	6.3750	43.8106	557.8243	7.2851	0.0000	0
2.	16.5000	18.9872	617.9068	5.7082	8.2500	74.8978	1234.1303	9.4927	0.0000	0
L2	16.7545	9.7080	326.3677	5.7909	8.3820	38.9367	653.1649	4.8549	2.5740	13.728
	25.5481	14.8618	1170.9321	8.8652	12.7813	91.6131	2343.4049	7.4323	4.0982	21.857
L3	25.5101	18.8727	1348.8043	8.4434	12.2094	110,4729	2699.3833	9.4382	3.7900	15.16
23	34.5245	26.7806	3853.9468	11.9812	17.2720	223.1326	7712.9647	13.3929	5.5440	22.176

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade A	Adjust. Factor A _f	Adjust. Factor A,	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft^2	in					in	in
L1 98.00-88.00				1	1	1		
L2 88.00-45.67				1	1	1		
L3 45.67-0.00				1	1	1		

Feed Line/Linear Appurtenances - Entered As Area

Description	Face	Allow Shield	Component	Placement	Total Number		C_AA_A	Weight
	or Leg	Shiela	Туре	ft	Tumber		ft²/ft	plf
LDF7-50A (1-5/8	C	No	Inside Pole	98.00 - 6.00	12	No Ice	0.00	0.82
FOAM)						1/2" Ice	0.00	0.82
LDF7-50A (1-5/8	C	No	Inside Pole	90.00 - 6.00	9	No Ice	0.00	0.82
FOAM)						1/2" Ice	0.00	0.82
VXL6-50 (1-1/4 FOAM)	C	No	Inside Pole	80.00 - 6.00	12	No Ice	0.00	0.50
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Ū					1/2" Ice	0.00	0.50
VXL6-50 (1-1/4 FOAM)	C	No	Inside Pole	70.00 - 6.00	15	No Ice	0.00	0.50
771E0 50 (1 1741 Offin)	Č	1.0		12 15 30 8 11 15 15 1		1/2" Ice	0.00	0.50
LDF5-50A (7/8 FOAM)	С	No	Inside Pole	90.00 - 6.00	1	No Ice	0.00	0.33
LDI 3 3011 (NOT OTHIN)		1.0				1/2" Ice	0.00	0.33

Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	A_R	A_F	C_AA_A	C_AA_A	Weight
Section	Elevation ft		ft^2	ft^2	In Face ft²	Out Face ft²	lb
L1	98.00-88.00	A	0.000	0.000	0.000	0.000	0.00
2.		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	113.82
L2	88.00-45.67	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	1131.35

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Tower Section	Tower Elevation	Face	A_R	A_F	C_AA_A In Face	C _A A _A Out Face	Weight
Section	ft		ft^2	ft²	ft²	ft^2	lb
L3	45.67-0.00	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	1231.75

Feed Line/Linear Appurtenances Section Areas - With Ice

T	T	Face	Ice	1	A_F	$C_A A_A$	$C_A A_A$	Weight
Tower Section	Tower Elevation	or	Thickness	A_R	Ap	In Face	Out Face	
	ft	Leg	in	ft^2	ft^2	ft^2	ft ²	lb
L1	98.00-88.00	A	0.500	0.000	0.000	0.000	0.000	0.00
x 7		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	113.82
L2	88.00-45.67	A	0.500	0.000	0.000	0.000	0.000	0.00
	00.00	В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	1131.35
L3	45.67-0.00	A	0.500	0.000	0.000	0.000	0.000	0.00
		В	(5)(5) \$ \$)	0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	1231.75

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C_AA_A Front	C_AA_A Side	Weight
			ft ft ft	٥	ft	X	ft²	ft²	lb
Pirod 10' PCS Frame (1)	С	None		0.0000	98.00	No Ice 1/2" Ice	9.00 13.20	9.00 13.20	250.00 350.00
Pirod 12' T-Frame Sector Mount (1)	C	From Leg	2.00 0.00 0.00	0.0000	90.00	No Ice 1/2" Ice	13.60 18.40	13.60 18.40	465.00 600.00
Pirod 12' T-Frame Sector Mount (1)	С	From Leg	2.00 0.00 0.00	0.0000	80.00	No Ice 1/2" Ice	13.60 18.40	13.60 18.40	465.00 600.00
Pirod 12' T-Frame Sector Mount (1)	С	From Leg	2.00 0.00 0.00	0.0000	70.00	No Ice 1/2" Ice	13.60 18.40	13.60 18.40	465.00 600.00
(4) DB844G45ZAXY	С	From Leg	4.00 0.00 0.00	0.0000	80.00	No Ice 1/2" Ice	7.00 7.41	3.97 4.34	21.00 64.04
(4) DB844G45ZAXY	В	From Leg	4.00 0.00 0.00	0.0000	80.00	No Ice 1/2" Ice	7.00 7.41	3.97 4.34	21.00 64.04
(4) DB844G45ZAXY	Α	From Leg	4.00 0.00 0.00	0.0000	80.00	No Ice 1/2" Ice	7.00 7.41	3.97 4.34	21.00 64.04
(2) RR90-17-DP	С	From Leg	4.00 0.00 0.00	0.0000	70.00	No Ice 1/2" Ice	4.36 4.77	1.97 2.31	18.00 40.42
(2) RR90-17-DP	В	From Leg	4.00 0.00 0.00	0.0000	70.00	No Ice 1/2" Ice	4.36 4.77	1.97 2.31	18.00 40.42

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C _A A _A Front	C_AA_A Side	Weigh
			Vert ft ft ft	٥	ft		ft²	ft²	lb
(2) RR90-17-DP	A	From Leg	4.00 0.00 0.00	0.0000	70.00	No Ice 1/2" Ice	4.36 4.77	1.97 2.31	18.00 40.42
7262.xx	С	From Leg	4.00 0.00 0.00	0.0000	70.00	No Ice 1/2" Ice	2.68 3.00	1.93 2.25	12.00 28.08
7262.xx	В	From Leg	4.00 0.00 0.00	0.0000	70.00	No Ice 1/2" Ice	2.68 3.00	1.93 2.25	12.00 28.08
7262.xx	Α	From Leg	4.00 0.00 0.00	0.0000	70.00	No Ice 1/2" Ice	2.68 3.00	1.93 2.25	12.00 28.00
2"x8' omni	Α	From Leg	5.00 0.00 0.00	0.0000	90.00	No Ice 1/2" Ice	1.60 2.42	1.60 2.42	30.00 42.4
APX16PV-16PVL	C	From Leg	4.00 0.00 0.00	0.0000	98.00	No Ice 1/2" Ice	6.65 7.08	1.98 2.30	40.0 71.0
APX16PV-16PVL	В	From Leg	4.00 0.00 0.00	0.0000	98.00	No Ice 1/2" Ice	6.65 7.08	1.98 2.30	40.0 71.0
APX16PV-16PVL	A	From Leg	4.00 0.00 0.00	0.0000	98.00	No Ice 1/2" Ice	6.65 7.08	1.98 2.30	40.0 71.0
(3) DUO1417-8670	С	From Leg	4.00 0.00 0.00	0.0000	90.00	No Ice 1/2" Ice	6.53 6.94	4.20 4.57	20.3 62.4
(3) DUO1417-8670	В	From Leg	4.00 0.00 0.00	0.0000	90.00	No Ice 1/2" Ice	6.53 6.94	4.20 4.57	20.3 62.4
(3) DUO1417-8670	Α	From Leg	4.00 0.00 0.00	0.0000	90.00	No Ice 1/2" Ice	6.53 6.94	4.20 4.57	20.3 62.4
APX16DWV-16DWVS	С	From Leg	4.00 0.00 0.00	0.0000	98.00	No Ice 1/2" Ice	6.70 7.13	2.00 2.33	20.0 51.3
APX16DWV-16DWVS	В	From Leg	4.00 0.00 0.00	0.0000	98.00	No Ice 1/2" Ice	6.70 7.13	2.00 2.33	20.0 51.3
APX16DWV-16DWVS	Α	From Leg	4.00 0.00 0.00	0.0000	98.00	No Ice 1/2" Ice	6.70 7.13	2.00 2.33	20.0 51.3

Com	pression	Checks
COIII	p. 000.0.	01100110

Pole Design Data

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Section No.	Elevation	Size	L	L_u	Kl/r	F_a	A	Actual P	Allow. P_a	Ratio P
	ft		ft	ft		ksi	in ²	lb	lb	P_a
L1	98 - 88 (1)	TP16.5x12.75x0.375	10.00	98.00	206.0	3.518	18.9872	-2319.28	66804.10	0.035
L2	88 - 45.67 (2)	TP25.16x16.5x0.1875 H1-3 (1.36 CR) - 2	42.33	98.00	136.8	7.984	14.4149	-5014.81	115084.00	0.044
L3	45.67 - 0 (3)	TP34x24.0342x0.25 H1-3 (1.36 CR) - 3	49.34	98.00	98.2	15.500	26.7806	-11063.40	415108.00	0.027

Pole Bending Design Data

Section No.	Elevation	Size	Actual M_x	Actual f_{bx}	Allow. F_{bx}	Ratio f_{bx}	Actual M_y	$Actual f_{by}$	Allow. F_{by}	Ratio f_{by}
	ft		lb-ft	ksi	ksi	$\overline{F_{bx}}$	lb-ft	ksi	ksi	F_{by}
L1	98 - 88 (1)	TP16.5x12.75x0.375	20749.6 7	-3.324	33.000	0.101	0.00	0.000	33.000	0.000
L2	88 - 45.67 (2)	TP25.16x16.5x0.1875	367717. 50	-51.210	39.000	1.313	0.00	0.000	39.000	0.000
L3	45.67 - 0 (3)	TP34x24.0342x0.25	970075. 00	-52.170	39.000	1.338	0.00	0.000	39.000	0.000

Pole Interaction Design Data

Section No.	Elevation	Size	Ratio P	Ratio f_{bx}	Ratio f_{by}	Comb. Stress	Allow. Stress	Criteria
	ft		P_a	F_{bx}	F_{by}	Ratio	Ratio	
L1	98 - 88 (1)	TP16.5x12.75x0.375	0.035	0.101	0.000	0.135	1.333	Н1-3
L2	88 - 45.67 (2)	TP25.16x16.5x0.1875	0.044	1.313	0.000	1.357	1.333	H1-3 X
L3	45.67 - 0 (3)	TP34x24.0342x0.25	0.027	1.338	0.000	1.364	1.333	H1-3 🗶

Section Capacity Table

Section	Elevation	Component	Size	Critical	P	SF*P _{allow}	%	Pass
No.	ft	Туре		Element	lb	lb	Capacity	Fail
L1	98 - 88	Pole	TP16.5x12.75x0.375	1	-2319.28	89049.86	10.2	Pass
L2	88 - 45.67	Pole	TP25.16x16.5x0.1875	2	-5014.81	153406.97	101.8	Fail X
L3	45.67 - 0	Pole	TP34x24.0342x0.25	3	-11063.40	553338.94	102.4	Fail 🗶
							Summary	1.
						Pole (L3)	102.4	Fail X
						RATING =	102.4	Fail 🗶

Spartan Engineering, LLC

Structural Engineering 50 SPRUCE ST. FRAMINGHAM, MA 01701 (508) 532-0876

JOB CTHHO39A	A A A A A A A A A A A A A A A A A A A
SHEET NO.	OF
CALCULATED BY FGL	DATE 12/15/08
CHECKED BY	DATE

	(508) 532-0876	SCALE		DATE
CHECK FOR	INDAMON FOR MONOROX	E Repend	عدہ	
Maxim	un OUERTORHING Mo	MEDET = 9	170,07 A.K	
Fouriesmon	Monumenon (BA:	POMIS TEC	PREVIOUS AN	ap, deted 3/10/06)
Ms on	LOX6 PIER	1:0"		
		3.25'		
for a side a second a commence of the commence	2163'50			
RESISTALL	OVERTURALLY MOMEN	17 :		
Comporter	E WI(K)	An	n (F)	plinare (ft.)
Former	21,63 (3,25) (0,15/cd) =	222.4K	10,815'	2405 fl.k
Pier	1.5'(6)2(0.15)= 8	-114	10.815	87.6
Oven BURDON	(21.63 × 1'-6x1)(0.1)=	43.2	10.815	467.1
Axaclo	11.08		10.85	119.8
				==3,080 A.K
FACI	roa of SATETY = 30	0f0 70.07	3.18 >1	okay
	Francis (5/	heasine		

Spartan Engineering, LLC

Structural Engineering 50 SPRUCE ST. FRAMINGHAM, MA 01701 (508) 532-0876

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CALCULATED BY Flag	DATE 12/15/08
CHECKED BY	DATE

SCAL

REFERENCE: "CIECULAR BASE R'S WI LATERE ECCENTRIC LONS" by Dayin LIM ACCULATE. STRESSES IN SHEAD BOUT of BASE PLATE + OD=46" T= 600 si Tb= 0.75(60)(1.33) = 59. RS to Tb = 0.75(60)(1.33) = 59. RS to Ta = 0.75(60)(1.33) = 59. R
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
CALCULARE MAXIMUM COMPLETE BAG. STRESS = 0.35 Sic A/A_2 = 0.35 (3ksi) $\sqrt{\frac{5184}{1809}} = 1.78 ksi$ $A_1 = 2.867$ A! = 1(21) + 2(14.85) = 16.9" BOTTOM of EQ.7 = $1 + 2 - (14.85) = 2.414$
CALCULARE MAXIMUM COMPLETE BAG. STRESS = 0.35 Sic A/A_2 = 0.35 (3ksi) $\sqrt{\frac{5184}{1809}} = 1.78 ksi$ $A_1 = 2.867$ A! = 1(21) + 2(14.85) = 16.9" BOTTOM of EQ.7 = $1 + 2 - (14.85) = 2.414$
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$A = 1(21) + 2(14.85) = 16.7$ Borrow of Eq. 7 = $1 + \frac{2}{21}(14.85) = 2.414$
$A = 1(21) + 2(14.85) = 16.7$ Borrow of Eq. 7 = $1 + \frac{2}{21}(14.85) = 2.414$
$A = 1(21) + 2(14.85) = 16.7$ Borrow of Eq. 7 = $1 + \frac{2}{21}(14.85) = 2.414$
Borron of Eq.7 = 1 + 2 (14.85) = 2.414
DASED UPON IPREADSHEET ACCULATIONS
MM. BASE R THICKNESS = 1.336" & (Provider = 1.5")
BASE R 15 ADEQUATE
Maximum Anchor BOLT TENSION = 149.41 K
Augustice Anche Bour Terision = 131.2 k
$\frac{149.41}{131.2} = 1.14 + 1.33 \text{ ok}$
Auchor Ross he Ascourre

Desing of Circular Base Plates

Design Based Upon "Circular Base Plates with Large Eccentric Loads" by Dajin Liu

Axial	11.08 kips
Moment	11641 in*kips

Plate Information

OD of Pipe	34 in
OD of Plate	48 in
Fy for Plate	60 ksi
No. Bolts	6
Bolt Circle	42 in
A1/A2	2.867
f'c	3 ksi

Is Plate Stiffened? yes

Crital Section 17 inches

Calculations:

Max. Concrete Brg. Stress 1.78 ksi eccentricity 1050.632 inches

Center of Tensile Bolt Group

A' 16.9

15.5 Assumed Bearing Length of Plate (in.)

Alpha 1.209 radians

69.26 degrees

Beta 44.89 inches

Area of Compressive Segment 505 In.^2
Area Center if Gravity 6.41 inches

Iterative Calculations

Right side of Eq 2 11828.25 in*kips Left side of Eq 2 11826.02 in*kips

% Error 1.000189

Total Tension In Bolts 360.67 kips

Max. Bolt Tension Calculation

Bottom of Eq 7 2.414

Max. Bolt Tension 149.41

Calculate Moment in Base Plate

Alpha 1 0.7837 radians

44.90 degrees

Beta 1 33.88 inches

Area of Compressive Segment 163.4 in.^2

Area Center if Gravity 2.84 inches

Moment in Plate 17.80 in*kips

Minimum Req'd Thickness of Plate

tp 1.336 inches

