

November 11, 2016

Melanie Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

RE: AT&T Wireless Modifications to Telecommunication Facility –
302 Ball Pond Rd., New Fairfield, CT 06812

Dear Ms. Bachman:

Enclosed please find and original and two (2) copies of a Notice of Exempt Modification including drawings and a check in the amount of six hundred twenty five (\$625.00) for the filing fee. In addition, I have included a single copy of each notification letter mailed this day to the municipality, and the owners of both the property and tower. Copies of the RF study and structural reports (without calculations) are also included.

I will submit electronic copies of the filings including the complete structural analysis and the RF table to you via e mail today.

Please feel free to contact me with any questions or comments. Thank you for your kind cooperation in this matter.

Respectfully submitted,

Jack Andrews
Zoning Manager, Empire Telecom
o/b/o AT&T Wireless
10130 Donleigh Drive
Columbia, MD 21046
443-677-0144
jandrews@empiretelecomm.com

Enclosures

Jack Andrews
Zoning Manager, Empire Telecom
o/b/o AT&T Wireless
10130 Donleigh Drive
Columbia, MD 21046
443-677-0144
jandrews@empiretelecomm.com

November 11, 2016

Melanie Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

NOTICE OF EXEMPT MODIFICATION

302 Ball Pond Rd., New Fairfield, CT 06812

Lat: 41-27-53.2 (41.46477778)
Long. 73-29-49 (-73.49694444)

Dear Ms. Bachman:

AT&T Wireless currently maintains nine (9) antennas at the 135 foot level of an existing 175 foot tall monopole tower located at 302 Ball Pond Rd., in New Fairfield, CT. The tower is owned by Town of New Fairfield. The property is owned by Town of New Fairfield. AT&T Wireless now seeks to replace three (3) existing RRU12 + A2 Remote Radio Units with three (3) new RRUS32B2 units. These replacement RRUs will be installed at the 135 foot level of the tower and be mounted on existing pipes next to the monopole, behind the antennas.

The facility was most recently approved by the Connecticut Siting Council in EM-CING-091-160310, dated March 28, 2016. Six (6) conditions were enumerated in the Council's decision:

- 1) Any deviation from the modification as specified in the Notice and supporting materials shall render this acknowledgement invalid;
- 2) Any material changes to the modification as proposed shall require the filing of a new Notice with the Council;
- 3) Within 45 days after the completion of construction the Council shall be notified in writing that the construction has been completed;
- 4) Any nonfunctioning antenna and antenna mounting equipment on this facility owned and operated by AT&T shall be removed within 60 days after the antenna ceased to function;
- 5) The validity of the action shall expire one year from the date of the letter; and
- 6) The applicant may request an extension of time beyond the one year deadline provided that such a request is submitted to the Council not less than 60 days prior to the expiration.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies section 16-50j-73 for construction that constitutes an exempt modification pursuant to RCSA section 16-50j-72(b)(2). In accordance with RCSA section 16-50j-73, a copy of this letter and attachments is being sent to the Honorable Susan Chapman, First Selectman of the Town of New Fairfield, and another letter to the Town of New Fairfield as both the tower owner and the property owner.

The planned modifications to the facility fall squarely within those activities expressly provided for in RCSA section 50j-72(b)(2).

1. The proposed modifications will not result in an increase in height of the existing structure.
2. The proposed modifications will not require an extension of the site boundary.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that will exceed state and local limits.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, AT&T Wireless respectfully submits that the proposed modifications to the above referenced telecommunications facility constitute an exempt modification under RCSA section 16-50j-72(b)(2).

Respectfully submitted,

Jack Andrews
Zoning Manager, Empire Telecom
o/b/o AT&T Wireless
10130 Donleigh Drive
Columbia, MD 21046
443-677-0144
jandrews@empiretelecomm.com

cc: Susan Chapman, First Selectman, Town of New Fairfield
Susan Chapman, First Selectman, Town of New Fairfield as Tower Owner and Property Owner

Jack Andrews
Zoning Manager, Empire Telecom o/b/o AT&T Wireless
10130 Donleigh Drive
Columbia, MD 21046
443-677-0144

November 11, 2016

The Honorable Susan Chapman,
First Selectman, Town of New Fairfield
New Fairfield Town Hall
4 Brush Hill Road
New Fairfield, CT 06812

RE: AT&T Wireless Modifications to Telecommunication Facility –
302 Ball Pond Rd., New Fairfield, CT 06812

Dear Selectman Chapman:

In order to accommodate technological changes, implement the Uniform Mobile Telecommunications System and enhance system performance in the State of Connecticut, AT&T Wireless (“AT&T”) will be changing its equipment configuration at the above referenced telecommunications facility. AT&T Wireless currently maintains nine (9) antennas at the 135 foot level of an existing 175 foot tall monopole tower located at 302 Ball Pond Rd., in New Fairfield, CT. The tower and property are owned by Town of New Fairfield. AT&T Wireless now seeks to replace three (3) existing RRU12 + A2 Remote Radio Units with three (3) new RRUS32B2 units. These replacement RRUs will be installed at the 135 foot level of the tower and be mounted on existing pipes next to the monopole, behind the antennas.

This letter is intended to serve as the required notice to the land owner and property owner. As required by the Regulations of Connecticut State Agencies (“RCSA”) section 16-50j-73, the Connecticut Siting Council (“CSC”) has been notified of the proposed changes and will review AT&T’s proposal. Please accept this letter as notification under RCSA section 16-50j-73 of construction which constitutes an exempt modification pursuant to RCSA section 16-50j-72(b)(2).

The enclosed letter to the CSC fully describes AT&T's proposal for the above referenced site. However, if you have any questions or require any additional information concerning our plans or the CSC procedures, please contact me at 443-677-0144 or contact Melanie Bachman, Acting Executive Director of the CSC at 860-872-2935.

Respectfully submitted,

Jack Andrews
Zoning Manager, Empire Telecom
o/b/o AT&T Wireless
10130 Donleigh Drive
Columbia, MD 21046
443-677-0144
jandrews@empiretelecomm.com

Enclosures

cc: Melanie Bachman, Connecticut Siting Council



RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

AT&T Existing Facility

Site ID: CT2070

New Fairfield Ctr
302 Ball Pond Road
New Fairfield, CT 06812

October 31, 2016

EBI Project Number: 6216004902

Site Compliance Summary	
Compliance Status:	COMPLIANT
Site total MPE% of FCC general public allowable limit:	9.22 %



October 31, 2016

AT&T Mobility – New England
Attn: Cameron Syme, RF Manager
550 Cochituate Road
Suite 550 – 13&14
Framingham, MA 06040

Emissions Analysis for Site: **CT2070 – New Fairfield Ctr**

EBI Consulting was directed to analyze the proposed AT&T facility located at **302 Ball Pond Road, New Fairfield, CT**, for the purpose of determining whether the emissions from the Proposed AT&T Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limits for the 700 and 850 MHz Bands are approximately $467 \mu\text{W}/\text{cm}^2$ and $567 \mu\text{W}/\text{cm}^2$ respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 2300 MHz (WCS) bands is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed AT&T Wireless antenna facility located at **302 Ball Pond Road, New Fairfield, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since AT&T is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 LTE channels (700 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 2) 2 LTE channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 3) 2 UMTS channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 4) 2 UMTS channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 5) 2 GSM channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.



- 6) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 7) For the following calculations the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturers supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antennas used in this modeling are the **CCI HPA-65R-BUU-H6**, **CCI HPA-65R-BUU-H8** and the **Powerwave 7770** for transmission in the 700 MHz, 850 MHz and 1900 MHz (PCS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturers supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 9) The antenna mounting height centerlines of the proposed antennas are **135 feet** above ground level (AGL) for **Sector A**, **135 feet** above ground level (AGL) for **Sector B** and **135 feet** above ground level (AGL) for Sector C.
- 10) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general public threshold limits.



AT&T Site Inventory and Power Data by Antenna

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	CCI HPA-65R-BUU-H8	Make / Model:	CCI HPA-65R-BUU-H6	Make / Model:	CCI HPA-65R-BUU-H6
Gain:	13.15 / 14.95 dBd	Gain:	13.15 / 14.95 dBd	Gain:	13.15 / 14.95 dBd
Height (AGL):	135 feet	Height (AGL):	135 feet	Height (AGL):	135 feet
Frequency Bands	700 MHz / 1900 MHz (PCS)	Frequency Bands	700 MHz / 1900 MHz (PCS)	Frequency Bands	700 MHz / 1900 MHz (PCS)
Channel Count	4	Channel Count	4	Channel Count	4
Total TX Power(W):	240 Watts	Total TX Power(W):	240 Watts	Total TX Power(W):	240 Watts
ERP (W):	6,229.75	ERP (W):	5,462.56	ERP (W):	5,462.56
Antenna A1 MPE%	1.96 %	Antenna B1 MPE%	1.64 %	Antenna C1 MPE%	1.64 %
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	Powerwave 7770	Make / Model:	Powerwave 7770	Make / Model:	Powerwave 7770
Gain:	11.4 / 13.4 dBd	Gain:	11.4 / 13.4 dBd	Gain:	11.4 / 13.4 dBd
Height (AGL):	135 feet	Height (AGL):	135 feet	Height (AGL):	135 feet
Frequency Bands	850 MHz / 1900 MHz (PCS)	Frequency Bands	850 MHz / 1900 MHz (PCS)	Frequency Bands	850 MHz / 1900 MHz (PCS)
Channel Count	4	Channel Count	4	Channel Count	4
Total TX Power(W):	120 Watts	Total TX Power(W):	120 Watts	Total TX Power(W):	120 Watts
ERP (W):	2,140.89	ERP (W):	2,140.89	ERP (W):	2,140.89
Antenna A2 MPE%	0.60 %	Antenna B2 MPE%	0.60 %	Antenna C2 MPE%	0.60 %
Antenna #:	3	Antenna #:	3	Antenna #:	3
Make / Model:	Powerwave 7770	Make / Model:	Powerwave 7770	Make / Model:	Powerwave 7770
Gain:	11.4 dBd	Gain:	11.4 dBd	Gain:	11.4 dBd
Height (AGL):	135 feet	Height (AGL):	135 feet	Height (AGL):	135 feet
Frequency Bands	850 MHz	Frequency Bands	850 MHz	Frequency Bands	850 MHz
Channel Count	2	Channel Count	2	Channel Count	2
Total TX Power(W):	60 Watts	Total TX Power(W):	60 Watts	Total TX Power(W):	60 Watts
ERP (W):	828.23	ERP (W):	828.23	ERP (W):	828.23
Antenna A3 MPE%	0.32 %	Antenna B3 MPE%	0.32 %	Antenna C3 MPE%	0.32 %

Site Composite MPE%	
Carrier	MPE%
AT&T – Max per sector	2.87 %
Town Fire Dept	0.23 %
Town Police Dept	0.15 %
Town Pub Wks Dept	0.21 %
Sprint	0.67 %
Clearwire	0.12 %
T-Mobile	2.08 %
Verizon Wireless	2.89 %
Site Total MPE %:	9.22 %

AT&T Sector A Total:	2.87 %
AT&T Sector B Total:	2.56 %
AT&T Sector C Total:	2.56 %
Site Total:	9.22 %

AT&T _ Frequency Band / Technology (Max for Sector A)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ($\mu\text{W}/\text{cm}^2$)	Frequency (MHz)	Allowable MPE ($\mu\text{W}/\text{cm}^2$)	Calculated % MPE
AT&T 700 MHz LTE	2	1,239.23	135	5.35	700 MHz	467	1.15%
AT&T 1900 MHz (PCS) LTE	2	1,875.65	135	8.10	1900 MHz (PCS)	1000	0.81%
AT&T 850 MHz UMTS	2	414.12	135	1.79	850 MHz	567	0.32%
AT&T 1900 MHz (PCS) UMTS	2	656.33	135	2.84	1900 MHz (PCS)	1000	0.28%
AT&T 850 MHz GSM	2	414.12	135	1.79	850 MHz	567	0.32%
							Total*: 2.87%

*NOTE: Totals may vary by 0.01% due to summing of remainders



Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general public exposure to RF Emissions.

The anticipated maximum composite contributions from the AT&T facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general public exposure to RF Emissions are shown here:

AT&T Sector	Power Density Value (%)
Sector A:	2.87 %
Sector B:	2.56 %
Sector C:	2.56 %
AT&T Maximum Total (per sector):	2.87 %
Site Total:	9.22 %
Site Compliance Status:	COMPLIANT

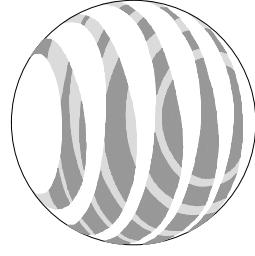
The anticipated composite MPE value for this site assuming all carriers present is **9.22 %** of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

PROJECT INFORMATION

SCOPE OF WORK	UNMANNED COMMUNICATIONS FACILITY MODIFICATIONS INCLUDING THE REPLACEMENT OF EXISTING TOP MOUNTED (3) FIBER TRUNKS 2-A2 WITH NEW (3) RRUS-32B UNITS, RE-USE EXISTING (1) FIBER TRUNK, EXISTING (2) DC TRUNKS, EXISTING (1) RAYCAP SURGE ARRESTOR AND ASSOCIATED JUMPER CABLES.
SITE NUMBER:	C12070
SITE NAME:	NEW FAIRFIELD CTR
SITE ADDRESS:	302 BALL POND ROAD NEW FAIRFIELD, CT 06812
TOWER OWNER:	TOWN OF NEW FAIRFIELD 4 BRUSH HILL ROAD NEW FAIRFIELD, CT 06812
APPLICANT:	AT&T MOBILITY 550 COCHITIATE RD SUITES 13 & 14 FRAMINGHAM, MA 01701
INC. CONTACT:	TEL: 866-915-5600
COORDINATES	LAT: N41°27'52.8" LONG: W73°29'43.0" ±825'
GROUND LEVEL:	N/A
DEED REFERENCE:	N/A
SITE PARCEL NO.:	N/A
CURRENT ZONING:	N/A
HORIZONTAL DATUM:	(NAD) 1983

at&t
Mobility



SITE NUMBER: CT2070 SITE NAME: NEW FAIRFIELD CTR PROJECT: LTE BWE Expansion

DRAWING INDEX

REV	01 TITLE SHEET	02 NOTES	03 SITE PLAN & EQUIPMENT PLAN	04 ELEVATION VIEW & ANTENNA LAYOUT	05 GROUNDDING DETAILS
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APPLICABLE BUILDING CODES AND STANDARDS

DIRECTIONS: FROM ROCKY HILL, TAKE I-91 SOUTH, TAKE EXIT 18, PROCEED WEST ON I-691, CONTINUE ON I-64 WEST. TAKE I-84 WEST EXIT 6, PROCEED NORTH ON CT RT-37 NORTH (PADAMAN RD). CONTINUE ON PEINBROKE RD (RT-37). TURN LEFT ON BUSH HILL RD (RT-39). TURN RIGHT ON BALL POND RD. SITE WILL BE ON LEFT.

SITE ACCESS: LOCKED GATE

BUILDING CODE: CONNECTICUT STATE BUILDING CODE

ELECTRICAL CODE: NATIONAL ELECTRICAL CODE LATEST EDITION

SUBCONTRACTOR'S WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING STANDARDS.

AMERICAN CONCRETE INSTITUTE (ACI) 318, BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE

AMERICAN INSTITUTE OF STEEL CONSTRUCTION (ASC), MANUAL OF STEEL CONSTRUCTION, ASD, NINTH EDITION

AMERICAN NATIONAL STANDARDS INSTITUTE/TELECOMMUNICATIONS INDUSTRY ASSOCIATION (ANSI/TIA) IEEE C62.41, RECOMMENDED PRACTICES ON SURGE VOLTAGES IN LOW VOLTAGE AC POWER CIRCUITS (FOR LOCATION CATEGORY 'C3' AND 'HIGH SYSTEM EXPOSURE')

TELECORDIA GR-1535, COAXIAL CABLE CONNECTIONS

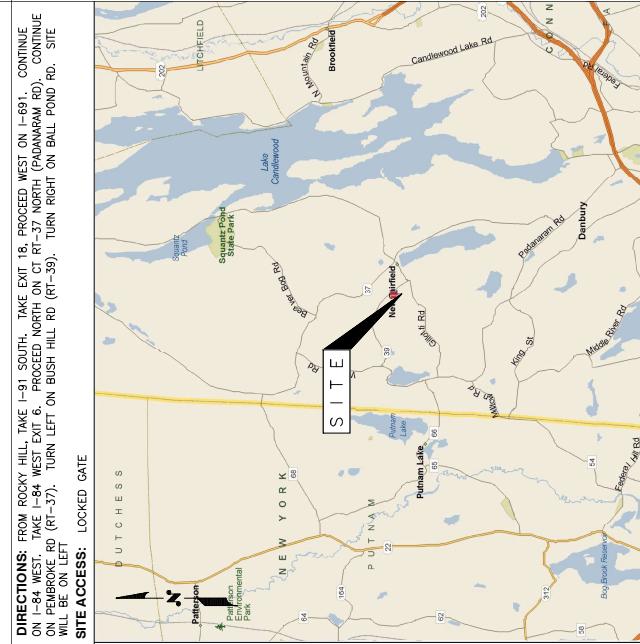
ANSI T1.311, FOR TELECOM - DC POWER SYSTEMS - TELECOM, ENVIRONMENTAL PROTECTION INSTITUTE, GROUND IMPEDANCE, AND EARTH SURFACE POTENTIALS OF A GROUND SYSTEM

IEEE 1100 (1999) RECOMMENDED PRACTICE FOR POWERING AND GROUNDING OF ELECTRONIC EQUIPMENT

IEEE 222-1995, APPENDIX B, ANTENNA SUPPORTING STRUCTURES

AT&T, 607, COMMERCIAL BUILDING GROUNDING AND BONDING REQUIREMENTS FOR TELECOMMUNICATIONS

AT LEAST 2 WORKING DAYS PRIOR TO DIGGING, THE CONTRACTOR IS REQUIRED TO CONNECT TO ONE CALL SYSTEM AT 1-800-222-4455



LOCATION MAP

CONTACT	COMPANY	PHONE NO.	PHONE NO.	GENERAL	GENERAL	GENERAL
CONTACT	VRC	(508) 981-9500	(508) 981-9500	△ 10/31/16	△ 10/29/16	△ 10/29/16
ENGINEERING:	MIGUEL NORIE	(484) 663-5349	(484) 663-5349	△ 10/29/16	△ 10/29/16	△ 10/29/16
SITE ACQUISITION:	DAVID COOPER			FOR CONSTRUCTION	FOR CONSTRUCTION	FOR CONSTRUCTION
CONSTRUCTION:	BILL DANIELS			FOR REVIEW	FOR REVIEW	FOR REVIEW
UTILITIES:	WORK REQUEST GROUP	NATIONAL GRID	(800) 375-7405	NO. DATE	NO. DATE	NO. DATE
POWER:		VERIZON	(800) 941-9900	REVISION	REVISION	REVISION
TELCO:				DESIGNED BY:	DESIGNED BY:	DESIGNED BY:
VERTICAL RESOURCES GRP.	EMPIRE telecom	489 Washington Street Auburn, WA 01501 Tel: (508) 981-9500 Fax: (508) 519 - 8539 mtnore@verticalresourcesgrp.com	SITE NUMBER: CT2070 SITE NAME: NEW FAIRFIELD CENTER 302 BALL POND ROAD NEW FAIRFIELD, CT 06812 FAIRFIELD COUNTY	DRAWN BY: M.A.	DRAWN BY: M.A.	DRAWN BY: M.A.
VRG	489 Washington Street Auburn, WA 01501 Tel: (508) 981-9500 Fax: (508) 519 - 8539 mtnore@verticalresourcesgrp.com	550 COCHITIATE RD SUITES 13 & 14 FRAMINGHAM, MA 01701	BY: CHC APPD	BY: CHC APPD	BY: CHC APPD	BY: CHC APPD
AT & T MOBILITY	at&t Mobility	550 COCHITIATE RD SUITES 13 & 14 FRAMINGHAM, MA 01701	TITLE SHEET	TITLE SHEET	TITLE SHEET	TITLE SHEET
50-145	01	50-145	01	50-145	01	50-145
REV	2	REV	2	REV	2	REV

GENERAL NOTES

1. THE CELLULAR INSTALLED IS AN UNMANAGED PRIVATE AND SECURED COMPOUND; IT IS ONLY ACQUIRED BY TRUSTED TECHNICIANS FOR PERIODIC MAINTENANCE AND SANITARY SEWER SERVICE. THE FACILITY IS NOT GOVERNED BY REGULATIONS REQUIRING PUBLIC ACCESS PER ADA REQUIREMENTS.
2. CONSTRUCTION, MAINTENANCE & OPERATION OF POWERED TOWER FACILITY WILL BE HELD IN ACCORDANCE WITH ALL APPLICABLE LOCAL STATE & FEDERAL REGULATIONS AND GUIDELINES.

THE TYPE, DIMENSIONS, MOUNTING HARDWARE, AND POSITIONS OF ALL EQUIPMENT IN THE COMPOUND ARE SHOWN IN ILLUSTRATIVE FASHION. THESE DRAWINGS ARE NOT INTENDED FOR CONSTRUCTION, ACTUAL HARDWARE DETAILS AND FINAL LOCATIONS MAY DIFFER SLIGHTLY FROM WHAT IS SHOWN.

• CONSTRUCTION, MAINTENANCE & OPERATION OF PROPOSED TOWER FACILITY WILL BE HELD IN ACCORDANCE WITH ALL APPLICABLE LOCAL, STATE & FEDERAL REGULATIONS AND GUIDELINES.

GENERAL NOTES

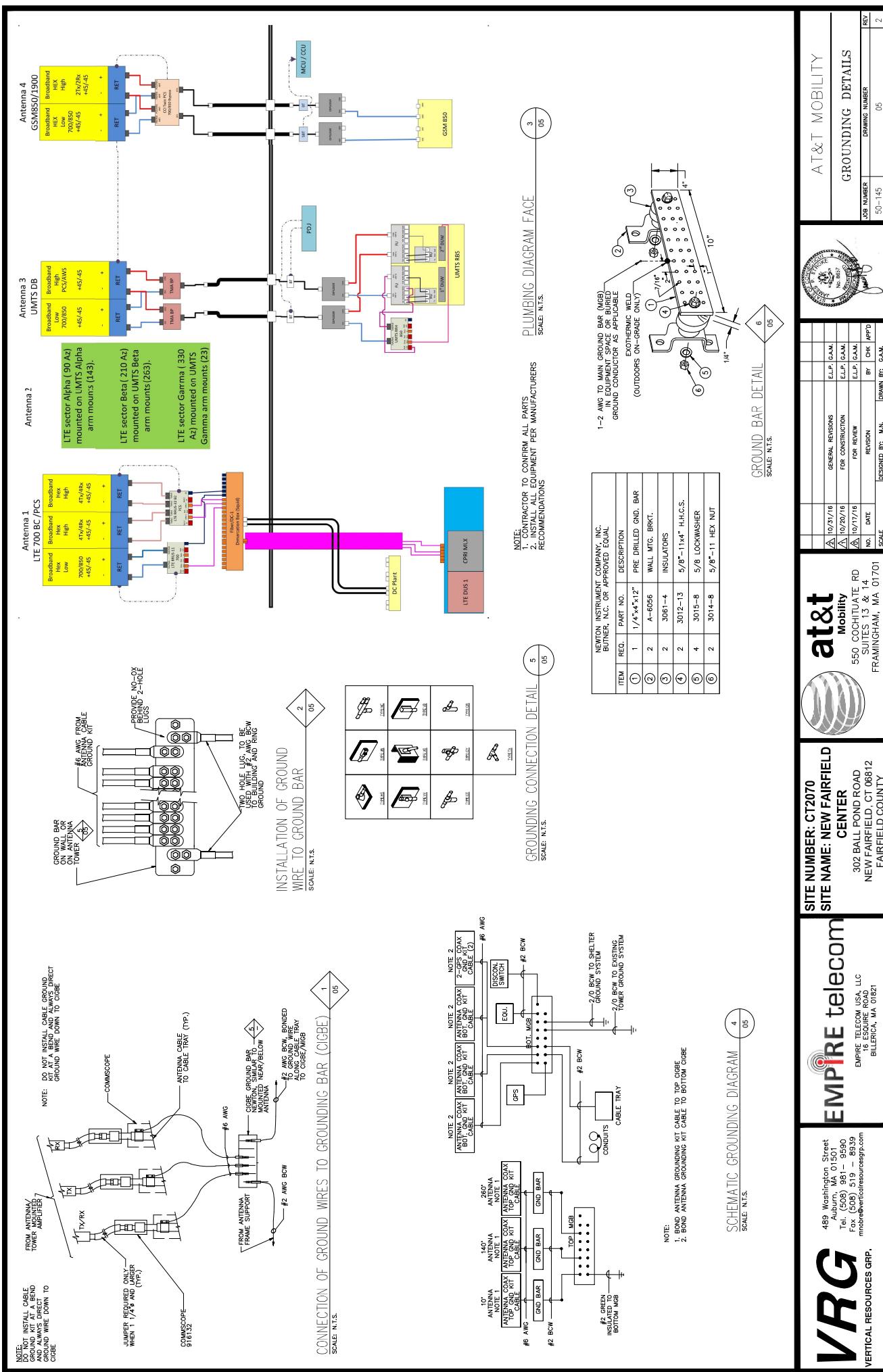
THE TYPE, DIMENSIONS, MOUNTING HARDWARE, AND THE POSITIONS OF ALL EQUIPMENT IN THE SYSTEM ARE SHOWN IN THE PLANS. THE DESIGNED SYSTEM MAY NOT BE IDENTICAL WITH THE CONSTRUCTION. ACTUAL HARDWARE DETAILS AND FINAL LOCATIONS MAY DIFFER SLIGHTLY FROM WHAT IS SHOWN.

2. THE CELLULAR INSTALLATION IS AN UNMANNED PRIVATE AND SECURED COMPOUND. IT IS ONLY ACCESSIBLE BY TRAINED TECHNICIANS FOR PERIODIC MAINTENANCE. NO RESIDENTS ARE ALLOWED TO ACCESSE ANY WATER OR SANITARY SEWER SERVICE. THE FACILITY IS NOT GOVERNED BY REGULATIONS REQUIRING PUBLIC ACCESS PER ADA REQUIREMENTS.

3. CONSTRUCTION, MAINTENANCE & OPERATION OF PROPOSED TOWER FACILITY WILL BE HELD IN ACCORDANCE WITH ALL APPLICABLE LOCAL, STATE & FEDERAL REGULATIONS AND GUIDELINES.

EQUIPMENT PLAN
SCALE: 1" = 2'

VRG 489 Washington Street
Auburn, MA 01601
(508) 981-9590
Fax (508) 519-8399
mbrace@verticalresourcesgrp.com



Rigorous Structural Analysis Report



AT&T - New Fairfield Center #CT2070 / FA #10035312

**Owner: Town of New Fairfield
New Fairfield, Connecticut**

November 02, 2016

MEI PROJECT ID: CT01113M-16V0



17950 PRESTON ROAD, SUITE 720 • DALLAS, TEXAS 75252 • TEL. 972-783-2578 FAX 972-783-2583
www.maloufengineering.com





November 02, 2016

Mr. Miguel Nobre
Vertical Resources Group
Auburn, MA 01501

RIGOROUS STRUCTURAL ANALYSIS

Structure/Make/Model:	175 ft Monopole		Fredd A. Nudd / MJ-180	
Client/Site Name/#:	Vertical Resources Group / AT&T			New Fairfield Center #CT2070 / FA #10035312
Owner/Site Name/#:	Town of Fairfield			New Fairfield Center
MEI Project ID:	CT01113M-16V0			
Location:	302 Ball Pond Rd New Fairfield, CT 06812		Fairfield County FCC #N/A	
	LAT	41-27-52.98 N	LON	73-29-49.03 W

EXECUTIVE SUMMARY:

Malouf Engineering Int'l (MEI), as requested, has performed a rigorous structural analysis of the above mentioned structure to assess the impact of the changed condition as noted in Table 1.

Based on the stress analysis performed, the existing structure is in conformance with the Connecticut Building Code / Int'l Building Code (IBC) / ANSI/TIA-222-G Standard for the loading considered under the criteria listed and referenced in the report sections – tower rated at 73.7% - Pole.

The installation of the proposed changed condition as noted in Table 1 is structurally acceptable.
Please refer to Appendix 1 for Schematic Lines Layout.

MEI appreciates the opportunity of providing our continuing professional services to you. If you have any questions or need further assistance on this or other projects please contact us.

Respectfully submitted,

MALOUF ENGINEERING INT'L, INC.

Analysis performed by:

Reviewed & Approved by:

A handwritten signature in blue ink, appearing to read 'E. Mark Malouf'.



E. Mark Malouf, PE
Connecticut #17715
972-783-2578 ext. 106
mmalouf@maloufengineering.com

11/2/2016

Helder Lopez, PE
Sr. Project Engineer

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1. INTRODUCTION & SCOPE

A rigorous structural analysis was performed by Malouf Engineering Int'l (MEI), as requested and authorized by Mr. Miguel Nobre, Vertical Resources Group, on behalf of AT&T, to determine the acceptance of the proposed changed conditions in conformance with the CT BC / IBC / ANSI/TIA-222-G Standard, "Structural Standard for Antenna Supporting Structures and Antennas". The scope of this independent analysis is to determine the overall stability and the adequacy of structural members, foundations, and member connections, as available and stated. This analysis considers the structure to have been properly installed and maintained with no structural defects. Installation procedures and related loading are not within the scope of this analysis and should be performed and evaluated by a competent person of the erection contractor.

The different report sections detail the applicable information used in this evaluation, relating to the tower data, the appurtenances configuration and the wind and ice loading considered.

2. SOURCE OF DATA

The following information has been used in this evaluation as source data that accurately represent the existing structure and the related appurtenances:

	Source	Information	Reference		
STRUCTURE					
Tower	MEI Records	Previous Structural Analysis	ID CT01113M-08V0 Dated 05/09/2008		
		Fredd A. Nudd Tower Design Drawing	Dated 03/19/2003 Rev. C		
	Vertical Resources / Mr. Miguel Nobre	Infinigy Structural Analysis	Job #158-093 Dated 02/16/2016		
Foundation	MEI Records	Previous Structural Analysis	ID CT01113M-08V0 Dated 05/09/2008		
	MEI Records	Fredd A. Nudd Foundation Design Dwg	Dated 03/19/2003 Rev. C		
		No geotechnical report provided – soil parameters as per URS analysis report.			
Material Grade	Available from supplied documents noted above-refer to Appendix				
CURRENT APPURTEANCES					
	Vertical Resources / Mr. Miguel Nobre	Infinigy Structural Analysis	Job #158-093 Dated 02/16/2016		
		Tower Photos	Dated 10/15/2016		
CHANGED CONDITION					
	Vertical Resources / Mr. Miguel Nobre	E-mail Instructions	Dated 10/31/2016		
		AT&T RF Data Sheet	Dated 06/10/2016		
		AT&T CDs	Dated 10/17/2016		

Background Information:

Based on available information, the following is known regarding this structure:

DESIGNER / FABRICATOR	Fredd A. Nudd / MJ-180
ORIGINAL DESIGN CRITERIA	TIA/EIA 222-F- 85 Mph + 0.50" Ice
PRIOR STRUCTURAL MODIFICATIONS	As per Infinigy modification design Project #379-016 dated 04/24/2015 – considered properly installed.



3. ANALYSIS CRITERIA

The structural analysis performed used the following criteria:

CODE / STANDARD	2016 Connecticut Building Code / 2012 Int'l Bldg Code / ANSI/TIA-222-G-2 Standard	
LOADING CASES	Full Wind:	115 Mph ultimate gust [equiv. 90 Mph (3-sec gust)] w/No Radial Ice**
	Iced Case:	50 Mph + 3/4" Radial Ice
	Service:	60 Mph
STRUCTURE CRITERIA	Structure Classification: Class II	
	Exposure Category: 'C' – Topographic Category: 1	

Appurtenances Configuration

The following appurtenances configuration is denoted by the summation of Tables 1 & 2:

Table 1: Tenant with Changed Condition Appurtenances Configuration

Elev (ft)	Tenant	Ants Qty	Appurtenance Model / Description	Mount Description	Lines Qty	Line size & Location	
135	AT&T	3	RRUS-32 B2 Boxes	[Onto Close Contact Mount]			
Current Appurtenances To Remain							
135	AT&T	6	7770.00 Panel Antennas	(3) LP T-Arm Mounts	12	1-5/8" 5/8" DC Power 3/8" Fiber-(I)	
		1	HPA-65R-BUU-H8 Panel Antenna				
		2	HPA-65R-BUU-H6 Panel Antennas				
		6	LGP21401 TMAs				
		3	DTMABP7819VG12A Twin TMAs				
		3	RRUS-11 Boxes		Close Contact Mount		
		1	Raycap DC6-48-60-18-8F Suppressor				
Current Appurtenances To Be Removed							
135	AT&T	3	RRUS-12 w/ A2 Backpack				

Table 2: Remaining Tenants Current and Reserved/Future Appurtenances

Elev (ft)	Tenant	Ants Qty	Appurtenance Model / Description	Mount Description	Lines Qty	Line size & Location		
175	Town	1	Lightning Rod	LP Platform Mount (w/ Empty Pipes)				
		4	PD220 Omni Antennas		4	1-5/8" -(I)		
		1	1ft Square Panel Antenna		1	1/2" -(I)		
		1	2ft Dia. Dish w/ Radome		1	1/2" -(I)		
155	Sprint	2	RR65-18-02DP Panel Antennas	LP Platform Mount	4	Hybrid Cables-(I)		
		1	RR45-19-02DPL4 Panel Antenna					
		1	APXVSPP18-C-A20 Panel Antenna					
		2	P40-16-XLPP-RR Panel Antennas					
		3	APXVTM14-C-120 Panel Antennas	Close Contact Mount				
		3	ALU-RRH 8x20 Boxes					
		3	1900 Mhz – RRH Boxes					
		3	800 Mhz – RRH Boxes					
145	T-Mobile	3	LNX-6515DS-VTM Panel Antennas	(3) LP T-Arm Mounts	18	1-5/8" HiCap Hybrid Cable-(I)		
		3	AIR21 B2A B4P Panel Antennas					
		3	RRUS-11 Boxes					
		3	AIR21 B4A B2P Panel Antennas					
		3	TMAs					
125	VzW	6	BXA-171085-12CF-EDIN-X Panel Ants.	(3) LP T-Arm Mounts	18	1-5/8" 1-1/4" (6x12) Hybrid (HFT1206- 24S49-xxx) or Equiv. -(I)		
		6	LPA-80080-6CF-x Panel Antennas					
		3	BXA-70063-6CF Panel Antennas					
		3	ALU-RRH 2x60 Boxes					
		1	RxxDC-3315-PF-48 OVP Box					
100	Town	1	PD220 Omni Antenna	Standoff Arm w/ Collar Mount	1	1-5/8" -(I)		
85	Sprint	3	GPS Antenna	Chain Collar Mount	3	1/2"- (I)		



Notes:

1. **As per 2012 IBC for ultimate 3-sec gust wind speed converted to nominal 3-sec gust wind speed as per Sect. 1609.3.1 as required to be used in ANSI/TIA-222-G Standard per exception 5 of Sect. 1609.1.1.
2. All elevations are measured from tower base.
3. Please note appurtenances not listed above are to be removed/not present as per data supplied.
4. (I) = Internal; (E) = External; (FZ) = Within Face Zone; (OFZ) = Outside Face Zone - as per TIA-222-G.
5. The above appurtenances represent MEI's understanding of the appurtenances configuration. If different than above, the analysis is invalid. Please contact MEI if any discrepancies are found.



4. ANALYSIS PROCEDURE

The subject structure is analyzed for feasibility of the installation of the proposed changed condition previously noted. The data records furnished were reviewed and a computer stress analysis was performed in accordance with the TIA-222 Standard provisions and with the agreed scope of work terms and the results of this analysis are reported.

Analysis Program

The computer program used to model the structure is a rigorous Finite Element Analysis program, tnxTower (ver. 7.07), a commercially available program by Tower Numerics Inc. The latticed structures members are modeled using beam/truss and cable members and the pole members using tubular beam elements. The structural parameters and geometry of the members are included in the model. The dead and temperature loads and the wind loads are internally calculated by the program for the different wind directions and then applied as external loads on the structure. Any applicable exemptions, as per Section 15.6 of the TIA-222-G Standard for existing structures originally designed in accordance with a previous revision of the TIA-222 Standard, have been taken.

Assumptions

This engineering study is based on the theoretical capacity of the members and is not a condition assessment of the structure. This analysis is based on information supplied, and therefore, its results are based on and as accurate as that supplied data. MEI has made no independent determination, nor is it required to, of its accuracy. The following assumptions were made for this structural stress analysis:

- This existing tower is assumed, for the purpose of this analysis, to have been properly maintained and to be in good condition with no structural defects and with no deterioration to its member capacities ('as-new' condition).
- The tower member sizes and configuration are considered accurate as supplied. The material grade is as per data supplied and/or as assumed and as stated.
- The appurtenances configuration is as supplied and/or as stated in the report. It is assumed to be complete and accurate. All antennas, mounts, coax and waveguides are assumed to be properly installed and supported as per manufacturer requirements.
- Some assumptions are made regarding antennas and mounts sizes and their projected areas based on best interpretation of data supplied and of best knowledge of antenna type & industry practice.
- Mounts/Platforms are considered adequate to support the loading. No actual analysis of the platform/mount itself is performed, with the analysis being limited to analyzing the structure.
- The soil parameters are as per data supplied or as assumed and stated in the calculations. Refer to the Appendix. If no data is available, the foundation system is assumed to support the structure with its new reactions.
- All welds and connections are assumed to develop at least the member capacity, unless determined otherwise and explicitly stated in this report.
- All prior structural modifications, if any, are assumed to be as per data supplied/available, and to have been properly installed and to be fully effective.

If any of the above assumptions are not valid or have been made in error, this analysis results may be invalidated. MEI should be contacted to review any contradictory information to determine its effect.

5. ANALYSIS RESULTS

The results of the structural stress analysis based on data available and with the previous listed criteria, indicated the following:

Table 3: Stress Analysis Results

Component Type	Maximum Stress Ratio	Controlling Elev. (ft) / Component	Pass/Fail	Comment
POLE	73.7%	41 - 0	Pass	
REINFORCING	68.9%	5 - 0	Pass	
BASE PLATE	71.5%	Bending	Pass	For effective Moment After reinforcement
ANCHOR RODS	52.8%	Combined	Pass	
FOUNDATION	56.3%	Overturning Moment	Pass	No geotechnical report provided – soil parameters as per URS 02/16/2004 analysis report.

Table 4: Serviceability Requirements

	Maximum Value	TIA Requirement (10dB)	Pass/Fail	Comment
TWIST/SWAY	1.1838 Deg.	1.8568 Deg.	Pass	2' Dia. Dish w/ Radome Elev. 175.00ft
	1.1838 Deg.	4 Deg. from Vert. or Horiz. Axis	Pass	
HORIZONTAL DISPLACEMENT	25.028 In./ 1.19% of Ht.	3.0% of Height	Pass	

Notes:

1. The Maximum Stress Ratio is the percentage that the maximum load in the member is relative to the allowable load as determined by Code requirements.
2. Refer to the Appendix 1 for more details on the member loads.
3. A maximum stress ratio between 100% and 105% may be considered as Acceptable according to industry standard practice.



6. FINDINGS & RECOMMENDATIONS

- Based on the rigorous stress analysis results, the subject structure is **rated at 73.7%** of its support capacity (controlling component: Pole) with the proposed changed condition considered. Please refer to Table 3 and to Appendix 1 for more details of the analysis results.
- Based on the stress analysis performed, the existing structure **is in conformance** with the IBC / ANSI/TIA **222-G** Standard for the loading considered under the criteria listed and referenced in the report sections.
- Please note that no geotechnical data is available. However based on soil parameters obtained from URS analysis report dated 02/16/2004 and comparison of new reactions with the design reactions, the foundation is considered acceptable.
- ***The installation of the proposed changed condition as noted in Table 1 is structurally acceptable.*** Please refer to Appendix 1 for Schematic Lines Layout.
- This structure has limited additional support capacity for the appurtenances and loading criteria considered. However, no changes to the configuration considered should be made without performing a new proper evaluation.

Rigging and temporary supports required for the erection/modification shall be determined, documented, furnished and installed by the erector/contractor accounting for the loads imposed on the structure due to the proposed construction method.



7. REPORT DISCLAIMER

The engineering services rendered by **Malouf Engineering International, Inc.** ('MEI') in connection with this Structural Analysis are limited to a computer analysis of the tower structure, size and capacity of its members. MEI does not analyze the fabrication, including welding and connection capacities, except as included in this Report.

The analysis performed and the conclusions contained herein are based on the assumption that the tower has been properly installed and maintained, including, but not limited to the following:

1. Proper alignment and plumbness.
2. Correct guy tensions, as applicable.
3. Correct bolt tightness or slip jacking of sleeved connections.
4. No significant deterioration or damage to any structural component.

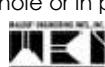
Furthermore, the information and conclusions contained in this Report were determined by application of the current "state-of-the-art" engineering and analysis procedures and formulae. MALOUF ENGINEERING INTERNATIONAL, INC. assumes no obligation to revise any of the information or conclusions contained in this Report in the event that such engineering and analysis procedures and formulae are hereafter modified or revised. In addition, under no circumstances will MALOUF ENGINEERING INTERNATIONAL, INC. have any obligation or responsibility whatsoever for or on account of consequential or incidental damages sustained by any person, firm or organization as a result of any information or conclusions contained in the Report, and the maximum liability of MALOUF ENGINEERING INTERNATIONAL, INC., if any, pursuant to this Report shall be limited to the total funds actually received by MALOUF ENGINEERING INTERNATIONAL, INC. for preparation of this Report.

Customer has requested MALOUF ENGINEERING INTERNATIONAL, INC. to prepare and submit to Customer an engineering analysis with respect to the Subject Tower and has further requested MALOUF ENGINEERING INTERNATIONAL, INC. to make appropriate recommendations regarding suggested structural modifications and changes to the Subject Tower. In making such request of MALOUF ENGINEERING INTERNATIONAL, INC., Customer has informed MALOUF ENGINEERING INTERNATIONAL, INC. that Customer will make a determination as to whether or not to implement any of the changes or modifications which may be suggested by MALOUF ENGINEERING INTERNATIONAL, INC. and that Customer will have any such changes or modifications made by riggers, erectors and other subcontractors of Customer's choice. MALOUF ENGINEERING INTERNATIONAL, INC. shall have the right to rely upon the accuracy of the information supplied by the customer and shall not be held responsible for the Customer's misrepresentation or omission of relevant fact whether intentional or otherwise.

Customer hereby agrees and acknowledges that MALOUF ENGINEERING INTERNATIONAL, INC. shall have no liability whatsoever to Customer or to others for any work or services performed by any persons other than MALOUF ENGINEERING INTERNATIONAL, INC. in connection with the implementation of services including but not limited to any services rendered for Customer or for others by riggers, erectors or other subcontractors. Customer acknowledges and agrees that any riggers, erectors or subcontractors retained or employed by Customer shall be solely responsible to Customer and to others for the quality of work performed by them and that MALOUF ENGINEERING INTERNATIONAL, INC. shall have no liability or responsibility whatsoever as a result of any negligence or breach of contract by any such rigger, erector or subcontractor and that Customer and rigger, erector, or subcontractor will provide MALOUF ENGINEERING INTERNATIONAL, INC. with a Certificate of Insurance naming MALOUF ENGINEERING INTERNATIONAL, INC. as additional insured.



APPENDIX 1 - ANALYSIS PRINTOUT & GRAPHICS



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
Lightning Rod (E)	175	(2) 7770.00 Panels w/ Pipe Mount (ATI / E)	135
PD220 (Town / E)	175	HPA-65R-BUU-H8 w/ Pipe Mounts (ATI / E)	135
PD220 (Town / E)	175	HPA-65R-BUU-H6 w/ Pipe Mounts (ATI / E)	135
PD220 (Town / E)	175	HPA-65R-BUU-H6 w/ Pipe Mounts (ATI / E)	135
1' Square Panel w/ Pipe Mount (Town / E/F)	175	HPA-65R-BUU-H6 w/ Pipe Mounts (ATI / E)	135
L.P. Platform Mount (w/ Empty Pipes) (E)	175	(2) LGP21401 TMA's (ATI / E)	135
2' Dia. Dish w/ Radome (Town / E/F)	175	(2) LGP21401 TMA's (ATI / E)	135
RR45-19-02DPL4 w/ Mount Pipe (Sprint / E)	155	(2) LGP21401 TMA's (ATI / E)	135
APXVSPP18-C-A20 w/ Pipe Mount (Sprint / E)	155	DTMABP7819VG12A Twin TMA (ATI / E)	135
(2) P40-16-XLPP-RR w/ Pipe Mount (Sprint / E)	155	DTMABP7819VG12A Twin TMA (ATI / E)	135
APXVTM14-C-120 w/ Pipe Mount (Sprint / E)	155	RRUS-11 (ATI / E)	135
APXVTM14-C-120 w/ Pipe Mount (Sprint / E)	155	RRUS-11 (ATI / E)	135
APXVTM14-C-120 w/ Pipe Mount (Sprint / E)	155	RRUS-11 (ATI / E)	135
APXVTM14-C-120 w/ Pipe Mount (Sprint / E)	155	Raycap DC6-48-60-18-8F SUPPRESSOR (ATI / E)	135
ALU-RRH 8x20 (Sprint / E)	155	RRUS-32 B2 (ATI / P)	135
ALU-RRH 8x20 (Sprint / E)	155	RRUS-32 B2 (ATI / P)	135
ALU-RRH 8x20 (Sprint / E)	155	RRUS-32 B2 (ATI / P)	135
1900 Mhz - RRH (Sprint / E)	155	Close Contact Mount (E)	135
1900 Mhz - RRH (Sprint / E)	155	L.P. T-Arm Mount (E)	135
1900 Mhz - RRH (Sprint / E)	155	L.P. T-Arm Mount (E)	135
800 Mhz - RRH (Sprint / E)	155	L.P. T-Arm Mount (E)	135
800 Mhz - RRH (Sprint / E)	155	(2) 7770.00 Panels w/ Pipe Mount (ATI / E)	135
800 Mhz - RRH (Sprint / E)	155	(2) BXA-171085-12CF-EDIN-X w/ Pipe Mount (VzW / E)	125
Close Contact Mount (E)	155	(2) BXA-171085-12CF-EDIN-X w/ Pipe Mount (VzW / E)	125
L.P. Platform Mount (E)	155	(2) LPA-80080-6CF-2 w/ Pipe Mount (VzW / E)	125
(2) RR65-18-02DP w/ Mount Pipe (Sprint / E)	155	(2) LPA-80080-6CF-2 w/ Pipe Mount (VzW / E)	125
LNX-6515DS-VTM w/ Pipe Mnt. (T-Mobile / E)	145	(2) LPA-80080-6CF-2 w/ Pipe Mount (VzW / E)	125
LNX-6515DS-VTM w/ Pipe Mnt. (T-Mobile / E)	145	(2) LPA-80080-6CF-2 w/ Pipe Mount (VzW / E)	125
AIR21 B2A B4P w/ pipe Mount (T-Mobile / E)	145	(2) LPA-80080-6CF-2 w/ Pipe Mount (VzW / E)	125
AIR21 B2A B4P w/ pipe Mount (T-Mobile / E)	145	BXA-70063-6CF w/ Pipe Mount (VzW / E)	125
AIR21 B2A B4P w/ pipe Mount (T-Mobile / E)	145	BXA-70063-6CF w/ Pipe Mount (VzW / E)	125
RRUS-11 (T-Mobile / E)	145	BXA-70063-6CF w/ Pipe Mount (VzW / E)	125
RRUS-11 (T-Mobile / E)	145	RRUS-11 (T-Mobile / E)	125
RRUS-11 (T-Mobile / E)	145	ALU-RRH 2x60 (VzW / E)	125
RRUS-11 (T-Mobile / E)	145	ALU-RRH 2x60 (VzW / E)	125
AIR21 B4A B2P w/ pipe Mount (T-Mobile / E)	145	ALU-RRH 2x60 (VzW / E)	125
AIR21 B4A B2P w/ pipe Mount (T-Mobile / E)	145	Raycap RxoDC-3315-PF-48 (VzW / E)	125
AIR21 B4A B2P w/ pipe Mount (T-Mobile / E)	145	L.P. T-Arm Mount (E)	125
AIR21 B4A B2P w/ pipe Mount (T-Mobile / E)	145	L.P. T-Arm Mount (E)	125
TMA's (T-Mobile / E)	145	(2) BXA-171085-12CF-EDIN-X w/ Pipe Mount (VzW / E)	125
TMA's (T-Mobile / E)	145	TMA's (T-Mobile / E)	125
TMA's (T-Mobile / E)	145	Standoff Arm w/ Collar Mount (E)	100
L.P. T-Arm Mount (E)	145	PD220 (Town / E)	100
L.P. T-Arm Mount (E)	145	GPS (Sprint / E/F)	85
L.P. T-Arm Mount (E)	145	GPS (Sprint / E/F)	85
LNX-6515DS-VTM w/ Pipe Mnt. (T-Mobile / E)	145	Chain Collar Mount (E)	85
(2) 7770.00 Panels w/ Pipe Mount (ATI / E)	135	GPS (Sprint / E/F)	85

ALL REACTIONS ARE FACTORED

AXIAL 104315 lb
SHEAR 14201 lb
TORQUE 3117 lb-ft
50 mph WIND - 0.7500 in IC

AXIAL 61762 lb
SHEAR 36793 lb
TORQUE 6604 lb-ft
REACTIONS - 90 mph WIN

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi	R7S - 150 ksi	120 ksi	150 ksi

TOWER DESIGN NOTES

1. Tower is located in Fairfield County, Connecticut.
2. Tower designed for Exposure C to the TIA-222-G Standard.
3. Tower designed for a 90 mph basic wind in accordance with the TIA-222-G Standard.
4. Tower is also designed for a 50 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Structure Class II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. IBC 2012 / 115 Mph (Ult.) = 90 Mph (3-Sec) as per CT Building Code
9. TOWER RATING: 73.7%



MALOUF ENGINEERING INT'L. INC.

17950 PRESTON RD. SUITE 720

DALLAS, TEXAS - 75252

Phone: (972) 783-2578

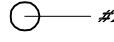
FAX: (972) 783-2583

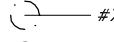
Job: 175 ft. MNP. / New Fairfield Center Site #CT2070 (FA 10035312)	Project: CT01113M-16V0	Drawn by: H Lopez	App'd:
Client: Vertical Resources Group / AT&T	Code: TIA-222-G	Date: 11/02/16	Scale: NTS
Path: C:\MEI\Projects\16\files\WNP\CT01113M-16V0\CT01113M-16V0.w-Anchor.er	Dwg No. E-1		

No.	QTY.	DESCRIPTION	ELEV.	TENANT
1	4	1 5/8	175'	Town / E
2	2	1/2	175'	Town / E
3	4	Hybrid Cable	155'	Sprint / E
4	18	1 5/8	145'	T-Mobile / E
5	1	HiCap Hybrid Cable	145'	T-Mobile / E
6	12	1 5/8	135'	AT&T / E
7	1	3/8 Fiber Cable	135'	AT&T / E
8	2	7/8" DC Power Cable	135'	AT&T / E
9	18	1 5/8	125'	VzW / E
10	1	1-1/4" (6x12) Hybrid or Equiv.	125'	VzW / E
11	1	1 5/8	100'	E
12	3	1/2	85'	Sprint / E

LEGEND:

E = EXISTING  #X

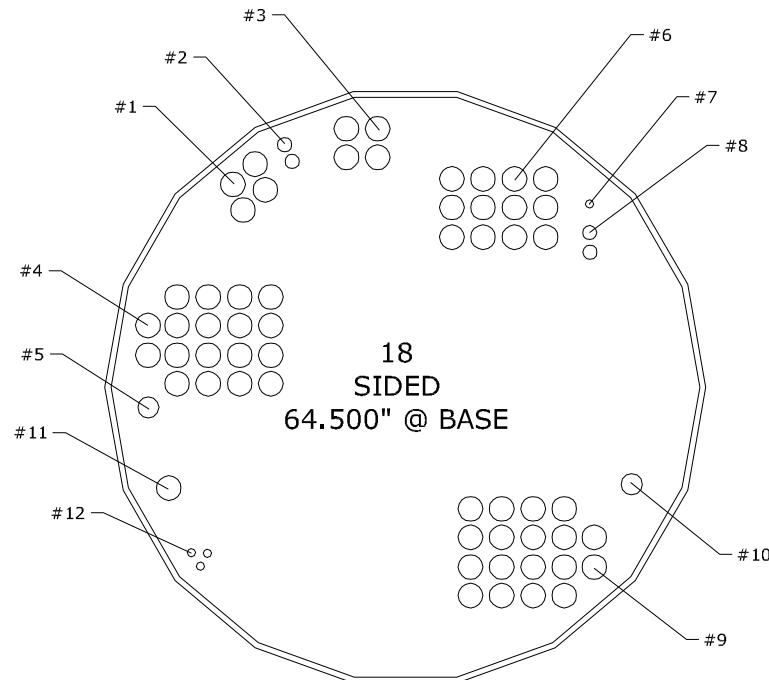
P = PROPOSED  #X

F = FUTURE  #X

R = REMOVE  #X

TO RELOCATE 

NOTE: LINE LAYOUT IS SCHEMATIC AND FOR ILLUSTRATION PURPOSES ONLY. ACTUAL LINE LOCATIONS WILL VARY WITHIN POLE SHAFT.



101

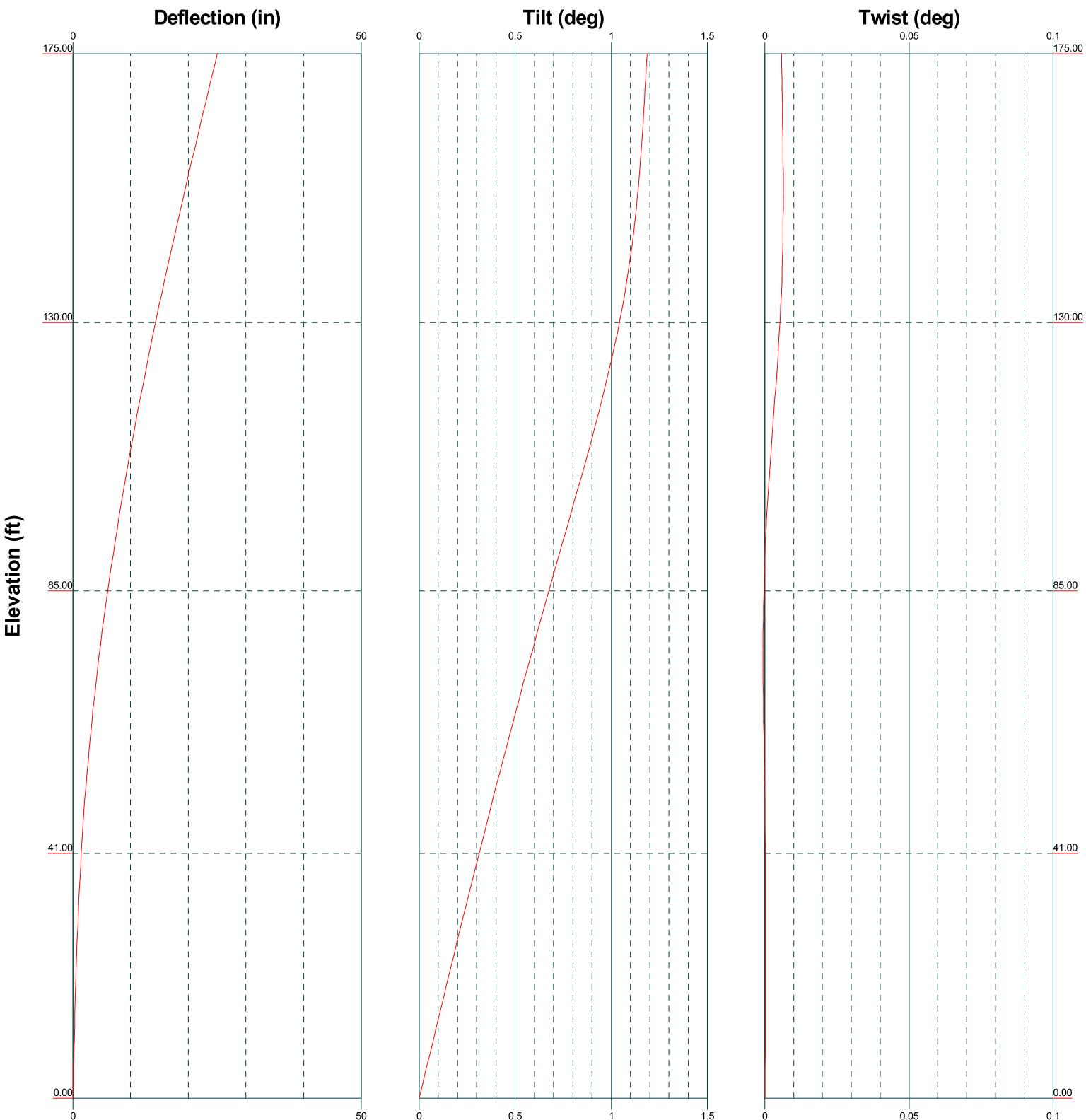
PLAN: SCHEMATIC Tx-LINE LAYOUT

SCALE: NOT TO SCALE

NOTES:

1. Tx LINE LAYOUT IS SCHEMATIC ONLY, BASED UPON LIMITED DATA PROVIDED.
2. NEW BRACKET SUPPORT SPECIFICATION BY OTHERS.

NOV 02, 2016



MALOUF ENGINEERING INT'L. INC.
 17950 PRESTON RD. SUITE 720
 DALLAS, TEXAS - 75252
 Phone: (972) 783-2578
 FAX: (972) 783-2583

maloufengineering.com

Job:	175 ft. MNP. / New Fairfield Center Site #CT2070 (FA 10035312)		
Project:	CT01113M-16V0	Drawn by:	H Lopez
Client:	Vertical Resources Group / AT&T	App'd:	
Code:	TIA-222-G	Date:	11/02/16
Path:	C:\MEI\Projects\16\files\WNP\CT01113M-16V0\CT01113M-16V0_w-Anchor.erl	Scale:	NTS
		Dwg No.	E-5

tnxTower MALOUF ENGINEERING INT'L. INC. 17950 PRESTON RD. SUITE 720 DALLAS, TEXAS - 75252 Phone: (972) 783-2578 FAX: (972) 783-2583	Job 175 ft. MNP. / New Fairfield Center Site #CT2070 (FA 10035312)	Page 1 of 10
	Project CT01113M-16V0	Date 10:47:29 11/02/16
	Client Vertical Resources Group / AT&T	Designed by H Lopez

Tower Input Data

There is a pole section.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Fairfield County, Connecticut.

Basic wind speed of 90 mph.

Structure Class II.

Exposure Category C.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 0.7500 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

IBC 2012 / 115 Mph (Ult.) = 90 Mph (3-Sec) as per CT Building Code.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Pole Reinforcing Data

Height Above Base ft	Segment Length ft	No. of Segments	Offset in	Grade	Type	Size	Unbraced Length ft	K	Bolt Hole Dia. in	Bolts per Row	Shear Lag Factor U
0.00	5.00	4	6.0000	R7S - 150 ksi (120 ksi)	Solid Round	1 3/4	2.50	0.80	0.0000	0	0.000

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Feed Line/Linear Appurtenances - Entered As Area

<i>Description</i>	<i>Component Type</i>	<i>Placement</i>	<i>Total Number</i>
			<i>ft</i>
Safety Line 3/8 (E)	CaAa (Out Of Face)	175.00 - 0.00	1
Step Bolts (E)	CaAa (Out Of Face)	175.00 - 0.00	1
1 5/8 (Town / E)	Inside Pole	175.00 - 0.00	4
1/2 (Town / E)	Inside Pole	175.00 - 0.00	2
Hybrid Cable (Sprint / E)	Inside Pole	155.00 - 0.00	4
1 5/8 (T-Mobile / E)	Inside Pole	145.00 - 0.00	18
HiCap Hybrid Cable (T-Mobile / E)	Inside Pole	145.00 - 0.00	1
1 5/8 (AT&T / E)	Inside Pole	135.00 - 0.00	12
3/8 Fiber Cable (AT&T / E)	Inside Pole	135.00 - 0.00	1
7/8" DC Power Cable (AT&T / E)	Inside Pole	135.00 - 0.00	2
1 5/8 (VzW / E)	Inside Pole	125.00 - 0.00	18
1-1/4" (6x12) Hybrid (HFT1206-24S49-xxx) or Equiv. (VzW / E)	Inside Pole	125.00 - 0.00	1
1 5/8 (E)	Inside Pole	100.00 - 0.00	1
1/2 (Sprint / E)	Inside Pole	85.00 - 0.00	3

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Discrete Tower Loads

Description	Face or Leg	Offset Type	Placement ft	Description	Face or Leg	Offset Type	Placement ft
Lightning Rod (E)	A	From Face	175.00	L.P. Platform Mount (E)	A	None	155.00
PD220 (Town / E)	A	From Leg	175.00	***** Blank Line *****			
PD220 (Town / E)	A	From Leg	175.00	LNX-6515DS-VTM w/ Pipe Mnt.	A	From Face	145.00
PD220 (Town / E)	B	From Leg	175.00	(T-Mobile / E)			
PD220 (Town / E)	C	From Leg	175.00	LNX-6515DS-VTM w/ Pipe Mnt.	B	From Face	145.00
I' Square Panel w/ Pipe Mount (Town / E/F)	C	From Leg	175.00	(T-Mobile / E)			
L.P. Platform Mount (w/ Empty Pipes) (E)	A	None	175.00	LNX-6515DS-VTM w/ Pipe Mnt.	C	From Face	145.00
***** Blank Line *****				(T-Mobile / E)			
(2) RR65-18-02DP w/ Mount Pipe (Sprint / E)	A	From Face	155.00	AIR21 B2A B4P w/ pipe Mount (T-Mobile / E)	A	From Face	145.00
RR45-19-02DPL4 w/ Mount Pipe (Sprint / E)	B	From Face	155.00	AIR21 B2A B4P w/ pipe Mount (T-Mobile / E)	B	From Face	145.00
APXVSPP18-C-A20 w/ Pipe Mount (Sprint / E)	B	From Face	155.00	(T-Mobile / E)			
(2) P40-16-XLPP-RR w/ Pipe Mount (Sprint / E)	C	From Face	155.00	RRUS-11 (T-Mobile / E)	A	From Face	145.00
APXVTM14-C-120 w/ Pipe Mount (Sprint / E)	A	From Face	155.00	RRUS-11 (T-Mobile / E)	B	From Face	145.00
APXVTM14-C-120 w/ Pipe Mount (Sprint / E)	B	From Face	155.00	RRUS-11 (T-Mobile / E)	C	From Face	145.00
APXVTM14-C-120 w/ Pipe Mount (Sprint / E)	C	From Face	155.00	(T-Mobile / E)			
ALU-RRH 8x20 (Sprint / E)	A	From Face	155.00	AIR21 B4A B2P w/ pipe Mount (T-Mobile / E)	A	From Face	145.00
ALU-RRH 8x20 (Sprint / E)	B	From Face	155.00	AIR21 B4A B2P w/ pipe Mount (T-Mobile / E)	B	From Face	145.00
ALU-RRH 8x20 (Sprint / E)	C	From Face	155.00	AIR21 B4A B2P w/ pipe Mount (T-Mobile / E)	C	From Face	145.00
1900 Mhz - RRH (Sprint / E)	A	From Face	155.00	(T-Mobile / E)			
1900 Mhz - RRH (Sprint / E)	B	From Face	155.00	TMA's (T-Mobile / E)	A	From Face	145.00
1900 Mhz - RRH (Sprint / E)	C	From Face	155.00	TMA's (T-Mobile / E)	B	From Face	145.00
1900 Mhz - RRH (Sprint / E)	A	From Face	155.00	TMA's (T-Mobile / E)	C	From Face	145.00
800 Mhz - RRH (Sprint / E)	B	From Face	155.00	L.P. T-Arm Mount (E)	A	From Face	145.00
800 Mhz - RRH (Sprint / E)	C	From Face	155.00	L.P. T-Arm Mount (E)	B	From Face	145.00
800 Mhz - RRH (Sprint / E)	A	From Face	155.00	L.P. T-Arm Mount (E)	C	From Face	145.00
Close Contact Mount (E)	A	None	155.00	***** Blank Line *****			
				(2) 7770.00 Panels w/ Pipe Mount (AT&T / E)	A	From Leg	135.00
				(2) 7770.00 Panels w/ Pipe Mount (AT&T / E)	A	From Leg	135.00
				(2) 7770.00 Panels w/ Pipe Mount (AT&T / E)	A	From Leg	135.00

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Description	Face or Leg	Offset Type	Placement ft	Description	Face or Leg	Offset Type	Placement ft
HPA-65R-BUU-H8 w/ Pipe Mounts (AT&T / E)	A	From Leg	135.00	EDIN-X w/ Pipe Mount (VzW / E)	C	From Leg	125.00
HPA-65R-BUU-H6 w/ Pipe Mounts (AT&T / E)	B	From Leg	135.00	(2) BXA-171085-12CF- EDIN-X w/ Pipe Mount (VzW / E)			
HPA-65R-BUU-H6 w/ Pipe Mounts (AT&T / E)	C	From Leg	135.00	(2) LPA-80080-6CF-2 w / Pipe Mount (VzW / E)	A	From Leg	125.00
(2) LGP21401 TMA'S (AT&T / E)	A	From Leg	135.00	(2) LPA-80080-6CF-2 w / Pipe Mount (VzW / E)	B	From Leg	125.00
(2) LGP21401 TMA'S (AT&T / E)	B	From Leg	135.00	(2) LPA-80080-6CF-2 w / Pipe Mount (VzW / E)	C	From Leg	125.00
(2) LGP21401 TMA'S (AT&T / E)	C	From Leg	135.00	BXA-70063-6CF w/ Pipe Mount (VzW / E)	A	From Leg	125.00
DTMABP7819VG12A Twin TMA (AT&T / E)	A	From Leg	135.00	BXA-70063-6CF w/ Pipe Mount (VzW / E)	B	From Leg	125.00
DTMABP7819VG12A Twin TMA (AT&T / E)	B	From Leg	135.00	BXA-70063-6CF w/ Pipe Mount (VzW / E)	C	From Leg	125.00
DTMABP7819VG12A Twin TMA (AT&T / E)	C	From Leg	135.00	ALU-RRH 2x60 (VzW / E)	A	From Leg	125.00
RRUS-11 (AT&T / E)	A	From Face	135.00	ALU-RRH 2x60 (VzW / E)	B	From Leg	125.00
RRUS-11 (AT&T / E)	B	From Face	135.00	ALU-RRH 2x60 (VzW / E)	C	From Leg	125.00
RRUS-11 (AT&T / E)	C	From Face	135.00	Raycap RxxDC-3315-PF-48 (VzW / E)	A	From Leg	125.00
Raycap DC6-48-60-18-8F SUPPRESSOR (AT&T / E)	B	From Face	135.00	L.P. T-Arm Mount (E)	A	From Leg	125.00
RRUS-32 B2 (AT&T / P)	A	From Face	135.00	L.P. T-Arm Mount (E)	B	From Leg	125.00
RRUS-32 B2 (AT&T / P)	B	From Face	135.00	L.P. T-Arm Mount (E)	C	From Leg	125.00
RRUS-32 B2 (AT&T / P)	C	From Face	135.00	***** Blank Line *****			
Close Contact Mount (E)	A	None	135.00	PD220 (Town / E)	A	From Leg	100.00
L.P. T-Arm Mount (E)	A	From Face	135.00	Standoff Arm w/ Collar Mount (E)	A	None	100.00
L.P. T-Arm Mount (E)	B	From Face	135.00	GPS (Sprint / E/F)	A	From Leg	85.00
L.P. T-Arm Mount (E)	C	From Face	135.00	GPS (Sprint / E/F)	B	From Leg	85.00
***** Blank Line *****				GPS (Sprint / E/F)	C	From Leg	85.00
(2) BXA-171085-12CF- EDIN-X w/ Pipe Mount (VzW / E)	A	From Leg	125.00	Chain Collar Mount (E)	B	None	85.00
(2) BXA-171085-12CF-	B	From Leg	125.00				

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Dishes

<i>Description</i>	<i>Dish Type</i>	<i>Elevation</i>	<i>Outside Diameter</i>
		<i>ft</i>	<i>ft</i>
2' Dia. Dish w/ Radome (Town / E/F)	Paraboloid w/Radome	175.00	2.00

Tower Mast Reaction Summary

<i>Load Combination</i>	<i>Vertical</i>	<i>Shear_x</i>	<i>Shear_z</i>	<i>Oversharing Moment, M_x</i>	<i>Oversharing Moment, M_z</i>	<i>Torque</i>
	<i>lb</i>	<i>lb</i>	<i>lb</i>	<i>lb-ft</i>	<i>lb-ft</i>	<i>lb-ft</i>
Dead Only	51467.98	0.00	-0.00	-1340.41	-134.19	0.00
1.2 Dead+1.6 Wind 0 deg - No Ice	61761.34	32.76	-36764.90	-4393737.98	-6156.19	398.64
0.9 Dead+1.6 Wind 0 deg - No Ice	46321.03	32.77	-36766.42	-4348360.90	-6035.95	402.08
1.2 Dead+1.6 Wind 30 deg - No Ice	61761.57	18121.37	-31847.36	-3806669.44	-2159473.66	-2974.85
0.9 Dead+1.6 Wind 30 deg - No Ice	46321.18	18121.42	-31847.44	-3767104.97	-2137235.74	-2954.91
1.2 Dead+1.6 Wind 60 deg - No Ice	61761.58	31334.98	-18385.02	-2198136.21	-3730712.29	-5512.94
0.9 Dead+1.6 Wind 60 deg - No Ice	46321.18	31334.92	-18384.99	-2175098.97	-3692328.13	-5481.57
1.2 Dead+1.6 Wind 90 deg - No Ice	61761.55	36163.75	-12.19	-3889.10	-4304452.12	-6604.35
0.9 Dead+1.6 Wind 90 deg - No Ice	46321.13	36163.22	-12.19	-3418.18	-4260117.88	-6569.91
1.2 Dead+1.6 Wind 120 deg - No Ice	61761.57	31325.76	18367.59	2191601.06	-3729011.88	-5967.35
0.9 Dead+1.6 Wind 120 deg - No Ice	46321.18	31325.76	18367.59	2169508.09	-3690679.31	-5939.20
1.2 Dead+1.6 Wind 150 deg - No Ice	61761.58	18085.89	31852.27	3804177.11	-2152973.39	-3697.62
0.9 Dead+1.6 Wind 150 deg - No Ice	46321.18	18085.85	31852.21	3765483.73	-2130815.96	-3683.62
1.2 Dead+1.6 Wind 180 deg - No Ice	61761.34	-32.76	36791.54	4395169.19	5816.71	-362.41
0.9 Dead+1.6 Wind 180 deg - No Ice	46321.03	-32.76	36793.06	4350640.68	5786.09	-366.37
1.2 Dead+1.6 Wind 210 deg - No Ice	61761.58	-18142.65	31885.05	3810138.41	2162990.21	3056.66
0.9 Dead+1.6 Wind 210 deg - No Ice	46321.18	-18142.61	31884.98	3771370.83	2140791.94	3035.99
1.2 Dead+1.6 Wind 240 deg - No Ice	61761.57	-31358.46	18424.30	2201956.32	3734632.98	5568.94
0.9 Dead+1.6 Wind 240 deg - No Ice	46321.18	-31358.54	18424.35	2179733.15	3696316.28	5537.37
1.2 Dead+1.6 Wind 270 deg - No Ice	61761.49	-36162.62	53.34	8085.28	4303937.92	6569.10
0.9 Dead+1.6 Wind 270 deg - No Ice	46321.13	-36163.22	53.35	8405.54	4259864.70	6535.13
1.2 Dead+1.6 Wind 300 deg - No Ice	61761.58	-31302.21	-18328.26	-2187776.98	3724405.42	5876.43
0.9 Dead+1.6 Wind 300 deg -	46321.18	-31302.15	-18328.23	-2164870.96	3686186.54	5849.01

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Load Combination	Vertical lb	Shear _x lb	Shear _z lb	Overspinning Moment, M _x lb-ft	Overspinning Moment, M _z lb-ft	Torque lb-ft
No Ice						
1.2 Dead+1.6 Wind 330 deg - No Ice	61761.57	-18064.61	-31814.59	-3800704.28	2148776.15	3652.47
0.9 Dead+1.6 Wind 330 deg - No Ice	46321.18	-18064.66	-31814.67	-3761214.93	2126758.82	3638.67
1.2 Dead+1.0 Ice+1.0 Temp	104314.65	-0.05	0.65	-9363.17	-650.33	-0.48
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	104314.63	5.44	-14194.55	-1772727.70	-1749.43	46.69
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	104314.63	7069.29	-12292.47	-1536459.62	-877961.24	-1525.38
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	104314.65	12234.50	-7095.19	-890814.20	-1518240.59	-2676.03
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	104314.63	14123.69	-0.26	-9754.81	-1752305.67	-3117.05
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	104314.63	12234.53	7095.39	871439.01	-1518212.96	-2736.20
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	104314.63	7065.20	12296.48	1517824.92	-877124.33	-1616.52
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	104314.63	-5.44	14201.26	1754625.77	401.91	-44.93
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	104314.63	-7074.62	12301.92	1518900.01	877639.57	1537.59
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	104314.63	-12239.97	7104.81	873301.90	1517940.63	2688.08
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	104314.63	-14123.69	10.62	-7603.46	1750958.57	3113.44
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	104314.65	-12229.07	-7085.77	-888951.63	1515818.25	2719.60
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	104314.63	-7059.88	-12287.03	-1535385.00	874751.16	1605.16
Dead+Wind 0 deg - Service	51467.97	8.14	-9137.70	-1086565.34	-1619.92	97.01
Dead+Wind 30 deg - Service	51467.97	4503.41	-7914.50	-941365.89	-533555.24	-745.55
Dead+Wind 60 deg - Service	51467.97	7787.13	-4568.90	-543997.57	-921678.55	-1375.26
Dead+Wind 90 deg - Service	51467.97	8987.29	-3.03	-1966.42	-1063421.32	-1643.35
Dead+Wind 120 deg - Service	51467.97	7784.86	4564.58	540377.33	-921270.26	-1484.24
Dead+Wind 150 deg - Service	51467.97	4494.57	7915.69	938742.81	-531955.68	-922.23
Dead+Wind 180 deg - Service	51467.97	-8.14	9144.32	1084924.11	1334.47	-94.82
Dead+Wind 210 deg - Service	51467.97	-4508.67	7923.83	940219.56	534228.55	757.19
Dead+Wind 240 deg - Service	51467.97	-7793.00	4578.69	542935.62	922461.49	1387.21
Dead+Wind 270 deg - Service	51467.97	-8987.29	13.26	987.98	1063135.65	1641.15
Dead+Wind 300 deg - Service	51467.97	-7778.99	-4554.80	-541439.10	919916.09	1470.09
Dead+Wind 330 deg - Service	51467.97	-4489.30	-7906.35	-939888.97	530711.36	912.78

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	175 - 130	25.028	39	1.1838	0.0072
L2	135 - 85	15.441	39	1.0692	0.0058
L3	91 - 41	6.948	39	0.7270	0.0024
L4	48 - 0	1.914	39	0.3694	0.0009

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Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
175.00	2' Dia. Dish w/ Radome	39	25.028	1.1838 (3 dB) 1.8568	0.0072 (3 dB) 1.8568	80278
155.00	(2) RR65-18-02DP w/ Mount Pipe	39	20.115	1.1446	0.0067	20069
145.00	LNX-6515DS-VTM w/ Pipe Mnt.	39	17.733	1.1137	0.0063	13379
135.00	(2) 7770.00 Panels w/ Pipe Mount	39	15.441	1.0692	0.0058	10149
125.00	(2) BXA-171085-12CF-EDIN-X w/ Pipe Mount	39	13.266	1.0080	0.0051	9015
100.00	PD220	39	8.441	0.8062	0.0031	7189
85.00	GPS	39	6.033	0.6748	0.0021	6532

Compression Checks

Pole Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	ϕP _n lb	Ratio P _u ϕP _n
L1	175 - 130 (1)	TP34.688x24x0.25	45.00	175.00	177.9	26.3842	-11667.00	188320.00	0.062
L2	130 - 85 (2)	TP44.688x33.0004x0.3125	50.00	175.00	137.7	42.6238	-26516.80	508161.00	0.052
L3	85 - 41 (3)	TP54.5x42.6605x0.375	50.00	175.00	112.7	62.4494	-41553.40	1109860.00	0.037
L4	41 - 0 (4)	TP64.5x52.0925x0.375	48.00	175.00	94.1	74.7864	-59523.20	1897630.00	0.031

Pole Bending Design Data

Section No.	Elevation ft	Size	M _{ux} lb-ft	ϕM _{nx} lb-ft	Ratio M _{ux} ϕM _{nx}	M _{uy} lb-ft	ϕM _{ny} lb-ft	Ratio M _{uy} ϕM _{ny}
L1	175 - 130 (1)	TP34.688x24x0.25	314904.17	1229983.33	0.256	0.00	1229983.33	0.000
L2	130 - 85 (2)	TP44.688x33.0004x0.3125	1403516.67	2537016.67	0.553	0.00	2537016.67	0.000
L3	85 - 41 (3)	TP54.5x42.6605x0.375	2718825.00	4508725.00	0.603	0.00	4508725.00	0.000
L4	41 - 0 (4)	TP64.5x52.0925x0.375	4212158.33	5975166.67	0.705	0.00	5975166.67	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V _u lb	ϕV _n lb	Ratio V _u ϕV _n	Actual T _u lb-ft	ϕT _n lb-ft	Ratio T _u ϕT _n
L1	175 - 130 (1)	TP34.688x24x0.25	14530.00	899151.00	0.016	225.15	2462975.00	0.000
L2	130 - 85 (2)	TP44.688x33.0004x0.3125	28269.10	1434670.00	0.020	399.71	5080233.33	0.000
L3	85 - 41 (3)	TP54.5x42.6605x0.375	32777.70	2088020.00	0.016	362.73	9028500.00	0.000
L4	41 - 0 (4)	TP64.5x52.0925x0.375	36438.90	2307960.00	0.016	362.42	11964916.00	0.000

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Pole Interaction Design Data

Section No.	Elevation	Ratio P_u	Ratio M_{ux}	Ratio M_{uy}	Ratio V_u	Ratio T_u	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
	ft	ϕP_n	ϕM_{nx}	ϕM_{ny}	ϕV_n	ϕT_n			
L1	175 - 130 (1)	0.062	0.256	0.000	0.016	0.000	0.318 ✓	1.000	4.8.2 ✓
L2	130 - 85 (2)	0.052	0.553	0.000	0.020	0.000	0.606 ✓	1.000	4.8.2 ✓
L3	85 - 41 (3)	0.037	0.603	0.000	0.016	0.000	0.641 ✓	1.000	4.8.2 ✓
L4	41 - 0 (4)	0.031	0.705	0.000	0.016	0.000	0.737 ✓	1.000	4.8.2 ✓

Reinforcing Design Data (Compression)

Section No.	Elevation	Size	L	L_u	Kl/r	A	P_u	ϕP_n	Ratio P_u
	ft		ft	ft		in ²	lb	lb	$\frac{\phi P_n}{P_u}$
L4	5 - 0	1 3/4	5.00	2.50	54.9 K=0.80	2.4053	-105532.00	153196.00	0.689 ¹

¹ $P_u / \phi P_n$ controls

Reinforcing Bending Design Data

Section No.	Elevation	Size	M_{ux}	ϕM_{nx}	Ratio M_{ux}	M_{uy}	ϕM_{ny}	Ratio M_{uy}
	ft		lb-ft	lb-ft	$\frac{\phi M_{nx}}{M_{ux}}$	lb-ft	lb-ft	$\frac{\phi M_{ny}}{M_{uy}}$
L4	5 - 0	1 3/4	0.00	8039.07	0.000	0.00	8039.07	0.000

Reinforcing Interaction Design Data

Section No.	Elevation	Size	Ratio P_u	Ratio M_{ux}	Ratio M_{uy}	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
	ft		ϕP_n	ϕM_{nx}	ϕM_{ny}			
L4	5 - 0	1 3/4	0.689	0.000	0.000	0.689 ¹ ✓	1.000	4.8.1 ✓

¹ $P_u / \phi P_n$ controls

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Tension Checks

Reinforcing Design Data (Tension)

Section No.	Elevation ft	Size	L	L _u	Kl/r	A	P _u	ϕP _n	Ratio P _u ϕP _n
L4	5 - 0	1 3/4	5.00	2.50	68.6	2.4053	102083.00	259770.00	0.393 ¹

¹ P_u / ϕP_n controls

Reinforcing Bending Design Data

Section No.	Elevation ft	Size	M _{nx}	ϕM _{nx}	Ratio M _{nx} ϕM _{nx}	M _{ny}	ϕM _{ny}	Ratio M _{ny} ϕM _{ny}
L4	5 - 0	1 3/4	66.72	8039.07	0.008	0.00	8039.07	0.000

Reinforcing Interaction Design Data

Section No.	Elevation ft	Size	Ratio P _u ϕP _n	Ratio M _{nx} ϕM _{nx}	Ratio M _{ny} ϕM _{ny}	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L4	5 - 0	1 3/4	0.393	0.008	0.000	0.393 ¹	1.000	4.8.1 ✓

¹ P_u / ϕP_n controls

Anchor Design Data

Plate Thickness	Number of Anchor Bolts	Anchor Bolt Size	Controlling Condition	Ratio
1.500	24	2.000	Bolt T	0.53 ✓

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Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	ϕP_{allow} lb	% Capacity	Pass Fail
L1	175 - 130	Pole	TP34.688x24x0.25	1	-11667.00	188320.00	31.8	Pass
L2	130 - 85	Pole	TP44.688x33.0004x0.3125	2	-26516.80	508161.00	60.6	Pass
L3	85 - 41	Pole	TP54.5x42.6605x0.375	3	-41553.40	1109860.00	64.1	Pass
L4	41 - 0	Pole	TP64.5x52.0925x0.375	4	-59523.20	1897630.00	73.7	Pass
L4	5 - 0	Reinforcing	1 3/4	8	-105532.00	153196.00	68.9	Pass
Summary								
Pole (L4) 73.7								
Base Plate 71.5								
Reinforcing 68.9								
(L4) RATING = 73.7								
Pass								



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TITLE: Individual Spread Footing Check
175' Monopole
New Fairfield Center #CT2070
DESCRIPTION:

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CHECKED BY: KMM
APPROVED BY: MM

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FOUNDATION DESIGN CALCULATIONS

Input:

Twr := 1 Tower Type, 1 = Monopole, 2 = SST, 3 = GT
Fdn := 1 Foundation Type, 1 = Spf, 2 = Mat, 3 = Caisson, 4 = Deadman Anchor, 5 = Pile Group w/ Head
6 = Single Pile or Anchoring Device, 7 = Non Battered Piles With Tapered Cross-Section
Class := 2 Structure Class, 1, 2, 3, Table 2-1
SoilReport := 1 Soil Report, 0 = Not Available, 1 = Available
SiteClass := "D" Seismic Site class

Geotechnical Report Source: No Geotechnical Report / Assumed soil parameters

Foundation Source: Foundation Specs as per URS analysis Proj. #CW1021 dated 2/16/04.

Wind / Seismic: DESIGN BASE SHEAR IS GOVERNED BY WIND LOADING

Foundation Loads / Tower Base Reactions

Load Case I: 1.2D+1.0Dg+1.6Wo ; Load Case II: 0.9D+1.0Dg+1.6Wo ; Load Case III: 1.2D+1.0Dg+1.0Di+1.0Wi+1.0Ti

n := 3	Number of load cases		k := 1..n	Dead Load Factors			
	Download	Uplift		Shear	Moment	Total Shear	Tower Weight
Case I	P _k := 61.761·kip	U _k := 0.0·kip	Sh _k := 36.793·kip	M _k := 4395.17·kip·ft	TSh _k := 0.0·kip	TWt _k := 31.06·kip	1.2
Case II	46.321·kip	0.0·kip	36.793·kip	4350.64·kip·ft	0.0·kip	31.06·kip	0.9
Case III	0.00·kip	0.0·kip	0.00·kip	0.00·kip·ft	0.0·kip	0.00·kip	1.2

Anchor Bolt Details (Verify details with tower manufacturer)

n_{bolt} := 0 No. of anchor bolts
L_{bolt} := 0.00·in Anchor Bolt Length-Total
L_{proj} := 0.00·in Projected length of anchor bolt
dia_{ancb} := 2.00·in Anchor bolt dia.
area_{ancb} := 3.1416·in² Anchor bolt - Tensile Area
BC_{anc} := 0.00·in Pole bolt circle
f_{yancbolt} := 75·ksi Anchor bolt, Yield Strength, A615-75
f_{uancbolt} := 100·ksi Anchor bolt, Ultimate Tensile Strength, A615-75

Foundation Dimensions

n_{ped} := 1 Number of pedestals, 1, 3 or 4, Use 1 for isolated spread footings, 3 or 4 for mat
TW := 0·ft Tower Face Width, ft - enter 0ft for Monopole, Guyed Tower, Isolated Footings
col_t := 0 Pedestal, 0=Round, 1=Square
Ped_s := 7.00·ft Ped diameter / side
E_g := 0.250·ft Ped Extension above soil grade
D_f := 6.00·ft Fdn Depth, grade to pad bottom
L_{pad} := 27.50·ft Length of pad
B := L Width of pad
T_f := 4.00·ft Thickness of pad
toe := 0 Has Toe =1, No Toe = 0



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ecc := 0·ft	Eccentricity, load & pad centroid
cc := 3.0·in	Concrete Cover
n _{vbars} := 50	No. vert. bars in pedestal
vbar := 11	Vert. Bar Size
tbar := 4	Tie Bar Size
S _p tie := 12·in	Tie Bar Spacing
n _{hbars} := 32	No. Horiz bars in pad
hbar := 10	Horiz. Bar Size

Material Parameters

γ _c := 0.145· $\frac{\text{kip}}{\text{ft}^3}$	Unit Wt. of Concrete	ϕ _{cshear} := 0.75	as per ACI-05, 9.3.2.3
f _c := 3000·psi	Concrete Compressive Strength	ϕ _{ccompr} := 0.65	as per ACI-05, 9.3.2.2
f _y := 60000·psi	Rebar yield strength	ϕ _{caxten} := 0.9	as per ACI-05, 9.3.2.1

Soil Properties

ϕ := 30-deg	Internal angle of friction	ϕ _{bg} := 0.6	as per Rev. G, bearing, GT
c _u := 0.0·ksf	Cohesion	ϕ _{bs} := 0.75	as per Rev. G, bearing, SS, MP
σ _{adh} := 0.0·ksf	Adhesion	ϕ _{up1} := 0.75	as per Rev. G, uplift
σ _p := 0.100· $\frac{\text{kip}}{\text{ft}^3}$	Passive Pressure, -1 auto calculate	ϕ _{up2} := 0.5	as per Rev. G, uplift Single Rock/Soil Bolt or anchor
γ _s := 0.100· $\frac{\text{kip}}{\text{ft}^3}$	Unit weight of soil	ϕ _{up3} := 0.4	as per Rev. G, uplift Non-battered piles with tapered cross-section
Brg _{ult} := 8.0·ksf	Ultimate Bearing Capacity	ϕ _{sh} := 0.75	as per Rev. G, Shear, Friction
μ _{soil} := 0.20	Coefficient of friction Concrete to Soil	ϕ _{lateral} := 0.75	as per Rev. G, Lateral Resistance
L _{neg} := 2.00·ft	Depth of soil neglected for lateral resistance		
L _{water} := -1·ft	Water Table, -1 No Water	γ _w := 0.062428· $\frac{\text{kip}}{\text{ft}^3}$	Unit Weight of Water
red _{up} := 1.0	Uplift Reduction Factor		
flag _{up} := 0	Flag to include Uplift reduction factor in the stability checks, 0 = No, 1 = Yes To Be Used When Ground Sloping		

Re-bar Properties

No := ((0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18)) ^T
d _b := ((0 0 0 0.375 0.5 0.625 0.75 0.875 1.00 1.128 1.27 1.41 0 0 1.693 0 0 0 2.257)) ^T ·in
A _b := ((0 0 0 0.11 0.20 0.31 0.44 0.60 0.79 1.00 1.27 1.56 0 0 2.25 0 0 0 4.00)) ^T ·in ²



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Total Resisting Moment - Alternate Way

$$q_k := \frac{T_{w_k}}{B_{-I}} \quad q_c := B_{rg_{ult}} \phi_{brg}$$

$$M_{Ralt_k} := \frac{L \cdot T_{W_k}}{2} \cdot \left(1 - \frac{q_k}{q_c} \right)$$

OVERTURNING MOMENT

$$L_{hs} := D_f + E_g \quad \text{Lever Arm}$$

$$M_{o_k} := M_k + S h_k \cdot L_{hs} + P_k \cdot ecc$$

Eccentricity

$$ec_k := \frac{Mo_k}{Tw_k}$$

Bearing - Combined Compression and Moment

$$q\max_k := \text{if } ec_k \leq \frac{L}{6}, \left[\frac{Tw_k}{B \cdot L} \cdot \left(1 + \frac{6 \cdot ec_k}{L} \right) \right], \left[\frac{Tw_k}{B \cdot (L - 2 \cdot ec_k)} \right]$$

$$q_{\min_k} := \text{if } [e c_k \leq \frac{L}{6}, \left[\frac{T w_k}{B \cdot L} \left(1 - \frac{6 e c_k}{L} \right) \right], 0 \cdot k s f]$$

$$q_{\max, \text{alt}} := \frac{\left(Tw_k\right)^2}{Tw_k \cdot L^2 - 2 \cdot Mo_k \cdot B}$$

$$Lp_k := \text{if } ec_k \leq \frac{L}{6}, L, (L - 2 \cdot ec_k)$$

Overspinning Moment	Resisting Moment	Alternate Resisting Moment	Total Download	Eccentricity	Contact Length
$Mo_k =$	$Mr_k =$	$Mralt_k =$	$Tw_k =$	$ec_k =$	$Lp_k =$
$4.625 \cdot 10^3$	$\cdot \text{kip}$	$1.08 \cdot 10^4$	$\cdot \text{kip} \cdot \text{ft}$	775.441	$\cdot \text{kip}$
$4.581 \cdot 10^3$		$8.134 \cdot 10^3$	$8.84 \cdot 10^3$	581.581	5.965
0		$9.951 \cdot 10^3$	$6.972 \cdot 10^3$	713.68	7.876
			$8.27 \cdot 10^3$	0	15.571
					11.748
					27.5

$q_{max_k} =$	$q_{min_k} =$	$q_{maxalt_k} =$
1.811 ·ksf	0 ·ksf	1.811 ·ksf



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Bearing Ratios

$$R_{br_k} := \frac{q_{max_k}}{B_{rg_{ult}} \phi_{brg}}$$

OTM - Stability Ratios

$$R_{mom_k} := \frac{M_{o_k}}{M_{r_k}} \quad R_{mom_{alt_k}} := \frac{M_{o_k}}{M_{r_{alt_k}}}$$

Shear Check

Total Resisting Shear

$$Sh_{r_k} := \phi_{sh} (T_{w_k} \mu_{soil} + P_{e_p})$$

$$R_{sh_k} := \frac{Sh_k}{Sh_{r_k}}$$

Combined Bearing and Moment - Ratios

Bearing Ratio	Overspinning Ratio	Contact Length
$R_{br_k} =$	$R_{mom_k} =$	$L_{p_k} =$
30.182	42.826	15.571
30.003	56.312	11.748
15.728	0	27.5

Shear Ratio

Total Shear	Shear Resistance	Shear Ratio
$Sh_k =$	$Sh_{r_k} =$	$R_{sh_k} =$
36.793	132.816	27.702
36.793	103.737	35.468
0	123.552	0

APPENDIX 2 – SOURCE / CHANGED CONDITION



From: Vertical Resources Group <mnobre@verticalresourcesgrp.com>
Sent: Monday, October 31, 2016 9:19 AM
To: 'Malouf Engr - Liz Adkins'
Cc: 'Mark Malouf'
Subject: RE: AT&T Site CT2070 FA10035312 175' Pole

AT&T coax is RFS 1.625" (4/sec, 12 tot.) existing to remain in place, (2) 5/8" DC power, (1) 3/8" fiber.

...

Miguel Nobre
Vertical Resources Group
489 Washington Street
Auburn, MA 01501
P: 508-981-9590
F: 508-519-8939

From: Vertical Resources Group [<mailto:mnobre@verticalresourcesgrp.com>]

Sent: Sunday, October 30, 2016 10:16 AM

To: Mark Malouf

Subject: RE: AT&T Site CT2070 FA10035312 175' Pole

...

AT&T Existing Loading:

(6) Powerwave 7770 (UMTS & GSM existing to remain)
(1) CCI HPA65RBUUH8 (LTE to remain)
(2) CCI HPA65RBUUH6 (LTE to remain)
(6) CCI LGP21401 (UMTS TMA to remain)
(3) CCI DTMABP7819VG12A (GSM to remain)
(3) Ericsson RRUS-11 (LTE700 to remain)
(3) Ericsson RRUS-12+A2 (LTE PCS TO BE REPLACED)
(1) Raycap Surge arrestor DC6-48-60-18-8F (to remain)
Mount Valmont RMV12

AT&T Existing Loading:

(6) Powerwave 7770 (UMTS & GSM existing to remain)
(1) CCI HPA65RBUUH8 (LTE to remain)
(2) CCI HPA65RBUUH6 (LTE to remain)
(6) CCI LGP21401 (UMTS TMA to remain)
(3) CCI DTMABP7819VG12A (GSM to remain)
(3) Ericsson RRUS-11 (LTE700 to remain)
(3) Ericsson RRUS-32 B2 (LTE PCS NEW)
(1) Raycap Surge arrestor DC6-48-60-18-8F (to remain)
Mount Valmont RMV12

Miguel Nobre
Vertical Resources Group
489 Washington Street
Auburn, MA 01501
P: 508-981-9590
F: 508-519-8939

Section 17A - FINAL SECTOR/CELL INFORMATION - SECTOR A (OR OMNI)

Section 17A - FINAL SECTOR/CELL INFORMATION - SECTOR A (OR OMNI)

Section 17B - FINAL SECTOR/CELL INFORMATION - SECTOR B

ANTENNA POSITION 4

Section 17C - FINAL SECTOR/CELL INFORMATION - SECTOR C

ANTENNA POSITION 4

