

QC Development
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860-670-9068
Mark.Roberts@QCDevelopment.net

December 8, 2016

Melanie A. Bachman Acting Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

Notice of Exempt Modification – New Cingular Wireless PCS, LLC (AT&T) – CT2841 208 Valley Road, New Canaan, CT 06840 N 41-09-58.5 W 73-28-13.7

Dear Ms. Bachman:

AT&T currently maintains three (3) antennas at the 86-foot level of the existing 120-foot Silhouette Monopole at 208 Valley Road, New Canaan, CT. The structure is owned by Tarpon Towers II LLC and the property is owned by Silver Hill Hospital, Inc. AT&T now intends to remove three (3) COMMSCOPE antennas and replace them with three (3) QUINTEL antennas. AT&T also intends to remove three (3) CCI TMAs and replace them with six (6) Kaelus TMAs. The new antennas would be installed at the 86-foot level of the tower.

This facility was approved by the Connecticut Siting Council in Docket 0401, on February 2, 2012. The Decision and Order included conditions requiring a concealed antenna tower design and limiting the tower height to 120 feet AGL. No modifications to the tower's concealment design or total height are proposed by AT&T. This modification therefore complies with the aforementioned approval.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Robert E. Mallozzi III, First Selectman for the Town of New Canaan, as well as the property owner and the

tower owner.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

- 1. The proposed modifications will not result in an increase in the height of the existing structure.
- 2. The proposed modifications will not require the extension of the site boundary.
- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
- 4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard.
- 5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
- 6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, AT&T respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitute an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Please feel free to call me at (860) 670-9068 with any questions regarding this matter. Thank you for your consideration.

Sincerely,

Mark Roberts

QC Development

Consultant for AT&T

Attachments

cc: Robert E. Mallozzi III - as elected official (via e-mail)

Tarpon Towers II LLC - as structure owner (via e-mail)

Silver Hill Hospital Inc. – as property owner

Power Density

Existing Loading on Tower

Carrier	# of Channels	ERP/Ch (W)	Antenna Centerline Height (ft)	Power Density (mW/cm^2)	Freq. Band (MHz**)	Limit S (mW /cm^2)	%МРЕ
Other Carriers*							3.59%
AT&T UMTS	2	500	86	0.0562	850	0.5667	0.99%
AT&T UMTS	2	500	86	0.0562	1900	1.0000	0.56%
AT&T LTE	2	500	86	0.0562	700	0.4667	1.20%
AT&T LTE	2	500	86	0.0562	2100	1.0000	0.56%
Site Total							6.91%

^{*}Per CSC Records (available upon request, includes calculation formulas)

Proposed Loading on Tower

Carrier	# of Channels	ERP/Ch (W)	Antenna Centerline Height (ft)	Power Density (mW/cm^2)	Freq. Band (MHz**)	Limit S (mW /cm^2)	%МРЕ
Other Carriers*	The second second			Contract of the same			3.59%
AT&T UMTS	2	500	86	0.0562	850	0.5667	0.99%
AT&T LTE	2	500	86	0.0562	700	0.4667	1.20%
AT&T LTE	2	500	86	0.0562	1900	1.0000	0.56%
Site Total							6.35%

^{*}Per CSC Records (available upon request, includes calculation formulas)

Note: Proposed Loading may also include corrections to certain Existing Loading values

^{**} If a range of frequencies are used, such as 880-894, enter the lowest value, i.e. 880

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WIRELESS COMMUNICATIONS FACILITY

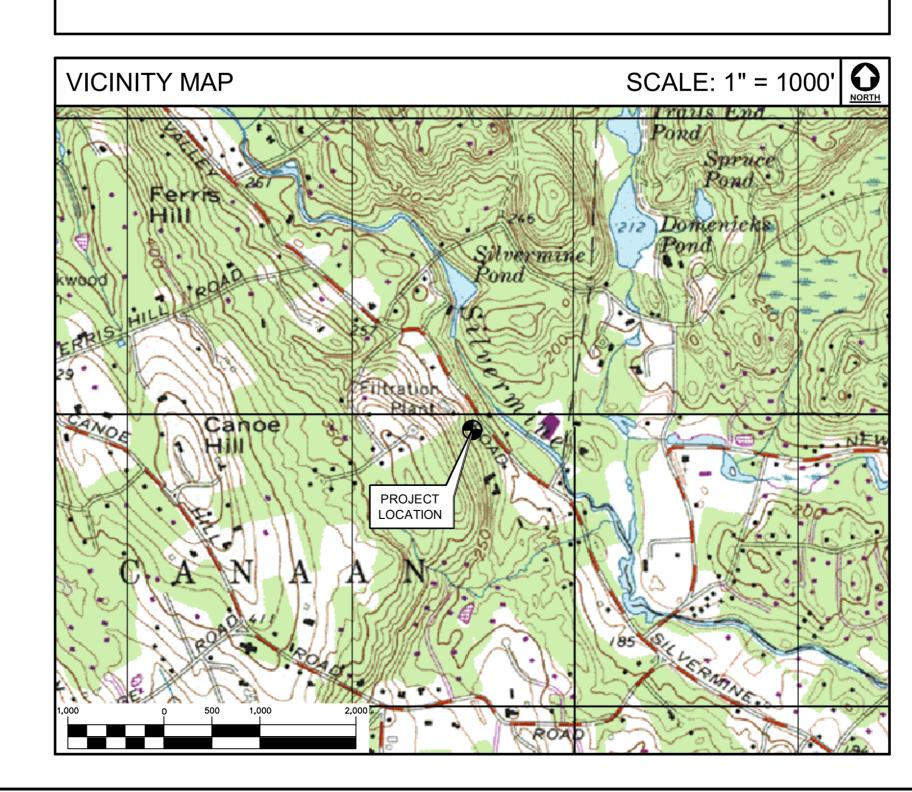
CT2841 - LTE BWE NEW CANAAN HOSPITAL 208 VALLEY ROAD NEW CANAAN, CT 06840

GENERAL NOTES

- ALL WORK SHALL BE IN ACCORDANCE WITH THE 2012 INTERNATIONAL BUILDING CODE AS MODIFIED BY THE 2016 CONNECTICUT STATE BUILDING CODE, INCLUDING THE TIA-222 REVISION "G" STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND SUPPORTING STRUCTURES, 2016 CONNECTICUT FIRE SAFETY CODE AND, NATIONAL ELECTRICAL CODE AND
- THE COMPOUND, TOWER, PRIMARY GROUND RING, ELECTRICAL SERVICE TO THE METER BANK AND TELEPHONE SERVICE TO THE DEMARCATION POINT ARE PROVIDED BY SITE OWNER. AS BUILT FIELD CONDITIONS REGARDING THESE ITEMS SHALL BE CONFIRMED BY THE CONTRACTOR. SHOULD ANY FIELD CONDITIONS PRECLUDE COMPLIANCE WITH THE DRAWINGS, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER AND SHALL NOT PROCEED WITH ANY AFFECTED WORK.
- 3. CONTRACTOR SHALL REVIEW ALL DRAWINGS AND SPECIFICATIONS IN THE CONTRACT DOCUMENT SET. CONTRACTOR SHALL COORDINATE ALL WORK SHOWN IN THE SET OF DRAWINGS. THE CONTRACTOR SHALL PROVIDE A COMPLETE SET OF DRAWINGS TO ALL SUBCONTRACTORS AND ALL RELATED PARTIES. THE SUBCONTRACTORS SHALL EXAMINE ALL THE DRAWINGS AND SPECIFICATIONS FOR THE INFORMATION THAT AFFECTS THEIR WORK.
- CONTRACTOR SHALL PROVIDE A COMPLETE BUILD-OUT WITH ALL FINISHES, STRUCTURAL, MECHANICAL, AND ELECTRICAL COMPONENTS AND PROVIDE ALL ITEMS AS SHOWN OR INDICATED ON THE DRAWINGS OR IN THE WRITTEN SPECIFICATIONS.
- 5. CONTRACTOR SHALL FURNISH ALL MATERIAL, LABOR AND EQUIPMENT TO COMPLETE THE WORK AND FURNISH A COMPLETED JOB ALL IN ACCORDANCE WITH LOCAL AND STATE GOVERNING AUTHORITIES AND OTHER AUTHORITIES HAVING LAWFUL JURISDICTION OVER THE WORK.
- 6. CONTRACTOR SHALL SECURE AND PAY FOR ALL PERMITS AND ALL INSPECTIONS REQUIRED AND SHALL ALSO PAY FEES REQUIRED FOR THE GENERAL CONSTRUCTION, PLUMBING, ELECTRICAL AND HVAC. PERMITS SHALL BE PAID FOR BY THE RESPECTIVE SUBCONTRACTORS.
- CONTRACTOR SHALL MAINTAIN A CURRENT SET OF DRAWINGS AND SPECIFICATIONS ON SITE AT ALL TIMES AND INSURE DISTRIBUTION OF NEW DRAWINGS TO SUBCONTRACTORS AND OTHER RELEVANT PARTIES AS SOON AS THEY ARE MADE AVAILABLE. ALL OLD DRAWINGS SHALL BE MARKED VOID AND REMOVED FROM THE CONTRACT AREA. THE CONTRACTOR SHALL FURNISH AN 'AS-BUILT' SET OF DRAWINGS TO OWNER UPON COMPLETION OF PROJECT.
- LOCATION OF EQUIPMENT, AND WORK SUPPLIED BY OTHERS THAT IS DIAGRAMMATICALLY INDICATED ON THE DRAWINGS SHALL BE DETERMINED BY THE CONTRACTOR. THE CONTRACTOR SHALL DETERMINE LOCATIONS AND DIMENSIONS SUBJECT TO STRUCTURAL CONDITIONS AND WORK OF THE SUBCONTRACTORS.
- 9. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURES AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY. MAINTAIN EXISTING BUILDING'S/PROPERTY'S OPERATIONS, COORDINATE WORK WITH BUILDING/PROPERTY OWNER.

- 10. DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF ANY WORK SHOULD BE INDICATED TO BE SUBSTANDARD TO ANY ORDINANCES, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE IN HIS WORK AND SHALL EXECUTE THE WORK CORRECTLY IN ACCORDANCE WITH SUCH ORDINANCES, LAWS, CODES. RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
- 11. ALL UTILITY WORK SHALL BE IN ACCORDANCE WITH LOCAL UTILITY COMPANY REQUIREMENTS AND SPECIFICATIONS.
- 12. ALL EQUIPMENT AND PRODUCTS PURCHASED ARE TO BE REVIEWED BY CONTRACTOR AND ALL APPLICABLE SUBCONTRACTORS FOR ANY CONDITION PER MFR.'S RECOMMENDATIONS. CONTRACTOR TO SUPPLY THESE ITEMS AT NO COST TO OWNER OR CONSTRUCTION MANAGER.
- 13. ANY AND ALL ERRORS, DISCREPANCIES, AND 'MISSED" ITEMS ARE TO BE BROUGHT TO THE ATTENTION OF THE AT&T CONSTRUCTION MANAGER DURING THE BIDDING PROCESS BY THE CONTRACTOR. ALL THESE ITEMS ARE TO BE INCLUDED IN THE BID. NO 'EXTRA' WILL BE ALLOWED FOR MISSED ITEMS.
- 14. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ON-SITE SAFETY FROM THE TIME THE JOB IS AWARDED UNTIL ALL WORK IS COMPLETE AND ACCEPTED BY THE OWNER.
- 15. CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO ENGINEER FOR APPROVAL. DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITTING TO THE CONSTRUCTION MANAGER FOR REVIEW.
- 16. THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES, AND EXISTING CONDITIONS AT THE SITE, PRIOR TO FABRICATION AND/OR INSTALLATION OF ANY WORK IN THE CONTRACT
- 17. COORDINATION, LAYOUT, FURNISHING AND INSTALLATION OF CONDUIT AND ALL APPURTENANCES REQUIRED FOR PROPER INSTALLATION OF ELECTRICAL AND TELECOMMUNICATION SERVICE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- 18. ALL EQUIPMENT AND PRODUCTS PURCHASED ARE TO BE REVIEWED BY CONTRACTOR AND ALL APPLICABLE SUB-CONTRACTORS FOR ANY CONDITION PER THE MANUFACTURER'S RECOMMENDATIONS. CONTRACTOR TO SUPPLY THESE ITEMS AT NO COST TO OWNER OR CONSTRUCTION MANAGER.
- 19. ALL DAMAGE CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD LIABLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.
- 20. THE CONTRACTOR SHALL CONTACT "CALL BEFORE YOU DIG" AT LEAST 48 HOURS PRIOR TO ANY EXCAVATIONS AT 1-800-922-4455. ALL UTILITIES SHALL BE IDENTIFIED AND CLEARLY MARKED PRIOR TO ANY EXCAVATION WORK. CONTRACTOR SHALL MAINTAIN AND PROTECT MARKED UTILITIES THROUGHOUT PROJECT COMPLETION.
- 21. CONTRACTOR SHALL COMPLY WITH OWNERS ENVIRONMENTAL ENGINEER ON ALL METHODS AND PROVISIONS FOR ALL EXCAVATION ACTIVITIES INCLUDING SOIL DISPOSAL, ALL BACKFILL MATERIALS TO BE PROVIDED BY THE CONTRACTOR.

SITE DIRECTIONS		
FROM: 500 ENTERPRISE DRIVE ROCKY HILL, CONNECTICUT	TO:	208 VALLEY ROAD NEW CANAAN, CONNECTICUT
1. HEAD NORTHEAST ON ENTERPRISE DR TOWARD CAPITAL BLVD 2. TURN LEFT ONTO CAPITAL BLVD 3. TURN LEFT ONTO WEST ST 4. TURN LEFT TO MERGE ONTO I-91 S TOWARD NEW HAVEN 5. TAKE EXIT 17 FOR CT-15 S/W CROSS PKWY 6. TAKE EXIT 38 FOR CT-123 7. TURN SLIGHT LEFT ONTO CT-123 8. TURN RIGHT ONTO SILVERMINE RD 9. TURN LEFT ONTO VALLEY RD. DESTINATION IS ON THE LEFT		0.37 MI 0.27 MI 0.30 MI 9.59 MI 49.00 MI 0.15 MI 1.68 MI 1.51 MI 0.48 MI



PROJECT SUMMARY

- THE PROPOSED SCOPE OF WORK CONSISTS OF A MODIFICATION TO THE EXISTING UNMANNED TELECOMMUNICATIONS FACILITY
- A. REMOVE AND REPLACE EXISTING LTE ANTENNA FOR PROPOSED LTE (12) PORT ANTENNA, (1) PER SECTOR.
- B. REMOVE AND REPLACE (3) EXISTING RRUS-12'S FOR PROPOSED RRUS-32 B2'S, (3) TOTAL ON EXISTING
- C. REMOVE AND REPLACE (3) EXISTING TMAS WITH (6) NEW KAELUS TMAs, (2) PER SECTOR.
- D. INSTALL (6) NEW COAX LINES, (2) PER SECTOR, FROM GROUND EQUIPMENT TO ANTENNAS ROUTED WITHIN THE
- EXISTING UNIPOLE. E. REMOVE AND REPLACE (6) DIPLEXERS FOR (12) PENTAPLEXERS INSTALLED AT GROUND LEVEL.
- F. INSTALL (12) SURGE ARRESTORS, (4) PER SECTOR. G. INSTALL (6) CURRENT INJECTORS, (2) PER SECTOR. H. INSTALL XMU WITHIN EXISTING PURCELL CABINET HOUSING LTE

PROJECT INFORMATION

AT&T SITE NUMBER: CT2841 NEW CANAAN HOSPITAL AT&T SITE NAME:

SITE ADDRESS: 208 VALLEY ROAD NEW CANAAN, CT 06840

LESSEE/APPLICANT: 500 ENTERPRISE DRIVE, SUITE 3A ROCKY HILL, CT 06067

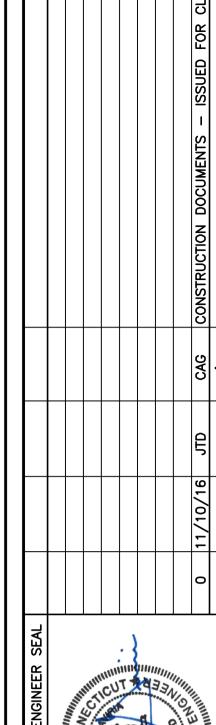
ENGINEER: CENTEK ENGINEERING, INC. 63-2 NORTH BRANFORD RD.

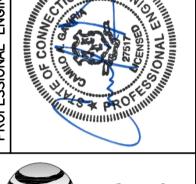
PROJECT COORDINATES: LATITUDE: 41'-09'-58.454 N LONGITUDE: 73°-28'-13.721" W GROUND ELEVATION: 261.5'± AMSL

BRANFORD, CT 06405

COORDINATES AND GROUND ELEVATION REFERENCED FROM FAA 2C CERTIFICATION AS PREPARED BY MARTINEZ, COUCH & ASSOCIATES, LLC DATED DECEMBER 15, 2009.

SHEET INDEX				
SHT. NO.	O. DESCRIPTION R			
T-1	TITLE SHEET	0		
N-1	NOTES AND SPECIFICATIONS	0		
C-1	PLANS AND ELEVATION	0		
C-2	LTE BWE EQUIPMENT DETAILS	0		
C-3	RRU MOUNTING CONFIGURATION	0		
E-1	TYPICAL ELECTRICAL DETAILS AND NOTES	0		









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11/03/16 SCALE: AS NOTED JOB NO. 16034.10

TITLE SHEET



NOTES AND SPECIFICATIONS

DESIGN BASIS

- GOVERNING CODE: 2012 INTERNATIONAL BUILDING CODE AS MODIFIED BY THE 2016 CT STATE SUPPLEMENT.
- 2. TIA/EIA-222 REVISION "G", ASCE MANUAL NO. 72 "DESIGN OF STEEL TRANSMISSION POLE STRUCTURES SECOND EDITION".
- 3. DESIGN CRITERIA:

WIND LOAD: (TOWER & FOUNDATION) NOMINAL DESIGN WIND SPEED (V) = 93 MPH (2016 CSBC: APPENDIX 'N')

GENERAL NOTES:

- 1. ALL CONSTRUCTION SHALL BE IN COMPLIANCE WITH THE GOVERNING BUILDING
- 2. DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF ANY WORK SHOULD BE INDICATED TO BE SUBSTANDARD TO ANY ORDINANCES, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE IN HIS WORK AND SHALL EXECUTE THE WORK CORRECTLY IN ACCORDANCE WITH SUCH ORDINANCES, LAWS, CODES, RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
- 3. BEFORE BEGINNING THE WORK, THE CONTRACTOR IS RESPONSIBLE FOR MAKING SUCH INVESTIGATIONS CONCERNING PHYSICAL CONDITIONS (SURFACE AND SUBSURFACE) AT OR CONTIGUOUS TO THE SITE WHICH MAY AFFECT PERFORMANCE AND COST OF THE WORK.
- 4. DIMENSIONS AND DETAILS SHALL BE CHECKED AGAINST EXISTING FIELD CONDITIONS.
- THE CONTRACTOR SHALL VERIFY AND COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS. SLEEVES AND ANCHOR BOLTS AS REQUIRED BY ALL TRADES.
- 6. ALL DIMENSIONS, ELEVATIONS, AND OTHER REFERENCES TO EXISTING STRUCTURES, SURFACE, AND SUBSURFACE CONDITIONS ARE APPROXIMATE. NO GUARANTEE IS MADE FOR THE ACCURACY OR COMPLETENESS OF THE INFORMATION SHOWN. THE CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS, ELEVATIONS, ANGLES WITH EXISTING CONDITIONS AND WITH ARCHITECTURAL AND SITE DRAWINGS BEFORE PROCEEDING WITH ANY WORK.
- 7. AS THE WORK PROGRESSES, THE CONTRACTOR SHALL NOTIFY THE OWNER OF ANY CONDITIONS WHICH ARE IN CONFLICT OR OTHERWISE NOT CONSISTENT WITH THE CONSTRUCTION DOCUMENTS AND SHALL NOT PROCEED WITH SUCH WORK UNTIL THE CONFLICT IS SATISFACTORILY RESOLVED.
- 8. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR PROVIDING AND MAINTAINING ADEQUATE SHORING, BRACING, AND BARRICADES AS MAY BE REQUIRED FOR THE PROTECTION OF EXISTING PROPERTY, CONSTRUCTION WORKERS, AND FOR PUBLIC SAFETY.
- 9. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURES AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY. MAINTAIN EXISTING SITE OPERATIONS, COORDINATE WORK WITH NORTHEAST UTILITIES
- 10. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER FOUNDATION REMEDIATION WORK IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND TO ENSURE THE SAFETY OF THE STRUCTURE AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, TEMPORARY BRACING, GUYS OR TIEDOWNS, WHICH MIGHT BE NECESSARY.
- ALL DAMAGE CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD LIABLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.
- 12. SHOP DRAWINGS, CONCRETE MIX DESIGNS, TEST REPORTS, AND OTHER SUBMITTALS PERTAINING TO STRUCTURAL WORK SHALL BE FORWARDED TO THE OWNER FOR REVIEW BEFORE FABRICATION AND/OR INSTALLATION IS MADE. SHOP DRAWINGS SHALL INCLUDE ERECTION DRAWINGS AND COMPLETE DETAILS OF CONNECTIONS AS WELL AS MANUFACTURER'S SPECIFICATION DATA WHERE APPROPRIATE. SHOP DRAWINGS SHALL BE CHECKED BY THE CONTRACTOR AND BEAR THE CHECKER'S INITIALS BEFORE BEING SUBMITTED FOR REVIEW.
- 13. NO DRILLING WELDING OR TAPING ON CL&P OWNED EQUIPMENT.
- REFER TO DRAWING T1 FOR ADDITIONAL NOTES AND REQUIREMENTS.

STRUCTURAL STEEL

- ALL STRUCTURAL STEEL IS DESIGNED BY ALLOWABLE STRESS DESIGN (ASD)
- STRUCTURAL STEEL (W SHAPES)---ASTM A992 (FY = 50 KSI) STRUCTURAL STEEL (OTHER SHAPES)---ASTM A36 (FY = 36 KSI) STRUCTURAL HSS (RECTANGULAR SHAPES) --- ASTM A500 GRADE B.
- (FY = 46 KSI)D. STRUCTURAL HSS (ROUND SHAPES) --- ASTM A500 GRADE B,
- (FY = 42 KSI)PIPE---ASTM A53 (FY = 35 KSI)
- CONNECTION BOLTS---ASTM A325-N
- U-BOLTS---ASTM A36 ANCHOR RODS---ASTM F 1554
- WELDING ELECTRODE --- ASTM E 70XX
- 2. CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO ENGINEER FOR APPROVAL. DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITTING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE THE FOLLOWING: SECTION PROFILES, SIZES, CONNECTION ATTACHMENTS, REINFORCING, ANCHORAGE, SIZE AND TYPE OF FASTENERS AND ACCESSORIES. INCLUDE ERECTION DRAWINGS, ELEVATIONS AND DETAILS.
- 3. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST PROVISIONS OF AISC MANUAL OF STEEL CONSTRUCTION.
- 4. PROVIDE ALL PLATES, CLIP ANGLES, CLOSURE PIECES, STRAP ANCHORS, MISCELLANEOUS PIECES AND HOLES REQUIRED TO COMPLETE THE STRUCTURE.
- 5. FIT AND SHOP ASSEMBLE FABRICATIONS IN THE LARGEST PRACTICAL SECTIONS FOR DELIVERY TO SITE.
- 6. INSTALL FABRICATIONS PLUMB AND LEVEL, ACCURATELY FITTED, AND FREE FROM DISTORTIONS OR DEFECTS.
- AFTER ERECTION OF STRUCTURES, TOUCHUP ALL WELDS, ABRASIONS AND NON-GALVANIZED SURFACES WITH A 95% ORGANIC ZINC RICH PAINT IN ACCORDANCE WITH ASTM 780.
- 8. ALL STEEL MATERIAL (EXPOSED TO WEATHER) SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT DIPPED GALVANIZED) COATINGS" ON IRONS AND STEEL PRODUCTS.
- 9. ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC COATING (HOT-DIP) ON IRON AND STEEL HARDWARE".
- 10. THE ENGINEER SHALL BE NOTIFIED OF ANY INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NON CONFORMING MATERIALS OR CONDITIONS TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER
- 11. CONNECTION ANGLES SHALL HAVE A MINIMUM THICKNESS OF 1/4 INCHES.
- 12. STRUCTURAL CONNECTION BOLTS SHALL CONFORM TO ASTM A325. ALL BOLTS SHALL BE 3/4" DIAMETER MINIMUM AND SHALL HAVE A MINIMUM OF TWO BOLTS, UNLESS OTHERWISE ON THE DRAWINGS.
- 13. LOCK WASHER ARE NOT PERMITTED FOR A325 STEEL ASSEMBLIES.
- SHOP CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED.
- 15. MILL BEARING ENDS OF COLUMNS, STIFFENERS, AND OTHER BEARING SURFACES TO TRANSFER LOAD OVER ENTIRE CROSS SECTION.
- 16. FABRICATE BEAMS WITH MILL CAMBER UP.
- 17. LEVEL AND PLUMB INDIVIDUAL MEMBERS OF THE STRUCTURE TO AN ACCURACY OF 1:500, BUT NOT TO EXCEED 1/4" IN THE FULL HEIGHT OF THE COLUMN.
- 18. COMMENCEMENT OF STRUCTURAL STEEL WORK WITHOUT NOTIFYING THE ENGINEER OF ANY DISCREPANCIES WILL BE CONSIDERED ACCEPTANCE OF PRECEDING WORK.
- 19. INSPECTION AND TESTING OF ALL WELDING AND HIGH STRENGTH BOLTING SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY.
- 20. FOUR COPIES OF ALL INSPECTION TEST REPORTS SHALL BE SUBMITTED TO THE ENGINEER WITHIN TEN (10) WORKING DAYS OF THE DATE OF INSPECTION.

PAINT NOTES

PAINTING SCHEDULE:

- 1. ANTENNA PANELS:
 - A. SHERWIN WILLIAMS POLANE-B B. COLOR TO BE MATCHED WITH EXISTING TOWER STRUCTURE.

2. <u>COAXIAL CABLES:</u>

- TWO COATS OF DTM ACRYLIC PRIMER/FINISH (2.5-5 MILS. DRY FINISH) COLOR TO BE FIELD MATCHED WITH EXISTING STRUCTURE.

EXAMINATION AND PREPARATION:

1. DO NOT APPLY PAINT IN SNOW, RAIN, FOG OR MIST OR WHEN RELATIVE HUMIDITY EXCEEDS 85%. DO NOT APPLY PAINT TO DAMP OR WET SURFACES.

A. ONE COAT OF DTM BONDING PRIMER (2-5 MILS. DRY FINISH)

- 2. VERIFY THAT SUBSTRATE CONDITIONS ARE READY TO RECEIVE WORK. EXAMINE SURFACE SCHEDULED TO BE FINISHED PRIOR TO COMMENCEMENT OF WORK, REPORT ANY CONDITION THAT MAY POTENTIALLY AFFECT PROPER APPLICATION.
- 3. TEST SHOP APPLIED PRIMER FOR COMPATIBILITY WITH SUBSEQUENT COVER MATERIALS.
- 4. PERFORM PREPARATION AND CLEANING PROCEDURE IN STRICT ACCORDANCE WITH COATING MANUFACTURER'S INSTRUCTIONS FOR EACH SUBSTRATE CONDITION.
- CORRECT DEFECTS AND CLEAN SURFACES WHICH AFFECT WORK OF THIS SECTION. REMOVE EXISTING COATINGS THAT EXHIBIT LOOSE SURFACE DEFECTS.
- IMPERVIOUS SURFACE: REMOVE MILDEW BY SCRUBBING WITH SOLUTION OF TRI-SODIUM PHOSPHATE AND BLEACH. RINSE WITH CLEAN WATER AND ALLOW SURFACE TO DRY.
- 7. ALUMINUM SURFACE SCHEDULED FOR PAINT FINISH: REMOVE SURFACE CONTAMINATION BY STEAM OR HIGH-PRESSURE WATER, REMOVE OXIDATION WITH ACID ETCH AND SOLVENT WASHING. APPLY ETCHING PRIMER IMMEDIATELY FOLLOWING CLEANING.
- 8. FERROUS METALS: CLEAN UNGALVANIZED FERROUS METAL SURFACES THAT HAVE NOT BEEN SHOP COATED; REMOVE OIL, GREASE, DIRT, LOOSE MILL SCALE, AND OTHER FOREIGN SUBSTANCES. USE SOLVENT OR MECHANICAL CLEANING METHODS THAT COMPLY WITH THE STEEL STRUCTURES PAINTING COUNCIL'S (SSPC) RECOMMENDATIONS. TOUCH UP BARE AREAS AND SHOP APPLIED PRIME COATS THAT HAVE BEEN DAMAGED. WIRE BRUSH, CLEAN WITH SOLVENTS RECOMMENDED BY PAINT MANUFACTURER, AND TOUCH UP WITH THE SAME PRIMER AS THE SHOP COAT.
- GALVANIZED SURFACES: CLEAN GALVANIZED SURFACES WITH NON-PETROLEUM-BASED SOLVENTS SO SURFACE IS FREE OF OIL AND SURFACE CONTAMINANTS. REMOVE PRETREATMENT FROM GALVANIZED SHEET METAL FABRICATED FROM COIL STOCK BY MECHANICAL METHODS.
- MATERIAL TO ENSURE ADEQUATE ADHESION. PANELS MUST BE WIPED WITH METHYL ETHYL KETONE (MEK).

10. ANTENNA PANELS: REMOVE ALL OIL, DUST, GREASE, DIRT, AND OTHER FOREIGN

11. COAXIAL CABLES: REMOVE ALL OIL, DUST, GREASE. DIRT, AND OTHER FOREIGN MATERIAL TO ENSURE ADEQUATE ADHESION.

CLEANING:

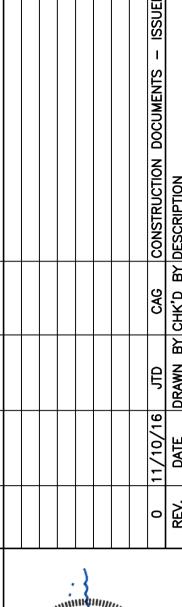
1. COLLECT WASTE MATERIAL, WHICH MAY CONSTITUTE A FIRE HAZARD, PLACE IN CLOSED METAL CONTAINERS AND REMOVE DAILY FROM SITE.

APPLICATION:

- 1. APPLY PRODUCTS IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
- 2. DO NOT APPLY FINISHES TO SURFACES THAT ARE NOT DRY.
- 3. APPLY EACH COAT TO UNIFORM FINISH.
- 4. APPLY EACH COAT OF PAINT SLIGHTLY DARKER THAN PRECEDING COAT UNLESS OTHERWISE APPROVED.
- 5. SAND METAL LIGHTLY BETWEEN COATS TO ACHIEVE REQUIRED FINISH.
- 6. VACUUM CLEAN SURFACES FREE OF LOOSE PARTICLES. USE TACK CLOTH JUST PRIOR TO APPLYING NEXT COAT.
- 7. ALLOW APPLIED COAT TO DRY BEFORE NEXT COAT IS APPLIED.

COMPLETED WORK:

- 1. SAMPLES: PREPARE 24" X 24" SAMPLE AREA FOR REVIEW.
- 2. MATCH APPROVED SAMPLES FOR COLOR, TEXTURE AND COVERAGE. REMOVE REFINISH OR REPAINT WORK NOT IN COMPLIANCE WITH SPECIFIED REQUIREMENTS.





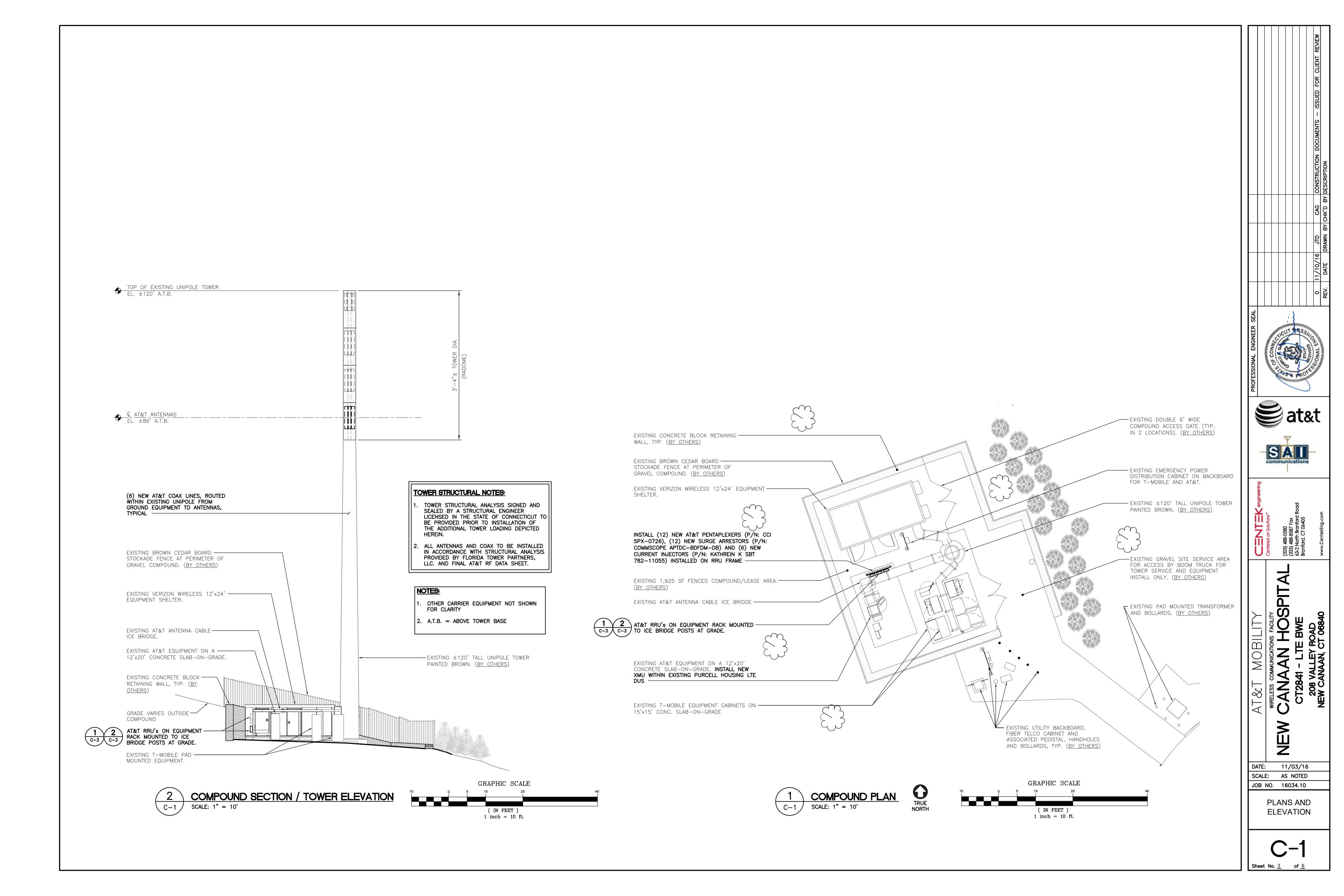


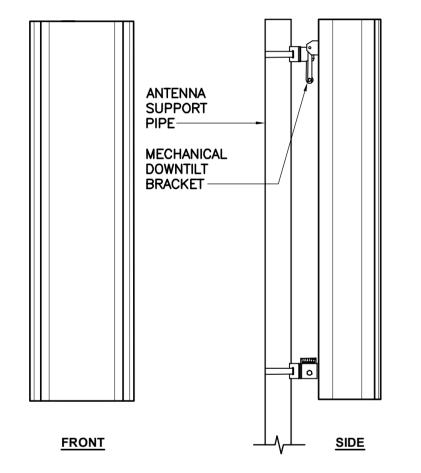


11/03/16 SCALE: AS NOTED JOB NO. 16034.10

NOTES AND **SPECIFICATIONS**



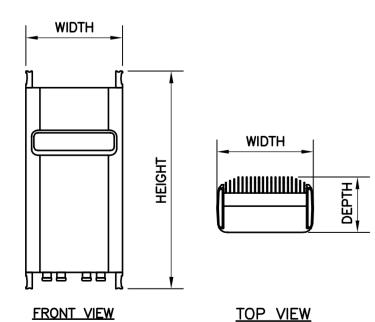






ALPH	IA/BETA/GAMMA ANTENNA		
EQUIPMENT	DIMENSIONS	WEIGHT	
MAKE: QUINTEL MODEL: QS66512-2	72.0"H × 12.0"W × 9.6"D	112.0-LBS	

5 PROPOSED ANTENNA DETAIL C-2 SCALE: 1/2" = 1'-0"

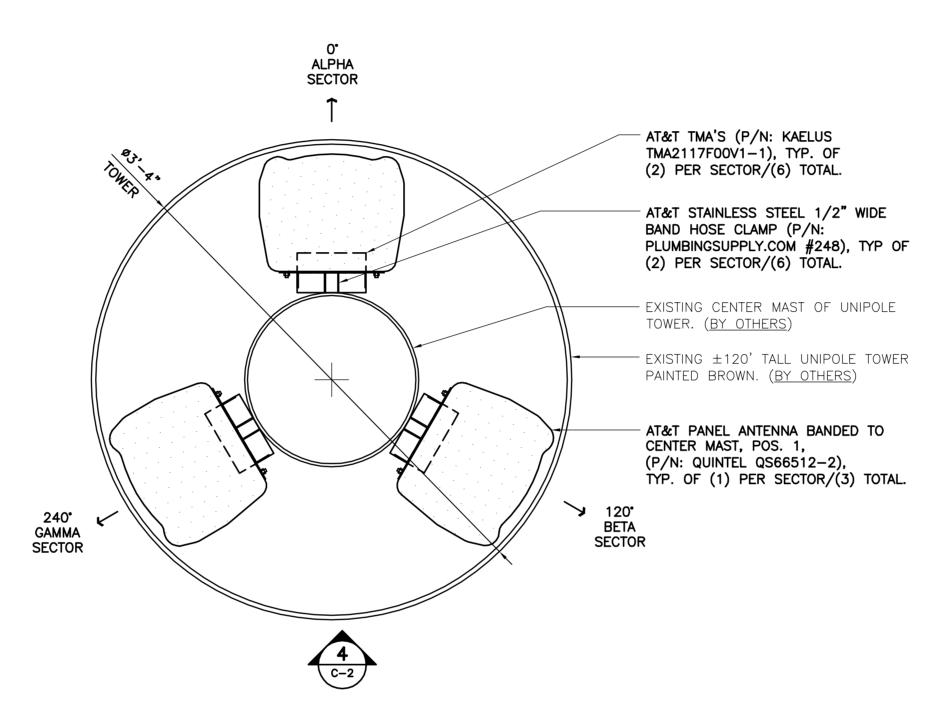


		<u> </u>	<u> </u>	
		RRU (REMOTE RADIO	UNIT)	
EQUIPMENT		DIMENSIONS	WEIGHT CLEARANCES	
 MAKE: MODEL:	ERICSSON RRUS-32 B2	27.17"H x 12.05"W x 7.01"D	52.91 LBS.	ABOVE: 16" MIN. BELOW: 12" MIN. FRONT: 36" MIN.

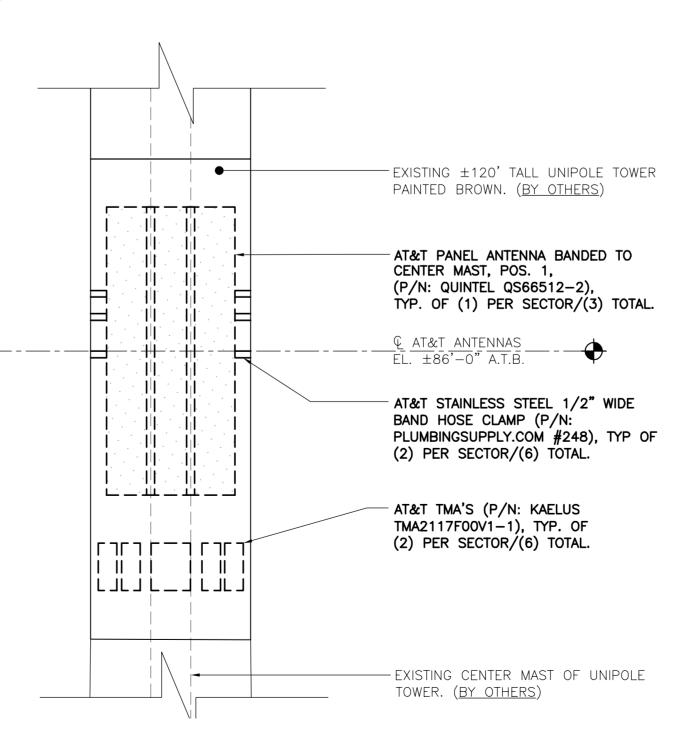
NOTES:

1. CONTRACTOR TO COORDINATE FINAL EQUIPMENT MODEL SELECTION WITH AT&T CONSTRUCTION MANAGER PRIOR TO ORDERING.



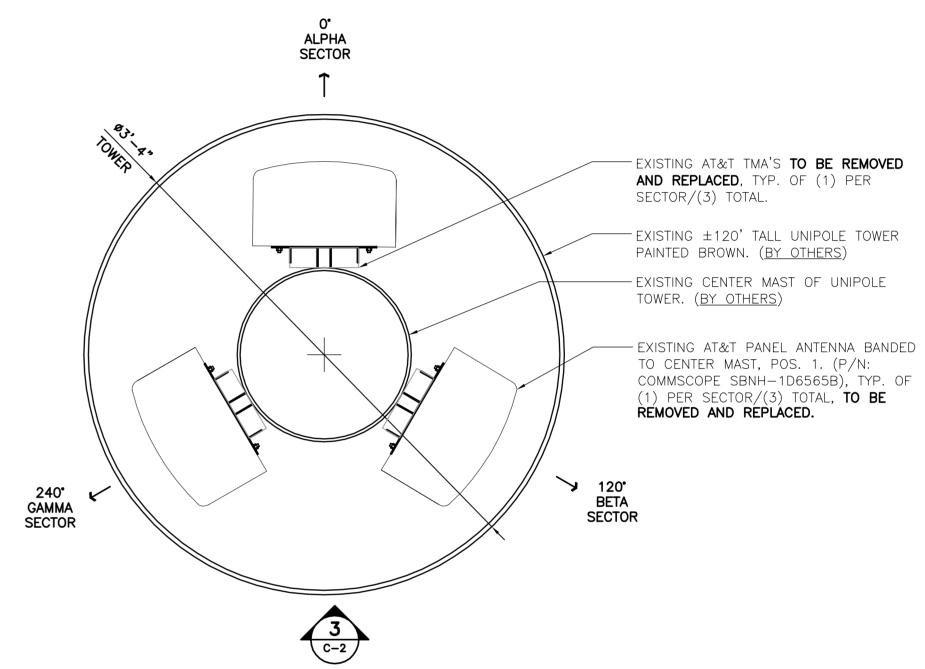


PROPOSED ANTENNA PLAN C-2 SCALE: 1 1/2" = 1'-0"

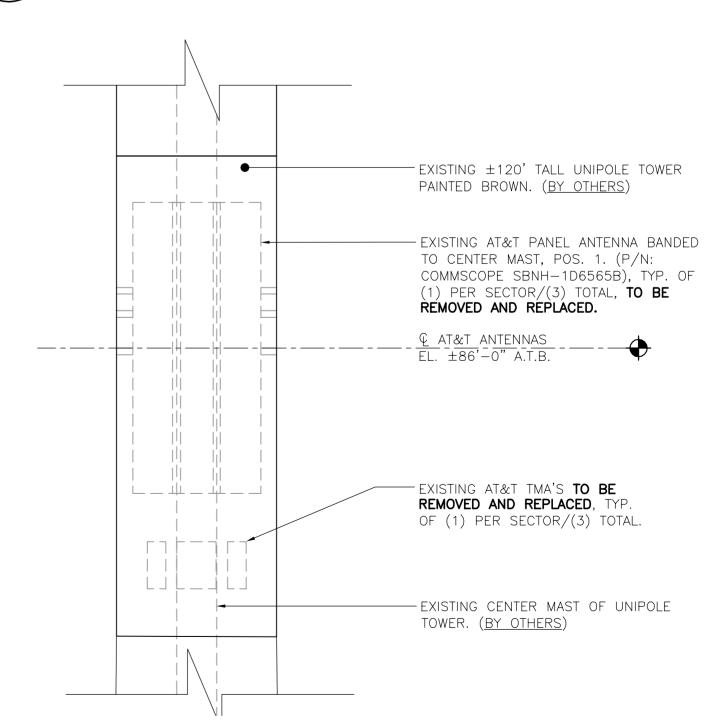


4 PROPOSED ANTENNA ELEVATION

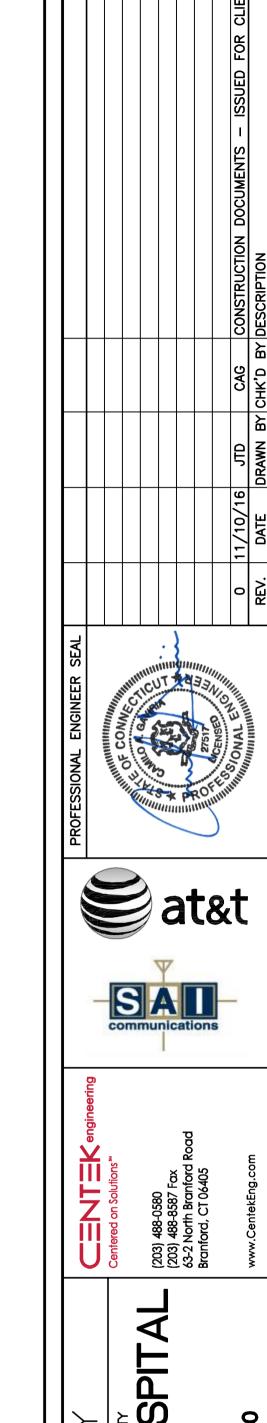
SCALE: 1/2" = 1'-0"



1 EXISTING ANTENNA PLAN C-2 SCALE: 1 1/2" = 1'-0"



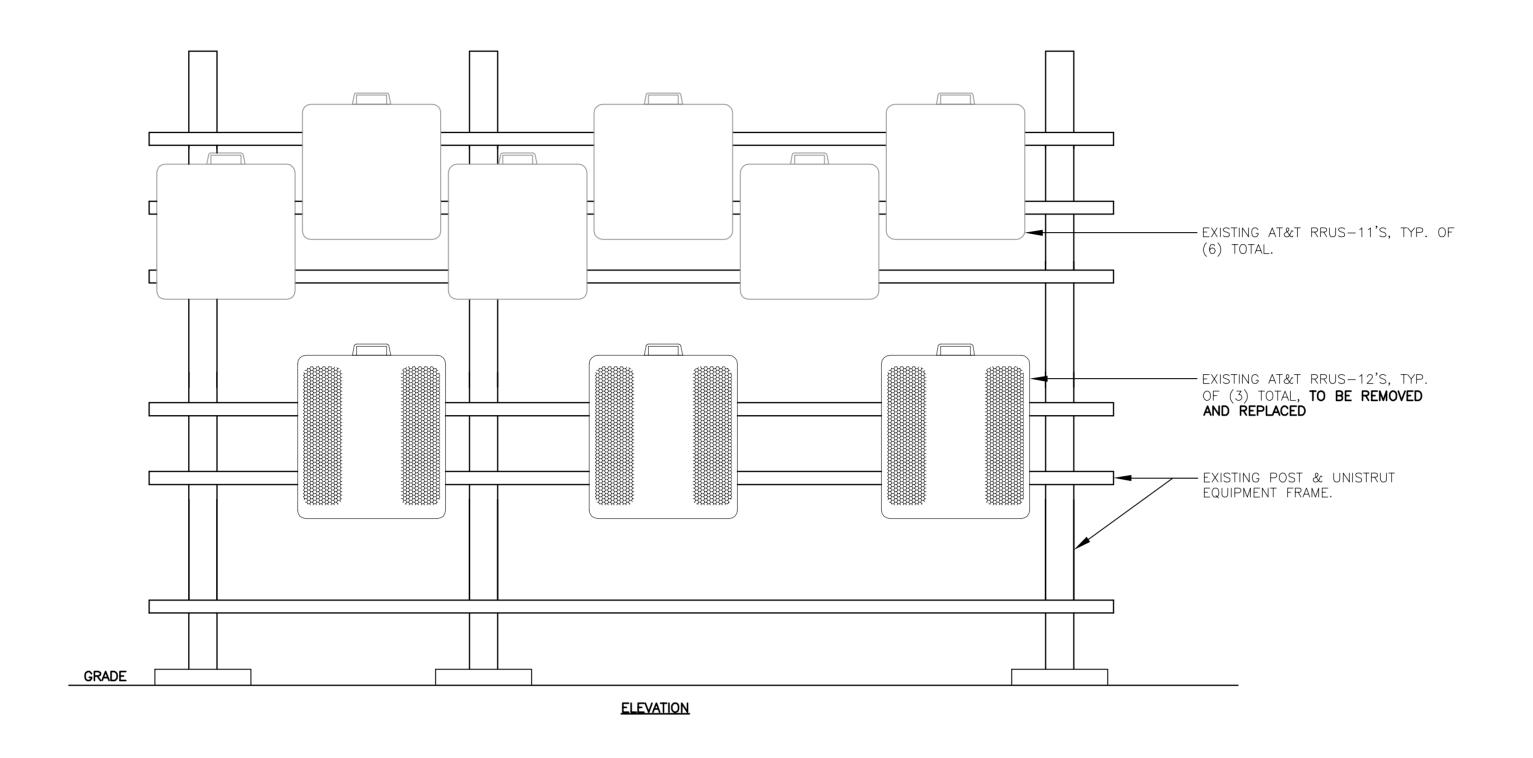




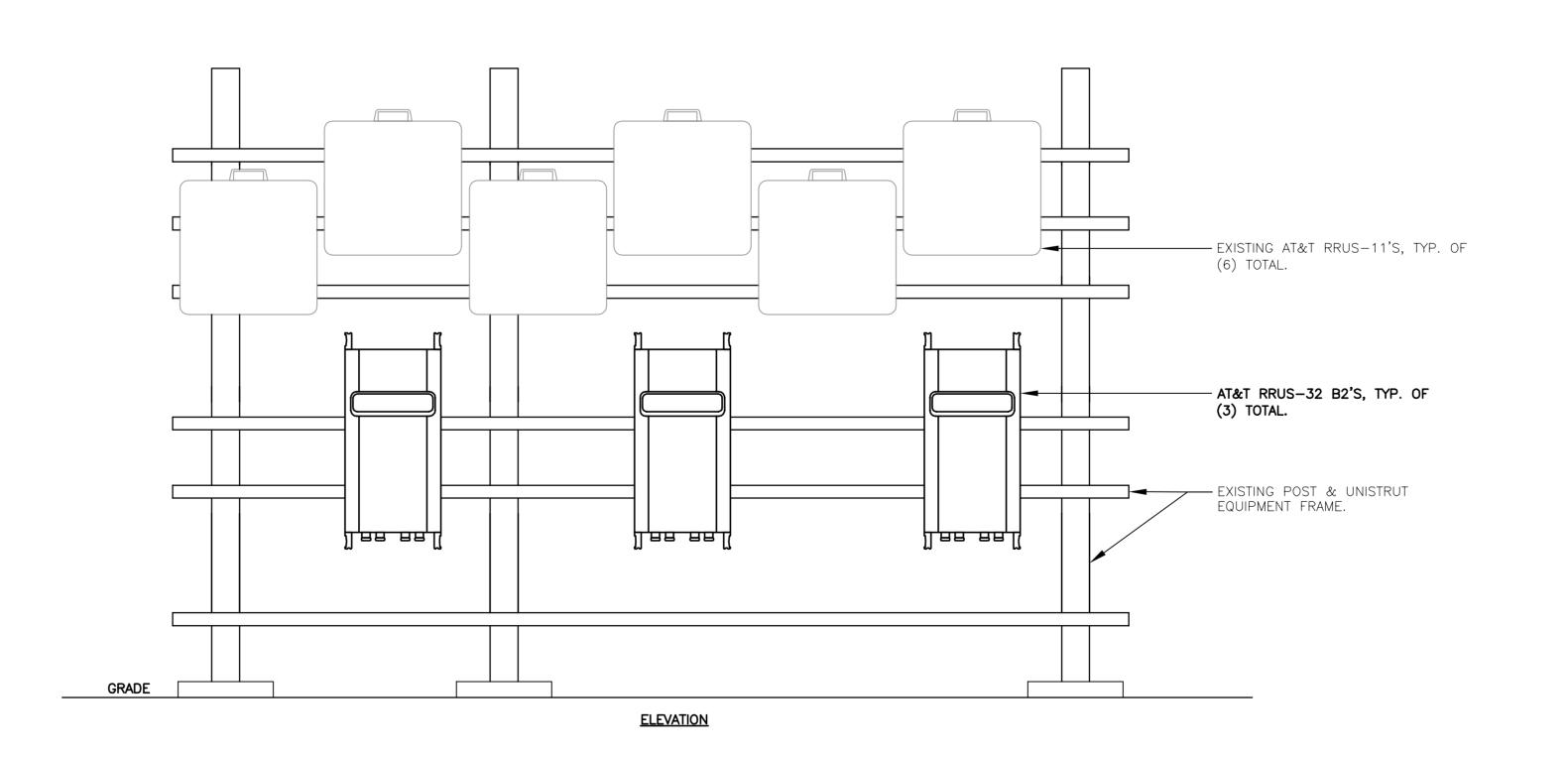
LTE BWE
EQUIPMENT
DETAILS

C-2

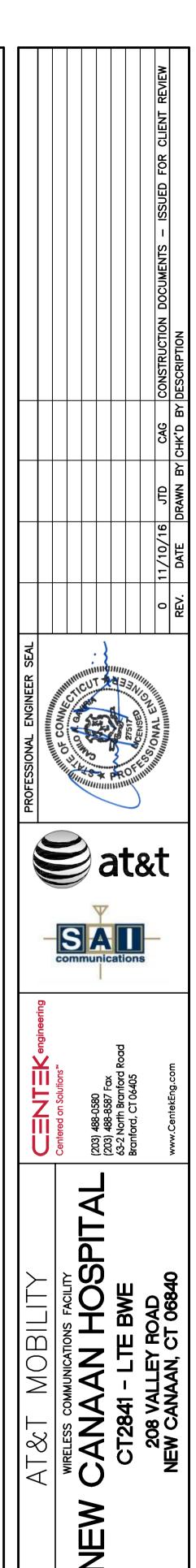
Sheet No. 4 of







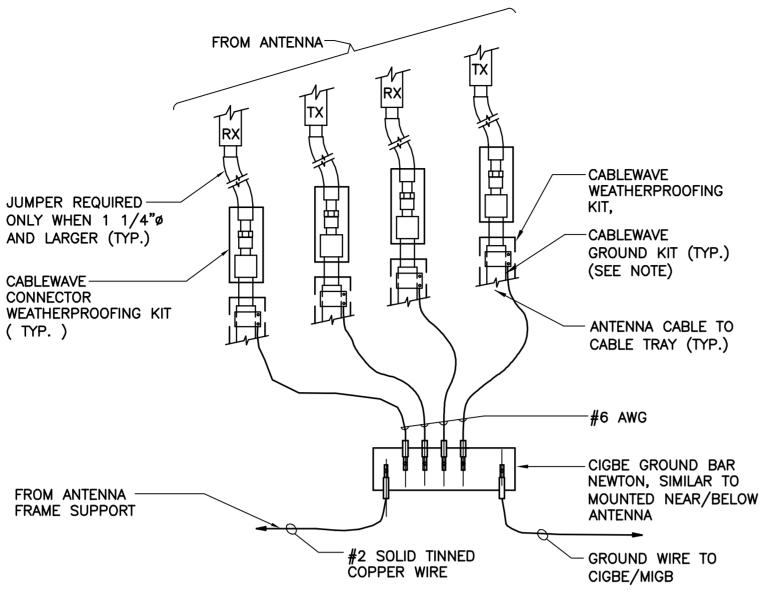




DATE: 11/03/16
SCALE: AS NOTED

JOB NO. 16034.10

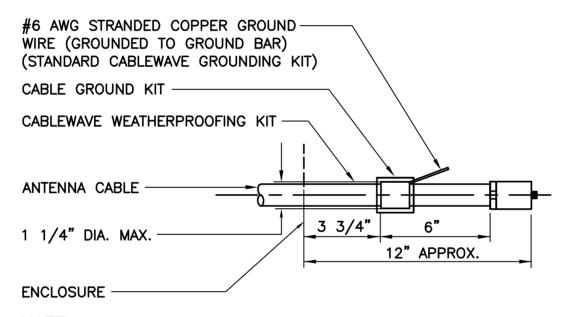
RRU MOUNTING CONFIGURATION



NOT

 DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO CIGBE

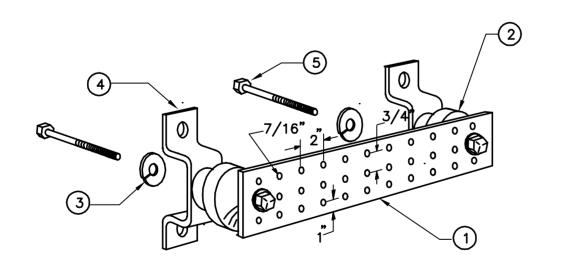
5 CONNECTION OF GROUND WIRES TO GROUND BAR E-1 NOT TO SCALE



NOTE:

 DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.

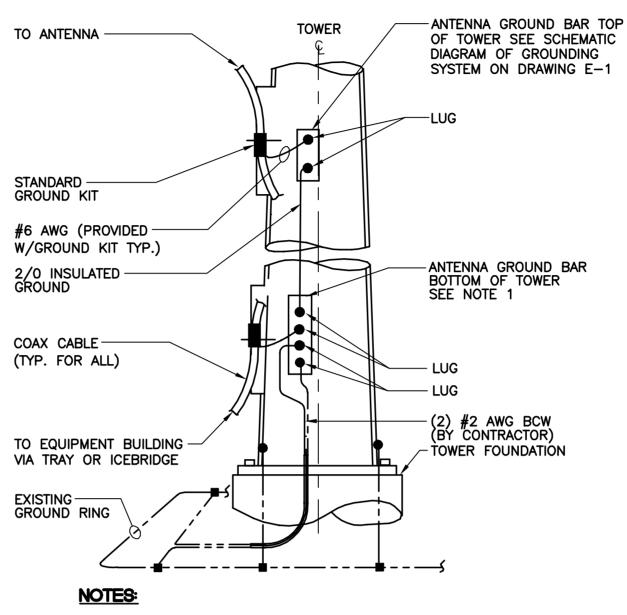




LEGEND

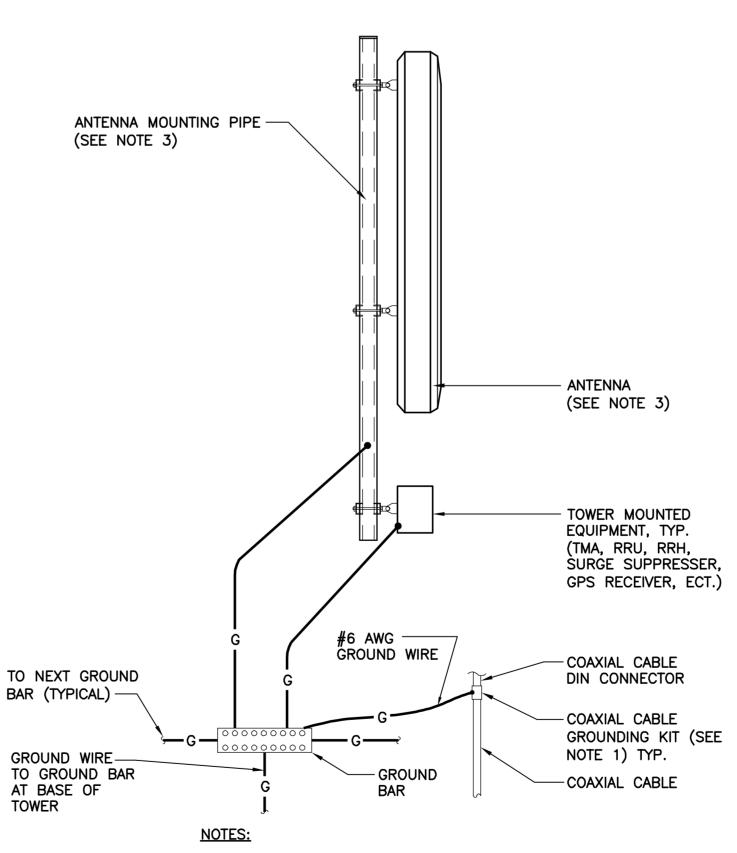
- 1. TINNED COPPER GROUND BAR, 1/4"x 4"x 20", NEWTON INSTRUMENT CO. HOLE CENTERS TO MATCH NEMA DOUBLE LUG.
- 2. INSULATORS, NEWTON INSTRUMENT CAT. NO. 2. 3061-4.
- 3. 3. 5/8" LOCK WASHERS, NEWTON INSTRUMENT CO. CAT. NO. 3015-8.
- 4. WALL MOUNTING BRACKET, NEWTON INSTRUMENT CO. 4. CAT NO. A-6056.
- 5. STAINLESS STEEL SECURITY SCREWS.



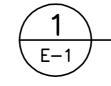


- 1. NUMBER OF GROUND BARS MAY VARY DEPENDING ON THE TYPE OF TOWER, LOCATION AND CONNECTION ORIENTATION. PROVIDE AS REQUIRED.
- 2. A SEPARATE GROUND BAR TO BE USED FOR GPS ANTENNA IF REQUIRED.





- 1. BOND COAXIAL CABLE GROUND KITS TO EACH OWNER'S GROUND BAR ALONG ENTIRE COAX RUN FROM ANTENNA TO SHELTER.
- 2. BOND ALL EQUIPMENT TO GROUND PER NEC AND MANUFACTURERS SPECIFICATIONS.
- 3. DETAIL IS TYPICAL FOR ALL ANTENNA SECTORS, INCLUDING GPS ANTENNA.



TYPICAL ANTENNA GROUNDING DETAIL

NOT TO SCALE

ELECTRICAL NOTES

- PRIOR TO START OF CONSTRUCTION CONTRACTOR SHALL COORDINATE WITH OWNER FOR ALL CONSTRUCTION STANDARDS AND SPECIFICATIONS, AND ALL MANUFACTURER DOCUMENTATION FOR ALL EQUIPMENT TO BE INSTALLED.
- 2. INSTALL ALL EQUIPMENT IN ACCORDANCE WITH LOCAL BUILDING CODE, NATIONAL ELECTRIC CODE, OWNER AND MANUFACTURER'S SPECIFICATIONS.
- 3. CONNECT ALL NEW EQUIPMENT TO EXISTING TELCO AS REQUIRED BY MANUFACTURER.
- 4. MAINTAIN ALL CLEARANCES REQUIRED BY NEC AND EQUIPMENT MANUFACTURER.
- 5. PRIOR TO INSTALLATION CONTRACTOR SHALL MEASURE EXISTING ELECTRICAL LOAD AND VERIFY EXISTING AVAILABLE CAPACITY FOR PROPOSED INSTALLATION. IF INADEQUATE CAPACITY IS AVAILABLE, CONTRACTOR SHALL COORDINATE WITH LOCAL ELECTRIC UTILITY COMPANY TO UPGRADE EXISTING ELECTRIC SERVICE.
- 6. CONTRACTOR SHALL INSPECT EXISTING GROUNDING AND LIGHTNING PROTECTION SYSTEM AND ENSURE THAT IT IS IN COMPLIANCE WITH NEC, AND SITE OWNER'S SPECIFICATIONS. THE RESULTS OF THIS INSPECTION SHALL BE PRESENTED TO OWNERS REPRESENTATIVE, AND ANY DEFICIENCIES SHALL BE CORRECTED.
- 7. ALL TRANSMISSION TOWER SITES CONTAIN AN EXTENSIVE BURIED GROUNDING SYSTEM. ALL GROUNDING WORK MUST BE COORDINATED WITH, AND APPROVED BY, THE TOWER OWNER'S SITE REPRESENTATIVE. ALL OF THE TOWER OWNER'S SPECIFICATIONS MUST BE STRICTLY FOLLOWED.
- 8. PROVIDE AND INSTALL GROUND KITS FOR ALL NEW COAXIAL CABLES AND BOND TO EXISTING OWNERS GROUNDING SYSTEM PER OWNERS SPECIFICATIONS AND NEC.
- 9. ALL CONDUCTORS SHALL BE TYPE THWN (INT. APPLICATION) AND XHHW (EXT. APPLICATION), 75 DEGREE C, 600 VOLT INSULATION, SOFT ANNEALED STRANDED COPPER. #10 AWG AND SMALLER SHALL BE SPLICED USING ACCEPTABLE SOLDERLESS PRESSURE CONNECTORS. #8 AWG AND LARGER SHALL BE SPLICED USING COMPRESSION SPLIT—BOLT TYPE CONNECTORS, #12 AWG SHALL BE THE MINIMUM SIZE CONDUCTOR FOR LINE VOLTAGE BRANCH CIRCUITS. REFER TO PANEL SCHEDULE FOR BRANCH CIRCUIT CONDUCTOR SIZE(S). CONDUCTORS SHALL BE COLOR CODED FOR CONSISTENT PHASE IDENTIFICATION:
- 10. MINIMUM BENDING RADIUS FOR CONDUCTORS SHALL BE 12 TIMES THE LARGEST DIAMETER OF BRANCH CIRCUIT CONDUCTOR.
- 11. THE ENTIRE ELECTRICAL INSTALLATION SHALL BE MADE IN STRICT ACCORDANCE WITH ALL LOCAL, STATE AND NATIONAL CODES AND REGULATIONS WHICH MAY APPLY AND NOTHING IN THE DRAWINGS OR SPECIFICATIONS SHALL BE INTERPRETED AS AN INFRINGEMENT OF SUCH CODES OR REGULATIONS.
- 12. THE ELECTRICAL CONTRACTOR IS TO BE RESPONSIBLE FOR THE COMPLETE INSTALLATION AND COORDINATION OF THE ENTIRE ELECTRICAL SERVICE. ALL ACTIVITIES TO BE COORDINATED THROUGH OWNER'S REPRESENTATIVE, DESIGN ENGINEER AND OTHER AUTHORITIES HAVING JURISDICTION OF TRADES.
- 13. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND PAY ALL FEES AS MAY BE REQUIRED FOR THE ELECTRICAL WORK AND FOR SCHEDULING OF ALL INSPECTIONS AS MAY BE REQUIRED BY THE LOCAL AUTHORITY.
- 14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION WITH THE SITE AND/OR BUILDING OWNER FOR NEW AND/OR DEMOLITION WORK INVOLVED.
- 15. THE CONTRACTOR SHALL GUARANTEE ALL NEW WORK FOR A PERIOD OF ONE YEAR FROM THE ACCEPTANCE DATE BY THE OWNER. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING WARRANTIES FROM ALL EQUIPMENT MANUFACTURERS FOR SUBMISSION TO THE OWNER.
- 16. DRAWINGS INDICATE GENERAL ARRANGEMENT OF WORK INCLUDED IN CONTRACT. CONTRACTOR SHALL WITHOUT EXTRA CHARGE, MAKE MODIFICATIONS TO THE LAYOUT OF THE WORK TO PREVENT CONFLICT WITH WORK OF OTHER TRADES AND FOR THE PROPER INSTALLATION OF WORK. CHECK ALL DRAWINGS AND VISIT JOB SITE TO VERIFY SPACE AND TYPE OF EXISTING CONDITIONS IN WHICH WORK WILL BE DONE, PRIOR TO SUBMITTAL OF BID.
- 17. ALL NON-CURRENT CARRYING PARTS OF THE ELECTRICAL AND TELEPHONE CONDUIT SYSTEMS SHALL BE MECHANICALLY AND ELECTRICALLY CONNECTED TO PROVIDE AN INDEPENDENT RETURN PATH TO THE EQUIPMENT GROUNDING SOURCES.
- 18. GROUNDING SYSTEM WILL BE IN ACCORDANCE WITH THE LATEST ACCEPTABLE EDITION OF THE NATIONAL ELECTRICAL CODE AND REQUIREMENTS PER LOCAL INSPECTOR HAVING JURISDICTION.
- 19. EACH EQUIPMENT GROUND CONDUCTOR SHALL BE SIZED IN ACCORDANCE WITH THE N.E.C. ARTICLE 250-122. (MIN. #12 AWG).
- 20. CONTRACTOR SHALL PROVIDE A CELLULAR GROUNDING SYSTEM WITH THE MAXIMUM AC RESISTANCE TO GROUND OF 5 OHM BETWEEN ANY POINT ON THE GROUNDING SYSTEM AS MEASURED BY 3-POINT GROUNDING TEST. (REFER TO SECTION 16960).

TESTS BY INDEPENDENT ELECTRICAL TESTING FIRM

EQUIPMENT.

A. CONTRACTOR SHALL RETAIN THE SERVICES OF A LOCAL INDEPENDENT ELECTRICAL TESTING FIRM (WITH MINIMUM 5 YEARS COMMERCIAL EXPERIENCE IN THE ELECTRICAL TESTING INDUSTRY) AS SPECIFIED BY OWNER TO PERFORM:

1. TESTING PROCEDURE INCLUDING THE MAKE AND MODEL OF TEST

- TEST 1: RESISTANCE TO GROUND TEST ON THE CELLULAR GROUNDING SYSTEM.
- THE TESTING FIRM SHALL INCLUDE THE FOLLOWING INFORMATION WITH THE REPORT:
- 2. CERTIFICATION OF TESTING EQUIPMENT CALIBRATION WITHIN SIX (6) MONTHS OF DATE OF TESTING. INCLUDE CERTIFICATION LAB ADDRESS AND TELEPHONE NUMBER.
- 3. GRAPHICAL DESCRIPTION OF TESTING METHOD ACTUALLY IMPLEMENTED.
- B. TESTING SHALL BE PERFORMED IN THE PRESENCE AND TO THE SATISFACTION OF OWNERS CONSTRUCTION REPRESENTATIVE. TESTING DATA SHALL BE INITIALED AND DATED BY THE CONSTRUCTION AND INCLUDED WITH THE WRITTEN REPORT ANALYSIS
- C. THE CONTRACTOR SHALL FORWARD SIX (6) COPIES OF THE INDEPENDENT ELECTRICAL TESTING FIRM REPORT/ANALYSIS TO ENGINEER A MINIMUM OF TEN (10) WORKING DAYS PRIOR TO THE JOB TURNOVER.
- D. CONTRACTOR TO PROVIDE A MINIMUM OF ONE (1) WEEK NOTICE TO OWNER AND ENGINEER FOR ALL TESTS REQUIRING WITNESSING.

ed on solutions 188-0580 188-8587 Fax orth Branford Road ord, CT 06405

PTAL (203) 488-(

ANAAN HOSP
CT2841 - LTE BWE
208 VALLEY ROAD

DATE: 11/03/16
SCALE: AS NOTED

TYPICAL ELECTRICAL DETAILS AND NOTES

JOB NO. 16034.10

E-1

Sheet No. 6 of 6

Structural Analysis 120-ft Monopole

Prepared For:
Tarpon Towers II, LLC
1001 3rd Ave. West, Suite 420
Bradenton, FL 34205

MFP Project #40916-151

Site Location:
CT1192 New Canaan
Fairfield Co., Connecticut
Lat/Long: 41°9'58.5", -73°28'13.7"

Analysis Type:
ANSI/TIA-222-G-2
Structure Rating - 68.8% Passing

November 21, 2016



Michael F. Plahovinsak, P.E. 1830| State Route 161 W, Plain City, 0H 43064 614-398-6250 - mike@mfpeng.com Page 2 of 5 11/21/2016

Project Summary:

I have completed a structural analysis of the existing monopole for the following new configuration:

- 86' AT&T:
 - o (3) Quintel QS66512-2 Antenna + (6) Kaelus TMA2117F00V1-1
 - o (12) 1 ¼" Cable

The pole has been analyzed in accordance with the requirements of the International Building Code per IBC section 3108.4, and the recommendations of the Telecommunications Industry Association "Structural Standard for Steel Antenna Supporting Structures" ANSI/TIA-222-G.

This analysis may be considered a "Rigorous Structural Analysis" as defined in ANSI/TIA-222-G 15.5.2.

As indicated in the conclusions of this analysis, I have determined that the existing pole and foundation have *sufficient capacity* to support the existing, reserved and proposed antenna loads as detailed herein. Based on the results of my analysis, structural modifications are not required at this time.

Source of Data:

Resource	Source	Job Number	Date
Pole and Foundation Drawings	Michael Plahovinsak, PE	23514-110	04/05/14
Geotechnical Report	Design Earth Technology	2012.06/2011.08	06/01/12

Structure Specifics:

• Manufacturer: TransAmerican Power Products

Manufacturer File #: TP-12359Year Built: 2014

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Analysis Criteria:

International Building Code 2006-2012 Section 3108.4 Structural Standards for Steel Antenna Supporting Structures ANSI/TIA-222-G 2

• TIA-222-G Wind Speed 110 mph (V_{asd} / 3-Second Gust) • TIA-222-G Wind w/ 3/4" Ice 50 mph (3-Sec Gust) • Operational Wind Speed 60 mph (3-Sec Gust)

Structure Class Exposure Category Topographic Category В

Ι

Appurtenance Listing:

II (I = 1.0)

Status	Elev.	Antenna / Mounting	Coax	Owner
Existing	117'	(3) Commscope SBNHH-1D65A Antenna	(18) 1 5/8"	T-Mobile
Existing	117	Internally Mounted		1-MODIIC
	106'	(3) Commscope LNX-6514DS-VTM Antenna		
Existing	100	Internally Mounted	(12) 1 1/4"	Verizon
Existing	06'	(3) Commscope HBX-6517DS-VTM Antenna	(12) 1 1/4	VEHZOH
	96'	Internally Mounted		
Duonosad	86'	(3) Quintel QS66512-2 Antenna + (6) TMA2117F00V1-1	(12) 1 1/4''	AT&T
Proposed	00	Internally Mounted	(12) 1 1/4	AI&I

All antenna lines assumed internally mounted, not exposed to the wind.

Page 4 of 5 11/21/2016

Foundation Analysis:

The existing monopole foundation design was analyzed in conjunction with site specific geotechnical report. The existing foundation has sufficient capacity to support the pole with the proposed antenna configuration.

Conclusion:

I have completed a structural analysis of the existing monopole and foundation in accordance with the project specifics outlined above. My analysis indicates that the existing monopole and foundation are structurally adequate when considering the existing plus proposed loading. Please refer to the attached calculations for an itemized listing of all member stress ratios. The existing pole is safe and adequate to support the proposed loads, and no structural reinforcing is required to support the above loading.

If you have any questions about the contents of this structural report or require any additional information, please feel free to contact my office.

Sincerely,

Michael F. Plahovinsak. P.E.

mike@mfpeng.com - 614.398-6250

Page 5 of 5 11/21/2016

Standard Conditions for Providing Structural Consulting Services on Existing Structures

- 1. The following standard conditions are a general overview of key issues regarding the work product supplied.
- 2. If the existing conditions are not as represented in this structural report or attached sketches, I should be contacted to evaluate the significance of the deviation and revise the structural assessment accordingly.
- 3. The structural analysis has been performed assuming that the structure is in "like new" condition. No allowance was made for excessive corrosion, damaged or missing structural members, loose bolts, etc. If there are any known deficiencies in the structure that potentially compromise structural integrity, I should be made aware of the deficiencies. If I am aware of a deficiency that exists in a structure at the time of my analysis, a general explanation of the structural concern due to the deficiency will be included in the structural report, but the deficiency will not be reflected in capacity calculations.
- 4. The structural analysis provided is an assessment of the primary load carrying capacity of the structure. I provide a limited scope of service in that I have not verified the capacity of every weld, plate, connection detail, etc. In most cases, structural fabrication details are unknown at the time of my analysis, and the detailed field measurement of this information is beyond the scope of my services. In instances where I have not performed connection capacity calculations, it is assumed that existing manufactured connections develop the full capacity of the primary members being connected.
- 5. The structural integrity of the existing foundation system can only be verified if exact foundation sizes and soils conditions are known. I will not accept any responsibility for the adequacy of the existing foundations unless this site-specific data is supplied.
- 6. Miscellaneous items such as antenna mounts, coax supports, etc. have not been designed, detailed, or specified as part of my work. It is assumed that material of adequate size and strength will be purchased from a reputable component manufacturer. The attached report and sketches are schematic in nature and should not be used to fabricate or purchase hardware and accessories to be attached to the structure. I recommend field measurement of the structure before fabricating or purchasing new hardware and accessories. I am not responsible for proper fit and clearance of hardware and accessory items in the field.
- 7. The structural analysis has been performed considering minimum code requirements or recommendations. If alternate wind, ice, or deflection criteria are to be considered, then I shall be made aware of the alternate criteria.

C	c		r	-	L		·	c	•
Section	œ		,	٥	ი	4	ກ	7	-
Length (ft)	38.00	/	38.00	0.50	9.50	10.00	10.00	10.00	10.00
Number of Sides	18		18	— <u>æ</u>	18	18	18	18	18
Thickness (in)	0.2500		0.2500	0.2188	0.2188	0.2188	0.2188	0.2188	0.2188
Socket Length (ft)			6.00						
Top Dia (in)	43.8453		40.0000	14.0000	14.0000	14.0000	14.0000	14.0000	14.0000
Bot Dia (in)	49.0000		45.1600	40.0000	14.0000	14.0000	14.0000	14.0000	14.0000
Grade			A572-65						
Weight (K) 10.7	7.		4.3	0.0	0.3	0.3	0.3	0.3	0.3
	<u>0.0 ft</u>	<u>32.0 ft</u>		70.5 ft	<u>00.0 11</u>	80.0 ft	90.0 ft	100.0 ft	110.0 ft
REACTIONS - 110 mph WIND	ALL REACTIONS ARE FACTORED AXIAL 37 K SHEAR 3 K MOMENT 166 kip-ft 50 mph WIND - 0.7500 in ICE AXIAL 21 K SHEAR 741 kip-ft PEACTIONS 110 mph WIND								

DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION	
(3) Andrew SBNHH-1D65A w/ mount	117	Radome Cylinder (40"Ø x 10')	95	
pipe (T-Mobile)		(3) Quintel QS66512-2 Panel w/ mount	86	
Radome Cylinder (40"Ø x 10')	115	pipe (ATT)		
(3) Andrew LNX-6514DS w/ mount pipe	106	(6) Kaelus TMA2117F00V1-1 (ATT)	86	
(Verizon)		Radome Cylinder (40"Ø x 10')	85	
Radome Cylinder (40"Ø x 10')	105	Radome Cylinder (40"Ø x 10')	75	
(3) Andrew HBX-6517DS-VTM w/ mount pipe (Verizon)	96			

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu	
Δ572-65	65 ksi	8∩ ksi				

TOWER DESIGN NOTES

- 1. Tower is located in Fairfield County, Connecticut.
 2. Tower designed for Exposure B to the TIA-222-G Standard.
 3. Tower designed for a 110 mph basic wind in accordance with the TIA-222-G Standard.
 4. Tower is also designed for a 50 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
 5. Deflections are based upon a 60 mph wind.
 6. Tower Structure Class II.
 7. Topographic Category 1 with Crest Height of 0.00 ft.

- 7. Topographic Category 1 with Crest Height of 0.00 ft8. TOWER RATING: 68.8%

Michael F. Plahovinsak, P.E.	^{Job:} 120-ft Monopole - MFP #40916-151
18301 State Route 161	Project: CT1192 New Canaan
Plain City, OH 43064	Client: Tarpon Towers Drawn by: Mike App'd:
Phone: 614-398-6250	Code: TIA-222-G Date: 11/21/16 Scale: NTS
FAX: mike@mfneng.com	Path: Dwg No. F-1

Michael F. Plahovinsak, P.E.

18301 State Route 161 Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com

Job		Page
	120-ft Monopole - MFP #40916-151	1 of 7
Project		Date
	CT1192 New Canaan	16:18:48 11/21/16
Client	T T	Designed by
	Tarpon Towers	Mike

Tower Input Data

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Fairfield County, Connecticut.

Basic wind speed of 110 mph.

Structure Class II.

Exposure Category B.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 0.7500 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Tapered Pole Section Geometry

Section	Elevation	Section	Splice	Number	Тор	Bottom	Wall	Bend	Pole Grade
		Length	Length	of	Diameter	Diameter	Thickness	Radius	
	ft	ft	ft	Sides	in	in	in	in	
L1	120.00-110.00	10.00	0.00	18	14.0000	14.0000	0.2188	0.8750	A572-65
									(65 ksi)
L2	110.00-100.00	10.00	0.00	18	14.0000	14.0000	0.2188	0.8750	A572-65
									(65 ksi)
L3	100.00-90.00	10.00	0.00	18	14.0000	14.0000	0.2188	0.8750	A572-65
									(65 ksi)
L4	90.00-80.00	10.00	0.00	18	14.0000	14.0000	0.2188	0.8750	A572-65
									(65 ksi)
L5	80.00-70.50	9.50	0.00	18	14.0000	14.0000	0.2188	0.8750	A572-65
									(65 ksi)
L6	70.50-70.00	0.50	0.00	18	14.0000	40.0000	0.2188	0.8750	A572-65
									(65 ksi)
L7	70.00-32.00	38.00	6.00	18	40.0000	45.1600	0.2500	1.0000	A572-65
									(65 ksi)
L8	32.00-0.00	38.00		18	43.8453	49.0000	0.2500	1.0000	A572-65
									(65 ksi)

Tapered Pole Properties

Section	Tip Dia.	Area	I	r	С	I/C	J_{\perp}	It/Q	w	w/t
	in	in ²	in ⁴	in	in	in ³	in ⁴	in ²	in	
L1	14.2160	9.5685	229.5928	4.8923	7.1120	32.2825	459.4877	4.7852	2.0790	9.504
	14.2160	9.5685	229.5928	4.8923	7.1120	32.2825	459.4877	4.7852	2.0790	9.504
L2	14.2160	9.5685	229.5928	4.8923	7.1120	32.2825	459.4877	4.7852	2.0790	9.504
	14.2160	9.5685	229.5928	4.8923	7.1120	32.2825	459.4877	4.7852	2.0790	9.504
L3	14.2160	9.5685	229.5928	4.8923	7.1120	32.2825	459.4877	4.7852	2.0790	9.504
	14.2160	9.5685	229.5928	4.8923	7.1120	32.2825	459.4877	4.7852	2.0790	9.504
L4	14.2160	9.5685	229.5928	4.8923	7.1120	32.2825	459.4877	4.7852	2.0790	9.504
	14.2160	9.5685	229.5928	4.8923	7.1120	32.2825	459.4877	4.7852	2.0790	9.504
L5	14.2160	9.5685	229.5928	4.8923	7.1120	32.2825	459.4877	4.7852	2.0790	9.504

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Section	Tip Dia.	Area	I	r	С	I/C	J	It/Q	w	w/t
	in	in^2	in^4	in	in	in^3	in^4	in^2	in	
	14.2160	9.5685	229.5928	4.8923	7.1120	32.2825	459.4877	4.7852	2.0790	9.504
L6	14.2160	9.5685	229.5928	4.8923	7.1120	32.2825	459.4877	4.7852	2.0790	9.504
	40.6171	27.6206	5522.3981	14.1223	20.3200	271.7716	11052.0627	13.8129	6.6550	30.423
L7	40.6171	31.5416	6296.4503	14.1113	20.3200	309.8647	12601.1856	15.7738	6.6000	26.4
	45.8567	35.6361	9080.5791	15.9430	22.9413	395.8183	18173.1067	17.8214	7.5082	30.033
L8	45.3481	34.5928	8306.1982	15.4763	22.2734	372.9202	16623.3258	17.2997	7.2768	29.107
	49.7559	38.6831	11614.7065	17.3062	24.8920	466.6040	23244.6960	19.3452	8.1840	32.736

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Component Type	Placement	Total Number		C_AA_A	Weight
	Leg		- 1	ft			ft²/ft	plf
1 5/8"	С	No	Inside Pole	117.00 - 0.00	18	No Ice	0.00	0.92
(T-Mobile)						1/2" Ice	0.00	0.92
						1" Ice	0.00	0.92
1 1/4"	C	No	Inside Pole	106.00 - 0.00	6	No Ice	0.00	0.66
(Verizon)						1/2" Ice	0.00	0.66
						1" Ice	0.00	0.66
1 1/4"	C	No	Inside Pole	96.00 - 0.00	6	No Ice	0.00	0.66
(Verizon)						1/2" Ice	0.00	0.66
						1" Ice	0.00	0.66
1 1/4"	C	No	Inside Pole	86.00 - 0.00	6	No Ice	0.00	0.66
(AT&T)						1/2" Ice	0.00	0.66
						1" Ice	0.00	0.66

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	0	ft		ft ²	ft ²	K
Radome Cylinder (40'Ø x 10')	С	None	J.	0.0000	115.00	No Ice 1/2" Ice 1" Ice	26.67 27.56 28.47	26.67 27.56 28.47	0.50 0.79 1.09
Radome Cylinder (40''Ø x 10')	С	None		0.0000	105.00	No Ice 1/2" Ice 1" Ice	26.67 27.56 28.47	26.67 27.56 28.47	0.50 0.79 1.09
Radome Cylinder (40'Ø x 10')	С	None		0.0000	95.00	No Ice 1/2" Ice	26.67 27.56	26.67 27.56	0.50 0.79
Radome Cylinder (40''Ø x 10')	C	None		0.0000	85.00	1" Ice No Ice 1/2" Ice	28.47 26.67 27.56	28.47 26.67 27.56	1.09 0.50 0.79
Radome Cylinder (40"Ø x 10') **	С	None		0.0000	75.00	1" Ice No Ice 1/2" Ice 1" Ice	28.47 26.67 27.56 28.47	28.47 26.67 27.56 28.47	1.09 0.50 0.79 1.09
(3) Andrew SBNHH-1D65A w/ mount pipe (T-Mobile) **	С	None		0.0000	117.00	No Ice 1/2" Ice 1" Ice	6.46 6.93 7.40	5.05 5.72 6.43	0.05 0.10 0.16
(3) Andrew LNX-6514DS w/ mount pipe (Verizon)	С	None		0.0000	106.00	No Ice 1/2" Ice 1" Ice	8.41 8.96 9.52	6.83 7.79 8.62	0.06 0.12 0.20

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	0	ft		ft²	ft ²	K
**									
(3) Andrew	C	None		0.0000	96.00	No Ice	5.24	4.73	0.04
HBX-6517DS-VTM w/						1/2" Ice	5.71	5.68	0.08
mount pipe (Verizon) **						1" Ice	6.18	6.50	0.13
(3) Quintel QS66512-2 Panel	C	None		0.0000	86.00	No Ice	8.40	8.22	0.13
w/ mount pipe						1/2" Ice	8.95	9.19	0.20
(ATT)						1" Ice	9.51	10.09	0.28
(6) Kaelus	C	None		0.0000	86.00	No Ice	1.17	0.23	0.03
TMA2117F00V1-1						1/2" Ice	1.32	0.31	0.04
(ATT)						1" Ice	1.48	0.39	0.05

Load Combinations

Comb.	Description
No.	
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 90 deg - No Ice
5	0.9 Dead+1.6 Wind 90 deg - No Ice
6	1.2 Dead+1.6 Wind 180 deg - No Ice
7	0.9 Dead+1.6 Wind 180 deg - No Ice
8	1.2 Dead+1.0 Ice+1.0 Temp
9	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
10	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
11	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
12	Dead+Wind 0 deg - Service
13	Dead+Wind 90 deg - Service
14	Dead+Wind 180 deg - Service

Maximum Member Forces

Section	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
No.	ft	Туре		Load		Moment	Moment
				Comb.	K	kip-ft	kip-ft
L1	120 - 110	Pole	Max Tension	2	0.00	0.00	-0.00
			Max. Compression	8	-3.30	0.00	0.00
			Max. Mx	4	-1.24	-5.81	0.00
			Max. My	2	-1.24	0.00	5.81
			Max. Vy	4	1.16	-5.81	0.00
			Max. Vx	2	-1.16	0.00	5.81
L2	110 - 100	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-6.87	0.00	0.00
			Max. Mx	4	-2.61	-23.07	0.00
			Max. My	2	-2.61	0.00	23.07
			Max. Vy	4	2.29	-23.07	0.00
			Max. Vx	2	-2.29	0.00	23.07
L3	100 - 90	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-10.13	0.00	0.00
			Max. Mx	4	-3.98	-51.36	0.00
			Max. My	2	-3.98	0.00	51.36

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Section	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
No.	ft	Туре		Load		Moment	Moment
				Comb.	K	kip-ft	kip-ft
			Max. Vy	4	3.36	-51.36	0.00
			Max. Vx	2	-3.36	0.00	51.36
L4	90 - 80	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-14.46	0.00	0.00
			Max. Mx	4	-5.98	-90.10	0.00
			Max. My	2	-5.98	0.00	90.10
			Max. Vy	4	4.37	-90.10	0.00
			Max. Vx	2	-4.37	0.00	90.10
L5	80 - 70.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-17.04	0.00	0.00
			Max. Mx	4	-7.36	-135.37	0.00
			Max. My	2	-7.36	0.00	135.37
			Max. Vy	4	5.19	-135.37	0.00
			Max. Vx	2	-5.19	0.00	135.37
L6	70.5 - 70	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-17.12	0.00	0.00
			Max. Mx	4	-7.42	-137.97	0.00
			Max. My	2	-7.42	0.00	137.97
			Max. Vy	4	5.22	-137.97	0.00
			Max. Vx	2	-5.22	0.00	137.97
L7	70 - 32	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-25.29	0.00	0.00
			Max. Mx	4	-12.85	-356.05	0.00
			Max. My	2	-12.85	0.00	356.05
			Max. Vy	4	8.39	-356.05	0.00
			Max. Vx	2	-8.39	0.00	356.05
L8	32 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-36.80	0.00	0.00
			Max. Mx	4	-20.78	-740.65	0.00
			Max. My	2	-20.78	0.00	740.65
			Max. Vy	4	11.84	-740.65	0.00
			Max. Vx	2	-11.84	0.00	740.65

Maximum Tower Deflections - Service Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	120 - 110	6.221	13	0.6008	0.0000
L2	110 - 100	4.965	13	0.5974	0.0000
L3	100 - 90	3.736	13	0.5705	0.0000
L4	90 - 80	2.608	13	0.4970	0.0000
L5	80 - 70.5	1.702	13	0.3550	0.0000
L6	70.5 - 70	1.197	13	0.1381	0.0000
L7	70 - 32	1.182	13	0.1375	0.0000
L8	38 - 0	0.404	13	0.0898	0.0000

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Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	0	ft
117.00	(3) Andrew SBNHH-1D65A w/	13	5.844	0.6006	0.0000	134324
	mount pipe					
115.00	Radome Cylinder (40"Ø x 10')	13	5.592	0.6003	0.0000	134324
106.00	(3) Andrew LNX-6514DS w/ mount	13	4.467	0.5914	0.0000	24747
	pipe					
105.00	Radome Cylinder (40"Ø x 10')	13	4.343	0.5891	0.0000	21391
96.00	(3) Andrew HBX-6517DS-VTM w/	13	3.267	0.5454	0.0000	8485
	mount pipe					
95.00	Radome Cylinder (40"Ø x 10')	13	3.153	0.5380	0.0000	7834
86.00	(3) Quintel QS66512-2 Panel w/	13	2.210	0.4589	0.0000	4244
	mount pipe					
85.00	Radome Cylinder (40"Ø x 10')	13	2.117	0.4468	0.0000	3994
75.00	Radome Cylinder (40"Ø x 10')	13	1.383	0.2128	0.0000	2554

Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	120 - 110	37.682	2	3.6448	0.0000
L2	110 - 100	30.062	2	3.6244	0.0000
L3	100 - 90	22.610	2	3.4606	0.0000
L4	90 - 80	15.773	2	3.0139	0.0000
L5	80 - 70.5	10.282	2	2.1511	0.0000
L6	70.5 - 70	7.224	2	0.8343	0.0000
L7	70 - 32	7.137	2	0.8308	0.0000
L8	38 - 0	2.437	2	0.5421	0.0000

Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	0	ft
117.00	(3) Andrew SBNHH-1D65A w/	2	35.391	3.6440	0.0000	22255
	mount pipe					
115.00	Radome Cylinder (40"Ø x 10')	2	33.865	3.6419	0.0000	22255
106.00	(3) Andrew LNX-6514DS w/ mount	2	27.042	3.5881	0.0000	4100
	pipe					
105.00	Radome Cylinder (40"Ø x 10')	2	26.294	3.5741	0.0000	3544
96.00	(3) Andrew HBX-6517DS-VTM w/	2	19.766	3.3084	0.0000	1405
	mount pipe					
95.00	Radome Cylinder (40"Ø x 10')	2	19.075	3.2634	0.0000	1297
86.00	(3) Quintel QS66512-2 Panel w/	2	13.362	2.7821	0.0000	701
	mount pipe					
85.00	Radome Cylinder (40"Ø x 10')	2	12.799	2.7087	0.0000	660
75.00	Radome Cylinder (40"Ø x 10')	2	8.353	1.2879	0.0000	421

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Pole Design Da	ata
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Section No.	Elevation	Size	L	L_u	Kl/r	A	P_u	ϕP_n	Ratio P_u
	ft		ft	ft		in^2	K	K	ϕP_n
L1	120 - 110 (1)	TP14x14x0.2188	10.00	0.00	0.0	9.5685	-1.24	710.89	0.002
L2	110 - 100 (2)	TP14x14x0.2188	10.00	0.00	0.0	9.5685	-2.61	710.89	0.004
L3	100 - 90 (3)	TP14x14x0.2188	10.00	0.00	0.0	9.5685	-3.98	710.89	0.006
L4	90 - 80 (4)	TP14x14x0.2188	10.00	0.00	0.0	9.5685	-5.98	710.89	0.008
L5	80 - 70.5 (5)	TP14x14x0.2188	9.50	0.00	0.0	9.5685	-7.36	710.89	0.010
L6	70.5 - 70 (6)	TP40x14x0.2188	0.50	0.00	0.0	9.5685	-7.40	710.89	0.010
L7	70 - 32 (7)	TP45.16x40x0.25	38.00	0.00	0.0	34.9896	-12.85	2102.06	0.006
L8	32 - 0 (8)	TP49x43.8453x0.25	38.00	0.00	0.0	38.6831	-20.78	2189.77	0.009

Pole Bending Design Data

Section	Elevation	Size	M_{ux}	ϕM_{nx}	Ratio	M_{uy}	ϕM_{ny}	Ratio
No.					M_{ux}			M_{uy}
	ft		kip-ft	kip-ft	ϕM_{nx}	kip-ft	kip-ft	ϕM_{ny}
L1	120 - 110 (1)	TP14x14x0.2188	5.81	199.87	0.029	0.00	199.87	0.000
L2	110 - 100 (2)	TP14x14x0.2188	23.07	199.87	0.115	0.00	199.87	0.000
L3	100 - 90 (3)	TP14x14x0.2188	51.36	199.87	0.257	0.00	199.87	0.000
L4	90 - 80 (4)	TP14x14x0.2188	90.10	199.87	0.451	0.00	199.87	0.000
L5	80 - 70.5 (5)	TP14x14x0.2188	135.37	199.87	0.677	0.00	199.87	0.000
L6	70.5 - 70 (6)	TP40x14x0.2188	135.37	199.87	0.677	0.00	199.87	0.000
L7	70 - 32 (7)	TP45.16x40x0.25	356.05	1910.18	0.186	0.00	1910.18	0.000
L8	32 - 0 (8)	TP49x43.8453x0.25	740.64	2201.13	0.336	0.00	2201.13	0.000

Pole Shear Design Data

Section No.	Elevation	Size	$Actual\ V_u$	ϕV_n	$Ratio$ V_u	$Actual$ T_u	ϕT_n	Ratio T
110.	ft		K	K	$\frac{v_u}{\phi V_n}$	kip-ft	kip-ft	$\frac{T_u}{\phi T_n}$
L1	120 - 110 (1)	TP14x14x0.2188	1.16	355.45	0.003	0.00	400.23	0.000
L2	110 - 100 (2)	TP14x14x0.2188	2.29	355.45	0.006	0.00	400.23	0.000
L3	100 - 90 (3)	TP14x14x0.2188	3.36	355.45	0.009	0.00	400.23	0.000
L4	90 - 80 (4)	TP14x14x0.2188	4.37	355.45	0.012	0.00	400.23	0.000
L5	80 - 70.5 (5)	TP14x14x0.2188	5.19	355.45	0.015	0.00	400.23	0.000
L6	70.5 - 70 (6)	TP40x14x0.2188	5.22	815.59	0.006	0.00	400.23	0.000
L7	70 - 32 (7)	TP45.16x40x0.25	8.39	1051.03	0.008	0.00	3825.03	0.000
L8	32 - 0 (8)	TP49x43.8453x0.25	11.84	1094.88	0.011	0.00	4407.63	0.000

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Pole Interaction Design Data

Section No.	Elevation	$Ratio$ P_u	Ratio M_{ux}	$Ratio$ M_{uy}	$Ratio$ V_u	$Ratio$ T_u	Comb. Stress	Allow. Stress	Criteria
	ft	ϕP_n	ϕM_{nx}	ϕM_{ny}	ϕV_n	ϕT_n	Ratio	Ratio	
L1	120 - 110 (1)	0.002	0.029	0.000	0.003	0.000	0.031	1.000	4.8.2
L2	110 - 100 (2)	0.004	0.115	0.000	0.006	0.000	0.119	1.000	4.8.2
L3	100 - 90 (3)	0.006	0.257	0.000	0.009	0.000	0.263	1.000	4.8.2
L4	90 - 80 (4)	0.008	0.451	0.000	0.012	0.000	0.459	1.000	4.8.2
L5	80 - 70.5 (5)	0.010	0.677	0.000	0.015	0.000	0.688	1.000	4.8.2
L6	70.5 - 70 (6)	0.010	0.677	0.000	0.006	0.000	0.688	1.000	4.8.2
L7	70 - 32 (7)	0.006	0.186	0.000	0.008	0.000	0.193	1.000	4.8.2
L8	32 - 0 (8)	0.009	0.336	0.000	0.011	0.000	0.346	1.000	4.8.2

Section Capacity Table

Section	Elevation	Component	Size	Critical	P	ϕP_{allow}	%	Pass
No.	ft	Туре		Element	K	K	Capacity	Fail
L1	120 - 110	Pole	TP14x14x0.2188	1	-1.24	710.89	3.1	Pass
L2	110 - 100	Pole	TP14x14x0.2188	2	-2.61	710.89	11.9	Pass
L3	100 - 90	Pole	TP14x14x0.2188	3	-3.98	710.89	26.3	Pass
L4	90 - 80	Pole	TP14x14x0.2188	4	-5.98	710.89	45.9	Pass
L5	80 - 70.5	Pole	TP14x14x0.2188	5	-7.36	710.89	68.8	Pass
L6	70.5 - 70	Pole	TP40x14x0.2188	6	-7.40	710.89	68.8	Pass
L7	70 - 32	Pole	TP45.16x40x0.25	7	-12.85	2102.06	19.3	Pass
L8	32 - 0	Pole	TP49x43.8453x0.25	8	-20.78	2189.77	34.6	Pass
							Summary	
						Pole (L5)	68.8	Pass
						RATING =	68.8	Pass

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Anchor Rod and Base Plate Calculation

ANSI/TIA-222-G-2

Axial:

Factored Base Reactions: Pole Shape: Anchor Rods: Base Plate:

49.00 in

Moment: 741 ft-kips 18-Sided (6) 2.25 in. A615 GR. 75 1.75 in. x 62 in. Round Shear: 12 kips $Pole\ Dia.\ (D_f)$: Anchor Rods Evenly Spaced fy = 60 ksi

Anchor Rod Calculation According to TIA-222-G section 4.9.9

 $\phi = 0.80 \text{ TIA } 4.9.9$

21 kips

 $I_{bolts} = 2352.00 \text{ in}^2 Momet of Inertia}$

 $P_u = 106 \; kips \; {\mbox{Tension Force}}$

 $V_u = 2 \text{ kips Shear Force}$ $R_{nf} = 325.00 \text{ kips Nominal Tensile Strength}$

 $\eta = 0.50 \,\, {}_{\text{for detail type (d)}}$

The following Interation Equation Shall Be Satisfied:

$$\left(\begin{array}{c}
\mathbf{P_u} + \frac{\mathbf{V_u}}{\eta} \\
\hline
& \phi \mathbf{R}_{nt}
\end{array}\right) \leq 1.0$$

 $0.423 \le 1$

Calculated Moment vs Factored Resistance

On a 56 in Bolt Circle

Base Plate Calculation According to TIA-222-G

 $\phi = 0.90 \text{ TIA } 4.7$

 $M_{PL} = 382.8 \text{ in-kip Plate Moment}$

L = 25.7 in Section Length

Z = 19.6 Plastic Section Modulus 382.75 in-kip ≤ 1061 in-kip

 $\mathbf{M_P} = 1178.6 \text{ in-kip Plastic Moment}$ $\mathbf{\phi} \ \mathbf{M_n} = 1060.7 \text{ in-kip Factored Resistance}$

Anchor Rods Are Adequate 42.3%

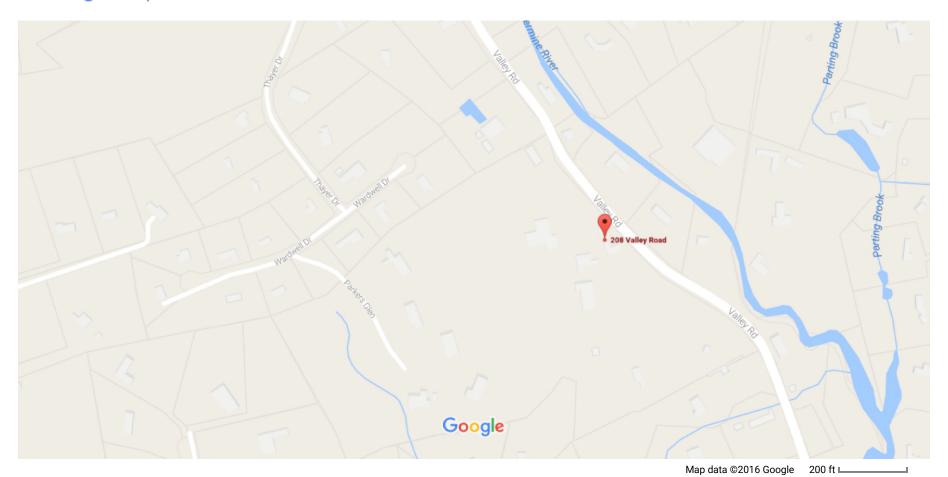
Base Plate is Adequate 36.1% ☑

Monopole Spread Footing Calculation

ANSI/TIA-222-G-2

Factored Base Read	ctions:	Footing Dimensions:		Concrete:
Moment:	741 ft-kips	16 ft x 16 ft	7 ft Square Pier	f'c = 4000 psi
Shear:	12 kips	x 2 ft thick	w/6 in Reveal	Steel $fy = 60 \text{ ksi}$
Axial:	21 kips	Bearing 6 ft B.G.	27.1 Yd3 Concrete	f = 0.75
Soil Backfill	100 pcf	Ultimate Bearing:	6000 psf	Water Table n/a
Equalities Weight				
Foundation Weight		21.0 kins		
Weight of		21.0 kips		
Weight of (109.875 kips		
Weight o		82.8 kips		
Bouyancy of		0.0 kips		
	Total	213.7 kips		
Overturning Resist	ance:			
Overturning M	Ioment (M _u)	819 ft-kips	741 ft-kips + (12 kips x 6.5	
Resisting Mo	oment (R _s)	1709.4 ft-kips	213.675 kips x 16 ft / 2	
φ x R _s >	$> M_{\rm u}$	$M_{overturning} / f M_{resist}$	63.9% OK	
Soil Bearing Pressu	ıre:			
Eccentric		3.83 ft	819 ft-kir	os / 213.675 kips
6(e)	•	23.0 ft >	16.0 ft	6e > 16
Maximum So		2531.5332 psf		d across corners
Soil Over	_	-600 psf		
Net Soil B		1931.5332 psf		
Resisting Soil	•	6000 psf		
Net Soil Beari	<u> </u>	Net Bearing / f R _s	42.9	% OK
Rending Moment in	n Pier:			
Bending Moment in		795 ft-kins	741 ft-kir	os + (12 kips x 4 5 ft)
Bending N	Moment	795 ft-kips	741 ft-kip	os + (12 kips x 4.5 ft)
O	Moment q'd (Loads)	795 ft-kips 9.09 in ² 35.28 in ²	•	os + (12 kips x 4.5 ft) sed on Square Pier)
Bending N Pier Steel Red	Moment q'd (Loads)	9.09 in ²	•	· · · ·
Bending N Pier Steel Red	Moment q'd (Loads) r Steel	9.09 in ²	•	· · · ·
Bending Min. Pier	Moment q'd (Loads) r Steel n Footing:	9.09 in ²	1/2% (Ba	
Bending Moment in	Moment q'd (Loads) r Steel n Footing: g Moment	9.09 in ² 35.28 in ²	1/2% (Ba	sed on Square Pier)

Google Maps 208 Valley Rd





New Search Back to Results View Property Print View Map

Location	Owner	Account	MBLU
208 VALLEY RD	SILVER HILL HOSPITAL INC	30126	0044/ 0108/ 0120/

Parcel Value

Item	Appraised Value	Assessed Value
Buildings	9,890,300	6,923,210
Extra Building Features	0	0
Outbuildings	67,700	47,390
Land	5,092,000	3,564,400
Total	15,050,000	10,535,000

Owner of Record

SILVER HILL HOSPITAL INC	
208 VALLEY RD	
NEW CANAAN, CT 06840	

Owner History

Name	Book/Page	Sale Date	Sale Price
SILVER HILL HOSPITAL INC	702/ 281	11/09/2004	0
SILVER HILL FOUNDATION INC	67/ 13	05/18/1940	136,567

Assessment History

Total Assessment
10,535,000
10,535,000
10,535,000
9,209,060
9,209,060
9,209,060
9,209,100
10,969,100
4,710,900
4,710,900
4,710,900
4,710,900
4,710,900
6,112,960

Building Permits

Permit ID	Issue Date	Ammount	Description	
16- 00064	01/28/2016	10,000	"REPAIR WATER DAMAGE AT MAIN HOUSE."	
15- 01238	12/09/2015	80,000	MARTIN CENTER - REPLACE EXISTING ENTRANCE STAIRS A	ND ROOF.
15- 01184	11/30/2015		RENOVATE 18 EXISTING RESTROOMS (WITH NEW FINISHES CONTROLS FOR PATIENT SAFETY.) NO INCREASE IN FIXTURE.	
15- 00466	06/01/2015	300,000	MAIN HOUSE - INTERIOR RENOVATIONS TO THE 2ND FLOOI	R
15- 00280	04/07/2015	90,000	'ENLARGE MED ROOM AND SWAP LOUNGE & TREATMENT I ADD AC UNITS TO MED, TREATMENT AND & NURSE STATIC	
14-1307	12/16/2014	72,000	CONSTRUCT A 12 X 24 SHELTER- FOR PROPANE GENERATOR	R, 6 ANTENNAS, UG PROPANE TANKS

14-0244	03/24/2014	400,000	"MARTIN CENTER BUILDING OFFICE: - RENOVATE EXISTING TO UPPER LEVEL, INCLUDES ADDING HVAC & EXTERIOR W NEW RESTROOM TO REPLACE ONE MOVED TO CREAT DATA COMPL	G OFFICE SPACE INCLUDING ADA ACCESS INDOWS [**REVISION- \$25,000: CREATE A CLOSET. NEW RESTROOM TO BE ADA
14-0297	03/19/2014	175,000	WIRELESS CELL TOWER ONLY.	
14-0296	03/19/2014	30,000	INSTALLATION OF EQUIPMENT ON $12\mathrm{x}20$ CONCRETE PAD, C 86^{\prime}	ONCRETE PAD & 3 PANEL ANTENNAS AT
14-0169	02/26/2014	1,600,000	"RESIDENTIAL BUILDING" RENOVATION TOTHE EXISTING INCLUDING ADA UPGRADES, NEW WINDOWS SIDING, ROOF HOUSE	7800 SQ FT RESIDENTIAL BUILDING - , MECHANICALS AND FINISHES FOR THE K
14-0168	02/12/2014	20,000	REMOVE POLE MOUNTED FLOOD LIGHTS & REPLACE WITH	CAMPUS STD LOW LIGHT POST LIGHTS.
12-0452	09/21/2012	1,500,000	COM ADDS & ALTS	
12-0359	04/02/2012	30,000	COM ADDS & ALTS	
11-0059	03/15/2011	1,234,000	COM ADDS & ALTS	
11-0037	01/19/2011	65,000	ASBESTOS ABATEMENT, EXPLORATION DEMO	
10-0086	03/24/2010	735,000	COM ADDS & ALTS	
09-0649	01/29/2010	0	SIDEWALKS & ACCESSIBLE ROUTE	
09-0109	04/14/2009	100,000	COM ADDS & ALTS	
08-0846	11/18/2008	25,000	INT ALTS AND DECK	
07-1210	02/28/2008	250,000	CHANGE OF USE INT. ALTS & RAMP R-4	
07-0675	08/20/2007	6,199,000	COM ADDITIONS AND ALTERATIONS	
07-0402	05/11/2007	50,000	COM ADDS & ALTS	
07-0309	04/25/2007	25,000	COM ADDS & ALTS	
01- 0773A	11/06/2001	0	COM CO	
01-0773	09/17/2001	20,000	NEW OUTSIDE STAIRS	
01-0096	03/12/2001	73,000	PATIENT ROOM REMO	
20343	01/03/2001	42,000		
1914- 0120	09/23/1998	150,000	SILVERHILL FOUNDATION, INC.	
1796- 0120	07/29/1996	1,000	SILVERHILL FOUNDATION, INC.	

Land Line Valuation

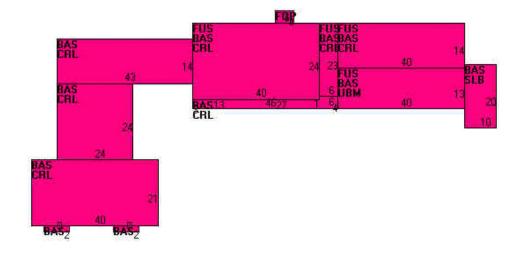
Size	Zone	Dev Map #	Appraised Value	Assessed Value
21.57 AC	2 AC	7319, 7350	5,092,000	3,564,400

Building Details - Click Buildings Below
Building 1 Building 2 Building 3 Building 4 Building 5 Building 6 Building 7 Building 8

Building 1



Building Sketch



Subarea Summary

	J		
Code	Description	Gross Area	Living Area
BAS	First Floor	4,572	4,572
CRL	Crawl Space	3,820	0
FOP	Open Porch	24	0
FUS	Upper Story, Finished	2,178	2,178
SLB	Slab	200	0
UBM	Basement, Unfinished	520	0
		Total Living Area:	6,750