



STATE OF CONNECTICUT
CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051
Phone: (860) 827-2935 Fax: (860) 827-2950
E-Mail: siting.council@ct.gov
www.ct.gov/csc

December 3, 2012

Kenneth C. Baldwin, Esq.
Robinson & Cole LLP
280 Trumbull Street
Hartford, CT 06103

RE: **EM-VER-086-121114** - Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at Old Colchester Road, Montville, Connecticut.

Dear Attorney Baldwin:

The Connecticut Siting Council (Council) hereby acknowledges your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies with the following conditions:

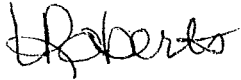
- Prior to antenna installation, the tower modifications identified in the Detailed Structural Analysis and Reinforcement prepared by URS Corporation dated October 11, 2012, and stamped by Richard Sambor shall be implemented;
- Not more than 45 days following completion of the antenna installation, a signed letter from a Professional Engineer duly licensed in the State of Connecticut shall be submitted to the Council to certify that the recommended modifications have been completed and the tower does not exceed 100 percent of the post-construction structural rating;
- Any deviation from the proposed modification as specified in this notice and supporting materials with Council shall render this acknowledgement invalid;
- Any material changes to this modification as proposed shall require the filing of a new notice with the Council;
- Not more than 45 days after completion of construction, the Council shall be notified in writing that construction has been completed;
- The validity of this action shall expire one year from the date of this letter; and
- The applicant may file a request for an extension of time beyond the one year deadline provided that such request is submitted to the Council not less than 60 days prior to the expiration;

The proposed modifications including the placement of all necessary equipment and shelters within the tower compound are to be implemented as specified here and in your notice dated November 12, 2012. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.



This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Thank you for your attention and cooperation.

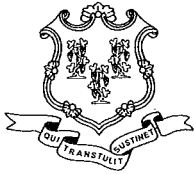
Very truly yours,



Linda Roberts
Executive Director

LR/CDM/jbw

c: The Honorable Ronald K. McDaniel, Mayor, Town of Montville
Marcia Vlaun, Town Planner, Town of Montville
Metrocast Communications



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November 15, 2012

The Honorable Ronald K. McDaniel
Mayor
Town of Montville
Town Hall
310 Norwich New London Turnpike
Uncasville, CT 06382

RE: **EM-VER-086-121114**- Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at Old Colchester Road, Montville, Connecticut.

Dear Mayor McDaniel:

The Connecticut Siting Council (Council) received a request to modify an existing telecommunications facility, pursuant to Regulations of Connecticut State Agencies Section 16-50j-72. A copy of which has already been provided to you.

If you have any questions or comments regarding the proposal, please call me or inform the Council by November 30, 2012.

Thank you for your cooperation and consideration.

Very truly yours,

Linda Roberts
Executive Director

LR/jbw

c: Marcia Vlaun, Town Planner, Town of Montville

280 Trumbull Street
Hartford, CT 06103-3597
Main (860) 275-8200
Fax (860) 275-8299
kbaldwin@rc.com
Direct (860) 275-8345

Also admitted in Massachusetts

November 12, 2012

Linda Roberts
Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

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CONNECTICUT
SITING COUNCIL

Re: **Notice of Exempt Modification – Antenna Swap
Old Colchester Road, Montville, Connecticut**

Dear Ms. Roberts:

Cellco Partnership d/b/a Verizon Wireless (“Cellco”) currently maintains twelve (12) wireless telecommunications antennas at the 305-foot level on an existing 370-foot tower at the above-referenced address. The tower and underlying property are owned by Metrocast Communications of CT LLC. Cellco’s use of the tower was approved by the Council in 1988. Cellco now intends to replace all of its existing antennas with six (6) LPA-80080-4CF cellular antennas; three (3) model BXA-171085-8BF PCS antennas; and three (3) model BXA-70063-6CF LTE antennas, all at the same 305-foot level on the tower. Cellco also intends to install six (6) coax cable diplexers behind its antennas. Attached behind Tab 1 are the specifications for Cellco’s replacement antennas and diplexers.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Ronald K. McDaniel, Mayor of the Town of Montville. A copy of this letter is also being sent to Metrocast Communications of CT LLC, the owner of the property on which the tower is located.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2), as amended.



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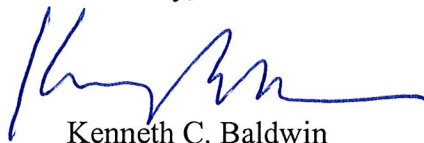
ROBINSON & COLE_{LLP}

Linda Roberts
November 12, 2012
Page 2

1. The proposed modifications will not result in an increase in the height of the existing tower. Cellco's replacement antennas will be located at the 305-foot level on the existing 370-foot tower.
2. The proposed modifications do not involve any change to ground-mounted equipment and, therefore, will not require the extension of the site boundaries.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative power density table for Cellco's modified facility is included behind Tab 2.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The tower and its foundation, with certain modifications, can support Cellco's proposed modifications. (See Detailed Structural Analysis and Reinforcement report attached behind Tab 3).

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,



Kenneth C. Baldwin

Enclosures

Copy to:

Ronald K. McDaniel, Montville Mayor
Metrocast Communications of CT LLC
Sandy M. Carter



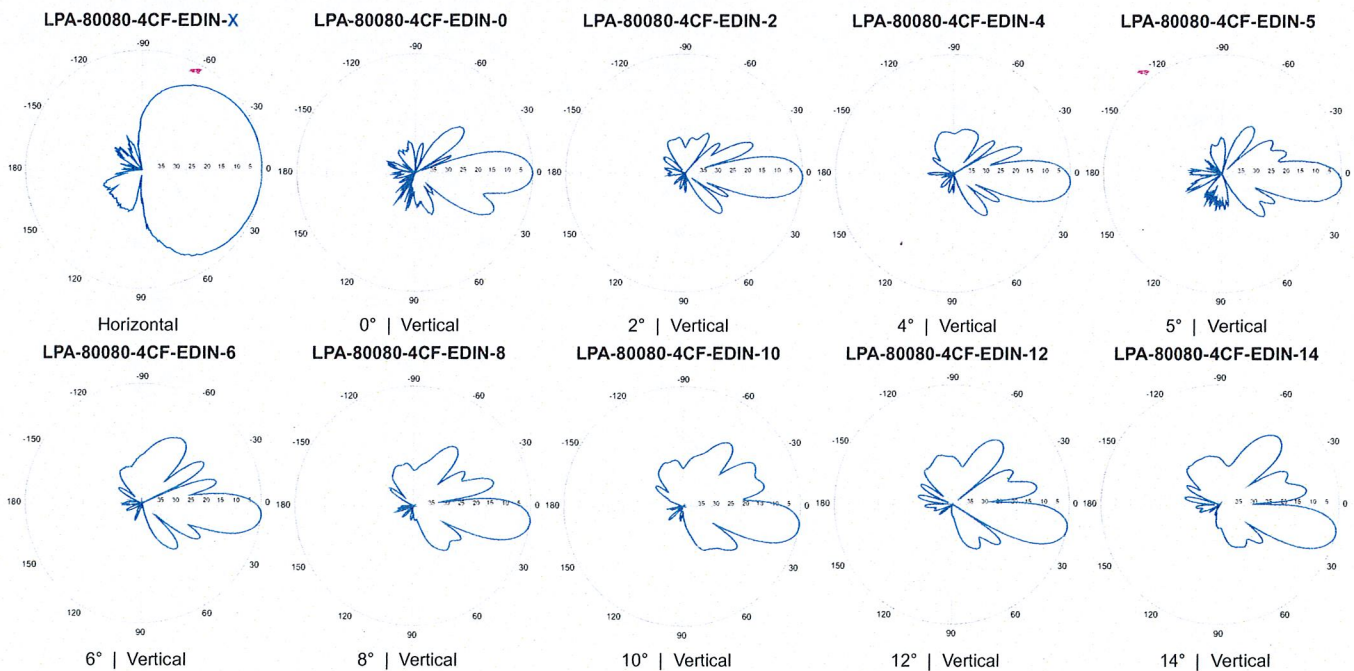
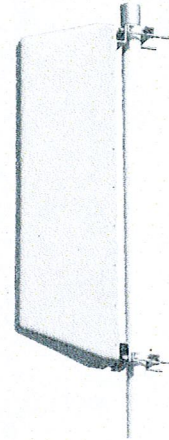
LPA-80080-4CF-EDIN-X

V-Pol | Log Periodic | 80° | 12.5 dBd

Replace "X" with desired electrical downtilt.

Antenna is also available with NE connector(s)
 Replace "EDIN" with "NE" in the model number
 when ordering.

Electrical Characteristics	
Frequency bands	806-960 MHz
Polarization	Vertical
Horizontal beamwidth	80°
Vertical beamwidth	15°
Gain	12.5 dBd (14.6 dBi)
Electrical downtilt (X)	0, 2, 4, 5, 6, 8, 10, 12, 14
Impedance	50Ω
VSWR	≤1.4:1
Upper sidelobe suppression (0°)	-14.2 dB
Front-to-back ratio (+/-30°)	-34.7 dB
Null fill	15% (-16.48 dB)
Input power	500 W
Lightning protection	Direct Ground
Connector(s)	1 Port / EDIN or NE / Female / Center (Back)
Mechanical Characteristics	
Dimensions Length x Width x Depth	1200 x 140 x 335 mm 47.2 x 5.5 x 13.2 in
Depth of antenna with z-bracket	375 mm 14.8 in
Weight without mounting brackets	5.4 kg 12 lbs
Survival wind speed	> 201 km/hr > 125 mph
Wind area	Front: 0.17 m ² Side: 0.40 m ² Front: 1.8 ft ² Side: 4.3 ft ²
Wind load @ 161 km/hr (100 mph)	Front: 254 N Side: 574 N Front: 57 lbf Side: 129 lbf
Mounting Options	
Mounting Options	Part Number
2-Point Mounting & Downtilt Bracket Kit (0-20°)	21699999
Lock-Down Brace	If the lock-down brace is used, the maximum diameter of the mounting pipe is 88.9 mm or 3.5 in.



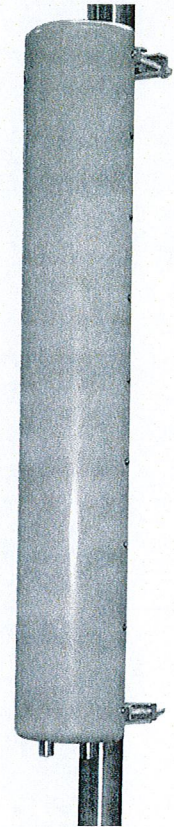
Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.

BXA-171085-8BF-EDIN-X

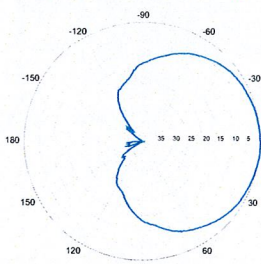
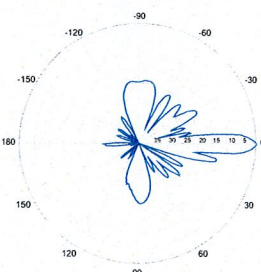
Replace "X" with desired electrical downtilt.

X-Pol | FET Panel | 85° | 16.4 dBi

Electrical Characteristics	1710-2170 MHz		
Frequency bands	1710-1880 MHz	1850-1990 MHz	1920-2170 MHz
Polarization	±45°	±45°	±45°
Horizontal beamwidth	88°	85°	80°
Vertical beamwidth	7°	7°	7°
Gain	13.5 dBd / 15.6 dBi	13.9 dBd / 16.0 dBi	14.3 dBd / 16.4 dBi
Electrical downtilt (X)	0, 2, 4		
Impedance	50Ω		
VSWR	≤1.5:1		
First upper sidelobe	< -17 dB		
Front-to-back isolation	> 30 dB		
In-band isolation	> 28 dB		
IM3 (20W carrier)	< -150 dBc		
Input power	300 W		
Lightning protection	Direct Ground		
Connector(s)	2 Ports / EDIN / Female / Bottom		
Operating temperature	-40° to +60° C / -40° to +140° F		
Mechanical Characteristics			
Dimensions Length x Width x Depth	1232 x 154 x 105 mm	48.5 x 6.1 x 4.1 in	
Depth with t-brackets	133 mm	5.2 in	
Weight without mounting brackets	4.8 kg	10.5 lbs	
Survival wind speed	296 km/hr	184 mph	
Wind area	Front: 0.19 m ² Side: 0.14 m ²	Front: 2.0 ft ² Side: 1.5 ft ²	
Wind load @ 161 km/hr (100 mph)	Front: 281 N Side: 223 N	Front: 63 lbf Side: 50 lbf	
Mounting Options	Part Number	Fits Pipe Diameter	Weight
2-Point Mounting Bracket Kit	26799997	50-102 mm 2.0-4.0 in	2.3 kg 5 lbs
2-Point Mounting & Downtilt Bracket Kit	26799999	50-102 mm 2.0-4.0 in	3.6 kg 8 lbs
Concealment Configurations	For concealment configurations, order BXA-171085-8BF-EDIN-X-FP		

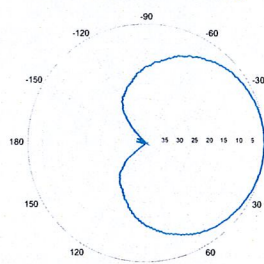
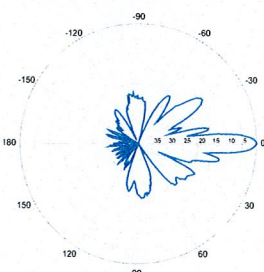


BXA-171085-8BF-EDIN-X

Horizontal | 1710-1880 MHz
BXA-171085-8BF-EDIN-0

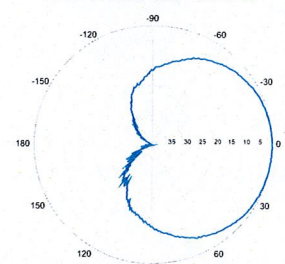
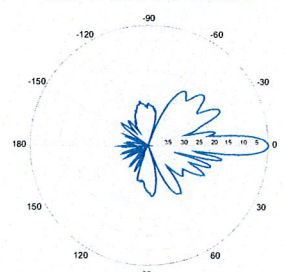
0° | Vertical | 1710-1880 MHz

BXA-171085-8BF-EDIN-X

Horizontal | 1850-1990 MHz
BXA-171085-8BF-EDIN-0

0° | Vertical | 1850-1990 MHz

BXA-171085-8BF-EDIN-X

Horizontal | 1920-2170 MHz
BXA-171085-8BF-EDIN-0

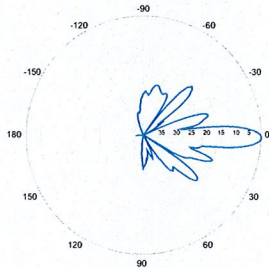
0° | Vertical | 1920-2170 MHz

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BXA-171085-8BF-EDIN-X

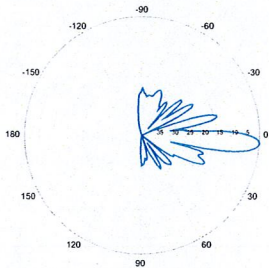
X-Pol | FET Panel | 85° | 16.4 dBi

BXA-171085-8BF-EDIN-2



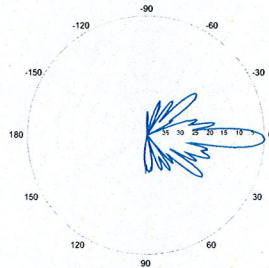
2° | Vertical | 1710-1880 MHz

BXA-171085-8BF-EDIN-4



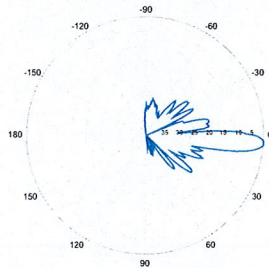
4° | Vertical | 1710-1880 MHz

BXA-171085-8BF-EDIN-2



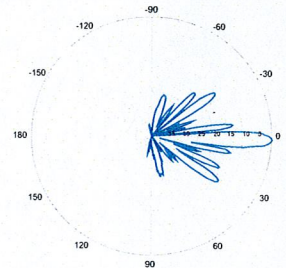
2° | Vertical | 1850-1990 MHz

BXA-171085-8BF-EDIN-4



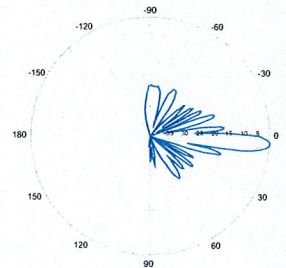
4° | Vertical | 1850-1990 MHz

BXA-171085-8BF-EDIN-2



2° | Vertical | 1920-2170 MHz

BXA-171085-8BF-EDIN-4



4° | Vertical | 1920-2170 MHz

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BXA-70063-6CF-EDIN-X

X-Pol | FET Panel | 63° | 14.5 dBd

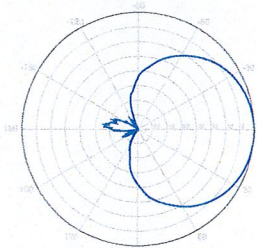
Replace "X" with desired electrical downtilt.

Antenna is also available with NE connector(s). Replace "EDIN" with "NE" in the model number when ordering.

Electrical Characteristics	696-900 MHz			
Frequency bands	696-806 MHz		806-900 MHz	
Polarization	±45°			
Horizontal beamwidth	65°		63°	
Vertical beamwidth	13°		11°	
Gain	14.0 dBd (16.1 dBi)		14.5 dBd (16.6 dBi)	
Electrical downtilt (X)	0, 2, 3, 4, 5, 6, 8, 10			
Impedance	50Ω			
VSWR	≤1.35:1			
Upper sidelobe suppression (0°)	-18.3 dB		-18.2 dB	
Front-to-back ratio (+/-30°)	-33.4 dB		-36.3 dB	
Null fill	5% (-26.02 dB)			
Isolation between ports	< -25 dB			
Input power with EDIN connectors	500 W			
Input power with NE connectors	300 W			
Lightning protection	Direct Ground			
Connector(s)	2 Ports / EDIN or NE / Female / Center (Back)			
Mechanical Characteristics				
Dimensions Length x Width x Depth	1804 x 285 x 132 mm		71.0 x 11.2 x 5.2 in	
Depth with z-brackets	172 mm		6.8 in	
Weight without mounting brackets	7.9 kg		17 lbs	
Survival wind speed	> 201 km/hr		> 125 mph	
Wind area	Front: 0.51 m ²	Side: 0.24 m ²	Front: 5.5 ft ²	Side: 2.6 ft ²
Wind load @ 161 km/hr (100 mph)	Front: 759 N	Side: 391 N	Front: 169 lbf	Side: 89 lbf
Mounting Options	Part Number	Fits Pipe Diameter		Weight
3-Point Mounting & Downtilt Bracket Kit	36210008	40-115 mm	1.57-4.5 in	6.9 kg 15.2 lbs
Concealment Configurations	For concealment configurations, order BXA-70063-6CF-EDIN-X-FP			

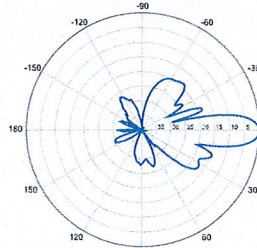


BXA-70063-6CF-EDIN-X



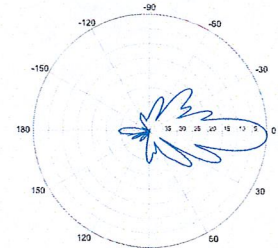
Horizontal | 750 MHz

BXA-70063-6CF-EDIN-0

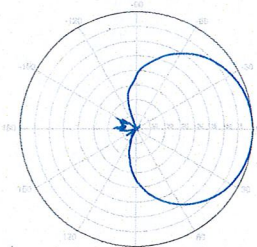


0° | Vertical | 750 MHz

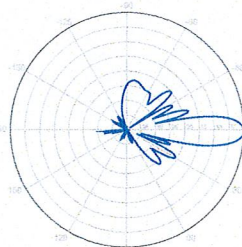
BXA-70063-6CF-EDIN-2



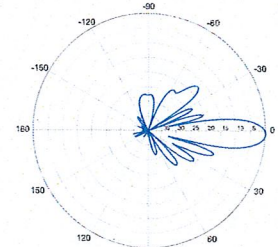
2° | Vertical | 750 MHz



Horizontal | 850 MHz



0° | Vertical | 850 MHz



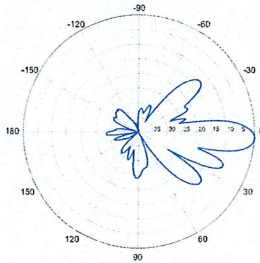
2° | Vertical | 850 MHz

Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.

BXA-70063-6CF-EDIN-X

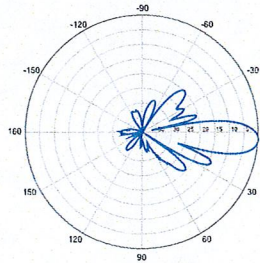
X-Pol | FET Panel | 63° | 14.5 dBd

BXA-70063-6CF-EDIN-3



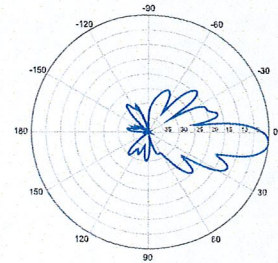
3° | Vertical | 750 MHz

BXA-70063-6CF-EDIN-4

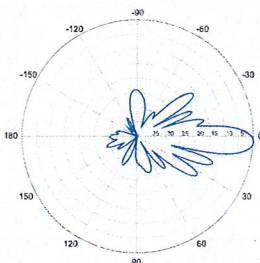


4° | Vertical | 750 MHz

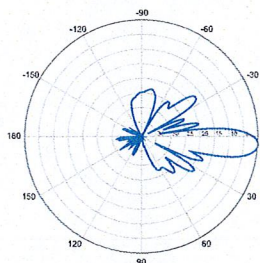
BXA-70063-6CF-EDIN-5



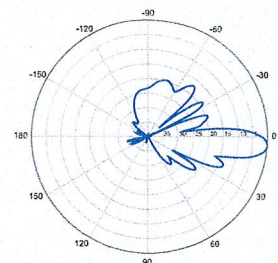
5° | Vertical | 750 MHz



3° | Vertical | 850 MHz

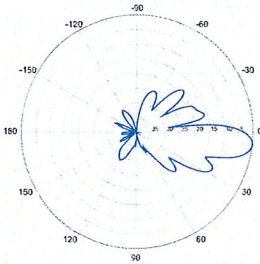


4° | Vertical | 850 MHz



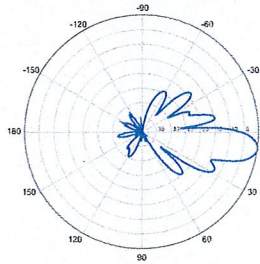
5° | Vertical | 850 MHz

BXA-70063-6CF-EDIN-6



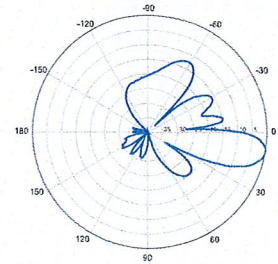
6° | Vertical | 750 MHz

BXA-70063-6CF-EDIN-8

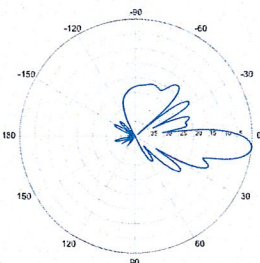


8° | Vertical | 750 MHz

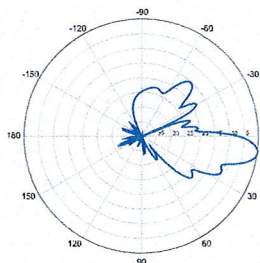
BXA-70063-6CF-EDIN-10



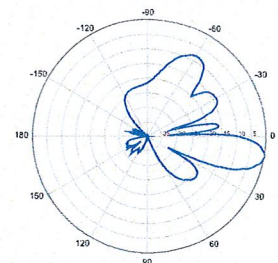
10° | Vertical | 750 MHz



6° | Vertical | 850 MHz



8° | Vertical | 850 MHz



10° | Vertical | 850 MHz

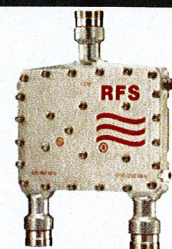
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ShareLite Wideband Diplexer – In-line 698-960 MHz/1710-2200 MHz, DC pass in high frequency path

Product Description

The ShareLite FD9R6004 Series of diplexers are designed to enable feeder sharing between systems in the 698-960 MHz range and in the 1710-2200 MHz range. The diplexer is equipped with in-line connector placement so it can be installed in the BTS cabinet or at the tower top. This is especially valuable in crowded sites or when the feeders are not easily accessible. Due to its wideband design, the FD9R6004 Series can accommodate many combining solutions between 698-960 MHz and 1710-2200 MHz systems such as LTE 700 MHz, Cellular 800 MHz with PCS, GSM900 with GSM1800, or GSM900 with UMTS. This diplexer features a highly selective filter. It provides a high level of isolation between ports, while keeping the insertion loss on both paths at an extremely low level. The FD9R6004 diplexers are available with various DC pass options, helpful in configurations with or without the Tower Mount Amplifiers installed.



Features/Benefits

- **LTE ready design**
- **Extremely Low Insertion Loss**
- **High level of Rejection between bands – Protection against interferences**
- **Extremely High Power Handling Capability**
- **Integrated DC block/bypass versions available**
- **Very compact & small size design – Easy installation and reduced tower load**
- **In-line long-neck connectors for easy connection & waterproofing**
- **Exceptional reliability & environmental protection (IP 67)**
- **Equipped with 1 * Breathable Vent – Prevent any humidity inside the product**
- **Mounting hardware for Wall and Pole mount provided (P/N SEM2-1A)**
- **Grounding already provided through the mounting bracket**
- **Kit available for easy dual mount**

Technical Specifications

Product Type	Diplexer/Cross Band Coupler
Application	LTE700, GSM900, UMTS, GSM1800, Cellular 800, PCS
Frequency Range 1, MHz	698-960
Frequency Range 2, MHz	1710-2200
Configuration	Sharelite Single diplexer, outdoor, DC pass in the 1710-2170MHz path, with mounting hardware SEM2-1A
Mounting	Wall Mounting: With 4 screws (maximum 6mm diameter); Pole Mounting: With included clamp set 40-110mm (1.57-4.33)
Return Loss All Ports Min/Typ, dB	19/23
Power Handling Continuous, Max, W	1250 at common port; 750 in low frequency path & 500 in high frequency path
Power Handling Peak, Max, W	15000 in low frequency path & 8000 in high frequency path
Impedance, Ohms	50
Insertion Loss, Path 1, dB	0.07 typ.
Insertion Loss, Path 2, dB	0.13 typ.
Rejection Between Bands Min/Typ, dB	58/64@698-960MHz; 57/70@1710-2200MHz
IMP Level at the COM Port, Typ, dBm	-112 @ 2x43
DC Pass in Low Frequency Path	No
DC Pass in High Frequency Path	Yes
Temperature Range, °C (°F)	-40 to +60 (-40 to +140)
Environmental	ETSI 300-019-2-4 Class 4.1E
Ingress Protection	IP 67
Lightning Protection	EN/IEC61000-4-5 Level 4
Connectors	In-line long-neck 7-16-Female
Weight, kg (lb)	1.2 (2.6)
Shipping Weight, kg (lb)	3.2 (7) for 2 * single units in 1 * box, 9.8 (21.6) for 6 * units = 3 * Boxes in 1 * overwrap
Dimensions, H x W x D, mm (in)	147 x 164 x 37 (5.8 x 6.5 x 1.5)
Shipping Dimensions, H x W x D, mm (in)	254 x 406 x 82 (10 x 16 x 3.2) for 2 * Single Units in 1 * box, 280 x 406 x 241 (11 x 16 x 9.5) for 6 * units = 3 * Boxes in 1 * overwrap
Volume, L	0.43
Housing	Aluminum

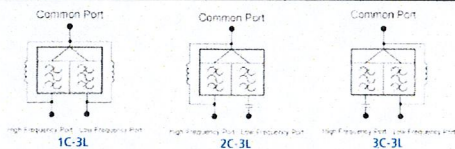
Notes

ShareLite Wideband Diplexer – In-line 698-960 MHz/1710-2200 MHz, DC pass in high frequency path

Other Documentation

FD9R6004/2C-3L Installation Instructions: Wideband_Diplexer_Installation_Rev5.pdf

Selection Guide Diplexer 698-960 / 1710-2200MHz					
	Model Number	Full DC Pass	DC Pass High Band	DC Pass Low Band	Mounting Hardware Included
Single	FD9R6004/1C-3L				X
	FD9R6004/2C-3L				X
	FD9R6004/3C-3L				X
Dual	KIT-FD9R6004/1C-DL				X
	KIT-FD9R6004/2C-DL				X
	KIT-FD9R6004/3C-DL				X



The FD9R6004 Series is upgradeable to a Dual Diplexer kit by means of 2 diplexers and mounting hardware kits SEM2-1A and SEM2-3

Mounting Hardware and Ground Cable Ordering Information	
Model Number	Description
SEM2-1A	Mounting Hardware, Pole mount $\phi 40-110$ mm (Included with the Single and Dual Diplexer) Wall Screws M6 (Not included with the product)
SEM2-3	Assembly kit for 2 pcs of FD9R6004/xC-3L (Can be ordered separately but included with the Dual Diplexer Kit)
CA020-2	Ground Cable, 2m, includes lugs (Optional)
CA030-2	Ground Cable, 2m, includes lugs (Optional)
SEM6	Mounting Hardware for 6 Diplexers, Tower Base (Optional)

DETAILED STRUCTURAL ANALYSIS AND REINFORCEMENT OF EXISTING 370' GUYED LATTICE TOWER FOR PROPOSED ANTENNA ARRANGEMENT

695 Old Colchester Road
Montville, Connecticut

prepared for



Verizon Wireless
99 East River Drive
East Hartford, Connecticut 06108

prepared by



URS CORPORATION
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36922267
VZ5-109 (Rev 1)

October 11, 2012

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 - **TNX TOWER FEEDLINE DISTRIBUTION**
 - **TNX TOWER DETAILED OUTPUT**
 - **FOUNDATION ANALYSIS**
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1. EXECUTIVE SUMMARY

This report summarizes the structural analysis and reinforcement of the existing 370' guyed lattice tower located at 695 Old Colchester Road in Montville, Connecticut. The analysis was conducted in accordance with the 2005 Connecticut State Building Code and the TIA/EIA-222-F standard requirements for a wind velocity of 95 mph (fastest mile) and 82 mph (fastest mile) concurrent with ½" ice. The antenna loading considered in the analysis consists of all existing and proposed antennas, transmission lines, and ancillary items as outlined in the Introduction Section of this report. The proposed Verizon Wireless modification is as follows:

Proposed Antenna and Mount	Carrier	Antenna Center Elevation
<u>Remove:</u> (6) SC-9012 / ALPE-9001 (6) 948F85T2E-M	Verizon (existing)	
<u>Install:</u> (3) BXA-70063-6CF (6) LPA-80080/4CF (3) BXA-171085-8BF (6) Diplexers	Verizon (proposed)	@ 305'

The results of an initial analysis indicated that the tower structure requires modification in order to support the proposed loading conditions. The required modifications are shown in SK-1 & SK-2 in Section 6 of this report. **Once these modifications are performed the tower and its foundation are considered structurally adequate with the wind load classification specified above and all the existing and proposed antenna loading.**

This analysis is based on:

- 1) The tower structure's theoretical capacity, not including any assessment of the condition of the tower.
- 2) Tower geometry and structural member sizes taken from Tower Mapping performed by CSB Communications, LLC, dated January 26, 2006 and revised February 23, 2006.
- 3) Tower base and guy anchor sizes taken from Foundation Inspection performed by Geotel Engineering, Inc., report number E08-383, dated July 30, 2008.
- 4) Geotechnical information taken from Geotechnical Engineering Report prepared by Terracon, project number J2085199, dated November 26, 2008.
- 5) Structural analysis and reinforcement performed by Malouf Engineering Intl., Inc, job number CT01162G-08V3 for SAI Communications / AT&T, signed and sealed December 10, 2008.
- 6) Existing tower inventory taken from inventory provided by Metrocast on July 12, 2012.
- 7) Antenna and mount configuration as specified on the following page of this report.

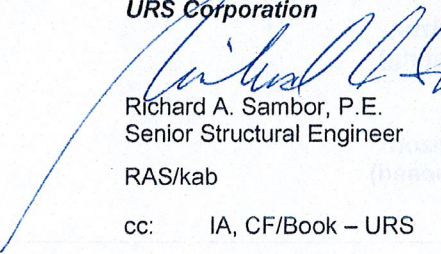
1. **EXECUTIVE SUMMARY** *(continued)*

This report is only valid as per the assumptions and data utilized in this report for antenna inventory, mounts and associated cables. The user of this report shall field verify the assumption of the antenna and mount configuration as well as the physical condition of the tower and connections. Notify the engineer in writing immediately if any of the information in this report is found to be other than specified.

If you should have any questions, please call.

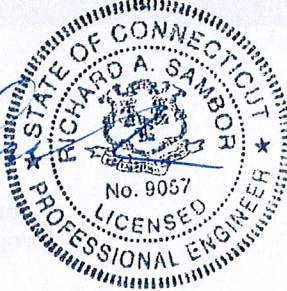
Sincerely,

URS Corporation


Richard A. Sambor, P.E.
Senior Structural Engineer

RAS/kab

cc: IA, CF/Book – URS



2. INTRODUCTION

The subject tower is located at 695 Old Colchester Road in Montville, Connecticut. The structure is a 370' guyed lattice tower designed and manufactured by PIROD.

The tower geometry and structural member sizes taken from Tower Mapping performed by CSB Communications, LLC, dated January 26, 2006 and revised February 23, 2006.

The inventory is summarized in the table below:

Antenna Type	Carrier	Mount	Antenna Centerline Elevation	Cable
(1) Search Antenna	WGBH	Leg Mounted	370'	(1) 7/8"
(1) 8' Parabolic Dish (1) Empty Pipe Mount	Unknown (existing)	Pipe Mount	355'	(1) 7/8"
(1) 20' Omni	Unknown (existing)	Pipe Mount	350'	(2) 7/8"
(1) 10' Omni	Unknown (existing)	3' Stand-Off	325'	(1) 1-5/8"
---	Verizon (existing)	(3) Sector Mounts	305'	(12) 1-5/8"
(3) BXA-70063-6CF (6) LPA-80080/4CF (3) BXA-171085-8BF (6) Diplexers	Verizon (proposed)	Shared with Above	305'	Shared with Above
(1) 20' Omni	Secret Service (existing)	Pipe Mount	250'	None
(6) CSS DUO1417-8686 (3) Powerwave 7770 (6) TMAs (6) Diplexers	AT&T (existing)	(3) Sector Mounts	242.5'	(12) 1-5/8"
(1) 4' Yagi	WFSB	Leg Mounted	200'	(1) 7/8"
(4) 5' Yagi	Hartford UHF (existing)	(2) Pipe Mounts	180'	(1) 7/8"
(1) 5' Yagi	WJAR	Leg Mounted	148'	(1) 1/2"
(1) 8' Yagi	WTNH	Leg Mounted	140'	(1) 7/8"
(1) 4' Yagi	Hartford UHF (existing)	Leg Mounted	125'	(1) 1/2"
(4) 5' PCS Antennas	FM	Leg Mounted	88'	(4) 1/2"
(1) 6' Yagi	WCTX	Leg Mounted	62'	(1) 7/8"
(1) 5' Yagi	WHPX	Leg Mounted	40'	(1) 7/8"

This structural analysis of the communications tower was performed by URS Corporation (URS) for Verizon Wireless. The purpose of this analysis was to investigate the structural integrity of the existing tower with its existing and proposed antenna loads. This analysis was conducted to evaluate stress on the tower and the effect of forces to the foundation of the tower resulting from existing and proposed antenna arrangements.

3. ANALYSIS METHODOLOGY AND LOADING CONDITIONS

The structural analysis was done in accordance with the 2005 Connecticut State Building Code, TIA/EIA-222-F—Structural Standard for Steel Antenna Towers and Antenna Supporting Structures, and the American Institute of Steel Construction (AISC) Manual of Steel Construction—Allowable Stress Design (ASD).

The analysis was conducted using TNX Tower 6.0. Two load conditions were evaluated as shown below which were compared to allowable stresses according to AISC and TIA/EIA.

Load Condition 1 = 95 mph (fastest mile) Wind Load (without ice) + Tower Dead Load
 Load Condition 2 = 82 mph (fastest mile) Wind Load (with ice) + Ice Load + Tower Dead Load

The TIA/EIA standard permits a one-third increase in allowable stresses for towers and monopoles less than 700 feet tall. For the purposes of this analysis, in computing the load capacity the allowable stresses of the tower members were increased by one-third.

4. FINDINGS AND EVALUATION

Stresses on the modified tower structure were evaluated to compare with allowable stresses in accordance with AISC. The calculated stresses under the proposed loading were within the allowable stresses. Detailed analysis and calculations for the proposed load condition are provided in section 6 of this report. Additionally, the modified guy anchors are structurally adequate for the proposed loading. **See drawings SK-1 & SK-2 for the required modifications.**

Table 1: Tower Component vs. Capacity Summary

COMPONENT (SECTION NO.)	CONTROLLING COMPONENT / ELEVATION	STRESS RATIO (% CAPACITY)	PASS/FAIL
Leg (T41)	SR 3.25 / 0-25'	87.4	Pass
Diagonal (T23)	SR 3/4 / 225'-231.25'	96.8	Pass
Horizontal (T34)	2" Crushed Tube 100'-125'	76.2	Pass
Top Girt (T40)	L2.5x2x0.25 / 25'-50'	59.7	Pass
Guy @ 350'	7/8 EHS	65.2	Pass
Guy @ 300'	7/8 EHS	69.5	Pass
Guy @ 225'	3/4 EHS	85.0	Pass
Guy @ 162.5'	3/4 EHS	90.5	Pass
Guy @ 100'	9/16 EHS	93.4	Pass
Guy @ 50'	9/16 EHS	82.0	Pass
Torque Arm Top (T12)	2L3x2.5x0.25 / 300'	31.8	Pass
Torque Arm Bottom (T12)	2L3x2.5x0.25 / 300'	71.9	Pass

Table 2: Foundation

COMPONENT / CONTROLLING ELEMENT	USAGE (% CAPACITY)
Base Foundation / Compression (%)	80.0
Inner Guy Anchor / Uplift (%)	24.8
Inner Guy Anchor / Shear (%)	41.0
160' Radius Guy Anchor / Uplift (%)	72.3
160' Radius Guy Anchor / Shear (%)	73.3
205' Radius Guy Anchor / Uplift (%)	82.9
205' Radius Guy Anchor / Shear (%)	66.9
Outer Guy Anchor / Uplift (%)	63.0
Outer Guy Anchor / Shear (%)	46.6

5. CONCLUSIONS

The results of an initial analysis indicated that the tower structure requires modification in order to support the proposed loading conditions. **The required modifications are shown in SK-1 & SK-2 located in Section 6 of this report. Once these modifications are performed the tower and its foundation are considered structurally adequate with the wind load classification specified above and all the existing and proposed antenna loading.**

Limitations/Assumptions:

This report is based on the following:

1. Tower inventory as listed in this report.
2. Tower is properly installed and maintained.
3. All members are as specified in the original design documents and are in good condition.
4. All required members are in place.
5. All bolts are in place and are properly tightened.
6. Tower is in plumb condition.
7. All member protective coatings are in good condition.
8. All tower members were properly designed, detailed, fabricated, and installed and have been properly maintained since erection.
9. Foundations were properly constructed to support original design loads as specified in the original design documents.
10. All coaxial cable is installed as specified in Section 6 of this report.

URS is not responsible for any modifications completed prior to or hereafter in which URS is not or was not directly involved. Modifications include but are not limited to:

- A. Adding antennas
- B. Removing/replacing antennas
- C. Adding coaxial cables

URS hereby states that this document represents the entire report and that it assumes no liability for any factual changes that may occur after the date of this report. All representations, recommendations, and conclusions are based upon information contained and set forth herein. If you are aware of any information which conflicts with that which is contained herein, or you are aware of any defects arising from original design, material, fabrication, or erection deficiencies, you should disregard this report and immediately contact URS. URS disclaims all liability for any representation, recommendation, or conclusion not expressly stated herein.

Ongoing and Periodic Inspection and Maintenance:

After the Contractor has successfully completed the installation and the work has been accepted, the owner will be responsible for the ongoing and periodic inspection and maintenance of the tower.

The owner shall refer to TIA/EIA-222-F for recommendations for maintenance and inspection. The frequency of the inspection and maintenance intervals is to be determined by the owner based upon actual site and environmental conditions. It is recommended that a complete and thorough inspection of the entire tower structural system be performed at least yearly and more frequently as conditions warrant. According to TIA/EIA-222-F section 14.1, Note 1: It is recommended that the structure be inspected after severe wind and/or ice storms or other extreme loading conditions.

6. DRAWINGS AND DATA

REINFORCEMENT DRAWINGS SK-1 & SK-2

STRUCTURAL NOTES

THE CONTRACTOR IS RESPONSIBLE FOR THE STABILITY OF THE STRUCTURE DURING CONSTRUCTION. IT IS RECOMMENDED THAT THE REPLACEMENT OF TOWER MEMBERS SHALL BE DONE ONE AT A TIME AND SHALL BE DONE WITH LESS THAN 15 MPH WIND PRESENT. NO MEMBER SHALL BE LEFT DISCONNECTED FOR THE NEXT WORKING DAY.

STRUCTURAL STEEL MATERIAL:

STRUCTURAL PLATES ASTM A372 GR60
STEEL BEAMS, CHANNELS & ANGLES ASTM A36

MODIFICATIONS SHOWN ARE FOR EACH FACE OR LEG UNLESS NOTED OTHERWISE

UNLESS OTHERWISE NOTED, ALL STEEL WILL BE GALVANIZED IN ACCORDANCE WITH ASTM 123 AFTER FABRICATION. TOUCH UP ALL DAMAGED GALVANIZED STEEL WITH APPROVED COLD ZINC, "GALVANOX", "DRY GALV", "ZINC-IT", OR APPROVED EQUIVALENT, IN ACCORDANCE WITH MANUFACTURERS GUIDELINES. TOUCH-UP DAMAGED NON GALVANIZED STEEL WITH SAME PAINT APPLIED IN SHOP OR FIELD.

EXISTING DIMENSIONS OF STRUCTURE SHOWN ON THESE DOCUMENTS ARE NOT GUARANTEED. CONTRACTOR SHALL TAKE FIELD DIMENSIONS AS NECESSARY TO ASSURE PROPER FIT OF ALL FINISHED WORK AND SHALL ASSUME FULL RESPONSIBILITY FOR THEIR ACCURACY. WHEN SHOP DRAWINGS BASED ON FIELD MEASUREMENT ARE SUBMITTED FOR REVIEW, DIMENSIONS ARE PROVIDED FOR THE ENGINEER'S REFERENCE ONLY.

ALL WELDING SHALL BE DONE BY A CERTIFIED WELDER IN ACCORDANCE WITH AWS STANDARDS, USING E70XX ELECTRODES UNLESS OTHERWISE NOTED. WHERE WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZES PER "PREQUALIFIED WELDED JOINTS" TABLES IN AISC "MANUAL OF STEEL CONSTRUCTION", NINTH EDITION.

IF WELDING GALVANIZED MATERIALS, USE PRECAUTIONS & PROCEDURES PER AWS D1.1.

MILL BEARING ENDS OF COLUMNS, STIFFENERS, AND OTHER BEARING SURFACES TO TRANSFER LOAD OVER ENTIRE CROSS SECTION.

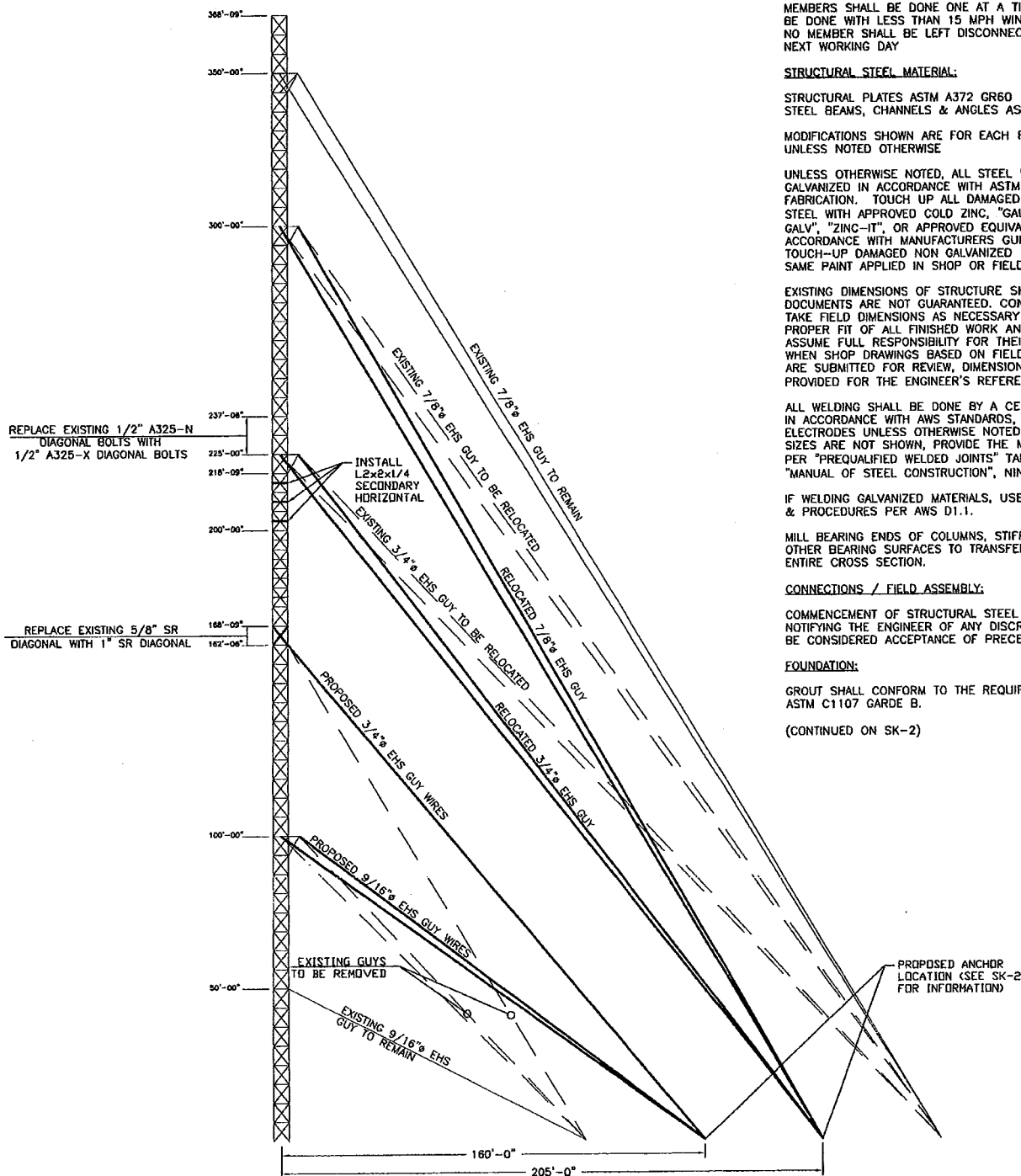
CONNECTIONS / FIELD ASSEMBLY:

COMMENCEMENT OF STRUCTURAL STEEL WORK WITHOUT NOTIFYING THE ENGINEER OF ANY DISCREPANCIES WILL BE CONSIDERED ACCEPTANCE OF PRECEDING WORK.

FOUNDATION:

GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C1107 GARDE B.

(CONTINUED ON SK-2)



1 TOWER ELEVATION
SK-1 SCALE: 1" = 50'-0"

GW NO:
36922267
Designed by:
KAB
Drawn by:
KAB
Checked by:
MJK
Approved by:
RAS

URS CORPORATION AES
500 ENTERPRISE DRIVE
ROCKY HILL, CONNECTICUT
1-(860)-529-8882

CELLCO PARTNERSHIP DBA
VERIZON WIRELESS
WIRELESS COMMUNICATIONS FACILITY

SITE
ADDRESS:

MONTVILLE CT
695 OLD COLCHESTER ROAD
MONTVILLE, CONNECTICUT 06353

REV.	DATE:	DESCRIPTION

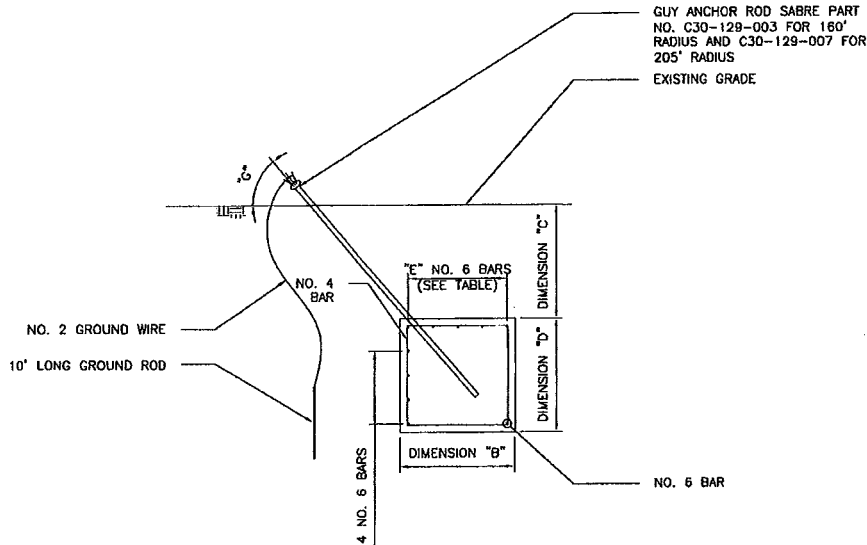
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Job No. VZ5-109 File No. Dwg. 1 of 2

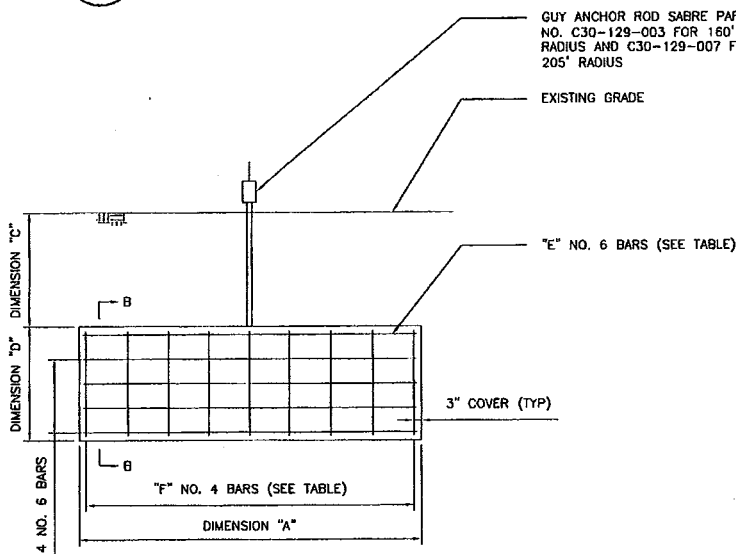
Dwg. No.

SK-1

ANCHOR RADIUS	A	B	C	D	E	F	G
160'-0"	10'-0"	4'-0"	6'-0"	4'-0"	5	8	38 DEG
205'-0"	16'-0"	6'-0"	6'-0"	4'-0"	7	12	40 DEG



2 SECTION B-B
SK-2 SCALE: N.T.S.



1 PROPOSED GUY ANCHOR
SK-2 SCALE: N.T.S.

SON

1. SOIL BEARING CAPACITY OF 5,000 PSF USED FOR FOUNDATION DESIGN. GENERAL CONTRACTOR RESPONSIBLE FOR VERIFYING BEARING CAPACITIES.
2. ALL SURFACES MUST BE FREE OF STANDING WATER PRIOR TO PLACING.
3. COMPACTED GRAVEL FILL PER CONNECTICUT DOT STANDARD SPEC. SECTION M.02.01 AND ASTM D1557.
4. CONTACT THE ENGINEER IF GROUND WATER IS IN ENCOUNTERED AND DEWATERING IS REQUIRED.

CONCRETE

1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318 AND THE SPECIFICATION CAST-IN-PLACE CONCRETE.
2. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI 28 DAYS. UNLESS NOTED OTHERWISE CONCRETE SHALL BE AIR ENTRAINMENT TO (4% TO 6%) AND SLUMP OF 3" TO 5".
3. REINFORCING STEEL SHALL CONFORM TO ASTM A 615, GRADE 60, DEFORMED UNLESS NOTED OTHERWISE. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185 WELDED STEEL WIRE FABRIC UNLESS NOTED OTHERWISE. SPICES SHALL BE CLASS "B" AND ALL HOOKS SHALL BE STANDARD UNLESS NOTED OTHERWISE.
4. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:

CONCRETE CAST AGAINST EARTH.....3 IN.

CONCRETE EXPOSED TO EARTH OR WEATHER:

#6 AND LARGER.....2 IN.

3/5 AND SMALLER & WWF1 1/2 IN.

CONCRETE NOT EXPOSED TO EARTH OR WEATHER OR NOT CAST AGAINST THE GROUND:

SLAB AND WALL.....3/4 IN.

BEAMS AND COLUMNS.....1 1/2 IN.

5. A CHAMFER 3/4" SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNO, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.
6. INSTALLATION OF CONCRETE EXPANSION/WEDGE ANCHOR, SHALL BE PER MANUFACTURER'S WRITTEN RECOMMENDED PROCEDURE, THE ANCHOR BOLT, DUB OR ROD SHALL BE CONFORM TO MANUFACTURER'S RECOMMENDATION FOR EMBEDMENT DEPTH OR SHOWN ON THE DRAWINGS. ANCHOR NEAR SHALL BE CUT WITHOUT PRIOR ENGINEERING APPROVAL WHEN DRILLING. COLD WEATHER CONCRETE PLACING SHALL BE IN ACCORDANCE WITH ACI-308.
7. FORMWORK SHALL BE PLACED ON FROZEN GROUND. UNCURED CONCRETE SHALL BE PROTECTED AGAINST FROST.
8. APPLY NON-SLIP BROOM FINISH IMMEDIATELY AFTER TROWEL FINISHING.

FOUNDATION NOTES

1. A PRESUMPTIVE SOIL BEARING CAPACITY OF 5000 PSF WAS USED FOR THE FOUNDATION DESIGN. THE GENERAL CONTRACTOR IS TO CONFIRM THE EXISTING SOIL BEARING PRESSURE.
2. ALL FOOTINGS SHALL BEAR ON EXISTING UNDISTURBED ORGANIC FREE SOIL. ALL UNSUITABLE SOIL SHALL BE REMOVED AS DIRECTED BY THE ENGINEER AND REPLACED WITH COMMERCE GRADE FILL PLACED IN 8" LAYERS AND COMPACTED TO 95% OF MODIFIED OPTIMUM DENSITY.
3. ALL FOOTINGS TO BE A MINIMUM OF 3'-6" BELOW FINISH GRADE UNLESS OTHERWISE NOTED. EXCEPT WHERE ROCK OR LEDGE OCCURS, PIN FOUNDATION TO ROCK.
4. INSTALLATION OF CONCRETE EXPANSION/WEDGE ANCHOR, SHALL BE PER MANUFACTURER'S WRITTEN RECOMMENDED PROCEDURE, THE ANCHOR BOLT, DOWEL OR ROD SHALL CONFORM TO MANUFACTURER'S RECOMMENDATION FOR EMBEDMENT DEPTH OR AS SHOWN ON THE DRAWINGS. NO REBAR SHALL BE CUT WITHOUT PRIOR ENGINEERING APPROVAL WHEN DRILLING HOLES IN CONCRETE.

OW NO: 36922267
Designed by: KAB
Drawn by: KAB
Checked by: MJK
Approved by: RAS

500 ENTERPRISE DRIVE
ROCKY HILL, CONNECTICUT
1-(860)-529-8882

**CELLCO PARTNERSHIP DBA
VERIZON WIRELESS
WIRELESS COMMUNICATIONS FACILITY**

SITE ADDRESS:

MONTVILLE CT
695 OLD COLCHESTER ROAD
MONTVILLE, CONNECTICUT 06353

Dwg. No.

SK-2

REV.	DATE:	DESCRIPTION
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Scale:

Date: 10-11-12

Job No. VZ5-109

File No.

Dwg. 2 of 2

TNX TOWER INPUT/OUTPUT SUMMARY

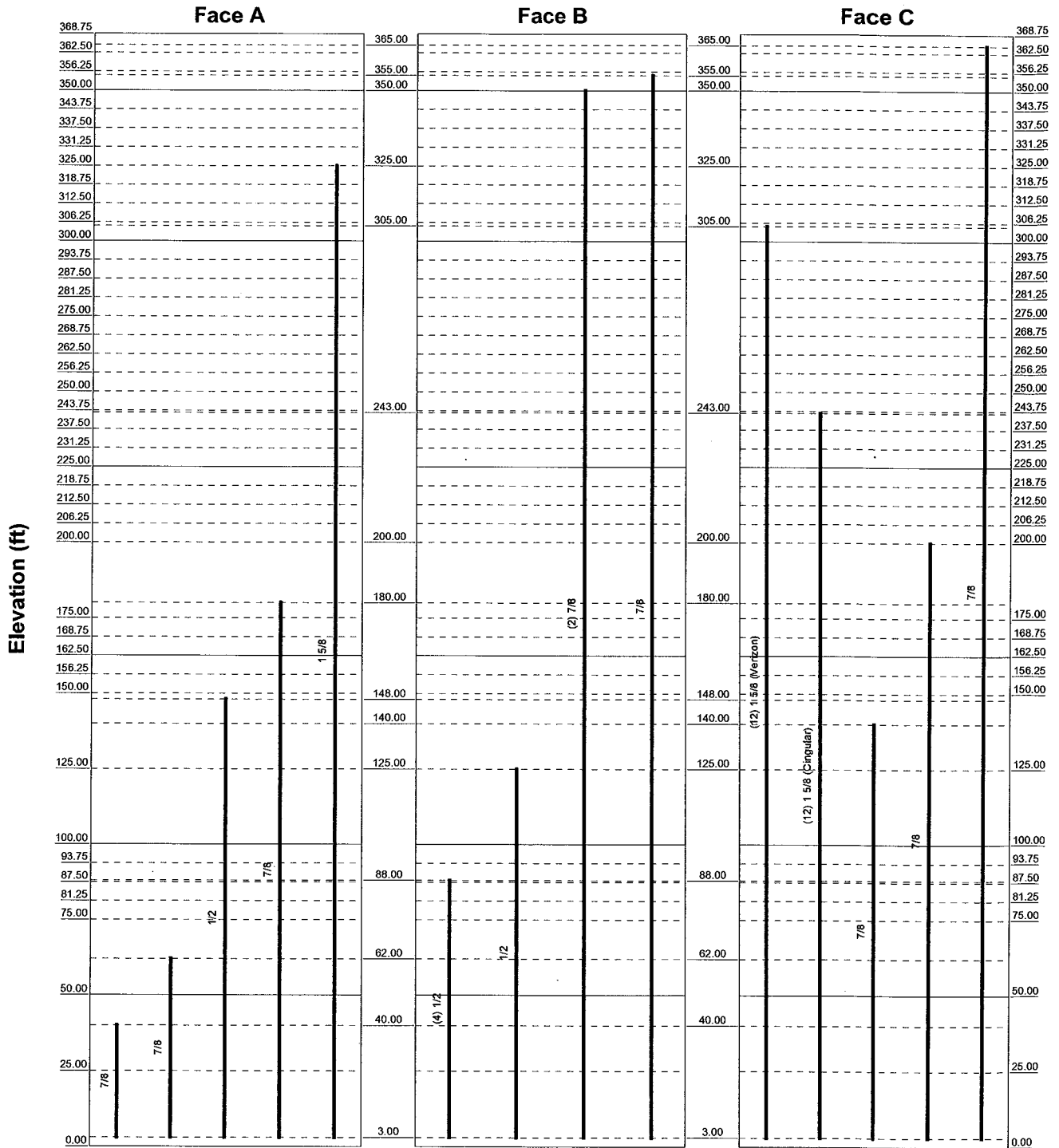
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TNX TOWER FEEDLINE DISTRIBUTION

Feedline Distribution Chart

0' - 368'9"

Round Flat App In Face App Out Face Truss Leg



URS Corporation			
500 Enterprise Drive, Suite 3B			
Rocky Hill, CT 06067			
Phone: (860) 529-8882			
FAX: (860) 529-3991			
Job: 370' Guyed Lattice Tower			
Project: Montville, CT			
Client: Verizon Wireless	Drawn by: Kevin Barker	App'd:	
Code: TIA/EIA-222-F	Date: 10/11/12	Scale: NTS	
Path: P:\08\ERI Files\Montville.eri			
			Dwg No. E-7

TNX TOWER DETAILED OUTPUT

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job 370' Guyed Lattice Tower	Page 1 of 98
	Project Montville, CT	Date 09:02:33 10/11/12
	Client Verizon Wireless	Designed by Kevin_Barker

Tower Input Data

The main tower is a 3x guyed tower with an overall height of 368.75 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 5.00 ft at the top and 5.00 ft at the base.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Basic wind speed of 95 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 82 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

Tension only take-up is 0.0313 in.

Pressures are calculated at each section.

Safety factor used in guy design is 2.

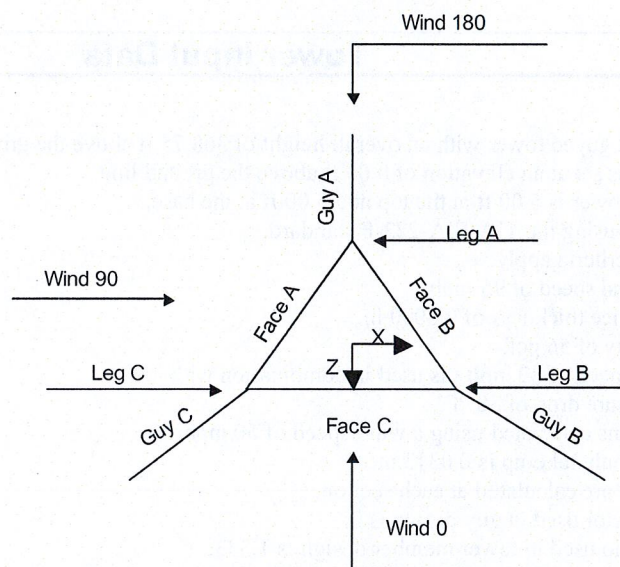
Stress ratio used in tower member design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Options

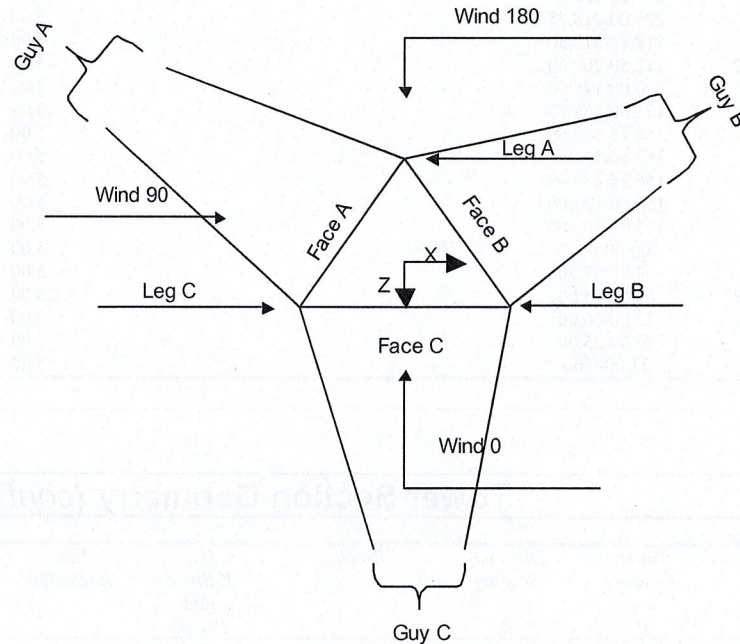
Consider Moments - Legs	Distribute Leg Loads As Uniform	Treat Feedline Bundles As Cylinder
Consider Moments - Horizontals	Assume Legs Pinned	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Diagonals	✓ Assume Rigid Index Plate	✓ Calculate Redundant Bracing Forces
Use Moment Magnification	✓ Use Clear Spans For Wind Area	Ignore Redundant Members in FEA
✓ Use Code Stress Ratios	✓ Use Clear Spans For KL/r	SR Leg Bolts Resist Compression
✓ Use Code Safety Factors - Guys	✓ Retension Guys To Initial Tension	✓ All Leg Panels Have Same Allowable
Escalate Ice	Bypass Mast Stability Checks	Offset Girt At Foundation
Always Use Max Kz	Use Azimuth Dish Coefficients	✓ Consider Feedline Torque
Use Special Wind Profile	Project Wind Area of Appurt.	Include Angle Block Shear Check
✓ Include Bolts In Member Capacity	✓ Autocalc Torque Arm Areas	Poles
✓ Leg Bolts Are At Top Of Section	✓ SR Members Have Cut Ends	Include Shear-Torsion Interaction
✓ Secondary Horizontal Braces Leg	✓ Sort Capacity Reports By Component	Always Use Sub-Critical Flow
Use Diamond Inner Bracing (4 Sided)	Triangulate Diamond Inner Bracing	Use Top Mounted Sockets
Add IBC .6D+W Combination		

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job 370' Guyed Lattice Tower	Page 2 of 98
	Project Montville, CT	Date 09:02:33 10/11/12
	Client Verizon Wireless	Designed by Kevin_Barker



Corner & Starmount Guyed Tower

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job 370' Guyed Lattice Tower	Page 3 of 98
	Project Montville, CT	Date 09:02:33 10/11/12
	Client Verizon Wireless	Designed by Kevin_Barker



Tower Section Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	ft			ft		ft
T1	368.75-362.50			5.00	1	6.25
T2	362.50-356.25			5.00	1	6.25
T3	356.25-350.00			5.00	1	6.25
T4	350.00-343.75			5.00	1	6.25
T5	343.75-337.50			5.00	1	6.25
T6-T7	337.50-325.00			5.00	2	6.25
T8-T10	325.00-306.25			5.00	3	6.25
T11	306.25-300.00			5.00	1	6.25
T12	300.00-293.75			5.00	1	6.25
T13	293.75-287.50			5.00	1	6.25
T14-T15	287.50-275.00			5.00	2	6.25
T16-T18	275.00-256.25			5.00	3	6.25
T19	256.25-250.00			5.00	1	6.25
T20	250.00-243.75			5.00	1	6.25
T21	243.75-237.50			5.00	1	6.25
T22	237.50-231.25			5.00	1	6.25

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Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	ft			ft		ft
T23	231.25-225.00			5.00	1	6.25
T24	225.00-218.75			5.00	1	6.25
T25	218.75-212.50			5.00	1	6.25
T26-T27	212.50-200.00			5.00	2	6.25
T28	200.00-175.00			5.00	1	25.00
T29	175.00-168.75			5.00	1	6.25
T30	168.75-162.50			5.00	1	6.25
T31	162.50-156.25			5.00	1	6.25
T32	156.25-150.00			5.00	1	6.25
T33	150.00-125.00			5.00	1	25.00
T34	125.00-100.00			5.00	1	25.00
T35	100.00-93.75			5.00	1	6.25
T36	93.75-87.50			5.00	1	6.25
T37-T38	87.50-75.00			5.00	2	6.25
T39	75.00-50.00			5.00	1	25.00
T40	50.00-25.00			5.00	1	25.00
T41	25.00-0.00			5.00	1	25.00

Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T1	368.75-362.50	6.25	X Brace	No	Yes	0.0000	0.0000
T2	362.50-356.25	6.25	X Brace	No	Yes	0.0000	0.0000
T3	356.25-350.00	6.25	X Brace	No	Yes	0.0000	0.0000
T4	350.00-343.75	6.25	X Brace	No	Yes	0.0000	0.0000
T5	343.75-337.50	6.25	TX Brace	No	Yes	0.0000	0.0000
T6-T7	337.50-325.00	6.25	TX Brace	No	Yes	0.0000	0.0000
T8-T10	325.00-306.25	6.25	TX Brace	No	Yes	0.0000	0.0000
T11	306.25-300.00	6.25	TX Brace	No	Yes	0.0000	0.0000
T12	300.00-293.75	6.25	X Brace	No	Yes	0.0000	0.0000
T13	293.75-287.50	6.25	TX Brace	No	Yes	0.0000	0.0000
T14-T15	287.50-275.00	6.25	TX Brace	No	Yes	0.0000	0.0000
T16-T18	275.00-256.25	6.25	TX Brace	No	Yes	0.0000	0.0000
T19	256.25-250.00	6.25	TX Brace	No	Yes	0.0000	0.0000
T20	250.00-243.75	6.25	TX Brace	No	Yes	0.0000	0.0000
T21	243.75-237.50	6.25	TX Brace	No	Yes	0.0000	0.0000
T22	237.50-231.25	6.25	TX Brace	No	Yes	0.0000	0.0000
T23	231.25-225.00	6.25	TX Brace	No	Yes	0.0000	0.0000
T24	225.00-218.75	6.25	X Brace	No	Yes	0.0000	0.0000
T25	218.75-212.50	6.25	TX Brace	No	Yes	0.0000	0.0000
T26-T27	212.50-200.00	6.25	TX Brace	No	Yes	0.0000	0.0000
T28	200.00-175.00	6.25	TX Brace	No	Yes	0.0000	0.0000
T29	175.00-168.75	6.25	TX Brace	No	Yes	0.0000	0.0000
T30	168.75-162.50	6.25	TX Brace	No	Yes	0.0000	0.0000
T31	162.50-156.25	6.25	TX Brace	No	Yes	0.0000	0.0000
T32	156.25-150.00	6.25	TX Brace	No	Yes	0.0000	0.0000
T33	150.00-125.00	6.25	TX Brace	No	Yes	0.0000	0.0000
T34	125.00-100.00	6.25	TX Brace	No	Yes	0.0000	0.0000
T35	100.00-93.75	6.25	X Brace	No	Yes	0.0000	0.0000
T36	93.75-87.50	6.25	TX Brace	No	Yes	0.0000	0.0000
T37-T38	87.50-75.00	6.25	TX Brace	No	Yes	0.0000	0.0000
T39	75.00-50.00	6.25	TX Brace	No	Yes	0.0000	0.0000

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Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T40	50.00-25.00	6.25	TX Brace	No	Yes	0.0000	0.0000
T41	25.00-0.00	6.25	TX Brace	No	Yes	0.0000	0.0000

Tower Section Geometry (cont'd)

Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
ft						
T1 368.75-362.50	Solid Round	2 3/4	A36 (36 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T2 362.50-356.25	Solid Round	2 3/4	A36 (36 ksi)	Double Equal Angle	2L3x3x5/16	A36 (36 ksi)
T3 356.25-350.00	Solid Round	2 3/4	A36 (36 ksi)	Double Equal Angle	2L3x3x5/16	A36 (36 ksi)
T4 350.00-343.75	Solid Round	3	A36 (36 ksi)	Single Angle	L3x2 1/2x1/4	A36 (36 ksi)
T5 343.75-337.50	Solid Round	3	A36 (36 ksi)	Solid Round	5/8	A36 (36 ksi)
T6-T7	Solid Round	3	A36 (36 ksi)	Solid Round	5/8	A36 (36 ksi)
337.50-325.00						
T8-T10	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	3/4	A36 (36 ksi)
325.00-306.25						
T11	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	3/4	A36 (36 ksi)
306.25-300.00						
T12	Solid Round	3 1/4	A36 (36 ksi)	Single Angle	L3x2 1/2x1/4	A36 (36 ksi)
300.00-293.75						
T13	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	3/4	A36 (36 ksi)
293.75-287.50						
T14-T15	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	5/8	A36 (36 ksi)
287.50-275.00						
T16-T18	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	5/8	A36 (36 ksi)
275.00-256.25						
T19	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	3/4	A36 (36 ksi)
256.25-250.00						
T20	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	3/4	A36 (36 ksi)
250.00-243.75						
T21	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	3/4	A36 (36 ksi)
243.75-237.50						
T22	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	3/4	A36 (36 ksi)
237.50-231.25						
T23	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	3/4	A36 (36 ksi)
231.25-225.00						
T24	Solid Round	3	A36 (36 ksi)	Double Equal Angle	2L2 1/2x2 1/2x1/4	A36 (36 ksi)
225.00-218.75						
T25	Solid Round	3	A36 (36 ksi)	Solid Round	5/8	A36 (36 ksi)
218.75-212.50						
T26-T27	Solid Round	3	A36 (36 ksi)	Solid Round	5/8	A36 (36 ksi)
212.50-200.00						
T28	Solid Round	3	A36 (36 ksi)	Solid Round	5/8	A36 (36 ksi)
200.00-175.00						
T29	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	5/8	A36 (36 ksi)
175.00-168.75						
T30	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
168.75-162.50						
T31	Solid Round	3 1/4	A36 (36 ksi)	Solid Round	5/8	A36 (36 ksi)
162.50-156.25						
T32	Solid Round	3 1/4	A36	Solid Round	5/8	A36

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Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
156.25-150.00			(36 ksi)			(36 ksi)
T33	Solid Round	3 1/4	A36	Solid Round	5/8	A36
150.00-125.00			(36 ksi)			(36 ksi)
T34	Solid Round	3 1/4	A36	Equal Angle	L2 1/2x2 1/2x3/16	A36
125.00-100.00			(36 ksi)			(36 ksi)
T35 100.00-93.75	Solid Round	3 1/4	A36	Double Equal Angle	2L2 1/2x2 1/2x1/4	A36
T36 93.75-87.50	Solid Round	3 1/4	A36	Solid Round	3/4	A36
			(36 ksi)			(36 ksi)
T37-T38	Solid Round	3 1/4	A36	Solid Round	5/8	A36
87.50-75.00			(36 ksi)			(36 ksi)
T39 75.00-50.00	Solid Round	3 1/4	A36	Solid Round	5/8	A36
			(36 ksi)			(36 ksi)
T40 50.00-25.00	Solid Round	3 1/4	A36	Solid Round	5/8	A36
			(36 ksi)			(36 ksi)
T41 25.00-0.00	Solid Round	3 1/4	A36	Solid Round	5/8	A36
			(36 ksi)			(36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 368.75-362.50	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
			(36 ksi)			(36 ksi)
T2 362.50-356.25	Double Angle	2L2 1/2x3x1/4	A36	Pipe		A36
			(36 ksi)			(36 ksi)
T3 356.25-350.00	Double Angle	2L2 1/2x3x1/4	A36	Pipe		A36
			(36 ksi)			(36 ksi)
T4 350.00-343.75	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
			(36 ksi)			(36 ksi)
T5 343.75-337.50	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
			(36 ksi)			(36 ksi)
T6-T7	Pipe	P1.25x.14	A36	Pipe		A36
337.50-325.00			(36 ksi)			(36 ksi)
T8-T10	Pipe	P1.25x.14	A36	Pipe		A36
325.00-306.25			(36 ksi)			(36 ksi)
T11	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
306.25-300.00			(36 ksi)			(36 ksi)
T12	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
300.00-293.75			(36 ksi)			(36 ksi)
T13	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
293.75-287.50			(36 ksi)			(36 ksi)
T14-T15	Pipe	P1.25x.14	A36	Pipe		A36
287.50-275.00			(36 ksi)			(36 ksi)
T16-T18	Pipe	P1.25x.14	A36	Pipe		A36
275.00-256.25			(36 ksi)			(36 ksi)
T19	Pipe	P1.25x.14	A36	Pipe		A36
256.25-250.00			(36 ksi)			(36 ksi)
T20	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
250.00-243.75			(36 ksi)			(36 ksi)
T21	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
243.75-237.50			(36 ksi)			(36 ksi)
T22	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
237.50-231.25			(36 ksi)			(36 ksi)
T23	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36

tnxTower

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Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
231.25-225.00			(36 ksi)			(36 ksi)
T24	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
225.00-218.75			(36 ksi)			(36 ksi)
T25	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
218.75-212.50			(36 ksi)			(36 ksi)
T26-T27	Pipe	P1.25x.14	A36	Pipe		A36
212.50-200.00			(36 ksi)			(36 ksi)
T28	Pipe	P1.25x.14	A36	Pipe		A36
200.00-175.00			(36 ksi)			(36 ksi)
T29	Pipe	P1.25x.14	A36	Pipe		A36
175.00-168.75			(36 ksi)			(36 ksi)
T30	Pipe	P1.25x.14	A36	Pipe		A36
168.75-162.50			(36 ksi)			(36 ksi)
T31	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
162.50-156.25			(36 ksi)			(36 ksi)
T32	Pipe	P1.25x.14	A36	Pipe		A36
156.25-150.00			(36 ksi)			(36 ksi)
T33	Pipe	P1.25x.14	A36	Pipe		A36
150.00-125.00			(36 ksi)			(36 ksi)
T34	Pipe	P1.25x.14	A36	Pipe		A36
125.00-100.00			(36 ksi)			(36 ksi)
T35 100.00-93.75	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
			(36 ksi)			(36 ksi)
T36 93.75-87.50	Double Angle	2L2 1/2x2x1/4	A36	Pipe		A36
			(36 ksi)			(36 ksi)
T37-T38	Pipe	P1.25x.14	A36	Pipe		A36
87.50-75.00			(36 ksi)			(36 ksi)
T39 75.00-50.00	Pipe	P1.25x.14	A36	Pipe		A36
			(36 ksi)			(36 ksi)
T40 50.00-25.00	Single Angle	L2 1/2x2x1/4	A36	Pipe		A36
			(36 ksi)			(36 ksi)
T41 25.00-0.00	Pipe	P1.25x.14	A36	Pipe		A36
			(36 ksi)			(36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T1 368.75-362.50	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T2 362.50-356.25	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T3 356.25-350.00	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T4 350.00-343.75	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T5 343.75-337.50	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T6-T7	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
337.50-325.00							
T8-T10	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
325.00-306.25							
T11	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
306.25-300.00							

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Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T12 300.00-293.75	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T13 293.75-287.50	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T14-T15 287.50-275.00	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T16-T18 275.00-256.25	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T19 256.25-250.00	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T20 250.00-243.75	None	Flat Bar		A36 (36 ksi)	Double Angle	2L2 1/2x2x1/4	A36 (36 ksi)
T21 243.75-237.50	None	Flat Bar		A36 (36 ksi)	Double Angle	2L2 1/2x2x1/4	A36 (36 ksi)
T22 237.50-231.25	None	Flat Bar		A36 (36 ksi)	Double Angle	2L2 1/2x2x1/4	A36 (36 ksi)
T23 231.25-225.00	None	Flat Bar		A36 (36 ksi)	Double Angle	2L2 1/2x2x1/4	A36 (36 ksi)
T24 225.00-218.75	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T25 218.75-212.50	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T26-T27 212.50-200.00	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T28 200.00-175.00	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T29 175.00-168.75	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T30 168.75-162.50	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T31 162.50-156.25	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T32 156.25-150.00	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T33 150.00-125.00	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T34 125.00-100.00	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T35 100.00-93.75	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T36 93.75-87.50	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T37-T38 87.50-75.00	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T39 75.00-50.00	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T40 50.00-25.00	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)
T41 25.00-0.00	None	Flat Bar		A36 (36 ksi)	Pipe	P1.25x.14	A36 (36 ksi)

Tower Section Geometry (cont'd)

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Tower Elevation	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
<i>ft</i>						
T25 218.75-212.50	Equal Angle	L2x2x1/4	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T26-T27 212.50-200.00	Equal Angle	L2x2x1/4	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T28 200.00-175.00	Pipe	P1.25x.14	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)

Tower Section Geometry (cont'd)

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A_f	Adjust. Factor A_r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals <i>in</i>	Double Angle Stitch Bolt Spacing Horizontals <i>in</i>
<i>ft</i>	<i>ft²</i>	<i>in</i>						
T1 368.75-362.50	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T2 362.50-356.25	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T3 356.25-350.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T4 350.00-343.75	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T5 343.75-337.50	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T6-T7 337.50-325.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T8-T10 325.00-306.25	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T11 306.25-300.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T12 300.00-293.75	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T13 293.75-287.50	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T14-T15 287.50-275.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T16-T18 275.00-256.25	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T19 256.25-250.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T20 250.00-243.75	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T21 243.75-237.50	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T22 237.50-231.25	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T23 231.25-225.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T24 225.00-218.75	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T25 218.75-212.50	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T26-T27 212.50-200.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T28	0.00	0.0000	A36	1	1	1	36.0000	36.0000

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	10 of 98
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Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A_f	Adjust. Factor A_r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft ²	in					in	in
200.00-175.00			(36 ksi)					
T29	0.00	0.0000	A36	1	1	1	36.0000	36.0000
175.00-168.75			(36 ksi)					
T30	0.00	0.0000	A36	1	1	1	36.0000	36.0000
168.75-162.50			(36 ksi)					
T31	0.00	0.0000	A36	1	1	1	36.0000	36.0000
162.50-156.25			(36 ksi)					
T32	0.00	0.0000	A36	1	1	1	36.0000	36.0000
156.25-150.00			(36 ksi)					
T33	0.00	0.0000	A36	1	1	1	36.0000	36.0000
150.00-125.00			(36 ksi)					
T34	0.00	0.0000	A36	1	1	1	36.0000	36.0000
125.00-100.00			(36 ksi)					
T35	0.00	0.0000	A36	1	1	1	36.0000	36.0000
100.00-93.75			(36 ksi)					
T36	0.00	0.0000	A36	1	1	1	36.0000	36.0000
93.75-87.50			(36 ksi)					
T37-T38	0.00	0.0000	A36	1	1	1	36.0000	36.0000
87.50-75.00			(36 ksi)					
T39	0.00	0.0000	A36	1	1	1	36.0000	36.0000
75.00-50.00			(36 ksi)					
T40	0.00	0.0000	A36	1	1	1	36.0000	36.0000
50.00-25.00			(36 ksi)					
T41 25.00-0.00	0.00	0.0000	A36	1	1	1	36.0000	36.0000
			(36 ksi)					

Tower Section Geometry (cont'd)

Tower Elevation	Calc K Single Angles	Calc K Solid Rounds	Legs	K Factors ¹						
				X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace
				X Y	X Y	X Y	X Y	X Y	X Y	X Y
T1	Yes	Yes	1	1	1	1	1	1	1	1
368.75-362.50				1	1	1	1	1	1	1
T2	Yes	Yes	1	1	1	1	1	1	1	1
362.50-356.25				1	1	1	1	1	1	1
T3	Yes	Yes	1	1	1	1	1	1	1	1
356.25-350.00				1	1	1	1	1	1	1
T4	Yes	Yes	1	1	1	1	1	1	1	1
350.00-343.75				1	1	1	1	1	1	1
T5	Yes	Yes	1	1	1	1	1	1	1	1
343.75-337.50				1	1	1	1	1	1	1
T6-T7	Yes	Yes	1	1	1	1	1	1	1	1
337.50-325.00				1	1	1	1	1	1	1
T8-T10	Yes	Yes	1	1	1	1	1	1	1	1
325.00-306.25				1	1	1	1	1	1	1
T11	Yes	Yes	1	1	1	1	1	1	1	1
306.25-300.00				1	1	1	1	1	1	1
T12	Yes	Yes	1	1	1	1	1	1	1	1
300.00-293.75				1	1	1	1	1	1	1
T13	Yes	Yes	1	1	1	1	1	1	1	1
293.75-287.50				1	1	1	1	1	1	1
T14-T15	Yes	Yes	1	1	1	1	1	1	1	1

Tower Elevation ft	Calc K Single Angles	Calc K Solid Rounds	Legs	K Factors ¹						
				X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace
				X Y	X Y	X Y	X Y	X Y	X Y	X Y
287.50-275.00				1	1	1	1	1	1	1
T16-T18	Yes	Yes	1	1	1	1	1	1	1	1
275.00-256.25				1	1	1	1	1	1	1
T19	Yes	Yes	1	1	1	1	1	1	1	1
256.25-250.00				1	1	1	1	1	1	1
T20	Yes	Yes	1	1	1	1	1	1	1	1
250.00-243.75				1	1	1	1	1	1	1
T21	Yes	Yes	1	1	1	1	1	1	1	1
243.75-237.50				1	1	1	1	1	1	1
T22	Yes	Yes	1	1	1	1	1	1	1	1
237.50-231.25				1	1	1	1	1	1	1
T23	Yes	Yes	1	1	1	1	1	1	1	1
231.25-225.00				1	1	1	1	1	1	1
T24	Yes	Yes	1	1	1	1	1	1	1	1
225.00-218.75				1	1	1	1	1	1	1
T25	Yes	Yes	1	1	1	1	1	1	1	1
218.75-212.50				1	1	1	1	1	1	1
T26-T27	Yes	Yes	1	1	1	1	1	1	1	1
212.50-200.00				1	1	1	1	1	1	1
T28	Yes	Yes	1	1	1	1	1	1	1	1
200.00-175.00				1	1	1	1	1	1	1
T29	Yes	Yes	1	1	1	1	1	1	1	1
175.00-168.75				1	1	1	1	1	1	1
T30	Yes	Yes	1	1	1	1	1	1	1	1
168.75-162.50				1	1	1	1	1	1	1
T31	Yes	Yes	1	1	1	1	1	1	1	1
162.50-156.25				1	1	1	1	1	1	1
T32	Yes	Yes	1	1	1	1	1	1	1	1
156.25-150.00				1	1	1	1	1	1	1
T33	Yes	Yes	1	1	1	1	1	1	1	1
150.00-125.00				1	1	1	1	1	1	1
T34	Yes	Yes	1	1	1	1	1	1	1	1
125.00-100.00				1	1	1	1	1	1	1
T35	Yes	Yes	1	1	1	1	1	1	1	1
100.00-93.75				1	1	1	1	1	1	1
T36	Yes	Yes	1	1	1	1	1	1	1	1
93.75-87.50				1	1	1	1	1	1	1
T37-T38	Yes	Yes	1	1	1	1	1	1	1	1
87.50-75.00				1	1	1	1	1	1	1
T39	Yes	Yes	1	1	1	1	1	1	1	1
75.00-50.00				1	1	1	1	1	1	1
T40	Yes	Yes	1	1	1	1	1	1	1	1
50.00-25.00				1	1	1	1	1	1	1
T41	Yes	Yes	1	1	1	1	1	1	1	1
25.00-0.00				1	1	1	1	1	1	1

¹Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

Tower Section Geometry (cont'd)

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Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T36 93.75-87.50	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T37-T38 87.50-75.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T39 75.00-50.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T40 50.00-25.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T41 25.00-0.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 368.75-362.50	Flange	0.7500	6	0.5000	2	0.6250	2	0.0000	0	0.6250	0	0.6250	2	0.6250	0
T2 362.50-356.25	Flange	A325N	0	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T3 356.25-350.00	Flange	A325N	0	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T4 350.00-343.75	Flange	A325N	6	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T5 343.75-337.50	Flange	A325N	0	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T6-T7 337.50-325.00	Flange	A325N	0	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T8-T10 325.00-306.25	Flange	A325N	6	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T11 306.25-300.00	Flange	A325N	0	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T12 300.00-293.75	Flange	A325N	6	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T13 293.75-287.50	Flange	A325N	0	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T14-T15 287.50-275.00	Flange	A325N	0	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T16-T18 275.00-256.25	Flange	A325N	6	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T19 256.25-250.00	Flange	A325N	0	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T20 250.00-243.75	Flange	A325N	6	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T21 243.75-237.50	Flange	A325N	0	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T22 237.50-231.25	Flange	A325N	0	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0
T23 231.25-225.00	Flange	A325N	0	A325N	2	A325N	2	A325N	0	A325N	0	A325N	2	A325N	0

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Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T24	Flange	0.7500	6	0.6250	2	0.6250	2	0.0000	0	0.6250	0	0.6250	2	0.6250	0
225.00-218.75		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T25	Flange	0.7500	0	0.5000	2	0.6250	2	0.0000	0	0.6250	0	0.6250	2	0.6250	0
218.75-212.50		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T26-T27	Flange	0.7500	0	0.5000	2	0.5000	2	0.0000	0	0.6250	0	0.6250	2	0.6250	0
212.50-200.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T28	Flange	0.7500	6	0.5000	2	0.5000	2	0.6250	0	0.6250	0	0.6250	2	0.6250	0
200.00-175.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T29	Flange	0.7500	6	0.5000	2	0.5000	2	0.0000	0	0.6250	0	0.6250	2	0.6250	0
175.00-168.75		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T30	Flange	0.7500	6	0.5000	2	0.5000	2	0.0000	0	0.6250	0	0.6250	2	0.6250	0
168.75-162.50		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T31	Flange	0.7500	6	0.5000	2	0.5000	2	0.0000	0	0.6250	0	0.6250	2	0.6250	0
162.50-156.25		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T32	Flange	0.7500	6	0.5000	2	0.5000	2	0.0000	0	0.6250	0	0.6250	2	0.6250	0
156.25-150.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T33	Flange	0.7500	6	0.5000	2	0.5000	2	0.6250	0	0.6250	0	0.6250	2	0.6250	0
150.00-125.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T34	Flange	0.7500	6	0.5000	2	0.5000	2	0.6250	0	0.6250	0	0.6250	2	0.6250	0
125.00-100.00		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T35	Flange	0.7500	6	0.6250	2	0.6250	2	0.0000	0	0.6250	0	0.6250	2	0.6250	0
100.00-93.75		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T36	Flange	0.7500	0	0.5000	2	0.6250	2	0.0000	0	0.6250	0	0.6250	2	0.6250	0
93.75-87.50		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T37-T38	Flange	0.7500	0	0.5000	2	0.5000	2	0.0000	0	0.6250	0	0.6250	2	0.6250	0
87.50-75.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T39	Flange	0.7500	6	0.5000	2	0.5000	2	0.6250	0	0.6250	0	0.6250	2	0.6250	0
75.00-50.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T40	Flange	0.7500	6	0.5000	2	0.5000	2	0.6250	0	0.6250	0	0.6250	2	0.6250	0
50.00-25.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T41 25.00-0.00	Flange	0.7500	6	0.5000	2	0.5000	2	0.6250	0	0.6250	0	0.6250	2	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	

Guy Data

Guy Elevation	Guy Grade	Guy Size	Initial Tension	%	Guy Modulus	Guy Weight	L _u	Anchor Radius	Anchor Azimuth Adj. °	Anchor Elevation	End Fitting Efficiency %
ft			K		ksi	plf	ft	ft		ft	
50	EHS	A 9/16	3.50	10%	21000	0.671	122.65	115.00	0.0000	0.00	100%
		B 9/16	3.50	10%	21000	0.671	122.65	115.00	0.0000	0.00	100%
		C 9/16	3.50	10%	21000	0.671	122.65	115.00	0.0000	0.00	100%
100	EHS	A 9/16	3.50	10%	21000	0.671	185.69	160.00	0.0000	0.00	100%
		B 9/16	3.50	10%	21000	0.671	185.69	160.00	0.0000	0.00	100%
		C 9/16	3.50	10%	21000	0.671	185.69	160.00	0.0000	0.00	100%
162.5	EHS	A 3/4	5.83	10%	19000	1.155	225.83	160.00	0.0000	0.00	100%
		B 3/4	5.83	10%	19000	1.155	225.83	160.00	0.0000	0.00	100%
		C 3/4	5.83	10%	19000	1.155	225.83	160.00	0.0000	0.00	100%
225	EHS	A 3/4	5.83	10%	19000	1.155	301.86	205.00	0.0000	0.00	100%
		B 3/4	5.83	10%	19000	1.155	301.86	205.00	0.0000	0.00	100%
		C 3/4	5.83	10%	19000	1.155	301.86	205.00	0.0000	0.00	100%
300	EHS	A 7/8	7.97	10%	19000	1.581	361.14	205.00	0.0000	0.00	100%
		B 7/8	7.97	10%	19000	1.581	361.14	205.00	0.0000	0.00	100%
		C 7/8	7.97	10%	19000	1.581	361.14	205.00	0.0000	0.00	100%

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350	EHS	A	7/8	7.97	10%	19000	1.581	427.79	250.00	0.0000	0.00	100%
		B	7/8	7.97	10%	19000	1.581	427.79	250.00	0.0000	0.00	100%
		C	7/8	7.97	10%	19000	1.581	427.79	250.00	0.0000	0.00	100%

Guy Data(cont'd)

Guy Elevation ft	Mount Type	Torque-Arm Spread ft	Torque-Arm Leg Angle °	Torque-Arm Style	Torque-Arm Grade	Torque-Arm Type	Torque-Arm Size
50	Corner						
100	Torque Arm	12.00	49.0000	Bat Ear	A36 (36 ksi)	Double Angle	2L3x2 1/2x1/4
162.5	Corner						
225	Torque Arm	12.00	49.0000	Bat Ear	A36 (36 ksi)	Double Angle	2L3x2 1/2x1/4
300	Torque Arm	12.00	49.0000	Bat Ear	A36 (36 ksi)	Double Angle	2L3x2 1/2x1/4
350	Torque Arm	12.00	49.0000	Bat Ear	A36 (36 ksi)	Double Angle	2L3x2 1/2x1/4

Guy Data (cont'd)

Guy Elevation ft	Diagonal Grade	Diagonal Type	Upper Diagonal Size	Lower Diagonal Size	Is Strap.	Pull-Off Grade	Pull-Off Type	Pull-Off Size
50.00	A572-50 (50 ksi)	Solid Round				A36 (36 ksi)	Flat Bar	
100.00	A572-50 (50 ksi)	Solid Round				A36 (36 ksi)	Flat Bar	
162.50	A572-50 (50 ksi)	Solid Round				A36 (36 ksi)	Solid Round	
225.00	A572-50 (50 ksi)	Solid Round				A36 (36 ksi)	Flat Bar	
300.00	A572-50 (50 ksi)	Solid Round				A36 (36 ksi)	Solid Round	
350.00	A572-50 (50 ksi)	Solid Round				A36 (36 ksi)	Solid Round	

Guy Data (cont'd)

Guy Elevation ft	Cable Weight A K	Cable Weight B K	Cable Weight C K	Cable Weight D K	Tower Intercept A ft	Tower Intercept B ft	Tower Intercept C ft	Tower Intercept D ft
50	0.08	0.08	0.08		1.44	1.44	1.44	
100	0.12	0.12	0.12		2.1 sec/pulse 3.28	2.1 sec/pulse 3.28	2.1 sec/pulse 3.28	
162.5	0.26	0.26	0.26		3.1 sec/pulse 4.98	3.1 sec/pulse 4.98	3.1 sec/pulse 4.98	

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	16 of 98
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	Client	Verizon Wireless	Designed by	Kevin_Barker

Guy Elevation	Cable Weight A	Cable Weight B	Cable Weight C	Cable Weight D	Tower Intercept A	Tower Intercept B	Tower Intercept C	Tower Intercept D
ft	K	K	K	K	ft	ft	ft	ft
225	0.35	0.35	0.35		3.9 sec/pulse 8.84	3.9 sec/pulse 8.84	3.9 sec/pulse 8.84	
300	0.57	0.57	0.57		5.1 sec/pulse 12.58	5.1 sec/pulse 12.58	5.1 sec/pulse 12.58	
350	0.68	0.68	0.68		6.1 sec/pulse 17.56	6.1 sec/pulse 17.56	6.1 sec/pulse 17.56	
					7.2 sec/pulse	7.2 sec/pulse	7.2 sec/pulse	

Guy Data (cont'd)

Guy Elevation ft	Calc K Single Angles	Calc K Solid Rounds	Torque Arm		Pull Off		Diagonal	
			K _x	K _y	K _x	K _y	K _x	K _y
50	No	No			1	1	1	1
100	No	No	1	1	1	1	1	1
162.5	No	No			1	1	1	1
225	No	No	1	1	1	1	1	1
300	No	No	1	1	1	1	1	1
350	No	No	1	1	1	1	1	1

Guy Data (cont'd)

Guy Elevation ft	Torque-Arm				Pull Off				Diagonal			
	Bolt Size in	Number	Net Width Deduct in	U	Bolt Size in	Number	Net Width Deduct in	U	Bolt Size in	Number	Net Width Deduct in	U
50	0.0000	0	0.0000	1	0.6250	0	0.0000	0.75	0.6250	0	0.0000	0.75
	A325N				A325N				A325N			
100	0.0000	0	0.0000	1	0.6250	0	0.0000	0.75	0.6250	0	0.0000	0.75
	A325N				A325N				A325N			
162.5	0.0000	0	0.0000	1	0.6250	0	0.0000	0.75	0.6250	0	0.0000	0.75
	A325N				A325N				A325N			
225	0.0000	0	0.0000	1	0.6250	0	0.0000	0.75	0.6250	0	0.0000	0.75
	A325N				A325N				A325N			
300	0.0000	0	0.0000	1	0.0000	0	0.0000	1	0.0000	0	0.0000	1
	A325N				A325N				A325N			
350	0.0000	0	0.0000	1	0.6250	0	0.0000	0.75	0.6250	0	0.0000	0.75
	A325N				A325N				A325N			

Guy Pressures

Guy Elevation ft	Guy Location	z ft	q _z psf	q _z Ice psf	Ice Thickness in
50	A	25.00	23	17	0.5000
	B	25.00	23	17	0.5000
	C	25.00	23	17	0.5000

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Guy Elevation ft	Guy Location	z ft	q _z psf	q _z Ice psf	Ice Thickness in
100	A	50.00	26	20	0.5000
	B	50.00	26	20	0.5000
	C	50.00	26	20	0.5000
162.5	A	81.25	30	22	0.5000
	B	81.25	30	22	0.5000
	C	81.25	30	22	0.5000
225	A	112.50	33	25	0.5000
	B	112.50	33	25	0.5000
	C	112.50	33	25	0.5000
300	A	150.00	36	27	0.5000
	B	150.00	36	27	0.5000
	C	150.00	36	27	0.5000
350	A	175.00	37	28	0.5000
	B	175.00	37	28	0.5000
	C	175.00	37	28	0.5000

Guy-Mast Forces (Excluding Wind) - No Ice

Guy Elevation ft	Guy Location	Chord Angle °	Guy Tension Top Bottom K	F _x K	F _y K	F _z K	M _x kip-ft	M _y kip-ft	M _z kip-ft
50	A	24.0358	3.53	0.00	1.47	-3.21	-4.25	0.00	0.00
			3.50						
	B	24.0358	3.53	2.78	1.47	1.61	2.13	0.00	-3.68
			3.50						
	C	24.0358	3.53	-2.78	1.47	1.61	2.13	-0.00	3.68
			3.50						
			Sum:	0.00	4.42	0.00	0.00	0.00	0.00
	A	32.5526	3.57	-0.11	1.96	-2.98	-6.80	18.25	-11.78
			3.50						
	A	32.5526	3.57	0.11	1.96	-2.98	-6.80	-18.25	11.78
			3.50						
	B	32.5526	3.57	2.63	1.96	1.39	13.60	18.25	0.00
			3.50						
	B	32.5526	3.57	2.52	1.96	1.59	-6.80	-18.25	-11.78
			3.50						
	C	32.5526	3.57	-2.52	1.96	1.59	-6.80	18.25	11.78
			3.50						
	C	32.5526	3.57	-2.63	1.96	1.39	13.60	-18.25	0.00
			3.50						
			Sum:	0.00	11.78	0.00	-0.00	0.00	0.00
162.5	A	45.9656	6.02	0.00	4.39	-4.12	-12.67	0.00	0.00
			5.83						
	B	45.9656	6.02	3.57	4.39	2.06	6.33	0.00	-10.97
			5.83						
	C	45.9656	6.02	-3.57	4.39	2.06	6.33	-0.00	10.97
			5.83						
			Sum:	0.00	13.17	0.00	0.00	0.00	0.00
	A	48.1361	6.09	-0.12	4.61	-3.97	-15.98	24.26	-27.67
			5.83						
	A	48.1361	6.09	0.12	4.61	-3.97	-15.98	-24.26	27.67
			5.83						
	B	48.1361	6.09	3.50	4.61	1.88	31.96	24.26	0.00
			5.83						
	B	48.1361	6.09	3.38	4.61	2.09	-15.98	-24.26	-27.67
			5.83						

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	18 of 98
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Guy Elevation	Guy Location	Chord Angle	Guy Tension Top Bottom K	F _x	F _y	F _z	M _x	M _y	M _z
ft		°		K	K	K	kip-ft	kip-ft	kip-ft
300	C	48.1361	6.09	-3.38	4.61	2.09	-15.98	24.26	27.67
			5.83						
	C	48.1361	6.09	-3.50	4.61	1.88	31.96	-24.26	0.00
			5.83						
	A	56.0956	Sum:	0.00	27.67	0.00	-0.00	0.00	0.00
			8.44	-0.14	7.10	-4.57	-24.58	27.91	-42.58
			7.97						
	A	56.0956	8.44	0.14	7.10	-4.57	-24.58	-27.91	42.58
			7.97						
	B	56.0956	8.44	4.03	7.10	2.17	49.17	27.91	0.00
			7.97						
	B	56.0956	8.44	3.89	7.10	2.40	-24.58	-27.91	-42.58
350			7.97						
	C	56.0956	8.44	-3.89	7.10	2.40	-24.58	27.91	42.58
			7.97						
	C	56.0956	8.44	-4.03	7.10	2.17	49.17	-27.91	0.00
			7.97						
			Sum:	0.00	42.58	0.00	-0.00	0.00	0.00
	A	54.8316	8.52	-0.12	7.08	-4.75	-24.52	28.87	-42.47
			7.97						
	A	54.8316	8.52	0.12	7.08	-4.75	-24.52	-28.87	42.47
			7.97						
	B	54.8316	8.52	4.17	7.08	2.27	49.04	28.87	0.00
			7.97						
	B	54.8316	8.52	4.05	7.08	2.47	-24.52	-28.87	-42.47
			7.97						
	C	54.8316	8.52	-4.05	7.08	2.47	-24.52	28.87	42.47
			7.97						
	C	54.8316	8.52	-4.17	7.08	2.27	49.04	-28.87	0.00
			7.97						
			Sum:	0.00	42.47	0.00	-0.00	0.00	0.00

Guy-Mast Forces (Excluding Wind) - Ice

Guy Elevation	Guy Location	Chord Angle	Guy Tension Top Bottom K	F _x	F _y	F _z	M _x	M _y	M _z
ft		°		K	K	K	kip-ft	kip-ft	kip-ft
50	A	24.0358	4.95	0.00	2.09	-4.49	-6.02	0.00	0.00
			4.89						
	B	24.0358	4.95	3.89	2.09	2.25	3.01	0.00	-5.21
100			4.89						
	C	24.0358	4.95	-3.89	2.09	2.25	3.01	-0.00	5.21
			4.89						
			Sum:	0.00	6.26	0.00	0.00	0.00	0.00
	A	32.5526	5.07	-0.16	2.81	-4.21	-9.75	25.83	-16.88
			4.94						
	A	32.5526	5.07	0.16	2.81	-4.21	-9.75	-25.83	16.88
			4.94						
	B	32.5526	5.07	3.73	2.81	1.97	19.49	25.83	0.00
			4.94						
	B	32.5526	5.07	3.57	2.81	2.25	-9.75	-25.83	-16.88
			4.94						

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Guy Elevation	Guy Location	Chord Angle	Guy Tension Top Bottom K	F _x	F _y	F _z	M _x	M _y	M _z
ft		°		K	K	K	kip-ft	kip-ft	kip-ft
162.5	C	32.5526	5.07 4.94	-3.57	2.81	2.25	-9.75	25.83	16.88
	C	32.5526	5.07 4.94	-3.73	2.81	1.97	19.49	-25.83	0.00
			Sum:	0.00	16.88	0.00	-0.00	0.00	0.00
	A	45.9656	8.27 7.96	0.00	6.05	-5.64	-17.47	0.00	0.00
	B	45.9656	8.27 7.96	4.88	6.05	2.82	8.74	0.00	-15.13
225	C	45.9656	8.27 7.96	-4.88	6.05	2.82	8.74	-0.00	15.13
			Sum:	0.00	18.16	0.00	0.00	0.00	0.00
	A	48.1361	8.44 8.01	-0.16	6.41	-5.48	-22.21	33.45	-38.47
	A	48.1361	8.44 8.01	0.16	6.41	-5.48	-22.21	-33.45	38.47
	B	48.1361	8.44 8.01	4.83	6.41	2.60	44.42	33.45	0.00
300	B	48.1361	8.44 8.01	4.67	6.41	2.88	-22.21	-33.45	-38.47
	C	48.1361	8.44 8.01	-4.67	6.41	2.88	-22.21	33.45	38.47
	C	48.1361	8.44 8.01	-4.83	6.41	2.60	44.42	-33.45	0.00
			Sum:	0.00	38.47	0.00	-0.00	0.00	0.00
	A	56.0956	11.55 10.83	-0.19	9.72	-6.24	-33.68	38.05	-58.34
350	A	56.0956	11.55 10.83	0.19	9.72	-6.24	-33.68	-38.05	58.34
	B	56.0956	11.55 10.83	5.49	9.72	2.96	67.36	38.05	0.00
	B	56.0956	11.55 10.83	5.31	9.72	3.28	-33.68	-38.05	-58.34
	C	56.0956	11.55 10.83	-5.31	9.72	3.28	-33.68	38.05	58.34
	C	56.0956	11.55 10.83	-5.49	9.72	2.96	67.36	-38.05	0.00
350			Sum:	0.00	58.34	0.00	-0.00	0.00	0.00
	A	54.8316	11.71 10.87	-0.16	9.75	-6.50	-33.76	39.52	-58.47
	A	54.8316	11.71 10.87	0.16	9.75	-6.50	-33.76	-39.52	58.47
	B	54.8316	11.71 10.87	5.70	9.75	3.11	67.52	39.52	0.00
	B	54.8316	11.71 10.87	5.55	9.75	3.38	-33.76	-39.52	-58.47
350	C	54.8316	11.71 10.87	-5.55	9.75	3.38	-33.76	39.52	58.47
	C	54.8316	11.71 10.87	-5.70	9.75	3.11	67.52	-39.52	0.00
			Sum:	0.00	58.47	0.00	-0.00	0.00	0.00

Guy-Mast Forces (Excluding Wind) - Service

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Guy Elevation	Guy Location	Chord Angle	Guy Tension Top Bottom K	F _x	F _y	F _z	M _x	M _y	M _z
ft		°		K	K	K	kip-ft	kip-ft	kip-ft
50	A	24.0358	3.53	0.00	1.47	-3.21	-4.25	0.00	0.00
			3.50						
	B	24.0358	3.53	2.78	1.47	1.61	2.13	0.00	-3.68
			3.50						
	C	24.0358	3.53	-2.78	1.47	1.61	2.13	-0.00	3.68
			3.50						
			Sum:	0.00	4.42	0.00	0.00	0.00	0.00
	A	32.5526	3.57	-0.11	1.96	-2.98	-6.80	18.25	-11.78
			3.50						
100	A	32.5526	3.57	0.11	1.96	-2.98	-6.80	-18.25	11.78
			3.50						
	B	32.5526	3.57	2.63	1.96	1.39	13.60	18.25	0.00
			3.50						
	B	32.5526	3.57	2.52	1.96	1.59	-6.80	-18.25	-11.78
			3.50						
	C	32.5526	3.57	-2.52	1.96	1.59	-6.80	18.25	11.78
			3.50						
	C	32.5526	3.57	-2.63	1.96	1.39	13.60	-18.25	0.00
			3.50						
			Sum:	0.00	11.78	0.00	-0.00	0.00	0.00
162.5	A	45.9656	6.02	0.00	4.39	-4.12	-12.67	0.00	0.00
			5.83						
	B	45.9656	6.02	3.57	4.39	2.06	6.33	0.00	-10.97
			5.83						
	C	45.9656	6.02	-3.57	4.39	2.06	6.33	-0.00	10.97
			5.83						
			Sum:	0.00	13.17	0.00	0.00	0.00	0.00
	A	48.1361	6.09	-0.12	4.61	-3.97	-15.98	24.26	-27.67
			5.83						
225	A	48.1361	6.09	0.12	4.61	-3.97	-15.98	-24.26	27.67
			5.83						
	B	48.1361	6.09	3.50	4.61	1.88	31.96	24.26	0.00
			5.83						
	B	48.1361	6.09	3.38	4.61	2.09	-15.98	-24.26	-27.67
			5.83						
	C	48.1361	6.09	-3.38	4.61	2.09	-15.98	24.26	27.67
			5.83						
	C	48.1361	6.09	-3.50	4.61	1.88	31.96	-24.26	0.00
			5.83						
			Sum:	0.00	27.67	0.00	-0.00	0.00	0.00
300	A	56.0956	8.44	-0.14	7.10	-4.57	-24.58	27.91	-42.58
			7.97						
	A	56.0956	8.44	0.14	7.10	-4.57	-24.58	-27.91	42.58
			7.97						
	B	56.0956	8.44	4.03	7.10	2.17	49.17	27.91	0.00
			7.97						
	B	56.0956	8.44	3.89	7.10	2.40	-24.58	-27.91	-42.58
			7.97						
	C	56.0956	8.44	-3.89	7.10	2.40	-24.58	27.91	42.58
			7.97						
	C	56.0956	8.44	-4.03	7.10	2.17	49.17	-27.91	0.00
			7.97						
			Sum:	0.00	42.58	0.00	-0.00	0.00	0.00
	A	54.8316	8.52	-0.12	7.08	-4.75	-24.52	28.87	-42.47
			7.97						
350	A	54.8316	8.52	0.12	7.08	-4.75	-24.52	-28.87	42.47
			7.97						
	B	54.8316	8.52	4.17	7.08	2.27	49.04	28.87	0.00
			7.97						

tnxTower

URS Corporation
 500 Enterprise Drive, Suite 3B
 Rocky Hill, CT 06067
 Phone: (860) 529-8882
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Job

370' Guyed Lattice Tower

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Project

Montville, CT

Date

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Client

Verizon Wireless

Designed by

Kevin_Barker

Guy Elevation	Guy Location	Chord Angle	Guy Tension Top Bottom K	F _x	F _y	F _z	M _x	M _y	M _z
ft		°		K	K	K	kip-ft	kip-ft	kip-ft
	B	54.8316	8.52 7.97	4.05	7.08	2.47	-24.52	-28.87	-42.47
	C	54.8316	8.52 7.97	-4.05	7.08	2.47	-24.52	28.87	42.47
	C	54.8316	8.52 7.97	-4.17	7.08	2.27	49.04	-28.87	0.00
			Sum:	0.00	42.47	0.00	-0.00	0.00	0.00

Guy-Tensioning Information

Temperature At Time Of Tensioning																	
Guy Elevation	H	V	0 F		20 F		40 F		60 F		80 F		100 F		120 F		
			Initial Tension	Intercept	Initial Tension	Intercept	Initial Tension	Intercept	Initial Tension	Intercept	Initial Tension	Intercept	Initial Tension	Intercept	Initial Tension	Intercept	
			ft	ft	K	ft	K	ft	K	ft	K	ft	K	ft	K	ft	K
50	A	112.11	50.00	4.793	1.05	4.359	1.15	3.927	1.28	3.500	1.44	3.079	1.63	2.669	1.88	2.275	2.21
	B	112.11	50.00	4.793	1.05	4.359	1.15	3.927	1.28	3.500	1.44	3.079	1.63	2.669	1.88	2.275	2.21
	C	112.11	50.00	4.793	1.05	4.359	1.15	3.927	1.28	3.500	1.44	3.079	1.63	2.669	1.88	2.275	2.21
100	A	156.65	100.00	4.570	2.51	4.209	2.73	3.851	2.98	3.500	3.28	3.157	3.63	2.825	4.05	2.509	4.56
	B	156.65	100.00	4.570	2.51	4.209	2.73	3.851	2.98	3.500	3.28	3.157	3.63	2.825	4.05	2.509	4.56
	C	156.65	100.00	4.570	2.51	4.209	2.73	3.851	2.98	3.500	3.28	3.157	3.63	2.825	4.05	2.509	4.56
162.5	A	157.11	162.50	6.959	4.18	6.580	4.42	6.203	4.68	5.830	4.98	5.463	5.31	5.103	5.68	4.751	6.09
	B	157.11	162.50	6.959	4.18	6.580	4.42	6.203	4.68	5.830	4.98	5.463	5.31	5.103	5.68	4.751	6.09
	C	157.11	162.50	6.959	4.18	6.580	4.42	6.203	4.68	5.830	4.98	5.463	5.31	5.103	5.68	4.751	6.09
225	A	201.63	225.00	6.832	7.56	6.493	7.95	6.159	8.38	5.830	8.84	5.508	9.35	5.193	9.90	4.887	10.50
	B	201.63	225.00	6.832	7.56	6.493	7.95	6.159	8.38	5.830	8.84	5.508	9.35	5.193	9.90	4.887	10.50
	C	201.63	225.00	6.832	7.56	6.493	7.95	6.159	8.38	5.830	8.84	5.508	9.35	5.193	9.90	4.887	10.50
300	A	201.63	300.00	8.922	11.26	8.601	11.67	8.284	12.11	7.970	12.58	7.661	13.07	7.356	13.60	7.057	14.16
	B	201.63	300.00	8.922	11.26	8.601	11.67	8.284	12.11	7.970	12.58	7.661	13.07	7.356	13.60	7.057	14.16
	C	201.63	300.00	8.922	11.26	8.601	11.67	8.284	12.11	7.970	12.58	7.661	13.07	7.356	13.60	7.057	14.16
350	A	246.61	350.00	8.938	15.71	8.610	16.29	8.287	16.91	7.970	17.56	7.659	18.25	7.354	18.99	7.056	19.76
	B	246.61	350.00	8.938	15.71	8.610	16.29	8.287	16.91	7.970	17.56	7.659	18.25	7.354	18.99	7.056	19.76
	C	246.61	350.00	8.938	15.71	8.610	16.29	8.287	16.91	7.970	17.56	7.659	18.25	7.354	18.99	7.056	19.76

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
1 5/8 (Verizon)	C	Yes	Ar (CfAe)	305.00 - 3.00	0.0000	-0.3	12	6	0.7500	1.9800		1.04
1 5/8 (Cingular)	C	No	Ar (Leg)	243.00 - 3.00	0.0000	-0.15	12	6	0.7500	1.9800		1.04
7/8	A	No	Ar (Leg)	40.00 - 3.00	0.0000	0.4	1	1	1.1100	1.1100		0.54
7/8	A	No	Ar (Leg)	62.00 - 3.00	0.0000	0.4	1	1	1.1100	1.1100		0.54
1/2	B	No	Ar (Leg)	88.00 - 3.00	0.0000	0	4	4	0.5800	0.5800		0.25
1/2	B	No	Ar (Leg)	125.00 - 3.00	0.0000	0	1	1	0.5800	0.5800		0.25
7/8	C	No	Ar (Leg)	140.00 - 3.00	0.0000	0	1	1	1.1100	1.1100		0.54
1/2	A	No	Ar (Leg)	148.00 - 3.00	0.0000	0	1	1	0.5800	0.5800		0.25
7/8	A	No	Ar (Leg)	180.00 - 3.00	0.0000	0	1	1	1.1100	1.1100		0.54
7/8	C	No	Ar (Leg)	200.00 - 3.00	0.0000	0	1	1	1.1100	1.1100		0.54
1 5/8	A	No	Ar (Leg)	325.00 - 3.00	0.0000	0	1	1	1.9800	1.9800		1.04
7/8	B	No	Ar (Leg)	350.00 - 3.00	0.0000	0	2	2	1.1100	1.1100		0.54
7/8	B	No	Ar (Leg)	355.00 - 3.00	0.0000	0	1	1	1.1100	1.1100		0.54

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	22 of 98
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	Client	Verizon Wireless	Designed by	Kevin_Barker

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
7/8	C	No	Ar (Leg)	365.00 - 3.00	0.0000	0	1	1	1.1100	1.1100		0.54

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
T1	368.75-362.50	A	0.231	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.231	0.000	0.000	0.000	0.00
T2	362.50-356.25	A	0.578	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.578	0.000	0.000	0.000	0.00
T3	356.25-350.00	A	0.578	0.000	0.000	0.000	0.00
		B	0.463	0.000	0.000	0.000	0.00
		C	1.041	0.000	0.000	0.000	0.00
T4	350.00-343.75	A	0.578	0.000	0.000	0.000	0.00
		B	1.734	0.000	0.000	0.000	0.01
		C	2.313	0.000	0.000	0.000	0.00
T5	343.75-337.50	A	0.578	0.000	0.000	0.000	0.00
		B	1.734	0.000	0.000	0.000	0.01
		C	2.313	0.000	0.000	0.000	0.00
T6	337.50-331.25	A	0.578	0.000	0.000	0.000	0.00
		B	1.734	0.000	0.000	0.000	0.01
		C	2.313	0.000	0.000	0.000	0.00
T7	331.25-325.00	A	0.578	0.000	0.000	0.000	0.00
		B	1.734	0.000	0.000	0.000	0.01
		C	2.313	0.000	0.000	0.000	0.00
T8	325.00-318.75	A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
		C	2.313	0.000	0.000	0.000	0.00
T9	318.75-312.50	A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
		C	2.313	0.000	0.000	0.000	0.00
T10	312.50-306.25	A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
		C	2.313	0.000	0.000	0.000	0.00
T11	306.25-300.00	A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
		C	7.262	0.000	0.000	0.000	0.07
T12	300.00-293.75	A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
		C	8.500	0.000	0.000	0.000	0.08
T13	293.75-287.50	A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
		C	8.500	0.000	0.000	0.000	0.08
T14	287.50-281.25	A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
		C	8.500	0.000	0.000	0.000	0.08
T15	281.25-275.00	A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
		C	8.500	0.000	0.000	0.000	0.08
T16	275.00-268.75	A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
		C	8.500	0.000	0.000	0.000	0.08
T17	268.75-262.50	A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	23 of 98
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	Client	Verizon Wireless	Designed by	Kevin_Barker

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
T18	262.50-256.25	C	8.500	0.000	0.000	0.000	0.08
		A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
T19	256.25-250.00	C	8.500	0.000	0.000	0.000	0.08
		A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
T20	250.00-243.75	C	8.500	0.000	0.000	0.000	0.08
		A	1.609	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
T21	243.75-237.50	C	8.500	0.000	0.000	0.000	0.08
		A	7.054	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
T22	237.50-231.25	C	13.945	0.000	0.000	0.000	0.15
		A	7.797	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
T23	231.25-225.00	C	14.688	0.000	0.000	0.000	0.16
		A	7.797	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
T24	225.00-218.75	C	14.688	0.000	0.000	0.000	0.16
		A	7.797	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
T25	218.75-212.50	C	14.688	0.000	0.000	0.000	0.16
		A	7.797	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
T26	212.50-206.25	C	14.688	0.000	0.000	0.000	0.16
		A	7.797	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
T27	206.25-200.00	C	14.688	0.000	0.000	0.000	0.16
		A	7.797	0.000	0.000	0.000	0.01
		B	2.766	0.000	0.000	0.000	0.01
T28	200.00-175.00	C	14.688	0.000	0.000	0.000	0.16
		A	33.962	0.000	0.000	0.000	0.03
		B	11.525	0.000	0.000	0.000	0.04
T29	175.00-168.75	C	61.063	0.000	0.000	0.000	0.65
		A	8.953	0.000	0.000	0.000	0.01
		B	3.344	0.000	0.000	0.000	0.01
T30	168.75-162.50	C	15.266	0.000	0.000	0.000	0.16
		A	8.953	0.000	0.000	0.000	0.01
		B	3.344	0.000	0.000	0.000	0.01
T31	162.50-156.25	C	15.266	0.000	0.000	0.000	0.16
		A	8.953	0.000	0.000	0.000	0.01
		B	3.344	0.000	0.000	0.000	0.01
T32	156.25-150.00	C	15.266	0.000	0.000	0.000	0.16
		A	8.953	0.000	0.000	0.000	0.01
		B	3.344	0.000	0.000	0.000	0.01
T33	150.00-125.00	C	15.266	0.000	0.000	0.000	0.16
		A	38.312	0.000	0.000	0.000	0.05
		B	14.487	0.000	0.000	0.000	0.04
T34	125.00-100.00	C	62.450	0.000	0.000	0.000	0.66
		A	39.333	0.000	0.000	0.000	0.05
		B	15.792	0.000	0.000	0.000	0.05
T35	100.00-93.75	C	64.583	0.000	0.000	0.000	0.66
		A	9.833	0.000	0.000	0.000	0.01
		B	3.948	0.000	0.000	0.000	0.01
T36	93.75-87.50	C	16.146	0.000	0.000	0.000	0.17
		A	9.833	0.000	0.000	0.000	0.01
		B	4.045	0.000	0.000	0.000	0.01
T37	87.50-81.25	C	16.243	0.000	0.000	0.000	0.17
		A	9.833	0.000	0.000	0.000	0.01
		B	5.156	0.000	0.000	0.000	0.02
		C	17.354	0.000	0.000	0.000	0.17

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	Client	Verizon Wireless	Designed by	Kevin_Barker

Tower Section	Tower Elevation ft	Face	A_R ft ²	A_F ft ²	$C_A A_A$ In Face ft ²	$C_A A_A$ Out Face ft ²	Weight K
T38	81.25-75.00	A	9.833	0.000	0.000	0.000	0.01
		B	5.156	0.000	0.000	0.000	0.02
		C	17.354	0.000	0.000	0.000	0.17
T39	75.00-50.00	A	40.443	0.000	0.000	0.000	0.05
		B	21.735	0.000	0.000	0.000	0.07
		C	69.417	0.000	0.000	0.000	0.66
T40	50.00-25.00	A	43.033	0.000	0.000	0.000	0.07
		B	24.325	0.000	0.000	0.000	0.07
		C	69.417	0.000	0.000	0.000	0.66
T41	25.00-0.00	A	38.683	0.000	0.000	0.000	0.06
		B	22.220	0.000	0.000	0.000	0.06
		C	61.087	0.000	0.000	0.000	0.58

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A_R ft ²	A_F ft ²	$C_A A_A$ In Face ft ²	$C_A A_A$ Out Face ft ²	Weight K
T1	368.75-362.50	A	0.500	0.440	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.440	0.000	0.000	0.000	0.00
T2	362.50-356.25	A	0.500	1.099	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		1.099	0.000	0.000	0.000	0.01
T3	356.25-350.00	A	0.500	1.099	0.000	0.000	0.000	0.00
		B		0.879	0.000	0.000	0.000	0.01
		C		1.978	0.000	0.000	0.000	0.01
T4	350.00-343.75	A	0.500	1.099	0.000	0.000	0.000	0.00
		B		3.297	0.000	0.000	0.000	0.03
		C		4.396	0.000	0.000	0.000	0.01
T5	343.75-337.50	A	0.500	1.099	0.000	0.000	0.000	0.00
		B		3.297	0.000	0.000	0.000	0.03
		C		4.396	0.000	0.000	0.000	0.01
T6	337.50-331.25	A	0.500	1.099	0.000	0.000	0.000	0.00
		B		3.297	0.000	0.000	0.000	0.03
		C		4.396	0.000	0.000	0.000	0.01
T7	331.25-325.00	A	0.500	1.099	0.000	0.000	0.000	0.00
		B		3.297	0.000	0.000	0.000	0.03
		C		4.396	0.000	0.000	0.000	0.01
T8	325.00-318.75	A	0.500	2.651	0.000	0.000	0.000	0.02
		B		4.849	0.000	0.000	0.000	0.03
		C		4.396	0.000	0.000	0.000	0.01
T9	318.75-312.50	A	0.500	2.651	0.000	0.000	0.000	0.02
		B		4.849	0.000	0.000	0.000	0.03
		C		4.396	0.000	0.000	0.000	0.01
T10	312.50-306.25	A	0.500	2.651	0.000	0.000	0.000	0.02
		B		4.849	0.000	0.000	0.000	0.03
		C		4.396	0.000	0.000	0.000	0.01
T11	306.25-300.00	A	0.500	2.651	0.000	0.000	0.000	0.02
		B		4.849	0.000	0.000	0.000	0.03
		C		5.638	5.688	0.000	0.000	0.18
T12	300.00-293.75	A	0.500	2.651	0.000	0.000	0.000	0.02
		B		4.849	0.000	0.000	0.000	0.03
		C		5.948	7.109	0.000	0.000	0.22
T13	293.75-287.50	A	0.500	2.651	0.000	0.000	0.000	0.02
		B		4.849	0.000	0.000	0.000	0.03
		C		5.948	7.109	0.000	0.000	0.22
T14	287.50-281.25	A	0.500	2.651	0.000	0.000	0.000	0.02

inxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	25 of 98
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	Client	Verizon Wireless	Designed by	Kevin_Barker

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
T15	281.25-275.00	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		5.948	7.109	0.000	0.000	0.22
		A		2.651	0.000	0.000	0.000	0.02
T16	275.00-268.75	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		5.948	7.109	0.000	0.000	0.22
		A		2.651	0.000	0.000	0.000	0.02
T17	268.75-262.50	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		5.948	7.109	0.000	0.000	0.22
		A		2.651	0.000	0.000	0.000	0.02
T18	262.50-256.25	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		5.948	7.109	0.000	0.000	0.22
		A		2.651	0.000	0.000	0.000	0.02
T19	256.25-250.00	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		5.948	7.109	0.000	0.000	0.22
		A		2.651	0.000	0.000	0.000	0.02
T20	250.00-243.75	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		5.948	7.109	0.000	0.000	0.22
		A		2.651	0.000	0.000	0.000	0.02
T21	243.75-237.50	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		7.314	13.366	0.000	0.000	0.41
		A		4.017	6.256	0.000	0.000	0.02
T22	237.50-231.25	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		7.500	14.219	0.000	0.000	0.44
		A		4.203	7.109	0.000	0.000	0.02
T23	231.25-225.00	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		7.500	14.219	0.000	0.000	0.44
		A		4.203	7.109	0.000	0.000	0.02
T24	225.00-218.75	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		7.500	14.219	0.000	0.000	0.44
		A		4.203	7.109	0.000	0.000	0.02
T25	218.75-212.50	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		7.500	14.219	0.000	0.000	0.44
		A		4.203	7.109	0.000	0.000	0.02
T26	212.50-206.25	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		7.500	14.219	0.000	0.000	0.44
		A		4.203	7.109	0.000	0.000	0.02
T27	206.25-200.00	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		7.500	14.219	0.000	0.000	0.44
		A		4.203	7.109	0.000	0.000	0.02
T28	200.00-175.00	B	0.500	4.849	0.000	0.000	0.000	0.03
		C		34.396	56.875	0.000	0.000	1.79
		A		20.275	0.000	0.000	0.000	0.11
T29	175.00-168.75	B	0.500	5.948	0.000	0.000	0.000	0.03
		C		8.599	14.219	0.000	0.000	0.45
		A		6.401	7.109	0.000	0.000	0.03
T30	168.75-162.50	B	0.500	5.948	0.000	0.000	0.000	0.03
		C		8.599	14.219	0.000	0.000	0.45
		A		6.401	7.109	0.000	0.000	0.03
T31	162.50-156.25	B	0.500	5.948	0.000	0.000	0.000	0.03
		C		8.599	14.219	0.000	0.000	0.45
		A		6.401	7.109	0.000	0.000	0.03
T32	156.25-150.00	B	0.500	5.948	0.000	0.000	0.000	0.03
		C		8.599	14.219	0.000	0.000	0.45
		A		6.401	7.109	0.000	0.000	0.03
T33	150.00-125.00	B	0.500	5.948	0.000	0.000	0.000	0.03
		C		37.033	56.875	0.000	0.000	1.81
		A		26.820	0.000	0.000	0.000	0.11
T34	125.00-100.00	B	0.500	33.292	28.438	0.000	0.000	0.12
		A		30.375	0.000	0.000	0.000	0.14

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	26 of 98
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	Client	Verizon Wireless	Designed by	Kevin_Barker

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A_R ft ²	A_F ft ²	$C_A A_A$ In Face ft ²	$C_A A_A$ Out Face ft ²	Weight K
T35	100.00-93.75	C	0.500	42.083	56.875	0.000	0.000	1.83
		A		8.323	7.109	0.000	0.000	0.03
		B		7.594	0.000	0.000	0.000	0.03
T36	93.75-87.50	C	0.500	10.521	14.219	0.000	0.000	0.46
		A		8.323	7.109	0.000	0.000	0.03
		B		7.660	0.145	0.000	0.000	0.04
T37	87.50-81.25	C	0.500	10.587	14.364	0.000	0.000	0.46
		A		8.323	7.109	0.000	0.000	0.03
		B		8.417	1.813	0.000	0.000	0.06
T38	81.25-75.00	C	0.500	11.344	16.031	0.000	0.000	0.46
		A		8.323	7.109	0.000	0.000	0.03
		B		8.417	1.813	0.000	0.000	0.06
T39	75.00-50.00	C	0.500	11.344	16.031	0.000	0.000	0.46
		A		35.402	28.438	0.000	0.000	0.14
		B		35.777	7.250	0.000	0.000	0.22
T40	50.00-25.00	C	0.500	45.375	64.125	0.000	0.000	1.83
		A		40.325	28.438	0.000	0.000	0.19
		B		40.700	7.250	0.000	0.000	0.22
T41	25.00-0.00	C	0.500	45.375	64.125	0.000	0.000	1.83
		A		37.033	25.025	0.000	0.000	0.18
		B		37.363	6.380	0.000	0.000	0.20
		C		39.930	56.430	0.000	0.000	1.61

Feed Line Shielding

Section	Elevation ft	Face	A_R ft ²	A_R Ice ft ²	A_F ft ²	A_F Ice ft ²
T1	368.75-362.50	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T2	362.50-356.25	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T3	356.25-350.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T4	350.00-343.75	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T5	343.75-337.50	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T6	337.50-331.25	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T7	331.25-325.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T8	325.00-318.75	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T9	318.75-312.50	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T10	312.50-306.25	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	27 of 98
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	Client	Verizon Wireless	Designed by	Kevin_Barker

Section	Elevation	Face	A_R	$A_{R_{Ice}}$	A_F	$A_{F_{Ice}}$
	ft		ft ²	ft ²	ft ²	ft ²
T11	306.25-300.00	C	0.000	0.000	0.000	0.000
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T12	300.00-293.75	C	0.158	0.610	0.165	0.231
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T13	293.75-287.50	C	0.000	0.485	0.999	1.398
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T14	287.50-281.25	C	0.198	0.763	0.206	0.289
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T15	281.25-275.00	C	0.302	0.908	0.000	0.000
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T16	275.00-268.75	C	0.302	0.908	0.000	0.000
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T17	268.75-262.50	C	0.302	0.908	0.000	0.000
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T18	262.50-256.25	C	0.302	0.908	0.000	0.000
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T19	256.25-250.00	C	0.302	0.908	0.000	0.000
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T20	250.00-243.75	C	0.335	0.954	0.000	0.000
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T21	243.75-237.50	C	0.198	0.763	0.206	0.289
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T22	237.50-231.25	C	0.198	0.763	0.206	0.289
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T23	231.25-225.00	C	0.198	0.763	0.206	0.289
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T24	225.00-218.75	C	0.198	0.763	0.206	0.289
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T25	218.75-212.50	C	0.000	0.485	0.867	1.213
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T26	212.50-206.25	C	0.165	0.832	0.371	0.520
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T27	206.25-200.00	C	0.302	1.024	0.165	0.231
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T28	200.00-175.00	C	0.302	1.024	0.165	0.231
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T29	175.00-168.75	C	1.756	4.861	0.000	0.000
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
T30	168.75-162.50	C	0.302	0.908	0.000	0.000
		A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.401	1.047	0.000	0.000

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	28 of 98
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	Client	Verizon Wireless	Designed by	Kevin_Barker

Section	Elevation	Face	A_R	$A_{R_{Ice}}$	A_F	$A_{F_{Ice}}$
	ft		ft ²	ft ²	ft ²	ft ²
T31	162.50-156.25	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.165	0.716	0.206	0.289
T32	156.25-150.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.302	0.908	0.000	0.000
T33	150.00-125.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	1.208	3.632	0.000	0.000
T34	125.00-100.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.548	2.708	2.641	3.697
T35	100.00-93.75	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.485	0.867	1.213
T36	93.75-87.50	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.198	0.763	0.206	0.289
T37	87.50-81.25	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.302	0.908	0.000	0.000
T38	81.25-75.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.302	0.908	0.000	0.000
T39	75.00-50.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	1.208	3.632	0.000	0.000
T40	50.00-25.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	1.071	3.440	0.206	0.289
T41	25.00-0.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	1.063	3.196	0.000	0.000

Feed Line Center of Pressure

Section	Elevation	CP_X	CP_Z	$CP_{X_{Ice}}$	$CP_{Z_{Ice}}$
	ft	in	in	in	in
T1	368.75-362.50	-0.3214	0.1856	-0.4376	0.2526
T2	362.50-356.25	-0.7171	0.4140	-0.9881	0.5705
T3	356.25-350.00	-0.1381	0.7177	-0.1877	0.9755
T4	350.00-343.75	1.2209	1.4097	1.6198	1.8704
T5	343.75-337.50	1.7931	2.0705	2.0849	2.4075
T6	337.50-331.25	1.8906	2.1831	2.1528	2.4859
T7	331.25-325.00	1.8906	2.1831	2.1528	2.4859
T8	325.00-318.75	1.6014	0.1999	1.8853	0.6397
T9	318.75-312.50	1.6025	0.2000	1.8868	0.6402
T10	312.50-306.25	1.6025	0.2000	1.8868	0.6402
T11	306.25-300.00	4.3266	3.4975	3.6446	2.8013
T12	300.00-293.75	3.5243	2.9282	3.1231	2.4437
T13	293.75-287.50	4.8831	4.1559	4.0181	3.2508
T14	287.50-281.25	5.1973	4.4398	4.2050	3.4194
T15	281.25-275.00	5.1973	4.4398	4.2050	3.4194
T16	275.00-268.75	5.1973	4.4398	4.2050	3.4194
T17	268.75-262.50	5.1973	4.4398	4.2050	3.4194
T18	262.50-256.25	5.1973	4.4398	4.2050	3.4194

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	29 of 98
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Section	Elevation	CP _X	CP _Z	CP _X	CP _Z
	ft	in	in	Ice in	Ice in
T19	256.25-250.00	5.0925	4.3451	4.1434	3.3638
T20	250.00-243.75	4.8831	4.1559	4.0181	3.2508
T21	243.75-237.50	-1.7045	6.0270	-1.0455	4.7298
T22	237.50-231.25	-2.3303	6.2047	-1.5394	4.8741
T23	231.25-225.00	-2.3303	6.2047	-1.5394	4.8741
T24	225.00-218.75	-2.2379	5.2824	-1.5940	4.2962
T25	218.75-212.50	-2.3307	5.9924	-1.5751	4.6212
T26	212.50-206.25	-2.3531	6.1618	-1.5698	4.7191
T27	206.25-200.00	-2.3531	6.1618	-1.5698	4.7191
T28	200.00-175.00	-2.6982	6.1696	-2.0161	4.7419
T29	175.00-168.75	-2.6478	5.8701	-1.9399	4.3844
T30	168.75-162.50	-2.6089	5.6468	-1.9409	4.2584
T31	162.50-156.25	-2.6206	5.7130	-1.9408	4.2963
T32	156.25-150.00	-2.6487	5.8721	-1.9407	4.3863
T33	150.00-125.00	-2.8137	5.6172	-2.1546	3.9785
T34	125.00-100.00	-2.5194	4.7360	-1.9101	3.5816
T35	100.00-93.75	-2.4985	4.6248	-1.9106	3.5149
T36	93.75-87.50	-2.5794	5.4651	-1.8349	4.0034
T37	87.50-81.25	-1.7754	5.8212	-1.0442	4.2387
T38	81.25-75.00	-1.7754	5.8212	-1.0442	4.2387
T39	75.00-50.00	-1.7544	5.6899	-1.0296	4.1056
T40	50.00-25.00	-1.7058	5.3625	-1.0000	3.7920
T41	25.00-0.00	-1.6256	5.0879	-0.9492	3.5686

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K
54"x12"x3" Yagi	A	From Leg	1.00	0.0000	40.00	No Ice	0.35	0.20
			0.00			1/2" Ice	0.45	0.30
			0.00					
54"x13"x3" Yagi	C	From Leg	1.00	0.0000	62.00	No Ice	0.38	0.21
			0.00			1/2" Ice	0.49	0.31
			0.00					
12' T-Frame Sector Mount (AT&T)	A	From Leg	3.25	0.0000	242.50	No Ice	13.60	0.47
			0.00			1/2" Ice	18.40	0.60
			0.00					
12' T-Frame Sector Mount (AT&T)	B	From Leg	3.25	0.0000	242.50	No Ice	13.60	0.47
			0.00			1/2" Ice	18.40	0.60
			0.00					
12' T-Frame Sector Mount (AT&T)	C	From Leg	3.25	0.0000	242.50	No Ice	13.60	0.47
			0.00			1/2" Ice	18.40	0.60
			0.00					
(2) DUO1417-8686 (AT&T)	A	From Leg	4.50	0.0000	242.50	No Ice	6.53	0.02
			0.00			1/2" Ice	6.94	0.06
			0.00					
(2) DUO1417-8686 (AT&T)	B	From Leg	4.50	0.0000	242.50	No Ice	6.53	0.02
			0.00			1/2" Ice	6.94	0.06
			0.00					
(2) DUO1417-8686 (AT&T)	C	From Leg	4.50	0.0000	242.50	No Ice	6.53	0.02
			0.00			1/2" Ice	6.94	0.06
			0.00					

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	30 of 98
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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
(2) TMA (AT&T)	A	From Face	0.00 4.50 0.00 0.00	0.0000	242.50	No Ice 1/2" Ice	1.00 1.50	1.00 1.50	0.01 0.01
(2) TMA (AT&T)	B	From Face	0.00 4.50 0.00 0.00	0.0000	242.50	No Ice 1/2" Ice	1.00 1.50	1.00 1.50	0.01 0.01
(2) TMA (AT&T)	C	From Face	0.00 4.50 0.00 0.00	0.0000	242.50	No Ice 1/2" Ice	1.00 1.50	1.00 1.50	0.01 0.01
13' T-Frame Sector Mount (Verizon)	A	From Leg	0.00 3.25 0.00 0.00	0.0000	304.25	No Ice 1/2" Ice	13.60 18.40	13.60 18.40	0.47 0.60
13' T-Frame Sector Mount (Verizon)	B	From Leg	0.00 3.25 0.00 0.00	0.0000	304.25	No Ice 1/2" Ice	13.60 18.40	13.60 18.40	0.47 0.60
13' T-Frame Sector Mount (Verizon)	C	From Leg	0.00 3.25 0.00 0.00	0.0000	304.25	No Ice 1/2" Ice	13.60 18.40	13.60 18.40	0.47 0.60
X-Style	A	From Leg	0.00 1.00 0.00 0.00	0.0000	88.00	No Ice 1/2" Ice	1.50 2.00	1.50 2.00	0.02 0.03
(2) X-Style	B	From Leg	0.00 1.00 0.00 0.00	0.0000	88.00	No Ice 1/2" Ice	1.50 2.00	1.50 2.00	0.02 0.03
X-Style	C	From Leg	0.00 1.00 0.00 0.00	0.0000	88.00	No Ice 1/2" Ice	1.50 2.00	1.50 2.00	0.02 0.03
49"x13"x1" Yagi	A	From Leg	0.00 1.00 0.00 0.00	0.0000	125.00	No Ice 1/2" Ice	0.13 0.22	0.48 0.80	0.20 0.25
8"x33"x4" Yagi	C	From Leg	0.00 1.00 0.00 0.00	0.0000	140.00	No Ice 1/2" Ice	1.28 1.53	3.73 4.39	0.30 0.45
54"x13"x3" Yagi	B	From Leg	0.00 1.00 0.00 0.00	0.0000	148.00	No Ice 1/2" Ice	0.38 0.49	1.58 1.95	0.21 0.31
(4) 5"x13"x3" Yagi	C	From Leg	0.00 1.00 0.00 0.00	0.0000	180.00	No Ice 1/2" Ice	0.38 0.49	1.75 2.16	0.25 0.33
(2) 5'3"x4" Pipe Mount	C	From Leg	0.00 1.00 0.00 0.00	0.0000	180.00	No Ice 1/2" Ice	1.88 2.21	1.88 2.21	0.06 0.07
6'8"x4" Pipe Mount	C	From Leg	0.00 0.50 0.00 0.00	0.0000	250.00	No Ice 1/2" Ice	2.60 3.01	2.60 3.01	0.07 0.09
3'-6" Standoff Mount	B	From Face	0.00 2.00 0.00 0.00	0.0000	325.00	No Ice 1/2" Ice	2.60 3.01	2.60 3.01	0.07 0.09
10' x 3" Dia Omni	B	From Face	0.00 3.00 0.00 0.00	0.0000	325.00	No Ice 1/2" Ice	3.00 4.03	3.00 4.03	0.03 0.05
6'x4" Pipe Mount	C	From Leg	0.00 0.50 0.00 0.00	0.0000	350.00	No Ice 1/2" Ice	2.09 2.46	2.09 2.46	0.05 0.07
10'6"x4" Pipe Mount	A	From Face	0.00 0.50 0.00 0.00	0.0000	355.00	No Ice 1/2" Ice	4.72 5.62	4.72 5.62	0.11 0.15

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	Client		Designed by
	Verizon Wireless		Kevin_Barker

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
6'-6" Standoff Mount	A	From Leg	0.00 3.00 0.00 0.00	0.0000	355.00	No Ice 1/2" Ice	2.30 3.13	2.30 3.13	0.04 0.06
3" Dia 20' Omni	C	From Leg	1.00 0.00 0.00	0.0000	350.00	No Ice 1/2" Ice	4.00 6.00	4.00 6.00	0.06 0.10
3" Dia 20' Omni	C	From Leg	1.00 0.00 0.00	0.0000	250.00	No Ice 1/2" Ice	4.00 6.00	4.00 6.00	0.06 0.10
54"x13"x3" Yagi	A	From Leg	1.00 0.00 0.00	0.0000	200.00	No Ice 1/2" Ice	0.38 0.49	1.58 1.95	0.21 0.31
BXA-70063/6CF (Verizon)	A	From Leg	4.50 0.00 0.00	0.0000	305.25	No Ice 1/2" Ice	7.73 8.27	3.76 4.19	0.02 0.06
BXA-70063/6CF (Verizon)	B	From Leg	4.50 0.00 0.00	0.0000	305.25	No Ice 1/2" Ice	7.73 8.27	3.76 4.19	0.02 0.06
BXA-70063/6CF (Verizon)	C	From Leg	4.50 0.00 0.00	0.0000	305.25	No Ice 1/2" Ice	7.73 8.27	3.76 4.19	0.02 0.06
LPA-80080-4CF-EDIN (Verizon)	A	From Leg	4.50 -6.00 0.00	0.0000	305.25	No Ice 1/2" Ice	2.62 2.92	6.06 6.45	0.02 0.05
LPA-80080-4CF-EDIN (Verizon)	A	From Leg	4.50 6.00 0.00	0.0000	305.25	No Ice 1/2" Ice	2.62 2.92	6.06 6.45	0.02 0.05
LPA-80080-4CF-EDIN (Verizon)	B	From Leg	4.50 -6.00 0.00	0.0000	305.25	No Ice 1/2" Ice	2.62 2.92	6.06 6.45	0.02 0.05
LPA-80080-4CF-EDIN (Verizon)	B	From Leg	4.50 6.00 0.00	0.0000	305.25	No Ice 1/2" Ice	2.62 2.92	6.06 6.45	0.02 0.05
LPA-80080-4CF-EDIN (Verizon)	C	From Leg	4.50 -6.00 0.00	0.0000	305.25	No Ice 1/2" Ice	2.62 2.92	6.06 6.45	0.02 0.05
LPA-80080-4CF-EDIN (Verizon)	C	From Leg	4.50 6.00 0.00	0.0000	305.25	No Ice 1/2" Ice	2.62 2.92	6.06 6.45	0.02 0.05
BXA-171085-8BF (Verizon)	A	From Leg	4.50 4.00 0.00	0.0000	305.25	No Ice 1/2" Ice	2.00 2.30	1.50 1.80	0.02 0.04
BXA-171085-8BF (Verizon)	B	From Leg	4.50 4.00 0.00	0.0000	305.25	No Ice 1/2" Ice	2.00 2.30	1.50 1.80	0.02 0.04
BXA-171085-8BF (Verizon)	C	From Leg	4.50 4.00 0.00	0.0000	305.25	No Ice 1/2" Ice	2.00 2.30	1.50 1.80	0.02 0.04
(2) Diplexer (Verizon)	A	From Leg	4.50 0.00 0.00	0.0000	305.25	No Ice 1/2" Ice	0.47 0.56	0.12 0.17	0.01 0.01
(2) Diplexer (Verizon)	B	From Leg	4.50 0.00 0.00	0.0000	305.25	No Ice 1/2" Ice	0.47 0.56	0.12 0.17	0.01 0.01
(2) Diplexer (Verizon)	C	From Leg	4.50 0.00 0.00	0.0000	305.25	No Ice 1/2" Ice	0.47 0.56	0.12 0.17	0.01 0.01

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	32 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
7770.00 (AT&T)	A	From Leg	0.00 4.50 0.00 0.00	0.0000	242.50	No Ice 1/2" Ice	10.03 10.61	5.60 6.15	0.02 0.07
7770.00 (AT&T)	B	From Leg	0.00 4.50 0.00 0.00	0.0000	242.50	No Ice 1/2" Ice	10.03 10.61	5.60 6.15	0.02 0.07
7770.00 (AT&T)	C	From Leg	0.00 4.50 0.00 0.00	0.0000	242.50	No Ice 1/2" Ice	10.03 10.61	5.60 6.15	0.02 0.07
(2) Diplexer (AT&T)	A	From Leg	0.00 4.50 0.00 0.00	0.0000	242.50	No Ice 1/2" Ice	0.47 0.56	0.12 0.17	0.01 0.01
(2) Diplexer (AT&T)	B	From Leg	0.00 4.50 0.00 0.00	0.0000	242.50	No Ice 1/2" Ice	0.47 0.56	0.12 0.17	0.01 0.01
(2) Diplexer (AT&T)	C	From Leg	0.00 4.50 0.00 0.00	0.0000	242.50	No Ice 1/2" Ice	0.47 0.56	0.12 0.17	0.01 0.01
Search Antenna	C	From Leg	0.00 1.00 0.00 0.00	0.0000	370.00	No Ice 1/2" Ice	1.28 1.53	3.73 4.39	0.30 0.45

Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert ft	Azimuth Adjustment °	3 dB Beam Width °	Elevation ft	Outside Diameter ft	Aperture Area ft²	Weight K	
9 FT DISH	A	Paraboloid w/o Radome	From Face	1.00 0.00 0.00	Worst		355.00	9.00	No Ice 1/2" Ice	63.62 66.00	0.32 0.73

Tower Pressures - No Ice

$$G_H = 1.075$$

Section Elevation ft	z ft	K _Z	q _z psf	A _G ft ²	F a c e	A _F ft ²	A _R ft ²	A _{leg} ft ²	Leg % ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²
T1 368.75-362.50	365.63	1.988	46	32.682	A	4.176	3.096	2.865	39.39	0.000	0.000
					B	4.176	2.865		40.69	0.000	0.000
					C	4.176	3.096		39.39	0.000	0.000
T2 362.50-356.25	359.38	1.978	46	32.682	A	4.812	3.443	2.865	34.70	0.000	0.000
					B	4.812	2.865		37.31	0.000	0.000

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	33 of 98
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	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	z	K _Z	q _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
ft	ft		psf	ft ²	e	ft ²	ft ²	ft ²			
T3	353.13	1.968	45	32.682	C	4.812	3.443		34.70	0.000	0.000
356.25-350.00					A	4.812	3.443	2.865	34.70	0.000	0.000
					B	4.812	3.327		35.19	0.000	0.000
					C	4.812	3.905		32.86	0.000	0.000
T4	346.88	1.958	45	32.813	A	4.804	3.703	3.125	36.73	0.000	0.000
350.00-343.75					B	4.804	4.859		32.34	0.000	0.000
					C	4.804	5.438		30.51	0.000	0.000
T5	340.63	1.948	45	32.813	A	0.990	4.495	3.125	56.98	0.000	0.000
343.75-337.50					B	0.990	5.651		47.06	0.000	0.000
					C	0.990	6.230		43.29	0.000	0.000
T6	334.38	1.938	45	32.813	A	0.000	5.152	3.125	60.65	0.000	0.000
337.50-331.25					B	0.000	6.309		49.54	0.000	0.000
					C	0.000	6.887		45.38	0.000	0.000
T7	328.13	1.928	45	32.813	A	0.000	5.152	3.125	60.65	0.000	0.000
331.25-325.00					B	0.000	6.309		49.54	0.000	0.000
					C	0.000	6.887		45.38	0.000	0.000
T8	321.88	1.917	44	32.943	A	0.000	6.600	3.385	51.29	0.000	0.000
325.00-318.75					B	0.000	7.757		43.65	0.000	0.000
					C	0.000	7.303		46.35	0.000	0.000
T9	315.63	1.906	44	32.943	A	0.000	6.595	3.385	51.33	0.000	0.000
318.75-312.50					B	0.000	7.752		43.67	0.000	0.000
					C	0.000	7.298		46.39	0.000	0.000
T10	309.38	1.895	44	32.943	A	0.000	6.595	3.385	51.33	0.000	0.000
312.50-306.25					B	0.000	7.752		43.67	0.000	0.000
					C	0.000	7.298		46.39	0.000	0.000
T11	303.13	1.884	44	32.943	A	0.985	5.941	3.385	48.88	0.000	0.000
306.25-300.00					B	0.985	7.097		41.89	0.000	0.000
					C	0.820	11.436		27.62	0.000	0.000
T12	296.88	1.873	43	32.943	A	4.770	4.995	3.385	34.67	0.000	0.000
300.00-293.75					B	4.770	6.151		31.00	0.000	0.000
					C	3.772	11.885		21.62	0.000	0.000
T13	290.63	1.862	43	32.943	A	0.985	5.941	3.385	48.88	0.000	0.000
293.75-287.50					B	0.985	7.097		41.89	0.000	0.000
					C	0.779	12.634		25.24	0.000	0.000
T14	284.38	1.85	43	32.943	A	0.000	6.438	3.385	52.59	0.000	0.000
287.50-281.25					B	0.000	7.594		44.58	0.000	0.000
					C	0.000	13.026		25.99	0.000	0.000
T15	278.13	1.839	42	32.943	A	0.000	6.438	3.385	52.59	0.000	0.000
281.25-275.00					B	0.000	7.594		44.58	0.000	0.000
					C	0.000	13.026		25.99	0.000	0.000
T16	271.88	1.827	42	32.943	A	0.000	6.438	3.385	52.59	0.000	0.000
275.00-268.75					B	0.000	7.594		44.58	0.000	0.000
					C	0.000	13.026		25.99	0.000	0.000
T17	265.63	1.815	42	32.943	A	0.000	6.438	3.385	52.59	0.000	0.000
268.75-262.50					B	0.000	7.594		44.58	0.000	0.000
					C	0.000	13.026		25.99	0.000	0.000
T18	259.38	1.802	42	32.943	A	0.000	6.438	3.385	52.59	0.000	0.000
262.50-256.25					B	0.000	7.594		44.58	0.000	0.000
					C	0.000	13.026		25.99	0.000	0.000
T19	253.13	1.79	41	32.943	A	0.000	6.595	3.385	51.33	0.000	0.000
256.25-250.00					B	0.000	7.752		43.67	0.000	0.000
					C	0.000	13.151		25.74	0.000	0.000
T20	246.88	1.777	41	32.943	A	0.985	5.941	3.385	48.88	0.000	0.000
250.00-243.75					B	0.985	7.097		41.89	0.000	0.000
					C	0.779	12.634		25.24	0.000	0.000
T21	240.63	1.764	41	32.943	A	0.985	11.386	3.385	27.37	0.000	0.000
243.75-237.50					B	0.985	7.097		41.89	0.000	0.000
					C	0.779	18.079		17.95	0.000	0.000
T22	234.38	1.751	40	32.943	A	0.985	12.129	3.385	25.82	0.000	0.000
237.50-231.25					B	0.985	7.097		41.89	0.000	0.000

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	34 of 98
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	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	z	K _Z	q _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
ft	ft		psf	ft ²		ft ²	ft ²	ft ²			
T23	228.13	1.737	40	32.943	C	0.779	18.821		17.27	0.000	0.000
231.25-225.00					A	0.985	12.129	3.385	25.82	0.000	0.000
					B	0.985	7.097		41.89	0.000	0.000
					C	0.779	18.821		17.27	0.000	0.000
T24	221.88	1.724	40	32.813	A	4.147	10.922	3.125	20.74	0.000	0.000
225.00-218.75					B	4.147	5.891		31.13	0.000	0.000
					C	3.280	17.813		14.82	0.000	0.000
T25	215.63	1.71	40	32.813	A	1.781	11.714	3.125	23.16	0.000	0.000
218.75-212.50					B	1.781	6.683		36.92	0.000	0.000
					C	1.410	18.439		15.74	0.000	0.000
T26	209.38	1.695	39	32.813	A	0.792	12.371	3.125	23.74	0.000	0.000
212.50-206.25					B	0.792	7.340		38.43	0.000	0.000
					C	0.627	18.960		15.96	0.000	0.000
T27	203.13	1.681	39	32.813	A	0.792	12.371	3.125	23.74	0.000	0.000
206.25-200.00					B	0.792	7.340		38.43	0.000	0.000
					C	0.627	18.960		15.96	0.000	0.000
T28	187.50	1.643	38	131.250	A	0.000	54.887	12.500	22.77	0.000	0.000
200.00-175.00					B	0.000	32.450		38.52	0.000	0.000
					C	0.000	80.231		15.58	0.000	0.000
T29	171.88	1.602	37	32.943	A	0.000	13.786	3.385	24.56	0.000	0.000
175.00-168.75					B	0.000	8.177		41.40	0.000	0.000
					C	0.000	19.796		17.10	0.000	0.000
T30	165.63	1.586	37	32.943	A	0.000	14.254	3.385	23.75	0.000	0.000
168.75-162.50					B	0.000	8.645		39.16	0.000	0.000
					C	0.000	20.166		16.79	0.000	0.000
T31	159.38	1.568	36	32.943	A	0.985	13.127	3.385	23.99	0.000	0.000
162.50-156.25					B	0.985	7.518		39.81	0.000	0.000
					C	0.779	19.275		16.88	0.000	0.000
T32	153.13	1.55	36	32.943	A	0.000	13.781	3.385	24.57	0.000	0.000
156.25-150.00					B	0.000	8.172		41.43	0.000	0.000
					C	0.000	19.792		17.11	0.000	0.000
T33	137.50	1.503	35	131.771	A	0.000	57.624	13.542	23.50	0.000	0.000
150.00-125.00					B	0.000	33.799		40.06	0.000	0.000
					C	0.000	80.555		16.81	0.000	0.000
T34	112.50	1.42	33	131.771	A	12.617	55.492	13.542	19.88	0.000	0.000
125.00-100.00					B	12.617	31.950		30.38	0.000	0.000
					C	9.976	80.194		15.02	0.000	0.000
T35	96.88	1.36	31	32.943	A	4.140	13.219	3.385	19.50	0.000	0.000
100.00-93.75					B	4.140	7.333		29.51	0.000	0.000
					C	3.273	19.531		14.85	0.000	0.000
T36	90.63	1.335	31	32.943	A	0.985	14.165	3.385	22.35	0.000	0.000
93.75-87.50					B	0.985	8.376		36.16	0.000	0.000
					C	0.779	20.376		16.00	0.000	0.000
T37	84.38	1.308	30	32.943	A	0.000	14.662	3.385	23.09	0.000	0.000
87.50-81.25					B	0.000	9.984		33.91	0.000	0.000
					C	0.000	21.880		15.47	0.000	0.000
T38	78.13	1.279	30	32.943	A	0.000	14.662	3.385	23.09	0.000	0.000
81.25-75.00					B	0.000	9.984		33.91	0.000	0.000
					C	0.000	21.880		15.47	0.000	0.000
T39	62.50	1.2	28	131.771	A	0.000	59.756	13.542	22.66	0.000	0.000
75.00-50.00					B	0.000	41.048		32.99	0.000	0.000
					C	0.000	87.521		15.47	0.000	0.000
T40	37.50	1.037	24	131.771	A	0.985	61.692	13.542	21.61	0.000	0.000
50.00-25.00					B	0.985	42.984		30.80	0.000	0.000
					C	0.779	87.004		15.43	0.000	0.000
T41	25.00-0.00	1	23	131.771	A	0.000	57.996	13.542	23.35	0.000	0.000
					B	0.000	41.533		32.60	0.000	0.000
					C	0.000	79.336		17.07	0.000	0.000

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	35 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Tower Pressure - With Ice

$$G_H = 1.075$$

Section Elevation	z	K _Z	q _z	t _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
ft	ft		psf	in	ft ²		ft ²	ft ²	ft ²			
T1 368.75-362.50	365.63	1.988	34	0.5000	33.203	A	4.176	6.016	3.906	38.33	0.000	0.000
						B	4.176	5.577		40.05	0.000	0.000
						C	4.176	6.016		38.33	0.000	0.000
T2 362.50-356.25	359.38	1.978	34	0.5000	33.203	A	4.812	6.676	3.906	34.00	0.000	0.000
						B	4.812	5.577		37.60	0.000	0.000
						C	4.812	6.676		34.00	0.000	0.000
T3 356.25-350.00	353.13	1.968	34	0.5000	33.203	A	4.812	6.676	3.906	34.00	0.000	0.000
						B	4.812	6.456		34.67	0.000	0.000
						C	4.812	7.555		31.59	0.000	0.000
T4 350.00-343.75	346.88	1.958	34	0.5000	33.333	A	4.804	6.933	4.167	35.50	0.000	0.000
						B	4.804	9.131		29.90	0.000	0.000
						C	4.804	10.230		27.71	0.000	0.000
T5 343.75-337.50	340.63	1.948	34	0.5000	33.333	A	0.990	7.721	4.167	47.84	0.000	0.000
						B	0.990	9.919		38.20	0.000	0.000
						C	0.990	11.018		34.70	0.000	0.000
T6 337.50-331.25	334.38	1.938	34	0.5000	33.333	A	0.000	8.378	4.167	49.73	0.000	0.000
						B	0.000	10.576		39.40	0.000	0.000
						C	0.000	11.675		35.69	0.000	0.000
T7 331.25-325.00	328.13	1.928	33	0.5000	33.333	A	0.000	8.378	4.167	49.73	0.000	0.000
						B	0.000	10.576		39.40	0.000	0.000
						C	0.000	11.675		35.69	0.000	0.000
T8 325.00-318.75	321.88	1.917	33	0.5000	33.464	A	0.000	10.344	4.427	42.80	0.000	0.000
						B	0.000	12.542		35.30	0.000	0.000
						C	0.000	12.089		36.62	0.000	0.000
T9 318.75-312.50	315.63	1.906	33	0.5000	33.464	A	0.000	10.334	4.427	42.84	0.000	0.000
						B	0.000	12.532		35.33	0.000	0.000
						C	0.000	12.079		36.65	0.000	0.000
T10 312.50-306.25	309.38	1.895	33	0.5000	33.464	A	0.000	10.334	4.427	42.84	0.000	0.000
						B	0.000	12.532		35.33	0.000	0.000
						C	0.000	12.079		36.65	0.000	0.000
T11 306.25-300.00	303.13	1.884	33	0.5000	33.464	A	0.985	9.680	4.427	41.51	0.000	0.000
						B	0.985	11.878		34.42	0.000	0.000
						C	6.442	12.057		23.93	0.000	0.000
T12 300.00-293.75	296.88	1.873	32	0.5000	33.464	A	4.770	8.734	4.427	32.78	0.000	0.000
						B	4.770	10.932		28.19	0.000	0.000
						C	10.482	11.546		20.10	0.000	0.000
T13 293.75-287.50	290.63	1.862	32	0.5000	33.464	A	0.985	9.680	4.427	41.51	0.000	0.000
						B	0.985	11.878		34.42	0.000	0.000
						C	7.806	12.215		22.11	0.000	0.000
T14 287.50-281.25	284.38	1.85	32	0.5000	33.464	A	0.000	10.177	4.427	43.50	0.000	0.000
						B	0.000	12.375		35.78	0.000	0.000
						C	7.109	12.566		22.50	0.000	0.000
T15 281.25-275.00	278.13	1.839	32	0.5000	33.464	A	0.000	10.177	4.427	43.50	0.000	0.000
						B	0.000	12.375		35.78	0.000	0.000
						C	7.109	12.566		22.50	0.000	0.000
T16 275.00-268.75	271.88	1.827	32	0.5000	33.464	A	0.000	10.177	4.427	43.50	0.000	0.000
						B	0.000	12.375		35.78	0.000	0.000
						C	7.109	12.566		22.50	0.000	0.000
T17 268.75-262.50	265.63	1.815	31	0.5000	33.464	A	0.000	10.177	4.427	43.50	0.000	0.000
						B	0.000	12.375		35.78	0.000	0.000
						C	7.109	12.566		22.50	0.000	0.000
T18 262.50-256.25	259.38	1.802	31	0.5000	33.464	A	0.000	10.177	4.427	43.50	0.000	0.000
						B	0.000	12.375		35.78	0.000	0.000

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	36 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	z	K _z	q _z	t _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
ft	ft		psf	in	ft ²		ft ²	ft ²	ft ²			
T19	253.13	1.79	31	0.5000	33.464	C	7.109	12.566		22.50	0.000	0.000
256.25-250.00						A	0.000	10.334	4.427	42.84	0.000	0.000
						B	0.000	12.532		35.33	0.000	0.000
						C	7.109	12.677		22.37	0.000	0.000
T20	246.88	1.777	31	0.5000	33.464	A	0.985	9.680	4.427	41.51	0.000	0.000
250.00-243.75						B	0.985	11.878		34.42	0.000	0.000
						C	7.806	12.215		22.11	0.000	0.000
T21	240.63	1.764	31	0.5000	33.464	A	7.241	11.046	4.427	24.21	0.000	0.000
243.75-237.50						B	0.985	11.878		34.42	0.000	0.000
						C	14.062	13.580		16.02	0.000	0.000
T22	234.38	1.751	30	0.5000	33.464	A	8.095	11.232	4.427	22.91	0.000	0.000
237.50-231.25						B	0.985	11.878		34.42	0.000	0.000
						C	14.915	13.767		15.44	0.000	0.000
T23	228.13	1.737	30	0.5000	33.464	A	8.095	11.232	4.427	22.91	0.000	0.000
231.25-225.00						B	0.985	11.878		34.42	0.000	0.000
						C	14.915	13.767		15.44	0.000	0.000
T24	221.88	1.724	30	0.5000	33.333	A	11.256	10.028	4.167	19.58	0.000	0.000
225.00-218.75						B	4.147	10.674		28.11	0.000	0.000
						C	17.152	12.840		13.89	0.000	0.000
T25	215.63	1.71	30	0.5000	33.333	A	8.891	11.221	4.167	20.72	0.000	0.000
218.75-212.50						B	1.781	11.867		30.53	0.000	0.000
						C	15.480	13.686		14.29	0.000	0.000
T26	209.38	1.695	29	0.5000	33.333	A	7.901	11.878	4.167	21.07	0.000	0.000
212.50-206.25						B	0.792	12.524		31.29	0.000	0.000
						C	14.779	14.151		14.40	0.000	0.000
T27	203.13	1.681	29	0.5000	33.333	A	7.901	11.878	4.167	21.07	0.000	0.000
206.25-200.00						B	0.792	12.524		31.29	0.000	0.000
						C	14.779	14.151		14.40	0.000	0.000
T28	187.50	1.643	28	0.5000	133.333	A	28.438	55.415	16.667	19.88	0.000	0.000
200.00-175.00						B	0.000	53.602		31.09	0.000	0.000
						C	56.875	62.862		13.92	0.000	0.000
T29	171.88	1.602	28	0.5000	33.464	A	7.109	13.936	4.427	21.04	0.000	0.000
175.00-168.75						B	0.000	13.483		32.84	0.000	0.000
						C	14.219	15.226		15.04	0.000	0.000
T30	165.63	1.586	27	0.5000	33.464	A	7.109	14.400	4.427	20.58	0.000	0.000
168.75-162.50						B	0.000	13.947		31.74	0.000	0.000
						C	14.219	15.551		14.87	0.000	0.000
T31	159.38	1.568	27	0.5000	33.464	A	8.095	13.273	4.427	20.72	0.000	0.000
162.50-156.25						B	0.985	12.819		32.07	0.000	0.000
						C	14.915	14.754		14.92	0.000	0.000
T32	153.13	1.55	27	0.5000	33.464	A	7.109	13.927	4.427	21.05	0.000	0.000
156.25-150.00						B	0.000	13.474		32.86	0.000	0.000
						C	14.219	15.217		15.04	0.000	0.000
T33	137.50	1.503	26	0.5000	133.854	A	28.438	61.373	17.708	19.72	0.000	0.000
150.00-125.00						B	0.000	56.923		31.11	0.000	0.000
						C	56.875	63.504		14.71	0.000	0.000
T34	112.50	1.42	25	0.5000	133.854	A	41.055	60.240	17.708	17.48	0.000	0.000
125.00-100.00						B	12.617	57.323		25.32	0.000	0.000
						C	65.795	66.324		13.40	0.000	0.000
T35	96.88	1.36	24	0.5000	33.464	A	11.249	14.406	4.427	17.26	0.000	0.000
100.00-93.75						B	4.140	13.677		24.85	0.000	0.000
						C	17.145	16.119		13.31	0.000	0.000
T36	93.75-87.50	1.335	23	0.5000	33.464	A	8.095	15.352	4.427	18.88	0.000	0.000
						B	1.130	14.689		27.99	0.000	0.000
						C	15.060	16.853		13.87	0.000	0.000
T37	87.50-81.25	1.308	23	0.5000	33.464	A	7.109	15.849	4.427	19.28	0.000	0.000
						B	1.813	15.942		24.93	0.000	0.000
						C	16.031	17.961		13.02	0.000	0.000
T38	81.25-75.00	1.279	22	0.5000	33.464	A	7.109	15.849	4.427	19.28	0.000	0.000
						B	1.813	15.942		24.93	0.000	0.000

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	37 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation ft	z ft	K _Z	q _z psf	t _z in	A _G ft ²	F a c e	A _F ft ²	A _R ft ²	A _{leg} ft ²	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
T39 75.00-50.00	62.50	1.2	21	0.5000	133.854	C	16.031	17.961		13.02	0.000	0.000
						A	28.438	65.504	17.708	18.85	0.000	0.000
						B	7.250	65.879		24.22	0.000	0.000
T40 50.00-25.00	37.50	1.037	18	0.5000	133.854	C	64.125	71.846		13.02	0.000	0.000
						A	29.423	69.774	17.708	17.85	0.000	0.000
						B	8.235	70.149		22.59	0.000	0.000
T41 25.00-0.00	12.50	1	17	0.5000	133.854	C	64.822	71.383		13.00	0.000	0.000
						A	25.025	67.136	17.708	19.21	0.000	0.000
						B	6.380	67.466		23.98	0.000	0.000
						C	56.430	66.837		14.37	0.000	0.000

Tower Pressure - Service

$$G_H = 1.075$$

Section Elevation ft	z ft	K _Z	q _z psf	A _G ft ²	F a c e	A _F ft ²	A _R ft ²	A _{leg} ft ²	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
T1 368.75-362.50	365.63	1.988	13	32.682	A	4.176	3.096	2.865	39.39	0.000	0.000
					B	4.176	2.865		40.69	0.000	0.000
					C	4.176	3.096		39.39	0.000	0.000
T2 362.50-356.25	359.38	1.978	13	32.682	A	4.812	3.443	2.865	34.70	0.000	0.000
					B	4.812	2.865		37.31	0.000	0.000
					C	4.812	3.443		34.70	0.000	0.000
T3 356.25-350.00	353.13	1.968	13	32.682	A	4.812	3.443	2.865	34.70	0.000	0.000
					B	4.812	3.327		35.19	0.000	0.000
					C	4.812	3.905		32.86	0.000	0.000
T4 350.00-343.75	346.88	1.958	13	32.813	A	4.804	3.703	3.125	36.73	0.000	0.000
					B	4.804	4.859		32.34	0.000	0.000
					C	4.804	5.438		30.51	0.000	0.000
T5 343.75-337.50	340.63	1.948	12	32.813	A	0.990	4.495	3.125	56.98	0.000	0.000
					B	0.990	5.651		47.06	0.000	0.000
					C	0.990	6.230		43.29	0.000	0.000
T6 337.50-331.25	334.38	1.938	12	32.813	A	0.000	5.152	3.125	60.65	0.000	0.000
					B	0.000	6.309		49.54	0.000	0.000
					C	0.000	6.887		45.38	0.000	0.000
T7 331.25-325.00	328.13	1.928	12	32.813	A	0.000	5.152	3.125	60.65	0.000	0.000
					B	0.000	6.309		49.54	0.000	0.000
					C	0.000	6.887		45.38	0.000	0.000
T8 325.00-318.75	321.88	1.917	12	32.943	A	0.000	6.600	3.385	51.29	0.000	0.000
					B	0.000	7.757		43.65	0.000	0.000
					C	0.000	7.303		46.35	0.000	0.000
T9 318.75-312.50	315.63	1.906	12	32.943	A	0.000	6.595	3.385	51.33	0.000	0.000
					B	0.000	7.752		43.67	0.000	0.000
					C	0.000	7.298		46.39	0.000	0.000
T10 312.50-306.25	309.38	1.895	12	32.943	A	0.000	6.595	3.385	51.33	0.000	0.000
					B	0.000	7.752		43.67	0.000	0.000
					C	0.000	7.298		46.39	0.000	0.000
T11 306.25-300.00	303.13	1.884	12	32.943	A	0.985	5.941	3.385	48.88	0.000	0.000
					B	0.985	7.097		41.89	0.000	0.000
					C	0.820	11.436		27.62	0.000	0.000
T12 300.00-293.75	296.88	1.873	12	32.943	A	4.770	4.995	3.385	34.67	0.000	0.000
					B	4.770	6.151		31.00	0.000	0.000
					C	3.772	11.885		21.62	0.000	0.000

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	38 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	z	K _Z	q _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
ft	ft		psf	ft ²		ft ²	ft ²	ft ²			
T13 293.75-287.50	290.63	1.862	12	32.943	A	0.985	5.941	3.385	48.88	0.000	0.000
					B	0.985	7.097		41.89	0.000	0.000
					C	0.779	12.634		25.24	0.000	0.000
T14 287.50-281.25	284.38	1.85	12	32.943	A	0.000	6.438	3.385	52.59	0.000	0.000
					B	0.000	7.594		44.58	0.000	0.000
					C	0.000	13.026		25.99	0.000	0.000
T15 281.25-275.00	278.13	1.839	12	32.943	A	0.000	6.438	3.385	52.59	0.000	0.000
					B	0.000	7.594		44.58	0.000	0.000
					C	0.000	13.026		25.99	0.000	0.000
T16 275.00-268.75	271.88	1.827	12	32.943	A	0.000	6.438	3.385	52.59	0.000	0.000
					B	0.000	7.594		44.58	0.000	0.000
					C	0.000	13.026		25.99	0.000	0.000
T17 268.75-262.50	265.63	1.815	12	32.943	A	0.000	6.438	3.385	52.59	0.000	0.000
					B	0.000	7.594		44.58	0.000	0.000
					C	0.000	13.026		25.99	0.000	0.000
T18 262.50-256.25	259.38	1.802	12	32.943	A	0.000	6.438	3.385	52.59	0.000	0.000
					B	0.000	7.594		44.58	0.000	0.000
					C	0.000	13.026		25.99	0.000	0.000
T19 256.25-250.00	253.13	1.79	11	32.943	A	0.000	6.595	3.385	51.33	0.000	0.000
					B	0.000	7.752		43.67	0.000	0.000
					C	0.000	13.151		25.74	0.000	0.000
T20 250.00-243.75	246.88	1.777	11	32.943	A	0.985	5.941	3.385	48.88	0.000	0.000
					B	0.985	7.097		41.89	0.000	0.000
					C	0.779	12.634		25.24	0.000	0.000
T21 243.75-237.50	240.63	1.764	11	32.943	A	0.985	11.386	3.385	27.37	0.000	0.000
					B	0.985	7.097		41.89	0.000	0.000
					C	0.779	18.079		17.95	0.000	0.000
T22 237.50-231.25	234.38	1.751	11	32.943	A	0.985	12.129	3.385	25.82	0.000	0.000
					B	0.985	7.097		41.89	0.000	0.000
					C	0.779	18.821		17.27	0.000	0.000
T23 231.25-225.00	228.13	1.737	11	32.943	A	0.985	12.129	3.385	25.82	0.000	0.000
					B	0.985	7.097		41.89	0.000	0.000
					C	0.779	18.821		17.27	0.000	0.000
T24 225.00-218.75	221.88	1.724	11	32.813	A	4.147	10.922	3.125	20.74	0.000	0.000
					B	4.147	5.891		31.13	0.000	0.000
					C	3.280	17.813		14.82	0.000	0.000
T25 218.75-212.50	215.63	1.71	11	32.813	A	1.781	11.714	3.125	23.16	0.000	0.000
					B	1.781	6.683		36.92	0.000	0.000
					C	1.410	18.439		15.74	0.000	0.000
T26 212.50-206.25	209.38	1.695	11	32.813	A	0.792	12.371	3.125	23.74	0.000	0.000
					B	0.792	7.340		38.43	0.000	0.000
					C	0.627	18.960		15.96	0.000	0.000
T27 206.25-200.00	203.13	1.681	11	32.813	A	0.792	12.371	3.125	23.74	0.000	0.000
					B	0.792	7.340		38.43	0.000	0.000
					C	0.627	18.960		15.96	0.000	0.000
T28 200.00-175.00	187.50	1.643	11	131.250	A	0.000	54.887	12.500	22.77	0.000	0.000
					B	0.000	32.450		38.52	0.000	0.000
					C	0.000	80.231		15.58	0.000	0.000
T29 175.00-168.75	171.88	1.602	10	32.943	A	0.000	13.786	3.385	24.56	0.000	0.000
					B	0.000	8.177		41.40	0.000	0.000
					C	0.000	19.796		17.10	0.000	0.000
T30 168.75-162.50	165.63	1.586	10	32.943	A	0.000	14.254	3.385	23.75	0.000	0.000
					B	0.000	8.645		39.16	0.000	0.000
					C	0.000	20.166		16.79	0.000	0.000
T31 162.50-156.25	159.38	1.568	10	32.943	A	0.985	13.127	3.385	23.99	0.000	0.000
					B	0.985	7.518		39.81	0.000	0.000
					C	0.779	19.275		16.88	0.000	0.000
T32 156.25-150.00	153.13	1.55	10	32.943	A	0.000	13.781	3.385	24.57	0.000	0.000
					B	0.000	8.172		41.43	0.000	0.000
					C	0.000	19.792		17.11	0.000	0.000

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	39 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	z	K _z	q _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _{AA} In Face ft ²	C _{AA} Out Face ft ²
ft	ft		psf	ft ²		ft ²	ft ²	ft ²			
T33 150.00-125.00	137.50	1.503	10	131.771	A	0.000	57.624	13.542	23.50	0.000	0.000
					B	0.000	33.799		40.06	0.000	0.000
					C	0.000	80.555		16.81	0.000	0.000
T34 125.00-100.00	112.50	1.42	9	131.771	A	12.617	55.492	13.542	19.88	0.000	0.000
					B	12.617	31.950		30.38	0.000	0.000
					C	9.976	80.194		15.02	0.000	0.000
T35 100.00-93.75	96.88	1.36	9	32.943	A	4.140	13.219	3.385	19.50	0.000	0.000
					B	4.140	7.333		29.51	0.000	0.000
					C	3.273	19.531		14.85	0.000	0.000
T36 93.75-87.50	90.63	1.335	9	32.943	A	0.985	14.165	3.385	22.35	0.000	0.000
					B	0.985	8.376		36.16	0.000	0.000
					C	0.779	20.376		16.00	0.000	0.000
T37 87.50-81.25	84.38	1.308	8	32.943	A	0.000	14.662	3.385	23.09	0.000	0.000
					B	0.000	9.984		33.91	0.000	0.000
					C	0.000	21.880		15.47	0.000	0.000
T38 81.25-75.00	78.13	1.279	8	32.943	A	0.000	14.662	3.385	23.09	0.000	0.000
					B	0.000	9.984		33.91	0.000	0.000
					C	0.000	21.880		15.47	0.000	0.000
T39 75.00-50.00	62.50	1.2	8	131.771	A	0.000	59.756	13.542	22.66	0.000	0.000
					B	0.000	41.048		32.99	0.000	0.000
					C	0.000	87.521		15.47	0.000	0.000
T40 50.00-25.00	37.50	1.037	7	131.771	A	0.985	61.692	13.542	21.61	0.000	0.000
					B	0.985	42.984		30.80	0.000	0.000
					C	0.779	87.004		15.43	0.000	0.000
T41 25.00-0.00	12.50	1	6	131.771	A	0.000	57.996	13.542	23.35	0.000	0.000
					B	0.000	41.533		32.60	0.000	0.000
					C	0.000	79.336		17.07	0.000	0.000

Tower Forces - No Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1 368.75-362.50	0.00	0.68	A	0.223	2.523	0.595	1	1	6.019	0.75	119.95	C
			B	0.215	2.545	0.594	1	1	5.877			
			C	0.223	2.523	0.595	1	1	6.019			
T2 362.50-356.25	0.00	1.09	A	0.253	2.43	0.603	1	1	6.887	0.82	131.55	C
			B	0.235	2.484	0.598	1	1	6.526			
			C	0.253	2.43	0.603	1	1	6.887			
T3 356.25-350.00	0.01	1.09	A	0.253	2.43	0.603	1	1	6.887	0.84	134.14	C
			B	0.249	2.44	0.602	1	1	6.814			
			C	0.267	2.388	0.606	1	1	7.180			
T4 350.00-343.75	0.01	0.77	A	0.259	2.41	0.604	1	1	7.042	0.90	144.03	C
		TA 0.79	B	0.295	2.311	0.614	1	1	7.789			
			C	0.312	2.264	0.62	1	1	8.174			
T5 343.75-337.50	0.01	0.61	A	0.167	2.709	0.584	1	1	3.616	0.57	91.97	C
			B	0.202	2.588	0.591	1	1	4.329			
			C	0.22	2.531	0.595	1	1	4.694			
T6 337.50-331.25	0.01	0.54	A	0.157	2.746	0.583	1	1	3.002	0.50	80.55	C
			B	0.192	2.622	0.589	1	1	3.715			
			C	0.21	2.563	0.592	1	1	4.080			
T7 331.25-325.00	0.01	0.54	A	0.157	2.746	0.583	1	1	3.002	0.50	80.11	C
			B	0.192	2.622	0.589	1	1	3.715			
			C	0.21	2.563	0.592	1	1	4.080			
T8 325.00-318.75	0.02	0.64	A	0.2	2.595	0.59	1	1	3.897	0.55	87.74	B
			B	0.235	2.482	0.598	1	1	4.641			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	40 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
T9 318.75-312.50	0.02	0.64	C	0.222	2.525	0.595	1	1	4.346			
			A	0.2	2.595	0.59	1	1	3.894	0.55	87.20	B
			B	0.235	2.482	0.598	1	1	4.637			
			C	0.222	2.526	0.595	1	1	4.343			
T10 312.50-306.25	0.02	0.64	A	0.2	2.595	0.59	1	1	3.894	0.54	86.71	B
			B	0.235	2.482	0.598	1	1	4.637			
			C	0.222	2.526	0.595	1	1	4.343			
T11 306.25-300.00	0.08	0.71	A	0.21	2.562	0.593	1	1	4.506	0.81	129.44	C
			B	0.245	2.452	0.601	1	1	5.249			
			C	0.372	2.122	0.641	1	1	8.146			
T12 300.00-293.75	0.10	0.85	A	0.296	2.306	0.615	1	1	7.841	1.07	171.57	C
		TA 0.79	B	0.332	2.216	0.626	1	1	8.621			
			C	0.475	1.934	0.685	1	1	11.916			
T13 293.75-287.50	0.10	0.71	A	0.21	2.562	0.593	1	1	4.506	0.86	137.24	C
			B	0.245	2.452	0.601	1	1	5.249			
			C	0.407	2.05	0.655	1	1	9.048			
T14 287.50-281.25	0.10	0.61	A	0.195	2.611	0.589	1	1	3.795	0.81	129.02	C
			B	0.231	2.497	0.597	1	1	4.534			
			C	0.395	2.073	0.65	1	1	8.464			
T15 281.25-275.00	0.10	0.61	A	0.195	2.611	0.589	1	1	3.795	0.80	128.20	C
			B	0.231	2.497	0.597	1	1	4.534			
			C	0.395	2.073	0.65	1	1	8.464			
T16 275.00-268.75	0.10	0.61	A	0.195	2.611	0.589	1	1	3.795	0.80	127.37	C
			B	0.231	2.497	0.597	1	1	4.534			
			C	0.395	2.073	0.65	1	1	8.464			
T17 268.75-262.50	0.10	0.61	A	0.195	2.611	0.589	1	1	3.795	0.79	126.53	C
			B	0.231	2.497	0.597	1	1	4.534			
			C	0.395	2.073	0.65	1	1	8.464			
T18 262.50-256.25	0.10	0.61	A	0.195	2.611	0.589	1	1	3.795	0.79	125.67	C
			B	0.231	2.497	0.597	1	1	4.534			
			C	0.395	2.073	0.65	1	1	8.464			
T19 256.25-250.00	0.10	0.64	A	0.2	2.595	0.59	1	1	3.894	0.79	125.83	C
			B	0.235	2.482	0.598	1	1	4.637			
			C	0.399	2.066	0.651	1	1	8.565			
T20 250.00-243.75	0.10	0.71	A	0.21	2.562	0.593	1	1	4.506	0.82	130.99	C
			B	0.245	2.452	0.601	1	1	5.249			
			C	0.407	2.05	0.655	1	1	9.048			
T21 243.75-237.50	0.17	0.71	A	0.376	2.114	0.642	1	1	8.294	1.13	180.33	C
			B	0.245	2.452	0.601	1	1	5.249			
			C	0.572	1.824	0.737	1	1	14.105			
T22 237.50-231.25	0.18	0.71	A	0.398	2.068	0.651	1	1	8.879	1.17	187.42	C
			B	0.245	2.452	0.601	1	1	5.249			
			C	0.595	1.807	0.751	1	1	14.905			
T23 231.25-225.00	0.18	0.71	A	0.398	2.068	0.651	1	1	8.879	1.16	185.98	C
			B	0.245	2.452	0.601	1	1	5.249			
			C	0.595	1.807	0.751	1	1	14.905			
T24 225.00-218.75	0.18	0.95	A	0.459	1.959	0.678	1	1	11.547	1.31	209.99	C
		TA 0.79	B	0.306	2.28	0.618	1	1	7.785			
			C	0.643	1.784	0.781	1	1	17.187			
T25 218.75-212.50	0.18	0.66	A	0.411	2.042	0.656	1	1	9.469	1.17	187.97	C
			B	0.258	2.414	0.604	1	1	5.817			
			C	0.605	1.801	0.757	1	1	15.362			
T26 212.50-206.25	0.18	0.58	A	0.401	2.062	0.652	1	1	8.858	1.13	181.03	C
			B	0.248	2.444	0.601	1	1	5.205			
			C	0.597	1.806	0.752	1	1	14.879			
T27 206.25-200.00	0.18	0.58	A	0.401	2.062	0.652	1	1	8.858	1.12	179.47	C
			B	0.248	2.444	0.601	1	1	5.205			
			C	0.597	1.806	0.752	1	1	14.879			
T28 200.00-175.00	0.72	2.28	A	0.418	2.029	0.659	1	1	36.181	4.48	179.01	C
			B	0.247	2.446	0.601	1	1	19.508			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	41 of 98
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	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T29	0.18	0.61	C	0.611	1.797	0.761	1	1	61.022			
175.00-168.75			A	0.418	2.029	0.659	1	1	9.089	1.07	171.45	C
			B	0.248	2.443	0.601	1	1	4.918			
			C	0.601	1.803	0.754	1	1	14.930			
T30	0.18	0.69	A	0.433	2.003	0.665	1	1	9.486	1.09	173.78	C
168.75-162.50			B	0.262	2.401	0.605	1	1	5.231			
			C	0.612	1.797	0.761	1	1	15.348			
T31	0.18	0.69	A	0.428	2.011	0.664	1	1	9.696	1.08	172.73	C
162.50-156.25			B	0.258	2.413	0.604	1	1	5.526			
			C	0.609	1.799	0.759	1	1	15.408			
T32	0.18	0.61	A	0.418	2.029	0.659	1	1	9.085	1.04	165.84	C
156.25-150.00			B	0.248	2.443	0.601	1	1	4.914			
			C	0.601	1.804	0.754	1	1	14.925			
T33	0.74	2.45	A	0.437	1.995	0.668	1	1	38.466	4.11	164.49	C
150.00-125.00			B	0.257	2.418	0.604	1	1	20.400			
			C	0.611	1.797	0.761	1	1	61.270			
T34	0.76	2.84	A	0.517	1.879	0.706	1	1	51.808	4.69	187.45	C
125.00-100.00			B	0.338	2.199	0.628	1	1	32.693			
			C	0.684	1.776	0.809	1	1	74.838			
T35	0.19	1.03	A	0.527	1.867	0.712	1	1	13.546	1.15	184.10	C
100.00-93.75		TA 0.79	B	0.348	2.176	0.632	1	1	8.773			
			C	0.692	1.776	0.814	1	1	19.179			
T36	0.19	0.71	A	0.46	1.958	0.678	1	1	10.587	0.99	157.80	C
93.75-87.50			B	0.284	2.339	0.611	1	1	6.105			
			C	0.642	1.784	0.78	1	1	16.679			
T37	0.20	0.61	A	0.445	1.982	0.671	1	1	9.838	1.00	160.73	C
87.50-81.25			B	0.303	2.288	0.617	1	1	6.159			
			C	0.664	1.778	0.795	1	1	17.395			
T38	0.20	0.61	A	0.445	1.982	0.671	1	1	9.838	0.98	157.24	C
81.25-75.00			B	0.303	2.288	0.617	1	1	6.159			
			C	0.664	1.778	0.795	1	1	17.395			
T39	0.79	2.45	A	0.453	1.968	0.675	1	1	40.328	3.69	147.53	C
75.00-50.00			B	0.312	2.266	0.619	1	1	25.429			
			C	0.664	1.778	0.795	1	1	69.578			
T40	0.80	2.47	A	0.476	1.934	0.685	1	1	43.268	3.21	128.36	C
50.00-25.00			B	0.334	2.21	0.627	1	1	27.927			
			C	0.666	1.778	0.796	1	1	70.063			
T41	0.71	2.45	A	0.44	1.99	0.669	1	1	38.787	2.68	107.26	C
25.00-0.00			B	0.315	2.256	0.621	1	1	25.778			
			C	0.602	1.803	0.755	1	1	59.889			
Sum Weight:	8.34	42.51								54.30		

Tower Forces - No Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1	0.00	0.68	A	0.223	2.523	0.595	0.8	1	5.184	0.65	103.31	C
368.75-362.50			B	0.215	2.545	0.594	0.8	1	5.041			
			C	0.223	2.523	0.595	0.8	1	5.184			
T2	0.00	1.09	A	0.253	2.43	0.603	0.8	1	5.924	0.71	113.17	C
362.50-356.25			B	0.235	2.484	0.598	0.8	1	5.563			
			C	0.253	2.43	0.603	0.8	1	5.924			
T3	0.01	1.09	A	0.253	2.43	0.603	0.8	1	5.924	0.73	116.16	C
356.25-350.00			B	0.249	2.44	0.602	0.8	1	5.852			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	42 of 98
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	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T4	0.01	0.77	C	0.267	2.388	0.606	0.8	1	6.218			
350.00-343.75		TA 0.79	A	0.259	2.41	0.604	0.8	1	6.081	0.79	127.10	C
			B	0.295	2.311	0.614	0.8	1	6.828			
T5	0.01	0.61	C	0.312	2.264	0.62	0.8	1	7.213			
343.75-337.50			A	0.167	2.709	0.584	0.8	1	3.418	0.55	88.09	C
			B	0.202	2.588	0.591	0.8	1	4.131			
T6	0.01	0.54	C	0.22	2.531	0.595	0.8	1	4.496			
337.50-331.25			A	0.157	2.746	0.583	0.8	1	3.002	0.50	80.55	C
			B	0.192	2.622	0.589	0.8	1	3.715			
T7	0.01	0.54	C	0.21	2.563	0.592	0.8	1	4.080			
331.25-325.00			A	0.157	2.746	0.583	0.8	1	3.002	0.50	80.11	C
			B	0.192	2.622	0.589	0.8	1	3.715			
T8	0.02	0.64	C	0.21	2.563	0.592	0.8	1	4.080			
325.00-318.75			A	0.2	2.595	0.59	0.8	1	3.897	0.55	87.74	B
			B	0.235	2.482	0.598	0.8	1	4.641			
T9	0.02	0.64	C	0.222	2.525	0.595	0.8	1	4.346			
318.75-312.50			A	0.2	2.595	0.59	0.8	1	3.894	0.55	87.20	B
			B	0.235	2.482	0.598	0.8	1	4.637			
T10	0.02	0.64	C	0.222	2.526	0.595	0.8	1	4.343			
312.50-306.25			A	0.2	2.595	0.59	0.8	1	3.894	0.54	86.71	B
			B	0.235	2.482	0.598	0.8	1	4.637			
T11	0.08	0.71	C	0.222	2.526	0.595	0.8	1	4.343			
306.25-300.00			A	0.21	2.562	0.593	0.8	1	4.309	0.79	126.84	C
			B	0.245	2.452	0.601	0.8	1	5.052			
T12	0.10	0.85	C	0.372	2.122	0.641	0.8	1	7.982			
300.00-293.75		TA 0.79	A	0.296	2.306	0.615	0.8	1	6.887	1.00	160.71	C
			B	0.332	2.216	0.626	0.8	1	7.667			
T13	0.10	0.71	C	0.475	1.934	0.685	0.8	1	11.161			
293.75-287.50			A	0.21	2.562	0.593	0.8	1	4.309	0.84	134.88	C
			B	0.245	2.452	0.601	0.8	1	5.052			
T14	0.10	0.61	C	0.407	2.05	0.655	0.8	1	8.892			
287.50-281.25			A	0.195	2.611	0.589	0.8	1	3.795	0.81	129.02	C
			B	0.231	2.497	0.597	0.8	1	4.534			
T15	0.10	0.61	C	0.395	2.073	0.65	0.8	1	8.464			
281.25-275.00			A	0.195	2.611	0.589	0.8	1	3.795	0.80	128.20	C
			B	0.231	2.497	0.597	0.8	1	4.534			
T16	0.10	0.61	C	0.395	2.073	0.65	0.8	1	8.464			
275.00-268.75			A	0.195	2.611	0.589	0.8	1	3.795	0.80	127.37	C
			B	0.231	2.497	0.597	0.8	1	4.534			
T17	0.10	0.61	C	0.395	2.073	0.65	0.8	1	8.464			
268.75-262.50			A	0.195	2.611	0.589	0.8	1	3.795	0.79	126.53	C
			B	0.231	2.497	0.597	0.8	1	4.534			
T18	0.10	0.61	C	0.395	2.073	0.65	0.8	1	8.464			
262.50-256.25			A	0.195	2.611	0.589	0.8	1	3.795	0.79	125.67	C
			B	0.231	2.497	0.597	0.8	1	4.534			
T19	0.10	0.64	C	0.395	2.073	0.65	0.8	1	8.464			
256.25-250.00			A	0.2	2.595	0.59	0.8	1	3.894	0.79	125.83	C
			B	0.235	2.482	0.598	0.8	1	4.637			
T20	0.10	0.71	C	0.399	2.066	0.651	0.8	1	8.565			
250.00-243.75			A	0.21	2.562	0.593	0.8	1	4.309	0.80	128.74	C
			B	0.245	2.452	0.601	0.8	1	5.052			
T21	0.17	0.71	C	0.407	2.05	0.655	0.8	1	8.892			
243.75-237.50			A	0.376	2.114	0.642	0.8	1	8.097	1.11	178.34	C
			B	0.245	2.452	0.601	0.8	1	5.052			
T22	0.18	0.71	C	0.572	1.824	0.737	0.8	1	13.949			
237.50-231.25			A	0.398	2.068	0.651	0.8	1	8.682	1.16	185.46	C
			B	0.245	2.452	0.601	0.8	1	5.052			
T23	0.18	0.71	C	0.595	1.807	0.751	0.8	1	14.749			
231.25-225.00			A	0.398	2.068	0.651	0.8	1	8.682	1.15	184.03	C
			B	0.245	2.452	0.601	0.8	1	5.052			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	44 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Tower Forces - No Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1 368.75-362.50	0.00	0.68	A	0.223	2.523	0.595	0.85	1	5.392	0.67	107.47	C
			B	0.215	2.545	0.594	0.85	1	5.250			
			C	0.223	2.523	0.595	0.85	1	5.392			
T2 362.50-356.25	0.00	1.09	A	0.253	2.43	0.603	0.85	1	6.165	0.74	117.76	C
			B	0.235	2.484	0.598	0.85	1	5.804			
			C	0.253	2.43	0.603	0.85	1	6.165			
T3 356.25-350.00	0.01	1.09	A	0.253	2.43	0.603	0.85	1	6.165	0.75	120.65	C
			B	0.249	2.44	0.602	0.85	1	6.092			
			C	0.267	2.388	0.606	0.85	1	6.458			
T4 350.00-343.75	0.01	0.77 TA 0.79	A	0.259	2.41	0.604	0.85	1	6.321	0.82	131.34	C
			B	0.295	2.311	0.614	0.85	1	7.068			
			C	0.312	2.264	0.62	0.85	1	7.453			
T5 343.75-337.50	0.01	0.61	A	0.167	2.709	0.584	0.85	1	3.467	0.56	89.06	C
			B	0.202	2.588	0.591	0.85	1	4.181			
			C	0.22	2.531	0.595	0.85	1	4.546			
T6 337.50-331.25	0.01	0.54	A	0.157	2.746	0.583	0.85	1	3.002	0.50	80.55	C
			B	0.192	2.622	0.589	0.85	1	3.715			
			C	0.21	2.563	0.592	0.85	1	4.080			
T7 331.25-325.00	0.01	0.54	A	0.157	2.746	0.583	0.85	1	3.002	0.50	80.11	C
			B	0.192	2.622	0.589	0.85	1	3.715			
			C	0.21	2.563	0.592	0.85	1	4.080			
T8 325.00-318.75	0.02	0.64	A	0.2	2.595	0.59	0.85	1	3.897	0.55	87.74	B
			B	0.235	2.482	0.598	0.85	1	4.641			
			C	0.222	2.525	0.595	0.85	1	4.346			
T9 318.75-312.50	0.02	0.64	A	0.2	2.595	0.59	0.85	1	3.894	0.55	87.20	B
			B	0.235	2.482	0.598	0.85	1	4.637			
			C	0.222	2.526	0.595	0.85	1	4.343			
T10 312.50-306.25	0.02	0.64	A	0.2	2.595	0.59	0.85	1	3.894	0.54	86.71	B
			B	0.235	2.482	0.598	0.85	1	4.637			
			C	0.222	2.526	0.595	0.85	1	4.343			
T11 306.25-300.00	0.08	0.71	A	0.21	2.562	0.593	0.85	1	4.358	0.80	127.49	C
			B	0.245	2.452	0.601	0.85	1	5.101			
			C	0.372	2.122	0.641	0.85	1	8.023			
T12 300.00-293.75	0.10	0.85 TA 0.79	A	0.296	2.306	0.615	0.85	1	7.126	1.02	163.42	C
			B	0.332	2.216	0.626	0.85	1	7.906			
			C	0.475	1.934	0.685	0.85	1	11.350			
T13 293.75-287.50	0.10	0.71	A	0.21	2.562	0.593	0.85	1	4.358	0.85	135.47	C
			B	0.245	2.452	0.601	0.85	1	5.101			
			C	0.407	2.05	0.655	0.85	1	8.931			
T14 287.50-281.25	0.10	0.61	A	0.195	2.611	0.589	0.85	1	3.795	0.81	129.02	C
			B	0.231	2.497	0.597	0.85	1	4.534			
			C	0.395	2.073	0.65	0.85	1	8.464			
T15 281.25-275.00	0.10	0.61	A	0.195	2.611	0.589	0.85	1	3.795	0.80	128.20	C
			B	0.231	2.497	0.597	0.85	1	4.534			
			C	0.395	2.073	0.65	0.85	1	8.464			
T16 275.00-268.75	0.10	0.61	A	0.195	2.611	0.589	0.85	1	3.795	0.80	127.37	C
			B	0.231	2.497	0.597	0.85	1	4.534			
			C	0.395	2.073	0.65	0.85	1	8.464			
T17 268.75-262.50	0.10	0.61	A	0.195	2.611	0.589	0.85	1	3.795	0.79	126.53	C
			B	0.231	2.497	0.597	0.85	1	4.534			
			C	0.395	2.073	0.65	0.85	1	8.464			
T18 262.50-256.25	0.10	0.61	A	0.195	2.611	0.589	0.85	1	3.795	0.79	125.67	C
			B	0.231	2.497	0.597	0.85	1	4.534			
			C	0.395	2.073	0.65	0.85	1	8.464			
T19 256.25-250.00	0.10	0.64	A	0.2	2.595	0.59	0.85	1	3.894	0.79	125.83	C
			B	0.235	2.482	0.598	0.85	1	4.637			
			C	0.399	2.066	0.651	0.85	1	8.565			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	45 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T20 250.00-243.75	0.10	0.71	A	0.21	2.562	0.593	0.85	1	4.358	0.81	129.30	C
			B	0.245	2.452	0.601	0.85	1	5.101			
			C	0.407	2.05	0.655	0.85	1	8.931			
T21 243.75-237.50	0.17	0.71	A	0.376	2.114	0.642	0.85	1	8.146	1.12	178.84	C
			B	0.245	2.452	0.601	0.85	1	5.101			
			C	0.572	1.824	0.737	0.85	1	13.988			
T22 237.50-231.25	0.18	0.71	A	0.398	2.068	0.651	0.85	1	8.731	1.16	185.95	C
			B	0.245	2.452	0.601	0.85	1	5.101			
			C	0.595	1.807	0.751	0.85	1	14.788			
T23 231.25-225.00	0.18	0.71	A	0.398	2.068	0.651	0.85	1	8.731	1.15	184.52	C
			B	0.245	2.452	0.601	0.85	1	5.101			
			C	0.595	1.807	0.751	0.85	1	14.788			
T24 225.00-218.75	0.18	0.95	A	0.459	1.959	0.678	0.85	1	10.925	1.27	203.98	C
		TA 0.79	B	0.306	2.28	0.618	0.85	1	7.163			
			C	0.643	1.784	0.781	0.85	1	16.695			
T25 218.75-212.50	0.18	0.66	A	0.411	2.042	0.656	0.85	1	9.202	1.16	185.39	C
			B	0.258	2.414	0.604	0.85	1	5.550			
			C	0.605	1.801	0.757	0.85	1	15.150			
T26 212.50-206.25	0.18	0.58	A	0.401	2.062	0.652	0.85	1	8.740	1.12	179.89	C
			B	0.248	2.444	0.601	0.85	1	5.086			
			C	0.597	1.806	0.752	0.85	1	14.785			
T27 206.25-200.00	0.18	0.58	A	0.401	2.062	0.652	0.85	1	8.740	1.11	178.34	C
			B	0.248	2.444	0.601	0.85	1	5.086			
			C	0.597	1.806	0.752	0.85	1	14.785			
T28 200.00-175.00	0.72	2.28	A	0.418	2.029	0.659	0.85	1	36.181	4.48	179.01	C
			B	0.247	2.446	0.601	0.85	1	19.508			
			C	0.611	1.797	0.761	0.85	1	61.022			
T29 175.00-168.75	0.18	0.61	A	0.418	2.029	0.659	0.85	1	9.089	1.07	171.45	C
			B	0.248	2.443	0.601	0.85	1	4.918			
			C	0.601	1.803	0.754	0.85	1	14.930			
T30 168.75-162.50	0.18	0.69	A	0.433	2.003	0.665	0.85	1	9.486	1.09	173.78	C
			B	0.262	2.401	0.605	0.85	1	5.231			
			C	0.612	1.797	0.761	0.85	1	15.348			
T31 162.50-156.25	0.18	0.69	A	0.428	2.011	0.664	0.85	1	9.549	1.07	171.42	C
			B	0.258	2.413	0.604	0.85	1	5.378			
			C	0.609	1.799	0.759	0.85	1	15.291			
T32 156.25-150.00	0.18	0.61	A	0.418	2.029	0.659	0.85	1	9.085	1.04	165.84	C
			B	0.248	2.443	0.601	0.85	1	4.914			
			C	0.601	1.804	0.754	0.85	1	14.925			
T33 150.00-125.00	0.74	2.45	A	0.437	1.995	0.668	0.85	1	38.466	4.11	164.49	C
			B	0.257	2.418	0.604	0.85	1	20.400			
			C	0.611	1.797	0.761	0.85	1	61.270			
T34 125.00-100.00	0.76	2.84	A	0.517	1.879	0.706	0.85	1	49.916	4.59	183.70	C
			B	0.338	2.199	0.628	0.85	1	30.800			
			C	0.684	1.776	0.809	0.85	1	73.341			
T35 100.00-93.75	0.19	1.03	A	0.527	1.867	0.712	0.85	1	12.925	1.12	179.39	C
		TA 0.79	B	0.348	2.176	0.632	0.85	1	8.152			
			C	0.692	1.776	0.814	0.85	1	18.688			
T36 93.75-87.50	0.19	0.71	A	0.46	1.958	0.678	0.85	1	10.439	0.98	156.70	C
			B	0.284	2.339	0.611	0.85	1	5.957			
			C	0.642	1.784	0.78	0.85	1	16.562			
T37 87.50-81.25	0.20	0.61	A	0.445	1.982	0.671	0.85	1	9.838	1.00	160.73	C
			B	0.303	2.288	0.617	0.85	1	6.159			
			C	0.664	1.778	0.795	0.85	1	17.395			
T38 81.25-75.00	0.20	0.61	A	0.445	1.982	0.671	0.85	1	9.838	0.98	157.24	C
			B	0.303	2.288	0.617	0.85	1	6.159			
			C	0.664	1.778	0.795	0.85	1	17.395			
T39 75.00-50.00	0.79	2.45	A	0.453	1.968	0.675	0.85	1	40.328	3.69	147.53	C
			B	0.312	2.266	0.619	0.85	1	25.429			
			C	0.664	1.778	0.795	0.85	1	69.578			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	46 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T40 50.00-25.00	0.80	2.47	A	0.476	1.934	0.685	0.85	1	43.120	3.20	128.14	C
			B	0.334	2.21	0.627	0.85	1	27.779			
			C	0.666	1.778	0.796	0.85	1	69.947			
T41 25.00-0.00	0.71	2.45	A	0.44	1.99	0.669	0.85	1	38.787	2.68	107.26	C
			B	0.315	2.256	0.621	0.85	1	25.778			
			C	0.602	1.803	0.755	0.85	1	59.889			
Sum Weight:	8.34	42.51								53.63		

Tower Forces - With Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1 368.75-362.50	0.00	0.88	A	0.307	2.278	0.618	1	1	7.894	0.67	106.54	C
			B	0.294	2.313	0.614	1	1	7.600			
			C	0.307	2.278	0.618	1	1	7.894			
T2 362.50-356.25	0.01	1.38	A	0.346	2.181	0.631	1	1	9.025	0.73	116.05	C
			B	0.313	2.262	0.62	1	1	8.270			
			C	0.346	2.181	0.631	1	1	9.025			
T3 356.25-350.00	0.02	1.38	A	0.346	2.181	0.631	1	1	9.025	0.75	120.12	C
			B	0.339	2.197	0.629	1	1	8.871			
			C	0.372	2.121	0.641	1	1	9.653			
T4 350.00-343.75	0.04	0.98	A	0.352	2.167	0.633	1	1	9.194	0.84	134.62	C
		TA 1.10	B	0.418	2.029	0.659	1	1	10.823			
			C	0.451	1.972	0.674	1	1	11.697			
T5 343.75-337.50	0.04	0.73	A	0.261	2.404	0.605	1	1	5.659	0.62	99.77	C
			B	0.327	2.226	0.625	1	1	7.185			
			C	0.36	2.148	0.636	1	1	7.999			
T6 337.50-331.25	0.04	0.63	A	0.251	2.433	0.602	1	1	5.045	0.58	92.60	C
			B	0.317	2.251	0.621	1	1	6.571			
			C	0.35	2.171	0.633	1	1	7.385			
T7 331.25-325.00	0.04	0.63	A	0.251	2.433	0.602	1	1	5.045	0.58	92.11	C
			B	0.317	2.251	0.621	1	1	6.571			
			C	0.35	2.171	0.633	1	1	7.385			
T8 325.00-318.75	0.05	0.74	A	0.309	2.272	0.619	1	1	6.400	0.61	97.29	B
			B	0.375	2.116	0.642	1	1	8.047			
			C	0.361	2.146	0.637	1	1	7.695			
T9 318.75-312.50	0.05	0.74	A	0.309	2.273	0.619	1	1	6.393	0.60	96.69	B
			B	0.375	2.117	0.642	1	1	8.040			
			C	0.361	2.146	0.636	1	1	7.688			
T10 312.50-306.25	0.05	0.74	A	0.309	2.273	0.619	1	1	6.393	0.60	96.14	B
			B	0.375	2.117	0.642	1	1	8.040			
			C	0.361	2.146	0.636	1	1	7.688			
T11 306.25-300.00	0.23	0.83	A	0.319	2.247	0.622	1	1	7.004	0.98	157.08	C
			B	0.384	2.096	0.645	1	1	8.651			
			C	0.553	1.841	0.726	1	1	15.193			
T12 300.00-293.75	0.27	1.06	A	0.404	2.057	0.653	1	1	10.474	1.22	194.86	C
		TA 1.10	B	0.469	1.943	0.682	1	1	12.229			
			C	0.658	1.779	0.791	1	1	19.614			
T13 293.75-287.50	0.27	0.83	A	0.319	2.247	0.622	1	1	7.004	1.06	170.26	C
			B	0.384	2.096	0.645	1	1	8.651			
			C	0.598	1.805	0.753	1	1	16.998			
T14 287.50-281.25	0.27	0.71	A	0.304	2.285	0.617	1	1	6.281	1.03	164.75	C
			B	0.37	2.127	0.64	1	1	7.917			
			C	0.588	1.812	0.746	1	1	16.487			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	47 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T15 281.25-275.00	0.27	0.71	A	0.304	2.285	0.617	1	1	6.281	1.02	163.71	C
			B	0.37	2.127	0.64	1	1	7.917			
			C	0.588	1.812	0.746	1	1	16.487			
T16 275.00-268.75	0.27	0.71	A	0.304	2.285	0.617	1	1	6.281	1.02	162.65	C
			B	0.37	2.127	0.64	1	1	7.917			
			C	0.588	1.812	0.746	1	1	16.487			
T17 268.75-262.50	0.27	0.71	A	0.304	2.285	0.617	1	1	6.281	1.01	161.57	C
			B	0.37	2.127	0.64	1	1	7.917			
			C	0.588	1.812	0.746	1	1	16.487			
T18 262.50-256.25	0.27	0.71	A	0.304	2.285	0.617	1	1	6.281	1.00	160.48	C
			B	0.37	2.127	0.64	1	1	7.917			
			C	0.588	1.812	0.746	1	1	16.487			
T19 256.25-250.00	0.27	0.74	A	0.309	2.273	0.619	1	1	6.393	1.00	160.21	C
			B	0.375	2.117	0.642	1	1	8.040			
			C	0.591	1.81	0.748	1	1	16.596			
T20 250.00-243.75	0.27	0.83	A	0.319	2.247	0.622	1	1	7.004	1.02	162.51	C
			B	0.384	2.096	0.645	1	1	8.651			
			C	0.598	1.805	0.753	1	1	16.998			
T21 243.75-237.50	0.46	0.83	A	0.546	1.847	0.722	1	1	15.220	1.60	256.32	C
			B	0.384	2.096	0.645	1	1	8.651			
			C	0.826	1.838	0.918	1	1	26.529			
T22 237.50-231.25	0.48	0.83	A	0.578	1.82	0.74	1	1	16.408	1.70	272.36	C
			B	0.384	2.096	0.645	1	1	8.651			
			C	0.857	1.869	0.945	1	1	27.920			
T23 231.25-225.00	0.48	0.83	A	0.578	1.82	0.74	1	1	16.408	1.69	270.27	C
			B	0.384	2.096	0.645	1	1	8.651			
			C	0.857	1.869	0.945	1	1	27.920			
T24 225.00-218.75	0.48	1.20 TA 1.10	A	0.639	1.785	0.778	1	1	19.057	1.84	294.23	C
			B	0.445	1.982	0.671	1	1	11.307			
			C	0.9	1.924	0.983	1	1	29.773			
T25 218.75-212.50	0.48	0.80	A	0.603	1.802	0.756	1	1	17.370	1.72	275.77	C
			B	0.409	2.046	0.655	1	1	9.560			
			C	0.875	1.891	0.96	1	1	28.625			
T26 212.50-206.25	0.48	0.71	A	0.593	1.808	0.75	1	1	16.804	1.68	268.95	C
			B	0.399	2.065	0.651	1	1	8.949			
			C	0.868	1.882	0.954	1	1	28.282			
T27 206.25-200.00	0.48	0.71	A	0.593	1.808	0.75	1	1	16.804	1.67	266.63	C
			B	0.399	2.065	0.651	1	1	8.949			
			C	0.868	1.882	0.954	1	1	28.282			
T28 200.00-175.00	1.98	2.73	A	0.629	1.789	0.772	1	1	71.202	6.97	278.81	C
			B	0.402	2.06	0.652	1	1	34.972			
			C	0.898	1.921	0.981	1	1	118.561			
T29 175.00-168.75	0.50	0.71	A	0.629	1.789	0.772	1	1	17.864	1.64	261.89	C
			B	0.403	2.058	0.653	1	1	8.801			
			C	0.88	1.897	0.965	1	1	28.909			
T30 168.75-162.50	0.50	0.80	A	0.643	1.784	0.781	1	1	18.351	1.66	264.95	C
			B	0.417	2.032	0.659	1	1	9.185			
			C	0.89	1.91	0.974	1	1	29.360			
T31 162.50-156.25	0.50	0.81	A	0.639	1.785	0.778	1	1	18.420	1.63	260.43	C
			B	0.413	2.04	0.657	1	1	9.405			
			C	0.887	1.906	0.971	1	1	29.240			
T32 156.25-150.00	0.50	0.71	A	0.629	1.789	0.772	1	1	17.854	1.58	253.23	C
			B	0.403	2.059	0.653	1	1	8.794			
			C	0.88	1.896	0.965	1	1	28.897			
T33 150.00-125.00	2.05	2.84	A	0.671	1.777	0.8	1	1	77.511	6.42	256.93	C
			B	0.425	2.016	0.662	1	1	37.696			
			C	0.899	1.923	0.982	1	1	119.267			
T34 125.00-100.00	2.09	3.54	A	0.757	1.79	0.862	1	1	92.986	7.08*	283.18	C
			B	0.523	1.872	0.709	1	1	53.273			
			C	0.987	2.073	1	1	1	132.119			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	48 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T35	0.52	1.28	A	0.767	1.795	0.87	1	1	23.778	1.70*	271.34	C
100.00-93.75		TA 1.10	B	0.532	1.861	0.715	1	1	13.912			
			C	0.994	2.088	1	1	1	33.264			
T36	0.52	0.83	A	0.701	1.776	0.82	1	1	20.689	1.59	255.16	C
93.75-87.50			B	0.473	1.938	0.684	1	1	11.177			
			C	0.954	2.01	1	1	1	31.914			
T37	0.54	0.71	A	0.686	1.776	0.81	1	1	19.947	1.63*	260.84	C
87.50-81.25			B	0.531	1.863	0.714	1	1	13.188			
			C	1	2.1	1	1	1	33.993			
T38	0.54	0.71	A	0.686	1.776	0.81	1	1	19.947	1.59*	255.17	C
81.25-75.00			B	0.531	1.863	0.714	1	1	13.188			
			C	1	2.1	1	1	1	33.993			
T39	2.20	2.84	A	0.702	1.776	0.821	1	1	82.230	5.99*	239.40	C
75.00-50.00			B	0.546	1.847	0.722	1	1	54.830			
			C	1	2.1	1	1	1	135.971			
T40	2.24	2.87	A	0.741	1.784	0.85	1	1	88.737	5.17*	206.89	C
50.00-25.00			B	0.586	1.814	0.745	1	1	60.488			
			C	1	2.1	1	1	1	136.205			
T41	1.98	2.84	A	0.689	1.776	0.812	1	1	79.524	4.49	179.58	C
25.00-0.00			B	0.552	1.842	0.725	1	1	55.308			
			C	0.921	1.955	1	1	1	123.267			
Sum Weight:	23.02	51.18			2A _R limit					77.98		

Tower Forces - With Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1	0.00	0.88	A	0.307	2.278	0.618	0.8	1	7.059	0.60	95.27	C
368.75-362.50			B	0.294	2.313	0.614	0.8	1	6.765			
			C	0.307	2.278	0.618	0.8	1	7.059			
T2	0.01	1.38	A	0.346	2.181	0.631	0.8	1	8.063	0.65	103.68	C
362.50-356.25			B	0.313	2.262	0.62	0.8	1	7.307			
			C	0.346	2.181	0.631	0.8	1	8.063			
T3	0.02	1.38	A	0.346	2.181	0.631	0.8	1	8.063	0.68	108.15	C
356.25-350.00			B	0.339	2.197	0.629	0.8	1	7.909			
			C	0.372	2.121	0.641	0.8	1	8.691			
T4	0.04	0.98	A	0.352	2.167	0.633	0.8	1	8.234	0.77	123.56	C
350.00-343.75		TA 1.10	B	0.418	2.029	0.659	0.8	1	9.862			
			C	0.451	1.972	0.674	0.8	1	10.736			
T5	0.04	0.73	A	0.261	2.404	0.605	0.8	1	5.461	0.61	97.31	C
343.75-337.50			B	0.327	2.226	0.625	0.8	1	6.987			
			C	0.36	2.148	0.636	0.8	1	7.801			
T6	0.04	0.63	A	0.251	2.433	0.602	0.8	1	5.045	0.58	92.60	C
337.50-331.25			B	0.317	2.251	0.621	0.8	1	6.571			
			C	0.35	2.171	0.633	0.8	1	7.385			
T7	0.04	0.63	A	0.251	2.433	0.602	0.8	1	5.045	0.58	92.11	C
331.25-325.00			B	0.317	2.251	0.621	0.8	1	6.571			
			C	0.35	2.171	0.633	0.8	1	7.385			
T8	0.05	0.74	A	0.309	2.272	0.619	0.8	1	6.400	0.61	97.29	B
325.00-318.75			B	0.375	2.116	0.642	0.8	1	8.047			
			C	0.361	2.146	0.637	0.8	1	7.695			
T9	0.05	0.74	A	0.309	2.273	0.619	0.8	1	6.393	0.60	96.69	B
318.75-312.50			B	0.375	2.117	0.642	0.8	1	8.040			

tnxTower

URS Corporation
 500 Enterprise Drive, Suite 3B
 Rocky Hill, CT 06067
 Phone: (860) 529-8882
 FAX: (860) 529-3991

Job	370' Guyed Lattice Tower	Page	49 of 98
Project	Montville, CT	Date	09:02:33 10/11/12
Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T10	0.05	0.74	C	0.361	2.146	0.636	0.8	1	7.688			
312.50-306.25			A	0.309	2.273	0.619	0.8	1	6.393	0.60	96.14	B
			C	0.375	2.117	0.642	0.8	1	8.040			
T11	0.23	0.83	B	0.361	2.146	0.636	0.8	1	7.688			
306.25-300.00			A	0.319	2.247	0.622	0.8	1	6.807	0.90	143.76	C
			B	0.384	2.096	0.645	0.8	1	8.454			
T12	0.27	1.06	C	0.553	1.841	0.726	0.8	1	13.905			
300.00-293.75		TA 1.10	A	0.404	2.057	0.653	0.8	1	9.520	1.09	174.03	C
			B	0.469	1.943	0.682	0.8	1	11.275			
T13	0.27	0.83	C	0.658	1.779	0.791	0.8	1	17.518			
293.75-287.50			A	0.319	2.247	0.622	0.8	1	6.807	0.97	154.63	C
			B	0.384	2.096	0.645	0.8	1	8.454			
T14	0.27	0.71	C	0.598	1.805	0.753	0.8	1	15.437			
287.50-281.25			A	0.304	2.285	0.617	0.8	1	6.281	0.94	150.54	C
			B	0.37	2.127	0.64	0.8	1	7.917			
T15	0.27	0.71	C	0.588	1.812	0.746	0.8	1	15.065			
281.25-275.00			A	0.304	2.285	0.617	0.8	1	6.281	0.93	149.59	C
			B	0.37	2.127	0.64	0.8	1	7.917			
T16	0.27	0.71	C	0.588	1.812	0.746	0.8	1	15.065			
275.00-268.75			A	0.304	2.285	0.617	0.8	1	6.281	0.93	148.62	C
			B	0.37	2.127	0.64	0.8	1	7.917			
T17	0.27	0.71	C	0.588	1.812	0.746	0.8	1	15.065			
268.75-262.50			A	0.304	2.285	0.617	0.8	1	6.281	0.92	147.64	C
			B	0.37	2.127	0.64	0.8	1	7.917			
T18	0.27	0.71	C	0.588	1.812	0.746	0.8	1	15.065			
262.50-256.25			A	0.304	2.285	0.617	0.8	1	6.281	0.92	146.64	C
			B	0.37	2.127	0.64	0.8	1	7.917			
T19	0.27	0.74	C	0.588	1.812	0.746	0.8	1	15.065			
256.25-250.00			A	0.309	2.273	0.619	0.8	1	6.393	0.92	146.48	C
			B	0.375	2.117	0.642	0.8	1	8.040			
T20	0.27	0.83	C	0.591	1.81	0.748	0.8	1	15.174			
250.00-243.75			A	0.319	2.247	0.622	0.8	1	6.807	0.92	147.58	C
			B	0.384	2.096	0.645	0.8	1	8.454			
T21	0.46	0.83	C	0.598	1.805	0.753	0.8	1	15.437			
243.75-237.50			A	0.546	1.847	0.722	0.8	1	13.772	1.43	229.14	C
			B	0.384	2.096	0.645	0.8	1	8.454			
T22	0.48	0.83	C	0.826	1.838	0.918	0.8	1	23.717			
237.50-231.25			A	0.578	1.82	0.74	0.8	1	14.789	1.52	243.26	C
			B	0.384	2.096	0.645	0.8	1	8.454			
T23	0.48	0.83	C	0.857	1.869	0.945	0.8	1	24.937			
231.25-225.00			A	0.578	1.82	0.74	0.8	1	14.789	1.51	241.39	C
			B	0.384	2.096	0.645	0.8	1	8.454			
T24	0.48	1.20	C	0.857	1.869	0.945	0.8	1	24.937			
225.00-218.75		TA 1.10	A	0.639	1.785	0.778	0.8	1	16.806	1.63	260.33	C
			B	0.445	1.982	0.671	0.8	1	10.478			
T25	0.48	0.80	C	0.9	1.924	0.983	0.8	1	26.342			
218.75-212.50			A	0.603	1.802	0.756	0.8	1	15.592	1.54	245.94	C
			B	0.409	2.046	0.655	0.8	1	9.204			
T26	0.48	0.71	C	0.875	1.891	0.96	0.8	1	25.529			
212.50-206.25			A	0.593	1.808	0.75	0.8	1	15.224	1.51	240.84	C
			B	0.399	2.065	0.651	0.8	1	8.791			
T27	0.48	0.71	C	0.868	1.882	0.954	0.8	1	25.326			
206.25-200.00			A	0.593	1.808	0.75	0.8	1	15.224	1.49	238.76	C
			B	0.399	2.065	0.651	0.8	1	8.791			
T28	1.98	2.73	C	0.868	1.882	0.954	0.8	1	25.326			
200.00-175.00			A	0.629	1.789	0.772	0.8	1	65.514	6.30	252.06	C
			B	0.402	2.06	0.652	0.8	1	34.972			
T29	0.50	0.71	C	0.898	1.921	0.981	0.8	1	107.186			
175.00-168.75			A	0.629	1.789	0.772	0.8	1	16.442	1.48	236.13	C
			B	0.403	2.058	0.653	0.8	1	8.801			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	50 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T30	0.50	0.80	C	0.88	1.897	0.965	0.8	1	26.066			
168.75-162.50			A	0.643	1.784	0.781	0.8	1	16.930	1.50	239.28	C
			B	0.417	2.032	0.659	0.8	1	9.185			
			C	0.89	1.91	0.974	0.8	1	26.516			
T31	0.50	0.81	A	0.639	1.785	0.778	0.8	1	16.801	1.46	233.87	C
162.50-156.25			B	0.413	2.04	0.657	0.8	1	9.208			
			C	0.887	1.906	0.971	0.8	1	26.257			
T32	0.50	0.71	A	0.629	1.789	0.772	0.8	1	16.433	1.43	228.31	C
156.25-150.00			B	0.403	2.059	0.653	0.8	1	8.794			
			C	0.88	1.896	0.965	0.8	1	26.053			
T33	2.05	2.84	A	0.671	1.777	0.8	0.8	1	71.823	5.81	232.43	C
150.00-125.00			B	0.425	2.016	0.662	0.8	1	37.696			
			C	0.899	1.923	0.982	0.8	1	107.892			
T34	2.09	3.54	A	0.757	1.79	0.862	0.8	1	84.775	6.52	260.90	C
125.00-100.00			B	0.523	1.872	0.709	0.8	1	50.750			
			C	0.987	2.073	1	0.8	1	118.960			
T35	0.52	1.28	A	0.767	1.795	0.87	0.8	1	21.529	1.58	252.51	C
100.00-93.75		TA 1.10	B	0.532	1.861	0.715	0.8	1	13.084			
			C	0.994	2.088	1	0.8	1	29.835			
T36	0.52	0.83	A	0.701	1.776	0.82	0.8	1	19.070	1.44	231.08	C
93.75-87.50			B	0.473	1.938	0.684	0.8	1	10.951			
			C	0.954	2.01	1	0.8	1	28.902			
T37	0.54	0.71	A	0.686	1.776	0.81	0.8	1	18.526	1.57	251.97	C
87.50-81.25			B	0.531	1.863	0.714	0.8	1	12.826			
			C	1	2.1	1	0.8	1	30.786			
T38	0.54	0.71	A	0.686	1.776	0.81	0.8	1	18.526	1.54	246.49	C
81.25-75.00			B	0.531	1.863	0.714	0.8	1	12.826			
			C	1	2.1	1	0.8	1	30.786			
T39	2.20	2.84	A	0.702	1.776	0.821	0.8	1	76.542	5.78	231.26	C
75.00-50.00			B	0.546	1.847	0.722	0.8	1	53.380			
			C	1	2.1	1	0.8	1	123.146			
T40	2.24	2.87	A	0.741	1.784	0.85	0.8	1	82.852	5.00	200.01	C
50.00-25.00			B	0.586	1.814	0.745	0.8	1	58.841			
			C	1	2.1	1	0.8	1	123.240			
T41	1.98	2.84	A	0.689	1.776	0.812	0.8	1	74.519	4.08	163.14	C
25.00-0.00			B	0.552	1.842	0.725	0.8	1	54.032			
			C	0.921	1.955	1	0.8	1	111.981			
Sum Weight:	23.02	51.18								71.82		

Tower Forces - With Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1	0.00	0.88	A	0.307	2.278	0.618	0.85	1	7.268	0.61	98.09	C
368.75-362.50			B	0.294	2.313	0.614	0.85	1	6.974			
			C	0.307	2.278	0.618	0.85	1	7.268			
T2	0.01	1.38	A	0.346	2.181	0.631	0.85	1	8.303	0.67	106.77	C
362.50-356.25			B	0.313	2.262	0.62	0.85	1	7.548			
			C	0.346	2.181	0.631	0.85	1	8.303			
T3	0.02	1.38	A	0.346	2.181	0.631	0.85	1	8.303	0.69	111.14	C
356.25-350.00			B	0.339	2.197	0.629	0.85	1	8.150			
			C	0.372	2.121	0.641	0.85	1	8.931			
T4	0.04	0.98	A	0.352	2.167	0.633	0.85	1	8.474	0.79	126.33	C
350.00-343.75		TA 1.10	B	0.418	2.029	0.659	0.85	1	10.102			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	51 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T5	0.04	0.73	C	0.451	1.972	0.674	0.85	1	10.976			
343.75-337.50			A	0.261	2.404	0.605	0.85	1	5.511	0.61	97.92	C
			B	0.327	2.226	0.625	0.85	1	7.037			
			C	0.36	2.148	0.636	0.85	1	7.850			
T6	0.04	0.63	A	0.251	2.433	0.602	0.85	1	5.045	0.58	92.60	C
337.50-331.25			B	0.317	2.251	0.621	0.85	1	6.571			
			C	0.35	2.171	0.633	0.85	1	7.385			
T7	0.04	0.63	A	0.251	2.433	0.602	0.85	1	5.045	0.58	92.11	C
331.25-325.00			B	0.317	2.251	0.621	0.85	1	6.571			
			C	0.35	2.171	0.633	0.85	1	7.385			
T8	0.05	0.74	A	0.309	2.272	0.619	0.85	1	6.400	0.61	97.29	B
325.00-318.75			B	0.375	2.116	0.642	0.85	1	8.047			
			C	0.361	2.146	0.637	0.85	1	7.695			
T9	0.05	0.74	A	0.309	2.273	0.619	0.85	1	6.393	0.60	96.69	B
318.75-312.50			B	0.375	2.117	0.642	0.85	1	8.040			
			C	0.361	2.146	0.636	0.85	1	7.688			
T10	0.05	0.74	A	0.309	2.273	0.619	0.85	1	6.393	0.60	96.14	B
312.50-306.25			B	0.375	2.117	0.642	0.85	1	8.040			
			C	0.361	2.146	0.636	0.85	1	7.688			
T11	0.23	0.83	A	0.319	2.247	0.622	0.85	1	6.857	0.92	147.09	C
306.25-300.00			B	0.384	2.096	0.645	0.85	1	8.503			
			C	0.553	1.841	0.726	0.85	1	14.227			
T12	0.27	1.06	A	0.404	2.057	0.653	0.85	1	9.759	1.12	179.24	C
300.00-293.75		TA 1.10	B	0.469	1.943	0.682	0.85	1	11.514			
			C	0.658	1.779	0.791	0.85	1	18.042			
T13	0.27	0.83	A	0.319	2.247	0.622	0.85	1	6.857	0.99	158.54	C
293.75-287.50			B	0.384	2.096	0.645	0.85	1	8.503			
			C	0.598	1.805	0.753	0.85	1	15.827			
T14	0.27	0.71	A	0.304	2.285	0.617	0.85	1	6.281	0.96	154.09	C
287.50-281.25			B	0.37	2.127	0.64	0.85	1	7.917			
			C	0.588	1.812	0.746	0.85	1	15.421			
T15	0.27	0.71	A	0.304	2.285	0.617	0.85	1	6.281	0.96	153.12	C
281.25-275.00			B	0.37	2.127	0.64	0.85	1	7.917			
			C	0.588	1.812	0.746	0.85	1	15.421			
T16	0.27	0.71	A	0.304	2.285	0.617	0.85	1	6.281	0.95	152.13	C
275.00-268.75			B	0.37	2.127	0.64	0.85	1	7.917			
			C	0.588	1.812	0.746	0.85	1	15.421			
T17	0.27	0.71	A	0.304	2.285	0.617	0.85	1	6.281	0.94	151.12	C
268.75-262.50			B	0.37	2.127	0.64	0.85	1	7.917			
			C	0.588	1.812	0.746	0.85	1	15.421			
T18	0.27	0.71	A	0.304	2.285	0.617	0.85	1	6.281	0.94	150.10	C
262.50-256.25			B	0.37	2.127	0.64	0.85	1	7.917			
			C	0.588	1.812	0.746	0.85	1	15.421			
T19	0.27	0.74	A	0.309	2.273	0.619	0.85	1	6.393	0.94	149.91	C
256.25-250.00			B	0.375	2.117	0.642	0.85	1	8.040			
			C	0.591	1.81	0.748	0.85	1	15.529			
T20	0.27	0.83	A	0.319	2.247	0.622	0.85	1	6.857	0.95	151.32	C
250.00-243.75			B	0.384	2.096	0.645	0.85	1	8.503			
			C	0.598	1.805	0.753	0.85	1	15.827			
T21	0.46	0.83	A	0.546	1.847	0.722	0.85	1	14.134	1.47	235.94	C
243.75-237.50			B	0.384	2.096	0.645	0.85	1	8.503			
			C	0.826	1.838	0.918	0.85	1	24.420			
T22	0.48	0.83	A	0.578	1.82	0.74	0.85	1	15.194	1.57	250.54	C
237.50-231.25			B	0.384	2.096	0.645	0.85	1	8.503			
			C	0.857	1.869	0.945	0.85	1	25.683			
T23	0.48	0.83	A	0.578	1.82	0.74	0.85	1	15.194	1.55	248.61	C
231.25-225.00			B	0.384	2.096	0.645	0.85	1	8.503			
			C	0.857	1.869	0.945	0.85	1	25.683			
T24	0.48	1.20	A	0.639	1.785	0.778	0.85	1	17.369	1.68	268.80	C
225.00-218.75		TA 1.10	B	0.445	1.982	0.671	0.85	1	10.685			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	52 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T25	0.48	0.80	C	0.9	1.924	0.983	0.85	1	27.200			
218.75-212.50			A	0.603	1.802	0.756	0.85	1	16.036	1.58	253.40	C
			B	0.409	2.046	0.655	0.85	1	9.293			
			C	0.875	1.891	0.96	0.85	1	26.303			
T26	0.48	0.71	A	0.593	1.808	0.75	0.85	1	15.619	1.55	247.86	C
212.50-206.25			B	0.399	2.065	0.651	0.85	1	8.831			
			C	0.868	1.882	0.954	0.85	1	26.065			
T27	0.48	0.71	A	0.593	1.808	0.75	0.85	1	15.619	1.54	245.73	C
206.25-200.00			B	0.399	2.065	0.651	0.85	1	8.831			
			C	0.868	1.882	0.954	0.85	1	26.065			
T28	1.98	2.73	A	0.629	1.789	0.772	0.85	1	66.936	6.47	258.75	C
200.00-175.00			B	0.402	2.06	0.652	0.85	1	34.972			
			C	0.898	1.921	0.981	0.85	1	110.030			
T29	0.50	0.71	A	0.629	1.789	0.772	0.85	1	16.797	1.52	242.57	C
175.00-168.75			B	0.403	2.058	0.653	0.85	1	8.801			
			C	0.88	1.897	0.965	0.85	1	26.777			
T30	0.50	0.80	A	0.643	1.784	0.781	0.85	1	17.285	1.54	245.70	C
168.75-162.50			B	0.417	2.032	0.659	0.85	1	9.185			
			C	0.89	1.91	0.974	0.85	1	27.227			
T31	0.50	0.81	A	0.639	1.785	0.778	0.85	1	17.206	1.50	240.51	C
162.50-156.25			B	0.413	2.04	0.657	0.85	1	9.257			
			C	0.887	1.906	0.971	0.85	1	27.003			
T32	0.50	0.71	A	0.629	1.789	0.772	0.85	1	16.788	1.47	234.54	C
156.25-150.00			B	0.403	2.059	0.653	0.85	1	8.794			
			C	0.88	1.896	0.965	0.85	1	26.764			
T33	2.05	2.84	A	0.671	1.777	0.8	0.85	1	73.245	5.96	238.55	C
150.00-125.00			B	0.425	2.016	0.662	0.85	1	37.696			
			C	0.899	1.923	0.982	0.85	1	110.735			
T34	2.09	3.54	A	0.757	1.79	0.862	0.85	1	86.828	6.70	268.12	C
125.00-100.00			B	0.523	1.872	0.709	0.85	1	51.381			
			C	0.987	2.073	1	0.85	1	122.250			
T35	0.52	1.28	A	0.767	1.795	0.87	0.85	1	22.091	1.62	259.76	C
100.00-93.75		TA 1.10	B	0.532	1.861	0.715	0.85	1	13.291			
			C	0.994	2.088	1	0.85	1	30.692			
T36	0.52	0.83	A	0.701	1.776	0.82	0.85	1	19.475	1.48	237.10	C
93.75-87.50			B	0.473	1.938	0.684	0.85	1	11.007			
			C	0.954	2.01	1	0.85	1	29.655			
T37	0.54	0.71	A	0.686	1.776	0.81	0.85	1	18.881	1.62	258.53	C
87.50-81.25			B	0.531	1.863	0.714	0.85	1	12.917			
			C	1	2.1	1	0.85	1	31.588			
T38	0.54	0.71	A	0.686	1.776	0.81	0.85	1	18.881	1.58	252.91	C
81.25-75.00			B	0.531	1.863	0.714	0.85	1	12.917			
			C	1	2.1	1	0.85	1	31.588			
T39	2.20	2.84	A	0.702	1.776	0.821	0.85	1	77.964	5.93	237.29	C
75.00-50.00			B	0.546	1.847	0.722	0.85	1	53.742			
			C	1	2.1	1	0.85	1	126.352			
T40	2.24	2.87	A	0.741	1.784	0.85	0.85	1	84.323	5.13	205.27	C
50.00-25.00			B	0.586	1.814	0.745	0.85	1	59.253			
			C	1	2.1	1	0.85	1	126.481			
T41	1.98	2.84	A	0.689	1.776	0.812	0.85	1	75.770	4.18	167.25	C
25.00-0.00			B	0.552	1.842	0.725	0.85	1	54.351			
			C	0.921	1.955	1	0.85	1	114.802			
Sum Weight:	23.02	51.18								73.66		

Tower Forces - Service - Wind Normal To Face

tnxTower

URS Corporation
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 Rocky Hill, CT 06067
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Project

Montville, CT

Date

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Client

Verizon Wireless

Designed by

Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1 368.75-362.50	0.00	0.68	A	0.223	2.523	0.595	1	1	6.019	0.21	33.23	C
			B	0.215	2.545	0.594	1	1	5.877			
			C	0.223	2.523	0.595	1	1	6.019			
T2 362.50-356.25	0.00	1.09	A	0.253	2.43	0.603	1	1	6.887	0.23	36.44	C
			B	0.235	2.484	0.598	1	1	6.526			
			C	0.253	2.43	0.603	1	1	6.887			
T3 356.25-350.00	0.01	1.09	A	0.253	2.43	0.603	1	1	6.887	0.23	37.16	C
			B	0.249	2.44	0.602	1	1	6.814			
			C	0.267	2.388	0.606	1	1	7.180			
T4 350.00-343.75	0.01	0.77 TA 0.79	A	0.259	2.41	0.604	1	1	7.042	0.25	39.90	C
			B	0.295	2.311	0.614	1	1	7.789			
			C	0.312	2.264	0.62	1	1	8.174			
T5 343.75-337.50	0.01	0.61	A	0.167	2.709	0.584	1	1	3.616	0.16	25.48	C
			B	0.202	2.588	0.591	1	1	4.329			
			C	0.22	2.531	0.595	1	1	4.694			
T6 337.50-331.25	0.01	0.54	A	0.157	2.746	0.583	1	1	3.002	0.14	22.31	C
			B	0.192	2.622	0.589	1	1	3.715			
			C	0.21	2.563	0.592	1	1	4.080			
T7 331.25-325.00	0.01	0.54	A	0.157	2.746	0.583	1	1	3.002	0.14	22.19	C
			B	0.192	2.622	0.589	1	1	3.715			
			C	0.21	2.563	0.592	1	1	4.080			
T8 325.00-318.75	0.02	0.64	A	0.2	2.595	0.59	1	1	3.897	0.15	24.30	B
			B	0.235	2.482	0.598	1	1	4.641			
			C	0.222	2.525	0.595	1	1	4.346			
T9 318.75-312.50	0.02	0.64	A	0.2	2.595	0.59	1	1	3.894	0.15	24.16	B
			B	0.235	2.482	0.598	1	1	4.637			
			C	0.222	2.526	0.595	1	1	4.343			
T10 312.50-306.25	0.02	0.64	A	0.2	2.595	0.59	1	1	3.894	0.15	24.02	B
			B	0.235	2.482	0.598	1	1	4.637			
			C	0.222	2.526	0.595	1	1	4.343			
T11 306.25-300.00	0.08	0.71	A	0.21	2.562	0.593	1	1	4.506	0.22	35.86	C
			B	0.245	2.452	0.601	1	1	5.249			
			C	0.372	2.122	0.641	1	1	8.146			
T12 300.00-293.75	0.10	0.85 TA 0.79	A	0.296	2.306	0.615	1	1	7.841	0.30	47.53	C
			B	0.332	2.216	0.626	1	1	8.621			
			C	0.475	1.934	0.685	1	1	11.916			
T13 293.75-287.50	0.10	0.71	A	0.21	2.562	0.593	1	1	4.506	0.24	38.02	C
			B	0.245	2.452	0.601	1	1	5.249			
			C	0.407	2.05	0.655	1	1	9.048			
T14 287.50-281.25	0.10	0.61	A	0.195	2.611	0.589	1	1	3.795	0.22	35.74	C
			B	0.231	2.497	0.597	1	1	4.534			
			C	0.395	2.073	0.65	1	1	8.464			
T15 281.25-275.00	0.10	0.61	A	0.195	2.611	0.589	1	1	3.795	0.22	35.51	C
			B	0.231	2.497	0.597	1	1	4.534			
			C	0.395	2.073	0.65	1	1	8.464			
T16 275.00-268.75	0.10	0.61	A	0.195	2.611	0.589	1	1	3.795	0.22	35.28	C
			B	0.231	2.497	0.597	1	1	4.534			
			C	0.395	2.073	0.65	1	1	8.464			
T17 268.75-262.50	0.10	0.61	A	0.195	2.611	0.589	1	1	3.795	0.22	35.05	C
			B	0.231	2.497	0.597	1	1	4.534			
			C	0.395	2.073	0.65	1	1	8.464			
T18 262.50-256.25	0.10	0.61	A	0.195	2.611	0.589	1	1	3.795	0.22	34.81	C
			B	0.231	2.497	0.597	1	1	4.534			
			C	0.395	2.073	0.65	1	1	8.464			
T19 256.25-250.00	0.10	0.64	A	0.2	2.595	0.59	1	1	3.894	0.22	34.86	C
			B	0.235	2.482	0.598	1	1	4.637			
			C	0.399	2.066	0.651	1	1	8.565			
T20 250.00-243.75	0.10	0.71	A	0.21	2.562	0.593	1	1	4.506	0.23	36.29	C
			B	0.245	2.452	0.601	1	1	5.249			
			C	0.407	2.05	0.655	1	1	9.048			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	54 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T21 243.75-237.50	0.17	0.71	A	0.376	2.114	0.642	1	1	8.294	0.31	49.95	C
			B	0.245	2.452	0.601	1	1	5.249			
			C	0.572	1.824	0.737	1	1	14.105			
T22 237.50-231.25	0.18	0.71	A	0.398	2.068	0.651	1	1	8.879	0.32	51.92	C
			B	0.245	2.452	0.601	1	1	5.249			
			C	0.595	1.807	0.751	1	1	14.905			
T23 231.25-225.00	0.18	0.71	A	0.398	2.068	0.651	1	1	8.879	0.32	51.52	C
			B	0.245	2.452	0.601	1	1	5.249			
			C	0.595	1.807	0.751	1	1	14.905			
T24 225.00-218.75	0.18	0.95 TA 0.79	A	0.459	1.959	0.678	1	1	11.547	0.36	58.17	C
			B	0.306	2.28	0.618	1	1	7.785			
			C	0.643	1.784	0.781	1	1	17.187			
T25 218.75-212.50	0.18	0.66	A	0.411	2.042	0.656	1	1	9.469	0.33	52.07	C
			B	0.258	2.414	0.604	1	1	5.817			
			C	0.605	1.801	0.757	1	1	15.362			
T26 212.50-206.25	0.18	0.58	A	0.401	2.062	0.652	1	1	8.858	0.31	50.15	C
			B	0.248	2.444	0.601	1	1	5.205			
			C	0.597	1.806	0.752	1	1	14.879			
T27 206.25-200.00	0.18	0.58	A	0.401	2.062	0.652	1	1	8.858	0.31	49.72	C
			B	0.248	2.444	0.601	1	1	5.205			
			C	0.597	1.806	0.752	1	1	14.879			
T28 200.00-175.00	0.72	2.28	A	0.418	2.029	0.659	1	1	36.181	1.24	49.59	C
			B	0.247	2.446	0.601	1	1	19.508			
			C	0.611	1.797	0.761	1	1	61.022			
T29 175.00-168.75	0.18	0.61	A	0.418	2.029	0.659	1	1	9.089	0.30	47.49	C
			B	0.248	2.443	0.601	1	1	4.918			
			C	0.601	1.803	0.754	1	1	14.930			
T30 168.75-162.50	0.18	0.69	A	0.433	2.003	0.665	1	1	9.486	0.30	48.14	C
			B	0.262	2.401	0.605	1	1	5.231			
			C	0.612	1.797	0.761	1	1	15.348			
T31 162.50-156.25	0.18	0.69	A	0.428	2.011	0.664	1	1	9.696	0.30	47.85	C
			B	0.258	2.413	0.604	1	1	5.526			
			C	0.609	1.799	0.759	1	1	15.408			
T32 156.25-150.00	0.18	0.61	A	0.418	2.029	0.659	1	1	9.085	0.29	45.94	C
			B	0.248	2.443	0.601	1	1	4.914			
			C	0.601	1.804	0.754	1	1	14.925			
T33 150.00-125.00	0.74	2.45	A	0.437	1.995	0.668	1	1	38.466	1.14	45.56	C
			B	0.257	2.418	0.604	1	1	20.400			
			C	0.611	1.797	0.761	1	1	61.270			
T34 125.00-100.00	0.76	2.84	A	0.517	1.879	0.706	1	1	51.808	1.30	51.92	C
			B	0.338	2.199	0.628	1	1	32.693			
			C	0.684	1.776	0.809	1	1	74.838			
T35 100.00-93.75	0.19	1.03 TA 0.79	A	0.527	1.867	0.712	1	1	13.546	0.32	51.00	C
			B	0.348	2.176	0.632	1	1	8.773			
			C	0.692	1.776	0.814	1	1	19.179			
T36 93.75-87.50	0.19	0.71	A	0.46	1.958	0.678	1	1	10.587	0.27	43.71	C
			B	0.284	2.339	0.611	1	1	6.105			
			C	0.642	1.784	0.78	1	1	16.679			
T37 87.50-81.25	0.20	0.61	A	0.445	1.982	0.671	1	1	9.838	0.28	44.52	C
			B	0.303	2.288	0.617	1	1	6.159			
			C	0.664	1.778	0.795	1	1	17.395			
T38 81.25-75.00	0.20	0.61	A	0.445	1.982	0.671	1	1	9.838	0.27	43.56	C
			B	0.303	2.288	0.617	1	1	6.159			
			C	0.664	1.778	0.795	1	1	17.395			
T39 75.00-50.00	0.79	2.45	A	0.453	1.968	0.675	1	1	40.328	1.02	40.87	C
			B	0.312	2.266	0.619	1	1	25.429			
			C	0.664	1.778	0.795	1	1	69.578			
T40 50.00-25.00	0.80	2.47	A	0.476	1.934	0.685	1	1	43.268	0.89	35.56	C
			B	0.334	2.21	0.627	1	1	27.927			
			C	0.666	1.778	0.796	1	1	70.063			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	55 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T41	0.71	2.45	A	0.44	1.99	0.669	1	1	38.787	0.74	29.71	C
25.00-0.00			B	0.315	2.256	0.621	1	1	25.778			
			C	0.602	1.803	0.755	1	1	59.889			
Sum Weight:	8.34	42.51								15.04		

Tower Forces - Service - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1	0.00	0.68	A	0.223	2.523	0.595	0.8	1	5.184	0.18	28.62	C
368.75-362.50			B	0.215	2.545	0.594	0.8	1	5.041			
			C	0.223	2.523	0.595	0.8	1	5.184			
T2	0.00	1.09	A	0.253	2.43	0.603	0.8	1	5.924	0.20	31.35	C
362.50-356.25			B	0.235	2.484	0.598	0.8	1	5.563			
			C	0.253	2.43	0.603	0.8	1	5.924			
T3	0.01	1.09	A	0.253	2.43	0.603	0.8	1	5.924	0.20	32.18	C
356.25-350.00			B	0.249	2.44	0.602	0.8	1	5.852			
			C	0.267	2.388	0.606	0.8	1	6.218			
T4	0.01	0.77	A	0.259	2.41	0.604	0.8	1	6.081	0.22	35.21	C
350.00-343.75		TA 0.79	B	0.295	2.311	0.614	0.8	1	6.828			
			C	0.312	2.264	0.62	0.8	1	7.213			
T5	0.01	0.61	A	0.167	2.709	0.584	0.8	1	3.418	0.15	24.40	C
343.75-337.50			B	0.202	2.588	0.591	0.8	1	4.131			
			C	0.22	2.531	0.595	0.8	1	4.496			
T6	0.01	0.54	A	0.157	2.746	0.583	0.8	1	3.002	0.14	22.31	C
337.50-331.25			B	0.192	2.622	0.589	0.8	1	3.715			
			C	0.21	2.563	0.592	0.8	1	4.080			
T7	0.01	0.54	A	0.157	2.746	0.583	0.8	1	3.002	0.14	22.19	C
331.25-325.00			B	0.192	2.622	0.589	0.8	1	3.715			
			C	0.21	2.563	0.592	0.8	1	4.080			
T8	0.02	0.64	A	0.2	2.595	0.59	0.8	1	3.897	0.15	24.30	B
325.00-318.75			B	0.235	2.482	0.598	0.8	1	4.641			
			C	0.222	2.525	0.595	0.8	1	4.346			
T9	0.02	0.64	A	0.2	2.595	0.59	0.8	1	3.894	0.15	24.16	B
318.75-312.50			B	0.235	2.482	0.598	0.8	1	4.637			
			C	0.222	2.526	0.595	0.8	1	4.343			
T10	0.02	0.64	A	0.2	2.595	0.59	0.8	1	3.894	0.15	24.02	B
312.50-306.25			B	0.235	2.482	0.598	0.8	1	4.637			
			C	0.222	2.526	0.595	0.8	1	4.343			
T11	0.08	0.71	A	0.21	2.562	0.593	0.8	1	4.309	0.22	35.14	C
306.25-300.00			B	0.245	2.452	0.601	0.8	1	5.052			
			C	0.372	2.122	0.641	0.8	1	7.982			
T12	0.10	0.85	A	0.296	2.306	0.615	0.8	1	6.887	0.28	44.52	C
300.00-293.75		TA 0.79	B	0.332	2.216	0.626	0.8	1	7.667			
			C	0.475	1.934	0.685	0.8	1	11.161			
T13	0.10	0.71	A	0.21	2.562	0.593	0.8	1	4.309	0.23	37.36	C
293.75-287.50			B	0.245	2.452	0.601	0.8	1	5.052			
			C	0.407	2.05	0.655	0.8	1	8.892			
T14	0.10	0.61	A	0.195	2.611	0.589	0.8	1	3.795	0.22	35.74	C
287.50-281.25			B	0.231	2.497	0.597	0.8	1	4.534			
			C	0.395	2.073	0.65	0.8	1	8.464			
T15	0.10	0.61	A	0.195	2.611	0.589	0.8	1	3.795	0.22	35.51	C
281.25-275.00			B	0.231	2.497	0.597	0.8	1	4.534			
			C	0.395	2.073	0.65	0.8	1	8.464			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	56 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K	e						ft ²	K	plf	
T16 275.00-268.75	0.10	0.61	A	0.195	2.611	0.589	0.8	1	3.795	0.22	35.28	C
			B	0.231	2.497	0.597	0.8	1	4.534			
			C	0.395	2.073	0.65	0.8	1	8.464			
T17 268.75-262.50	0.10	0.61	A	0.195	2.611	0.589	0.8	1	3.795	0.22	35.05	C
			B	0.231	2.497	0.597	0.8	1	4.534			
			C	0.395	2.073	0.65	0.8	1	8.464			
T18 262.50-256.25	0.10	0.61	A	0.195	2.611	0.589	0.8	1	3.795	0.22	34.81	C
			B	0.231	2.497	0.597	0.8	1	4.534			
			C	0.395	2.073	0.65	0.8	1	8.464			
T19 256.25-250.00	0.10	0.64	A	0.2	2.595	0.59	0.8	1	3.894	0.22	34.86	C
			B	0.235	2.482	0.598	0.8	1	4.637			
			C	0.399	2.066	0.651	0.8	1	8.565			
T20 250.00-243.75	0.10	0.71	A	0.21	2.562	0.593	0.8	1	4.309	0.22	35.66	C
			B	0.245	2.452	0.601	0.8	1	5.052			
			C	0.407	2.05	0.655	0.8	1	8.892			
T21 243.75-237.50	0.17	0.71	A	0.376	2.114	0.642	0.8	1	8.097	0.31	49.40	C
			B	0.245	2.452	0.601	0.8	1	5.052			
			C	0.572	1.824	0.737	0.8	1	13.949			
T22 237.50-231.25	0.18	0.71	A	0.398	2.068	0.651	0.8	1	8.682	0.32	51.37	C
			B	0.245	2.452	0.601	0.8	1	5.052			
			C	0.595	1.807	0.751	0.8	1	14.749			
T23 231.25-225.00	0.18	0.71	A	0.398	2.068	0.651	0.8	1	8.682	0.32	50.98	C
			B	0.245	2.452	0.601	0.8	1	5.052			
			C	0.595	1.807	0.751	0.8	1	14.749			
T24 225.00-218.75	0.18	0.95 TA 0.79	A	0.459	1.959	0.678	0.8	1	10.717	0.35	55.95	C
			B	0.306	2.28	0.618	0.8	1	6.956			
			C	0.643	1.784	0.781	0.8	1	16.531			
T25 218.75-212.50	0.18	0.66	A	0.411	2.042	0.656	0.8	1	9.112	0.32	51.11	C
			B	0.258	2.414	0.604	0.8	1	5.461			
			C	0.605	1.801	0.757	0.8	1	15.080			
T26 212.50-206.25	0.18	0.58	A	0.401	2.062	0.652	0.8	1	8.700	0.31	49.73	C
			B	0.248	2.444	0.601	0.8	1	5.047			
			C	0.597	1.806	0.752	0.8	1	14.754			
T27 206.25-200.00	0.18	0.58	A	0.401	2.062	0.652	0.8	1	8.700	0.31	49.30	C
			B	0.248	2.444	0.601	0.8	1	5.047			
			C	0.597	1.806	0.752	0.8	1	14.754			
T28 200.00-175.00	0.72	2.28	A	0.418	2.029	0.659	0.8	1	36.181	1.24	49.59	C
			B	0.247	2.446	0.601	0.8	1	19.508			
			C	0.611	1.797	0.761	0.8	1	61.022			
T29 175.00-168.75	0.18	0.61	A	0.418	2.029	0.659	0.8	1	9.089	0.30	47.49	C
			B	0.248	2.443	0.601	0.8	1	4.918			
			C	0.601	1.803	0.754	0.8	1	14.930			
T30 168.75-162.50	0.18	0.69	A	0.433	2.003	0.665	0.8	1	9.486	0.30	48.14	C
			B	0.262	2.401	0.605	0.8	1	5.231			
			C	0.612	1.797	0.761	0.8	1	15.348			
T31 162.50-156.25	0.18	0.69	A	0.428	2.011	0.664	0.8	1	9.499	0.30	47.36	C
			B	0.258	2.413	0.604	0.8	1	5.329			
			C	0.609	1.799	0.759	0.8	1	15.252			
T32 156.25-150.00	0.18	0.61	A	0.418	2.029	0.659	0.8	1	9.085	0.29	45.94	C
			B	0.248	2.443	0.601	0.8	1	4.914			
			C	0.601	1.804	0.754	0.8	1	14.925			
T33 150.00-125.00	0.74	2.45	A	0.437	1.995	0.668	0.8	1	38.466	1.14	45.56	C
			B	0.257	2.418	0.604	0.8	1	20.400			
			C	0.611	1.797	0.761	0.8	1	61.270			
T34 125.00-100.00	0.76	2.84	A	0.517	1.879	0.706	0.8	1	49.285	1.26	50.54	C
			B	0.338	2.199	0.628	0.8	1	30.169			
			C	0.684	1.776	0.809	0.8	1	72.843			
T35 100.00-93.75	0.19	1.03 TA 0.79	A	0.527	1.867	0.712	0.8	1	12.718	0.31	49.26	C
			B	0.348	2.176	0.632	0.8	1	7.945			
			C	0.692	1.776	0.814	0.8	1	18.524			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	57 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T36	0.19	0.71	A	0.46	1.958	0.678	0.8	1	10.390	0.27	43.30	C
93.75-87.50			B	0.284	2.339	0.611	0.8	1	5.908			
			C	0.642	1.784	0.78	0.8	1	16.523			
T37	0.20	0.61	A	0.445	1.982	0.671	0.8	1	9.838	0.28	44.52	C
87.50-81.25			B	0.303	2.288	0.617	0.8	1	6.159			
			C	0.664	1.778	0.795	0.8	1	17.395			
T38	0.20	0.61	A	0.445	1.982	0.671	0.8	1	9.838	0.27	43.56	C
81.25-75.00			B	0.303	2.288	0.617	0.8	1	6.159			
			C	0.664	1.778	0.795	0.8	1	17.395			
T39	0.79	2.45	A	0.453	1.968	0.675	0.8	1	40.328	1.02	40.87	C
75.00-50.00			B	0.312	2.266	0.619	0.8	1	25.429			
			C	0.664	1.778	0.795	0.8	1	69.578			
T40	0.80	2.47	A	0.476	1.934	0.685	0.8	1	43.071	0.89	35.48	C
50.00-25.00			B	0.334	2.21	0.627	0.8	1	27.730			
			C	0.666	1.778	0.796	0.8	1	69.908			
T41	0.71	2.45	A	0.44	1.99	0.669	0.8	1	38.787	0.74	29.71	C
25.00-0.00			B	0.315	2.256	0.621	0.8	1	25.778			
			C	0.602	1.803	0.755	0.8	1	59.889			
Sum Weight:	8.34	42.51								14.79		

Tower Forces - Service - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1	0.00	0.68	A	0.223	2.523	0.595	0.85	1	5.392	0.19	29.77	C
368.75-362.50			B	0.215	2.545	0.594	0.85	1	5.250			
			C	0.223	2.523	0.595	0.85	1	5.392			
T2	0.00	1.09	A	0.253	2.43	0.603	0.85	1	6.165	0.20	32.62	C
362.50-356.25			B	0.235	2.484	0.598	0.85	1	5.804			
			C	0.253	2.43	0.603	0.85	1	6.165			
T3	0.01	1.09	A	0.253	2.43	0.603	0.85	1	6.165	0.21	33.42	C
356.25-350.00			B	0.249	2.44	0.602	0.85	1	6.092			
			C	0.267	2.388	0.606	0.85	1	6.458			
T4	0.01	0.77	A	0.259	2.41	0.604	0.85	1	6.321	0.23	36.38	C
350.00-343.75		TA 0.79	B	0.295	2.311	0.614	0.85	1	7.068			
			C	0.312	2.264	0.62	0.85	1	7.453			
T5	0.01	0.61	A	0.167	2.709	0.584	0.85	1	3.467	0.15	24.67	C
343.75-337.50			B	0.202	2.588	0.591	0.85	1	4.181			
			C	0.22	2.531	0.595	0.85	1	4.546			
T6	0.01	0.54	A	0.157	2.746	0.583	0.85	1	3.002	0.14	22.31	C
337.50-331.25			B	0.192	2.622	0.589	0.85	1	3.715			
			C	0.21	2.563	0.592	0.85	1	4.080			
T7	0.01	0.54	A	0.157	2.746	0.583	0.85	1	3.002	0.14	22.19	C
331.25-325.00			B	0.192	2.622	0.589	0.85	1	3.715			
			C	0.21	2.563	0.592	0.85	1	4.080			
T8	0.02	0.64	A	0.2	2.595	0.59	0.85	1	3.897	0.15	24.30	B
325.00-318.75			B	0.235	2.482	0.598	0.85	1	4.641			
			C	0.222	2.525	0.595	0.85	1	4.346			
T9	0.02	0.64	A	0.2	2.595	0.59	0.85	1	3.894	0.15	24.16	B
318.75-312.50			B	0.235	2.482	0.598	0.85	1	4.637			
			C	0.222	2.526	0.595	0.85	1	4.343			
T10	0.02	0.64	A	0.2	2.595	0.59	0.85	1	3.894	0.15	24.02	B
312.50-306.25			B	0.235	2.482	0.598	0.85	1	4.637			
			C	0.222	2.526	0.595	0.85	1	4.343			

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	58 of 98
	Project	Montville, CT	Date	09:02:33 10/11/12
	Client	Verizon Wireless	Designed by	Kevin_Barker

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T11 306.25-300.00	0.08	0.71	A	0.21	2.562	0.593	0.85	1	4.358	0.22	35.32	C
			B	0.245	2.452	0.601	0.85	1	5.101			
			C	0.372	2.122	0.641	0.85	1	8.023			
T12 300.00-293.75	0.10	0.85	A	0.296	2.306	0.615	0.85	1	7.126	0.28	45.27	C
		TA 0.79	B	0.332	2.216	0.626	0.85	1	7.906			
			C	0.475	1.934	0.685	0.85	1	11.350			
T13 293.75-287.50	0.10	0.71	A	0.21	2.562	0.593	0.85	1	4.358	0.23	37.53	C
			B	0.245	2.452	0.601	0.85	1	5.101			
			C	0.407	2.05	0.655	0.85	1	8.931			
T14 287.50-281.25	0.10	0.61	A	0.195	2.611	0.589	0.85	1	3.795	0.22	35.74	C
			B	0.231	2.497	0.597	0.85	1	4.534			
			C	0.395	2.073	0.65	0.85	1	8.464			
T15 281.25-275.00	0.10	0.61	A	0.195	2.611	0.589	0.85	1	3.795	0.22	35.51	C
			B	0.231	2.497	0.597	0.85	1	4.534			
			C	0.395	2.073	0.65	0.85	1	8.464			
T16 275.00-268.75	0.10	0.61	A	0.195	2.611	0.589	0.85	1	3.795	0.22	35.28	C
			B	0.231	2.497	0.597	0.85	1	4.534			
			C	0.395	2.073	0.65	0.85	1	8.464			
T17 268.75-262.50	0.10	0.61	A	0.195	2.611	0.589	0.85	1	3.795	0.22	35.05	C
			B	0.231	2.497	0.597	0.85	1	4.534			
			C	0.395	2.073	0.65	0.85	1	8.464			
T18 262.50-256.25	0.10	0.61	A	0.195	2.611	0.589	0.85	1	3.795	0.22	34.81	C
			B	0.231	2.497	0.597	0.85	1	4.534			
			C	0.395	2.073	0.65	0.85	1	8.464			
T19 256.25-250.00	0.10	0.64	A	0.2	2.595	0.59	0.85	1	3.894	0.22	34.86	C
			B	0.235	2.482	0.598	0.85	1	4.637			
			C	0.399	2.066	0.651	0.85	1	8.565			
T20 250.00-243.75	0.10	0.71	A	0.21	2.562	0.593	0.85	1	4.358	0.22	35.82	C
			B	0.245	2.452	0.601	0.85	1	5.101			
			C	0.407	2.05	0.655	0.85	1	8.931			
T21 243.75-237.50	0.17	0.71	A	0.376	2.114	0.642	0.85	1	8.146	0.31	49.54	C
			B	0.245	2.452	0.601	0.85	1	5.101			
			C	0.572	1.824	0.737	0.85	1	13.988			
T22 237.50-231.25	0.18	0.71	A	0.398	2.068	0.651	0.85	1	8.731	0.32	51.51	C
			B	0.245	2.452	0.601	0.85	1	5.101			
			C	0.595	1.807	0.751	0.85	1	14.788			
T23 231.25-225.00	0.18	0.71	A	0.398	2.068	0.651	0.85	1	8.731	0.32	51.11	C
			B	0.245	2.452	0.601	0.85	1	5.101			
			C	0.595	1.807	0.751	0.85	1	14.788			
T24 225.00-218.75	0.18	0.95	A	0.459	1.959	0.678	0.85	1	10.925	0.35	56.50	C
		TA 0.79	B	0.306	2.28	0.618	0.85	1	7.163			
			C	0.643	1.784	0.781	0.85	1	16.695			
T25 218.75-212.50	0.18	0.66	A	0.411	2.042	0.656	0.85	1	9.202	0.32	51.35	C
			B	0.258	2.414	0.604	0.85	1	5.550			
			C	0.605	1.801	0.757	0.85	1	15.150			
T26 212.50-206.25	0.18	0.58	A	0.401	2.062	0.652	0.85	1	8.740	0.31	49.83	C
			B	0.248	2.444	0.601	0.85	1	5.086			
			C	0.597	1.806	0.752	0.85	1	14.785			
T27 206.25-200.00	0.18	0.58	A	0.401	2.062	0.652	0.85	1	8.740	0.31	49.40	C
			B	0.248	2.444	0.601	0.85	1	5.086			
			C	0.597	1.806	0.752	0.85	1	14.785			
T28 200.00-175.00	0.72	2.28	A	0.418	2.029	0.659	0.85	1	36.181	1.24	49.59	C
			B	0.247	2.446	0.601	0.85	1	19.508			
			C	0.611	1.797	0.761	0.85	1	61.022			
T29 175.00-168.75	0.18	0.61	A	0.418	2.029	0.659	0.85	1	9.089	0.30	47.49	C
			B	0.248	2.443	0.601	0.85	1	4.918			
			C	0.601	1.803	0.754	0.85	1	14.930			
T30 168.75-162.50	0.18	0.69	A	0.433	2.003	0.665	0.85	1	9.486	0.30	48.14	C
			B	0.262	2.401	0.605	0.85	1	5.231			
			C	0.612	1.797	0.761	0.85	1	15.348			

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Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T31	0.18	0.69	A	0.428	2.011	0.664	0.85	1	9.549	0.30	47.48	C
162.50-156.25			B	0.258	2.413	0.604	0.85	1	5.378			
			C	0.609	1.799	0.759	0.85	1	15.291			
T32	0.18	0.61	A	0.418	2.029	0.659	0.85	1	9.085	0.29	45.94	C
156.25-150.00			B	0.248	2.443	0.601	0.85	1	4.914			
			C	0.601	1.804	0.754	0.85	1	14.925			
T33	0.74	2.45	A	0.437	1.995	0.668	0.85	1	38.466	1.14	45.56	C
150.00-125.00			B	0.257	2.418	0.604	0.85	1	20.400			
			C	0.611	1.797	0.761	0.85	1	61.270			
T34	0.76	2.84	A	0.517	1.879	0.706	0.85	1	49.916	1.27	50.89	C
125.00-100.00			B	0.338	2.199	0.628	0.85	1	30.800			
			C	0.684	1.776	0.809	0.85	1	73.341			
T35	0.19	1.03	A	0.527	1.867	0.712	0.85	1	12.925	0.31	49.69	C
100.00-93.75		TA 0.79	B	0.348	2.176	0.632	0.85	1	8.152			
			C	0.692	1.776	0.814	0.85	1	18.688			
T36	0.19	0.71	A	0.46	1.958	0.678	0.85	1	10.439	0.27	43.41	C
93.75-87.50			B	0.284	2.339	0.611	0.85	1	5.957			
			C	0.642	1.784	0.78	0.85	1	16.562			
T37	0.20	0.61	A	0.445	1.982	0.671	0.85	1	9.838	0.28	44.52	C
87.50-81.25			B	0.303	2.288	0.617	0.85	1	6.159			
			C	0.664	1.778	0.795	0.85	1	17.395			
T38	0.20	0.61	A	0.445	1.982	0.671	0.85	1	9.838	0.27	43.56	C
81.25-75.00			B	0.303	2.288	0.617	0.85	1	6.159			
			C	0.664	1.778	0.795	0.85	1	17.395			
T39	0.79	2.45	A	0.453	1.968	0.675	0.85	1	40.328	1.02	40.87	C
75.00-50.00			B	0.312	2.266	0.619	0.85	1	25.429			
			C	0.664	1.778	0.795	0.85	1	69.578			
T40	0.80	2.47	A	0.476	1.934	0.685	0.85	1	43.120	0.89	35.50	C
50.00-25.00			B	0.334	2.21	0.627	0.85	1	27.779			
			C	0.666	1.778	0.796	0.85	1	69.947			
T41	0.71	2.45	A	0.44	1.99	0.669	0.85	1	38.787	0.74	29.71	C
25.00-0.00			B	0.315	2.256	0.621	0.85	1	25.778			
			C	0.602	1.803	0.755	0.85	1	59.889			
Sum Weight:	8.34	42.51								14.86		

Force Totals (Does not include forces on guys)

Load Case	Vertical Forces	Sum of Forces X	Sum of Forces Z	Sum of Torques
	K	K	K	kip-ft
Leg Weight	29.84			
Bracing Weight	12.67			
Total Member Self-Weight	42.51			
Guy Weight	11.35			
Total Weight	69.16			
Wind 0 deg - No Ice		0.00	-79.19	-18.37
Wind 90 deg - No Ice		78.52	0.00	16.23
Wind 180 deg - No Ice		0.00	78.29	18.35
Member Ice	8.68			
Guy Ice	6.84			
Total Weight Ice	102.80			
Wind 0 deg - Ice		0.00	-99.70	-16.08
Wind 90 deg - Ice		95.38	0.00	19.21
Wind 180 deg - Ice		0.00	93.54	15.70
Total Weight	69.16			

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Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Torques kip-ft
Wind 0 deg - Service		0.00	-21.94	-5.09
Wind 90 deg - Service		21.75	0.00	4.50
Wind 180 deg - Service		0.00	21.69	5.08

Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice+Guy
3	Dead+Wind 90 deg - No Ice+Guy
4	Dead+Wind 180 deg - No Ice+Guy
5	Dead+Ice+Temp+Guy
6	Dead+Wind 0 deg+Ice+Temp+Guy
7	Dead+Wind 90 deg+Ice+Temp+Guy
8	Dead+Wind 180 deg+Ice+Temp+Guy
9	Dead+Wind 0 deg - Service+Guy
10	Dead+Wind 90 deg - Service+Guy
11	Dead+Wind 180 deg - Service+Guy

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	368.75 - 362.5	Leg	Max Tension	4	0.35	0.00	-0.00
			Max. Compression	8	-0.94	0.04	0.16
			Max. Mx	3	-0.14	0.19	0.00
			Max. My	4	-0.75	0.06	0.22
			Max. Vy	3	-0.14	0.00	-0.00
			Max. Vx	2	0.21	0.00	0.00
		Diagonal	Max Tension	2	0.41	0.00	0.00
			Max. Compression	2	-0.48	0.00	0.00
			Max. Mx	8	0.34	0.01	-0.00
			Max. My	6	0.10	0.01	0.00
			Max. Vy	8	-0.01	0.01	-0.00
			Max. Vx	6	0.00	0.00	0.00
		Top Girt	Max Tension	2	0.11	0.00	0.00
			Max. Compression	3	-0.09	0.00	0.00
			Max. Mx	5	-0.01	0.03	0.00
			Max. My	6	-0.03	0.00	-0.00
			Max. Vy	5	0.03	0.00	0.00
			Max. Vx	6	-0.00	0.00	0.00
T2	362.5 - 356.25	Leg	Max Tension	4	0.89	-0.13	0.11
			Max. Compression	2	-1.51	-0.52	0.73
			Max. Mx	3	-0.31	-1.01	-0.01
			Max. My	4	-1.21	-0.23	-1.15
			Max. Vy	3	0.23	-1.01	-0.01
			Max. Vx	2	-0.27	0.26	1.15
		Diagonal	Max Tension	4	0.62	0.00	0.00
			Max. Compression	2	-0.88	0.00	0.00

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T3	356.25 - 350	Top Girt	Max. Mx	8	-0.55	-0.03	-0.00
			Max. My	3	-0.47	-0.02	0.01
			Max. Vy	8	0.03	-0.03	-0.00
			Max. Vx	3	0.00	0.00	0.00
			Max Tension	4	0.33	0.00	0.00
			Max. Compression	2	-0.18	0.00	0.00
		Leg	Max. Mx	5	0.04	0.04	0.00
			Max. My	6	0.21	0.00	-0.00
			Max. Vy	5	0.03	0.00	0.00
			Max. Vx	6	-0.00	0.00	0.00
			Max Tension	4	6.38	0.53	-0.71
			Max. Compression	2	-9.11	-0.35	0.82
		Diagonal	Max. Mx	3	-1.31	1.68	-0.02
			Max. My	4	-5.28	-0.21	1.87
			Max. Vy	3	-2.16	-1.01	-0.01
			Max. Vx	4	-2.42	-0.23	-1.15
			Max Tension	2	5.43	0.00	0.00
			Max. Compression	4	-5.20	0.00	0.00
		Top Girt	Max. Mx	6	0.47	-0.06	-0.03
			Max. My	2	-4.24	0.00	-0.08
			Max. Vy	6	-0.03	-0.06	-0.03
			Max. Vx	2	0.02	0.00	-0.08
			Max Tension	2	1.37	0.00	0.00
			Max. Compression	4	-1.30	0.00	0.00
T4	350 - 343.75	Leg	Max. Mx	5	0.04	0.04	0.00
			Max. My	6	-0.51	0.00	-0.00
			Max. Vy	5	0.03	0.00	0.00
			Max. Vx	6	-0.00	0.00	0.00
			Max Tension	4	5.89	0.36	-0.82
			Max. Compression	2	-8.47	0.20	0.16
		Diagonal	Max. Mx	3	-1.46	-1.10	-0.02
			Max. My	2	1.99	0.21	1.18
			Max. Vy	3	-0.72	-0.73	0.15
			Max. Vx	2	0.89	0.21	1.18
			Max Tension	6	3.60	0.00	0.00
			Max. Compression	7	-3.90	0.00	0.00
		Top Girt	Max. Mx	8	0.09	0.03	-0.00
			Max. My	2	-3.16	0.03	0.01
			Max. Vy	8	-0.02	0.03	-0.00
			Max. Vx	2	-0.00	0.03	0.01
			Max Tension	4	9.71	0.00	0.00
			Max. Compression	2	-6.92	0.00	0.00
		Guy A	Max. Mx	5	1.32	0.03	0.00
			Max. My	6	5.94	0.00	0.00
			Max. Vy	5	0.03	0.00	0.00
			Max. Vx	6	0.00	0.00	0.00
			Bottom Tension	8	25.15		
			Top Tension	8	25.99		
		Guy B	Top Cable Vert	8	21.83		
			Top Cable Norm	8	14.11		
			Top Cable Tan	8	0.02		
			Bot Cable Vert	8	-19.87		
			Bot Cable Norm	8	15.43		
			Bot Cable Tan	8	0.02		
			Bottom Tension	6	20.24		
			Top Tension	6	21.08		
			Top Cable Vert	6	17.64		
			Top Cable Norm	6	11.50		
			Top Cable Tan	6	0.93		
			Bot Cable Vert	6	-16.04		
			Bot Cable Norm	6	12.30		

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T5	343.75 - 337.5	Guy C	Bot Cable Tan	6	1.03			
			Bottom Tension	7	23.54			
			Top Tension	7	24.38			
			Top Cable Vert	7	20.48			
			Top Cable Norm	7	13.22			
			Top Cable Tan	7	0.47			
			Bot Cable Vert	7	-18.59			
			Bot Cable Norm	7	14.44			
		Torque Arm Top	Bot Cable Tan	7	0.53			
			Max Tension	8	23.92	0.00	0.00	
			Max. Compression	1	0.00	0.00	0.00	
			Max. Mx	7	16.75	0.06	0.00	
			Max. My	6	21.96	0.00	-0.00	
			Max. Vy	7	-0.04	0.00	0.00	
			Max. Vx	6	0.00	0.00	0.00	
		Torque Arm Bottom	Max Tension	1	0.00	0.00	0.00	
			Max. Compression	8	-27.40	0.00	0.00	
			Max. Mx	6	-22.90	0.08	0.00	
			Max. My	6	-8.93	0.00	0.00	
			Max. Vy	6	-0.04	0.00	0.00	
			Max. Vx	6	-0.00	0.00	0.00	
			Max Tension	1	0.00	0.00	0.00	
		Leg	Max Tension	1	0.00	0.00	0.00	
			Max. Compression	8	-44.97	0.03	0.03	
			Max. Mx	3	-35.34	-0.20	-0.10	
			Max. My	2	-31.22	-0.06	0.29	
			Max. Vy	3	-0.08	-0.20	-0.10	
			Max. Vx	2	0.10	-0.06	0.29	
			Diagonal Top Girt	Max Tension	6	4.09	0.00	0.00
				Max Tension	1	0.00	0.00	0.00
		Max. Compression		8	-4.16	0.00	0.00	
		Max. Mx		5	-2.00	0.03	0.00	
		Max. My		6	-2.97	0.00	-0.00	
		Max. Vy		5	0.03	0.00	0.00	
Max. Vx	6	0.00		0.00	0.00			
T6	337.5 - 331.25	Leg	Max Tension	1	0.00	0.00	0.00	
			Max. Compression	8	-46.13	-0.02	-0.02	
			Max. Mx	7	-44.71	0.07	0.01	
			Max. My	6	-39.69	0.00	-0.09	
			Max. Vy	3	0.05	-0.04	-0.01	
			Max. Vx	2	-0.06	-0.00	0.05	
		Diagonal Top Girt	Max Tension	6	3.71	0.00	0.00	
			Max Tension	1	0.00	0.00	0.00	
			Max. Compression	10	-2.56	0.00	0.00	
			Max. Mx	5	-2.45	0.01	0.00	
			Max. My	6	-2.31	0.00	-0.00	
			Max. Vy	5	-0.01	0.00	0.00	
			Max. Vx	6	0.00	0.00	0.00	
		Leg	Max Tension	1	0.00	0.00	0.00	
			Max. Compression	8	-46.62	0.04	0.00	
			Max. Mx	7	-14.48	0.06	0.02	
			Max. My	6	-41.26	0.02	-0.08	
			Max. Vy	3	-0.05	-0.04	-0.01	
			Max. Vx	2	0.06	-0.00	0.05	
Diagonal Top Girt	Max Tension	6	3.26	0.00	0.00			
	Max Tension	1	0.00	0.00	0.00			
	Max. Compression	11	-2.46	0.00	0.00			
	Max. Mx	5	-2.33	0.01	0.00			
	Max. My	6	-2.25	0.00	-0.00			
	Max. Vy	5	-0.01	0.00	0.00			
	Max. Vx	6	0.00	0.00	0.00			
	Max Tension	1	0.00	0.00	0.00			
Leg	325 - 318.75	Max Tension	1	0.00	0.00	0.00		

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T9	318.75 - 312.5	Diagonal Top Girt	Max. Compression	8	-47.42	-0.02	-0.04	
			Max. Mx	7	-46.55	0.05	0.01	
			Max. My	6	-43.23	0.02	-0.09	
			Max. Vy	3	-0.19	-0.01	0.01	
			Max. Vx	4	-0.21	-0.02	0.03	
			Max Tension	4	3.69	0.00	0.00	
			Max Tension	1	0.00	0.00	0.00	
			Max. Compression	4	-2.94	0.00	0.00	
			Max. Mx	5	-2.79	0.01	0.00	
			Max. My	6	-2.73	0.00	-0.00	
			Max. Vy	5	-0.01	0.00	0.00	
			Max. Vx	6	0.00	0.00	0.00	
		Leg	Max Tension	1	0.00	0.00	0.00	
			Max. Compression	8	-45.77	0.01	0.11	
			Max. Mx	3	-36.83	0.13	-0.00	
			Max. My	6	-43.90	-0.02	-0.12	
			Max. Vy	3	-0.07	-0.05	0.01	
			Max. Vx	2	0.07	-0.00	0.05	
			Diagonal Top Girt	Max Tension	4	3.88	0.00	0.00
				Max Tension	1	0.00	0.00	0.00
				Max. Compression	11	-3.37	0.00	0.00
				Max. Mx	5	-3.23	0.01	0.00
				Max. My	6	-3.16	0.00	-0.00
				Max. Vy	5	-0.01	0.00	0.00
Max. Vx	6	0.00		0.00	0.00			
T10	312.5 - 306.25	Leg		Max Tension	1	0.00	0.00	0.00
				Max. Compression	6	-45.72	-0.04	0.37
				Max. Mx	3	-35.62	-0.51	0.01
				Max. My	2	-15.29	0.03	0.49
				Max. Vy	3	0.14	-0.51	0.01
			Max. Vx	2	-0.14	0.03	0.47	
			Diagonal Top Girt	Max Tension	4	4.25	0.00	0.00
				Max Tension	1	0.00	0.00	0.00
				Max. Compression	11	-3.45	0.00	0.00
				Max. Mx	5	-3.33	0.01	0.00
				Max. My	6	-3.22	0.00	-0.00
				Max. Vy	5	-0.01	0.00	0.00
		Leg	Max. Vx	6	0.00	0.00	0.00	
			Max Tension	1	0.00	0.00	0.00	
			Max. Compression	6	-45.42	0.07	0.98	
			Max. Mx	3	-36.39	-1.16	-0.00	
			Max. My	4	-31.40	-0.08	-1.20	
			Max. Vy	3	-1.30	-0.51	0.01	
			Diagonal Top Girt	Max. Vx	2	1.30	0.03	0.47
				Max Tension	4	7.34	0.00	0.00
				Max Tension	1	0.00	0.00	0.00
				Max. Compression	4	-4.70	0.00	0.00
				Max. Mx	5	-3.73	0.03	0.00
				Max. My	6	-4.09	0.00	-0.00
T11	306.25 - 300	Leg		Max. Vy	5	0.03	0.00	0.00
				Max. Vx	6	0.00	0.00	0.00
				Max Tension	1	0.00	0.00	0.00
				Max. Compression	6	-38.53	-0.05	0.03
				Max. Mx	3	-28.45	-1.16	-0.00
				Max. My	4	-22.09	-0.08	-1.20
			Diagonal Top Girt	Max. Vy	3	-0.66	-1.16	-0.00
				Max. Vx	4	-0.66	-0.08	-1.20
				Max Tension	7	3.01	0.00	0.00
				Max. Compression	8	-7.95	0.00	0.00
				Max. Mx	7	0.97	0.05	-0.00
				Max. My	7	-7.65	0.01	0.01

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T13	293.75 - 287.5	Top Girt	Max. Vy	7	-0.02	0.05	-0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	8	11.72	0.00	0.00
			Max. Compression	2	-7.76	0.00	0.00
			Max. Mx	5	0.36	0.03	0.00
			Max. My	6	6.01	0.00	0.00
		Guy A	Max. Vy	5	0.03	0.00	0.00
			Max. Vx	6	0.00	0.00	0.00
			Bottom Tension	8	26.98		
			Top Tension	8	27.70		
			Top Cable Vert	8	23.43		
			Top Cable Norm	8	14.76		
		Guy B	Top Cable Tan	8	0.02		
			Bot Cable Vert	8	-21.81		
			Bot Cable Norm	8	15.87		
			Bot Cable Tan	8	0.03		
			Bottom Tension	6	22.04		
			Top Tension	6	22.76		
		Guy C	Top Cable Vert	6	19.21		
			Top Cable Norm	6	12.19		
			Top Cable Tan	6	0.71		
			Bot Cable Vert	6	-17.88		
			Bot Cable Norm	6	12.86		
			Bot Cable Tan	6	0.87		
		Torque Arm Top	Bottom Tension	7	25.96		
			Top Tension	7	26.68		
			Top Cable Vert	7	22.57		
			Top Cable Norm	7	14.23		
			Top Cable Tan	7	0.34		
			Bot Cable Vert	7	-21.00		
		Torque Arm Bottom	Bot Cable Norm	7	15.25		
			Bot Cable Tan	7	0.46		
			Max Tension	7	24.06	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	7	14.93	0.06	0.00
			Max. My	6	22.57	0.00	-0.00
T14	287.5 - 281.25	Leg	Max. Vy	7	0.04	0.00	0.00
			Max. Vx	6	0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-30.17	0.00	0.00
			Max. Mx	6	-24.45	0.08	0.00
			Max. My	6	-6.64	0.00	0.00
		Diagonal	Max. Vy	6	-0.04	0.00	0.00
			Max. Vx	6	-0.00	0.00	0.00
			Max Tension	7	0.08	0.01	0.14
			Max. Compression	8	-2.82	0.00	0.00
			Max. Mx	5	-1.17	0.03	0.00
			Max. My	6	-1.90	0.00	-0.00
		Top Girt	Max. Vy	5	0.03	0.00	0.00
			Max. Vx	6	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-101.09	-0.01	0.09
			Max. Mx	7	-98.62	0.13	0.04
			Max. My	6	-91.28	-0.00	-0.16

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	65 of 98
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	Client	Verizon Wireless	Designed by	Kevin_Barker

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T15	281.25 - 275	Diagonal Top Girt	Max. Vy	7	-0.07	-0.05	0.06
			Max. Vx	6	0.08	-0.02	-0.00
			Max Tension	7	5.78	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
		Leg	Max. Compression	7	-3.96	0.00	0.00
			Max. Mx	5	-2.47	0.01	0.00
			Max. My	6	-3.17	0.00	-0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	6	0.00	0.00	0.00
			Max Tension	6	3.41	0.01	-0.15
			Max. Compression	8	-104.95	-0.02	0.06
			Max. Mx	7	-10.94	0.18	0.01
			Max. My	6	-94.29	-0.00	-0.20
			Max. Vy	7	0.05	0.10	0.01
			Max. Vx	6	-0.06	-0.01	-0.10
		Diagonal Top Girt	Max Tension	7	4.60	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	7	-3.28	0.00	0.00
			Max. Mx	5	-1.97	0.01	0.00
T16	275 - 268.75	Leg	Max. My	6	-2.63	0.00	-0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	6	0.00	0.00	0.00
			Max Tension	6	8.58	0.01	-0.11
			Max. Compression	8	-107.82	-0.02	0.06
			Max. Mx	7	-7.12	0.16	0.01
			Max. My	6	-96.93	-0.01	-0.18
			Max. Vy	3	0.05	0.04	0.03
			Max. Vx	6	-0.06	-0.01	-0.08
		Diagonal Top Girt	Max Tension	7	3.31	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	7	-2.51	0.00	0.00
			Max. Mx	5	-1.98	0.01	0.00
T17	268.75 - 262.5	Leg	Max. My	7	-2.15	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	6	11.51	0.02	-0.09
			Max. Compression	8	-109.40	-0.01	0.07
			Max. Mx	7	-4.89	0.15	0.02
			Max. My	6	-99.37	-0.01	-0.17
			Max. Vy	3	-0.05	0.04	0.03
			Max. Vx	6	0.06	-0.01	-0.08
		Diagonal Top Girt	Max Tension	3	2.43	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	4	-2.36	0.00	0.00
			Max. Mx	5	-1.97	0.01	0.00
T18	262.5 - 256.25	Leg	Max. My	7	-2.17	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	6	12.88	0.01	-0.12
			Max. Compression	8	-109.48	-0.02	0.03
			Max. Mx	7	-106.49	0.13	-0.00
			Max. My	6	-100.32	-0.01	-0.17
			Max. Vy	3	0.05	-0.00	0.00
			Max. Vx	4	0.06	-0.01	-0.02
		Diagonal Top Girt	Max Tension	3	2.04	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	4	-2.33	0.00	0.00
			Max. Mx	5	-1.94	0.01	0.00
		Leg	Max. My	7	-2.15	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	66 of 98
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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T19	256.25 - 250	Leg	Max Tension	6	11.31	0.02	-0.05
			Max. Compression	8	-108.86	-0.04	0.15
			Max. Mx	7	-6.40	0.21	0.06
			Max. My	6	11.20	0.03	-0.25
			Max. Vy	3	-0.08	-0.00	0.00
			Max. Vx	4	-0.08	-0.01	-0.02
		Diagonal	Max Tension	3	3.10	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	4	-2.85	0.00	0.00
			Max. Mx	5	-2.38	0.01	0.00
			Max. My	7	-2.60	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
		Top Girt	Max Tension	6	9.02	0.03	-0.25
			Max. Compression	8	-106.13	0.01	-0.39
			Max. Mx	3	-11.28	-0.52	-0.02
			Max. My	2	-4.77	-0.01	0.53
T20	250 - 243.75	Leg	Max. Vy	3	0.16	-0.52	-0.02
			Max. Vx	2	-0.16	0.00	0.51
			Max Tension	2	3.34	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	4	-3.44	0.00	0.00
			Max. Mx	5	-2.96	0.03	0.00
		Diagonal	Max. My	3	-2.85	0.00	0.00
			Max. Vy	5	0.03	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
			Max Tension	6	0.40	-0.01	0.41
			Max. Compression	8	-102.89	0.00	-0.29
			Max. Mx	3	-81.12	1.31	-0.00
			Max. My	4	-40.29	0.01	1.31
			Max. Vy	3	-1.48	-0.52	-0.02
			Max. Vx	2	1.47	-0.00	0.51
		Top Girt	Max Tension	7	7.89	0.00	0.00
			Max Tension	6	0.01	0.00	0.00
			Max. Compression	4	-4.34	0.00	0.00
			Max. Mx	5	-3.03	0.03	0.00
T21	243.75 - 237.5	Leg	Max. My	3	-3.90	0.00	0.00
			Max. Vy	5	0.03	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-95.88	0.02	-0.36
			Max. Mx	7	-85.59	0.34	0.04
		Diagonal	Max. My	6	-95.88	0.02	-0.36
			Max. Vy	7	-0.15	-0.28	0.01
			Max. Vx	6	0.15	0.01	0.24
			Max Tension	7	11.15	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	7	-5.97	0.00	0.00
		Top Girt	Max. Mx	5	-3.00	0.03	0.00
			Max. My	3	-5.72	0.00	0.00
			Max. Vy	5	0.03	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
T22	237.5 - 231.25	Leg	Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-88.89	-0.05	0.78
			Max. Mx	7	-86.82	-0.78	0.05
			Max. My	8	-69.18	0.09	-0.87
			Max. Vy	7	0.23	-0.76	-0.02
			Max. Vx	8	0.23	-0.08	-0.83
		Diagonal	Max Tension	7	12.31	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	7	-7.34	0.00	0.00
		Top Girt	Max. Mx	5	-3.00	0.03	0.00
			Max. My	3	-5.72	0.00	0.00
			Max. Vy	5	0.03	0.00	0.00
T23	231.25 - 225	Leg	Max. Vx	3	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-88.89	-0.05	0.78
			Max. Mx	7	-86.82	-0.78	0.05
			Max. My	8	-69.18	0.09	-0.87
			Max. Vy	7	0.23	-0.76	-0.02
		Diagonal	Max. Vx	8	0.23	-0.08	-0.83
			Max Tension	7	12.31	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	7	-7.34	0.00	0.00
		Top Girt	Max. Mx	5	-3.00	0.03	0.00
			Max. My	3	-5.72	0.00	0.00
			Max. Vy	5	0.03	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T24	225 - 218.75	Leg	Max. Mx	5	-3.12	0.03	0.00
			Max. My	3	-6.80	0.00	0.00
			Max. Vy	5	0.03	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-68.31	0.12	0.12
		Diagonal	Max. Mx	7	-67.96	-0.78	0.05
			Max. My	8	-51.06	0.09	-0.87
			Max. Vy	3	-0.53	-0.76	0.02
			Max. Vx	4	-0.55	0.08	-0.85
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	7	-15.34	0.00	0.00
		Top Girt	Max. Mx	7	-3.86	-0.10	0.01
			Max. My	7	-13.22	0.03	-0.03
			Max. Vy	7	0.04	-0.10	0.01
			Max. Vx	7	-0.01	0.00	0.00
			Max Tension	8	-18.24	0.00	0.00
			Max. Compression	2	-5.46	0.00	0.00
		Guy A	Max. Mx	5	2.95	0.03	0.00
			Max. My	3	1.20	0.00	0.00
			Max. Vy	5	0.03	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
			Bottom Tension	8	24.36		
			Top Tension	8	24.79		
		Guy B	Top Cable Vert	8	18.79		
			Top Cable Norm	8	16.17		
			Top Cable Tan	8	0.01		
			Bot Cable Vert	8	-17.69		
			Bot Cable Norm	8	16.75		
			Bot Cable Tan	8	0.02		
		Guy C	Bottom Tension	6	20.59		
			Top Tension	6	21.02		
			Top Cable Vert	6	15.91		
			Top Cable Norm	6	13.73		
			Top Cable Tan	6	0.45		
			Bot Cable Vert	6	-14.99		
		Torque Arm Top	Bot Cable Norm	6	14.11		
			Bot Cable Tan	6	0.66		
			Bottom Tension	7	24.23		
			Top Tension	7	24.66		
			Top Cable Vert	7	18.68		
			Top Cable Norm	7	16.09		
		Torque Arm Bottom	Top Cable Tan	7	0.19		
			Bot Cable Vert	7	-17.60		
			Bot Cable Norm	7	16.65		
			Bot Cable Tan	7	0.35		
			Max Tension	7	22.88	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
T25	218.75 - 212.5	Leg	Max. Mx	5	7.93	0.06	0.00
			Max. My	3	3.94	0.00	0.00
			Max. Vy	5	0.04	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-123.49	-0.00	-0.24
		Torque Arm Bottom	Max. Mx	7	-38.60	-0.41	0.11

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	370' Guyed Lattice Tower	Page	68 of 98
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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T26	212.5 - 206.25	Diagonal Secondary Horizontal	Max. My	6	-22.11	-0.11	0.35
			Max. Vy	7	-0.18	-0.41	0.11
			Max. Vx	6	0.18	-0.11	0.35
			Max Tension	7	7.66	0.00	0.00
			Max Tension	6	2.14	0.00	0.00
			Max. Compression	6	-2.14	0.00	0.00
		Top Girt	Max. Mx	5	1.01	-0.02	0.00
			Max. My	3	1.54	0.00	-0.00
			Max. Vy	5	0.01	0.00	0.00
			Max. Vx	3	0.00	0.00	0.00
			Max Tension	6	6.20	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
		Leg	Max. Mx	5	2.36	0.03	0.00
			Max. My	3	2.17	0.00	0.00
			Max. Vy	5	0.03	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-127.24	0.00	-0.14
		Diagonal Secondary Horizontal	Max. Mx	7	-32.29	0.48	0.02
			Max. My	6	-13.56	0.06	-0.51
			Max. Vy	7	0.23	-0.17	-0.02
			Max. Vx	6	-0.22	-0.04	0.12
			Max Tension	7	5.89	0.00	0.00
			Max Tension	6	2.20	0.00	0.00
		Top Girt	Max. Compression	6	-2.20	0.00	0.00
			Max. Mx	5	1.02	-0.02	0.00
			Max. My	7	2.18	0.00	-0.00
			Max. Vy	5	0.01	0.00	0.00
			Max. Vx	7	0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
T27	206.25 - 200	Leg	Max. Compression	7	-4.05	0.00	0.00
			Max. Mx	5	-1.44	0.01	0.00
			Max. My	3	-2.66	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
		Diagonal Secondary Horizontal	Max. Compression	6	-129.64	-0.01	-0.12
			Max. Mx	7	-27.97	0.36	0.04
			Max. My	6	-7.72	0.05	-0.37
			Max. Vy	7	-0.20	-0.17	-0.02
			Max. Vx	6	0.18	-0.04	0.12
			Max Tension	7	3.59	0.00	0.00
		Top Girt	Max Tension	6	2.25	0.00	0.00
			Max. Compression	6	-2.25	0.00	0.00
			Max. Mx	5	1.02	-0.02	0.00
			Max. My	7	2.24	0.00	-0.00
			Max. Vy	5	0.01	0.00	0.00
			Max. Vx	7	0.00	0.00	0.00
T28	200 - 175	Leg	Max Tension	7	0.00	0.00	0.00
			Max. Compression	7	-2.80	0.00	0.00
			Max. Mx	5	-1.31	0.01	0.00
			Max. My	7	-2.80	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
		Top Girt	Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-131.19	-0.02	-0.22
			Max. Mx	7	-39.75	0.53	0.03
			Max. My	6	-23.13	0.11	-0.63

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T29	175 - 168.75	Diagonal Horizontal	Max. Vy	7	-0.36	-0.19	-0.02
			Max. Vx	6	0.38	-0.03	0.24
			Max Tension	6	6.09	0.00	0.00
			Max Tension	6	2.27	0.00	0.00
			Max. Compression	6	-3.24	0.00	0.00
			Max. Mx	5	1.05	0.01	0.00
		Secondary Horizontal	Max. My	7	2.25	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	6	2.27	0.00	0.00
			Max. Compression	6	-2.27	0.00	0.00
			Max. Mx	5	1.05	0.01	0.00
		Top Girt	Max. My	7	2.25	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	11	-1.67	0.00	0.00
			Max. Mx	5	-1.30	0.01	0.00
		Leg	Max. My	7	-1.51	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-123.17	0.05	0.21
			Max. Mx	7	-49.93	-0.25	0.03
T30	168.75 - 162.5	Diagonal Top Girt	Max. My	8	-81.86	0.01	-0.27
			Max. Vy	7	0.10	-0.17	0.01
			Max. Vx	6	-0.11	0.05	0.21
			Max Tension	7	8.23	0.00	0.00
			Max Tension	6	0.22	0.00	0.00
			Max. Compression	6	-4.56	0.00	0.00
		Leg	Max. Mx	5	-1.37	0.01	0.00
			Max. My	7	-4.43	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-118.46	-0.04	0.34
		Diagonal Top Girt	Max. Mx	7	-109.33	-0.45	0.02
			Max. My	8	-100.89	0.02	-0.50
			Max. Vy	7	0.10	-0.45	0.02
			Max. Vx	8	0.09	0.02	-0.50
			Max Tension	7	9.92	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
T31	162.5 - 156.25	Leg	Max. Compression	6	-5.66	0.00	0.00
			Max. Mx	5	-2.65	0.01	0.00
			Max. My	7	-5.66	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
		Diagonal Top Girt	Max. Compression	6	-129.74	-0.04	-0.24
			Max. Mx	7	-122.15	-0.45	0.02
			Max. My	8	-113.66	0.02	-0.50
			Max. Vy	7	-0.16	-0.45	0.02
			Max. Vx	8	-0.15	0.02	-0.50
			Max Tension	8	3.87	0.00	0.00
		Leg	Max Tension	6	8.89	0.00	0.00
			Max. Compression	11	-1.33	0.00	0.00
			Max. Mx	5	0.08	0.03	0.00
			Max. My	7	1.48	0.00	0.00
			Max. Vy	5	0.03	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00

<i>tnxTower</i> <i>URS Corporation</i> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job 370' Guyed Lattice Tower	Page 70 of 98
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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T32	156.25 - 150	Guy A	Bottom Tension	8	25.98		
			Top Tension	8	26.29		
			Top Cable Vert	8	19.11		
			Top Cable Norm	8	18.05		
			Top Cable Tan	8	0.00		
			Bot Cable Vert	8	-18.33		
			Bot Cable Norm	8	18.40		
		Guy B	Bot Cable Tan	8	0.00		
			Bottom Tension	6	22.69		
			Top Tension	6	22.99		
			Top Cable Vert	6	16.70		
			Top Cable Norm	6	15.81		
			Top Cable Tan	6	0.25		
			Bot Cable Vert	6	-16.04		
		Guy C	Bot Cable Norm	6	16.04		
			Bot Cable Tan	6	0.53		
			Bottom Tension	7	26.06		
			Top Tension	7	26.37		
			Top Cable Vert	7	19.16		
			Top Cable Norm	7	18.11		
			Top Cable Tan	7	0.08		
		Leg	Bot Cable Vert	7	-18.40		
			Bot Cable Norm	7	18.46		
			Bot Cable Tan	7	0.30		
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-131.18	0.01	-0.06
			Max. Mx	7	-125.58	0.23	0.02
			Max. My	6	-130.97	-0.04	-0.24
		Diagonal Top Girt	Max. Vy	7	0.09	0.01	-0.01
			Max. Vx	6	-0.08	-0.01	-0.03
			Max Tension	8	2.80	0.00	0.00
			Max Tension	6	0.09	0.00	0.00
			Max. Compression	8	-1.95	0.00	0.00
Max. Mx	5		-1.47	0.01	0.00		
Max. My	7		-1.70	0.00	0.00		
T33	150 - 125	Leg	Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-132.41	0.01	-0.09
			Max. Mx	7	-74.45	-0.29	0.09
			Max. My	6	-128.44	0.05	0.35
			Max. Vy	7	0.13	-0.29	0.09
		Diagonal Horizontal	Max. Vx	6	-0.15	0.05	0.35
			Max Tension	6	6.75	0.00	0.00
			Max Tension	6	2.29	0.00	0.00
			Max. Compression	6	-3.91	0.00	0.00
			Max. Mx	5	1.20	0.01	0.00
			Max. My	7	2.23	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
		Top Girt	Max. Vx	7	-0.00	0.00	0.00
Max Tension	6		0.20	0.00	0.00		
Max. Compression	10		-1.72	0.00	0.00		
Max. Mx	5		-1.40	0.01	0.00		
Max. My	7		-0.99	0.00	0.00		
Max. Vy	5		-0.01	0.00	0.00		
Max. Vx	7		-0.00	0.00	0.00		
T34	125 - 100	Leg	Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-130.63	-0.07	0.49
			Max. Mx	7	-90.94	-0.66	-0.18
			Max. My	6	-104.75	0.07	0.80
			Max. Vy	7	0.18	-0.66	-0.18

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T35	100 - 93.75	Diagonal	Max. Vx	6	-0.21	0.07	0.80
			Max Tension	7	14.18	0.00	0.00
		Horizontal	Max Tension	6	2.26	0.00	0.00
			Max. Compression	7	-8.36	0.00	0.00
		Top Girt	Max. Mx	5	1.29	0.01	0.00
			Max. My	7	2.16	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-5.00	0.00	0.00
			Max. Mx	5	-2.15	0.01	0.00
			Max. My	7	-4.70	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
		Leg	Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-106.97	-0.04	0.30
			Max. Mx	7	-62.64	-0.66	-0.18
			Max. My	6	-76.92	0.07	0.80
			Max. Vy	3	-0.39	-0.53	-0.13
			Max. Vx	6	0.42	0.07	0.80
		Diagonal	Max Tension	1	0.00	0.00	0.00
			Max. Compression	7	-16.36	0.00	0.00
			Max. Mx	7	-13.03	-0.10	0.01
			Max. My	7	-16.32	0.01	-0.03
			Max. Vy	7	0.04	-0.10	0.01
		Top Girt	Max. Vx	7	-0.01	0.00	0.00
			Max Tension	8	19.04	0.00	0.00
			Max. Compression	6	-5.30	0.00	0.00
			Max. Mx	5	3.79	0.03	0.00
			Max. My	7	4.41	0.00	0.00
		Guy A	Max. Vy	5	0.03	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Bottom Tension	8	15.52		
			Top Tension	8	15.65		
			Top Cable Vert	8	8.55		
			Top Cable Norm	8	13.10		
			Top Cable Tan	8	0.00		
			Bot Cable Vert	8	-8.16		
			Bot Cable Norm	8	13.20		
			Bot Cable Tan	8	0.01		
		Guy B	Bottom Tension	6	14.26		
			Top Tension	6	14.39		
			Top Cable Vert	6	7.87		
			Top Cable Norm	6	12.05		
			Top Cable Tan	6	0.18		
			Bot Cable Vert	6	-7.51		
			Bot Cable Norm	6	12.12		
			Bot Cable Tan	6	0.32		
		Guy C	Bottom Tension	7	16.22		
			Top Tension	7	16.35		
			Top Cable Vert	7	8.93		
			Top Cable Norm	7	13.70		
			Top Cable Tan	7	0.05		
			Bot Cable Vert	7	-8.53		
			Bot Cable Norm	7	13.80		
			Bot Cable Tan	7	0.14		
		Torque Arm Top	Max Tension	7	16.98	0.00	0.00
			Max. Compression	6	-0.84	0.00	0.00
			Max. Mx	5	4.57	0.06	0.00
			Max. My	7	15.10	0.00	-0.00
			Max. Vy	5	-0.04	0.00	0.00

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T36	93.75 - 87.5	Torque Arm Bottom	Max. Vx	7	0.00	0.00	0.00
			Max Tension	6	0.82	0.00	0.00
			Max. Compression	7	-14.03	0.00	0.00
			Max. Mx	6	-12.22	0.08	0.00
			Max. My	7	0.67	0.00	-0.00
			Max. Vy	6	-0.04	0.00	0.00
		Leg	Max. Vx	7	0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-121.20	0.05	0.02
			Max. Mx	7	-113.24	-0.44	-0.14
			Max. My	8	-103.95	0.06	-0.38
			Max. Vy	7	-0.12	-0.44	-0.14
		Diagonal Top Girt	Max. Vx	8	-0.10	0.06	-0.38
			Max Tension	7	7.54	0.00	0.00
			Max Tension	8	7.00	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	5	3.94	0.03	0.00
			Max. My	7	5.12	0.00	0.00
T37	87.5 - 81.25	Leg	Max. Vy	5	0.03	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-123.75	-0.01	-0.08
			Max. Mx	7	-118.54	0.11	0.02
			Max. My	6	-123.75	-0.01	-0.11
		Diagonal Top Girt	Max. Vy	7	-0.08	-0.11	0.07
			Max. Vx	8	-0.07	0.00	-0.11
			Max Tension	7	5.68	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	7	-4.12	0.00	0.00
			Max. Mx	5	-1.54	0.01	0.00
		Leg	Max. My	7	-4.12	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-125.11	0.00	-0.04
			Max. Mx	7	-108.87	0.15	0.01
T38	81.25 - 75	Leg	Max. My	6	-124.78	-0.01	-0.13
			Max. Vy	7	0.06	0.05	0.01
			Max. Vx	6	-0.05	0.00	-0.04
		Diagonal Top Girt	Max Tension	7	3.61	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	7	-2.92	0.00	0.00
		Leg	Max. Mx	5	-1.14	0.01	0.00
			Max. My	7	-2.92	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-125.51	0.01	-0.03
T39	75 - 50	Leg	Max. Mx	7	-117.94	-0.82	-0.13
			Max. My	6	-122.49	-0.06	0.82
			Max. Vy	7	0.22	-0.82	-0.13
			Max. Vx	6	-0.21	-0.06	0.82
		Diagonal Horizontal	Max Tension	6	4.84	0.00	0.00
			Max Tension	6	2.17	0.00	0.00
			Max. Compression	6	-2.83	0.00	0.00
		Top Girt	Max. Mx	5	1.40	0.01	0.00
			Max. My	7	2.13	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	7	-1.61	0.00	0.00

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T40	50 - 25	Leg	Max. Mx	5	-1.14	0.01	0.00
			Max. My	7	-1.61	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	7	-134.38	0.03	-0.01
			Max. Mx	7	-124.33	-0.82	-0.13
			Max. My	6	-127.77	-0.06	0.82
			Max. Vy	7	-0.21	-0.82	-0.13
			Max. Vx	6	0.20	-0.06	0.82
		Diagonal	Max Tension	7	5.97	0.00	0.00
			Max Tension	7	2.33	0.00	0.00
			Max. Compression	7	-3.39	0.00	0.00
		Horizontal	Max. Mx	5	1.46	0.01	0.00
			Max. My	7	2.33	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
		Top Girt	Max. Vx	7	-0.00	0.00	0.00
			Max Tension	6	6.56	0.00	0.00
			Max. Compression	4	-0.25	0.00	0.00
		Guy A	Max. Mx	5	1.19	-0.02	0.00
			Max. My	7	0.88	0.00	-0.00
			Max. Vy	5	0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Bottom Tension	8	13.67		
			Top Tension	8	13.74		
			Top Cable Vert	8	5.68		
			Top Cable Norm	8	12.51		
			Top Cable Tan	8	0.00		
			Bot Cable Vert	8	-5.46		
		Guy B	Bot Cable Norm	8	12.54		
			Bot Cable Tan	8	0.00		
			Bottom Tension	6	12.66		
			Top Tension	6	12.73		
			Top Cable Vert	6	5.27		
			Top Cable Norm	6	11.58		
			Top Cable Tan	6	0.09		
			Bot Cable Vert	6	-5.05		
			Bot Cable Norm	6	11.61		
			Bot Cable Tan	6	0.18		
		Guy C	Bottom Tension	7	14.28		
			Top Tension	7	14.34		
			Top Cable Vert	7	5.93		
			Top Cable Norm	7	13.06		
			Top Cable Tan	7	0.02		
			Bot Cable Vert	7	-5.70		
			Bot Cable Norm	7	13.09		
			Bot Cable Tan	7	0.09		
		Leg	Max Tension	1	0.00	0.00	0.00
			Max. Compression	7	-134.61	0.08	-0.02
			Max. Mx	7	-128.82	-1.99	0.66
			Max. My	8	-124.20	0.02	-2.05
			Max. Vy	7	0.44	-1.99	0.66
			Max. Vx	8	0.44	0.02	-2.05
		Diagonal	Max Tension	7	4.12	0.00	0.00
			Max Tension	7	2.33	0.00	0.00
			Max. Compression	7	-2.33	0.00	0.00
		Horizontal	Max. Mx	5	1.49	0.01	0.00
			Max. My	7	2.33	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
		Top Girt	Max. Vx	7	-0.00	0.00	0.00
			Max Tension	6	0.12	0.00	0.00
T41	25 - 0	Leg	Max. Mx	7	-128.82	-1.99	0.66
			Max. My	8	-124.20	0.02	-2.05
			Max. Vy	7	0.44	-1.99	0.66
			Max. Vx	8	0.44	0.02	-2.05
		Diagonal	Max Tension	7	4.12	0.00	0.00
			Max Tension	7	2.33	0.00	0.00
			Max. Compression	7	-2.33	0.00	0.00
		Horizontal	Max. Mx	5	1.49	0.01	0.00
			Max. My	7	2.33	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
		Top Girt	Max. Vx	7	-0.00	0.00	0.00
			Max Tension	6	0.12	0.00	0.00

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
			Max. Compression	11	-1.43	0.00	0.00
			Max. Mx	5	-1.02	0.01	0.00
			Max. My	7	-0.52	0.00	0.00
			Max. Vy	5	-0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
		Base Beam	Max Tension	6	0.09	-366.42	-0.02
			Max. Compression	7	-3.64	2.05	0.01
			Max. Mx	6	-128.02	-367.56	-3.96
			Max. My	6	-127.98	-367.50	3.98
			Max. Vy	6	-128.02	-367.56	-3.96
			Max. Vx	6	1.38	-367.50	3.98

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Guy C @ 115 ft Elev 0 ft Azimuth 240 deg	Max. Vert	4	-0.66	-1.36	0.71
	Max. H _x	4	-0.66	-1.36	0.71
	Max. H _z	7	-5.70	-11.39	6.47
	Min. Vert	7	-5.70	-11.39	6.47
	Min. H _x	7	-5.70	-11.39	6.47
	Min. H _z	4	-0.66	-1.36	0.71
Guy B @ 115 ft Elev 0 ft Azimuth 120 deg	Max. Vert	3	-0.14	0.29	0.20
	Max. H _x	6	-5.05	9.96	5.96
	Max. H _z	6	-5.05	9.96	5.96
	Min. Vert	6	-5.05	9.96	5.96
	Min. H _x	3	-0.14	0.29	0.20
	Min. H _z	3	-0.14	0.29	0.20
Guy A @ 115 ft Elev 0 ft Azimuth 0 deg	Max. Vert	2	-0.07	-0.00	-0.22
	Max. H _x	8	-5.46	0.00	-12.54
	Max. H _z	2	-0.07	-0.00	-0.22
	Min. Vert	8	-5.46	0.00	-12.54
	Min. H _x	7	-3.08	-0.20	-7.10
	Min. H _z	8	-5.46	0.00	-12.54
Guy C @ 160 ft Elev 0 ft Azimuth 240 deg	Max. Vert	4	-4.36	-5.01	2.35
	Max. H _x	4	-4.36	-5.01	2.35
	Max. H _z	7	-34.93	-39.46	22.05
	Min. Vert	7	-34.93	-39.46	22.05
	Min. H _x	7	-34.93	-39.46	22.05
	Min. H _z	4	-4.36	-5.01	2.35
Guy B @ 160 ft Elev 0 ft Azimuth 120 deg	Max. Vert	3	-1.22	1.14	0.90
	Max. H _x	6	-30.81	33.99	20.93
	Max. H _z	6	-30.81	33.99	20.93
	Min. Vert	6	-30.81	33.99	20.93
	Min. H _x	3	-1.22	1.14	0.90
	Min. H _z	3	-1.22	1.14	0.90

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Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Guy A @ 160 ft Elev 0 ft Azimuth 0 deg	Max. Vert	2	-0.32	-0.00	-0.31
	Max. H _x	8	-34.34	0.02	-44.28
	Max. H _z	2	-0.32	-0.00	-0.31
	Min. Vert	8	-34.34	0.02	-44.28
	Min. H _x	7	-18.24	-1.21	-23.68
	Min. H _z	8	-34.34	0.02	-44.28
Guy C @ 205 ft Elev 0 ft Azimuth 240 deg	Max. Vert	4	-17.76	-12.16	5.04
	Max. H _x	4	-17.76	-12.16	5.04
	Max. H _z	7	-75.24	-54.72	29.58
	Min. Vert	7	-75.24	-54.72	29.58
	Min. H _x	7	-75.24	-54.72	29.58
	Min. H _z	4	-17.76	-12.16	5.04
Guy B @ 205 ft Elev 0 ft Azimuth 120 deg	Max. Vert	3	-8.49	4.30	3.47
	Max. H _x	6	-63.67	43.69	28.90
	Max. H _z	6	-63.67	43.69	28.90
	Min. Vert	6	-63.67	43.69	28.90
	Min. H _x	3	-8.49	4.30	3.47
	Min. H _z	3	-8.49	4.30	3.47
Guy A @ 205 ft Elev 0 ft Azimuth 0 deg	Max. Vert	2	-5.42	-0.00	-3.03
	Max. H _x	4	-65.41	0.02	-52.79
	Max. H _z	2	-5.42	-0.00	-3.03
	Min. Vert	8	-78.53	0.01	-64.82
	Min. H _x	7	-43.12	-3.61	-34.30
	Min. H _z	8	-78.53	0.01	-64.82
Guy C @ 250 ft Elev 0 ft Azimuth 240 deg	Max. Vert	11	-11.43	-7.38	3.86
	Max. H _x	11	-11.43	-7.38	3.86
	Max. H _z	6	-31.45	-19.81	13.88
	Min. Vert	7	-36.80	-25.31	13.33
	Min. H _x	7	-36.80	-25.31	13.33
	Min. H _z	4	-13.39	-8.58	3.48
Guy B @ 250 ft Elev 0 ft Azimuth 120 deg	Max. Vert	3	-7.71	3.81	2.95
	Max. H _x	6	-31.52	19.87	13.89
	Max. H _z	6	-31.52	19.87	13.89
	Min. Vert	6	-31.52	19.87	13.89
	Min. H _x	3	-7.71	3.81	2.95
	Min. H _z	3	-7.71	3.81	2.95
Guy A @ 250 ft Elev 0 ft Azimuth 0 deg	Max. Vert	2	-5.36	-0.00	-3.03
	Max. H _x	4	-34.31	0.02	-25.98
	Max. H _z	2	-5.36	-0.00	-3.03
	Min. Vert	8	-39.15	0.01	-30.43
	Min. H _x	7	-25.03	-2.50	-18.37
	Min. H _z	8	-39.15	0.01	-30.43
Mast	Max. Vert	6	383.27	-0.04	0.61
	Max. H _x	4	269.08	-0.01	-1.02
	Max. H _z	7	371.06	-1.18	0.72
	Max. M _x	1	0.00	-0.02	0.04
	Max. M _z	1	0.00	-0.02	0.04

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Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Max. Torsion	1	0.00	-0.02	0.04
	Min. Vert	1	199.82	-0.02	0.04
	Min. H _x	7	371.06	-1.18	0.72
	Min. H _z	8	357.45	-0.05	-1.58
	Min. M _x	1	0.00	-0.02	0.04
	Min. M _z	1	0.00	-0.02	0.04
	Min. Torsion	1	0.00	-0.02	0.04

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overtuning Moment, M _x kip-ft	Overtuning Moment, M _z kip-ft	Torque kip-ft
Dead Only	199.82	0.02	-0.04	0.00	0.00	0.00
Dead+Wind 0 deg - No Ice+Guy	289.38	0.02	-0.38	0.00	0.00	0.00
Dead+Wind 90 deg - No Ice+Guy	281.56	0.65	-0.47	0.00	0.00	0.00
Dead+Wind 180 deg - No Ice+Guy	269.08	0.01	1.02	0.00	0.00	0.00
Dead+Ice+Temp+Guy	253.74	0.06	-0.11	0.00	0.00	0.00
Dead+Wind 0 deg+Ice+Temp+Guy	383.27	0.04	-0.61	0.00	0.00	0.00
Dead+Wind 90 deg+Ice+Temp+Guy	371.06	1.18	-0.72	0.00	0.00	0.00
Dead+Wind 180 deg+Ice+Temp+Guy	357.45	0.05	1.58	0.00	0.00	0.00
Dead+Wind 0 deg - Service+Guy	202.84	0.02	-0.56	0.00	0.00	0.00
Dead+Wind 90 deg - Service+Guy	203.29	0.53	-0.01	0.00	0.00	0.00
Dead+Wind 180 deg - Service+Guy	203.82	0.02	0.44	0.00	0.00	0.00

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-69.16	0.00	-0.00	69.16	-0.00	0.001%
2	0.00	-69.84	-97.40	-0.00	69.84	97.39	0.003%
3	96.71	-69.16	-0.00	-96.71	69.16	0.00	0.002%
4	-0.00	-68.47	96.50	0.00	68.47	-96.50	0.000%
5	0.00	-102.80	0.00	-0.00	102.80	0.00	0.002%
6	0.00	-103.99	-130.18	-0.00	103.99	130.18	0.002%
7	125.83	-102.80	-0.00	-125.83	102.80	0.00	0.001%
8	-0.00	-101.60	124.02	-0.00	101.60	-124.02	0.001%
9	0.00	-69.35	-26.98	-0.00	69.35	26.98	0.001%
10	26.79	-69.16	-0.00	-26.79	69.16	0.00	0.001%
11	-0.00	-68.97	26.73	-0.00	68.97	-26.73	0.006%

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Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	7	0.00000001	0.00001645
2	Yes	14	0.00005755	0.00008993
3	Yes	14	0.00000001	0.00006183
4	Yes	10	0.00000001	0.00007565
5	Yes	6	0.00000001	0.00001237
6	Yes	15	0.00000001	0.00006124
7	Yes	15	0.00000001	0.00004432
8	Yes	11	0.00000001	0.00003926
9	Yes	9	0.00000001	0.00005026
10	Yes	9	0.00000001	0.00004221
11	Yes	8	0.00000001	0.00007717

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	368.75 - 362.5	2.390	11	0.0405	0.1005
T2	362.5 - 356.25	2.416	11	0.0405	0.0999
T3	356.25 - 350	2.443	11	0.0406	0.1000
T4	350 - 343.75	2.470	11	0.0421	0.0975
T5	343.75 - 337.5	2.504	11	0.0434	0.0968
T6	337.5 - 331.25	2.536	11	0.0417	0.0826
T7	331.25 - 325	2.565	11	0.0398	0.0674
T8	325 - 318.75	2.591	11	0.0378	0.0530
T9	318.75 - 312.5	2.613	11	0.0362	0.0448
T10	312.5 - 306.25	2.631	11	0.0348	0.0369
T11	306.25 - 300	2.648	11	0.0336	0.0290
T12	300 - 293.75	2.655	11	0.0333	0.0217
T13	293.75 - 287.5	2.679	11	0.0328	0.0216
T14	287.5 - 281.25	2.702	11	0.0288	0.0197
T15	281.25 - 275	2.720	11	0.0242	0.0178
T16	275 - 268.75	2.730	11	0.0191	0.0239
T17	268.75 - 262.5	2.730	11	0.0136	0.0316
T18	262.5 - 256.25	2.722	11	0.0101	0.0385
T19	256.25 - 250	2.704	11	0.0156	0.0444
T20	250 - 243.75	2.678	11	0.0209	0.0480
T21	243.75 - 237.5	2.644	11	0.0258	0.0498
T22	237.5 - 231.25	2.594	11	0.0297	0.0508
T23	231.25 - 225	2.536	11	0.0318	0.0505
T24	225 - 218.75	2.475	11	0.0317	0.0490
T25	218.75 - 212.5	2.437	11	0.0311	0.0493
T26	212.5 - 206.25	2.403	11	0.0342	0.0629
T27	206.25 - 200	2.361	11	0.0379	0.0752
T28	200 - 175	2.312	11	0.0419	0.0857
T29	175 - 168.75	2.031	11	0.0558	0.1114
T30	168.75 - 162.5	1.943	11	0.0568	0.1113
T31	162.5 - 156.25	1.863	11	0.0565	0.1107
T32	156.25 - 150	1.796	11	0.0570	0.1088
T33	150 - 125	1.727	11	0.0580	0.1054
T34	125 - 100	1.404	11	0.0624	0.0737
T35	100 - 93.75	1.066	11	0.0572	0.0554
T36	93.75 - 87.5	0.997	9	0.0541	0.0556
T37	87.5 - 81.25	0.937	9	0.0528	0.0664

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T38	81.25 - 75	0.878	9	0.0521	0.0805
T39	75 - 50	0.816	9	0.0518	0.0934
T40	50 - 25	0.541	9	0.0503	0.1303
T41	25 - 0	0.294	9	0.0518	0.1524

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
370.00	Search Antenna	11	2.390	0.0405	0.1005	358632
355.00	9 FT DISH	11	2.448	0.0408	0.0995	Inf
350.00	Guy	11	2.470	0.0421	0.0975	47046
325.00	3'-6" Standoff Mount	11	2.591	0.0378	0.0530	123771
305.25	BXA-70063/6CF	11	2.649	0.0335	0.0276	50014
304.25	13' T-Frame Sector Mount	11	2.650	0.0334	0.0262	137695
300.00	Guy	11	2.655	0.0333	0.0217	22367
250.00	6'8"x4" Pipe Mount	11	2.678	0.0209	0.0480	66453
242.50	12' T-Frame Sector Mount	11	2.635	0.0267	0.0501	28149
225.00	Guy	11	2.475	0.0317	0.0490	17339
200.00	54"x13"x3" Yagi	11	2.312	0.0419	0.0857	53771
180.00	(4) 5"x13"x3" Yagi	11	2.097	0.0541	0.1095	54784
162.50	Guy	11	1.863	0.0565	0.1107	30582
148.00	54"x13"x3" Yagi	11	1.703	0.0584	0.1037	78943
140.00	8"x33"x4" Yagi	11	1.605	0.0601	0.0945	93940
125.00	49"x13"x1" Yagi	11	1.404	0.0624	0.0737	152458
100.00	Guy	11	1.066	0.0572	0.0554	56948
88.00	X-Style	9	0.942	0.0528	0.0653	294262
62.00	54"x13"x3" Yagi	9	0.673	0.0509	0.1152	841294
50.00	Guy	9	0.541	0.0503	0.1303	88792
40.00	54"x12"x3" Yagi	9	0.442	0.0504	0.1400	346415

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	368.75 - 362.5	15.837	2	0.4325	0.4629
T2	362.5 - 356.25	15.976	2	0.4327	0.4607
T3	356.25 - 350	16.117	2	0.4333	0.4609
T4	350 - 343.75	16.255	2	0.4382	0.4519
T5	343.75 - 337.5	16.425	2	0.4411	0.4493
T6	337.5 - 331.25	16.809	6	0.4319	0.3928
T7	331.25 - 325	17.387	6	0.4207	0.3328
T8	325 - 318.75	17.942	6	0.4081	0.2757
T9	318.75 - 312.5	18.472	6	0.3966	0.2422
T10	312.5 - 306.25	18.982	6	0.3850	0.2103
T11	306.25 - 300	19.475	6	0.3736	0.1779
T12	300 - 293.75	19.924	6	0.3642	0.1379
T13	293.75 - 287.5	20.418	6	0.3535	0.1372
T14	287.5 - 281.25	20.916	6	0.3276	0.1297
T15	281.25 - 275	21.393	6	0.2974	0.1180

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T16	275 - 268.75	21.805	6	0.2638	0.1394
T17	268.75 - 262.5	22.152	6	0.2274	0.1749
T18	262.5 - 256.25	22.440	6	0.1894	0.2069
T19	256.25 - 250	22.667	6	0.1506	0.2354
T20	250 - 243.75	22.834	6	0.1501	0.2533
T21	243.75 - 237.5	22.944	6	0.1678	0.2645
T22	237.5 - 231.25	22.953	6	0.1822	0.2608
T23	231.25 - 225	22.881	6	0.1901	0.2418
T24	225 - 218.75	22.766	6	0.1902	0.2117
T25	218.75 - 212.5	22.759	6	0.1873	0.2129
T26	212.5 - 206.25	22.795	6	0.2022	0.2989
T27	206.25 - 200	22.743	6	0.2251	0.4066
T28	200 - 175	22.590	6	0.2492	0.4871
T29	175 - 168.75	21.033	6	0.3505	0.6903
T30	168.75 - 162.5	20.404	6	0.3805	0.6815
T31	162.5 - 156.25	19.828	6	0.4039	0.6658
T32	156.25 - 150	19.319	6	0.4310	0.6676
T33	150 - 125	18.751	6	0.4598	0.6519
T34	125 - 100	15.818	6	0.5635	0.4396
T35	100 - 93.75	12.463	6	0.5786	0.2479
T36	93.75 - 87.5	11.724	6	0.5683	0.2487
T37	87.5 - 81.25	11.041	6	0.5683	0.3402
T38	81.25 - 75	10.355	6	0.5715	0.4599
T39	75 - 50	9.627	6	0.5767	0.5606
T40	50 - 25	6.347	6	0.5884	0.8402
T41	25 - 0	3.449	6	0.6068	1.0752

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
370.00	Search Antenna	2	15.837	0.4325	0.4629	72099
355.00	9 FT DISH	2	16.144	0.4340	0.4591	149983
350.00	Guy	2	16.255	0.4382	0.4519	12800
325.00	3'-6" Standoff Mount	6	17.942	0.4081	0.2757	18551
305.25	BXA-70063/6CF	6	19.548	0.3719	0.1709	11147
304.25	13' T-Frame Sector Mount	6	19.620	0.3703	0.1635	19706
300.00	Guy	6	19.924	0.3642	0.1379	5453
250.00	6'8"x4" Pipe Mount	6	22.834	0.1501	0.2533	9078
242.50	12' T-Frame Sector Mount	6	22.954	0.1710	0.2651	4404
225.00	Guy	6	22.766	0.1902	0.2117	2865
200.00	54"x13"x3" Yagi	6	22.590	0.2492	0.4871	5366
180.00	(4) 5'x13"x3" Yagi	6	21.478	0.3207	0.6766	4203
162.50	Guy	6	19.828	0.4039	0.6658	4009
148.00	54"x13"x3" Yagi	6	18.552	0.4692	0.6420	6588
140.00	8'x33"x4" Yagi	6	17.687	0.5063	0.5852	7888
125.00	49"x13"x1" Yagi	6	15.818	0.5635	0.4396	13061
100.00	Guy	6	12.463	0.5786	0.2479	7205
88.00	X-Style	6	11.095	0.5681	0.3308	27560
62.00	54"x13"x3" Yagi	6	7.927	0.5837	0.7215	72622
50.00	Guy	6	6.347	0.5884	0.8402	7390
40.00	54"x12"x3" Yagi	6	5.176	0.5947	0.9345	29240

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Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T1	368.75	Leg	A325N	0.7500	6	0.06	19.44	0.003	✓	1.333 Bolt Tension
		Diagonal	A325N	0.5000	2	0.24	4.12	0.058	✓	1.333 Bolt Shear
		Top Girt	A325N	0.6250	2	0.06	12.89	0.004	✓	1.333 Bolt Shear
T2	362.5	Diagonal	A325N	0.5000	2	0.44	8.25	0.053	✓	1.333 Bolt Shear
		Top Girt	A325N	0.5000	2	0.16	8.25	0.020	✓	1.333 Bolt Shear
T3	356.25	Diagonal	A325N	0.5000	2	2.72	8.25	0.329	✓	1.333 Bolt Shear
		Top Girt	A325N	0.5000	2	0.68	8.25	0.083	✓	1.333 Bolt Shear
T4	350	Leg	A325N	0.7500	6	0.98	19.44	0.051	✓	1.333 Bolt Tension
		Diagonal	A325N	0.6250	2	1.95	6.44	0.303	✓	1.333 Bolt Shear
		Top Girt	A325N	0.6250	2	4.86	12.89	0.377	✓	1.333 Bolt Shear
T5	343.75	Diagonal	A325N	0.5000	2	2.04	4.12	0.496	✓	1.333 Bolt Shear
		Top Girt	A325N	0.6250	2	2.08	12.89	0.161	✓	1.333 Bolt Shear
T6	337.5	Diagonal	A325N	0.5000	2	1.85	4.12	0.450	✓	1.333 Bolt Shear
		Top Girt	A325N	0.5000	2	1.26	4.12	0.307	✓	1.333 Bolt Shear
T7	331.25	Diagonal	A325N	0.5000	2	1.63	4.12	0.396	✓	1.333 Bolt Shear
		Top Girt	A325N	0.5000	2	1.22	4.12	0.296	✓	1.333 Bolt Shear
T8	325	Leg	A325N	0.7500	6	0.00	19.44	0.000	✓	1.333 Bolt Tension
		Diagonal	A325N	0.5000	2	1.84	4.12	0.447	✓	1.333 Bolt Shear
		Top Girt	A325N	0.5000	2	1.47	4.12	0.356	✓	1.333 Bolt Shear
T9	318.75	Leg	A325N	0.7500	6	0.00	19.44	0.000	✓	1.333 Bolt Tension
		Diagonal	A325N	0.5000	2	1.94	4.12	0.470	✓	1.333 Bolt Shear
		Top Girt	A325N	0.5000	2	1.67	4.12	0.404	✓	1.333 Bolt Shear
T10	312.5	Leg	A325N	0.7500	6	0.00	19.44	0.000	✓	1.333 Bolt Tension
		Diagonal	A325N	0.5000	2	2.12	4.12	0.515	✓	1.333 Bolt Shear
		Top Girt	A325N	0.5000	2	1.71	4.12	0.416	✓	1.333 Bolt Shear
T11	306.25	Diagonal	A325N	0.5000	2	3.67	4.12	0.891	✓	1.333 Bolt Shear
		Top Girt	A325N	0.6250	2	2.35	12.89	0.183	✓	1.333 Bolt Shear
T12	300	Leg	A325N	0.7500	6	0.00	19.44	0.000	✓	1.333 Bolt Tension
		Diagonal	A325N	0.6250	2	3.98	6.44	0.617	✓	1.333 Bolt Shear
		Top Girt	A325N	0.6250	2	5.86	12.89	0.455	✓	1.333 Bolt Shear
T13	293.75	Diagonal	A325N	0.5000	2	3.47	4.12	0.842	✓	1.333 Bolt Shear
		Top Girt	A325N	0.6250	2	1.41	12.89	0.109	✓	1.333 Bolt Shear
T14	287.5	Diagonal	A325N	0.5000	2	2.89	4.12	0.701	✓	1.333 Bolt Shear
		Top Girt	A325N	0.5000	2	1.98	4.12	0.480	✓	1.333 Bolt Shear
T15	281.25	Diagonal	A325N	0.5000	2	2.30	4.12	0.558	✓	1.333 Bolt Shear
		Top Girt	A325N	0.5000	2	1.64	4.12	0.398	✓	1.333 Bolt Shear
T16	275	Leg	A325N	0.7500	6	1.43	19.44	0.074	✓	1.333 Bolt Tension
		Diagonal	A325N	0.5000	2	1.66	4.12	0.402	✓	1.333 Bolt Shear

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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T17	268.75	Top Girt	A325N	0.5000	2	1.26	4.12	0.304 ✓	1.333	Bolt Shear
		Leg	A325N	0.7500	6	1.92	19.44	0.099 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	1.22	4.12	0.295 ✓	1.333	Bolt Shear
T18	262.5	Top Girt	A325N	0.5000	2	1.18	4.12	0.286 ✓	1.333	Bolt Shear
		Leg	A325N	0.7500	6	2.15	19.44	0.110 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	1.02	4.12	0.248 ✓	1.333	Bolt Shear
T19	256.25	Top Girt	A325N	0.5000	2	1.17	4.12	0.283 ✓	1.333	Bolt Shear
		Diagonal	A325N	0.5000	2	1.55	4.12	0.376 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.5000	2	1.42	4.12	0.345 ✓	1.333	Bolt Shear
T20	250	Leg	A325N	0.7500	6	1.50	19.44	0.077 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	1.67	4.12	0.405 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.5000	2	1.72	8.25	0.208 ✓	1.333	Bolt Shear
T21	243.75	Diagonal	A325N	0.5000	2	3.94	4.12	0.956 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.6250	2	2.17	12.89	0.168 ✓	1.333	Bolt Shear
		Diagonal	A325X	0.5000	2	5.57	5.89	0.946 ✓	1.333	Bolt Shear
T22	237.5	Top Girt	A325N	0.6250	2	2.99	12.89	0.232 ✓	1.333	Bolt Shear
		Diagonal	A325X	0.5000	2	6.15	5.89	1.045 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.6250	2	3.67	12.89	0.285 ✓	1.333	Bolt Shear
T23	231.25	Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.6250	2	7.67	12.89	0.595 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.6250	2	9.12	12.89	0.708 ✓	1.333	Bolt Shear
T24	225	Diagonal	A325N	0.5000	2	3.83	4.12	0.929 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.6250	2	3.10	12.89	0.241 ✓	1.333	Bolt Shear
		Diagonal	A325N	0.5000	2	2.95	4.12	0.715 ✓	1.333	Bolt Shear
T25	218.75	Top Girt	A325N	0.5000	2	2.02	4.12	0.491 ✓	1.333	Bolt Shear
		Diagonal	A325N	0.5000	2	1.79	4.12	0.435 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.5000	2	1.40	4.12	0.339 ✓	1.333	Bolt Shear
T26	212.5	Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	3.04	4.12	0.738 ✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	1.62	6.44	0.252 ✓	1.333	Bolt Shear
T27	206.25	Top Girt	A325N	0.5000	2	0.78	4.12	0.190 ✓	1.333	Bolt Shear
		Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	4.11	4.12	0.998 ✓	1.333	Bolt Shear
T28	200	Top Girt	A325N	0.5000	2	2.28	4.12	0.553 ✓	1.333	Bolt Shear
		Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	4.96	4.12	1.203 ✓	1.333	Bolt Shear
T29	175	Top Girt	A325N	0.5000	2	2.83	4.12	0.686 ✓	1.333	Bolt Shear
		Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	1.94	4.12	0.469 ✓	1.333	Bolt Shear
T30	168.75	Top Girt	A325N	0.5000	2	4.45	8.25	0.539 ✓	1.333	Bolt Shear
		Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	1.94	4.12	0.469 ✓	1.333	Bolt Shear
T31	162.5	Top Girt	A325N	0.5000	2	4.45	8.25	0.539 ✓	1.333	Bolt Shear
		Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	1.94	4.12	0.469 ✓	1.333	Bolt Shear

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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T32	156.25	Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	1.40	4.12	0.340 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.5000	2	0.97	4.12	0.236 ✓	1.333	Bolt Shear
T33	150	Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	3.38	4.12	0.819 ✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	1.95	6.44	0.303 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.5000	2	0.84	4.12	0.203 ✓	1.333	Bolt Shear
T34	125	Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325X	0.5000	2	7.09	5.89	1.204 ✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	4.18	6.44	0.649 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.5000	2	2.50	4.12	0.607 ✓	1.333	Bolt Shear
T35	100	Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.6250	2	8.18	12.89	0.635 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.6250	2	9.52	12.89	0.739 ✓	1.333	Bolt Shear
T36	93.75	Diagonal	A325N	0.5000	2	3.77	4.12	0.915 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.6250	2	3.50	12.89	0.272 ✓	1.333	Bolt Shear
T37	87.5	Diagonal	A325N	0.5000	2	2.84	4.12	0.688 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.5000	2	2.06	4.12	0.499 ✓	1.333	Bolt Shear
T38	81.25	Diagonal	A325N	0.5000	2	1.81	4.12	0.438 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.5000	2	1.46	4.12	0.354 ✓	1.333	Bolt Shear
T39	75	Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	2.42	4.12	0.587 ✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	1.41	6.44	0.219 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.5000	2	0.80	4.12	0.195 ✓	1.333	Bolt Shear
T40	50	Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	2.99	4.12	0.724 ✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	1.69	6.44	0.263 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.5000	2	3.28	4.12	0.795 ✓	1.333	Bolt Shear
T41	25	Leg	A325N	0.7500	6	0.00	19.44	0.000 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	2	2.06	4.12	0.500 ✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	1.17	6.44	0.181 ✓	1.333	Bolt Shear
		Top Girt	A325N	0.5000	2	0.70	4.12	0.171 ✓	1.333	Bolt Shear

Guy Design Data

Section No.	Elevation ft	Size	Initial Tension K	Breaking Load K	Actual T K	Allowable T _a K	Required S.F.	Actual S.F.
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Section No.	Elevation ft	Size	Initial Tension K	Breaking Load K	Actual T K	Allowable T _a K	Required S.F.	Actual S.F.
T4	350.00 (A) (748)	7/8 EHS	7.97	79.70	25.28	39.85	2.000	3.152 ✓
	350.00 (A) (749)	7/8 EHS	7.97	79.70	25.99	39.85	2.000	3.066 ✓
	350.00 (B) (742)	7/8 EHS	7.97	79.70	20.36	39.85	2.000	3.914 ✓
	350.00 (B) (743)	7/8 EHS	7.97	79.70	21.08	39.85	2.000	3.781 ✓
	350.00 (C) (736)	7/8 EHS	7.97	79.70	23.91	39.85	2.000	3.333 ✓
	350.00 (C) (737)	7/8 EHS	7.97	79.70	24.38	39.85	2.000	3.269 ✓
T12	300.00 (A) (730)	7/8 EHS	7.97	79.70	27.45	39.85	2.000	2.904 ✓
	300.00 (A) (731)	7/8 EHS	7.97	79.70	27.70	39.85	2.000	2.878 ✓
	300.00 (B) (724)	7/8 EHS	7.97	79.70	21.03	39.85	2.000	3.790 ✓
	300.00 (B) (725)	7/8 EHS	7.97	79.70	22.76	39.85	2.000	3.502 ✓
	300.00 (C) (718)	7/8 EHS	7.97	79.70	25.48	39.85	2.000	3.128 ✓
	300.00 (C) (719)	7/8 EHS	7.97	79.70	26.68	39.85	2.000	2.987 ✓
T24	225.00 (A) (712)	3/4 EHS	5.83	58.30	24.41	29.15	2.000	2.388 ✓
	225.00 (A) (713)	3/4 EHS	5.83	58.30	24.79	29.15	2.000	2.352 ✓
	225.00 (B) (706)	3/4 EHS	5.83	58.30	20.11	29.15	2.000	2.900 ✓
	225.00 (B) (707)	3/4 EHS	5.83	58.30	21.02	29.15	2.000	2.774 ✓
	225.00 (C) (700)	3/4 EHS	5.83	58.30	23.31	29.15	2.000	2.501 ✓
	225.00 (C) (701)	3/4 EHS	5.83	58.30	24.66	29.15	2.000	2.365 ✓
T31	162.50 (A) (699)	3/4 EHS	5.83	58.30	26.29	29.15	2.000	2.218 ✓
	162.50 (B) (698)	3/4 EHS	5.83	58.30	22.99	29.15	2.000	2.536 ✓
	162.50 (C) (697)	3/4 EHS	5.83	58.30	26.37	29.15	2.000	2.211 ✓
T35	100.00 (A) (691)	9/16 EHS	3.50	35.00	15.06	17.50	2.000	2.324 ✓
	100.00 (A) (692)	9/16 EHS	3.50	35.00	15.65	17.50	2.000	2.237 ✓
	100.00 (B) (685)	9/16 EHS	3.50	35.00	14.39	17.50	2.000	2.432 ✓
	100.00 (B) (686)	9/16 EHS	3.50	35.00	13.95	17.50	2.000	2.509 ✓
	100.00 (C) (679)	9/16 EHS	3.50	35.00	15.37	17.50	2.000	2.277 ✓
	100.00 (C) (680)	9/16 EHS	3.50	35.00	16.35	17.50	2.000	2.140 ✓
T40	50.00 (A) (678)	9/16 EHS	3.50	35.00	13.74	17.50	2.000	2.547 ✓
	50.00 (B) (677)	9/16 EHS	3.50	35.00	12.73	17.50	2.000	2.750 ✓
	50.00 (C) (676)	9/16 EHS	3.50	35.00	14.34	17.50	2.000	2.440 ✓

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Compression Checks

Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _n ft	Kl/r	Mast Stability Index	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	368.75 - 362.5	2 3/4	6.25	6.25	109.1 K=1.00	1.00	11.794	5.9396	-0.74	70.05	0.011*
T2	362.5 - 356.25	2 3/4	6.25	6.25	109.1 K=1.00	1.00	11.794	5.9396	-1.51	70.05	0.022
T3	356.25 - 350	2 3/4	6.25	6.25	109.1 K=1.00	1.00	11.794	5.9396	-9.11	70.05	0.130
T4	350 - 343.75	3	6.25	6.25	100.0 K=1.00	1.00	12.978	7.0686	-8.47	91.73	0.092
T5	343.75 - 337.5	3	6.25	6.25	100.0 K=1.00	1.00	12.978	7.0686	-44.97	91.73	0.490
T6	337.5 - 331.25	3	6.25	6.25	100.0 K=1.00	1.00	12.978	7.0686	-46.13	91.73	0.503
T7	331.25 - 325	3	6.25	6.25	100.0 K=1.00	1.00	12.978	7.0686	-46.62	91.73	0.508
T8	325 - 318.75	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-47.42	115.55	0.410
T9	318.75 - 312.5	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-45.77	115.55	0.396
T10	312.5 - 306.25	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-45.72	115.55	0.396
T11	306.25 - 300	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-45.42	115.55	0.393
T12	300 - 293.75	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-38.53	115.55	0.333
T13	293.75 - 287.5	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-95.84	115.55	0.829
T14	287.5 - 281.25	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-101.09	115.55	0.875
T15	281.25 - 275	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-104.95	115.55	0.908
T16	275 - 268.75	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-107.82	115.55	0.933
T17	268.75 - 262.5	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-109.40	115.55	0.947
T18	262.5 - 256.25	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-109.48	115.55	0.948
T19	256.25 - 250	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-108.86	115.55	0.942
T20	250 - 243.75	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-106.14	115.55	0.919
T21	243.75 - 237.5	3 1/4	6.25	6.25	92.3 K=1.00	1.00	13.928	8.2958	-102.89	115.55	0.890
T22	237.5 - 231.25	3 1/4	6.25	6.25	92.3	1.00	13.928	8.2958	-95.88	115.55	0.830

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	Mast Stability Index	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T23	231.25 - 225	3 1/4	6.25	6.25	K=1.00 92.3	1.00	13.928	8.2958	-88.89	115.55	0.769 ✓
T24	225 - 218.75	3	6.25	6.25	K=1.00 100.0	1.00	12.978	7.0686	-68.31	91.73	0.745 ✓
T25	218.75 - 212.5	3	6.25	3.13	K=1.00 50.0	1.00	18.351	7.0686	-123.49	129.71	0.952 ✓
T26	212.5 - 206.25	3	6.25	3.13	K=1.00 50.0	1.00	18.351	7.0686	-127.24	129.71	0.981 ✓
T27	206.25 - 200	3	6.25	3.13	K=1.00 50.0	1.00	18.351	7.0686	-129.64	129.71	0.999 ✓
T28	200 - 175	3	25.00	3.13	K=1.00 50.0	1.00	18.351	7.0686	-131.19	129.71	1.011 ✓
T29	175 - 168.75	3 1/4	6.25	6.25	K=1.00 92.3	1.00	13.928	8.2958	-123.18	115.55	1.066 ✓
T30	168.75 - 162.5	3 1/4	6.25	6.25	K=1.00 92.3	1.00	13.928	8.2958	-118.46	115.55	1.025 ✓
T31	162.5 - 156.25	3 1/4	6.25	6.25	K=1.00 92.3	1.00	13.928	8.2958	-129.74	115.55	1.123 ✓
T32	156.25 - 150	3 1/4	6.25	6.25	K=1.00 92.3	1.00	13.928	8.2958	-131.18	115.55	1.135 ✓
T33	150 - 125	3 1/4	25.00	6.25	K=1.00 92.3	1.00	13.928	8.2958	-132.41	115.55	1.146 ✓
T34	125 - 100	3 1/4	25.00	6.25	K=1.00 92.3	1.00	13.928	8.2958	-130.63	115.55	1.131 ✓
T35	100 - 93.75	3 1/4	6.25	6.25	K=1.00 92.3	1.00	13.928	8.2958	-106.97	115.55	0.926 ✓
T36	93.75 - 87.5	3 1/4	6.25	6.25	K=1.00 92.3	1.00	13.928	8.2958	-121.20	115.55	1.049 ✓
T37	87.5 - 81.25	3 1/4	6.25	6.25	K=1.00 92.3	1.00	13.928	8.2958	-123.75	115.55	1.071 ✓
T38	81.25 - 75	3 1/4	6.25	6.25	K=1.00 92.3	1.00	13.928	8.2958	-125.11	115.55	1.083 ✓
T39	75 - 50	3 1/4	25.00	6.25	K=1.00 92.3	1.00	13.928	8.2958	-125.51	115.55	1.086 ✓
T40	50 - 25	3 1/4	25.00	6.25	K=1.00 92.3	1.00	13.928	8.2958	-134.38	115.55	1.163 ✓
T41	25 - 0	3 1/4	25.00	6.25	K=1.00 92.3	1.00	13.928	8.2958	-134.61	115.55	1.165 ✓

* DL controls

Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	368.75 - 362.5	L2 1/2x2 1/2x1/4	8.00	3.64	96.7 K=1.09	13.385	1.1900	-0.48	15.93	0.030
T2	362.5 - 356.25	2L3x3x5/16	8.00	3.64	65.5 K=1.38	16.885	3.5500	-0.88	59.94	0.015
T3	356.25 - 350	2L3x3x5/16	8.00	3.64	65.5 K=1.38	16.885	3.5500	-5.20	59.94	0.087
T4	350 - 343.75	L3x2 1/2x1/4	8.00	3.60	91.4 K=1.12	14.032	1.3100	-3.90	18.38	0.212
T12	300 - 293.75	L3x2 1/2x1/4	8.00	3.57	90.9 K=1.12	14.100	1.3100	-7.95	18.47	0.430
T24	225 - 218.75	2L2 1/2x2 1/2x1/4	8.00	3.59	72.0 K=1.29	16.222	2.3800	-15.34	38.61	0.397
T35	100 - 93.75	2L2 1/2x2 1/2x1/4	8.00	3.57	71.8 K=1.29	16.243	2.3800	-16.36	38.66	0.423

Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T28	200 - 175	P1.25x.14	5.00	4.75	105.6 K=1.00	12.254	0.6685	-3.24	8.19	0.396
T33	150 - 125	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-3.91	8.23	0.475
T34	125 - 100	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-8.36	8.23	1.015
T39	75 - 50	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-2.83	8.23	0.343
T40	50 - 25	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-3.39	8.23	0.412
T41	25 - 0	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-2.33	8.23	0.283

Secondary Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T25	218.75 - 212.5	L2x2x1/4	5.00	4.75	135.9 K=0.93	8.091	0.9380	-2.14	7.59	0.282
T26	212.5 - 206.25	L2x2x1/4	5.00	4.75	135.9 K=0.93	8.091	0.9380	-2.20	7.59	0.290
T27	206.25 - 200	L2x2x1/4	5.00	4.75	135.9 K=0.93	8.091	0.9380	-2.25	7.59	0.296
T28	200 - 175	P1.25x.14	5.00	4.75	105.6 K=1.00	12.254	0.6685	-2.27	8.19	0.277

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Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	368.75 - 362.5	2L2 1/2x2x1/4	5.00	4.34	93.2 K=1.40	13.815	2.1300	-0.09	29.43	0.003
T2	362.5 - 356.25	2L2 1/2x3x1/4	5.00	4.42	95.2 K=1.35	13.577	2.6300	-0.18	35.71	0.005
T3	356.25 - 350	2L2 1/2x3x1/4	5.00	4.42	95.2 K=1.35	13.577	2.6300	-1.30	35.71	0.036
T4	350 - 343.75	2L2 1/2x2x1/4	5.00	4.34	93.2 K=1.40	13.815	2.1300	-6.92	29.43	0.235
T5	343.75 - 337.5	2L2 1/2x2x1/4	5.00	4.32	93.1 K=1.41	13.835	2.1300	-4.16	29.47	0.141
T6	337.5 - 331.25	P1.25x.14	5.00	4.75	105.6 K=1.00	12.254	0.6685	-2.53	8.19	0.309*
T7	331.25 - 325	P1.25x.14	5.00	4.75	105.6 K=1.00	12.254	0.6685	-2.41	8.19	0.295*
T8	325 - 318.75	P1.25x.14	5.00	4.75	105.6 K=1.00	12.254	0.6685	-2.88	8.19	0.351*
T9	318.75 - 312.5	P1.25x.14	5.00	4.75	105.6 K=1.00	12.254	0.6685	-3.32	8.19	0.406*
T10	312.5 - 306.25	P1.25x.14	5.00	4.75	105.6 K=1.00	12.254	0.6685	-3.43	8.19	0.418*
T11	306.25 - 300	2L2 1/2x2x1/4	5.00	4.30	92.9 K=1.41	13.854	2.1300	-3.84	29.51	0.130*
T12	300 - 293.75	2L2 1/2x2x1/4	5.00	4.30	92.9 K=1.41	13.854	2.1300	-7.76	29.51	0.263
T13	293.75 - 287.5	2L2 1/2x2x1/4	5.00	4.30	92.9 K=1.41	13.854	2.1300	-2.82	29.51	0.095
T14	287.5 - 281.25	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-3.96	8.23	0.480
T15	281.25 - 275	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-3.28	8.23	0.398
T16	275 - 268.75	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-2.12	8.23	0.258*
T17	268.75 - 262.5	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-2.12	8.23	0.257*
T18	262.5 - 256.25	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-2.09	8.23	0.254*
T19	256.25 - 250	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-2.57	8.23	0.312*
T20	250 - 243.75	2L2 1/2x2x1/4	5.00	4.38	93.5 K=1.40	13.786	2.1300	-3.20	29.36	0.109*
T21	243.75 - 237.5	2L2 1/2x2x1/4	5.00	4.30	92.9 K=1.41	13.854	2.1300	-3.29	29.51	0.111*
T22	237.5 - 231.25	2L2 1/2x2x1/4	5.00	4.30	92.9 K=1.41	13.854	2.1300	-5.97	29.51	0.202
T23	231.25 - 225	2L2 1/2x2x1/4	5.00	4.30	92.9 K=1.41	13.854	2.1300	-7.34	29.51	0.249
T24	225 - 218.75	2L2 1/2x2x1/4	5.00	4.30	92.9 K=1.41	13.854	2.1300	-5.46	29.51	0.185

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T26	212.5 - 206.25	P1.25x.14	5.00	4.75	105.6 K=1.00	12.254	0.6685	-4.05	8.19	0.494
T27	206.25 - 200	P1.25x.14	5.00	4.75	105.6 K=1.00	12.254	0.6685	-2.80	8.19	0.342
T28	200 - 175	P1.25x.14	5.00	4.75	105.6 K=1.00	12.254	0.6685	-1.57	8.19	0.192*
T29	175 - 168.75	P1.25x.14	5.00	4.75	105.6 K=1.00	12.254	0.6685	-4.56	8.19	0.556
T30	168.75 - 162.5	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-5.66	8.23	0.688
T31	162.5 - 156.25	2L2 1/2x2x1/4	5.00	4.38	93.5 K=1.40	13.786	2.1300	-1.26	29.36	0.043
T32	156.25 - 150	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-1.74	8.23	0.212*
T33	150 - 125	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-1.67	8.23	0.203*
T34	125 - 100	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-5.00	8.23	0.608
T35	100 - 93.75	2L2 1/2x2x1/4	5.00	4.30	92.9 K=1.41	13.854	2.1300	-5.30	29.51	0.180
T37	87.5 - 81.25	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-4.12	8.23	0.500
T38	81.25 - 75	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-2.92	8.23	0.354
T39	75 - 50	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-1.49	8.23	0.181*
T40	50 - 25	L2 1/2x2x1/4	5.00	4.38	122.3 K=0.99	9.943	1.0600	-0.25	10.54	0.024
T41	25 - 0	P1.25x.14	5.00	4.73	105.2 K=1.00	12.315	0.6685	-1.41	8.23	0.171*

* DL controls

Torque-Arm Top Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T35	100 - 93.75 (681)	2L3x2 1/2x1/4	6.03	5.89	74.8 K=1.00	15.921	2.6300	-0.84	41.87	0.020
T35	100 - 93.75 (687)	2L3x2 1/2x1/4	6.03	5.89	74.8 K=1.00	15.921	2.6300	-0.58	41.87	0.014
T35	100 - 93.75 (693)	2L3x2 1/2x1/4	6.03	5.89	74.8 K=1.00	15.921	2.6300	-0.77	41.87	0.018

Torque-Arm Bottom Design Data

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T4	350 - 343.75 (740)	2L3x2 1/2x1/4	8.68	8.50	108.0 K=1.00	11.943	2.6300	-25.87	31.41	0.824 ✓
T4	350 - 343.75 (741)	2L3x2 1/2x1/4	8.68	8.50	108.0 K=1.00	11.943	2.6300	-26.80	31.41	0.853 ✓
T4	350 - 343.75 (746)	2L3x2 1/2x1/4	8.68	8.50	108.0 K=1.00	11.943	2.6300	-25.49	31.41	0.811 ✓
T4	350 - 343.75 (747)	2L3x2 1/2x1/4	8.68	8.50	108.0 K=1.00	11.943	2.6300	-22.97	31.41	0.731 ✓
T4	350 - 343.75 (752)	2L3x2 1/2x1/4	8.68	8.50	108.0 K=1.00	11.943	2.6300	-22.06	31.41	0.702 ✓
T4	350 - 343.75 (753)	2L3x2 1/2x1/4	8.68	8.50	108.0 K=1.00	11.943	2.6300	-27.40	31.41	0.872 ✓
T12	300 - 293.75 (722)	2L3x2 1/2x1/4	8.68	8.49	107.8 K=1.00	11.968	2.6300	-28.14	31.48	0.894 ✓
T12	300 - 293.75 (723)	2L3x2 1/2x1/4	8.68	8.49	107.8 K=1.00	11.968	2.6300	-30.02	31.48	0.954 ✓
T12	300 - 293.75 (728)	2L3x2 1/2x1/4	8.68	8.49	107.8 K=1.00	11.968	2.6300	-29.06	31.48	0.923 ✓
T12	300 - 293.75 (729)	2L3x2 1/2x1/4	8.68	8.49	107.8 K=1.00	11.968	2.6300	-24.53	31.48	0.779 ✓
T12	300 - 293.75 (734)	2L3x2 1/2x1/4	8.68	8.49	107.8 K=1.00	11.968	2.6300	-24.74	31.48	0.786 ✓
T12	300 - 293.75 (735)	2L3x2 1/2x1/4	8.68	8.49	107.8 K=1.00	11.968	2.6300	-30.17	31.48	0.958 ✓
T24	225 - 218.75 (704)	2L3x2 1/2x1/4	8.68	8.50	108.0 K=1.00	11.943	2.6300	-23.50	31.41	0.748 ✓
T24	225 - 218.75 (705)	2L3x2 1/2x1/4	8.68	8.50	108.0 K=1.00	11.943	2.6300	-25.04	31.41	0.797 ✓
T24	225 - 218.75 (710)	2L3x2 1/2x1/4	8.68	8.50	108.0 K=1.00	11.943	2.6300	-25.35	31.41	0.807 ✓
T24	225 - 218.75 (711)	2L3x2 1/2x1/4	8.68	8.50	108.0 K=1.00	11.943	2.6300	-21.43	31.41	0.682 ✓
T24	225 - 218.75 (716)	2L3x2 1/2x1/4	8.68	8.50	108.0 K=1.00	11.943	2.6300	-21.79	31.41	0.694 ✓
T24	225 - 218.75 (717)	2L3x2 1/2x1/4	8.68	8.50	108.0 K=1.00	11.943	2.6300	-24.42	31.41	0.777 ✓
T35	100 - 93.75 (683)	2L3x2 1/2x1/4	8.68	8.49	107.8 K=1.00	11.968	2.6300	-12.30	31.48	0.391 ✓
T35	100 - 93.75 (684)	2L3x2 1/2x1/4	8.68	8.49	107.8 K=1.00	11.968	2.6300	-12.88	31.48	0.409 ✓
T35	100 - 93.75 (689)	2L3x2 1/2x1/4	8.68	8.49	107.8 K=1.00	11.968	2.6300	-14.03	31.48	0.446 ✓
T35	100 - 93.75 (690)	2L3x2 1/2x1/4	8.68	8.49	107.8 K=1.00	11.968	2.6300	-11.10	31.48	0.353 ✓
T35	100 - 93.75 (695)	2L3x2 1/2x1/4	8.68	8.49	107.8 K=1.00	11.968	2.6300	-11.82	31.48	0.376 ✓
T35	100 - 93.75 (696)	2L3x2 1/2x1/4	8.68	8.49	107.8 K=1.00	11.968	2.6300	-12.49	31.48	0.397 ✓

Tension Checks

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Leg Design Data (Tension)										
Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	368.75 - 362.5	2 3/4	6.25	6.25	109.1	21.600	5.9396	0.35	128.29	0.003
T2	362.5 - 356.25	2 3/4	6.25	6.25	109.1	21.600	5.9396	0.89	128.29	0.007
T3	356.25 - 350	2 3/4	6.25	6.25	109.1	21.600	5.9396	6.38	128.29	0.050
T4	350 - 343.75	3	6.25	6.25	100.0	21.600	7.0686	5.89	152.68	0.039
T15	281.25 - 275	3 1/4	6.25	6.25	92.3	21.600	8.2958	3.41	179.19	0.019
T16	275 - 268.75	3 1/4	6.25	6.25	92.3	21.600	8.2958	8.58	179.19	0.048
T17	268.75 - 262.5	3 1/4	6.25	6.25	92.3	21.600	8.2958	11.51	179.19	0.064
T18	262.5 - 256.25	3 1/4	6.25	6.25	92.3	21.600	8.2958	12.88	179.19	0.072
T19	256.25 - 250	3 1/4	6.25	6.25	92.3	21.600	8.2958	11.31	179.19	0.063
T20	250 - 243.75	3 1/4	6.25	6.25	92.3	21.600	8.2958	9.02	179.19	0.050
T21	243.75 - 237.5	3 1/4	6.25	6.25	92.3	21.600	8.2958	0.40	179.19	0.002

Diagonal Design Data (Tension)										
Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	368.75 - 362.5	L2 1/2x2 1/2x1/4	8.00	3.64	59.6	29.000	0.7753	0.41	22.48	0.018
T2	362.5 - 356.25	2L3x3x5/16	8.00	3.64	49.7	29.000	2.3695	0.62	68.72	0.009
T3	356.25 - 350	2L3x3x5/16	8.00	3.64	49.7	29.000	2.3695	5.43	68.72	0.079
T4	350 - 343.75	L3x2 1/2x1/4	8.00	3.60	61.0	29.000	0.8419	3.60	24.41	0.148
T5	343.75 - 337.5	5/8	8.00	7.60	584.0	21.600	0.3068	4.09	6.63	0.617
T6	337.5 - 331.25	5/8	8.00	7.60	584.0	21.600	0.3068	3.71	6.63	0.560
T7	331.25 - 325	5/8	8.00	7.60	584.0	21.600	0.3068	3.26	6.63	0.493
T8	325 - 318.75	3/4	8.00	7.59	485.6	21.600	0.4418	3.69	9.54	0.387
T9	318.75 - 312.5	3/4	8.00	7.59	485.6	21.600	0.4418	3.88	9.54	0.407

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T10	312.5 - 306.25	3/4	8.00	7.59	485.6	21.600	0.4418	4.25	9.54	0.445
T11	306.25 - 300	3/4	8.00	7.57	484.5	21.600	0.4418	7.34	9.54	0.770
T12	300 - 293.75	L3x2 1/2x1/4	8.00	3.57	60.4	29.000	0.8419	3.01	24.41	0.123
T13	293.75 - 287.5	3/4	8.00	7.57	484.5	21.600	0.4418	6.94	9.54	0.727
T14	287.5 - 281.25	5/8	8.00	7.57	581.4	21.600	0.3068	5.78	6.63	0.872
T15	281.25 - 275	5/8	8.00	7.57	581.4	21.600	0.3068	4.60	6.63	0.694
T16	275 - 268.75	5/8	8.00	7.57	581.4	21.600	0.3068	3.31	6.63	0.500
T17	268.75 - 262.5	5/8	8.00	7.57	581.4	21.600	0.3068	2.43	6.63	0.367
T18	262.5 - 256.25	5/8	8.00	7.57	581.4	21.600	0.3068	1.67	6.63	0.252*
T19	256.25 - 250	3/4	8.00	7.57	484.5	21.600	0.4418	2.49	9.54	0.261*
T20	250 - 243.75	3/4	8.00	7.57	484.5	21.600	0.4418	2.67	9.54	0.280*
T21	243.75 - 237.5	3/4	8.00	7.57	484.5	21.600	0.4418	7.89	9.54	0.827
T22	237.5 - 231.25	3/4	8.00	7.57	484.5	21.600	0.4418	11.15	9.54	1.168
T23	231.25 - 225	3/4	8.00	7.57	484.5	21.600	0.4418	12.31	9.54	1.290
T25	218.75 - 212.5	5/8	8.00	7.60	584.0	21.600	0.3068	7.66	6.63	1.156
T26	212.5 - 206.25	5/8	8.00	7.60	584.0	21.600	0.3068	5.89	6.63	0.890
T27	206.25 - 200	5/8	8.00	7.60	584.0	21.600	0.3068	3.59	6.63	0.541
T28	200 - 175	5/8	8.00	7.60	584.0	21.600	0.3068	6.09	6.63	0.919
T29	175 - 168.75	5/8	8.00	7.59	582.7	21.600	0.3068	8.23	6.63	1.241
T30	168.75 - 162.5	1	8.00	7.57	363.4	21.600	0.7854	9.92	16.96	0.585
T31	162.5 - 156.25	5/8	8.00	7.57	581.4	21.600	0.3068	3.87	6.63	0.584
T32	156.25 - 150	5/8	8.00	7.57	581.4	21.600	0.3068	2.80	6.63	0.423
T33	150 - 125	5/8	8.00	7.57	581.4	21.600	0.3068	6.75	6.63	1.019
T34	125 - 100	L2 1/2x2 1/2x3/16	8.00	7.22	116.8	29.000	0.5886	14.18	17.07	0.831
T36	93.75 - 87.5	3/4	8.00	7.57	484.5	21.600	0.4418	7.54	9.54	0.790
T37	87.5 - 81.25	5/8	8.00	7.57	581.4	21.600	0.3068	5.68	6.63	0.856

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T38	81.25 - 75	5/8	8.00	7.57	581.4	21.600	0.3068	3.61	6.63	0.545 ✓
T39	75 - 50	5/8	8.00	7.57	581.4	21.600	0.3068	4.84	6.63	0.730 ✓
T40	50 - 25	5/8	8.00	7.57	581.4	21.600	0.3068	5.97	6.63	0.901 ✓
T41	25 - 0	5/8	8.00	7.57	581.4	21.600	0.3068	4.12	6.63	0.622 ✓

* DL controls

Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T28	200 - 175	P1.25x.14	5.00	4.75	105.6	21.600	0.6685	2.27	14.44	0.157 ✓
T33	150 - 125	P1.25x.14	5.00	4.73	105.2	21.600	0.6685	2.29	14.44	0.159 ✓
T34	125 - 100	P1.25x.14	5.00	4.73	105.2	21.600	0.6685	2.26	14.44	0.157 ✓
T39	75 - 50	P1.25x.14	5.00	4.73	105.2	21.600	0.6685	2.17	14.44	0.151 ✓
T40	50 - 25	P1.25x.14	5.00	4.73	105.2	21.600	0.6685	2.33	14.44	0.161 ✓
T41	25 - 0	P1.25x.14	5.00	4.73	105.2	21.600	0.6685	2.33	14.44	0.161 ✓

Secondary Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T25	218.75 - 212.5	L2x2x1/4	5.00	4.75	93.6	21.600	0.9380	2.14	20.26	0.106 ✓
T26	212.5 - 206.25	L2x2x1/4	5.00	4.75	93.6	21.600	0.9380	2.20	20.26	0.109 ✓
T27	206.25 - 200	L2x2x1/4	5.00	4.75	93.6	21.600	0.9380	2.25	20.26	0.111 ✓
T28	200 - 175	P1.25x.14	5.00	4.75	105.6	21.600	0.6685	2.27	14.44	0.157 ✓

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Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	368.75 - 362.5	2L2 1/2x2x1/4	5.00	4.34	73.0	29.000	1.3162	0.11	38.17	0.003
T2	362.5 - 356.25	2L2 1/2x3x1/4	5.00	4.42	76.0	29.000	1.7381	0.33	50.41	0.006
T3	356.25 - 350	2L2 1/2x3x1/4	5.00	4.42	76.0	29.000	1.7381	1.37	50.41	0.027
T4	350 - 343.75	2L2 1/2x2x1/4	5.00	4.34	73.0	29.000	1.3162	9.71	38.17	0.254
T12	300 - 293.75	2L2 1/2x2x1/4	5.00	4.30	72.4	29.000	1.3162	11.72	38.17	0.307
T13	293.75 - 287.5	2L2 1/2x2x1/4	5.00	4.30	72.4	29.000	1.3162	0.35	38.17	0.009
T21	243.75 - 237.5	2L2 1/2x2x1/4	5.00	4.30	72.4	29.000	1.3162	0.01	38.17	0.000
T24	225 - 218.75	2L2 1/2x2x1/4	5.00	4.30	72.4	29.000	1.3162	18.24	38.17	0.478
T25	218.75 - 212.5	2L2 1/2x2x1/4	5.00	4.32	72.7	29.000	1.3162	6.20	38.17	0.162
T27	206.25 - 200	P1.25x.14	5.00	4.75	105.6	21.600	0.6685	0.00	14.44	0.000
T29	175 - 168.75	P1.25x.14	5.00	4.75	105.6	21.600	0.6685	0.22	14.44	0.015
T31	162.5 - 156.25	2L2 1/2x2x1/4	5.00	4.38	72.4	29.000	1.3631	8.89	39.53	0.225
T32	156.25 - 150	P1.25x.14	5.00	4.73	105.2	21.600	0.6685	0.09	14.44	0.006
T33	150 - 125	P1.25x.14	5.00	4.73	105.2	21.600	0.6685	0.20	14.44	0.014
T35	100 - 93.75	2L2 1/2x2x1/4	5.00	4.30	72.4	29.000	1.3162	19.04	38.17	0.499
T36	93.75 - 87.5	2L2 1/2x2x1/4	5.00	4.30	72.4	29.000	1.3162	7.00	38.17	0.184
T40	50 - 25	L2 1/2x2x1/4	5.00	4.38	95.8	29.000	0.6778	6.56	19.66	0.334
T41	25 - 0	P1.25x.14	5.00	4.73	105.2	21.600	0.6685	0.12	14.44	0.008

Torque-Arm Top Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T4	350 - 343.75 (738)	2L3x2 1/2x1/4	6.03	5.91	75.1	21.600	2.6300	23.92	56.81	0.421
T4	350 - 343.75 (739)	2L3x2 1/2x1/4	6.03	5.91	75.1	21.600	2.6300	23.18	56.81	0.408
T4	350 - 343.75	2L3x2 1/2x1/4	6.03	5.91	75.1	21.600	2.6300	21.96	56.81	0.387

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
	(744)									✓
T4	350 - 343.75 (745)	2L3x2 1/2x1/4	6.03	5.91	75.1	21.600	2.6300	21.21	56.81	0.373
T4	350 - 343.75 (750)	2L3x2 1/2x1/4	6.03	5.91	75.1	21.600	2.6300	22.41	56.81	0.394
T4	350 - 343.75 (751)	2L3x2 1/2x1/4	6.03	5.91	75.1	21.600	2.6300	18.44	56.81	0.325
T12	300 - 293.75 (720)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	23.87	56.81	0.420
T12	300 - 293.75 (721)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	24.06	56.81	0.424
T12	300 - 293.75 (726)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	22.57	56.81	0.397
T12	300 - 293.75 (727)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	22.10	56.81	0.389
T12	300 - 293.75 (732)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	23.39	56.81	0.412
T12	300 - 293.75 (733)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	18.67	56.81	0.329
T24	225 - 218.75 (702)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	22.76	56.81	0.401
T24	225 - 218.75 (703)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	22.88	56.81	0.403
T24	225 - 218.75 (708)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	21.04	56.81	0.370
T24	225 - 218.75 (709)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	21.85	56.81	0.385
T24	225 - 218.75 (714)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	22.59	56.81	0.398
T24	225 - 218.75 (715)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	18.58	56.81	0.327
T35	100 - 93.75 (681)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	16.17	56.81	0.285
T35	100 - 93.75 (682)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	15.27	56.81	0.269
T35	100 - 93.75 (687)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	13.81	56.81	0.243
T35	100 - 93.75 (688)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	16.98	56.81	0.299
T35	100 - 93.75 (693)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	15.44	56.81	0.272
T35	100 - 93.75 (694)	2L3x2 1/2x1/4	6.03	5.89	74.8	21.600	2.6300	14.79	56.81	0.260

Torque-Arm Bottom Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T35	100 - 93.75	2L3x2 1/2x1/4	8.68	8.49	107.8	21.600	2.6300	0.82	56.81	0.014

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Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
T3	356.25 - 350	Diagonal	2L3x3x5/16	36	-5.20	79.90	6.5	Pass
T4	350 - 343.75	Diagonal	L3x2 1/2x1/4	43	-3.90	24.50	24.7 (b) 15.9	Pass
T5	343.75 - 337.5	Diagonal	5/8	58	4.09	8.83	46.3	Pass
T6	337.5 - 331.25	Diagonal	5/8	70	3.71	8.83	42.0	Pass
T7	331.25 - 325	Diagonal	5/8	82	3.26	8.83	37.0	Pass
T8	325 - 318.75	Diagonal	3/4	95	3.69	12.72	29.0	Pass
T9	318.75 - 312.5	Diagonal	3/4	107	3.88	12.72	33.6 (b) 30.5	Pass
T10	312.5 - 306.25	Diagonal	3/4	119	4.25	12.72	35.3 (b) 33.4	Pass
T11	306.25 - 300	Diagonal	3/4	131	7.34	12.72	38.6 (b) 57.7	Pass
T12	300 - 293.75	Diagonal	L3x2 1/2x1/4	143	-7.95	24.62	66.8 (b) 32.3	Pass
T13	293.75 - 287.5	Diagonal	3/4	152	6.94	12.72	46.3 (b) 54.6	Pass
T14	287.5 - 281.25	Diagonal	5/8	164	5.78	8.83	63.2 (b) 65.4	Pass
T15	281.25 - 275	Diagonal	5/8	176	4.60	8.83	52.1	Pass
T16	275 - 268.75	Diagonal	5/8	188	3.31	8.83	37.5	Pass
T17	268.75 - 262.5	Diagonal	5/8	200	2.43	8.83	27.5	Pass
T18	262.5 - 256.25	Diagonal	5/8	216	1.67	6.63	25.2	Pass
T19	256.25 - 250	Diagonal	3/4	228	2.49	9.54	26.1	Pass
T20	250 - 243.75	Diagonal	3/4	240	2.67	9.54	28.2 (b) 28.0	Pass
T21	243.75 - 237.5	Diagonal	3/4	247	7.89	12.72	30.4 (b) 62.0	Pass
T22	237.5 - 231.25	Diagonal	3/4	259	11.15	12.72	71.8 (b) 87.6	Pass
T23	231.25 - 225	Diagonal	3/4	271	12.31	12.72	96.8	Pass
T24	225 - 218.75	Diagonal	2L2 1/2x2 1/2x1/4	288	-15.34	51.46	29.8	Pass
T25	218.75 - 212.5	Diagonal	5/8	296	7.66	8.83	44.6 (b) 86.7	Pass
T26	212.5 - 206.25	Diagonal	5/8	311	5.89	8.83	66.7	Pass
T27	206.25 - 200	Diagonal	5/8	326	3.59	8.83	40.6	Pass
T28	200 - 175	Diagonal	5/8	345	6.09	8.83	68.9	Pass
T29	175 - 168.75	Diagonal	5/8	391	8.23	8.83	93.1	Pass
T30	168.75 - 162.5	Diagonal	1	403	9.92	22.61	43.9	Pass
T31	162.5 - 156.25	Diagonal	5/8	417	3.87	8.83	90.3 (b) 43.8	Pass
T32	156.25 - 150	Diagonal	5/8	429	2.80	8.83	31.7	Pass
T33	150 - 125	Diagonal	5/8	444	6.75	8.83	76.4	Pass
T34	125 - 100	Diagonal	L2 1/2x2 1/2x3/16	478	14.18	22.75	62.3	Pass
T35	100 - 93.75	Diagonal	2L2 1/2x2 1/2x1/4	517	-16.36	51.53	90.3 (b) 31.7	Pass
T36	93.75 - 87.5	Diagonal	3/4	530	7.54	12.72	47.6 (b) 59.3	Pass
T37	87.5 - 81.25	Diagonal	5/8	542	5.68	8.83	68.6 (b) 64.2	Pass
T38	81.25 - 75	Diagonal	5/8	554	3.61	8.83	40.9	Pass
T39	75 - 50	Diagonal	5/8	567	4.84	8.83	54.8	Pass
T40	50 - 25	Diagonal	5/8	632	5.97	8.83	67.6	Pass
T41	25 - 0	Diagonal	5/8	643	4.12	8.83	46.7	Pass
T28	200 - 175	Horizontal	P1.25x.14	347	-3.24	10.92	29.7	Pass
T33	150 - 125	Horizontal	P1.25x.14	447	-3.91	10.97	35.6	Pass
T34	125 - 100	Horizontal	P1.25x.14	484	-8.36	10.97	76.2	Pass
T39	75 - 50	Horizontal	P1.25x.14	572	-2.83	10.97	25.8	Pass
T40	50 - 25	Horizontal	P1.25x.14	628	-3.39	10.97	30.9	Pass
T41	25 - 0	Horizontal	P1.25x.14	658	-2.33	10.97	21.2	Pass

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Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
T25	218.75 - 212.5	Secondary Horizontal	L2x2x1/4	301	-2.14	10.12	21.1	Pass
T26	212.5 - 206.25	Secondary Horizontal	L2x2x1/4	316	-2.20	10.12	21.8	Pass
T27	206.25 - 200	Secondary Horizontal	L2x2x1/4	331	-2.25	10.12	22.2	Pass
T28	200 - 175	Secondary Horizontal	P1.25x.14	349	-2.27	10.92	20.8	Pass
T1	368.75 - 362.5	Top Girt	2L2 1/2x2x1/4	5	-0.09	39.23	0.2	Pass
T2	362.5 - 356.25	Top Girt	2L2 1/2x3x1/4	16	0.33	67.19	0.5	Pass
T3	356.25 - 350	Top Girt	2L2 1/2x3x1/4	28	-1.30	47.60	2.7	Pass
T4	350 - 343.75	Top Girt	2L2 1/2x2x1/4	40	9.71	50.88	19.1	Pass
T5	343.75 - 337.5	Top Girt	2L2 1/2x2x1/4	52	-4.16	39.28	28.3 (b)	Pass
T6	337.5 - 331.25	Top Girt	P1.25x.14	65	-2.53	8.19	12.1 (b)	Pass
T7	331.25 - 325	Top Girt	P1.25x.14	77	-2.41	8.19	30.9	Pass
T8	325 - 318.75	Top Girt	P1.25x.14	89	-2.88	8.19	29.5	Pass
T9	318.75 - 312.5	Top Girt	P1.25x.14	101	-3.32	8.19	35.1	Pass
T10	312.5 - 306.25	Top Girt	P1.25x.14	113	-3.43	8.19	40.6	Pass
T11	306.25 - 300	Top Girt	2L2 1/2x2x1/4	125	-3.84	29.51	41.8	Pass
T12	300 - 293.75	Top Girt	2L2 1/2x2x1/4	136	11.72	50.88	13.0	Pass
T13	293.75 - 287.5	Top Girt	2L2 1/2x2x1/4	148	-2.82	39.34	23.0	Pass
T14	287.5 - 281.25	Top Girt	P1.25x.14	160	-3.96	10.97	34.1 (b)	Pass
T15	281.25 - 275	Top Girt	P1.25x.14	172	-3.28	10.97	7.2	Pass
T16	275 - 268.75	Top Girt	P1.25x.14	184	-2.12	8.23	8.2 (b)	Pass
T17	268.75 - 262.5	Top Girt	P1.25x.14	196	-2.12	8.23	36.0	Pass
T18	262.5 - 256.25	Top Girt	P1.25x.14	208	-2.09	8.23	29.9	Pass
T19	256.25 - 250	Top Girt	P1.25x.14	220	-2.57	8.23	25.8	Pass
T20	250 - 243.75	Top Girt	2L2 1/2x2x1/4	232	-3.20	29.36	25.7	Pass
T21	243.75 - 237.5	Top Girt	2L2 1/2x2x1/4	244	-3.29	29.51	25.4	Pass
T22	237.5 - 231.25	Top Girt	2L2 1/2x2x1/4	256	-5.97	39.34	31.2	Pass
T23	231.25 - 225	Top Girt	2L2 1/2x2x1/4	268	-7.34	39.34	10.9	Pass
T24	225 - 218.75	Top Girt	2L2 1/2x2x1/4	280	18.24	50.88	15.6 (b)	Pass
T25	218.75 - 212.5	Top Girt	2L2 1/2x2x1/4	292	6.20	50.88	11.1	Pass
T26	212.5 - 206.25	Top Girt	P1.25x.14	307	-4.05	10.92	12.6 (b)	Pass
T27	206.25 - 200	Top Girt	P1.25x.14	322	-2.80	10.92	15.2	Pass
T28	200 - 175	Top Girt	P1.25x.14	337	-1.57	8.19	17.4 (b)	Pass
T29	175 - 168.75	Top Girt	P1.25x.14	390	-4.56	10.92	18.6	Pass
T30	168.75 - 162.5	Top Girt	P1.25x.14	402	-5.66	10.97	21.4 (b)	Pass
T31	162.5 - 156.25	Top Girt	2L2 1/2x2x1/4	412	8.89	52.69	35.9	Pass
T32	156.25 - 150	Top Girt	P1.25x.14	425	-1.74	8.23	53.1 (b)	Pass
T33	150 - 125	Top Girt	P1.25x.14	437	-1.67	8.23	12.2	Pass
T34	125 - 100	Top Girt	P1.25x.14	477	-5.00	10.97	18.0 (b)	Pass
T35	100 - 93.75	Top Girt	2L2 1/2x2x1/4	514	19.04	50.88	37.0	Pass
T36	93.75 - 87.5	Top Girt	2L2 1/2x2x1/4	526	7.00	50.88	21.2	Pass
T37	87.5 - 81.25	Top Girt	P1.25x.14	538	-4.12	10.97	20.3	Pass
T38	81.25 - 75	Top Girt	P1.25x.14	550	-2.92	10.97	20.6	Pass
T39	75 - 50	Top Girt	P1.25x.14	564	-1.49	8.23	18.1	Pass
T40	50 - 25	Top Girt	L2 1/2x2x1/4	601	6.56	26.20	25.0	Pass

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Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
T41	25 - 0	Top Girt	P1.25x.14	641	-1.41	8.23	59.7 (b)	Pass
T4	350 - 343.75	Guy A@350	7/8	749	25.99	39.85	17.1	Pass
T12	300 - 293.75	Guy A@300	7/8	731	27.70	39.85	65.2	Pass
T24	225 - 218.75	Guy A@225	3/4	713	24.79	29.15	69.5	Pass
T31	162.5 - 156.25	Guy A@162.5	3/4	699	26.29	29.15	85.0	Pass
T35	100 - 93.75	Guy A@100	9/16	692	15.65	17.50	90.2	Pass
T40	50 - 25	Guy A@50	9/16	678	13.74	17.50	89.4	Pass
T4	350 - 343.75	Guy B@350	7/8	743	21.08	39.85	78.5	Pass
T12	300 - 293.75	Guy B@300	7/8	725	22.76	39.85	52.9	Pass
T24	225 - 218.75	Guy B@225	3/4	707	21.02	29.15	57.1	Pass
T31	162.5 - 156.25	Guy B@162.5	3/4	698	22.99	29.15	72.1	Pass
T35	100 - 93.75	Guy B@100	9/16	685	14.39	17.50	78.9	Pass
T40	50 - 25	Guy B@50	9/16	677	12.73	17.50	82.2	Pass
T4	350 - 343.75	Guy C@350	7/8	737	24.38	39.85	72.7	Pass
T12	300 - 293.75	Guy C@300	7/8	719	26.68	39.85	61.2	Pass
T24	225 - 218.75	Guy C@225	3/4	701	24.66	29.15	67.0	Pass
T31	162.5 - 156.25	Guy C@162.5	3/4	697	26.37	29.15	84.6	Pass
T35	100 - 93.75	Guy C@100	9/16	680	16.35	17.50	90.5	Pass
T40	50 - 25	Guy C@50	9/16	676	14.34	17.50	93.4	Pass
T4	350 - 343.75	Torque Arm Top@350	2L3x2 1/2x1/4	738	23.92	75.73	82.0	Pass
T12	300 - 293.75	Torque Arm Top@300	2L3x2 1/2x1/4	721	24.06	75.73	31.6	Pass
T24	225 - 218.75	Torque Arm Top@225	2L3x2 1/2x1/4	703	22.88	75.73	31.8	Pass
T35	100 - 93.75	Torque Arm Top@100	2L3x2 1/2x1/4	688	16.98	75.73	30.2	Pass
T4	350 - 343.75	Torque Arm Bottom@350	2L3x2 1/2x1/4	753	-27.40	41.87	22.4	Pass
T12	300 - 293.75	Torque Arm Bottom@300	2L3x2 1/2x1/4	735	-30.17	41.96	65.4	Pass
T24	225 - 218.75	Torque Arm Bottom@225	2L3x2 1/2x1/4	710	-25.35	41.87	71.9	Pass
T35	100 - 93.75	Torque Arm Bottom@100	2L3x2 1/2x1/4	689	-14.03	41.96	60.5	Pass
							Summary	
							Leg (T41)	87.4 Pass
							Diagonal (T23)	96.8 Pass
							Horizontal (T34)	76.2 Pass
							Secondary Horizontal (T27)	22.2 Pass
							Top Girt (T40)	59.7 Pass
							Guy A (T31)	90.2 Pass
							Guy B (T35)	82.2 Pass
							Guy C (T35)	93.4 Pass
							Torque Arm Top (T12)	31.8 Pass
							Torque Arm Bottom (T12)	71.9 Pass
							Bolt Checks	90.3 Pass
							RATING =	96.8 Pass

FOUNDATION ANALYSIS

FOUNDATION ANALYSIS

TOWER FORCES:

Moment Caused by Tower	$M_t := 0 \cdot \text{ft} \cdot \text{kips}$
Shear at Base of Tower	$S_t := 2 \text{ kip}$
Max Compressive Force	$C_t := 383 \cdot \text{kip}$
Height of Tower	$H_t := 370 \cdot \text{ft}$

FOOTING DIMENSIONS:

Overall Depth of Footing	$D_f := 3.5 \text{ ft}$
Length of Pier	$L_p := 3 \cdot \text{ft}$
Extension of Pier Above Grade	$L_{pag} := 1.5 \cdot \text{ft}$
Diameter of Pier	$d_p := 3 \cdot \text{ft}$
Thickness of Footing	$T_f := 2 \cdot \text{ft}$
Width of Footing:	$W_f := 7 \text{ ft}$

PROPERTIES:

Internal Friction Angle of Soil	$\phi_s := 30 \cdot \text{deg}$
Allowable Bearing Capacity	$q_s := \frac{4}{3} \cdot 8000 \cdot \text{psf}$
Note: 1/3 increase in allowable bearing capacity for transient loadings, such as wind and seismic per geotech report.	
Unit Weight of Soil	$\gamma_s := 120 \cdot \text{pcf}$
Unit Weight of Concrete	$\gamma_c := 150 \cdot \text{pcf}$
Depth to Neglect	$n := 0 \text{ ft}$
Cohesion of Clay Type Soil	$c := 0 \cdot \text{ksf}$
Note: Use 0 for Sandy Soil	
Seismic Zone Factor:	$Z := 2$
UBC Fig 23-2	
Coefficient of Friction between Concrete:	$\mu := 0.45$

STABILITY OF FOOTING

Coefficient of Lateral Soil Pressure:	$K_p := \frac{1 + \sin(\phi_s)}{1 - \sin(\phi_s)}$	$K_p = 3$
Passive Pressure:	$P_{pn} := K_p \cdot \gamma_s \cdot n + c \cdot 2 \cdot \sqrt{K_p}$ $P_{pt} := K_p \cdot \gamma_s \cdot (D_f - T_f) + c \cdot 2 \cdot \sqrt{K_p}$ $P_{top} := \text{if}[n < (D_f - T_f), P_{pt}, P_{pn}]$ $P_{bot} := K_p \cdot \gamma_s \cdot D_f + c \cdot 2 \cdot \sqrt{K_p}$ $P_{ave} := \frac{P_{top} + P_{bot}}{2}$ $T_p := \text{if}[n < (D_f - T_f), T_f, (D_f - n)]$ $A_p := W_f \cdot T_p$	$P_{pn} = 0 \cdot \text{ksf}$ $P_{pt} = 0.54 \cdot \text{ksf}$ $P_{top} = 0.54 \cdot \text{ksf}$ $P_{bot} = 1.26 \cdot \text{ksf}$ $P_{ave} = 0.9 \cdot \text{ksf}$ $T_p = 2 \cdot \text{ft}$ $A_p = 14 \cdot \text{ft}^2$
Ultimate Shear:	$S_u := P_{ave} \cdot A_p$	$S_u = 12.6 \cdot \text{kip}$

Job	370' Guyed Lattice Tower - Montville, CT	Project No.	VZ5-109	Page	of
Description	Spread Footing w/ Pier Analysis	Computed by	KAB	Sheet	2 of 2
		Checked by		Date	10/11/12
				Date	

Weight of Concrete Pad:	$WT_c := \left[\left(W_f^2 \cdot T_f \right) + d_p^2 L_p \right] \cdot \gamma_c$	$WT_c = 18.75 \cdot \text{kip}$
Weight of Soil above Footing:	$WT_{s1} := \left[W_f^2 \cdot \left(L_p - L_{pag} \right) - \frac{d_p^2 \cdot \pi}{4} \cdot \left(L_p - L_{pag} \right) \right] \cdot \gamma_s$	$WT_{s1} = 7.5477 \cdot \text{kip}$
Weight of Soil Wedge at back face:	$WT_{s2} := \left(\frac{D_f^2 \cdot \tan(\phi_s)}{2} \cdot W_f \right) \cdot \gamma_s$	$WT_{s2} = 2.9705 \cdot \text{kip}$
Total Weight:	$WT_{tot} := WT_c + WT_{s1} + C_t$	$WT_{tot} = 409.2977 \cdot \text{kip}$
Resisting Moment:	$M_r := \left(WT_{tot} \right) \cdot \frac{W_f}{2} + S_u \cdot \frac{T_f}{3} + WT_{s2} \cdot \left(W_f + \frac{D_f \cdot \tan(\phi_s)}{3} \right)$	$M_r = 1463.7359 \cdot \text{kip} \cdot \text{ft}$
Overturing Moment:	$M_{ot} := M_t + S_t \cdot (L_p + T_f)$	$M_{ot} = 10 \cdot \text{kip} \cdot \text{ft}$
Factor of Safety:	$FS := \frac{M_r}{M_{ot}}$ $FS_{req} := 2$ $SafetyCheck := \text{if}(FS > FS_{req}, "Okay", "No Good")$	$FS = 146.37$ $SafetyCheck = "Okay"$

BEARING PRESSURE CAUSED BY FOOTING

$A_{mat} := W_f^2$	$A_{mat} = 49 \cdot \text{ft}^2$
$S := \frac{W_f^3}{6}$	$S = 57.1667 \cdot \text{ft}^3$
$P_{max} := \frac{WT_{tot}}{A_{mat}} + \frac{M_{ot}}{S}$	$P_{max} = 8.5279 \cdot \text{ksf}$
$P_{min} := \frac{WT_{tot}}{A_{mat}} - \frac{M_{ot}}{S}$	$P_{min} = 8.1781 \cdot \text{ksf}$
$q_s = 10.6667 \cdot \text{ksf}$	
$MaxPressure := \text{if}(P_{max} < q_s, "Okay", "No Good")$	$MaxPressure = "Okay"$
$MinPressure := \text{if}[(P_{min} \geq 0) \cdot (P_{min} < q_s), "Okay", "No Good"]$	$MinPressure = "Okay"$

GUY ANCHOR ANALYSIS

Job : Old Colchester Rd - Montville, CT
 Description: Anchor Block Evaluation
 Inner Anchor Block

Project No.: VZ5-109
 Computed by: KAB
 Checked by:

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 Sheet 1 of 2
 Date 10/11/12
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CHECK UPLIFT RESISTANCE

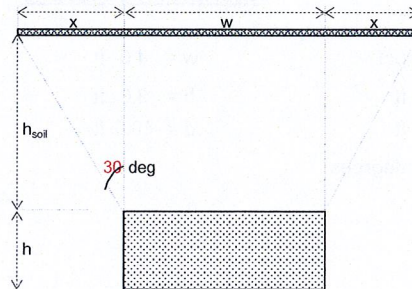
RESULTS FROM COMPUTER ANALYSIS:

Uplift = 6 kips
 Sliding = 13 kips

CONCRETE PARAMETERS:

$\gamma_{conc} = 150$ pcf
 $w = 4$ ft
 $h = 3$ ft
 $d = 10$ ft

Vol. = 120 ft³
 $W_c = 18.00$ kips



Foundation Section

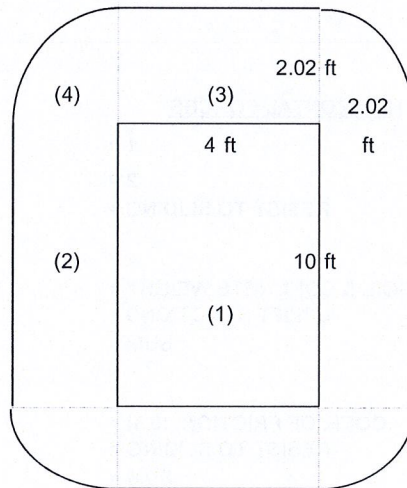
SOIL PARAMETERS:

$\gamma_{soil} = 120$ pcf
 $h_{soil} = 3.5$ ft
 $x = 2.02$ ft

Soil Weight (Wr):

(1) = 16.80 kips
 (2) = 8.49 kips
 (3) = 3.39 kips
 (4) = 1.80 kips

* (5) Anchor Reinf. = 0 kips
 Total = 30.48 kips



Foundation Plan View

CHECK UPLIFT (PER EIA/TIA-222-F STANDARD):

$$W_r / 2.0 + W_c / 1.25 > \text{UPLIFT}$$

$$29.64 > 6 \quad \text{OK}$$

$$(W_r + W_c) / 1.5 > \text{UPLIFT}$$

$$32.32 > 6 \quad \text{OK}$$

CHECK UPLIFT (PER 2005 CT BLDG CODE 3108.4):

$$(W_r + W_c) / 2.0 > \text{UPLIFT}$$

$$24.24 > 6 \quad \text{OK}$$

→ GUY ANCHORS AGAINST UPLIFT ARE ADEQUATE

Job : Old Colchester Rd - Montville, CT
 Description: Anchor Block Evaluation
 Inner Anchor Block

Project No.: VZ5-109
 Computed by: KAB
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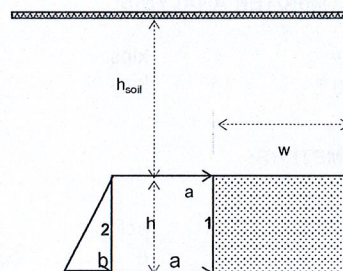
CHECK SLIDING RESISTANCE

SOIL PARAMETERS

$\gamma_{soil} = 120$ pcf
 $h_{soil} = 3.5$ ft
 $h = 3$ ft
 $\phi = 30$ degrees

ANCHOR PARAMETERS

$w = 4.0$ ft
 $h = 3.0$ ft
 $d = 10.0$ ft



Foundation Elevation View

$K_a = 0.33$

$K_p = 3.00$

$\Delta = 2.67$

HORIZONTAL FORCES

1 =	33.60	k
2 =	8.64	k
RESIST TO SLIDING =	<u>42.24</u>	k

SOIL & CONCRETE WEIGHT =	$W_r + W_c = 48.48$	k
UPLIFT REACTIONS =	<u>-6</u>	k
SUM =	<u>42.48</u>	k

COEF. OF FRICTION, (0.5) =	21.24	k
RESIST TO SLIDING =	<u>42.24</u>	k
SUM =	<u>63.48</u>	k

SF AGAINST SLIDING

SF = 4.88 > 2.0 OK

→ GUY ANCHORS AGAINST SLIDING ARE ADEQUATE

Job : Old Colchester Rd - Montville, CT
 Description: Anchor Block Evaluation
 160' Radius Anchor Block

Project No.: VZ5-109
 Computed by: KAB
 Checked by:

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CHECK UPLIFT RESISTANCE

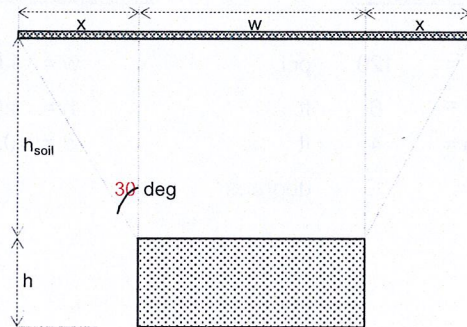
RESULTS FROM COMPUTER ANALYSIS:

Uplift = 35 kips
 Sliding = 45 kips

CONCRETE PARAMETERS:

$\gamma_{conc} = 150$ pcf
 $w = 4$ ft
 $h = 4$ ft
 $d = 10$ ft

Vol. = 160 ft³
 $W_c = 24.00$ kips



Foundation Section

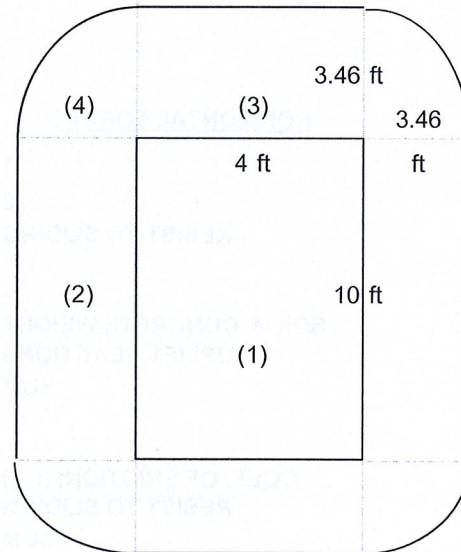
SOIL PARAMETERS:

$\gamma_{soil} = 120$ pcf
 $h_{soil} = 6$ ft
 $x = 3.46$ ft

Soil Weight (W_r):

(1) = 28.80 kips
 (2) = 24.94 kips
 (3) = 9.98 kips
 (4) = 9.05 kips

*(5) Anchor Reinf. = 0 kips
 Total = 72.77 kips



Foundation Plan View

CHECK UPLIFT (PER EIA/TIA-222-F STANDARD):

$$W_r / 2.0 + W_c / 1.25 > \text{UPLIFT}$$

$$55.58 > 35 \quad \text{OK}$$

$$(W_r + W_c) / 1.5 > \text{UPLIFT}$$

$$64.51 > 35 \quad \text{OK}$$

CHECK UPLIFT (PER 2005 CT BLDG CODE 3108.4):

$$(W_r + W_c) / 2.0 > \text{UPLIFT}$$

$$48.38 > 35 \quad \text{OK}$$

→ GUY ANCHORS AGAINST UPLIFT ARE ADEQUATE

Job : Old Colchester Rd - Montville, CT
 Description: Anchor Block Evaluation
 160' Radius Anchor Block

Project No.: VZ5-109
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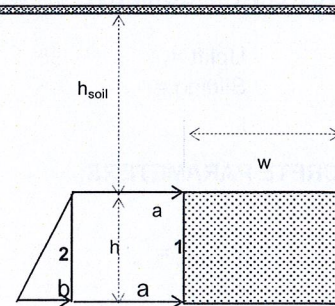
CHECK SLIDING RESISTANCE

SOIL PARAMETERS

$\gamma_{soil} = 120$ pcf
 $h_{soil} = 6$ ft
 $h = 4$ ft
 $\phi = 30$ degrees

ANCHOR PARAMETERS

$w = 4.0$ ft
 $h = 4.0$ ft
 $d = 10.0$ ft



Foundation Elevation View

$K_a = 0.33$

$K_p = 3.00$

$\Delta = 2.67$

HORIZONTAL FORCES

1 = 76.80 k
 2 = 15.36 k
 RESIST TO SLIDING = 92.16 k

SOIL & CONCRETE WEIGHT = $W_r + W_c = 96.77$ k
 UPLIFT REACTIONS = -35 k
 SUM = 61.77 k

COEF. OF FRICTION, (0.5) = 30.88 k
 RESIST TO SLIDING = 92.16 k
 SUM = 123.04 k

SF AGAINST SLIDING

SF = 2.73 > 2.0 OK

→ GUY ANCHORS AGAINST SLIDING ARE ADEQUATE

Job : Old Colchester Rd - Montville, CT
 Description: Anchor Block Evaluation
 205' Radius Anchor Block

Project No.: VZ5-109
 Computed by: KAB
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CHECK UPLIFT RESISTANCE

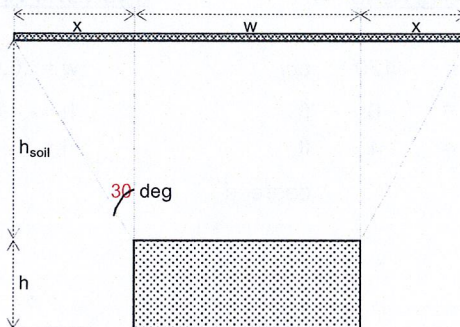
RESULTS FROM COMPUTER ANALYSIS:

Uplift = 79 kips
 Sliding = 65 kips

CONCRETE PARAMETERS:

$\gamma_{conc} = 150$ pcf
 $w = 6$ ft
 $h = 4$ ft
 $d = 16$ ft

 Vol. = 384 ft³
 Wc = 57.60 kips



Foundation Section

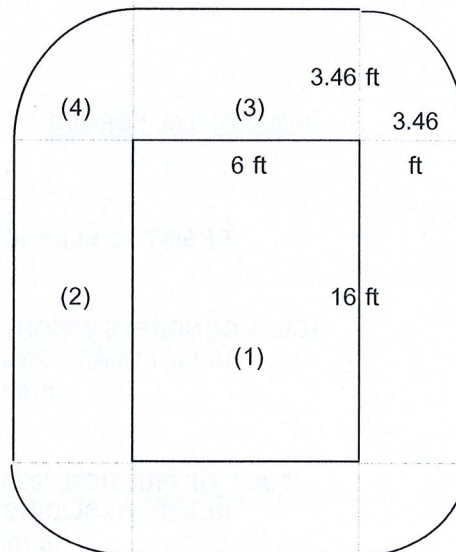
SOIL PARAMETERS:

$\gamma_{soil} = 120$ pcf
 $h_{soil} = 6$ ft
 $x = 3.46$ ft

Soil Weight (Wr):

(1) = 69.12 kips
 (2) = 39.91 kips
 (3) = 14.96 kips
 (4) = 9.05 kips

* (5) Anchor Reinf. = 0 kips
 Total = 133.04 kips



Foundation Plan View

FORCE
 ←

CHECK UPLIFT (PER EIA/TIA-222-F STANDARD):

$$Wr / 2.0 + Wc / 1.25 > \text{UPLIFT}$$

$$112.60 > 79 \quad \text{OK}$$

$$(Wr + Wc) / 1.5 > \text{UPLIFT}$$

$$127.09 > 79 \quad \text{OK}$$

CHECK UPLIFT (PER 2005 CT BLDG CODE 3108.4):

$$(Wr + Wc) / 2.0 > \text{UPLIFT}$$

$$95.32 > 79 \quad \text{OK}$$

→ GUY ANCHORS AGAINST UPLIFT ARE ADEQUATE

Job : Old Colchester Rd - Montville, CT
 Description: Anchor Block Evaluation
 205' Radius Anchor Block

Project No.: VZ5-109
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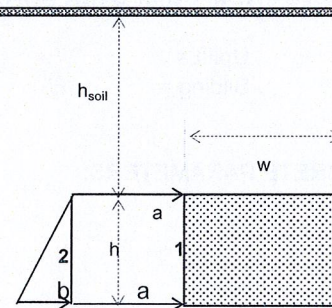
CHECK SLIDING RESISTANCE

SOIL PARAMETERS

$\gamma_{soil} = 120$ pcf
 $h_{soil} = 6$ ft
 $h = 4$ ft
 $\phi = 30$ degrees

ANCHOR PARAMETERS

$w = 6.0$ ft
 $h = 4.0$ ft
 $d = 16.0$ ft



Foundation Elevation View

$K_a = 0.33$

$K_p = 3.00$

$\Delta = 2.67$

HORIZONTAL FORCES

1 =	122.88 k
2 =	15.36 k
RESIST TO SLIDING =	<u>138.24 k</u>

SOIL & CONCRETE WEIGHT =	$W_r + W_c = 190.64$ k
UPLIFT REACTIONS =	<u>-79 k</u>
SUM =	111.64 k

COEF. OF FRICTION, (0.5) =	55.82 k
RESIST TO SLIDING =	<u>138.24 k</u>
SUM =	194.06 k

SF AGAINST SLIDING

$SF = 2.99 > 2.0$ OK

→ GUY ANCHORS AGAINST SLIDING ARE ADEQUATE

Job : Old Colchester Rd - Montville, CT
 Description: Anchor Block Evaluation
 Outer Anchor Block

Project No.: VZ5-109
 Computed by: KAB
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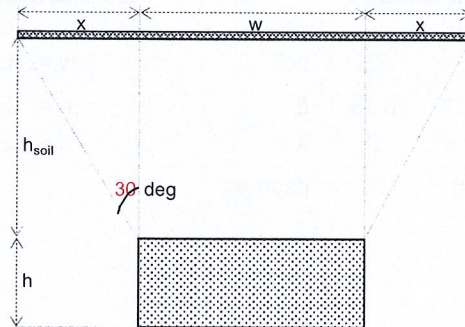
CHECK UPLIFT RESISTANCE

RESULTS FROM COMPUTER ANALYSIS:

Uplift = 39 kips
 Sliding = 30 kips

CONCRETE PARAMETERS:

$\gamma_{conc} = 150$ pcf
 $w = 4$ ft
 $h = 3$ ft
 $d = 12$ ft
 Vol. = 144 ft³
 $W_c = 21.60$ kips



Foundation Section

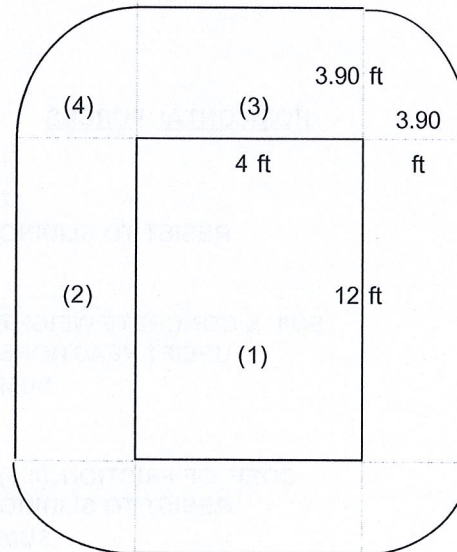
SOIL PARAMETERS:

$\gamma_{soil} = 120$ pcf
 $h_{soil} = 6.75$ ft
 $x = 3.90$ ft

Soil Weight (W_r):

(1) = 38.88 kips
 (2) = 37.88 kips
 (3) = 12.63 kips
 (4) = 12.88 kips

* (5) Anchor Reinf. = 0 kips
 Total = 102.27 kips



Foundation Plan View

FORCE
 ←

Note: Previous reinforcement accounted for in soil height. 3' @ 130pcf = 3.25' @ 120pcf

CHECK UPLIFT (PER EIA/TIA-222-F STANDARD):

$$W_r / 2.0 + W_c / 1.25 > \text{UPLIFT}$$

68.41 > 39 OK

$$(W_r + W_c) / 1.5 > \text{UPLIFT}$$

82.58 > 39 OK

CHECK UPLIFT (PER 2005 CT BLDG CODE 3108.4):

$$(W_r + W_c) / 2.0 > \text{UPLIFT}$$

61.93 > 39 OK

→ GUY ANCHORS AGAINST UPLIFT ARE ADEQUATE

Job : Old Colchester Rd - Montville, CT
 Description: Anchor Block Evaluation
 Outer Anchor Block

Project No.: VZ5-109
 Computed by: KAB
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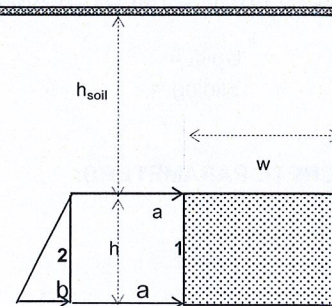
CHECK SLIDING RESISTANCE

SOIL PARAMETERS

$\gamma_{soil} = 120$ pcf
 $h_{soil} = 6.75$ ft
 $h = 3$ ft
 $\phi = 30$ degrees

ANCHOR PARAMETERS

$w = 4.0$ ft
 $h = 3.0$ ft
 $d = 12.0$ ft



Foundation Elevation View

$K_a = 0.33$

$K_p = 3.00$

$\Delta = 2.67$

HORIZONTAL FORCES

1 = 77.76 k

2 = 8.64 k

RESIST TO SLIDING = 86.40 k

SOIL & CONCRETE WEIGHT =

$W_r + W_c = 123.87$ k

UPLIFT REACTIONS =

-39 k

SUM =

84.87 k

COEF. OF FRICTION, (0.5) =

42.43 k

RESIST TO SLIDING =

86.40 k

SUM =

128.83 k

SF AGAINST SLIDING

SF = 4.29 > 2.0 OK

→ GUY ANCHORS AGAINST SLIDING ARE ADEQUATE