



Crown Castle  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065

August 4, 2020

Melanie A. Bachman  
Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

**RE: Notice of Exempt Modification for AT&T - 841294**  
**230 Guinea Road, Monroe, CT 06468**  
**Latitude: 41° 20' 30.68" / Longitude: -73° 16' 28.28"**

Dear Ms. Bachman:

AT&T currently maintains nine (9) antennas at the 230-foot mount on the existing 240-foot Self-Support Tower, located at 230 Guinea Road, Monroe, CT. The property is owned by the Town of Monroe and the Tower is owned by Crown Castle. AT&T now intends to remove and replace three (3) existing antennas with six (6) new antennas. The new antennas will be installed at the 230-ft level of the tower. AT&T is also proposing tower mount modifications as shown on the enclosed Mount Analysis.

The facility was approved by the Connecticut Siting Council in Docket No. 114 on January 16, 1990. The approval was given with conditions which this exempt modification follows.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Ken Kellogg, First Selectman for the Town of Monroe, as both the municipality and property owner, Rick Schultz, Town Planner, and Crown Castle is the tower owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

Melanie A. Bachman

Page 2

For the foregoing reasons, AT&T respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Anne Marie Zsamba.

Sincerely,

Anne Marie Zsamba  
Site Acquisition Specialist  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065  
(201) 236-9224  
AnneMarie.Zsamba@crowncastle.com

Attachments

cc: Ken Kellogg, First Selectman (*via email only to kkellogg@monroect.org*)  
Monroe Town Hall

7 Fan Hill Road  
Monroe, CT 06468

Rick Schultz, Town Planner (*via email only to rschultz@monroect.org*)

Monroe Town Hall  
7 Fan Hill Road  
Monroe, CT 06468

Crown Castle, Tower Owner

**From:** Zsamba, Anne Marie  
**To:** "rschultz@monroect.org"  
**Subject:** Notice of Exempt Modification - AT&T - 230 Guinea Road, Monroe - 841294  
**Date:** Tuesday, August 4, 2020 11:50:00 AM  
**Attachments:** [EM-AT&T-230 GUINEA RD MONROE-841294.pdf](#)

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Dear Town Planner Schultz:

Attached please find AT&T's exempt modification application that is being submitted to the Connecticut Siting Council, today Tuesday, August 4, 2020.

In light of the present circumstances with Covid-19, The Council has advised that electronic notification of this filing is acceptable. If you could kindly confirm receipt. Thank you.

Best,  
Anne Marie Zsamba

**ANNE MARIE ZSAMBA**  
Site Acquisition Specialist  
T: (201) 236-9224  
M: (518) 350-3639  
F: (724) 416-6112

**CROWN CASTLE**  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065  
[CrownCastle.com](http://CrownCastle.com)

**From:** Zsamba, Anne Marie  
**To:** "[kkellogg@monroect.org](mailto:kkellogg@monroect.org)"  
**Subject:** Notice of Exempt Modification - AT&T - 230 Guinea Road, Monroe - 841294  
**Date:** Tuesday, August 4, 2020 11:50:00 AM  
**Attachments:** [EM-AT&T-230 GUINEA RD MONROE-841294.pdf](#)

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Dear First Selectman Kellogg:

Attached please find AT&T's exempt modification application that is being submitted to the Connecticut Siting Council, today Tuesday, August 4, 2020.

In light of the present circumstances with Covid-19, The Council has advised that electronic notification of this filing is acceptable. If you could kindly confirm receipt. Thank you.

Best,  
Anne Marie Zsamba

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Site Acquisition Specialist  
T: (201) 236-9224  
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F: (724) 416-6112

**CROWN CASTLE**  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065  
[CrownCastle.com](http://CrownCastle.com)

# Exhibit A

**Original Facility Approval**

DOCKET NO. 114 - An application : Connecticut  
of SNET Cellular, Inc., for a :  
Certificate of Environmental : Siting  
Compatibility and Public Need : Council  
for a cellular telephone tower :  
and associated equipment in the :  
Town of Monroe, Connecticut. : January 16, 1990

DECISION AND ORDER

Pursuant to the foregoing Findings of Fact and Opinion, the Connecticut Siting Council finds that the effects associated with the construction, operation, and maintenance of a cellular telephone facility at the proposed Monroe site, including effects on the natural environment; ecological integrity and balance; public health and safety; scenic, historic, and recreational values; forests and parks; air and water purity; and fish and wildlife are not significant either alone or cumulatively with other effects, are not in conflict with the policies of the State concerning such effects, and are not sufficient reason to deny the application, and therefore directs that a Certificate of Environmental Compatibility and Public Need, as provided by Section 16-50k of the General Statutes of Connecticut (CGS), be issued to SNET Cellular, Inc. (SNET), for the construction, operation, and maintenance of a cellular telecommunications tower, associated equipment, and building at the proposed site in Monroe, Connecticut.

The facility shall be constructed, operated, and maintained substantially as specified in the Council's record in this matter, and subject to the following conditions:

1. All SNET cellular antennas shall extend no higher than 252 feet above ground level (AGL). If the Town of Monroe and SNET reach an agreement to place the Town of Monroe's antennas for public radio station WMNR on the tower, then the tower shall be no higher than 260 feet AGL for the attachment of such town antennas; otherwise the tower shall be no higher than 240 feet AGL. Prior to the raising of the tower from 240 feet AGL to 260 feet AGL, notice of such sharing and raising of the tower shall be provided to the Council.
2. The facility shall be constructed in accordance with the State of Connecticut Basic Building Code.
3. The Certificate Holder shall prepare a Development and Management (D&M) Plan for this site in compliance with Sections 16-50j-75 through 16-50j-77 of the Regulations of State Agencies. The D&M plan shall include detailed plans for site preparation including a profile and cross-section of the proposed access road, placement of the proposed tower and equipment building within the leased parcel, and erosion and sedimentation control.

4. The Certificate Holder shall comply with any future radio frequency (RF) standard, promulgated by State or federal regulatory agencies. Upon the establishment of any new governmental RF standards, the facility granted in this Decision and Order shall be brought into compliance with such standards.
5. The Certificate Holder or its successor shall provide the Council a recalculated report of power density if and when additional channels over the proposed 45 channels, higher wattage over the proposed 100 watts per channel, or other circumstances in operation cause a change in power density above the levels originally calculated in the application.
6. The Certificate Holder or its successor shall permit public or private entities to share space on the proposed Monroe tower for fair consideration, or shall provide any requesting entity with specific legal, technical, environmental, or economic reasons precluding such tower sharing.
7. If this facility does not initially provide, or permanently ceases to provide cellular service following completion of construction, this Decision and Order shall be void, and the tower and all associated equipment in this application shall be dismantled and removed or reapplied for any new use shall be made to the Council before any such new use is made.
8. Unless otherwise approved by the Council, this Decision and Order shall be void if all construction authorized herein is not completed within three years of the issuance of this Decision and Order, or within three years after the completion of any appeal from this Decision and Order. Pursuant to Section 16-50p, we hereby direct that a copy of the Findings of Fact, Opinion, and Decision and Order be served on each person listed below. A notice of issuance shall be published in the Bridgeport Post and the Monroe Courier.  
  
By this Decision and Order, the Council disposes of the legal rights, duties, and privileges of each party named or admitted to the proceeding in accordance with section 16-50j-17 of the Regulations of State Agencies.

The parties or intervenors to this proceeding are:

SNET Cellular, Inc.  
227 Church Street  
New Haven, CT 06506  
(Applicant)

Peter J. Tyrrell  
SNET Cellular, Inc.  
Room 1021  
227 Church Street  
New Haven, CT 06506  
(Its Representative)

Metro Mobile CTS of  
Fairfield County, Inc.  
(Intervenor)

Micheal W. Riley  
Vice-President North East Region  
Metro Mobile CTS, Inc.  
110 East 59th Street  
New York, New York 10022  
(Its Representatives)

Philip Mayberry, General Manager  
David S. Malko  
Metro Mobile CTS of  
Fairfield County, Inc.  
50 Rockland Road  
South Norwalk, Connecticut 06854

Paul M. Hancock, General Partner  
Housatonic Cable Vision Company  
2 East Street  
P.O. Box 1540  
New Milford, Connecticut 06766  
(Party)

Howard L. Slater, Esq.  
Bryne, Slater, Sandler,  
Shulman, & Rouse, P.C.  
330 Main Street  
P.O. Box 3216  
Hartford, Connecticut 06103  
Attn: Jennifer Young Gaudet, Esq.  
(Its Representative)



CERTIFICATION

The undersigned members of the Connecticut Siting Council hereby certify that they have heard this case in Docket 114 or read the record thereof, and that we voted as follows:


Dated at New Britain, Connecticut the 16 day of January, 1990.

Council Members

Vote Cast

  
Gloria Dibble Pond  
Chairperson

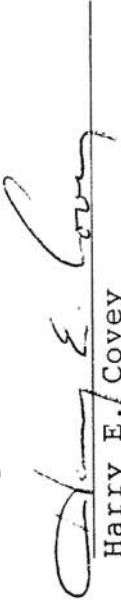
Yes

  
Commissioner Peter Boucher  
Designee: Robert A. Pulito

Yes

  
Commissioner Leslie Carothers  
Designee: Brian Emerick

Yes

  
Harry E. Covey

Yes

  
Mortimer A. Gelston

Yes

\_\_\_\_\_  
Daniel P. Lynch, Jr.

Absent

\_\_\_\_\_  
Paulann H. Sheets

Absent

\_\_\_\_\_  
William H. Smith

Absent

\_\_\_\_\_  
Colin C. Tait

Absent

# Exhibit B

**Property Card**

# 230 GUINEA RD

Location 230 GUINEA RD Map/Lot 081/008/00//  
Acct# 08100800 Owner MONROE TOWN OF (OPEN SPACE)  
Appraisal \$23,400

Assessment \$16,400

PID 11950 Building Count 1

Survey 1814 C Affordable

## Current Value

Appraisal			
Valuation Year	Improvements	Land	Total
2019	\$0	\$23,400	\$23,400
Assessment			
Valuation Year	Improvements	Land	Total
2019	\$0	\$16,400	\$16,400

## Owner of Record

Owner MONROE TOWN OF (OPEN SPACE) Sale Price \$0  
Co-Owner Certificate 1  
Address 7 FAN HILL RD Book & Page 297/119  
MONROE, CT 06468-1800 Sale Date 10/30/1985  
Instrument

## Ownership History

Ownership History				
Owner	Sale Price	Certificate	Book & Page	Sale Date
MONROE TOWN OF (OPEN SPACE)	\$0	1	297/119	10/30/1985

## Building Information

Building 1 : Section 1

Year Built:  
Living Area: 0

Building Attributes

Field	Description
Style	Vacant Land
Model	
Stories:	
Occupancy	
Exterior Wall 1	
Exterior Wall 2	
Roof Structure:	
Roof Cover	
Interior Wall 1	
Interior Wall 2	
Interior Flr 1	
Interior Flr 2	
Heat Fuel	
Heat Type:	
AC Type:	
Total Bedrooms:	
Total Bthrms:	
Total Half Baths:	
Total Xtra Fixtrs:	
Total Rooms:	
Bath Style:	
Kitchen Style:	
Fireplaces	
Wdstv Flues	
Basement Gar.	
Attic	
Basement	
In Law Apt	

### Building Photo



(<http://images.vgsi.com/photos/MonroeCTPhotos/default.jpg>)

### Building Layout

([http://images.vgsi.com/photos/MonroeCTPhotos/Sketches/11950\\_11950.j](http://images.vgsi.com/photos/MonroeCTPhotos/Sketches/11950_11950.j))

Building Sub-Areas (sq ft)	Legend
No Data for Building Sub-Areas	

### Extra Features

Extra Features	Legend
No Data for Extra Features	

### Land

Land Use	Land Line Valuation
Use Code 903 Description Municipal Zone RF2	Size (Acres) 3.02 Appraised Value \$23,400

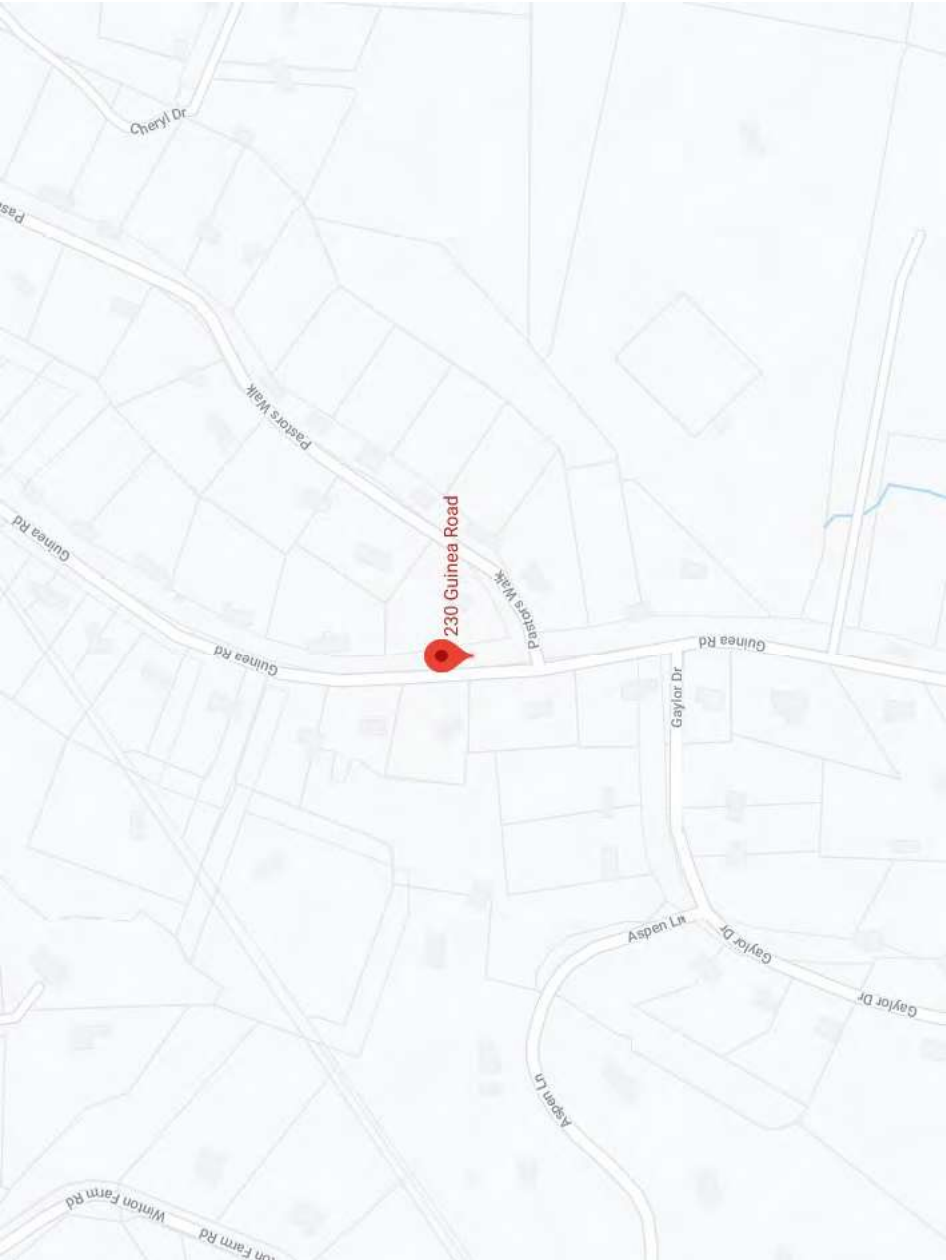
**Outbuildings**

Outbuildings		Legend
		No Data for Outbuildings

**Valuation History**

Appraisal				
Valuation Year	Improvements	Land	Total	
2019	\$0	\$23,400	\$23,400	\$23,400
2019	\$0	\$23,400	\$23,400	\$23,400
2018	\$0	\$24,200	\$24,200	\$24,200

Assessment				
Valuation Year	Improvements	Land	Total	
2019	\$0	\$16,400	\$16,400	\$16,400
2019	\$0	\$16,400	\$16,400	\$16,400
2018	\$0	\$16,900	\$16,900	\$16,900



# Exhibit C

## Construction Drawings



**AT&T SITE NUMBER:** CTL02144  
**AT&T SITE NAME:** MONROE-GUINEA ROAD  
**AT&T FA CODE:** 10035068  
**AT&T PACE NUMBER:** MRCTB047199, MRCTB047301, MRCTB047183,

**BUSINESS UNIT #:** 841294  
**SITE ADDRESS:** 230 GUINEA ROAD  
 MONROE, CT 06468

**AT&T PTN NUMBER:** 2051A0VCW3, 2051A0VALK, 2051A0VBES

**COUNTY:** FAIRFIELD COUNTY  
**TOWER HEIGHT:** 240'  
**SITE TYPE:** SELF SUPPORT

**PROJECT: AT&T 3C & 5G NR**



AT&T SITE NUMBER: CTL02144

**BU #: 841294**  
**MONROE-GUINEA ROAD**  
 230 GUINEA ROAD  
 MONROE, CT 06468

EXISTING  
 240' SELF SUPPORT

**ISSUED FOR:**

REV	DATE	DRWN	DESCRIPTION	DES. QA
A	7/16/20	JJD	PRELIMINARY REVIEW	30P
B	7/27/20	MJC	PRELIMINARY REVIEW	18P
C	8/3/20	MJC	CONSTRUCTION	BMC



**B&T ENGINEERING, INC.**  
 PEC.0001564  
 Expires 2/10/21

IF IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

**SHEET NUMBER:** **REVISION:**

**T-1** **0**

**SITE INFORMATION**

**CROWN CASTLE USA INC.**  
 MONROE-GUINEA ROAD  
 230 GUINEA ROAD  
 MONROE, CT 06468  
**COUNTY:** FAIRFIELD COUNTY  
**MAP/PARCEL #:** NOT PROVIDED  
**AREA OF CONSTRUCTION:** EXISTING  
**LATITUDE:** 41.3418531  
**LONGITUDE:** -73.2745269  
**LAT/LONG TYPE:** NAD83  
**GROUND ELEVATION:** 588'  
**CURRENT ZONING:** RF2  
**JURISDICTION:** FAIRFIELD COUNTY  
**OCCUPANCY CLASSIFICATION:** 0  
**TYPE OF CONSTRUCTION:** IIB  
**A.D.A. COMPLIANCE:** FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION  
**PROPERTY OWNER:** SOUTHWESTERN BELL  
 754 PEACHTREE ST NE 16TH FLR  
 ATLANTA, GA 30308  
**TOWER OWNER:** CROWN CASTLE USA INC  
 2000 CORPORATE DRIVE  
 CANONSBURG, PA 15317  
**CARRIER/APPLICANT:** AT&T TOWER ASSET GROUP  
 7900 XERXES AVE S  
 BLOOMINGTON, MN 55431  
**ELECTRIC PROVIDER:** NOT PROVIDED  
**TELCO PROVIDER:** NOT PROVIDED

**DRAWING INDEX**

SHEET #	SHEET DESCRIPTION
T-1	TITLE SHEET
T-2	GENERAL NOTES
C-1	SITE PLAN
C-1.1	EQUIPMENT PLAN
C-2	FINAL ELEVATION & ANTENNA PLANS
C-3	FINAL ANTENNA & FEEDLINE SCHEDULE
C-4	EQUIPMENT DETAILS
G-1	GROUNDING SCHEMATIC
G-2	GROUNDING DETAILS
ATTACHED	PLUMBING DIAGRAM

ALL DRAWINGS CONTAINED HEREIN ARE FORMATTED FOR 24x36. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

**PROJECT DESCRIPTION**

THE PURPOSE OF THIS PROJECT IS TO ENHANCE BROADBAND CONNECTIVITY AND CAPACITY TO THE EXISTING ELIGIBLE WIRELESS FACILITY.

**TOWER SCOPE OF WORK**

- MODIFY SECTOR MOUNTS PER RECOMMENDATIONS IN MOUNT ANALYSIS REPORT BY POWER OF DESIGN GROUP DATED 5/28/20
- REMOVE (3) POWERWAVE - 7770 ANTENNAS
- REMOVE (3) ERICSSON - RRUS-11 B12 RRHS
- REMOVE (6) POWERWAVE - LCP 21901 DIPLNERS
- REMOVE (3) POWERWAVE - TT19-888H11-1401 TMSAS
- INSTALL (3) CCL - DMP6R-H06DA ANTENNAS
- INSTALL (3) CCL - OP65R-B06DA ANTENNAS
- INSTALL (3) ERICSSON - 4449 B5/B12 RRHS
- INSTALL (3) ERICSSON - 4478 B14 RRHS
- INSTALL (1) RAYCAP - DG6-48-60-0-8F SQUID

**GROUND SCOPE OF WORK**

- REMOVE (1) GALAXY 24 VOLT POWER PLANT & BATTERIES
- INSTALL (1) NETSURE POWER PLANT WITH (3) STRINGS OF BATTERIES
- INSTALL (1) RAYCAP DC12-48-60-25E
- INSTALL (9) UP CONVERTERS (MOUNTING KIT, eSURE POWER EXTEND CONVERTER, 26-POSITION PANEL, NETSURE 7100 # 565459)
- INSTALL (2) ROSENBERGER LEONI - WR-VG86ST-8RD

**LOCATION MAP**



DIRECTIONS FROM BRADLEY INTERNATIONAL AIRPORT:  
 TAKE TERMINAL RD TO BRADLEY FIELD CONNECTOR CT-20. TAKE RAMP ONTO L-91. AT EXIT 32A-32B TAKE RAMP ONTO L-84 [US-6]. AT EXIT 11 TURN RIGHT ONTO MILE HILL RD [WASSERMAN WAY]. TURN RIGHT ONTO CT-34 [BERKSHIRE RD]. KEEP RIGHT ONTO TODDY HILL RD. ROAD NAME CHANGES TO BOTSFORD HILL RD. TURN LEFT ONTO CT-25 [S MAIN ST]. TURN RIGHT ONTO BEAR HILLS RD. TURN LEFT ONTO PINE TREE HILL RD. ROAD NAME CHANGES TO GUINEA RD. ARRIVE AT 230 GUINEA RD, MONROE, CT 06468.

**SITE PHOTO**



**APPLICABLE CODES/REFERENCE DOCUMENTS**

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:

CODE TYPE	CODE
BUILDING	2015 IBC
MECHANICAL	2015 IMC
ELECTRICAL	2017 NEC

**NOTE:**  
 PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE CROWN NOC AT (800) 788-7011 & CROWN CONSTRUCTION MANAGER

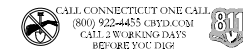
**REFERENCE DOCUMENTS:**

STRUCTURAL ANALYSIS: GPD ENGINEERING  
 DATED: 6/14/20

MOUNT ANALYSIS: POWER OF DESIGN  
 DATED: 5/28/20

RFDS REVISION: 2  
 DATED: 5/27/20

ORDER ID: 517061  
 REVISION: 0

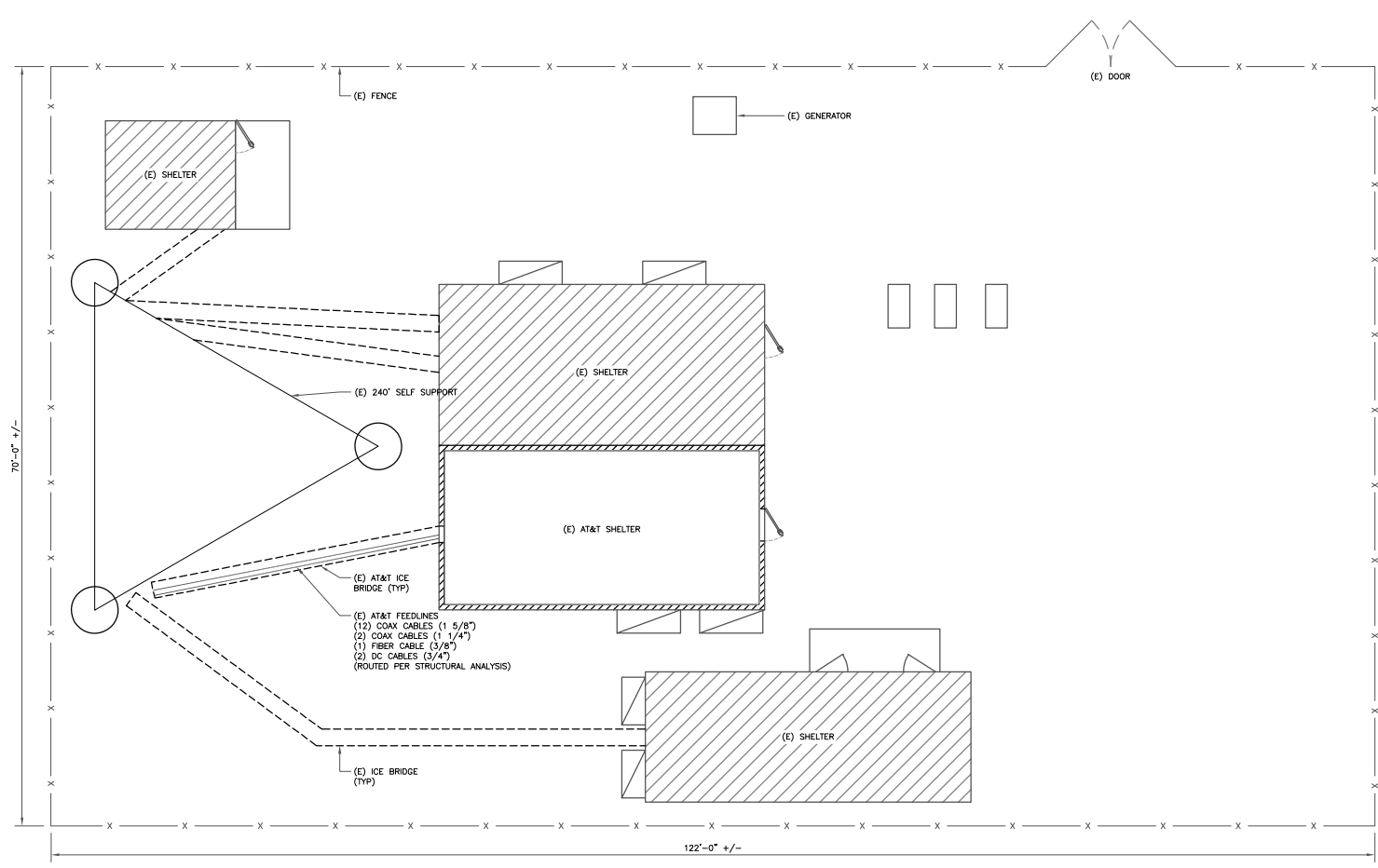


143764.001\_Monroe-Guinea Road\_841294\_Ctl.dwg - Sheet1-1 - User: rccannon - Aug. 03, 2020 - 4:40pm





1:43764.0011\_Monroe-Guinea Road\_841294\_C1.dwg - Sheet C-1 - User: rconson - Aug 03, 2020 - 4:40pm



1 SITE PLAN  
 SCALE: 3/8"=1'-0" (FULL SIZE)  
 3/16"=1'-0" (11x17)



575 MOROSGO DRIVE  
 ATLANTA, GA 30324-3300

3530 TORINGDON WAY, SUITE 300  
 CHARLOTTE, NC 28277

1717 S. BOULDER  
 SUITE 300  
 TULSA, OK 74110  
 PH: (918) 587-4830  
 www.btgrp.com

AT&T SITE NUMBER: CTL02144

BU #: 841294  
**MONROE-GUINEA ROAD**

230 GUINEA ROAD  
 MONROE, CT 06468

EXISTING  
 240' SELF SUPPORT

ISSUED FOR:

REV	DATE	DRWN	DESCRIPTION	DES	QA
A	7/16/20	JJD	PRELIMINARY REVIEW	30P	
B	7/27/20	MJC	PRELIMINARY REVIEW	138P	
C	8/3/20	MJC	CONSTRUCTION	81C	

8/3/20

B&T ENGINEERING, INC.  
 PEC.0001564  
 Expires 2/10/21

IF IN A QUESTION OF LAW FOR ANY PERSON,  
 UNLESS THEY ARE ACTING UNDER THE DIRECTION  
 OF A LICENSED PROFESSIONAL ENGINEER,  
 TO ALTER THIS DOCUMENT.

SHEET NUMBER: **C-1** REVISION: **0**



575 MOROSGO DRIVE  
ATLANTA, GA 30324-3300



3530 TORINGDON WAY, SUITE 300  
CHARLOTTE, NC 28277



1717 S. BOULDER  
SUITE 300  
TULSA, OK 74119  
PH: (918) 587-4830  
www.blgrp.com

AT&T SITE NUMBER: CTL02144

BU #: 841294  
MONROE-GUINEA ROAD

230 GUINEA ROAD  
MONROE, CT 06468

EXISTING  
240' SELF SUPPORT

ISSUED FOR:

REV	DATE	DRSN	DESCRIPTION	DES.	QA
A	7/16/20	JJD	PRELIMINARY REVIEW	JRP	
B	7/27/20	MJC	PRELIMINARY REVIEW	JRP	
0	8/3/20	MJC	CONSTRUCTION	BMC	

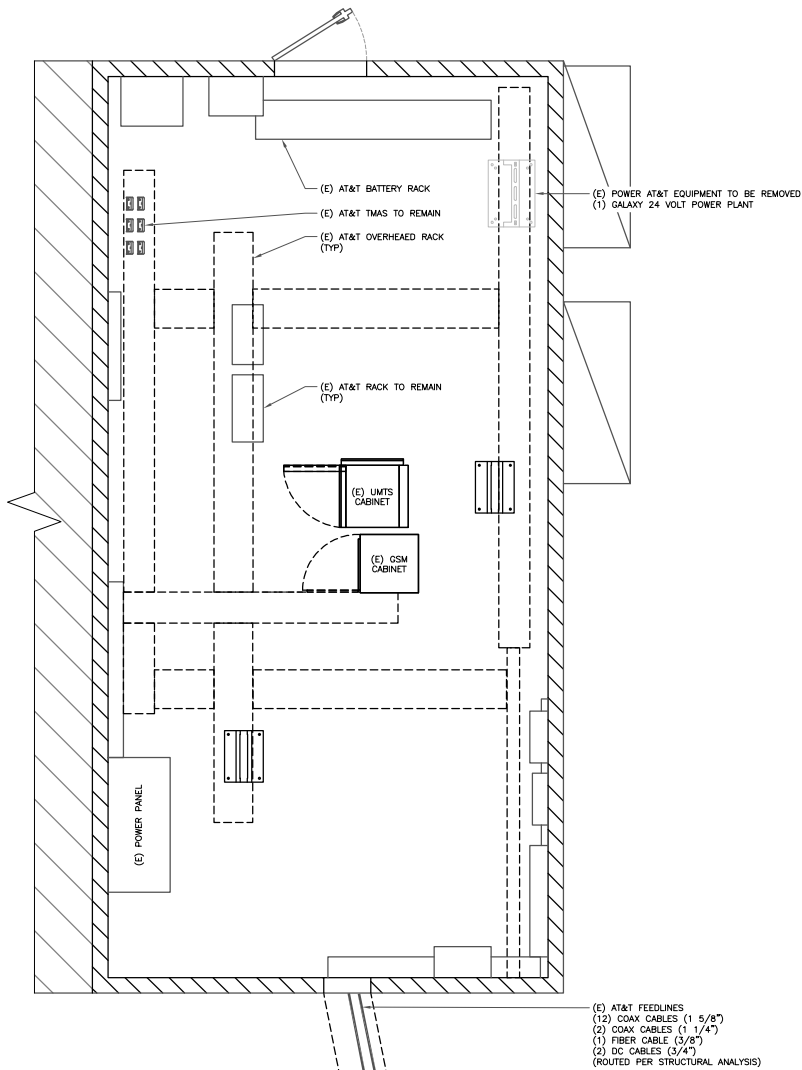


8/3/20

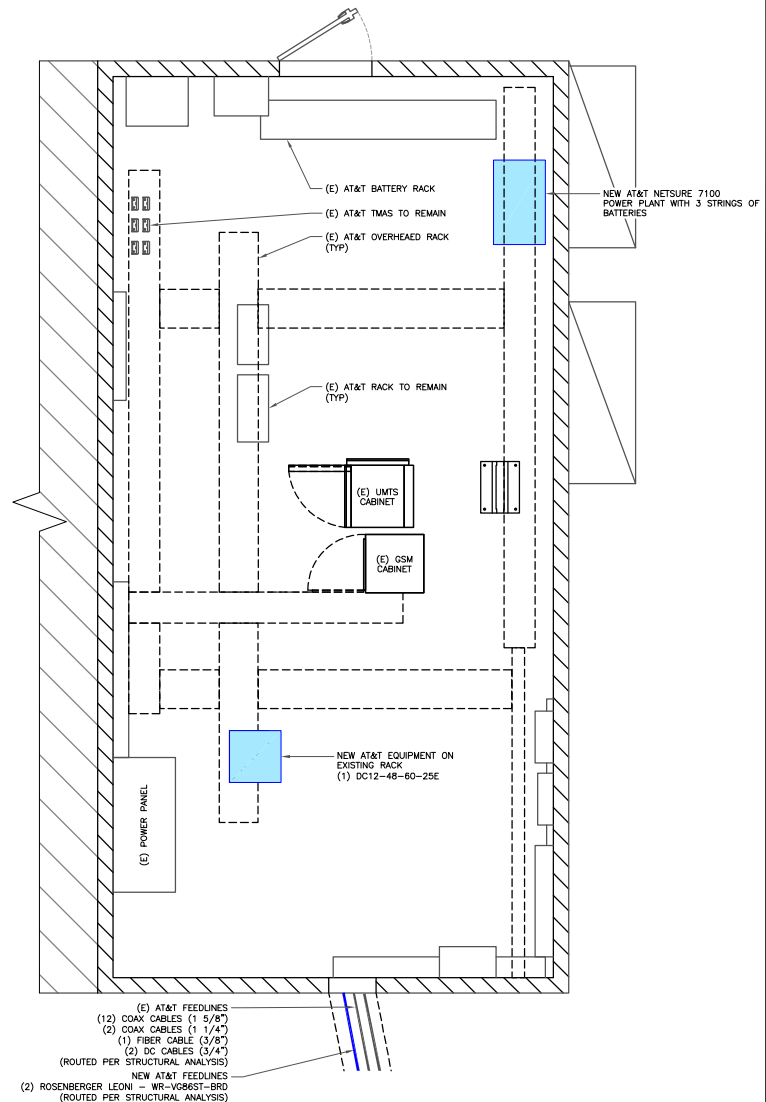
B&T ENGINEERING, INC.  
PEC.0001564  
Expires 2/10/21

IF IN A VIOLENT OR LITIGIOUS PERSON,  
UNLESS THEY ARE ACTING UNDER THE DIRECTION  
OF A LICENSED PROFESSIONAL ENGINEER,  
TO ALTER THE DOCUMENT.

SHEET NUMBER: **C-1.1** REVISION: **0**



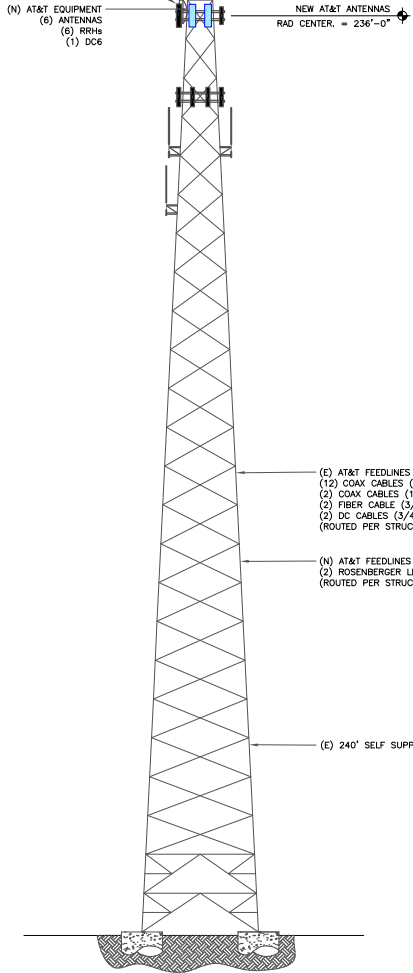
1 SITE PLAN  
SCALE: 3/8"=1'-0" (FULL SIZE)  
3/16"=1'-0" (1:12)



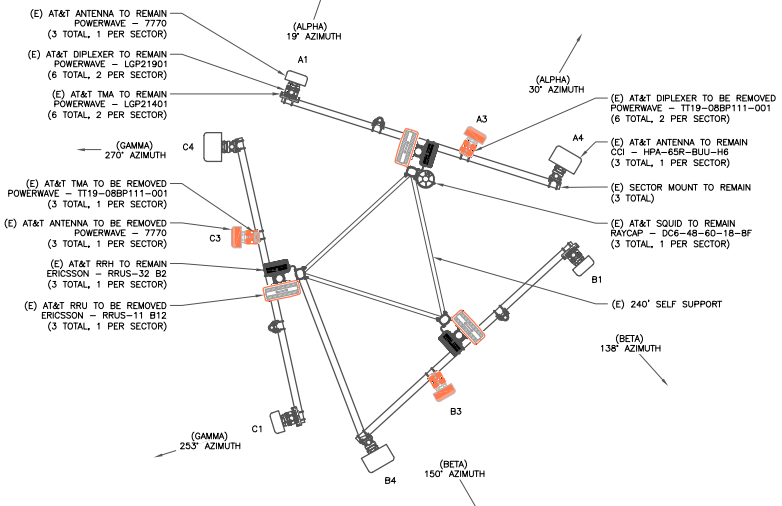
1 PROPOSED SITE PLAN  
SCALE:



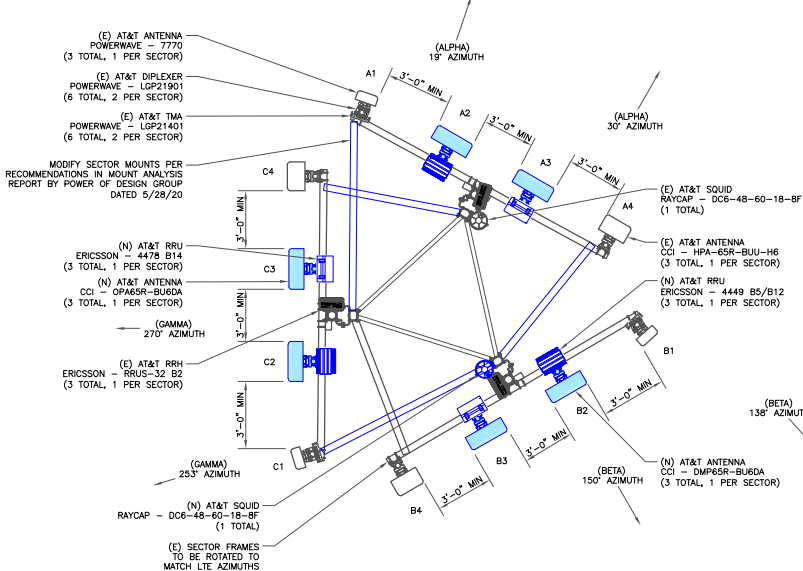
MODIFY SECTOR MOUNTS PER RECOMMENDATIONS IN MOUNT ANALYSIS REPORT BY POWER OF DESIGN GROUP DATED 5/28/20



1 FINAL ELEVATION  
SCALE: NOT TO SCALE



2 EXISTING ANTENNA PLAN  
SCALE: NOT TO SCALE



3 FINAL ANTENNA PLAN  
SCALE: NOT TO SCALE

**"LOOK UP" - CROWN CASTLE USA, INC. SAFETY CLIMB REQUIREMENT:**  
THE INTEGRITY OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY ON THE STRUCTURE. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO: PINCHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM ITS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTIONAL WEAR, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO IMPEDE/BLOCK ITS INTENDED USE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGGED OUT AND REPORTED TO YOUR CROWN CASTLE USA, INC. PAC OR CALL THE NCC TO GENERATE A SAFETY CLIMB MAINTENANCE AND CONTRACTOR NOTICE TICKET.

- NOTES:**
1. REFERENCE C-3 FOR FINAL EQUIPMENT SCHEDULE.
  2. REFERENCE C-4 FOR NEW EQUIPMENT SPECIFICATIONS.
  3. CONTRACTOR TO VERIFY ALL ANTENNA TIP HEIGHTS DO NOT EXCEED BEACON BASE HEIGHT.

**MOUNT ANALYSIS NOTES:**

1. THE DESIGN DEPICTED IN THESE DRAWINGS IS VALID WHEN ACCOMPANIED BY A CORRESPONDING PASSING MOUNT ANALYSIS.
2. CONSTRUCTION MANAGER / GENERAL CONTRACTOR SHALL REVIEW THE MOUNT ANALYSIS FOR ANY CONDITIONS PRIOR TO INSTALLATION.
3. ANY REQUIRED MOUNT MODIFICATION DESIGN OR MOUNT REPLACEMENT SHALL BE APPROVED BY EOR.



AT&T SITE NUMBER: CTL02144

BU #: 841294  
MONROE-GUINEA ROAD

230 GUINEA ROAD  
MONROE, CT 06468

EXISTING  
240' SELF SUPPORT

ISSUED FOR:

REV	DATE	DRSN	DESCRIPTION	DES. QA
A	7/16/20	JJD	PRELIMINARY REVIEW	JJD
B	7/27/20	MJC	PRELIMINARY REVIEW	TRF
C	8/3/20	MJC	CONSTRUCTION	BMC



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1:43764.001\_Monroe-Guinea Road\_841294\_C3a.dwg - Sheet C-2 - User: rcranon - Aug 03, 2020 - 4:49pm



AT&T SITE NUMBER: CTL02144

BU #: 841294  
**MONROE-GUINEA ROAD**  
 230 GUINEA ROAD  
 MONROE, CT 06468  
 EXISTING  
 240' SELF SUPPORT

**ISSUED FOR:**

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B	7/27/20	MJC	PRELIMINARY REVIEW	18P
C	8/3/20	MJC	CONSTRUCTION	8M



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C-3

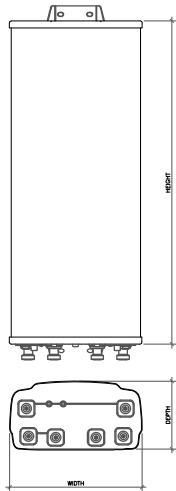
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**FINAL ANTENNA AND FEEDLINE SCHEDULE**

POS.	TECH	STATUS	AZIMUTH	ANTENNA TYPE	ANTENNA RAD CENTER	MECHANICAL DOWNTILT	ELECTRICAL DOWNTILT	MAIN COAX SIZE	MAIN COAX LENGTH	COAX QTY	TMA QTY AND MODEL	SURGE PROTECTION	DC/FIBER CABLES	RRHs QTY & MODEL ON TOWER	LOCATION	DIPLEXER ON TOWER	DIPLEXER ON GROUND	RET CABLE
<b>ALPHA SECTOR</b>																		
A1	UMTS 850	EXISTING	19°	POWERWAVE 7770	236'-0"	0°	4°	1 5/8"	260'-0"	2	(2) LGP 21401	DC6-48-60-18-8F	(1) 3/8" FIBER (2) 1/2" DC (12) COAX	-	TOWER	Y	N	Y
A2	LTE 700/ LTE 850/5G	NEW	30°	CCI DMP65R-BU6DA	236'-0"	0°	2°/2°/2°	-	-	-	-			(1) 4449 B5/B12	TOWER	N	N	N
A3	LTE 700	NEW	30°	CCI OPA65R-BU6DA	236'-0"	0°	2°	-	-	-	-			(1) 4478 B14	TOWER	N	N	N
A4	LTE 1900	EXISTING	30°	CCI HPA-65R-BUU-H6	236'-0"	0°	3°/3°	1 5/8"	-	2	-			(1) RRUS-32 B2	TOWER	N	N	N
<b>BETA SECTOR</b>																		
B1	UMTS 850	EXISTING	138°	POWERWAVE 7770	236'-0"	0°	4°	1 5/8"	260'-0"	2	(2) LGP 21401	DC6-48-60-0-8F	(2) 1/2" DC	-	TOWER	Y	N	Y
B2	LTE 700/ LTE 850/5G	NEW	150°	CCI DMP65R-BU6DA	236'-0"	0°	2°/2°/2°	-	-	-	-			(1) 4449 B5/B12	TOWER	N	N	N
B3	LTE 700	NEW	150°	CCI OPA65R-BU6DA	236'-0"	0°	2°	-	-	-	-			(1) 4478 B14	TOWER	N	N	N
B4	LTE 1900	EXISTING	150°	CCI HPA-65R-BUU-H6	236'-0"	0°	4°/4°	1 5/8"	-	2	-			(1) RRUS-32 B2	TOWER	N	N	N
<b>GAMMA SECTOR</b>																		
C1	UMTS 850	EXISTING	253°	POWERWAVE 7770	236'-0"	0°	2°	1 5/8"	260'-0"	2	(2) LGP 21401	-	-	-	TOWER	Y	N	Y
C2	LTE 700/ LTE 850/5G	NEW	270°	CCI DMP65R-BU6DA	236'-0"	0°	0°/0°/0°	-	-	-	-			(1) 4449 B5/B12	TOWER	N	N	N
C3	LTE 700	NEW	270°	CCI OPA65R-BU6DA	236'-0"	0°	4°	-	-	-	-			(1) 4478 B14	TOWER	N	N	N
C4	LTE 1900	EXISTING	270°	CCI HPA-65R-BUU-H6	236'-0"	0°	2°/2°	1 5/8"	-	2	-			(1) RRUS-32 B2	TOWER	N	N	N

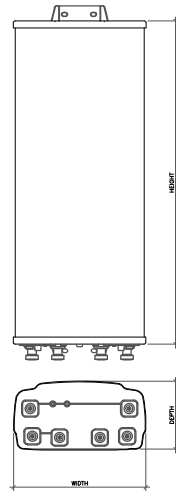
NOTE: BOLD DENOTES NEW EQUIPMENT

1 FINAL ANTENNA AND FEEDLINE SCHEDULE  
 SCALE: NOT TO SCALE



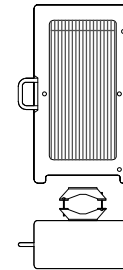
ANTENNA DIMENSIONS (INCHES)				
MODEL	HEIGHT	WIDTH	DEPTH	WEIGHT
DMP65R-BU6DA	71"	20.7"	7.7"	79.4 lbs

① CCI - DMP65R-BU6DA  
SCALE: NOT TO SCALE



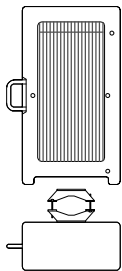
ANTENNA DIMENSIONS (INCHES)				
MODEL	HEIGHT	WIDTH	DEPTH	WEIGHT
OPA65R-BU6DA	71"	21"	7.8"	60.2 lbs

② CCI - OPA65R-BU6DA  
SCALE: NOT TO SCALE



ERICSSON - 4478 B14  
WEIGHT (FULLY EQUIPPED): 59.4 LBS  
SIZE (HxWxD): 18.1x13.4x8.26 IN.

③ ERICSSON - 4478 B14  
SCALE: NOT TO SCALE

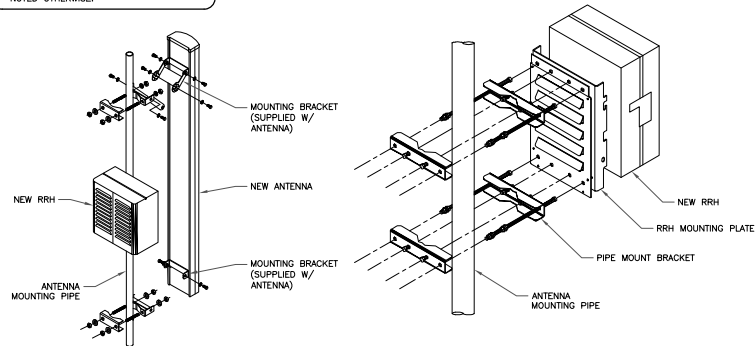


ERICSSON - 4449 B5/B12  
WEIGHT (FULLY EQUIPPED): 75 LBS  
SIZE (HxWxD): 15x13.2x10.4 IN.

④ ERICSSON - 4449 B5/B12  
SCALE: NOT TO SCALE

**INSTALLER NOTES:**

1. COMPLY WITH MANUFACTURERS INSTRUCTIONS TO ENSURE THAT ALL RRHs RECEIVE ELECTRICAL POWER WITHIN 24 HOURS OF BEING REMOVED FROM THE MANUFACTURER'S PACKAGING.
2. DO NOT OPEN RRH PACKAGES IN THE RAIN.
3. ALL PIPES, BRACKETS, AND MISCELLANEOUS HARDWARE TO BE GALVANIZED UNLESS NOTED OTHERWISE.



⑤ ANTENNA WITH RRH MOUNTING DETAIL  
SCALE: NOT TO SCALE



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MONROE-GUINEA ROAD

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EXISTING  
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C	8/3/20	MJC	CONSTRUCTION	81C



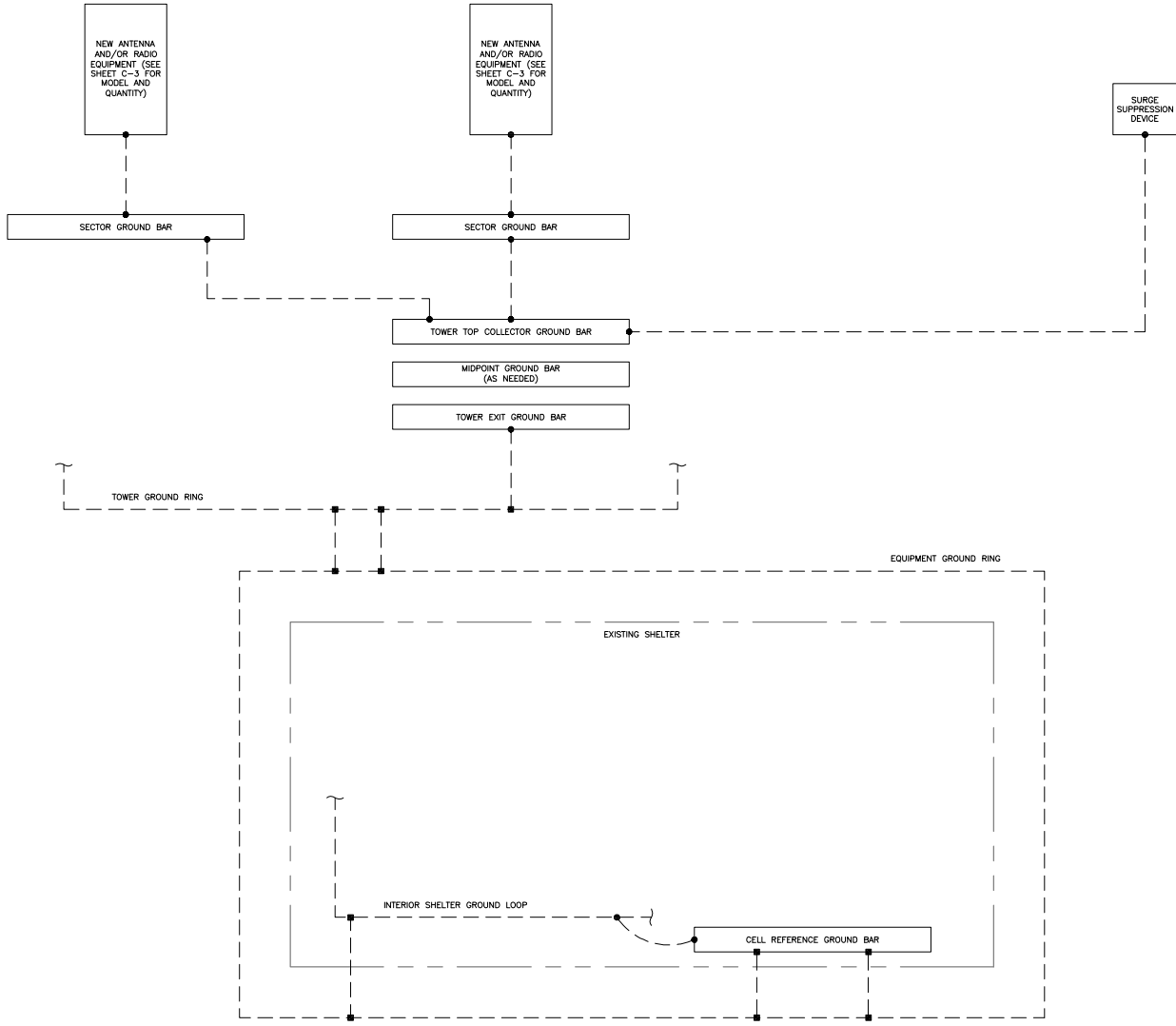
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C-4 0

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1 GROUNDING SCHEMATIC  
SCALE: NOT TO SCALE

**GROUNDING PLAN LEGEND:**  
 --- GROUND WIRE  
 ■ EXOTHERMIC WELD  
 ● MECHANICAL CONNECTION  
 ○ COPPER GROUND ROD  
 ⊗ GROUND ROD W/ TEST WELL

**CELL REFERENCE GROUND BAR:** POINT OF GROUND REFERENCE FOR ALL COMMUNICATIONS EQUIPMENT FRAMES. ALL BONDS ARE MADE WITH #2 STRANDED GREEN INSULATED COPPER CONDUCTORS. BOND TO GROUND RING WITH (2) #2 SOLID TINNED COPPER CONDUITS (ATT-TP-76416 7.6.7).

**HATCH-PLATE GROUND BAR:** BOND TO THE INTERIOR GROUND RING WITH (2) #2 STRANDED GREEN INSULATED COPPER CONDUCTORS. WHEN A HATCH-PLATE AND A CELL REFERENCE GROUND BAR ARE BOTH PRESENT, THE CELL SITE REFERENCE GROUND BAR MUST BE CONNECTED TO THE HATCH-PLATE AND TO THE INTERIOR GROUND RING USING (2) #2 STRANDED GREEN INSULATED COPPER CONDUCTORS.

**EXTERIOR CABLE ENTRY PORT GROUND BARS:** LOCATED AT THE ENTRANCE TO THE CELL SITE BUILDING. BOND TO GROUND RING WITH A #2 SOLID TINNED COPPER CONDUCTORS WITH AN EXOTHERMIC WELD AND INSPECTION SLEEVE (ATT-TP-76416 7.6.7.2).

**DURING ALL DC POWER SYSTEM CHANGES** INCLUDING DC SYSTEM CHANGE OUTS, RECTIFIER REPLACEMENTS OR ADDITIONS, BREAKER DISTRIBUTION CHANGES, BATTERY ADDITIONS, BATTERY REPLACEMENTS AND INSTALLATIONS OR CHANGES TO DC CONVERTER SYSTEMS IT SHALL BE REQUIRED THAT SERVICES CONTRACTORS VERIFY ALL DC POWER SYSTEMS ARE EQUIPPED WITH MASTER DC SYSTEM RETURN GROUND CONDUCTOR FROM THE DC POWER SYSTEM COMMON RETURN BUS DIRECTLY CONNECTED TO THE CELL SITE REFERENCE GROUND BAR PER TP76300 SECTION H 6 AND TP76416 FIGURE 7-11 REQUIREMENTS.



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MONROE-GUINEA ROAD

230 GUINEA ROAD  
MONROE, CT 06468

EXISTING  
240' SELF SUPPORT

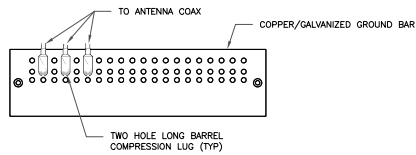
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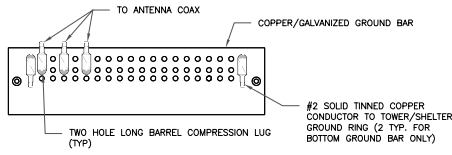
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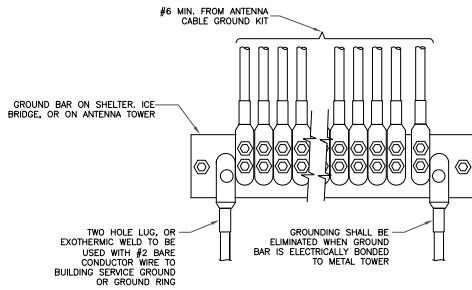
- NOTES:
1. DOUBLING UP "OR STACKING" OF CONNECTIONS IS NOT PERMITTED.
  2. EXTERIOR ANTIOXIDANT JOINT COMPOUND TO BE USED ON ALL EXTERIOR CONNECTIONS.
  3. GROUND BAR SHALL NOT BE ISOLATED FROM TOWER. MOUNT DIRECTLY TO TOWER STEEL.

1 ANTENNA GROUND BAR DETAIL  
SCALE: NOT TO SCALE

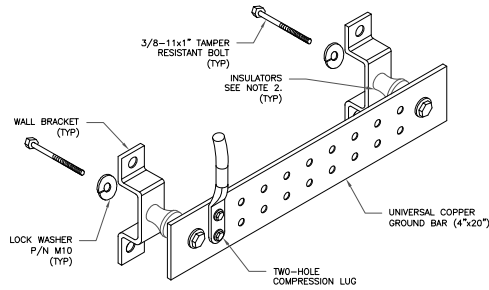


- NOTES:
1. EXTERIOR ANTIOXIDANT JOINT COMPOUND TO BE USED ON ALL EXTERIOR CONNECTIONS.
  2. GROUND BAR SHALL NOT BE ISOLATED FROM TOWER. MOUNT DIRECTLY TO TOWER STEEL (TOWER ONLY).
  3. GROUND BAR SHALL BE ISOLATED FROM BUILDING OR SHELTER.

2 TOWER/SHELTER GROUND BAR DETAIL  
SCALE: NOT TO SCALE

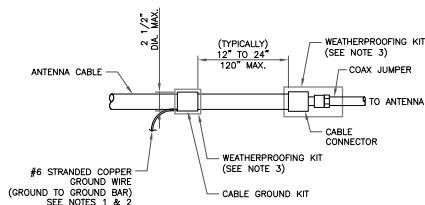


4 GROUNDWIRE INSTALLATION  
SCALE: NOT TO SCALE



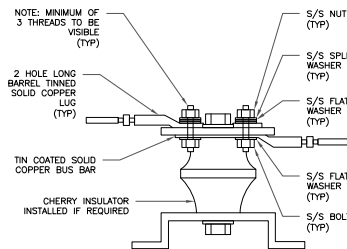
- NOTES:
1. DOWN LEAD (HOME RUN) CONDUCTORS ARE NOT TO BE INSTALLED ON CROWN CASTLE USA INC. TOWER. PER THE GROUNDING DOWN CONDUCTOR POLICY QAS-ST0-10091. NO MODIFICATION OR DRILLING TO TOWER STEEL IS ALLOWED IN ANY FORM OR FASHION. CAD-WELDING ON THE TOWER AND/OR IN THE AIR ARE NOT PERMITTED.
  2. OMIT INSULATOR WHEN MOUNTING TO TOWER STEEL OR PLATFORM STEEL. USE INSULATORS WHEN ATTACHING TO BUILDING OR SHELTERS.

5 GROUND BAR DETAIL  
SCALE: NOT TO SCALE



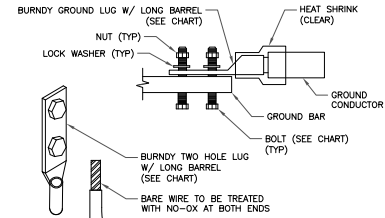
- NOTES:
1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.
  2. GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.
  3. WEATHER PROOFING SHALL BE TWO-PART TAPE KIT, COLD SHRINK SHALL NOT BE USED.

6 CABLE GROUND KIT CONNECTION  
SCALE: NOT TO SCALE



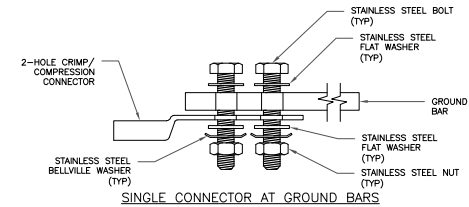
7 LUG DETAIL  
SCALE: NOT TO SCALE

WIRE SIZE	BURNDY LUG	BOLT SIZE
#6 GREEN INSULATED	YA6C-2TC38	3/8" - 16 NC SS 2 BOLT
#2 SOLID TINNED	YA3C-2TC38	3/8" - 16 NC SS 2 BOLT
#2 STRANDED	YA2C-2TC38	3/8" - 16 NC SS 2 BOLT
#2/0 STRANDED	YA26-2TC38	3/8" - 16 NC SS 2 BOLT
#4/0 STRANDED	YA28-2N	1/2" - 16 NC SS 2 BOLT

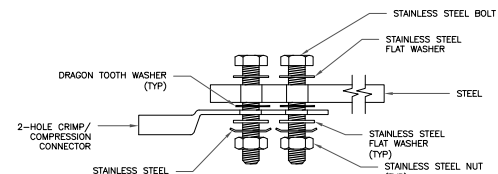


- NOTE:
- ALL GROUNDING LUGS ARE TO BE INSTALLED PER MANUFACTURER'S SPECIFICATIONS. ALL HARDWARE BOLTS, NUTS, LOCK WASHERS SHALL BE STAINLESS STEEL. ALL HARDWARE ARE TO BE AS FOLLOWS: BOLT, FLAT WASHER, GROUND BAR, GROUND LUG, FLAT WASHER AND NUT.

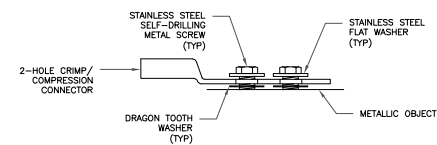
3 MECHANICAL LUG CONNECTION  
SCALE: NOT TO SCALE



SINGLE CONNECTOR AT GROUND BARS



SINGLE CONNECTOR AT STEEL OBJECTS



SINGLE CONNECTOR AT METALLIC/STEEL OBJECTS

8 HARDWARE DETAIL FOR EXTERIOR CONNECTIONS  
SCALE: NOT TO SCALE



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0	8/3/20	MJC	CONSTRUCTION	BMC

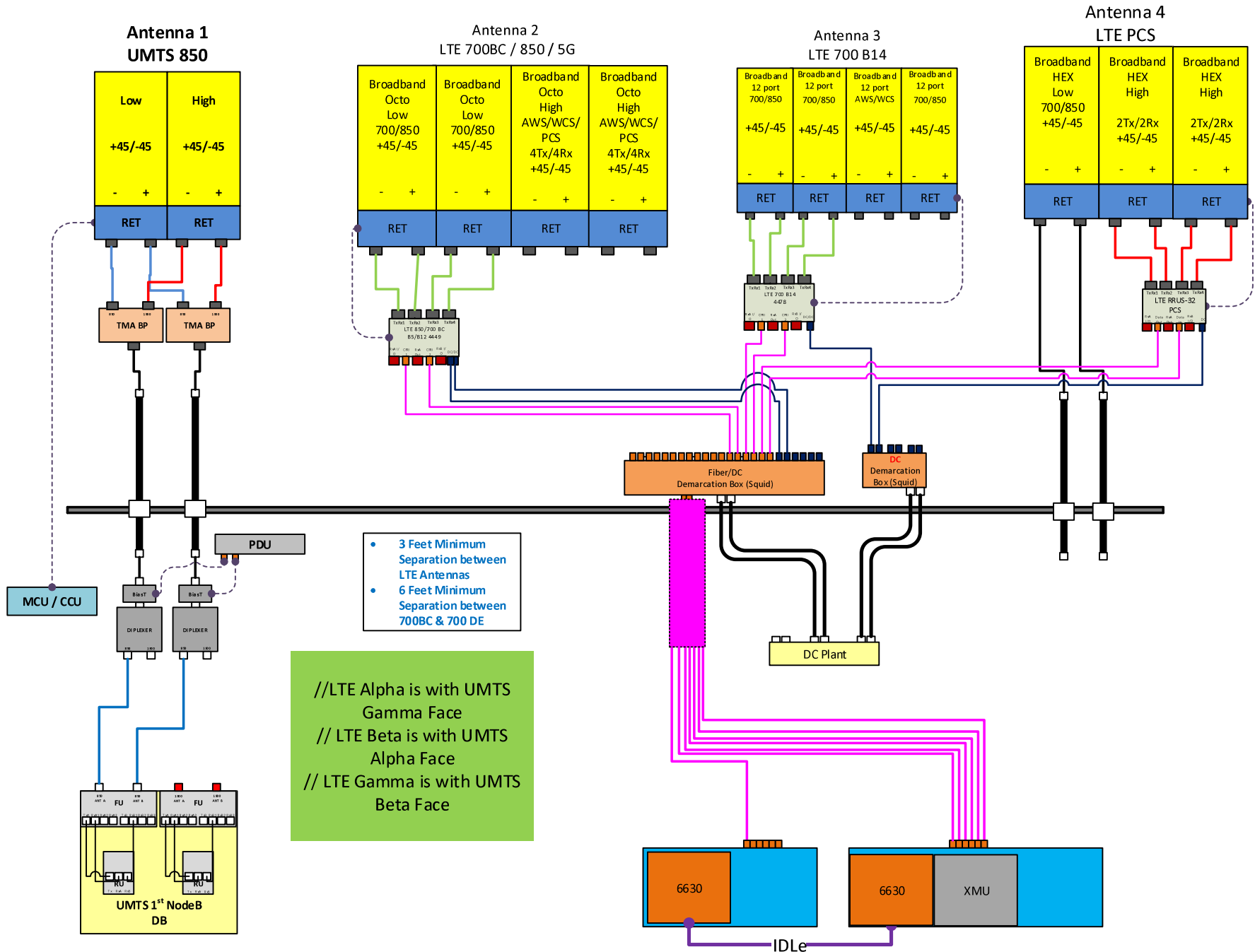


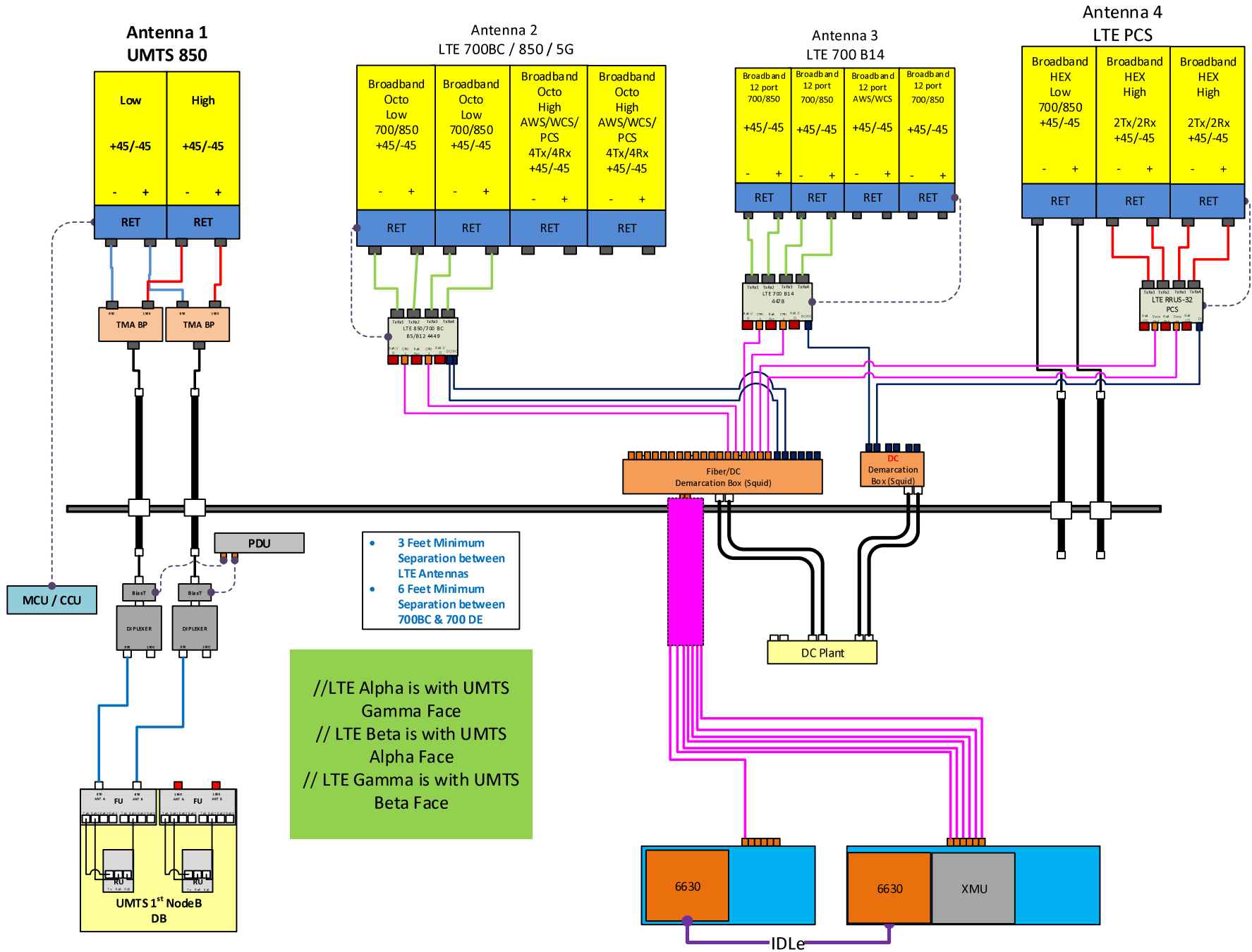
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PEC.0001564  
Expires 2/10/21

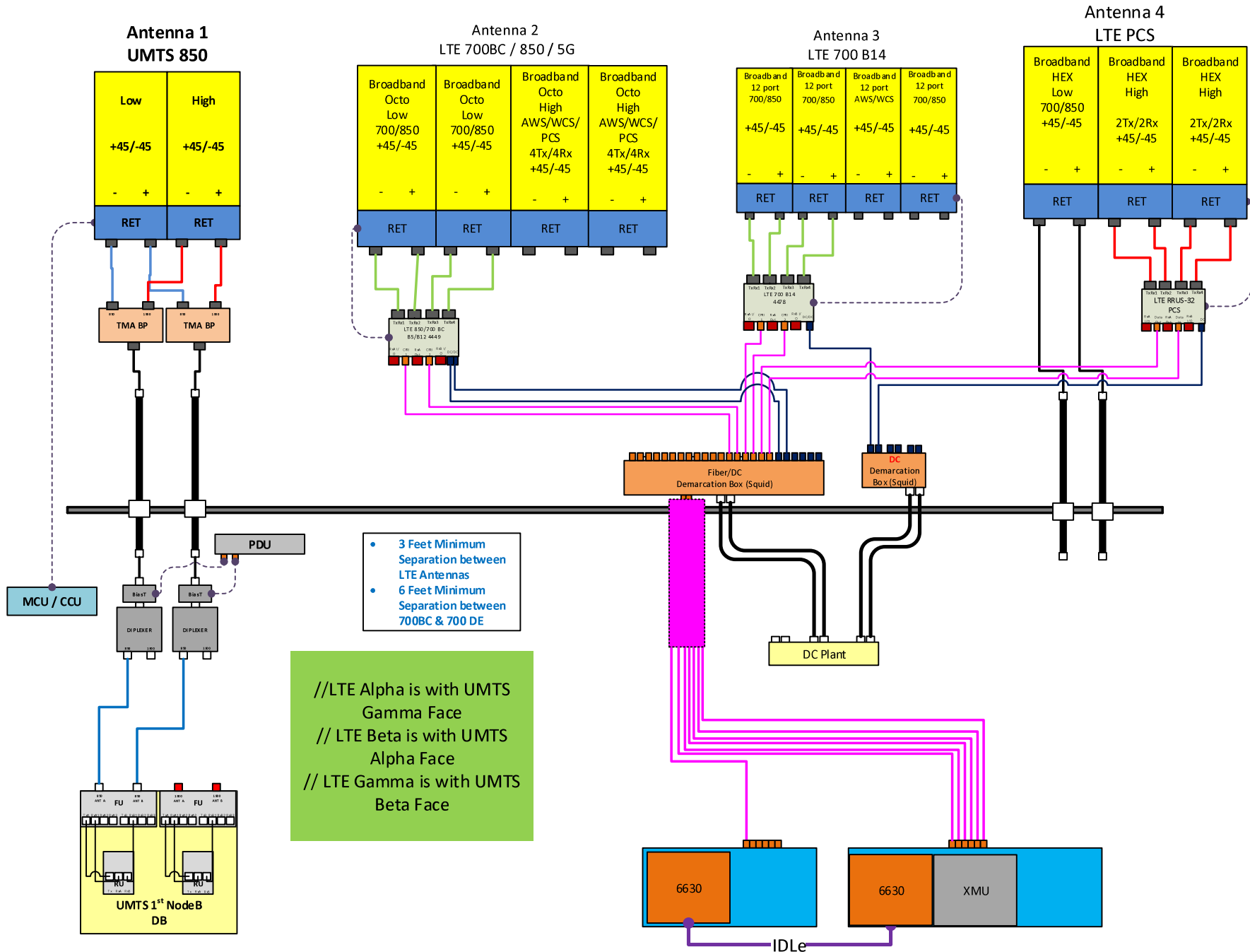
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# Exhibit D

## Structural Analysis Report

Date: **June 4, 2020**

Denice Nicholson  
Crown Castle  
3 Corporate Dr  
Clifton Park, NY 12065



520 South Main Street Suite 2531  
Akron, Ohio 44311  
(216) 927-8663

**Subject:**

**Structural Analysis Report**

**Carrier Designation:**

**AT&T Mobility Co Co-Locate**

**Carrier Site Name:**

CTL02144

**Carrier Site Number:**

60427

**Crown Castle Designation:**

**Crown Castle BU Number:**

841294

**Crown Castle Site Name:**

MONROE-GUINEA ROAD

**Crown Castle JDE Job Number:**

605386

**Crown Castle Work Order Number:**

1856333

**Crown Castle Order Number:**

517061 Rev. 0

**Engineering Firm Designation:**

**GPD Project Number:**

2020777.841294.15

**Site Data:**

**230 Guinea Road, Monroe, CT 06468, Fairfield County  
Latitude 41° 20' 30.68", Longitude -73° 16' 30.68"  
242.917 Foot – Modified Rohn Self Support Tower**

Dear Denice Nicholson,

We are pleased to submit this **"Structural Analysis Report"** to determine the structural integrity of the above mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Proposed Equipment Configuration

**Sufficient Capacity – 75.7%**

This analysis utilizes an ultimate 3-second gust wind speed of 120 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: Matt Steward

Respectfully submitted by:

Christopher J. Schecks, P.E.  
Connecticut #: 0030026



6/4/2020

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- 1) INTRODUCTION**
- 2) ANALYSIS CRITERIA**
  - Table 1 - Proposed Equipment Configuration
  - Table 2 - Other Considered Equipment
- 3) ANALYSIS PROCEDURE**
  - Table 3 - Documents Provided
  - 3.1) Analysis Method
  - 3.2) Assumptions
- 4) ANALYSIS RESULTS**
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  - Table 5 - Tower Component Stresses vs. Capacity – LC7
  - 4.1) Recommendations
  - Table 6 – Dish Twist/Sway Results for 60 MPH Service Wind Speed
- 5) APPENDIX A**
  - tnxTower Output
- 6) APPENDIX B**
  - Base Level Drawing
- 7) APPENDIX C**
  - Additional Calculations

**1) INTRODUCTION**

This tower is a 242.9 ft self-support tower designed by Rohn in June of 1990.  
 The tower has been modified multiple times to accommodate additional loading.

**2) ANALYSIS CRITERIA**

**TIA-222 Revision:** TIA-222-H  
**Risk Category:** II  
**Wind Speed:** 120 mph  
**Exposure Category:** B  
**Topographic Factor:** 1  
**Ice Thickness:** 1.5 in  
**Wind Speed with Ice:** 50 mph  
**Service Wind Speed:** 60 mph

**Table 1 - Proposed Equipment Configuration**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	
230.0	232.0	3	CCI Antennas	HPA-65R-BUU-H6			
		3	CCI Antennas	DMP65R-BU8D			
		3	CCI Antennas	OPA65R-BU6D			
			3	Powerwave Technologies	7770.00		
			3	Ericsson	RADIO 4449 B5/12	1	3/8
		230.0	3	Ericsson	RRUS 32 B2	4	3/4
			3	Ericsson	RRUS 4478 B14	12	1-5/8
			6	Powerwave Technologies	LGP13519		
			2	Raycap	DC6-48-60-18-8F		
			3		12.5 ft Frame Mount		
201.0	207.0	2	Kathrein	OG-4			
	201.0	2	-	Side Arm Mount [SO 306-1]	2	1-1/4	

Elevations have been taken from the bottom of the tower steel, which is 2.0' above ground level

**Table 2 - Other Considered Equipment**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
240.0	242.0	1	Decibel	DB806-XC	9	1/2
		1	Kathrein	FMO		
240.0	240.0	2	Commscope	USX6-6W-6GR	19	1-5/8
		4	SAF	FREEMILE 24 GHZ FODU		
		1	-	Side Arm Mount [SO 303-3]		
215.0	218.0	6	Commscope	JAHH-65B-R3B	1	1/2
		3	Andrew	HBXX-6517DS-A2M		
		3	Andrew	LNX-8514DS-A1M		
		3	Alcatel Lucent	RRH2X60-700		
		3	Alcatel Lucent	B66A RRH4X45		
		1	-	Sector Mount [SM 503-3]		
186.0	212.0	2	RFS/Celwave	DB-T1-6Z-8AB-0Z	2	7/8 1/2
	188.0	1	Andrew	DB589-A		
	186.0	1	-	Side Arm Mount [SO 301-1]		
	184.0	1	Andrew	DB589-A		
12.0	12.0	1	Kathrein	TY-840	1	1/2

Elevations have been taken from the bottom of the tower steel, which is 2.0' above ground level

**3) ANALYSIS PROCEDURE**

**Table 3 - Documents Provided**

Document	Remarks	Reference	Source
Tower Drawings	Rohn File #: 25692, dated: 07/05/1990	4841385	CCISITES
Foundation Investigation	GPD Project #: 2015777.841294.06, dated 06/11/2015	4468667	CCISITES
Geotechnical Report	WEI Project #: 2009-901, dated 09/16/2009	4468666	CCISITES
Boring Log Review	GPD Project #: 2015777.841294.07, dated 06/17/2015	5751301	CCISITES
Modification Design	GPD Project #: 2009268.80 Rev. A, dated 10/12/2009	4601540	CCISITES
Post-Mod Inspection	GPD Project #: 2009591.00, dated 01/13/2010	4601541	CCISITES
Modification Design	GPD Project #: 2014777.841294.04, dated 09/22/2014	5306639	CCISITES
Post-Mod Inspection	GPD Project #: 2015777.841294.05, dated 06/17/2015	5750961	CCISITES



### 3.1) Analysis Method

tnxTower (version 8.0.5.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Caste has calculated and provided the effective area for panel antennas using approved methods following the intent of the of the TIA-222 standard.

### 3.2) Assumptions

- 1) The tower and structures were maintained in accordance with the TIA-222 standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions or items in Table 3 are not valid or have been made in error. GPD should be notified to determine the effect on the structural integrity of the tower.

## 4) ANALYSIS RESULTS

**Table 4 - Section Capacity (Summary)**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	244.917 - 224.792	Leg	ROHN 2.5 STD	3	-16.02	66.58	24.1	Pass
T2	224.792 - 204.625	Leg	ROHN 3 EH	39	-45.93	115.74	39.7	Pass
T3	204.625 - 184.438	Leg	ROHN 3.5 EH	69	-74.24	131.32	56.5	Pass
T4	184.438 - 164.229	Leg	ROHN 4 EH	90	-103.19	167.14	61.7	Pass
T5	164.229 - 144.021	Leg	ROHN 5 EH	111	-131.94	250.61	52.6	Pass
T6	144.021 - 123.813	Leg	ROHN 5 EH	132	-157.78	209.91	75.2	Pass
T7	123.813 - 103.604	Leg	ROHN 6 EH	147	-186.12	317.44	58.6	Pass
T8	103.604 - 83.3333	Leg	ROHN 6 EH	162	-214.93	317.01	67.8	Pass
T9	83.3333 - 63	Leg	ROHN 6 EH	177	-243.98	316.57	77.1	Pass
T10	63 - 42.6667	Leg	ROHN 8 EHS	192	-273.24	404.01	67.6	Pass
T11	42.6667 - 22.3334	Leg	ROHN 8 EHS	207	-279.00	404.01	69.1	Pass
T12	22.3334 - 2	Leg	ROHN 8 EH	240	-307.42	529.11	58.1	Pass
T1	244.917 - 224.792	Diagonal	L1-3/4x1-3/4x3/16	11	-3.76	11.77	31.9 61.8 (b)	Pass
T2	224.792 - 204.625	Diagonal	L1-3/4x1-3/4x3/16	47	-4.25	6.64	63.9 74.7 (b)	Pass
T3	204.625 - 184.438	Diagonal	L2-1/2x2-1/2x3/16	74	-4.75	12.21	38.9 73.0 (b)	Pass
T4	184.438 - 164.229	Diagonal	L2-1/2x2-1/2x1/4	94	-5.36	12.04	44.5 61.1 (b)	Pass
T5	164.229 - 144.021	Diagonal	L2-1/2x2-1/2x5/16	115	-5.99	11.52	52.0 55.5 (b)	Pass
T6	144.021 - 123.813	Diagonal	L 3 x 3 x 5/16	136	-7.25	13.67	53.0	Pass
T7	123.813 - 103.604	Diagonal	L3-1/2x3-1/2x1/4	151	-7.97	15.14	52.6 71.8 (b)	Pass
T8	103.604 - 83.3333	Diagonal	L 4 x 4 x 5/16	166	-8.81	23.53	37.5 52.7 (b)	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T9	83.3333 - 63	Diagonal	L 4 x 4 x 5/16	181	-9.64	19.99	48.2 57.3 (b)	Pass
T10	63 - 42.6667	Diagonal	L 4 x 4 x 5/16	196	-10.41	17.40	59.9 62.1 (b)	Pass
T11	42.6667 - 22.3334	Diagonal	P3 STD	219	-18.28	32.58	56.1	Pass
T12	22.3334 - 2	Diagonal	P3 STD	252	-17.89	31.16	57.4	Pass
T11	42.6667 - 22.3334	Horizontal	ROHN 2.5 STD	215	-9.53	15.91	59.9	Pass
T12	22.3334 - 2	Horizontal	P3 STD	248	-9.91	26.65	37.2	Pass
T1	244.917 - 224.792	Top Girt	L 2 x 2 x 1/8	4	-0.35	4.27	8.3	Pass
T2	224.792 - 204.625	Top Girt	L1-3/4x1-3/4x3/16	40	-0.06	4.09	1.5	Pass
T11	42.6667 - 22.3334	Redund Horiz 1 Bracing	ROHN 1.5 STD	226	-4.84	13.67	35.4	Pass
T12	22.3334 - 2	Redund Horiz 1 Bracing	ROHN 1.5 STD	253	-5.33	11.55	46.2	Pass
T11	42.6667 - 22.3334	Redund Diag 1 Bracing	ROHN 2 STD	221	-4.41	8.96	49.2	Pass
T12	22.3334 - 2	Redund Diag 1 Bracing	ROHN 2 STD	260	-4.58	8.51	53.8	Pass
T11	42.6667 - 22.3334	Redund Hip 1 Bracing	ROHN 1.5 STD	222	-0.03	12.19	0.2	Pass
T12	22.3334 - 2	Redund Hip 1 Bracing	ROHN 1.5 STD	255	-0.02	10.40	0.2	Pass
T11	42.6667 - 22.3334	Redund Hip Diagonal 1 Bracing	Rohn 2.5 STD	223	-0.08	10.66	0.8	Pass
T12	22.3334 - 2	Redund Hip Diagonal 1 Bracing	Rohn 2.5 STD	265	-0.08	9.78	0.8	Pass
T11	42.6667 - 22.3334	Inner Bracing	ROHN 2 STD	237	-0.01	6.55	0.5	Pass
T12	22.3334 - 2	Inner Bracing	P3 STD	268	-0.02	25.30	0.4	Pass
						Summary	ELC:	Load Case 7
						Leg (T9)	77.1	Pass
						Diagonal (T2)	74.7	Pass
						Horizontal (T11)	59.9	Pass
						Top Girt (T1)	8.3	Pass
						Redund Horiz 1 Bracing (T12)	46.2	Pass
						Redund Diag 1 Bracing (T12)	53.8	Pass
						Redund Hip 1 Bracing (T11)	0.2	Pass
						Redund Hip Diagonal 1 Bracing (T12)	0.8	Pass
						Inner Bracing (T11)	0.5	Pass
						Bolt Checks	74.7	Pass
						Rating =	77.1	Pass

**Table 5 - Tower Component Stresses vs. Capacity – LC7**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1,2	Anchor Rods	0	23.6	Pass
1,2	Base Foundation Structural	0	42.5	Pass
1,2	Base Foundation Soil Interaction	0	57.7	Pass
<b>Structure Rating (max from all components) =</b>				<b>77.1%</b>

Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.
- 2) Rating per TIA-222-H, section 15.5

**4.1) Recommendations**

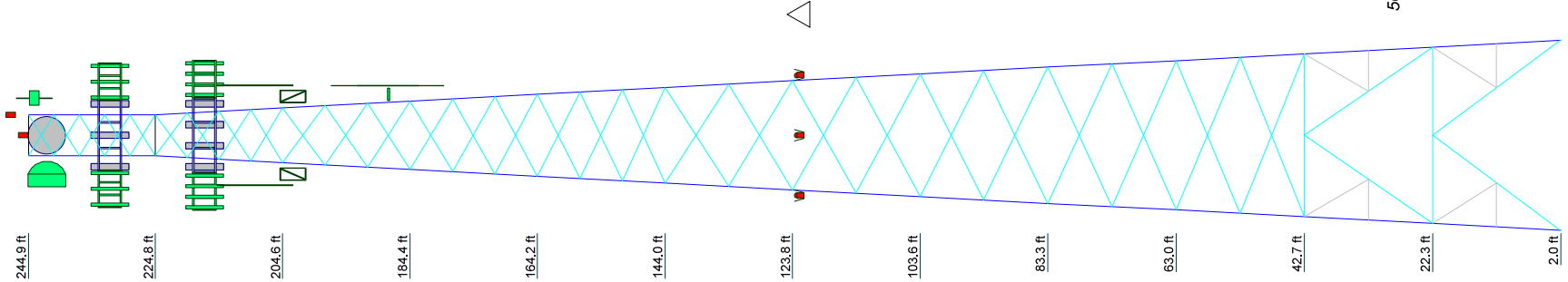
The tower has sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

**Table 6 - Dish Tilt/Sway Results for 60 mph Service Wind Speed**

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
242.00	USX6-6W-6GR	39	6.160	0.2480	0.0219	342124

**APPENDIX A**  
**TNXTOWER OUTPUT**

244.9 ft



Section	Legs	Diagonals	Diagonal Grade	Top Girts	Horizontals	Red. Horizontals	Red. Diagonals	Red. Hips	Inner Bracing	Face Width (ft)	# Panels @ (ft)	Weight (k)
T11	ROHN 8 EH	P3 STD	A572-50		P3 STD	ROHN 1.5 STD	ROHN 2 STD	ROHN 1.5 STD	P3 STD	28.0303	1 @ 20.25	36.5
T10	ROHN 8 EHS									25.8835	1 @ 20.3333	44
T9			L 4 x 4 x 5/16							23.7367	4 @ 10.1667	48
T8	ROHN 6 EH		L3-1/2x3-1/2x1/4							19.4432	2 @ 10.1354	40
T7			L 3 x 3 x 5/16							17.2964	4 @ 10.1042	32
T6	ROHN 5 EH		A36							15.1496	6 @ 6.73611	27
T5	ROHN 4 EH		L2-1/2x2-1/2x5/16							13.0028	3 @ 6.72917	14
T4	ROHN 4 EH		L2-1/2x2-1/2x1/4							10.8561	3 @ 6.72917	14
T3	ROHN 3.5 EH		L2-1/2x2-1/2x3/16							8.70928	4 @ 5.04167	11
T2	ROHN 3 EH		L1-3/4x1-3/4x3/16							6.5625	5 @ 4.2025	09
T1	ROHN 2.5 STD		L1-3/4x1-3/4x3/16									09

### MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

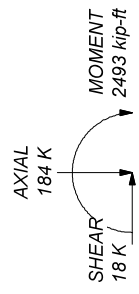
### TOWER DESIGN NOTES

1. Tower is located in Fairfield County, Connecticut
2. Tower designed for Exposure B to the TIA-222-H Standard.
3. Tower designed for a 120 mph basic wind in accordance with the TIA-222-H Standard.
4. Tower is also designed for a 50 mph basic wind with 1.50 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Risk Category II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING: 77.1%

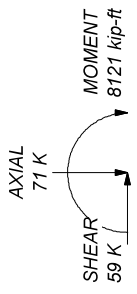
ALL REACTIONS ARE FACTORED

MAX. CORNER REACTIONS AT BASE:  
 DOWN: 334 K  
 SHEAR: 36 K

UPLIFT: -279 K  
 SHEAR: 31 K



TORQUE 11 kip-ft  
 50 mph WIND - 1.5000 in ICE



TORQUE 50 kip-ft  
 REACTIONS - 120 mph WIND



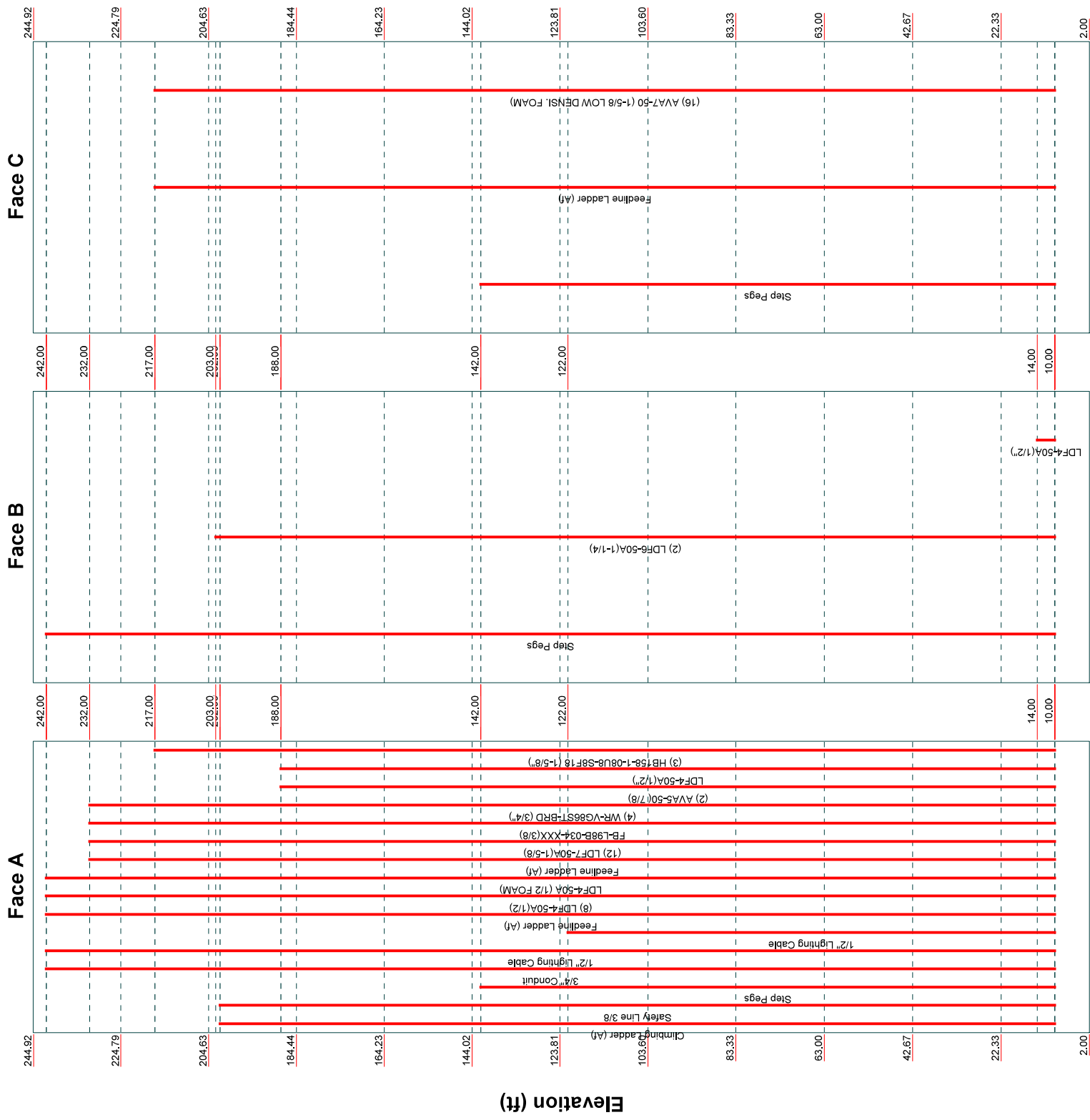
**GPD**  
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 Akron, Ohio 44311  
 Phone: (330) 572-2100  
 FAX: (330) 572-2101

Job: **BU #: 841294 MONROE-GUINEA ROAD**  
 Project: 2020777.841294.15  
 Client: Crown Castle, Inc  
 Code: TIA-222-H  
 Path: T:\Crown\841294\55\_Structure\B0\_Structure\B0\_Structure\09\_Rev\_003\_Modeling\841294.dwg  
 Drawn by: rsteward  
 Date: 06/04/20  
 Scale: NTS  
 Dwg No. E-1

# Feed Line Distribution Chart

2' - 244'11-1/32"

Round Flat App In Face App Out Face Truss Leg



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 Project: **2020777-841294.15**  
 Client: Crown Castle, Inc  
 Code: TIA-222-H  
 Date: 06/04/20  
 Drawn by: msteward  
 App'd:  
 Scale: NTS  
 Path: T:\Crown841294155\_Structure\B0\_Structure\B0\_Structure.dwg  
 Dwg No. E-7

<b><i>tnxTower</i></b>  <b>GPD</b> 520 South Main Street Suite 2531 Akron, Ohio 44311 Phone: (330) 572-2100 FAX: (330) 572-2101	<b>Job</b>  BU #: 841294 MONROE-GUINEA ROAD	<b>Page</b>  1 of 22
	<b>Project</b>  2020777.841294.15	<b>Date</b>  14:51:29 06/04/20
	<b>Client</b>  Crown Castle, Inc	<b>Designed by</b>  msteward

## Tower Input Data

The main tower is a 3x free standing tower with an overall height of 244.92 ft above the ground line. The base of the tower is set at an elevation of 2.00 ft above the ground line. The face width of the tower is 6.56 ft at the top and 30.18 ft at the base. This tower is designed using the TIA-222-H standard.

The following design criteria apply:

Tower is located in Fairfield County, Connecticut.

Tower base elevation above sea level: 586.00 ft.

Basic wind speed of 120 mph.

Risk Category II.

Exposure Category B.

Simplified Topographic Factor Procedure for wind speed-up calculations is used.

Topographic Category: 1.

Crest Height: 0.00 ft.

Nominal ice thickness of 1.5000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

Pressures are calculated at each section.

Tower analysis based on target reliabilities in accordance with Annex S.

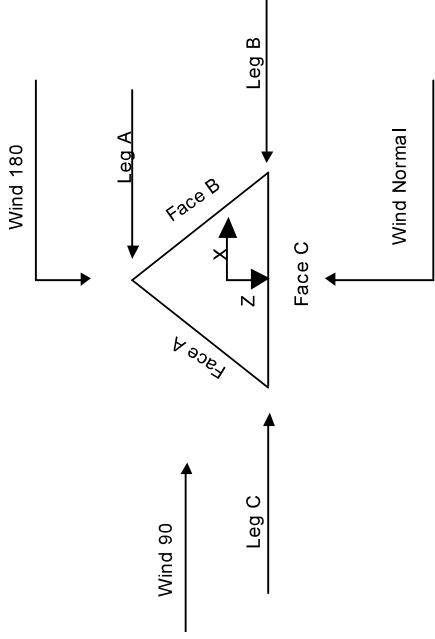
Load Modification Factors used:  $K_{es}(F_w) = 0.95$ ,  $K_{es}(t_i) = 0.85$ .

Stress ratio used in tower member design is 1.05.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Horizontals	Assume Legs Pinned	Calculate Redundant Bracing Forces
Consider Moments - Diagonals	Assume Rigid Index Plate	Ignore Redundant Members in FEA
Use Moment Magnification	Use Clear Spans For Wind Area	SR Leg Bolts Resist Compression
Use Code Stress Ratios	Use Clear Spans For KL/r	All Leg Panels Have Same Allowable
Use Code Safety Factors - Guys	Retension Guys To Initial Tension	Offset Girt At Foundation
Escalate Ice	Bypass Mast Stability Checks	Consider Feed Line Torque
Always Use Max Kz	Use Azimuth Dish Coefficients	Include Angle Block Shear Check
Include Bolts In Member Capacity	Project Wind Area of Appurt.	Use TIA-222-H Bracing Resist. Exemption
Leg Bolts Are At Top Of Section	Autocalc Torque Arm Areas	Use TIA-222-H Tension Splice Exemption
Secondary Horizontal Braces Leg	Add IBC .6D+W Combination	Poles
Use Diamond Inner Bracing (4 Sided)	Sort Capacity Reports By Component	Include Shear-Torsion Interaction
SR Members Have Cut Ends	Triangulate Diamond Inner Bracing	Always Use Sub-Critical Flow
SR Members Are Concentric	Treat Feed Line Bundles As Cylinder	Use Top Mounted Sockets
	Ignore KL/r For 60 Deg. Angle Legs	Pole Without Linear Attachments
		Pole With Shroud Or No Appurtenances
		Outside and Inside Corner Radii Are Known



Triangular Tower

**Tower Section Geometry**

Tower Section	Tower Elevation ft	Assembly Database	Description	Section Width ft	Number of Sections	Section Length ft
T1	244.92-224.79			6.56	1	20.13
T2	224.79-204.63			6.56	1	20.17
T3	204.63-184.44			8.71	1	20.19
T4	184.44-164.23			10.86	1	20.21
T5	164.23-144.02			13.00	1	20.21
T6	144.02-123.81			15.15	1	20.21
T7	123.81-103.60			17.30	1	20.21
T8	103.60-83.33			19.44	1	20.27
T9	83.33-63.00			21.59	1	20.33
T10	63.00-42.67			23.74	1	20.33
T11	42.67-22.33			25.88	1	20.33
T12	22.33-2.00			28.03	1	20.33

**Tower Section Geometry (cont'd)**

Tower Section	Tower Elevation ft	Diagonal Spacing ft	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset in	Bottom Girt Offset in
T1	244.92-224.79	4.03	X Brace	No	No	0.0000	0.0000
T2	224.79-204.63	5.04	X Brace	No	No	0.0000	0.0000
T3	204.63-184.44	6.73	X Brace	No	No	0.0000	0.0000
T4	184.44-164.23	6.74	X Brace	No	No	0.0000	0.0000
T5	164.23-144.02	6.74	X Brace	No	No	0.0000	0.0000
T6	144.02-123.81	10.10	X Brace	No	No	0.0000	0.0000
T7	123.81-103.60	10.10	X Brace	No	No	0.0000	0.0000
T8	103.60-83.33	10.14	X Brace	No	No	0.0000	0.0000



# ***tnxTower***

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### **Project**

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### **Designed by**

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Tower Section	Tower Elevation <i>ft</i>	Diagonal Spacing <i>ft</i>	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset <i>in</i>	Bottom Girt Offset <i>in</i>
T9	83.33-63.00	10.17	X Brace	No	No	0.0000	0.0000
T10	63.00-42.67	10.17	X Brace	No	No	0.0000	0.0000
T11	42.67-22.33	20.33	K1 Down	No	Yes	0.0000	0.0000
T12	22.33-2.00	20.25	K1 Down	No	Yes	0.0000	1.0000

## **Tower Section Geometry (cont'd)**

Tower Elevation <i>ft</i>	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 244.92-224.79	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Equal Angle	L1-3/4x1-3/4x3/16	A36 (36 ksi)
T2 224.79-204.63	Pipe	ROHN 3 EH	A572-50 (50 ksi)	Equal Angle	L1-3/4x1-3/4x3/16	A36 (36 ksi)
T3 204.63-184.44	Pipe	ROHN 3.5 EH	A572-50 (50 ksi)	Equal Angle	L2-1/2x2-1/2x3/16	A36 (36 ksi)
T4 184.44-164.23	Pipe	ROHN 4 EH	A572-50 (50 ksi)	Equal Angle	L2-1/2x2-1/2x1/4	A36 (36 ksi)
T5 164.23-144.02	Pipe	ROHN 5 EH	A572-50 (50 ksi)	Equal Angle	L2-1/2x2-1/2x5/16	A36 (36 ksi)
T6 144.02-123.81	Pipe	ROHN 5 EH	A572-50 (50 ksi)	Equal Angle	L 3 x 3 x 5/16	A36 (36 ksi)
T7 123.81-103.60	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Equal Angle	L3-1/2x3-1/2x1/4	A36 (36 ksi)
T8 103.60-83.33	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Equal Angle	L 4 x 4 x 5/16	A36 (36 ksi)
T9 83.33-63.00	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Equal Angle	L 4 x 4 x 5/16	A36 (36 ksi)
T10 63.00-42.67	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Equal Angle	L 4 x 4 x 5/16	A36 (36 ksi)
T11 42.67-22.33	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Pipe	P3 STD	A572-50 (50 ksi)
T12 22.33-2.00	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Pipe	P3 STD	A572-50 (50 ksi)

## **Tower Section Geometry (cont'd)**

Tower Elevation <i>ft</i>	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 244.92-224.79	Equal Angle	L 2 x 2 x 1/8	A36 (36 ksi)	Flat Bar		A36 (36 ksi)
T2 224.79-204.63	Equal Angle	L1-3/4x1-3/4x3/16	A36 (36 ksi)	Equal Angle		A36 (36 ksi)

# tnxTower

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## Tower Section Geometry (cont'd)

Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T11 42.67-22.33	None	Solid Round		A36 (36 ksi)	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)
T12 22.33-2.00	None	Solid Round		A36 (36 ksi)	Pipe	P3 STD	A572-50 (50 ksi)

## Tower Section Geometry (cont'd)

Tower Elevation ft	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
T11 42.67-22.33	Solid Round		A36 (36 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T12 22.33-2.00	Solid Round		A36 (36 ksi)	Pipe	P3 STD	A572-50 (50 ksi)

## Tower Section Geometry (cont'd)

Tower Elevation ft	Redundant Bracing Grade	Redundant Type	Redundant Size	K Factor
T11 42.67-22.33	A572-50 (50 ksi)	Horizontal (1) Diagonal (1) Hip (1)	Pipe ROHN 1.5 STD ROHN 2 STD ROHN 1.5 STD	1 1 1
T12 22.33-2.00	A572-50 (50 ksi)	Hip Diagonal (1) Horizontal (1) Diagonal (1) Hip (1) Hip Diagonal (1)	Pipe Rohn 2.5 STD ROHN 1.5 STD ROHN 2 STD ROHN 1.5 STD Rohn 2.5 STD	1 1 1 1 1

## Tower Section Geometry (cont'd)

Tower Elevation ft	Gusset Area (per face)	Gusset Thickness in	Gusset Grade	Adjust. Factor $A_j$	Adjust. Factor $A_r$	Weight Mult.	Double Angle Stitch Spacing Diagonals in	Double Angle Stitch Spacing Horizontals in	Double Angle Stitch Spacing Redundants in
T1 244.92-224.79	0.00	0.1875	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T2 224.79-204.63	0.00	0.1875	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T3 204.63-184.44	0.00	0.1875	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T4 184.44-164.23	0.00	0.2500	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T5 164.23-144.02	0.00	0.3125	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000

# tnxTower

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Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor $A_f$	Adjust. Factor $A_r$	Weight Mult.	Double Angle Spacing Diagonals	Double Angle Spacing Horizontals	Double Angle Spacing Redundants
ft	ft <sup>2</sup>	in					in	in	in
T6 144.02-123.81	0.00	0.3125	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T7 123.81-103.60	0.00	0.2500	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T8 103.60-83.33	0.00	0.3125	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T9 83.33-63.00	0.00	0.3125	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T10 63.00-42.67	0.00	0.3125	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T11 42.67-22.33	0.00	0.3750	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T12 22.33-2.00	0.00	0.3750	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000

## Tower Section Geometry (cont'd)

Tower Elevation	Calc K Single Angles	Calc K Solid Rounds	Legs	X Brace		K Brace		K Factors <sup>1</sup>		Horiz.	Sec. Horiz.	Inner Brace	
				Diags	X	Diags	Y	Single Diags	Girts				
T1 244.92-224.79	Yes	Yes	I	X	Y	X	Y	X	Y	X	Y	X	Y
T2 224.79-204.63	Yes	Yes	I	X	Y	X	Y	X	Y	X	Y	X	Y
T3 204.63-184.44	Yes	Yes	I	X	Y	X	Y	X	Y	X	Y	X	Y
T4 184.44-164.23	Yes	Yes	I	X	Y	X	Y	X	Y	X	Y	X	Y
T5 164.23-144.02	Yes	Yes	I	X	Y	X	Y	X	Y	X	Y	X	Y
T6 144.02-123.81	Yes	Yes	I	X	Y	X	Y	X	Y	X	Y	X	Y
T7 123.81-103.60	Yes	Yes	I	X	Y	X	Y	X	Y	X	Y	X	Y
T8 103.60-83.33	Yes	Yes	I	X	Y	X	Y	X	Y	X	Y	X	Y
T9 83.33-63.00	Yes	Yes	I	X	Y	X	Y	X	Y	X	Y	X	Y
T10 63.00-42.67	Yes	Yes	I	X	Y	X	Y	X	Y	X	Y	X	Y
T11 42.67-22.33	Yes	Yes	I	X	Y	X	Y	X	Y	X	Y	X	Y
T12 22.33-2.00	Yes	Yes	I	X	Y	X	Y	X	Y	X	Y	X	Y

<sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

**inxTower**

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**Tower Section Geometry (cont'd)**

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 244.92-224.79	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T2 224.79-204.63	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T3 204.63-184.44	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T4 184.44-164.23	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T5 164.23-144.02	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 144.02-123.81	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 123.81-103.60	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 103.60-83.33	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T9 83.33-63.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T10 63.00-42.67	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T11 42.67-22.33	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T12 22.33-2.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

**Tower Section Geometry (cont'd)**

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal		
	Leg Connection Type	Bolt Size	No.	in	Bolt Size	No.	in	Bolt Size	No.	in	Bolt Size	No.	in	Bolt Size	No.
T1 244.92-224.79	Flange	0.7500	4	0.5000	1	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T2 224.79-204.63	Flange	A325N	4	A325N	1	A325N	1	A325N	0	A325N	0	A325N	0	A325N	0
T3 204.63-184.44	Flange	0.8750	4	0.5000	1	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T4 184.44-164.23	Flange	A325N	4	A325N	1	A325N	1	A325N	0	A325N	0	A325N	0	A325N	0
T5 164.23-144.02	Flange	1.0000	4	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T6 144.02-123.81	Flange	A325N	6	A325X	1	A325N	1	A325N	0	A325N	0	A325N	0	A325N	0
T7 123.81-103.60	Flange	1.0000	6	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T8 103.60-83.33	Flange	A325N	6	A325N	1	A325N	1	A325N	0	A325N	0	A325N	0	A325N	0
T9 83.33-63.00	Flange	1.0000	6	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T10 63.00-42.67	Flange	A325N	8	A325N	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0

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Tower Elevation ft	Leg Connection Type	Leg	Diagonal		Top Girt	Bottom Girt	Mid Girt	Long Horizontal		Short Horizontal	
			Bolt Size	No.				Bolt Size	No.	Bolt Size	No.
T11 42.67-22.33	Flange	1.0000 A325N	0.7500 A325N	3	0.6250 A325N	0	0.6250 A325N	0	0.7500 A325N	2	0.6250 A325N
T12 22.33-2.00	Flange	1.0000 A354-BC	0.7500 A325N	3	0.6250 A325N	0	0.6250 A325N	0	0.7500 A325N	2	0.6250 A325N

## Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement	Face Offset	Lateral Offset (Frac FW)	#	Clear Per Row	Width or Diameter	Perimeter	Weight
					ft	in			in	in	in	plf
Climbing Ladder (Af)	A	No	No	Af (CaAa)	202.00 - 10.00	0.0000	0.5	1	1	3.8400	3.8400	4.81
Safety Line 3/8	A	No	No	Ar (CaAa)	202.00 - 10.00	0.0000	0.5	1	1	0.3750	0.3750	0.22
Step Pegs	C	No	No	Ar (CaAa)	142.00 - 10.00	0.0000	0.5	1	1	0.8000	0.8000	2.72
Step Pegs	A	No	No	Ar (CaAa)	142.00 - 10.00	0.0000	0.5	1	1	0.8000	0.8000	2.72
Step Pegs	B	No	No	Ar (CaAa)	242.00 - 10.00	0.0000	0.5	1	1	0.8000	0.8000	2.72
3/4" Conduit	A	No	No	Ar (CaAa)	242.00 - 10.00	0.0000	0.4825	1	1	0.7500	0.7500	0.50
1/2" Lighting Cable	A	No	No	Ar (CaAa)	242.00 - 10.00	0.0000	0.4825	1	1	0.6250	0.6250	0.50
1/2" Lighting Cable	A	No	No	Ar (CaAa)	122.00 - 10.00	0.0000	0.4825	1	1	0.6250	0.6250	0.50
Feedline Ladder (Af)	A	No	No	Af (CaAa)	242.00 - 10.00	0.0000	0.25	1	1	3.0000	3.0000	8.40
LDF4-50A(1/2)	A	No	No	Ar (CaAa)	242.00 - 10.00	0.0000	0.25	8	8	0.6250	0.6250	0.15
LDF4-50A (1/2 FOAM)	A	No	No	Ar (CaAa)	242.00 - 10.00	0.0000	0.48	1	1	0.6300	0.6300	0.15
Feedline Ladder (Af)	A	No	No	Af (CaAa)	232.00 - 10.00	0.0000	-0.4	1	1	3.0000	3.0000	8.40
LDF7-50A(1-5/8)	A	No	No	Ar (CaAa)	232.00 - 10.00	0.0000	-0.4	12	4	1.0000	1.9800	0.82
FB-L98B-034-XXX(3/8)	A	No	No	Ar (CaAa)	232.00 - 10.00	0.0000	-0.35	1	1	0.3937	0.3937	0.06
WR-VG86ST-BRD (3/4")	A	No	No	Ar (CaAa)	232.00 - 10.00	0.0000	-0.3	4	2	0.7950	0.7950	0.60
AVA5-50(7/8)	A	No	No	Ar (CaAa)	188.00 - 10.00	0.0000	-0.42	2	1	1.0000	1.1020	0.30
LDF4-50A(1-2")	A	No	No	Ar (CaAa)	188.00 - 10.00	0.0000	-0.42	1	1	0.6300	0.6300	0.15
LDF6-50A(1-1/4)	B	No	No	Ar (CaAa)	203.00 - 10.00	0.0000	0.49	2	2	1.0000	1.5500	0.60
Feedline Ladder (Af)	C	No	No	Af (CaAa)	217.00 - 10.00	0.0000	-0.375	1	1	3.0000	3.0000	8.40
AVA7-50 (1-5/8 LOW DENSI. FOAM)	C	No	No	Ar (CaAa)	217.00 - 10.00	0.0000	-0.4	16	16	1.0000	1.9800	0.72
HB158-1-08U 8-S8F18	A	No	No	Ar (CaAa)	217.00 - 10.00	2.0000	-0.45	3	3	1.0000	1.9800	1.70

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Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
(1-5/8") LDF4-50A(1/2")	B	No	No	Ar (CaAa)	14.00 - 10.00	0.0000	0.45	1	0.6300	0.6300		0.15

## Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>1</sub> A <sub>1</sub> Front	C <sub>1</sub> A <sub>1</sub> Side	Weight K
Flash Beacon Lighting	A	From Leg	0.00 0.00 0.00	0.0000	245.00	2.70 3.10 3.50	2.70 3.10 3.50	0.05 0.07 0.09
Lighting Rod 3/4" x 4'	B	From Leg	0.00 0.00 2.00	0.0000	245.00	4.30 0.30 0.71	4.30 0.30 0.71	0.13 0.03 0.03
Side Arm Mount [SO 303-3]	B	None		0.0000	242.00	1.00 1.52 7.67	1.00 1.52 7.67	0.04 0.06 0.34
(2) FREEMILE 24 GHZ FODU	A	From Leg	3.00 0.00 0.00	0.0000	242.00	11.04 14.57 22.36	11.04 14.57 22.36	0.48 0.65 1.14
(2) FREEMILE 24 GHZ FODU	C	From Leg	3.00 0.00 0.00	0.0000	242.00	0.68 0.79 1.15	0.68 0.79 1.15	0.00 0.01 0.04
DB806-XC	B	From Leg	3.00 0.00 2.00	0.0000	242.00	0.90 1.14 1.68	0.90 1.14 1.68	0.02 0.04 0.03
FMO	B	From Leg	3.00 0.00 2.00	0.0000	242.00	0.41 0.59 0.81	0.41 0.59 0.81	0.02 0.04 0.18
DMP65R-BU8D	A	From Leg	3.00 0.00 0.00	0.0000	232.00	0.90 15.86 16.80	0.90 15.86 16.80	0.02 0.18 0.20
DMP65R-BU8D	B	From Leg	3.00 0.00 0.00	0.0000	232.00	0.41 17.75 19.71	0.41 17.75 19.71	0.02 0.04 0.55
DMP65R-BU8D	C	From Leg	3.00 0.00 0.00	0.0000	232.00	0.41 15.86 16.80	0.41 15.86 16.80	0.02 0.18 0.20
HPA-65R-BUU-H16	A	From Leg	3.00 0.00 2.00	0.0000	232.00	9.24 9.22 10.00	9.24 9.22 10.00	0.36 0.31 0.11

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Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C <sub>1</sub> A <sub>1</sub> Front	C <sub>1</sub> A <sub>1</sub> Side	Weight
			Horz Lateral	Vert	ft					
HPA-65R-BUU-H16	B	From Leg	3.00	0.00	0.00	232.00	No Ice	9.22	4.65	0.05
			0.00	0.00	0.00		1/2" Ice	10.00	5.36	0.11
			2.00	0.00	0.00		1" Ice	10.79	6.09	0.17
HPA-65R-BUU-H16	C	From Leg	3.00	0.00	0.00	232.00	No Ice	9.22	7.60	0.32
			0.00	0.00	0.00		1/2" Ice	10.00	4.65	0.05
			2.00	0.00	0.00		1" Ice	10.79	5.36	0.11
OPA65R-BU6D	A	From Leg	3.00	0.00	0.00	232.00	2" Ice	12.43	7.60	0.32
			0.00	0.00	0.00		No Ice	12.22	4.54	0.06
			0.00	0.00	0.00		1/2" Ice	12.98	5.19	0.14
OPA65R-BU6D	B	From Leg	3.00	0.00	0.00	232.00	1" Ice	13.75	5.86	0.22
			0.00	0.00	0.00		2" Ice	15.35	7.24	0.40
			0.00	0.00	0.00		No Ice	12.22	4.54	0.06
OPA65R-BU6D	C	From Leg	3.00	0.00	0.00	232.00	1" Ice	13.75	5.86	0.22
			0.00	0.00	0.00		2" Ice	15.35	7.24	0.40
			0.00	0.00	0.00		No Ice	12.22	4.54	0.06
7770.00	A	From Leg	3.00	0.00	0.00	232.00	5.51	2.93	0.04	
			0.00	0.00	0.00		5.87	3.27	0.07	
			0.00	0.00	0.00		6.23	3.63	0.11	
7770.00	B	From Leg	3.00	0.00	0.00	232.00	6.99	4.35	0.20	
			0.00	0.00	0.00		5.51	2.93	0.04	
			0.00	0.00	0.00		5.87	3.27	0.07	
7770.00	C	From Leg	3.00	0.00	0.00	232.00	6.23	3.63	0.11	
			0.00	0.00	0.00		6.99	4.35	0.20	
			0.00	0.00	0.00		5.51	2.93	0.04	
RADIO 4449 B5/B12	A	From Leg	3.00	0.00	0.00	232.00	1.64	1.30	0.07	
			0.00	0.00	0.00		1.80	1.45	0.09	
			0.00	0.00	0.00		1.97	1.60	0.11	
RADIO 4449 B5/B12	B	From Leg	3.00	0.00	0.00	232.00	2.33	1.92	0.16	
			0.00	0.00	0.00		1.64	1.30	0.07	
			0.00	0.00	0.00		1.80	1.45	0.09	
RADIO 4449 B5/B12	C	From Leg	3.00	0.00	0.00	232.00	2.33	1.92	0.16	
			0.00	0.00	0.00		1.64	1.30	0.07	
			0.00	0.00	0.00		1.80	1.45	0.09	
RRUS 32 B2	A	From Leg	3.00	0.00	0.00	232.00	2.33	1.92	0.16	
			0.00	0.00	0.00		2.73	1.67	0.05	
			0.00	0.00	0.00		2.95	1.86	0.07	
RRUS 32 B2	B	From Leg	3.00	0.00	0.00	232.00	3.18	2.05	0.10	
			0.00	0.00	0.00		3.66	2.46	0.16	
			0.00	0.00	0.00		2.73	1.67	0.05	
RRUS 32 B2	C	From Leg	3.00	0.00	0.00	232.00	3.18	2.05	0.10	
			0.00	0.00	0.00		3.66	2.46	0.16	
			0.00	0.00	0.00		2.95	1.86	0.07	
RRUS 4478 B14	A	From Leg	3.00	0.00	0.00	232.00	3.18	2.05	0.10	
			0.00	0.00	0.00		3.66	2.46	0.16	
			0.00	0.00	0.00		2.95	1.86	0.07	

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Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C <sub>1</sub> A <sub>1</sub> Front	C <sub>1</sub> A <sub>1</sub> Side	Weight
			Horz Lateral	Vert	ft					
			ft	ft	ft	ft	ft <sup>2</sup>	ft <sup>2</sup>		
			0.00				1/2" Ice	2.01	1.20	0.08
			0.00				1" Ice	2.19	1.34	0.09
							2" Ice	2.57	1.66	0.14
RRUS 4478 B14	B	From Leg	3.00		0.0000	232.00	No Ice	1.84	1.06	0.06
			0.00				1/2" Ice	2.01	1.20	0.08
			0.00				1" Ice	2.19	1.34	0.09
							2" Ice	2.57	1.66	0.14
RRUS 4478 B14	C	From Leg	3.00		0.0000	232.00	No Ice	1.84	1.06	0.06
			0.00				1/2" Ice	2.01	1.20	0.08
			0.00				1" Ice	2.19	1.34	0.09
							2" Ice	2.57	1.66	0.14
(2) LGP13519	A	From Leg	3.00		0.0000	232.00	No Ice	0.29	0.18	0.01
			0.00				1/2" Ice	0.36	0.24	0.01
			0.00				1" Ice	0.44	0.31	0.01
							2" Ice	0.62	0.47	0.02
(2) LGP13519	B	From Leg	3.00		0.0000	232.00	No Ice	0.29	0.18	0.01
			0.00				1/2" Ice	0.36	0.24	0.01
			0.00				1" Ice	0.44	0.31	0.01
							2" Ice	0.62	0.47	0.02
(2) LGP13519	C	From Leg	3.00		0.0000	232.00	No Ice	0.29	0.18	0.01
			0.00				1/2" Ice	0.36	0.24	0.01
			0.00				1" Ice	0.44	0.31	0.01
							2" Ice	0.62	0.47	0.02
(2) DC6-48-60-18-8F Surge Suppression Unit	A	From Leg	3.00		0.0000	232.00	No Ice	0.92	0.92	0.02
			0.00				1/2" Ice	1.46	1.46	0.04
			0.00				1" Ice	1.64	1.64	0.06
							2" Ice	2.04	2.04	0.11
(3) 12.5 ft Frame Mount	B	None			0.0000	232.00	No Ice	27.93	27.93	1.28
							1/2" Ice	37.68	37.68	1.76
							1" Ice	47.43	47.43	2.24
							2" Ice	66.93	66.93	3.19
8' STD P2 pipe	A	From Leg	3.00		0.0000	232.00	No Ice	1.90	1.90	0.03
			0.00				1/2" Ice	2.73	2.73	0.04
			0.00				1" Ice	3.40	3.40	0.06
							2" Ice	4.40	4.40	0.12
8' STD P2 pipe	C	From Leg	3.00		0.0000	232.00	No Ice	1.90	1.90	0.03
			0.00				1/2" Ice	2.73	2.73	0.04
			0.00				1" Ice	3.40	3.40	0.06
							2" Ice	4.40	4.40	0.12
Sector Mount [SM 503-3]	B	None			0.0000	217.00	No Ice	30.43	30.43	1.69
							1/2" Ice	43.02	43.02	2.30
							1" Ice	55.43	55.43	3.10
							2" Ice	79.89	79.89	5.27
(2) JAHH-65B-R3B w/ Mount Pipe	A	From Leg	4.00		0.0000	217.00	No Ice	5.50	4.38	0.10
			0.00				1/2" Ice	5.97	4.84	0.17
			3.00				1" Ice	6.45	5.30	0.25
							2" Ice	7.44	6.26	0.46
(2) JAHH-65B-R3B w/ Mount Pipe	B	From Leg	4.00		0.0000	217.00	No Ice	5.50	4.38	0.10
			0.00				1/2" Ice	5.97	4.84	0.17
			3.00				1" Ice	6.45	5.30	0.25
							2" Ice	7.44	6.26	0.46
(2) JAHH-65B-R3B w/ Mount Pipe	C	From Leg	4.00		0.0000	217.00	No Ice	5.50	4.38	0.10
			0.00				1/2" Ice	5.97	4.84	0.17
			3.00				1" Ice	6.45	5.30	0.25
							2" Ice	7.44	6.26	0.46
LNX-8514DS-A1M w/ Mount Pipe	A	From Leg	4.00		0.0000	217.00	No Ice	5.56	4.47	0.08
			0.00				1/2" Ice	6.07	4.97	0.17



# tnxTower

**GPD**  
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## Job

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## Project

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## Date

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## Client

Crown Castle, Inc

## Designed by

msteward

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>1</sub> A <sub>1</sub> Front	C <sub>1</sub> A <sub>1</sub> Side	Weight
			Horz Lateral	Vert					
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
			3.00				6.59	5.47	0.26
							7.65	6.52	0.49
LNx-8514DS-A1M w/ Mount Pipe	B	From Leg	4.00		0.0000	217.00	5.56	4.47	0.08
			0.00				6.07	4.97	0.17
			3.00				6.59	5.47	0.26
							7.65	6.52	0.49
LNx-8514DS-A1M w/ Mount Pipe	C	From Leg	4.00		0.0000	217.00	5.56	4.47	0.08
			0.00				6.07	4.97	0.17
			3.00				6.59	5.47	0.26
							7.65	6.52	0.49
HBXX-6517DS-A2M w/ Mount Pipe	A	From Leg	4.00		0.0000	217.00	7.97	5.99	0.08
			0.00				8.73	6.72	0.14
			3.00				9.51	7.47	0.21
							11.11	9.02	0.40
HBXX-6517DS-A2M w/ Mount Pipe	B	From Leg	4.00		0.0000	217.00	7.97	5.99	0.08
			0.00				8.73	6.72	0.14
			3.00				9.51	7.47	0.21
							11.11	9.02	0.40
HBXX-6517DS-A2M w/ Mount Pipe	C	From Leg	4.00		0.0000	217.00	7.97	5.99	0.08
			0.00				8.73	6.72	0.14
			3.00				9.51	7.47	0.21
							11.11	9.02	0.40
B66A RRH4X45	A	From Leg	4.00		0.0000	217.00	2.54	1.61	0.06
			0.00				2.75	1.79	0.08
			3.00				2.97	1.98	0.10
							3.43	2.37	0.16
B66A RRH4X45	B	From Leg	4.00		0.0000	217.00	2.54	1.61	0.06
			0.00				2.75	1.79	0.08
			3.00				2.97	1.98	0.10
							3.43	2.37	0.16
B66A RRH4X45	C	From Leg	4.00		0.0000	217.00	2.54	1.61	0.06
			0.00				2.75	1.79	0.08
			3.00				2.97	1.98	0.10
							3.43	2.37	0.16
RRH2X60-700	A	From Leg	4.00		0.0000	217.00	3.50	1.82	0.06
			0.00				3.76	2.05	0.08
			3.00				2.97	1.98	0.10
							4.58	2.79	0.17
RRH2X60-700	B	From Leg	4.00		0.0000	217.00	3.50	1.82	0.06
			0.00				3.76	2.05	0.08
			3.00				2.97	1.98	0.10
							4.58	2.79	0.17
RRH2X60-700	C	From Leg	4.00		0.0000	217.00	3.50	1.82	0.06
			0.00				3.76	2.05	0.08
			3.00				2.97	1.98	0.10
							4.58	2.79	0.17
(2) DB-T1-6Z-8AB-0Z	C	From Leg	1.00		0.0000	217.00	4.80	2.00	0.04
			0.00				5.07	2.19	0.08
			-3.00				5.35	2.39	0.12
							5.93	2.81	0.21
							0.41	2.26	0.04
			2.00		0.0000	203.00	0.81	3.83	0.06
			0.00				1.23	5.48	0.09
			0.00				2.08	9.37	0.19
Side Arm Mount [SO 306-1]	B	From Leg	2.00		0.0000	203.00	0.41	2.26	0.04
			0.00				0.81	3.83	0.06
			0.00				1.23	5.48	0.09
			2.00		0.0000	203.00	0.41	2.26	0.04
			0.00				0.81	3.83	0.06
			0.00				1.23	5.48	0.09
Side Arm Mount [SO 306-1]	C	From Leg	2.00		0.0000	203.00	0.41	2.26	0.04
			0.00				0.81	3.83	0.06
			0.00				1.23	5.48	0.09

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Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C <sub>1</sub> A <sub>1</sub> Front	C <sub>1</sub> A <sub>1</sub> Side	Weight
			Horz Lateral	Vert	ft					
			ft	ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
OG-4	B	From Leg	4.00			0.0000	203.00	2.08	9.37	0.19
			0.00					4.31	4.31	0.02
			6.00					7.14	7.14	0.06
OG-4	C	From Leg	4.00			0.0000	203.00	7.86	7.86	0.11
			0.00					9.34	9.34	0.23
			6.00					4.31	4.31	0.02
Side Arm Mount [SO 301-1]	B	From Leg	1.50			0.0000	188.00	7.14	7.14	0.06
			0.00					7.86	7.86	0.11
			0.00					9.34	9.34	0.23
DB589-A	B	From Leg	3.00			0.0000	188.00	0.46	0.91	0.02
			0.00					0.65	1.30	0.03
			2.00					0.87	1.71	0.05
DB589-A	B	From Leg	3.00			0.0000	188.00	1.41	2.62	0.09
			0.00					2.76	2.76	0.01
			2.00					4.17	4.17	0.03
17" Side Light Mount	A	From Face	3.00			0.0000	188.00	5.59	5.59	0.06
			0.00					8.49	8.49	0.15
			-2.00					2.76	2.76	0.01
17" Side Light Mount	C	From Face	0.50			0.0000	122.00	4.17	4.17	0.03
			0.00					5.59	5.59	0.06
			0.00					8.49	8.49	0.15
Side Light	A	From Leg	0.50			0.0000	122.00	2.27	2.27	0.06
			0.00					3.42	3.42	1.15
			0.00					4.58	4.58	2.26
Side Light	B	From Leg	1.00			0.0000	122.00	6.93	6.93	4.55
			0.00					0.47	0.47	0.01
			0.00					0.60	0.60	0.01
Side Light	C	From Leg	1.00			0.0000	122.00	0.87	0.87	0.01
			0.00					0.33	0.33	0.01
			0.00					0.47	0.47	0.01
TY-840	B	From Face	1.00			0.0000	14.00	0.60	0.60	0.01
			0.00					0.87	0.87	0.01
			0.00					0.25	0.25	0.00
							0.45	0.45	0.00	
							0.65	0.65	0.00	
							1.05	1.05	0.01	

# tnxTower

## GPD

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### Project

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## Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weight
				ft	°	°	ft	ft	ft <sup>2</sup>	K
USX6-6W-6GR	A	Paraboloid w/Shroud (HP)	From Leg	1.00 0.00 0.00	28.0000		242.00	6.00	No Ice 28.27	0.33
USX6-6W-6GR	C	Paraboloid w/Shroud (HP)	From Leg	1.00 0.00 0.00	-33.0000		242.00	6.00	1/2" Ice 29.07 1" Ice 29.86 2" Ice 31.44 No Ice 28.27 1/2" Ice 29.07 1" Ice 29.86 2" Ice 31.44	0.48 0.63 0.93 0.33 0.48 0.63 0.93

## Maximum Tower Deflections - Service Wind

Section No.	Elevation	Horz. Deflection	Gov. Load Comb.	Tilt	Twist
	ft	in		°	°
T1	244.917 - 224.792	6.314	39	0.2490	0.0229
T2	224.792 - 204.625	5.262	39	0.2390	0.0158
T3	204.625 - 184.438	4.266	39	0.2167	0.0169
T4	184.438 - 164.229	3.383	39	0.1878	0.0173
T5	164.229 - 144.021	2.626	39	0.1586	0.0156
T6	144.021 - 123.813	1.980	39	0.1352	0.0137
T7	123.813 - 103.604	1.441	39	0.1099	0.0120
T8	103.604 - 83.3333	0.992	39	0.0908	0.0100
T9	83.3333 - 63	0.629	39	0.0707	0.0085
T10	63 - 42.6667	0.349	39	0.0500	0.0069
T11	42.6667 - 22.3334	0.149	39	0.0317	0.0053
T12	22.3334 - 2	0.044	39	0.0138	0.0025

## Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
ft			in	°	°	ft
245.00	Flash Beacon Lighting	39	6.314	0.2490	0.0229	342124
242.00	USX6-6W-6GR	39	6.160	0.2480	0.0219	342124
232.00	DMP65R-BU8D	39	5.635	0.2436	0.0182	132435
217.00	Sector Mount [SM 503-3]	39	4.867	0.2317	0.0136	59033
203.00	Side Arm Mount [SO 306-1]	39	4.190	0.2145	0.0171	38672
188.00	Side Arm Mount [SO 301-1]	39	3.529	0.1932	0.0175	36919
122.00	17" Side Light Mount	39	1.397	0.1079	0.0118	57857
14.00	TY-840	39	0.022	0.0077	0.0014	129943

# tnxTower

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### Job

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### Project

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### Client

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## Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	244.917 - 224.792	24.128	3	0.9475	0.0869
T2	224.792 - 204.625	20.119	3	0.9122	0.0601
T3	204.625 - 184.438	16.314	3	0.8287	0.0639
T4	184.438 - 164.229	12.936	3	0.7188	0.0656
T5	164.229 - 144.021	10.039	3	0.6070	0.0592
T6	144.021 - 123.813	7.569	3	0.5173	0.0518
T7	123.813 - 103.604	5.507	3	0.4205	0.0452
T8	103.604 - 83.3333	3.789	3	0.3474	0.0378
T9	83.3333 - 63	2.401	3	0.2706	0.0321
T10	63 - 42.6667	1.331	3	0.1913	0.0261
T11	42.6667 - 22.3334	0.567	2	0.1214	0.0198
T12	22.3334 - 2	0.167	2	0.0528	0.0094

## Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
245.00	Flash Beacon Lighting	3	24.128	0.9475	0.0869	97180
242.00	USX6-6W-6GR	3	23.542	0.9439	0.0830	97180
232.00	DMP65R-BU8D	3	21.543	0.9290	0.0691	37618
217.00	Sector Mount [SM 503-3]	3	18.611	0.8853	0.0513	15995
203.00	Side Arm Mount [SO 306-1]	3	16.024	0.8204	0.0649	10361
188.00	Side Arm Mount [SO 301-1]	3	13.497	0.7391	0.0663	9773
122.00	17" Side Light Mount	3	5.340	0.4129	0.0446	15094
14.00	TY-840	2	0.083	0.0295	0.0053	34012

## Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load per Bolt K	Ratio Load Allowable	Criteria
T1	244.917	Leg Diagonal	A325N	0.7500	4	2.94	30.10	0.098	Bolt Tension
			A325N	0.5000	1	3.70	5.71	0.648	Member Block Shear
T2	224.792	Top Girt Leg Diagonal	A325N	0.5000	1	0.35	4.13	0.083	Member Bearing
			A325N	0.8750	4	9.41	41.56	0.226	Bolt Tension
			A325N	0.5000	1	4.48	5.71	0.785	Member Block Shear
T3	204.625	Top Girt	A325N	0.5000	1	0.04	5.71	0.007	Member Block Shear
		Leg Diagonal	A325N	0.8750	4	15.92	41.56	0.383	Bolt Tension
T4	184.438	Leg Diagonal	A325N	1.0000	4	4.75	6.20	0.767	Member Bearing
			A325X	0.5000	1	22.39	54.52	0.411	Bolt Tension
T5	164.229	Leg Diagonal	A325N	1.0000	4	5.30	8.27	0.641	Member Bearing
			A325X	0.5000	1	28.64	54.52	0.525	Bolt Tension
T6	144.021	Leg Diagonal	A325N	1.0000	6	6.02	10.33	0.583	Member Bearing
			A325N	0.6250	1	22.79	54.52	0.418	Bolt Tension
T7	123.813	Leg Diagonal	A325N	1.0000	6	7.21	13.05	0.552	Gusset Bearing
			A325N	1.0000	6	26.77	54.52	0.491	Bolt Tension

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load per Bolt K	Ratio Load/Allowable	Allowable Ratio	Criteria
T8	103.604	Diagonal Leg	A325X	0.6250	1	7.87	10.44	0.754	1.05	Gusset Bearing
		Diagonal	A325N	1.0000	6	30.72	54.52	0.564	1.05	Bolt Tension
T9	83.3333	Diagonal Leg	A325N	0.7500	1	8.73	15.77	0.554	1.05	Gusset Bearing
		Diagonal	A325N	1.0000	6	34.66	54.52	0.636	1.05	Bolt Tension
T10	63	Diagonal Leg	A325N	0.7500	1	9.48	15.77	0.601	1.05	Gusset Bearing
		Diagonal	A325N	1.0000	8	28.92	54.52	0.530	1.05	Bolt Tension
T11	42.6667	Diagonal Leg	A325N	0.7500	1	10.27	15.77	0.652	1.05	Gusset Bearing
		Diagonal	A325N	1.0000	8	29.31	54.52	0.538	1.05	Bolt Tension
		Horizontal	A325N	0.7500	3	6.09	19.88	0.306	1.05	Bolt Shear
T12	22.3334	Diagonal	A325N	0.7500	3	4.89	19.88	0.246	1.05	Bolt Shear
		Horizontal	A325N	0.7500	2	5.96	19.88	0.300	1.05	Bolt Shear
					2	4.95	19.88	0.249	1.05	Bolt Shear

## Compression Checks

## Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	K/l <sub>r</sub>	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> /φP <sub>n</sub>
T1	244.917 - 224.792	ROHN 2.5 STD	20.13	4.02	51.0	1.7040	-16.02	63.41	0.253 <sup>1</sup>
T2	224.792 - 204.625	ROHN 3 EH	20.20	5.05	53.3	3.0159	-45.93	110.22	0.417 <sup>1</sup>
T3	204.625 - 184.438	ROHN 3.5 EH	20.23	6.74	61.9	3.6784	-74.24	125.07	0.594 <sup>1</sup>
T4	184.438 - 164.229	ROHN 4 EH	20.25	6.75	54.8	4.4074	-103.19	159.18	0.648 <sup>1</sup>
T5	164.229 - 144.021	ROHN 5 EH	20.25	6.75	44.0	6.1120	-131.94	238.68	0.553 <sup>1</sup>
T6	144.021 - 123.813	ROHN 5 EH	20.25	10.12	66.1	6.1120	-157.78	199.91	0.789 <sup>1</sup>
T7	123.813 - 103.604	ROHN 6 EH	20.25	10.12	55.3	8.4049	-186.12	302.33	0.616 <sup>1</sup>
T8	103.604 - 83.3333	ROHN 6 EH	20.31	10.15	55.5	8.4049	-214.93	301.91	0.712 <sup>1</sup>
T9	83.3333 - 63	ROHN 6 EH	20.37	10.19	55.7	8.4049	-243.98	301.49	0.809 <sup>1</sup>
T10	63 - 42.6667	ROHN 8 EHS	20.37	10.19	41.9	9.7193	-273.24	384.77	0.710 <sup>1</sup>
T11	42.6667 - 22.3334	ROHN 8 EHS	20.37	10.19	41.9	9.7193	-279.00	384.77	0.725 <sup>1</sup>
T12	22.3334 - 2	ROHN 8 EH	20.37	10.14	42.3	12.7627	-307.42	503.91	0.610 <sup>1</sup>
					K=1.00				

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

**Diagonal Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub>		Ratio P <sub>u</sub>	
							K	φP <sub>u</sub>	K	φP <sub>u</sub>
T1	244.917 - 224.792	L1-3/4x1-3/4x3/16	7.70	3.60	125.9 K=1.00	0.6211	-3.76	11.21	0.335 <sup>1</sup>	
T2	224.792 - 204.625	L1-3/4x1-3/4x3/16	9.83	4.80	167.7 K=1.00	0.6211	-4.25	6.32	0.671 <sup>1</sup>	
T3	204.625 - 184.438	L2-1/2x2-1/2x3/16	12.47	6.15	149.0 K=1.00	0.9023	-4.75	11.63	0.408 <sup>1</sup>	
T4	184.438 - 164.229	L2-1/2x2-1/2x1/4	14.33	7.05	172.2 K=1.00	1.1875	-5.36	11.47	0.467 <sup>1</sup>	
T5	164.229 - 144.021	L2-1/2x2-1/2x5/16	16.25	7.97	195.5 K=1.00	1.4648	-5.99	10.97	0.546 <sup>1</sup>	
T6	144.021 - 123.813	L 3 x 3 x 5/16	19.57	9.71	197.8 K=1.00	1.7800	-7.25	13.02	0.556 <sup>1</sup>	
T7	123.813 - 103.604	L3-1/2x3-1/2x1/4	21.44	10.59	183.1 K=1.00	1.6900	-7.97	14.42	0.552 <sup>1</sup>	
T8	103.604 - 83.3333	L 4 x 4 x 5/16	23.37	11.54	175.1 K=1.00	2.4000	-8.81	22.41	0.393 <sup>1</sup>	
T9	83.3333 - 63	L 4 x 4 x 5/16	25.33	12.52	190.0 K=1.00	2.4000	-9.64	19.03	0.506 <sup>1</sup>	
T10	63 - 42.6667	L 4 x 4 x 5/16	27.31	13.42	203.6 K=1.00	2.4000	-10.41	16.57	0.629 <sup>1</sup>	
T11	42.6667 - 22.3334	KL/R > 200 (C) - 196 P3 STD	24.70	12.35	127.4 K=1.00	2.2285	-18.28	31.03	0.589 <sup>1</sup>	
T12	22.3334 - 2	P3 STD	25.26	12.63	130.2 K=1.00	2.2285	-17.89	29.68	0.603 <sup>1</sup>	

<sup>1</sup> P<sub>u</sub> / φP<sub>u</sub> controls**Horizontal Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub>		Ratio P <sub>u</sub>	
							K	φP <sub>u</sub>	K	φP <sub>u</sub>
T11	42.6667 - 22.3334	ROHN 2.5 STD	25.88	12.58	159.4 K=1.00	1.7040	-9.53	15.16	0.629 <sup>1</sup>	
T12	22.3334 - 2	P3 STD	28.03	13.66	140.8 K=1.00	2.2285	-9.91	25.38	0.390 <sup>1</sup>	

<sup>1</sup> P<sub>u</sub> / φP<sub>u</sub> controls**Top Girt Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub>		Ratio P <sub>u</sub>	
							K	φP <sub>u</sub>	K	φP <sub>u</sub>
T1	244.917 - 224.792	L 2 x 2 x 1/8	6.56	6.11	184.6 K=1.00	0.4844	-0.35	4.07	0.087 <sup>1</sup>	
T2	224.792 - 204.625	L1-3/4x1-3/4x3/16	6.56	6.11	213.6 K=1.00	0.6211	-0.06	3.89	0.016 <sup>1</sup>	

<sup>1</sup> P<sub>u</sub> / φP<sub>u</sub> controls

**Redundant Horizontal (1) Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub>		Ratio $\frac{P_u}{\phi P_n}$
							K	K	
T11	42.6667 - 22.3334	ROHN 1.5 STD	6.47	6.11	117.8 K=1.00	0.7995	-4.84	13.02	0.372 <sup>1</sup>
T12	22.3334 - 2	ROHN 1.5 STD	7.01	6.65	128.1 K=1.00	0.7995	-5.33	11.00	0.485 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls**Redundant Diagonal (1) Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub>		Ratio $\frac{P_u}{\phi P_n}$
							K	K	
T11	42.6667 - 22.3334	ROHN 2 STD	11.78	11.06	168.7 K=1.00	1.0745	-4.41	8.53	0.516 <sup>1</sup>
T12	22.3334 - 2	ROHN 2 STD	12.02	11.35	173.1 K=1.00	1.0745	-4.58	8.10	0.565 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls**Redundant Hip (1) Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub>		Ratio $\frac{P_u}{\phi P_n}$
							K	K	
T11	42.6667 - 22.3334	ROHN 1.5 STD	6.47	6.47	124.7 K=1.00	0.7995	-0.03	11.61	0.002 <sup>1</sup>
T12	22.3334 - 2	ROHN 1.5 STD	7.01	7.01	135.1 K=1.00	0.7995	-0.02	9.90	0.002 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls**Redundant Hip Diagonal (1) Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub>		Ratio $\frac{P_u}{\phi P_n}$
							K	K	
T11	42.6667 - 22.3334	Rohn 2.5 STD	15.37	15.37	194.7 K=1.00	1.7040	-0.08	10.15	0.008 <sup>1</sup>
T12	22.3334 - 2	Rohn 2.5 STD	16.05	16.05	203.3 K=1.00	1.7040	-0.08	9.31	0.008 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

# inxTower

**GPD**  
520 South Main Street Suite 2531  
Akron, Ohio 44311  
Phone: (330) 572-2100  
FAX: (330) 572-2101

## Job

BU #: 841294 MONROE-GUINEA ROAD

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## Project

2020777.841294.15

## Date

14:51:29 06/04/20

## Client

Crown Castle, Inc

## Designed by

msteward

## Inner Bracing Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub>		Ratio	
							K	φP <sub>u</sub>	K	φP <sub>u</sub>
T11	42.6667 - 22.3334	ROHN 2 STD	12.94	12.94	197.3 K=1.00	1.0745	-0.01	6.24	0.002 <sup>1</sup>	
T12	22.3334 - 2	P3 STD	14.02	14.02	144.5 K=1.00	2.2285	-0.02	24.10	0.001 <sup>1</sup>	

<sup>1</sup> P<sub>u</sub> / φP<sub>u</sub> controls

## Tension Checks

## Leg Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub>		Ratio	
							K	φP <sub>u</sub>	K	φP <sub>u</sub>
T1	244.917 - 224.792	ROHN 2.5 STD	20.13	4.02	51.0	1.7040	11.75	76.68	0.153 <sup>1</sup>	
T2	224.792 - 204.625	ROHN 3 EH	20.20	5.05	53.3	3.0159	37.62	135.72	0.277 <sup>1</sup>	
T3	204.625 - 184.438	ROHN 3.5 EH	20.23	6.74	61.9	3.6784	63.68	165.53	0.385 <sup>1</sup>	
T4	184.438 - 164.229	ROHN 4 EH	20.25	6.75	54.8	4.4074	89.58	198.34	0.452 <sup>1</sup>	
T5	164.229 - 144.021	ROHN 5 EH	20.25	6.75	44.0	6.1120	114.55	275.04	0.416 <sup>1</sup>	
T6	144.021 - 123.813	ROHN 5 EH	20.25	10.12	66.1	6.1120	136.76	275.04	0.497 <sup>1</sup>	
T7	123.813 - 103.604	ROHN 6 EH	20.25	10.12	55.3	8.4049	160.62	378.22	0.425 <sup>1</sup>	
T8	103.604 - 83.3333	ROHN 6 EH	20.31	10.15	55.5	8.4049	184.34	378.22	0.487 <sup>1</sup>	
T9	83.3333 - 63	ROHN 6 EH	20.37	10.19	55.7	8.4049	207.98	378.22	0.550 <sup>1</sup>	
T10	63 - 42.6667	ROHN 8 EHS	20.37	10.19	41.9	9.7193	231.35	437.37	0.529 <sup>1</sup>	
T11	42.6667 - 22.3334	ROHN 8 EHS	20.37	10.19	41.9	9.7193	234.93	437.37	0.537 <sup>1</sup>	
T12	22.3334 - 2	ROHN 8 EH	20.37	0.08	0.3	12.7627	280.54	574.32	0.488 <sup>1</sup>	

<sup>1</sup> P<sub>u</sub> / φP<sub>u</sub> controls

## Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub>		Ratio	
							K	φP <sub>u</sub>	K	φP <sub>u</sub>
T1	244.917 - 224.792	L1-3/4x1-3/4x3/16	7.70	3.60	82.9	0.3779	3.70	16.44	0.225 <sup>1</sup>	
T2	224.792 - 204.625	L1-3/4x1-3/4x3/16	9.38	4.57	104.5	0.3779	4.48	16.44	0.272 <sup>1</sup>	
T3	204.625 -	L2-1/2x2-1/2x3/16	11.88	5.85	91.8	0.5889	4.75	25.62	0.186 <sup>1</sup>	



Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	K/l/r	A in <sup>2</sup>	P <sub>n</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_n}{\phi P_n}$
T4	184.438 - 164.229	L2-1/2x2-1/2x1/4	14.33	7.05	111.6	0.7734	5.30	33.64	0.158 <sup>1</sup>
T5	164.229 - 144.021	L2-1/2x2-1/2x5/16	16.25	7.97	127.2	0.9521	6.02	41.42	0.145 <sup>1</sup>
T6	144.021 - 123.813	L 3 x 3 x 5/16	19.57	9.71	127.9	1.1592	7.21	50.43	0.143 <sup>1</sup>
T7	123.813 - 103.604	L3-1/2x3-1/2x1/4	21.44	10.59	117.9	1.1269	7.87	49.02	0.161 <sup>1</sup>
T8	103.604 - 83.3333	L 4 x 4 x 5/16	23.37	11.54	113.0	1.5949	8.73	69.38	0.126 <sup>1</sup>
T9	83.3333 - 63	L 4 x 4 x 5/16	25.33	12.52	122.5	1.5949	9.48	69.38	0.137 <sup>1</sup>
T10	63 - 42.6667	L 4 x 4 x 5/16	27.31	13.42	131.2	1.5949	10.27	69.38	0.148 <sup>1</sup>
T11	42.6667 - 22.3334	P3 STD	24.70	12.35	127.4	2.2285	17.41	100.28	0.174 <sup>1</sup>
T12	22.3334 - 2	P3 STD	25.26	12.63	130.2	2.2285	16.95	100.28	0.169 <sup>1</sup>

<sup>1</sup> P<sub>n</sub> / φP<sub>n</sub> controls**Horizontal Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	K/l/r	A in <sup>2</sup>	P <sub>n</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_n}{\phi P_n}$
T11	42.6667 - 22.3334	ROHN 2.5 STD	25.88	12.58	159.4	1.7040	9.78	76.68	0.128 <sup>1</sup>
T12	22.3334 - 2	P3 STD	28.03	13.66	140.8	2.2285	9.84	100.28	0.098 <sup>1</sup>

<sup>1</sup> P<sub>n</sub> / φP<sub>n</sub> controls**Top Girt Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	K/l/r	A in <sup>2</sup>	P <sub>n</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_n}{\phi P_n}$
T1	244.917 - 224.792	L 2 x 2 x 1/8	6.56	6.11	121.2	0.3047	0.35	13.25	0.026 <sup>1</sup>
T2	224.792 - 204.625	L1-3/4x1-3/4x3/16	6.56	6.11	141.3	0.3779	0.04	16.44	0.003 <sup>1</sup>

<sup>1</sup> P<sub>n</sub> / φP<sub>n</sub> controls**Redundant Horizontal (1) Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	K/l/r	A in <sup>2</sup>	P <sub>n</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_n}{\phi P_n}$
T11	42.6667 - 22.3334	ROHN 1.5 STD	6.47	6.11	117.8	0.7995	4.84	35.98	0.135 <sup>1</sup>
T12	22.3334 - 2	ROHN 1.5 STD	7.01	6.65	128.1	0.7995	5.33	35.98	0.148 <sup>1</sup>

<sup>1</sup> P<sub>n</sub> / φP<sub>n</sub> controls

**Redundant Diagonal (1) Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T11	42.6667 - 22.3334	ROHN 2 STD	11.78	11.06	168.7	1.0745	4.41	48.35	0.091 <sup>1</sup>
T12	22.3334 - 2	ROHN 2 STD	12.02	11.35	173.1	1.0745	4.58	48.35	0.095 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls**Redundant Hip (1) Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T11	42.6667 - 22.3334	ROHN 1.5 STD	6.47	6.47	124.7	0.7995	0.02	35.98	0.000 <sup>1</sup>
T12	22.3334 - 2	ROHN 1.5 STD	7.01	7.01	135.1	0.7995	0.01	35.98	0.000 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls**Redundant Hip Diagonal (1) Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T11	42.6667 - 22.3334	Rohn 2.5 STD	15.37	15.37	194.7	1.7040	0.07	76.68	0.001 <sup>1</sup>
T12	22.3334 - 2	Rohn 2.5 STD	16.05	16.05	203.3	1.7040	0.07	76.68	0.001 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls**Inner Bracing Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T11	42.6667 - 22.3334	ROHN 2 STD	12.94	12.94	197.3	1.0745	0.00	48.35	0.000 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls**Section Capacity Table**

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	φP <sub>allow</sub> K	% Capacity	Pass Fail
T1	244.917 - 224.792	Leg	ROHN 2.5 STD	3	-16.02	66.58	24.1	Pass

# tnxTower

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520 South Main Street Suite 2531  
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## Job

BU #: 841294 MONROE-GUINEA ROAD

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## Project

2020777.841294.15

## Date

14:51:29 06/04/20

## Client

Crown Castle, Inc

## Designed by

msteward

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
T2	224.792 - 204.625	Leg	ROHN 3 EH	39	-45.93	115.74	39.7	Pass
T3	204.625 - 184.438	Leg	ROHN 3.5 EH	69	-74.24	131.32	56.5	Pass
T4	184.438 - 164.229	Leg	ROHN 4 EH	90	-103.19	167.14	61.7	Pass
T5	164.229 - 144.021	Leg	ROHN 5 EH	111	-131.94	250.61	52.6	Pass
T6	144.021 - 123.813	Leg	ROHN 5 EH	132	-157.78	209.91	75.2	Pass
T7	123.813 - 103.604	Leg	ROHN 6 EH	147	-186.12	317.44	58.6	Pass
T8	103.604 - 83.3333	Leg	ROHN 6 EH	162	-214.93	317.01	67.8	Pass
T9	83.3333 - 63	Leg	ROHN 6 EH	177	-243.98	316.57	77.1	Pass
T10	63 - 42.6667	Leg	ROHN 8 EHS	192	-273.24	404.01	67.6	Pass
T11	42.6667 - 22.3334	Leg	ROHN 8 EHS	207	-279.00	404.01	69.1	Pass
T12	22.3334 - 2	Leg	ROHN 8 EH	240	-307.42	529.11	58.1	Pass
T1	244.917 - 224.792	Diagonal	L1-3/4x1-3/4x3/16	11	-3.76	11.77	31.9	Pass
T2	224.792 - 204.625	Diagonal	L1-3/4x1-3/4x3/16	47	-4.25	6.64	61.8 (b)	Pass
T3	204.625 - 184.438	Diagonal	L2-1/2x2-1/2x3/16	74	-4.75	12.21	74.7 (b)	Pass
T4	184.438 - 164.229	Diagonal	L2-1/2x2-1/2x1/4	94	-5.36	12.04	38.9	Pass
T5	164.229 - 144.021	Diagonal	L2-1/2x2-1/2x5/16	115	-5.99	11.52	73.0 (b)	Pass
T6	144.021 - 123.813	Diagonal	L 3 x 3 x 5/16	136	-7.25	13.67	61.1 (b)	Pass
T7	123.813 - 103.604	Diagonal	L3-1/2x3-1/2x1/4	151	-7.97	15.14	55.5 (b)	Pass
T8	103.604 - 83.3333	Diagonal	L 4 x 4 x 5/16	166	-8.81	23.53	52.6	Pass
T9	83.3333 - 63	Diagonal	L 4 x 4 x 5/16	181	-9.64	19.99	71.8 (b)	Pass
T10	63 - 42.6667	Diagonal	L 4 x 4 x 5/16	196	-10.41	17.40	37.5	Pass
T11	42.6667 - 22.3334	Diagonal	P3 STD	219	-18.28	32.58	52.7 (b)	Pass
T12	22.3334 - 2	Diagonal	P3 STD	252	-17.89	31.16	48.2	Pass
T11	42.6667 - 22.3334	Horizontal	ROHN 2.5 STD	215	-9.53	15.91	57.3 (b)	Pass
T12	22.3334 - 2	Horizontal	P3 STD	248	-9.91	26.65	62.1 (b)	Pass
T1	244.917 - 224.792	Top Girt	L 2 x 2 x 1/8	4	-0.35	4.27	59.9	Pass
T2	224.792 - 204.625	Top Girt	L1-3/4x1-3/4x3/16	40	-0.06	4.09	8.3	Pass
T11	42.6667 - 22.3334	Redund Horz 1 Bracing	ROHN 1.5 STD	220	-4.84	13.67	1.5	Pass
T12	22.3334 - 2	Redund Horz 1 Bracing	ROHN 1.5 STD	253	-5.33	11.55	35.4	Pass
T11	42.6667 - 22.3334	Redund Diag 1 Bracing	ROHN 2 STD	221	-4.41	8.96	46.2	Pass
T12	22.3334 - 2	Redund Diag 1 Bracing	ROHN 2 STD	254	-4.58	8.51	49.2	Pass
T11	42.6667 - 22.3334	Redund Hip 1 Bracing	ROHN 1.5 STD	222	-0.03	12.19	53.8	Pass
T12	22.3334 - 2	Redund Hip 1 Bracing	ROHN 1.5 STD	255	-0.02	10.40	0.2	Pass

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
T11	42.6667 - 22.3334	Redund Hip Diagonal I Bracing	Rohn 2.5 STD	223	-0.08	10.66	0.8	Pass
T12	22.3334 - 2	Redund Hip Diagonal I Bracing	Rohn 2.5 STD	256	-0.08	9.78	0.8	Pass
T11	42.6667 - 22.3334	Inner Bracing	ROHN 2 STD	236	-0.01	6.55	0.5	Pass
T12	22.3334 - 2	Inner Bracing	P3 STD	268	-0.02	25.30	0.4	Pass

Summary ELC: Load Case 7

Leg (T9) Pass

Diagonal (T12) Pass

Horizontal (T11) Pass

Top Girt (T1) Pass

Redund Horz I Bracing (T12) Pass

Redund Diag I Bracing (T12) 53.8 Pass

Redund Hip I Bracing (T11) 0.2 Pass

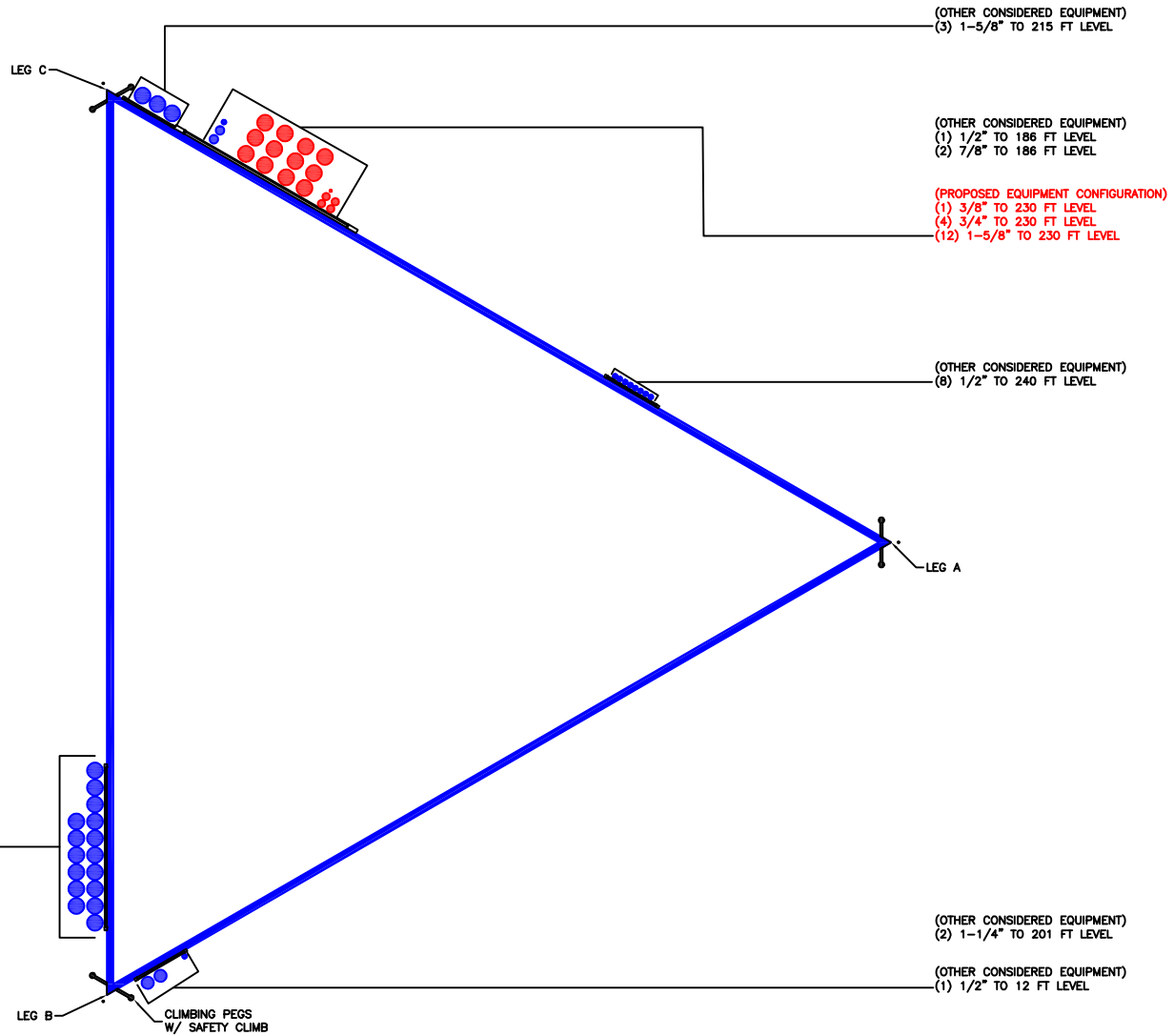
Redund Hip Diagonal I Bracing (T12) 0.8 Pass

Inner Bracing (T12) 0.5 Pass

Bolt Checks (T11) 74.7 Pass

Rating = 77.1 Pass

**APPENDIX B**  
**BASE LEVEL DRAWING**



CROWN REGION ADDRESS  
USA

WT	HC	BT	SM	CM	SM	SL	ST
20/1/19	UPDATED PER WORK ORDER 1802282						
09/03/19	UPDATED PER WORK ORDER 1802288						
10/05/19	UPDATED PER WORK ORDER 1740072						
09/09/19	UPDATED PER WORK ORDER 1786433						
17/09/19	UPDATED PER WORK ORDER 1792020						
18/11/19	UPDATED PER WORK ORDER 1810801						
18/11/19	UPDATED PER WORK ORDER 1811399						
03/12/19	UPDATED PER WORK ORDER 1812347						
20/05/20	UPDATED PER WORK ORDER 1826332						

DRAWN BY: VJL  
CHECKED BY: AGT  
DRAWING DATE: 18/03/14

SITE NUMBER:

SITE NAME:

SITE NAME:

MONROE-GUINEA ROAD

BUSINESS UNIT NUMBER:

841294

SITE ADDRESS:

230 GUINEA ROAD  
MONROE, CT 06468  
FAIRFIELD COUNTY  
USA

SHEET TITLE:

**BASE LEVEL**

SHEET NUMBER:

N.T.S.

**A1-0**

**APPENDIX C**  
**ADDITIONAL CALCULATIONS**



### Project Information

BU #	841294
Site Name	MONROE-GUINEA ROAD
Order #	517061 Rev 0

### Tower Information

Tower Type	Self Support
TIA-222 Rev	H

Apply TIA-222-H Section 15.5

### Applied Loads

	Comp.	Uplift
Axial (k)		279.00
Shear (k)		31.00

### Anchor Rod Data

Quantity:	10
Diameter (in):	1
<u>Material Grade:</u>	A354-BC
Grout Considered:	Yes
$l_{ar}$ (in):	0
Eta Factor, $\eta$ :	
Thread Type:	N-Included
Configuration:	Symmetrical

Fy=109 ksi Fu=125 ksi  
Not Considered,  $l_{ar} \leq 1(d)$

### Anchor Rod Results

Axial, $P_{u,t}$ (kips)	27.90
Shear, $V_u$ (kips)	3.10
Moment, $M_u$ (kip-in)	-
Axial Cap., $\phi P_{n,t}$ (kips)	56.81
Shear Cap., $\phi V_n$ (kips)	36.82
Moment Cap., $\phi M_n$ (kip-in)	-
Stress Rating	23.6%

Pass



## Drilled Pier Foundation



BU #: 841294  
 Site Name: MONROE-GUINEA RC  
 Order Number: 517061 Rev 0

TIA-222 Revision: H  
 Tower Type: Self Support

Applied Loads		
	Comp.	Uplift
Moment (kip-ft)		
Axial Force (kips)	334	279
Shear Force (kips)	36	31

Material Properties		
Concrete Strength, f <sub>c</sub> :	3	ksi
Rebar Strength, F <sub>y</sub> :	60	ksi

Pier Design Data		
Depth	20	ft
Ext. Above Grade	2	ft
Pier Section 1		
<i>From 2' above grade to 5' below grade</i>		
Pier Diameter	4	ft
Rebar Quantity	12	
Rebar Size	9	
Clear Cover to Ties	8.5	in
Tie Size	4	
Pier Section 2		
<i>From 5' below grade to 20' below grade</i>		
Pier Diameter	3.5	ft
Rebar Quantity	12	
Rebar Size	9	
Clear Cover to Ties	6.5	in
Tie Size	4	

Rebar & Pier Options  
 Embedded Pole Inputs  
 Belled Pier Inputs

### Analysis Results

Soil Vertical Check	Compression	Uplift
Skin Friction (kips)	643.34	428.89
End Bearing (kips)	453.30	-
Weight of Concrete (kips)	41.81	31.36
Total Capacity (kips)	1096.64	460.25
Axial (kips)	375.81	279.00
Rating*	32.6%	57.7%

Check Limitation	
Apply TIA-222-H Section 15.5:	<input checked="" type="checkbox"/>
N/A	<input type="checkbox"/>

Soil Interaction Rating*	57.7%
--------------------------	-------

\*Rating per TIA-222-H Section 15.5

### Soil Profile

Groundwater Depth	NA	# of Layers	3
-------------------	----	-------------	---

Layer	Top (ft)	Bottom (ft)	Thickness (ft)	γ <sub>soil</sub> (pcf)	γ <sub>concrete</sub> (pcf)	Cohesion (ksf)	Angle of Friction (degrees)	Calculated Ultimate Skin Friction Comp (ksf)	Calculated Ultimate Skin Friction Uplift (ksf)	Ultimate Skin Friction Comp Override (ksf)	Ultimate Skin Friction Uplift Override (ksf)	Ult. Gross Bearing Capacity (ksf)	SPT Blow Count	Soil Type
1	0	3.5	3.5	120	150			0.000	0.000					Cohesionless
2	3.5	7	3.5	120	150		34	0.000	0.000	0.00	0.00			Cohesionless
3	7	20	13	150	150	10		4.500	4.500	6.00	4.00	62.82		Cohesive



**Caisson Foundation LPile Summary**  
**MONROE-GUINEA ROAD BU #: 841294**  
**2020777.841294.15**

Reinforcement Check	
Allowable Stress Ratio =	1.00
Apply TIA-222-H Section 15.5?	Yes
<b>Case 1 - Uplift</b>	
Nominal Moment Capacity (Mn) =	7612.4 k-in
=	634.4 k-ft
$\phi$ =	0.9
Factored Moment Capacity ( $\phi Mn$ ) =	570.93 k-ft
Maximum Bending Moment (Mu) =	3059750 in-lbs
=	255.0 k-ft
<b>Mu/<math>\phi Mn</math> =</b>	<b>42.5%</b>
<b>OK</b>	
<b>Case 2 - Compression</b>	
Nominal Moment Capacity (Mn) =	15782.1 k-in
=	1315.2 k-ft
$\phi$ =	0.9
Factored Moment Capacity ( $\phi Mn$ ) =	1183.66 k-ft
Maximum Bending Moment (Mu) =	4019135 in-lbs
=	334.9 k-ft
<b>Mu/<math>\phi Mn</math> =</b>	<b>26.9%</b>
<b>OK</b>	

Deflection Check	
Load Type	Design
Allowable Deflection	1.5 in
Max Deflection from LPILE	0.5041 in
<b>Deflections are Acceptable</b>	

LPile for Windows, Version 2019-11.001

Analysis of Individual Piles and Drilled Shafts  
Subjected to Lateral Loading Using the p-y Method  
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Files Used for Analysis

Path to file locations:  
\Crown\841294\15\5\_Structural\00\_Structure\00\_Rev 0\01\_Calcs\

Name of input data file:  
841294 Lpile.lp11

Name of output report file:  
841294 Lpile.lp11

Name of plot output file:  
841294 Lpile.lp11

Name of runtime message file:  
841294 Lpile.lp11

Date and Time of Analysis

Date: June 4, 2020 Time: 15:02:18

Problem Title

Project Name: BU #: 841294 MONROE-GUINEA ROAD

Job Number: 2020777.841294.14

Client: Crown Castle Inc

Engineer:NT

841294 Lpile.lp110

Description:

-----  
Program Options and Settings  
-----

Computational Options:

- Use unfactored loads in computations (conventional analysis)
- Engineering Units Used for Data Input and Computations:
  - US Customary System Units (pounds, feet, inches)

Analysis Control Options:

- Maximum number of iterations allowed = 1000
- Deflection tolerance for convergence = 1.0000E-04 in
- Maximum allowable deflection = 100.0000 in
- Number of pile increments = 100

Loading Type and Number of Cycles of Loading:

- Static loading specified
- Use of p-y modification factors for p-y curves not selected
- Analysis uses layering correction (Method of Georgiadis)
- No distributed lateral loads are entered
- Loading by lateral soil movements acting on pile not selected
- Input of shear resistance at the pile tip not selected
- Input of moment resistance at the pile tip not selected
- Computation of pile-head foundation stiffness matrix not selected
- Push-over analysis of pile not selected
- Buckling analysis of pile not selected

Output Options:

- Output files use decimal points to denote decimal symbols.
- Values of pile-head deflection, bending moment, shear force, and soil reaction are printed for full length of pile.
- Printing Increment (nodal spacing of output points) = 1
- No p-y curves to be computed and reported for user-specified depths
- Print using wide report formats

-----  
Pile Structural Properties and Geometry  
-----

Number of pile sections defined = 1  
Total length of pile = 22.000 ft  
Depth of ground surface below top of pile = 2.0000 ft

Pile diameters used for p-y curve computations are defined using 2 points.

p-y curves are computed using pile diameter values interpolated with depth over the length of the pile. A summary of values of pile diameter vs. depth follows.

Point No.	Depth Below Pile Head feet	Pile Diameter inches
1	0.0000	42.0000
2	22.000	42.0000

Input Structural Properties for Pile Sections:

Pile Section No. 1:

Section 1 is a round drilled shaft, bored pile, or CIDH pile  
 Length of section = 22.000000 ft  
 Shaft Diameter = 42.000000 in  
 Shear capacity of section = 0.0000 lbs

-----  
 Ground Slope and Pile Batter Angles  
 -----

Ground Slope Angle = 0.000 degrees  
 = 0.000 radians  
 Pile Batter Angle = 0.000 degrees  
 = 0.000 radians

-----  
 Soil and Rock Layering Information  
 -----

The soil profile is modelled using 3 layers

Layer 1 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 2.000000 ft  
 Distance from top of pile to bottom of layer = 5.500000 ft  
 Effective unit weight at top of layer = 120.000000 pcf  
 Effective unit weight at bottom of layer = 120.000000 pcf  
 Friction angle at top of layer = 0.010000 deg.  
 Friction angle at bottom of layer = 0.010000 deg.  
 Subgrade k at top of layer = 0.0000 pci  
 Subgrade k at bottom of layer = 0.0000 pci

NOTE: Default values for subgrade k will be computed for this layer.

Layer 2 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 5.500000 ft  
 Distance from top of pile to bottom of layer = 9.000000 ft  
 Effective unit weight at top of layer = 120.000000 pcf  
 Effective unit weight at bottom of layer = 120.000000 pcf  
 Friction angle at top of layer = 34.000000 deg.  
 Friction angle at bottom of layer = 34.000000 deg.  
 Subgrade k at top of layer = 0.0000 pci  
 Subgrade k at bottom of layer = 0.0000 pci

NOTE: Default values for subgrade k will be computed for this layer.

Layer 3 is soft clay, p-y criteria by Matlock, 1970

Distance from top of pile to top of layer = 9.000000 ft  
 Distance from top of pile to bottom of layer = 22.000000 ft  
 Effective unit weight at top of layer = 150.000000 pcf  
 Effective unit weight at bottom of layer = 150.000000 pcf  
 Undrained cohesion at top of layer = 10000. psf  
 Undrained cohesion at bottom of layer = 10000. psf  
 Epsilon-50 at top of layer = 0.0000

Epsilon-50 at bottom of layer  
 841294 Lpile.lp110  
 = 0.0000

NOTE: Default values for Epsilon-50 will be computed for this layer.

(Depth of the lowest soil layer extends 0.000 ft below the pile tip)

\*\*\*\* Warning - Possible Input Data Error \*\*\*\*

Values entered for effective unit weights of soil were outside the limits of 20 pcf to 140 pcf.

The maximum input value, in layer 3, for effective unit weight = 150.00 pcf

This data may be erroneous. Please check your data.

-----  
 Summary of Input Soil Properties  
 -----

Layer Num.	Soil Type Name (p-y Curve Type)	Layer Depth ft	Effective Unit Wt. pcf	Undrained Cohesion psf	Angle of Friction deg.	E50 or krm	kpy or pci
1	Sand (Reese, et al.)	2.0000	120.0000	--	0.01000	--	default
2	Sand (Reese, et al.)	5.5000	120.0000	--	34.0000	--	default
3	Soft Clay	9.0000 22.0000	120.0000 150.0000	-- 10000.	34.0000 --	-- default	default --

-----  
 Static Loading Type  
 -----

Static loading criteria were used when computing p-y curves for all analyses.

-----  
 Pile-head Loading and Pile-head Fixity Conditions  
 -----

Number of loads specified = 2

Load No.	Load Type	Condition 1	Condition 2	Axial Thrust Force, lbs	Compute Top y vs. Pile Length
1	1	V = 31000. lbs	M = 0.0000 in-lbs	-279000.	No
2	1	V = 36000. lbs	M = 0.0000 in-lbs	334000.	No

V = shear force applied normal to pile axis  
 M = bending moment applied to pile head  
 y = lateral deflection normal to pile axis  
 S = pile slope relative to original pile batter angle  
 R = rotational stiffness applied to pile head  
 Values of top y vs. pile lengths can be computed only for load types with specified shear loading (Load Types 1, 2, and 3).  
 Thrust force is assumed to be acting axially for all pile batter angles.

841294 Lpile.lp110  
 Computations of Nominal Moment Capacity and Nonlinear Bending Stiffness

-----  
 Axial thrust force values were determined from pile-head loading conditions

Number of Pile Sections Analyzed = 1

Pile Section No. 1:  
 -----

Dimensions and Properties of Drilled Shaft (Bored Pile):  
 -----

Length of Section = 22.000000 ft  
 Shaft Diameter = 42.000000 in  
 Concrete Cover Thickness (to edge of long. rebar) = 3.000000 in  
 Number of Reinforcing Bars = 12 bars  
 Yield Stress of Reinforcing Bars = 60000. psi  
 Modulus of Elasticity of Reinforcing Bars = 29000000. psi  
 Gross Area of Shaft = 1385. sq. in.  
 Total Area of Reinforcing Steel = 12.000000 sq. in.  
 Area Ratio of Steel Reinforcement = 0.87 percent  
 Edge-to-Edge Bar Spacing = 7.897538 in  
 Maximum Concrete Aggregate Size = 0.750000 in  
 Ratio of Bar Spacing to Aggregate Size = 10.53  
 Offset of Center of Rebar Cage from Center of Pile = 0.0000 in

Axial Structural Capacities:  
 -----

Nom. Axial Structural Capacity = 0.85 Fc Ac + Fy As = 4222.278 kips  
 Tensile Load for Cracking of Concrete = -537.570 kips  
 Nominal Axial Tensile Capacity = -720.000 kips

Reinforcing Bar Dimensions and Positions Used in Computations:

Bar Number	Bar Diam. inches	Bar Area sq. in.	X inches	Y inches
1	1.128000	1.000000	17.436000	0.00000
2	1.128000	1.000000	15.100019	8.718000
3	1.128000	1.000000	8.718000	15.100019
4	1.128000	1.000000	0.00000	17.436000
5	1.128000	1.000000	-8.718000	15.100019
6	1.128000	1.000000	-15.100019	8.718000
7	1.128000	1.000000	-17.436000	0.00000
8	1.128000	1.000000	-15.100019	-8.718000
9	1.128000	1.000000	-8.718000	-15.100019
10	1.128000	1.000000	0.00000	-17.436000
11	1.128000	1.000000	8.718000	-15.100019
12	1.128000	1.000000	15.100019	-8.718000

NOTE: The positions of the above rebars were computed by LPile

Minimum spacing between any two bars not equal to zero = 7.898 inches between bars 7 and 8.

Ratio of bar spacing to maximum aggregate size = 10.53

Concrete Properties:  
 -----

Compressive Strength of Concrete = 3000. psi  
 Modulus of Elasticity of Concrete = 3122019. psi

841294 Lpile.lp110  
 = -410.791918 psi  
 = 0.001634  
 = -0.0001160  
 = 0.750000 in

Number of Axial Thrust Force Values Determined from Pile-head Loadings = 2

Number	Axial Thrust Force kips
1	-279.000
2	334.000

Definitions of Run Messages and Notes:  
 -----

C = concrete in section has cracked in tension.  
 Y = stress in reinforcing steel has reached yield stress.  
 T = ACI 318 criteria for tension-controlled section met, tensile strain in reinforcement exceeds 0.005 while simultaneously compressive strain in concrete more than 0.003. See ACI 318, Section 10.3.4.  
 Z = depth of tensile zone in concrete section is less than 10 percent of section depth.

Bending Stiffness (EI) = Computed Bending Moment / Curvature.  
 Position of neutral axis is measured from edge of compression side of pile.  
 Compressive stresses and strains are positive in sign.  
 Tensile stresses and strains are negative in sign.

Axial Thrust Force = -279.000 kips

Bending Curvature rad/in.	Bending Moment in-kip	Bending Stiffness kip-in <sup>2</sup>	Depth to N Axis in	Max Comp Strain in/in	Max Tens Strain in/in	Max Conc Stress ksi	Max Steel Stress ksi	Run Msg
6.25000E-07	368.4202213	589472354.	-63.0643576	-0.00003942	-0.00006567	-0.1434945	-1.8471977	
0.00000125	736.8211491	589456919.	-21.0625887	-0.00002633	-0.00007883	-0.0968719	-2.1718313	
0.00000188	736.8211491	392971280.	-406.5860088	-0.0007623	-0.0008411	0.00000	-24.2205830	C
0.00000210	1474.	589404045.	-0.0921235	-2.30309E-07	-0.0001052	-0.0027736	-2.8233039	
0.00000313	1474.	471523236.	-235.5515260	-0.0007361	-0.0008673	0.00000	-24.8676383	C
0.00000375	1474.	392936030.	-192.7929053	-0.0007230	-0.0008805	0.00000	-25.1911659	C
0.00000438	1474.	336802311.	-162.2510334	-0.0007098	-0.0008936	0.00000	-25.5146936	C
0.00000500	1474.	294702023.	-139.3446295	-0.0006967	-0.0009067	0.00000	-25.8382213	C
0.00000563	1474.	261957353.	-121.5285375	-0.0006836	-0.0009198	0.00000	-26.1617489	C
0.00000625	1474.	235761618.	-107.2756639	-0.0006705	-0.0009330	0.00000	-26.4852764	C
0.00000688	1474.	214328744.	-95.6142219	-0.0006573	-0.0009461	0.00000	-26.8088043	C
0.00000750	1474.	196468015.	-85.8963536	-0.0006442	-0.0009592	0.00000	-27.1323319	C
0.00000813	1474.	181355091.	-77.6735419	-0.0006311	-0.0009723	0.00000	-27.4558597	C
0.00000875	1474.	168401156.	-70.6254176	-0.0006180	-0.0009855	0.00000	-27.7793870	C
0.00000938	1474.	157174412.	-64.5170433	-0.0006048	-0.0009986	0.00000	-28.1029150	C
0.00001000	1474.	147351011.	-59.1722157	-0.0005917	-0.0010117	0.00000	-28.4264427	C
0.00001063	1474.	138683305.	-54.4561913	-0.0005786	-0.0010248	0.00000	-28.7499700	C
0.00001125	1474.	130978677.	-50.2641697	-0.0005655	-0.0010380	0.00000	-29.0734978	C
0.00001188	1474.	124085062.	-46.5134135	-0.0005523	-0.0010511	0.00000	-29.3970254	C
0.00001250	1474.	117880809.	-43.1377329	-0.0005392	-0.0010642	0.00000	-29.7205531	C
0.00001313	1474.	112267437.	-40.0835457	-0.0005261	-0.0010773	0.00000	-30.0440808	C
0.00001375	1474.	107164372.	-37.3070119	-0.0005130	-0.0010905	0.00000	-30.3676085	C
0.00001438	1474.	102505051.	-34.7719158	-0.0004987	-0.0011036	0.00000	-30.6911361	C
0.00001500	1474.	98234008.	-32.4408777	-0.0004867	-0.0011167	0.00000	-31.0146638	C
0.00001563	1474.	94304647.	-30.3101467	-0.0004736	-0.0011298	0.00000	-31.3381915	C
0.00001625	1474.	90677545.	-28.3366719	-0.0004605	-0.0011430	0.00000	-31.6617191	C
0.00001688	1474.	87319118.	-26.5093804	-0.0004473	-0.0011561	0.00000	-31.9852467	C
0.00001750	1474.	84200578.	-24.8126098	-0.0004342	-0.0011692	0.00000	-32.3087745	C
0.00001813	1474.	81297110.	-23.2328578	-0.0004211	-0.0011823	0.00000	-32.6323021	C



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0.00001875	1474.	78587206.	-21.7584226	-0.0004080	-0.0011955	0.00000	-32.9558298	C
0.00001938	1474.	76052135.	-20.3791122	-0.0003948	-0.0012086	0.00000	-33.2793573	C
0.00002000	1474.	73675506.	-19.0860088	-0.0003817	-0.0012217	0.00000	-33.6028850	C
0.00002063	1474.	71442915.	-17.8712752	-0.0003686	-0.0012348	0.00000	-33.9264127	C
0.00002125	1474.	69341652.	-16.7272966	-0.0003555	-0.0012480	0.00000	-34.2499404	C
0.00002188	1474.	67360462.	-15.6500482	-0.0003423	-0.0012611	0.00000	-34.5734680	C
0.00002250	1474.	65489338.	-14.6319858	-0.0003292	-0.0012742	0.00000	-34.8969957	C
0.00002313	1474.	63719356.	-13.6689538	-0.0003161	-0.0012873	0.00000	-35.2205233	C
0.00002375	1474.	62042531.	-12.7566077	-0.0003030	-0.0013005	0.00000	-35.5440509	C
0.00002438	1474.	60451697.	-11.8910486	-0.0002898	-0.0013136	0.00000	-35.8675786	C
0.00002503	1474.	57502834.	-10.2865975	-0.0002636	-0.0013398	0.00000	-36.5146340	C
0.00002688	1474.	54828283.	-8.8313977	-0.0002373	-0.0013661	0.00000	-37.1616893	C
0.00002813	1488.	52907068.	-7.5055490	-0.0002111	-0.0013923	0.00000	-37.8087446	C
0.00002938	1554.	52906984.	-6.2925385	-0.0001848	-0.0014186	0.00000	-38.4557999	C
0.00003063	1620.	52906907.	-5.1785492	-0.0001586	-0.0014448	0.00000	-39.1028552	C
0.00003188	1686.	52906836.	-4.1519317	-0.0001323	-0.0014711	0.00000	-39.7499105	C
0.00003313	1753.	52906771.	-3.2027947	-0.0001061	-0.0014973	0.00000	-40.3969659	C
0.00003438	1819.	52906710.	-2.3226859	-0.00007984	-0.0015236	0.00000	-41.0440211	C
0.00003563	1885.	52906654.	-1.5043391	-0.00005359	-0.0015498	0.00000	-41.6910765	C
0.00003688	1951.	52906601.	-0.7414734	-0.00002734	-0.0015761	0.00000	-42.3381318	C
0.00003813	2017.	52906552.	-0.0286317	-0.00000109	-0.0016023	0.00000	-42.9851871	C
0.00003938	2086.	52981091.	0.6286413	0.00002475	-0.0016290	0.0602397	-43.6440139	C
0.00004063	2166.	53328170.	1.2062766	0.00004900	-0.0016572	0.1468169	-44.3490115	C
0.00004188	2257.	54627377.	1.7148885	0.00007181	-0.0016869	0.2269953	-45.0959515	C
0.00004313	2356.	55487887.	2.1631415	0.00009329	-0.0017180	0.3013949	-45.8815034	C
0.00004438	2462.	55487887.	2.5604499	0.0001136	-0.0017501	0.3708643	-46.7001159	C
0.00004563	2575.	56427805.	2.9157745	0.0001330	-0.0017832	0.4362974	-47.5454729	C
0.00004688	2691.	57408278.	3.2372551	0.0001517	-0.0018170	0.4985652	-48.4110752	C
0.00004813	2811.	58413518.	3.5292975	0.0001698	-0.0018514	0.5580365	-49.2944553	C
0.00004938	2935.	59444593.	3.7931254	0.0001873	-0.0018865	0.6146213	-50.1970621	C
0.00005063	3060.	60449726.	4.0393272	0.0002045	-0.0019218	0.6697895	-51.1064189	C
0.00005188	3188.	61464652.	4.2629966	0.0002211	-0.0019576	0.7225417	-52.0318231	C
0.00005313	3318.	62447992.	4.4728702	0.0002376	-0.0019936	0.7741403	-52.9622654	C
0.00005438	3449.	63427199.	4.6651552	0.0002537	-0.0020301	0.8237974	-53.9052269	C
0.00005563	3580.	64362946.	4.8484368	0.0002697	-0.0020666	0.8728322	-54.8487715	C
0.00005688	3714.	65301673.	5.0147048	0.0002852	-0.0021035	0.9197503	-55.8070899	C
0.00005813	3848.	66195003.	5.1745897	0.0003008	-0.0021405	0.9662803	-56.7641134	C
0.00005938	3982.	67068412.	5.3237669	0.0003161	-0.0021777	1.0115800	-57.7279655	C
0.00006188	4253.	68736874.	5.4631535	0.0003312	-0.0022150	1.0557137	-58.6982493	C
0.00006313	4384.	69453034.	5.5975821	0.0003464	-0.0022524	1.0994757	-59.6673071	C
0.00006438	4502.	69937620.	5.7203265	0.0003611	-0.0022902	1.1415790	-60.0000000	CY
0.00006563	4612.	70280401.	5.8239012	0.0003749	-0.0023288	1.1805597	-60.0000000	CY
0.00006688	4722.	70607683.	5.9177903	0.0003884	-0.0023679	1.2180611	-60.0000000	CY
0.00006813	4830.	70895551.	6.0086507	0.0004018	-0.0024069	1.2552620	-60.0000000	CY
0.00006938	4922.	70951202.	6.0949330	0.0004152	-0.0024460	1.2918358	-60.0000000	CY
0.00007063	4990.	70951202.	6.1617449	0.0004275	-0.0024863	1.3249083	-60.0000000	CY
0.00007188	5049.	70649799.	6.2076860	0.0004384	-0.0025278	1.3541037	-60.0000000	CY
0.00007313	5108.	70244368.	6.2461366	0.0004489	-0.0025698	1.3819003	-60.0000000	CY
0.00007438	5167.	69850172.	6.2834031	0.0004595	-0.0026118	1.4094856	-60.0000000	CY
0.00007563	5402.	69468353.	6.3196400	0.0004700	-0.0026537	1.4368778	-60.0000000	CY
0.00007688	5636.	68053024.	6.4552958	0.0005124	-0.0028214	1.5444979	-60.0000000	CY
0.00007813	5860.	66798676.	6.5736880	0.0005547	-0.0029891	1.6480331	-60.0000000	CY
0.00007938	5974.	63301078.	6.6714268	0.0005963	-0.0031575	1.7461776	-60.0000000	CY
0.00010044	6064.	61022496.	6.6827518	0.0006307	-0.0033331	1.8241767	-60.0000000	CY
0.00010494	6154.	58958848.	6.6781427	0.0006636	-0.0035101	1.8963983	-60.0000000	CY
0.00011094	6243.	57080594.	6.6735790	0.0006967	-0.0036870	1.9666021	-60.0000000	CY
0.00011444	6332.	55363364.	6.6734132	0.0007633	-0.0040405	2.0347660	-60.0000000	CY
0.0001194	6421.	53786870.	6.6743072	0.0007967	-0.0042170	2.1648830	-60.0000000	CY
0.00012444	6509.	52334086.	6.6762253	0.0008304	-0.0043934	2.2267891	-60.0000000	CY
0.0001294	6597.	50990614.	6.6790712	0.0008641	-0.0045696	2.2856509	-60.0000000	CY
0.00013444	6684.	49744198.	6.6827642	0.0008980	-0.0047458	2.3441728	-60.0000000	CY
0.0001394	6771.	48578001.	6.6865197	0.0009319	-0.0049218	2.3994347	-60.0000000	CY
0.00014444	6842.	47391480.	6.6795968	0.0009644	-0.0050994	2.4499017	-60.0000000	CY
0.0001494	6881.	46065564.	6.6480705	0.0009931	-0.0052807	2.4925134	-60.0000000	CY
0.0001544	6904.	44723404.	6.6067816	0.0010199	-0.0054638	2.5307640	-60.0000000	CY
0.0001594	6926.	43456768.	6.5675756	0.0010467	-0.0056470	2.5674032	-60.0000000	CY

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Bending Curvature rad/in.	Bending Moment in-kip	Bending Stiffness kip-in2	Depth to N Axis in	Max Comp Strain in/in	Max Tens Strain in/in	Max Conc Stress ksi	Max Steel Stress ksi	Run Msg
0.0001644	6948.	42266349.	6.5312156	0.0010736	-0.0058302	2.6026545	-60.0000000	CY
0.0001694	6969.	41144676.	6.4962413	0.0011003	-0.0060134	2.6362430	-60.0000000	CY
0.0001744	6990.	40085438.	6.4617980	0.0011268	-0.0061970	2.6680344	-60.0000000	CY
0.0001794	7011.	39084514.	6.4297165	0.0011533	-0.0063804	2.6984645	-60.0000000	CY
0.0001844	7032.	38137151.	6.3998108	0.0011800	-0.0065638	2.7275211	-60.0000000	CY
0.0001894	7052.	37239096.	6.3719150	0.0012067	-0.0067471	2.7551914	-60.0000000	CY
0.0001944	7073.	36386535.	6.3458801	0.0012335	-0.0069303	2.7814626	-60.0000000	CY
0.0001994	7093.	35576034.	6.3215726	0.0012604	-0.0071134	2.8063215	-60.0000000	CY
0.0002044	7113.	34804498.	6.2988720	0.0012873	-0.0072964	2.8297546	-60.0000000	CY
0.0002094	7133.	34069124.	6.2776697	0.0013144	-0.0074794	2.8517481	-60.0000000	CY
0.0002144	7153.	33367374.	6.2578672	0.0013415	-0.0076622	2.8722877	-60.0000000	CY
0.0002194	7173.	32696937.	6.2393753	0.0013688	-0.0078450	2.8913591	-60.0000000	CY
0.0002244	7192.	32055710.	6.2221130	0.0013961	-0.0080277	2.9089473	-60.0000000	CY
0.0002294	7212.	31441774.	6.2060066	0.0014235	-0.0082102	2.9250371	-60.0000000	CY
0.0002344	7231.	30853372.	6.1909888	0.0014510	-0.0083927	2.9396128	-60.0000000	CY
0.0002394	7250.	30288893.	6.1769982	0.0014786	-0.0085751	2.9526585	-60.0000000	CY
0.0002444	7269.	29746859.	6.1639786	0.0015063	-0.0087574	2.9641577	-60.0000000	CY
0.0002494	7288.	29225909.	6.1518787	0.0015341	-0.0089396	2.9740934	-60.0000000	CY
0.0002544	7307.	28724791.	6.1406513	0.0015620	-0.0091217	2.9824484	-60.0000000	CY
0.0002594	7325.	28242345.	6.1302531	0.0015900	-0.0093037	2.9892046	-60.0000000	CY
0.0002644	7344.	27777503.	6.1206444	0.0016181	-0.0094856	2.9943437	-60.0000000	CY
0.0002694	7362.	27329274.	6.1117888	0.0016464	-0.0096674	2.9978468	-60.0000000	CY
0.0002744	7380.	26896738.	6.1036526	0.0016747	-0.0098491	2.9996944	-60.0000000	CY
0.0003044	7483.	24586355.	6.0661339	0.0018464	-0.0109374	2.9956246	-60.0000000	CY
0.0003344	7557.	22601272.	6.0177319	0.0020122	-0.0120316	2.9942823	-60.0000000	CY
0.0003644	7580.	20802963.	5.9264033	0.0021594	-0.0131443	2.9998930	-60.0000000	CY
0.0003944	7588.	19240811.	5.8384099	0.0023025	-0.0142612	2.9934372	-60.0000000	CY
0.0004244	7595.	17897013.	5.7079818	0.0024472	-0.0153766	2.9976015	-60.0000000	CY
0.0004544	7601.	16728339.	5.6571290	0.0025936	-0.0164902	2.9909745	-60.0000000	CY
0.0004844	7606.	15702780.	5.6091713	0.0027402	-0.0176036	2.9998301	-60.0000000	CY
0.0005144	7610.	14794213.	5.5682825	0.0028852	-0.0187185	2.9868200	-60.0000000	CY
0.0005444	7613.	13985112.	5.5332632	0.0030312	-0.0198325	2.9912011	-60.0000000	CYT
0.0005744	7616.	13259958.	5.5038131	0.0031782	-0.0209456	2.9993826	-60.0000000	CYT
0.0006044	7619.	12606001.	5.4784877	0.0033264	-0.0220574	2.9910964	-60.0000000	CYT
0.0006344	7621.	12013447.	5.4784877	0.0034754	-0.0231683	2.9771487	-60.0000000	CYT
0.0006644	7623.	11474123.	5.4564189	0.0036251	-0.0242786	2.9884792	-60.0000000	CYT
0.0006944	7625.	10981118.	5.4372287	0.0037755	-0.0253883	2.9976865	-60.0000000	CYT
0.0007244	7627.	10528613.	5.4207507	0.0039267	-0.0264971	2.9976494	-60.0000000	CYT

Axial Thrust Force = 334,000 kips

Bending Curvature rad/in.	Bending Moment in-kip	Bending Stiffness kip-in2	Depth to N Axis in	Max Comp Strain in/in	Max Tens Strain in/in	Max Conc Stress ksi	Max Steel Stress ksi	Run Msg
6.25000E-07	366.1758073	585881292.	121.9418046	0.00007621	0.00004996	0.2729402	2.1531015	
0.00000125	732.3391879	585871350.	71.5015145	0.00008938	0.00003688	0.3183860	2.4777424	
0.00000188	1098.	585850721.	54.7016883	0.0001026	0.00002382	0.3635378	2.8031231	
0.00000250	1465.	585820926.	46.3119793	0.0001158	0.00001078	0.4083933	3.1292435	
0.00000313	1831.	585782179.	41.2863175	0.0001290	-0.00000223	0.4529504	3.4561038	
0.00000375	2196.	585691966.	37.9422580	0.0001423	-0.00001522	0.4972013	3.7836580	
0.00000438	2561.	585441187.	35.5577491	0.0001556	-0.00002818	0.5411242	4.1117332	
0.00000500	2925.	585003210.	33.7719207	0.0001689	-0.00004114	0.5847003	4.4401785	
0.00000563	3287.	584396396.	32.3846039	0.0001822	-0.00005409	0.6279174	4.7688948	
0.00000625	3648.	583648514.	31.2758867	0.0001955	-0.00006703	0.6707673	5.0978170	
0.00000688	4007.	582785676.	30.3695711	0.0002088	-0.00008096	0.7132446	5.4269020	
0.00000750	4364.	581829600.	29.6149214	0.0002221	-0.00009289	0.7553457	5.7561204	
0.00000813	4719.	580797776.	28.9768534	0.0002354	-0.0001058	0.7970679	6.0854524	
0.00000875	4719.	539312221.	26.2926995	0.0002301	-0.0001374	0.7797007	5.8724600	C
0.00000938	4719.	503358073.	25.5791305	0.0002398	-0.0001539	0.8099689	6.0979197	C
0.00001000	4719.	471898193.	24.9376832	0.0002494	-0.0001706	0.8394972	6.3184281	C
0.00001063	4719.	444139476.	24.3553164	0.0002588	-0.0001875	0.8682862	6.5338881	C
0.00001125	4719.	419465061.	23.8254043	0.0002680	-0.0002045	0.8964581	6.7453507	C
0.00001188	4719.	397387952.	23.3402225	0.0002772	-0.0002216	0.9240417	6.9530078	C
0.00001250	4719.	377518555.	22.8936559	0.0002862	-0.0002388	0.9510675	7.1570752	C
0.00001313	4719.	359541481.	22.4808211	0.0002951	-0.0002562	0.9775681	7.3577937	C

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0.00001375	4719.	343198686.	22.0977908	0.0003038	1.0035778	7.5554315	C
0.00001438	4719.	328277004.	21.7413921	0.0003125	1.0291336	7.7502866	C
0.00001500	4801.	320078753.	21.4091000	0.0003211	1.0542765	7.9427085	C
0.00001563	4893.	313160901.	21.0989408	0.0003297	1.0790537	8.1331138	C
0.00001625	4984.	306681718.	20.8088346	0.0003381	1.1034916	-8.5018991	C
0.00001688	5072.	309562323.	20.5347143	0.0003465	1.1275035	-8.9630430	C
0.00001750	5159.	294824903.	20.2778845	0.0003549	1.1512469	-9.4253486	C
0.00001813	5246.	289407291.	20.0352569	0.0003631	1.1746565	-9.8894993	C
0.00001875	5330.	284282800.	19.8054652	0.0003714	1.1977387	-10.3554658	C
0.00001938	5414.	279448387.	19.5887504	0.0003795	1.2205783	-10.8224145	C
0.00002000	5497.	274849682.	19.3820179	0.0003876	1.2430719	-11.2914296	C
0.00002063	5579.	270501524.	19.1866424	0.0003957	1.2653549	-11.7611457	C
0.00002125	5660.	266360942.	19.0000951	0.0004038	1.2873386	-12.2325038	C
0.00002188	5741.	262423225.	18.8224935	0.0004117	1.3090788	-12.7049494	C
0.00002250	5820.	258686536.	18.6540447	0.0004197	1.3306360	-13.1778608	C
0.00002313	5899.	255099065.	18.4913939	0.0004276	1.3518467	-13.6529902	C
0.00002375	5978.	251696666.	18.3375665	0.0004355	1.3729387	-14.1279385	C
0.00002438	6056.	248436091.	18.1895854	0.0004434	1.3937611	-14.6043305	C
0.00002563	6210.	242335527.	17.9123114	0.0004590	1.4348018	-15.5593198	C
0.00002688	6362.	236723419.	17.6560238	0.0004745	1.4749731	-16.5180553	C
0.00002813	6513.	231575554.	17.4213859	0.0004900	1.5145400	-17.4777133	C
0.00002938	6662.	226786139.	17.2010797	0.0005053	1.5531645	-18.4421739	C
0.00003063	6810.	222373842.	16.9991831	0.0005206	1.5913129	-19.4062566	C
0.00003188	6956.	218242912.	16.8081907	0.0005358	1.6285614	-20.3748974	C
0.00003313	7102.	214414237.	16.6323069	0.0005509	1.6653671	-21.3428714	C
0.00003438	7247.	210820012.	16.4658828	0.0005660	1.7013955	-22.3141669	C
0.00003563	7391.	207458020.	16.3104680	0.0005811	1.7368740	-23.2861539	C
0.00003688	7534.	204314580.	16.1660782	0.0005961	1.7719035	-24.2576187	C
0.00003813	7676.	201334226.	16.0272464	0.0006110	1.8060952	-25.2334068	C
0.00003938	7817.	198535498.	15.8978038	0.0006260	1.8398555	-26.2085389	C
0.00004063	7959.	195901574.	15.7768869	0.0006409	1.8731830	-27.1830112	C
0.00004188	8098.	193396571.	15.6606498	0.0006558	1.9057889	-28.1605672	C
0.00004313	8238.	191023786.	15.5507735	0.0006706	1.9378806	-29.1385950	C
0.00004438	8377.	188778024.	15.4476012	0.0006855	1.9695437	-30.1159618	C
0.00004563	8516.	186648798.	15.3505849	0.0007004	2.0007763	-31.0926635	C
0.00004688	8654.	184611171.	15.2566028	0.0007152	2.0313189	-32.0722743	C
0.00004813	8791.	182669075.	15.1672568	0.0007299	2.0613631	-33.0522283	C
0.00004938	8928.	180819408.	15.0829030	0.0007447	2.0909808	-34.0315119	C
0.00005063	9065.	179055281.	15.0031746	0.0007595	2.1201699	-35.0101204	C
0.00005188	9201.	177370469.	14.9277403	0.0007744	2.1489287	-35.9880493	C
0.00005313	9337.	175748913.	14.8542145	0.0007891	2.1770418	-36.9685071	C
0.00005438	9472.	174193309.	14.7838487	0.0008039	2.2046601	-37.9493121	C
0.00005563	9607.	172702394.	14.7170735	0.0008186	2.2318512	-38.9294270	C
0.00005688	9741.	171271867.	14.6536549	0.0008334	2.2586132	-39.9088465	C
0.00005813	9875.	169897792.	14.5933795	0.0008482	2.2849441	-40.8875657	C
0.00005938	10009.	168576567.	14.5360519	0.0008631	2.3108417	-41.8655792	C
0.00006063	10143.	167303473.	14.4811462	0.0008779	2.3362677	-42.8434909	C
0.00006188	10276.	166069732.	14.4270659	0.0008927	2.3610697	-43.8239021	C
0.00006313	10408.	164880210.	14.3755225	0.0009075	2.3854409	-44.8035901	C
0.00006438	10540.	163732296.	14.3263713	0.0009223	2.4093792	-45.7825492	C
0.00006563	10672.	162623577.	14.2794785	0.0009371	2.4328824	-46.7607735	C
0.00006688	10804.	161551821.	14.2347207	0.0009519	2.4559484	-47.7382570	C
0.00006813	10935.	160514959.	14.1919835	0.0009668	2.4785750	-48.7149937	C
0.00006938	11066.	159511069.	14.1511606	0.0009817	2.5007599	-49.6909774	C
0.00007063	11197.	158538367.	14.1121534	0.0009967	2.5225009	-50.6662018	C
0.00007188	11327.	157595192.	14.0748704	0.0010116	2.5437958	-51.6406605	C
0.00007313	11457.	156675926.	14.0378914	0.0010265	2.5645056	-52.6171777	C
0.00007438	11586.	155782838.	14.0023172	0.0010414	2.5847508	-53.5933456	C
0.00007563	11717.	154527553.	13.8747093	0.0011013	2.6612546	-57.4899970	C
0.00007688	11847.	152484799.	13.6422912	0.0011612	2.7300385	-60.0000000	CY
0.00007813	11978.	149284799.	13.4229152	0.0012193	2.7891746	-60.0000000	CY
0.00007938	12109.	145483949.	13.2127427	0.0012742	2.8383812	-60.0000000	CY
0.00008063	12240.	141016079.	13.5016635	0.0013265	2.8791214	-60.0000000	CY
0.00008188	12371.	136044994.	13.3486459	0.0013791	2.9141135	-60.0000000	CY
0.00008313	12502.	131521155.	13.2127247	0.0014314	2.9430024	-60.0000000	CY
0.00008438	12633.	127367892.	13.0867336	0.0014840	2.9661326	-60.0000000	CY
0.00008563	12764.	123553652.	12.9749612	0.0015370	2.9833747	-60.0000000	CY
0.00008688	12895.	120031666.	12.8755615	0.0015900			

Load No.	Axial Thrust kips	Nominal Thrust in-kips	Nominal Mom. Cap. in-kip	Max. Comp. Strain	Moment Capacity for Section 1			
0.0001244	14476.	116389346.	841294 Lpile.lp110	0.0015879	-0.0036359	2.9942002	-60.0000000	CY
0.0001294	14560.	112538336.	12.7666430	0.0016354	-0.0037983	2.9992803	-60.0000000	CY
0.0001344	14637.	108924247.	12.6409686	0.0016831	-0.0039607	2.9986112	-60.0000000	CY
0.0001394	14712.	105555909.	12.5251532	0.0017310	-0.0041227	2.9979890	-60.0000000	CY
0.0001444	14784.	102397167.	12.4200630	0.0017788	-0.0042850	2.9999540	-60.0000000	CY
0.0001494	14853.	99433392.	12.3203728	0.0018265	-0.0044472	2.9992044	-60.0000000	CY
0.0001544	14921.	96651425.	12.2278647	0.0018746	-0.0046091	2.9971115	-60.0000000	CY
0.0001594	14987.	94034306.	12.1434095	0.0019231	-0.0047707	2.9998456	-60.0000000	CY
0.0001644	15051.	91567083.	11.9957171	0.0019718	-0.0049320	2.9980667	-60.0000000	CY
0.0001694	15114.	89233165.	11.9281432	0.0020203	-0.0050934	2.9999774	-60.0000000	CY
0.0001744	15174.	87022055.	11.8645271	0.0020689	-0.0052549	2.9981354	-60.0000000	CY
0.0001794	15234.	84928908.	11.8058306	0.0021177	-0.0054161	2.9999650	-60.0000000	CY
0.0001844	15293.	82942554.	11.7519644	0.0021668	-0.0055770	2.9974559	-60.0000000	CY
0.0001894	15350.	81056595.	11.7021663	0.0022161	-0.0057377	2.9997799	-60.0000000	CY
0.0001944	15407.	79262391.	11.6563151	0.0022657	-0.0058981	2.9958755	-60.0000000	CY
0.0002004	15462.	77553863.	11.6139563	0.0023155	-0.0060582	2.9990020	-60.0000000	CY
0.0002094	15517.	75925423.	11.5747019	0.0023656	-0.0062182	2.9995010	-60.0000000	CY
0.0002144	15569.	74358183.	11.5362080	0.0024154	-0.0063784	2.9967736	-60.0000000	CY
0.0002194	15615.	72838328.	11.4953894	0.0024643	-0.0065394	2.9993202	-60.0000000	CY
0.0002244	15669.	71328180.	11.4492957	0.0025117	-0.0067021	2.9994133	-60.0000000	CY
0.0002294	15685.	69835467.	11.3993663	0.0025577	-0.0068660	2.9952034	-60.0000000	CY
0.0002344	15698.	68381792.	11.3484484	0.0026031	-0.0070307	2.9980120	-60.0000000	CY
0.0002394	15710.	66977567.	11.2983960	0.0026481	-0.0071957	2.9995903	-60.0000000	CY
0.0002444	15723.	65631048.	11.2509807	0.0026932	-0.0073605	2.9991689	-60.0000000	CY
0.0002494	15735.	64338179.	11.2062347	0.0027385	-0.0075252	2.9943354	-60.0000000	CY
0.0002544	15746.	63096598.	11.1636684	0.0027839	-0.0076898	2.9967096	-60.0000000	CY
0.0002594	15757.	61902388.	11.1232795	0.0028293	-0.0078545	2.9987806	-60.0000000	CY
0.0002644	15766.	60749580.	11.0793831	0.0028737	-0.0080200	2.9998208	-60.0000000	CY
0.0002694	15775.	59636712.	11.0395180	0.0029186	-0.0081852	2.9983211	-60.0000000	CY
0.0002744	15784.	58562191.	11.0024035	0.0029638	-0.0083500	2.9939487	-60.0000000	CY
0.0003044	15823.	57526486.	10.9669313	0.0030091	-0.0085147	2.9952538	-60.0000000	CYT
0.0003344	15848.	51988068.	10.7972735	0.0032864	-0.0094973	2.9940181	-60.0000000	CYT
0.0003644	15867.	47395650.	10.6752045	0.0035695	-0.0104742	2.9916797	-60.0000000	CYT
		43547010.	10.5708885	0.0038518	-0.0114520	2.9999992	-60.0000000	CYT

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Summary of Results for Nominal (Unfactored) Moment Capacity for Section 1  
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Moment values interpolated at maximum compressive strain = 0.003  
or maximum developed moment if pile fails at smaller strains.

Load No.	Axial Thrust kips	Nominal Thrust in-kips	Nominal Mom. Cap. in-kip	Max. Comp. Strain
1	-279.000	7612.424	0.00300000	0.00300000
2	334.000	15782.102	0.00300000	0.00300000

Note that the values of moment capacity in the table above are not factored by a strength reduction factor ( $\phi$ -factor).

In ACI 318, the value of the strength reduction factor depends on whether the transverse reinforcing steel bars are tied hoops ( $\phi$ .65) or spirals ( $\phi$ .70).

The above values should be multiplied by the appropriate strength reduction factor to compute ultimate moment capacity according to ACI 318, Section 9.3.2.2 or the value required by the design standard being followed.

The following table presents factored moment capacities and corresponding bending stiffnesses computed for common resistance factor values used for reinforced concrete sections.

Axial Load No.	Resist. Factor for Moment	Nominal Moment Cap in-kips	Ult. (Fac) Ax. Thrust kips	Ult. (Fac) Moment Cap in-kips	Bend. Stiff. at Ult Mom kip-in^2
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				841294 Lpile.lp110			
1	0.65	7612.	-181.350000	4948.	70835672.		
2	0.65	15782.	217.100000	10258.	166229516.		
1	0.75	7612.	-195.300000	5709.	66397441.		
2	0.75	15782.	233.800000	11837.	154163523.		
1	0.90	7612.	-209.250000	6851.	47083452.		
2	0.90	15782.	250.500000	14204.	122260678.		

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Layering Correction Equivalent Depths of Soil & Rock Layers  
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Layer No.	Top of Layer Below Pile Head ft	Equivalent Top Depth Below Grnd Surf ft	Same Layer Type As Layer Above		Layer is Rock or is Below Rock Layer	F0 Integral for Layer lbs	F1 Integral for Layer lbs
			Layer Above	Layer Below			
1	2.0000	0.00	N.A.	No	No	0.00	69.9721
2	5.5000	0.1867	Yes	No	No	69.9721	33414.
3	9.0000	0.3163	No	No	No	33484.	N.A.

Notes: The F0 integral of Layer n+1 equals the sum of the F0 and F1 integrals for Layer n. Layering correction equivalent depths are computed only for soil types with both shallow-depth and deep-depth expressions for peak lateral load transfer. These soil types are soft and stiff clays, non-liquefied sands, and cemented c-phi soil.

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Computed Values of Pile Loading and Deflection  
for Lateral Loading for Load Case Number 1  
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Pile-head conditions are Shear and Moment (Loading Type 1)

Shear force at pile head = 31000.0 lbs  
Applied moment at pile head = 0.0 in-lbs  
Axial thrust load on pile head = -279000.0 lbs

Depth X feet	Deflect. y inches	Bending Moment in-lbs	Shear Force lbs	Slope S radians	Total Stress psi*	Bending Stiffness in-lb^2	Soil Res. P lb/inch	Soil Spr. Es* <sup>h</sup> lb/inch	Distrib. Lat. Load lb/inch
0.00	0.5041	-2.91E+04	31000.	-0.00477	0.00	5.89E+11	0.00	0.00	0.00
0.2200	0.4915	78326.	31000.	-0.00477	0.00	5.89E+11	0.00	0.00	0.00
0.4400	0.4789	156653.	31000.	-0.00477	0.00	5.89E+11	0.00	0.00	0.00
0.6600	0.4663	234980.	31000.	-0.00477	0.00	5.89E+11	0.00	0.00	0.00
0.8800	0.4537	313307.	31000.	-0.00477	0.00	5.89E+11	0.00	0.00	0.00
1.1000	0.4411	391636.	31000.	-0.00477	0.00	5.89E+11	0.00	0.00	0.00
1.3200	0.4285	469966.	31000.	-0.00476	0.00	5.89E+11	0.00	0.00	0.00
1.5400	0.4160	548298.	31000.	-0.00476	0.00	5.89E+11	0.00	0.00	0.00
1.7600	0.4034	626631.	31000.	-0.00476	0.00	5.89E+11	0.00	0.00	0.00
1.9800	0.3908	704967.	31000.	-0.00476	0.00	5.89E+11	0.00	0.00	0.00
2.2000	0.3783	783305.	31000.	-0.00475	0.00	4.01E+11	-0.00335	0.02340	0.00
2.4200	0.3658	861646.	31000.	-0.00475	0.00	4.17E+11	-0.00681	0.04913	0.00
2.6400	0.3532	939992.	31000.	-0.00474	0.00	5.56E+11	-0.01002	0.07487	0.00
2.8600	0.3407	1018341.	31000.	-0.00474	0.00	5.66E+11	-0.01298	0.1006	0.00
3.0800	0.3282	1096693.	31000.	-0.00473	0.00	5.28E+11	-0.01571	0.1263	0.00
3.3000	0.3157	1175049.	31000.	-0.00473	0.00	5.79E+11	-0.01819	0.1521	0.00
3.5200	0.3033	1253409.	31000.	-0.00472	0.00	5.87E+11	-0.02042	0.1778	0.00
3.7400	0.2908	1331773.	31000.	-0.00472	0.00	5.83E+11	-0.02242	0.2035	0.00

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3.9600	0.2784	1410141.	31000.	-0.00471	0.00	5.78E+11	-0.02417	0.2293	0.00
4.1800	0.2659	1488514.	31000.	-0.00470	0.00	3.41E+11	-0.02569	0.2550	0.00
4.4000	0.2535	1566895.	31000.	-0.00466	0.00	5.40E+10	-0.02696	0.2807	0.00
4.6200	0.2413	1645332.	30999.	-0.00458	0.00	5.29E+10	-0.02802	0.3065	0.00
4.8400	0.2294	1723830.	30999.	-0.00449	0.00	5.29E+10	-0.02886	0.3322	0.00
5.0600	0.2176	1802391.	30999.	-0.00440	0.00	5.29E+10	-0.02951	0.3579	0.00
5.2800	0.2061	1881018.	30999.	-0.00431	0.00	5.29E+10	-0.02996	0.3837	0.00
5.5000	0.1949	1959713.	30982.	-0.00422	0.00	5.29E+10	-12.9142	174.9673	0.00
5.7200	0.1839	2038391.	30868.	-0.00412	0.00	5.29E+10	-73.9033	1061.	0.00
5.9400	0.1731	2116629.	30619.	-0.00401	0.00	5.31E+10	-114.7534	1750.	0.00
6.1600	0.1627	2194144.	30262.	-0.00391	0.00	5.35E+10	-155.6647	2526.	0.00
6.3800	0.1525	2270654.	29797.	-0.00380	0.00	5.40E+10	-196.2152	3397.	0.00
6.6000	0.1426	2345879.	29227.	-0.00369	0.00	5.46E+10	-235.9895	4369.	0.00
6.8200	0.1330	2419542.	28553.	-0.00357	0.00	5.51E+10	-274.5798	5449.	0.00
7.0400	0.1238	2491377.	27779.	-0.00345	0.00	5.57E+10	-311.5871	6647.	0.00
7.2600	0.1148	2561127.	26908.	-0.00333	0.00	5.63E+10	-348.5411	8016.	0.00
7.4800	0.1062	2628536.	25941.	-0.00321	0.00	5.69E+10	-384.0266	9551.	0.00
7.7000	0.09783	2693359.	24882.	-0.00309	0.00	5.74E+10	-417.5432	11268.	0.00
7.9200	0.08983	2755363.	23739.	-0.00297	0.00	5.80E+10	-448.7942	13189.	0.00
8.1400	0.08217	2814331.	22516.	-0.00284	0.00	5.84E+10	-477.4945	15342.	0.00
8.3600	0.07484	2870065.	21221.	-0.00271	0.00	5.89E+10	-503.3711	17757.	0.00
8.5800	0.06785	2922386.	19862.	-0.00258	0.00	5.93E+10	-526.1641	20473.	0.00
8.8000	0.06120	2971135.	18448.	-0.00245	0.00	5.97E+10	-545.6285	23536.	0.00
9.0200	0.05490	3016178.	14746.	-0.00232	0.00	6.01E+10	-2259.	108611.	0.00
9.2400	0.04895	3045576.	8863.	-0.00219	0.00	6.03E+10	-2198.	118552.	0.00
9.4600	0.04336	3059750.	3143.	-0.00205	0.00	6.04E+10	-2134.	129971.	0.00
9.6800	0.03811	3059147.	-2403.	-0.00192	0.00	6.04E+10	-2067.	143190.	0.00
9.9000	0.03322	3044235.	-7766.	-0.00179	0.00	6.03E+10	-1996.	158627.	0.00
10.1200	0.02868	3015510.	-12937.	-0.00165	0.00	6.01E+10	-1921.	176836.	0.00
10.3400	0.02449	2973494.	-17903.	-0.00152	0.00	5.98E+10	-1842.	198566.	0.00
10.5600	0.02064	2918738.	-22656.	-0.00139	0.00	5.93E+10	-1758.	224851.	0.00
10.7800	0.01714	2851823.	-27181.	-0.00126	0.00	5.88E+10	-1670.	257164.	0.00
11.0000	0.01398	2773365.	-31465.	-0.00114	0.00	5.74E+10	-1576.	297663.	0.00
11.2200	0.01115	2684014.	-35495.	-0.00101	0.00	5.65E+10	-1477.	349655.	0.00
11.4400	0.00865	2584461.	-39253.	-8.88E-04	0.00	5.56E+10	-1370.	418469.	0.00
11.6600	0.00646	2475447.	-42720.	-7.69E-04	0.00	5.47E+10	-1256.	513290.	0.00
11.8800	0.00458	2357765.	-45872.	-6.53E-04	0.00	5.37E+10	-1131.	651501.	0.00
12.1000	0.00301	2232282.	-48676.	-5.42E-04	0.00	5.30E+10	-993.0158	870831.	0.00
12.3200	0.00173	2099958.	-51086.	-4.34E-04	0.00	5.29E+10	-832.7154	1274107.	0.00
12.5400	7.16E-04	1961908.	-53012.	-3.33E-04	0.00	5.29E+10	-626.6880	2309587.	0.00
12.7600	-3.43E-05	1819562.	-53526.	-2.39E-04	0.00	5.29E+10	237.6274	1.83E+07	0.00
12.9800	-5.45E-04	1678938.	-52440.	-1.52E-04	0.00	5.29E+10	585.2710	2833972.	0.00
13.2000	-8.35E-04	1542456.	-50769.	-7.13E-05	0.00	5.29E+10	680.5866	2151877.	0.00
13.4200	-9.22E-04	1410774.	-48933.	-2.96E-05	0.00	5.76E+11	709.9679	2033913.	0.00
13.6400	-9.91E-04	1284045.	-47027.	-2.29E-05	0.00	5.03E+11	734.1702	1955771.	0.00
13.8600	-0.00104	1162438.	-45065.	-1.64E-05	0.00	4.83E+11	752.3787	1904933.	0.00
14.0800	-0.00108	1046079.	-43061.	-1.08E-05	0.00	5.61E+11	765.5391	1875468.	0.00
14.3000	-0.00110	935059.	-41023.	-5.40E-06	0.00	4.26E+11	778.1371	1868358.	0.00
14.5200	-0.00111	829467.	-38959.	-1.91E-07	0.00	4.75E+11	785.5986	1875032.	0.00
14.7400	-0.00110	729353.	-36878.	4.34E-06	0.00	4.33E+11	791.2857	1898190.	0.00
14.9600	-0.00108	634758.	-34785.	7.99E-06	0.00	5.89E+11	794.1242	1935488.	0.00
15.1800	-0.00106	545700.	-32688.	1.06E-05	0.00	5.89E+11	794.9582	1982997.	0.00
15.4000	-0.00103	462183.	-30590.	1.29E-05	0.00	5.89E+11	793.9196	2040746.	0.00
15.6200	-9.90E-04	384202.	-28498.	1.48E-05	0.00	5.89E+11	791.1190	2109028.	0.00
15.8400	-9.49E-04	3111736.	-26415.	1.63E-05	0.00	5.89E+11	786.6502	2188382.	0.00
16.0600	-8.04E-04	244753.	-24347.	1.76E-05	0.00	5.89E+11	780.5929	2279593.	0.00
16.2800	-8.56E-04	183212.	-22296.	1.85E-05	0.00	5.89E+11	773.0140	2383711.	0.00
16.5000	-8.06E-04	127059.	-20267.	1.92E-05	0.00	5.89E+11	763.9698	2502084.	0.00
16.7200	-7.55E-04	76231.	-18264.	1.97E-05	0.00	5.89E+11	753.5057	2636410.	0.00
16.9400	-7.02E-04	30655.	-16290.	1.99E-05	0.00	5.89E+11	741.6567	2788809.	0.00
17.1600	-6.49E-04	-9753.	-14350.	2.00E-05	0.00	5.89E+11	728.4461	2961937.	0.00
17.3800	-5.97E-04	-45083.	-12446.	1.99E-05	0.00	5.89E+11	713.8849	3159135.	0.00
17.6000	-5.44E-04	-75438.	-10582.	1.96E-05	0.00	5.89E+11	697.9685	3384650.	0.00
17.8200	-4.93E-04	-100928.	-8762.	1.92E-05	0.00	5.89E+11	680.6736	3643965.	0.00
18.0400	-4.43E-04	-121675.	-6990.	1.87E-05	0.00	5.89E+11	661.9523	3944286.	0.00
18.2600	-3.94E-04	-137809.	-5269.	1.81E-05	0.00	5.89E+11	641.7245	4295302.	0.00
18.4800	-3.47E-04	-149470.	-3604.	1.75E-05	0.00	5.89E+11	619.8648	4710416.	0.00

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18.7000	-3.02E-04	-156812.	-1999.	1.68E-05	0.00	5.89E+11	596.1841	5208808.	0.00
18.9200	-2.59E-04	-159999.	-458.9317	1.61E-05	0.00	5.89E+11	570.3989	5819111.	0.00
19.1400	-2.17E-04	-159211.	1010.	1.54E-05	0.00	5.89E+11	542.0795	6586393.	0.00
19.3600	-1.78E-04	-154646.	2399.	1.47E-05	0.00	5.89E+11	510.5572	7586596.	0.00
19.5800	-1.40E-04	-146523.	3700.	1.40E-05	0.00	5.89E+11	474.7393	8960015.	0.00
19.8000	-1.04E-04	-135092.	4897.	1.34E-05	0.00	5.89E+11	432.6921	1.10E+07	0.00
20.0200	-6.94E-05	-120645.	5971.	1.28E-05	0.00	5.89E+11	380.4935	1.45E+07	0.00
20.2400	-3.63E-05	-103547.	6879.	1.23E-05	0.00	5.89E+11	307.7567	2.24E+07	0.00
20.4600	-4.53E-06	-84304.	7479.	1.19E-05	0.00	5.89E+11	146.7052	8.55E+07	0.00
20.6800	2.63E-05	-64039.	7296.	1.15E-05	0.00	5.89E+11	-285.6585	2.87E+07	0.00
20.9000	5.63E-05	-45765.	6432.	1.13E-05	0.00	5.89E+11	-368.6790	1.73E+07	0.00
21.1200	8.58E-05	-30061.	5382.	1.11E-05	0.00	5.89E+11	-426.4626	1.31E+07	0.00
21.3400	1.15E-04	-17330.	4195.	1.10E-05	0.00	5.89E+11	-472.9858	1.09E+07	0.00
21.5600	1.44E-04	-7894.	2894.	1.09E-05	0.00	5.89E+11	-513.0136	9408634.	0.00
21.7800	1.73E-04	-2035.	1492.	1.09E-05	0.00	5.89E+11	-548.7971	8384128.	0.00
22.0000	2.02E-04	0.00	0.00	1.09E-05	0.00	5.89E+11	-581.5772	3807210.	0.00

\* This analysis computed pile response using nonlinear moment-curvature relationships. Values of total stress due to combined axial and bending stresses are computed only for elastic sections only and do not equal the actual stresses in concrete and steel. Stresses in concrete and steel may be interpolated from the output for nonlinear bending properties relative to the magnitude of bending moment developed in the pile.

Output Summary for Load Case No. 1:

Pile-head deflection = 0.50407996 inches  
 Computed slope at pile head = -0.00477056 radians  
 Maximum bending moment = 3059750. inch-lbs  
 Maximum shear force = -53526. lbs  
 Depth of maximum bending moment = 9.46000000 feet below pile head  
 Depth of maximum shear force = 12.76000000 feet below pile head  
 Number of iterations = 37  
 Number of zero deflection points = 2

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 Computed Values of Pile Loading and Deflection  
 for Lateral Loading for Load Case Number 2  
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Pile-head conditions are Shear and Moment (Loading Type 1)

Shear force at pile head = 36000.0 lbs  
 Applied moment at pile head = 0.0 in-lbs  
 Axial thrust load on pile head = 334000.0 lbs

Depth X feet	Deflect. y inches	Bending Moment in-lbs	Shear Force lbs	Slope S radians	Total Stress psi*	Bending Stiffness in-lb^2	Soil Res. p lb/inch	Soil Spr. Es*H lb/inch	Distrib. Lat. Load lb/inch
0.00	0.1004	1.17E+05	36000.	-8.67E-04	0.00	5.86E+11	0.00	0.00	0.00
0.2200	0.09810	95805.	36000.	-8.67E-04	0.00	5.86E+11	0.00	0.00	0.00
0.4400	0.09581	191609.	36000.	-8.66E-04	0.00	5.86E+11	0.00	0.00	0.00
0.6600	0.09352	287413.	36000.	-8.65E-04	0.00	5.86E+11	0.00	0.00	0.00
0.8800	0.09124	383215.	36000.	-8.64E-04	0.00	5.86E+11	0.00	0.00	0.00
1.1000	0.08896	479016.	36000.	-8.62E-04	0.00	5.86E+11	0.00	0.00	0.00
1.3200	0.08669	574815.	36000.	-8.60E-04	0.00	5.86E+11	0.00	0.00	0.00
1.5400	0.08442	670612.	36000.	-8.57E-04	0.00	5.86E+11	0.00	0.00	0.00
1.7600	0.08217	766406.	36000.	-8.54E-04	0.00	5.86E+11	0.00	0.00	0.00
1.9800	0.07992	862197.	36000.	-8.50E-04	0.00	5.86E+11	0.00	0.00	0.00
2.2000	0.07768	957985.	36000.	-8.46E-04	0.00	5.86E+11	-6.88E-04	0.02340	0.00
2.4200	0.07545	1053769.	36000.	-8.41E-04	0.00	5.86E+11	-0.00140	0.04913	0.00
2.6400	0.07324	1149549.	36000.	-8.36E-04	0.00	5.86E+11	-0.00208	0.07487	0.00
2.8600	0.07104	1245324.	36000.	-8.31E-04	0.00	5.86E+11	-0.00271	0.1006	0.00

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3.0800	0.06885	1341094.	36000.	-8.25E-04	0.00	5.86E+11	-0.00329	0.1263	0.00
3.3000	0.06668	1436858.	36000.	-8.19E-04	0.00	5.86E+11	-0.00384	0.1521	0.00
3.5200	0.06453	1532618.	36000.	-8.12E-04	0.00	5.86E+11	-0.00435	0.1778	0.00
3.7400	0.06239	1628370.	36000.	-8.05E-04	0.00	5.86E+11	-0.00481	0.2035	0.00
3.9600	0.06028	1724117.	36000.	-7.97E-04	0.00	5.86E+11	-0.00523	0.2293	0.00
4.1800	0.05818	1819856.	36000.	-7.89E-04	0.00	5.86E+11	-0.00567	0.2550	0.00
4.4000	0.05611	1915589.	36000.	-7.81E-04	0.00	5.86E+11	-0.00597	0.2807	0.00
4.6200	0.05406	2011313.	36000.	-7.72E-04	0.00	5.86E+11	-0.00628	0.3065	0.00
4.8400	0.05203	2107030.	36000.	-7.63E-04	0.00	5.86E+11	-0.00655	0.3322	0.00
5.0600	0.05003	2202738.	36000.	-7.53E-04	0.00	5.86E+11	-0.00678	0.3579	0.00
5.2800	0.04806	2298437.	36000.	-7.43E-04	0.00	5.86E+11	-0.00698	0.3837	0.00
5.5000	0.04611	2394127.	35988.	-7.32E-04	0.00	5.86E+11	-8.8791	508.3975	0.00
5.7200	0.04419	2489746.	35909.	-7.21E-04	0.00	5.85E+11	-50.8044	3035.	0.00
5.9400	0.04230	2585001.	35738.	-7.10E-04	0.00	5.85E+11	-78.8875	4924.	0.00
6.1600	0.04044	2679695.	35493.	-6.98E-04	0.00	5.85E+11	-107.0324	6987.	0.00
6.3800	0.03861	2773633.	35173.	-6.86E-04	0.00	5.85E+11	-134.9673	9228.	0.00
6.6000	0.03682	2866620.	34781.	-6.73E-04	0.00	5.85E+11	-162.4277	11647.	0.00
6.8200	0.03506	2958463.	34317.	-6.60E-04	0.00	5.85E+11	-189.1571	14244.	0.00
7.0400	0.03333	3048975.	33783.	-6.46E-04	0.00	5.85E+11	-214.9080	17021.	0.00
7.2600	0.03165	3137978.	33181.	-6.32E-04	0.00	5.85E+11	-241.5021	20147.	0.00
7.4800	0.02999	3225285.	32545.	-6.18E-04	0.00	5.84E+11	-240.4258	21161.	0.00
7.7000	0.02838	3310904.	31915.	-6.03E-04	0.00	5.84E+11	-236.6344	22011.	0.00
7.9200	0.02681	3394860.	31296.	-5.88E-04	0.00	5.84E+11	-232.1471	22861.	0.00
8.1400	0.02528	3477185.	30690.	-5.73E-04	0.00	5.84E+11	-227.0112	23710.	0.00
8.3600	0.02379	3557913.	30098.	-5.57E-04	0.00	5.84E+11	-221.2752	24560.	0.00
8.5800	0.02234	3637085.	29522.	-5.40E-04	0.00	5.84E+11	-214.9884	25409.	0.00
8.8000	0.02093	3714745.	28964.	-5.24E-04	0.00	5.83E+11	-208.2013	26259.	0.00
9.0200	0.01957	3790938.	26573.	-5.07E-04	0.00	5.83E+11	-1603.	216228.	0.00
9.2400	0.01826	3855944.	22366.	-4.90E-04	0.00	5.83E+11	-1584.	229032.	0.00
9.4600	0.01699	3909896.	18212.	-4.72E-04	0.00	5.83E+11	-1563.	242970.	0.00
9.6800	0.01576	3952937.	14114.	-4.54E-04	0.00	5.83E+11	-1542.	258182.	0.00
9.9000	0.01459	3985217.	10074.	-4.36E-04	0.00	5.83E+11	-1494.	274827.	0.00
10.1200	0.01346	4006896.	6096.	-4.18E-04	0.00	5.83E+11	-1471.	293093.	0.00
10.3400	0.01238	4018143.	2185.	-4.00E-04	0.00	5.83E+11	-1469.	313202.	0.00
10.5600	0.01135	4019135.	-1658.	-3.82E-04	0.00	5.83E+11	-1442.	335413.	0.00
10.7800	0.01037	4010062.	-5428.	-3.63E-04	0.00	5.83E+11	-1414.	360040.	0.00
11.0000	0.00943	3991119.	-9121.	-3.45E-04	0.00	5.83E+11	-1384.	387455.	0.00
11.2200	0.00854	3962513.	-12734.	-3.27E-04	0.00	5.83E+11	-1353.	418111.	0.00
11.4400	0.00770	3924462.	-16263.	-3.09E-04	0.00	5.83E+11	-1320.	452565.	0.00
11.6600	0.00691	3877192.	-19704.	-2.92E-04	0.00	5.83E+11	-1286.	491505.	0.00
11.8800	0.00616	3820942.	-23052.	-2.74E-04	0.00	5.83E+11	-1251.	535791.	0.00
12.1000	0.00546	3755961.	-26304.	-2.57E-04	0.00	5.83E+11	-1213.	586524.	0.00
12.3200	0.00480	3682510.	-29455.	-2.40E-04	0.00	5.84E+11	-1174.	645126.	0.00
12.5400	0.00419	3600864.	-32499.	-2.24E-04	0.00	5.84E+11	-1133.	713475.	0.00
12.7600	0.00362	3511309.	-35432.	-2.08E-04	0.00	5.84E+11	-1089.	794111.	0.00
12.9800	0.00309	3414150.	-38247.	-1.92E-04	0.00	5.84E+11	-1043.	890563.	0.00
13.2000	0.00261	3309705.	-40938.	-1.77E-04	0.00	5.84E+11	-994.9501	1007907.	0.00
13.4200	0.00216	3198312.	-43496.	-1.62E-04	0.00	5.85E+11	-943.3000	1153749.	0.00
13.6400	0.00175	3080332.	-45913.	-1.48E-04	0.00	5.85E+11	-887.8125	1340112.	0.00
13.8600	0.00138	2956152.	-48177.	-1.35E-04	0.00	5.85E+11	-827.4850	1587366.	0.00
14.0800	0.00104	2826193.	-50274.	-1.21E-04	0.00	5.85E+11	-760.7002	1933462.	0.00
14.3000	7.35E-04	2690921.	-52181.	-1.09E-04	0.00	5.85E+11	-684.5965	2459605.	0.00
14.5200	4.63E-04	2550867.	-53868.	-9.72E-05	0.00	5.85E+11	-593.3005	3383126.	0.00
14.7400	2.22E-04	2406668.	-55273.	-8.60E-05	0.00	5.86E+11	-470.8439	5611442.	0.00
14.9600	8.70E-06	2259178.	-56195.	-7.55E-05	0.00	5.86E+11	-227.8352	6.91E+07	0.00
15.1800	-1.77E-04	2110090.	-55923.	-6.57E-05	0.00	5.86E+11	434.1245	6466743.	0.00
15.4000	-3.38E-04	1964020.	-54629.	-5.65E-05	0.00	5.86E+11	546.5051	4267944.	0.00
15.6200	-4.76E-04	1821751.	-53090.	-4.80E-05	0.00	5.86E+11	618.8503	3435886.	0.00
15.8400	-5.91E-04	1683788.	-51387.	-4.01E-05	0.00	5.86E+11	671.8159	2999600.	0.00
16.0600	-6.87E-04	1550501.	-49559.	-3.28E-05	0.00	5.86E+11	712.6673	2738554.	0.00
16.2800	-7.64E-04	1422174.	-47635.	-2.61E-05	0.00	5.86E+11	744.9297	2573037.	0.00
16.5000	-8.25E-04	1299034.	-45634.	-1.99E-05	0.00	5.86E+11	770.6167	2466891.	0.00
16.7200	-8.70E-04	1181259.	-43573.	-1.44E-05	0.00	5.86E+11	790.9994	2401336.	0.00
16.9400	-9.00E-04	1068993.	-41464.	-9.28E-06	0.00	5.86E+11	806.9368	2365742.	0.00
17.1600	-9.19E-04	962347.	-39318.	-4.71E-06	0.00	5.86E+11	819.0377	2353772.	0.00
17.3800	-9.25E-04	861404.	-37144.	-5.98E-07	0.00	5.86E+11	827.7482	2361572.	0.00
17.6000	-9.22E-04	766228.	-34951.	3.07E-06	0.00	5.86E+11	833.4028	2386846.	0.00



841294 Lpile.lp110

17.8200	-9.09E+04	676857.	-32747.	6.32E-06	0.00	5.86E+11	836.2551	2428365.	0.00
18.0400	-8.88E-04	593312.	-30539.	9.18E-06	0.00	5.86E+11	836.4978	2485698.	0.00
18.2600	-8.61E-04	515594.	-28334.	1.17E-05	0.00	5.86E+11	834.2741	2559079.	0.00
18.4800	-8.27E-04	443689.	-26137.	1.38E-05	0.00	5.86E+11	829.6860	2649370.	0.00
18.7000	-7.88E-04	377565.	-23956.	1.57E-05	0.00	5.86E+11	822.7978	2758084.	0.00
18.9200	-7.44E-04	317174.	-21796.	1.73E-05	0.00	5.86E+11	813.6372	2887496.	0.00
19.1400	-6.96E-04	262452.	-19663.	1.86E-05	0.00	5.86E+11	802.1942	3040830.	0.00
19.3600	-6.46E-04	213320.	-17563.	1.96E-05	0.00	5.86E+11	788.4158	3222586.	0.00
19.5800	-5.93E-04	169683.	-15503.	2.05E-05	0.00	5.86E+11	772.1984	3439063.	0.00
19.8000	-5.38E-04	131426.	-13490.	2.12E-05	0.00	5.86E+11	753.3736	3699225.	0.00
20.0200	-4.81E-04	98420.	-11529.	2.17E-05	0.00	5.86E+11	731.6861	4016179.	0.00
20.2400	-4.23E-04	70513.	-9631.	2.21E-05	0.00	5.86E+11	706.7580	4409833.	0.00
20.4600	-3.64E-04	47531.	-7803.	2.23E-05	0.00	5.86E+11	678.0279	4912003.	0.00
20.6800	-3.05E-04	29275.	-6057.	2.25E-05	0.00	5.86E+11	644.6410	5577107.	0.00
20.9000	-2.46E-04	15512.	-4407.	2.26E-05	0.00	5.86E+11	605.2351	6507402.	0.00
21.1200	-1.86E-04	5967.	-2872.	2.27E-05	0.00	5.86E+11	557.4684	7923374.	0.00
21.3400	-1.26E-04	306.7584	-1481.	2.27E-05	0.00	5.86E+11	496.7833	1.04E+07	0.00
21.5600	-6.60E-05	-1891.	-280.8051	2.27E-05	0.00	5.86E+11	412.1272	1.65E+07	0.00
21.7800	-6.16E-06	-1216.	365.6662	2.27E-05	0.00	5.86E+11	77.6238	3.33E+07	0.00
22.0000	5.37E-05	0.00	0.00	2.27E-05	0.00	5.86E+11	-354.6437	8721911.	0.00

\* This analysis computed pile response using nonlinear moment-curvature relationships. Values of total stress due to combined axial and bending stresses are computed only for elastic sections only and do not equal the actual stresses in concrete and steel. Stresses in concrete and steel may be interpolated from the output for nonlinear bending properties relative to the magnitude of bending moment developed in the pile.

Output Summary for Load Case No. 2:

Pile-head deflection = 0.10038885 inches  
 Computed slope at pile head = -0.00086734 radians  
 Maximum bending moment = 4019135. inch-lbs  
 Maximum shear force = -56195. lbs  
 Depth of maximum bending moment = 10.56000000 feet below pile head  
 Depth of maximum shear force = 14.96000000 feet below pile head  
 Number of iterations = 13  
 Number of zero deflection points = 2

-----  
 Summary of Pile-head Responses for Conventional Analyses  
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Definitions of Pile-head Loading Conditions:

Load Type 1: Load 1 = Shear, V, lbs, and Load 2 = Moment, M, in-lbs  
 Load Type 2: Load 1 = Shear, V, lbs, and Load 2 = Slope, S, radians  
 Load Type 3: Load 1 = Shear, V, lbs, and Load 2 = Rot. Stiffness, R, in-lbs/rad.  
 Load Type 4: Load 1 = Top Deflection, Y, inches, and Load 2 = Moment, M, in-lbs  
 Load Type 5: Load 1 = Top Deflection, Y, inches, and Load 2 = Slope, S, radians

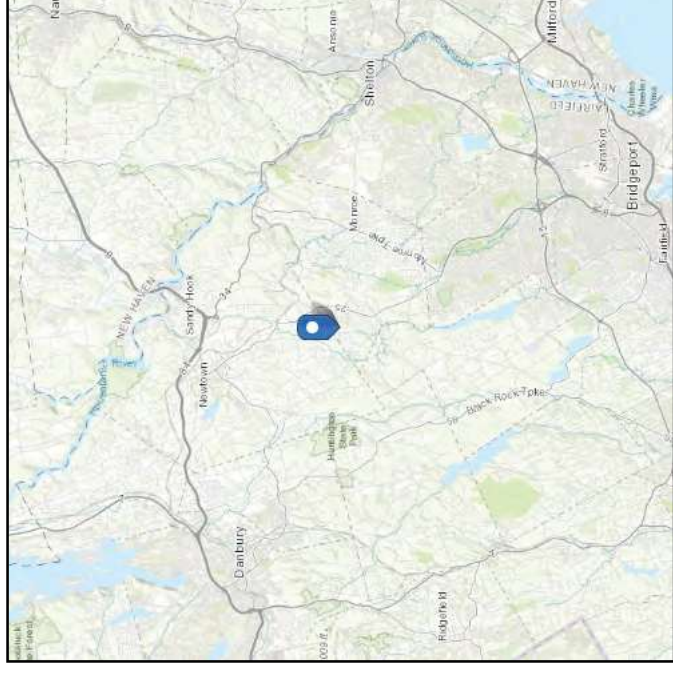
Load Case No.	Load Type	Pile-head Load 1	Load Type 2	Pile-head Load 2	Axial Load lbs	Pile-head Deflection inches	Pile-head Rotation radians	Max Shear in Pile lbs	Max Moment in Pile in-lbs
1	V, lb	31000.	M, in-lb	0.00	-279000.	0.5041	-0.00477	-53526.	3059750.
2	V, lb	36000.	M, in-lb	0.00	334000.	0.1004	-8.67E-04	-56195.	4019135.

Maximum pile-head deflection = 0.5040799570 inches  
 Maximum pile-head rotation = -0.0047705635 radians = -0.273333 deg.

The analysis ended normally.

# ASCE 7 Hazards Report

**Address:** No Address at This Location  
**Standard:** ASCE/SEI 7-10  
**Elevation:** 610.45 ft (NAVD 88)  
**Risk Category:** II  
**Latitude:** 41.341856  
**Soil Class:** D - Stiff Soil  
**Longitude:** -73.275189



## Wind

### Results:

**76 Vmph**  
 Wind Speed: 120 Vmph  
 10-year MRI: 76 Vmph  
 25-year MRI: 86 Vmph  
 50-year MRI: 91 Vmph  
 100-year MRI: 98 Vmph

### Data Source:

ASCE/SEI 7-10, Fig. 26.5-1A and Figs. CC-1-CC-4, incorporating errata of March 12, 2014

### Date Accessed:

Tue Dec 04 2018

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.

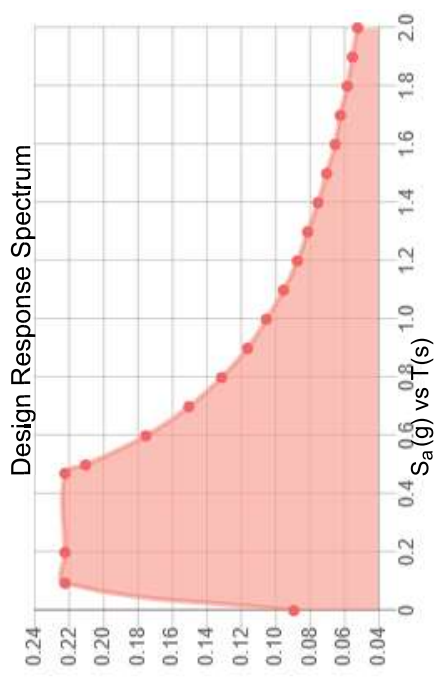
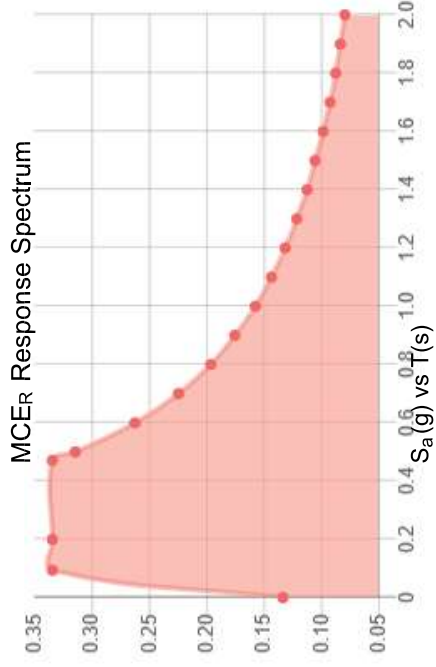
Mountainous terrain, gorges, ocean promontories, and special wind regions should be examined for unusual wind conditions.

**Site Soil Class:** D - Stiff Soil

**Results:**

$S_S$ :	0.209	$S_{DS}$ :	0.222
$S_1$ :	0.065	$S_{D1}$ :	0.105
$F_a$ :	1.600	$T_L$ :	6.000
$F_v$ :	2.400	$PGA$ :	0.113
$S_{MS}$ :	0.334	$PGA_M$ :	0.177
$S_{M1}$ :	0.157	$F_{PGA}$ :	1.575
		$I_e$ :	1

**Seismic Design Category** B



**Data Accessed:**

**Date Source:**

Tue Dec 04 2018

USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-10 Ch. 21 are available from USGS.

## Ice

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### Results:

Ice Thickness: 0.75 in.

Concurrent Temperature: 15 F

Gust Speed: 50 mph

**Data Source:** Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

**Date Accessed:** Tue Dec 04 2018

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

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The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided “as is” and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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# Exhibit E

## Mount Analysis

Date: **May 28, 2020**

Darcy Tarr  
Crown Castle  
3530 Toringdon Way, Suite 300  
Charlotte, NC 28277  
(704)-405-6619



POD Group  
1033 E Turkeyfoot Lake Rd. Suite 206  
Akron, OH 44312  
(330) 961-7432  
[mhoudeshellpodgrp.com](mailto:mhoudeshellpodgrp.com)

**Subject:**

**Mount Analysis Report**

**Carrier Designation:**

**AT&T**  
**Carrier Site Number:** 60427  
**Carrier Site Name:** CTL02144  
**PACE Number:** MRCTB047199  
**FA Number:** 10035068

**Crown Castle Designation:**

**Crown Castle BU Number:** 841294  
**Crown Castle Site Name:** MONROE-GUINEA ROAD  
**Crown Castle JDE Job Number:** 605386  
**Crown Castle Order Number:** 517061 Rev. 0

**Engineering Firm Designation:**

**POD Report Designation:** 20-64617

**Site Data:**

**230 Guinea Road, Monroe, Fairfield County, CT 06468**  
**Latitude 41° 20' 30.68" Longitude -73° 16' 28.28"**

**Structure Information:**

**Tower Height & Type:** 240 ft Self-Support  
**Mount Elevation:** 230 ft  
**Mount Type:** 12.5 ft Frame Mount

Dear Darcy Tarr,

POD Group is pleased to submit this "Mount Analysis Report" to determine the structural integrity of AT&T's antenna mounting system with the proposed appurtenance and equipment addition on the abovementioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

**12.5 Frame Mount (Typical)**

**Sufficient\***  
**\*See Section 4.1 of this report for the loading and structural modifications required in order for the mount to support the loading listed in Table 1.**

This analysis has been performed in accordance with the TIA-222-H Standard based upon an ultimate 3-second gust wind speed of 117 mph. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount structural analysis prepared by: Dario Pelemis

Respectfully submitted by:

A handwritten signature in black ink, appearing to read 'Jason Cherfonis', is written over the 'Respectfully submitted by:' text.

Jason Cherfonis, P.E.  
Connecticut PE #: 0032793



5/28/2020

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**1) INTRODUCTION**

This mount is a existing 12.5 ft Frame Mount. This mount is installed at the 230 ft elevation on 3 sectors of the 240 ft Self-Support tower.

**2) ANALYSIS CRITERIA**

**TIA-222 Revision:** TIA-222-H  
**Risk Category:** II  
**Ultimate Wind Speed:** 117 mph  
**Exposure Category:** B  
**Topographic Factor at Base:** 1  
**Topographic Factor at Mount:** 1  
**Ice Thickness:** 1 in  
**Wind Speed with Ice:** 50 mph  
**Seismic S<sub>s</sub>:** 0.212  
**Seismic S<sub>1</sub>:** 0.055  
**Live Loading Wind Speed:** 30 mph  
**Man Live Load at Mid/End-Points:** 250 lb  
**Man Live Load at Mount Pipes:** 500 lb

**Table 1 - Final Equipment Configuration**

Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount / Modification Details	Note	
230	232	3	CCI Antennas	HPA-65R-BUU-H6	12.5 ft Frame Mount		
		3	CCI Antennas	DMP65R-BU8D			
		3	CCI Antennas	OPA65R-BU6D			
3	Powerwave Technologies	7770.00					
230	230	3	Ericsson	RRUS 4449 B5/B12			
		3	Ericsson	RRUS 32 B2			
		3	Ericsson	RRUS 4478 B14			
201	207	6	Powerwave Technologies	LGP13519			
		2	Raycap	DC6-48-60-18-8F			2
201	207	2	Kathrein	OG-4		-	1

Notes:

- 1) Not on mount being analyzed, therefore not considered in analysis
- 2) Tower Mounted and not considered in analysis



**3) ANALYSIS PROCEDURE**

**Table 2 - Documents Provided**

Document	Remarks	Reference	Source
Application	-	Crown Application #: 517061 Rev. 0 Dated: 05/28/2020	Crown
Elevation Drawings	-	Crown Drawing #: A1-230 Dated: 12/03/2019	Crown
Structural Analysis	-	GPD Job #: 2020777.841294.14 Dated: 11/26/2019	Crown
RFDS	-	AT&T FA #: 10035068 Dated: 03/24/2020	Crown

**3.1) Analysis Method**

RISA-3D (Version 17.0.4), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases. Selected output from the analysis are included in the Appendices.

A tool internally developed, using Microsoft Excel, by POD Group, was used to calculate wind loading on all appurtenances, dishes, and mount members for various load cases. Selected output from the calculations is included in Appendix B.

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 Tower Mount Analysis (Revision B). In addition, this analysis is in accordance with AT&T's mount technical directive.

### 3.2) Assumptions

- 1) The antenna mounting system was properly fabricated, installed, and maintained in good condition in accordance with its original design, TIA Standards, and/or manufacturer's specifications.
- 2) The configuration of antennas, mounts, and other appurtenances are as specified in Table 1 and the referenced drawings.
- 3) All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- 4) The weight of the mount was increased 10% in the analysis to account for connections, coax, and jumpers.
- 5) Member sizes have been assumed from photos of the site and experience with similar mounting systems. If the sizes assumed in this report differ from the actual member sizes, POD Group shall be contacted immediately, and the results of the analysis shall be considered null and void.
- 6) All structural members shall be verified in accordance with AT&T Mount Technical Directive.
- 7) The analysis will be required to be revised if the existing conditions in the field differ from those shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
- 8) Additional mount pipes are considered in Alpha and Gamma sectors to accommodate proposed loading.
- 9) Mount pipes have been adjusted to meet the edge to edge spacing requirement between the antennas as described in the radio frequency data sheet.
- 10) Steel grades have been assumed as follows, unless noted otherwise:
  - a. Angle  
ASTM A36 (GR 36)
  - b. Pipe  
ASTM A53 (GR 35)
  - c. Connection Bolts  
ASTM A325

If any of these assumptions are not valid or have been made in error, this analysis may be affected, and POD Group should be allowed to review any new information to determine its effect on the structural integrity of the mount.

**4) ANALYSIS RESULTS**

**Table 3 - Mount Component Stresses vs. Capacity (12.5 ft Frame Mount)**

Notes	Component	Critical Member	Centerline (ft)	% Capacity	Pass / Fail
1	Face	BOTTOMFACE	230	71.0	Pass
	Mount Pipe	MP ALPHA4	230	42.6	Pass
	Pipe	TB PIPE	230	33.5	Pass
	Support Pipe	SUPPIPE	230	14.2	Pass
	Tieback	TIEBACK	230	9.9	Pass

<b>Structure Rating (max from all components) =</b>	<b>71.0%</b>
---	--------------

Notes:

- 1) All sectors are typical

**Table 4 - Tieback Connection Data Table**

Tower Connection Node No.	Existing / Proposed	Resultant End Reaction (lb)	Connected Member Type	Connected Member Size	Member Compressive Capacity (lb) <sup>2</sup>	Notes
N53	Existing	330	Pipe	ROHN 2.5 STD	3329	1

Notes:

- 1) Tieback connection point is within 25% of either end of the connected tower member
- 2) Reduced member compressive capacity according to CED-STD-10294 *Standard for Installation of Mounts and Appendances*

**4.1) Recommendations**

The mount has sufficient capacity to carry the proposed loading configuration. In order for the results of the analysis to be considered valid, the loading modification listed below must be completed.

1. Installation of 8' STD P2 pipe on sectors Alpha and Gamma, 5' from the right end of the mount when looking from the front.
2. Recommended P2 Pipe Shall be connected with (2 per sector, 4 total) SitePro1, P/N SCX2-K
  - o All critical measurements and manufacturer specifications for the above specified modification part shall be field verified prior to material ordering.
  - o The contractor shall provide shop drawings to POD Group prior to material ordering and/or fabrication of the above specified modification part.
  - o Any substitutes, additions, or alterations shall be approved by POD Group prior to material ordering and/or fabrication.

If any of these guidelines are not met, POD Group shall not be held liable.

**Table 5 – AT&T Specification**

Wind Speed (mph)	Ice Thickness (in)	Height (ft)	Exposure	Class	Topo	# of Pipes	Allowable EPA per Pipe (ft sq.)	Allowable Weight per Sector (lbs)
117	1	230	B	II	1	4	13.86	2520

## **5) DISCLAIMER OF WARRANTIES**

POD Group has not performed a site visit to the structure to verify the member sizes or antenna/coax loading unless noted otherwise. If the existing conditions are not as represented in this report, we should be contacted immediately to evaluate the significance of the discrepancy. This is not a condition assessment of the structure or foundation. This report does not replace a full structure inspection. The structure, foundations, and mounting systems are assumed to have been properly fabricated, erected, maintained, in good condition, twist free, and plumb.

The engineering services rendered by POD Group in connection with this Structural Analysis are limited to a computer analysis of the structure and theoretical capacity of its main structural members. No allowance was made for any damaged, bent, missing, loose, or rusted members (above and below ground). No allowance was made for loose bolts or cracked welds.

POD Group does not analyze the fabrication of the structure (including welding). It is not possible to have all the very detailed information needed to perform a thorough analysis of every structural sub-component and connection of an existing structure. POD Group provides a limited scope of service in that we cannot verify the adequacy of every weld, plate connection detail, etc. The purpose of this report is to assess the feasibility of adding appurtenances usually accompanied by transmission lines to the structure.

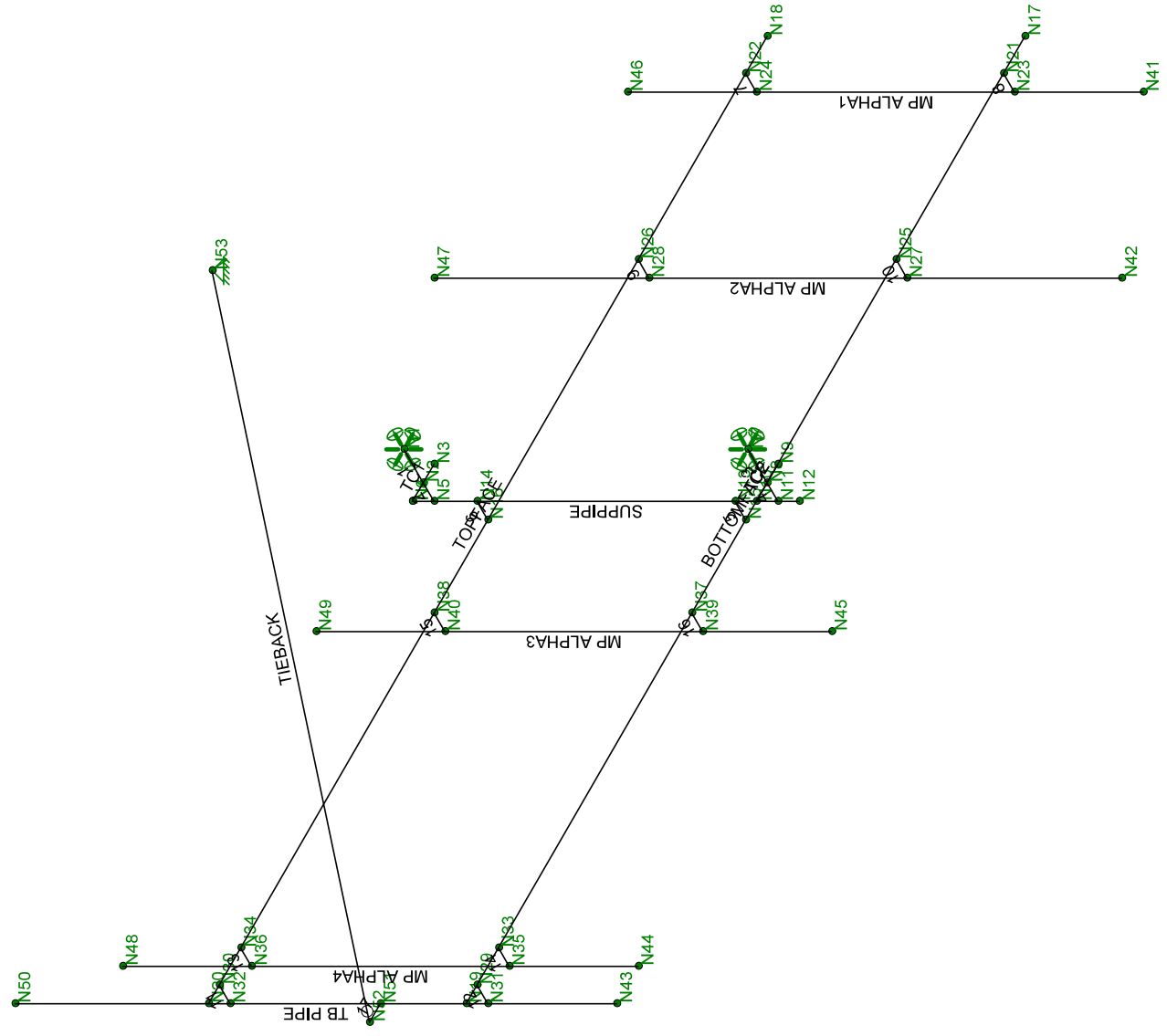
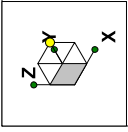
It is the owner's responsibility to determine the amount of ice accumulation in excess of the code specified amount, if any, that should be considered in the structural analysis.

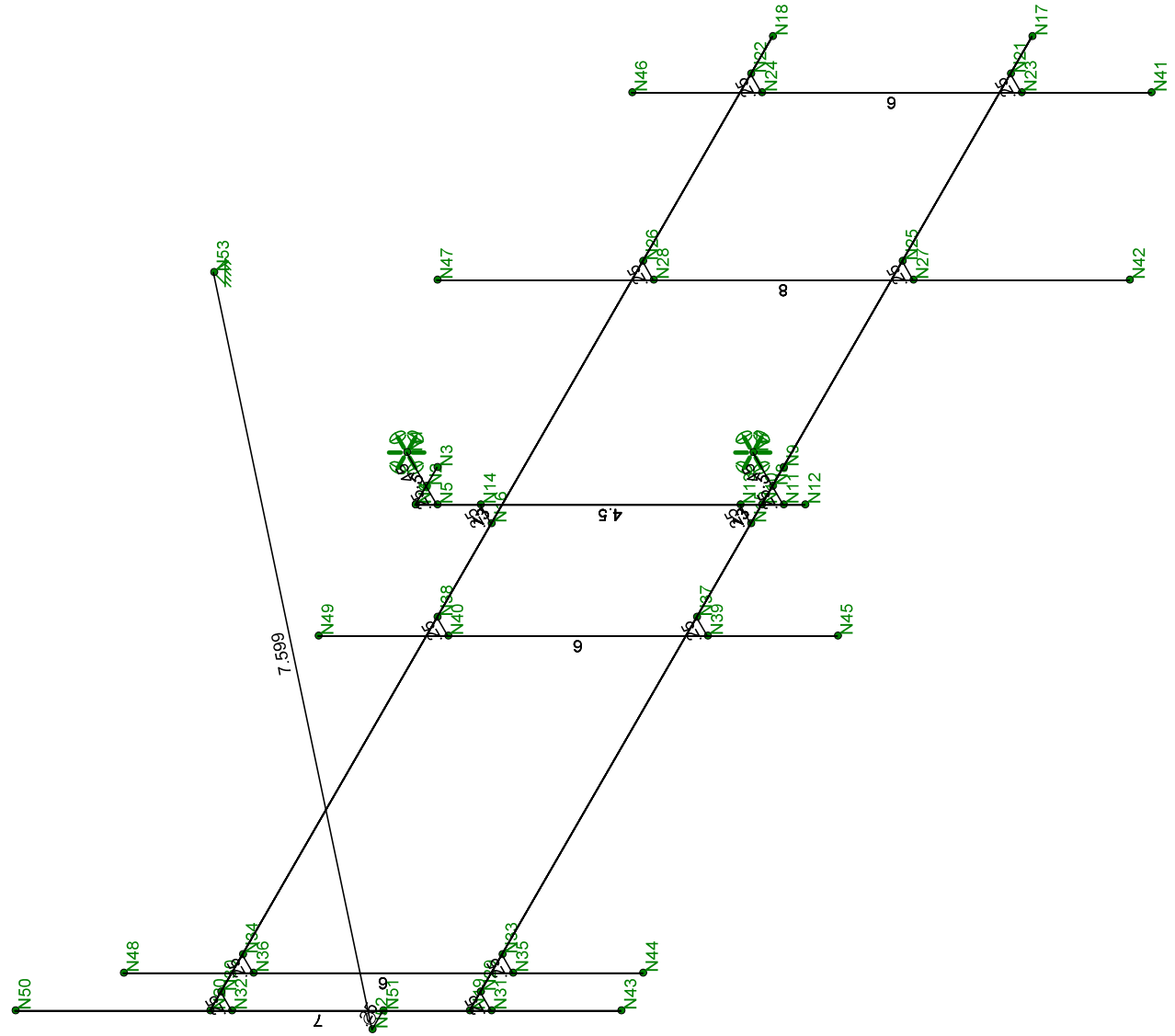
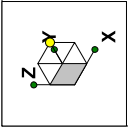
The attached sketches are a schematic representation of the analyzed structure. If any material is fabricated from these sketches, the contractor shall be responsible for field verifying the existing conditions, proper fit, and clearance in the field. Any mentions of structural modifications are reasonable estimates and should not be used as a precise construction document. Precise modification drawings are obtainable from POD Group, but are beyond the scope of this report.

POD Group makes no warranties, expressed and/or implied, in connection with this report and disclaims any liability arising from material, fabrication, and erection of this structure. POD Group will not be responsible whatsoever, for or on account of, consequential or incidental damages sustained by any person, firm, or organization as a result of any data or conclusions contained in this report. The maximum liability of POD Group pursuant to this report will be limited to the total fee received for preparation of this report.

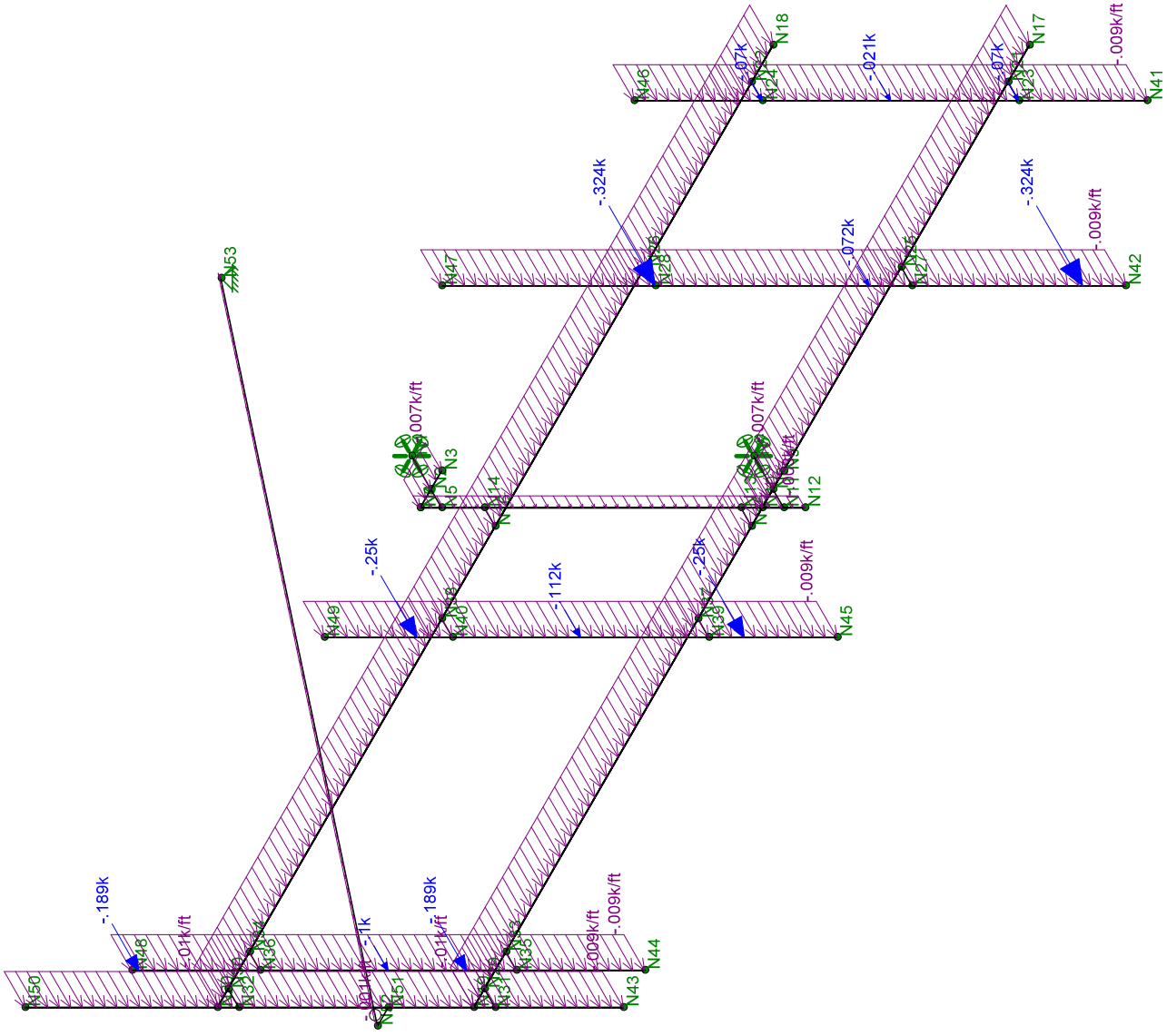
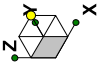
## APPENDIX A

### Wire Frame and Rendered Models

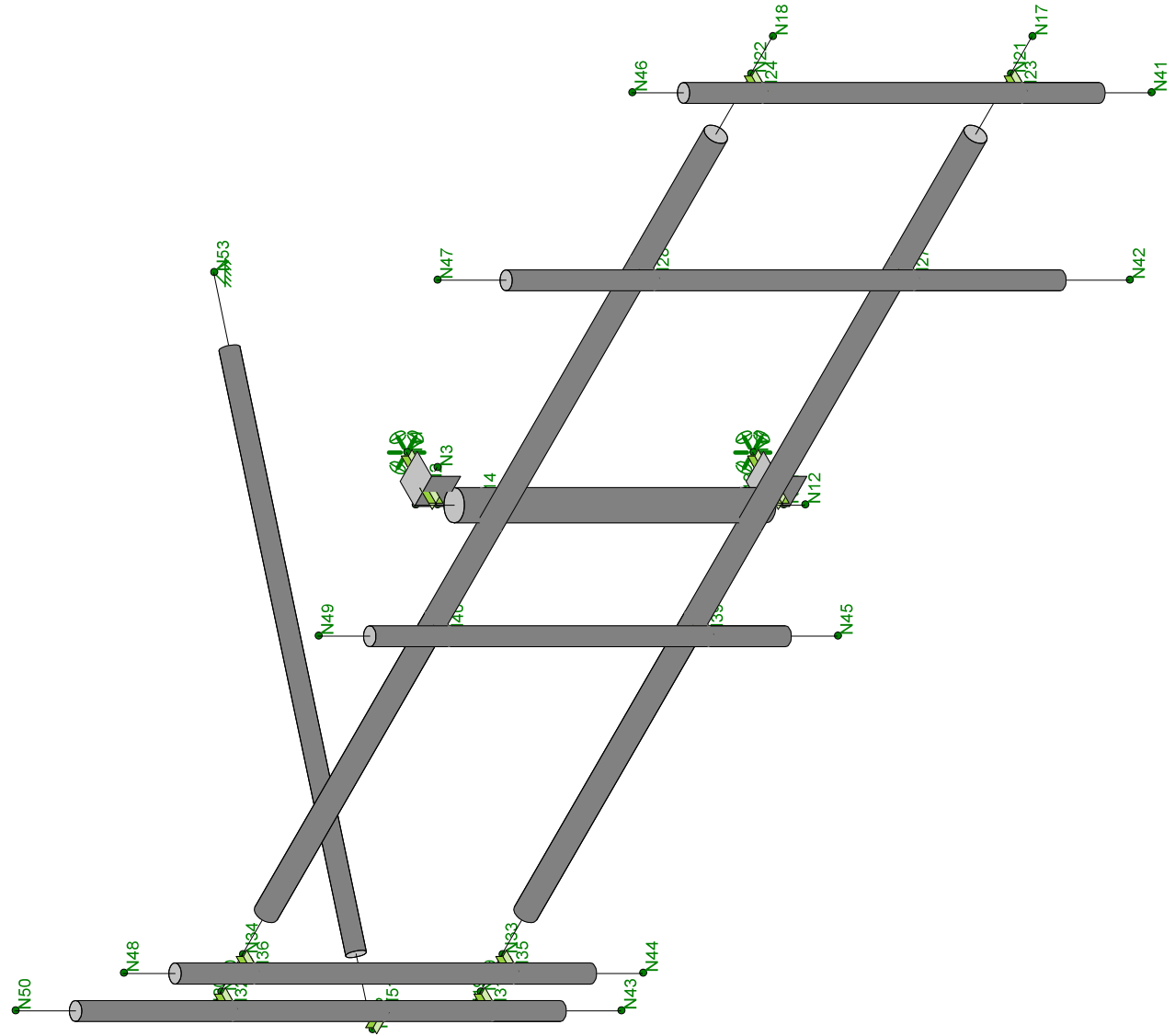
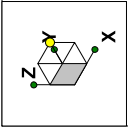


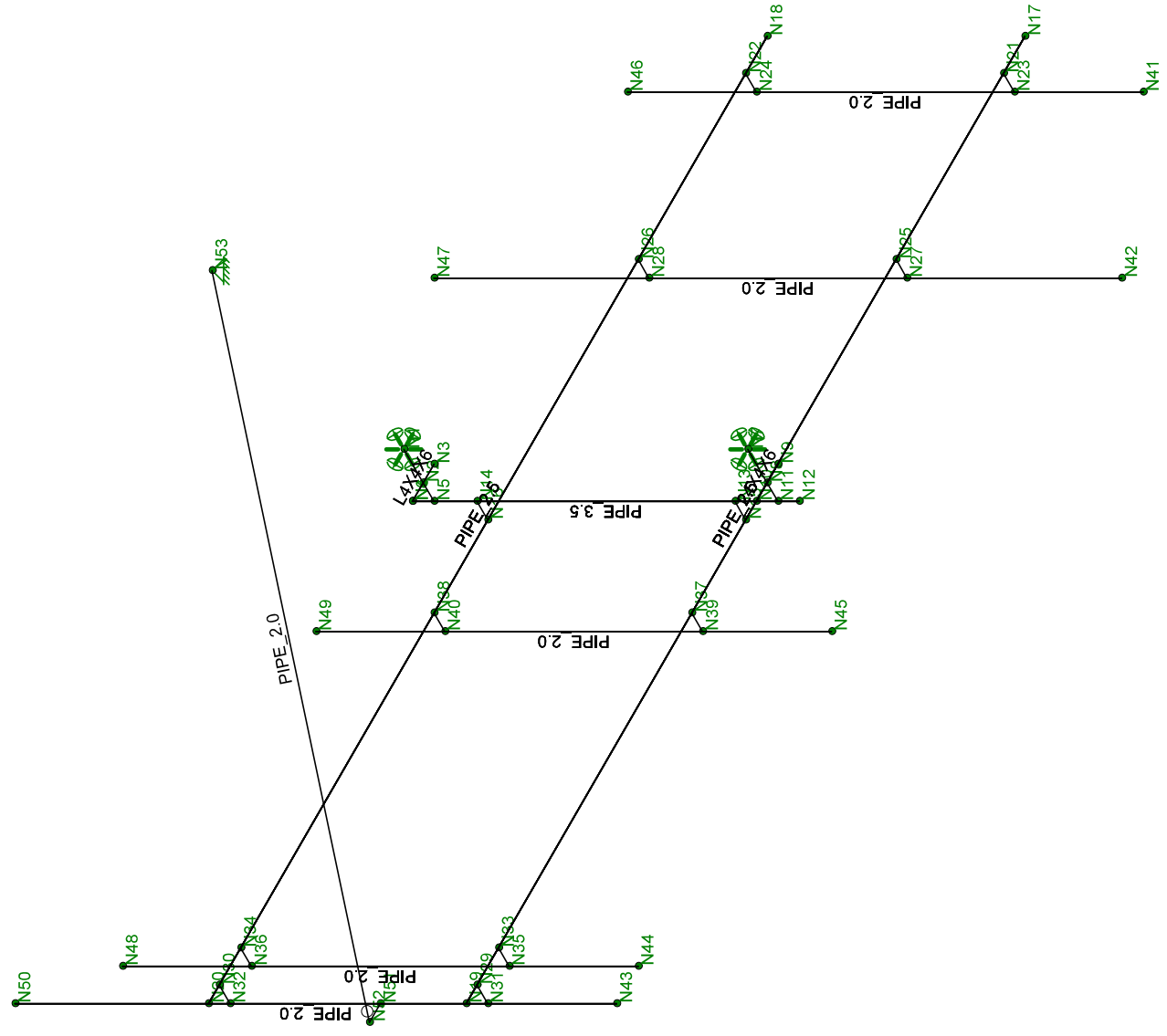
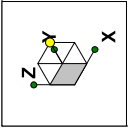


Member Length (ft) Displayed









## **APPENDIX B**

### **Software Input Calculations**



POD Job #  
20548187  
Site Name  
MORROE-GUINEA ROAD

Mount Type MF  
V (Wind Speed) 117  
Z<sub>0</sub> 584  
V<sub>1</sub> 1  
V<sub>2</sub> 50  
Kz1 1  
Exposure B  
Z<sub>e</sub> 1000  
K<sub>z</sub> 0.7  
G<sub>z</sub> 1  
K<sub>e</sub> 0.38  
K<sub>d</sub> 0.35  
K<sub>g</sub> 0.3

Risk Category II  
I (column) 1  
Sms 0.339  
S<sub>m1</sub> 0.132  
S<sub>m2</sub> 0.088  
S<sub>1</sub> 0.055  
Soil Site Class D (assumed)  
F<sub>s</sub> 1.600  
Seismic Analysis Not Required  
As 11A-222-H 16.7  
Cs, Min 11A-222-H 16.7  
Cs, Max 0.11306667 11A-222-H 27.7,1.1

**Appurtenance Information**

Model	Shielded	% Shielded	Centerline	Centerline on MP	Spacing (in)	Amount	Sector	Quantity	MP #
OPASR-BU5D	No	0	230	3	46	A	A	1	2
HPA-ESR-BUJ-HR	No	0	230	4	46	A	A	1	4
OPASR-BU5D	No	0	230	3	46	A	A	1	3
7770 4448 57612	No	0	230	3	36	A	A	1	1
RRJUS 32 8P	No	0	230	3	36	A	A	1	4
RRJUS 4775 B14	No	0	230	3	36	A	A	1	4
LOP13139	No	0	230	3	36	A	A	1	3
								2	1

**Mount Information**

Elevation (ft) 230  
K<sub>1</sub> 1.25  
K<sub>2</sub> 1.14  
K<sub>3</sub> 1.21

Length (ft) Width (in) Centerline  
6 2.375 230

**Round Members**

Length (ft) Width (in) Frame Member # of Members  
Eccs 12.5 2.875 No 2  
Support Pipe 12.5 2.875 No 2  
Tieback 7.599 2.375 No 1

**Flat Members**

Member Length (ft) Width (in) Shape A B C D Frame Member # of Members  
Tower Connection 0.5 4 Angle 4 0.375 No 2



**Appearance Wind Calculations**

Model	Height	Width	Depth	Weight (lb)	Kz	qr (lb/ft <sup>2</sup> )	[EPA] <sub>w</sub> (ft <sup>2</sup> )	[EPA] <sub>h</sub> (ft <sup>2</sup> )	Front	Side	Wind Force (Kips)	Beta	Gamma
DWPSR-BUSD	56.0	20.7	7.7	105.6	1.25	40.87	15.86	5.95	0.648	0.243	0.547	0.547	0.248
HPA-ESR-BUJ-H6	72.0	14.8	9.0	47.9	1.26	40.87	9.22	4.65	0.378	0.191	0.331	0.331	0.191
OPASR-BUSD	71.2	21.0	7.8	68.5	1.25	40.87	12.22	4.54	0.499	0.186	0.421	0.421	0.186
7770	35.0	10.0	5.0	35.0	1.25	40.87	1.77	1.36	0.180	0.054	0.121	0.121	0.054
7770	44.85	15.0	7.5	37.5	1.25	40.87	1.77	1.36	0.180	0.054	0.121	0.121	0.054
RRUS 32 B2	21.1	13.1	7.0	52.9	1.25	40.87	2.46	1.50	0.100	0.061	0.091	0.091	0.061
RRUS 4478 B14	16.5	13.4	7.7	59.9	1.25	40.87	1.66	0.95	0.068	0.039	0.061	0.061	0.039
LGP1319	7.9	4.4	2.7	5.3	1.25	40.87	0.26	0.16	0.011	0.007	0.010	0.010	0.007
DC6-46-60-184F	31.3	11.0	7.7	32.8	1.25	40.87	1.09	1.21	0.045	0.050	0.046	0.046	0.050

**Appearance Ice Calculations**

Model	Height	Width	Depth	Weight (lb)	Kz	qr (lb/ft <sup>2</sup> )	[EPA] <sub>w</sub> (ft <sup>2</sup> )	[EPA] <sub>h</sub> (ft <sup>2</sup> )	Front	Side	Wind Force (Kips)	Alpha	Beta	Gamma
DWPSR-BUSD	56.0	20.7	7.7	105.6	1.25	40.87	15.86	5.95	0.648	0.243	0.547	0.547	0.248	
HPA-ESR-BUJ-H6	72.0	14.8	9.0	47.9	1.26	40.87	9.22	4.65	0.378	0.191	0.331	0.331	0.191	
OPASR-BUSD	71.2	21.0	7.8	68.5	1.25	40.87	12.22	4.54	0.499	0.186	0.421	0.421	0.186	
7770	35.0	10.0	5.0	35.0	1.25	40.87	1.77	1.36	0.180	0.054	0.121	0.121	0.054	
7770	44.85	15.0	7.5	37.5	1.25	40.87	1.77	1.36	0.180	0.054	0.121	0.121	0.054	
RRUS 32 B2	21.1	13.1	7.0	52.9	1.25	40.87	2.46	1.50	0.100	0.061	0.091	0.091	0.061	
RRUS 4478 B14	16.5	13.4	7.7	59.9	1.25	40.87	1.66	0.95	0.068	0.039	0.061	0.061	0.039	
LGP1319	7.9	4.4	2.7	5.3	1.25	40.87	0.26	0.16	0.011	0.007	0.010	0.010	0.007	
DC6-46-60-184F	31.3	11.0	7.7	32.8	1.25	40.87	1.09	1.21	0.045	0.050	0.046	0.046	0.050	

**Board Members**

Member	q <sub>s</sub> (lb/ft <sup>2</sup> )	Ar	C	EPA (ft <sup>2</sup> )	Load (kV)	EPA (ft <sup>2</sup> )	Arice	Refice	Cs	EPA (ft <sup>2</sup> )	Load (kV)
Face	40.87	5.99	31.06	1.82	2.99	0.010	7.46	11.05	0.63	1.82	5.70
Support Type	40.87	1.27	31.06	1.20	0.99	0.003	7.46	3.92	0.63	1.20	3.76
Interior	40.87	1.50	25.96	1.20	0.99	0.003	7.46	3.94	0.63	1.20	2.97

**Flat Members**

Member	q <sub>s</sub> (lb/ft <sup>2</sup> )	At	Cs	Wind Calculations	EPA	Load (kV)	Ice Calculations	Arice	Refice	Cs	EPA (ft <sup>2</sup> )	Load (kV)
Tower Connection	40.87	0.33	0.33	0.007	0.18	0.001	7.46	0.54	0.63	1.82	0.28	0.002

**Appearance Seismic Calculations**

Model	Height	Width	Depth	Weight	Cs	Ev	Th
DWPSR-BUSD	56.0	20.7	7.7	105.6	0.113	1.000	0.005
HPA-ESR-BUJ-H6	72.0	14.8	9.0	47.9	0.113	1.000	0.002
OPASR-BUSD	71.2	21.0	7.8	68.5	0.113	1.000	0.003
7770	35.0	10.0	5.0	35.0	0.113	1.000	0.002
7770	44.85	15.0	7.5	37.5	0.113	1.000	0.002
RRUS 32 B2	21.1	13.1	7.0	52.9	0.113	1.000	0.002
RRUS 4478 B14	16.5	13.4	7.7	59.9	0.113	1.000	0.003
LGP1319	7.9	4.4	2.7	5.3	0.113	1.000	0.001
DC6-46-60-184F	31.3	11.0	7.7	32.8	0.113	1.000	0.001

## **APPENDIX C**

### **Software Analysis Output**



Company : POD Group  
 Designer : DP  
 Job Number : 20-64617  
 Model Name : 841294

May 28, 2020  
 5:05 PM  
 Checked By: \_\_\_\_\_

### Hot Rolled Steel Design Parameters

Label	Shape	Length[ft]	Lbyy[ft]	Lbzz[ft]	Lcomp_top[ft]	Lcomp_bot[ft]	L_torg...	Kyy	Kzz	Cb	Function
1	TC1	L4X4X6	.5		Lbyy						Lateral
2	TC2	L4X4X6	.5		Lbyy						Lateral
3	BOTTOMF...	PIPE 2.5	13		Lbyy						Lateral
4	TOPFACE	PIPE 2.5	13		Lbyy						Lateral
5	SUPPIPE	PIPE 3.5	4.5		Lbyy						Lateral
6	MP ALPHA1	PIPE 2.0	6		Lbyy						Lateral
7	MP ALPHA2	PIPE 2.0	8		Lbyy						Lateral
8	MP ALPHA3	PIPE 2.0	6		Lbyy						Lateral
9	MP ALPHA4	PIPE 2.0	6		Lbyy						Lateral
10	TB PIPE	PIPE 2.0	7		Lbyy						Lateral
11	TIEBACK	PIPE 2.0	7.599		Lbyy						Lateral

### Member Primary Data

Label	I Joint	J Joint	K Joint	Rotate(...)	Section/Shape	Type	Design List	Material	Design R...
1	N1	N2			RIGID	None	None	RIGID	Typical
2	TC1	N3	N4		L4X4X6	Beam	Single Angle	A36 Gr.36	Typical
3	2	N2	N5		RIGID	None	None	RIGID	Typical
4	3	N7	N8		RIGID	None	None	RIGID	Typical
5	TC2	N9	N10		L4X4X6	Beam	Single Angle	A36 Gr.36	Typical
6	4	N8	N11		RIGID	None	None	RIGID	Typical
7	5	N13	N15		RIGID	None	None	RIGID	Typical
8	6	N14	N16		RIGID	None	None	RIGID	Typical
9	BOTTOMFACE	N19	N17		PIPE 2.5	Beam	Pipe	A53 Gr.B	Typical
10	TOPFACE	N20	N18		PIPE 2.5	Beam	Pipe	A53 Gr.B	Typical
11	SUPPIPE	N12	N6		PIPE 3.5	Beam	Pipe	A53 Gr.B	Typical
12	7	N22	N24		RIGID	None	None	RIGID	Typical
13	8	N21	N23		RIGID	None	None	RIGID	Typical
14	9	N26	N28		RIGID	None	None	RIGID	Typical
15	10	N25	N27		RIGID	None	None	RIGID	Typical
16	11	N30	N32		RIGID	None	None	RIGID	Typical
17	12	N29	N31		RIGID	None	None	RIGID	Typical
18	13	N34	N36		RIGID	None	None	RIGID	Typical
19	14	N33	N35		RIGID	None	None	RIGID	Typical
20	15	N38	N40		RIGID	None	None	RIGID	Typical
21	16	N37	N39		RIGID	None	None	RIGID	Typical
22	MP ALPHA1	N41	N46		PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
23	MP ALPHA2	N42	N47		PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
24	MP ALPHA3	N45	N49		PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
25	MP ALPHA4	N44	N48		PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
26	TB PIPE	N43	N50		PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
27	TIEBACK	N52	N53		PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
28	17	N51	N52		RIGID	None	None	RIGID	Typical

### Member Advanced Data

Label	I Release	J Release	I Offset[in]	J Offset[in]	T/C Only	Physical Defl Ra...	Analysis ...	Inactive	Seismic...
1						Yes	** NA **		None
2	TC1					Yes	** NA **		None
3	2					Yes	** NA **		None
4	3					Yes	** NA **		None
5	TC2					Yes	** NA **		None
6	4					Yes	** NA **		None
7	5					Yes	** NA **		None
8	6					Yes	** NA **		None

**Member Advanced Data (Continued)**

Label	I Release	J Release	I Offset[in]	J Offset[in]	T/C Only	Physical Defl Ra...	Analysis ...	Inactive	Seismic...
9	BOTTOMFACE				Yes				None
10	TOPFACE				Yes				None
11	SUPPIPE				Yes				None
12	7				Yes	** NA **			None
13	8				Yes	** NA **			None
14	9				Yes	** NA **			None
15	10				Yes	** NA **			None
16	11				Yes	** NA **			None
17	12				Yes	** NA **			None
18	13				Yes	** NA **			None
19	14				Yes	** NA **			None
20	15				Yes	** NA **			None
21	16				Yes	** NA **			None
22	MP ALPHA1				Yes	Default			None
23	MP ALPHA2				Yes				None
24	MP ALPHA3				Yes				None
25	MP ALPHA4				Yes				None
26	TB PIPE				Yes				None
27	TIEBACK	BenPIN			Yes	Default			None
28	17				Yes	** NA **			None

**Hot Rolled Steel Properties**

Label	E [ksj]	G [ksj]	Nu	Therm (1/E5 ...)	Density[k/ft^3]	Yield[ksj]	Ry	F <sub>u</sub> [ksj]	Rt
1	A992	29000	11154	.3	.65	.49	50	65	1.1
2	A36 Gr.36	29000	11154	.3	.65	.49	36	58	1.2
3	A572 Gr.50	29000	11154	.3	.65	.49	50	65	1.1
4	A500 Gr.B RND	29000	11154	.3	.65	.527	42	58	1.3
5	A500 Gr.B Rect	29000	11154	.3	.65	.527	46	58	1.3
6	A53 Gr.B	29000	11154	.3	.65	.49	35	60	1.2
7	A1085	29000	11154	.3	.65	.49	50	65	1.15
8	A913 Gr.65	29000	11154	.3	.65	.49	65	80	1.1

**Member Point Loads (BLC 1 : Live Load)**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
1	TOPFACE	Z	0

**Member Point Loads (BLC 2 : Wind Load (0))**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
1	MP ALPHA2	Y	5.5
2	MP ALPHA2	Y	.5
3	MP ALPHA4	Y	5.917
4	MP ALPHA4	Y	2.083
5	MP ALPHA3	Y	4.917
6	MP ALPHA3	Y	1.083
7	MP ALPHA1	Y	4.5
8	MP ALPHA1	Y	1.5
9	MP ALPHA2	Y	3
10	MP ALPHA4	Y	3
11	MP ALPHA3	Y	3
12	MP ALPHA1	Y	3
13	MP ALPHA3	Y	3



**Member Point Loads (BLC 3 : Dead Load)**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
1			
MP ALPHA2	Z	-.053	5.5
MP ALPHA2	Z	-.053	.5
MP ALPHA4	Z	-.024	5.917
MP ALPHA4	Z	-.024	2.083
MP ALPHA3	Z	-.032	4.917
MP ALPHA3	Z	-.032	1.083
MP ALPHA1	Z	-.018	4.5
MP ALPHA1	Z	-.018	1.5
MP ALPHA2	Z	-.071	3
MP ALPHA4	Z	-.053	3
MP ALPHA3	Z	-.06	3
MP ALPHA1	Z	-.011	3
MP ALPHA3	Z	-.033	3

**Member Point Loads (BLC 4 : Wind Load (30))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
1			
MP ALPHA2	Y	-.237	5.5
MP ALPHA2	Y	-.237	.5
MP ALPHA2	X	-.137	5.5
MP ALPHA2	X	-.137	.5
MP ALPHA4	Y	-.143	5.917
MP ALPHA4	Y	-.143	2.083
MP ALPHA4	X	-.083	5.917
MP ALPHA4	X	-.083	2.083
MP ALPHA3	Y	-.182	4.917
MP ALPHA3	Y	-.182	1.083
MP ALPHA3	X	-.105	4.917
MP ALPHA3	X	-.105	1.083
MP ALPHA1	Y	-.052	4.5
MP ALPHA1	Y	-.052	1.5
MP ALPHA1	X	-.03	4.5
MP ALPHA1	X	-.03	1.5
MP ALPHA2	Y	-.058	3
MP ALPHA2	X	-.034	3
MP ALPHA4	Y	-.079	3
MP ALPHA4	X	-.045	3
MP ALPHA3	Y	-.052	3
MP ALPHA3	X	-.03	3
MP ALPHA1	Y	-.017	3
MP ALPHA1	X	-.01	3
MP ALPHA3	Y	-.04	3
MP ALPHA3	X	-.023	3

**Member Point Loads (BLC 5 : Wind Load (60))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
1			
MP ALPHA2	Y	-.086	5.5
MP ALPHA2	Y	-.086	.5
MP ALPHA2	X	-.149	5.5
MP ALPHA2	X	-.149	.5
MP ALPHA4	Y	-.059	5.917
MP ALPHA4	Y	-.059	2.083
MP ALPHA4	X	-.103	5.917
MP ALPHA4	X	-.103	2.083
MP ALPHA3	Y	-.066	4.917
MP ALPHA3	Y	-.066	1.083
MP ALPHA3	X	-.114	4.917

**Member Point Loads (BLC 5 : Wind Load (60)) (Continued)**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]	
12	MP ALPHA3	X	-1.14	1.083
13	MP ALPHA1	Y	-.021	4.5
14	MP ALPHA1	Y	-.021	1.5
15	MP ALPHA1	X	-.036	4.5
16	MP ALPHA1	X	-.036	1.5
17	MP ALPHA2	Y	-.028	3
18	MP ALPHA2	X	-.049	3
19	MP ALPHA4	Y	-.036	3
20	MP ALPHA4	X	-.062	3
21	MP ALPHA3	Y	-.023	3
22	MP ALPHA3	X	-.04	3
23	MP ALPHA1	Y	-.008	3
24	MP ALPHA1	X	-.013	3
25	MP ALPHA3	Y	-.024	3
26	MP ALPHA3	X	-.042	3

**Member Point Loads (BLC 6 : Wind Load (90))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]	
1	MP ALPHA2	X	-.122	5.5
2	MP ALPHA2	X	-.122	.5
3	MP ALPHA4	X	-.095	5.917
4	MP ALPHA4	X	-.095	2.083
5	MP ALPHA3	X	-.093	4.917
6	MP ALPHA3	X	-.093	1.083
7	MP ALPHA1	X	-.032	4.5
8	MP ALPHA1	X	-.032	1.5
9	MP ALPHA2	X	-.052	3
10	MP ALPHA4	X	-.061	3
11	MP ALPHA3	X	-.039	3
12	MP ALPHA1	X	-.013	3
13	MP ALPHA3	X	-.05	3

**Member Point Loads (BLC 7 : Wind Load (120))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]	
1	MP ALPHA2	Y	.086	5.5
2	MP ALPHA2	Y	.086	.5
3	MP ALPHA2	X	-.149	5.5
4	MP ALPHA2	X	-.149	.5
5	MP ALPHA4	Y	.059	5.917
6	MP ALPHA4	Y	.059	2.083
7	MP ALPHA4	X	-.103	5.917
8	MP ALPHA4	X	-.103	2.083
9	MP ALPHA3	Y	.066	4.917
10	MP ALPHA3	Y	.066	1.083
11	MP ALPHA3	X	-.114	4.917
12	MP ALPHA3	X	-.114	1.083
13	MP ALPHA1	Y	.021	4.5
14	MP ALPHA1	Y	.021	1.5
15	MP ALPHA1	X	-.036	4.5
16	MP ALPHA1	X	-.036	1.5
17	MP ALPHA2	Y	.028	3
18	MP ALPHA2	X	-.049	3
19	MP ALPHA4	Y	.036	3
20	MP ALPHA4	X	-.062	3
21	MP ALPHA3	Y	.023	3
22	MP ALPHA3	X	-.04	3

**Member Point Loads (BLC 7 : Wind Load (120)) (Continued)**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
23	Y	.008	3
24	X	-.013	3
25	Y	.024	3
26	X	-.042	3

**Member Point Loads (BLC 8 : Wind Load (150))**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
1	Y	.237	5.5
2	Y	.237	.5
3	X	-.137	5.5
4	X	-.137	.5
5	Y	.143	5.917
6	Y	.143	2.083
7	X	-.083	5.917
8	X	-.083	2.083
9	Y	.182	4.917
10	Y	.182	1.083
11	X	-.105	4.917
12	X	-.105	1.083
13	Y	.052	4.5
14	Y	.052	1.5
15	X	-.03	4.5
16	X	-.03	1.5
17	Y	.058	3
18	X	-.034	3
19	Y	.079	3
20	X	-.045	3
21	Y	.052	3
22	X	-.03	3
23	Y	.017	3
24	X	-.01	3
25	Y	.04	3
26	X	-.023	3

**Member Point Loads (BLC 9 : Wind Load (180))**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
1	Y	.324	5.5
2	Y	.324	.5
3	Y	.189	5.917
4	Y	.189	2.083
5	Y	.25	4.917
6	Y	.25	1.083
7	Y	.07	4.5
8	Y	.07	1.5
9	Y	.072	3
10	Y	.1	3
11	Y	.068	3
12	Y	.021	3
13	Y	.045	3

**Member Point Loads (BLC 10 : Wind Load (210))**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
1	Y	.237	5.5
2	Y	.237	.5
3	X	.137	5.5



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 Designer : DP  
 Job Number : 20-64617  
 Model Name : 841294

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**Member Point Loads (BLC 10 : Wind Load (210)) (Continued)**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
4	X	.137	.5
5	Y	.143	5.917
6	Y	.143	2.083
7	X	.083	5.917
8	X	.083	2.083
9	Y	.182	4.917
10	Y	.182	1.083
11	X	.105	4.917
12	X	.105	1.083
13	Y	.052	4.5
14	Y	.052	1.5
15	X	.03	4.5
16	X	.03	1.5
17	Y	.058	3
18	X	.034	3
19	Y	.079	3
20	X	.045	3
21	Y	.052	3
22	X	.03	3
23	Y	.017	3
24	X	.01	3
25	Y	.04	3
26	X	.023	3

**Member Point Loads (BLC 11 : Wind Load (240))**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	Y	.086	5.5
2	Y	.086	.5
3	X	.149	5.5
4	X	.149	.5
5	Y	.059	5.917
6	Y	.059	2.083
7	X	.103	5.917
8	X	.103	2.083
9	Y	.066	4.917
10	Y	.066	1.083
11	X	.114	4.917
12	X	.114	1.083
13	Y	.021	4.5
14	Y	.021	1.5
15	X	.036	4.5
16	X	.036	1.5
17	Y	.028	3
18	X	.049	3
19	Y	.036	3
20	X	.062	3
21	Y	.023	3
22	X	.04	3
23	Y	.008	3
24	X	.013	3
25	Y	.024	3
26	X	.042	3

**Member Point Loads (BLC 12 : Wind Load (270))**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	X	.122	5.5

**Member Point Loads (BLC 12 : Wind Load (270)) (Continued)**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
2	X	.122	.5
3	X	.095	5.917
4	X	.095	2.083
5	X	.093	4.917
6	X	.093	1.083
7	X	.032	4.5
8	X	.032	1.5
9	X	.052	3
10	X	.061	3
11	X	.039	3
12	X	.013	3
13	X	.05	3

**Member Point Loads (BLC 13 : Wind Load (300))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
1	Y	-.086	5.5
2	Y	-.086	.5
3	X	.149	5.5
4	X	.149	.5
5	Y	-.059	5.917
6	Y	-.059	2.083
7	X	.103	5.917
8	X	.103	2.083
9	Y	-.066	4.917
10	Y	-.066	1.083
11	X	.114	4.917
12	X	.114	1.083
13	Y	-.021	4.5
14	Y	-.021	1.5
15	X	.036	4.5
16	X	.036	1.5
17	Y	-.028	3
18	X	.049	3
19	Y	-.036	3
20	X	.062	3
21	Y	-.023	3
22	X	.04	3
23	Y	-.008	3
24	X	.013	3
25	Y	-.024	3
26	X	.042	3

**Member Point Loads (BLC 14 : Wind Load (330))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
1	Y	-.237	5.5
2	Y	-.237	.5
3	X	.137	5.5
4	X	.137	.5
5	Y	-.143	5.917
6	Y	-.143	2.083
7	X	.083	5.917
8	X	.083	2.083
9	Y	-.182	4.917
10	Y	-.182	1.083
11	X	.105	4.917
12	X	.105	1.083

**Member Point Loads (BLC 14 : Wind Load (330)) (Continued)**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
13	Y	-.052	4.5
14	Y	-.052	1.5
15	X	.03	4.5
16	X	.03	1.5
17	Y	-.058	3
18	X	.034	3
19	Y	-.079	3
20	X	.045	3
21	Y	-.052	3
22	X	.03	3
23	Y	-.017	3
24	X	.01	3
25	Y	-.04	3
26	X	.023	3

**Member Point Loads (BLC 15 : Maintenance (0))**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	Y	-.021	5.5
2	Y	-.021	.5
3	Y	-.012	5.917
4	Y	-.012	2.083
5	Y	-.016	4.917
6	Y	-.016	1.083
7	Y	-.005	4.5
8	Y	-.005	1.5
9	Y	-.005	3
10	Y	-.007	3
11	Y	-.004	3
12	Y	-.001	3
13	Y	-.003	3

**Member Point Loads (BLC 16 : Maintenance (30))**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	Y	-.016	5.5
2	Y	-.016	.5
3	X	-.009	5.5
4	X	-.009	.5
5	Y	-.009	5.917
6	Y	-.009	2.083
7	X	-.005	5.917
8	X	-.005	2.083
9	Y	-.012	4.917
10	Y	-.012	1.083
11	X	-.007	4.917
12	X	-.007	1.083
13	Y	-.003	4.5
14	Y	-.003	1.5
15	X	-.002	4.5
16	X	-.002	1.5
17	Y	-.004	3
18	X	-.002	3
19	Y	-.005	3
20	X	-.003	3
21	Y	-.003	3
22	X	-.002	3
23	Y	-.001	3

**Member Point Loads (BLC 16 : Maintenance (30)) (Continued)**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
24	X	-0.00634	3
25	Y	-0.003	3
26	X	-0.002	3

**Member Point Loads (BLC 17 : Maintenance (60))**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
1	Y	-0.006	5.5
2	Y	-0.006	.5
3	X	-0.1	5.5
4	X	-0.1	.5
5	Y	-0.04	5.917
6	Y	-0.04	2.083
7	X	-0.07	5.917
8	X	-0.07	2.083
9	Y	-0.04	4.917
10	Y	-0.04	1.083
11	X	-0.08	4.917
12	X	-0.08	1.083
13	Y	-0.01	4.5
14	Y	-0.01	1.5
15	X	-0.02	4.5
16	X	-0.02	1.5
17	Y	-0.02	3
18	X	-0.03	3
19	Y	-0.02	3
20	X	-0.04	3
21	Y	-0.02	3
22	X	-0.03	3
23	Y	-0.00503	3
24	X	-0.0087	3
25	Y	-0.02	3
26	X	-0.03	3

**Member Point Loads (BLC 18 : Maintenance (90))**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
1	X	-0.08	5.5
2	X	-0.08	.5
3	X	-0.06	5.917
4	X	-0.06	2.083
5	X	-0.06	4.917
6	X	-0.06	1.083
7	X	-0.02	4.5
8	X	-0.02	1.5
9	X	-0.03	3
10	X	-0.04	3
11	X	-0.03	3
12	X	-0.00873	3
13	X	-0.03	3

**Member Point Loads (BLC 19 : Maintenance (120))**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
1	Y	.006	5.5
2	Y	.006	.5
3	X	-0.1	5.5
4	X	-0.1	.5

**Member Point Loads (BLC 19 : Maintenance (120)) (Continued)**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
5	Y	.004	5.917
6	Y	.004	2.083
7	X	-.007	5.917
8	X	-.007	2.083
9	Y	.004	4.917
10	Y	.004	1.083
11	X	-.008	4.917
12	X	-.008	1.083
13	Y	.001	4.5
14	Y	.001	1.5
15	X	-.002	4.5
16	X	-.002	1.5
17	Y	.002	3
18	X	-.003	3
19	Y	.002	3
20	X	-.004	3
21	Y	.002	3
22	X	-.003	3
23	Y	.000503	3
24	X	-.00087	3
25	Y	.002	3
26	X	-.003	3

**Member Point Loads (BLC 20 : Maintenance (150))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
1	Y	.016	5.5
2	Y	.016	.5
3	X	-.009	5.5
4	X	-.009	.5
5	Y	.009	5.917
6	Y	.009	2.083
7	X	-.005	5.917
8	X	-.005	2.083
9	Y	.012	4.917
10	Y	.012	1.083
11	X	-.007	4.917
12	X	-.007	1.083
13	Y	.003	4.5
14	Y	.003	1.5
15	X	-.002	4.5
16	X	-.002	1.5
17	Y	.004	3
18	X	-.002	3
19	Y	.005	3
20	X	-.003	3
21	Y	.003	3
22	X	-.002	3
23	Y	.001	3
24	X	-.000634	3
25	Y	.003	3
26	X	-.002	3

**Member Point Loads (BLC 21 : Maintenance (180))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
1	Y	.021	5.5
2	Y	.021	.5



**Member Point Loads (BLC 21 : Maintenance (180)) (Continued)**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft.%]
3	Y	.012	5.917
4	Y	.012	2.083
5	Y	.016	4.917
6	Y	.016	1.083
7	Y	.005	4.5
8	Y	.005	1.5
9	Y	.005	3
10	Y	.007	3
11	Y	.004	3
12	Y	.001	3
13	Y	.003	3

**Member Point Loads (BLC 22 : Maintenance (210))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft.%]
1	Y	.016	5.5
2	Y	.016	.5
3	X	.009	5.5
4	X	.009	.5
5	Y	.009	5.917
6	Y	.009	2.083
7	X	.005	5.917
8	X	.005	2.083
9	Y	.012	4.917
10	Y	.012	1.083
11	X	.007	4.917
12	X	.007	1.083
13	Y	.003	4.5
14	Y	.003	1.5
15	X	.002	4.5
16	X	.002	1.5
17	Y	.004	3
18	X	.002	3
19	Y	.005	3
20	X	.003	3
21	Y	.003	3
22	X	.002	3
23	Y	.001	3
24	X	.000634	3
25	Y	.003	3
26	X	.002	3

**Member Point Loads (BLC 23 : Maintenance (240))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft.%]
1	Y	.006	5.5
2	Y	.006	.5
3	X	.01	5.5
4	X	.01	.5
5	Y	.004	5.917
6	Y	.004	2.083
7	X	.007	5.917
8	X	.007	2.083
9	Y	.004	4.917
10	Y	.004	1.083
11	X	.008	4.917
12	X	.008	1.083
13	Y	.001	4.5

**Member Point Loads (BLC 23 : Maintenance (240)) (Continued)**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
14	Y	.001	1.5
15	X	.002	4.5
16	X	.002	1.5
17	Y	.002	3
18	X	.003	3
19	Y	.002	3
20	X	.004	3
21	Y	.002	3
22	X	.003	3
23	Y	.000503	3
24	X	.00087	3
25	Y	.002	3
26	X	.003	3

**Member Point Loads (BLC 24 : Maintenance (270))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
1	X	.008	5.5
2	X	.008	.5
3	X	.006	5.917
4	X	.006	2.083
5	X	.006	4.917
6	X	.006	1.083
7	X	.002	4.5
8	X	.002	1.5
9	X	.003	3
10	X	.004	3
11	X	.003	3
12	X	.000873	3
13	X	.003	3

**Member Point Loads (BLC 25 : Maintenance (300))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
1	Y	-.006	5.5
2	Y	-.006	.5
3	X	.01	5.5
4	X	.01	.5
5	Y	-.004	5.917
6	Y	-.004	2.083
7	X	.007	5.917
8	X	.007	2.083
9	Y	-.004	4.917
10	Y	-.004	1.083
11	X	.008	4.917
12	X	.008	1.083
13	Y	-.001	4.5
14	Y	-.001	1.5
15	X	.002	4.5
16	X	.002	1.5
17	Y	-.002	3
18	X	.003	3
19	Y	-.002	3
20	X	.004	3
21	Y	-.002	3
22	X	.003	3
23	Y	-.000503	3
24	X	.00087	3

**Member Point Loads (BLC 25 : Maintenance (300)) (Continued)**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft. %]
25	Y	-.002	3
26	X	.003	3

**Member Point Loads (BLC 26 : Maintenance (330))**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft. %]
1	Y	-.016	5.5
2	Y	-.016	.5
3	X	.009	5.5
4	X	.009	.5
5	Y	-.009	5.917
6	Y	-.009	2.083
7	X	.005	5.917
8	X	.005	2.083
9	Y	-.012	4.917
10	Y	-.012	1.083
11	X	.007	4.917
12	X	.007	1.083
13	Y	-.003	4.5
14	Y	-.003	1.5
15	X	.002	4.5
16	X	.002	1.5
17	Y	-.004	3
18	X	.002	3
19	Y	-.005	3
20	X	.003	3
21	Y	-.003	3
22	X	.002	3
23	Y	-.001	3
24	X	.000634	3
25	Y	-.003	3
26	X	.002	3

**Member Point Loads (BLC 27 : Ice Dead Load)**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft. %]
1	Z	-.126	5.5
2	Z	-.126	.5
3	Z	-.082	5.917
4	Z	-.082	2.083
5	Z	-.097	4.917
6	Z	-.097	1.083
7	Z	-.044	4.5
8	Z	-.044	1.5
9	Z	-.05	3
10	Z	-.057	3
11	Z	-.043	3
12	Z	-.017	3
13	Z	-.074	3

**Member Point Loads (BLC 28 : Ice Wind Load (0))**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft. %]
1	Y	-.061	5.5
2	Y	-.061	.5
3	Y	-.038	5.917
4	Y	-.038	2.083
5	Y	-.047	4.917

**Member Point Loads (BLC 28 : Ice Wind Load (0)) (Continued)**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft.%]
6	Y	-047	1.083
7	Y	-015	4.5
8	Y	-015	1.5
9	Y	-01	3
10	Y	-014	3
11	Y	-01	3
12	Y	-005	3
13	Y	-015	3

**Member Point Loads (BLC 29 : Ice Wind Load (30))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft.%]
1	Y	-045	5.5
2	Y	-045	.5
3	X	-026	5.5
4	X	-026	.5
5	Y	-029	5.917
6	Y	-029	2.083
7	X	-017	5.917
8	X	-017	2.083
9	Y	-035	4.917
10	Y	-035	1.083
11	X	-02	4.917
12	X	-02	1.083
13	Y	-011	4.5
14	Y	-011	1.5
15	X	-007	4.5
16	X	-007	1.5
17	Y	-008	3
18	X	-005	3
19	Y	-011	3
20	X	-006	3
21	Y	-008	3
22	X	-004	3
23	Y	-004	3
24	X	-002	3
25	Y	-013	3
26	X	-007	3

**Member Point Loads (BLC 30 : Ice Wind Load (60))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft.%]
1	Y	-018	5.5
2	Y	-018	.5
3	X	-031	5.5
4	X	-031	.5
5	Y	-013	5.917
6	Y	-013	2.083
7	X	-022	5.917
8	X	-022	2.083
9	Y	-014	4.917
10	Y	-014	1.083
11	X	-024	4.917
12	X	-024	1.083
13	Y	-005	4.5
14	Y	-005	1.5
15	X	-008	4.5
16	X	-008	1.5

**Member Point Loads (BLC 30 : Ice Wind Load (60)) (Continued)**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
17	Y	-.004	3
18	X	-.007	3
19	Y	-.005	3
20	X	-.009	3
21	Y	-.004	3
22	X	-.006	3
23	Y	-.002	3
24	X	-.003	3
25	Y	-.007	3
26	X	-.013	3

**Member Point Loads (BLC 31 : Ice Wind Load (90))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
1	X	-.027	5.5
2	X	-.027	.5
3	X	-.022	5.917
4	X	-.022	2.083
5	X	-.021	4.917
6	X	-.021	1.083
7	X	-.008	4.5
8	X	-.008	1.5
9	X	-.008	3
10	X	-.009	3
11	X	-.006	3
12	X	-.004	3
13	X	-.015	3

**Member Point Loads (BLC 32 : Ice Wind Load (120))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft. %]
1	Y	.018	5.5
2	Y	.018	.5
3	X	-.031	5.5
4	X	-.031	.5
5	Y	.013	5.917
6	Y	.013	2.083
7	X	-.022	5.917
8	X	-.022	2.083
9	Y	.014	4.917
10	Y	.014	1.083
11	X	-.024	4.917
12	X	-.024	1.083
13	Y	.005	4.5
14	Y	.005	1.5
15	X	-.008	4.5
16	X	-.008	1.5
17	Y	.004	3
18	X	-.007	3
19	Y	.005	3
20	X	-.009	3
21	Y	.004	3
22	X	-.006	3
23	Y	.002	3
24	X	-.003	3
25	Y	.007	3
26	X	-.013	3

**Member Point Loads (BLC 33 : Ice Wind Load (150))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft.%]
1	Y	.045	5.5
2	Y	.045	.5
3	X	-.026	5.5
4	X	-.026	.5
5	Y	.029	5.917
6	Y	.029	2.083
7	X	-.017	5.917
8	X	-.017	2.083
9	Y	.035	4.917
10	Y	.035	1.083
11	X	-.02	4.917
12	X	-.02	1.083
13	Y	.011	4.5
14	Y	.011	1.5
15	X	-.007	4.5
16	X	-.007	1.5
17	Y	.008	3
18	X	-.005	3
19	Y	.011	3
20	X	-.006	3
21	Y	.008	3
22	X	-.004	3
23	Y	.004	3
24	X	-.002	3
25	Y	.013	3
26	X	-.007	3

**Member Point Loads (BLC 34 : Ice Wind Load (180))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft.%]
1	Y	.061	5.5
2	Y	.061	.5
3	Y	.038	5.917
4	Y	.038	2.083
5	Y	.047	4.917
6	Y	.047	1.083
7	Y	.015	4.5
8	Y	.015	1.5
9	Y	.01	3
10	Y	.014	3
11	Y	.01	3
12	Y	.005	3
13	Y	.015	3

**Member Point Loads (BLC 35 : Ice Wind Load (210))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft.%]
1	Y	.045	5.5
2	Y	.045	.5
3	X	.026	5.5
4	X	.026	.5
5	Y	.029	5.917
6	Y	.029	2.083
7	X	.017	5.917
8	X	.017	2.083
9	Y	.035	4.917
10	Y	.035	1.083
11	X	.02	4.917

**Member Point Loads (BLC 35 : Ice Wind Load (210)) (Continued)**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft.%]
12	X	.02	1.083
13	Y	.011	4.5
14	Y	.011	1.5
15	X	.007	4.5
16	X	.007	1.5
17	Y	.008	3
18	X	.005	3
19	Y	.011	3
20	X	.006	3
21	Y	.008	3
22	X	.004	3
23	Y	.004	3
24	X	.002	3
25	Y	.013	3
26	X	.007	3

**Member Point Loads (BLC 36 : Ice Wind Load (240))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft.%]
1	Y	.018	5.5
2	Y	.018	.5
3	X	.031	5.5
4	X	.031	.5
5	Y	.013	5.917
6	Y	.013	2.083
7	X	.022	5.917
8	X	.022	2.083
9	Y	.014	4.917
10	Y	.014	1.083
11	X	.024	4.917
12	X	.024	1.083
13	Y	.005	4.5
14	Y	.005	1.5
15	X	.008	4.5
16	X	.008	1.5
17	Y	.004	3
18	X	.007	3
19	Y	.005	3
20	X	.009	3
21	Y	.004	3
22	X	.006	3
23	Y	.002	3
24	X	.003	3
25	Y	.007	3
26	X	.013	3

**Member Point Loads (BLC 37 : Ice Wind Load (270))**

Member Label	Direction	Magnitude[k.k-ft]	Location[ft.%]
1	X	.027	5.5
2	X	.027	.5
3	X	.022	5.917
4	X	.022	2.083
5	X	.021	4.917
6	X	.021	1.083
7	X	.008	4.5
8	X	.008	1.5
9	X	.008	3

**Member Point Loads (BLC 37 : Ice Wind Load (270)) (Continued)**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
10	X	.009	3
11	X	.006	3
12	X	.004	3
13	X	.015	3

**Member Point Loads (BLC 38 : Ice Wind Load (300))**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
1	Y	-.018	5.5
2	Y	-.018	.5
3	X	.031	5.5
4	X	.031	.5
5	Y	-.013	5.917
6	Y	-.013	2.083
7	X	.022	5.917
8	X	.022	2.083
9	Y	-.014	4.917
10	Y	-.014	1.083
11	X	.024	4.917
12	X	.024	1.083
13	Y	-.005	4.5
14	Y	-.005	1.5
15	X	.008	4.5
16	X	.008	1.5
17	X	-.004	3
18	X	.007	3
19	Y	-.005	3
20	X	.009	3
21	Y	-.004	3
22	X	.006	3
23	Y	-.002	3
24	X	.003	3
25	Y	-.007	3
26	X	.013	3

**Member Point Loads (BLC 39 : Ice Wind Load (330))**

Member Label	Direction	Magnitude[k, k-ft]	Location[ft, %]
1	Y	-.045	5.5
2	Y	-.045	.5
3	X	.026	5.5
4	X	.026	.5
5	Y	-.029	5.917
6	Y	-.029	2.083
7	X	.017	5.917
8	X	.017	2.083
9	Y	-.035	4.917
10	Y	-.035	1.083
11	X	.02	4.917
12	X	.02	1.083
13	Y	-.011	4.5
14	Y	-.011	1.5
15	X	.007	4.5
16	X	.007	1.5
17	Y	-.008	3
18	X	.005	3
19	Y	-.011	3
20	X	.006	3



**Member Point Loads (BLC 39 : Ice Wind Load (330)) (Continued)**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
21	Y	-.008	3
22	X	.004	3
23	Y	-.004	3
24	X	.002	3
25	Y	-.013	3
26	X	.007	3

**Member Point Loads (BLC 40 : Earthquake (x-direction))**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	X	-.006	5.5
2	X	-.006	.5
3	X	-.003	5.917
4	X	-.003	2.083
5	X	-.004	4.917
6	X	-.004	1.083
7	X	-.002	4.5
8	X	-.002	1.5
9	X	-.008	3
10	X	-.006	3
11	X	-.007	3
12	X	-.001	3
13	X	-.004	3

**Member Point Loads (BLC 41 : Earthquake (y-direction))**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	Y	-.006	5.5
2	Y	-.006	.5
3	Y	-.003	5.917
4	Y	-.003	2.083
5	Y	-.004	4.917
6	Y	-.004	1.083
7	Y	-.002	4.5
8	Y	-.002	1.5
9	Y	-.008	3
10	Y	-.006	3
11	Y	-.007	3
12	Y	-.001	3
13	Y	-.004	3

**Member Point Loads (BLC 42 : Earthquake (z-direction))**

Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	Z	-.002	5.5
2	Z	-.002	.5
3	Z	-.001	5.917
4	Z	-.001	2.083
5	Z	-.001	4.917
6	Z	-.001	1.083
7	Z	-.000791	4.5
8	Z	-.000791	1.5
9	Z	-.003	3
10	Z	-.002	3
11	Z	-.003	3
12	Z	-.000479	3
13	Z	-.001	3

**Member Distributed Loads (BLC 2 : Wind Load (0))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,....,Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PY	-01	0
2	TOPFACE	PY	-01	0
3	TC1	PY	-007	0
4	TC2	PY	-007	0
5	SUPPIPE	PY	-003	0
6	MP ALPHA1	PY	-009	0
7	MP ALPHA2	PY	-009	0
8	MP ALPHA3	PY	-009	0
9	MP ALPHA4	PY	-009	0
10	TB PIPE	PY	-009	0
11	TIEBACK	PY	-003	0

**Member Distributed Loads (BLC 4 : Wind Load (30))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,....,Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PY	-009	0
2	TOPFACE	PY	-009	0
3	TC1	PY	-006	0
4	TC2	PY	-006	0
5	SUPPIPE	PY	-003	0
6	MP ALPHA1	PY	-008	0
7	MP ALPHA2	PY	-008	0
8	MP ALPHA3	PY	-008	0
9	MP ALPHA4	PY	-008	0
10	TB PIPE	PY	-008	0
11	TIEBACK	PY	-003	0
12	BOTTOMFACE	PX	-005	0
13	TOPFACE	PX	-005	0
14	TC1	PX	-004	0
15	TC2	PX	-004	0
16	SUPPIPE	PX	-002	0
17	MP ALPHA1	PX	-004	0
18	MP ALPHA2	PX	-004	0
19	MP ALPHA3	PX	-004	0
20	MP ALPHA4	PX	-004	0
21	TB PIPE	PX	-004	0

**Member Distributed Loads (BLC 5 : Wind Load (60))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,....,Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PY	-005	0
2	TOPFACE	PY	-005	0
3	TC1	PY	-004	0
4	TC2	PY	-004	0
5	SUPPIPE	PY	-002	0
6	MP ALPHA1	PY	-004	0
7	MP ALPHA2	PY	-004	0
8	MP ALPHA3	PY	-004	0
9	MP ALPHA4	PY	-004	0
10	TB PIPE	PY	-004	0
11	TIEBACK	PY	-002	0
12	BOTTOMFACE	PX	-009	0
13	TOPFACE	PX	-009	0
14	TC1	PX	-006	0
15	TC2	PX	-006	0
16	SUPPIPE	PX	-003	0
17	MP ALPHA1	PX	-008	0

**Member Distributed Loads (BLC 5 : Wind Load (60)) (Continued)**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft....,Start Location[ft, %]	Start Location[ft, %]	End Location[ft, %]
18	MP ALPHA2	PX	-008	0	0
19	MP ALPHA3	PX	-008	0	0
20	MP ALPHA4	PX	-008	0	0
21	TB PIPE	PX	-008	0	0

**Member Distributed Loads (BLC 6 : Wind Load (90))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft....,Start Location[ft, %]	Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PX	-003	0	0
2	TOPFACE	PX	-003	0	0
3	TC1	PX	-007	0	0
4	TC2	PX	-007	0	0
5	SUPPIPE	PX	-003	0	0
6	MP ALPHA1	PX	-009	0	0
7	MP ALPHA2	PX	-009	0	0
8	MP ALPHA3	PX	-009	0	0
9	MP ALPHA4	PX	-009	0	0
10	TB PIPE	PX	-009	0	0
11	TIEBACK	PX	-003	0	0

**Member Distributed Loads (BLC 7 : Wind Load (120))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft....,Start Location[ft, %]	Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PY	.005	0	0
2	TOPFACE	PY	.005	0	0
3	TC1	PY	.004	0	0
4	TC2	PY	.004	0	0
5	SUPPIPE	PY	.002	0	0
6	MP ALPHA1	PY	.004	0	0
7	MP ALPHA2	PY	.004	0	0
8	MP ALPHA3	PY	.004	0	0
9	MP ALPHA4	PY	.004	0	0
10	TB PIPE	PY	.004	0	0
11	TIEBACK	PY	.002	0	0
12	BOTTOMFACE	PX	-009	0	0
13	TOPFACE	PX	-009	0	0
14	TC1	PX	-006	0	0
15	TC2	PX	-006	0	0
16	SUPPIPE	PX	-003	0	0
17	MP ALPHA1	PX	-008	0	0
18	MP ALPHA2	PX	-008	0	0
19	MP ALPHA3	PX	-008	0	0
20	MP ALPHA4	PX	-008	0	0
21	TB PIPE	PX	-008	0	0

**Member Distributed Loads (BLC 8 : Wind Load (150))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft....,Start Location[ft, %]	Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PY	.009	0	0
2	TOPFACE	PY	.009	0	0
3	TC1	PY	.006	0	0
4	TC2	PY	.006	0	0
5	SUPPIPE	PY	.003	0	0
6	MP ALPHA1	PY	.008	0	0
7	MP ALPHA2	PY	.008	0	0
8	MP ALPHA3	PY	.008	0	0
9	MP ALPHA4	PY	.008	0	0
10	TB PIPE	PY	.008	0	0

**Member Distributed Loads (BLC 8 : Wind Load (150)) (Continued)**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft... Start Location[ft, %]	End Location[ft, %]
11	TIEBACK	.003	.003	0
12	BOTTOMFACE	-.005	-.005	0
13	TOPFACE	-.005	-.005	0
14	TC1	-.004	-.004	0
15	TC2	-.004	-.004	0
16	SUPPIPE	-.002	-.002	0
17	MP ALPHA1	-.004	-.004	0
18	MP ALPHA2	-.004	-.004	0
19	MP ALPHA3	-.004	-.004	0
20	MP ALPHA4	-.004	-.004	0
21	TB PIPE	-.004	-.004	0

**Member Distributed Loads (BLC 9 : Wind Load (180))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft... Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	.01	.01	0
2	TOPFACE	.01	.01	0
3	TC1	.007	.007	0
4	TC2	.007	.007	0
5	SUPPIPE	.003	.003	0
6	MP ALPHA1	.009	.009	0
7	MP ALPHA2	.009	.009	0
8	MP ALPHA3	.009	.009	0
9	MP ALPHA4	.009	.009	0
10	TB PIPE	.009	.009	0
11	TIEBACK	.003	.003	0

**Member Distributed Loads (BLC 10 : Wind Load (210))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft... Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	.009	.009	0
2	TOPFACE	.009	.009	0
3	TC1	.006	.006	0
4	TC2	.006	.006	0
5	SUPPIPE	.003	.003	0
6	MP ALPHA1	.008	.008	0
7	MP ALPHA2	.008	.008	0
8	MP ALPHA3	.008	.008	0
9	MP ALPHA4	.008	.008	0
10	TB PIPE	.008	.008	0
11	TIEBACK	.003	.003	0
12	BOTTOMFACE	.005	.005	0
13	TOPFACE	.005	.005	0
14	TC1	.004	.004	0
15	TC2	.004	.004	0
16	SUPPIPE	.002	.002	0
17	MP ALPHA1	.004	.004	0
18	MP ALPHA2	.004	.004	0
19	MP ALPHA3	.004	.004	0
20	MP ALPHA4	.004	.004	0
21	TB PIPE	.004	.004	0

**Member Distributed Loads (BLC 11 : Wind Load (240))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft... Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	.005	.005	0
2	TOPFACE	.005	.005	0
3	TC1	.004	.004	0

**Member Distributed Loads (BLC 11 : Wind Load (240)) (Continued)**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,F,ksf]	Start Location[ft,%]	End Location[ft,%]
4	TC2	PY	.004	0	0
5	SUPPIPE	PY	.002	0	0
6	MP ALPHA1	PY	.004	0	0
7	MP ALPHA2	PY	.004	0	0
8	MP ALPHA3	PY	.004	0	0
9	MP ALPHA4	PY	.004	0	0
10	TB PIPE	PY	.004	0	0
11	TIEBACK	PY	.002	0	0
12	BOTTOMFACE	PX	.009	0	0
13	TOPFACE	PX	.009	0	0
14	TC1	PX	.006	0	0
15	TC2	PX	.006	0	0
16	SUPPIPE	PX	.003	0	0
17	MP ALPHA1	PX	.008	0	0
18	MP ALPHA2	PX	.008	0	0
19	MP ALPHA3	PX	.008	0	0
20	MP ALPHA4	PX	.008	0	0
21	TB PIPE	PX	.008	0	0

**Member Distributed Loads (BLC 12 : Wind Load (270))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,F,ksf]	Start Location[ft,%]	End Location[ft,%]
1	BOTTOMFACE	PX	.003	0	0
2	TOPFACE	PX	.003	0	0
3	TC1	PX	.007	0	0
4	TC2	PX	.007	0	0
5	SUPPIPE	PX	.003	0	0
6	MP ALPHA1	PX	.009	0	0
7	MP ALPHA2	PX	.009	0	0
8	MP ALPHA3	PX	.009	0	0
9	MP ALPHA4	PX	.009	0	0
10	TB PIPE	PX	.009	0	0
11	TIEBACK	PX	.003	0	0

**Member Distributed Loads (BLC 13 : Wind Load (300))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,F,ksf]	Start Location[ft,%]	End Location[ft,%]
1	BOTTOMFACE	PY	-.005	0	0
2	TOPFACE	PY	-.005	0	0
3	TC1	PY	-.004	0	0
4	TC2	PY	-.004	0	0
5	SUPPIPE	PY	-.002	0	0
6	MP ALPHA1	PY	-.004	0	0
7	MP ALPHA2	PY	-.004	0	0
8	MP ALPHA3	PY	-.004	0	0
9	MP ALPHA4	PY	-.004	0	0
10	TB PIPE	PY	-.004	0	0
11	TIEBACK	PY	-.002	0	0
12	BOTTOMFACE	PX	.009	0	0
13	TOPFACE	PX	.009	0	0
14	TC1	PX	.006	0	0
15	TC2	PX	.006	0	0
16	SUPPIPE	PX	.003	0	0
17	MP ALPHA1	PX	.008	0	0
18	MP ALPHA2	PX	.008	0	0
19	MP ALPHA3	PX	.008	0	0
20	MP ALPHA4	PX	.008	0	0
21	TB PIPE	PX	.008	0	0

**Member Distributed Loads (BLC 14 : Wind Load (330))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft... Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PY	-0.09	0
2	TOPFACE	PY	-0.09	0
3	TC1	PY	-0.06	0
4	TC2	PY	-0.06	0
5	SUPPIPE	PY	-0.03	0
6	MP ALPHA1	PY	-0.08	0
7	MP ALPHA2	PY	-0.08	0
8	MP ALPHA3	PY	-0.08	0
9	MP ALPHA4	PY	-0.08	0
10	TB PIPE	PY	-0.08	0
11	TIEBACK	PY	-0.03	0
12	BOTTOMFACE	PX	.005	0
13	TOPFACE	PX	.005	0
14	TC1	PX	.004	0
15	TC2	PX	.004	0
16	SUPPIPE	PX	.002	0
17	MP ALPHA1	PX	.004	0
18	MP ALPHA2	PX	.004	0
19	MP ALPHA3	PX	.004	0
20	MP ALPHA4	PX	.004	0
21	TB PIPE	PX	.004	0

**Member Distributed Loads (BLC 27 : Ice Dead Load)**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft... Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	Z	-0.06	0
2	TOPFACE	Z	-0.06	0
3	TC1	Z	-0.1	0
4	TC2	Z	-0.1	0
5	SUPPIPE	Z	-0.05	0
6	MP ALPHA1	Z	-0.05	0
7	MP ALPHA2	Z	-0.05	0
8	MP ALPHA3	Z	-0.05	0
9	MP ALPHA4	Z	-0.05	0
10	TB PIPE	Z	-0.05	0
11	TIEBACK	Z	-0.06	0

**Member Distributed Loads (BLC 28 : Ice Wind Load (0))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft... Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PY	-0.03	0
2	TOPFACE	PY	-0.03	0
3	TC1	PY	-0.02	0
4	TC2	PY	-0.02	0
5	SUPPIPE	PY	-0.01	0
6	MP ALPHA1	PY	-0.03	0
7	MP ALPHA2	PY	-0.03	0
8	MP ALPHA3	PY	-0.03	0
9	MP ALPHA4	PY	-0.03	0
10	TB PIPE	PY	-0.03	0
11	TIEBACK	PY	-0.01	0

**Member Distributed Loads (BLC 29 : Ice Wind Load (30))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft... Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PY	-0.03	0
2	TOPFACE	PY	-0.03	0
3	TC1	PY	-0.02	0

**Member Distributed Loads (BLC 29 : Ice Wind Load (30)) (Continued)**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,F,ksf]	Start Location[ft, %]	End Location[ft, %]
4	TC2	PY	-0.002	-0.002	0
5	SUPPIPE	PY	-0.000866	-0.000866	0
6	MP ALPHA1	PY	-0.003	-0.003	0
7	MP ALPHA2	PY	-0.003	-0.003	0
8	MP ALPHA3	PY	-0.003	-0.003	0
9	MP ALPHA4	PY	-0.003	-0.003	0
10	TB PIPE	PY	-0.003	-0.003	0
11	TIEBACK	PY	-0.000866	-0.000866	0
12	BOTTOMFACE	PX	-0.002	-0.002	0
13	TOPFACE	PX	-0.002	-0.002	0
14	TC1	PX	-0.001	-0.001	0
15	TC2	PX	-0.001	-0.001	0
16	SUPPIPE	PX	-0.0005	-0.0005	0
17	MP ALPHA1	PX	-0.002	-0.002	0
18	MP ALPHA2	PX	-0.002	-0.002	0
19	MP ALPHA3	PX	-0.002	-0.002	0
20	MP ALPHA4	PX	-0.002	-0.002	0
21	TB PIPE	PX	-0.002	-0.002	0

**Member Distributed Loads (BLC 30 : Ice Wind Load (60))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,F,ksf]	Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PY	-0.002	-0.002	0
2	TOPFACE	PY	-0.002	-0.002	0
3	TC1	PY	-0.001	-0.001	0
4	TC2	PY	-0.001	-0.001	0
5	SUPPIPE	PY	-0.0005	-0.0005	0
6	MP ALPHA1	PY	-0.002	-0.002	0
7	MP ALPHA2	PY	-0.002	-0.002	0
8	MP ALPHA3	PY	-0.002	-0.002	0
9	MP ALPHA4	PY	-0.002	-0.002	0
10	TB PIPE	PY	-0.002	-0.002	0
11	TIEBACK	PY	-0.0005	-0.0005	0
12	BOTTOMFACE	PX	-0.003	-0.003	0
13	TOPFACE	PX	-0.003	-0.003	0
14	TC1	PX	-0.002	-0.002	0
15	TC2	PX	-0.002	-0.002	0
16	SUPPIPE	PX	-0.000866	-0.000866	0
17	MP ALPHA1	PX	-0.003	-0.003	0
18	MP ALPHA2	PX	-0.003	-0.003	0
19	MP ALPHA3	PX	-0.003	-0.003	0
20	MP ALPHA4	PX	-0.003	-0.003	0
21	TB PIPE	PX	-0.003	-0.003	0

**Member Distributed Loads (BLC 31 : Ice Wind Load (90))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,F,ksf]	Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PX	-0.001	-0.001	0
2	TOPFACE	PX	-0.001	-0.001	0
3	TC1	PX	-0.002	-0.002	0
4	TC2	PX	-0.002	-0.002	0
5	SUPPIPE	PX	-0.001	-0.001	0
6	MP ALPHA1	PX	-0.003	-0.003	0
7	MP ALPHA2	PX	-0.003	-0.003	0
8	MP ALPHA3	PX	-0.003	-0.003	0
9	MP ALPHA4	PX	-0.003	-0.003	0
10	TB PIPE	PX	-0.003	-0.003	0
11	TIEBACK	PX	-0.001	-0.001	0

**Member Distributed Loads (BLC 32 : Ice Wind Load (120))**

Member Label	Direction	Start Magnitude[k/ft.F.ksf]	End Magnitude[k/ft.... Start Location[ft.%]	End Location[ft.%]
1	BOTTOMFACE	PY	.002	0
2	TOPFACE	PY	.002	0
3	TC1	PY	.001	0
4	TC2	PY	.001	0
5	SUPPIPE	PY	.0005	0
6	MP ALPHA1	PY	.002	0
7	MP ALPHA2	PY	.002	0
8	MP ALPHA3	PY	.002	0
9	MP ALPHA4	PY	.002	0
10	TB PIPE	PY	.002	0
11	TIEBACK	PY	.0005	0
12	BOTTOMFACE	PX	-.003	0
13	TOPFACE	PX	-.003	0
14	TC1	PX	-.002	0
15	TC2	PX	-.002	0
16	SUPPIPE	PX	-.000866	0
17	MP ALPHA1	PX	-.003	0
18	MP ALPHA2	PX	-.003	0
19	MP ALPHA3	PX	-.003	0
20	MP ALPHA4	PX	-.003	0
21	TB PIPE	PX	-.003	0

**Member Distributed Loads (BLC 33 : Ice Wind Load (150))**

Member Label	Direction	Start Magnitude[k/ft.F.ksf]	End Magnitude[k/ft.... Start Location[ft.%]	End Location[ft.%]
1	BOTTOMFACE	PY	.003	0
2	TOPFACE	PY	.003	0
3	TC1	PY	.002	0
4	TC2	PY	.002	0
5	SUPPIPE	PY	.000866	0
6	MP ALPHA1	PY	.003	0
7	MP ALPHA2	PY	.003	0
8	MP ALPHA3	PY	.003	0
9	MP ALPHA4	PY	.003	0
10	TB PIPE	PY	.003	0
11	TIEBACK	PY	.000866	0
12	BOTTOMFACE	PX	-.002	0
13	TOPFACE	PX	-.002	0
14	TC1	PX	-.001	0
15	TC2	PX	-.001	0
16	SUPPIPE	PX	-.0005	0
17	MP ALPHA1	PX	-.002	0
18	MP ALPHA2	PX	-.002	0
19	MP ALPHA3	PX	-.002	0
20	MP ALPHA4	PX	-.002	0
21	TB PIPE	PX	-.002	0

**Member Distributed Loads (BLC 34 : Ice Wind Load (180))**

Member Label	Direction	Start Magnitude[k/ft.F.ksf]	End Magnitude[k/ft.... Start Location[ft.%]	End Location[ft.%]
1	BOTTOMFACE	PY	.003	0
2	TOPFACE	PY	.003	0
3	TC1	PY	.002	0
4	TC2	PY	.002	0
5	SUPPIPE	PY	.001	0
6	MP ALPHA1	PY	.003	0
7	MP ALPHA2	PY	.003	0
8	MP ALPHA3	PY	.003	0



**Member Distributed Loads (BLC 34 : Ice Wind Load (180)) (Continued)**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,...,Start Location[ft, %]	Start Location[ft, %]	End Location[ft, %]
9	MP ALPHA4	PY	.003	0	0
10	TB PIPE	PY	.003	0	0
11	TIEBACK	PY	.001	0	0

**Member Distributed Loads (BLC 35 : Ice Wind Load (210))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,...,Start Location[ft, %]	Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PY	.003	0	0
2	TOPFACE	PY	.003	0	0
3	TC1	PY	.002	0	0
4	TC2	PY	.002	0	0
5	SUPPIPE	PY	.000866	0	0
6	MP ALPHA1	PY	.003	0	0
7	MP ALPHA2	PY	.003	0	0
8	MP ALPHA3	PY	.003	0	0
9	MP ALPHA4	PY	.003	0	0
10	TB PIPE	PY	.003	0	0
11	TIEBACK	PY	.000866	0	0
12	BOTTOMFACE	PX	.002	0	0
13	TOPFACE	PX	.002	0	0
14	TC1	PX	.001	0	0
15	TC2	PX	.001	0	0
16	SUPPIPE	PX	.0005	0	0
17	MP ALPHA1	PX	.002	0	0
18	MP ALPHA2	PX	.002	0	0
19	MP ALPHA3	PX	.002	0	0
20	MP ALPHA4	PX	.002	0	0
21	TB PIPE	PX	.002	0	0

**Member Distributed Loads (BLC 36 : Ice Wind Load (240))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,...,Start Location[ft, %]	Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PY	.002	0	0
2	TOPFACE	PY	.002	0	0
3	TC1	PY	.001	0	0
4	TC2	PY	.001	0	0
5	SUPPIPE	PY	.0005	0	0
6	MP ALPHA1	PY	.002	0	0
7	MP ALPHA2	PY	.002	0	0
8	MP ALPHA3	PY	.002	0	0
9	MP ALPHA4	PY	.002	0	0
10	TB PIPE	PY	.002	0	0
11	TIEBACK	PY	.0005	0	0
12	BOTTOMFACE	PX	.003	0	0
13	TOPFACE	PX	.003	0	0
14	TC1	PX	.002	0	0
15	TC2	PX	.002	0	0
16	SUPPIPE	PX	.000866	0	0
17	MP ALPHA1	PX	.003	0	0
18	MP ALPHA2	PX	.003	0	0
19	MP ALPHA3	PX	.003	0	0
20	MP ALPHA4	PX	.003	0	0
21	TB PIPE	PX	.003	0	0

**Member Distributed Loads (BLC 37 : Ice Wind Load (270))**

Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/ft,...,Start Location[ft, %]	Start Location[ft, %]	End Location[ft, %]
1	BOTTOMFACE	PX	.001	0	0

**Member Distributed Loads (BLC 37 : Ice Wind Load (270)) (Continued)**

Member Label	Direction	Start Magnitude[k/ft.F.ksf]	End Magnitude[k/ft.... Start Location[ft.%]	Start Location[ft.%]	End Location[ft.%]
2	TOPFACE	.001	.001	0	0
3	TC1	.002	.002	0	0
4	TC2	.002	.002	0	0
5	SUPPIPE	.001	.001	0	0
6	MP ALPHA1	.003	.003	0	0
7	MP ALPHA2	.003	.003	0	0
8	MP ALPHA3	.003	.003	0	0
9	MP ALPHA4	.003	.003	0	0
10	TB PIPE	.003	.003	0	0
11	TIEBACK	.001	.001	0	0

**Member Distributed Loads (BLC 38 : Ice Wind Load (300))**

Member Label	Direction	Start Magnitude[k/ft.F.ksf]	End Magnitude[k/ft.... Start Location[ft.%]	Start Location[ft.%]	End Location[ft.%]
1	BOTTOMFACE	-.002	-.002	0	0
2	TOPFACE	-.002	-.002	0	0
3	TC1	-.001	-.001	0	0
4	TC2	-.001	-.001	0	0
5	SUPPIPE	-.0005	-.0005	0	0
6	MP ALPHA1	-.002	-.002	0	0
7	MP ALPHA2	-.002	-.002	0	0
8	MP ALPHA3	-.002	-.002	0	0
9	MP ALPHA4	-.002	-.002	0	0
10	TB PIPE	-.002	-.002	0	0
11	TIEBACK	-.0005	-.0005	0	0
12	BOTTOMFACE	.003	.003	0	0
13	TOPFACE	.003	.003	0	0
14	TC1	.002	.002	0	0
15	TC2	.002	.002	0	0
16	SUPPIPE	.000866	.000866	0	0
17	MP ALPHA1	.003	.003	0	0
18	MP ALPHA2	.003	.003	0	0
19	MP ALPHA3	.003	.003	0	0
20	MP ALPHA4	.003	.003	0	0
21	TB PIPE	.003	.003	0	0

**Member Distributed Loads (BLC 39 : Ice Wind Load (330))**

Member Label	Direction	Start Magnitude[k/ft.F.ksf]	End Magnitude[k/ft.... Start Location[ft.%]	Start Location[ft.%]	End Location[ft.%]
1	BOTTOMFACE	-.003	-.003	0	0
2	TOPFACE	-.003	-.003	0	0
3	TC1	-.002	-.002	0	0
4	TC2	-.002	-.002	0	0
5	SUPPIPE	-.000866	-.000866	0	0
6	MP ALPHA1	-.003	-.003	0	0
7	MP ALPHA2	-.003	-.003	0	0
8	MP ALPHA3	-.003	-.003	0	0
9	MP ALPHA4	-.003	-.003	0	0
10	TB PIPE	-.003	-.003	0	0
11	TIEBACK	-.000866	-.000866	0	0
12	BOTTOMFACE	.002	.002	0	0
13	TOPFACE	.002	.002	0	0
14	TC1	.001	.001	0	0
15	TC2	.001	.001	0	0
16	SUPPIPE	.0005	.0005	0	0
17	MP ALPHA1	.002	.002	0	0
18	MP ALPHA2	.002	.002	0	0
19	MP ALPHA3	.002	.002	0	0

**Member Distributed Loads (BLC 39 : Ice Wind Load (330)) (Continued)**

Member Label	Direction	Start Magnitude[k/ft.F,ksf]	End Magnitude[k/ft... Start Location[ft.%]	Start Location[ft.%]	End Location[ft.%]
20 MP ALPHA4	PX	.002	.002	0	0
21 TB PIPE	PX	.002	.002	0	0

**Envelope Joint Reactions**

Joint	X [k]			Y [k]			Z [k]			LC MX [k-ft]			LC MY [k-ft]			LC MZ [k-ft]			LC
	max	min		max	min		max	min		max	min		max	min		max	min		
1 N1	.973	10	1.273	2	1.168	15	.081	2	.627	25	0								41
2	max	-66	-1.108	20	.443	35	-1	21	-.388	5	0								1
3 N7	max	.744	14	1.23	2	1.164	.045	20	.615	19	0								41
4	min	-.979	34	-1.411	20	.444	17	-.987	3	-.394	35	0							1
5 N53	max	.139	32	.299	32	.04	33	0	41	0	41	0							41
6	min	-.138	14	-.297	14	.017	14	0	1	0	1	0							1
7 Totals:	max	1.279	8	2.558	2	2.35	18												
8	min	-1.279	26	-2.558	20	1.012	35												

**Basic Load Cases**

BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed Area(Me... Surface(...
1 Live Load	DL					1	
2 Wind Load (0)	DL					13	11
3 Dead Load	DL			-1.1		13	
4 Wind Load (30)	DL					26	21
5 Wind Load (60)	DL					26	21
6 Wind Load (90)	DL					13	11
7 Wind Load (120)	DL					26	21
8 Wind Load (150)	DL					26	21
9 Wind Load (180)	DL					13	11
10 Wind Load (210)	DL					26	21
11 Wind Load (240)	DL					26	21
12 Wind Load (270)	DL					13	11
13 Wind Load (300)	DL					26	21
14 Wind Load (330)	DL					26	21
15 Maintenance (0)	DL					13	
16 Maintenance (30)	DL					26	
17 Maintenance (60)	DL					26	
18 Maintenance (90)	DL					13	
19 Maintenance (120)	DL					26	
20 Maintenance (150)	DL					26	
21 Maintenance (180)	DL					13	
22 Maintenance (210)	DL					26	
23 Maintenance (240)	DL					26	
24 Maintenance (270)	DL					13	
25 Maintenance (300)	DL					26	
26 Maintenance (330)	DL					26	
27 Ice Dead Load	DL					13	11
28 Ice Wind Load (0)	DL					13	11
29 Ice Wind Load (30)	DL					26	21
30 Ice Wind Load (60)	DL					26	21
31 Ice Wind Load (90)	DL					13	11
32 Ice Wind Load (120)	DL					26	21
33 Ice Wind Load (150)	DL					26	21
34 Ice Wind Load (180)	DL					13	11
35 Ice Wind Load (210)	DL					26	21
36 Ice Wind Load (240)	DL					26	21
37 Ice Wind Load (270)	DL					13	11
38 Ice Wind Load (300)	DL					26	21

**Basic Load Cases (Continued)**

BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed Area(Me... Surface(...
39 Ice Wind Load (330)	DL					26	21
40 Earthquake (x-directi...	DL	-.124				13	
41 Earthquake (y-directi...	DL		-.124			13	
42 Earthquake (z-directi...	DL			-.05		13	

**Load Combinations**

Description	Solve	PDelta	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1 1.4D	Yes	Y	3	1.4															
2 1.2D + 1.0W(0)	Yes	Y	3	1.2	2	1													
3 1.2D + 1.0Di + 1.0Wf(...	Yes	Y	3	1.2	27	1	28	1											
4 1.2D + 1.5L + 1.0W(0)	Yes	Y	3	1.2	1	1.5	15	1											
5 1.2D + 1.0W(30)	Yes	Y	3	1.2	4	1													
6 1.2D + 1.0Di + 1.0Wf(...	Yes	Y	3	1.2	27	1	29	1											
7 1.2D + 1.5L + 1.0W(...	Yes	Y	3	1.2	1	1.5	16	1											
8 1.2D + 1.0W(60)	Yes	Y	3	1.2	5	1													
9 1.2D + 1.0Di + 1.0Wf(...	Yes	Y	3	1.2	27	1	30	1											
10 1.2D + 1.5L + 1.0W(...	Yes	Y	3	1.2	1	1.5	17	1											
11 1.2D + 1.0W(90)	Yes	Y	3	1.2	6	1													
12 1.2D + 1.0Di + 1.0Wf(...	Yes	Y	3	1.2	27	1	31	1											
13 1.2D + 1.5L + 1.0W(...	Yes	Y	3	1.2	1	1.5	18	1											
14 1.2D + 1.0W(120)	Yes	Y	3	1.2	7	1													
15 1.2D + 1.0Di + 1.0Wf(...	Yes	Y	3	1.2	27	1	32	1											
16 1.2D + 1.5L + 1.0W(...	Yes	Y	3	1.2	1	1.5	19	1											
17 1.2D + 1.0W(150)	Yes	Y	3	1.2	8	1													
18 1.2D + 1.0Di + 1.0Wf(...	Yes	Y	3	1.2	27	1	33	1											
19 1.2D + 1.5L + 1.0W(...	Yes	Y	3	1.2	1	1.5	20	1											
20 1.2D + 1.0W(180)	Yes	Y	3	1.2	9	1													
21 1.2D + 1.0Di + 1.0Wf(...	Yes	Y	3	1.2	27	1	34	1											
22 1.2D + 1.5L + 1.0W(...	Yes	Y	3	1.2	1	1.5	21	1											
23 1.2D + 1.0W(210)	Yes	Y	3	1.2	10	1													
24 1.2D + 1.0Di + 1.0Wf(...	Yes	Y	3	1.2	27	1	35	1											
25 1.2D + 1.5L + 1.0W(...	Yes	Y	3	1.2	1	1.5	22	1											
26 1.2D + 1.0W(240)	Yes	Y	3	1.2	11	1													
27 1.2D + 1.0Di + 1.0Wf(...	Yes	Y	3	1.2	27	1	36	1											
28 1.2D + 1.5L + 1.0W(...	Yes	Y	3	1.2	1	1.5	23	1											
29 1.2D + 1.0W(270)	Yes	Y	3	1.2	12	1													
30 1.2D + 1.0Di + 1.0Wf(...	Yes	Y	3	1.2	27	1	37	1											
31 1.2D + 1.5L + 1.0W(...	Yes	Y	3	1.2	1	1.5	24	1											
32 1.2D + 1.0W(300)	Yes	Y	3	1.2	13	1													
33 1.2D + 1.0Di + 1.0Wf(...	Yes	Y	3	1.2	27	1	38	1											
34 1.2D + 1.5L + 1.0W(...	Yes	Y	3	1.2	1	1.5	25	1											
35 1.2D + 1.0W(330)	Yes	Y	3	1.2	14	1													
36 1.2D + 1.0Di + 1.0Wf(...	Yes	Y	3	1.2	27	1	39	1											
37 1.2D + 1.5L + 1.0W(...	Yes	Y	3	1.2	1	1.5	26	1											
38 1.2D + 1.0E(x) + 1.0E...	Yes	Y	3	1.2	40	1	42	1	1	1									
39 1.2D + 1.0E(y) + 1.0E...	Yes	Y	3	1.2	41	1	42	1	1	1									
40 1.2D - 1.0E(x) + 1.0E(...	Yes	Y	3	1.2	40	-1	42	1	1	1									
41 1.2D - 1.0E(y) + 1.0E(...	Yes	Y	3	1.2	41	-1	42	1	1	1									

**Envelope AISC 15th(360-16): LRFD Steel Code Checks**

Member	Shape	Code Check	Lo...	LC	She...	Loc[ft]	Dir	LC	phi*	phi*	phi*	phi*	phi*	Eqn
1 BOTTOMFACE	PIPE 2.5	.710	6.5	20	.160	6.5	20	13.46	50.7...	3.596	3.596	3.596	3.596	H1...
2 TOPFACE	PIPE 2.5	.681	6.5	5	.114	6.5	2	13.46	50.7...	3.596	3.596	3.596	3.596	H1...
3 MP ALPHA4	PIPE 2.0	.426	4.5	10	.083	3	19	20.8...	32.13	1.872	1.872	1.872	1.872	H1...



Company : POD Group  
 Designer : DP  
 Job Number : 20-64617  
 Model Name : 841294

May 28, 2020  
 5:05 PM  
 Checked By: \_\_\_\_\_

**Envelope AISC 15th(360-16): LRFD Steel Code Checks (Continued)**

Member	Shape	Code Check	Lo...	LC	She...Locifl	Dir	LC	phi*	phi*	phi*	phi*	phi*	Eqn
4	MP ALPHA3	.395	4.5	7	.082	1.5	22	20.8...	32.13	1.872	1.872	1.872	H1-...
5	MP ALPHA2	.391	2.5	20	.076	2.5	2	14.9...	32.13	1.872	1.872	1.872	H1-...
6	TB PIPE	.335	4...	16	.075	2.771	19	17.8...	32.13	1.872	1.872	1.872	H1-...
7	MP ALPHA1	.170	4.5	6	.032	1.5	6	20.8...	32.13	1.872	1.872	1.872	H1-...
8	SUPPIPE	.142	3.75	22	.138	.281	34	72.51	78.75	7.954	7.954	7.954	H1-...
9	TIEBACK	.043	3...	15	.004	0	30	16.0...	32.13	1.872	1.872	1.872	H1-...
10	TC1	.000	.25	21	.000	.25	z	36	92.3...	92.6...	4.398	9.886	H2-1
11	TC2	.000	.25	21	.000	.25	z	36	92.3...	92.6...	4.398	9.886	H2-1

## **APPENDIX D**

### **Wind Speed Documentation**



Search Information

Coordinates: 41.341856, -73.274522
Elevation: 587 ft
Timestamp: 2020-05-28T13:28:33.866Z
Hazard Type: Wind



Table with 3 columns: ASCE 7-16, ASCE 7-10, ASCE 7-05. Rows include MRI 10-Year, MRI 25-Year, MRI 50-Year, MRI 100-Year, Risk Category I, Risk Category II, Risk Category III, Risk Category IV.

You are in a wind-borne debris region if you are also within 1 mile of the coastal mean high water line.

The results indicated here DO NOT reflect any state or local amendments to the values or any delineation lines made during the building code adoption process. Users should confirm any output obtained from this tool with the local Authority Having Jurisdiction before proceeding with design.

Disclaimer

Hazard loads are interpolated from data provided in ASCE 7 and rounded up to the nearest whole integer. Per ASCE 7, islands and coastal areas outside the last contour should use the last wind speed contour of the coastal area - in some cases, this website will extrapolate past the last wind speed contour and therefore, provide a wind speed that is slightly higher. NOTE: For queries near wind-borne debris region boundaries, the resulting determination is sensitive to rounding which may affect whether or not it is considered to be within a wind-borne debris region.

Mountainous terrain, gorges, ocean promontories, and special wind regions shall be examined for unusual wind conditions.

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# Exhibit F

## **Power Density/RF Emissions Report**



# Fullerton Engineering Consultants, LLC.

RF Engineering & Consultant Services

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## Radio Frequency Emissions Analysis Report

**AT&T** Existing Facility

**Site ID: CTL02144**

Project Type: AT&T LTE 5C

Monroe\_Guinea Road  
230 Guinea Road  
Monroe, CT 06468

**July 13, 2020**

**Fullerton Project Number: 2020.0182.0014**

Site Compliance Summary	
Compliance Status:	<b>COMPLIANT</b>
Site total MPE% of FCC general population allowable limit:	<b>3.14 %</b>

# Fullerton Engineering Consultants, LLC.

RF Engineering & Consultant Services

July 13, 2020

Crown Castle on Behalf of AT&T  
Attn: Anne Marie Zsamba, Site Acquisition Specialist  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065

## Emissions Analysis for Site: **CTL02144 – Monroe\_Guinea Road**

Fullerton Engineering Consultants, LLC (“Fullerton”) was directed to analyze the proposed upgrades to the AT&T facility located at **230 Guinea Road, Monroe, CT**, for the purpose of determining whether the emissions from the proposed AT&T antenna installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The number of  $\mu\text{W}/\text{cm}^2$  calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The general population exposure limits for the 700 MHz & 850 MHz bands are approximately 467  $\mu\text{W}/\text{cm}^2$  and 567  $\mu\text{W}/\text{cm}^2$  respectively. The general population exposure limit for the 1900 MHz (PCS) band is 1000  $\mu\text{W}/\text{cm}^2$ . Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

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Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

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## CALCULATIONS

Calculations were performed for the proposed upgrades to the AT&T antenna facility located at **230 Guinea Road, Monroe, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since AT&T is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was focused at the base of the tower. For this report, the sample point is the top of a 6-foot person standing at the base of the tower.

Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. All power values expressed and analyzed are maximum power levels expected to be used on all radios.

All emissions values for additional carriers were taken from the Connecticut Siting Council (CSC) active MPE database. Values in this database are provided by the individual carriers themselves.

For each sector the following channel counts, frequency bands and power levels were utilized as shown in *Table 1*:

Technology	Frequency Band	Channel Count	Transmit Power per Channel (W)
UMTS	850 MHz	1	20
LTE	700 MHz (Band 12)	4	40
LTE / 5G NR	850 MHz	4	40
LTE	700 MHz (Band 14)	4	40
LTE	1900 MHz (PCS)	4	40

*Table 1: Channel Data Table*

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The following antennas listed in *Table 2* were used in the modeling for transmission in the 700 MHz, 850 MHz and 1900 MHz (PCS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

Sector	Antenna Number	Antenna Make / Model	Antenna Centerline (ft)
A	1	Powerwave 7770	230
A	2	CCI DMP65R-BU6D	230
A	3	CCI OPA65R-BU6D	230
A	4	CCI HPA-65R-BUU-H6	232
B	1	Powerwave 7770	230
B	2	CCI DMP65R-BU6D	230
B	3	CCI OPA65R-BU6D	230
B	4	CCI HPA-65R-BUU-H6	232
C	1	Powerwave 7770	230
C	2	CCI DMP65R-BU6D	230
C	3	CCI OPA65R-BU6D	230
C	4	CCI HPA-65R-BUU-H6	232

*Table 2: Antenna Data*

All calculations were done with respect to uncontrolled / general population threshold limits.

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Cable losses were factored in the calculations for this site. For each **700 MHz** Remote Radio Unit (RRU) there was **0.18 dB** of cable loss calculated into the system gains / losses for this site. For each **850 MHz** Remote Radio Unit (RRU) there was **0.20 dB** of cable loss calculated into the system gains / losses for this site. For each **850 MHz** ground mounted radio there was **1.60 dB** of cable loss calculated into the system gains / losses for this site. For each **1900 MHz (PCS)** Remote Radio Unit (RRU) there was **0.32 dB** of cable loss calculated into the system gains / losses for this site. These values were calculated based upon the manufacturers specifications for **10 feet of 1/2"** coax for all Remote Radio Units (RRU) and **260 feet of 1-5/8"** for all ground mounted radios.

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## RESULTS

Per the calculations completed for the proposed AT&T configurations *Table 3* shows resulting emissions power levels and percentages of the FCC’s allowable general population limit.

Antenna ID	Antenna Make / Model	Frequency Bands	Antenna Gain (dBd)	Channel Count	Total TX Power (W)	ERP (W)	MPE %
Antenna A1	Powerwave 7770	850 MHz	11.4	1	20	191.00	0.02
Antenna A2	CCI DMP65R-BU6D	700 MHz (Band 12) / 850 MHz	11.85 / 12.45	8	240	3,693.33	0.53
Antenna A3	CCI OPA65R-BU6D	700 MHz (Band 14)	11.85	4	160	2,518.37	0.39
Antenna A4	CCI HPA-65R-BUU-H6	1900 MHz (PCS)	15.95	4	160	5,849.52	0.41
Sector A Composite MPE%							
Antenna B1	Powerwave 7770	850 MHz	11.4	1	20	191.00	0.02
Antenna B2	CCI DMP65R-BU6D	700 MHz (Band 12) / 850 MHz	11.85 / 12.45	8	240	3,693.33	0.53
Antenna B3	CCI OPA65R-BU6D	700 MHz (Band 14)	11.85	4	160	2,518.37	0.39
Antenna B4	CCI HPA-65R-BUU-H6	1900 MHz (PCS)	15.95	4	160	5,849.52	0.41
Sector B Composite MPE%							
Antenna C1	Powerwave 7770	850 MHz	11.4	1	20	191.00	0.02
Antenna C2	CCI DMP65R-BU6D	700 MHz (Band 12) / 850 MHz	11.85 / 12.45	8	240	3,693.33	0.53
Antenna C3	CCI OPA65R-BU6D	700 MHz (Band 14)	11.85	4	160	2,518.37	0.39
Antenna C4	CCI HPA-65R-BUU-H6	1900 MHz (PCS)	15.95	4	160	5,849.52	0.41
Sector C Composite MPE%							
<b>1.35</b>							

*Table 3: AT&T Emissions Levels*

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The Following table (table 4) shows all additional carriers on site and their MPE% as recorded in the CSC active MPE database for this facility along with the newly calculated maximum AT&T MPE contributions per this report. FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. For this site, all three sectors have the same configuration yielding the same results on all three sectors. Table 5 below shows a summary for each AT&T Sector as well as the composite MPE value for the site.

Site Composite MPE%	
Carrier	MPE%
AT&T – Max Per Sector Value	1.35 %
PageNet	0.20 %
RAW Mobile Data	0.01 %
Nextel	0.25 %
CL&P	0.09 %
Verizon Wireless	1.24 %
<b>Site Total MPE %:</b>	<b>3.14 %</b>

Table 4: All Carrier MPE Contributions

AT&T Sector A Total:	1.35 %
AT&T Sector B Total:	1.35 %
AT&T Sector C Total:	1.35 %
<hr/>	
Site Total:	3.14 %

Table 5: Site MPE Summary



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FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. *Table 6* below details a breakdown by frequency band and technology for the MPE power values for the maximum calculated AT&T sector(s). For this site, all three sectors have the same configuration yielding the same results on all three sectors.

AT&T _ Frequency Band / Technology Max Power Values (Per Sector)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ( $\mu\text{W}/\text{cm}^2$ )	Frequency (MHz)	Allowable MPE ( $\mu\text{W}/\text{cm}^2$ )	Calculated % MPE
AT&T 850 MHz UMTS	1	191.00	230	0.14	850 MHz	567	0.02%
AT&T 700 MHz LTE (Band 12)	4	587.57	230	1.68	700 MHz	467	0.36%
AT&T 850 MHz LTE / 5G NR	4	335.76	230	0.96	850 MHz	567	0.17%
AT&T 700 MHz LTE (Band 14)	4	629.59	230	1.80	700 MHz	467	0.39%
AT&T 1900 MHz (PCS) LTE	4	1,462.38	232	4.12	1900 MHz (PCS)	1000	0.41%
<b>Total:</b>						<b>1000</b>	<b>1.35%</b>

*Table 6: AT&T Maximum Sector MPE Power Values*

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## Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the AT&T facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

AT&T Sector	Power Density Value (%)
Sector A:	1.35 %
Sector B:	1.35 %
Sector C:	1.35 %
AT&T Maximum Total (per sector):	1.35 %
Site Total:	3.14 %
Site Compliance Status:	<b>COMPLIANT</b>

The anticipated composite MPE value for this site assuming all carriers present is **3.14 %** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.



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