

April 24, 2018

Melanie A. Bachman Acting Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

RE: Notice of Exempt Modification for T-Mobile / L700 Crown Site BU: 876355

T-Mobile Site ID: CT11264C

Located at: 474-480 Main St., Monroe, CT 06468 Latitude: 41° 19′ 31.99″ / Longitude: -73° 15′ 57.05″

Dear Ms. Bachman,

T-Mobile currently maintains twelve (12) antennas at the 194-foot level of the existing 194-foot monopole at 474-480 Main St., Monroe, CT. The tower is owned by Crown Castle. The property is owned by Sprint PCS and Global Signal Acq. II LLC (Crown Castle). T-Mobile now intends to remove and replace six (6) antennas, move three (3) of the antennas to the 192-foot level, replace six (6) TMAs, add three (3) BiasTs, and replace current T-Arm mounts with a new full platform mount for all antennas. They also propose to add one equipment cabinet to the ground. All work will be completed within the existing area.

This facility was approved by the Town of Monroe on October 17, 2000 in No. 10461. The approval did not include any conditions.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. §16-50j-72(b)(2). In accordance with R.S.C.A. § 16-50j-73, a copy of this letter is being sent to First Selectman, Kenneth Kellogg; Zoning Enforcement Officer, Joe Chapman; as well as the property owner and the tower owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.

2. The proposed modification will not require the extension of the site boundary.

3. The proposed modification will not increase noise levels at the facility by six decibels or more,

or to levels that exceed state and local criteria.

4. The operation of the replacement antennas will not increase radio frequency emissions at the

facility to a level at or above the Federal Communication Commission safety standard.

5. The proposed modifications will not cause a change or alteration in the physical or

environmental characteristics of the site.

6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-

72(b)(2). Please send approval/rejection letter to Attn: Amanda Cornwall.

Sincerely,

Amanda Cornwall

Real Estate Specialist

12 Gill Street, Suite 5800, Woburn, MA 01801

339-205-7017

Amanda.Cornwall@crowncastle.com

Attachments:

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes

Tab 2: Exhibit-2: Structural Modification Report

Tab 4: Exhibit-3: General Power Density Table report (RF Emissions Analysis Report)

cc: Kenneth Kellogg, First SelectmanTown of Monroe7 Fan Hill Road

Monroe, CT 06468

Joe Chapman, Zoning Enforcement Officer 7 Fan Hill Road Monroe, CT 06468

Crown Castle (Tower Owner & Property Owner) 12 Gill Street, Suite 5800 Woburn, Ma 01801 Ex.

Shipping

Tracking

Printing Services

Locations

Support

Amenda

IMPORTANT!

FedEx is closely monitoring the winter storms across portions of the U.S. Learn More

772070282560 Ship date: Actual delivery: Tue 4/24/2018 Wed 4/25/2018 10:21 am Crown Castle Delivered Zoning Enforcement Officer Amanda Cornwall Suite 5800 Signed for by: B.KRADUSKI 7 Fan Hill Road Fedex Fedex Fedex Fedex Fedex Fedex Fedex MONROE, CT US 06468 Woburn, MA US 01801 339 205-7017 FerEx FerEx FerEx FerEx FerEx FerEx 203 452-2816 Fedly loof x feet x fee Travel History Date/Time Activity Location - 4/25/2018 - Wednesday 10:21 am Delivered MONROE, CT 9:25 am On FedEx vehicle for delivery STRATFORD, CT 7:55 am At local FedEx facility 3:55 am Departed FedEx location NEWARK NJ 4/24/2018 - Tuesday 11:20 pm Arrived at FedEx location Left FedEx origin facility 8:05 pm WILMINGTON, MA 6:27 pm Picked up WILMINGTON, MA Shipment information sent to FedEx 2:38 pm Shipment Facts Tracking Number 772070282560 Service FedEx Priority Overnight Reference 1766.6680 Weight 0.5 lbs / 0.23 kgs Delivery attempts 1 **Delivered To** Receptionist/Front Desk Total pieces 1 Total shipment weight 0.5 lbs / 0.23 kgs Not Available 907651 Invoice number Shipper reference Packaging FedEx Envelope Special handling section Deliver Weekday Standard 4/25/2018 by 10:30 am

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Amanda

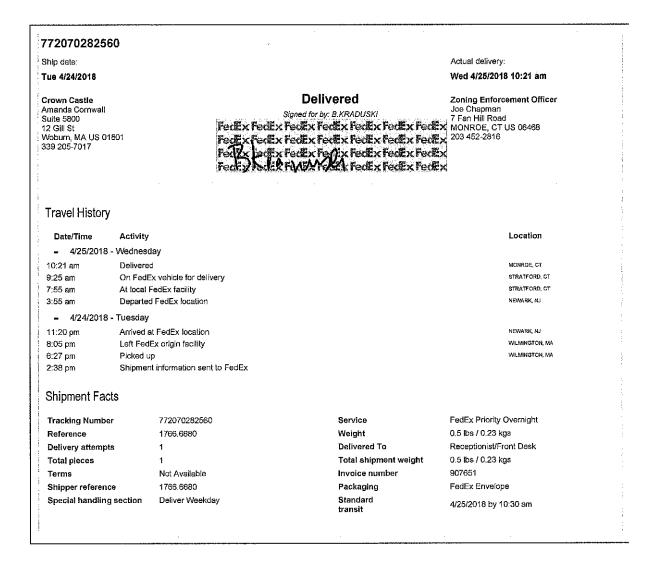
IMPORTANTI

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Town of Monroe



OFFICE OF THE TOWN ENGINEERING DEPARTMENT

Town Hall 7 Fan Hill Road Monroe, Connecticut 06468 Phone: (203) 452-5437 (203) 452-5438

July 10,2000

Paul T. Tusch Cacase, Tusch, Santagam 777 Summer Street P.O. Box 15859 Stamford, CT. 06901-0859

Re: Sprint PCS

474-480 Main Street Special Exception Permit

Dear Mr. Tusch:

Please be advised that this department has reviewed the plans (4 pages) submitted for the above project and, although the design concept is generally acceptable, the following item should be addressed:

1) if the access roadway is to have a gravel surface (ie; not asphalt paved), construct the road using a minimum 6" depth of 3/4" medium coarse process gravel, shaped and crowned to control water runoff and compacted to 95%. Construct sufficient riprap leak offs to control erosion in road shoulder areas.

It is required that installation of the security fencing commence immediately following erection of the tower and continue non stop (without interruption) until completely installed.

If you have any questions, please contact my office at (203) 452-5438.

Very Truly Yours,

Sherwood Lovejoy, P.F.

Director of Public Work

Town Engineer

SL/fjm QMS & JU,



TOWN OF MONROE

OFFICE OF THE FIRE MARSHAL

June 27, 2000

Attorney Paul T. Tusch Cacase, Tusch, Santagata 777 Summer Street P. O. Box 15859 Stamford, CT 06901-0859

RE: Sprint PCS Tower, 474-480 Main Street

Dear Attorney Tusch,

I have reviewed the proposed Sprint PCS Tower located at TLC, 474-480 Main Street, and my only requirements would be:

- Knox box system
- Access road be at least 20' wide

If you have any questions, please call me.

Sincerely,

Anthony Carpenter

Fire Marshal



TOWN OF MONROE, CONNECTICUT PROVISIONAL CERTIFICATE ZONING COMPLIANCE

This is to certify that the propose	d <u>Communication Tower</u>	- eglipment compound
	(struct	ure, addition, use)
located at No. 480 (Lo	1 No) Main And	(Street Road Drive)
Application dated 19/17		ten ocoatic
has been examined and based on the in	nformation contained in said application the proj	
the Town of Monroe, dated	7-31-00	
•	(Effective date of last amendment)	
This provisional certificate expi	res one year from the date herein, or upon issu	nance of a permanent certificate of zoning
compliance, whichever is first. Failure	to obtain said permanent certificate prior to u	se shall constitute a violation of the Zoning
Regulations of the Town of Monroe.		
Nº 10461	Dated at Monroe, Connecticut this 27	14 day of October 3000
42 10401	(Zoning Enforcement Officer)	(Planning Administrator)

Town of Monroe conservation and water resources commission/inland wetland commission

TOWN HALL 7 Fan Hill Road Monroe, Connecticut 06468 Phone (203) 452-5467 Fax (203) 261-6197

November 16, 2000

CERTIFIED MAIL RETURN RECEIPT REQUESTED 7009 3400 0007 9991 7695

Sprint PCS 1 International Blvd Suite 800 Mahwah, NJ 07495

CONDITIONAL APPROVAL Inland Wetlands Permit No. 00-23

Applicant:

Sprint PCS

Property Owner

Property Location: 474-480 Main Street Assessor's Map No. 45 Parcel No. 21A &

22B

Plans & Preparer: URS Corporation AES 500 Enterprise Drive, Rocky Hill CT

PERMIT APPROVED (date): October 25, 2000. All appropriate conditions must be satisfied prior to site disturbance. THIS APPROVAL IS NOT AN AUTHORIZATION TO START CONSTRUCTION.

PERMIT EXPIRES: October 25, 2005

Permit duration is five (5) years. Additional extensions must be requested prior to expiration. A renewal fee will be required. THIS PERMIT CANNOT BE REINSTATED IF IT EXPIRES.

THIS PERMIT IS NOT TRANSFERABLE UNLESS THE NEW OWNER PROVIDES THE COMMISSION WITH A SIGNED ACKNOWLEDGMENT THAT HE UNDERSTANDS AND ACCEPTS THE CONDITIONS OF APPROVAL.

Commission's findings and resolution: The following resolution was adopted by the Inland Wetlands Commission.

Page 2

Be it resolved that Inland Wetland Permit Application No. 00-23 is hereby approved based upon the findings and subject to the modifications and conditions hereinafter set forth.

The Commission reviewed the application and the site plan and determined there will be no significant impact and the application does not warrant a public hearing. There was also no public interest demonstrated.

The Commission finds that the proposed activities are located entirely within the regulated setback and there will be no direct wetland disturbance.

MODIFICATIONS AND CONDITIONS:

- 1) The excavated trench shall be refilled, seeded and stabilized immediately after completion of the utility installation.
- 2) Access to the construction area will be by existing roads.

STANDARD CONDITIONS:

- 1) Regulated activities herein shall be implemented by the permittee in accordance with the timing, location, duration and intent proposed and approved by the Commission.
- 2) Notice of assignment or transfer of the permit must be given to the Commission immediately. Failure to do so may invalidate your permit.
- 3) Install sediment and erosion controls prior to soil disturbance and maintain them during construction and remove them prior to requesting final inspection.
- 4) Any changes in the approved plans must be approved by the Commission. This includes changes required by any other agency.
- 5) The posting of a cash or passbook savings account may be required at any time during construction by the Inland Wetlands Commission for erosion controls or any required wetland mitigation measures, in an amount to be determined by the Commission or its agent.
- 6) For the purpose of making site inspections of sediment and erosion controls, the permittee shall provide forty-eight (48) hours notice prior to site disturbance.
- 7) Anti tracking aprons shall be installed on all road and driveway exits with six (6) inches in depth of crushed stone spread to the traveled width, forty (40) feet long and underlain with construction fabric.
- 8) In the event an appeal is taken from this decision the applicant shall provide the Commission with three (3) sets of all plans, reports and documents in support of the application within thirty (30) days.
- 9) Heating oil tanks will not be buried anywhere on the property.

This application is approved with the above conditions and/or modifications. This decision and these conditions are consistent with the purposes of the wetland regulations which are designed to protect the citizens of Monroe by providing a balance between the need for growth, development and enjoyment of the Town's natural resources with the need to protect its' environment and ecological stability.

cc: Dean Gustafson, Applicants Agent

imes P. White, Jr.

Chairman of the Commission

Town of Monroe conservation and water resources commission/inland wetland commission

TOWN HALL
7 Fan Hill Road
Monroe, Connected 06468
Phone (203) 452-5467
Fax (203) 261-6197

July 11, 2000

URS Greiner Woodward Clyde 500 Enterprise Drive Rocky Hill, CT 06067

RE: Sprint PCS Upper Stepney

Dear Mr. Clyde:

Based on my review of the site plan for Sprint PCS Upper Stepney deced June 23, 2000. An Inland Wetland permit will not be required for this project.

Please contact me if you have any questions.

Yours truly,

Richard B. Jacobson Wetland Consultzer

gw

cc: Planning and Zoning

474 MAIN ST

Location 474 MAIN ST

Map/Lot 045/022/0Z//

Acct# 0450220Z

Owner SPRINT PCS

Assessment \$239,700

Appraisal \$342,400

PID 16240

Building Count 1

Survey 1676 B

Affordable

Current Value

Appraisal				
Valuation Year Improvements Land Total				
2014	\$125,000	\$217,400	\$342,400	
	Assessment			
Valuation Year	Improvements	Land	Total	
2014	\$87,500	\$152,200	\$239,700	

Owner of Record

Owner

SPRINT PCS

Co-Owner GLOBAL SIGNAL ACQ II LLC

Address

PMB 331 4017 WASHINGTON RD

MCMURRAY, PA 15317

Sale Price

\$0

Certificate 1

Book & Page 943/187

Sale Date 04/27/2001

Ownership History

Ownership History				
Owner Sale Price Certificate Book & Page Sale Date				
SPRINT PCS	\$0	1	943/ 187	04/27/2001

Building Information

Building 1: Section 1

Year Built:

Living Area:

0

Building Attributes				
Field Description				
Style	Vacant Land			
Model				

Stories:	
Occupancy	
Exterior Wall 1	
Heat Fuel	
Heat Type:	
AC Type:	
Total Bedrooms:	
Total Bthrms:	
Total Half Baths:	
Total Rooms:	
Fireplaces	
Basement Gar.	
Basement	
In Law Apt	

Building Photo



 $\hline (http://images.vgsi.com/photos/MonroeCTPhotos//<math>\00\00\64/02$.

Building Layout

(http://images.vgsi.com/photos/MonroeCTPhotos//Sketches/1624

Building Sub-Areas (sq ft)	<u>Legend</u>
No Data for Building Sub-Areas	

Extra Features

Extra Features	<u>Legend</u>
No Data for Extra Features	

Land

Land Use		Land Line Valuation		
Use Code	431	Size (Acres)	0.06	
Description	TEL REL TW	Appraised Value	\$217,400	
Zone	B1			
Neighborhood				
Alt Land Approved	No			

Outbuildings

Category

Outbuildings <u>Le</u>					<u>Legend</u>	
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #
TT4	TOWER MONOPOLE			1 UNITS	\$125,000	1

Valuation History

Appraisal			
Valuation Year	Improvements	Land	Total

2017	\$125,000	\$217,400	\$342,400
2015	\$125,000	\$217,400	\$342,400

Assessment				
Valuation Year Improvements Land Total				
2017	\$87,500	\$152,200	\$239,700	
2015	\$87,500	\$152,200	\$239,700	

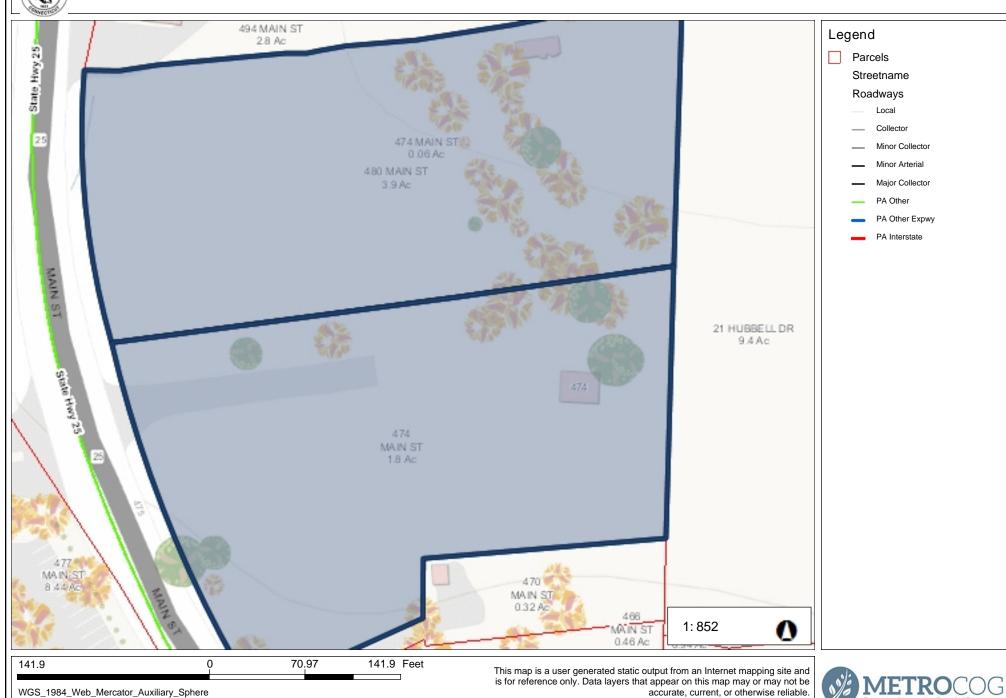
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Town of Monroe

Created by Greater Bridgeport Regional Council

Map Title

THIS MAP IS NOT TO BE USED FOR NAVIGATION



SITE NAME: MONROE/ RT 59/ RT 25

474-480 MAIN STREET MONROE, CT 06468 FAIRFIELD COUNTY

T-MOBILE SITE NUMBER: CT11264C

CROWN BU NUMBER: 876355

RF DESIGN GUIDELINE: 704G

GENERAL NOTES

THIS DOCUMENT IS THE CREATION, DESIGN, PROPERTY AND COPYRIGHTED WORK OF T-MOBILE. ANY DUPLICATION OR USE WITHOUT EXPRESS WRITTEN CONSENT IS STRICTLY PROHIBITED. DUPLICATION AND USE BY GOVERNMENT AGENCIES FOR THE PURPOSES OF CONDUCTING THEIR LAWFULLY AUTHORIZED REGULATORY AND ADMINISTRATIVE FUNCTIONS IS SPECIFICALLY ALLOWED.

THE FACILITY IS AN UNMANNED PRIVATE AND SECURED EQUIPMENT INSTALLATION. IT IS ONLY ACCESSED BY TRAINED TECHNICIANS FOR PERIODIC ROUTINE MAINTENANCE AND THEREFORE DOES NOT REQUIRE ANY WATER OR SANITARY SEWER SERVICE. THE FACILITY IS NOT GOVERNED BY REGULATIONS REQUIRING PUBLIC ACCESS PER ADA REQUIREMENTS.

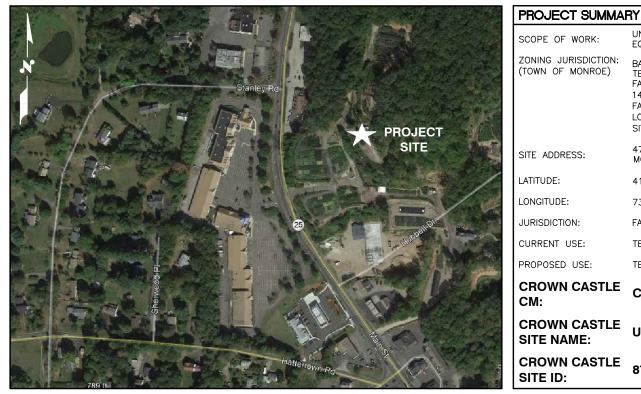
CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE T-MOBILE NORTHEAST, LLC REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

SPECIAL STRUCTURAL NOTES

CONTRACTOR SCOPE OF WORK SHALL INCLUDE ALL REQUIRED STRUCTURAL MODIFICATIONS, RE-BUNDLING OF COAXIAL CABLES OR OTHER SPECIAL MODIFICATIONS AS OUTLINED THEREIN.

STRUCTURAL DESIGNS AND DETAILS FOR ANTENNA MOUNTS AND GLOBAL STRUCTURAL STABILITY ANALYSIS COMPLETED ON BEHALF OF T-MOBILE ARE INCLUSIVE OF THE ENTIRE SUPPORT STRUCTURE, EXISTING ANTENNA MOUNTS AND ALL OTHER ASPECTS OF THE STRUCTURE THAT WILL SUPPORT THE T-MOBILE L700 EQUIPMENT DEPLOYMENT AS DEPICTED HEREIN.

HUDSON DESIGN ASSUMES THAT THE EQUIPMENT IS PROPERLY CONSTRUCTED AND MAINTAINED. ALL STRUCTURAL MEMBERS AND THEIR CONNECTION ARE ASSUMED TO BE IN GOOD CONDITION AND ARE FREE FROM DEFECTS WITH NO DETERIORATION TO ITS MEMBER CAPACITIES



APPROVALS PROJECT MANAGER DATE CONSTRUCTION DATE RF ENGINEERING DATE ZONING / SITE ACQ. DATE **OPERATIONS** DATE TOWER OWNER DATE

72 HOURS



UNDERGROUND SERVICE ALERT

CONTRACTOR TO NOTIFY CROWN CM PRIOR TO CONSTRUCTION START

T-MOBILE TECHNICIAN SITE SAFETY NOTES SPECIAL RESTRICTIONS LOCATION SECTOR A: ANTENNA/TMA/RRH ACCESS NOT PERMITTED SECTOR B: ANTENNA/TMA/RRH ACCESS NOT PERMITTED SECTOR C: ANTENNA/TMA/RRH ACCESS NOT PERMITTED UNRESTRICTED CAUTION: OSHA-APPROVED GPS/LMU: PORTABLE 8' STEP-LADDER REQUIRED RADIO CABINETS: UNRESTRICTED PPC DISCONNECT: UNRESTRICTED MAIN CIRCUIT D/C: UNRESTRICTED NIU/T DEMARC: UNRESTRICTED OTHER/SPECIAL: NONE

UNMANNED TELECOMMUNICATIONS FACILITY T-MOBILE SCOPE OF WORK: **EQUIPMENT MODERNIZATION** ZONING JURISDICTION: BASED ON INFORMATION PROVIDED BY T-MOBILE. THIS (TOWN OF MONROE) TELECOMMUNICATIONS EQUIPMENT DEPLOYMENT IS AN ELIGIBLE FACILITY UNDER THE TAX RELIEF ACT OF 2012, 47 USC 1455(A), AND IS SUBJECT TO AN EXPEDITED ELIGIBLE FACILITIES REQUEST/REVIEW AND ZONING PRE-EMPTION FOR LOCAL DISCRETIONARY PERMITS (VARIANCE, SPECIAL PERMIT, SITE PLAN REVIEW). 474-480 MAIN STREET SITE ADDRESS: MONROE, CT 06468 LATITUDE: 41° 19' 31.99" N LONGITUDE: 73° 15' 57.05" W JURISDICTION: FAIRFIELD COUNTY

TELECOMMUNICATIONS FACILITY

TELECOMMUNICATIONS FACILITY

CROWN CASTLE CHRIS MILLER (585) 739-1780

CROWN CASTLE UPPER STEPNEY-TLC SITE NAME:

CROWN CASTLE 876355 SITE ID:

CURRENT USE:

PROPOSED USE:

DRAWING INDEX					
SHEET NO.	DESCRIPTION	REV.			
T-1	TITLE SHEET	0			
GN-1	GENERAL NOTES	0			
A-1	COMPOUND & EQUIPMENT PLANS	0			
A-2	ANTENNA LAYOUTS & ELEVATION	0			
A-3	ANTENNA DETAILS	0			
A-4	EQUIPMENT DETAILS	0			
A-5	ANTENNA AND COAX SCHEDULE & PLUMBING DIAGRAM	0			
E-1	ONE-LINE DIAGRAM AND GROUNDING DETAILS	0			

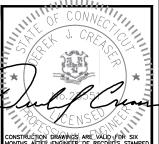
T-MOBILE NORTHEAST LLC

103 MONARCH DRIVE LIVERPOOL, NY 13088 (315) 265-1882



WOBURN, MA 0180





CHECKED BY:

APPROVED BY: DJC

SUBMITTALS

0 03/28/18 ISSUED FOR CONSTRUCTION GA

SITE NUMBER: CT11264C CROWN CASTLE SITE ID:

876355 MONROE/RT 59/ RT 25

SITE ADDRESS: 474-480 MAIN STREET MONROE, CT 06468 FAIRFIELD COUNTY

TITLE SHEET

(L700)

SHEET NUMBER

1-1

GROUNDING NOTES

- 1. THE SUBCONTRACTOR SHALL REVIEW AND INSPECT THE EXISTING FACILITY GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRICT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ), THE SITE—SPECIFIC (UL, LPI, OR NFPA) LIGHTING PROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND TIA GROUNDING STANDARDS. THE SUBCONTRACTOR SHALL REPORT ANY VIOLATIONS OR ADVERSE FINDINGS TO THE CONTRACTOR FOR RESOLUTION.
- ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GES'S) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
- 3. THE SUBCONTRACTOR SHALL PERFORM IEEE FALL—OF—POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR NEW GROUND ELECTRODE SYSTEMS. THE SUBCONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
- 4. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS FOUIPMENT.
- 5. EACH BTS CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, 6 AWG STRANDED COPPER OR LARGER FOR INDOOR BTS 2 AWG STRANDED COPPER FOR DISTRICT
- 6. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
- 7. APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- 8. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO GROUND BAR.
- 9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
- 10. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC
- 11. METAL CONDUIT SHALL BE MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH 6 AWS COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- 12. ALL NEW STRUCTURES WITH A FOUNDATION AND/OR FOOTING HAVING 20 FT. OR MORE OF 1/2 IN. OR GREATER ELECTRICALLY CONDUCTIVE REINFORCING STEEL MUST HAVE IT BONDED TO THE GROUND RING USING AN EXOTHERMIC WELD CONNECTION USING #2 AWG SOLID BARE TINNED COPPER GROUND WIRE, PER NEC 250 50

GENERAL NOTES

1. FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:

CONTRACTOR — CROWN CASTLE INTERNATIONAL SUBCONTRACTOR — GENERAL CONTRACTOR (CONSTRUCTION)
OWNER — T—MOBILE

- PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING SUBCONTRACTOR SHALL VISIT
 THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM
 THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION
 DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF
 CONTRACTOR.
- 3. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES. SUBCONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 4. DRAWINGS PROVIDED HERE ARE NOT TO BE SCALED AND ARE INTENDED TO SHOW OUTLINE ONLY.
- 5. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
- 6. "KITTING LIST" SUPPLIED WITH THE BID PACKAGE IDENTIFIES ITEMS THAT WILL BE SUPPLIED BY CONTRACTOR. ITEMS NOT INCLUDED IN THE BILL OF MATERIALS AND KITTING LIST SHALL BE SUPPLIED BY THE SUBCONTRACTOR.
- THE SUBCONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 8. IF THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE SUBCONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION SPACE FOR APPROVAL BY THE CONTRACTOR
- 9. SUBCONTRACTOR SHALL DETERMINE ACTUAL ROUTING OF CONDUIT, POWER AND T1 CABLES, GROUNDING CABLES AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING. SUBCONTRACTOR SHALL UTILIZE EXISTING TRAYS AND/OR SHALL ADD NEW TRAYS AS NECESSARY. SUBCONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH THE CONTRACTOR.
- 10. THE SUBCONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT SUBCONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.
- 11. SUBCONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 12. SUBCONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION.
- 13. ALL CONCRETE REPAIR WORK SHALL BE DONE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301.

- 14. ANY NEW CONCRETE NEEDED FOR THE CONSTRUCTION SHALL BE AIR—ENTRAINED AND SHALL HAVE 4000 PSI STRENGTH AT 28 DAYS. ALL CONCRETE WORK SHALL BE DONE IN ACCORDANCE WITH ACI 318 CODE REQUIREMENTS.
- 15. ALL STRUCTURAL STEEL WORK SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH AISC SPECIFICATIONS. ALL STRUCTURAL STEEL SHALL BE ASTM A36 (Fy = 36 ksi) UNLESS OTHERWISE NOTED. PIPES SHALL BE ASTM A53 TYPE E (Fy = 36 ksi). ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED. TOUCHUP ALL SCRATCHES AND OTHER MARKS IN THE FIELD AFTER STEEL IS ERECTED USING A COMPATIBLE ZINC RICH PAINT.
- 16. CONSTRUCTION SHALL COMPLY WITH L700 SPECIFICATIONS AND "GENERAL CONSTRUCTION SERVICES FOR CONSTRUCTION OF T-MOBILE SITES."
- 17. SUBCONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK, ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS MUST BE VERIFIED. SUBCONTRACTOR SHALL NOTIFY THE CONTRACTOR OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH CONSTRUCTION.
- 18. THE EXISTING CELL SITE IS IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION WORK BY SUBCONTRACTOR SHALL NOT DISRUPT THE EXISTING NORMAL OPERATION. ANY WORK ON EXISTING EQUIPMENT MUST BE COORDINATED WITH CONTRACTOR. ALSO, WORK SHOULD BE SCHEDULED FOR AN APPROPRIATE MAINTENANCE WINDOW USUALLY IN LOW TRAFFIC PERIODS AFTER MIDNIGHT.
- 19. SINCE THE CELL SITE IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE ADVISED TO BE WORN TO ALERT OF ANY DANGEROUS EXPOSURE LEVELS.
- 20. APPLICABLE BUILDING CODES:
 SUBCONTRACTOR'S WORK SHALL COMPLY WITH ALL APPLICABLE NATIONAL, STATE,
 AND LOCAL CODES AS ADOPTED BY THE LOCAL AUTHORITY HAVING JURISDICTION
 (AHJ) FOR THE LOCATION. THE EDITION OF THE AHJ ADOPTED CODES AND
 STANDARDS IN EFFECT ON THE DATE OF CONTRACT AWARD SHALL GOVERN THE

BUILDING CODE: IBC 2012 WITH 2016 CT STATE BUILDING CODE AMENDMENTS ELECTRICAL CODE: REFER TO ELECTRICAL DRAWINGS

SUBCONTRACTOR'S WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING STANDARDS:

AMERICAN CONCRETE INSTITUTE (ACI) 318; BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE:

AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC)

MANUAL OF STEEL CONSTRUCTION, ASD, FOURTEENTH EDITION;

TELECOMMUNICATIONS INDUSTRY ASSOCIATION (TIA) 222-G, STRUCTURAL STANDARDS FOR STEEL

EQUIPMENT AND ANTENNA SUPPORTING STRUCTURES; REFER TO ELECTRICAL DRAWINGS FOR SPECIFIC ELECTRICAL STANDARDS.

FOR ANY CONFLICTS BETWEEN SECTIONS OF LISTED CODES AND STANDARDS REGARDING MATERIAL, METHODS OF CONSTRUCTION, OR OTHER REQUIREMENTS, THE MOST RESTRICTIVE REQUIREMENT SHALL GOVERN. WHERE THERE IS CONFLICT BETWEEN A GENERAL REQUIREMENT AND A SPECIFIC REQUIREMENT, THE SPECIFIC REQUIREMENT SHALL GOVERN.

T-MOBILE NORTHEAST LLC

103 MONARCH DRIVE LIVERPOOL, NY 13088 {315} 265-1882



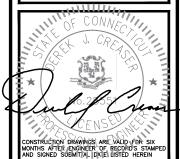
12 GILL STREET, SUITE 5800 WOBURN, MA 01801



BEECHWOOD DRIVE

TEL: (978) 557-5553 FAX: (978) 336-5586

BB



- 4 8 4 7

CHECKED BY:

APPROVED BY:

PROVED BY: DJC

SUBMITTALS

DATE DESCRIPTION

0 03/28/18 ISSUED FOR CONSTRUCTION GA

SITE NUMBER:
CT11264C
CROWN CASTLE SITE ID:

876355 SITE NAME: MONROE/ RT 59/ RT 25

SITE ADDRESS: 474—480 MAIN STREET MONROE, CT 06468 FAIRFIELD COUNTY

SHEET TITLE

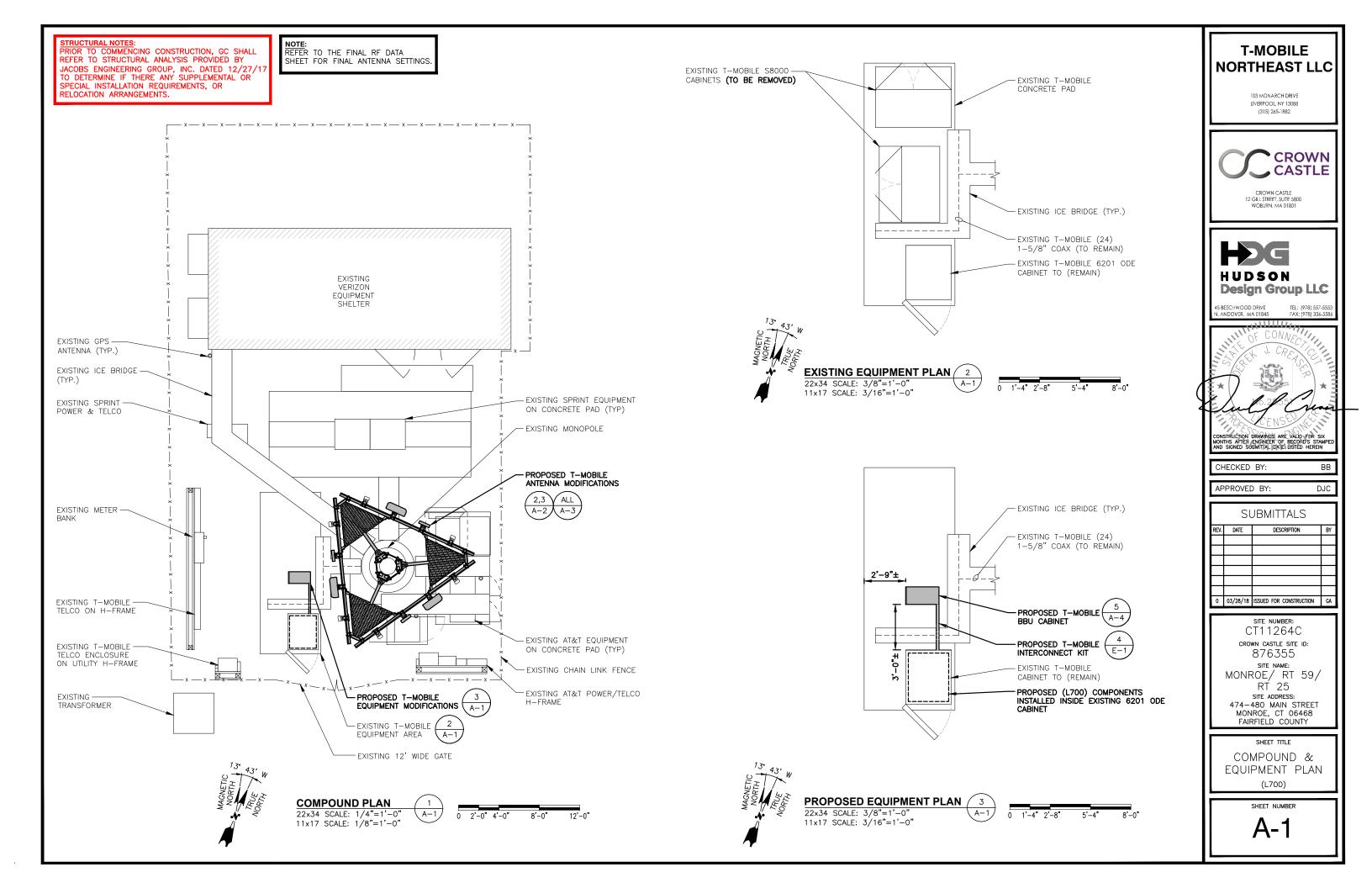
GENERAL NOTES

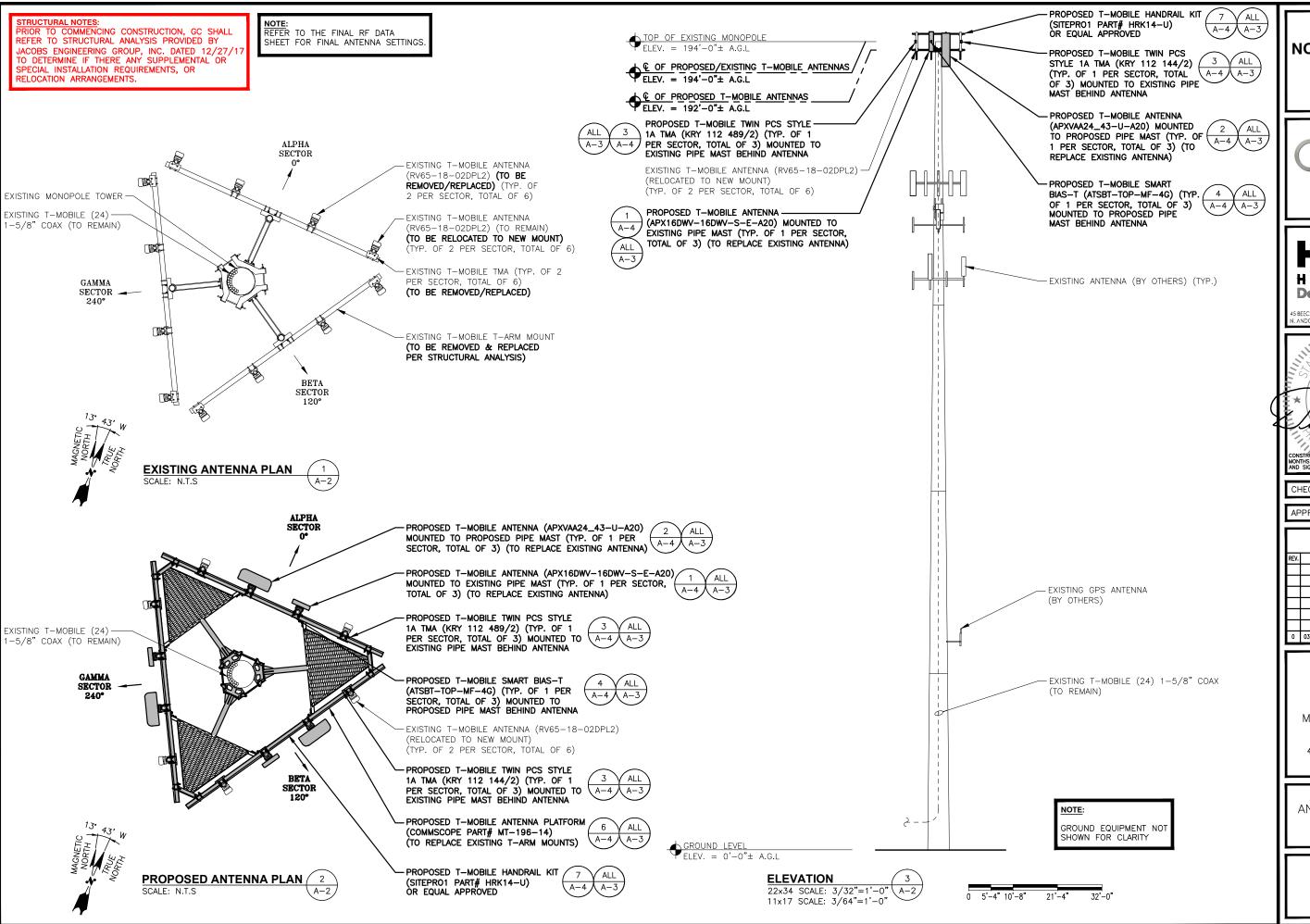
(L700)

SHEET NUMBER

GN-1

			ABBREVIATIONS		
AGL	ABOVE GRADE LEVEL	EQ	EQUAL	REQ	REQUIRED
AWG	AMERICAN WIRE GAUGE	GC	GENERAL CONTRACTOR	RF	RADIO FREQUENCY
BBU	BATTERY BACKUP UNIT	GRC	GALVANIZED RIGID CONDUIT	TBD	TO BE DETERMINED
BTCW	BARE TINNED SOLID COPPER WIRE	MGB	MASTER GROUND BAR	TBR	TO BE REMOVED
BGR	BURIED GROUND RING	MIN	MINIMUM	TBRR	TO BE REMOVED AND REPLACED
BTS	BASE TRANSCEIVER STATION	Р	PROPOSED	TYP	TYPICAL
E	EXISTING	NTS	NOT TO SCALE	UG	UNDER GROUND
EGB	EQUIPMENT GROUND BAR	RAD	RADIATION CENTER LINE (ANTENNA)	VIF	VERIFY IN FIELD
EGR	EQUIPMENT GROUND RING	REF	REFERENCE		





T-MOBILE NORTHEAST LLC

103 MONARCH DRIVE LIVERPOOL, NY 13088 {315} 265-1882



CROWN CASTLE 12 GILL STREET, SUITE 5800 WOBURN, MA 01801



45 BEECHWOOD DRIVE

TEL: (978) 557-55 5 FAX: (978) 336-55

BB

DJC

CONSTRUCTION DRAWINGS ARE VALID FOR SIX MONTHS AFFER ENGINEER OF RECORDS TAMPED AND SIGNED SUBMITTAL IDITEL BUSINED HEREIN

CHECKED BY:

ECKED BI:

APPROVED BY:

SUBMITTALS

REV.	DATE	DESCRIPTION	BY
0	03/28/18	ISSUED FOR CONSTRUCTION	GA
_			

SITE NUMBER: CT11264C

crown castle site id:
876355
site name:
MONROE/ RT 59/
RT 25

SITE ADDRESS: 474—480 MAIN STREET MONROE, CT 06468 FAIRFIELD COUNTY

SHEET TITLE

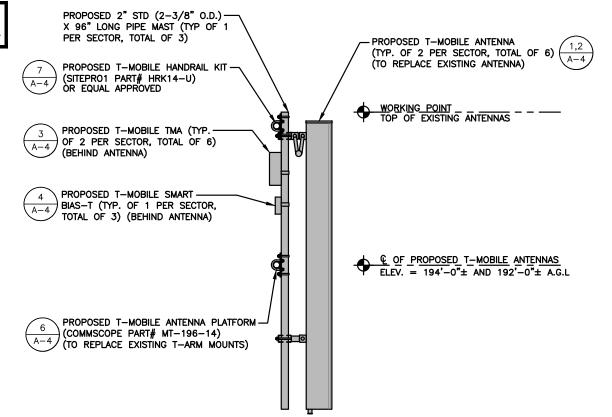
ANTENNA LAYOUTS & ELEVATION (L700)

SHEET NUMBER

A-2

RIOR TO COMMENCING CONSTRUCTION, GC SHALL REFER TO STRUCTURAL ANALYSIS PROVIDED BY JACOBS ENGINEERING GROUP, INC. DATED 12/27/1
TO DETERMINE IF THERE ANY SUPPLEMENTAL OR SPECIAL INSTALLATION REQUIREMENTS, OR RELOCATION ARRANGEMENTS.

REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.



ANTENNA MOUNTING DETAIL

22x34 SCALE: 3/4"=1'-0" 11x17 SCALE: 3/8"=1'-0"

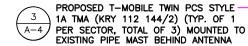


6/20/2017 14:44

T-MOBILE ELEVATION PHOTO DETAIL (2)

SCALE: N.T.S

EXISTING T-MOBILE ANTENNA (RV65-18-02DPL2) (RELOCATED TO NEW MOUNT) (TYP. OF 2 PER SECTOR, TOTAL OF 6) (TO REMAIN)



\A-4

PROPOSED T-MOBILE SMART BIAS-T (ATSBT-TOP-MF-4G) (TYP. OF 1 PER SECTOR, TOTAL OF 3) MOUNTED TO EXISTING PIPE MAST BEHIND ANTENNA

PROPOSED T-MOBILE ANTENNA (APX16DWV-16DWV-S-E-A20)/ MOUNTED TO EXISTING PIPE MAST (TYP. OF 1 PER SECTOR, TOTAL OF 3) (TO REPLACE EXISTING ANTENNA)

PROPOSED T-MOBILE ANTENNA (APXVAA24_43-U-A20) MOUNTED TO PROPOSED PIPE MAST (TYP. OF 1 PER SECTOR, TOTAL OF 3) (TO REPLACE EXISTING ANTENNA)

PROPOSED T-MOBILE HANDRAIL KIT (SITEPRO1 PART# HRK14-U) OR EQUAL APPROVED

PROPOSED T-MOBILE TWIN PCS STYLE 1A TMA (KRY 112 489/2) (TYP. OF 1
PER SECTOR, TOTAL OF 3) MOUNTED TO
EXISTING PIPE MAST BEHIND ANTENNA



(COMMSCOPE PART# MT-196-14) (TO REPLACE EXISTING T-ARM MOUNTS)

PROPOSED T-MOBILE ANTENNA PLATFORM

T-MOBILE NORTHEAST LLC

103 MONARCH DRIVE LIVERPOOL, NY 13088 (315) 265-1882



CROWN CASTLE 12 GILL STREET, SUITE 5800 WOBURN, MA 01801



ВВ

DJC



CONSTRUCTION DRAWINGS ARE VALID FOR SIX MONTHS AFTER ENGINEER OF RECORDS STAMP AND SIGNED SUBMITIAL DATE LISTED HEREIN

CHECKED BY:

APPROVED BY:

SUBMITTALS

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REV.	DATE	DESCRIPTION	BY					
0	03/28/18	ISSUED FOR CONSTRUCTION	GA					

SITE NUMBER: CT11264C

CROWN CASTLE SITE ID: 876355 SITE NAME: MONROE/ RT 59/ RŤ 25

SITE ADDRESS: 474-480 MAIN STREET MONROE, CT 06468 FAIRFIELD COUNTY

SHEET TITLE

ANTENNA DETAILS

(L700)

A-3

WEIGHT

PRIOR TO COMMENCING CONSTRUCTION, GC SHALL REFER TO STRUCTURAL ANALYSIS PROVIDED BY JACOBS ENGINEERING GROUP, INC. DATED 12/27/1
TO DETERMINE IF THERE ANY SUPPLEMENTAL OR SPECIAL INSTALLATION REQUIREMENTS, OR RELOCATION ARRANGEMENTS.

NOTE: REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.

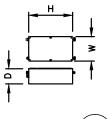
TMA D	MENSIONS
MODEL #	KRY 112 489/2
MANUF.	ERICSSON
HEIGHT	11"
WIDTH	6.1"
DEPTH	3.9"
WEIGHT	15.4 LBS

TMA	TMA DIMENSIONS						
MODEL #	KRY 112 144/2						
MANUF.	ERICSSON						
HEIGHT	8.65"						
WIDTH	6.65"						
DEPTH	3.19"						
WEIGHT	9.70 LBS						

L19/G19 ANTENNA DIMENSIONS APX16DWV-16DWV-S-E-A20 MANUF. RFS/CELWAVE HEIGHT 55.9" WIDTH 13.3" DEPTH 3.15"

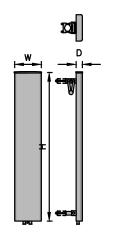
40.7 LBS

L700 ANTENNA DIMENSIONS						
MODEL #	APXVAA24_43-U-A20					
MANUF.	RFS/CELWAVE					
HEIGHT	96"					
WIDTH	24"					
DEPTH	8.5"					
WEIGHT	101.4 LBS					



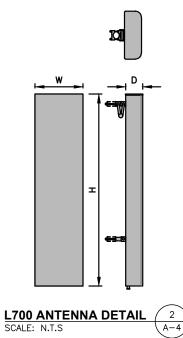


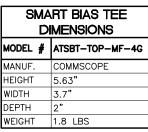


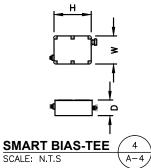


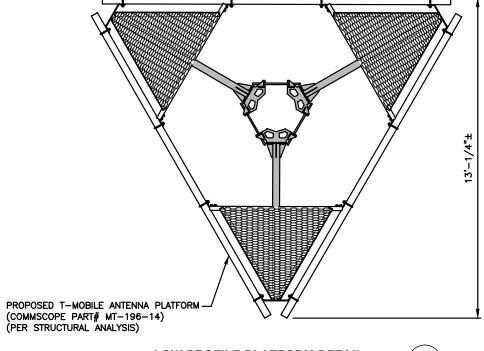
L19/G19 ANTENNA DETAIL (1

SCALE: N.T.S







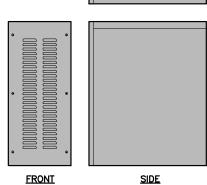


14'-6"±

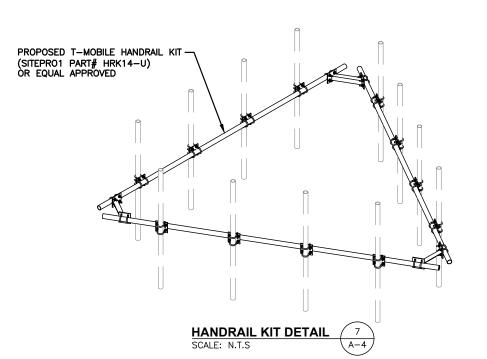








BBU DIMENSIONS				
MODEL #	PTS8003			
MANUF.	PTS			
WIDTH	14.0"			
DEPTH	26.3"			
HEIGHT	32.3"			
WEIGHT W/O BATTERIES	60 LBS			
MOUNT BASE WITH (4) 1/2" DROP IN ANCHORS WITH 2" MINIMUM EMBEDMENT (INSTALL PER MANUFACTURER'S INSTALLATION GUIDELINES)				



BATTERY BACKUP UNIT CABINET (BBU) (5 SCALE: N.T.S

A-4

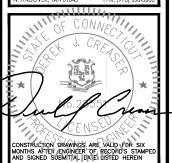
T-MOBILE NORTHEAST LLC

103 MONARCH DRIVE LIVERPOOL, NY 13088 (315) 265-1882



CROWN CASTLE 12 GILL STREET, SUITE 5800 WOBURN, MA 01801





CHECKED BY:

DJC APPROVED BY:

SUBMITTALS

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	0	03/28/18	ISSUED FOR CONSTRUCTION	GA			

SITE NUMBER: CT11264C CROWN CASTLE SITE ID:

876355 MONROE/ RT 59/ RŤ 25

SITE ADDRESS: 474-480 MAIN STREET MONROE, CT 06468 FAIRFIELD COUNTY

SHEET TITLE

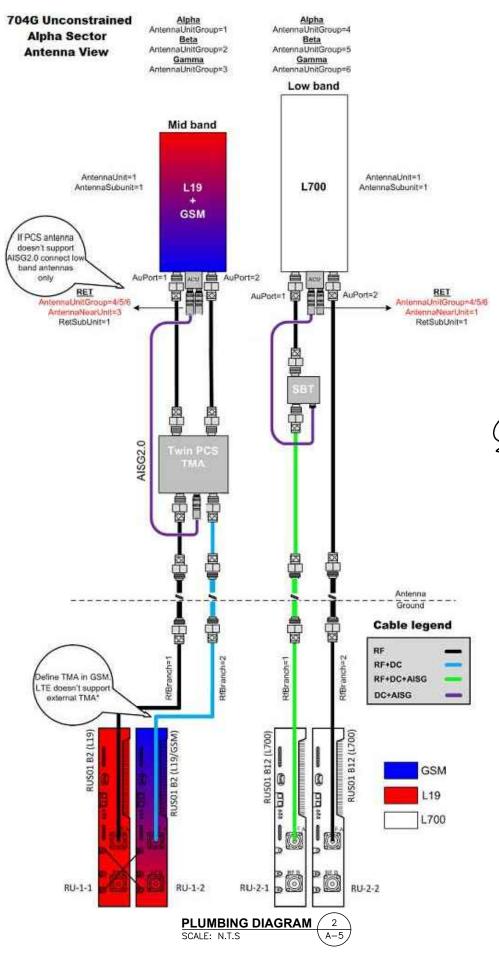
EQUIPMENT DETAILS

(L700)

	PROPOSED ANTENNA AND COAXIAL CABLE SCHEDULE										
SECTOR MARK	ANTENNA MODEL	AZIMUT H	E-TILT	M-TILT	ANTENNA CENTERLINE	SECTOR	TMA/SBiasT/RRU	TX/RX	CABLE FEED LINES	JUMPER TYPE	CABLE LENGTH
A-1 L19/G19	RFS/CELWAVE APX16DWV-16DWV-S-E-A20 (55.9x13.3x3.15)	0°	0	0	194'	RIGHT ALPHA	0/1/0	TX-RX-1	(2) (E) 1-5/8" COAX	_	260'±
A-2 L700	RFS/CELWAVE APXVAA24_43-U-A20 (96x24x8.5)	0,	0	0	192'	LEFT ALPHA	0/0/0	TX-RX-1	(2) (E) 1-5/8" COAX	_	260'±
B-1 L19/G19	RFS/CELWAVE APX16DWV-16DWV-S-E-A20 (55.9X13.3X3.15)	120°	0	0	194'	RIGHT BETA	0/1/0	TX-RX-1 TX-RX-2	(2) (E) 1-5/8" COAX	_	260'±
B-2 L700	RFS/CELWAVE APXVAA24_43-U-A20 (96X24X8.5)	120°	0	0	192'	LEFT BETA	0/0/0	TX-RX-1 TX-RX-2	(2) (E) 1-5/8" COAX	_	260'±
C-1 L19/G19	RFS/CELWAVE APX16DWV-16DWV-S-E-A20 (55.9X13.3X3.15)	240°	0	0	194'	RIGHT GAMMA	0/1/0	TX-RX-1 TX-RX-2	(2) (E) 1-5/8" COAX	_	260'±
C-2 L700	RFS/CELWAVE APXVAA24_43-U-A20 (96X24X8.5)	240°	0	0	192'	LEFT GAMMA	0/0/0	TX-RX-1 TX-RX-2	(2) (E) 1-5/8" COAX	_	260'±

INFORMATION BASED ON CCI APP. REVISION #0 DATED 11/30/2017. CHECK WITH RF ENGINEER FOR LATEST RFDS.

PROPOSED ANTENNA AND COAXIAL CABLE SCHEDULE SCALE: N.T.S



T-MOBILE NORTHEAST LLC

103 MONARCH DRIVE LIVERPOOL, NY 13088 (315) 265-1882



CROWN CASTLE 12 GILL STREET, SUITE 5800 WOBURN, MA 01801



ВВ



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DJC APPROVED BY:

SUBMITTALS

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SITE NUMBER: CT11264C CROWN CASTLE SITE ID:

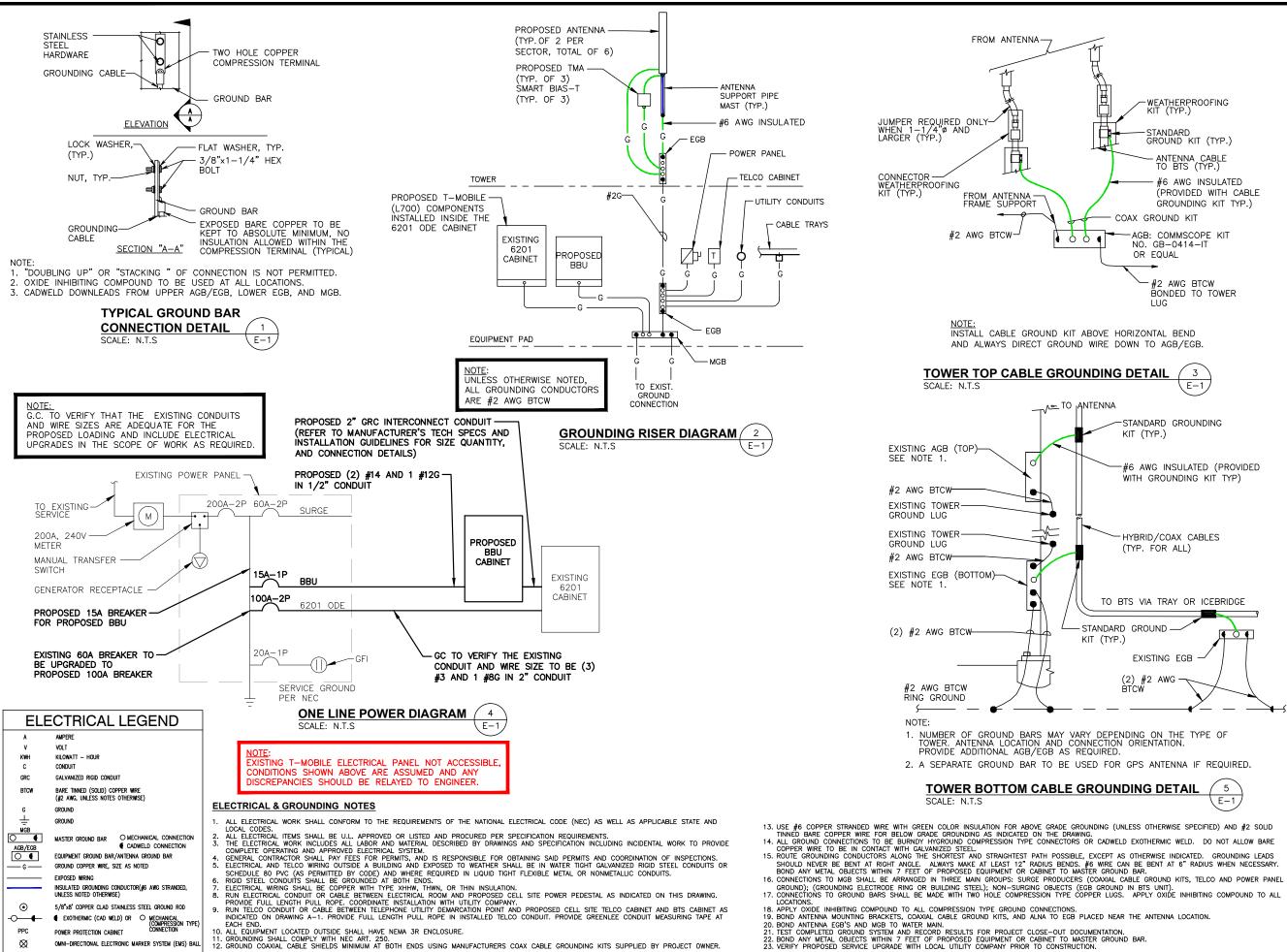
876355 SITE NAME: MONROE/ RT 59/ RT 25

SITE ADDRESS: 474-480 MAIN STREET MONROE, CT 06468 FAIRFIELD COUNTY

SHEET TITLE ANTENNA AND COAX SCHEDULE & PLUMBING DIAGRAM

SHEET NUMBER

A-5



T-MOBILE NORTHEAST LLC

103 MONARCH DRIVE LIVERPOOL, NY 13088 (315) 265-1882

-WEATHERPROOFING KIT (TYP.)

GRÖUND KIT (TYP.)

- ANTENNA CABLE TO BTS (TYP.)

-AGB: COMMSCOPE KIT NO. GB-0414-IT

#2 AWG BTCW

BONDED TO TOWER

#6 AWG INSULATED

(PROVIDED WITH CABLE

GROUNDING KIT TYP.)

STANDARD

COAX GROUND KIT

OR EQUAL

LUG

STANDARD GROUNDING

#6 AWG INSULATED (PROVIDED

WITH GROUNDING KIT TYP)

HYBRID/COAX CABLES

TO BTS VIA TRAY OR ICEBRIDGE

EXISTING EGB

(2) #2 AWG

(TYP. FOR ALL)

-STANDARD GROUND

KIT (TYP.)

KIT (TYP.)

00

ANTENNA مر



CROWN CASTLE 12 GILL STREET, SUITE 5800 WOBURN, MA 0180

HUDSON **Design Group LLC**

BB

DJC

MONTHS AFTER ENGINEER OF RECORDS STAMF AND SIGNED SUBMIT AL DATE LISTED HEREIN

CHECKED BY:

APPROVED BY:

SUBMITTALS 0 03/28/18 ISSUED FOR CONSTRUCTION

> SITE NUMBER CT11264C CROWN CASTLE SITE ID:

876355 SITE NAME: MONROE/RT 59/

RT 25 SITE ADDRESS:

474-480 MAIN STREET MONROE, CT 06468 FAIRFIELD COUNTY

SHEET TITLE ONE-LINE DIAGRAM & GROUNDING DETAILS (L700)

SHEET NUMBER

Date: December 27, 2017

Charles McGuirt Crown Castle 3530 Toringdon Way, Suite 300 Charlotte, NC 28277



Subject: Structural Analysis Report

Carrier Designation: T-Mobile Co-Locate

Carrier Site Number: CT11264C

Carrier Site Name: Monroe/ Rt 59/ Rt 25

(770)701-2500

Crown Castle Designation: Crown Castle BU Number: 876355

Crown Castle Site Name: UPPER STEPNEY - TLC

Crown Castle JDE Job Number: 474072 Crown Castle Work Order Number: 1503489 Crown Castle Application Number: 418243 Rev. 0

Engineering Firm Designation: Jacobs Engineering Group, Inc. Project Number: 1503489

Site Data: 474-480 Main St., MONROE, Fairfield County, CT

Latitude 41° 19' 31.99", Longitude -73° 15' 57.05"

191.5 Foot - Monopole Tower

Dear Charles McGuirt,

Jacobs Engineering Group, Inc. is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 1121179, in accordance with application 418243, revision 0.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment

Sufficient Capacity

Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

This analysis has been performed in accordance with the 2016 Connecticut State Building Code based upon an ultimate 3-second gust wind speed of 120 mph converted to a nominal 3-second gust wind speed of 93 mph per Section 1609.3 and Appendix N as required for use in the TIA-222-G Standard per Exception #5 of Section 1609.1.1. Exposure Category B with a maximum topographic factor, Kzt, of 1.000 and Risk Category II were used in this analysis.

All modifications and equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

We at *Jacobs Engineering Group, Inc.* appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects, please give us a call.

Structural analysis prepared by:

Reviewed by:

Nikhil Sharma Structural Engineer Paul L. Mucci, PE Senior Project Engineer

tnxTower Report - version 7.0.7.0

TABLE OF CONTENTS

1) INTRODUCTION

2) ANALYSIS CRITERIA

Table 1 - Proposed Antenna and Cable Information

Table 2 - Existing and Reserved Antenna and Cable Information

Table 3 - Design Antenna and Cable Information

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

3.1) Analysis Method

3.2) Assumptions

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Table 6 - Tower Components vs. Capacity

4.1) Recommendations

5) APPENDIX A

tnxTower Output

6) APPENDIX B

Base Level Drawing

7) APPENDIX C

Additional Calculations

1) INTRODUCTION

This tower is a 191.5 ft Monopole tower designed by Engineered Endeavors, Inc. in October of 2000. The tower was originally designed for a wind speed of 90 mph per TIA/EIA-222-F.

2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA-222-G Structural Standard for Antenna Supporting Structures and Antennas using a 3-second gust wind speed of 93 mph with no ice, 50 mph with 0.75-inch ice thickness and 60 mph under service loads, exposure category B with topographic category 1.

Table 1 - Proposed Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note	
		3	commscope	ATSBT-TOP-MF-4G				
	194.0	3	rfs celwave	APX16DWV-16DWV-S-E- A20				
400.0	192.0	3	ericsson	KRY 112 144/2				
192.0			3	ericsson	KRY 112 489/2	-	-	-
		3	rfs celwave	APXVAA24_43-U-A20				
		1	commscope	MT-196-14				
		1	commscope	MT-195-14 Handrail Kit				

Table 2 - Existing and Reserved Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
		6	ems wireless	RV65-18-02DPL2	24	1-5/8	1
192.0	194.0	6	ems wireless	RV65-18-02DPL2 w/ Mount Pipe			
192.0		6	ericsson	KRY 112 144/1	-	-	3
	192.0	1	tower mounts (crown)	T-Arm Mount [TA 602-3]			
		3	alcatel lucent	RRH2x60-700			
			commscope	JAHH-65B-R3B w/ Mount Pipe	1	1-5/8	2
			rfs celwave	DB-B1-6C-8AB-0Z			
			alcatel lucent	AWS4 (B66) 4x45 RRH			
160.0	160.0	2	antel	BXA-171063-12BF w/ Mount Pipe		1-5/8	
100.0	100.0	1	antel	BXA-171063-8BF-2 w/ Mount Pipe	10		
		4	antel	LPA-80063/6CF w/ Mount Pipe	13		1
		2	antel	LPA-80080/4CF w/ Mount Pipe			
		1	rfs celwave	DB-B1-6C-8AB-0Z			

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
		1	tower mounts (crown)	Platform Mount [LP 303-1]			
		3	alcatel lucent	PCS 1900MHz 4x45W- 65MHz			
154.0	154.0	1	tower mounts (crown)	Side Arm Mount [SO 102-3]	_	_	1
		3	alcatel lucent	800 EXTERNAL NOTCH FILTER			
		3	alcatel lucent	800MHZ 2X50W RRH			
	154.0	3	alcatel lucent	TD-RRH8x20-25			
	152.0	1	rfs celwave	APXVTM14-C-120 w/ Mount Pipe	1	1-1/4	2
150.0		3	rfs celwave	APXVSPP18-C-A20 w/ Mount Pipe	3	1-1/4	
		9	rfs celwave	ACU-A20-N			1
		1	tower mounts (crown)	Platform Mount [LP 712-1]			
		3	ericsson	RRUS-11			
		3	powerwave technologies	7770.00 w/ Mount Pipe			
137.0	140.0	3	powerwave technologies	P65-16-XLH-RR w/ Mount Pipe	1 2	3/8 5/8	1
137.0		1	raycap	DC6-48-60-18-8F	6	1-1/4	'
		6	powerwave technologies	LGP21401	O	, .	
	137.0	1	tower mounts (crown)	Platform Mount [LP 303-1]			
	52.0	1	kathrein	OG-860/1920/GPS-A			
50.0	50.0	1	tower mounts (crown)	Side Arm Mount [SO 701-	1	1/2	1

Notes:

- Existing Equipment 1) 2) 3)
- Reserved Equipment Equipment to Be Removed; Not considered in this analysis

Table 3 - Design Antenna and Cable Information

Mounting Level (ft)	Elevation	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
191.5	191.5	12	dapa	48000	-	-
181.5	181.5	12	dapa	48000	-	-
171.5	171.5	12	dapa	48000	-	-
161.5	161.5	12	dapa	48000	-	-
150.0	150.0	12	dapa	48000	-	-
140.0	140.0	12	dapa	48000	-	-

- 1	Mounting Level (ft)	Flouration	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
	50.0	50.0	1	generic	GPS Antenna	-	-

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Clarence Welti Associates, Inc.	1531885	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Engineered Endeavors, Inc.	1631625	CCISITES
4-TOWER MANUFACTURER DRAWINGS	Engineered Endeavors, Inc.	1631582	CCISITES

3.1) Analysis Method

tnxTower (version 7.0.7.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Jacobs Engineering Group, Inc. should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	191.5 - 172.46	Pole	TP20.46x15.5x0.188	1	-3.222	852.683	27.3	Pass
L2	172.46 - 127.753	Pole	TP31.6x19.282x0.313	2	-17.008	2220.140	47.3	Pass
L3	127.753 - 83.0833	Pole	TP42.19x29.815x0.438	3	-28.691	4156.060	46.4	Pass
L4	83.0833 - 40.4567	Pole	TP52.59x39.847x0.5	4	-38.689	5432.330	43.2	Pass
L5	40.4567 - 0	Pole	TP62x49.727x0.5	5	-66.977	6834.140	46.1	Pass
							Summary	
						Pole (L2)	47.3	Pass
						RATING =	47.3	Pass

Table 6 - Tower Component Stresses vs. Capacity - LC7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	44.2	Pass
1	Base Plate	0	55.5	Pass
1	Base Foundation (Structural)	0	57.5	Pass
1	Base Foundation (Soil Interaction)	0	48.5	Pass

Structure Rating (max from all components) = 57.5%
--

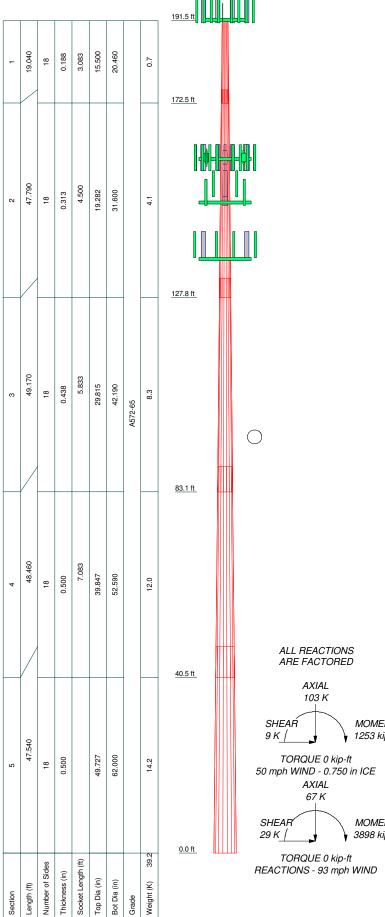
Notes:

4.1) Recommendations

The tower and its base foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

¹⁾ See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

APPENDIX A TNXTOWER OUTPUT



DESIGNED APPURTENANCE LOADING

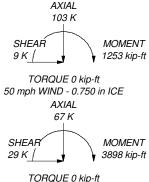
TYPE	ELEVATION	TYPE	ELEVATION
(2) RV65-18-02DPL2	192	800 EXTERNAL NOTCH FILTER	154
(2) RV65-18-02DPL2	192	800 EXTERNAL NOTCH FILTER	154
(2) RV65-18-02DPL2	192	800 EXTERNAL NOTCH FILTER	154
APX16DWV-16DWV-S-E-A20	192	800MHZ 2X50W RRH	154
APX16DWV-16DWV-S-E-A20	192	800MHZ 2X50W RRH	154
APX16DWV-16DWV-S-E-A20	192	800MHZ 2X50W RRH	154
APXVAA24_43-U-A20	192	Side Arm Mount [SO 102-3]	154
APXVAA24_43-U-A20	192	APXVSPP18-C-A20 w/ Mount Pipe	150
APXVAA24_43-U-A20	192	APXVSPP18-C-A20 w/ Mount Pipe	150
(2) KRY 112 489/2	192	APXVSPP18-C-A20 w/ Mount Pipe	150
KRY 112 489/2	192	APXVTM14-C-120 w/ Mount Pipe	150
ATSBT-TOP-MF-4G	192	TD-RRH8x20-25	150
ATSBT-TOP-MF-4G	192	TD-RRH8x20-25	150
ATSBT-TOP-MF-4G	192	TD-RRH8x20-25	150
KRY 112 144/2	192	(3) ACU-A20-N	150
(2) KRY 112 144/2	192	(3) ACU-A20-N	150
Platform Mount [LP 302-1]	192	(3) ACU-A20-N	150
Lightning Rod 5/8" x 5'	191.5	(2) 6' x 2" Mount Pipe	150
(2) LPA-80063/6CF w/ Mount Pipe	160	(2) 6' x 2" Mount Pipe	150
(2) LPA-80063/6CF w/ Mount Pipe	160	(2) 6' x 2" Mount Pipe	150
(2) LPA-80080/4CF w/ Mount Pipe	160	Transition Ladder	150
BXA-171063-12BF w/ Mount Pipe	160	Platform Mount [LP 712-1]	150
BXA-171063-12BF w/ Mount Pipe	160	7770.00 w/ Mount Pipe	137
BXA-171063-8BF-2 w/ Mount Pipe	160	7770.00 w/ Mount Pipe	137
AWS4 (B66) 4x45 RRH	160	7770.00 w/ Mount Pipe	137
AWS4 (B66) 4x45 RRH	160	P65-16-XLH-RR w/ Mount Pipe	137
AWS4 (B66) 4x45 RRH	160	P65-16-XLH-RR w/ Mount Pipe	137
DB-B1-6C-8AB-0Z	160	P65-16-XLH-RR w/ Mount Pipe	137
(2) JAHH-65B-R3B w/ Mount Pipe	160	(2) LGP21401	137
(2) JAHH-65B-R3B w/ Mount Pipe	160	(2) LGP21401	137
(2) JAHH-65B-R3B w/ Mount Pipe	160	(2) LGP21401	137
RRH2x60-700	160	RRUS-11	137
RRH2x60-700	160	RRUS-11	137
RRH2x60-700	160	RRUS-11	137
DB-B1-6C-8AB-0Z	160	DC6-48-60-18-8F	137
Platform Mount [LP 303-1]	160	Platform Mount [LP 303-1]	137
PCS 1900MHz 4x45W-65MHz	154	OG-860/1920/GPS-A	50
PCS 1900MHz 4x45W-65MHz	154	Side Arm Mount [SO 701-1]	50
PCS 1900MHz 4x45W-65MHz	154		

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

- Tower is located in Fairfield County, Connecticut.
 Tower designed for Exposure B to the TIA-222-G Standard.
 Tower designed for a 93 mph basic wind in accordance with the TIA-222-G Standard.
 Tower is also designed for a 50 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
- Deflections are based upon a 60 mph wind.
- Tower Structure Class II.
 Topographic Category 1 with Crest Height of 0.000 ft
 TOWER RATING: 47.3%



Jacobs Engineering Group, Inc. **JACOBS** 5449 Bells Ferry Road

Acworth, GA 30102 Phone: 770-701-2500 FAX: 770-701-2501

Job: UPPER STEPNEY - 1			
Project: BU#876355 WO#15034			
	Drawn by: HolderKG	App'd:	
^{Code:} TIA-222-G	Scale:	NTS	
Path:	Dwg No	· F-1	

tnxTower

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

Acworth, GA 30102 Phone: 770-701-2500 FAX: 770-701-2501

Job	Page
UPPER STEPNEY - TLC	1 of 17
Project	Date
BU#876355 WO#1503489	14:07:52 12/27/17
Client	Designed by
Crown Castle	HolderKG

Tower Input Data

There is a pole section.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Fairfield County, Connecticut.

Basic wind speed of 93 mph.

Structure Class II.

Exposure Category B.

Topographic Category 1.

Crest Height 0.000 ft.

Nominal ice thickness of 0.750 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals

Use Moment Magnification

- √ Use Code Stress Ratios
- ✓ Use Code Safety Factors Guys Escalate Ice Always Use Max Kz Use Special Wind Profile Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends

SR Members Are Concentric

Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area
 Use Clear Spans For KL/r
 Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- Vose Azimula Bisi Coefficients
 ✓ Project Wind Area of Appurt.
 Autocalc Torque Arm Areas
 Add IBC .6D+W Combination
 Sort Capacity Reports By Component
 Triangulate Diamond Inner Bracing
 Treat Feed Line Bundles As Cylinder

Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation

- ✓ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-G Bracing Resist. Exemption Use TIA-222-G Tension Splice Exemption Poles
- √ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets

Tapered Pole Section Geometry

Section	Elevation	Section	Splice	Number	Тор	Bottom	Wall	Bend	Pole Grade
		Length	Length	of	Diameter	Diameter	Thickness	Radius	
	ft	ft	ft	Sides	in	in	in	in	
L1	191.500-172.46	19.040	3.083	18	15.500	20.460	0.188	0.750	A572-65
	0								(65 ksi)
L2	172.460-127.75 3	47.790	4.500	18	19.282	31.600	0.313	1.250	A572-65 (65 ksi)

tnxTower

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

Acworth, GA 30102 Phone: 770-701-2500 FAX: 770-701-2501

	Job		Page
	UPPEI	R STEPNEY - TLC	2 of 17
	Project		Date
•	BU#87	6355 WO#1503489	14:07:52 12/27/17
	Client		Designed by
		Crown Castle	HolderKG

Section	Elevation	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft	Sides	in	in	in	in	
L3	127.753-83.083	49.170	5.833	18	29.815	42.190	0.438	1.750	A572-65 (65 ksi)
L4	83.083-40.457	48.460	7.083	18	39.847	52.590	0.500	2.000	A572-65 (65 ksi)
L5	40.457-0.000	47.540		18	49.727	62.000	0.500	2.000	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia.	Area	I	r	С	I/C	J	It/Q	w	w/t
	in	in^2	in^4	in	in	in^3	in^4	in^2	in	
L1	15.739	9.113	269.950	5.436	7.874	34.284	540.256	4.557	2.398	12.789
	20.776	12.065	626.423	7.197	10.394	60.270	1253.670	6.033	3.271	17.445
L2	20.386	18.815	855.356	6.734	9.795	87.324	1711.837	9.409	2.844	9.099
	32.087	31.033	3838.018	11.107	16.053	239.087	7681.086	15.520	5.012	16.037
L3	31.425	40.794	4448.064	10.429	15.146	293.678	8901.981	20.401	4.477	10.234
	42.841	57.979	12769.382	14.822	21.433	595.795	25555.567	28.995	6.655	15.212
L4	42.019	62.444	12213.654	13.968	20.242	603.375	24443.379	31.228	6.133	12.266
	53.401	82.667	28338.539	18.492	26.716	1060.744	56714.366	41.341	8.376	16.752
L5	52.351	78.124	23918.500	17.476	25.261	946.836	47868.472	39.069	7.872	15.744
	62.956	97.600	46637.979	21.833	31.496	1480.759	93337.326	48.810	10.032	20.064

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade Adjus	t. Factor Adju A _f Factor A _d	or	Stitch Bolt Spacing	Double Angle Stitch Bolt Spacing	Stitch Bolt Spacing
ft	ft^2	in				Diagonals in	Horizontals in	Redundants in
 L1	J			1 1	1			
191.500-172.4								
60								
L2				1 1	1			
172.460-127.7								
53								
L3				1 1	1			
127.753-83.08								
3								
L4				1 1	1			
83.083-40.457								
L5				1 1	1			
40.457-0.000								

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Sector	Component	Placement	Total	Number	Start/End	Width or	Perimeter	Weight
		Type		Number	Per Row	Position	Diameter		
			ft				in	in	klf
AVA7-50(1-5/8")	В	Surface Ar (CaAa)	160.000 - 0.000	2	2	0.350 0.370	2.010		0.001
* Safety Line 3/8	С	Surface Ar (CaAa)	191.500 - 0.000	1	1	0.000	0.375		0.000

4 T	Job	Page
tnxTower	UPPER STEPNEY - TLC	3 of 17
Jacobs Engineering Group, Inc. 5449 Bells Ferry Road	Project BU#876355 WO#1503489	Date 14:07:52 12/27/17
Acworth, GA 30102 Phone: 770-701-2500 FAX: 770-701-2501	Client Crown Castle	Designed by HolderKG

	F	eed L	ine/Line	ar Appurte	enance	es - En	tered A	s Area
Description	Face or	Allow Shield	Component Type	Placement	Total Number		$C_A A_A$	Weight
	Leg		- 1	ft			ft²/ft	klf
*								
LDF7-50A(1-5/8")	A	No	Inside Pole	191.500 - 0.000	24	No Ice	0.000	0.001
						1/2" Ice	0.000	0.001
						1" Ice	0.000	0.001
*								
AVA7-50(1-5/8")	В	No	Inside Pole	160.000 - 0.000	12	No Ice	0.000	0.001
						1/2" Ice	0.000	0.001
						1" Ice	0.000	0.001
*								
HB114-1-0813U4-M5J(1	A	No	Inside Pole	150.000 - 0.000	3	No Ice	0.000	0.001
-1/4)						1/2" Ice	0.000	0.001
						1" Ice	0.000	0.001
HB114-21U3M12-XXX	A	No	Inside Pole	150.000 - 0.000	1	No Ice	0.000	0.001
F(1-1/4)						1/2" Ice	0.000	0.001
						1" Ice	0.000	0.001
*								
LDF6-50A(1-1/4")	В	No	Inside Pole	137.000 - 0.000	6	No Ice	0.000	0.001
						1/2" Ice	0.000	0.001
						1" Ice	0.000	0.001
FB-L98B-002-75000(В	No	Inside Pole	137.000 - 0.000	1	No Ice	0.000	0.000
3/8")						1/2" Ice	0.000	0.000
						1" Ice	0.000	0.000
WR-VG82ST-BRDA(C	No	Inside Pole	137.000 - 0.000	2	No Ice	0.000	0.000
5/8")						1/2" Ice	0.000	0.000
						1" Ice	0.000	0.000
2" Rigid Conduit	C	No	Inside Pole	137.000 - 0.000	1	No Ice	0.000	0.003
-						1/2" Ice	0.000	0.003
						1" Ice	0.000	0.003
*								
LDF4-50A(1/2")	Α	No	Inside Pole	50.000 - 0.000	1	No Ice	0.000	0.000
						1/2" Ice	0.000	0.000

Feed Line/Linear Appurtenances Section Areas

1" Ice

0.000

0.000

Tower	Tower	Face	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation				In Face	Out Face	
	ft		ft^2	ft^2	ft ²	ft ²	K
L1	191.500-172.460	A	0.000	0.000	0.000	0.000	0.375
		В	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.714	0.000	0.004
L2	172.460-127.753	A	0.000	0.000	0.000	0.000	0.987
		В	0.000	0.000	12.963	0.000	0.353
		C	0.000	0.000	1.677	0.000	0.041
L3	127.753-83.083	A	0.000	0.000	0.000	0.000	1.094
		В	0.000	0.000	17.957	0.000	0.617
		C	0.000	0.000	1.675	0.000	0.162
L4	83.083-40.457	A	0.000	0.000	0.000	0.000	1.046
		В	0.000	0.000	17.136	0.000	0.589
		C	0.000	0.000	1.599	0.000	0.155
L5	40.457-0.000	A	0.000	0.000	0.000	0.000	0.997
		В	0.000	0.000	16.264	0.000	0.559

tnxTower

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

Acworth, GA 30102 Phone: 770-701-2500 FAX: 770-701-2501

i		
Job		Page
	UPPER STEPNEY - TLC	4 of 17
Project		Date
	BU#876355 WO#1503489	14:07:52 12/27/17
Client		Designed by
	Crown Castle	HolderKG

Tower	Tower	Face	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation				In Face	Out Face	
	ft		ft ²	ft^2	ft^2	ft^2	K
		С	0.000	0.000	1.517	0.000	0.147

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation	or	Thickness			In Face	Out Face	
	ft	Leg	in	ft^2	ft^2	ft ²	ft^2	K
L1	191.500-172.460	A	1.779	0.000	0.000	0.000	0.000	0.375
		В		0.000	0.000	0.000	0.000	0.000
		C		0.000	0.000	7.488	0.000	0.093
L2	172.460-127.753	A	1.744	0.000	0.000	0.000	0.000	0.987
		В		0.000	0.000	30.544	0.000	0.719
		C		0.000	0.000	17.582	0.000	0.251
L3	127.753-83.083	A	1.683	0.000	0.000	0.000	0.000	1.094
		В		0.000	0.000	41.919	0.000	1.111
		C		0.000	0.000	17.253	0.000	0.364
L4	83.083-40.457	A	1.596	0.000	0.000	0.000	0.000	1.046
		В		0.000	0.000	39.360	0.000	1.039
		C		0.000	0.000	15.950	0.000	0.335
L5	40.457-0.000	A	1.424	0.000	0.000	0.000	0.000	0.997
		В		0.000	0.000	36.474	0.000	0.958
		C		0.000	0.000	14.433	0.000	0.303

Feed Line Center of Pressure

Section	Elevation	CP_X	CP_Z	CP_X	CP_Z
				Ice	Ice
	ft	in	in	in	in
L1	191.500-172.460	0.000	0.055	0.000	0.408
L2	172.460-127.753	0.413	0.146	0.603	0.459
L3	127.753-83.083	0.534	0.174	0.816	0.522
L4	83.083-40.457	0.543	0.178	0.884	0.563
L5	40.457-0.000	0.548	0.180	0.924	0.582

Shielding Factor Ka

Tower	Feed Line	Description	Feed Line	K_a	K_a
Section	Record No.		Segment Elev.	No Ice	Ice
L1	18	Safety Line 3/8	172.46 -	1.0000	1.0000
			191.50		
L1	5	AVA7-50(1-5/8")	172.46 -	1.0000	1.0000
			160.00		
L2	5	AVA7-50(1-5/8")	127.75 -	1.0000	1.0000
			160.00		
L2	18	Safety Line 3/8	127.75 -	1.0000	1.0000
			172.46		
L3	5	AVA7-50(1-5/8")	83.08 - 127.75	1.0000	1.0000
L3	18	Safety Line 3/8	83.08 - 127.75	1.0000	1.0000

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

Job		Page
	UPPER STEPNEY - TLC	5 of 17
Project		Date
	BU#876355 WO#1503489	14:07:52 12/27/17
Client		Designed by
	Crown Castle	HolderKG

Tower	Feed Line	Description	Feed Line	K_a	K_a
Section	Record No.		Segment Elev.	No Ice	Ice
L4	5	AVA7-50(1-5/8")	40.46 - 83.08	1.0000	1.0000
L4	18	Safety Line 3/8	40.46 - 83.08	1.0000	1.0000

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		$C_A A_A$ Front	C_AA_A Side	Weight	
			Vert ft ft ft	o	ft		ft²	ft²	K	
Lightning Rod 5/8" x 5'	С	From Leg	0.000	0.000	191.500	No Ice	0.313	0.313	0.006	
			0.000			1/2" Ice	0.826	0.826	0.010	
delete 1.400 delete			2.000			1" Ice	1.322	1.322	0.016	
level 192			4.000	0.000	102.000	NT T	2 200	1.007	0.011	
(2) RV65-18-02DPL2	Α	From Leg	4.000	0.000	192.000	No Ice	3.300	1.987	0.011	
			0.000			1/2" Ice	3.644	2.316	0.029	
(2) DVICE 10 02DDV2	-		2.000	0.000	102.000	1" Ice	3.996	2.651	0.052	
(2) RV65-18-02DPL2	В	From Leg	4.000	0.000	192.000	No Ice	3.300	1.987	0.011	
			0.000			1/2" Ice	3.644	2.316	0.029	
(2) DIVES 10 02DDI 2			2.000	0.000	102 000	1" Ice	3.996	2.651	0.052	
(2) RV65-18-02DPL2	C	From Leg	4.000	0.000	192.000	No Ice	3.300	1.987	0.011	
			0.000			1/2" Ice	3.644	2.316	0.029	
ADVICDUM ICDUM CE A		г т	2.000	0.000	102 000	1" Ice	3.996	2.651	0.052	
APX16DWV-16DWV-S-E-A	A	From Leg	4.000	0.000	192.000	No Ice	6.586	2.150	0.041	
20			0.000			1/2" Ice	6.962	2.490	0.074	
ADVICDUM ICDUM CE A	D	г т	2.000	0.000	102 000	1" Ice	7.344	2.837	0.113	
APX16DWV-16DWV-S-E-A	В	From Leg	4.000	0.000	192.000	No Ice 1/2" Ice	6.586	2.150	0.041	
20			0.000				6.962	2.490	0.074	
ADVICDUM ICDUM C E A	C	F I	2.000	0.000	102.000	1" Ice No Ice	7.344 6.586	2.837 2.150	0.113	
APX16DWV-16DWV-S-E-A 20	C	From Leg	4.000 0.000	0.000	192.000	1/2" Ice	6.962	2.130	0.041 0.074	
20			2.000			1" Ice	7.344	2.490	0.074	
APXVAA24_43-U-A20	A	From Leg	4.000	0.000	192.000	No Ice	20.267	2.837 8.744	0.113	
APA VAA24_45-U-A20	А	From Leg	0.000	0.000	192.000	1/2" Ice	20.267	9.342	0.101	
			0.000			1" Ice	21.570	9.947	0.213	
APXVAA24_43-U-A20	В	From Leg	4.000	0.000	192.000	No Ice	20.267	9.947 8.744	0.334	
AFA VAA24_43-U-A20	ь	From Leg	0.000	0.000	192.000	1/2" Ice	20.207	9.342	0.101	
			0.000			1" Ice	21.570	9.342	0.213	
APXVAA24_43-U-A20	C	From Leg	4.000	0.000	192.000	No Ice	20.267	9.947 8.744	0.334	
Al X VAA24_43-0-A20	C	110III Leg	0.000	0.000	192.000	1/2" Ice	20.207	9.342	0.101	
			0.000			1" Ice	21.570	9.342	0.213	
(2) KRY 112 489/2	A	From Leg	4.000	0.000	192.000	No Ice	0.559	0.365	0.015	
(2) KK1 112 489/2	А	From Leg	0.000	0.000	192.000	1/2" Ice	0.559	0.303	0.013	
			0.000			1" Ice	0.764	0.542	0.020	
KRY 112 489/2	В	From Leg	4.000	0.000	192.000	No Ice	0.764	0.342	0.027	
KK1 112 409/2	ь	110m Lcg	0.000	0.000	172.000	1/2" Ice	0.658	0.303	0.013	
			0.000			1" Ice	0.058	0.542	0.020	
ATSBT-TOP-MF-4G	A	From Leg	4.000	0.000	192.000	No Ice	0.704	0.095	0.027	
	11	1 Ioni Log	0.000	0.000	1,2.000	1/2" Ice	0.174	0.093	0.002	
			2.000			1" Ice	0.223	0.193	0.005	
ATSBT-TOP-MF-4G	В	From Leg	4.000	0.000	192.000	No Ice	0.292	0.195	0.000	
	5	110m Log	0.000	0.000	1,2.000	1/2" Ice	0.174	0.140	0.002	
			2.000			1" Ice	0.229	0.140	0.003	

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

	Job	Page
	UPPER STEPNEY - TLC	6 of 17
	Project	Date
•	BU#876355 WO#1503489	14:07:52 12/27/17
	Client	Designed by
	Crown Castle	HolderKG

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		$C_A A_A$ Front	C_AA_A Side	Weigh
	Leg	J1	Lateral	J					
			Vert ft	0	ft		ft ²	ft ²	K
			ft ft		Ji		Ji	Ji	11
ATSBT-TOP-MF-4G	С	From Leg	4.000	0.000	192.000	No Ice	0.174	0.095	0.002
			0.000			1/2" Ice	0.229	0.140	0.003
IZD IV 110 144/0	D	Б 1	2.000	0.000	102 000	1" Ice	0.292	0.193	0.006
KRY 112 144/2	В	From Leg	4.000 0.000	0.000	192.000	No Ice 1/2" Ice	0.479	0.232 0.299	0.010
			0.000			1" Ice	0.568 0.664	0.299	0.014
(2) KRY 112 144/2	C	From Leg	4.000	0.000	192.000	No Ice	0.479	0.232	0.010
(2) IRCT 112 111/2	C	Trom Leg	0.000	0.000	1,2.000	1/2" Ice	0.568	0.299	0.014
			0.000			1" Ice	0.664	0.376	0.019
latform Mount [LP 302-1]	C	None		0.000	192.000	No Ice	33.030	33.030	1.70
						1/2" Ice	44.600	44.600	2.193
						1" Ice	56.170	56.170	2.678
level 160									
(2) LPA-80063/6CF w/	Α	From Leg	4.000	0.000	160.000	No Ice	9.831	10.215	0.05
Mount Pipe			0.000			1/2" Ice 1" Ice	10.400	11.384	0.14:
(2) LPA-80063/6CF w/	В	From Leg	0.000 4.000	0.000	160.000	No Ice	10.933 9.831	12.269 10.215	0.24
Mount Pipe	ь	110III Leg	0.000	0.000	100.000	1/2" Ice	10.400	11.384	0.03
Would Tipe			0.000			1" Ice	10.400	12.269	0.24
(2) LPA-80080/4CF w/	C	From Leg	4.000	0.000	160.000	No Ice	2.856	6.569	0.03
Mount Pipe			0.000			1/2" Ice	3.220	7.195	0.07
1			0.000			1" Ice	3.592	7.837	0.12
BXA-171063-12BF w/	A	From Leg	4.000	0.000	160.000	No Ice	4.971	5.228	0.04
Mount Pipe			0.000			1/2" Ice	5.521	6.389	0.08
			0.000			1" Ice	6.036	7.261	0.13
BXA-171063-12BF w/	В	From Leg	4.000	0.000	160.000	No Ice	4.971	5.228	0.04
Mount Pipe			0.000			1/2" Ice	5.521	6.389	0.08
DVA 171062 ODE 2	0	Б. Т	0.000	0.000	1.60.000	1" Ice	6.036	7.261	0.13
BXA-171063-8BF-2 w/	C	From Leg	4.000	0.000	160.000	No Ice	3.179	3.353	0.02
Mount Pipe			0.000			1/2" Ice 1" Ice	3.555 3.930	3.971	0.06
AWS4 (B66) 4x45 RRH	A	From Leg	0.000 4.000	0.000	160.000	No Ice	2.660	4.595 1.586	0.09
AW54 (D00) 4A45 KKII	Λ	110iii Leg	0.000	0.000	100.000	1/2" Ice	2.878	1.769	0.08
			0.000			1" Ice	3.104	1.959	0.10
AWS4 (B66) 4x45 RRH	В	From Leg	4.000	0.000	160.000	No Ice	2.660	1.586	0.06
11101 (200) 1110 1441	2	Trom Log	0.000	0.000	100.000	1/2" Ice	2.878	1.769	0.08
			0.000			1" Ice	3.104	1.959	0.10
AWS4 (B66) 4x45 RRH	C	From Leg	4.000	0.000	160.000	No Ice	2.660	1.586	0.06
			0.000			1/2" Ice	2.878	1.769	0.08
			0.000			1" Ice	3.104	1.959	0.10
DB-B1-6C-8AB-0Z	C	From Leg	4.000	0.000	160.000	No Ice	4.800	2.000	0.04
			0.000			1/2" Ice	5.070	2.193	0.08
(A) 141H1 (5D DAD /		Б. т	0.000	0.000	1.60.000	1" Ice	5.348	2.393	0.12
(2) JAHH-65B-R3B w/	A	From Leg	4.000	0.000	160.000	No Ice	9.351	7.646	0.08
Mount Pipe			0.000			1/2" Ice 1" Ice	9.921 10.455	8.833 9.734	0.16
(2) JAHH-65B-R3B w/	В	From Leg	4.000	0.000	160.000	No Ice	9.351	9.734 7.646	0.24
Mount Pipe	ь	rioni Leg	0.000	0.000	100.000	1/2" Ice	9.921	8.833	0.08
Wiount I ipc			0.000			1" Ice	10.455	9.734	0.10
(2) JAHH-65B-R3B w/	C	From Leg	4.000	0.000	160.000	No Ice	9.351	7.646	0.08
Mount Pipe	Č	200	0.000	2.200		1/2" Ice	9.921	8.833	0.16
r ·			0.000			1" Ice	10.455	9.734	0.24
RRH2x60-700	A	From Leg	4.000	0.000	160.000	No Ice	3.500	1.816	0.06
		S	0.000			1/2" Ice	3.761	2.052	0.08
			0.000			1" Ice	4.029	2.289	0.10
RRH2x60-700	В	From Leg	4.000	0.000	160.000	No Ice	3.500	1.816	0.06
			0.000			1/2" Ice	3.761	2.052	0.083

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

	Job	Page
	UPPER STEPNEY - TLC	7 of 17
	Project	Date
•	BU#876355 WO#1503489	14:07:52 12/27/17
	Client	Designed by
	Crown Castle	HolderKG

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		$C_A A_A$ Front	$C_A A_A$ Side	Weigh
	Leg		Lateral						
			Vert ft	0	ft		ft²	ft²	K
			ft ft		Ji		Ji	Ji	K
			0.000			1" Ice	4.029	2.289	0.109
RRH2x60-700	C	From Leg	4.000	0.000	160.000	No Ice	3.500	1.816	0.060
			0.000			1/2" Ice	3.761	2.052	0.083
			0.000			1" Ice	4.029	2.289	0.109
DB-B1-6C-8AB-0Z	В	From Leg	4.000	0.000	160.000	No Ice	4.800	2.000	0.04
			0.000			1/2" Ice	5.070	2.193	0.08
			0.000			1" Ice	5.348	2.393	0.12
latform Mount [LP 303-1]	C	None		0.000	160.000	No Ice	14.660	14.660	1.25
						1/2" Ice	18.870	18.870	1.48
						1" Ice	23.080	23.080	1.71
level 154		Б. Т	2.000	0.000	154,000	NT T	2 222	2 220	0.06
PCS 1900MHz	A	From Leg	2.000	0.000	154.000	No Ice	2.322	2.238	0.06
4x45W-65MHz			0.000			1/2" Ice	2.527	2.441	0.08
DCS 1000MHz	D	Enom Loo	0.000	0.000	154,000	1" Ice	2.739	2.651	0.11 0.06
PCS 1900MHz	В	From Leg	2.000	0.000	154.000	No Ice	2.322	2.238	
4x45W-65MHz			0.000			1/2" Ice 1" Ice	2.527 2.739	2.441 2.651	0.08
PCS 1900MHz	С	From Leg	0.000	0.000	154,000	No Ice	2.739	2.238	0.11 0.06
4x45W-65MHz	C	From Leg	2.000 0.000	0.000	154.000	1/2" Ice	2.527	2.236	0.08
4x43 W-03MHZ						1" Ice	2.739	2.651	0.08
00 EXTERNAL NOTCH	Α.	From Leg	0.000 2.000	0.000	154.000	No Ice	0.660	0.321	0.11
FILTER	A	110III Leg	0.000	0.000	134.000	1/2" Ice	0.763	0.321	0.01
FILTER			0.000			1" Ice	0.703	0.338	0.01
00 EXTERNAL NOTCH	В	From Leg	2.000	0.000	154.000	No Ice	0.660	0.483	0.02
FILTER	ь	110m Lcg	0.000	0.000	134.000	1/2" Ice	0.763	0.321	0.01
TILTER			0.000			1" Ice	0.703	0.483	0.01
00 EXTERNAL NOTCH	C	From Leg	2.000	0.000	154.000	No Ice	0.660	0.321	0.01
FILTER	C	Trom Leg	0.000	0.000	134.000	1/2" Ice	0.763	0.398	0.01
TIETER			0.000			1" Ice	0.873	0.483	0.02
800MHZ 2X50W RRH	A	From Leg	2.000	0.000	154.000	No Ice	2.134	1.773	0.05
50000000 27130 W Takiri	2.1	Trom Leg	0.000	0.000	151.000	1/2" Ice	2.320	1.946	0.07
			0.000			1" Ice	2.512	2.127	0.09
800MHZ 2X50W RRH	В	From Leg	2.000	0.000	154.000	No Ice	2.134	1.773	0.05
			0.000			1/2" Ice	2.320	1.946	0.07
			0.000			1" Ice	2.512	2.127	0.09
800MHZ 2X50W RRH	C	From Leg	2.000	0.000	154.000	No Ice	2.134	1.773	0.05
		Ç	0.000			1/2" Ice	2.320	1.946	0.07
			0.000			1" Ice	2.512	2.127	0.09
de Arm Mount [SO 102-3]	C	None		0.000	154.000	No Ice	3.000	3.000	0.08
						1/2" Ice	3.480	3.480	0.11
						1" Ice	3.960	3.960	0.14
level 150									
APXVSPP18-C-A20 w/	A	From Leg	4.000	0.000	150.000	No Ice	8.262	6.946	0.08
Mount Pipe			0.000			1/2" Ice	8.822	8.127	0.15
	_		2.000			1" Ice	9.346	9.021	0.22
APXVSPP18-C-A20 w/	В	From Leg	4.000	0.000	150.000	No Ice	8.262	6.946	0.08
Mount Pipe			0.000			1/2" Ice	8.822	8.127	0.15
ADVAGDD10 G AGO	C	г .	2.000	0.000	150.000	1" Ice	9.346	9.021	0.22
APXVSPP18-C-A20 w/	C	From Leg	4.000	0.000	150.000	No Ice	8.262	6.946	0.08
Mount Pipe			0.000			1/2" Ice	8.822	8.127	0.15
ADVI/TM14 C 120/	C	Enoug I	2.000	0.000	150,000	1" Ice	9.346	9.021	0.22
APXVTM14-C-120 w/	C	From Leg	4.000	0.000	150.000	No Ice	6.580	4.959	0.07
			0.000 2.000			1/2" Ice 1" Ice	7.031 7.473	5.754 6.472	0.13 0.19
Mount Pipe						i ice	1413		0.19
•	Α.	Erom I as		0.000	150,000				
TD-RRH8x20-25	A	From Leg	4.000 0.000	0.000	150.000	No Ice 1/2" Ice	4.045 4.298	1.533 1.712	0.07 0.09

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

	Job	Page
	UPPER STEPNEY - TLC	8 of 17
	Project	Date
•	BU#876355 WO#1503489	14:07:52 12/27/17
	Client	Designed by
	Crown Castle	HolderKG

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		$C_A A_A$ Front	$C_A A_A$ Side	Weigh
	Leg		Lateral						
			Vert	0	£.		<i>c</i> .2	£2	ν
			ft ft	-	ft		ft^2	ft^2	K
			ft						
TD-RRH8x20-25	В	From Leg	4.000	0.000	150.000	No Ice	4.045	1.533	0.070
			0.000			1/2" Ice	4.298	1.712	0.097
TD DD11020-25	C	E I	4.000	0.000	150,000	1" Ice	4.557	1.899	0.128
TD-RRH8x20-25	C	From Leg	4.000 0.000	0.000	150.000	No Ice 1/2" Ice	4.045 4.298	1.533 1.712	0.070 0.097
			4.000			1" Ice	4.298	1.712	0.097
(3) ACU-A20-N	Α	From Leg	4.000	0.000	150.000	No Ice	0.067	0.117	0.001
(0)			0.000			1/2" Ice	0.104	0.162	0.002
			0.000			1" Ice	0.148	0.215	0.004
(3) ACU-A20-N	В	From Leg	4.000	0.000	150.000	No Ice	0.067	0.117	0.001
			0.000			1/2" Ice	0.104	0.162	0.002
	~		0.000			1" Ice	0.148	0.215	0.004
(3) ACU-A20-N	C	From Leg	4.000	0.000	150.000	No Ice	0.067	0.117	0.001
			0.000			1/2" Ice 1" Ice	0.104 0.148	0.162 0.215	0.002
(2) 6' x 2" Mount Pipe	A	From Leg	0.000 4.000	0.000	150.000	No Ice	1.425	1.425	0.004 0.022
(2) 6 x 2 Would Tipe	Λ	110III Leg	0.000	0.000	130.000	1/2" Ice	1.925	1.925	0.022
			2.000			1" Ice	2.294	2.294	0.048
(2) 6' x 2" Mount Pipe	В	From Leg	4.000	0.000	150.000	No Ice	1.425	1.425	0.022
r			0.000			1/2" Ice	1.925	1.925	0.033
			2.000			1" Ice	2.294	2.294	0.048
(2) 6' x 2" Mount Pipe	C	From Leg	4.000	0.000	150.000	No Ice	1.425	1.425	0.022
			0.000			1/2" Ice	1.925	1.925	0.033
	~		2.000			1" Ice	2.294	2.294	0.048
Transition Ladder	C	From Leg	2.000	0.000	150.000	No Ice	6.000	6.000	0.160
			0.000 -4.000			1/2" Ice 1" Ice	8.000 10.000	8.000 10.000	0.240
Platform Mount [LP 712-1]	C	None	-4.000	0.000	150.000	No Ice	24.530	24.530	0.320 1.335
Trationiii Wount [E1 /12-1]	C	None		0.000	130.000	1/2" Ice	29.940	29.940	1.646
						1" Ice	35.350	35.350	1.956
level 137									
7770.00 w/ Mount Pipe	Α	From Leg	4.000	0.000	137.000	No Ice	5.746	4.254	0.055
			0.000			1/2" Ice	6.179	5.014	0.103
			3.000			1" Ice	6.607	5.711	0.157
7770.00 w/ Mount Pipe	В	From Leg	4.000	0.000	137.000	No Ice	5.746	4.254	0.055
			0.000			1/2" Ice	6.179	5.014	0.103
7770 00 /M /P'	0	г т	3.000	0.000	127.000	1" Ice	6.607	5.711	0.157
7770.00 w/ Mount Pipe	C	From Leg	4.000 0.000	0.000	137.000	No Ice 1/2" Ice	5.746 6.179	4.254 5.014	0.055 0.103
			3.000			1" Ice	6.607	5.711	0.103
P65-16-XLH-RR w/ Mount	Α	From Leg	4.000	0.000	137.000	No Ice	8.371	6.362	0.137
Pipe	11	Trom Leg	0.000	0.000	137.000	1/2" Ice	8.931	7.538	0.144
F -			3.000			1" Ice	9.457	8.427	0.218
P65-16-XLH-RR w/ Mount	В	From Leg	4.000	0.000	137.000	No Ice	8.371	6.362	0.079
Pipe			0.000			1/2" Ice	8.931	7.538	0.144
			3.000			1" Ice	9.457	8.427	0.218
P65-16-XLH-RR w/ Mount	C	From Leg	4.000	0.000	137.000	No Ice	8.371	6.362	0.079
Pipe			0.000			1/2" Ice	8.931	7.538	0.144
(2) I CD21401		E	3.000	0.000	127 000	1" Ice	9.457	8.427	0.218
(2) LGP21401	A	From Leg	4.000	0.000	137.000	No Ice 1/2" Ice	1.104 1.239	0.207 0.274	0.014 0.021
			0.000 3.000			1/2" Ice 1" Ice	1.239	0.274	0.021
(2) LGP21401	В	From Leg	4.000	0.000	137.000	No Ice	1.104	0.348	0.030
(2) LGI 21701	ъ	110m Leg	0.000	0.000	137.000	1/2" Ice	1.239	0.274	0.014
			3.000			1" Ice	1.381	0.348	0.021
(2) LGP21401	C	From Leg	4.000	0.000	137.000	No Ice	1.104	0.207	0.014
* /			0.000			1/2" Ice	1.239	0.274	0.021

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

Acworth, GA 30102 Phone: 770-701-2500 FAX: 770-701-2501

Job		Page
	UPPER STEPNEY - TLC	9 of 17
Project		Date
	BU#876355 WO#1503489	14:07:52 12/27/17
Client		Designed by
	Crown Castle	HolderKG

Description	Face	Offset	Offsets:	Azimuth	Placement		$C_A A_A$	$C_A A_A$	Weight
	or	Type	Horz	Adjustment			Front	Side	
	Leg		Lateral						
			Vert	0			. 2	. 2	
			ft	0	ft		ft^2	ft^2	K
			ft						
			ft						
			3.000			1" Ice	1.381	0.348	0.030
RRUS-11	Α	From Leg	4.000	0.000	137.000	No Ice	2.522	1.068	0.055
			0.000			1/2" Ice	2.719	1.211	0.074
			3.000			1" Ice	2.923	1.361	0.097
RRUS-11	В	From Leg	4.000	0.000	137.000	No Ice	2.522	1.068	0.055
			0.000			1/2" Ice	2.719	1.211	0.074
			3.000			1" Ice	2.923	1.361	0.097
RRUS-11	C	From Leg	4.000	0.000	137.000	No Ice	2.522	1.068	0.055
			0.000			1/2" Ice	2.719	1.211	0.074
			3.000			1" Ice	2.923	1.361	0.097
DC6-48-60-18-8F	C	From Leg	4.000	0.000	137.000	No Ice	0.917	0.917	0.033
			0.000			1/2" Ice	1.458	1.458	0.051
			3.000			1" Ice	1.643	1.643	0.071
Platform Mount [LP 303-1]	C	None		0.000	137.000	No Ice	14.660	14.660	1.250
						1/2" Ice	18.870	18.870	1.481
						1" Ice	23.080	23.080	1.713

OG-860/1920/GPS-A	Α	From Leg	2.000	0.000	50.000	No Ice	0.308	0.367	0.002
			0.000			1/2" Ice	0.395	0.457	0.005
			2.000			1" Ice	0.490	0.555	0.010
Side Arm Mount [SO 701-1]	A	From Leg	1.500	0.000	50.000	No Ice	0.850	1.670	0.065
		_	0.000			1/2" Ice	1.140	2.340	0.079
			0.000			1" Ice	1.430	3.010	0.093

Load Combinations

Comb.	Description
No.	
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 30 deg - No Ice
5	0.9 Dead+1.6 Wind 30 deg - No Ice
6	1.2 Dead+1.6 Wind 60 deg - No Ice
7	0.9 Dead+1.6 Wind 60 deg - No Ice
8	1.2 Dead+1.6 Wind 90 deg - No Ice
9	0.9 Dead+1.6 Wind 90 deg - No Ice
10	1.2 Dead+1.6 Wind 120 deg - No Ice
11	0.9 Dead+1.6 Wind 120 deg - No Ice
12	1.2 Dead+1.6 Wind 150 deg - No Ice
13	0.9 Dead+1.6 Wind 150 deg - No Ice
14	1.2 Dead+1.6 Wind 180 deg - No Ice
15	0.9 Dead+1.6 Wind 180 deg - No Ice
16	1.2 Dead+1.6 Wind 210 deg - No Ice
17	0.9 Dead+1.6 Wind 210 deg - No Ice
18	1.2 Dead+1.6 Wind 240 deg - No Ice
19	0.9 Dead+1.6 Wind 240 deg - No Ice
20	1.2 Dead+1.6 Wind 270 deg - No Ice
21	0.9 Dead+1.6 Wind 270 deg - No Ice
22	1.2 Dead+1.6 Wind 300 deg - No Ice

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

Acworth, GA 30102 Phone: 770-701-2500 FAX: 770-701-2501

٦	Job	Page
	UPPER STEPNEY - TLC	10 of 17
	Project	Date
•	BU#876355 WO#1503489	14:07:52 12/27/17
	Client	Designed by
	Crown Castle	HolderKG

Comb.	Description
No.	·
23	0.9 Dead+1.6 Wind 300 deg - No Ice
24	1.2 Dead+1.6 Wind 330 deg - No Ice
25	0.9 Dead+1.6 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
No.	ft	Type		Load		Moment	Moment
				Comb.	K	kip-ft	kip-ft
L1	191.5 - 172.46	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-8.799	-0.033	-0.018
			Max. Mx	8	-3.221	-91.799	0.029
			Max. My	2	-3.222	-0.021	91.754
			Max. Vy	20	-5.827	91.762	0.027
			Max. Vx	14	5.823	-0.018	-91.686
			Max. Torque	10			0.045
L2	172.46 - 127.753	Pole	Max Tension	1	0.000	0.000	0.000
	127.755		Max. Compression	26	-39.209	0.612	-2.147
			Max. Mx	20	-17.016	636.599	1.533
			Max. My	14	-17.018	-1.513	-636.278
			Max. Vy	20	-19.826	636.599	1.533
			Max. Vx	14	19.811	-1.513	-636.278
			Max. Torque	25			-1.383
L3	127.753 - 83.0833	Pole	Max Tension	1	0.000	0.000	0.000
	03.0033		Max. Compression	26	-54.824	-0.219	-2.701
			Max. Mx	20	-28.696	1563.044	4.736
			Max. My	14	-28.697	-4.874	-1562.235
			Max. Vy	20	-22.943	1563.044	4.736
			Max. Vx	14	22.928	-4.874	-1562.235
			Max. Torque	22			0.491
L4	83.0833 - 40.4567	Pole	Max Tension	1	0.000	0.000	0.000

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

Acworth, GA 30102 Phone: 770-701-2500 FAX: 770-701-2501

	Job	Page
	UPPER STEPNEY - TLC	11 of 17
	Project	Date
•	BU#876355 WO#1503489	14:07:52 12/27/17
	Client	Designed by
	Crown Castle	HolderKG

Section	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
No.	ft	Type		Load		Moment	Moment
				Comb.	K	kip-ft	kip-ft
			Max. Compression	26	-75.455	-1.200	-2.778
			Max. Mx	20	-44.648	2577.178	8.054
			Max. My	14	-44.649	-8.097	-2575.589
			Max. Vy	20	-26.054	2577.178	8.054
			Max. Vx	14	26.013	-8.097	-2575.589
			Max. Torque	22			0.490
L5	40.4567 - 0	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-103.461	-2.486	-3.538
			Max. Mx	20	-66.977	3888.258	11.442
			Max. My	14	-66.977	-11.745	-3884.996
			Max. Vy	20	-29.020	3888.258	11.442
			Max. Vx	14	28.980	-11.745	-3884.996
			Max. Torque	12			-0.309

Maximum	Reactions
IVIGALILIGILI	INCACTIONS

Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, Z
		Load	K	K	K
		Comb.			
Pole	Max. Vert	31	103.461	-7.980	-4.605
	Max. H _x	20	66.990	28.990	0.071
	Max. H _z	2	66.990	0.071	28.950
	Max. M _x	2	3883.679	0.071	28.950
	Max. M _z	8	3887.550	-28.990	-0.071
	Max. Torsion	24	0.306	14.557	25.107
	Min. Vert	5	50.242	-14.434	25.036
	Min. H _x	8	66.990	-28.990	-0.071
	Min. H _z	14	66.990	-0.071	-28.950
	$Min. M_x$	14	-3884.996	-0.071	-28.950
	Min. M _z	20	-3888.258	28.990	0.071
	Min. Torsion	12	-0.309	-14.557	-25.107

Tower Mast Reaction Summary

Load Combination	Vertical	$Shear_x$	$Shear_z$	Overturning Moment, M _x	Overturning Moment, M ₂	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Dead Only	55.825	0.000	0.000	0.499	0.255	0.000
1.2 Dead+1.6 Wind 0 deg - No	66.990	-0.071	-28.950	-3883.679	12.459	-0.232
Ice						
0.9 Dead+1.6 Wind 0 deg - No	50.242	-0.071	-28.950	-3837.812	12.207	-0.214
Ice						
1.2 Dead+1.6 Wind 30 deg - No	66.990	14.434	-25.036	-3357.274	-1933.147	-0.099
Ice						
0.9 Dead+1.6 Wind 30 deg - No	50.242	14.434	-25.036	-3317.632	-1910.328	-0.087
Ice						
1.2 Dead+1.6 Wind 60 deg - No	66.990	25.071	-14.413	-1931.056	-3360.669	0.058
Ice						
0.9 Dead+1.6 Wind 60 deg - No	50.242	25.071	-14.413	-1908.332	-3320.921	0.061
Ice						
1.2 Dead+1.6 Wind 90 deg - No	66.990	28.990	0.071	12.762	-3887.550	0.200
Ice						
0.9 Dead+1.6 Wind 90 deg - No	50.242	28.990	0.071	12.431	-3841.572	0.194

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

Job		Page
	UPPER STEPNEY - TLC	12 of 17
Project		Date
,	BU#876355 WO#1503489	14:07:52 12/27/17
Client		Designed by
	Crown Castle	HolderKG

Load Combination	Vertical	$Shear_x$	$Shear_z$	Overturning Moment, M_x	Overturning Moment, M_z	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Ice 1.2 Dead+1.6 Wind 120 deg - No Ice	66.990	25.142	14.537	1953.308	-3372.722	0.292
0.9 Dead+1.6 Wind 120 deg - No Ice	50.242	25.142	14.537	1929.966	-3332.824	0.278
1.2 Dead+1.6 Wind 150 deg - No Ice	66.990	14.557	25.107	3370.646	-1954.079	0.309
No Ice 0.9 Dead+1.6 Wind 150 deg - No Ice	50.242	14.557	25.107	3330.499	-1930.997	0.290
1.2 Dead+1.6 Wind 180 deg - No Ice	66.990	0.071	28.950	3884.996	-11.745	0.241
0.9 Dead+1.6 Wind 180 deg - No Ice	50.242	0.071	28.950	3838.774	-11.691	0.223
1.2 Dead+1.6 Wind 210 deg - No Ice	66.990	-14.434	25.036	3358.589	1933.858	0.106
0.9 Dead+1.6 Wind 210 deg - No Ice	50.242	-14.434	25.036	3318.593	1910.842	0.094
1.2 Dead+1.6 Wind 240 deg - No Ice	66.990	-25.071	14.413	1932.372	3361.378	-0.060
0.9 Dead+1.6 Wind 240 deg - No Ice	50.242	-25.071	14.413	1909.294	3321.433	-0.063
1.2 Dead+1.6 Wind 270 deg - No Ice	66.990	-28.990	-0.071	-11.442	3888.258	-0.209
0.9 Dead+1.6 Wind 270 deg - No Ice	50.242	-28.990	-0.071	-11.467	3842.083	-0.202
1.2 Dead+1.6 Wind 300 deg - No Ice	66.990	-25.142	-14.537	-1951.987	3373.433	-0.299
0.9 Dead+1.6 Wind 300 deg - No Ice	50.242	-25.142	-14.537	-1929.001	3333.337	-0.284
1.2 Dead+1.6 Wind 330 deg - No Ice	66.990	-14.557	-25.107	-3369.326	1954.793	-0.306
0.9 Dead+1.6 Wind 330 deg - No Ice	50.242	-14.557	-25.107	-3329.535	1931.513	-0.287
1.2 Dead+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 0 deg+1.0	103.461 103.461	0.000 -0.011	0.000 -9.190	3.538 -1241.949	-2.486 -0.635	0.000 -0.260
Ice+1.0 Temp 1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	103.461	4.594	-7.954	-1074.069	-624.644	-0.164
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	103.461	7.969	-4.586	-617.372	-1081.975	-0.024
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	103.461	9.208	0.011	5.771	-1250.085	0.123
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	103.461	7.980	4.605	628.388	-1083.931	0.236
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	103.461	4.614	7.965	1083.652	-628.035	0.287
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	103.461	0.011	9.190	1249.574	-4.551	0.261
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	103.461	-4.594	7.954	1081.697	619.459	0.165
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	103.461	-7.969	4.586	625.000	1076.792	0.025
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	103.461	-9.208	-0.011	1.855	1244.903	-0.122
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	103.461	-7.980	-4.605	-620.764	1078.749	-0.235
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	103.461	-4.614	-7.965	-1076.028	622.850	-0.286
Dead+Wind 0 deg - Service	55.825	-0.017	-6.738 5.827	-897.421	3.086	-0.052
Dead+Wind 30 deg - Service Dead+Wind 60 deg - Service	55.825 55.825	3.360 5.836	-5.827 -3.355	-775.719 -446.017	-446.685 -776.689	-0.022 0.015

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

Acworth, GA 30102 Phone: 770-701-2500 FAX: 770-701-2501

Job		Page
	UPPER STEPNEY - TLC	13 of 17
Project		Date
	BU#876355 WO#1503489	14:07:52 12/27/17
Client		Designed by
Crown Castle		HolderKG

Load	Vertical	$Shear_x$	$Shear_z$	Overturning	Overturning	Torque
Combination				Moment, M_x	Moment, M_z	
	K	K	K	kip-ft	kip-ft	kip-ft
Dead+Wind 90 deg - Service	55.825	6.748	0.017	3.340	-898.502	0.047
Dead+Wind 120 deg - Service	55.825	5.852	3.384	451.948	-779.485	0.067
Dead+Wind 150 deg - Service	55.825	3.388	5.844	779.602	-451.528	0.069
Dead+Wind 180 deg - Service	55.825	0.017	6.738	898.509	-2.506	0.053
Dead+Wind 210 deg - Service	55.825	-3.360	5.827	776.806	447.264	0.022
Dead+Wind 240 deg - Service	55.825	-5.836	3.355	447.105	777.269	-0.015
Dead+Wind 270 deg - Service	55.825	-6.748	-0.017	-2.252	899.082	-0.048
Dead+Wind 300 deg - Service	55.825	-5.852	-3.384	-450.860	780.065	-0.067
Dead+Wind 330 deg - Service	55.825	-3.388	-5.844	-778.514	452.108	-0.069

Solution Summary

	Su	m of Applied Forces	S		Sum of Reaction	ıs	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
1	0.000	-55.825	0.000	0.000	55.825	0.000	0.000%
2	-0.071	-66.990	-28.950	0.071	66.990	28.950	0.000%
3	-0.071	-50.242	-28.950	0.071	50.242	28.950	0.000%
4	14.434	-66.990	-25.036	-14.434	66.990	25.036	0.000%
5	14.434	-50.242	-25.036	-14.434	50.242	25.036	0.000%
6	25.071	-66.990	-14.413	-25.071	66.990	14.413	0.000%
7	25.071	-50.242	-14.413	-25.071	50.242	14.413	0.000%
8	28.990	-66.990	0.071	-28.990	66.990	-0.071	0.000%
9	28.990	-50.242	0.071	-28.990	50.242	-0.071	0.000%
10	25.142	-66.990	14.537	-25.142	66.990	-14.537	0.000%
11	25.142	-50.242	14.537	-25.142	50.242	-14.537	0.000%
12	14.557	-66.990	25.107	-14.557	66.990	-25.107	0.000%
13	14.557	-50.242	25.107	-14.557	50.242	-25.107	0.000%
14	0.071	-66.990	28.950	-0.071	66.990	-28.950	0.000%
15	0.071	-50.242	28.950	-0.071	50.242	-28.950	0.000%
16	-14.434	-66.990	25.036	14.434	66.990	-25.036	0.000%
17	-14.434	-50.242	25.036	14.434	50.242	-25.036	0.000%
18	-25.071	-66.990	14.413	25.071	66.990	-14.413	0.000%
19	-25.071	-50.242	14.413	25.071	50.242	-14.413	0.000%
20	-28.990	-66.990	-0.071	28.990	66.990	0.071	0.000%
21	-28.990	-50.242	-0.071	28.990	50.242	0.071	0.000%
22	-25.142	-66.990	-14.537	25.142	66.990	14.537	0.000%
23	-25.142	-50.242	-14.537	25.142	50.242	14.537	0.000%
24	-14.557	-66.990	-25.107	14.557	66.990	25.107	0.000%
25	-14.557	-50.242	-25.107	14.557	50.242	25.107	0.000%
26	0.000	-103.461	0.000	-0.000	103.461	-0.000	0.000%
27	-0.011	-103.461	-9.190	0.011	103.461	9.190	0.000%
28	4.594	-103.461	-7.953	-4.594	103.461	7.954	0.000%
29	7.969	-103.461	-4.586	-7.969	103.461	4.586	0.000%
30	9.208	-103.461	0.011	-9.208	103.461	-0.011	0.000%
31	7.980	-103.461	4.605	-7.980	103.461	-4.605	0.000%
32	4.614	-103.461	7.965	-4.614	103.461	-7.965	0.000%
33	0.011	-103.461	9.190	-0.011	103.461	-9.190	0.000%
34	-4.594	-103.461	7.953	4.594	103.461	-7.954	0.000%
35	-7.969	-103.461	4.586	7.969	103.461	-4.586	0.000%
36	-9.208	-103.461	-0.011	9.208	103.461	0.011	0.000%
37	-7.980	-103.461	-4.605	7.980	103.461	4.605	0.000%
38	-4.614	-103.461	-7.965	4.614	103.461	7.965	0.000%
39	-0.017	-55.825	-6.738	0.017	55.825	6.738	0.000%
40	3.360	-55.825	-5.827	-3.360	55.825	5.827	0.000%
41	5.836	-55.825	-3.355	-5.836	55.825	3.355	0.000%
42	6.748	-55.825	0.017	-6.748	55.825	-0.017	0.000%
43	5.852	-55.825	3.384	-5.852	55.825	-3.384	0.000%

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

Acworth, GA 30102 Phone: 770-701-2500 FAX: 770-701-2501

Job		Page
	UPPER STEPNEY - TLC	14 of 17
Project		Date
	BU#876355 WO#1503489	14:07:52 12/27/17
Client		Designed by
	Crown Castle	HolderKG

	Sui	m of Applied Forces	S		Sum of Reaction	s	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
44	3.388	-55.825	5.844	-3.388	55.825	-5.844	0.000%
45	0.017	-55.825	6.738	-0.017	55.825	-6.738	0.000%
46	-3.360	-55.825	5.827	3.360	55.825	-5.827	0.000%
47	-5.836	-55.825	3.355	5.836	55.825	-3.355	0.000%
48	-6.748	-55.825	-0.017	6.748	55.825	0.017	0.000%
49	-5.852	-55.825	-3.384	5.852	55.825	3.384	0.000%
50	-3.388	-55.825	-5.844	3.388	55.825	5.844	0.000%

Non-Linear Convergence Results

Load	Converged?	Number	Displacement	Force
Combination	Convergeu:	of Cycles	Tolerance	Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	4	0.00000001	0.00050100
3	Yes	4	0.00000001	0.00030100
4	Yes	6	0.00000001	0.00020072
5	Yes	5	0.0000001	0.00047950
6	Yes	6	0.0000001	0.00047930
7	Yes	5	0.0000001	0.00047616
8	Yes	4	0.0000001	0.00047616
9	Yes	4		
10	Yes	6	0.00000001	0.00026032
			0.00000001	0.00005663
11	Yes	5	0.00000001	0.00048661
12	Yes	6	0.00000001	0.00005637
13	Yes	5	0.00000001	0.00048457
14	Yes	4	0.00000001	0.00050599
15	Yes	4	0.00000001	0.00021904
16	Yes	6	0.00000001	0.00005561
17	Yes	5	0.00000001	0.00047778
18	Yes	6	0.00000001	0.00005597
19	Yes	5	0.00000001	0.00048112
20	Yes	4	0.00000001	0.00046880
21	Yes	4	0.00000001	0.00017392
22	Yes	6	0.00000001	0.00005628
23	Yes	5	0.00000001	0.00048365
24	Yes	6	0.00000001	0.00005653
25	Yes	5	0.00000001	0.00048570
26	Yes	4	0.00000001	0.00001680
27	Yes	5	0.00000001	0.00050161
28	Yes	5	0.00000001	0.00066110
29	Yes	5	0.00000001	0.00066131
30	Yes	5	0.00000001	0.00050537
31	Yes	5	0.00000001	0.00067580
32	Yes	5	0.00000001	0.00067068
33	Yes	5	0.00000001	0.00050678
34	Yes	5	0.00000001	0.00066831
35	Yes	5	0.00000001	0.00066850
36	Yes	5	0.00000001	0.00050421
37	Yes	5	0.00000001	0.00066125
38	Yes	5	0.00000001	0.00066581
39	Yes	4	0.00000001	0.00004316
40	Yes	4	0.0000001	0.00004310
41	Yes	4	0.0000001	0.00027434
42	Yes	4	0.0000001	0.00027434
43	Yes	4	0.0000001	0.00004393
44	Yes	4	0.0000001	0.00028768
44	ies	4	0.0000001	0.00028403

tnxTower	Job	UPPER STEPNEY - TLC	Page 15 of 17
Jacobs Engineering Group, Inc. 5449 Bells Ferry Road	Project	BU#876355 WO#1503489	Date 14:07:52 12/27/17
Acworth, GA 30102 Phone: 770-701-2500 FAX: 770-701-2501	Client	Crown Castle	Designed by HolderKG

45	Yes	4	0.00000001	0.00004341
46	Yes	4	0.00000001	0.00027875
47	Yes	4	0.00000001	0.00028448
48	Yes	4	0.00000001	0.00004333
49	Yes	4	0.00000001	0.00028210
50	Yes	4	0.00000001	0.00028566

Maximum Tower Deflections - Service Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	۰
L1	191.5 - 172.46	27.410	44	1.473	0.000
L2	175.543 - 127.753	22.639	44	1.358	0.000
L3	132.253 - 83.0833	11.950	44	0.952	0.000
L4	88.9167 - 40.4567	5.034	49	0.559	0.000
L5	47.54 - 0	1.398	49	0.272	0.000

Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	٥	0	ft
192.000	(2) RV65-18-02DPL2	44	27.410	1.473	0.000	20699
191.500	Lightning Rod 5/8" x 5'	44	27.410	1.473	0.000	20699
160.000	(2) LPA-80063/6CF w/ Mount Pipe	44	18.372	1.223	0.000	6129
154.000	PCS 1900MHz 4x45W-65MHz	44	16.850	1.167	0.000	5991
150.000	APXVSPP18-C-A20 w/ Mount Pipe	44	15.875	1.128	0.000	5903
137.000	7770.00 w/ Mount Pipe	44	12.934	0.999	0.000	5634
50.000	OG-860/1920/GPS-A	49	1.537	0.287	0.000	7508

Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	٥
L1	191.5 - 172.46	118.489	22	6.376	0.002
L2	175.543 - 127.753	97.905	22	5.877	0.002
L3	132.253 - 83.0833	51.723	22	4.123	0.001
L4	88.9167 - 40.4567	21.795	22	2.424	0.000
L5	47.54 - 0	6.050	22	1.176	0.000

Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	0	ft

Jacobs Engineering Group, Inc. 5449 Bells Ferry Road

Acworth, GA 30102 Phone: 770-701-2500 FAX: 770-701-2501

Job		Page
	UPPER STEPNEY - TLC	16 of 17
Project		Date
	BU#876355 WO#1503489	14:07:52 12/27/17
Client		Designed by
	Crown Castle	HolderKG

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	0	ft
192.000	(2) RV65-18-02DPL2	22	118.489	6.376	0.002	4909
191.500	Lightning Rod 5/8" x 5'	22	118.489	6.376	0.002	4909
160.000	(2) LPA-80063/6CF w/ Mount Pipe	22	79.481	5.297	0.002	1442
154.000	PCS 1900MHz 4x45W-65MHz	22	72.905	5.052	0.001	1408
150.000	APXVSPP18-C-A20 w/ Mount Pipe	22	68.693	4.884	0.001	1386
137.000	7770.00 w/ Mount Pipe	22	55.981	4.327	0.001	1319
50.000	OG-860/1920/GPS-A	22	6.655	1.242	0.000	1735

Compression Checks

	Pole Design Data								
Section No.	Elevation	Size	L	L_u	Kl/r	A	P_u	ϕP_n	Ratio P _u
	ft		ft	ft		in^2	K	K	ϕP_n
L1	191.5 - 172.46 (1)	TP20.46x15.5x0.188	19.040	0.000	0.0	11.587	-3.222	852.683	0.004
L2	172.46 - 127.753 (2)	TP31.6x19.282x0.313	47.790	0.000	0.0	29.883	-17.008	2220.140	0.008
L3	127.753 - 83.0833 (3)	TP42.19x29.815x0.438	49.170	0.000	0.0	55.940	-28.691	4156.060	0.007
L4	83.0833 - 40.4567 (4)	TP52.59x39.847x0.5	48.460	0.000	0.0	73.118	-38.689	5432.330	0.007
L5	40.4567 - 0 (5)	TP62x49.727x0.5	47.540	0.000	0.0	97.601	-66.977	6834.140	0.010

	Pole Bending Design Data							
Section No.	Elevation	Size	M_{ux}	ϕM_{nx}	Ratio M _{ux}	M_{uy}	ϕM_{ny}	Ratio M _{uy}
	ft		kip-ft	kip-ft	ϕM_{nx}	kip-ft	kip-ft	ϕM_{ny}
L1	191.5 - 172.46 (1)	TP20.46x15.5x0.188	91.802	340.775	0.269	0.000	340.775	0.000
L2	172.46 - 127.753 (2)	TP31.6x19.282x0.313	638.001	1372.008	0.465	0.000	1372.008	0.000
L3	127.753 - 83.0833 (3)	TP42.19x29.815x0.438	1567.100	3432.575	0.457	0.000	3432.575	0.000
L4	83.0833 - 40.4567 (4)	TP52.59x39.847x0.5	2181.142	5131.458	0.425	0.000	5131.458	0.000
L5	40.4567 - 0 (5)	TP62x49.727x0.5	3897.517	8640.417	0.451	0.000	8640.417	0.000

			Pole Sh	ear Des	ign Da	ıta		
Section No.	Elevation	Size	Actual V _u	ϕV_n	Ratio V _u	Actual T _u	ϕT_n	Ratio T _u
	ft		K	K	ϕV_n	kip-ft	kip-ft	ϕT_n
L1	191.5 - 172.46	TP20.46x15.5x0.188	5.826	426.342	0.014	0.027	682.383	0.000

tnxTower Job UPPER STEPNEY - TLC Page Jacobs Engineering Group, Inc. 5449 Bells Ferry Road Acworth, GA 30102 Project Date 14:07:52 12/27/17 Designed by

Crown Castle

HolderKG

Section No.	Elevation	Size	Actual V_u	ϕV_n	Ratio V_u	Actual T _u	ϕT_n	Ratio T_u
	ft		K	K	ϕV_n	kip-ft	kip-ft	ϕT_n
L2	(1) 172.46 - 127.753 (2)	TP31.6x19.282x0.313	19.887	1110.070	0.018	0.492	2747.367	0.000
L3	127.753 - 83.0833 (3)	TP42.19x29.815x0.438	23.004	2078.030	0.011	0.490	6873.533	0.000
L4	83.0833 - 40.4567 (4)	TP52.59x39.847x0.5	25.087	2746.780	0.009	0.490	10275.500	0.000
L5	40.4567 - 0 (5)	TP62x49.727x0.5	29.072	3417.070	0.009	0.292	17302.000	0.000

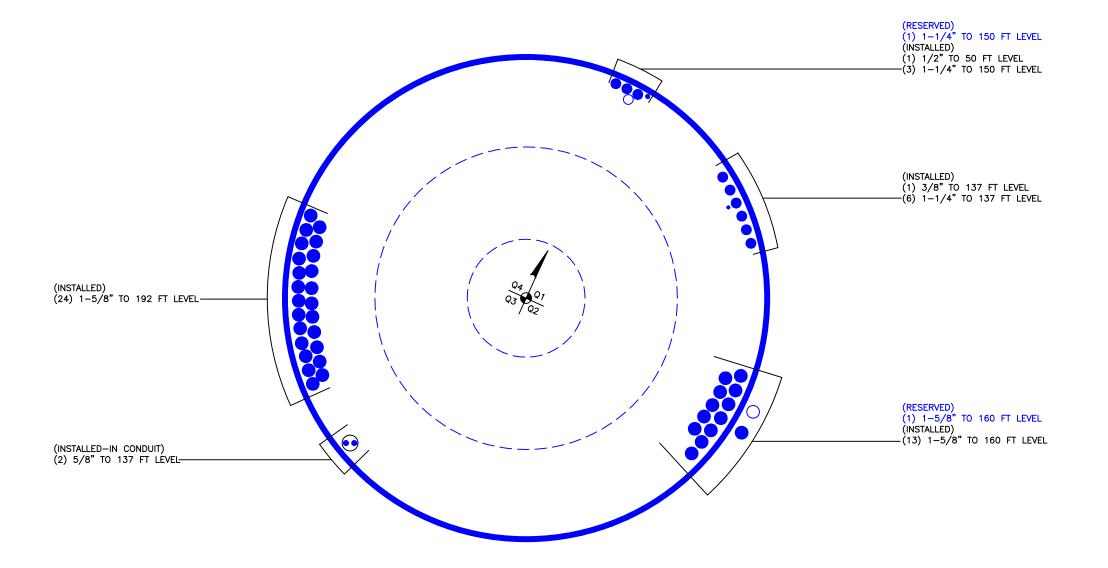
Phone: 770-701-2500 FAX: 770-701-2501

Section No.	Elevation	$Ratio$ P_u	$Ratio$ M_{ux}	$Ratio$ M_{uy}	$Ratio$ V_u	$Ratio$ T_u	Comb. Stress	Allow. Stress	Criteria
	ft	ϕP_n	ϕM_{nx}	ϕM_{ny}	ϕV_n	ϕT_n	Ratio	Ratio	
L1	191.5 - 172.46 (1)	0.004	0.269	0.000	0.014	0.000	0.273	1.000	4.8.2
L2	172.46 - 127.753 (2)	0.008	0.465	0.000	0.018	0.000	0.473	1.000	4.8.2
L3	127.753 - 83.0833 (3)	0.007	0.457	0.000	0.011	0.000	0.464	1.000	4.8.2
L4	83.0833 - 40.4567 (4)	0.007	0.425	0.000	0.009	0.000	0.432	1.000	4.8.2
L5	40.4567 - 0 (5)	0.010	0.451	0.000	0.009	0.000	0.461	1.000	4.8.2

Section Capacity Table								
Section No.	Elevation ft	Component Type	Size	Critical Element	P K	φP _{allow} K	% Capacity	Pass Fail
L1	191.5 - 172.46	Pole	TP20.46x15.5x0.188	1	-3.222	852.683	27.3	Pass
L2	172.46 - 127.753	Pole	TP31.6x19.282x0.313	2	-17.008	2220.140	47.3	Pass
L3	127.753 - 83.0833	Pole	TP42.19x29.815x0.438	3	-28.691	4156.060	46.4	Pass
L4	83.0833 - 40.4567	Pole	TP52.59x39.847x0.5	4	-38.689	5432.330	43.2	Pass
L5	40.4567 - 0	Pole	TP62x49.727x0.5	5	-66.977	6834.140	46.1 Summary	Pass
						Pole (L2)	47.3	Pass
						RATING =	47.3	Pass

APPENDIX B BASE LEVEL DRAWING





BUSINESS UNIT: 876355 TOWER ID: C_BASELEVEL

BASE LEVEL DRAWING

CROWN REGION ADDRESS

USA

BMH EAN SAC SAC WAWW WAWW CH

| 19/11/13 | UPDATED PER WORK ORDER # 678087 | 24/03/14 | UPDATED PER WORK ORDER # 732936 | 28/5/2014 | UPDATED PER WORK ORDER 773458 | 15/10/2014 | UPDATED PER WORK ORDER 1392509 | 12/04/17 | UPDATED PER WORK ORDER 1392509 | 1392576 | 28/07/17 | UPDATED PER WORK ORDER 13924194 | 09/08/17 | UPDATED PER WORK ORDER 1324194 | 08/08/17 | UPDATED PER WORK ORDER 1324194 | 08/08/17 | UPDATED PER WORK ORDER 1347644

DRAWN BY: **JEP**CHECKED BY: **JW**DRAWING DATE: 08/08/07

SITE NUMBER: SITE NAME:

SITE NAME

UPPER STEPNEY-TLC

BUSINESS UNIT NUMBER

876355

SITE ADDRESS

474-480 MAIN ST. MONROE, CT 06468 FAIRFILED COUNTY USA

SHEET TITLE

BASE LEVEL

SHEET NUMBER

APPENDIX C ADDITIONAL CALCULATIONS

Stiffened or Unstiffened, Ungrouted, Circular Base Plate - Any Rod Material

TIA Rev G Assumption: Clear space between bottom of leveling nut and top of concrete **not** exceeding (1)*(Rod Diameter)

Site Data

BU#: 876355

Site Name: UPPER STEPNEY - TLC

App #: 418243 Rev#0

Pole Manufacturer: Other

Reactions					
Mu:	3898	ft-kips			
Axial, Pu:	67	kips			
Shear, Vu:	29	kips			
Eta Factor, η	0.5	TIA G (Fig. 4-4)			

If No etiffeners Criteria:	AISC LRED	<-Only Applcable to Unstiffened Cases
ii ivo suireners, Chiena.	AIGC LKFD	<-Only Applicable to Unstillened Cases

Anchor Rod Data Qty: 24 Diam: 2.25 in Rod Material: A615-J Strength (Fu): 100 ksi Yield (Fy): 75 ksi **Bolt Circle:** 71 in

Plate Data					
Diam:	77	in			
Thick:	2.25	in			
Grade:	60	ksi			
Single-Rod B-eff:	8.20	in			

Stiffener Da	Stiffener Data (Welding at both sides)						
Config:	0	*					
Weld Type:							
Groove Depth:		in **					
Groove Angle:		degrees					
Fillet H. Weld:		< Disregard					
Fillet V. Weld:		in					
Width:		in					
Height:		in					
Thick:		in					
Notch:		in					
Grade:		ksi					
Weld str.:		ksi					

Pole Data					
Diam:	62	in			
Thick:	0.5	in			
Grade:	65	ksi			
# of Sides:	18	"0" IF Round			
Fu	80	ksi			
Reinf. Fillet Weld	0	"0" if None			

Base Plate Results	Flexural Check
Base Plate Stress:	30.0 ksi
Allowable Plate Stress:	54.0 ksi
Base Plate Stress Ratio:	55.5% Pass

Rigid	
AISC LRFD	
φ*Fy	
Y.L. Length:	
34.60	

Rigid

AISC LRFD

φ*Tn

115.0 Kips

260.0 Kips

44.2% Pass

<u>n/a</u>

Stiffener Results

Anchor Rod Results

Max Rod (Cu+ Vu/ή):

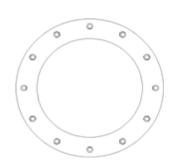
Allowable Axial, Φ*Fu*Anet:

Anchor Rod Stress Ratio:

Horizontal Weld: n/a
Vertical Weld: n/a
Plate Flex+Shear, fb/Fb+(fv/Fv)^2: n/a
Plate Tension+Shear, ft/Ft+(fv/Fv)^2 n/a
Plate Comp. (AISC Bracket): n/a

Pole Results

Pole Punching Shear Check: n/a





Analysis Date: 12/27/2017

^{* 0 =} none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

^{**} Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

Pier and Pad Foundation

BU #: 876355
Site Name: UPPER STEPNEY
App. Number: 418243 Rev#0



TIA-222 Revision: G
Tower Type: Monopole

Block Foundation?:	

Superstructure Analysis Reactions			
Compression, P _{comp} :	67	kips	
Base Shear, Vu_comp:	29	kips	
_			
Moment, M _u :	3898	ft-kips	
Tower Height, H:	191.5	ft	
BP Dist. Above Fdn, bp _{dist} :	2	in	

Foundation Analysis Checks					
Capacity Demand Rating					
Lateral (Sliding) (kips)	364.75	29.00	8.0%	Pass	
Bearing Pressure (ksf)	9.41	2.17	23.1%	Pass	
Overturning (kip*ft)	8404.27	4076.83	48.5%	Pass	
Pier Flexure (Comp.) (kip*ft)	6931.00	3985.00	57.5%	Pass	
Pier Compression (kip)	35802.00	97.38	0.3%	Pass	
Pad Flexure (kip*ft)	2756.54	1476.68	53.6%	Pass	
Pad Shear - 1-way (kips)	1075.81	191.70	17.8%	Pass	
Pad Shear - 2-way (ksi)	0.19	0.04	18.9%	Pass	

Pier Properties				
Pier Shape:	Square			
Pier Diameter, dpier :	7.5	ft		
Ext. Above Grade, E:	1.00	ft		
Pier Rebar Size, Sc :	8			
Pier Rebar Quantity, mc :	51			
Pier Tie/Spiral Size, St :	4			
Pier Tie/Spiral Quantity, mt:	8			
Pier Reinforcement Type:	Tie			
Pier Clear Cover, cc _{pier} :	5	in		

Soil Rating:	48.5%
Structural Rating:	57.5%

Pad Properties				
Depth, D :	5.0	ft		
Pad Width, W :	30.0	ft		
Pad Thickness, T :	3.0	ft		
Pad Rebar Size, Sp :	8			
Pad Rebar Quantity, mp :	25			
Pad Clear Cover, cc _{pad} :	3	in		

Material Properties			
Rebar Grade, Fy :	60000	psi	
Concrete Compressive Strength, F'c:	4000	psi	
Dry Concrete Density, δ c :	150	pcf	

Soil Properties				
Total Soil Unit Weight, γ :	110	pcf		
Ultimate Net Bearing, Qnet:	12.000	ksf		
Cohesion, Cu :	0.000	ksf		
Friction Angle, $oldsymbol{arphi}$:	30	degrees		
SPT Blow Count, N_{blows} :				
Base Friction, μ :	0.7			
Neglected Depth, N:	3.50	ft		
Foundation Bearing on Rock?	Yes			
Groundwater Depth, gw :	N/A	ft		

<--Toggle between Gross and Net



RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CT11264C

Monroe / Rt 59 / Rt 25 474-480 Main Street Monroe, CT 06468

January 8, 2018

EBI Project Number: 6218000059

Site Compliance Summary		
Compliance Status:	COMPLIANT	
Site total MPE% of FCC general population allowable limit:	6.32 %	



January 8, 2018

T-Mobile USA Attn: Jason Overbey, RF Manager 35 Griffin Road South Bloomfield, CT 06002

Emissions Analysis for Site: CT11264C - Monroe / Rt 59 / Rt 25

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **474-480 Main Street**, **Monroe**, **CT**, for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter (μ W/cm²). The number of μ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter (μ W/cm²). The general population exposure limit for the 700 MHz Band is approximately 467 μ W/cm², and the general population exposure limit for the 1900 MHz (PCS) and 2100 MHz (AWS) bands is 1000 μ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at **474-480 Main Street, Monroe, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 GSM channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) 2 LTE channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 3) 2 LTE channels (AWS Band 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel
- 4) 1 LTE channel (700 MHz Band) was considered for each sector of the proposed installation. This channel has a transmit power of 30 Watts.
- 5) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.



- 6) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antennas used in this modeling are the RFS APX16DWV-16DWVS-E-A20 for 1900 MHz (PCS) and 2100 MHz (AWS) channels and the RFS APXVAA24-43-U-A20 for 700 MHz channels. This is based on feedback from the carrier with regards to anticipated antenna selection. The RFS APX16DWV-16DWVS-E-A20 has a maximum gain of 16.3 dBd at its main lobe at 1900 MHz and 2100 MHz. The RFS APXVAA24-43-U-A20 has a maximum gain of 13.55 dBd at its main lobe at 700 MHz. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antenna mounting height centerlines of the proposed antennas are **194 feet & 192 feet** above ground level (AGL).
- 9) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 10) All calculations were done with respect to uncontrolled / general population threshold limits.



T-Mobile Site Inventory and Power Data

Sector:	A	Sector:	В	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	RFS APX16DWV- 16DWVS-E-A20	Make / Model:	RFS APX16DWV- 16DWVS-E-A20	Make / Model:	RFS APX16DWV- 16DWVS-E-A20
Gain:	16.3 dBd	Gain:	16.3 dBd	Gain:	16.3 dBd
Height (AGL):	194	Height (AGL):	194	Height (AGL):	194
Frequency Bands	1900 MHz (PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz (PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz (PCS) / 2100 MHz (AWS)
Channel Count	6	Channel Count	6	Channel Count	6
Total TX Power(W):	300	Total TX Power(W):	300	Total TX Power(W):	300
ERP (W):	12,572.17	ERP (W):	12,572.17	ERP (W):	12,572.17
Antenna A1 MPE%	1.28	Antenna B1 MPE%	1.28	Antenna C1 MPE%	1.28
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	RFS APXVAA24- 43-U-A20	Make / Model:	RFS APXVAA24- 43-U-A20	Make / Model:	RFS APXVAA24- 43-U-A20
Gain:	13.55 dBd	Gain:	13.55 dBd	Gain:	13.55 dBd
Height (AGL):	192	Height (AGL):	192	Height (AGL):	192
Frequency Bands	700 MHz	Frequency Bands	700 MHz	Frequency Bands	700 MHz
Channel Count	1	Channel Count	1	Channel Count	1
Total TX Power(W):	30	Total TX Power(W):	30	Total TX Power(W):	30
ERP (W):	679.39	ERP (W):	679.39	ERP (W):	679.39
Antenna A2 MPE%	0.15	Antenna B2 MPE%	0.15	Antenna C2 MPE%	0.15

Site Composite MPE%				
Carrier	MPE%			
T-Mobile (Per Sector Max)	1.43 %			
AT&T	1.09 %			
Sprint	0.58 %			
Verizon Wireless	3.22 %			
Site Total MPE %:	6.32 %			

T-Mobile Sector A Total:	1.43 %
T-Mobile Sector B Total:	1.43 %
T-Mobile Sector C Total:	1.43 %
Site Total:	6.32 %

T-Mobile _Max Values per sector	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm²)	Frequency (MHz)	Allowable MPE (µW/cm²)	Calculated % MPE
T-Mobile AWS - 2100 MHz LTE	2	2,559.48	194	5.21	AWS - 2100 MHz	1000	0.52%
T-Mobile PCS - 1900 MHz LTE	2	2,559.48	194	5.21	PCS - 1900 MHz	1000	0.52%
T-Mobile PCS - 1900 MHz GSM	2	1,167.14	194	2.37	PCS - 1900 MHz	1000	0.24%
T-Mobile 700 MHz LTE	1	679.39	192	0.71	700 MHz	467	0.15%
						Total:	1.43%



Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)	
Sector A:	1.43 %	
Sector B:	1.43 %	
Sector C:	1.43 %	
T-Mobile Per Sector	1.43 %	
Maximum:	1.45 %	
Site Total:	6.32 %	
Site Compliance Status:	COMPLIANT	

The anticipated composite MPE value for this site assuming all carriers present is **6.32%** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.