



Together with Nextel

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Real Estate Consultant

March, 31st 2014

Hand Delivered

Ms. Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

RE: Sprint Spectrum L.P. notice of intent to modify an existing telecommunications facility located at 430 Boston Post Rd, Milford, CT 06461. Known to Sprint Spectrum L.P. as site CT23XC552.

Dear Ms. Roberts:

In order to accommodate technological changes, implement Code Division Multiple Access (“CDMA”) and/or Long Term Evolution (“LTE”) capabilities, and enhance system performance in the state of Connecticut, Sprint Spectrum L.P. plans to modify the equipment configurations at many of its existing cell sites. Please accept this letter and attachments as notification, pursuant to R.C.S.A. Section 16-50j-73, of construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2). In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter and its attachments is being sent to the chief elected official of the municipality in which affected cell site is located.

CDMA employs Spread-Spectrum technology and special coding scheme to allow multiple users to be multiplexed over the same physical channel. LTE is a new high-performance air interface for cellular mobile communications. It is designed to increase the capacity and speed of mobile telephone networks.

As part of the project the new multi-mode 800/1900 antenna will replace existing antennas. These antennas will provide more flexibility for optimization by allowing fast and easy electrical tilt adjustment from remote location and will enable the transmission of multiple technologies from a single antenna. As Sprint Nextel’s network evolves to meet the demands of its customers, it is essential for Sprint Nextel to install modern

equipment and antennas in order to provide reliable wireless voice and data services. The proposed equipment will include multi-mode radios that will allow Sprint Nextel to transmit at different frequencies using different technologies, including LTE technology. Likewise, the proposed antennas are quad-pole multi-band high gain antennas that will allow Sprint to operate using its multiple frequency bands and technologies, including LTE technology. The proposed equipment and antennas will improve the reliability, coverage and capacity of Sprint Nextel's voice and data networks across Sprint Nextel's various FCC licensed frequency bands and significantly increase the data speeds of Sprint Nextel's network by utilizing the latest LTE technology. Without the proposed modifications Sprint Nextel will be unable to provide reliable wireless voice and data service using the latest technologies.

Sprint Spectrum L.P. will have an interim (testing) period during the modification/installation prior to the final configuration. This antenna configuration is shown on the attached drawings of the planned modifications. Also included is the power density calculation reflecting the change in Sprint's operations at the site and documentation of the structural sufficiency of the tower to accommodate the revised antenna configuration.

The changes to the facility do not constitute modification as defined Connecticut General Statutes ("C.G.S.") Section 16-50i(d) because the general physical characteristics of the facility will not be significantly changed or altered. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for the R.C.S.A. Section 16-50j-72(b)(2).

1. The height of the overall structure will not be affected.
2. The proposed changes will not extend the site boundaries. There will be no effect on the site compound.
3. The proposed changes will not increase the noise level at the existing facility by 6 decibels or more.
4. Radio Frequency power density may increase due to the use of one or more CDMA transmissions. Moreover, LTE will utilize additional radio frequencies newly licensed by the FCC for cellular mobile communications. However, the changes will not increase the calculated "worst case" power density for the combined operations at the site to a level at or above the applicable standard for uncontrolled environments as calculated for a mixed frequency site.

For the foregoing reasons Sprint Spectrum L.P. respectfully submits that the proposed changes at the referenced site constitute exempt modifications under R.C.S.A. Section 16-50j-72(b)(2).

Please feel free to call me at (845)-499-4712 or email jnotaro@Transcendwireless.com with questions concerning this matter. Thank you for your consideration.

Sincerely,

Jennifer Notaro
Real Estate Consultant

RADIO FREQUENCY EMISSIONS ANALYSIS REPORT
EVALUATION OF HUMAN EXPOSURE POTENTIAL
TO NON-IONIZING EMISSIONS

Sprint Existing Facility

Site ID: CT23XC552

Milford AT&T
430 Boston Post Road
Milford, CT 06460

August 28, 2012

August 28, 2012

Sprint
Attn: RF Engineering Manager
1 International Boulevard, Suite 800
Mahwah, NJ 07495

Re: Emissions Values for Site **CT23XC552 – Milford AT&T**

EBI Consulting was directed to analyze the proposed upgrades to the existing Sprint facility located at 430 Boston Post Road, Milford, CT, for the purpose of determining whether the emissions from the proposed Sprint equipment upgrades on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limit for the cellular band is approximately $567 \mu\text{W}/\text{cm}^2$, and the general population exposure limit for the PCS band is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed upgrades to the existing Sprint Wireless antenna facility located at 430 Boston Post Road, Milford, CT, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. All calculations were performed assuming the main lobe of the antenna was focused at the base of the tower to present a worst case scenario. Actual values seen from this site will be dramatically less than those shown in this report. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all emissions were calculated using the following assumptions:

- 1) 3 CDMA Carriers (1900 MHz) were considered for each sector of the proposed installation.
- 2) 1 CDMA Carrier (850 MHz) was considered for each sector of the proposed installation
- 3) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 4) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The actual gain in this direction was used per the manufactures supplied specifications.
- 5) The antenna used in this modeling is the RFS APXVSP18-C-A20. This is based on feedback from the carrier with regards to anticipated antenna selection. This antenna has a 15.9 dBd gain value at its main lobe at 1900 MHz and 13.4 dBd at its main lobe for 850 MHz. All calculations were performed assuming the main lobe of the antenna was focused at the base of the tower to present a worst case scenario.

- 6) The antenna mounting height centerline of the proposed antennas is **100.3 feet** above ground level (AGL)
- 7) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculation were done with respect to uncontrolled / general public and Controlled / Occupational threshold limits

Site ID	CT23XC552 - Milford AT&T
Site Address	430 Boston Post Road, Milford, CT 06460
Site Type	Self Support Tower

Sector 1

Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain in direction of sample point (dBd)	Antenna Height (ft)	analysis height	Cable Size	Cable Loss (dB)	Additional Loss	ERP	Power Density Value	Power Density Percentage
1a	RFS	APXVSP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	3	60	15.9	100.3	94.3	1/2 "	0.5	0	2080.4211	84.10733	8.41073%
1a	RFS	APXVSP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	13.4	100.3	94.3	1/2 "	0.5	0	389.96892	15.76568	2.78054%
Sector total Power Density Value:																11.191%	

Sector 2

Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain in direction of sample point (dBd)	Antenna Height (ft)	analysis height	Cable Size	Cable Loss (dB)	Additional Loss	ERP	Power Density Value	Power Density Percentage
2a	RFS	APXVSP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	3	60	15.9	100.3	94.3	1/2 "	0.5	0	2080.4211	84.10733	8.41073%
2a	RFS	APXVSP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	13.4	100.3	94.3	1/2 "	0.5	0	389.96892	15.76568	2.78054%
Sector total Power Density Value:																11.191%	

Sector 3

Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain in direction of sample point (dBd)	Antenna Height (ft)	analysis height	Cable Size	Cable Loss (dB)	Additional Loss	ERP	Power Density Value	Power Density Percentage
3a	RFS	APXVSP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	3	60	15.9	100.3	94.3	1/2 "	0.5	0	2080.4211	84.10733	8.41073%
3a	RFS	APXVSP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	13.4	100.3	94.3	1/2 "	0.5	0	389.96892	15.76568	2.78054%
Sector total Power Density Value:																11.191%	

Site Composite MPE %	
Carrier	MPE %
Sprint	33.574%
Town	0.300%
T-Mobile	7.350%
Pocket	10.640%
XM	21.120%
Clearwire	3.300%
Verizon Wireless	26.540%
AT&T	12.270%
Total Site MPE %	115.094%

Summary

All calculations performed for this analysis yielded results that were above the allowable limits for general public exposure to RF Emissions. However, the area surrounding the tower is a controlled fenced compound, occupational threshold limits would apply to this area.

The anticipated Maximum Composite contributions from the Sprint facility are **33.574% (11.191% from each sector)** of the allowable FCC established general public limit considering all three sectors simultaneously sampled at the ground level. This is equal to **6.715% (2.238% from each sector)** of the allowable FCC established occupational limit considering all three sectors simultaneously sampled at the ground level

The anticipated composite MPE value for this site assuming all carriers present is **115.094%** of the allowable FCC established general public limit sampled at the ground level. This is equal to **23.019%** of the allowable FCC established occupational limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. Although values could potentially exceed the FCC established general public limit at the base of the tower, this area is well within the FCC established occupational limit for this same area and should be considered in compliance since it is a controlled area.



Scott Heffernan
RF Engineering Director

EBI Consulting
21 B Street
Burlington, MA 01803

Date: **March 07, 2014**

Patrick Byrum
Crown Castle
3530 Toringdon Way, Suite 300
Charlotte, NC 28277



Crown Castle
2000 Corporate Drive
Canonsburg, PA
(724) 416-2000

Subject: Structural Analysis Report

Carrier Designation: **Sprint PCS Co-Locate**
Carrier Site Number: CT23XC552
Carrier Site Name: Milford / AT&T

Crown Castle Designation: **Crown Castle BU Number:** 842870
Crown Castle Site Name: MILFORD
Crown Castle JDE Job Number: 262584
Crown Castle Work Order Number: 717495
Crown Castle Application Number: 220333 Rev. 4

Engineering Firm Designation: **Crown Castle Project Number:** 717495

Site Data: **434 BOSTON POST ROAD, MILFORD, New Haven County, CT**
Latitude 41° 13' 42.7", Longitude -73° 4' 12.5"
150 Foot - Self Support Tower

Dear Patrick Byrum,

Crown Castle is pleased to submit this "**Structural Analysis Report**" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 717495, in accordance with application 220333, revision 4.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment

Sufficient Capacity

Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

The analysis has been performed in accordance with the TIA/EIA-222-F standard and 2005 CT State Building Code with 2009 amendment based upon a wind speed of 85 mph fastest mile.

All modifications and equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

We at *Crown Castle* appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Structural analysis prepared by: Melanie Perna, E.I.T. / CMS

Respectfully submitted by:

Jamal A. Huwel, P.E.
Manager Engineering

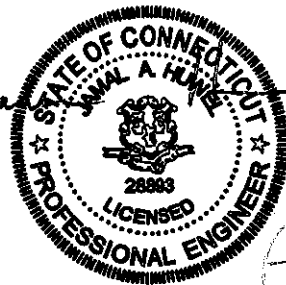


TABLE OF CONTENTS

1) INTRODUCTION

2) ANALYSIS CRITERIA

Table 1 - Proposed Antenna and Cable Information

Table 2 - Existing and Reserved Antenna and Cable Information

Table 3 - Design Antenna and Cable Information

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

3.1) Analysis Method

3.2) Assumptions

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Table 6 – Tower Components vs. Capacity

4.1) Recommendations

5) APPENDIX A

tnxTower Output

6) APPENDIX B

Base Level Drawing

7) APPENDIX C

Additional Calculations

1) INTRODUCTION

This tower is a 150 ft Self Support tower designed by PiROD Inc. in March of 2000. The tower was originally designed for a wind speed of 85 mph per TIA/EIA-222-F.

2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 85 mph with no ice, 37.6 mph with 0.75 inch ice thickness and 50 mph under service loads.

Table 1 - Proposed Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
100.0	100.0	3	alcatel lucent	800MHz 2X50W RRH W/FILTER	3	1-1/4	-
		3	alcatel lucent	PCS 1900MHz 2x40W			
		6	rfs celwave	APXVSP18-C-A20 w/ Mount Pipe			

Table 2 - Existing and Reserved Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
151.0	151.0	3	scala	PR-950	3	1-5/8	1
		1	tower mounts	Platform Mount [LP 405-1]			
141.0	141.0	3	ericsson	RRUS 11-700	3	1-1/4	2
		3	ericsson	RRUS 11-AWS			
		1	raycap	DC6-48-60-18-8F			
		3	kmw communications	AM-X-CD-14-65-00T-RET w/ Mount Pipe	12	1-5/8	1
		6	powerwave technologies	7770.00 w/ Mount Pipe			
		6	powerwave technologies	LGP21401			
1	tower mounts	Sector Mount [SM 411-3]					
133.0	138.0	2	generic	10' x 2" Omni	2	1-5/8	1
	133.0	2	tower mounts	Side Arm Mount [SO 301-1]			
127.0	137.0	1	generic	20" x 2" Omni	1	1-5/8	1
	127.0	1	tower mounts	Side Arm Mount [SO 301-1]			
117.0	117.0	1	generic	1 sq. ft antenna	2	1-5/8	1
		1	tower mounts	Side Arm Mount [SO 301-1]			
112.0	112.0	1	tower mounts	Sector Mount [SM 307-3]	18	1-5/8	1
	109.0	6	andrew	ETW200VS12UB			
		6	ems wireless	RR90-17-02DP			
		3	rfs celwave	APX16DWV-16DWVS-C			

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
100.0	100.0	9	andrew	HBX-6516DS-R2M w/ Mount Pipe	3	1-1/4	3
		2	cci	CE-1819-200MC			
		1	tower mounts	Sector Mount [SM 407-3]	6	1-1/4	1
88.0	90.0	6	andrew	ETB19G8-12UB	12	1-5/8	1
		6	antel	LPA-185063/8CFx2 w/ Mount Pipe			
		6	antel	LPA-80063/4CF w/ Mount Pipe			
	88.0	1	tower mounts	Sector Mount [SM 408-3]			
85.0	85.0	1	allgon	7130.14.05.00 w/ Mount Pipe	1	1-5/8	1
		1	tower mounts	Side Arm Mount [SO 301-1]			
65.0	65.0	3	rfs celwave	APXV18-206517S-C w/ Mount Pipe	6	1-5/8	1
50.0	50.0	1	pctel	GPS-TMG-26N	1	1/2	1
43.0	43.0	1	gabriel electronics	SSP2-23	1	7/8	1
		1	tower mounts	Side Arm Mount [SO 301-1]			

Notes:

- 1) Existing Equipment
- 2) Reserved Equipment
- 3) Equipment to be Removed; Not Considered in Analysis

Table 3 - Design Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
150	150	4	celwave	PD201	7	1-5/8
		3	scala	PR950		
140	140	12	allgon	7184	12	1-5/8
125	125	1	celwave	PD201	1	1-5/8
115	115	1	celwave	PD201	2	1-5/8
		1	celwave	PD220-DT		
110	110	12	allgon	7184	12	1-5/8
100	100	12	allgon	7184	12	1-5/8
90	90	12	allgon	7184	12	1-5/8
80	80	12	allgon	7184	12	1-5/8

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Clarence Welti Associates, Inc.	4529406	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	PIROD	4529405	CCISITES
4-TOWER MANUFACTURER DRAWINGS	PIROD	4480661	CCISITES
4-TOWER STRUCTURAL ANALYSIS REPORTS	B&T Engineering	4529407	CCISITES

3.1) Analysis Method

tnxTower (version 6.1.4.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by TIA/EIA-222-F.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	150 - 130	Leg	1 1/2	1	19.75	33.50	59.0	Pass
T2	130 - 110	Leg	2	67	-58.97	97.25	60.6	Pass
T3	110 - 100	Leg	Pirod 105244	131	-60.65	122.94	49.3	Pass
T4	100 - 80	Leg	Pirod 105216	143	-95.66	122.94	77.8	Pass
T5	80 - 60	Leg	Pirod 105217	164	-138.99	184.67	75.3	Pass
T6	60 - 40	Leg	Pirod 105218	182	-174.50	258.24	67.6	Pass
T7	40 - 20	Leg	Pirod 105218	197	-208.26	258.24	80.6	Pass
T8	20 - 0	Leg	Pirod 105219	212	-239.75	343.62	69.8	Pass
T1	150 - 130	Diagonal	3/4	10	-2.67	4.34	61.5	Pass
T2	130 - 110	Diagonal	7/8	76	-3.69	6.87	53.7	Pass
T3	110 - 100	Diagonal	L2 1/2x2 1/2x3/16	137	-7.12	12.23	58.2 59.7 (b)	Pass
T4	100 - 80	Diagonal	L2 1/2x2 1/2x3/16	149	-9.57	9.65	99.2	Pass
T5	80 - 60	Diagonal	L3x3x3/16	170	-7.16	13.37	53.5 64.1 (b)	Pass
T6	60 - 40	Diagonal	L3x3x3/16	188	-7.18	10.67	67.3	Pass
T7	40 - 20	Diagonal	L3x3x5/16	203	-7.75	13.74	56.4	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail	
T8	20 - 0	Diagonal	L3x3x5/16	218	-9.17	11.34	80.9	Pass	
T1	150 - 130	Horizontal	3/4	23	-0.26	2.43	10.8	Pass	
T2	130 - 110	Horizontal	3/4	122	-0.72	2.26	31.7	Pass	
T4	100 - 80	Horizontal	L3x3x3/16	153	-4.55	15.58	29.2 50.5 (b)	Pass	
T1	150 - 130	Top Girt	7/8	5	-0.44	5.37	8.2	Pass	
T2	130 - 110	Top Girt	7/8	70	-1.33	4.30	31.0	Pass	
T3	110 - 100	Top Girt	L3x3x3/16	132	-0.67	17.74	3.8 8.2 (b)	Pass	
T4	100 - 80	Top Girt	L3x3x3/16	144	-3.83	17.44	22.0 42.2 (b)	Pass	
T5	80 - 60	Top Girt	L3x3x3/16	165	-4.07	12.35	32.9 43.8 (b)	Pass	
T1	150 - 130	Bottom Girt	7/8	7	-1.22	4.27	28.5	Pass	
T2	130 - 110	Bottom Girt	7/8	71	-1.15	3.49	32.9	Pass	
							Summary		
							Leg (T7)	80.6	Pass
							Diagonal (T4)	99.2	Pass
							Horizontal (T4)	50.5	Pass
							Top Girt (T5)	43.8	Pass
							Bottom Girt (T2)	32.9	Pass
							Bolt Checks	79.4	Pass
							Rating =	99.2	Pass

Table 6 - Tower Component Stresses vs. Capacity – LC7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
-	Anchor Rods	0	43.1	Pass
1, 2	Base Foundation (Compared w/ Design Loads)	0	92.4	Pass

Structure Rating (max from all components) =	99.2%
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Notes:

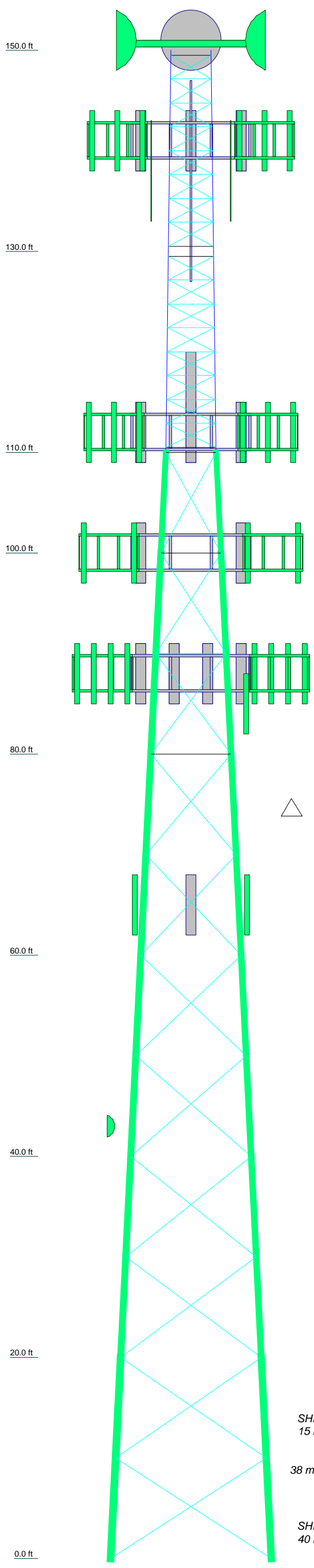
- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.
- 2) Foundation capacity determined by comparing analysis reactions to original design reactions.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the existing, reserved, and proposed loads. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

Section	T1	T2	T3	T4	T5	T6	T7	T8
Legs	SR 1 1/2	SR 2	Pirod 105244	Pirod 105216	Pirod 105217	Pirod 105218	Pirod 105219	
Leg Grade	SR 3/4	SR 7/8	L2 1/2x2 1/2x3/16	A572-50	L3x3x3/16	A36	L3x3x5/16	
Diagonals	A572-50							
Diagonal Grade	SR 7/8	SR 7/8						
Top Girts	SR 7/8	SR 7/8						
Bottom Girts	SR 3/4	SR 3/4	N.A.					
Horizontals	4.5	16 @ 2.375	5	6	8	10	11 @ 10	12
Face Width (ft)	1.3	1.1	2.0	2.5	2.9	3.4	4.6	
# Panels @ (ft)	0.8							
Weight (K)								



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
PR-950	151	(2) RR90-17-02DP	112
PR-950	151	APX16DWV-16DWVS-C	112
PR-950	151	APX16DWV-16DWVS-C	112
Platform Mount [LP 405-1]	151	APX16DWV-16DWVS-C	112
(3) 6' x 2" Mount Pipe	151	(2) ETW200VS12UB	112
(3) 6' x 2" Mount Pipe	151	(2) ETW200VS12UB	112
(3) 6' x 2" Mount Pipe	151	(2) ETW200VS12UB	112
(2) 7770.00 w/ Mount Pipe	141	Sector Mount [SM 307-3]	112
(2) 7770.00 w/ Mount Pipe	141	800MHz 2X50W RRH W/FILTER	100
(2) 7770.00 w/ Mount Pipe	141	PCS 1900MHz 2x40W	100
(2) LGP21401	141	(2) APXVSP18-C-A20 w/ Mount Pipe	100
(2) LGP21401	141	800MHz 2X50W RRH W/FILTER	100
(2) LGP21401	141	PCS 1900MHz 2x40W	100
Sector Mount [SM 411-3]	141	(2) APXVSP18-C-A20 w/ Mount Pipe	100
AM-X-CD-14-65-00T-RET w/ Mount Pipe	141	800MHz 2X50W RRH W/FILTER	100
AM-X-CD-14-65-00T-RET w/ Mount Pipe	141	PCS 1900MHz 2x40W	100
AM-X-CD-14-65-00T-RET w/ Mount Pipe	141	(2) APXVSP18-C-A20 w/ Mount Pipe	100
RRUS 11-700	141	Sector Mount [SM 407-3]	100
RRUS 11-700	141	(2) LPA-80063/4CF w/ Mount Pipe	88
RRUS 11-700	141	(2) LPA-80063/4CF w/ Mount Pipe	88
RRUS 11-AWS	141	(2) LPA-80063/4CF w/ Mount Pipe	88
RRUS 11-AWS	141	(2) LPA-185063/8CFx2 w/ Mount Pipe	88
RRUS 11-AWS	141	(2) LPA-185063/8CFx2 w/ Mount Pipe	88
DC6-48-60-18-8F	141	(2) LPA-185063/8CFx2 w/ Mount Pipe	88
6' x 2" Mount Pipe	141	(2) ETB19G8-12UB	88
6' x 2" Mount Pipe	141	(2) ETB19G8-12UB	88
6' x 2" Mount Pipe	141	(2) ETB19G8-12UB	88
10' x 2" Omni	133	Sector Mount [SM 408-3]	88
10' x 2" Omni	133	7130.14.05.00 w/ Mount Pipe	85
Side Arm Mount [SO 301-1]	133	Side Arm Mount [SO 301-1]	85
Side Arm Mount [SO 301-1]	133	APXV18-206517S-C w/ Mount Pipe	65
20' x 2" Omni	127	APXV18-206517S-C w/ Mount Pipe	65
Side Arm Mount [SO 301-1]	127	APXV18-206517S-C w/ Mount Pipe	65
1 sq. ft antenna	117	GPS-TMG-26N	50
Side Arm Mount [SO 301-1]	117	Side Arm Mount [SO 301-1]	43
(2) RR90-17-02DP	112	SSP2-23	43
(2) RR90-17-02DP	112		

MATERIAL STRENGTH

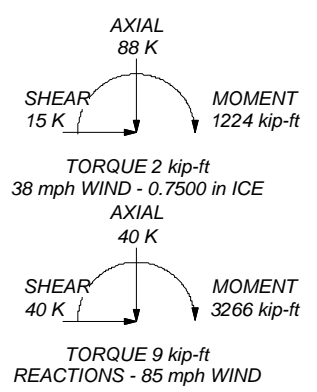
GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

TOWER DESIGN NOTES

1. Tower is located in New Haven County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 38 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 99.2%

MAX. CORNER REACTIONS AT BASE:
 DOWN: 249 K
 SHEAR: 27 K

UPLIFT: -217 K
 SHEAR: 24 K



<p>Crown Castle 2000 Corporate Drive Canonsburg, PA 15317 We Are Solutions Phone: (724) 416-2000 FAX:</p>	Job: BU# 842870
	Project:
	Client: Crown Castle
	Code: TIA/EIA-222-F
Path: R:\SA Models - Letters\Work Area\MPerna\842870\temp\842870.enr	
Drawn by: cschanck	App'd:
Date: 03/06/14	Scale: NTS
Dwg No. E-1	

Tower Input Data

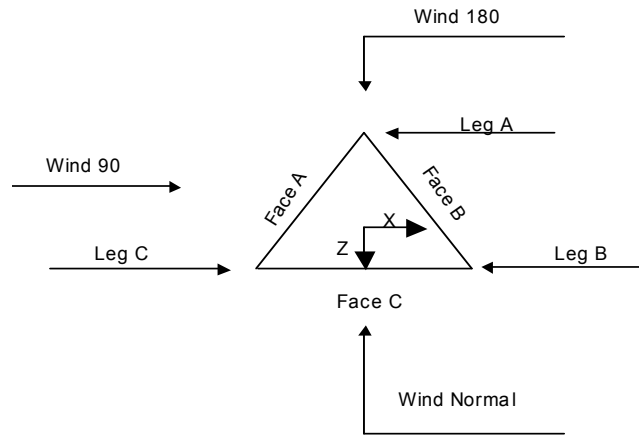
The main tower is a 3x free standing tower with an overall height of 150' above the ground line.
 The base of the tower is set at an elevation of 0' above the ground line.
 The face width of the tower is 4' at the top and 16' at the base.
 This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

- 1) Tower is located in New Haven County, Connecticut.
- 2) Basic wind speed of 85 mph.
- 3) Nominal ice thickness of 0.7500 in.
- 4) Ice thickness is considered to increase with height.
- 5) Ice density of 56 pcf.
- 6) A wind speed of 38 mph is used in combination with ice.
- 7) Temperature drop of 50 °F.
- 8) Deflections calculated using a wind speed of 50 mph.
- 9) Pressures are calculated at each section.
- 10) Stress ratio used in tower member design is 1.333.
- 11) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification ✓ Use Code Stress Ratios ✓ Use Code Safety Factors - Guys ✓ Escalate Ice Always Use Max Kz Use Special Wind Profile ✓ Include Bolts In Member Capacity Leg Bolts Are At Top Of Section ✓ Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) Add IBC .6D+W Combination	Distribute Leg Loads As Uniform Assume Legs Pinned ✓ Assume Rigid Index Plate ✓ Use Clear Spans For Wind Area ✓ Use Clear Spans For KL/r Retension Guys To Initial Tension ✓ Bypass Mast Stability Checks ✓ Use Azimuth Dish Coefficients ✓ Project Wind Area of Appurt. Autocalc Torque Arm Areas SR Members Have Cut Ends ✓ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Use TIA-222-G Tension Splice Capacity Exemption	Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules ✓ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA ✓ SR Leg Bolts Resist Compression ✓ All Leg Panels Have Same Allowable ✓ Offset Girt At Foundation ✓ Consider Feedline Torque ✓ Include Angle Block Shear Check <div style="text-align: center; background-color: #e0e0e0; padding: 2px;">Poles</div> Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets
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Triangular Tower

Tower Section Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	ft			ft		ft
T1	150'-130'			4'	1	20'
T2	130'-110'			4'6"	1	20'
T3	110'-100'			5'	1	10'
T4	100'-80'			6'	1	20'
T5	80'-60'			8'	1	20'
T6	60'-40'			10'	1	20'
T7	40'-20'			12'	1	20'
T8	20'-0'			14'	1	20'

Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T1	150'-130'	2'4-9/16"	X Brace	No	Steps	6.0000	6.0000
T2	130'-110'	2'4-9/16"	X Brace	No	Steps	6.0000	6.0000
T3	110'-100'	10'	X Brace	No	Yes	0.0000	0.0000
T4	100'-80'	10'	X Brace	No	Yes	0.0000	0.0000
T5	80'-60'	10'	X Brace	No	No	0.0000	0.0000
T6	60'-40'	10'	X Brace	No	No	0.0000	0.0000
T7	40'-20'	10'	X Brace	No	No	0.0000	0.0000
T8	20'-0'	10'	X Brace	No	No	0.0000	0.0000

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 150'-130'	Solid Round	1 1/2	A572-50 (50 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T2 130'-110'	Solid Round	2	A572-50 (50 ksi)	Solid Round	7/8	A572-50 (50 ksi)
T3 110'-100'	Truss Leg	Pirod 105244	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T4 100'-80'	Truss Leg	Pirod 105216	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T5 80'-60'	Truss Leg	Pirod 105217	A572-50 (50 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)
T6 60'-40'	Truss Leg	Pirod 105218	A572-50 (50 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)
T7 40'-20'	Truss Leg	Pirod 105218	A572-50 (50 ksi)	Equal Angle	L3x3x5/16	A36 (36 ksi)
T8 20'-0'	Truss Leg	Pirod 105219	A572-50 (50 ksi)	Equal Angle	L3x3x5/16	A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 150'-130'	Solid Round	7/8	A572-50 (50 ksi)	Solid Round	7/8	A572-50 (50 ksi)
T2 130'-110'	Solid Round	7/8	A572-50 (50 ksi)	Solid Round	7/8	A572-50 (50 ksi)
T3 110'-100'	Equal Angle	L3x3x3/16	A36 (36 ksi)	Equal Angle		A36 (36 ksi)
T4 100'-80'	Equal Angle	L3x3x3/16	A36 (36 ksi)	Equal Angle		A36 (36 ksi)
T5 80'-60'	Equal Angle	L3x3x3/16	A36 (36 ksi)	Equal Angle		A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T1 150'-130'	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T2 130'-110'	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T4 100'-80'	None	Flat Bar		A36 (36 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _r	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in
T1 150'-130'	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A_r	Adjust. Factor A_r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft ²	in					in	in
T2 130'-110'	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T3 110'-100'	0.00	0.0000	A36 (36 ksi)	1.03	1	1.05	36.0000	36.0000
T4 100'-80'	0.00	0.0000	A36 (36 ksi)	1.03	1	1.05	36.0000	36.0000
T5 80'-60'	0.00	0.0000	A36 (36 ksi)	1.03	1	1.05	36.0000	36.0000
T6 60'-40'	0.00	0.0000	A36 (36 ksi)	1.03	1	1.05	36.0000	36.0000
T7 40'-20'	0.00	0.0000	A36 (36 ksi)	1.03	1	1.05	36.0000	36.0000
T8 20'-0'	0.00	0.0000	A36 (36 ksi)	1.03	1	1.05	36.0000	36.0000

Tower Section Geometry (cont'd)

Tower Elevation	Calc K Single Angles	Calc K Solid Rounds	Legs	K Factors ¹							
				X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace	
ft				X Y	X Y	X Y	X Y	X Y	X Y	X Y	
T1 150'-130'	No	Yes	1	1	1	1	1	1	1	1	
T2 130'-110'	No	Yes	1	1	1	1	1	1	1	1	
T3 110'-100'	Yes	No	1	1	1	1	1	1	1	1	
T4 100'-80'	Yes	No	1	1	1	1	1	1	1	1	
T5 80'-60'	Yes	No	1	1	1	1	1	1	1	1	
T6 60'-40'	Yes	No	1	1	1	1	1	1	1	1	
T7 40'-20'	Yes	No	1	1	1	1	1	1	1	1	
T8 20'-0'	Yes	No	1	1	1	1	1	1	1	1	

¹Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

Tower Section Geometry (cont'd)

Tower Elevation	Truss-Leg K Factors					
	Leg Panels	Truss-Legs Used As Leg Members		Leg Panels	Truss-Legs Used As Inner Members	
ft		X Brace Diagonals	Z Brace Diagonals		X Brace Diagonals	Z Brace Diagonals
T3 110'-100'	1	0.5	0.85	1	0.5	0.85
T4 100'-80'	1	0.5	0.85	1	0.5	0.85
T5 80'-60'	1	0.5	0.85	1	0.5	0.85
T6 60'-40'	1	0.5	0.85	1	0.5	0.85
T7 40'-20'	1	0.5	0.85	1	0.5	0.85
T8 20'-0'	1	0.5	0.85	1	0.5	0.85

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 150'-130'	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T2 130'-110'	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T3 110'-100'	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T4 100'-80'	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T5 80'-60'	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 60'-40'	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 40'-20'	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 20'-0'	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg Bolt Size in	Leg No.	Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
				Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 150'-130'	Sleeve DS	0.6250	5	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T2 130'-110'	Flange	1.0000	6	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T3 110'-100'	Flange	1.0000	6	1.0000	1	1.0000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T4 100'-80'	Flange	1.0000	6	1.0000	1	1.0000	1	0.6250	0	0.6250	0	1.0000	1	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T5 80'-60'	Flange	1.0000	6	1.0000	1	1.0000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T6 60'-40'	Flange	1.0000	6	1.0000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T7 40'-20'	Flange	1.0000	6	1.0000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T8 20'-0'	Flange	1.2500	6	1.2500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A687		A325N		A325N		A325N		A325N		A325N		A325N	

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Shield Leg	Allow	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
LDF7-50A(1 5/8")	B	No	Ar (Leg)	150' - 133'	0.0000	0.1	3	3	0.7500	1.9800		0.82
LDF7-50A(1 5/8")	B	No	Ar (Leg)	133' - 127'	0.0000	0.1	5	5	0.7500	1.9800		0.82
LDF7-50A(1 5/8")	B	No	Ar (Leg)	127' - 117'	0.0000	0.1	6	6	0.7500	1.9800		0.82
LDF7-50A(1 5/8")	B	No	Ar (Leg)	117' - 85'	0.0000	0.1	8	6	0.7500	1.9800		0.82
LDF7-50A(1 5/8")	B	No	Ar (Leg)	85' - 0'	0.0000	0.1	9	6	0.7500	1.9800		0.82
T-Brackets (Af) ***	B	No	Af (Leg)	150' - 0'	0.0000	0.1	1	1	1.0000	1.0000	4.0000	8.40
LDF7-50A(1 5/8")	A	No	Ar (Leg)	141' - 0'	0.0000	0.1	12	6	0.7500	1.9800		0.82

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
LDF6-50A(1-1/4")	A	No	Ar (Leg)	141' - 0'	0.0000	0.16	3	2	0.7500	1.5500		0.66
T-Brackets (Af) ***	A	No	Af (Leg)	150' - 0'	0.0000	0.1	1	1	1.0000	1.0000	4.0000	8.40
LDF7-50A(1 5/8")	C	No	Ar (Leg)	112' - 0'	0.0000	0.1	18	6	0.7500	1.9800		0.82
T-Brackets (Af) ***	C	No	Af (Leg)	112' - 0'	0.0000	0.1	1	1	1.0000	1.0000	4.0000	8.40
LDF6-50A(1-1/4")	B	Yes	Ar (CfAe)	100' - 0'	-2.5000	-0.06	9	6	0.7500	1.5500		0.66
Feedline Ladder (Af) ***	B	Yes	Af (CfAe)	100' - 0'	-0.0100	-0.06	1	1	3.0000	3.0000	12.0000	8.40
LDF7-50A(1 5/8")	A	Yes	Ar (CfAe)	88' - 0'	0.0000	-0.18	12	12	0.7500	1.9800		0.82
Feedline Ladder (Af) ***	A	Yes	Af (CfAe)	88' - 0'	0.0000	-0.18	1	1	3.0000	3.0000	12.0000	8.40
LDF7-50A(1 5/8")	B	Yes	Ar (CfAe)	65' - 0'	-0.0100	-0.23	6	6	0.7500	1.9800		0.82
Feedline Ladder (Af) ***	B	Yes	Af (CfAe)	65' - 0'	-0.0100	-0.23	1	1	3.0000	3.0000	12.0000	8.40
LDF4-50A(1/2) ***	B	Yes	Ar (CfAe)	50' - 0'	-0.5000	-0.11	1	1	0.6300	0.6300		0.15
LDF5-50A(7/8") ***	B	No	Ar (Leg)	43' - 0'	0.0000	0.06	1	1	0.7500	1.0900		0.33

Feed Line/Linear Appurtenances Section Areas

Tower Section n	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
T1	150'-130'	A	13.732	1.667	0.000	0.000	0.30
		B	24.622	3.333	0.000	0.000	0.22
		C	10.890	1.667	0.000	0.000	0.00
T2	130'-110'	A	26.947	1.833	0.000	0.000	0.40
		B	44.272	3.333	0.000	0.000	0.28
		C	21.285	1.833	0.000	0.000	0.05
T3	110'-100'	A	22.383	1.667	0.000	0.000	0.20
		B	22.383	1.667	0.000	0.000	0.15
		C	19.800	1.667	0.000	0.000	0.23
T4	100'-80'	A	60.607	5.333	0.000	0.000	0.55
		B	60.267	8.333	0.000	0.000	0.59
		C	39.600	3.333	0.000	0.000	0.46
T5	80'-60'	A	84.367	8.333	0.000	0.000	0.77
		B	65.217	9.583	0.000	0.000	0.67
		C	39.600	3.333	0.000	0.000	0.46
T6	60'-40'	A	84.367	8.333	0.000	0.000	0.77
		B	80.864	13.333	0.000	0.000	0.87
		C	39.873	3.333	0.000	0.000	0.46
T7	40'-20'	A	84.367	8.333	0.000	0.000	0.77
		B	82.933	13.333	0.000	0.000	0.88
		C	41.417	3.333	0.000	0.000	0.46
T8	20'-0'	A	84.367	8.333	0.000	0.000	0.77
		B	82.933	13.333	0.000	0.000	0.88
		C	41.417	3.333	0.000	0.000	0.46

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Sectio n	Tower Elevation ft	Face or Leg	Ice Thickness in	A_R ft ²	A_F ft ²	$C_A A_A$ In Face ft ²	$C_A A_A$ Out Face ft ²	Weight K
T1	150'-130'	A	0.892	6.507	18.270	0.000	0.000	0.74
		B		12.780	32.384	0.000	0.000	0.46
		C		6.273	14.114	0.000	0.000	0.00
T2	130'-110'	A	0.876	12.343	32.832	0.000	0.000	1.15
		B		17.940	55.876	0.000	0.000	0.68
		C		6.841	28.316	0.000	0.000	0.13
T3	110'-100'	A	0.862	8.900	28.248	0.000	0.000	0.57
		B		8.900	28.248	0.000	0.000	0.38
		C		6.173	26.332	0.000	0.000	0.65
T4	100'-80'	A	0.846	20.091	79.198	0.000	0.000	1.58
		B		23.046	82.473	0.000	0.000	1.51
		C		12.240	52.593	0.000	0.000	1.29
T5	80'-60'	A	0.821	23.428	113.189	0.000	0.000	2.22
		B		24.220	89.699	0.000	0.000	1.70
		C		12.072	52.481	0.000	0.000	1.28
T6	60'-40'	A	0.788	22.995	112.972	0.000	0.000	2.18
		B		30.711	111.591	0.000	0.000	2.20
		C		12.522	52.337	0.000	0.000	1.26
T7	40'-20'	A	0.750	22.483	112.717	0.000	0.000	2.13
		B		35.433	111.250	0.000	0.000	2.19
		C		15.917	52.167	0.000	0.000	1.24
T8	20'-0'	A	0.750	22.483	112.717	0.000	0.000	2.13
		B		35.433	111.250	0.000	0.000	2.19
		C		15.917	52.167	0.000	0.000	1.24

Feed Line Shielding

Section	Elevation ft	Face	A_R ft ²	A_R Ice ft ²	A_F ft ²	A_F Ice ft ²
T1	150'-130'	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T2	130'-110'	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T3	110'-100'	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T4	100'-80'	A	0.000	1.624	1.747	2.506
		B	0.000	2.055	2.007	3.171
		C	0.000	0.000	0.000	0.000
T5	80'-60'	A	0.000	3.052	3.897	5.577
		B	0.000	1.977	2.333	3.613
		C	0.000	0.000	0.000	0.000
T6	60'-40'	A	0.000	2.261	3.017	4.302
		B	0.000	2.494	3.100	4.745
		C	0.000	0.000	0.000	0.000
T7	40'-20'	A	0.000	1.999	2.815	3.999
		B	0.000	2.252	2.926	4.504
		C	0.000	0.000	0.000	0.000
T8	20'-0'	A	0.000	1.904	2.681	3.808
		B	0.000	2.145	2.786	4.289
		C	0.000	0.000	0.000	0.000

Feed Line Center of Pressure

Section	Elevation	CP _x	CP _z	CP _x Ice	CP _z Ice
	ft	in	in	in	in
T1	150'-130'	2.9964	-2.4955	1.6370	-1.5053
T2	130'-110'	3.1227	-2.7628	2.0943	-1.6310
T3	110'-100'	0.0000	-0.5825	0.0000	-0.2376
T4	100'-80'	-0.7150	-1.6137	-0.5069	-0.9235
T5	80'-60'	-3.1706	-2.4687	-2.3262	-1.5289
T6	60'-40'	-2.7461	-5.0301	-2.0559	-3.3740
T7	40'-20'	-2.7972	-5.7015	-1.8699	-3.8116
T8	20'-0'	-3.0998	-6.3969	-2.0686	-4.2771

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight								
			Lateral	Vert						ft	°	ft	ft ²	ft ²	K		
PR-950	A	From Leg	4.00	0'	0.0000	151'	No Ice	6.35	6.35	0.04							
							1/2" Ice	11.43	11.43	0.05							
							1" Ice	16.51	16.51	0.06							
							2" Ice	26.67	26.67	0.08							
							4" Ice	46.99	46.99	0.13							
							PR-950	B	From Leg	4.00	0'	0.0000	151'	No Ice	6.35	6.35	0.04
PR-950	B	From Leg	4.00	0'	0.0000	151'	1/2" Ice	11.43	11.43	0.05							
							1" Ice	16.51	16.51	0.06							
							2" Ice	26.67	26.67	0.08							
							4" Ice	46.99	46.99	0.13							
							PR-950	C	From Leg	4.00	0'	0.0000	151'	No Ice	6.35	6.35	0.04
							PR-950	C	From Leg	4.00	0'	0.0000	151'	1/2" Ice	11.43	11.43	0.05
1" Ice	16.51	16.51	0.06														
2" Ice	26.67	26.67	0.08														
4" Ice	46.99	46.99	0.13														
Platform Mount [LP 405-1]	C	None			0.0000	151'								No Ice	20.80	20.80	1.80
(3) 6" x 2" Mount Pipe	A	From Leg	4.00	0'	0.0000	151'								1/2" Ice	28.10	28.10	2.07
							1" Ice	35.40	35.40	2.33							
							2" Ice	50.00	50.00	2.86							
							4" Ice	79.20	79.20	3.93							
							No Ice	1.43	1.43	0.02							
							1/2" Ice	1.92	1.92	0.03							
(3) 6" x 2" Mount Pipe	B	From Leg	4.00	0'	0.0000	151'	1" Ice	2.29	2.29	0.05							
							2" Ice	3.06	3.06	0.09							
							4" Ice	4.70	4.70	0.23							
							No Ice	1.43	1.43	0.02							
							1/2" Ice	1.92	1.92	0.03							
							1" Ice	2.29	2.29	0.05							
(3) 6" x 2" Mount Pipe	C	From Leg	4.00	0'	0.0000	151'	2" Ice	3.06	3.06	0.09							
							4" Ice	4.70	4.70	0.23							
							No Ice	1.43	1.43	0.02							
							1/2" Ice	1.92	1.92	0.03							
							1" Ice	2.29	2.29	0.05							
							2" Ice	3.06	3.06	0.09							
*** (2) 7770.00 w/ Mount Pipe	A	From Leg	4.00	0'	0.0000	141'	4" Ice	4.70	4.70	0.23							
							No Ice	6.12	4.25	0.06							
							1/2" Ice	6.63	5.01	0.10							
							1" Ice	7.13	5.71	0.16							
							2" Ice	8.16	7.16	0.29							

Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C _{AA} _{Front}	C _{AA} _{Side}	Weight	
			Horz	Lateral	Vert						ft
			ft	ft	ft	°	ft	ft ²	ft ²	K	
							2" Ice	10.36	10.41	0.66	
(2) 7770.00 w/ Mount Pipe	B	From Leg	4.00	0'	0'	0.0000	141'	4" Ice	6.12	4.25	0.06
								No Ice	6.63	5.01	0.10
								1/2" Ice	7.13	5.71	0.16
								1" Ice	8.16	7.16	0.29
								2" Ice	10.36	10.41	0.66
(2) 7770.00 w/ Mount Pipe	C	From Leg	4.00	0'	0'	0.0000	141'	4" Ice	6.12	4.25	0.06
								No Ice	6.63	5.01	0.10
								1/2" Ice	7.13	5.71	0.16
								1" Ice	8.16	7.16	0.29
								2" Ice	10.36	10.41	0.66
(2) LGP21401	A	From Leg	4.00	0'	0'	0.0000	141'	4" Ice	1.29	0.23	0.01
								No Ice	1.45	0.31	0.02
								1/2" Ice	1.61	0.40	0.03
								1" Ice	1.97	0.61	0.05
								2" Ice	2.79	1.12	0.14
(2) LGP21401	B	From Leg	4.00	0'	0'	0.0000	141'	4" Ice	1.29	0.23	0.01
								No Ice	1.45	0.31	0.02
								1/2" Ice	1.61	0.40	0.03
								1" Ice	1.97	0.61	0.05
								2" Ice	2.79	1.12	0.14
(2) LGP21401	C	From Leg	4.00	0'	0'	0.0000	141'	4" Ice	1.29	0.23	0.01
								No Ice	1.45	0.31	0.02
								1/2" Ice	1.61	0.40	0.03
								1" Ice	1.97	0.61	0.05
								2" Ice	2.79	1.12	0.14
Sector Mount [SM 411-3]	C	None			0.0000	141'	4" Ice	21.88	21.88	1.07	
							No Ice	30.68	30.68	1.48	
							1/2" Ice	39.48	39.48	1.90	
							1" Ice	57.08	57.08	2.73	
							2" Ice	92.28	92.28	4.40	
AM-X-CD-14-65-00T-RET w/ Mount Pipe	A	From Leg	4.00	0'	0'	0.0000	141'	4" Ice	5.74	4.02	0.03
								No Ice	6.20	4.63	0.08
								1/2" Ice	6.66	5.28	0.13
								1" Ice	7.62	6.68	0.25
								2" Ice	9.67	9.74	0.61
AM-X-CD-14-65-00T-RET w/ Mount Pipe	B	From Leg	4.00	0'	0'	0.0000	141'	4" Ice	5.74	4.02	0.03
								No Ice	6.20	4.63	0.08
								1/2" Ice	6.66	5.28	0.13
								1" Ice	7.62	6.68	0.25
								2" Ice	9.67	9.74	0.61
AM-X-CD-14-65-00T-RET w/ Mount Pipe	C	From Leg	4.00	0'	0'	0.0000	141'	4" Ice	5.74	4.02	0.03
								No Ice	6.20	4.63	0.08
								1/2" Ice	6.66	5.28	0.13
								1" Ice	7.62	6.68	0.25
								2" Ice	9.67	9.74	0.61
RRUS 11-700	A	From Leg	4.00	0'	0'	0.0000	141'	4" Ice	2.94	1.25	0.06
								No Ice	3.17	1.41	0.07
								1/2" Ice	3.41	1.59	0.10
								1" Ice	3.91	1.96	0.15
								2" Ice	5.02	2.82	0.30
RRUS 11-700	B	From Leg	4.00	0'	0'	0.0000	141'	4" Ice	2.94	1.25	0.06
								No Ice	3.17	1.41	0.07
								1/2" Ice	3.41	1.59	0.10

Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight	
			Horz	Lateral	Vert						ft
RRUS 11-700	C	From Leg	4.00	0'	0'	0.0000	141'	1" Ice	3.91	1.96	0.15
								2" Ice	5.02	2.82	0.30
								4" Ice			
								No Ice	2.94	1.25	0.06
								1/2" Ice	3.17	1.41	0.07
								1" Ice	3.41	1.59	0.10
RRUS 11-AWS	A	From Leg	4.00	0'	0'	0.0000	141'	1" Ice	3.91	1.96	0.15
								2" Ice	5.02	2.82	0.30
								4" Ice			
								No Ice	2.94	1.25	0.06
								1/2" Ice	3.17	1.41	0.07
								1" Ice	3.41	1.59	0.10
RRUS 11-AWS	B	From Leg	4.00	0'	0'	0.0000	141'	1" Ice	3.91	1.96	0.15
								2" Ice	5.02	2.82	0.30
								4" Ice			
								No Ice	2.94	1.25	0.06
								1/2" Ice	3.17	1.41	0.07
								1" Ice	3.41	1.59	0.10
RRUS 11-AWS	C	From Leg	4.00	0'	0'	0.0000	141'	1" Ice	3.91	1.96	0.15
								2" Ice	5.02	2.82	0.30
								4" Ice			
								No Ice	2.94	1.25	0.06
								1/2" Ice	3.17	1.41	0.07
								1" Ice	3.41	1.59	0.10
DC6-48-60-18-8F	C	From Leg	4.00	0'	0'	0.0000	141'	1" Ice	2.09	2.09	0.10
								2" Ice	3.10	3.10	0.21
								4" Ice			
								No Ice	1.27	1.27	0.02
								1/2" Ice	1.46	1.46	0.04
								1" Ice	1.66	1.66	0.05
6' x 2" Mount Pipe	A	From Leg	4.00	0'	0'	0.0000	141'	1" Ice	3.06	3.06	0.09
								2" Ice	4.70	4.70	0.23
								4" Ice			
								No Ice	1.43	1.43	0.02
								1/2" Ice	1.92	1.92	0.03
								1" Ice	2.29	2.29	0.05
6' x 2" Mount Pipe	B	From Leg	4.00	0'	0'	0.0000	141'	1" Ice	3.06	3.06	0.09
								2" Ice	4.70	4.70	0.23
								4" Ice			
								No Ice	1.43	1.43	0.02
								1/2" Ice	1.92	1.92	0.03
								1" Ice	2.29	2.29	0.05
6' x 2" Mount Pipe	C	From Leg	4.00	0'	0'	0.0000	141'	1" Ice	3.06	3.06	0.09
								2" Ice	4.70	4.70	0.23
								4" Ice			
								No Ice	1.43	1.43	0.02
								1/2" Ice	1.92	1.92	0.03
								1" Ice	2.29	2.29	0.05
10' x 2" Omni	B	From Leg	2.00	0'	5'	0.0000	133'	1" Ice	6.10	6.10	0.08
								2" Ice	10.20	10.20	0.15
								4" Ice			
								No Ice	2.00	2.00	0.02
								1/2" Ice	3.02	3.02	0.03
								1" Ice	4.05	4.05	0.05
10' x 2" Omni	C	From Leg	2.00	0'	5'	0.0000	133'	1" Ice	6.10	6.10	0.08
								2" Ice	10.20	10.20	0.15
								4" Ice			
								No Ice	2.00	2.00	0.02
								1/2" Ice	3.02	3.02	0.03
								1" Ice	4.05	4.05	0.05
Side Arm Mount [SO 301-	B	From Leg	1.00			0.0000	133'	No Ice	1.00	0.90	0.02

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} _{Front}	C _{AA} _{Side}	Weight	
			Horz	Lateral						Vert
			ft	ft	°	ft	ft ²	ft ²	K	
1]			0'			1/2"	1.39	1.42	0.03	
			0'			Ice	1.78	1.94	0.04	
						1" Ice	2.56	2.98	0.06	
						2" Ice	4.12	5.06	0.10	
						4" Ice				
Side Arm Mount [SO 301-1]	C	From Leg	1.00	0.0000		133'	No Ice	1.00	0.90	0.02
			0'				1/2"	1.39	1.42	0.03
			0'				Ice	1.78	1.94	0.04
							1" Ice	2.56	2.98	0.06
							2" Ice	4.12	5.06	0.10
							4" Ice			

20" x 2" Omni	A	From Leg	2.00	0.0000		127'	No Ice	4.00	4.00	0.03
			0'				1/2"	6.03	6.03	0.06
			10'				Ice	8.05	8.05	0.09
							1" Ice	12.10	12.10	0.15
							2" Ice	20.20	20.20	0.27
							4" Ice			
Side Arm Mount [SO 301-1]	A	From Leg	1.00	0.0000		127'	No Ice	1.00	0.90	0.02
			0'				1/2"	1.39	1.42	0.03
			0'				Ice	1.78	1.94	0.04
							1" Ice	2.56	2.98	0.06
							2" Ice	4.12	5.06	0.10
							4" Ice			

1 sq. ft antenna	A	From Leg	2.00	0.0000		117'	No Ice	1.40	0.23	0.02
			0'				1/2"	1.56	0.33	0.03
			0'				Ice	1.72	0.43	0.03
							1" Ice	2.04	0.63	0.05
							2" Ice	2.68	1.03	0.08
							4" Ice			
Side Arm Mount [SO 301-1]	A	From Leg	1.00	0.0000		117'	No Ice	1.00	0.90	0.02
			0'				1/2"	1.39	1.42	0.03
			0'				Ice	1.78	1.94	0.04
							1" Ice	2.56	2.98	0.06
							2" Ice	4.12	5.06	0.10
							4" Ice			

(2) RR90-17-02DP	A	From Leg	4.00	0.0000		112'	No Ice	4.36	1.97	0.02
			0'				1/2"	4.77	2.31	0.04
			-3'				Ice	5.20	2.66	0.07
							1" Ice	6.08	3.37	0.14
							2" Ice	7.95	4.89	0.33
							4" Ice			
(2) RR90-17-02DP	B	From Leg	4.00	0.0000		112'	No Ice	4.36	1.97	0.02
			0'				1/2"	4.77	2.31	0.04
			-3'				Ice	5.20	2.66	0.07
							1" Ice	6.08	3.37	0.14
							2" Ice	7.95	4.89	0.33
							4" Ice			
(2) RR90-17-02DP	C	From Leg	4.00	0.0000		112'	No Ice	4.36	1.97	0.02
			0'				1/2"	4.77	2.31	0.04
			-3'				Ice	5.20	2.66	0.07
							1" Ice	6.08	3.37	0.14
							2" Ice	7.95	4.89	0.33
							4" Ice			
APX16DWV-16DWVS-C	A	From Leg	4.00	0.0000		112'	No Ice	7.23	2.15	0.04
			0'				1/2"	7.68	2.49	0.07
			-3'				Ice	8.14	2.84	0.11
							1" Ice	9.09	3.55	0.20
							2" Ice	11.09	5.08	0.46
							4" Ice			
APX16DWV-16DWVS-C	B	From Leg	4.00	0.0000		112'	No Ice	7.23	2.15	0.04
			0'				1/2"	7.68	2.49	0.07
			-3'				Ice	8.14	2.84	0.11

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A _A Front	C _A A _A Side	Weight	
			Horz	Lateral						Vert
			ft	ft	°	ft	ft ²	ft ²	K	
APX16DWV-16DWVS-C	C	From Leg	4.00	0'	0.0000	112'	1" Ice	9.09	3.55	0.20
							2" Ice	11.09	5.08	0.46
							4" Ice			
							No Ice	7.23	2.15	0.04
							1/2" Ice	7.68	2.49	0.07
							1" Ice	8.14	2.84	0.11
							2" Ice	9.09	3.55	0.20
(2) ETW200VS12UB	A	From Leg	4.00	0'	0.0000	112'	2" Ice	11.09	5.08	0.46
							4" Ice			
							No Ice	0.18	0.47	0.01
							1/2" Ice	0.25	0.57	0.01
							1" Ice	0.32	0.67	0.02
							1" Ice	0.49	0.90	0.03
							2" Ice	0.94	1.47	0.09
(2) ETW200VS12UB	B	From Leg	4.00	0'	0.0000	112'	4" Ice			
							No Ice	0.18	0.47	0.01
							1/2" Ice	0.25	0.57	0.01
							1" Ice	0.32	0.67	0.02
							1" Ice	0.49	0.90	0.03
							2" Ice	0.94	1.47	0.09
							4" Ice			
(2) ETW200VS12UB	C	From Leg	4.00	0'	0.0000	112'	No Ice	0.18	0.47	0.01
							1/2" Ice	0.25	0.57	0.01
							1" Ice	0.32	0.67	0.02
							1" Ice	0.49	0.90	0.03
							2" Ice	0.94	1.47	0.09
							4" Ice			
							No Ice	0.18	0.47	0.01
Sector Mount [SM 307-3]	C	None			0.0000	112'	1/2" Ice	36.28	36.28	2.15
							Ice	46.34	46.34	2.68
							1" Ice	66.46	66.46	3.73
							2" Ice	106.70	106.70	5.85
							4" Ice			
							No Ice	26.22	26.22	1.62
							1/2" Ice	36.28	36.28	2.15
*** 800MHz 2X50W RRH W/FILTER	A	From Leg	4.00	0'	0.0000	100'	Ice	46.34	46.34	2.68
							1" Ice	66.46	66.46	3.73
							2" Ice	106.70	106.70	5.85
							4" Ice			
							No Ice	2.40	2.25	0.06
							1/2" Ice	2.61	2.46	0.09
							Ice	2.83	2.68	0.11
PCS 1900MHz 2x40W	A	From Leg	4.00	0'	0.0000	100'	1" Ice	3.30	3.13	0.17
							2" Ice	4.34	4.15	0.34
							4" Ice			
							No Ice	2.74	1.46	0.04
							1/2" Ice	2.97	1.65	0.06
							Ice	3.21	1.84	0.08
							1" Ice	3.71	2.27	0.14
(2) APXVSP18-C-A20 w/ Mount Pipe	A	From Leg	4.00	0'	0.0000	100'	2" Ice	4.82	3.22	0.28
							4" Ice			
							No Ice	8.50	6.95	0.08
							1/2" Ice	9.15	8.13	0.15
							Ice	9.77	9.02	0.23
							1" Ice	11.03	10.84	0.41
							2" Ice	13.68	14.85	0.91
800MHz 2X50W RRH W/FILTER	B	From Leg	4.00	0'	0.0000	100'	4" Ice			
							No Ice	2.40	2.25	0.06
							1/2" Ice	2.61	2.46	0.09
							Ice	2.83	2.68	0.11
							1" Ice	3.30	3.13	0.17
							2" Ice	4.34	4.15	0.34
							4" Ice			
PCS 1900MHz 2x40W	B	From Leg	4.00	0'	0.0000	100'	No Ice	2.74	1.46	0.04
							1/2" Ice	2.97	1.65	0.06
							Ice	3.21	1.84	0.08
							1" Ice	3.71	2.27	0.14
							2" Ice	4.82	3.22	0.28
							4" Ice			
							No Ice	2.74	1.46	0.04
(2) APXVSP18-C-A20 w/ Mount Pipe	B	From Leg	4.00	0'	0.0000	100'	1/2" Ice	2.97	1.65	0.06
							Ice	3.21	1.84	0.08
							1" Ice	3.71	2.27	0.14
							2" Ice	4.82	3.22	0.28
							4" Ice			
							No Ice	8.50	6.95	0.08
							No Ice	8.50	6.95	0.08

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment t °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
Mount Pipe			0' 0'			1/2" 9.15 Ice 9.77 1" Ice 11.03 2" Ice 13.68 4" Ice	8.13 9.02 10.84 14.85	0.15 0.23 0.41 0.91
800MHz 2X50W RRH W/FILTER	C	From Leg	4.00 0' 0'	0.0000	100'	No Ice 2.40 1/2" 2.61 Ice 2.83 1" Ice 3.30 2" Ice 4.34 4" Ice	2.25 2.46 2.68 3.13 4.15	0.06 0.09 0.11 0.17 0.34
PCS 1900MHz 2x40W	C	From Leg	4.00 0' 0'	0.0000	100'	No Ice 2.74 1/2" 2.97 Ice 3.21 1" Ice 3.71 2" Ice 4.82 4" Ice	1.46 1.65 1.84 2.27 3.22	0.04 0.06 0.08 0.14 0.28
(2) APXVSP18-C-A20 w/ Mount Pipe	C	From Leg	4.00 0' 0'	0.0000	100'	No Ice 8.50 1/2" 9.15 Ice 9.77 1" Ice 11.03 2" Ice 13.68 4" Ice	6.95 8.13 9.02 10.84 14.85	0.08 0.15 0.23 0.41 0.91
Sector Mount [SM 407-3]	C	None		0.0000	100'	No Ice 20.49 1/2" 30.39 Ice 40.29 1" Ice 60.09 2" Ice 99.69 4" Ice	20.49 30.39 40.29 60.09 99.69	0.96 1.38 1.80 2.64 4.32

(2) LPA-80063/4CF w/ Mount Pipe	A	From Leg	4.00 0' 2'	0.0000	88'	No Ice 7.25 1/2" 7.72 Ice 8.20 1" Ice 9.19 2" Ice 11.32 4" Ice	7.26 7.96 8.67 10.16 13.39	0.04 0.10 0.18 0.34 0.80
(2) LPA-80063/4CF w/ Mount Pipe	B	From Leg	4.00 0' 2'	0.0000	88'	No Ice 7.25 1/2" 7.72 Ice 8.20 1" Ice 9.19 2" Ice 11.32 4" Ice	7.26 7.96 8.67 10.16 13.39	0.04 0.10 0.18 0.34 0.80
(2) LPA-80063/4CF w/ Mount Pipe	C	From Leg	4.00 0' 2'	0.0000	88'	No Ice 7.25 1/2" 7.72 Ice 8.20 1" Ice 9.19 2" Ice 11.32 4" Ice	7.26 7.96 8.67 10.16 13.39	0.04 0.10 0.18 0.34 0.80
(2) LPA-185063/8CFx2 w/ Mount Pipe	A	From Leg	4.00 0' 2'	0.0000	88'	No Ice 3.21 1/2" 3.60 Ice 4.02 1" Ice 4.90 2" Ice 6.79 4" Ice	3.92 4.52 5.16 6.54 9.58	0.03 0.06 0.10 0.20 0.51
(2) LPA-185063/8CFx2 w/ Mount Pipe	B	From Leg	4.00 0' 2'	0.0000	88'	No Ice 3.21 1/2" 3.60 Ice 4.02 1" Ice 4.90 2" Ice 6.79 4" Ice	3.92 4.52 5.16 6.54 9.58	0.03 0.06 0.10 0.20 0.51
(2) LPA-185063/8CFx2 w/ Mount Pipe	C	From Leg	4.00 0' 2'	0.0000	88'	No Ice 3.21 1/2" 3.60 Ice 4.02 1" Ice 4.90 2" Ice 6.79	3.92 4.52 5.16 6.54 9.58	0.03 0.06 0.10 0.20 0.51

Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight	
			Horz	Lateral	Vert						ft
(2) ETB19G8-12UB	A	From Leg	4.00	0'	2'	0.0000	88'	4" Ice			
								No Ice	1.06	0.45	0.02
								1/2"	1.21	0.57	0.03
								Ice	1.37	0.71	0.03
								1" Ice	1.72	1.00	0.06
(2) ETB19G8-12UB	B	From Leg	4.00	0'	2'	0.0000	88'	4" Ice			
								No Ice	1.06	0.45	0.02
								1/2"	1.21	0.57	0.03
								Ice	1.37	0.71	0.03
								1" Ice	1.72	1.00	0.06
(2) ETB19G8-12UB	C	From Leg	4.00	0'	2'	0.0000	88'	4" Ice			
								No Ice	1.06	0.45	0.02
								1/2"	1.21	0.57	0.03
								Ice	1.37	0.71	0.03
								1" Ice	1.72	1.00	0.06
Sector Mount [SM 408-3]	C	None			0.0000	88'	4" Ice				
							No Ice	22.45	22.45	1.02	
							1/2"	33.50	33.50	1.47	
							Ice	44.55	44.55	1.93	
							1" Ice	66.65	66.65	2.84	
*** 7130.14.05.00 w/ Mount Pipe	B	From Leg	2.00	0'	0'	0.0000	85'	2" Ice	2.51	1.69	0.14
								4" Ice			
								No Ice	3.17	3.62	0.02
								1/2"	3.48	4.07	0.06
								Ice	3.81	4.54	0.09
Side Arm Mount [SO 301-1]	B	From Leg	1.00	0'	0'	0.0000	85'	1" Ice	4.50	5.55	0.19
								2" Ice	6.06	7.93	0.46
								4" Ice			
								No Ice	1.00	0.90	0.02
								1/2"	1.39	1.42	0.03
*** APXV18-206517S-C w/ Mount Pipe	A	From Leg	1.00	0'	0'	0.0000	65'	Ice	1.78	1.94	0.04
								1" Ice	2.56	2.98	0.06
								2" Ice	4.12	5.06	0.10
								4" Ice			
								No Ice	5.40	4.70	0.05
APXV18-206517S-C w/ Mount Pipe	B	From Leg	1.00	0'	0'	0.0000	65'	1/2"	5.96	5.86	0.10
								Ice	6.48	6.73	0.15
								1" Ice	7.55	8.51	0.28
								2" Ice	9.92	12.28	0.68
								4" Ice			
APXV18-206517S-C w/ Mount Pipe	C	From Leg	1.00	0'	0'	0.0000	65'	No Ice	5.40	4.70	0.05
								1/2"	5.96	5.86	0.10
								Ice	6.48	6.73	0.15
								1" Ice	7.55	8.51	0.28
								2" Ice	9.92	12.28	0.68
*** GPS-TMG-26N	C	From Leg	1.00	0'	0'	0.0000	50'	No Ice	0.16	0.16	0.00
								1/2"	0.21	0.21	0.00
								Ice	0.28	0.28	0.01
								1" Ice	0.44	0.44	0.01
								2" Ice	0.86	0.86	0.05
4" Ice											

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _A A _{Front} ft ²	C _A A _{Side} ft ²	Weight K
Side Arm Mount [SO 301-1]	C	From Leg	1.00 0' 0'	0.0000	43'	No Ice 1/2" Ice 1" 2" 4" Ice	1.00 0.90 1.39 1.42 1.78 1.94 2.56 2.98 4.12 5.06	0.02 0.03 0.04 0.06 0.10

Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	3 dB Beam Width °	Elevation ft	Outside Diameter ft	Aperture Area ft ²	Weight K	
SSP2-23	C	Paraboloid w/o Radome	From Leg	2.00 0' 0'	0.0000		43'	2.15	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.58 3.92 0.00 0.00 0.00	0.04 0.06 0.08 0.12 0.20

Truss-Leg Properties

Section Designation	Area in ²	Area Ice in ²	Self Weight K	Ice Weight K	Equiv. Diameter r in	Equiv. Diameter r Ice in	Leg Area in ²
Pirod 105244	1026.8606	2520.2170	0.56	0.49	7.1310	17.5015	3.6816
Pirod 105216	2169.0308	4785.0529	0.47	0.96	7.5314	16.6148	3.6816
Pirod 105217	2296.2363	4895.6001	0.59	0.94	7.9730	16.9986	5.3014
Pirod 105218	2425.3141	4973.0624	0.72	0.90	8.4212	17.2676	7.2158
Pirod 105218	2425.3141	4788.5245	0.72	0.83	8.4212	16.6268	7.2158
Pirod 105219	2597.9095	5128.1841	1.09	0.88	9.0205	17.8062	9.4248

Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 60 deg - No Ice
5	Dead+Wind 90 deg - No Ice
6	Dead+Wind 120 deg - No Ice
7	Dead+Wind 150 deg - No Ice
8	Dead+Wind 180 deg - No Ice
9	Dead+Wind 210 deg - No Ice
10	Dead+Wind 240 deg - No Ice
11	Dead+Wind 270 deg - No Ice
12	Dead+Wind 300 deg - No Ice
13	Dead+Wind 330 deg - No Ice

Comb. No.	Description
14	Dead+Ice+Temp
15	Dead+Wind 0 deg+Ice+Temp
16	Dead+Wind 30 deg+Ice+Temp
17	Dead+Wind 60 deg+Ice+Temp
18	Dead+Wind 90 deg+Ice+Temp
19	Dead+Wind 120 deg+Ice+Temp
20	Dead+Wind 150 deg+Ice+Temp
21	Dead+Wind 180 deg+Ice+Temp
22	Dead+Wind 210 deg+Ice+Temp
23	Dead+Wind 240 deg+Ice+Temp
24	Dead+Wind 270 deg+Ice+Temp
25	Dead+Wind 300 deg+Ice+Temp
26	Dead+Wind 330 deg+Ice+Temp
27	Dead+Wind 0 deg - Service
28	Dead+Wind 30 deg - Service
29	Dead+Wind 60 deg - Service
30	Dead+Wind 90 deg - Service
31	Dead+Wind 120 deg - Service
32	Dead+Wind 150 deg - Service
33	Dead+Wind 180 deg - Service
34	Dead+Wind 210 deg - Service
35	Dead+Wind 240 deg - Service
36	Dead+Wind 270 deg - Service
37	Dead+Wind 300 deg - Service
38	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	150 - 130	Leg	Max Tension	4	19.75	-0.45	-0.00
			Max. Compression	6	-23.51	0.45	0.01
			Max. Mx	10	-23.39	-0.62	-0.01
			Max. My	3	-1.84	-0.00	0.55
			Max. Vy	2	-2.14	0.46	-0.01
		Diagonal	Max. Vx	9	-1.87	-0.01	0.38
			Max Tension	13	2.58	0.00	0.00
			Max. Compression	11	-2.67	0.00	0.00
			Max. Mx	26	0.87	-0.00	-0.00
			Max. My	5	-2.16	-0.00	0.00
		Horizontal	Max. Vy	26	0.00	-0.00	-0.00
			Max. Vx	5	-0.00	-0.00	0.00
			Max Tension	8	0.39	0.00	0.00
			Max. Compression	2	-0.26	0.00	0.00
			Max. Mx	14	0.14	0.01	0.00
		Top Girt	Max. Vy	14	0.01	0.00	0.00
			Max Tension	6	0.42	0.00	0.00
			Max. Compression	4	-0.44	0.00	0.00
		Bottom Girt	Max. Mx	14	-0.02	0.01	0.00
			Max. Vy	14	-0.01	0.00	0.00
Max Tension	8		1.31	0.00	0.00		
Max. Compression	2		-1.22	0.00	0.00		
Max. Mx	14		0.02	0.01	0.00		
T2	130 - 110	Leg	Max. Vy	14	-0.01	0.00	0.00
			Max Tension	4	52.24	0.21	-0.01
			Max. Compression	2	-58.97	1.77	-0.04
			Max. Mx	8	51.81	-1.78	0.04
			Max. My	9	-1.90	-0.02	1.32
		Diagonal	Max. Vy	8	3.99	-1.78	0.04
			Max. Vx	11	2.70	-0.01	-1.29
			Max Tension	13	3.65	0.00	0.00
			Max. Compression	7	-3.69	0.00	0.00
			Max. Mx	15	1.10	-0.00	-0.00
			Max. My	5	-3.15	-0.00	0.00
			Max. Vy	15	0.01	-0.00	-0.00
			Max. Vx	5	-0.00	0.00	0.00

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T3	110 - 100	Horizontal	Max Tension	8	0.82	0.00	0.00	
			Max. Compression	2	-0.72	0.00	0.00	
			Max. Mx	14	0.11	0.01	0.00	
			Max. Vy	14	-0.01	0.00	0.00	
			Top Girt	Max Tension	10	1.35	0.00	0.00
				Max. Compression	12	-1.33	0.00	0.00
		Max. Mx		14	0.01	0.01	0.00	
		Max. Vy		14	-0.01	0.00	0.00	
		Bottom Girt		Max Tension	8	1.24	0.00	0.00
				Max. Compression	2	-1.15	0.00	0.00
			Max. Mx	14	0.07	0.01	0.00	
			Max. Vy	14	-0.01	0.00	0.00	
			Leg	Max Tension	4	54.82	-1.77	0.03
				Max. Compression	2	-60.65	3.16	0.00
		Max. Mx		2	-60.65	3.16	0.00	
		Max. My		5	-3.02	0.01	3.20	
		Max. Vy		4	0.22	-3.13	0.02	
		Max. Vx		13	-0.28	-0.01	3.18	
		Diagonal		Max Tension	5	6.17	0.00	0.00
				Max. Compression	5	-7.12	0.00	0.00
				Max. Mx	5	3.43	0.05	0.00
				Max. My	6	-6.95	-0.04	0.01
				Max. Vy	16	0.02	0.03	-0.00
				Max. Vx	26	0.00	0.00	0.00
		Top Girt	Max Tension	8	0.93	0.00	0.00	
			Max. Compression	2	-0.67	0.00	0.00	
			Max. Mx	14	0.24	-0.03	0.00	
			Max. My	14	0.23	0.00	0.00	
Max. Vy	14		0.02	0.00	0.00			
Max. Vx	14		-0.00	0.00	0.00			
T4	100 - 80		Leg	Max Tension	4	84.74	-3.57	0.00
				Max. Compression	2	-95.66	5.12	-0.03
				Max. Mx	2	-95.66	5.12	-0.03
				Max. My	3	-4.08	-0.09	-4.17
		Max. Vy		4	-0.84	-3.13	0.02	
		Max. Vx		3	-0.96	-0.09	-4.17	
		Diagonal	Max Tension	12	8.21	0.00	0.00	
			Max. Compression	6	-9.57	0.00	0.00	
			Max. Mx	2	2.31	0.06	-0.00	
			Max. My	19	-4.46	0.00	0.00	
			Max. Vy	16	0.02	0.04	-0.00	
			Max. Vx	19	-0.00	0.00	0.00	
		Horizontal	Max Tension	4	5.67	0.00	0.00	
			Max. Compression	2	-4.55	0.00	0.00	
Max. Mx	14		1.18	-0.05	0.00			
Max. My	14		1.12	0.00	0.00			
Max. Vy	14		-0.03	0.00	0.00			
Max. Vx	14		-0.00	0.00	0.00			
Top Girt	Max Tension		4	4.74	0.00	0.00		
	Max. Compression		2	-3.83	0.00	0.00		
	Max. Mx		14	0.91	-0.04	0.00		
	Max. My		14	0.84	0.00	0.00		
	Max. Vy	14	0.03	0.00	0.00			
	Max. Vx	14	0.00	0.00	0.00			
	T5	80 - 60	Leg	Max Tension	12	122.49	-3.22	-0.02
				Max. Compression	2	-138.99	4.72	0.01
Max. Mx				2	-118.15	5.12	-0.03	
Max. My				13	-6.90	-0.29	6.34	
Max. Vy				6	-0.34	4.70	0.01	
Max. Vx				5	0.48	-0.28	6.32	
Diagonal			Max Tension	7	7.20	0.00	0.00	
			Max. Compression	6	-7.26	0.00	0.00	
			Max. Mx	2	6.15	0.10	-0.01	
			Max. My	13	-7.04	-0.05	-0.02	
			Max. Vy	15	-0.03	0.07	0.01	
			Max. Vx	13	0.00	0.00	0.00	
Top Girt			Max Tension	4	4.92	0.00	0.00	
			Max. Compression	2	-4.07	0.00	0.00	
	Max. Mx	14	0.94	-0.07	0.00			

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T6	60 - 40	Leg	Max. My	14	0.90	0.00	0.00
			Max. Vy	14	0.04	0.00	0.00
			Max. Vx	14	-0.00	0.00	0.00
			Max Tension	12	154.10	-3.96	0.00
			Max. Compression	2	-174.50	5.35	0.05
			Max. Mx	2	-174.50	5.35	0.05
		Diagonal	Max. My	5	-9.34	0.08	5.00
			Max. Vy	4	0.25	-5.16	0.09
			Max. Vx	5	-0.26	0.08	5.00
			Max Tension	9	7.16	0.00	0.00
			Max. Compression	9	-7.21	0.00	0.00
			Max. Mx	2	6.11	0.08	0.00
T7	40 - 20	Leg	Max. My	23	-2.57	0.03	-0.01
			Max. Vy	25	0.03	0.06	-0.01
			Max. Vx	23	0.00	0.00	0.00
			Max Tension	12	183.32	-3.87	-0.01
			Max. Compression	2	-208.26	4.79	0.00
			Max. Mx	2	-191.37	5.35	0.05
		Diagonal	Max. My	5	-9.94	0.08	5.00
			Max. Vy	21	0.52	-4.69	-0.02
			Max. Vx	11	-0.29	-0.15	-4.85
			Max Tension	9	7.59	0.00	0.00
			Max. Compression	9	-7.75	0.00	0.00
			Max. Mx	2	5.98	0.12	0.01
T8	20 - 0	Leg	Max. My	24	3.13	0.09	-0.01
			Max. Vy	25	0.05	0.09	0.01
			Max. Vx	24	0.00	0.00	0.00
			Max Tension	12	209.62	-4.45	-0.02
			Max. Compression	2	-239.75	0.00	0.00
			Max. Mx	15	-114.88	6.42	-0.04
		Diagonal	Max. My	11	-12.52	-0.31	-7.44
			Max. Vy	21	-0.86	-4.69	-0.02
			Max. Vx	11	-0.84	-0.31	-7.44
			Max Tension	4	8.47	0.00	0.00
			Max. Compression	10	-9.17	0.00	0.00
			Max. Mx	25	0.46	0.12	-0.01

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg C	Max. Vert	10	248.36	22.89	-13.61
	Max. H _x	10	248.36	22.89	-13.61
	Max. H _z	4	-217.00	-20.38	12.11
	Min. Vert	4	-217.00	-20.38	12.11
	Min. H _x	4	-217.00	-20.38	12.11
Leg B	Min. H _z	10	248.36	22.89	-13.61
	Max. Vert	6	247.71	-22.95	-13.47
	Max. H _x	12	-217.25	20.40	11.98
	Max. H _z	12	-217.25	20.40	11.98
	Min. Vert	12	-217.25	20.40	11.98
Leg A	Min. H _x	6	247.71	-22.95	-13.47
	Min. H _z	6	247.71	-22.95	-13.47
	Max. Vert	2	249.10	-0.15	26.66
	Max. H _x	11	13.85	1.60	1.14
	Max. H _z	2	249.10	-0.15	26.66
	Min. Vert	8	-216.43	0.12	-23.65
	Min. H _x	5	14.18	-1.64	1.20
	Min. H _z	8	-216.43	0.12	-23.65

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead Only	40.08	0.00	0.00	-7.44	4.77	0.00
Dead+Wind 0 deg - No Ice	40.08	0.14	-40.35	-3266.48	-1.44	-3.48
Dead+Wind 30 deg - No Ice	40.08	19.81	-34.18	-2786.72	-1601.81	-6.82
Dead+Wind 60 deg - No Ice	40.08	33.95	-19.63	-1605.59	-2758.77	-8.63
Dead+Wind 90 deg - No Ice	40.08	39.47	-0.09	-11.41	-3201.51	-8.32
Dead+Wind 120 deg - No Ice	40.08	34.98	20.05	1616.70	-2816.20	-5.59
Dead+Wind 150 deg - No Ice	40.08	19.68	34.13	2769.46	-1595.79	-1.09
Dead+Wind 180 deg - No Ice	40.08	-0.02	39.15	3184.07	5.90	3.53
Dead+Wind 210 deg - No Ice	40.08	-19.72	34.18	2771.73	1607.39	7.23
Dead+Wind 240 deg - No Ice	40.08	-34.91	20.18	1622.46	2823.29	9.07
Dead+Wind 270 deg - No Ice	40.08	-39.42	0.01	-6.83	3208.97	7.91
Dead+Wind 300 deg - No Ice	40.08	-33.87	-19.56	-1602.21	2764.71	5.10
Dead+Wind 330 deg - No Ice	40.08	-19.68	-34.13	-2784.29	1605.41	1.09
Dead+Ice+Temp	88.29	0.00	-0.00	-19.86	16.62	0.00
Dead+Wind 0 deg+Ice+Temp	88.29	0.01	-14.96	-1223.76	16.03	-0.93
Dead+Wind 30 deg+Ice+Temp	88.29	7.20	-12.47	-1039.52	-571.98	-1.75
Dead+Wind 60 deg+Ice+Temp	88.29	12.25	-7.08	-601.86	-990.41	-2.13
Dead+Wind 90 deg+Ice+Temp	88.29	14.39	-0.01	-20.29	-1159.86	-2.06
Dead+Wind 120 deg+Ice+Temp	88.29	12.95	7.47	581.57	-1025.27	-1.44
Dead+Wind 150 deg+Ice+Temp	88.29	7.19	12.46	999.49	-571.32	-0.28
Dead+Wind 180 deg+Ice+Temp	88.29	-0.00	14.15	1143.55	16.83	0.84
Dead+Wind 210 deg+Ice+Temp	88.29	-7.20	12.47	999.78	604.93	1.78
Dead+Wind 240 deg+Ice+Temp	88.29	-12.95	7.48	582.21	1058.45	2.37
Dead+Wind 270 deg+Ice+Temp	88.29	-14.39	0.00	-19.70	1192.94	2.03
Dead+Wind 300 deg+Ice+Temp	88.29	-12.25	-7.07	-601.39	1023.26	1.30
Dead+Wind 330 deg+Ice+Temp	88.29	-7.19	-12.46	-1039.22	604.57	0.28
Dead+Wind 0 deg - Service	40.08	0.05	-13.96	-1135.13	2.62	-1.20
Dead+Wind 30 deg - Service	40.08	6.86	-11.83	-969.13	-551.14	-2.36
Dead+Wind 60 deg - Service	40.08	11.75	-6.79	-560.44	-951.47	-2.99
Dead+Wind 90 deg - Service	40.08	13.66	-0.03	-8.82	-1104.67	-2.88
Dead+Wind 120 deg - Service	40.08	12.10	6.94	554.55	-971.35	-1.93
Dead+Wind 150 deg - Service	40.08	6.81	11.81	953.42	-549.06	-0.38
Dead+Wind 180 deg - Service	40.08	-0.01	13.55	1096.89	5.16	1.22
Dead+Wind 210 deg - Service	40.08	-6.82	11.83	954.21	559.31	2.50
Dead+Wind 240 deg - Service	40.08	-12.08	6.98	556.54	980.03	3.14
Dead+Wind 270 deg - Service	40.08	-13.64	0.00	-7.23	1113.49	2.74
Dead+Wind 300 deg - Service	40.08	-11.72	-6.77	-559.26	959.76	1.76
Dead+Wind 330 deg - Service	40.08	-6.81	-11.81	-968.29	558.62	0.38

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-40.08	0.00	0.00	40.08	0.00	0.000%
2	0.14	-40.08	-40.35	-0.14	40.08	40.35	0.000%
3	19.81	-40.08	-34.18	-19.81	40.08	34.18	0.000%
4	33.95	-40.08	-19.63	-33.95	40.08	19.63	0.000%
5	39.47	-40.08	-0.09	-39.47	40.08	0.09	0.000%
6	34.98	-40.08	20.05	-34.98	40.08	-20.05	0.000%
7	19.68	-40.08	34.13	-19.68	40.08	-34.13	0.000%
8	-0.02	-40.08	39.15	0.02	40.08	-39.15	0.000%
9	-19.72	-40.08	34.18	19.72	40.08	-34.18	0.000%
10	-34.91	-40.08	20.18	34.91	40.08	-20.18	0.000%
11	-39.42	-40.08	0.01	39.42	40.08	-0.01	0.000%
12	-33.87	-40.08	-19.56	33.87	40.08	19.56	0.000%
13	-19.68	-40.08	-34.13	19.68	40.08	34.13	0.000%
14	0.00	-88.29	0.00	0.00	88.29	0.00	0.000%
15	0.01	-88.29	-14.96	-0.01	88.29	14.96	0.000%
16	7.20	-88.29	-12.47	-7.20	88.29	12.47	0.000%
17	12.25	-88.29	-7.08	-12.25	88.29	7.08	0.000%
18	14.39	-88.29	-0.01	-14.39	88.29	0.01	0.000%
19	12.95	-88.29	7.47	-12.95	88.29	-7.47	0.000%
20	7.19	-88.29	12.46	-7.19	88.29	-12.46	0.000%
21	-0.00	-88.29	14.15	0.00	88.29	-14.15	0.000%
22	-7.20	-88.29	12.47	7.20	88.29	-12.47	0.000%
23	-12.95	-88.29	7.48	12.95	88.29	-7.48	0.000%
24	-14.39	-88.29	0.00	14.39	88.29	-0.00	0.000%
25	-12.25	-88.29	-7.07	12.25	88.29	7.07	0.000%
26	-7.19	-88.29	-12.46	7.19	88.29	12.46	0.000%
27	0.05	-40.08	-13.96	-0.05	40.08	13.96	0.000%
28	6.86	-40.08	-11.83	-6.86	40.08	11.83	0.000%
29	11.75	-40.08	-6.79	-11.75	40.08	6.79	0.000%
30	13.66	-40.08	-0.03	-13.66	40.08	0.03	0.000%
31	12.10	-40.08	6.94	-12.10	40.08	-6.94	0.000%
32	6.81	-40.08	11.81	-6.81	40.08	-11.81	0.000%
33	-0.01	-40.08	13.55	0.01	40.08	-13.55	0.000%
34	-6.82	-40.08	11.83	6.82	40.08	-11.83	0.000%
35	-12.08	-40.08	6.98	12.08	40.08	-6.98	0.000%
36	-13.64	-40.08	0.00	13.64	40.08	-0.00	0.000%
37	-11.72	-40.08	-6.77	11.72	40.08	6.77	0.000%
38	-6.81	-40.08	-11.81	6.81	40.08	11.81	0.000%

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	150 - 130	6.177	27	0.3800	0.0191
T2	130 - 110	4.563	27	0.3543	0.0185
T3	110 - 100	3.134	27	0.2942	0.0133
T4	100 - 80	2.539	27	0.2611	0.0097
T5	80 - 60	1.557	27	0.1883	0.0062
T6	60 - 40	0.854	27	0.1292	0.0044
T7	40 - 20	0.373	27	0.0835	0.0027
T8	20 - 0	0.099	27	0.0362	0.0014

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
151'	PR-950	27	6.177	0.3800	0.0191	103351
141'	(2) 7770.00 w/ Mount Pipe	27	5.439	0.3716	0.0192	57417
133'	10' x 2" Omni	27	4.797	0.3602	0.0188	30397

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
127'	20" x 2" Omni	27	4.332	0.3471	0.0181	22470
117'	1 sq. ft antenna	27	3.601	0.3174	0.0157	15630
112'	(2) RR90-17-02DP	27	3.263	0.3008	0.0140	13708
100'	800MHz 2X50W RRH W/FILTER	27	2.539	0.2611	0.0097	18077
88'	(2) LPA-80063/4CF w/ Mount Pipe	27	1.917	0.2173	0.0071	16962
85'	7130.14.05.00 w/ Mount Pipe	27	1.776	0.2062	0.0067	16532
65'	APXV18-206517S-C w/ Mount Pipe	27	1.007	0.1421	0.0048	20885
50'	GPS-TMG-26N	27	0.587	0.1060	0.0036	23260
43'	SSP2-23	27	0.432	0.0904	0.0029	23198

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	150 - 130	17.779	2	1.0927	0.0552
T2	130 - 110	13.136	2	1.0189	0.0535
T3	110 - 100	9.026	2	0.8466	0.0383
T4	100 - 80	7.313	2	0.7515	0.0280
T5	80 - 60	4.486	2	0.5421	0.0178
T6	60 - 40	2.460	2	0.3717	0.0128
T7	40 - 20	1.073	2	0.2404	0.0077
T8	20 - 0	0.284	2	0.1041	0.0040

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
151'	PR-950	2	17.779	1.0927	0.0552	36107
141'	(2) 7770.00 w/ Mount Pipe	2	15.658	1.0687	0.0554	20060
133'	10' x 2" Omni	2	13.812	1.0361	0.0544	10619
127'	20" x 2" Omni	2	12.473	0.9985	0.0522	7842
117'	1 sq. ft antenna	2	10.371	0.9132	0.0454	5450
112'	(2) RR90-17-02DP	2	9.398	0.8657	0.0405	4781
100'	800MHz 2X50W RRH W/FILTER	2	7.313	0.7515	0.0280	6290
88'	(2) LPA-80063/4CF w/ Mount Pipe	2	5.520	0.6255	0.0206	5894
85'	7130.14.05.00 w/ Mount Pipe	2	5.116	0.5936	0.0194	5743
65'	APXV18-206517S-C w/ Mount Pipe	2	2.900	0.4091	0.0139	7253
50'	GPS-TMG-26N	2	1.692	0.3050	0.0103	8076
43'	SSP2-23	2	1.243	0.2602	0.0085	8053

Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T1	150	Leg	A325N	0.6250	5	4.70	12.89	0.365 ✓	1.333	Bolt DS
T2	130	Leg	A325N	1.0000	6	8.71	34.53	0.252 ✓	1.333	Bolt Tension
T3	110	Leg	A325N	1.0000	6	9.14	34.56	0.264 ✓	1.333	Bolt Tension

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T4	100	Diagonal	A325N	1.0000	1	6.17	7.75	0.796 ✓	1.333	Member Block Shear
		Top Girt	A325N	1.0000	1	0.93	8.43	0.110 ✓	1.333	Member Block Shear
		Leg	A325N	1.0000	6	14.12	34.56	0.409 ✓	1.333	Bolt Tension
		Diagonal	A325N	1.0000	1	8.21	7.75	1.059 ✓	1.333	Member Block Shear
		Horizontal	A325N	1.0000	1	5.67	8.43	0.673 ✓	1.333	Member Block Shear
T5	80	Top Girt	A325N	1.0000	1	4.74	8.43	0.562 ✓	1.333	Member Block Shear
		Leg	A325N	1.0000	6	20.41	34.56	0.591 ✓	1.333	Bolt Tension
		Diagonal	A325N	1.0000	1	7.20	8.43	0.855 ✓	1.333	Member Block Shear
T6	60	Top Girt	A325N	1.0000	1	4.92	8.43	0.584 ✓	1.333	Member Block Shear
		Leg	A325N	1.0000	6	25.68	34.56	0.743 ✓	1.333	Bolt Tension
T7	40	Diagonal	A325N	1.0000	1	7.16	8.43	0.849 ✓	1.333	Member Block Shear
		Leg	A325N	1.0000	6	30.55	34.56	0.884 ✓	1.333	Bolt Tension
T8	20	Diagonal	A325N	1.0000	1	7.59	14.05	0.540 ✓	1.333	Member Block Shear
		Leg	A687	1.2500	6	34.94	60.75	0.575 ✓	1.333	Bolt Tension
		Diagonal	A325N	1.2500	1	8.47	14.95	0.566 ✓	1.333	Member Block Shear

Compression Checks

Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P/P _a
T1	150 - 130	1 1/2	20'	2'4-9/16"	76.0 K=1.00	19.799	1.7672	-23.51	34.99	0.672 ✓
T2	130 - 110	2	20'	2'4-9/16"	57.0 K=1.00	23.222	3.1416	-58.97	72.95	0.808 ✓
T3	110 - 100	Pirod 105244	10'1/4"	10'1/4"	45.4 K=1.00	25.051	3.6816	-60.65	92.23	0.658 ✓
T4	100 - 80	Pirod 105216	20'3/8"	10'1/4"	45.4 K=1.00	25.051	3.6816	-95.66	92.23	1.037 ✓
T5	80 - 60	Pirod 105217	20'3/8"	10'1/4"	37.8 K=1.00	26.132	5.3014	-138.99	138.54	1.003 ✓
T6	60 - 40	Pirod 105218	20'3/8"	10'1/4"	32.4 K=1.00	26.848	7.2158	-174.50	193.73	0.901 ✓
T7	40 - 20	Pirod 105218	20'3/8"	10'1/4"	32.4 K=1.00	26.848	7.2158	-208.26	193.73	1.075 ✓
T8	20 - 0	Pirod 105219	20'3/8"	10'1/4"	28.4 K=1.00	27.351	9.4248	-239.75	257.78	0.930 ✓

Truss-Leg Diagonal Data

Section No.	Elevation ft	Diagonal Size	L_d ft	Kl/r	F_a ksi	A in^2	Actual V K	Allow. V_a K	Stress Ratio
T3	110 - 100	0.5	1'5-3/4"	121.0	10.193	0.1963	0.28	2.24	0.125
T4	100 - 80	0.5	1'5-3/4"	121.0	10.133	0.1963	0.96	2.23	0.431
T5	80 - 60	0.5	1'5-5/8"	120.0	10.279	0.1963	0.48	2.26	0.212
T6	60 - 40	0.5	1'5-17/32"	119.0	10.423	0.1963	0.27	2.29	0.117
T7	40 - 20	0.5	1'5-17/32"	119.0	10.423	0.1963	0.52	2.29	0.226
T8	20 - 0	0.625	1'5-13/32"	94.4	13.671	0.3068	0.87	4.69	0.185



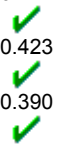
Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L_u ft	Kl/r	F_a ksi	A in^2	Actual P K	Allow. P_a K	Ratio $\frac{P}{P_a}$
T1	150 - 130	3/4	5'19/32"	2'5-5/8"	142.4 K=0.90	7.368	0.4418	-2.67	3.26	0.820
T2	130 - 110	7/8	5'6"	2'8-1/32"	132.0 K=0.90	8.576	0.6013	-3.69	5.16	0.716
T3	110 - 100	L2 1/2x2 1/2x3/16	11'5-1/32"	4'11-3/4"	120.8 K=1.00	10.170	0.9020	-7.12	9.17	0.776
T4	100 - 80	L2 1/2x2 1/2x3/16	12'6"	5'7-9/16"	136.4 K=1.00	8.025	0.9020	-9.57	7.24	1.322
T5	80 - 60	L3x3x3/16	13'9-19/32"	6'3-31/32"	127.4 K=1.00	9.200	1.0900	-7.16	10.03	0.714
T6	60 - 40	L3x3x3/16	15'2-7/8"	7'31/32"	142.6 K=1.00	7.345	1.0900	-7.18	8.01	0.897
T7	40 - 20	L3x3x5/16	16'9-19/32"	7'10-9/16"	160.6 K=1.00	5.791	1.7800	-7.75	10.31	0.752
T8	20 - 0	L3x3x5/16	18'5-13/32"	8'8-5/32"	176.8 K=1.00	4.779	1.7800	-9.17	8.51	1.078



Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L_u ft	Kl/r	F_a ksi	A in^2	Actual P K	Allow. P_a K	Ratio $\frac{P}{P_a}$
T1	150 - 130	3/4	4'4-7/16"	4'2-7/8"	190.1 K=0.70	4.131	0.4418	-0.26	1.83	0.144
T2	130 - 110	3/4	4'6-27/32"	4'4-29/32"	197.4 K=0.70	3.834	0.4418	-0.72	1.69	0.423
T4	100 - 80	L3x3x3/16	7'	5'6-31/32"	116.2 K=1.03	10.720	1.0900	-4.55	11.68	0.390



Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L_u ft	Kl/r	F_a ksi	A in^2	Actual P K	Allow. P_a K	Ratio $\frac{P}{P_a}$
T1	150 - 130	7/8	4'1/8"	3'10-	149.3	6.701	0.6013	-0.44	4.03	0.109

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a	
T2	130 - 110	7/8	4'6-1/8"	11/16" 4'4-3/16"	K=0.70 K=0.70	166.9	5.362	0.6013	-1.33	3.22	0.413
T3	110 - 100	L3x3x3/16	5'	4'5-1/32"	K=1.17	104.5	12.208	1.0900	-0.67	13.31	0.050
T4	100 - 80	L3x3x3/16	6'	4'6-31/32"	K=1.15	106.1	12.002	1.0900	-3.83	13.08	0.293
T5	80 - 60	L3x3x3/16	8'	6'6-31/32"	K=1.00	132.6	8.499	1.0900	-4.07	9.26	0.439

Bottom Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	150 - 130	7/8	4'5-7/8"	4'4-5/16"	167.5 K=0.70	5.321	0.6013	-1.22	3.20	0.380
T2	130 - 110	7/8	4'11-7/8"	4'9-27/32"	185.1 K=0.70	4.358	0.6013	-1.15	2.62	0.439

Tension Checks

Leg Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	150 - 130	1 1/2	20'	2'4-9/16"	76.0	32.500	0.7732	19.75	25.13	0.786
T2	130 - 110	2	20'	2'4-9/16"	57.0	30.000	3.1416	52.24	94.25	0.554
T3	110 - 100	Pirod 105244	10'1/4"	10'1/4"	45.4	30.000	3.6816	54.82	110.45	0.496
T4	100 - 80	Pirod 105216	20'3/8"	10'1/4"	45.4	30.000	3.6816	84.74	110.45	0.767
T5	80 - 60	Pirod 105217	20'3/8"	10'1/4"	37.8	30.000	5.3014	122.49	159.04	0.770
T6	60 - 40	Pirod 105218	20'3/8"	10'1/4"	32.4	30.000	7.2158	154.10	216.47	0.712
T7	40 - 20	Pirod 105218	20'3/8"	10'1/4"	32.4	30.000	7.2158	183.32	216.47	0.847
T8	20 - 0	Pirod 105219	20'3/8"	10'1/4"	28.4	30.000	9.4248	209.63	282.74	0.741

Truss-Leg Diagonal Data

Section No.	Elevation ft	Diagonal Size	L _d ft	Kl/r	F _a ksi	A in ²	Actual V K	Allow. V _a K	Stress Ratio
T3	110 - 100	0.5	1'5-3/4"	121.0	10.193	0.1963	0.28	2.24	0.125

Section No.	Elevation ft	Diagonal Size	L_d ft	Kl/r	F_a ksi	A in^2	Actual V K	Allow. V_a K	Stress Ratio
T4	100 - 80	0.5	1'5-3/4"	121.0	10.133	0.1963	0.96	2.23	0.431
T5	80 - 60	0.5	1'5-5/8"	120.0	10.279	0.1963	0.48	2.26	0.212
T6	60 - 40	0.5	1'5-17/32"	119.0	10.423	0.1963	0.27	2.29	0.117
T7	40 - 20	0.5	1'5-17/32"	119.0	10.423	0.1963	0.52	2.29	0.226
T8	20 - 0	0.625	1'5-13/32"	94.4	13.671	0.3068	0.87	4.69	0.185

Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L_u ft	Kl/r	F_a ksi	A in^2	Actual P K	Allow. P_a K	Ratio $\frac{P}{P_a}$
T1	150 - 130	3/4	5'19/32"	2'5-5/8"	158.2	30.000	0.4418	2.58	13.25	0.195
T2	130 - 110	7/8	5'6"	2'8-1/32"	146.6	30.000	0.6013	3.65	18.04	0.203
T3	110 - 100	L2 1/2x2 1/2x3/16	11'5-1/32"	4'11-3/4"	80.1	29.000	0.5183	6.17	15.03	0.410
T4	100 - 80	L2 1/2x2 1/2x3/16	12'6"	5'7-9/16"	90.0	29.000	0.5183	8.21	15.03	0.546
T5	80 - 60	L3x3x3/16	13'9-19/32"	6'3-31/32"	83.5	29.000	0.6593	7.20	19.12	0.377
T6	60 - 40	L3x3x3/16	14'6"	6'8-3/4"	88.6	29.000	0.6593	7.16	19.12	0.374
T7	40 - 20	L3x3x5/16	16'1/8"	7'5-7/8"	100.3	29.000	1.0713	7.59	31.07	0.244
T8	20 - 0	L3x3x5/16	18'5-13/32"	8'8-5/32"	116.2	29.000	1.0127	8.47	29.37	0.288

Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L_u ft	Kl/r	F_a ksi	A in^2	Actual P K	Allow. P_a K	Ratio $\frac{P}{P_a}$
T1	150 - 130	3/4	4'4-7/16"	4'2-7/8"	271.6	30.000	0.4418	0.39	13.25	0.030
T2	130 - 110	3/4	4'6-27/32"	4'4-29/32"	281.9	30.000	0.4418	0.82	13.25	0.062
T4	100 - 80	L3x3x3/16	7'	5'6-31/32"	76.7	29.000	0.6593	5.67	19.12	0.297

Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L_u ft	Kl/r	F_a ksi	A in^2	Actual P K	Allow. P_a K	Ratio $\frac{P}{P_a}$
T1	150 - 130	7/8	4'1/8"	3'10-11/16"	213.3	30.000	0.6013	0.42	18.04	0.023
T2	130 - 110	7/8	4'6-1/8"	4'4-3/16"	238.4	30.000	0.6013	1.35	18.04	0.075

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P/P _a
T3	110 - 100	L3x3x3/16	5'	4'5-1/32"	61.8	29.000	0.6593	0.93	19.12	0.048
T4	100 - 80	L3x3x3/16	6'	4'6-31/32"	63.9	29.000	0.6593	4.74	19.12	0.248
T5	80 - 60	L3x3x3/16	8'	6'6-31/32"	89.5	29.000	0.6593	4.92	19.12	0.257



Bottom Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P/P _a
T1	150 - 130	7/8	4'5-7/8"	4'4-5/16"	239.3	30.000	0.6013	1.31	18.04	0.072
T2	130 - 110	7/8	4'11-7/8"	4'9-27/32"	264.5	30.000	0.6013	1.24	18.04	0.069



Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail	
T1	150 - 130	Leg	1 1/2	1	19.75	33.50	59.0	Pass	
T2	130 - 110	Leg	2	67	-58.97	97.25	60.6	Pass	
T3	110 - 100	Leg	Pirol 105244	131	-60.65	122.94	49.3	Pass	
T4	100 - 80	Leg	Pirol 105216	143	-95.66	122.94	77.8	Pass	
T5	80 - 60	Leg	Pirol 105217	164	-138.99	184.67	75.3	Pass	
T6	60 - 40	Leg	Pirol 105218	182	-174.50	258.24	67.6	Pass	
T7	40 - 20	Leg	Pirol 105218	197	-208.26	258.24	80.6	Pass	
T8	20 - 0	Leg	Pirol 105219	212	-239.75	343.62	69.8	Pass	
T1	150 - 130	Diagonal	3/4	10	-2.67	4.34	61.5	Pass	
T2	130 - 110	Diagonal	7/8	76	-3.69	6.87	53.7	Pass	
T3	110 - 100	Diagonal	L2 1/2x2 1/2x3/16	137	-7.12	12.23	58.2	Pass	
							59.7 (b)		
T4	100 - 80	Diagonal	L2 1/2x2 1/2x3/16	149	-9.57	9.65	99.2	Pass	
T5	80 - 60	Diagonal	L3x3x3/16	170	-7.16	13.37	53.5	Pass	
							64.1 (b)		
T6	60 - 40	Diagonal	L3x3x3/16	188	-7.18	10.67	67.3	Pass	
T7	40 - 20	Diagonal	L3x3x5/16	203	-7.75	13.74	56.4	Pass	
T8	20 - 0	Diagonal	L3x3x5/16	218	-9.17	11.34	80.9	Pass	
T1	150 - 130	Horizontal	3/4	23	-0.26	2.43	10.8	Pass	
T2	130 - 110	Horizontal	3/4	122	-0.72	2.26	31.7	Pass	
T4	100 - 80	Horizontal	L3x3x3/16	153	-4.55	15.58	29.2	Pass	
							50.5 (b)		
T1	150 - 130	Top Girt	7/8	5	-0.44	5.37	8.2	Pass	
T2	130 - 110	Top Girt	7/8	70	-1.33	4.30	31.0	Pass	
T3	110 - 100	Top Girt	L3x3x3/16	132	-0.67	17.74	3.8	Pass	
							8.2 (b)		
T4	100 - 80	Top Girt	L3x3x3/16	144	-3.83	17.44	22.0	Pass	
							42.2 (b)		
T5	80 - 60	Top Girt	L3x3x3/16	165	-4.07	12.35	32.9	Pass	
							43.8 (b)		
T1	150 - 130	Bottom Girt	7/8	7	-1.22	4.27	28.5	Pass	
T2	130 - 110	Bottom Girt	7/8	71	-1.15	3.49	32.9	Pass	
							Summary		
							Leg (T7)	80.6	Pass
							Diagonal (T4)	99.2	Pass
							Horizontal	50.5	Pass

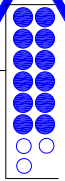
Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
						(T4)		
						Top Girt (T5)	43.8	Pass
						Bottom Girt (T2)	32.9	Pass
						Bolt Checks	79.4	Pass
						RATING =	99.2	Pass

APPENDIX B
BASE LEVEL DRAWING



(RESERVED)
(3) 1-1/4" TO 141 FT LEVEL
(INSTALLED)
(12) 1-5/8" TO 141 FT LEVEL

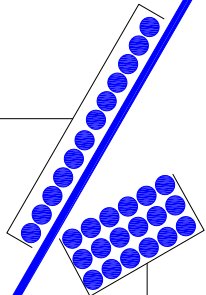
LEG A



(INSTALLED)
(6) 1-5/8" TO 65 FT LEVEL

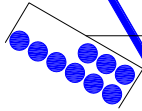
(PROPOSED)
(3) 1-1/4" TO 100 FT LEVEL
(INSTALLED—TO BE REMOVED)
(3) 1-1/4" TO 100 FT LEVEL
(INSTALLED)
(6) 1-1/4" TO 100 FT LEVEL
(1) 1/2" TO 50 FT LEVEL
(1) 7/8" TO 43 FT LEVEL

(INSTALLED)
(12) 1-5/8" TO 88 FT LEVEL



(INSTALLED)
(1) 1-5/8" TO 85 FT LEVEL
(2) 1-5/8" TO 117 FT LEVEL
(1) 1-5/8" TO 127 FT LEVEL
(2) 1-5/8" TO 133 FT LEVEL
(3) 1-5/8" TO 150 FT LEVEL

LEG B



LEG C

(INSTALLED)
(18) 1-5/8" TO 112 FT LEVEL

APPENDIX C
ADDITIONAL CALCULATIONS

FOUNDATION REACTION COMPARISON

BU# 842870

WO# 717495

REACTIONS	DESIGN REACTIONS	CURRENT REACTIONS	% CAPACITY
UPLIFT (kips)	237.8	217.0	91.3%
COMPRESSION (kips)	269.5	249.0	92.4%
MAX SHEAR (kips)	44.7	40.0	89.5%

Design loads from: CCI sites Doc #4480661



Know what's below.
Call before you dig.

APPROVALS			
	DATE	APPROVED	DISAPPROVED
		AS NOTED	REVISE
SPRINT REPRESENTATIVES		<input type="checkbox"/>	<input type="checkbox"/>
SPRINT RF ENGINEER		<input type="checkbox"/>	<input type="checkbox"/>
SITE OWNER		<input type="checkbox"/>	<input type="checkbox"/>
		<input type="checkbox"/>	<input type="checkbox"/>



SITE ID: CT23XC552
SITE NAME: MILFORD / AT&T

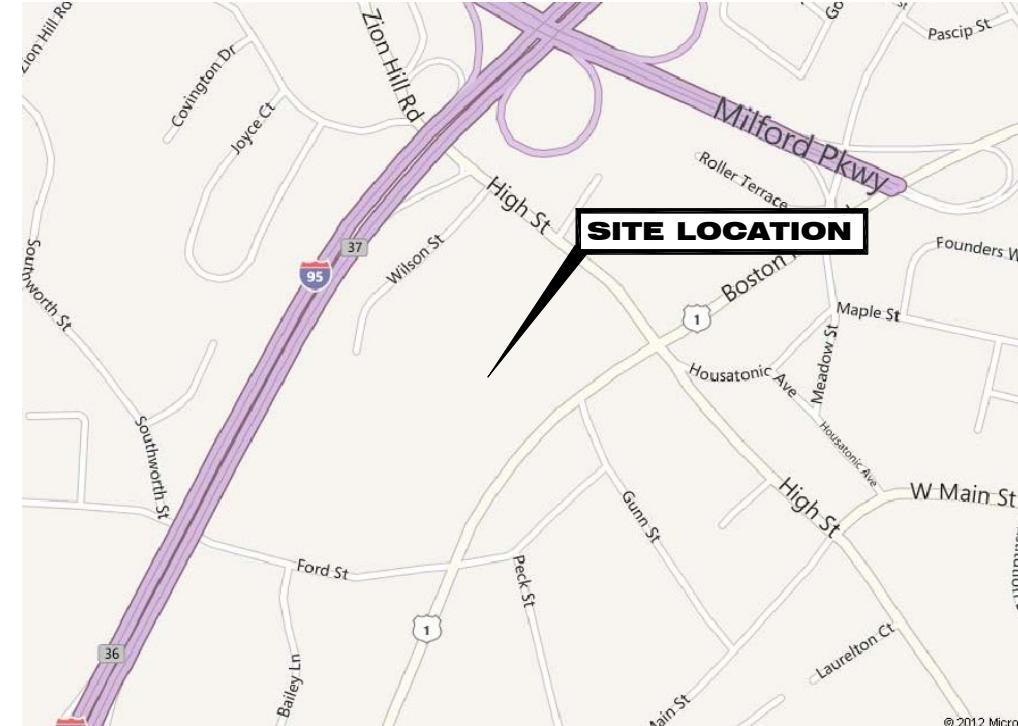
THE STRUCTURAL ENGINEERING CONCERNING THE STRUCTURAL STABILITY OF THE TOWER/POLE AND FOUNDATION IS BEING COMPLETED BY OTHERS. KMB DESIGN GROUP, LLC HAS NOT BEEN REQUESTED TO PERFORM ANY STRUCTURAL ANALYSIS SERVICES TO VERIFY THAT THE TOWER/POLE AND/OR FOUNDATION IS CAPABLE OF SUPPORTING THE PROPOSED EQUIPMENT DEPICTED WITHIN THESE SIGNED AND SEALED DRAWINGS. SIGNED AND SEALED DRAWINGS REVISED TO STATE "ISSUED FOR CONSTRUCTION" SHALL BE PROVIDED TO THE PROFESSIONAL ENGINEERS RESPONSIBLE FOR THE STRUCTURAL ANALYSIS OF THE TOWER/POLE AND FOUNDATION. KMB DESIGN GROUP, LLC SHALL BE NOTIFIED SHOULD THE STRUCTURAL ANALYSIS RESULT IN SOME ELEMENTS NOT BEING STRUCTURALLY CAPABLE OF SUPPORTING THE PROPOSED DESIGN DEPICTED. THE CONTRACTOR SHALL NOT COMMENCE CONSTRUCTION WITHOUT OBTAINING (A) A SIGNED AND SEALED COPY OF THE PLANS "ISSUED FOR CONSTRUCTION"; (B) STRUCTURAL ANALYSIS REPORT STATING THAT THE TOWER/POLE/FOUNDATION IS CAPABLE OF SUPPORTING THE PROPOSED LOADING REFERENCING THE SIGNED AND SEALED PLANS BY KMB DESIGN GROUP, LLC; (C) SPRINT PLATFORM ANALYSIS STATING THAT THE SPRINT PLATFORM IS CAPABLE OF SUPPORTING THE PROPOSED DESIGN AS REFERENCED WITHIN THE SIGNED AND SEALED PLANS BY KMB DESIGN GROUP, LLC.



NETWORK VISION CONSTRUCTION DRAWINGS



AERIAL VIEW
SCALE: NTS



LOCATION MAP
SCALE: NTS

SITE INFORMATION

PARCEL ID: 64-930-6
ZONING CLASSIFICATION: GU (GOVERNMENT USE)
ZONING JURISDICTION: MILFORD TOWNSHIP

PROJECT INFORMATION:

SITE ADDRESS:
430 BOSTON POST ROAD
MILFORD, CT 06460
NEW HAVEN COUNTY
COORDINATES:
LATITUDE: N 41° 13' 42.65"
LONGITUDE: W 73° 4' 12.46"] DATUM : NAD 83

STRUCTURE HEIGHT:
±150'-0" (TOP OF LATTICE TOWER)

PROJECT DIRECTORY:

PROPERTY OWNER:
TOWN OF MILFORD
70 W. RIVER STREET, 2ND FLOOR
MILFORD, CT 06460

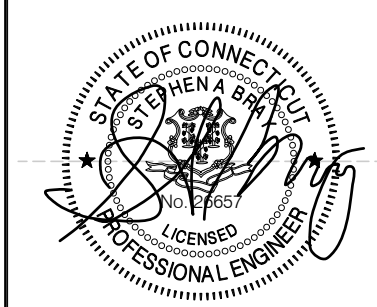
APPLICANT:
SPRINT-NEXTEL
6200 SPRINT PARKWAY
OVERLAND PARK, KS 66251

ENGINEER:
KMB DESIGN GROUP, LLC
1800 ROUTE 34, SUITE 209
WALL, NJ 07719
KEITH DRENNAN - PROJECT MANAGER
(732) 280-5623

POWER COMPANY:
THE UNITED ILLUMINATING CO.
157 CHURCH STREET
NEW HAVEN, CT 06510
1-800-7-CALL UI



Stephen A. Bray
PROFESSIONAL ENGINEER



CT LICENSE: 26657 11/5/12

PROJECT NUMBER:
332.1487

SITE INFORMATION:
430 BOSTON POST ROAD
MILFORD, CT 06460
NEW HAVEN COUNTY

CT23XC552

PROJECT TYPE:
NETWORK VISION

DRAWN BY: KAZ CHECKED BY: DATE: 03-15-12

SHEET TITLE:
COVER SHEET

SHEET NUMBER: REV:

A01 1

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CODES & STANDARDS

These documents are in compliance & all construction to be in accordance with the following codes & standards as applicable:
State Building Code: 2005 Connecticut Supplement which includes the 2009 Amendment (effective August 1, 2009) to the 2005 State Building Code.
2003 International Building Code
2003 International Residential Code
2003 International Existing Building Code
2003 International Mechanical Code
2003 International Plumbing Code
2006 International Energy Conservation Code (new edition adopted with changes-effective August 1, 2009)
2009 International Energy Conservation Code with Connecticut Amendments (effective October 7, 2011)
ICC/ANSI A117.1-2003 Accessible and Usable Buildings and Facilities
2005 National Electrical Code (NFPA-70)

DRIVING DIRECTIONS

- DEPART 1 INTERNATIONAL BLVD, MAHWAH, NJ 07495
- HEAD NORTH ON INTERNATIONAL BLVD TOWARD QUEENSLAND RD.
- TURN RIGHT ONTO PARK LN.
- CONTINUE STRAIGHT ONTO LEISURE LN
- SLIGHT RIGHT ONTO NJ-17 N
- MERGE ONTO I-287 N/NJ-17 N VIA THE RAMP ON THE LEFT TO I-87/NY THRUWAY
- ENTER NEW YORK
- KEEP RIGHT AT THE FORK, FOLLOW SIGNS FOR I-87 S/I-287/TAPPAN ZEE BR/NEW YORK CITY/NEW YORK THRUWAY AND MERGE ONTO I-287 E/I-87 N
- CONTINUE TO FOLLOW I-287 E
- TAKE THE EXIT ONTO I-95 N
- ENTER CONNECTICUT
- TAKE EXIT 33 FOR US-1/CT-130/FERRY BLVD TOWARD DEVON
- MERGE ONTO CT-130 E/FERRY BLVD
- CONTINUE TO FOLLOW FERRY BLVD
- CONTINUE ONTO BRIDGEPORT AVE
- CONTINUE ONTO BOSTON POST RD
- DESTINATION WILL BE ON THE LEFT

DWG #	DRAWING TITLES
A01	COVER SHEET
C01	GENERAL NOTES 1 OF 2
C01A	GENERAL NOTES 2 OF 2
C02	COMPOUND PLAN
C02A	ELEVATION
C03	EQUIPMENT PLANS
C03A	EQUIPMENT & ANTENNA SPECIFICATIONS
C04	EXISTING ANTENNA PLAN (ALL SECTORS)
C04A	INTERIM ANTENNA PLAN (ALL SECTORS)
C04B	FINAL ANTENNA PLAN (ALL SECTORS)
C04C	RRH PLANS & DETAILS (ALL SECTORS)
C05	SITE DETAILS
C06	RF SCHEDULE
C06A	RF DATA SHEET
C07	AAV DRAWINGS - COVER SHEET
C07A	AAV DRAWINGS - SITE PHOTOS
C07B	AAV DRAWINGS - KEY & EQUIPMENT PLAN
C07C	AAV DRAWINGS - DETAILS
E01	ELECTRICAL NOTES
E02	ELECTRICAL & GROUNDING DETAILS

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DIVISION 2 - SITE WORK

- The Contractor shall call utilities prior to the start of construction.
- All existing active sewer, water, gas, electric, and other utilities where encountered in the work, shall be protected at all times, and where required for the proper execution of the work, shall be relocated as directed by Engineers. Extreme caution should be used by the Contractor when excavating or pier drilling around or near utilities. The Contractor shall provide safety training for the working crew. This will include but not limited to:
 - Fall protection
 - Confined space
 - Electrical safety
 - Trenching & excavation
- All site work shall be as indicated on the drawing and stipulated in the specification project summary.
- If necessary, rubbish, stumps, debris, sticks, stones, and other refuse shall be removed from the site and disposed of legally.
- The site shall be graded to cause surface water flow away from the equipment shelter and monopole areas.
- No fill or embankment material shall be placed on frozen ground. Frozen materials, snow or ice shall not be placed in any fill or embankment.
- The sub grade shall be compacted and brought to a smooth uniform grade prior to finished surface application.
- All existing inactive sewer, water, gas, electric and other utilities, which interfere with the execution of the work, shall be removed and/or capped, plugged or otherwise discontinued at points which will not interfere with the execution of the work, subject to the approval of engineering.
- The areas of the Owners property disturbed by the work and not covered by the building or driveway, shall be graded to a uniform slope, fertilized, seeded, and covered with mulch as specified in the specification of landscape work.
- The Contractor shall minimize disturbance to existing site during construction. Erosion control measures, shall be in conformance with the local guidelines for erosion and sediment control.
- All back fill shall be compacted to 95% modified proctor density as determined by ASTM standard test procedures.

DIVISION 3 - CONCRETE

- Design and construction of all concrete elements shall conform to the latest editions of the following applicable codes: ACI 301 "Specifications for Structural Concrete for Buildings"; ACI 318 "Building Code Requirements for Reinforced Concrete".
- Mix design shall be approved by Owner's representative prior to placing concrete.
- Concrete shall be normal weight, 6% air entrained (±1.5%) with a maximum 4" slump, and have a minimum 28-day compressive strength of 3000 psi unless otherwise noted.
- Maximum aggregate size shall be 1".
- The following materials shall be used:

Portland cement:	ASTM C 150, TYPE I
Reinforcement:	ASTM A 185
Normal weight aggregate:	ASTM C 33
Water:	Drinkable
Admixtures:	Non-chloride containing
- Reinforcing details shall be in accordance with the latest edition of ACI 315.
- Reinforcing steel shall conform to ASTM A 615, grade 60, deformed unless noted otherwise. Welded wire fabric shall conform to ASTM A 185 welded steel wire fabric unless noted otherwise. Splices shall be class "B" and all hooks shall be standard, unless otherwise noted.
- The following minimum concrete cover shall be provided for reinforcing steel unless shown otherwise on drawings:
 - Concrete cast against earth 3"
 - Concrete exposed to earth or weather:
 - #6 and larger 2"
 - #5 and smaller 1 1/2"
 - Concrete not exposed to earth or weather or not cast against the ground:
 - Slab and wall 3/4"
 - Beams and columns 1 1/2"
- A 1" chamfer shall be provided at all exposed edges of concrete, unless otherwise noted, in accordance with ACI 30 section 4.2.4.
- Installation of concrete anchor, shall be per manufacturers written recommended procedure, the anchor bolt, dowel or rod shall conform to manufacturer's recommendation for embedment depth or as shown on the drawing. No rebar shall be cut without prior engineering approval when drilling holes in concrete.
- Curing compounds shall conform to ASTM C-309.
- Admixtures shall conform to the appropriate ASTM standard as referenced in ACI-301.
- Do not weld or tack weld reinforcing steel.
- All dowels, anchor bolts, embedded steel, electrical conduits, pipe sleeves, grounds and all other embedded items and formed details shall be in place before start of concrete placement.
- Locate additional construction joints required to facilitate construction as acceptable to Engineer. Place reinforcement continuously through joint.
- Reinforcement shall be cold bent whenever bending is required.
- Place concrete in a uniform manner to prevent the formation of cold joints and other planes of weakness. Vibrate the concrete to fully embed reinforcing. Do not use vibrators to transport concrete through chutes or formwork.
- Do not place concrete in water, ice, or on frozen ground.
- Do not allow concrete sub base to freeze during concrete curing and setting period, or a minimum of 14 days after placement.
- For cold -weather and hot-weather concrete placement, conform to applicable ACI codes and recommendations. In either case, materials containing chloride, calcium, salts, etc. shall not be used. Protect fresh concrete from weather for 7 days minimum.

DIVISION 5 - METALS

SECTION 05120 - STRUCTURAL STEEL

- Codes and specifications:
 - The fabrication/erection shall conform to the requirements of the following codes and specifications, latest edition, unless otherwise noted:
 - The local building code.
 - AISC-specification for structural steel buildings, allowable stress design, 1989.
 - ASTM A992 structural steel (for all w sections only).
 - ASTM A36 structural steel (all other sections).
 - ASTM A53, type E, grade B, electric resistance welded steel pipe.
 - ASTM 123 zinc (hot-dip galvanized) coatings on iron and steel products.
 - ASTM 153 zinc coated (hot-dip) iron and steel hardware.
 - AWS D1.1 structural welding code.
 - EIA/TIA-222 structural standards for steel antenna towers and antenna supporting structures.
- Design parameters:
 - The structural steel antenna mounting frames are designed to provide support for antennas and all hardware and accessories associated with antennas.
- Fabrication and installation requirements:
 - The antenna supports, antennas and mounting hardware shall be constructed plumb, level and true.
 - All structural elements and fasteners shall be galvanized in accordance with ASTM A123 and A153.
 - Welds should be shop made wherever possible, conforming to AISC specification and AWS requirements. All welds are to be of the size and type indicated. Contractor shall employ a licensed welder and shall provide the engineer with their name and a copy of their license prior to commencing any field welding.
 - Contractor shall provide fire watch during all welding operations, brazing and soldering and other work requiring the use of an open flame. Two (2) hand held 30 lb fire extinguishers and adequate water supply shall be maintained on site. Fire watch plan shall be submitted to the client for approval prior to welding.
 - All bolted connections shall be A325 high strength bolts 5/8" diameter minimum size unless otherwise noted. Bolts shall be supplied with flat washers. Bolts shall be tightened in accordance with the AISC snug tight condition, unless otherwise noted.
 - Protective galvanized coatings which were damaged or removed during erection or transportation shall be restored by painting with zinc-rich primer.
 - All threaded rods shall be 1/2" diameter A36 steel unless otherwise noted.
 - Temporary structures for staging and construction shall be capable of withstanding forces specified by the local building code current edition.
- Inspections:
 - All structural steel antenna frames, and connections shall be inspected prior to installation of antennas.
 - All antenna cable trays, supports, channels and clamps shall be inspected prior to installation of antenna cables.
 - Coordinate all inspections with the client's Construction Manager.
 - Contractor to make notifications 72 hours prior to any required inspections.



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1	11-06-12	REVISED PER CLIENT COMMENTS	JLS	KCD	
0	09-26-12	ISSUED FOR CONSTRUCTION	CCR	KCD	
REV.	DATE	REVISION DESCRIPTION	DRAWN BY	CHKD. BY	



Stephen A. Bray
PROFESSIONAL ENGINEER



CT LICENSE: 26657 11/5/12

PROJECT NUMBER: **332.1487**

SITE INFORMATION:
430 BOSTON POST ROAD
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CT23XC552

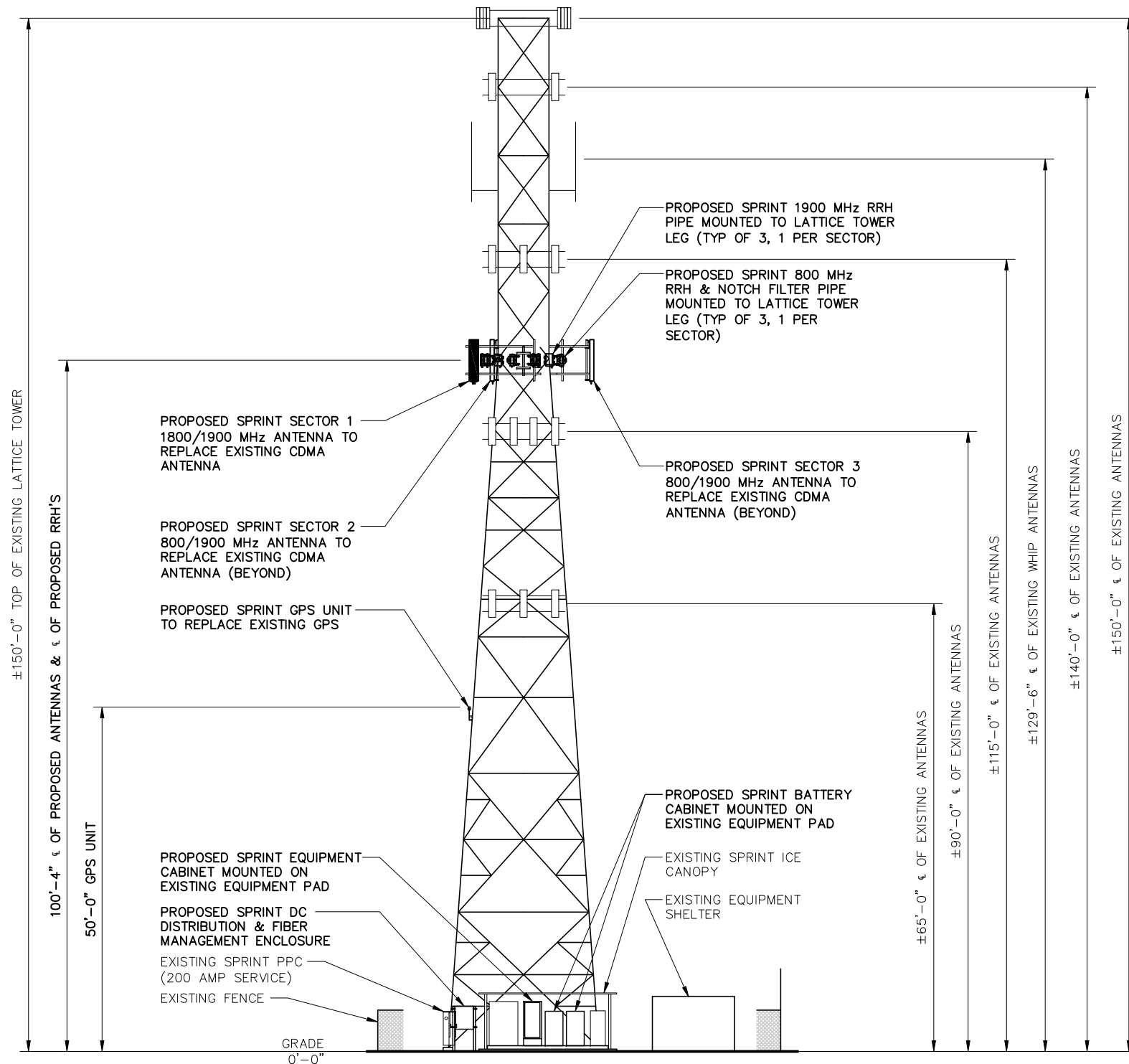
PROJECT TYPE:
NETWORK VISION

DRAWN BY: KAZ	CHECKED BY:	DATE: 03-15-12
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SHEET TITLE:
GENERAL NOTES
2 OF 2

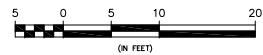
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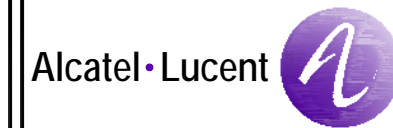
THE STRUCTURAL ENGINEERING CONCERNING THE STRUCTURAL STABILITY OF THE TOWER/POLE AND FOUNDATION IS BEING COMPLETED BY OTHERS. KMB DESIGN GROUP, LLC HAS NOT BEEN REQUESTED TO PERFORM ANY STRUCTURAL ANALYSIS SERVICES TO VERIFY THAT THE TOWER/POLE AND/OR FOUNDATION IS CAPABLE OF SUPPORTING THE PROPOSED EQUIPMENT DEPICTED WITHIN THESE SIGNED AND SEALED DRAWINGS. SIGNED AND SEALED DRAWINGS REVISED TO STATE "ISSUED FOR CONSTRUCTION" SHALL BE PROVIDED TO THE PROFESSIONAL ENGINEERS RESPONSIBLE FOR THE STRUCTURAL ANALYSIS OF THE TOWER/POLE AND FOUNDATION. KMB DESIGN GROUP, LLC SHALL BE NOTIFIED SHOULD THE STRUCTURAL ANALYSIS RESULT IN SOME ELEMENTS NOT BEING STRUCTURALLY CAPABLE OF SUPPORTING THE PROPOSED DESIGN DEPICTED. THE CONTRACTOR SHALL NOT COMMENCE CONSTRUCTION WITHOUT OBTAINING (A) A SIGNED AND SEALED COPY OF THE PLANS "ISSUED FOR CONSTRUCTION"; (B) STRUCTURAL ANALYSIS REPORT STATING THAT THE TOWER/POLE/FOUNDATION IS CAPABLE OF SUPPORTING THE PROPOSED LOADING REFERENCING THE SIGNED AND SEALED PLANS BY KMB DESIGN GROUP, LLC; (C) SPRINT PLATFORM ANALYSIS STATING THAT THE SPRINT PLATFORM IS CAPABLE OF SUPPORTING THE PROPOSED DESIGN AS REFERENCED WITHIN THE SIGNED AND SEALED PLANS BY KMB DESIGN GROUP, LLC.

- NOTES:**
1. FINAL ANTENNA & EQUIPMENT CONFIGURATION SHOWN ON THIS PLAN. SEE EQUIPMENT & ANTENNA PLAN SHEETS FOR EXISTING AND INTERIM CONFIGURATION.
 2. EXISTING TOWER INVENTORY PROVIDED BY OTHERS.
 3. ALL ANTENNA AND CABLING WORK ON THE TOWER SHALL BE IN ACCORDANCE WITH STRUCTURAL REPORT FOR THE TOWER (BY OTHERS).

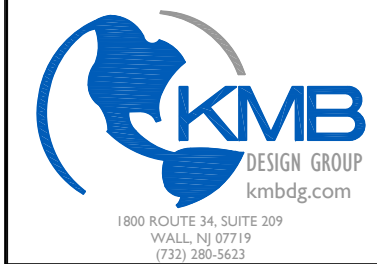


1 **NORTHEAST ELEVATION**

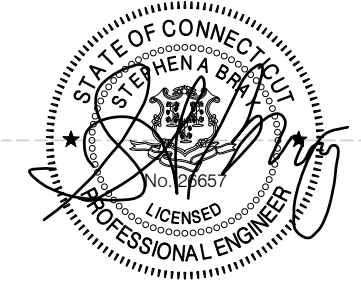
11x17 SCALE: 1" = 20' 24x36 SCALE: 1" = 10'



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1	11-06-12	REVISED PER CLIENT COMMENTS	JLS	KCD
0	09-26-12	ISSUED FOR CONSTRUCTION	CCR	KCD
REV.	DATE	REVISION DESCRIPTION	DRAWN BY	CHKD. BY



Stephen A. Bray
PROFESSIONAL ENGINEER



CT LICENSE: 26657 11/5/12

PROJECT NUMBER: **332.1487**

SITE INFORMATION:
430 BOSTON POST ROAD
MILFORD, CT 06460
NEW HAVEN COUNTY

CT23XC552

PROJECT TYPE:
NETWORK VISION

DRAWN BY: **KAZ** CHECKED BY: DATE: **03-15-12**

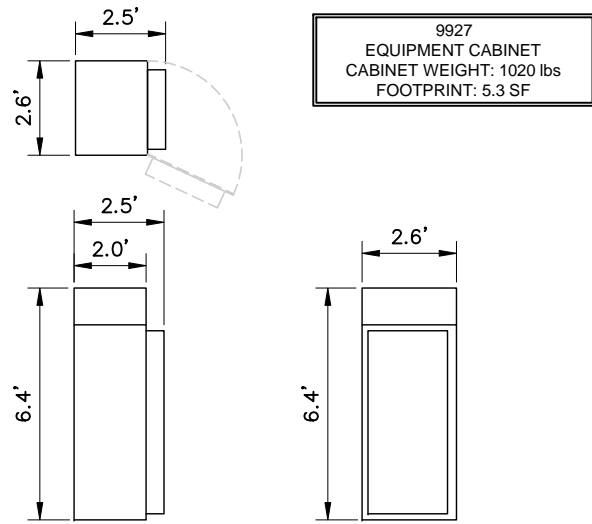
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ELEVATION

SHEET NUMBER: REV.:

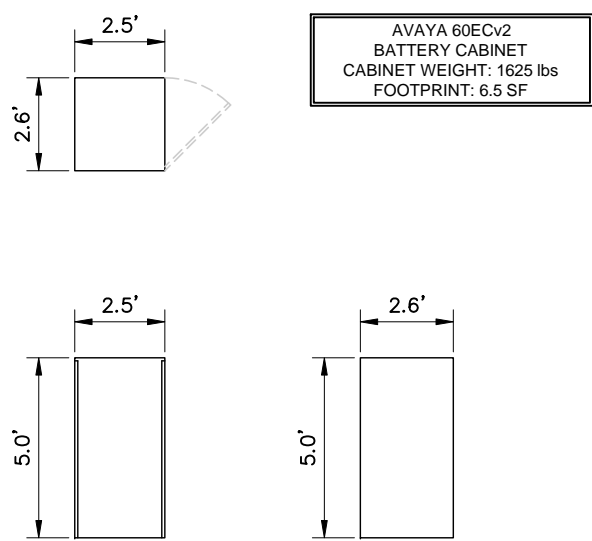
C02A **1**

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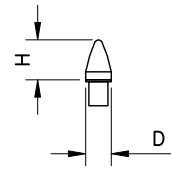
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9927
 EQUIPMENT CABINET
 CABINET WEIGHT: 1020 lbs
 FOOTPRINT: 5.3 SF



AVAYA 60ECv2
 BATTERY CABINET
 CABINET WEIGHT: 1625 lbs
 FOOTPRINT: 6.5 SF



MANUF.: PCTEL
 MODEL #: GPS-TMG-HR-26NCM
 HEIGHT: 5.0"
 DIAMETER: 3.2"
 WEIGHT: 0.6 lbs

DETAIL NOT USED

1 EQUIPMENT CABINET SPECIFICATIONS

2 BATTERY CABINET SPECIFICATION

3 GPS UNIT SPECIFICATIONS

4

11x17 SCALE: 3/16" = 1'-0"

24x36 SCALE: 3/8" = 1'-0"

11x17 SCALE: 3/16" = 1'-0"

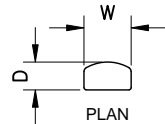
24x36 SCALE: 3/8" = 1'-0"

11x17 SCALE: 1/4" = 1'-0"

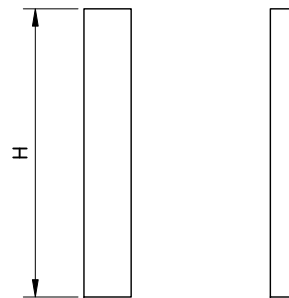
24x36 SCALE: 1/2" = 1'-0"

DETAIL NOT USED

DETAIL NOT USED



MANUF.: RFS
 MODEL: APXVSP18-C-A20
 LENGTH: 72.0"
 WIDTH: 11.8"
 DEPTH: 7.0"
 WEIGHT: 64.5 lbs
 AREA: 5.9 SF



DETAIL NOT USED

5

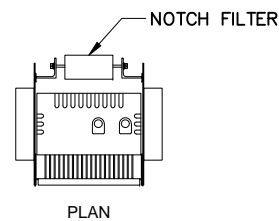
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7 ANTENNA SPECIFICATIONS - 800/1900 MHz

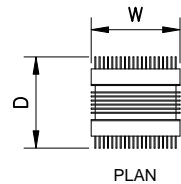
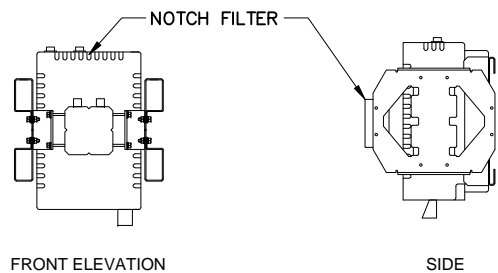
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SCALE: NTS

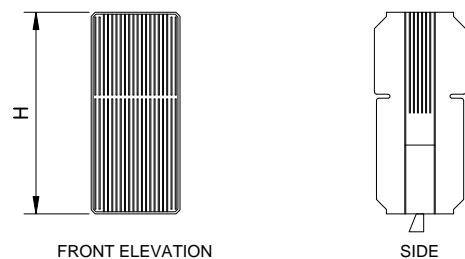
DETAIL NOT USED



MANUF.: ALCATEL LUCENT
 MODEL #: FD-RRH-2x50-800
 HEIGHT: 19"
 WIDTH: 13"
 DEPTH: 12.2"
 WEIGHT: ±64 lbs
 DIMENSIONS AND WEIGHT
 INCLUDES FILTER



MANUF.: ALCATEL LUCENT
 MODEL #: RRH 1900 4x45 65MHz
 HEIGHT: 25.1"
 WIDTH: 11.1"
 DEPTH: 10.69"
 WEIGHT: ±59.5 lbs



9 RRH SPECIFICATIONS - 800 MHz

10 RRH SPECIFICATIONS - 1900 MHz

11

11x17 SCALE: 1/2" = 1'-0"

24x36 SCALE: 1" = 1'-0"

11x17 SCALE: 1/2" = 1'-0"

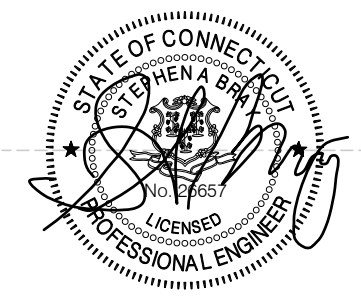
24x36 SCALE: 1" = 1'-0"



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REV.	DATE	REVISION DESCRIPTION	DRAWN BY	CHKD. BY



Stephen A. Bray
PROFESSIONAL ENGINEER



CT LICENSE: 26657 11/5/12

PROJECT NUMBER: **332.1487**

SITE INFORMATION:
430 BOSTON POST ROAD
MILFORD, CT 06460
NEW HAVEN COUNTY

CT23XC552

PROJECT TYPE:
NETWORK VISION

DRAWN BY: KAZ CHECKED BY: DATE: 03-15-12

SHEET TITLE:
EQUIPMENT & ANTENNA SPECIFICATIONS

SHEET NUMBER: REV.:

C03A 1

K:\332_Sprint\332.1487_C03A.dwg, 11/5/2012 4:47:10 PM, jaymiewski

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EXISTING SPRINT SINGLE POLE CDMA ANTENNA

SECTOR 1
40° AZIMUTH

EXISTING SPRINT SINGLE POLE CDMA ANTENNA

EXISTING SPRINT SINGLE POLE CDMA ANTENNA

SECTOR 3
260° AZIMUTH

EXISTING LATTICE TOWER LEG (TYP)

EXISTING SPRINT SINGLE POLE CDMA ANTENNA

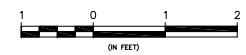
EXISTING SPRINT SINGLE POLE CDMA ANTENNA

SECTOR 2
145° AZIMUTH

EXISTING SPRINT SINGLE POLE CDMA ANTENNA

EXISTING LATTICE TOWER

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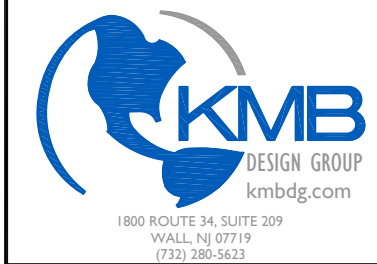


1 EXISTING ANTENNA PLAN @ ±100'-4" AGL (ALL SECTORS)

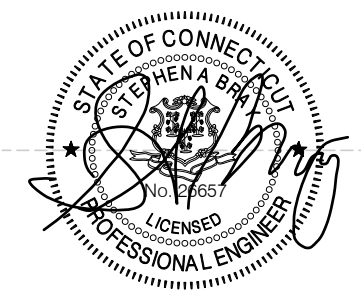
11x17 SCALE: 3/8" = 1'-0" 24x36 SCALE: 3/4" = 1'-0"



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1	11-06-12	REVISED PER CLIENT COMMENTS	JLS	KCD
0	09-26-12	ISSUED FOR CONSTRUCTION	CCR	KCD
REV.	DATE	REVISION DESCRIPTION	DRAWN BY	CHKD. BY



Stephen A. Bray
PROFESSIONAL ENGINEER



CT LICENSE: 26657 11/5/12

PROJECT NUMBER: **332.1487**

SITE INFORMATION:
 430 BOSTON POST ROAD
 MILFORD, CT 06460
 NEW HAVEN COUNTY
CT23XC552

PROJECT TYPE:
NETWORK VISION

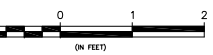
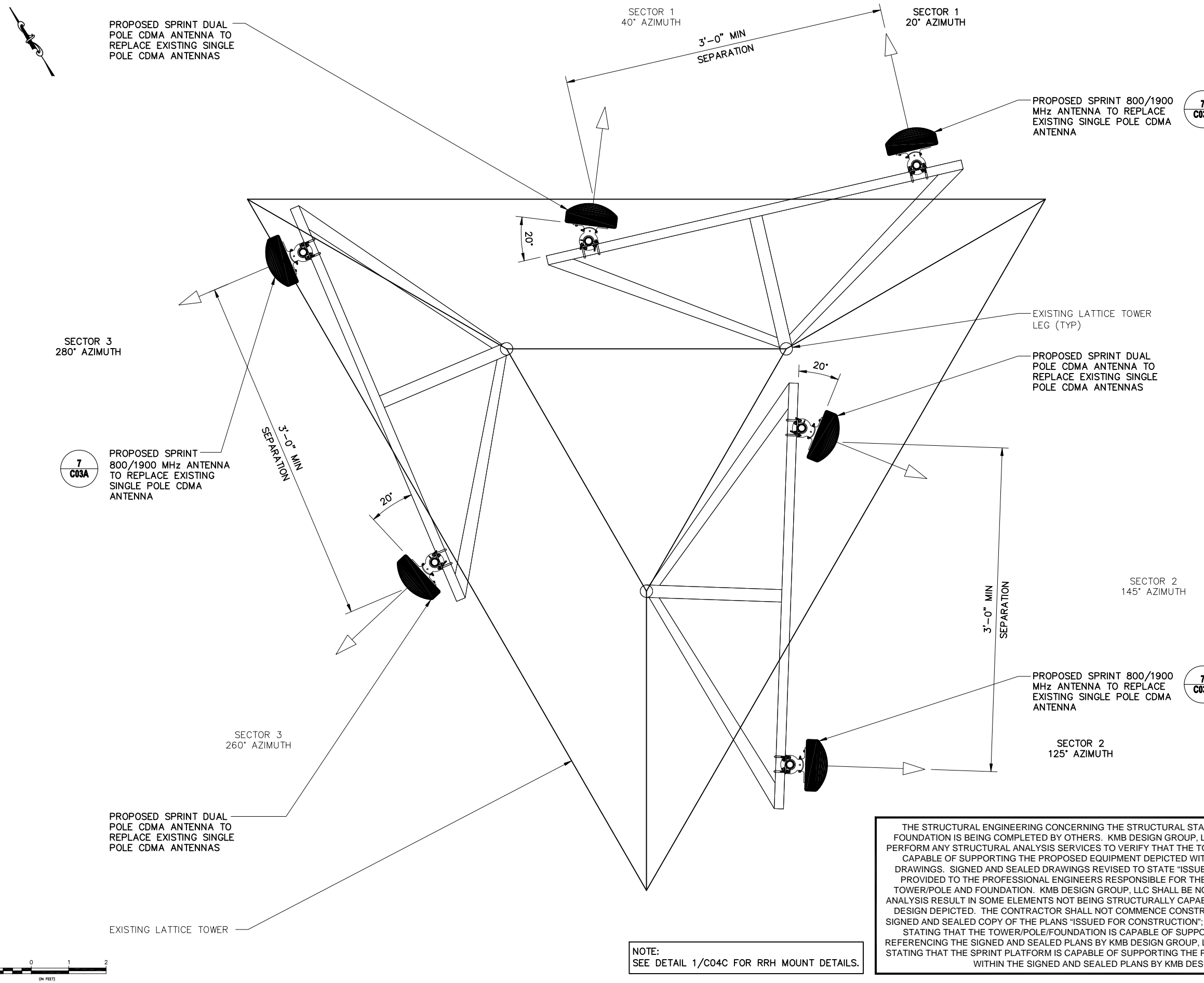
DRAWN BY: **KAZ** CHECKED BY: DATE: **03-15-12**

SHEET TITLE:
EXISTING ANTENNA PLAN (ALL SECTORS)

SHEET NUMBER: **C04** REV.: **1**

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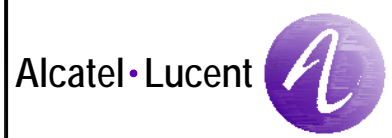
1 INTERIM ANTENNA PLAN @ ±100'-4" AGL (ALL SECTORS)

11x17 SCALE: 3/8" = 1'-0"

24x36 SCALE: 3/4" = 1'-0"

NOTE:
SEE DETAIL 1/C04C FOR RRH MOUNT DETAILS.

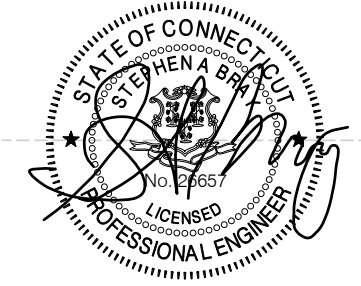
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REV.	DATE	REVISION DESCRIPTION	DRAWN BY	CHKD. BY



Stephen A. Bray
PROFESSIONAL ENGINEER



CT LICENSE: 26657 11/5/12

PROJECT NUMBER: **332.1487**

SITE INFORMATION:
430 BOSTON POST ROAD
MILFORD, CT 06460
NEW HAVEN COUNTY
CT23XC552

PROJECT TYPE:
NETWORK VISION

DRAWN BY: **KAZ** CHECKED BY: DATE: **03-15-12**

SHEET TITLE:
INTERIM ANTENNA PLAN (ALL SECTORS)

SHEET NUMBER: REV.:

C04A 1

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SPRINT DUAL POLE CDMA ANTENNA TO BE REMOVED. EXISTING EMPTY MAST TO REMAIN

SECTOR 1
20° AZIMUTH

EXISTING SPRINT 800/1900 MHz ANTENNA TO REMAIN

SECTOR 3
280° AZIMUTH

EXISTING SPRINT 800/1900 MHz ANTENNA TO REMAIN

EXISTING LATTICE TOWER LEG (TYP)

SPRINT DUAL POLE CDMA ANTENNA TO BE REMOVED. EXISTING EMPTY MAST TO REMAIN

EXISTING SPRINT 800/1900 MHz ANTENNA TO REMAIN

SECTOR 2
125° AZIMUTH

SPRINT DUAL POLE CDMA ANTENNA TO BE REMOVED. EXISTING EMPTY MAST TO REMAIN

EXISTING LATTICE TOWER



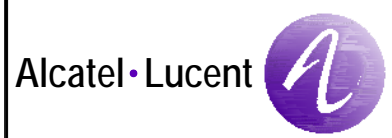
NOTE:
SEE DETAIL 2/C04C FOR RRH MOUNT DETAILS.

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1 FINAL ANTENNA PLAN @ ±100'-4" AGL (ALL SECTORS)

11x17 SCALE: 3/8" = 1'-0"

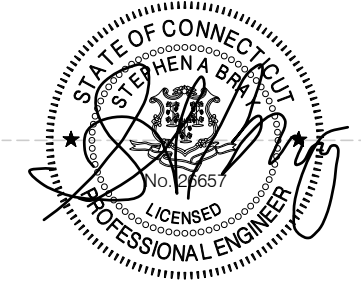
24x36 SCALE: 3/4" = 1'-0"



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1	11-06-12	REVISED PER CLIENT COMMENTS	JLS	KCD
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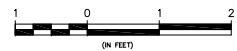
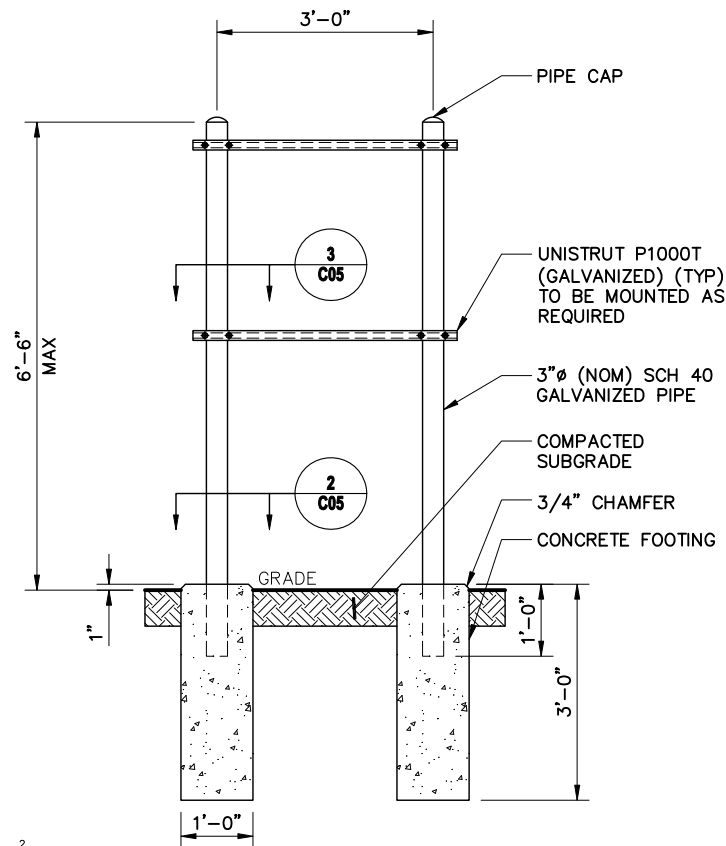
SHEET TITLE:
FINAL ANTENNA PLAN (ALL SECTORS)

SHEET NUMBER: REV.:

C04B 1

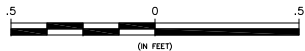
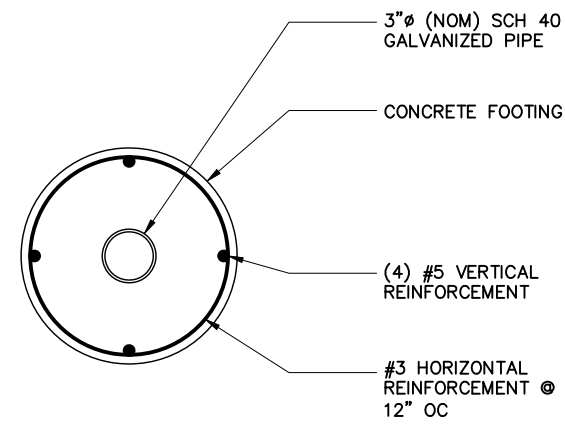
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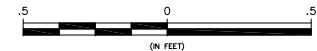
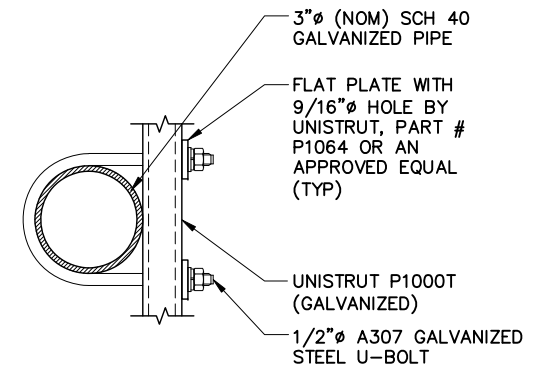
1 UNISTRUT FRAME DETAIL

11x17 SCALE: 3/8" = 1'-0" 24x36 SCALE: 3/4" = 1'-0"



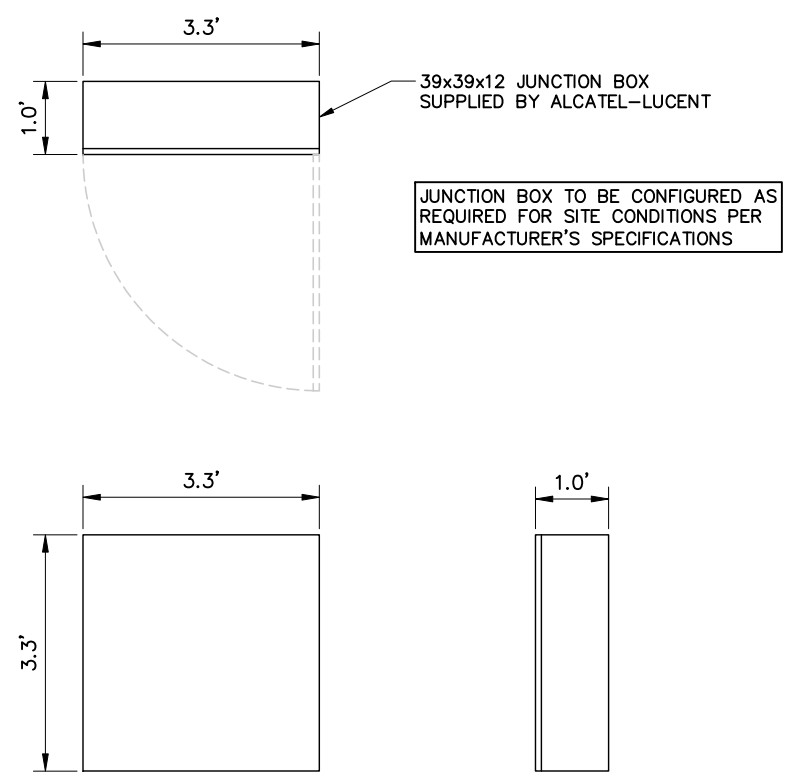
2 CONCRETE PIER DETAIL

11x17 SCALE: 1 1/2" = 1'-0" 24x36 SCALE: 3" = 1'-0"



3 UNISTRUT CONNECTION DETAIL

11x17 SCALE: 1 1/2" = 1'-0" 24x36 SCALE: 3" = 1'-0"



4 DC DISTRIBUTION & FIBER MGMT ENCLOSURE DETAIL

11x17 SCALE: 3/8" = 1'-0" 24x36 SCALE: 3/4" = 1'-0"



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1800 ROUTE 34, SUITE 209
WALL, NJ 07719
(732) 280-5623

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DRAWN BY:	CHECKED BY:	DATE:
KAZ		03-15-12

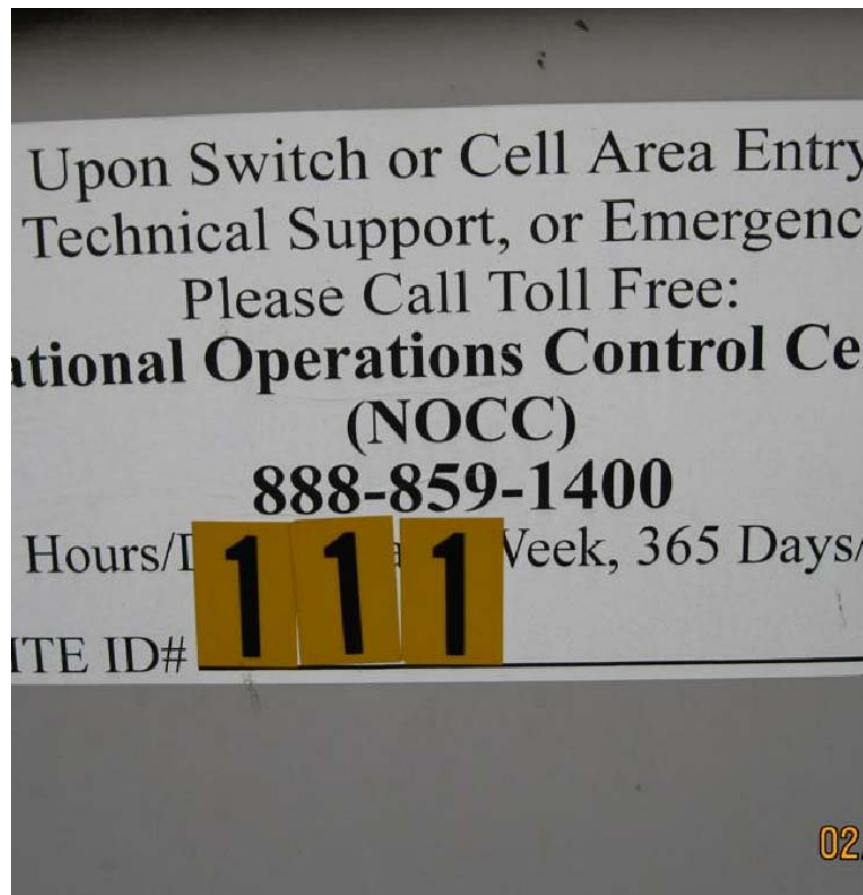
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SITE DETAILS

SHEET NUMBER: REV.:

C05 1

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1 EXISTING SIGNAGE



2 PROPOSED MEET POINT



3 EXISTING EQUIPMENT AREA



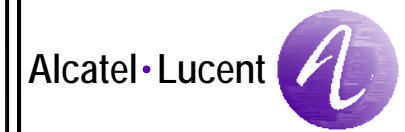
4 EXISTING TELCO CABINET



5 EXISTING POWER SOURCE



6 PROPOSED NID EQUIPMENT LOCATION



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1	11-06-12	REVISED PER CLIENT COMMENTS	JLS	KCD
0	09-26-12	ISSUED FOR CONSTRUCTION	CCR	KCD
REV.	DATE	REVISION DESCRIPTION	DRAWN BY	CHKD. BY



Stephen A. Bray
PROFESSIONAL ENGINEER



CT LICENSE: 26657 11/5/12

PROJECT NUMBER: **332.1487**

SITE INFORMATION:
430 BOSTON POST ROAD
MILFORD, CT 06460
NEW HAVEN COUNTY

CT23XC552

PROJECT TYPE:
NETWORK VISION

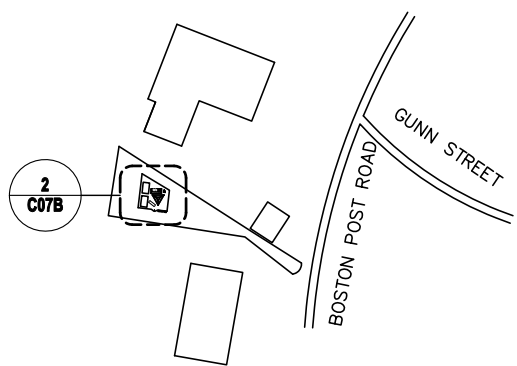
DRAWN BY: **KAZ** CHECKED BY: DATE: **03-15-12**

SHEET TITLE:
**AAV DRAWINGS
SITE PHOTOS**

SHEET NUMBER: REV.:

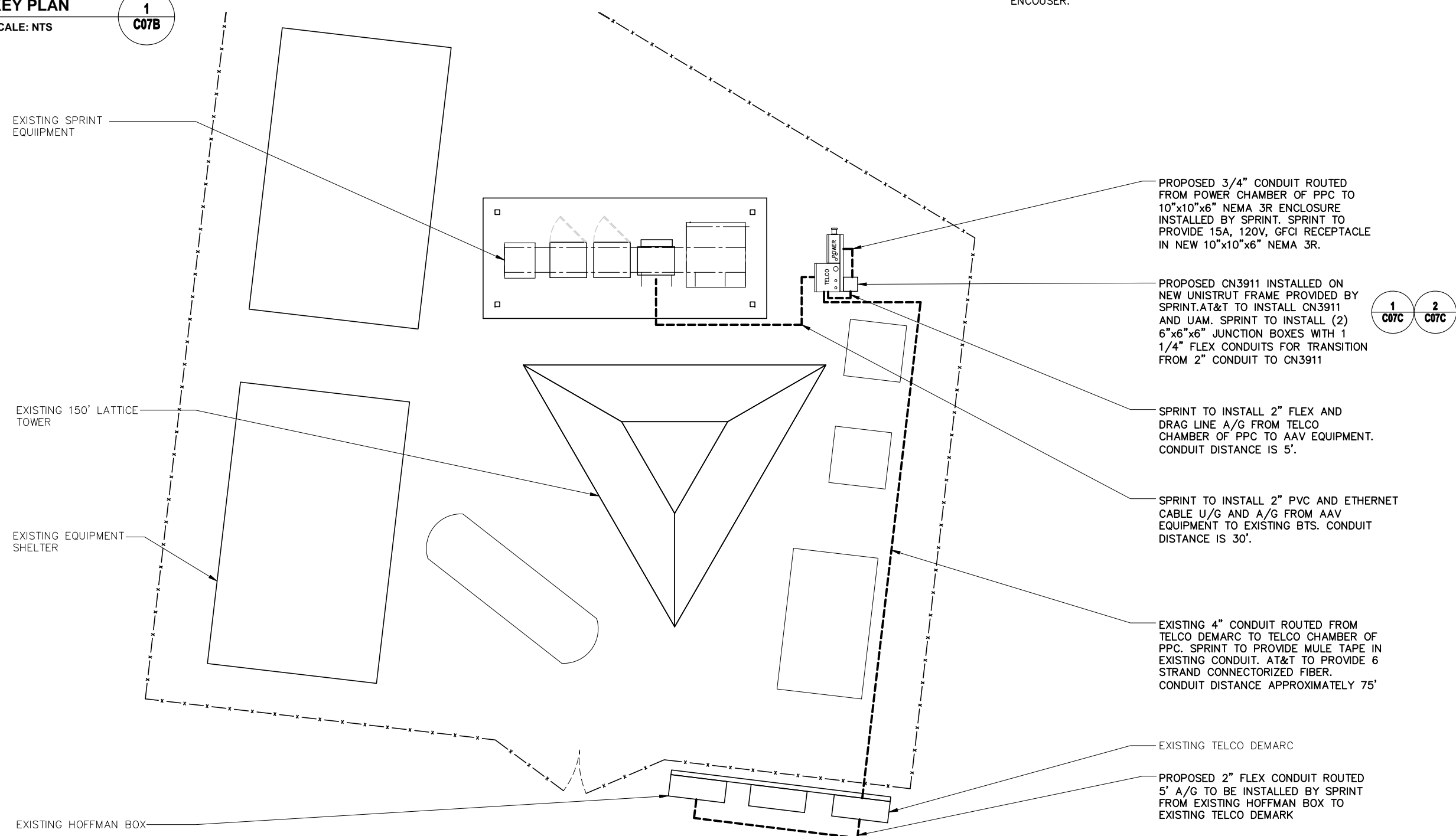
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KEY PLAN
SCALE: NTS

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- AAV SCOPE OF WORK NOTES:**
1. FIBER RUN: PROPOSED AT&T 6 STRAND FIBER AND MULE TAPE TO ORIGINATE FROM EXISTING TELCO DEMARC AND RUN WITHIN EXISTING 4"Ø CONDUIT TO TELCO CHAMBER OF PPC. (APPROX ±75')
 2. INSTALL CIENA 3911 ON NEW UNISTRUT FRAME. UNISTRUT FRAME TO BE PROVIDED AND INSTALLED BY SPRINT.
 3. PROPOSED ETHERNET CABLE AND DRAG LINE TO BE ROUTED FROM TELCO CHAMBER TO CIENA 3911 VIA PROPOSED 2" FLEX A/G CONDUIT INSTALLED BY SPRINT. (APPROX. DISTANCE IS 5')
 4. PROPOSED ETHERNET CABLE ROUTED FROM TELCO CHAMBER TO EXISTING SPRINT BTS CABINET VIA PROPOSED 2" PVC CONDUIT (APPROX ±30' TOTAL)
 5. POWER RUN: PROPOSED 3/4"Ø CONDUIT ROUTED FROM THE POWER CHAMBER OF PPC TO PROPOSED 10"x10"x6" NEMA 3R ENCLOSURE.
 6. INSTALL NEW 15A, 120V, GFCI ON NEW UNISTRUT FRAME IN 10"x10"x6" ENCOUSER.

PROPOSED 3/4" CONDUIT ROUTED FROM POWER CHAMBER OF PPC TO 10"x10"x6" NEMA 3R ENCLOSURE INSTALLED BY SPRINT. SPRINT TO PROVIDE 15A, 120V, GFCI RECEPTACLE IN NEW 10"x10"x6" NEMA 3R.

PROPOSED CN3911 INSTALLED ON NEW UNISTRUT FRAME PROVIDED BY SPRINT. AT&T TO INSTALL CN3911 AND UAM. SPRINT TO INSTALL (2) 6"x6"x6" JUNCTION BOXES WITH 1 1/4" FLEX CONDUITS FOR TRANSITION FROM 2" CONDUIT TO CN3911

SPRINT TO INSTALL 2" FLEX AND DRAG LINE A/G FROM TELCO CHAMBER OF PPC TO AAV EQUIPMENT. CONDUIT DISTANCE IS 5'.

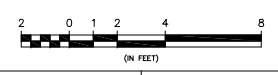
SPRINT TO INSTALL 2" PVC AND ETHERNET CABLE U/G AND A/G FROM AAV EQUIPMENT TO EXISTING BTS. CONDUIT DISTANCE IS 30'.

EXISTING 4" CONDUIT ROUTED FROM TELCO DEMARC TO TELCO CHAMBER OF PPC. SPRINT TO PROVIDE MULE TAPE IN EXISTING CONDUIT. AT&T TO PROVIDE 6 STRAND CONNECTORIZED FIBER. CONDUIT DISTANCE APPROXIMATELY 75'

EXISTING TELCO DEMARC

PROPOSED 2" FLEX CONDUIT ROUTED 5' A/G TO BE INSTALLED BY SPRINT FROM EXISTING HOFFMAN BOX TO EXISTING TELCO DEMARC

1 C07C 2 C07C



2 EQUIPMENT PLAN

11x17 SCALE: 1/8"=1'-0" 24x36 SCALE: 1/4"=1'-0"



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1	11-06-12	REVISED PER CLIENT COMMENTS	JLS	KCD	
0	09-26-12	ISSUED FOR CONSTRUCTION	CCR	KCD	
REV.	DATE	REVISION DESCRIPTION	DRAWN BY	CHKD. BY	



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NEW HAVEN COUNTY
CT23XC552

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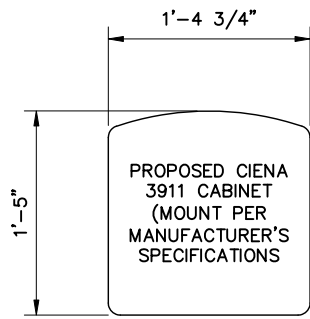
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**AAV DRAWINGS
EQUIPMENT PLAN**

SHEET NUMBER: REV.:

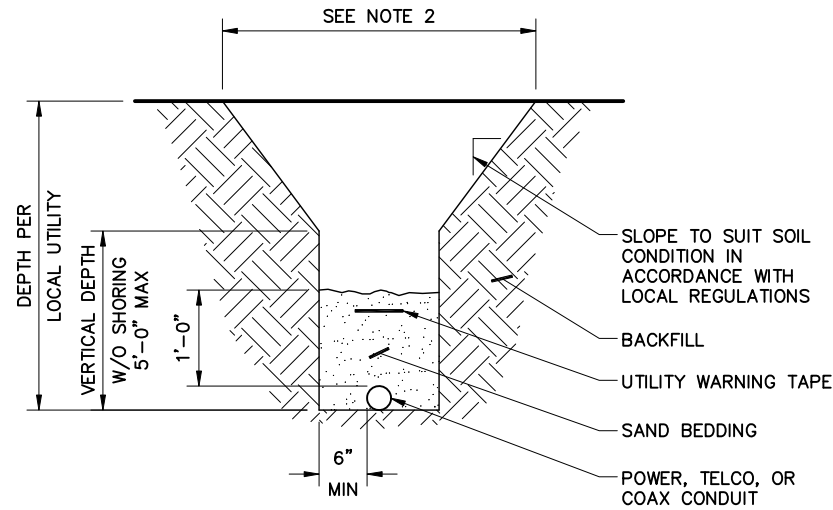
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1 TYP CIENA 3911 SPECIFICATIONS

SCALE: NTS

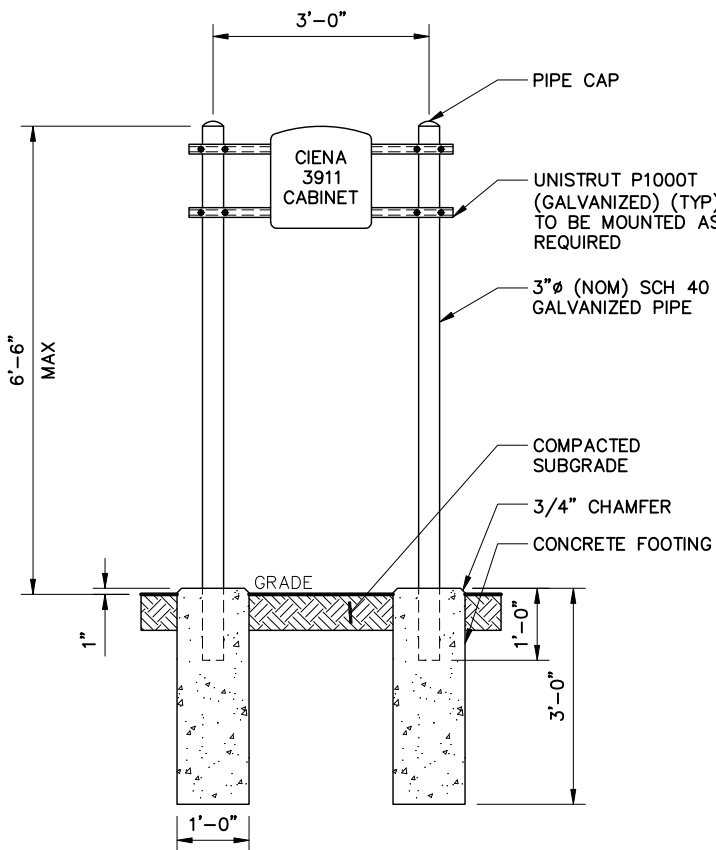


3 TYP TRENCH DETAIL

SCALE: NTS

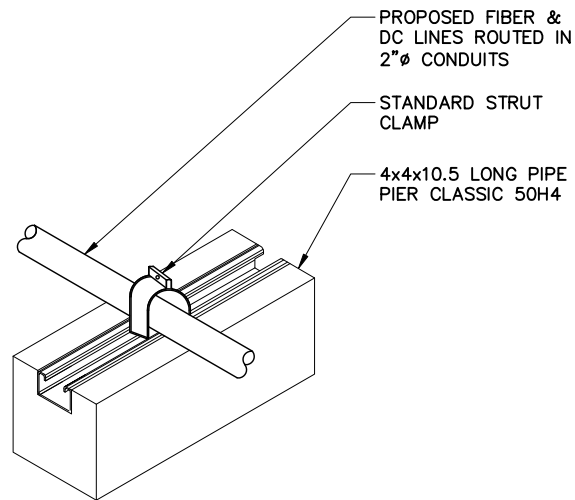
NOTES:

1. DETAIL SHOWN IS FOR ONE CONDUIT. MULTIPLE CONDUITS CAN BE PLACED IN THE SAME TRENCH AS LONG AS A MINIMUM SEPARATION PER THE LOCAL UTILITY COMPANIES IS MAINTAINED. IN ALL CASES THE MINIMUM CENTER TO CENTER SPACING BETWEEN CONDUITS IS 1'-0" (NOT REQUIRED FOR COAX).
2. CONTRACTOR SHALL RESTORE THE TRENCH TO ITS ORIGINAL CONDITIONS BY EITHER SEEDING OR SODDING GRASS AREAS, OR REPLACING ASPHALT OR CONCRETE AREAS TO ITS ORIGINAL CROSS SECTION.



2 UNISTRUT FRAME DETAIL

11x17 SCALE: 3/8" = 1'-0" 24x36 SCALE: 3/4" = 1'-0"



4 TYP CONDUIT ALONG CONCRETE DETAIL

SCALE: NTS



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DETAILS**

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