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Steven L. Levine Real Estate Consultant

HAND DELIVERED

April 14, 2014

Attorney Melanie Bachman Acting Executive Director Connecticut Siting Council 10 Franklin Square New Britain, Connecticut 06051

Re: New Cingular Wireless PCS, LLC notice of intent to modify an existing telecommunications facility located at 50 Fairchild Road, Middletown (owner, SBA)

Dear Ms. Bachman:

In order to accommodate technological changes, implement Uniform Mobile Telecommunications System ("UMTS") and/or Long Term Evolution ("LTE") capabilities, and enhance system performance in the State of Connecticut, New Cingular Wireless PCS, LLC ("AT&T") plans to modify the equipment configurations at many of its existing cell sites. Please accept this letter and attachments as notification, pursuant to R.C.S.A. Section 16-50j-73, of construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2). In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter and attachments is being sent to the chief elected official of the municipality in which the affected cell site is located.

UMTS technology offers services to mobile computer and phone users anywhere in the world. Based on the Global System for Mobile ("GSM") communication standard, UMTS is the planned worldwide standard for mobile users. UMTS, fully implemented, gives computer and phone users high-speed access to the Internet as they travel. They have the same capabilities even when they roam, through both terrestrial wireless and satellite transmissions.

LTE is a high-performance air interface for cellular mobile communications. It is designed to increase the capacity and speed of mobile telephone networks.

Attached is a summary of the planned modifications, including power density calculations reflecting the change in AT&T's operations at the site. Also included is documentation of the structural sufficiency of the tower to accommodate the revised antenna configuration.

The changes to the facility do not constitute modifications as defined in Connecticut General

Statutes ("C.G.S.") Section 16-50i(d) because the general physical characteristics of the facility will not be significantly changed or altered. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in R.C.S.A. Section 16-50j-72(b)(2).

1. The height of the overall structure will be unaffected.

2. The proposed changes will not extend the site boundaries. There will be no effect on the site compound other than some enlarged equipment pads as may be noted in the attachments.

3. The proposed changes will not increase the noise level at the existing facility by six decibels or more.

4. Radio frequency power density may increase due to use of one or more GSM channel for UMTS transmissions. Moreover, LTE will utilize additional radio frequencies newly-licensed by the FCC for cellular mobile communications. However, the changes will not increase the calculated "worst case" power density for the combined operations at the site to a level at or above the applicable standard for uncontrolled environments as calculated for a mixed frequency site.

For the foregoing reasons, AT&T respectfully submits that the proposed changes at the referenced site constitute exempt modifications under R.C.S.A. Section 16-50j-72(b)(2).

Please feel free to call me at (860) 830-0380 with questions concerning this matter. Thank you for your consideration.

Sincerely,

Steven L. Levine Real Estate Consultant

cc: Mayor Daniel T. Drew, City of Middletown

Attachments

NEW CINGULAR WIRELESS PCS, LLC Equipment Modification

50 Fairchild Road, Middletown CSC Approvals: Dockets 316 and 316A; Petition 988 AT&T Site CT2547

Tower Owner/Manager:	SBA
Equipment Configuration:	Monopole (Proposed conversion to a Guyed Monopole)
Current and/or approved:	Low profile platform @ 130 ft. Nine PowerWave P65-16-XLH-RR antennas @ 130 ft c.l. Six PowerWave TMA's @ 130 ft Six remote radio heads @130 ft Twelve runs 1 5/8 inch coax Equipment shelter
Planned Modifications:	 Remove existing platform and all antennas, TMA's, and associated equipment from 130 ft level. Remove six 1-5/8 inch coax lines Six to remain. Install recommended structural modifications, including guy lines. Install one Commscope MTC3607R antenna platform with handrails @ 130 ft level. Reinstall three PowerWave P65-16-XLH-RR antennas @ 130 ft c.l. Install nine CCI HPA-65R-BUU-H6 antennas @ 130 ft c.l. Install three CCI TMA's @ 130 ft. Install 18 remote radio heads and six associated A2 modules @ 130 ft. Install three Raycap DC6-48-60-18-8F surge arrestors @ 130 ft.

Power Density:

Calculations for AT&T's current operations at the site indicate a radio frequency electromagnetic radiation power density, measured at the monopole base, of approximately 56.2 % of the standard adopted by the FCC. As depicted in the second table below, the total radio frequency

electromagnetic radiation power density for AT&T's planned operations would be approximately 56.1 % of the standard.

Company	Centerline Ht (feet)	Frequency (MHz)	Number of Channels	Power Per Channel (Watts)	Power Density (mW/cm ²)	Standard Limits (mW/cm ²)	Percent of Limit
Other Users *							47.05
AT&TITE *	130	740	1	500	0.0106	0.4933	2.16
AT&T GSM *	130	1900 Band	1	427	0.0091	1.0000	0.91
AT&T GSM *	130	880 - 894	3	296	0.0189	0.5867	3.22
AT&TUMTS *	130	1900 Band	1	500	0.0106	1.0000	1.06
AT&TUMTS *	130	880 - 894	1	500	0.0106	0.5867	1.81
Total							56.2%

Existing

* Per CSC records.

Proposed

Company	Centerline Ht (feet)	Frequency (MHz)	Number of Channels	Power Per Channel (Watts)	Power Density (mW/cm ²)	Standard Limits (mW/cm ²)	Percent of Limit
Other Users *							47.05
AT&TITE	130	700 Band	1	500	0.0106	0.4667	2.28
AT&TITE	130	1900 Band	1	500	0.0106	1.0000	1.06
AT&TITE	130	2300 Band	1	500	0.0106	1.0000	1.06
ATETIME	130	880 - 894	2	500	0.0213	0.5867	3.63
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Total							00.170

* Per CSC records.

Structural information:

The attached structural analysis demonstrates that the tower and foundation will have adequate structural capacity to accommodate the proposed equipment modifications upon completion of recommended structural modifications described in attachments hereto. (FDH Engineering, 4-4-14)

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FDH Engineering, Inc., 6521 Meridien Drive Raleigh, NC 27616, Ph. 919.755.1012

Structural Analysis for SBA Network Services, Inc.

130' Monopole Tower

SBA Site Name: Middletown 2 SBA Site ID: CT13064-A-05 AT&T Site Name: Middletown AT&T Site ID: CT2547

FDH Project Number 1423HD1400

	Analysis Results	
Tower Components	97.0%	Sufficient
Foundation	57.9%	Sufficient

Prepared By:

David Zambrano, El Project Engineer

FDH Engineering, Inc. 6521 Meridien Drive Raleigh, NC 27616 (919) 755-1012 info@fdh-inc.com Reviewed By:

Bradley R. Newman, PE Senior Project Engineer CT PE License No. 29630



April 4, 2014

Prepared pursuant to ANSI/TIA-222-G Structural Standard for Antenna Supporting Structures and Antennas and 2005 Connecticut State Building Code

Document No. ENG-RPT-501S

Revision Date: 06/17/11

EXECUTIVE SUMMARY

At the request of SBA Network Services, Inc., FDH Engineering, Inc. performed a structural analysis of the monopole located in Middletown, CT to determine whether the tower is structurally adequate to support both the existing and proposed loads pursuant to the *Structural Standard for Antenna Supporting Structures and Antennas, ANSI/TIA-222-G* and 2005 Connecticut State Building Code (CSBC). Information pertaining to the existing/proposed antenna loading, current tower geometry, geotechnical data, and member sizes was obtained from:

- Radian Communication Services (File No. 060-3494) original design drawings dated December 15, 2006
- Gemini Geotechnical Associates, Inc. (Site No. 999-0049) Geotechnical Engineering Report dated November 30, 2006
- □ FDH Engineering, Inc.(Project No. 11-01248E S1) Modification & 10' Extension Drawings for a 120' Monopole dated September 21, 2011
- □ FDH Engineering, Inc. (Project No. 11-01248E S1) Post Construction Inspection Report dated December 14, 2011
- FDH Engineering, Inc. (Job No. 12-08192E S2) Modification Drawings for a 130' Monopole dated November 14, 2012
- FDH Engineering, Inc. (Project No. 12-11103T C1) TIA Inspection Report dated February 18, 2013
- FDH Engineering, Inc. (Project No. 1423HD1400) Modification Drawings for a 130' Monopole dated April 4, 2014
- SBA Network Services, Inc.

The basic design wind speed per the ANSI/TIA-222-G standard and 2005 CSBC is 110 mph without ice and 50 mph with 3/4" radial ice. Ice is considered to increase in thickness with height. Furthermore, this structure was analyzed as a Class II structure with Exposure Category C, Topographical Factor of 1, and Spectral Response Accelerations of $S_s = 0.236$ and $S_1 = 0.062$.

Note: Per Section 2.7.3 of the *ANSI/TIA-222-G* standard, the seismic/earthquake loading effects can be ignored if spectral response acceleration at short periods (S_s) is less than or equal to 1.00. The tower's location mandates a design S_s of less than 1.00, thus seismic loading was not considered as part of the analysis of this structure.

Conclusions

With the existing and proposed antennas from AT&T in place at 130 ft, the tower meets the requirements of the *ANSI/TIA-222-G* standard and 2005 *CSBC* provided the **Recommendations** listed below are satisfied. Furthermore, provided the foundation was designed and constructed to support the original design reactions (see Radian File No. 060-3494) and the proposed modifications have been correctly installed (see FDH Engineering, Inc. Project No. 1423HD1400), the foundations should have the necessary capacity to support both the proposed and existing loading. For a more detailed description of the analysis of the tower, see the **Results** section of this report.

Our structural analysis has been performed assuming all information provided to FDH Engineering, Inc. is accurate (i.e., the steel data, tower layout, existing antenna loading, and proposed antenna loading) and that the tower has been properly erected and maintained per the original design drawings.

Recommendations

To ensure the requirements of the ANSI/TIA-222-G standard and 2005 CSBC are met with the existing and proposed loading in place, we have the following recommendations:

- 1. The proposed feed lines must be installed inside the monopole shaft.
- 2. The proposed TMAs should be installed directly behind the existing and/or proposed antennas
- 3. RRU/RRH Stipulation: The equipment may be installed in any arrangement determined by the client.
- 4. Modifications outlined in FDH Engineering, Inc. (Project No. 1423HD1400) Modification Drawings for a 130' Monopole dated April 4, 2014 must be correctly installed in order for this analysis to be valid.

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APPURTENANCE LISTING

The proposed and existing antennas with their corresponding cables/coax lines are shown in **Table 1**. If the actual layout determined in the field deviates from the layout, FDH Engineering, Inc. should be contacted to perform a revised analysis.

Table 1 - Appurtenance Loading

Existing Loading:

Antenna Elevation (ft)	Description	Feed Lines	Carrier	Mount Elevation (ft)	Mount Type
130	(9) Powerwave P65-16-XLH-RR (6) Powerwave TT19-08BP111-001 TMAs (6) Powerwave 7010 RETs (6) Ericsson RRUS-11 RRUs	(18) 1-5/8"	AT&T	129	(1) Low-Profile Platform
120	(6) RFS APXV86-906515	(12) 1-5/8"	Nextel	120	(6) Pipe Mounts
111.5	(3) Andrew CBC721-DF]]	
110.5	(3) Antel BXA-70063-6CF-2 (3) Antel BXD-63606380CF	(12) 1-5/8"	Verizon	110	(3) T-Arms
108.5	(3) Andrew CBC721-DF				
99.5	(3) APXV18-206517S	(6) 1-5/8"	Pocket	99.5	(3) Pipe Mounts
94	(1) 1'4" x 6.5" x 6" Surge Protector			94	Direct Mount
91	(3) Kathrein 840 10054 (3) Samsung RASSPI-2213-RRH	(3) 5/16 (2) 1/2	Clearwire	90 E	(2) T Arme
90.8	(1) Andrew VHLP2-18-DW1	(3) 5/8		09.5	(3) 1-Allis
90.7	(1) VHLP800-11-DW1	(3) //4			

Proposed Loading:

Antenna Elevation (ft)	Description	Feed Lines	Carrier	Mount Elevation (ft)	Mount Type
130	 (3) Powerwave P65-16-XLH-RR (9) CCI HPA-65R-BUU-H6 (3) CCI DTMABP7819VG12A-BP TMAs (6) Ericsson RRUS-11 RRUs (6) Ericsson RRUS-12 RRUs (3) Ericsson RRUS-32 RRUs (3) Ericsson RRUS-22 RRUs (6) Ericsson A2 Modules (3) Raycap DC6-48-60-18-8F Surge Arrestors 	(6) 1-5/8" (1) 1.496" Fiber (6) 0.645" Power	AT&T	129	(1) 12.5' Platform w/ Handrails [Commscope P/N MTC3607R]

RESULTS

The following yield strength of steel for individual members was used for analysis:

Member Type	Yield Strength
Tower Shaft Sections	65 ksi
Upper Flange Plate	50 ksi
Guy Lugs	65 ksi
Flange Bolts	F _u = 150 ksi
Lower Flange Plate	36 ksi
Inner Anchor Bolts	F _u = 125 ksi
Outer Anchor Rods	F _u = 100 ksi
Base Plate	50 ksi

Table 2 - Material Strength

Table 3 displays the summary of the ratio (as a percentage) of force in the member to their capacities. Values greater than 100% indicate locations where the maximum force in the member exceeds its capacity. *Note: Capacities up to 100% are considered acceptable.* **Table 4** displays the maximum foundation reactions.

If the assumptions outlined in this report differ from actual field conditions, FDH Engineering, Inc. should be contacted to perform a revised analysis. Furthermore, as no information pertaining to the allowable twist and sway requirements for the existing or proposed appurtenances was provided, deflection and rotation were not taken into consideration when performing this analysis.

See the Appendix for detailed modeling information.

Table 3 - Summary of Working Percentage of Structural Components

Section No.	Elevation ft	Component Type	Size	% Capacity	Pass Fail
	130 - 120	Pole	TP18x18x0.25	51.9	Pass
		Upper Flange Plate	PL 31.5"ø x 1" thick	95.1	Pass
	120	Flange Bolts	(6) 1" ø on a 28.25"ø BC	91.3	Pass
		Lower Flange Plate	PL 31.5"ø x 1" thick	95.1	Pass
	120 - 87.42	Pole	TP29.89x21.74x0.1875	75.7	Pass
L2 -	107.25	Guy Wire Connection	(3) 42" Lugs, (6) 7/8" Wires	97.0	Pass
L3	87.42 - 43.3367	Pole	TP36.31x28.5352x0.25	42.1	Pass
	43.3367 - 8.0833	Pole	TP42.5x35.0541x0.312	31.0	Pass
	8.0833 - 0	Modified Pole	TP42.5x35.0541x0.3125 w/ Flat Plate Modifications	22.3	Pass
L4		Inner Anchor Bolts	(14) 1.5"ø on a 47.25"ø BC	11.3	Pass
,	0	Outer Anchor Rods	(4) 2.25"ø on a 56.75"ø BC	17.9	Pass
		Base Plate	PL 51.75"ø x 1.5" thick	10.2	Pass

Table 4 - Maximum Base Reactions

Base Reactions	22	Current Analysis* Original Design (ANSI/TIA-222-G) (ANSI/TIA-222-G			Original Design (ANSI/TIA-222-G)		
Dase Reactions	Horizontal	Vertical	Moment	Horizontai	Vertical	Moment	
Base	6	173 k	623 k-ft	20 k	39 k	1,864 k-ft	
Anchors	36	84 k			-		

* Foundations determined to be adequate per independent analysis.

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Revision Date: 06/17/11

GENERAL COMMENTS

This engineering analysis is based upon the theoretical capacity of the structure. It is not a condition assessment of the tower and its foundation. It is the responsibility of SBA Network Services, Inc. to verify that the tower modeled and analyzed is the correct structure (with accurate antenna loading information) modeled. If there are substantial modifications to be made or the assumptions made in this analysis are not accurate, FDH Engineering, Inc. should be notified immediately to perform a revised analysis.

LIMITATIONS

All opinions and conclusions are considered accurate to a reasonable degree of engineering certainty based upon the evidence available at the time of this report. All opinions and conclusions are subject to revision based upon receipt of new or additional/updated information. All services are provided exercising a level of care and diligence equivalent to the standard and care of our profession. No other warranty or guarantee, expressed or implied, is offered. Our services are confidential in nature and we will not release this report to any other party without the client's consent. The use of this engineering work is limited to the express purpose for which it was commissioned and it may not be reused, copied, or distributed for any other purpose without the written consent of FDH Engineering, Inc.







DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
Lightning Rod	130	(2) APXV86-906515 W/ Mount Pipe	120
P65-16-XLH-RR W/ Mount Pipe	129	(2) APXV86-906515 W/ Mount Pipe	120
P65-16-XLH-RR W/ Mount Pipe	129	(2) APXV86-906515 W/ Mount Pipe	120
P65-16-XLH-RR W/ Mount Pipe	129	BXA-70063-6CF-2 w/ Mount Pipe	110
(3) HPA-65R-BUU-H6 w/ Mount Pipe	129	BXA-70063-6CF-2 w/ Mount Pipe	110
(3) HPA-65R-BUU-H6 w/ Mount Pipe	129	BXA-70063-6CF-2 w/ Mount Pipe	110
(3) HPA-65R-BUU-H6 w/ Mount Pipe	129	BXD-63606380CF W/ Mount Pipe	110
DTMABP7819VG12A TMA	129	BXD-63606380CF W/ Mount Pipe	110
DIMABP7819VG12ATMA	129	BXD-63606380CF W/ Mount Pipe	110
DTMABP7819VG12A TMA	129	CBC721-DF	110
(2) RRUS-11	129	CBC721-DF	110
(2) RRUS-11	129	CBC721-DF	110
(2) RRUS-11	129	CBC721-DF	110
(2) RRUS-12	129	CBC721-DF	110
(2) RRUS-12	129	CBC721-DF	110
(2) RRUS-12	129	(3) T-Arms	110
RRUS-32	129	APXV18-206517S W/Mount Pipe	99.5
RRUS-32	129	APXV18-206517S W/Mount Pipe	99.5
RRUS-32	129	APXV18-206517S W/Mount Pipe	99.5
RRUS-E2	129	1'4" x 6.5" x 6" Surge Protector	94
RRUS-E2	129	VHLP2-18-DW1	90
RRUS-E2	129	VHLP800-11-DW1	90
RRUS A2 MODULE	129	RASSPI-2213-RRH	89.5
RRUS A2 MODULE	129	RASSPI-2213-RRH	89.5
RRUS A2 MODULE	129	RASSPI-2213-RRH	89.5
DC6-48-60-18-8F	129	(3) T-Arms	89.5
DC6-48-60-18-8F	129	840 10054 w/ Mount Pipe	89.5
DC6-48-60-18-8F	129	840 10054 w/ Mount Pipe	89.5
(1) Platform w/ Handrails MNT [Commscope P/N MTC3607]	129	840 10054 w/ Mount Pipe	89.5
Kicker Support [Commscope P/N	129		

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A500-50	50 ksi	62 ksi	A572-65	65 ksi	80 ksi

TOWER DESIGN NOTES

TOWER DESIGN NOTES
1. Tower is located in Middlesex County, Connecticut.
2. Tower designed for Exposure C to the TIA-222-G Standard.
3. Tower designed for a 110 mph basic wind in accordance with the TIA-222-G Standard.
4. Tower is also designed for a 50 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Structure Class II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING. 97%

R=45.00 ft

36 K

84 K

92 K

ALL REACTIONS ARE FACTORED

-	FDH Engineering, Inc.	^{Job:} Middletown - CT13064-	A-05	
EDH	6521 Meridien Drive, Suite 107	Project: 1423HD1400		
	Raleigh NC 27616	Client: SBA Network Services, Inc.	Drawn by: David Zambranc	App'd:
Tower Analysis	Phone: (919) 755-1012	Code: TIA-222-G	Date: 04/04/14	Scale: NTS
FAX: (919) 755-1031	Path:	Continuent berteinen 2 (Tablication	Dwg No. E-1	



Centek Engineering, Inc. 3-2 North Branford Road Branford, Connecticut 06405 Phone: (203) 488-0580 Fax: (203) 488-8587

Steven L. Levine Real Estate Consultant

April 23, 2014

Mayor Daniel T. Drew City of Middletown Municipal Bldg., 245 DeKoven Drive Middletown, CT 06457

Re: New Cingular Wireless PCS, LLC notice of intent to modify an existing telecommunications facility located at 50 Fairchild Road, Middletown (Owner, SBA)

Dear Mayor Drew:

In order to accommodate technological changes, implement Uniform Mobile Telecommunications System ("UMTS") and Long Term Evolution ("LTE") capabilities, and enhance system performance in the State of Connecticut, New Cingular Wireless PCS, LLC ("AT&T") will be changing its equipment configuration at certain cell sites.

As required by Regulations of Connecticut State Agencies ("R.C.S.A.") Section 16-50j-73, the Connecticut Siting Council has been notified of the changes and will review AT&T's proposal. Please accept this letter as notification under Section 16-50j-73 of construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2).

The enclosed Notice fully sets forth the AT&T proposal. However, if you have any questions or require any further information on the plans for the site or the Siting Council's procedures, please contact the undersigned at 860-830-0380 or Ms. Melanie Bachman, Acting Executive Director, Connecticut Siting Council at (860) 827-2935.

Sincerely,

Steven L. Levine Real Estate Consultant

Enclosure



FDH Engineering, Inc., 6521 Meridien Drive Raleigh, NC 27616, Ph. 919.755.1012

Structural Analysis for SBA Network Services, Inc.

130' Monopole Tower

SBA Site Name: Middletown 2 SBA Site ID: CT13064-A-05 AT&T Site Name: Middletown AT&T Site ID: CT2547

FDH Project Number 1423HD1400

Analysis Results

Tower Components	97.0%	Sufficient				
Foundation	57.9%	Sufficient				

Prepared By:

David Zambrano, El Project Engineer

FDH Engineering, Inc. 6521 Meridien Drive Raleigh, NC 27616 (919) 755-1012 info@fdh-inc.com Reviewed By:

Bradley R. Newman, PE Senior Project Engineer CT PE License No. 29630

April 4, 2014

Prepared pursuant to ANSI/TIA-222-G Structural Standard for Antenna Supporting Structures and Antennas and 2005 Connecticut State Building Code

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EXECUTIVE SUMMARY

At the request of SBA Network Services, Inc., FDH Engineering, Inc. performed a structural analysis of the monopole located in Middletown, CT to determine whether the tower is structurally adequate to support both the existing and proposed loads pursuant to the *Structural Standard for Antenna Supporting Structures and Antennas, ANSI/TIA-222-G* and 2005 Connecticut State Building Code (CSBC). Information pertaining to the existing/proposed antenna loading, current tower geometry, geotechnical data, and member sizes was obtained from:

- Radian Communication Services (File No. 060-3494) original design drawings dated December 15, 2006
- Gemini Geotechnical Associates, Inc. (Site No. 999-0049) Geotechnical Engineering Report dated November 30, 2006
- □ FDH Engineering, Inc.(Project No. 11-01248E S1) Modification & 10' Extension Drawings for a 120' Monopole dated September 21, 2011
- FDH Engineering, Inc. (Project No. 11-01248E S1) Post Construction Inspection Report dated December 14, 2011
- FDH Engineering, Inc. (Job No. 12-08192E S2) Modification Drawings for a 130' Monopole dated November 14, 2012
- FDH Engineering, Inc. (Project No. 12-11103T C1) TIA Inspection Report dated February 18, 2013
- FDH Engineering, Inc. (Project No. 1423HD1400) Modification Drawings for a 130' Monopole dated April 4, 2014
- SBA Network Services, Inc.

The basic design wind speed per the ANSI/TIA-222-G standard and 2005 CSBC is 110 mph without ice and 50 mph with 3/4" radial ice. Ice is considered to increase in thickness with height. Furthermore, this structure was analyzed as a Class II structure with Exposure Category C, Topographical Factor of 1, and Spectral Response Accelerations of $S_s = 0.236$ and $S_1 = 0.062$.

Note: Per Section 2.7.3 of the *ANSI/TIA-222-G* standard, the seismic/earthquake loading effects can be ignored if spectral response acceleration at short periods (S_S) is less than or equal to 1.00. The tower's location mandates a design S_S of less than 1.00, thus seismic loading was not considered as part of the analysis of this structure.

Conclusions

With the existing and proposed antennas from AT&T in place at 130 ft, the tower meets the requirements of the *ANSI/TIA-222-G* standard and *2005 CSBC* provided the **Recommendations** listed below are satisfied. Furthermore, provided the foundation was designed and constructed to support the original design reactions (see Radian File No. 060-3494) and the proposed modifications have been correctly installed (see FDH Engineering, Inc. Project No. 1423HD1400), the foundations should have the necessary capacity to support both the proposed and existing loading. For a more detailed description of the analysis of the tower, see the **Results** section of this report.

Our structural analysis has been performed assuming all information provided to FDH Engineering, Inc. is accurate (i.e., the steel data, tower layout, existing antenna loading, and proposed antenna loading) and that the tower has been properly erected and maintained per the original design drawings.

Recommendations

To ensure the requirements of the *ANSI/TIA-222-G* standard and *2005 CSBC* are met with the existing and proposed loading in place, we have the following recommendations:

- 1. The proposed feed lines must be installed inside the monopole shaft.
- 2. The proposed TMAs should be installed directly behind the existing and/or proposed antennas
- 3. RRU/RRH Stipulation: The equipment may be installed in any arrangement determined by the client.
- 4. Modifications outlined in FDH Engineering, Inc. (Project No. 1423HD1400) Modification Drawings for a 130' Monopole dated April 4, 2014 must be correctly installed in order for this analysis to be valid.

APPURTENANCE LISTING

The proposed and existing antennas with their corresponding cables/coax lines are shown in **Table 1**. If the actual layout determined in the field deviates from the layout, FDH Engineering, Inc. should be contacted to perform a revised analysis.

Table 1 - Appurtenance Loading

Existing Loading:

Antenna Elevation (ft)	Description	Feed Lines	Carrier	Mount Elevation (ft)	Mount Type	
130	(9) Powerwave P65-16-XLH-RR (6) Powerwave TT19-08BP111-001 TMAs (6) Powerwave 7010 RETs (6) Ericsson RRUS-11 RRUs	(18) 1-5/8"	AT&T	129	(1) Low-Profile Platform	
120	(6) RFS APXV86-906515	(12) 1-5/8"	Nextel	120	(6) Pipe Mounts	
111.5	(3) Andrew CBC721-DF					
110.5	(3) Antel BXA-70063-6CF-2 (3) Antel BXD-63606380CF	(12) 1-5/8"	Verizon	110	(3) T-Arms	
108.5	(3) Andrew CBC721-DF					
99.5	(3) APXV18-206517S	(6) 1-5/8"	Pocket	99.5	(3) Pipe Mounts	
94	(1) 1'4" x 6.5" x 6" Surge Protector	(0) = (1.0		94	Direct Mount	
91	(3) Kathrein 840 10054 (3) Samsung RASSPI-2213-RRH	(3) 5/16 (2) 1/2	Clearwire	00 F		
90.8	(1) Andrew VHLP2-18-DW1	(3) 5/8		89.5	(3) I-Arms	
90.7	(1) VHLP800-11-DW1	(3) 1/4				

Proposed Loading:

Antenna Elevation (ft)	Description	Feed Lines	Carrier	Mount Elevation (ft)	Mount Type
130	 (3) Powerwave P65-16-XLH-RR (9) CCI HPA-65R-BUU-H6 (3) CCI DTMABP7819VG12A-BP TMAs (6) Ericsson RRUS-11 RRUs (6) Ericsson RRUS-12 RRUs (3) Ericsson RRUS-32 RRUs (3) Ericsson RRUS-E2 RRUs (6) Ericsson A2 Modules (3) Raycap DC6-48-60-18-8F Surge Arrestors 	(6) 1-5/8" (1) 1.496" Fiber (6) 0.645" Power	AT&T	129	(1) 12.5' Platform w/ Handrails [Commscope P/N MTC3607R]

RESULTS

The following yield strength of steel for individual members was used for analysis:

Member Type	Yield Strength
Tower Shaft Sections	65 ksi
Upper Flange Plate	50 ksi
Guy Lugs	65 ksi
Flange Bolts	F _u = 150 ksi
Lower Flange Plate	36 ksi
Inner Anchor Bolts	F _u = 125 ksi
Outer Anchor Rods	F _u = 100 ksi
Base Plate	50 ksi

Table 2 - Material Strength

Table 3 displays the summary of the ratio (as a percentage) of force in the member to their capacities. Values greater than 100% indicate locations where the maximum force in the member exceeds its capacity. *Note: Capacities up to 100% are considered acceptable.* **Table 4** displays the maximum foundation reactions.

If the assumptions outlined in this report differ from actual field conditions, FDH Engineering, Inc. should be contacted to perform a revised analysis. Furthermore, as no information pertaining to the allowable twist and sway requirements for the existing or proposed appurtenances was provided, deflection and rotation were not taken into consideration when performing this analysis.

See the Appendix for detailed modeling information.

Table 3 - Summary of Working Percentage of Structural Components

Section No.	Elevation ft	Component Type	Size	% Capacity	Pass Fail
	130 - 120	Pole	TP18x18x0.25	51.9	Pass
L1		Upper Flange Plate	PL 31.5"ø x 1" thick	95.1	Pass
	120	Flange Bolts	(6) 1" ø on a 28.25"ø BC	91.3	Pass
		Lower Flange Plate	PL 31.5"ø x 1" thick	95.1	Pass
10	120 - 87.42	Pole	TP29.89x21.74x0.1875	75.7	Pass
LZ	107.25	Guy Wire Connection	(3) 42" Lugs, (6) 7/8" Wires	97.0	Pass
L3	87.42 - 43.3367	Pole	TP36.31x28.5352x0.25	42.1	Pass
	43.3367 - 8.0833	Pole	TP42.5x35.0541x0.312	31.0	Pass
L4	8.0833 - 0	Modified Pole	TP42.5x35.0541x0.3125 w/ Flat Plate Modifications	22.3	Pass
		Inner Anchor Bolts	(14) 1.5"ø on a 47.25"ø BC	11.3	Pass
	0	Outer Anchor Rods	(4) 2.25"ø on a 56.75"ø BC	17.9	Pass
		Base Plate	PL 51.75"ø x 1.5" thick	10.2	Pass

Table 4 - Maximum Base Reactions

Current Analysis* Base Reactions (ANSI/TIA-222-G)			Original Design (ANSI/TIA-222-G)			
	Horizontal	Vertical	Moment	Horizontal	Vertical	Moment
Base	6	173 k	623 k-ft	20 k	39 k	1,864 k-ft
Anchors	36	84 k				

* Foundations determined to be adequate per independent analysis.

GENERAL COMMENTS

This engineering analysis is based upon the theoretical capacity of the structure. It is not a condition assessment of the tower and its foundation. It is the responsibility of SBA Network Services, Inc. to verify that the tower modeled and analyzed is the correct structure (with accurate antenna loading information) modeled. If there are substantial modifications to be made or the assumptions made in this analysis are not accurate, FDH Engineering, Inc. should be notified immediately to perform a revised analysis.

LIMITATIONS

All opinions and conclusions are considered accurate to a reasonable degree of engineering certainty based upon the evidence available at the time of this report. All opinions and conclusions are subject to revision based upon receipt of new or additional/updated information. All services are provided exercising a level of care and diligence equivalent to the standard and care of our profession. No other warranty or guarantee, expressed or implied, is offered. Our services are confidential in nature and we will not release this report to any other party without the client's consent. The use of this engineering work is limited to the express purpose for which it was commissioned and it may not be reused, copied, or distributed for any other purpose without the written consent of FDH Engineering, Inc.

APPENDIX

Document No. ENG-RPT-501S





DESIGNED APPURTENANCE LOADING

ТҮРЕ	ELEVATION	ТҮРЕ	ELEVATION
Lightning Rod	130	(2) APXV86-906515 W/ Mount Pipe	120
P65-16-XLH-RR W/ Mount Pipe	129	(2) APXV86-906515 W/ Mount Pipe	120
P65-16-XLH-RR W/ Mount Pipe	129	(2) APXV86-906515 W/ Mount Pipe	120
P65-16-XLH-RR W/ Mount Pipe	129	BXA-70063-6CF-2 w/ Mount Pipe	110
(3) HPA-65R-BUU-H6 w/ Mount Pipe	129	BXA-70063-6CF-2 w/ Mount Pipe	110
(3) HPA-65R-BUU-H6 w/ Mount Pipe	129	BXA-70063-6CF-2 w/ Mount Pipe	110
(3) HPA-65R-BUU-H6 w/ Mount Pipe	129	BXD-63606380CF W/ Mount Pipe	110
DTMABP7819VG12A TMA	129	BXD-63606380CF W/ Mount Pipe	110
DTMABP7819VG12A TMA	129	BXD-63606380CF W/ Mount Pipe	110
DTMABP7819VG12A TMA	129	CBC721-DF	110
(2) RRUS-11	129	CBC721-DF	110
(2) RRUS-11	129	CBC721-DF	110
(2) RRUS-11	129	CBC721-DF	110
(2) RRUS-12	129	CBC721-DF	110
(2) RRUS-12	129	CBC721-DF	110
(2) RRUS-12	129	(3) T-Arms	110
RRUS-32	129	APXV18-206517S W/Mount Pipe	99.5
RRUS-32	129	APXV18-206517S W/Mount Pipe	99.5
RRUS-32	129	APXV18-206517S W/Mount Pipe	99.5
RRUS-E2	129	1'4" x 6.5" x 6" Surge Protector	94
RRUS-E2	129	VHLP2-18-DW1	90
RRUS-E2	129	VHLP800-11-DW1	90
RRUS A2 MODULE	129	RASSPI-2213-RRH	89.5
RRUS A2 MODULE	129	RASSPI-2213-RRH	89.5
RRUS A2 MODULE	129	RASSPI-2213-RRH	89.5
DC6-48-60-18-8F	129	(3) T-Arms	89.5
DC6-48-60-18-8F	129	840 10054 w/ Mount Pipe	89.5
DC6-48-60-18-8F	129	840 10054 w/ Mount Pipe	89.5
(1) Platform w/ Handrails MNT [Commscope P/N MTC3607]	129	840 10054 w/ Mount Pipe	89.5
Kicker Support [Commscope P/N MTC3237]	129	1	

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A500-50	50 ksi	62 ksi	A572-65	65 ksi	80 ksi

TOWER DESIGN NOTES

84 K

R

- IOWER DESIGN NOTES
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 Deflections are based upon a 60 mph wind.
 Tower Structure Class II.
 Topographic Category 1 with Crest Height of 0.00 ft
 TOWER RATING: 97%

36 K R=45.00 ft

ALL REACTIONS ARE FACTORED

