

Northeast Site Solutions Victoria Masse 420 Main St Unit 1 Box 2 Sturbridge, MA 01566 victoria@northeastsitesolutions.com

February 3, 2022

Members of the Siting Council Connecticut Siting Council Ten Franklin Square New Britain, CT 06051

RE: Tower Share Application 11 West Peak Drive, Meriden, CT Latitude: 41.561200 N

Longitude: -72.8441 W Site#: BOHVN00204B

### Dear Ms. Bachman:

This letter and attachments are submitted on behalf of Dish Wireless LLC. Dish Wireless LLC plans to install antennas and related equipment to the tower site located at 11 West Peak Drive, Meriden, Connecticut.

Dish Wireless LLC proposes to install three (3) 600/1900/2100 5G MHz antenna and six (6) RRUs, at the 117-foot level of the existing 125-foot self-support tower, one (1) Fiber cable will also be installed. Dish Wireless LLC equipment cabinets will be placed within 7x5 lease area. Included are plans by Infinigy, dated December 7, 2021, Exhibit C. Also included is a structural analysis prepared by Armor Tower Engineering, dated September 22, 2021 confirming that the existing tower is structurally capable of supporting the proposed equipment. Attached as Exhibit D. This facility was approved by the Connecticut Siting Council, Petition No. 67 on March 26, 1981. Please see attached Exhibit A.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies 16-50aa, of Dish Wireless LLC intent to share a telecommunications facility pursuant to R.C.S.A. 16-50j-88. In accordance with R.C.S.A., a copy of this letter is being sent to Kevin M. Scarpati, Mayor, Paul Dickson, Acting Director of Planning, Development & Enforcement, as well as the property owner Frontier Communications and EIP Holdings II LLC, tower owner.

The planned modifications of the facility fall squarely within those activities explicitly provided for in R.C.S.A. 16-50j-89.

- 1. The proposed modifications will not result in an increase in the height of the existing structure. The top of the tower is 125-feet; Dish Wireless LLC proposed antennas will be located at a center line height of 117-feet.
- 2. The proposed modification will not result in the increase of the site boundary as depicted on the attached site plan.
- 3. The proposed modification will not increase the noise levels at the facility by six decibels or more, or to levels that exceed local and state criteria. The incremental effect of the proposed changes will be negligent.



4.The operation of the proposed antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard. As indicated in the attached power density calculations, the combined site operations will result in a total density of 15.83% as evidenced by Exhibit F.

Connecticut General Statutes 16-50-aa indicates that the Council must approve the shared use of a telecommunications facility provided it finds the shared use is technically, legally, environmentally, and economically feasible and meets public safety concerns. As demonstrated in this letter, Dish Wireless LLC respectfully indicates that the shared use of this facility satisfies these criteria.

A. Technical Feasibility. The existing self-support tower has been deemed structurally capable of supporting Dish Wireless LLC proposed loading. The structural analysis is included in Exhibit D.

B. Legal Feasibility. As referenced above, C.G.S. 16-50aa has been authorized to issue orders approving the shared use of an existing tower such as this self-support tower in Meriden. Under the authority granted to the Council, an order of the Council approving the requested shared use would permit Dish Wireless LLC to obtain a building permit for the proposed installation. Further, a letter of Authorization is included as Exhibit G, authorizing Dish Wireless LLC to file this application for shared use.

C. Environmental Feasibility. The proposed shared use of this facility would have a minimal environmental impact. The installation of Dish Wireless LLC equipment at the 117-foot level of the existing 125-foot tower would have an insignificant visual impact on the area around the self-support tower. Dish Wireless LLC ground equipment would be installed within the existing facility compound. Dish Wireless LLC shared use would therefore not cause any significant alteration in the physical or environmental characteristics of the existing site. Additionally, as evidenced by Exhibit F, the proposed antennas would not increase radio frequency emissions to a level at or above the Federal Communications Commission safety standard.

D. Economic Feasibility. Dish Wireless LLC will be entering into an agreement with the owner of this facility to mutually agreeable terms. As previously mentioned, the Letter of Authorization has been provided by the owner to assist Dish Wireless LLC with this tower share application.

E. Public Safety Concerns. As discussed above, the tower is structurally capable of supporting Dish Wireless LLC proposed loading. Dish Wireless LLC is not aware of any public safety concerns relative to the proposed sharing of the existing tower. Dish Wireless LLC intentions of providing new and improved wireless service through the shared use of this facility is expected to enhance the safety and welfare of local residents and individuals traveling through Meriden.

Sincerely,

Victoria Masse Mobile: 860-306-2326 Fax: 413-521-0558

Office: 420 Main Street, Unit 1 Box 2, Sturbridge, MA 01566

Email: victoria@northeastsitesolutions.com



Attachments

Cc: Kevin M. Scarpati, Mayor City of Meriden 142 East Main Street Meriden, CT 06450

Paul Dickson, Acting Director of Planning, Development & Enforcement City of Meriden 142 East Main Street Meriden, CT 06450

Frontier Communications, Property Owner 401 Merrit 7 Norwalk, CT 06851

EIP Holdings II LLC, Tower Owner 100 Summer Street, Suite 1600 Boston, MA 02110

# Exhibit A

**Original Facility Approval** 

## STATE OF CONNECTICUT



# DEPARTMENT OF BUSINESS REGULATION POWER FACILITY EVALUATION COUNCIL

Petition No. 67 Wolcott, Connecticut March 26, 1981

Mr. Doocy, Mr. Clapp, Mr. Wood, and Mr. Reid met Mr. Kischell and Mr. Bailey of the Southern New England Telephone Company to review the first half of Petition No. 67. Telecommunication facilities were viewed in Wolcott, Waterbury, and Meriden. The second half of Petition No. 67 involves facilities in Shelton, Norwalk, and Bridgeport. These were reviewed on March 31, 1981.

The first half of this petition involves the following changes at the Barry Avenue site in Wolcott: (a) replacing an existing 90 foot tall triangular lattice steel tower with an 80 foot tall square lattice steel tower; (b) replacing two microwave dishes and two reflectors with four new microwave dishes; (c) adding a 12' x 16' concrete radio building and a new fuel storage tank at the base of the tower and extending the fence to encompass the new facilities. Additional changes include: (d) adding two microwave antennae to the Waterbury East Tower in Waterbury and another concrete radio building; and (e) adding one microwave antenna to the West Peak tower in Meriden.

The Wolcott site is in a single family dwelling residential area near the top of Clinton Hill. The tower is visible from several locations within the area. The tower base and radio building are partially screened by vegetation from the nearest residence and are not visible from other residences. The new tower will be located several feet northeast of the existing tower at approximately the same ground elevation. The proposed tower will be 80 feet tall and more narrow than the existing tower; it will be square instead of triangular. The new microwave antennae are to be mounted on a platform at the top of the tower.

The soil appears shallow but stable, and a few bedrock outcrops appear on the site. The proposed tower will require new foundations which will be set in soil or bedrock. If the soil is too shallow or the bedrock unsuitable, some blasting may be necessary.

. A new concrete building will be constructed at the base of the tower and will accommodate the generator used for emergency power. The existing fence will be extended to enclose this facility.

The existing tower will remain in place for approximately six months or until the new facility is operating properly. Then the existing tower will be dismantled and removed.

According to the SNETCO representatives, this proposal has been approved by the Wolcott Planning and Zoning Commission.

The Waterbury East tower is located adjacent to a water tower and several other cable TV or telecommunication towers on top of Long Hill in Waterbury. The site is surrounded by single and multiple family dwellings, commercial, and industrial properties. Both the telecommunication tower and the water tower are visible

Phone 566-5612

State Office Building — Hartford, Connecticut 06115

from many viewpoints in the Waterbury area. Two microwave antennae are to be mounted at the 80 foot level to the existing 90 foot tower. Once the new facilities are operating, two narrow 80 foot tall towers presently on the site can be removed. These two towers now support reflectors which relay signals from the Waterbury central office to Wolcott. A new radio building will be constructed at the base of the tower and the existing fence will be extended to surround this new building. The radio building will house an emergency generator, the new radio equipment, and future radio equipment when existing facilities are replaced. An existing building presently storing a temporary generator may be removed after the new building is constructed. According to SNETCO representatives, this proposal has received planning and zoning approval.

The Meriden tower is adjacent to West Peak State Park and several telecommunication towers on the top of West Peak. The existing telecommunication facilities on West Peak are relatively well screened from most locations within the state park, but they are a prominent feature on the ridge top as seen from viewpoints in the Meriden area and can be seen up to many miles away on clear days.

The telephone company's tower presently supports seven microwave antennae. SNETCO proposes to add one microwave dish to the existing tower at the 90 foot level to complete a route from Meriden to the Wolcott Tower. The existing North Branform to Wolcott route will be eliminated, and an antenna at the North Branford tower may be removed when the Meriden to Wolcott route is in service. No additional buildings are proposed at this site.

Duncan C. Reid Environmentalist March 30, 1981

## STATE OF CONNECTICUT

# DEPARTMENT OF BUSINESS REGULATION POWER FACILITY EVALUATION COUNCIL

Petition No. 67 Norwalk, Connecticut March 31, 1981

Commissioner Boucher, Mr. Clapp, Christopher Wood and Duncan Reid met Mr. Bailey and Mr. Kischell of the Southern New England Telephone Company to review the second part of Petition No. 67 which involved facilities in Norwalk, Bridgeport, and Shelton. The first part of this petition involves facilities located in Wolcott, Waterbury, and Meriden which were visited on Thursday, March 26th.

In Norwalk one dish is to be mounted on an existing 350 foot tower located at a telephone company service center immediately north of Route 1. The dish will be directed toward the existing tower in Bridgeport. The general area around the Norwalk site appears to be commercial, residential, and industrial. The tower is visible from many locations in the area.

The Bridgeport tower (40 feet tall) is located on top of the Central Office Building in downtown Bridgeport. One dish will be mounted at approximately the 30 foot level and directed tower the new dish in Norwalk. The location of the tower on top of the office building diminishes its visual impact.

The 181 foot tower in Shelton is located in a rural residential area. One 5 foot dish will be removed and a 12 foot dish mounted in the same location and directed toward an existing facility in Derby. A new and large dish is required in Shelton to prevent interference with transmissions from Shelton to New Haven. This tower is visible from selected locations within the immediate area and from some distant viewpoints.

No additional radio buildings, generators, or fuel tanks, are planned for the facilities in Norwalk, Bridgeport, and Shelton.

Duncan C. Reid Environmentalist March 31, 1981

# Exhibit B

**Property Card** 



**PROPERTY** 

# PROPERTY INFORMATION Location: 11 WEST PEAK DR

Map/Lot: 1214-0352-0021-0000

INFORMATION Owner(s):

SOUTHERN NEW ENGLAND TEL CO C/O FRONTIER COMMUNICATIONS

Owner Address:

ATTN: TAX DEPT 401 MERRIT 7

NORWALK, CT 06851

### **BUILDING INFORMATION**

Card 1 Number:

Total Units: 1

OVERVIEW	ı
Building ID	19560
Finished Area	1,542
Comm/Rental Units	1
Living Units	0
Building Type	MultiPurp
Year Built	1979
Effective Yr Built	
Building Number	1
Condo Name	

INTERIOR DETAIL	-S		
Rooms			
BedRooms			
Full Bath	0		
Full Bath Rating			
Half Bath	0		
Half Bath Rating			
Kitchens	0		
Kitchen Rating			
Fireplaces	0		

CONSTRUCT	ION DETAILS
Exterior	Concrete Blo
Roof Structure	Gable
Roof Cover	Asphalt
Quality	C+
Heat Fuel	Gas
<b>Heat Type</b>	Forced Air
Prcnt. Heated	100.00
Prcnt. AC	0.00
Stories	1 story
Foundation	Conc Slab

Sub Area **Summary** 

Building ID	Description	Total Area	Fin. Area	Perimeter
19560	1st FLOOR	1,542	1,542	204

Special **Features** 

BuildingID	Description	Quantity	Area	Length	Width	YearBuilt	Quality
19560	SHED FRAME	1	160			1979	Average
19560	FENCE 4	1	400			1979	Average

APPRAISAL INFORMATION Tax District: 1 District Name: OUTER DISTRICT District Mill Rate: 40.86

**Grand List** Year: 2019

Land Appraised	Building Appraised	Yard Appraised	Total Appraised Value	Land Assessed	Building Assessed	Yard Assessed	Special Land Value	Total Assessed Value
\$600,000	\$137,000	\$4,200	\$741,200	\$420,000	\$95,900	\$2,940	\$0	\$518,840

**Previous** Year: 2018

Land	Building	Yard	Appraised	Land	Building	Yard	Assessed
Value	Value	Items	Value	Value	Value	Items	Value
\$600,000	\$137,000	\$4,200	\$741,200	\$420,000	\$95,900	\$2,940	\$518,840

### **LAND INFORMATION**

Land Use	Zoning	Land Area	Neighborhood Description
Comm Bldg	R-R	0.82874	N OF W MAIN E&W OF RT 71

<sup>\*</sup>Confirm zoning with Planning Office.

Zoning map is the official document to determine zone.

### **SALES INFORMATION**

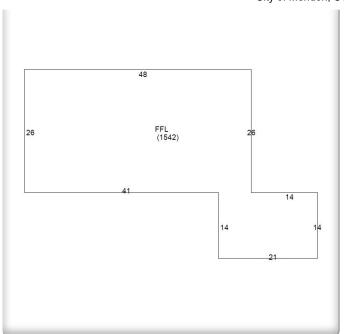
	Grantee Deed Type	Grantor	Page	Book	Sale Price	Sale Date
3/8/1947 \$0 281 483			483	281	\$0	3/8/1947

### **ASSESSOR'S PERMIT HISTORY**

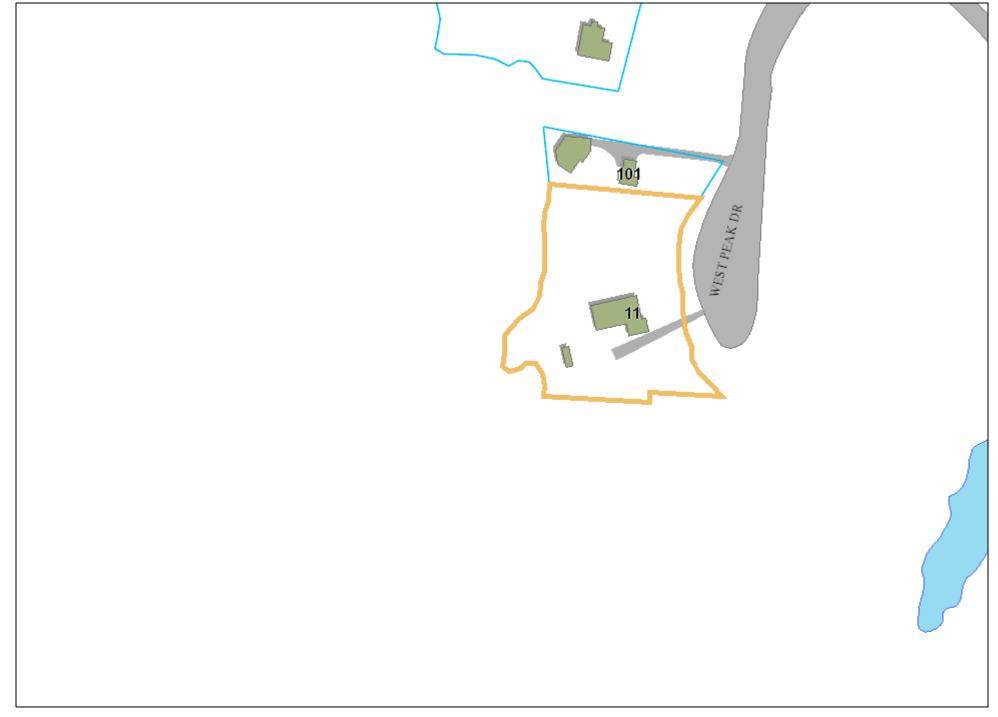
No data found.

### **PROPERTY IMAGES**





19562 1214-0352-0021-0000





Date: 11/12/2020

CITY OF MERIDEN, CT GIS

11 WEST PEAK DR



# Exhibit C

**Construction Drawings** 

# wireless...

DISH Wireless L.L.C. SITE ID:

# BOHVN00204B

DISH Wireless L.L.C. SITE ADDRESS:

# **WEST PEAK DRIVE** MERIDEN, CT 06451

# CONNECTICUT CODE COMPLIANCE

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:

CODE TYPE

2018 CT STATE BUILDING CODE/2015 IBC W/ CT AMENDMENTS 2018 CT STATE BUILDING CODE/2015 IMC W/ CT AMENDMENTS **MECHANICAL** 2018 CT STATE BUILDING CODE/2017 NEC W/ CT AMENDMENTS ELECTRICAL

	SHEET INDEX
SHEET NO.	SHEET TITLE
T-1	TITLE SHEET
A-1	OVERALL AND ENLARGED SITE PLAN
A-2	ELEVATION, ANTENNA LAYOUT AND SCHEDULE
A-3	EQUIPMENT PLATFORM AND H-FRAME DETAILS
A-4	EQUIPMENT DETAILS
A-5	EQUIPMENT DETAILS
A-6	EQUIPMENT DETAILS
E-1	ELECTRICAL/FIBER ROUTE PLAN AND NOTES
E-2	ELECTRICAL DETAILS
E-3	ELECTRICAL ONE-LINE, FAULT CALCS & PANEL SCHEDULE
G-1	GROUNDING PLANS AND NOTES
G-2	GROUNDING DETAILS
G-3	GROUNDING DETAILS
RF-1	RF CABLE COLOR CODE
GN-1	LEGEND AND ABBREVIATIONS
GN-2	GENERAL NOTES
GN-3	GENERAL NOTES
GN-4	GENERAL NOTES

# SCOPE OF WORK

THIS IS NOT AN ALL INCLUSIVE LIST. CONTRACTOR SHALL UTILIZE SPECIFIED EQUIPMENT PART OR ENGINEER APPROVED EQUIVALENT. CONTRACTOR SHALL VERIFY ALL NEEDED EQUIPMENT TO PROVIDE A FUNCTIONAL SITE. THE PROJECT GENERALLY CONSISTS OF THE FOLLOWING:

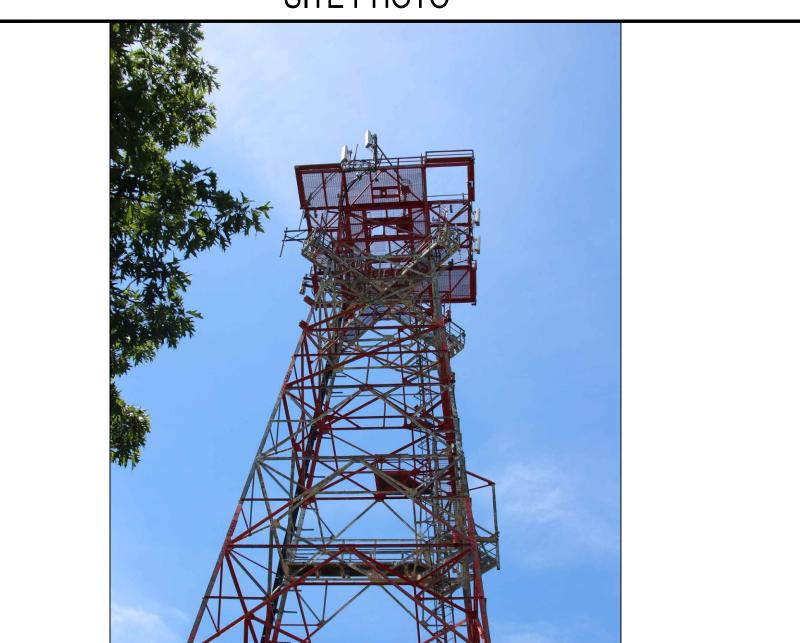
# TOWER SCOPE OF WORK:

- INSTALL (3) PROPOSED PANEL ANTENNAS (1 PER SECTOR) INSTALL (3) PROPOSED ANTENNA SECTOR MOUNTS (1 PER SECTOR) INSTALL PROPOSED JUMPERS
- INSTALL (6) PROPOSED RRUS (2 PER SECTOR)
- INSTALL (1) PROPOSED OVER VOLTAGE PROTECTION DEVICE (OVP)
- INSTALL (1) PROPOSED HYBRID CABLE

# **GROUND SCOPE OF WORK:**

- INSTALL (1) PROPOSED METAL PLATFORM
- INSTALL (1) PROPOSED ICE BRIDGE
- INSTALL (1) PROPOSED PPC CABINET
- INSTALL (1) PROPOSED EQUIPMENT CABINET
- INSTALL (1) PROPOSED POWER CONDUIT INSTALL (1) PROPOSED TELCO CONDUIT
- INSTALL (1) PROPOSED TELCO-FIBER BOX
- INSTALL (1) PROPOSED GPS UNIT
- INSTALL (1) PROPOSED SAFETY SWITCH (IF REQUIRED) INSTALL (1) PROPOSED FIBER NID (IF REQUIRED)
- INSTALL (1) PROPOSED METER SOCKET

# SITE PHOTO





UNDERGROUND SERVICE ALERT CBYD 811 UTILITY NOTIFICATION CENTER OF CONNECTICUT (800) 922-4455 WWW.CBYD.COM

CALL 2 WORKING DAYS UTILITY NOTIFICATION PRIOR TO CONSTRUCTION

# **GENERAL NOTES**

THE FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION. A TECHNICIAN WILL VISIT THE SITE AS REQUIRED FOR ROUTINE MAINTENANCE. THE PROJECT WILL NOT RESULT IN ANY SIGNIFICANT DISTURBANCE OR EFFECT ON DRAINAGE. NO SANITARY SEWER SERVICE, POTABLE WATER, OR TRASH DISPOSAL IS REQUIRED AND NO COMMERCIAL SIGNAGE IS PROPOSED.

# 11"x17" PLOT WILL BE HALF SCALE UNLESS OTHERWISE NOTED

CONTRACTOR SHALL VERIFY ALL PLANS, EXISTING DIMENSIONS, AND CONDITIONS ON THE JOB SITE, AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK.

### PROPERTY OWNER: **EVEREST APPLICANT:** DISH Wireless L.L.C. WEST PEAK DRIVE 5701 SOUTH SANTA FE DRIVE ADDRESS: MERIDEN, CT 06451 LITTLETON, CO 80120 **TOWER TYPE: SELF SUPPORT TOWER** TOWER OWNER: EIP COMMUNICATIONS I. LLC TOWER CO SITE ID: TWO ALLEGHENY CENTER NOVA TOWER 2, SUITE 703 TOWER APP NUMBER: PITTSBURGH PA 15212 SITE DESIGNER: INFINIGY COUNTY: NEW HAVEN 1033 WATERVLIET SHAKER RD ALBANY, NY 12205 LATITUDE (NAD 83): 41° 33′ 40.01″ N 41.561200 N (518) 690-0790 LONGITUDE (NAD 83): 72° 50′ 39.00″ W 72.8441 W CONNECTICUT SITING COUNCIL ZONING JURISDICTION: SITE ACQUISITION: APRIL PARROTT **ZONING DISTRICT:** CONSTRUCTION MANAGER: JAVIER SOTO PARCEL NUMBER: SYED ZAIDI OCCUPANCY GROUP: **RF ENGINEER:** CONSTRUCTION TYPE: POWER COMPANY: TELEPHONE COMPANY: TBD

PROJECT DIRECTORY

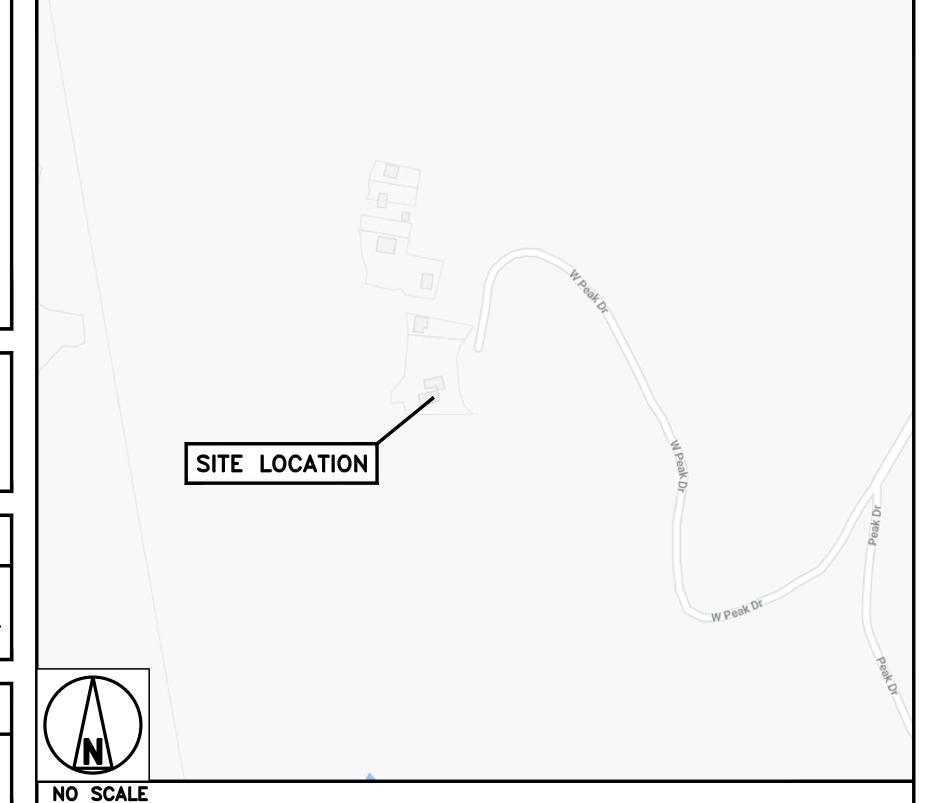
# **DIRECTIONS**

## DIRECTIONS FROM MERIDEN MARKHAM MUNICIPAL AIRPORT:

SITE INFORMATION

HEAD NORTH ON EVANSVILLE AVE TOWARD BAKER AVE, TURN LEFT ONTO CT-70 / MAIN ST, TURN RIGHT TO STAY ON CT-70 / RIVER RD, TURN RIGHT ONTO OREGON RD, TURN LEFT ONTO COE AVE, TURN RIGHT ONTO ALLEN AVE, TURN RIGHT ONTO JOHNSON AVE, TURN LEFT ONTO W MAIN ST TURN RIGHT ONTO HUBBARD PARK DR, TURNS RESTRICTED AT SPECIFIC TIMES, ROAD RESTRICTED AT SPECIFIC TIMES, ROAD NAME CHANGES TO RESERVOIR AVE, ROAD RESTRICTED AT SPECIFIC TIMES, ROAD NAME CHANGES TO WEST PEAK DR, KEEP STRAIGHT, HEADING TOWARD PARK DR, KEEP STRAIGHT TO GET ONTO PARK DR, ROAD RESTRICTED AT SPECIFIC TIMES, TURN LEFT ONTO W PEAK DR POAD RESTRICTED AT SPECIFIC TIMES. LEFT ONTO W PEAK DR, ROAD RESTRICTED AT SPECIFIC TIMES, ARRIVE AT WEST PEAK DRIVE MERIDEN, CT 06451



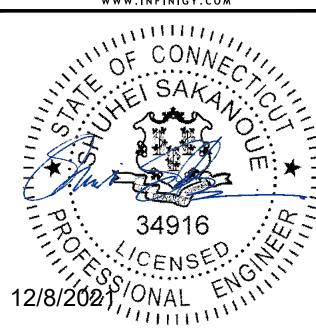




5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120



the solutions are endless HOFFMAN ESTATES, IL 60169 PHONE: 847-648-4068 | FAX: 518-690-0793



UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

DRAWN BY:	CHECKED BY:	APPROVED BY
RCD	SS	CJW

RFDS REV #: N/A

# CONSTRUCTION DOCUMENTS

		SUBMITTALS
REV	DATE	DESCRIPTION
0	11/01/21	ISSUED FOR PERMIT
1	11/16/21	REVISED FOR PERMIT
2	12/07/21	REVISED FOR PERMIT
	A&E F	PROJECT NUMBER

1197-F0001-C

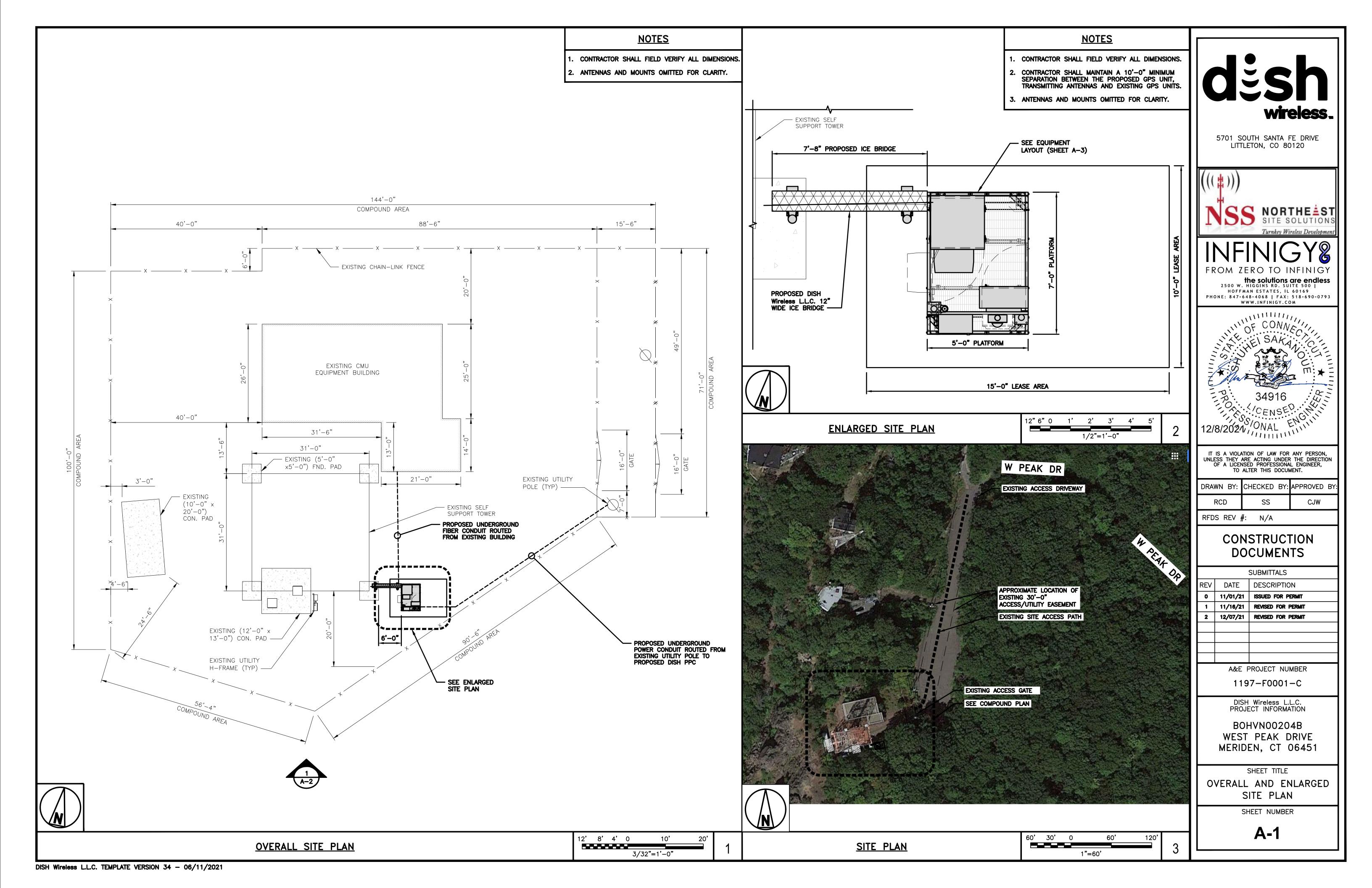
DISH Wireless L.L.C. PROJECT INFORMATION

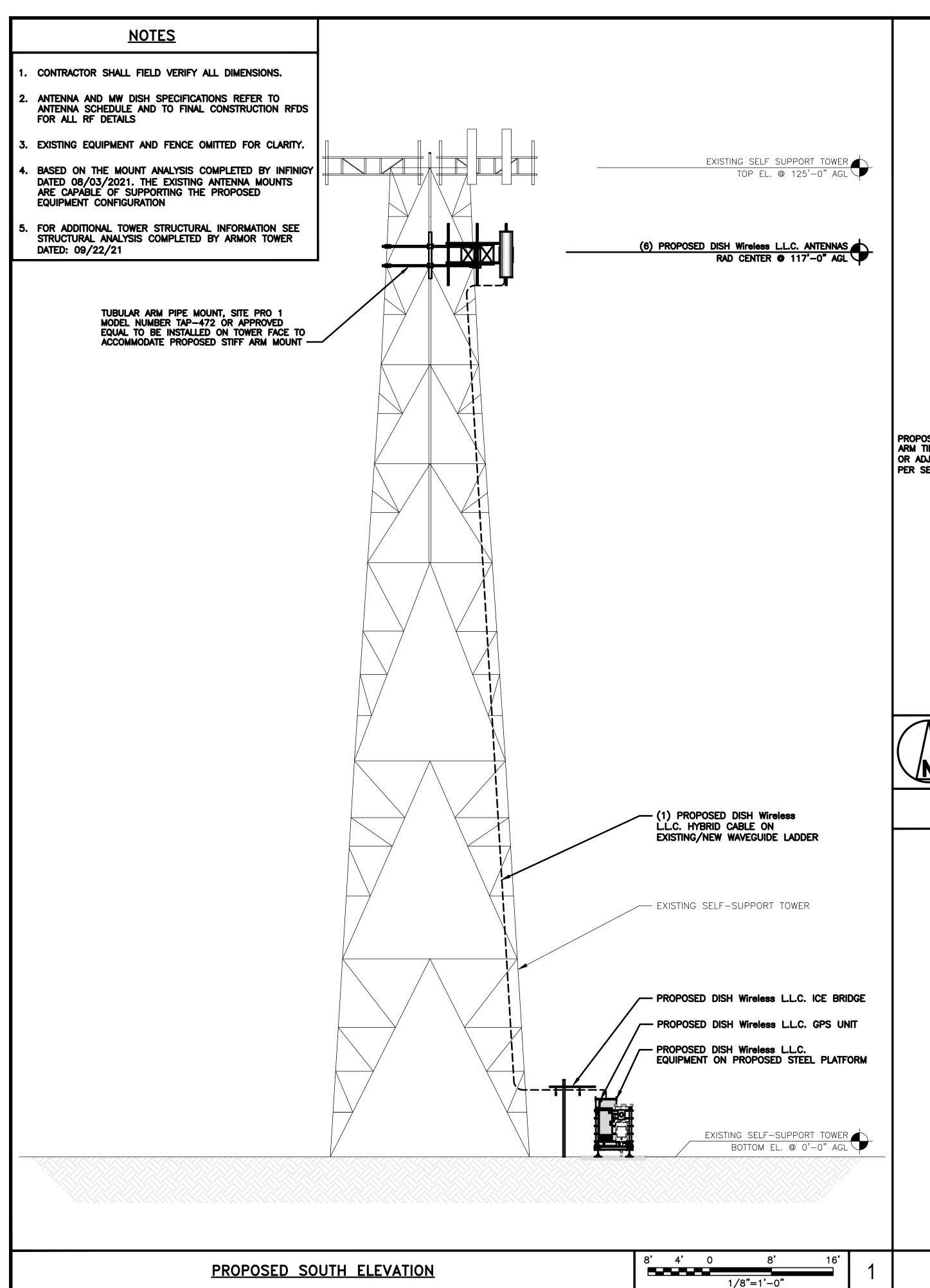
BOHVN00204B WEST PEAK DRIVE MERIDEN, CT 06451

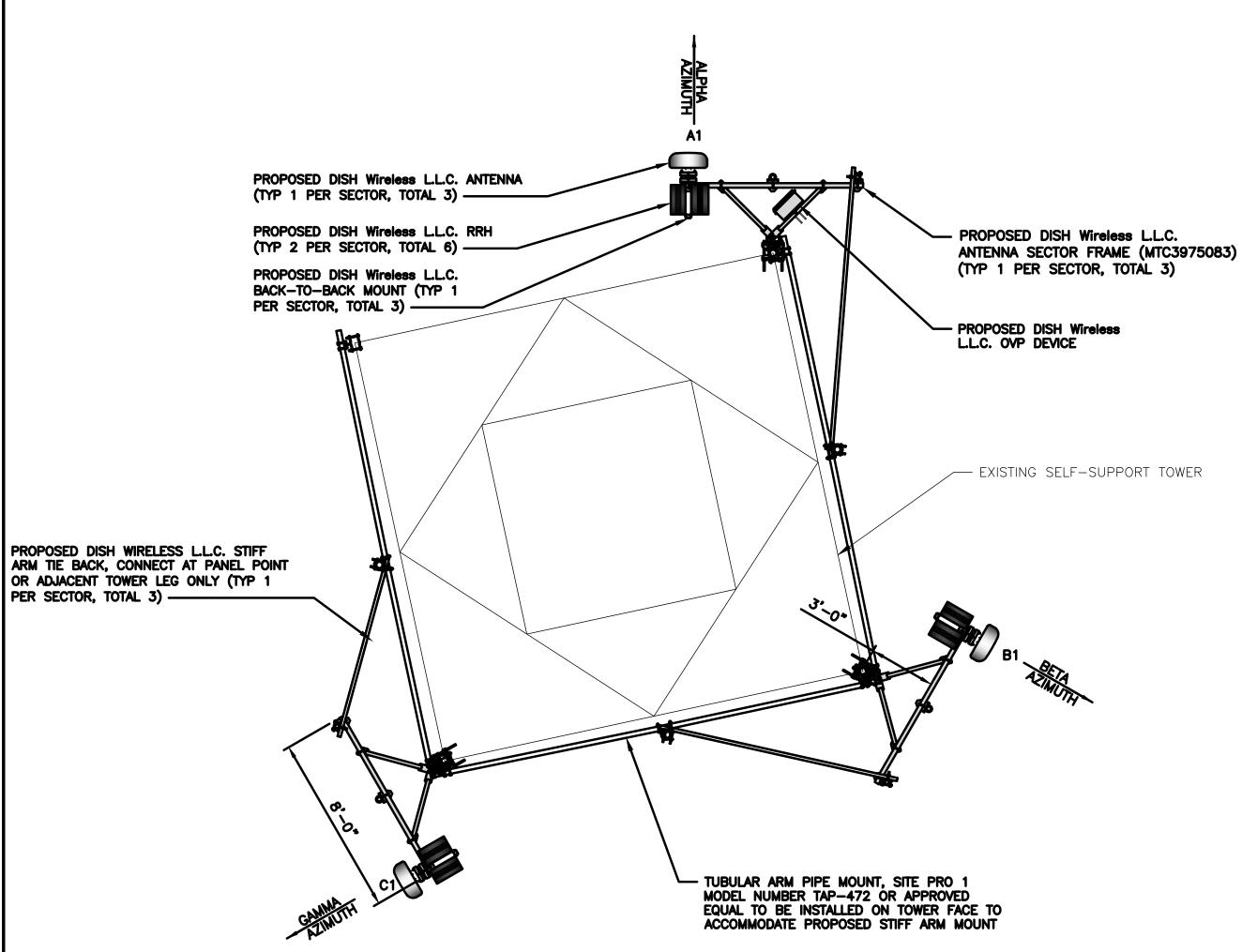
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SHEET NUMBER

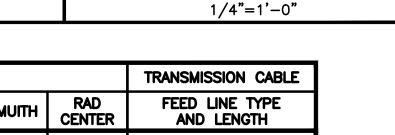
**T-1** 







ANTENNA LAYOUT



4' 2' 0

			TRANSMISSION CABLE					
SECTOR	POSITION	EXISTING OR PROPOSED	MANUFACTURER — MODEL NUMBER	TECHNOLOGY	SIZE (HxW)	AZMUITH	RAD CENTER	FEED LINE TYPE AND LENGTH
ALPHA	A1	PROPOSED	JMA WIRELESS - MX08FR0665-21	5G	72.0" × 20.0"	o	117'-0"	(4) LIIOU CARACITY
BETA	B1	PROPOSED	JMA WIRELESS - MX08FR0665-21	5G	72.0" × 20.0"	120°	117'-0"	(1) HIGH-CAPACITY HYBRID CABLE (160' LONG)
GAMMA	C1	PROPOSED	JMA WIRELESS - MX08FR0665-21	5G	72.0" × 20.0"	240°	117'-0"	(100 LONG)

# **NOTES**

- 1. CONTRACTOR TO REFER TO FINAL CONSTRUCTION RFDS FOR ALL RF DETAILS.
- 2. ANTENNA OR RRH MODELS MAY CHANGE DUE TO EQUIPMENT AVAILABILITY. ALL EQUIPMENT CHANGES MUST BE APPROVED AND REMAIN IN COMPLIANCE WITH THE PROPOSED DESIGN AND STRUCTURAL ANALYSES.

		KKH	
SECTOR	POSITION	MANUFACTURER — MODEL NUMBER	TECHNOLOGY
ALPHA	<b>A1</b>	FUJITSU - TA08025-B604	5G
	A1	FUJITSU - TA08025-B605	5G
	B1	FUJITSU - TA08025-B604	5G
BETA	B1	FUJITSU - TA08025-B605	5G
GAMMA	C1	FUJITSU - TA08025-B604	5G
	C1	FUJITSU - TA08025-B605	5G

- 1. CONTRACTOR TO REFER TO FINAL CONSTRUCTION RFDS FOR ALL RF
- 2. ANTENNA AND RRH MODELS MAY CHANGE DUE TO EQUIPMENT AVAILABILITY. ALL EQUIPMENT CHANGES MUST BE APPROVED AND REMAIN IN COMPLIANCE WITH THE PROPOSED DESIGN AND STRUCTURAL ANALYSES.

SHEET NUMBER

**A-2** 

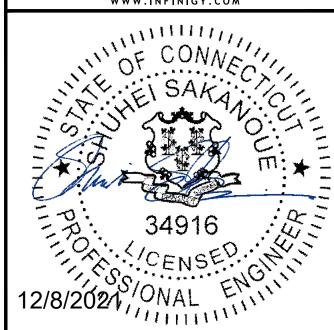
ANTENNA SCHEDULE

NO SCALE

5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120



the solutions are endless 2500 W. HIGGINS RD. SUITE 500 | HOFFMAN ESTATES, IL 60169 PHONE: 847-648-4068 | FAX: 518-690-0793 WWW.INFINIGY.COM



IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

l	DRAWN	BY:	CHECKED	BY:	APPROVED	В
	RCI	)	SS		CJW	

RFDS REV #: N/A

# CONSTRUCTION **DOCUMENTS**

		SUBMITTALS					
REV	DATE	DESCRIPTION					
0	11/01/21	ISSUED FOR PERMIT					
1	11/16/21	REVISED FOR PERMIT					
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	A&E PROJECT NUMBER						
	110	7_F0001_C					

119/-10001-0

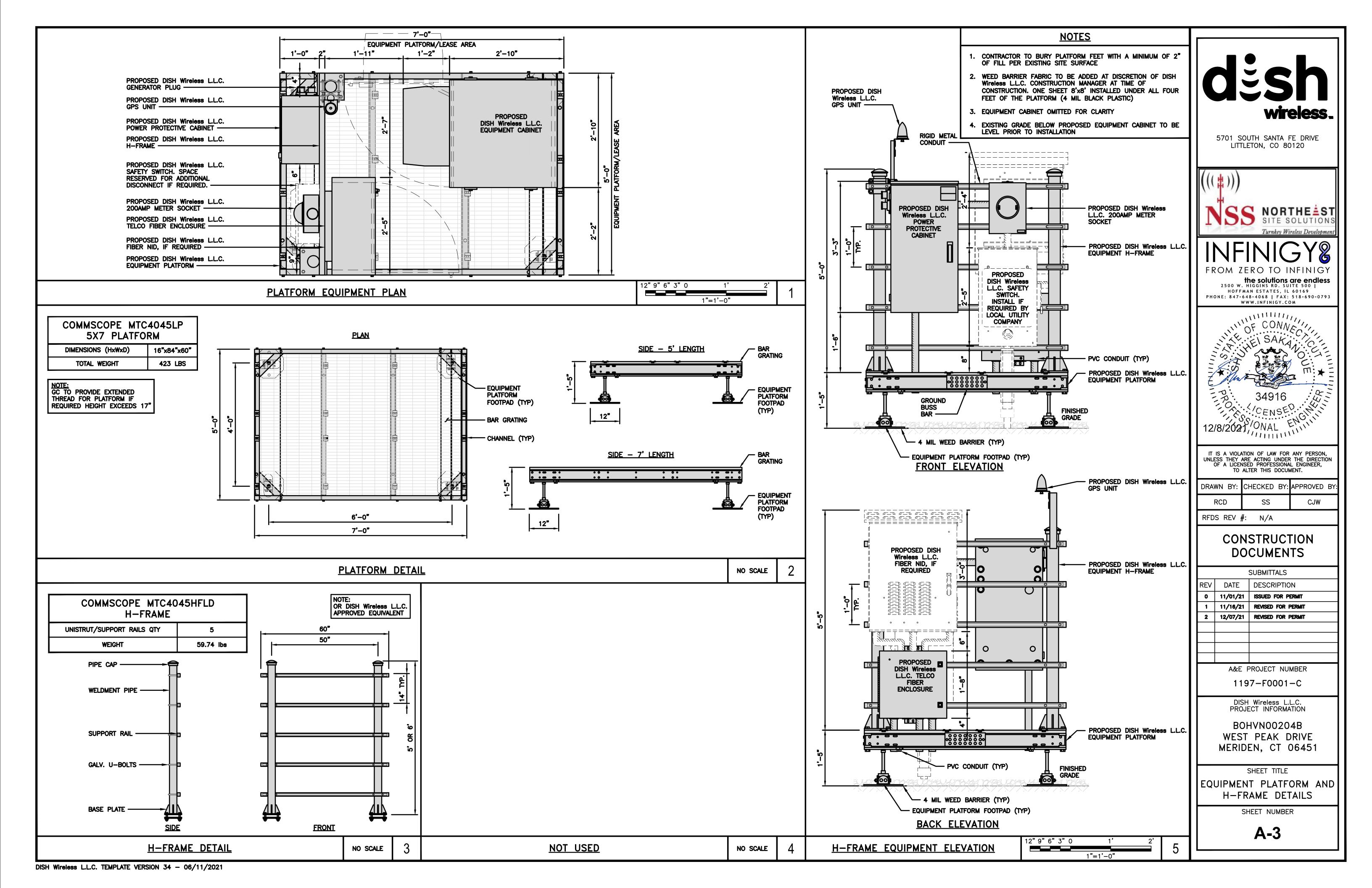
DISH Wireless L.L.C. PROJECT INFORMATION BOHVN00204B

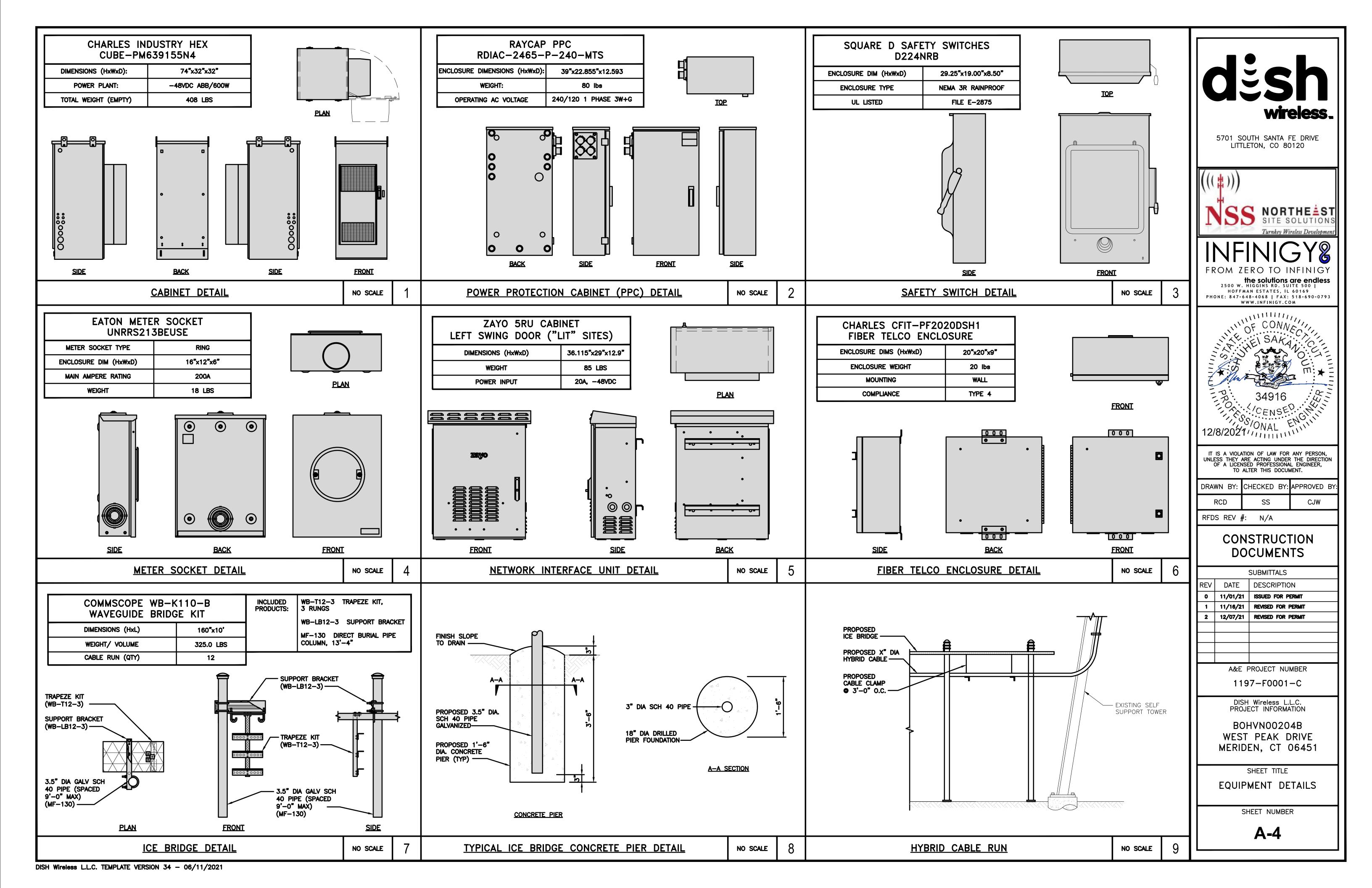
WEST PEAK DRIVE MERIDEN, CT 06451

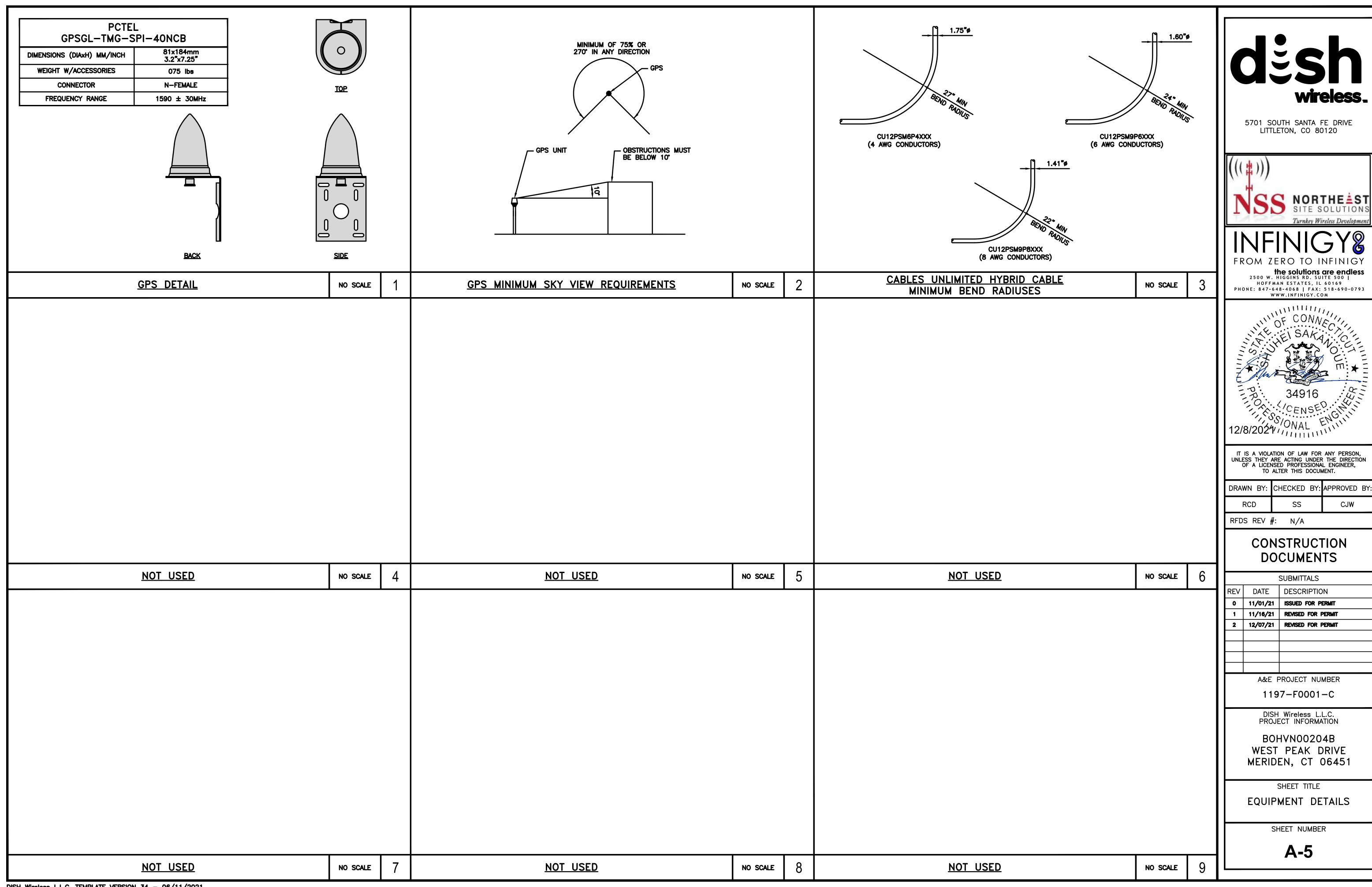
SHEET TITLE

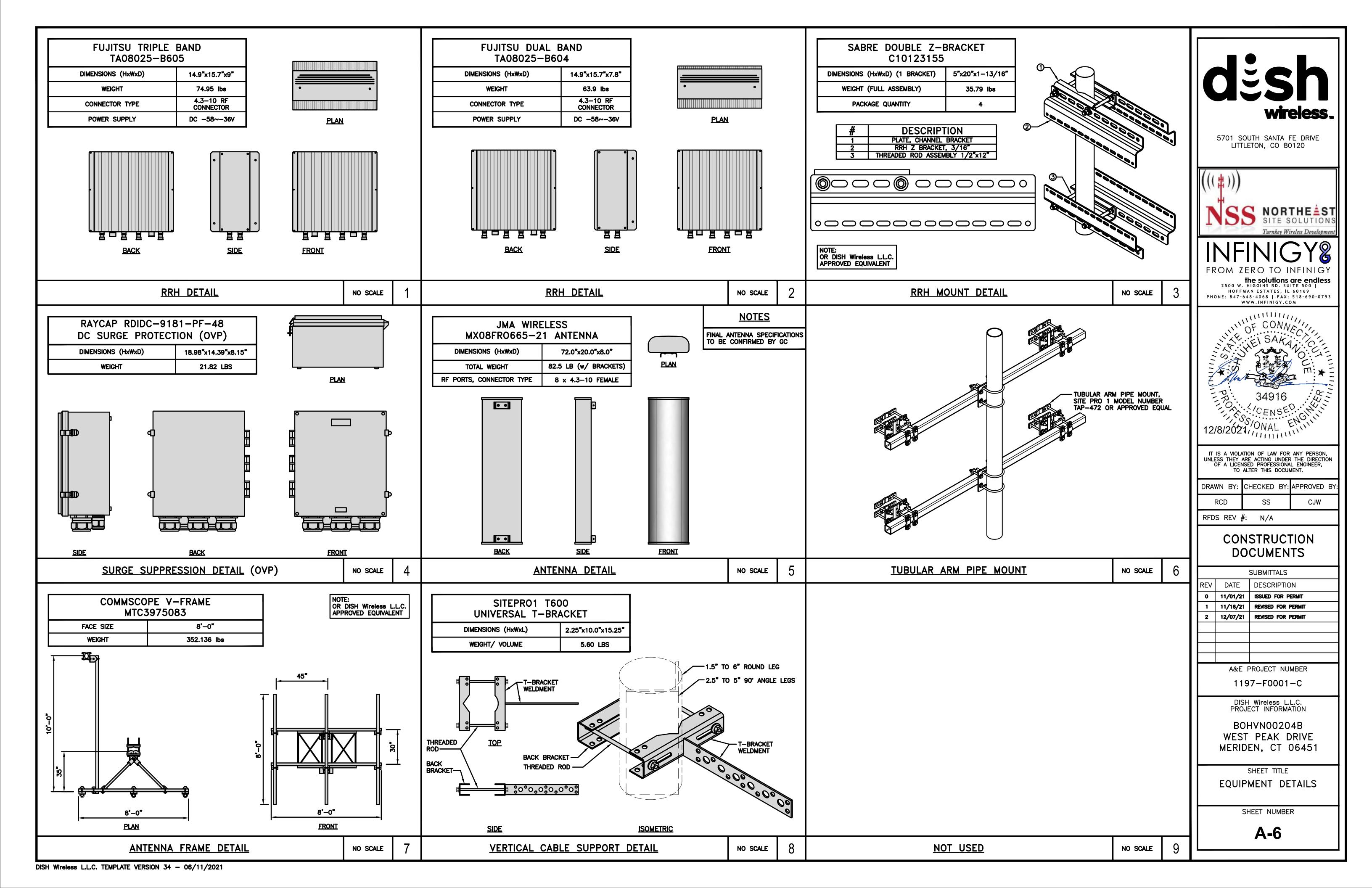
ELEVATION, ANTENNA

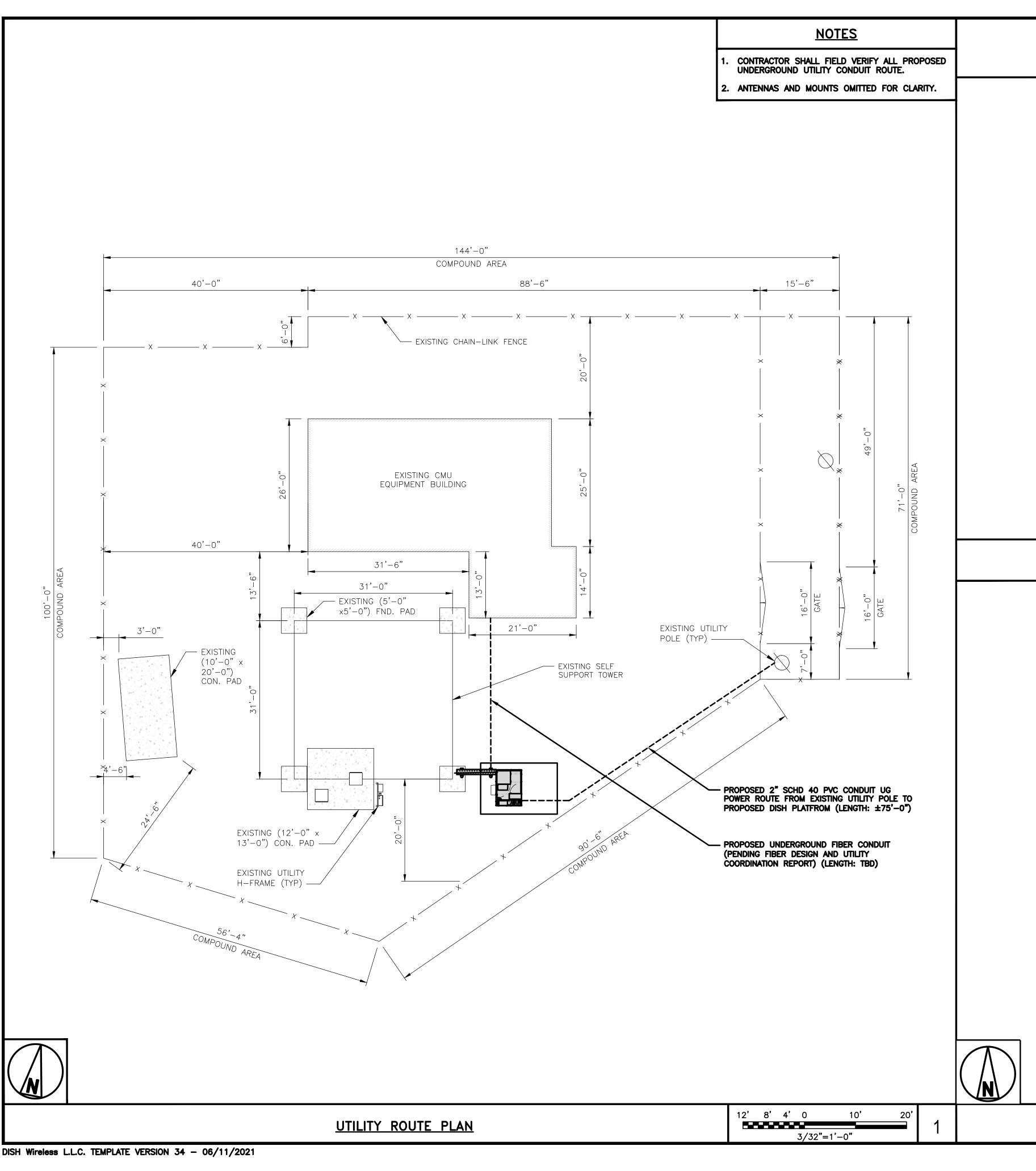
LAYOUT AND SCHEDULE







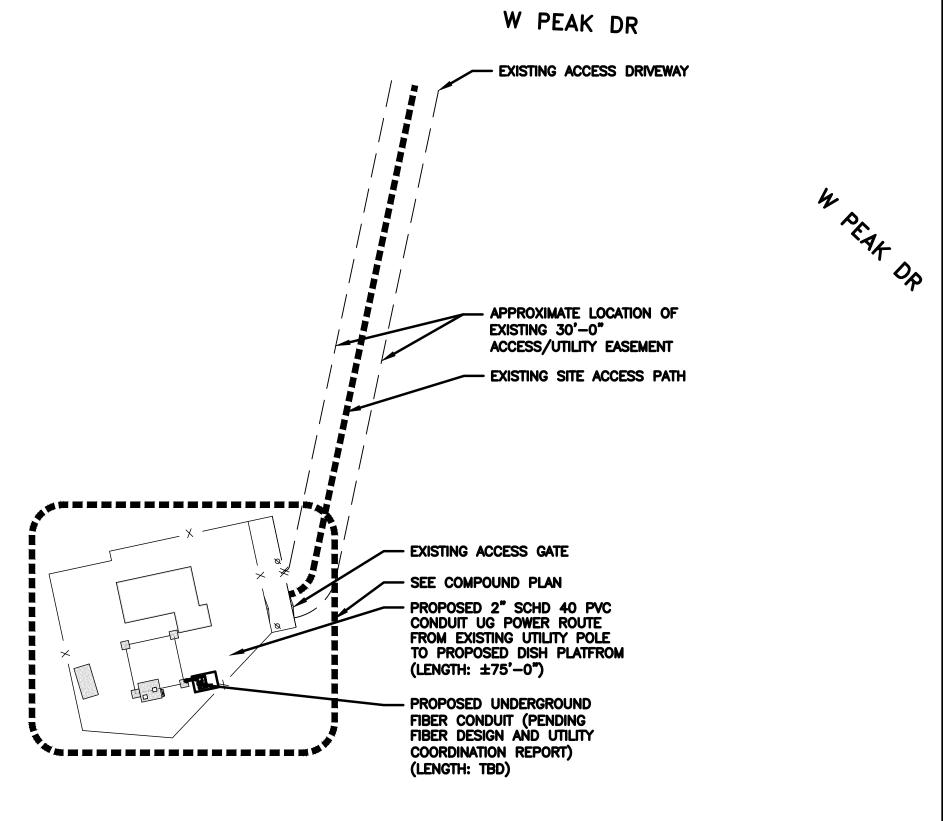




DC POWER WIRING SHALL BE COLOR CODED AT EACH END FOR IDENTIFYING +24V AND -48V CONDUCTORS. RED MARKINGS SHALL IDENTIFY +24V AND BLUE MARKINGS SHALL IDENTIFY -48V.

- 1. CONTRACTOR SHALL INSPECT THE EXISTING CONDITIONS PRIOR TO SUBMITTING A BID. ANY QUESTIONS ARISING DURING THE BID PERIOD IN REGARDS TO THE CONTRACTOR'S FUNCTIONS, THE SCOPE OF WORK, OR ANY OTHER ISSUE RELATED TO THIS PROJECT SHALL BE BROUGHT UP DURING THE BID PERIOD WITH THE PROJECT MANAGER FOR CLARIFICATION, NOT AFTER THE CONTRACT HAS BEEN AWARDED.
- 2. ALL ELECTRICAL WORK SHALL BE DONE IN ACCORDANCE WITH CURRENT NATIONAL ELECTRICAL CODES AND ALL STATE AND LOCAL CODES, LAWS, AND ORDINANCES. PROVIDE ALL COMPONENTS AND WIRING SIZES AS REQUIRED TO MEET NEC STANDARDS.
- 3. LOCATION OF EQUIPMENT, CONDUIT AND DEVICES SHOWN ON THE DRAWINGS ARE APPROXIMATE AND SHALL BE COORDINATED WITH FIELD CONDITIONS PRIOR TO CONSTRUCTION.
- 4. CONDUIT ROUGH-IN SHALL BE COORDINATED WITH THE MECHANICAL EQUIPMENT TO AVOID LOCATION CONFLICTS. VERIFY WITH THE MECHANICAL EQUIPMENT CONTRACTOR AND COMPLY AS REQUIRED.
- 5. CONTRACTOR SHALL PROVIDE ALL BREAKERS, CONDUITS AND CIRCUITS AS REQUIRED FOR A COMPLETE SYSTEM.
- 6. CONTRACTOR SHALL PROVIDE PULL BOXES AND JUNCTION BOXES AS REQUIRED BY THE NEC ARTICLE 314.
- 7. CONTRACTOR SHALL PROVIDE ALL STRAIN RELIEF AND CABLE SUPPORTS FOR ALL CABLE ASSEMBLIES. INSTALLATION SHALL BE IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS.
- 8. ALL DISCONNECTS AND CONTROLLING DEVICES SHALL BE PROVIDED WITH ENGRAVED PHENOLIC NAMEPLATES INDICATING EQUIPMENT CONTROLLED, BRANCH CIRCUITS INSTALLED ON, AND PANEL FIELD LOCATIONS FED FROM.
- 9. INSTALL AN EQUIPMENT GROUNDING CONDUCTOR IN ALL CONDUITS PER THE SPECIFICATIONS AND NEC 250. THE EQUIPMENT GROUNDING CONDUCTORS SHALL BE BONDED AT ALL JUNCTION BOXES, PULL BOXES, AND ALL DISCONNECT SWITCHES, AND EQUIPMENT CABINETS.
- 10. ALL NEW MATERIAL SHALL HAVE A U.L. LABEL.
- 11. PANEL SCHEDULE LOADING AND CIRCUIT ARRANGEMENTS REFLECT POST—CONSTRUCTION EQUIPMENT.
- 12. CONTRACTOR SHALL BE RESPONSIBLE FOR AS-BUILT PANEL SCHEDULE AND SITE DRAWINGS.
- 13. ALL TRENCHES IN COMPOUND TO BE HAND DUG

**ELECTRICAL NOTES** 



60' 30' 0

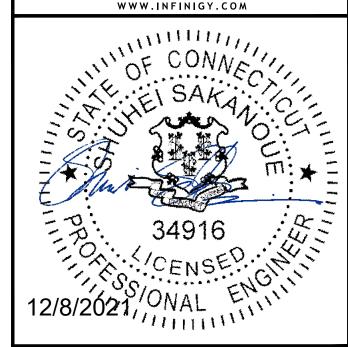
1"=60'

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	RCD	SS		CJW	
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CONSTRUCTION

**DOCUMENTS** 

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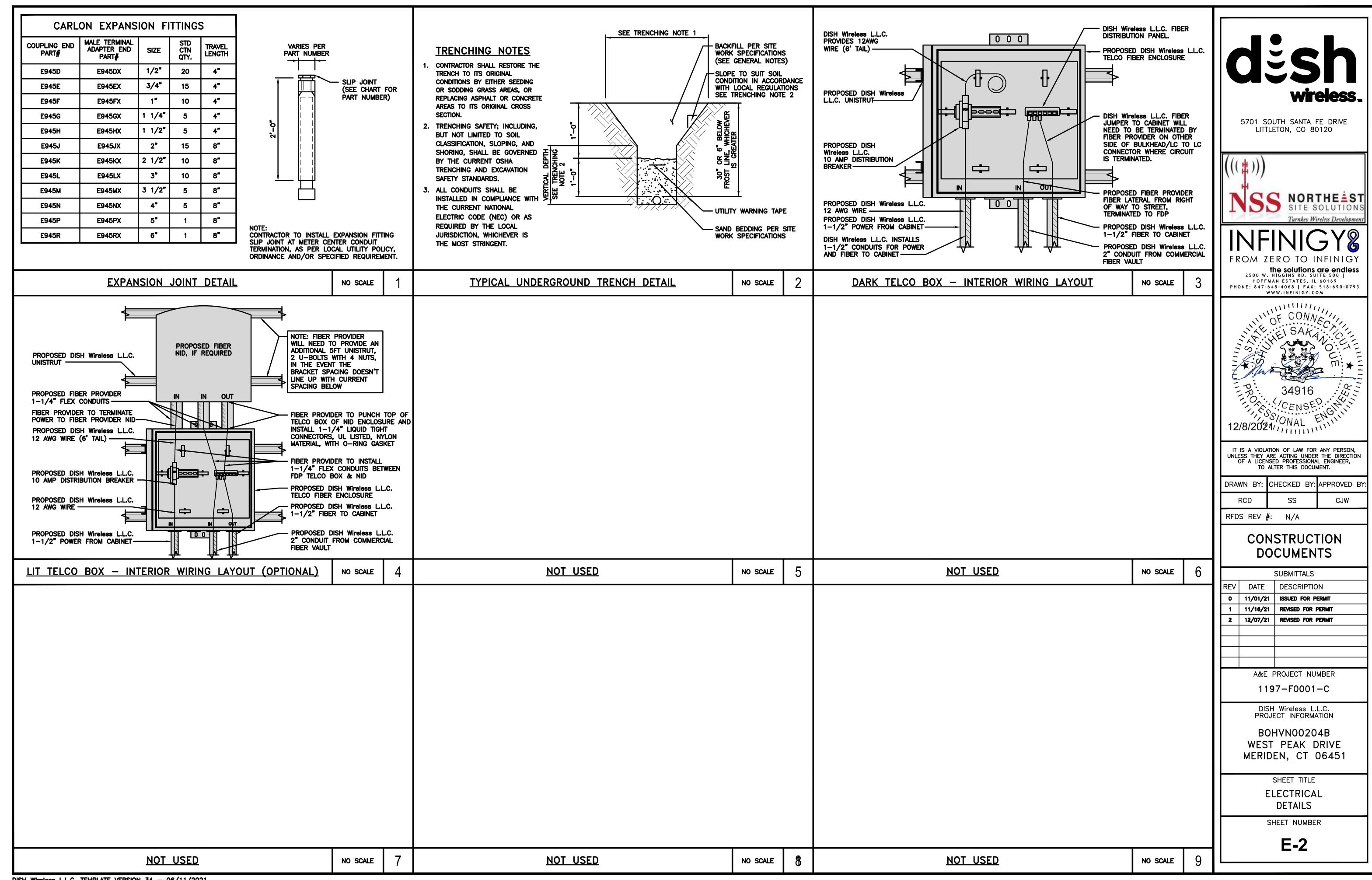
BOHVN00204B WEST PEAK DRIVE MERIDEN, CT 06451

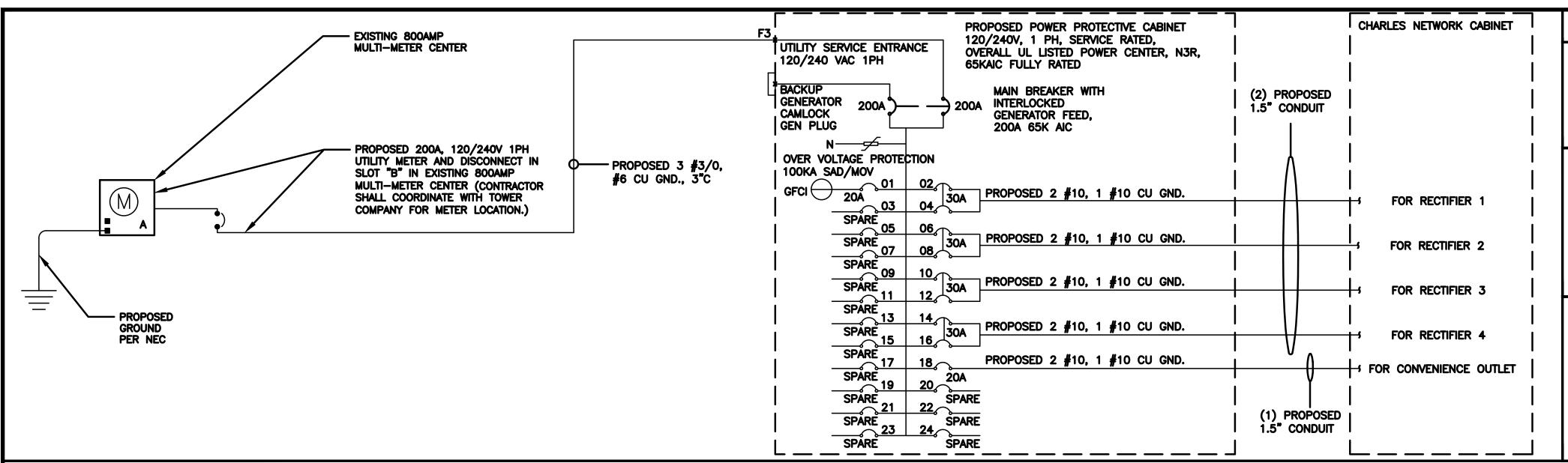
SHEET TITLE ELECTRICAL/FIBER ROUTE PLAN AND NOTES

SHEET NUMBER

E-1

OVERALL UTILITY ROUTE PLAN





# **NOTES**

THERE ARE A TOTAL OF (10) CURRENT CARRYING CONDUCTORS IN A SINGLE CONDUIT. ADJUSTABLE FACTOR OF 50% PER NEC TABLE 310.15(B)(3)(a) SHALL

> #10 FOR 15A/1P BREAKER:  $0.5 \times 40A = 15.0A$ #8 FOR 20A-25A/2P BREAKER: 0.5 x 55A = 27.5A

CONDUIT SIZING: ASSUME 1.5" EMT AT 40% FILL PER NEC 358, TABLE 4 - 0.814A SQ. IN AREA WIRES: USING THWN-2. CU. (INCLUDING 3 GROUND WIRES)

#6 - 0.0507 SQ. IN X 8 = 0.4056 SQ. IN #8 - 0.0366 SQ. IN X 2 = 0.0732 SQ. IN #10 - 0.0211 SQ. IN X 4 = 0.0844 SQ. IN <GROUND

#12 - 0.0133 SQ. IN X 1 = 0.0133 SQ. IN <GROUND = 0.5765 SQ. IN

1.5" EMT CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OR (15) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

CONDUIT SIZING: ASSUME 3.0" SCH 40 PVC AT 40% FILL PER NEC 352, TABLE 4 - 1.216A SQ. IN AREA

USING THHN, CU. (INCLUDING 2 GROUND WIRES)

#3/0 - 0.1318 SQ. IN X 3 = 0.3954 SQ. IN #2 - 0.0521 SQ. IN X 1 = 0.0521 SQ. IN

= 0.4475 SQ. IN

3.0" EMT CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OR (3) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

(CHARLES ABB GE INFINITY DC PLANT) WITH MULTI-METER CENTER 120V240V 1PH SOURCE

NO SCALE

PROPOSED PANEL SCHEDULE												
LOAD SERVED VOLT AMPS (WATTS)  TRIP CKT # PHASE CKT # TRIP (WATTS)  LOAD SERVED										LOAD SERVED		
CDADE	L1	L2				<b>A</b> .	$\Rightarrow$			L1	L2	
-SPARE- -SPARE-				3		B	壮	2 4	30A	2880	2880	ABB/GE INFINITY RECTIFIER 1
-SPARE-				5	<b>\</b>	A	以	6	30A	2880	2880	ABB/GE INFINITY RECTIFIER 2
-SPARE- -SPARE-				9	<b>\</b>	A	以	10 12	30A	2880	2880	ABB/GE INFINITY RECTIFIER 3
-SPARE-				13	$\searrow$	Ā	对	14	30A	2880		ABB/GE INFINITY
-SPARE- -SPARE-				15 17	K	B	갋	16 18	20A	1920	2880	RÉCTIFIER 4 CHARLES GFCI OUTLET
-SPARE-				19		B	_	20				-SPARE-
-SPARE- -SPARE-				21 23	$\sum$	AB	_	22 24				-SPARE- -SPARE-
VOLT AMPS				23		ВГ		24		13440	11520	-SPARE-
200A MCB, 1¢, 3W,	120/24	OV.	L1			L2				70 770	77525	
MB RATING: 65,000			134	40	1	1520	一	VOL	T AMPS	5		
			140			96		AMP				
					40 75				AMPS 125%			

PANEL SCHEDULE (CHARLES ABB GE INFINITY DC PLANT) WITH MULTI-METER CENTER 120V240V 1PH SOURCE

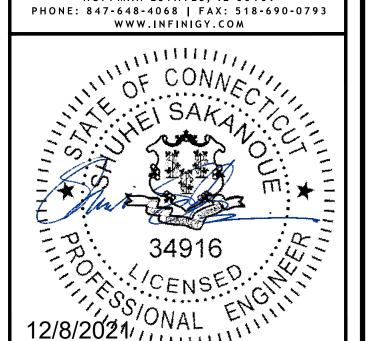
NO SCALE

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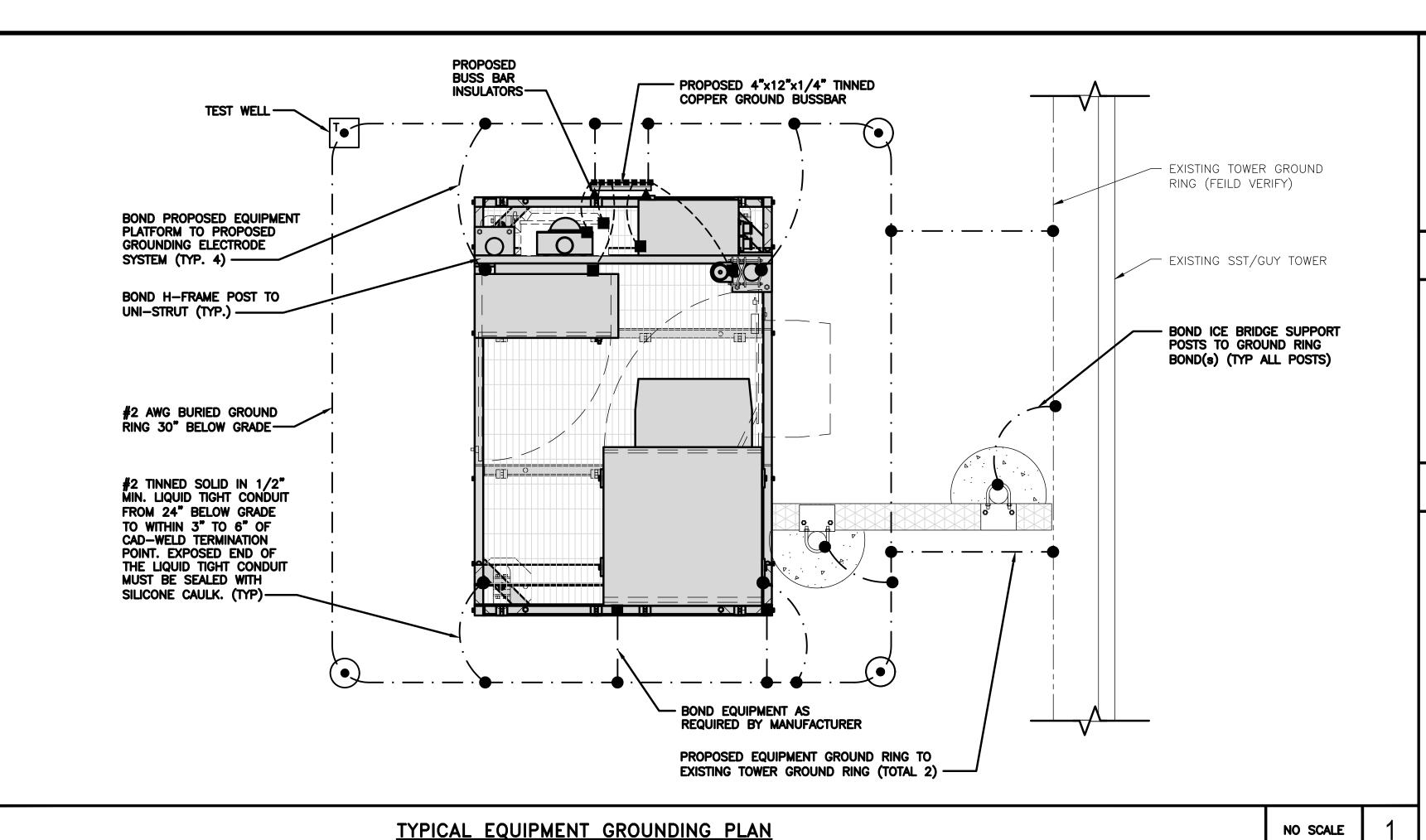
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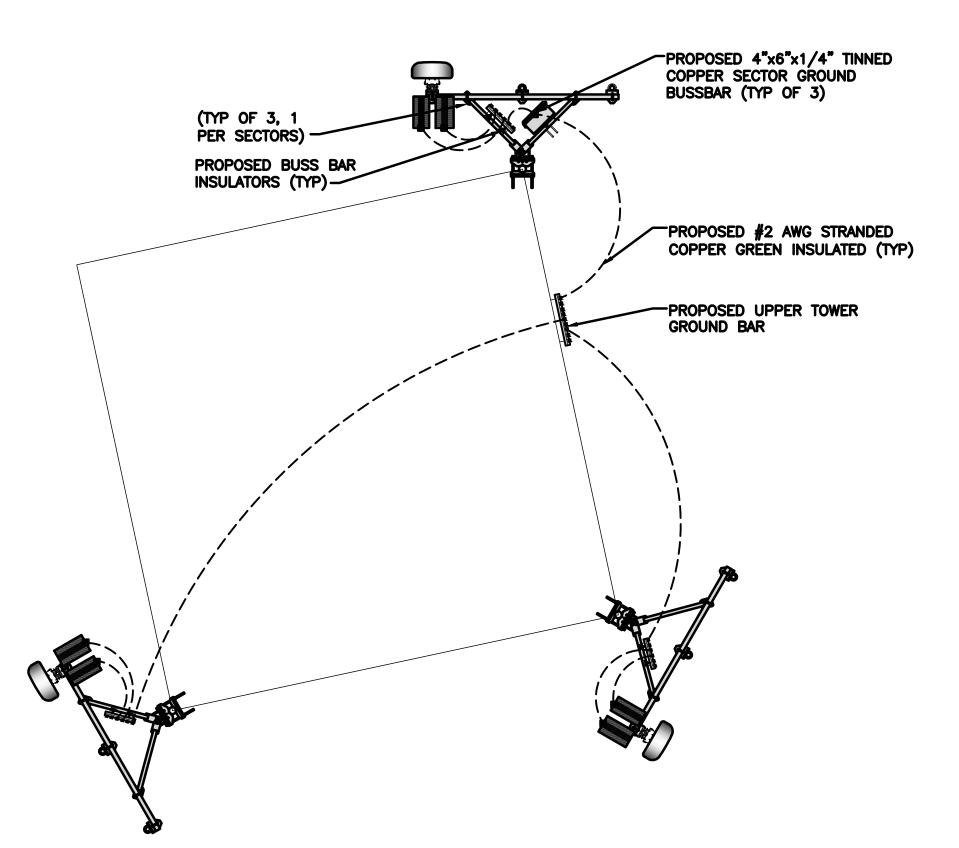
ELECTRICAL ONE-LINE, FAULT CALCS & PANEL SCHEDULE

SHEET NUMBER

**E-3** 

NOT USED **FAULT CALCULATIONS** NO SCALE NO SCALE





TYPICAL ANTENNA GROUNDING PLAN

**NOTES** 

ANTENNAS AND OVP SHOWN ARE GENERIC AND NOT REFERENCING TO A SPECIFIC MANUFACTURER. THIS LAYOUT IS FOR REFERENCE EXOTHERMIC CONNECTION TEST GROUND ROD WITH INSPECTION SLEEVE MECHANICAL CONNECTION #6 AWG STRANDED & INSULATED

GROUND BUS BAR

GROUND ROD

#2 AWG SOLID COPPER TINNED

▲ BUSS BAR INSULATOR

# **GROUNDING LEGEND**

- 1. GROUNDING IS SHOWN DIAGRAMMATICALLY ONLY.
- 2. CONTRACTOR SHALL GROUND ALL EQUIPMENT AS A COMPLETE SYSTEM. GROUNDING SHALL BE IN COMPLIANCE WITH NEC SECTION 250 AND DISH Wireless L.L.C. GROUNDING AND BONDING REQUIREMENTS AND MANUFACTURER'S SPECIFICATIONS.
- 3. ALL GROUND CONDUCTORS SHALL BE COPPER; NO ALUMINUM CONDUCTORS SHALL BE USED.

# **GROUNDING KEY NOTES**

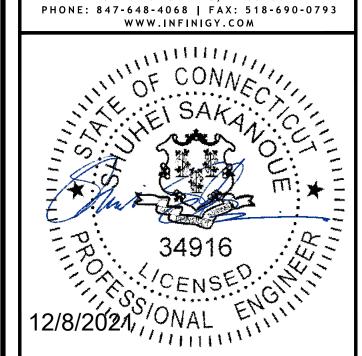
- A EXTERIOR GROUND RING: #2 AWG SOLID COPPER, BURIED AT A DEPTH OF AT LEAST 30 INCHES BELOW GRADE, OR 6 INCHES BELOW THE FROST LINE AND APPROXIMATELY 24 INCHES FROM THE EXTERIOR WALL OR FOOTING.
- TOWER GROUND RING: THE GROUND RING SYSTEM SHALL BE INSTALLED AROUND AN ANTENNA TOWER'S LEGS, AND/OR GUY ANCHORS. WHERE SEPARATE SYSTEMS HAVE BEEN PROVIDED FOR THE TOWER AND THE BUILDING, AT LEAST TWO BONDS SHALL BE MADE BETWEEN THE TOWER RING GROUND SYSTEM AND THE BUILDING RING GROUND SYSTEM USING MINIMUM #2 AWG SOLID COPPER CONDUCTORS.
- C INTERIOR GROUND RING: #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTOR EXTENDED AROUND THE PERIMETER OF THE EQUIPMENT AREA. ALL NON-TELECOMMUNICATIONS BELATED METALLIC OF LEGES FOLLOWS. PERIMETER OF THE EQUIPMENT AREA. ALL NON-TELECOMMUNICATIONS RELATED METALLIC OBJECTS FOUND WITHIN A SITE SHALL BE GROUNDED TO THE INTERIOR GROUND RING WITH #6 AWG STRANDED GREEN INSULATED CONDUCTOR.
- BOND TO INTERIOR GROUND RING: #2 AWG SOLID TINNED COPPER WIRE PRIMARY BONDS SHALL BE PROVIDED AT LEAST AT FOUR POINTS ON THE INTERIOR GROUND RING, LOCATED AT THE CORNERS OF THE
- E GROUND ROD: UL LISTED COPPER CLAD STEEL. MINIMUM 1/2" DIAMETER BY EIGHT FEET LONG. GROUND RODS SHALL BE INSTALLED WITH INSPECTION SLEEVES. GROUND RODS SHALL BE DRIVEN TO THE DEPTH OF GROUND ROD: UL LISTED COPPER CLAD STEEL. MINIMUM 1/2" DIAMETER BY EIGHT FEET LONG. GROUND GROUND RING CONDUCTOR.
- CELL REFERENCE GROUND BAR: POINT OF GROUND REFERENCE FOR ALL COMMUNICATIONS EQUIPMENT FRAMES. ALL BONDS ARE MADE WITH #2 AWG LINESCE MOTES OF THE PROPERTY OF FRAMES. ALL BONDS ARE MADE WITH #2 AWG UNLESS NOTED OTHERWISE STRANDED GREEN INSULATED COPPER CONDUCTORS. BOND TO GROUND RING WITH (2) #2 SOLID TINNED COPPER CONDUCTORS.
- HATCH PLATE GROUND BAR: BOND TO THE INTERIOR GROUND RING WITH TWO #2 AWG STRANDED GREEN G HATCH PLATE GROUND BAR: BOND TO THE INTERIOR GROUND RING WITH TWO #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTORS. WHEN A HATCH-PLATE AND A CELL REFERENCE GROUND BAR ARE BOTH PRESENT, THE CRGB MUST BE CONNECTED TO THE HATCH-PLATE AND TO THE INTERIOR GROUND RING USING (2) TWO #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTORS EACH.
- EXTERIOR CABLE ENTRY PORT GROUND BARS: LOCATED AT THE ENTRANCE TO THE CELL SITE BUILDING. BOND TO GROUND RING WITH A #2 AWG SOLID TINNED COPPER CONDUCTORS WITH AN EXOTHERMIC WELD AND INSPECTION SLEEVE.
- ( | ) TELCO GROUND BAR: BOND TO BOTH CELL REFERENCE GROUND BAR OR EXTERIOR GROUND RING.
- FRAME BONDING: THE BONDING POINT FOR TELECOM EQUIPMENT FRAMES SHALL BE THE GROUND BUS THAT IS NOT ISOLATED FROM THE EQUIPMENTS METAL FRAMEWORK.
- K INTERIOR UNIT BONDS: METAL FRAMES, CABINETS AND INDIVIDUAL METALLIC UNITS LOCATED WITH THE AREA OF THE INTERIOR GROUND RING REQUIRE A #6 AWG STRANDED GREEN INSULATED COPPER BOND TO THE INTERIOR GROUND RING.
- L FENCE AND GATE GROUNDING: METAL FENCES WITHIN 7 FEET OF THE EXTERIOR GROUND RING OR OBJECTS BONDED TO THE EXTERIOR GROUND RING SHALL BE BONDED TO THE GROUND RING WITH A #2 AWG SOLID TINNED COPPER CONDUCTOR AT AN INTERVAL NOT EXCEEDING 25 FEET. BONDS SHALL BE MÄDE AT EACH GATE POST AND ACROSS GATE OPENINGS.
- EXTERIOR UNIT BONDS: METALLIC OBJECTS, EXTERNAL TO OR MOUNTED TO THE BUILDING, SHALL BE BONDED TO THE EXTERIOR GROUND RING. USING #2 TINNED SOLID COPPER WIRE
- N ICE BRIDGE SUPPORTS: EACH ICE BRIDGE LEG SHALL BE BONDED TO THE GROUND RING WITH #2 AWG BARE TINNED COPPER CONDUCTOR. PROVIDE EXOTHERMIC WELDS AT BOTH THE ICE BRIDGE LEG AND BURIED GROUND RING.
- DURING ALL DC POWER SYSTEM CHANGES INCLUDING DC SYSTEM CHANGE OUTS, RECTIFIER REPLACEMENTS OR ADDITIONS, BREAKER DISTRIBUTION CHANGES, BATTERY ADDITIONS, BATTERY REPLACEMENTS AND INSTALLATIONS OR CHANGES TO DC CONVERTER SYSTEMS IT SHALL BE REQUIRED THAT SERVICE CONTRACTORS VERIFY ALL DC POWER SYSTEMS ARE EQUIPPED WITH A MASTER DC SYSTEM RETURN GROUND CONDUCTOR FROM THE DC POWER SYSTEM COMMON RETURN BUS DIRECTLY CONNECTED TO THE CELL SITE REFERENCE GROUND BAR
- (P) TOWER TOP COLLECTOR BUSS BAR IS TO BE MECHANICALLY BONDED TO PROPOSED ANTENNA MOUNT COLLAR. REFER TO DISH Wireless L.L.C. GROUNDING NOTES.

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SHEET TITLE GROUNDING PLANS AND NOTES

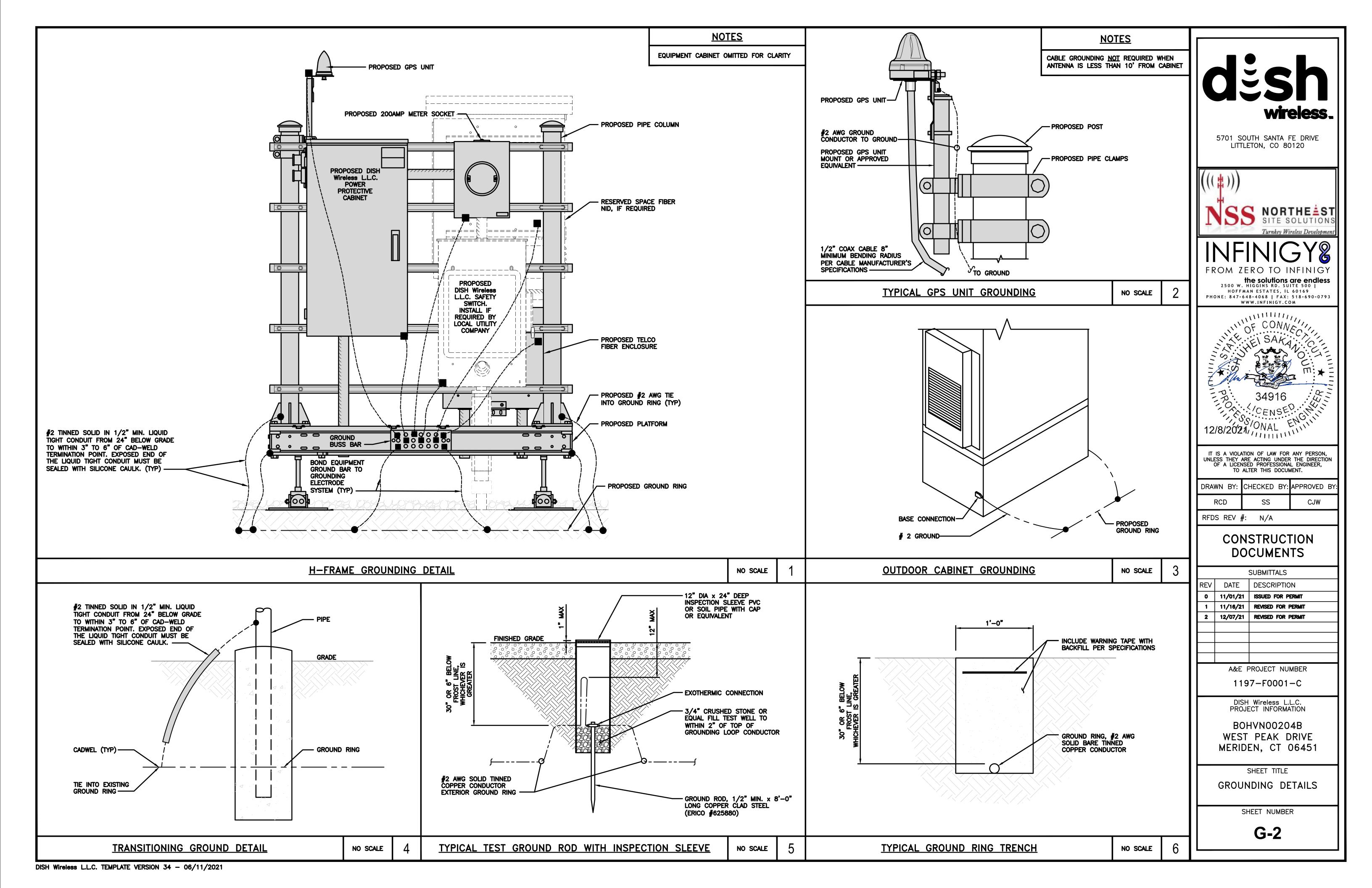
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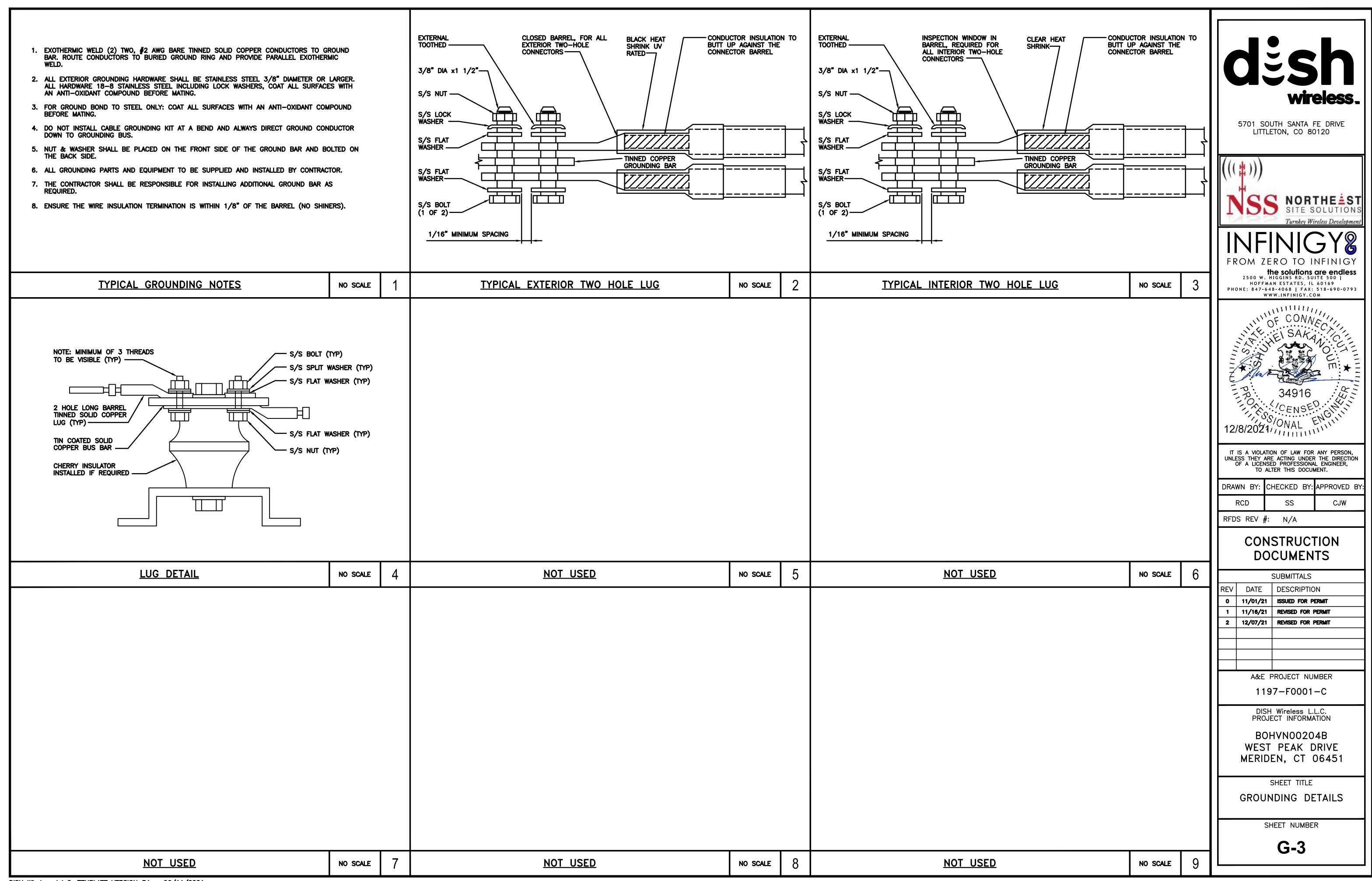
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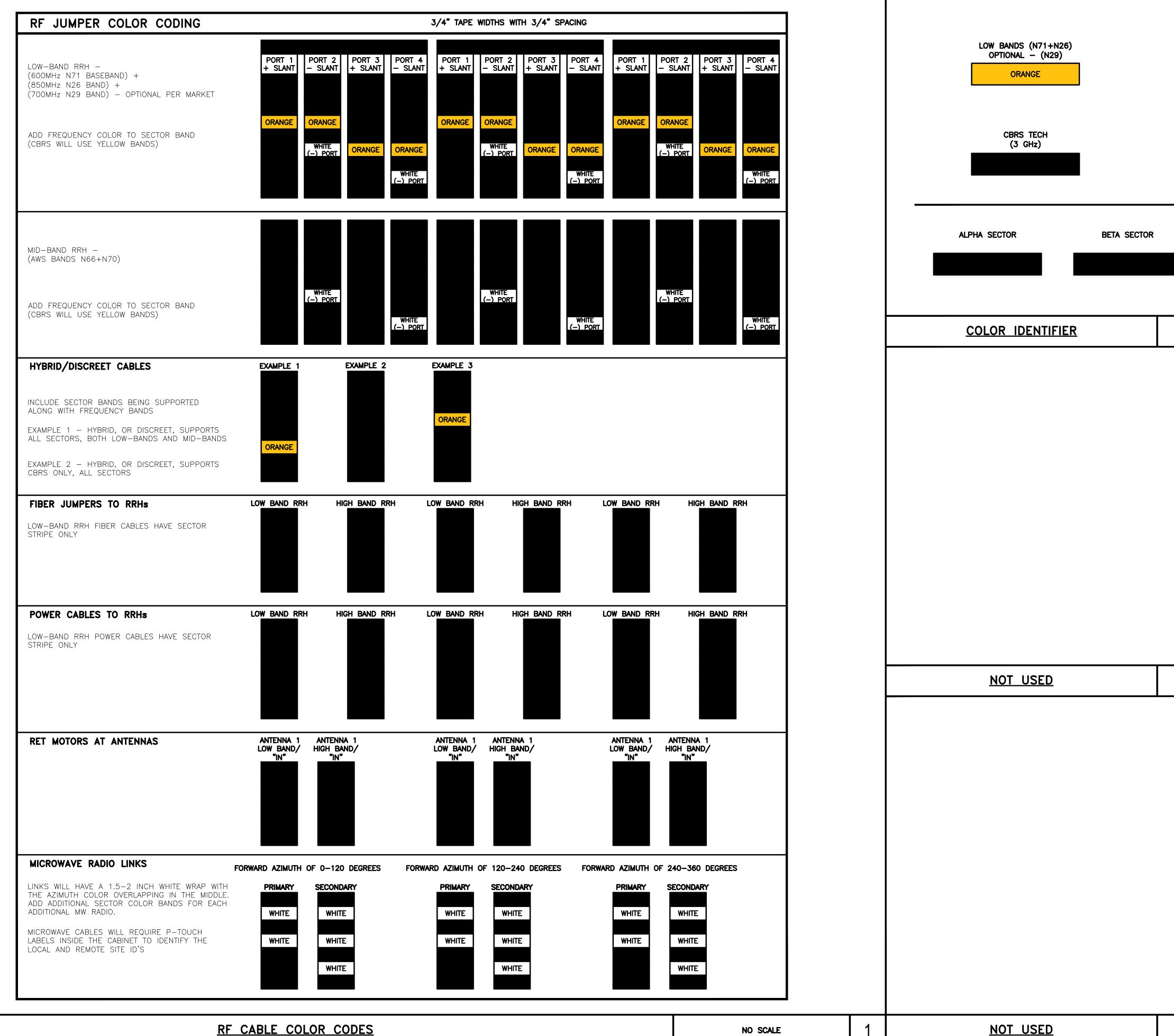
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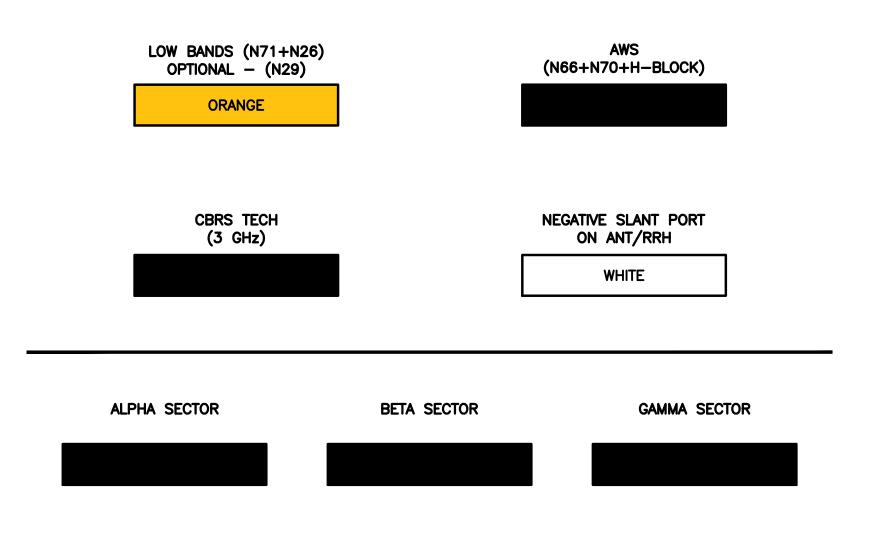
**GROUNDING KEY NOTES** 

NO SCALE









NO SCALE

NO SCALE

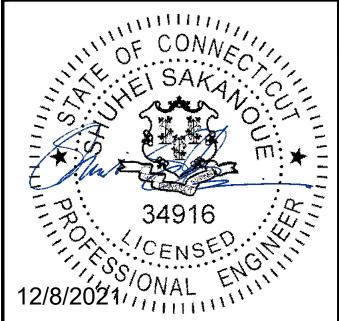
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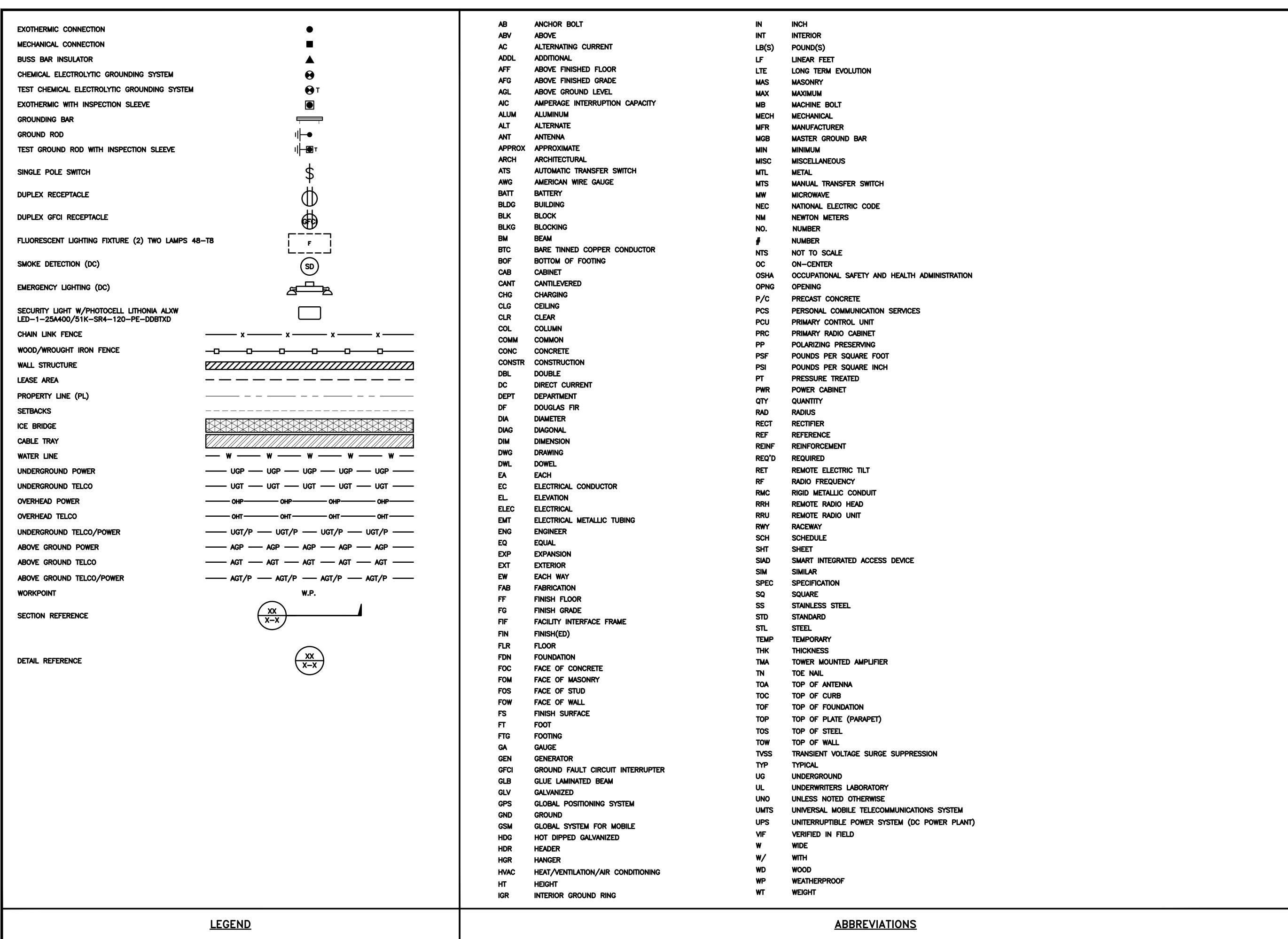
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SHEET TITLE CABLE COLOR CODES

SHEET NUMBER

RF-1



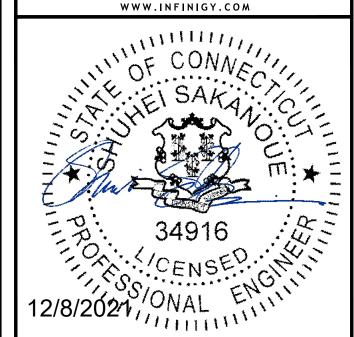


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SHEET TITLE

LEGEND AND

ABBREVIATIONS

SHEET NUMBER

## **SITE ACTIVITY REQUIREMENTS:**

- 1. NOTICE TO PROCEED NO WORK SHALL COMMENCE PRIOR TO CONTRACTOR RECEIVING A WRITTEN NOTICE TO PROCEED (NTP) AND THE ISSUANCE OF A PURCHASE ORDER. PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE DISH Wireless L.L.C. AND TOWER OWNER NOC & THE DISH Wireless L.L.C. AND TOWER CONSTRUCTION MANAGER.
- 2. "LOOK UP" DISH Wireless L.L.C. AND TOWER OWNER SAFETY CLIMB REQUIREMENT:

THE INTEGRITY OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY ON THE STRUCTURE. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO: PINCHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM ITS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTIONAL WEAR, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO IMPEDE/BLOCK ITS INTENDED USE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGGED OUT AND REPORTED TO YOUR DISH Wireless L.L.C. AND DISH Wireless L.L.C. AND TOWER OWNER POC OR CALL THE NOC TO GENERATE A SAFETY CLIMB MAINTENANCE AND CONTRACTOR NOTICE TICKET.

- 3. PRIOR TO THE START OF CONSTRUCTION, ALL REQUIRED JURISDICTIONAL PERMITS SHALL BE OBTAINED. THIS INCLUDES, BUT IS NOT LIMITED TO, BUILDING, ELECTRICAL, MECHANICAL, FIRE, FLOOD ZONE, ENVIRONMENTAL, AND ZONING. AFTER ONSITE ACTIVITIES AND CONSTRUCTION ARE COMPLETED, ALL REQUIRED PERMITS SHALL BE SATISFIED AND CLOSED OUT ACCORDING TO LOCAL JURISDICTIONAL REQUIREMENTS.
- 4. ALL CONSTRUCTION MEANS AND METHODS; INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS, AND RESCUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONTAINED HEREIN, AND SHALL MEET ANSI/ASSE A10.48 (LATEST EDITION); FEDERAL, STATE, AND LOCAL REGULATIONS; AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL ADHERE TO ANSI/ASSE A10.48 (LATEST EDITION) AND DISH Wireless L.L.C. AND TOWER OWNER STANDARDS, INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION, TO CERTIFY THE SUPPORTING STRUCTURE(S) IN ACCORDANCE WITH ANSI/TIA—322 (LATEST EDITION).
- 5. ALL SITE WORK TO COMPLY WITH DISH Wireless L.L.C. AND TOWER OWNER INSTALLATION STANDARDS FOR CONSTRUCTION ACTIVITIES ON DISH Wireless L.L.C. AND TOWER OWNER TOWER SITE AND LATEST VERSION OF ANSI/TIA-1019-A-2012 "STANDARD FOR INSTALLATION, ALTERATION, AND MAINTENANCE OF ANTENNA SUPPORTING STRUCTURES AND ANTENNAS."
- 6. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY DISH Wireless L.L.C. AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- 7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 8. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 9. THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES INCLUDING PRIVATE LOCATES SERVICES PRIOR TO THE START OF CONSTRUCTION.
- 10. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION E) CONSTRUCTION SAFETY PROCEDURES.
- 11. ALL SITE WORK SHALL BE AS INDICATED ON THE STAMPED CONSTRUCTION DRAWINGS AND DISH PROJECT SPECIFICATIONS, LATEST APPROVED REVISION.
- 12. CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH AT THE COMPLETION OF THE WORK. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
- 13. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF DISH Wireless L.L.C. AND TOWER OWNER, AND/OR LOCAL UTILITIES.
- 14. THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE TECHNICAL SPECIFICATION FOR SITE SIGNAGE REQUIRED BY LOCAL JURISDICTION AND SIGNAGE REQUIRED ON INDIVIDUAL PIECES OF EQUIPMENT, ROOMS, AND SHELTERS.
- 15. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE CARRIER'S EQUIPMENT AND TOWER AREAS.
- 16. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
- 17. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION AS SPECIFIED ON THE CONSTRUCTION DRAWINGS AND/OR PROJECT SPECIFICATIONS.
- 18. CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.
- 19. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.
- 20. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS AND RADIOS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 21. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.
- 22. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.

## **GENERAL NOTES:**

1.FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:

CONTRACTOR: GENERAL CONTRACTOR RESPONSIBLE FOR CONSTRUCTION

CARRIER: DISH Wireless L.L.C.

TOWER OWNER:TOWER OWNER

- 2. THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE ENGINEERS IN THIS OR SIMILAR LOCALITIES. IT IS ASSUMED THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKPEOPLE WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.
- 3. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY FOR PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, FORMWORK, SHORING, ETC. SITE VISITS BY THE ENGINEER OR HIS REPRESENTATIVE WILL NOT INCLUDE INSPECTION OF THESE ITEMS AND IS FOR STRUCTURAL OBSERVATION OF THE FINISHED STRUCTURE ONLY.
- 4. NOTES AND DETAILS IN THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE GREATER, MORE STRICT REQUIREMENTS, SHALL GOVERN. IF FURTHER CLARIFICATION IS REQUIRED CONTACT THE ENGINEER OF RECORD.
- 5. SUBSTANTIAL EFFORT HAS BEEN MADE TO PROVIDE ACCURATE DIMENSIONS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.
- 6. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CARRIER POC AND TOWER OWNER.
- 7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 8. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
- 9. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 10. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CARRIER AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- 11. CONTRACTOR IS TO PERFORM A SITE INVESTIGATION, BEFORE SUBMITTING BIDS, TO DETERMINE THE BEST ROUTING OF ALL CONDUITS FOR POWER, AND TELCO AND FOR GROUNDING CABLES AS SHOWN IN THE POWER, TELCO, AND GROUNDING PLAN DRAWINGS.
- 12. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF DISH Wireless L.L.C. AND TOWER OWNER
- 13. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 4. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY ASIS.

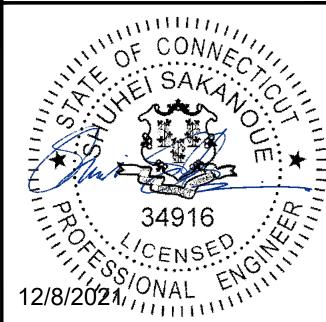


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	DRAWN	BY:	CHECKED	BY:	APPROVED	BY
	RCE	)	SS		CJW	

RFDS REV #: N/A

# CONSTRUCTION DOCUMENTS

SUBMITTALS					
REV	DATE	DESCRIPTION			
0	11/01/21	ISSUED FOR PERMIT			
1	11/16/21	REVISED FOR PERMIT			
2 12/07/21		REVISED FOR PERMIT			
·	A&E F	PROJECT NUMBER			
i					

DISH Wireless L.L.C.

PROJECT INFORMATION

1197-F0001-C

BOHVN00204B WEST PEAK DRIVE MERIDEN, CT 06451

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

## CONCRETE, FOUNDATIONS, AND REINFORCING STEEL:

- 1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST—IN—PLACE CONCRETE.
- 2. UNLESS NOTED OTHERWISE, SOIL BEARING PRESSURE USED FOR DESIGN OF SLABS AND FOUNDATIONS IS ASSUMED TO BE 1000 psf.
- 3. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f'c) OF 3000 psi AT 28 DAYS, UNLESS NOTED OTHERWISE. NO MORE THAN 90 MINUTES SHALL ELAPSE FROM BATCH TIME TO TIME OF PLACEMENT UNLESS APPROVED BY THE ENGINEER OF RECORD. TEMPERATURE OF CONCRETE SHALL NOT EXCEED 90°F AT TIME OF PLACEMENT.
- 4. CONCRETE EXPOSED TO FREEZE—THAW CYCLES SHALL CONTAIN AIR ENTRAINING ADMIXTURES. AMOUNT OF AIR ENTRAINMENT TO BE BASED ON SIZE OF AGGREGATE AND F3 CLASS EXPOSURE (VERY SEVERE). CEMENT USED TO BE TYPE II PORTLAND CEMENT WITH A MAXIMUM WATER—TO—CEMENT RATIO (W/C) OF 0.45.
- 5. ALL STEEL REINFORCING SHALL CONFORM TO ASTM A615. ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185. ALL SPLICES SHALL BE CLASS "B" TENSION SPLICES, UNLESS NOTED OTHERWISE. ALL HOOKS SHALL BE STANDARD 90 DEGREE HOOKS, UNLESS NOTED OTHERWISE. YIELD STRENGTH (Fy) OF STANDARD DEFORMED BARS ARE AS FOLLOWS:

#4 BARS AND SMALLER 40 ksi

#5 BARS AND LARGER 60 ksi

- 6. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:
- CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3"
- CONCRETE EXPOSED TO EARTH OR WEATHER:
- #6 BARS AND LARGER 2"
- #5 BARS AND SMALLER 1-1/2"
- CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
- SLAB AND WALLS 3/4"
- BEAMS AND COLUMNS 1-1/2"
- 7. A TOOLED EDGE OR A 3/4" CHAMFER SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNLESS NOTED OTHERWISE, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.

## **ELECTRICAL INSTALLATION NOTES:**

- 1. ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES/ORDINANCES.
- 2. CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED AND TRIP HAZARDS ARE ELIMINATED.
- 3. WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC.
- 4. ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC.
- 4.1. ALL EQUIPMENT SHALL BEAR THE UNDERWRITERS LABORATORIES LABEL OF APPROVAL, AND SHALL CONFORM TO REQUIREMENT OF THE NATIONAL ELECTRICAL CODE.
- 4.2. ALL OVERCURRENT DEVICES SHALL HAVE AN INTERRUPTING CURRENT RATING THAT SHALL BE GREATER THAN THE SHORT CIRCUIT CURRENT TO WHICH THEY ARE SUBJECTED, 22,000 AIC MINIMUM. VERIFY AVAILABLE SHORT CIRCUIT CURRENT DOES NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRENT ADOPTED CODE PRE THE GOVERNING JURISDICTION.
- 5. EACH END OF EVERY POWER PHASE CONDUCTOR, GROUNDING CONDUCTOR, AND TELCO CONDUCTOR OR CABLE SHALL BE LABELED WITH COLOR—CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2" PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA.
- 6. ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH LAMICOID TAGS SHOWING THEIR RATED VOLTAGE, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING AND BRANCH CIRCUIT ID NUMBERS (i.e. PANEL BOARD AND CIRCUIT ID'S).
- 7. PANEL BOARDS (ID NUMBERS) SHALL BE CLEARLY LABELED WITH PLASTIC LABELS.
- 8. TIE WRAPS ARE NOT ALLOWED.
- 9. ALL POWER AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE COPPER CONDUCTOR (#14 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 10. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE COPPER CONDUCTOR (#6 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 11. POWER AND CONTROL WIRING IN FLEXIBLE CORD SHALL BE MULTI-CONDUCTOR, TYPE SOOW CORD (#14 OR LARGER) UNLESS OTHERWISE SPECIFIED.
- 12. POWER AND CONTROL WIRING FOR USE IN CABLE TRAY SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#14 OR LARGER), WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 13. ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP—STYLE, COMPRESSION WIRE LUGS AND WIRE NUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRE NUTS SHALL BE RATED FOR OPERATION NOT LESS THAN 75° C (90° C IF AVAILABLE).
- 14. RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.
- 15. ELECTRICAL METALLIC TUBING (EMT), INTERMEDIATE METAL CONDUIT (IMC), OR RIGID METAL CONDUIT (RMC) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.

- . ELECTRICAL METALLIC TUBING (EMT) OR METAL—CLAD CABLE (MC) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.
- 17. SCHEDULE 40 PVC UNDERGROUND ON STRAIGHTS AND SCHEDULE 80 PVC FOR ALL ELBOWS/90s AND ALL APPROVED ABOVE GRADE PVC CONDUIT.
- 18. LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.
- 19. CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION—TYPE AND APPROVED FOR THE LOCATION USED. SET SCREW FITTINGS ARE NOT ACCEPTABLE.
- 20. CABINETS, BOXES AND WIRE WAYS SHALL BE LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND THE NEC.
- 21. WIREWAYS SHALL BE METAL WITH AN ENAMEL FINISH AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARDS (WIREMOLD SPECMATE WIREWAY).
- 22. SLOTTED WIRING DUCT SHALL BE PVC AND INCLUDE COVER (PANDUIT TYPE E OR EQUAL).
- 23. CONDUITS SHALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-PERFORATED STRAPS AND HANGERS. EXPLOSIVE DEVICES (i.e. POWDER-ACTUATED) FOR ATTACHING HANGERS TO STRUCTURE WILL NOT BE PERMITTED. CLOSELY FOLLOW THE LINES OF THE STRUCTURE, MAINTAIN CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES. CHANGES IN DIRECTION TO ROUTE AROUND OBSTACLES SHALL BE MADE WITH CONDUIT OUTLET BODIES. CONDUIT SHALL BE INSTALLED IN A NEAT AND WORKMANLIKE MANNER. PARALLEL AND PERPENDICULAR TO STRUCTURE WALL AND CEILING LINES. ALL CONDUIT SHALL BE FISHED TO CLEAR OBSTRUCTIONS. ENDS OF CONDUITS SHALL BE TEMPORARILY CAPPED FLUSH TO FINISH GRADE TO PREVENT CONCRETE, PLASTER OR DIRT FROM ENTERING. CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED MALLEABLE IRON BUSHING ON INSIDE AND GALVANIZED MALLEABLE IRON LOCKNUT ON OUTSIDE AND INSIDE.
- 24. EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES AND PULL BOXES SHALL BE GALVANIZED OR EPOXY—COATED SHEET STEEL. SHALL MEET OR EXCEED UL 50 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND NEMA 3 (OR BETTER) FOR EXTERIOR LOCATIONS.
- 25. METAL RECEPTACLE, SWITCH AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY—COATED OR NON—CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- 26. NONMETALLIC RECEPTACLE, SWITCH AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2 (NEWEST REVISION) AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- 27. THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CARRIER AND/OR DISH Wireless L.L.C. AND TOWER OWNER BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.
- 28. THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD LIFE AND PROPERTY.
- 29. INSTALL LAMICOID LABEL ON THE METER CENTER TO SHOW "DISH Wireless L.L.C.".
- 30. ALL EMPTY/SPARE CONDUITS THAT ARE INSTALLED ARE TO HAVE A METERED MULE TAPE PULL CORD INSTALLED.

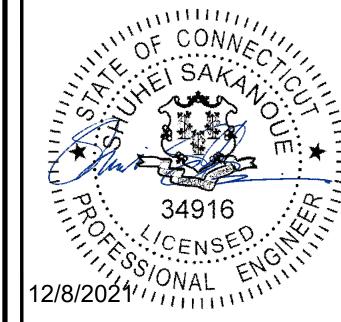


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	RCD		SS		CJW	
	RFDS REV ;		#: N/A			

# CONSTRUCTION DOCUMENTS

	SUBMITTALS				
REV DATE DESCRIPTION		DESCRIPTION			
0	11/01/21	ISSUED FOR PERMIT			
1	11/16/21	REVISED FOR PERMIT			
2	12/07/21	REVISED FOR PERMIT			
	A&E F	PROJECT NUMBER			

1197-F0001-C

DISH Wireless L.L.C. PROJECT INFORMATION

BOHVN00204B WEST PEAK DRIVE MERIDEN, CT 06451

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

## **GROUNDING NOTES:**

- 1. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION AND AC POWER GES'S) SHALL BE BONDED TOGETHER AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
- 2. THE CONTRACTOR SHALL PERFORM IEEE FALL—OF—POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS, THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT AND PROVIDE TESTING RESULTS.
- 4. METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- 5. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
- 6. EACH CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 STRANDED COPPER OR LARGER FOR INDOOR BTS; #2 BARE SOLID TINNED COPPER FOR OUTDOOR BTS.
- 7. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED BACK TO BACK CONNECTIONS ON OPPOSITE SIDE OF THE GROUND BUS ARE PERMITTED.
- 8. ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING SHALL BE #2 SOLID TINNED COPPER UNLESS OTHERWISE INDICATED.
- 9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
- 10. USE OF 90° BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45° BENDS CAN BE ADEQUATELY SUPPORTED.
- 11. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
- 12. ALL GROUND CONNECTIONS ABOVE GRADE (INTERIOR AND EXTERIOR) SHALL BE FORMED USING HIGH PRESS CRIMPS.
- 13. COMPRESSION GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.
- 14. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR.
- 15. APPROVED ANTIOXIDANT COATINGS (i.e. CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- 16. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.
- 17. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
- 18. BOND ALL METALLIC OBJECTS WITHIN 6 ft OF MAIN GROUND RING WITH (1) #2 BARE SOLID TINNED COPPER GROUND CONDUCTOR.
- 19. GROUND CONDUCTORS USED FOR THE FACILITY GROUNDING AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDUITIONS, NON-METALLIC MATERIAL SUCH AS PVC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (i.e., NONMETALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT
- 20. ALL GROUNDS THAT TRANSITION FROM BELOW GRADE TO ABOVE GRADE MUST BE #2 BARE SOLID TINNED COPPER IN 3/4" NON-METALLIC, FLEXIBLE CONDUIT FROM 24" BELOW GRADE TO WITHIN 3" TO 6" OF CAD-WELD TERMINATION POINT. THE EXPOSED END OF THE CONDUIT MUST BE SEALED WITH SILICONE CAULK. (ADD TRANSITIONING GROUND STANDARD DETAIL AS WELL).
- 21. BUILDINGS WHERE THE MAIN GROUNDING CONDUCTORS ARE REQUIRED TO BE ROUTED TO GRADE, THE CONTRACTOR SHALL ROUTE TWO GROUNDING CONDUCTORS FROM THE ROOFTOP, TOWERS, AND WATER TOWERS GROUNDING RING, TO THE EXISTING GROUNDING SYSTEM, THE GROUNDING CONDUCTORS SHALL NOT BE SMALLER THAN 2/O COPPER. ROOFTOP GROUNDING RING SHALL BE BONDED TO THE EXISTING GROUNDING SYSTEM, THE BUILDING STEEL COLUMNS, LIGHTNING PROTECTION SYSTEM, AND BUILDING MAIN WATER LINE (FERROUS OR NONFERROUS METAL PIPING ONLY). DO NOT ATTACH GROUNDING TO FIRE SPRINKLER SYSTEM PIPES.

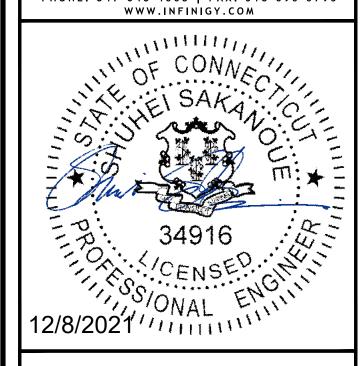


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RCD	SS		CJM		
DEDO DEL //					

RFDS REV #: N/A

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1197-F0001-C

DISH Wireless L.L.C. PROJECT INFORMATION

BOHVN00204B WEST PEAK DRIVE MERIDEN, CT 06451

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

# Exhibit D

# **Structural Analysis Report**



# Reanalysis of a 125 ft Self-Supporting Tower

Site Number Dish Wireless BOHVN00204B

**Everest Infrastructure 638281** 

Site Name: West Peak

County: New Haven, CT

Location: 11 West Peak Drv, Meriden, CT

Checked by:

- Patrick Propert

Structural Design Engineer III

PEN.0034951
//CENSED

9/22/2021

EVEREST - INFRASTRUCTURE PARTNERS -

Two Allegheny Ctr

Nova Tower 2, Ste 703

Pittsburgh, PA 15212

September 2021

September 22, 2021

Tom Rigg Everest Infrastructure Partners Two Allegheny Ctr Nova Center 2, Ste 703 Pittsburgh, PA 15212



RE:

Dish Wireless – BOHVN00204B Everest Infrastructure 638281

11 West Peak Drv, Meriden, CT

### Tom:

We have completed the structural analysis of the subject tower and have found it to be adequate within the scope of this analysis to support the proposed antenna loading. The tower was analyzed according to the code wind and ice parameters outlined in the *Code Requirements Table* following this letter.

The subject tower is a 125' square self-supporting tower consisting of all-bolted sections with angle legs and bracing. Tower face dimensions range from 12'6" at the top to 31'3" at the base. Foundation capacities are based on manufacturer's design details.

The loading used in the analysis consisted of the existing antennas/lines as well as the following for Dish Wireless at 117' on (3) Commscope MTC3975083 antenna frames:

- (3) MX08FRO665-20 antennas
- (3) TA08025-B604 RRUs, (3) TA08025-B605 RRUs
- (1) RDIDC-9181-PF-48 fed with (1) 1-5/8" hybrid cable installed as shown on E-7.

The results of the analysis showed all tower and foundation elements to be loaded within allowable limits with a maximum stress rating of 87%. We recommend a post-construction inspection be completed by a structural engineer to document that tower mounted equipment has been placed in compliance with the requirements of this analysis. For a detailed listing of tower performance, please see pages 13 to 14 of the calculations.

We appreciate the opportunity to provide our professional services to Everest Infrastructure and Dish Wireless and if you have any questions concerning this analysis, please contact us.

Sincerely,

ARMOR TOWER, INC.

Patrick Botimer

Structural Design Engineer V

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### **CODE REQUIREMENTS**

Governing code: CT State Building Code
Code basis: 2018 IBC

Code basis: 2018 IBC
Referenced standard: ANSI/TIA 222-H

Basic wind speed: (3-sec. gust):

Vult: 118 mph with no ice

50 mph with 1" concurrent ice

County of site location: New Haven

ASCE 7 Special wind region:

Structure/Risk Category:

Exposure Category:

Topographic Category:

3 - hill

Crest Height/Tower Base Elevation 922 ft/1004 ft Spectral Response:  $S_s=0.200, S_1=0.055$ 

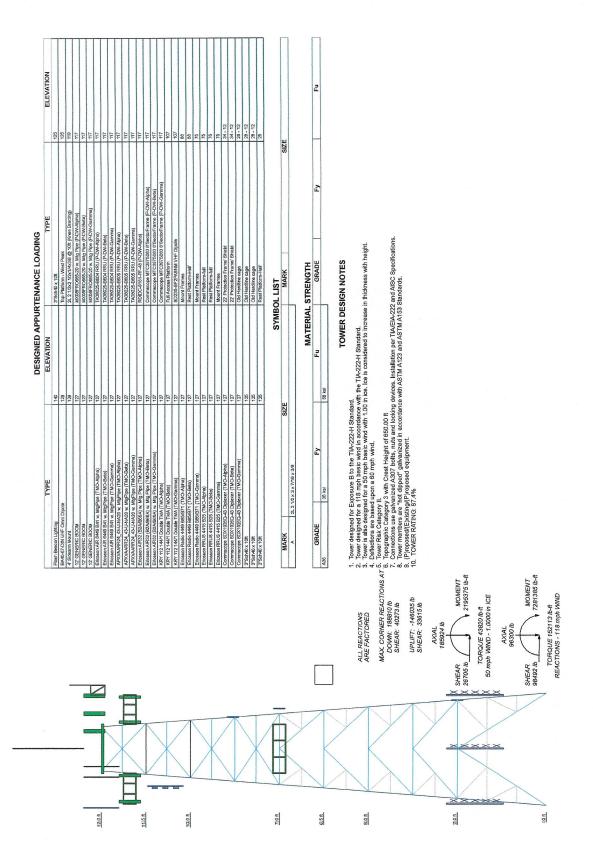
### PRIMARY ASSUMPTIONS CONSIDERED IN THIS PROJECT

1. Leg A is assumed to be oriented Northeast.

- 2. Allowable steel stresses are defined by AISC-LRFD-99/360-16 and all welds conform to AWS D1.1 specification.
- 3. If reserved antennas/feed lines by other carriers or the tower owner are to be considered in this analysis, it is the responsibility of Everest Infrastructure and its affiliates to provide this information.
- 4. Any deviation from the analyzed antenna loading will require a re-analysis of the tower for verification of structural integrity. This analysis has considered the proposed feed lines to be stacked and located as shown on drawing E-7.
- 5. This analysis assumes all tower members are galvanized adequately to prevent corrosion of the steel and that all tower members are in "like new" condition with no physical deterioration. This analysis also assumes the tower has been maintained properly per TIA 222-H Annex J recommended inspection and maintenance procedures for tower owners and is in a plumb condition. Armor Tower has not completed a condition assessment of the tower.
- 6. No accounting for residual stresses due to incorrect tower erection can be made. This analysis assumes all bolts are appropriately tightened providing necessary connection continuity and that the installation of the tower was performed by a qualified tower erector.
- 7. No conclusions, expressed or implied, shall indicate that Armor Tower has made an evaluation of the original design, materials, fabrication, or potential installation or erection deficiencies. Any information contrary to that assumed for the purpose of preparing this analysis could alter the findings and conclusions stated herein.
- 8. Tower member sizes and geometry are based on a tower mapping completed by this office in 2017. Field measurements included NDT-ultrasonic thickness testing. Existing antenna loading is based on our 2017 tower analysis. It is our assumption that this data is complete and accurately reflects the existing conditions of the tower and equipment. Armor Tower has not been commissioned to field-



- validate this data. Armor Tower reserves the right to add to or modify this report as more information becomes available. Proposed equipment was outlined in 638281 Dish Colo App (EIP) \_9.7.2021.xlsx.
- 9. The investigation of the load carrying capacities of the antenna supporting frames/mounts is outside the scope of this analysis. Antenna mount certification has been completed by others.
- 10. Armor Tower can assist the contractor in providing a Class IV rigging plan for equipment lifting.



000	Armor Tower Inc.	Armor Tower Inc   125' SQR SELF-SUPPORTING TOWER ANALYSIS	<b>VER ANALYSIS</b>
SWC X	9 North Main	Project Dish Wireless BOHVN00204B West Peak/Meriden, CT	V/Meriden, CT
	Cortland NY 13045	Cortland NY 13045 Client: Everest Infrastructure - 638281 Drawn by: PB	App'd:
	Phone: 607-591-5381	Code: TIA-222-H Date: 09/22/2	Date: 09/22/21 Scale: NTS
	FAX: 866-870-0840	Path:	Dwg No. E1

T.88TEh (dl) IrlgieW	a.tets1	872096	4262.2	3,0313	99629	2,6786	0.878.0
# Panels @ (ft)	1 @ 25			ZI @ 8	97		
Eace Width (ft) 31.25	S.T.S	27.6S	21.87S	oz	62.81	376.41	-21
BriossB tenni	L2 1/2×2 1/2×1/4	8/6x8116x5/1 2x5/1 2.15	SL2 1/2X2X	8/E×91/6	AN	٧	
sziroH-duS Jess	2F3×2 1/2×1/4×3/8			A.N			
Red. Diagonals	91/8/2/1-2/2/1-27	81/5X2/1 5X2/1 S.1	ZXZ/1 Z7	91/62	ΥN	γ	
Red. Horizontals	L2 1/2x2x3/16	NEXEXCI	91		A,N	٧	
Seo, Horizontals		٧n			13x2 1/2x1/4	C.6x8.2	1/2×2×1/4
aletnozinoH	3F3×5 1\5×2\18×3\8	2L2 1/2x2 1/2x1/4x3/8 2L		BIEXPITXS/T SXELIS	'N	A	
ahi S doT		Aμ			SLEXZ 1/2x1/4x3/8	A	C8×13.4
ebsið lenogeið			36A				
slenogeiC	SL3X3 1/2X3/8X3/8	ברג וובאב וובאב	8/Exp/1:		b/LXEXb7	F3×3 1/5×1/4	F3 1/5×3 1/5×1/4
Leg Grade			)EA	5			
reda	8/4×8×8J	8/2×8×3J	×9×97	3/4	8/9×9×97	9×97	ZIX
geopou	и	91	91	91	£T.	Z1	1.1

### Feed Line Plan 25'

Round Flat App In Face App Out Fac

					MEI FORM, ST,
				SION CHI	ELES. FORM (NO. 12T)
				AVAT SO CLE HYDRIO	
		8		6), 6)	
Ladder (Af) (P-DW-117)		, and the second			
873 (1-5/8") (P-DW-117)					
					-
	< <			0	
	- wada				
				(2) (3	VA" Digid Conduit
				L2 1/2	x2 1/2x44 Redunda
					9/4" Rigid Conduit  x2 1/2x4 A Bedunda
				NA FOR	FORM
				TO SO POLIS	\$
		Sec. 20		rp. rpgin	

404400		Job: 125' SQR SELF-SUPPOR		
ARMUK	9 North Main	Project: Dish Wireless BOHVN00204	4 West Peak	Meriden, CT
TOWER	Cortland, NY 13045	Client: Everest Infrastructure - 638281	Drawn by: PB	App'd:
	Phone: 607-591-5381	Code: TIA-222-H	Date: 09/22/21	Scale: NTS
		Path:		Dwg No. E-7



Cortland, NY 13045 Phone: 607-591-5381 FAX: 866-870-0840

Job 125' SQR SELF-SUPPORTING TOWER ANALYSIS .	Page 1 of 29
Project Dish Wireless BOHVN00204B West Peak/Meriden, CT	Date 08:21:04 09/22/21
Client Everest Infrastructure - 638281	Designed by PB

### **Tower Input Data**

The main tower is a 4x free standing tower with an overall height of 125.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 12.50 ft at the top and 31.25 ft at the base.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

Tower base elevation above sea level: 1004.00 ft.

Basic wind speed of 118 mph.

Risk Category II.

Exposure Category B.

Simplified Topographic Factor Procedure for wind speed-up calculations is used.

Topographic Category: 3.

Crest Height: 650.00 ft.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

Connections use galvanized A307 bolts, nuts and locking devices. Installation per TIA/EIA-222 and AISC Specifications.

Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.

(P)roposed/(E)xisting/(P)roposed equipment.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

### **Options**

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

- √ Use Code Stress Ratios
- √ Use Code Safety Factors Guys
   Escalate Ice
   Always Use Max Kz
   Use Special Wind Profile
- √ Include Bolts In Member Capacity
  Leg Bolts Are At Top Of Section
- ✓ Secondary Horizontal Braces Leg
   ✓ Use Diamond Inner Bracing (4 Sided)
   SR Members Have Cut Ends
   SR Members Are Concentric

Distribute Leg Loads As Uniform Assume Legs Pinned

- Assume Legs Pinned

  √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area
  Use Clear Spans For KL/r
  Retension Guys To Initial Tension
  Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt. Autocalc Torque Arm Areas Add IBC .6D+W Combination
- √ Sort Capacity Reports By Component
- √ Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs

Use ASCE 10 X-Brace Ly Rules

- √ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression
- ✓ All Leg Panels Have Same Allowable Offset Girt At Foundation
- √ Consider Feed Line Torque
- √ Include Angle Block Shear Check
  Use TIA-222-H Bracing Resist. Exemption
  Use TIA-222-H Tension Splice Exemption
  Poles

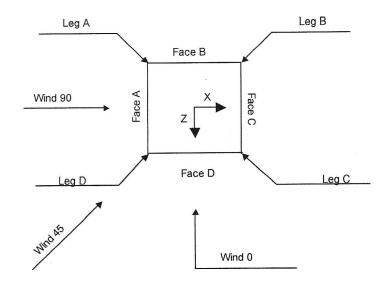
Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known



Armor Tower Inc 9 North Main Cortland, NY 13045

Phone: 607-591-5381 FAX: 866-870-0840

Job 125' SQR SELF-SUPPORTING TOWER ANALYSIS .	Page 2 of 29
Project Dish Wireless BOHVN00204B West Peak/Meriden, CT	Date 08:21:04 09/22/21
Client Everest Infrastructure - 638281	Designed by



Square Tower

Tower	Saction	n Geometry	,
IOMEI	Section	i Geometry	

Tower	Tower	Assembly	Description	Section	Number	Section	
Section	Elevation	Database		Width	of	Length	
					Sections		
	ft			ft		ft	
T1	125.00-112.50		Appendix open some oververen from the SAS SAS SAS SAS SAS SAS SAS SAS SAS SA	12.50	1	12.50	
T2	112.50-100.00			14.38	1	12.50	
T3	100.00-75.00			16.25	1	25.00	
T4	75.00-62.50			20.00	1	12.50	
T5	62.50-50.00			21.88	1	12.50	
T6	50.00-25.00			23.75	1	25.00	
T7	25.00-0.00			27.50	1	25.00	

# **Tower Section Geometry** (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft		Ena Panels	End Panels		in
T1	125.00-112.50	12.50	X Brace	No	Yes	0.0000	0.0000
T2	112.50-100.00	12.50	X Brace	No	Yes	0.0000	0.0000
Т3	100.00-75.00	12.50	X Brace	No	Yes	0.0000	0.0000
T4	75.00-62.50	12.50	K1 Down	No	Yes	0.0000	0.0000
T5	62.50-50.00	12.50	K1 Down	No	Yes	0.0000	0.0000
T6	50.00-25.00	12.50	K1 Down	No	Yes	0.0000	0.0000
T7	25.00-0.00	25.00	K2 Down	No	Yes	0.0000	0.0000



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Job 125' SQR SELF-SUPPORTING TOWER ANALYSIS .	Page 3 of 29
Project Dish Wireless BOHVN00204B West Peak/Meriden, CT	Date 08:21:04 09/22/21
Client  Everest Infrastructure - 638281	Designed by

Tower	Leg	Leg	Leg	Diagonal	Diagonal	Diagonal
Elevation	Туре	Size	Grade	Туре	Size	Grade
ft			1.0.6	G: 1 1 1	L3 1/2x3 1/2x1/4	A36
T1 125.00-112.50	Equal Angle	L6x6x1/2	A36 (36 ksi)	Single Angle	L3 1/2X3 1/2X1/4	(36 ksi)
T2 112.50-100.00	Equal Angle	L6x6x1/2	A36 (36 ksi)	Single Angle	L3x3 1/2x1/4	A36 (36 ksi)
T3 100.00-75.00	Equal Angle	L6x6x5/8	A36 (36 ksi)	Single Angle	L4x3x1/4	A36 (36 ksi)
T4 75.00-62.50	Equal Angle	L6x6x3/4	A36 (36 ksi)	Double Angle	2L2 1/2x2 1/2x1/4x3/8	A36 (36 ksi)
T5 62.50-50.00	Equal Angle	L6x6x3/4	A36 (36 ksi)	Double Angle	2L2 1/2x2 1/2x1/4x3/8	A36 (36 ksi)
T6 50.00-25.00	Equal Angle	L6x6x7/8	A36 (36 ksi)	Double Angle	2L2 1/2x2 1/2x1/4x3/8	A36 (36 ksi)
T7 25.00-0.00	Equal Angle	L8x8x7/8	A36 (36 ksi)	Double Angle	2L3x3 1/2x3/8x3/8	A36 (36 ksi)

# **Tower Section Geometry** (cont'd)

Tower Elevation	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 125 00-112.50	Channel	C9x13.4	A36	Flat Bar	ners valentine til en	A36
11 125.00-112.50	Chamber	2,1112.1	(36 ksi)			(36 ksi)
T2 112.50-100.00	Double Angle	2L 3 1/2 x 3 x 7/16 x 3/8	A36	Flat Bar		A36
			(36 ksi)			(36 ksi)
T3 100.00-75.00	Double Angle	2L3x2 1/2x1/4x3/8	A36	Flat Bar		A36
15 100.00 75.00	13 100.00-73.00 Dodole Aligie		(36 ksi)			(36 ksi)

# **Tower Section Geometry** (cont'd)

Tower	No.	Mid Girt	Mid Girt	Mid Girt	Horizontal	Horizontal	Horizontal
Elevation	of	Туре	Size	Grade	Туре	Size	Grade
	Mid						
ft	Girts						
T3 100.00-75.00	None	Flat Bar		A36	Double Angle	2L3x2 1/2x1/4x3/8	A36
				(36 ksi)			(36 ksi)
T4 75.00-62.50	None	Wide Flange		A36	Double Angle	2L3x2 1/2x1/4x3/8	A36
				(36 ksi)			(36 ksi)
T5 62.50-50.00	None	Flat Bar		A36	Double Equal	2L2 1/2x2 1/2x1/4x3/8	A36
				(36 ksi)	Angle		(36 ksi)
T6 50.00-25.00	None	Flat Bar		A36	Double Angle	2L2 1/2x2 1/2x1/4x3/8	A36
				(36 ksi)			(36 ksi)
T7 25.00-0.00	None	Flat Bar		A36	Double Angle	2L3x2 1/2x5/16x3/8	A36
X, 20.00 0.00	110110			(36 ksi)	Č		(36 ksi)

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TOWER
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Cortland, NY 13045 Phone: 607-591-5381 FAX: 866-870-0840

### Job 125' SQR SELF-SUPPORTING TOWER ANALYSIS .

Dish Wireless BOHVN00204B West Peak/Meriden, CT

Client Everest Infrastructure - 638281

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Tower Elevation	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
ft						
T1 125.00-112.50	Single Angle	L2 1/2x2x1/4	A36	Double Angle		A36
			(36 ksi)			(36 ksi)
T2 112.50-100.00	Channel	C6x8.2	A36	Double Angle		A36
			(36 ksi)			(36 ksi)
T3 100.00-75.00	Single Angle	L3x2 1/2x1/4	A36	Double Angle		A36
			(36 ksi)			(36 ksi)
T4 75.00-62.50	Solid Round		A36	Double Angle	2L2 1/2x2x3/16x3/8	A36
			(36 ksi)			(36 ksi)
T5 62.50-50.00	Solid Round		A36	Double Angle	2L2 1/2x2x3/16x3/8	A36
			(36 ksi)			(36 ksi)
T6 50.00-25.00	Solid Round		A36	Double Angle	2L2 1/2x2 1/2x3/16x3/8	A36
			(36 ksi)	Special Control Control		(36 ksi)
T7 25.00-0.00	Solid Round		A36	Equal Angle	L2 1/2x2 1/2x1/4	A36
			(36 ksi)			(36 ksi)

# **Tower Section Geometry** (cont'd)

Tower Elevation	Redundant Bracing Grade		Redundant Type	Redundant Size	K Factor
ft					
T4	A36	Horizontal (1)	Equal Angle	L2x2x3/16	0.9
75.00-62.50	(36 ksi)	Diagonal (1)	Single Angle	L2 1/2x2x3/16	0.9
T5	A36	Horizontal (1)	Equal Angle	L2x2x3/16	0.9
62.50-50.00	(36 ksi)	Diagonal (1)	Single Angle	L2 1/2x2x3/16	0.9
T6	A36	Horizontal (1)	Equal Angle	L2x2x3/16	0.9
50.00-25.00	(36 ksi)	Diagonal (1)	Single Angle	L2 1/2x2 1/2x3/16	0.9
Γ7 25.00-0.00	A36	Horizontal (1)	Arbitrary Shape	L2 1/2x2x3/16	0.9
	(36 ksi)	Horizontal (2)	, ,	2L2 1/2x2 1/2x1/4x3/8	
		Diagonal (1)	Arbitrary Shape	L2-1/2x2-1/2x3/16	0.9
		Diagonal (2)	2 mg mag 1	2L2 1/2x2x1/4x3/8	
		Sub-Horizontal	Double Angle	2L3x2 1/2x1/4x3/8	1

Tower Elevation ft	Gusset Area (per face) ft²	Gusset Thickness in	Gusset Grade	Adjust. Factor $A_f$	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Hortzontals in	Double Angle Stitch Bolt Spacing Redundants in
T1	4.80	0.3750	A36	1	1	1.03	24.0000	24.0000	24.0000
125.00-112.50			(36 ksi)						
T2	4.80	0.3750	A36	1	1	1.03	24.0000	24.0000	24.0000
112.50-100.00			(36 ksi)						
T3	6.10	0.3750	A36	1	1	1.03	24.0000	24.0000	24.0000
100.00-75.00			(36 ksi)						
T4 75.00-62.50	3.50	0.3750	A36	1	1	1.03	24.0000	24.0000	24.0000
			(36 ksi)						
T5 62.50-50.00	3.50	0.3750	A36	1	1	1.03	24.0000	24.0000	24.0000
			(36 ksi)						
T6 50.00 <b>-2</b> 5.00	3.50	0.3750	A36	1	1	1.03	24.0000	24.0000	24.0000
			(36 ksi)						4



Cortland, NY 13045 Phone: 607-591-5381 FAX: 866-870-0840

## ЈоБ 125' SQR SELF-SUPPORTING TOWER ANALYSIS .

Project
Dish Wireless BOHVN00204B West Peak/Meriden, CT

Client

Everest Infrastructure - 638281

Date 08:21:04 09/22/21

Designed by

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PB

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Double Angle Double Angle Double Angle Gusset Gusset Grade Adjust. Factor Adjust. Weight Mult. Tower Gusset Stitch Bolt Stitch Bolt Stitch Bolt Elevation Area Thickness Factor  $A_r$ Spacing Spacing Spacing (per face) Horizontals Redundants Diagonals in in in in 1.03 24.0000 24.0000 24.0000 0.3750 A36 1 9.80 T7 25.00-0.00

(36 ksi)

## **Tower Section Geometry** (cont'd)

						K Fac	ctors <sup>i</sup>			
Tower Elevation	Calc K Single	Calc K Solid	Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace
	Angles	Rounds		X	X	X	X	X	X	X
ft				Y	Y	Y	Y	Y	<u> </u>	Y
T1	No	No	1	0.9	1	1	1	1	1	1
125.00-112.50				0.9	1	1	1	1	1	1
T2	No	No	1	0.9	1	1	1	1	1	1
112.50-100.00				0.9	1	1	1	1	1	1
T3	No	No	1	0.9	1	1	1	1	1	1
100.00-75.00				0.9	1	1	1	1	1	1
T4	No	No	1	1	0.9	1	1	0.9	1	0.5
75.00-62.50				1	0.9	1	1	0.9	1	0.5
T5	No	No	1	1	0.9	1	1	0.9	1	0.5
62.50-50.00				1	0.9	1	1	0.9	1	0.5
T6	No	No	1	1	0.9	1	1	0.9	1	0.5
50.00-25.00				1	0.9	1	1	0.9	1	0.5
T7 25.00-0.00	No	No	1	1	0.9	1	1	0.9	1	0.5
				1	0.9	1	1	0.9	1	0.5

<sup>&</sup>lt;sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

Tower Elevation ft	Leg		Diagonal		Top G	Top Girt		ı Girt	Mid	Girt	Long Ho	rizontal	Short Ho	rizontal
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
Tl	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
125.00-112.50	1				0.0000		0.0000		0.0000	0.77	0.0000	0.575	0.0000	0.75
T2	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
112.50-100.00 T3 100.00-75.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T4 75.00-62.50	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T5 62.50-50.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 50.00-25.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 25.00-0.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75



Cortland, NY 13045 Phone: 607-591-5381 FAX: 866-870-0840

### Job 125' SQR SELF-SUPPORTING TOWER ANALYSIS .

Project
Dish Wireless BOHVN00204B West Peak/Meriden, CT

Client

Everest Infrastructure - 638281 Designed by PB

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Tower Elevation ft	Reduna Horizo		Reduna Diago		Redund Sub-Diag		2100000	Redundant l Sub-Horizontal		Redundant Vertical		ınt Hip	Redundant Hip Diagonal	
•	Net Width	U	Net Width	U	Net Width	U	Net Width	U	Net Width	U	Net Width	U	Net Width	U
	Deduct in		Deduct in		Deduct in		Wiain Deduct		Deduct		Wiain Deduct		Deduct	
	***						in		in		in		in	
T1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
125.00-112.50							0.0000	0.77	0.0000	0.75	0.0000	0.75	0.0000	0.75
T2 112.50-100.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T3	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
100.00-75.00	0.0000	0.75	0.0000	0.70	0.0000	0.70	0,000	31,7						
T4 75.00-62.50	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T5 62.50-50.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 50.00-25.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 25.00-0.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

# **Tower Section Geometry** (cont'd)

Tower				Connection	on Offsets			,				
Elevation		Diag	gonal			K-Bracing						
	Vert.	Horiz.	Vert.	Horiz.	Vert.	Horiz.	Vert.	Horiz.				
	Тор	Top	Bot.	Bot.	Тор	Top	Bot.	Bot.				
ft	in	in	in	in	in	in	in	in				
T1	8.0000	8.0000	8.0000	8.0000	0.0000	0.0000	0.0000	0.0000				
125.00-112.50							COCCERNIC					
T2	8.0000	8.0000	8.0000	8.0000	0.0000	0.0000	0.0000	0.0000				
112.50-100.00												
T3	8.0000	8.0000	8.0000	8.0000	0.0000	0.0000	0.0000	0.0000				
100.00-75.00												
T4 75.00-62.50	0.0000	0.0000	0.0000	0.0000	5.0000	5.0000	10.0000	5.0000				
T5 62.50-50.00	0.0000	0.0000	0.0000	0.0000	5.0000	5.0000	10.0000	5.0000				
T6 50.00-25.00	0.0000	0.0000	0.0000	0.0000	5.0000	5.0000	10.0000	5.0000				
T7 25.00-0.00	0.0000	0.0000	0.0000	0.0000	5.0000	5.0000	10.0000	5.0000				

Tower Elevation ft	Leg Connection Type	Leg	Leg		Diagonal		Top Girt		Bottom Girt		irt	Long Hori	zontal	Short Hor	izontal
		Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.
		in		in		in		in		in		in		in	
T1	Sleeve DS	0.7500	16	0.7500	5	0.7500	4	0.0000	0	0.7500	0	0.7500	3	0.7500	2
125.00-112.50		A307		A307		A307		A325N		A307		A307		A307	
T2	Sleeve DS	0.7500	0	0.7500	4	0.7500	3	0.6250	0	0.7500	0	0.7500	3	0.7500	2
112.50-100.00		A307		A307		A307		A325N		A307		A307		A307	
T3	Sleeve DS	0.7500	16	0.7500	4	0.7500	3	0.6250	O	0.6250	0	0.7500	3	0.7500	2
100.00-75.00		A307		A307		A307		A325N		A325N		A307		A307	
T4 75.00-62.50	Sleeve DS	0.7500	0	0.7500	2	0.6250	0	0.6250	0	0.7500	0	0.7500	2	0.7500	0
		A307		A307		A325N		A325N		A325N		A307		A307	
T5 62.50-50.00	Sleeve DS	0.7500	20	0.7500	- 2	0.6250	0	0.6250	0	0.6250	0	0.7500	2	0.7500	0
		A307		A307		A325N		A325N		A325N		A307		A307	

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Tower Elevation ft	Leg Connection Type	Leg		Diagor	ial	Top G	irt	Bottom (	Girt	Mid G	irt	Long Hori	zontal	Short Hori	izontal
		Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.						
		in		in		in		in		in		in		in	
T6 50.00-25.00	Sleeve DS	0.7500	24	0.7500	2	0.6250	0	0.6250	0	0.6250	0	0.7500	2	0.7500	0
		A307		A307		A325N		A325N		A325N		A307		A307	
T7 25.00-0.00	Sleeve DS	0.7500	28	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.7500	2	0.7500	0
		A307		A307		A325N		A325N		A325N		A307		A307	

# Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
Climbing Ladder **	D	No '	No	Af (CaAa)	125.00 <b>-</b> 0.00	-12.000 0	-0.48	1	1	6.0000	6.0000	n de la companya de l	7.80
*TMobile* *(18) coax existing, (12) 2B removed													
AVA7-50 (1-5/8 LOW DENSI. FOAM) (TMO-127')	В	No	No	Ar (CaAa)	125.00 <b>-</b> 0.00	2.0000	0.4	6	3	1.9800	1.9800		0.72
Main Hybrid Fiber Cable (TMO-127')	В	No	No	Ar (CaAa)	125.00 - 0.00	4.0000	0.45	6	3	1.4300	1.4300		1.63
LDF6-50A 1-1/4 FOAM) (Omni-129')	D	No	No	Ar (CaAa)	125.00 <b>-</b> 0.00	-15.000 0	-0.43	1	1	1.5500	1.5500		0.66
LDF6-50A 1-1/4 FOAM) (Omni-107')	D	No	No	Ar (CaAa)	107.00 <b>-</b> 0.00	-12.000 0	<b>-</b> 0.46	1	1	1.5500	1.5500		0.66
3/4" Rigid Conduit	С	No	No	Ar (CaAa)	125.00 <b>-</b> 0.00	5.0000	0.2	2	2	0.7500	0.7500		0.80
**													
**													
** L2 1/2x2 1/2x1/4 (Redundant Vert) *	С	No	No	Af (CaAa)	75.00 - 25.00	0.0000	0.25	1	1	1.2500	1.2500		2.00
*Proposed Dish Wireless Sept2021													
FXL1873 (1-5/8") (P-DW-117)	A	No	No	Ar (CaAa)	117.00 <b>-</b> 5.00	1.0000	0.4	1	1	1.6250	1.6250		0.67
Feedline Ladder (Af) (P-DW-117)	A	No	No	Af (CaAa)	117.00 <b>-</b> 5.00	1.0000	0.45	1	1	3.0000	3.0000		8.40



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Job		
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## Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Exclude From	Component Type	Placement	Total Number	$C_A A_A$	Weight
	Leg		Torque	21	ft		ft²/ft	plf
			Calculation					
**							NATIONAL PROPERTY AND	
*TMobile*								
**								
**								
*								

## Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	$A_R$	$A_F$	$C_A A_A$	$C_AA_A$	Weight
Section	Elevation				In Face	Out Face	
	ft		$ft^2$	$ft^2$	$ft^2$	ft²	lb
T1	125.00-112.50	Α	0.000	0.000	2.981	0.000	40.81
		В	0.000	0.000	25.575	0.000	176.25
		C	0.000	0.000	1.875	0.000	20.00
		D	0.000	0.000	14.438	0.000	105.75
T2	112.50-100.00	Α	0.000	0.000	8.281	0.000	113.38
		В	0.000	0.000	25.575	0.000	176.25
		C	0.000	0.000	1.875	0.000	20.00
		D	0.000	0.000	15.522	0.000	110.37
T3	100.00-75.00	A	0.000	0.000	16.563	0.000	226.75
		В	0.000	0.000	51.150	0.000	352.50
		C	0.000	0.000	3.750	0.000	40.00
		D	0.000	0.000	32.750	0.000	228.00
T4	75.00-62.50	Α	0.000	0.000	8.281	0.000	113.38
		В	0.000	0.000	25.575	0.000	176.25
		C	0.000	0.000	4.479	0.000	45.00
		D	0.000	0.000	16.375	0.000	114.00
T5	62.50-50.00	A	0.000	0.000	8.281	0.000	113.38
		В	0.000	0.000	25.575	0.000	176.25
		C	0.000	0.000	4.479	0.000	45.00
		D	0.000	0.000	16.375	0.000	114.00
T6	50.00-25.00	Α	0.000	0.000	16.563	0.000	226.75
		В	0.000	0.000	51.150	0.000	352.50
		C	0.000	0.000	8.958	0.000	90.00
		D	0.000	0.000	32.750	0.000	ZZ8.00
T7	25.00-0.00	A	0.000	0.000	13.250	0.000	181.40
		В	0.000	0.000	51.150	0.000	352.50
		C	0.000	0.000	3.750	0.000	40.00
		D	0.000	0.000	32.750	0.000	228.00

# Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	$A_R$	$A_F$	$C_AA_A$	$C_AA_A$	Weight
Section	Elevation	or	Thickness			In Face	Out Face	8
***************************************	ft	Leg	in	fl <sup>2</sup>	$ft^2$	$ft^2$	$ft^2$	lb
T1	125.00-112.50	A	1.388	0.000	0.000	5 481	0.000	105.75



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Tower Section	Tower Elevation	Face or	Ice Thickness	$A_R$	$A_F$	$C_A A_A$ In Face	$C_A A_A$ Out Face	Weight
Deciion	ft	Leg	in	ft²	$ft^2$	ft <sup>2</sup>	ft²	lb
		В		0.000	0.000	43.078	0.000	875.73
		C		0.000	0.000	9.595	0.000	88.91
		D		0.000	0.000	21.380	0.000	287.90
T2	112.50-100.00	A	1.382	0.000	0.000	15.194	0.000	292.68
		В		0.000	0.000	43.026	0.000	873.71
		C		0.000	0.000	9.569	0.000	88.50
		D		0.000	0.000	24.370	0.000	326.13
T3	100.00-75.00	A	1.370	0.000	0.000	30.265	0.000	581.08
		В		0.000	0.000	85.840	0.000	1739.26
		C		0.000	0.000	19.032	0.000	175.35
		D		0.000	0.000	53.304	0.000	708.08
T4	75.00-62.50	Α	1.352	0.000	0.000	15.044	0.000	287.42
		В		0.000	0.000	42.765	0.000	863.67
		C		0.000	0.000	15.424	0.000	164.85
		D		0.000	0.000	26.518	0.000	349.73
T5	62.50-50.00	Ā	1.336	0.000	0.000	14.960	0.000	284.51
	02.00	В		0.000	0.000	42.619	0.000	858.09
		Ĉ		0.000	0.000	15.309	0.000	162.65
		D		0.000	0.000	26.393	0.000	345.72
T6	50.00-25.00	Ā	1.298	0.000	0.000	29.541	0.000	556.02
707	0.1	В		0.000	0.000	84.580	0.000	1691.09
		c		0.000	0.000	30.099	0.000	315.52
		D		0.000	0.000	52.218	0.000	673.50
T7	25.00-0.00	A	1.182	0.000	0.000	22.707	0.000	413.97
		В		0.000	0.000	82.568	0.000	1615.49
		C		0.000	0.000,	17.397	0.000	150.89
		D		0.000	0.000	50.483	0.000	620.40

# **Feed Line Center of Pressure**

Section	Elevation	$CP_X$	$CP_Z$	$CP_X$ $Ice$	CP <sub>z</sub> Ice
	ft	in	in	in	in
T1	125.00-112.50	11.0240	-6.4511	14.1851	-6.1814
T2	112.50-100.00	10.7780	-8.0878	12.9472	-8.0533
T3	100.00-75.00	12.6430	-8.7426	15.1302	<b>-</b> 7.9929
T4	75.00-62.50	16.1750	-9.7824	20.8070	-6.9529
T5	62.50-50.00	17.3807	-10.4368	22.0911	-7.3038
T6	50.00-25.00	19.3789	-11.5430	24.0358	-7.8829
T7	25.00-0.00	19.1883	-11.3065	22.3044	-9.2873

# **Shielding Factor Ka**

Tower	Feed Line	Description	Feed Line	$K_a$	$K_a$
Section	Record No.		Segment Elev.	No Ice	Ice
T1	1	Climbing Ladder	112.50 -	0.6000	0.6000
		7	125.00		
T1	5	AVA7-50 (1-5/8 LOW	112.50 -	0.6000	0.6000
		DENSI. FOAM)	125.00		
T1	6	Main Hybrid Fiber Cable	112.50 -	0.6000	0.6000
			125.00		and the first of



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Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.		Segment Elev.	No Ice	Ice
T1	8	LDF6-50A (1-1/4 FOAM)	112.50 -	0.6000	0.6000
			125.00		
T1	10	3/4" Rigid Conduit	112.50 -	0.6000	0.6000
			125.00	0.0000	0.0000
T1	22	FXL1873 (1-5/8")	112.50 -	0.6000	0.6000
TC1	22	Feedline Ladder (Af)	117.00 112.50 -	0.6000	0.6000
T1	23	reedine Ladder (AI)	117.00	0.0000	0.0000
T2	1	Climbing Ladder	100.00 -	0.6000	0.6000
12	1	Chinoling Dadder	112.50	0.0000	0.0000
T2	5	AVA7-50 (1-5/8 LOW	100.00 -	0.6000	0.6000
		DENSI. FOAM)	112.50		
T2	6	Main Hybrid Fiber Cable	100.00 -	0.6000	0.6000
			112.50		
T2	8	LDF6-50A (1-1/4 FOAM)	100.00 -	0.6000	0.6000
			112.50		
T2	9	LDF6-50A (1-1/4 FOAM)	100.00 -	0.6000	0.6000
	-		107.00		
T2	10	3/4" Rigid Conduit		0.6000	0.6000
			112.50		
T2	22	FXL1873 (1-5/8")	100.00 -	0.6000	0.6000
			112.50	0.5000	0.6000
T2	23	Feedline Ladder (Af)	100.00 -	0.6000	0.6000
TO	1	Climbing Ladder	112.50	0.6000	0.6000
T3 T3	1 5	AVA7-50 (1-5/8 LOW		0.6000	0.6000
13	3	DENSI. FOAM)	73.00 - 100.00	0.0000	0.0000
Т3	6	Main Hybrid Fiber Cable	75 00 - 100 00	0.6000	0.6000
T3	8	LDF6-50A (1-1/4 FOAM)	75.00 - 100.00	0.6000	0.6000
T3	ا ق	LDF6-50A (1-1/4 FOAM)		0.6000	0.6000
T3	10	3/4" Rigid Conduit		0.6000	0.6000
T3	22	FXL1873 (1-5/8")		0.6000	0.6000
T3	23	Feedline Ladder (Af)	75.00 - 100.00	0.6000	0.6000
T4	1	Climbing Ladder	62.50 - 75.00	0.6000	0.6000
T4	5	AVA7-50 (1-5/8 LOW	62.50 - 75.00	0.6000	0.6000
		DENSI. FOAM)		,	
T4	6	Main Hybrid Fiber Cable		0.6000	0.6000
T4	8	LDF6-50A (1-1/4 FOAM)		0.6000	0.6000
T4	9	LDF6-50A (1-1/4 FOAM)		0.6000	0.6000
T4	10	3/4" Rigid Conduit L2 1/2x2 1/2x1/4		0.6000 1.0000	0.6000 1.0000
T4 T4	19 22	FXL1873 (1-5/8")		0.6000	0.6000
T4	23	Feedline Ladder (Af)		0.6000	0.6000
T5	1	Climbing Ladder		0.6000	0.6000
T5	5	AVA7-50 (1-5/8 LOW	50.00 - 62.50	0.6000	0.6000
10		DENSI. FOAM)			
T5	6	Main Hybrid Fiber Cable		0.6000	0.6000
T5	8	LDF6-50A (1-1/4 FOAM)	50.00 - 62.50	0.6000	0.6000
T5	9	LDF6-50A (1-1/4 FOAM)		0.6000	0.6000
T5	10	3/4" Rigid Conduit	50.00 - 62.50	0.6000	0.6000
T5	19	L2 1/2x2 1/2x1/4	50.00 - 62.50	1.0000	1.0000
T5	22	FXL1873 (1-5/8") Feedline Ladder (Af)		0.6000	0.6000 0.6000
T5 T6	23	Climbing Ladder		0.6000 0.6000	0.6000
T6	5	AVA7-50 (1-5/8 LOW		0.6000	0.6000
10	3	DENSI. FOAM)		0.0000	0.0000
Т6	6	Main Hybrid Fiber Cable		0.6000	0.6000
T6	8	LDF6-50A (1-1/4 FOAM)		0.6000	0.6000
T6	9	LDF6-50A (1-1/4 FOAM)	25.00 - 50.00	0.6000	0.6000
T6	10	3/4" Rigid Conduit		0.6000	0.6000
T6	19	L2 1/2x2 1/2x1/4		1.0000	1.0000
T6	22	FXL1873 (1-5/8")	25.00 - 50.00	0.6000	0.6000

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FAX: 866-870-0840

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Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T6	23	Feedline Ladder (Af)	25.00 - 50.00	0.6000	0.6000
T7	1	Climbing Ladder	0.00 - 25.00	0.6000	0.6000
T7	5	AVA7-50 (1-5/8 LOW	0.00 - 25.00	0.6000	0.6000
		DENSI. FOAM)			
T7	6	Main Hybrid Fiber Cable	0.00 - 25.00	0.6000	0.6000
T7	8	LDF6-50A (1-1/4 FOAM)	0.00 - 25.00	0.6000	0.6000
T7	9	LDF6-50A (1-1/4 FOAM)	0.00 - 25.00	0.6000	0.6000
T7	10	3/4" Rigid Conduit	0.00 - 25.00	0.6000	0.6000
T7	22	FXL1873 (1-5/8")	5.00 - 25.00	0.6000	0.6000
T7	23	Feedline Ladder (Af)	5.00 - 25.00	0.6000	0.6000

				20 10 10 10 10 10		
	ina	anta.	TANK	AM	00	de
u	156	ELE	Tow	CIL	_Ua	$\mathbf{u}\mathbf{s}$

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		$C_AA_A$ Front	C₄A₄ Side	Weight
			ft ft ft	٥	ft		ft²	fî²	lb
Flash Beacon Lighting	A	None	namental and a second discovered and a second	0.0000	140.00	No Ice 1/2" Ice 1" Ice	2.70 3.10 3.50	2.70 3.10 3.50	50.00 70.00 90.00
**						1 100	5.50	3.50	70.00
Top Platform - West Peak	С	None		0.0000	125.00	No Ice 1/2" Ice 1" Ice	147.00 198.00 249.00	147.00 198.00 249.00	9100.00 12300.00 15500.00
2L 2 1/2x2 1/2x1/4x3/8 @ 10ft (Knee Bracing) **	A	None		0.0000	119.00	No Ice 1/2" Ice 1" Ice	62.30 84.10 105.90	62.30 84.10 105.90	1360.00 1850.00 2340.00
Full Access Platform	С	None		0.0000	107.00	No Ice 1/2" Ice 1" Ice	100.00 135.00 170.00	100.00 135.00 170.00	5100.00 6900.00 8700.00
SD235-SF2PASNM VHF Dipole	A	From Leg	5.00 1.00 0.00	0.0000	107.00	No Ice 1/2" Ice 1" Ice	3.43 5.68 7.93	3.43 5.68 7.93	25.00 37.00 49.00
**									
*T-Mobile* 12' GENERIC BOOM	A	From Leg	10.00 8.00 0.00	45.0000	127.00	No Ice 1/2" Ice 1" Ice	16.60 19.80 23.00	16.60 19.80 23.00	560.00 700.00 840.00
12' GENERIC BOOM	С	From Leg	10.00 10.00 0.00	45.0000	127.00	No Ice 1/2" Ice 1" Ice	16.60 19.80 23.00	16.60 19.80 23.00	560.00 700.00 840.00
12' GENERIC BOOM	D	From Leg	10.00 -10.00 0.00	45.0000	127.00	No Ice 1/2" Ice 1" Ice	16.60 19.80 23.00	16.60 19.80 23.00	560.00 700.00 840.00
*									
Proposed TMobile Sept2020 Ericsson AIR 6449 B41 w. MtgPipe (TMO-Alpha)	A	From Face	10.00 8.00 0.00	0.0000	127.00	No Ice 1/2" Ice 1" Ice	5.72 6.03 6.36	3.00 3.41 3.84	135.95 182.56 234.46
Ericsson AIR 6449 B41 w. MtgPipe	C	From Face	10.00 10.00	0.0000	127.00	No Ice 1/2" Ice	5.72 6.03	3.00 3.41	135.95 182.56

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### Job 125' SQR SELF-SUPPORTING TOWER ANALYSIS .

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		$C_AA_A$ Front	$C_AA_A$ Side	Weigh
	LUS		Vert						
			ft ft	٥	ft		ft²	ft²	lb
(TMO-Beta)		and an address of the second of the second			and a survey are surples of the contract of th	1" Ice	6.36	3.84	234.46
Ericsson AIR 6449 B41 w.	D	From Face	10.00	0.0000	127.00	No Ice	5.72	3.00	135.95
	D	Prom Pace	-10.00	0.0000	127.00	1/2" Ice	6.03	3.41	182.56
MtgPipe						1" Ice	6.36	3.84	234.46
(TMO-Gamma)		F F	0.00	0.0000	127.00			10.79	182.20
APXVAARR24_43-U-NA20	A	From Face	10.00	0.0000	127.00	No Ice	20.24		315.89
w. MtgPipe			8.00			1/2" Ice	20.89	12.21	
(TMO-Alpha)	0	F F	0.00	0.0000	127.00	1" Ice	21.55	13.49	460.20
APXVAARR24_43-U-NA20	C	From Face	10.00	0.0000	127.00	No Ice	20.24	10.79	182.20
w. MtgPipe			10.00			1/2" Ice	20.89	12.21	315.89
(TMO-Beta)	-		0.00	0.0000	107.00	1" Ice	21.55	13.49	460.20
APXVAARR24_43-U-NA20	D	From Face	10.00	0.0000	127.00	No Ice	20.24	10.79	182.20
w. MtgPipe			-10.00			1/2" Ice	20.89	12.21	315.89
(TMO-Gamma)			0.00			1" Ice	21.55	13.49	460.20
Ericsson AIR32 (B2A/B66A)	A	From Face	10.00	0.0000	127.00	No Ice	6.51	5.58	146.80
w. Mtg Pipe			8.00			1/2" Ice	6.89	6.18	203.30
(TMO-Alpha)			0.00			1" Ice	7.27	6.80	266.34
Ericsson AIR32 (B2A/B66A)	C	From Face	10.00	0.0000	127.00	No Ice	6.51	5.58	146.80
w. Mtg Pipe			10.00			1/2" Ice	6.89	6.18	203.30
(TMO-Beta)			0.00			1" Ice	7.27	6.80	266.34
Ericsson AIR32 (B2A/B66A)	D	From Face	10.00	0.0000	127.00	No Ice	6.51	5.58	146.80
w. Mtg Pipe			-10.00			1/2" Ice	6.89	6.18	203.30
(TMO-Gamma)			0.00			1" Ice	7.27	6.80	266.34
RY 112 144/1 Double TMA	A	From Face	10.00	0.0000	127.00	No Ice	0.35	0.16	11.00
(TMO-Alpha)			8.00			1/2" Ice	0.43	0.22	14.10
()			0.00			1" Ice	0.51	0.28	18.42
CRY 112 144/1 Double TMA	C	From Face	10.00	0.0000	127.00	No Ice	0.35	0.16	11.00
(TMO-Beta)	_		10.00			1/2" Ice	0.43	0.22	14.10
(=====,			0.00			1" Ice	0.51	0.28	18.42
RY 112 144/1 Double TMA	D	From Face	10.00	0.0000	127.00	No Ice	0.35	0.16	11.00
(TMO-Gamma)			-10.00	0.0000	127.00	1/2" Ice	0.43	0.22	14.10
(1110 Gaillia)			0.00			1" Ice	0.51	0.28	18.42
Ericsson Radio 4449	Α	From Face	10.00	0.0000	127.00	No Ice	0.00	0.00	0.00
B85/B71	11	Trom ruce	8.00	0.0000	127.00	1/2" Ice	0.00	0.00	0.00
(TMO-Alpha)			0.00			1" Ice	0.00	0.00	0.00
	C	From Face		0.0000	127.00	No Ice			
Ericsson Radio 4449	C	rioiii race	10.00	0.0000	127.00		0.00	0.00	0.00
B85/B71			10.00			1/2" Ice	0.00	0.00	0.00
(TMO-Beta)	ъ		0.00	0.0000	107.00	1" Ice	0.00	0.00	0.00
Ericsson Radio 4449	D	From Face	10.00	0.0000	127.00	No Ice	0.00	0.00	0.00
B85/B71			-10.00			1/2" Ice	0.00	0.00	0.00
(TMO-Gamma)			0.00			1" Ice	0.00	0.00	0.00
Ericsson RRUS 4415 B25	Α	From Face	10.00	0.0000	127.00	No Ice	1.64	0.68	46.00
(TMO-Alpha)			8.00			1/2" Ice	1.80	0.79	58.43
E . DDIIG 4415 D05	-	г г	0.00	0.0000	105.00	1" Ice	1.97	0.91	73.23
Ericsson RRUS 4415 B25	C	From Face	10.00	0.0000	127.00	No Ice	1.64	0.68	46.00
(TMO-Beta)			10.00			1/2" Ice	1.80	0.79	58.43
E-i DDIIS 4415 D25	D	F F	0.00	0.0000	107.00	1" Ice	1.97	0.91	73.23
Ericsson RRUS 4415 B25	D	From Face	10.00	0.0000	127.00	No Ice	1.64	0.68	46.00
(TMO-Gamma)			-10.00			1/2" Ice	1.80	0.79	58.43
D	,	r r	0.00	0.0000	107.00	1" Ice	1.97	0.91	73.23
Commscope SDX1926Q-43	Α	From Face	10.00	0.0000	127.00	No Ice	0.24	0.17	6.20
Diplexer			8.00			1/2" Ice	0.31	0.22	8.67
(TMO-Alpha)			0.00			1" Ice	0.38	0.29	12.24
Commscope SDX1926Q-43	В	From Face	10.00	0.0000	127.00	No Ice	0.24	0.17	6.20
Diplexer			10.00			1/2" Ice	0.31	0.22	8.67
(TMO-Beta)			0.00	2004 124007	SESSION VALUE IN	1" Ice	0.38	0.29	12.24
Commscope SDX1926Q-43	D	From Face	10.00	0.0000	127.00	No Ice	0.24	0.17	6.20
Diplexer			-10.00			1/2" Ice	0.31	0.22	8.67



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Leg	$C_AA_A$ Side	Weigh
True		
(TMO-Gamma)	ft²	lb
3"Sch40 x 10ft	0.29	12.24
3"Sch40 x 10ft		
Solidaria   Soli	2.92	75.00
3"Sch40 x 10ft	4.54	99.95
3"Sch40 x 10ft	5.30	131.52
Section   1/2"   Ice     4.54   1/2"   Ice     5.30   1/2"   Ice     4.54   1/2"   Ice     5.30   1/2"   Ice     4.54   1/2"   Ice     5.30   1/2"   Ice   1	2.92	75.00
3"Sch40 x 10ft D From Face 10.00 0.0000 125.00 No Ice 2.92 1/2" Ice 5.30 0.00 1/2" Ice 5.30 1/2" Ice 5.30 1/2" Ice 5.30 3"Sch40 x 10ft D From Face 10.00 0.0000 125.00 No Ice 2.92 1/2" Ice 5.30 No Ice 2.92 1/2" Ice 4.54 1/2" Ice 5.30 No Ice 2.92 1/2" Ice 4.54 1/2" Ice 5.30 1/2" Ice 3.30 1/2" Ice 0.96 1/2" Ice 0.96 1/2" Ice 0.96 1/2" Ice 0.96 1/2" Ice 1.41 1/2" Ice	4.54	99.95
3"Sch40 x 10ft	5.30	131.52
Solid   1/2"   Ice     4.54     1/2"   Ice     5.30     1/2"   Ice   1.41     1/2"   Ice   1.41     1/2"   Ice     1.41     1/2"   Ice     1.41     1/2"   Ice     1.41     1/2"   Ice     1.41     1/2"   Ice     1.41	2.92	75.00
3"Sch40 x 10ft D From Face 10.00 0.0000 125.00 No Ice 2.92 1/2" Ice 4.54 1/" Ice 5.30	4.54	99.95
3"Sch40 x 10ft	5.30	131.52
Section   Sect	2.92	75.00
*Proposed TMobile Sept2020  * BA40-67-DIN UHF Omni	4.54	99.95
**  BA40-67-DIN UHF Omni Dipole Dipol	5.30	131.52
Dipole		
Dipole	2.00	11.00
C   From Leg   10.00   0.0000   129.00   No Ice   0.51	3.30	18.00
4' Sidearm Mount C From Leg 10.00 0.0000 129.00 No Ice 0.51 1/2" Ice 0.96 1" Ice 1.41  ***  Mount Frames D From Face 2.00 0.0000 75.00 No Ice 30.00 1/2" Ice 40.50 0.00 1" Ice 51.00  Mount Frames D From Face 2.00 0.0000 75.00 No Ice 30.00 1/2" Ice 40.50 1" Ice 51.00  Mount Frames D From Face 2.00 0.0000 88.00 No Ice 30.00 1/2" Ice 40.50 1" Ice 51.00  Mount Frames D From Face 2.00 0.0000 88.00 No Ice 30.00 1/2" Ice 40.50 1/2" Ice 40.50 1/2" Ice 40.50 1/2" Ice 51.00  ***  ***  Rest Platform-Half C From Face 0.00 0.0000 25.00 No Ice 35.20 10.00 1/2" Ice 47.00 1/2" Ice 47.00 1/2" Ice 51.00  **  Rest Platform-full D From Face 1.50 0.0000 75.00 No Ice 40.90 1/2" Ice 52.70 0.00 1/2" Ice 52.70 1" Ice 64.50 Rest Platform-full C From Face 1.50 0.0000 75.00 No Ice 40.90 1/2" Ice 52.70 1" Ice 64.50 Rest Platform-Half C From Face 1.50 0.0000 75.00 No Ice 40.90 1/2" Ice 52.70 1" Ice 64.50 Rest Platform-Half C From Face 1.50 0.0000 75.00 No Ice 35.20 1/2" Ice 47.00 1/2" Ice 52.70 1" Ice 64.50 1/2" Ice 52.70 1" Ice 64.50 1/2" Ice 47.00 1/2" Ice 52.70 1/2" Ice 64.50 1/2" Ice 66.50 1/2" Ice 67.00 1/2" Ice 67.00 1/2" I	4.60	25.00
-5.00	2.54	43.00
Mount Frames	4.03	64.00
Mount Frames	5.52	85.00
Mount Frames	3.34	83.00
Mount Frames   C   From Face   2.00   0.0000   75.00   No Ice   30.00	20.00	750.00
Mount Frames   C   From Face   2.00   0.0000   75.00   No Ice   30.00	27.00	1012.50
Mount Frames	34.00	1275.00
Mount Frames	20.00	750.00
Mount Frames   D   From Face   2.00   0.0000   88.00   No Ice   30.00   1/2" Ice   40.50   0.000   1   Ice   51.00	27.00	1012.50
Mount Frames   D   From Face   2.00   0.0000   88.00   No Ice   30.00   1/2" Ice   40.50   0.00   1   Ice   51.00	34.00	1275.00
-6.00	20.00	750.00
**  **  Rest Platform-Half  C From Face  0.00  0.000  10.00  10.00  11/2" Ice  47.00  11/2" Ice  47.00	27.00	1012.50
**  Rest Platform-Half  C From Face  0.00 0.000 0.00000 0.00000 0.00000 0.00000 0.00000 0.00000 0.00000 0.00000 0.00000 0.000000	34.00	1275.00
Rest Platform-Half   C   From Face   0.00   0.0000   25.00   No Ice   35.20		
10.00		
Rest Platform-full	22.00	1066.60
Rest Platform-full	27.00	1523.70
0.00	33.00	1980.80
Rest Platform-full   C   From Face   1.50   0.0000   75.00   No Ice   40.90	38.00	1235.80
Rest Platform-full   C   From Face   1.50   0.0000   75.00   No Ice   40.90	44.00	1765.40
0.00	50.00	2295.00
Rest Platform-Half C From Face 0.00 0.0000 88.00 No Ice 35.20 5.00 1/2" Ice 47.00 1" Ice 58.80 **  22' Protection Frame/ Shield C From Face 1.00 0.000 34.00 - 12.00 No Ice 59.42 0.00 1" Ice 117.64 22' Protection Frame/ Shield A From Face 1.00 0.000 34.00 - 12.00 No Ice 69.20 1" Ice 117.64 22' Protection Frame/ Shield A From Face 1.00 0.0000 34.00 - 12.00 No Ice 69.20	38.00	1235.80
Rest Platform-Half C From Face 0.00 0.0000 88.00 No Ice 35.20 5.00 1/2" Ice 47.00 0.00 1" Ice 58.80 **  22' Protection Frame/ Shield C From Face 1.00 0.000 34.00 - 12.00 No Ice 69.20 1/2" Ice 93.42 0.00 1" Ice 117.64 22' Protection Frame/ Shield A From Face 1.00 0.000 34.00 - 12.00 No Ice 69.20 0.00 1" Ice 117.64	44.00	1765.40
5.00 1/2" Ice 47.00 1" Ice 58.80  **  22' Protection Frame/ Shield C From Face 1.00 0.0000 34.00 - 12.00 No Ice 69.20 1/2" Ice 93.42 0.00 1" Ice 117.64  22' Protection Frame/ Shield A From Face 1.00 0.0000 34.00 - 12.00 No Ice 69.20	50.00	2295.00
**  0.00  1" Ice 58.80  22' Protection Frame/ Shield C From Face 1.00 0.0000 34.00 - 12.00 No Ice 69.20 -9.50 1/2" Ice 93.42 0.00 1" Ice 117.64  22' Protection Frame/ Shield A From Face 1.00 0.0000 34.00 - 12.00 No Ice 69.20	22.00	1066.60
** 22' Protection Frame/ Shield C From Face 1.00 0.0000 34.00 - 12.00 No Ice 69.20 -9.50 1/2" Ice 93.42 0.00 1" Ice 117.64 22' Protection Frame/ Shield A From Face 1.00 0.0000 34.00 - 12.00 No Ice 69.20	27.00	1523.70
22' Protection Frame/ Shield     C     From Face     1.00     0.0000     34.00 - 12.00     No Ice     69.20       -9.50     1/2" Ice     93.42       0.00     1" Ice     117.64       22' Protection Frame/ Shield     A     From Face     1.00     0.0000     34.00 - 12.00     No Ice     69.20	32.00	1980.80
-9.50 1/2" Ice 93.42 0.00 1" Ice 117.64 22' Protection Frame/ Shield A From Face 1.00 0.0000 34.00 - 12.00 No Ice 69.20	21.70	1000 0
0.00 1" Ice 117.64 22' Protection Frame/ Shield A From Face 1.00 0.0000 34.00 - 12.00 No Ice 69.20	31.70	1000.00
22' Protection Frame/ Shield A From Face 1.00 0.0000 34.00 - 12.00 No Ice 69.20	42.80	1350.00
	53.89	1700.00
	31.70 42.80	1000.00 1350.00
-9.50 1/2" Ice 93.42 0.00 1" Ice 117.64	53.89	1700.00
**	33.07	1700.00
Old Hardline cage A From Face 1.00 0.0000 28.00 - 12.00 No Ice 31.50	9.40	790.00



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Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		$C_AA_A$ Front	$C_AA_A$ Side	Weigh
			Vert ft ft ft	۰	ft		ft²	ft²	lb
			0.00			1/2" Ice	42.52	12.69	1066.5
			0.00			1" Ice	53.55	15.98	1343.0
Old Hardline cage	C	From Face	1.00	0.0000	28.00 - 12.00	No Ice	31.50	9.40	790.00
			0.00			1/2" Ice	42.52	12.69	1066.5
			0.00			1" Ice	53.55	15.98	1343.0
Old Hardline cage	C	From	0.00	0.0000	28.00 - 12.00	No Ice	31.50	9.40	790.0
		Centroid-Fa ce	0.00			1/2" Ice 1" Ice	42.52 53.55	12.69 15.98	1066.5 1343.0
*									
*Proposed Dish Wireless Sept2021									
MX08FRO665-20 w. Mtg	A	From Leg	3.00	0.0000	117.00	No Ice	12.49	7.29	93.90
Pipe			0.00			1/2" Ice	12.99	8.25	183.5
(P-DW-Alpha)			0.00			1" Ice	13.49	9.08	281.6
MX08FRO665-20 w. Mtg	В	From Leg	3.00	0.0000	117.00	No Ice	12.49	7.29	93.90
Pipe			0.00			1/2" Ice	12.99	8.25	183.5
(P-DW-Beta)			0.00			1" Ice	13.49	9.08	281.6
MX08FRO665-20 w. Mtg	C	From Leg	3.00	0.0000	117.00	No Ice	12.49	7.29	93.90
Pipe			0.00			1/2" Ice	12.99	8.25	183.5
(P-DW-Gamma)			0.00			1" Ice	13.49	9.08	281.6
TA08025-B604 RRU	Α	From Leg	3.00	0.0000	117.00	No Ice	1.98	1.04	64.00
(P-DW-Alpha)			0.00			1/2" Ice	2.15	1.18	80.85
	_		0.00			1" Ice	2.33	1.32	100.4
TA08025-B604 RRU	В	From Leg	3.00	0.0000	117.00	No Ice	1.98	1.04	64.00
(P-DW-Beta)			0.00			1/2" Ice	2.15	1.18	80.85
T	~	D .	0.00	0.0000	117.00	1" Ice	2.33	1.32	100.4
TA08025-B604 RRU	C	From Leg	3.00	0.0000	117.00	No Ice	1.98	1.04	64.00
(P-DW-Gamma)			0.00			1/2" Ice	2.15	1.18	80.85
T100005 D605 DBH		T T	0.00	0.0000	117.00	1" Ice No Ice	2.33 1.98	1.32 1.20	100.4 75.00
TA08025-B605 RRU	Α	From Leg	3.00 0.00	0.0000	117.00	1/2" Ice	2.15	1.34	93.09
(P-DW-Alpha)			0.00			1" Ice	2.13	1.34	113.9
TA08025-B605 RRU	В	From Leg	3.00	0.0000	117.00	No Ice	1.98	1.20	75.00
(P-DW-Beta)	D	110m Leg	0.00	0.0000	117.00	1/2" Ice	2.15	1.34	93.09
(i B ii Beill)			0.00			1" Ice	2.33	1.49	113.9
TA08025-B605 RRU	C	From Leg	3.00	0.0000	117.00	No Ice	1.98	1.20	75.00
(P-DW-Gamma)			0.00			1/2" Ice	2.15	1.34	93.09
			0.00			1" Ice	2.33	1.49	113.9
RDIDC-9181-PF-48	Α	From Leg	3.00	0.0000	117.00	No Ice	2.31	1.29	22.00
(P-DW-Alpha)			0.00			1/2" Ice	2.50	1.45	41.25
, ,			0.00			1" Ice	2.70	1.61	63.41
Commscope MTC3975083	Α	From Leg	2.00	0.0000	117.00	No Ice	10.80	6.50	352.0
9'SectorFrame			0,00			1/2" Icc	13.60	9.20	440.0
(P-DW-Alpha)		_	0.00			1" Ice	16.40	9.90	528.0
Commscope MTC3975083	В	From Leg	2.00	0.0000	117.00	No Ice	10.80	6.50	352.0
8'SectorFrame			0.00			1/2" Ice	13.60	8.20	440.0
(P-DW-Beta)		D T	0.00	0.0000	117.00	1" Ice	16.40	9.90	528.0
Commscope MTC3975083	C	From Leg	2.00	0.0000	117.00	No Ice	10.80	6.50	352.0
8'SectorFrame			0.00			1/2" Ice	13.60	8.20	440.0
(P-DW-Gamma)			0.00			1" Ice	16.40	9.90	528.0



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# **Load Combinations**

Comb.		Description
No.		
1	Dead Only	
2	1.2 Dead+1.0 Wind 0 deg - No Ice	
3	0.9 Dead+1.0 Wind 0 deg - No Ice	
4	1.2 Dead+1.0 Wind 45 deg - No Ice	
5	0.9 Dead+1.0 Wind 45 deg - No Ice	
6	1.2 Dead+1.0 Wind 90 deg - No Ice	
7	0.9 Dead+1.0 Wind 90 deg - No Ice	*
8	1.2 Dead+1.0 Wind 135 deg - No Ice	
9	0.9 Dead+1.0 Wind 135 deg - No Ice	
10	1.2 Dead+1.0 Wind 180 deg - No Ice	<i>y</i> .
11	0.9 Dead+1.0 Wind 180 deg - No Ice	
12	1.2 Dead+1.0 Wind 225 deg - No Ice	
13	0.9 Dead+1.0 Wind 225 deg - No Ice	
14	1.2 Dead+1.0 Wind 270 deg - No Ice	
15	0.9 Dead+1.0 Wind 270 deg - No Ice	
16	1.2 Dead+1.0 Wind 315 deg - No Ice	
17	0.9 Dead+1.0 Wind 315 deg - No Ice	
18	1.2 Dead+1.0 Ice+1.0 Temp	
19	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	
20	1.2 Dead+1.0 Wind 45 deg+1.0 Ice+1.0 Temp	
21	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	
22	1.2 Dead+1.0 Wind 135 deg+1.0 Ice+1.0 Temp	
23	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	
24	1.2 Dead+1.0 Wind 225 deg+1.0 Ice+1.0 Temp	
25	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	
26	1.2 Dead+1.0 Wind 315 deg+1.0 Ice+1.0 Temp	
27	Dead+Wind 0 deg - Service	
28	Dead+Wind 45 deg - Service	
29	Dead+Wind 90 deg - Service	
30	Dead+Wind 135 deg - Service	
31	Dead+Wind 180 deg - Service	
32	Dead+Wind 225 deg - Service	
33	Dead+Wind 270 deg - Service	
34	Dead+Wind 315 deg - Service	

# **Maximum Reactions**

Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, Z
		Load Comb.	lb	lb	lb
Leg D	Max. Vert	12	184734.21	29802.84	-26564.79
-	Max. H.	12	184734.21	29802.84	-26564.79
	Max. H <sub>z</sub>	5	-143522.23	-24890.48	22191.00
	Min. Vert	5	-143522.23	-24890.48	22191.00
	Min. H <sub>x</sub>	5	-143522.23	-24890.48	22191.00
	Min. H <sub>z</sub>	12	184734.21	29802.84	-26564.79
Leg C	Max. Vert	8	188809.90	-28397.97	-28555.78
	Max. H <sub>x</sub>	17	-142170.30	23485.04	23568.76
	Max. H <sub>z</sub>	17	-142170.30	23485.04	23568.76
	Min. Vert	17	-142170.30	23485.04	23568.76
	Min. H <sub>x</sub>	8	188809.90	-28397.97	-28555.78
	Min. Hz	8	188809.90	-28397.97	-28555.78
Leg B	Max. Vert	4	185784.02	-27550.52	28922.26
-	Max. H <sub>x</sub>	13	-142733.33	23067.08	-23934.67
	Max. Hz	4	185784.02	-27550.52	28922.26
	Min. Vert	13	-142733.33	23067.08	-23934.67

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Location	Condition	Gov. Load	Vertical lb	Horizontal, X lb	Horizontal, lb
		Comb.			
	Min. H <sub>x</sub>	4	185784.02	-27550.52	28922.26
	Min. Hz	13	-142733.33	23067.08	-23934.67
Leg A	Max. Vert	16	183657.98	29191.75	27166.38
C	Max. H <sub>x</sub>	16	183657.98	29191.75	27166.38
	Max. H <sub>z</sub>	16	183657.98	29191.75	27166.38
	Min. Vert	9	-146034.92	-24707.75	-22793.15
	Min. H <sub>x</sub>	9	-146034.92	-24707.75	-22793.15
	Min. Hz	9	-146034.92	-24707.75	-22793.15

# **Tower Mast Reaction Summary**

Load Combination	Vertical	Shear <sub>x</sub>	Shear <sub>z</sub>	Overturning Moment, M <sub>x</sub>	Overturning Moment, M <sub>z</sub>	Torque
	lb	lb	lb	lb-ft	lb-ft	lb-ft
Dead Only	80250.15	0.00	0.00	53406.45	-80768.19	0.00
1.2 Dead+1.0 Wind 0 deg - No	96300.18	-187.70	-91894.20	-6731946.54	-75381.13	152112.86
Ice						
0.9 Dead+1.0 Wind 0 deg - No	72225.13	-187.70	-91894.20	-6747968.48	-51150.68	152112.86
Ice						
1.2 Dead+1.0 Wind 45 deg - No	96300.18	71563.29	-67124.05	-4901672.30	-5205165.98	125437.75
Ice	, , , , , , , , , , , , , , , , , , , ,	7.10.00.125				
0.9 Dead+1.0 Wind 45 deg - No	72225.13	71563.29	-67124.05	-4917694.23	-5180935.52	125437.75
Ice						
1.2 Dead+1.0 Wind 90 deg - No	96300.18	94009.59	187.70	85628.62	-6837851.99	47453.49
Ice	, , , , , , , , , , , , , , , , , , , ,	, 1003103	20,,,,	02020102		
0.9 Dead+1.0 Wind 90 deg - No	72225.13	94009.59	187.70	69606.68	-6813621.53	47453,49
Ice	12223.13	J <del>4</del> 007.37	107.70	0,000.00	-0013021.33	47455.47
1.2 Dead+1.0 Wind 135 deg -	96300.18	71828.74	67389.50	5060311.18	-5235629.11	-64923.47
No Ice	90300.16	/1020.74	07309.30	3000311.18	-3233029.11	-04923.47
	72225.13	71828.74	67389.50	5044289.25	-5211398.65	-64923.48
0.9 Dead+1.0 Wind 135 deg -	12223.13	/1828.74	0/389.30	3044289.23	-3211398.03	-04923.40
No Ice	06200 10	105.50	0100100	6060100.00	110460.50	150110.00
1.2 Dead+1.0 Wind 180 deg -	96300.18	187.70	91894.20	6860122.02	-118462.52	-152112.86
No Ice						
0.9 Dead+1.0 Wind 180 deg -	72225.13	187.70	91894.20	6844100.08	-94232.06	-152112.86
No Ice						
1.2 Dead+1.0 Wind 225 deg -	96300.18	-71563.29	67124.05	5029847.77	5011322.33	-125437.75
No Ice						
0.9 Dead+1.0 Wind 225 deg -	72225.13	-71563.29	67124.05	5013825.84	5035552.78	-125437.75
No Ice						
1.2 Dead+1.0 Wind 270 deg -	96300.18	<b>-</b> 94009.59	-187.70	42546.86	6644008.34	-47453.49
No Ice						
0.9 Dead+1.0 Wind 270 deg -	72225.13	-94009.59	-187.70	26524.93	6668238.79	-47453.49
No Ice						
1.2 Dead+1.0 Wind 315 deg -	96300.18	-71828.74	-67389.50	-4932135.71	5041785.46	64923.48
No Ice						
0.9 Dead+1.0 Wind 315 deg -	72225.13	<b>-</b> 71828.74	-67389.50	-4948157.64	5066015.91	64923.48
No Ice						
1.2 Dead+1.0 Ice+1.0 Temp	185924.44	0.00	0.00	73790.11	-241867.19	0.00
1.2 Dead+1.0 Wind 0 deg+1.0	185924.44	-31.63	-24313.48	-1770662.54	-238349.89	43819.92
Ice+1.0 Temp						
1.2 Dead+1.0 Wind 45 deg+1.0	185924.44	19565.09	-18083.63	-1290826.28	-1649209.85	41697.67
Ice+1.0 Temp						
1.2 Dead+1.0 Wind 90 deg+1.0	185924.44	25681.68	31.63	77307.45	-2101989.55	19517.87
Ice+1.0 Temp	AMERICAN (1971) 1975 1975					
1.2 Dead+1.0 Wind 135	185924.44	19609.83	18128.37	1443380.77	-1654184.06	-15552.93
deg+1.0 Ice+1.0 Temp					and the second second	
1.2 Dead+1.0 Wind 180	185924.44	31.63	24313.48	1918242.75	-245384.48	-43819.91

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Load	Vertical	$Shear_x$	Shearz	Overturning	Overturning	Torque
Combination	lb	lb	lb	Moment, $M_x$ $lb$ -ft	Moment, $M_z$ $lb$ -ft	lb-ft
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 225	185924.44	-19565.09	18083.63	1438406.49	1165475.48	-41697.66
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 270	185924.44	-25681.68	-31.63	70272.76	1618255.18	-19517.87
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 315	185924.44	-19609.83	-18128.37	-1295800.56	1170449.68	15552.94
deg+1.0 Ice+1.0 Temp Dead+Wind 0 deg - Service	80250.15	-48.53	-23758.91	-1703683.71	-75198.92	39328.23
Dead+Wind 45 deg - Service Dead+Wind 90 deg - Service	80250.15 80250.15	18502.43 24305.84	-17354.68 48.53	-1230472.90 58975.76	-1401486.30 -1823611.38	32431.48 12268.93
Dead+Wind 135 deg - Service	80250.15	18571.06	17423.31	1345162.01	-1409362.43 -86337.46	-16785.73 -39328.23
Dead+Wind 180 deg - Service Dead+Wind 225 deg - Service	80250.15 80250.15	48.53 -18502.43	23758.91 17354.68	1810496.61 1337285.80	1239949.92	-32431.48
Dead+Wind 270 deg - Service Dead+Wind 315 deg - Service	80250.15 80250.15	-24305.84 -18571.06	-48.53 -17423.31	47837.13 -1238349.11	1662075.00 1247826.05	-12268.93 16785.73

## **Solution Summary**

	Sui	m of Applied Forces			Sum of Reaction:		
Load	PX	PY	PZ	PX	PY	PZ	% Erroi
Comb.	lb	lb	lb	lb	lb	lb	
1	0.00	-80250.15	0.00	-0.00	80250.15	-0.00	0.000%
2	-187.70	-96300.18	-91894.20	187.70	96300.18	91894.20	0.000%
3	-187.70	-72225.13	-91894.20	187.70	72225.13	91894.20	0.000%
4	71563.29	-96300.18	-67124.05	-71563.29	96300.18	67124.05	0.000%
5	71563.29	-72225.13	-67124.05	-71563.29	72225.13	67124.05	0.000%
6	94009.59	-96300.18	187.70	<b>-</b> 94009.59	96300.18	-187.70	0.000%
7	94009.59	-72225.13	187.70	-94009.59	72225.13	-187.70	0.000%
8	71828.74	-96300.18	67389.50	-71828.74	96300.18	<b>-</b> 67389.50	0.000%
9	71828.74	-72225.13	67389.50	-71828.74	72225.13	-67389.50	0.000%
10	187.70	-96300.18	91894.20	-187.70	96300.18	-91894.20	0.000%
11	187.70	-72225.13	91894.20	-187.70	72225.13	-91894.20	0.000%
12	-71563.29	-96300.18	67124.05	71563.29	96300.18	-67124.05	0.000%
13	-71563.29	-72225.13	67124.05	71563.29	72225.13	-67124.05	0.000%
14	-94009.59	-96300.18	-187.70	94009.59	96300.18	187.70	0.000%
15	-94009.59	-72225.13	-187.70	94009.59	72225.13	187.70	0.000%
16	-71828.74	-96300.18	-67389.50	71828.74	96300.18	67389.50	0.000%
17	-71828.74	-72225.13	-67389.50	71828.74	72225.13	67389.50	0.000%
18	0.00	-185924.44	0.00	-0.00	185924.44	-0.00	0.000%
19	-31.63	-185924.44	-24313.48	31.63	185924.44	24313.48	0.000%
20	19565.09	-185924.44	-18083.63	-19565.09	185924.44	18083.63	0.000%
21	25681.68	-185924.44	31.63	-25681.68	185924.44	-31.63	0.000%
22	19609.83	-185924.44	18128.37	-19609.83	185924.44	-18128.37	0.000%
23	31.63	-185924.44	24313.48	-31.63	185924.44	-24313.48	0.000%
24	-19565.09	-185924.44	18083.63	19565.09	185924.44	-18083.63	0.000%
25	-25681.68	-185924.44	-31.63	25681.68	185924.44	31.63	0.000%
26	-19609.83	-185924.44	-18128.37	19609.83	185924.44	18128.37	0.000%
27	-48.53	-80250.15	-23758.92	48.53	80250.15	23758.91	0.000%
28	18502.43	-80250.15	-17354.68	-18502.43	80250.15	17354.68	0.000%
29	24305.84	-80250.15	48.53	-24305.84	80250.15	-48.53	0.000%
30	18571.06	-80250.15	17423.31	-18571.06	80250.15	-17423.31	0.000%
31	48.53	-80250.15	23758.92	-48.53	80250.15	-23758.91	0.000%
32	-18502.43	-80250.15	17354.68	18502.43	80250.15	-17354.68	0.000%
33	-24305.84	-80250.15	-48.53	24305.84	80250.15	48.53	0.000%
34	-18571.06	-80250.15	-17423.31	18571.06	80250.15	17423.31	0.000%



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## **Maximum Tower Deflections - Service Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.	ft	Deflection in	Load Comb.	0	۰
T1	125 - 112.5	0.564	30	0.0249	0,0083
T2	112.5 - 100	0.492	30	0.0243	0.0077
T3	100 - 75	0.416	30	0.0229	0.0073
T4	75 - 62.5	0.273	30	0.0179	0.0063
T5	62.5 - 50	0.210	30	0.0150	0.0051
T6	50 - 25	0.157	34	0.0115	0.0040
T7	25 - 0	0.071	34	0.0046	0.0021

# Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	٥	fi
140.00	Flash Beacon Lighting	30	0.564	0.0249	0.0083	568997
129.00	BA40-67-DIN UHF Omni Dipole	30	0.564	0.0249	0.0083	568997
127.00	12' GENERIC BOOM	30	0.564	0.0249	0.0083	568997
125.00	Top Platform - West Peak	30	0.564	0.0249	0.0083	568997
119.00	2L 2 1/2x2 1/2x1/4x3/8 @ 10ft	30	0.530	0.0247	0.0080	474162
117.00	MX08FRO665-20 w. Mtg Pipe	30	0.518	0.0246	0.0079	355707
107.00	Full Access Platform	30	0.459	0.0238	0.0076	575932
88.00	Mount Frames	30	0.345	0.0207	0.0070	409726
75.00	Mount Frames	30	0.273	0.0179	0.0063	292258
34.00	22' Protection Frame/ Shield	34	0.100	0.0069	0.0028	281723
28.50	22' Protection Frame/ Shield	34	0.082	0.0054	0.0023	280616
28.00	Old Hardline cage	34	0.081	0.0053	0.0023	280447
25.00	Rest Platform-Half	34	0.071	0.0046	0.0021	287008
23.00	22' Protection Frame/ Shield	34	0.065	0.0041	0.0019	303156
22.67	Old Hardline cage	34	0.064	0.0040	0.0019	306850
17.50	22' Protection Frame/ Shield	34	0.049	0.0030	0.0015	395011
17.33	Old Hardline cage	34	0.048	0.0029	0.0014	398810
12.00	22' Protection Frame/ Shield	34	0.033	0.0020	0.0010	576057

# **Maximum Tower Deflections - Design Wind**

Section No.	Elevation	110rz. Deflection	Gov. Load	Tilt	Twist
IVO.	ft	in	Comb.	۰	•
T1	125 - 112.5	2.094	8	0.0867	0.0322
T2	112.5 - 100	1.836	8	0.0858	0.0299
T3	100 - 75	1.563	8	0.0809	0.0283
T4	75 - 62.5	1.043	8	0.0631	0.0242
T5	62.5 - 50	0.809	8	0.0536	0.0196
T6	50 - 25	0.604	16	0.0414	0.0155
T7	25 - 0	0.270	16	0.0164	0.0080



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# Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	0	ft
140.00	Flash Beacon Lighting	8	2.094	0.0867	0.0322	164840
129.00	BA40-67-DIN UHF Omni Dipole	8	2.094	0.0867	0.0322	164840
127.00	12' GENERIC BOOM	8	2.094	0.0867	0.0322	164840
125.00	Top Platform - West Peak	8	2.094	0.0867	0.0322	164840
119.00	2L 2 1/2x2 1/2x1/4x3/8 @ 10ft	8	1.972	0.0866	0.0309	137367
117.00	MX08FRO665-20 w. Mtg Pipe	8	1.931	0.0864	0.0304	103051
107.00	Full Access Platform	8	1.717	0.0842	0.0292	173980
88.00	Mount Frames	8	1.307	0.0730	0.0269	134184
75.00	Mount Frames	8	1.043	0.0631	0.0242	107491
34.00	22' Protection Frame/ Shield	16	0.380	0.0247	0.0107	79090
28.50	22' Protection Frame/ Shield	16	0.311	0.0195	0.0091	77519
28.00	Old Hardline cage	16	0.305	0.0190	0.0089	77478
25.00	Rest Platform-Half	16	0.270	0.0164	0.0080	79319
23.00	22' Protection Frame/ Shield	16	0.246	0.0147	0.0074	83791
22.67	Old Hardline cage	16	0.242	0.0145	0.0073	84813
17.50	22' Protection Frame/ Shield	16	0.184	0.0106	0.0057	109183
17.33	Old Hardline cage	16	0.182	0.0105	0.0056	110233
12.00	22' Protection Frame/ Shield	16	0.125	0.0070	0.0039	159225

# **Bolt Design Data**

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt lb	Allowable Load per Bolt lb	Ratio Load Allowable	Allowable Ratio	Criteria
T1	125	Leg	A307	0.7500	16	1325.81	24850.50	0.053	1	Bolt DS
		Diagonal	A307	0.7500	5	1332.93	10222.50	0.130	1	Member Block Shear
		Secondary Horizontal	A307	0.7500	2	745.53	10467.20	0.071	1	Member Block Shear
		Top Girt	A307	0.7500	4	288.42	12425.20	0.023	1	<b>Bolt Shear</b>
T2	112.5	Diagonal	A307	0.7500	4	2183.47	9855.47	0.222	1	Member Block Shear
		Secondary Horizontal	A307	0.7500	2	628.03	12425.20	0.051	1	Bolt Shear
		Top Girt	A307	0.7500	3	1174.74	24850.50	0.047	1	<b>Bolt Shear</b>
T3	100	Leg	A307	0.7500	16	7730.82	24850.50	0.311	1	Bolt DS
		Diagonal	A307	0.7500	4	2774.02	10535.20	0.263	1	Member Block Shear
		Horizontal	A307	0.7500	3	1082.34	20843.80	0.052	1	Member Block Shear
		Secondary Horizontal	A307	0.7500	2	465.14	11146.90	0.042	1	Member Block Shear
		Top Girt	A307	0.7500	3	1104.93	20843.80	0.053	1	Member Block Shear
T4	75	Diagonal	A307	0.7500	2	9065.35	20118.80	0.451	1	Member Block Shear
		Horizontal	A307	0.7500	2	6233.59	22293.80	0.280	1	Member Block Shear
T5	62.5	Leg	A307	0.7500	20	9469.42	24850.50	0.381	1	Bolt DS



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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt lb	Allowable Load per Bolt lb	Ratio Load Allowable	Allowable Ratio	Criteria
		Diagonal	A307	0.7500	2	9095.41	20118.80	0.452	1	Member Block Shear
		Horizontal	A307	0.7500	2	6438.73	20934.40	0.308	1	Member Block Shear
T6	50	Leg	A307	0.7500	24	11036.90	24850.50	0.444	1	Bolt DS
		Diagonal	A307	0.7500	2	9111.83	20118.80	0.453	1	Member Block Shear
		Horizontal	A307	0.7500	2	6959.02	20934.40	0.332	1	Member Block Shear
T7	25	Leg	A307	0.7500	28	10009.70	24850.50	0.403	1	Bolt DS
		Diagonal	A307	0.7500	4	8295.65	24850.50	0.334	1	Bolt Shear
		Horizontal	A307	0.7500	2	9083.76	24850.50	0.366	1	Bolt Shear

## **Compression Checks**

## Leg Design Data (Compression)

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio $P_u$
	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
T1	125 - 112.5	L6x6x1/2	12.57	6.72	68.4 K=1.00	5.7500	-10606.50	176569.00	0.060 1
T2	112.5 - 100	L6x6x1/2	12.57	6.67	67.8 K=1.00	5.7500	-25981.40	177053.00	0.147 1
T3	100 - 75	L6x6x5/8	25.14	6.63	67.4 K=1.00	7.1100	-61846.60	219395.00	0.282 1
T4	75 - 62.5	L6x6x3/4	12.57	6.29	64.5 K=1.00	8.4400	-75377.60	264139.00	0.285 1
T5	62.5 - 50	L6x6x3/4	12.57	6.29	64.5 K=1.00	8.4400	-94694.20	264139.00	0.359 1
T6	50 - 25	L6x6x7/8	25.14	6.29	64.5 K=1.00	9.7300	-132443.00	304511.00	0.435 1
T7	25 - 0	L8x8x7/8	25.14	8.38	64.1 K=1.00	13.2000	-140742.00	413897.00	0.340 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

# Diagonal Design Data (Compression)

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
T1	125 - 112.5	L3 1/2x3 1/2x1/4	16.49	8.82	137.3	1.6900	-7958.67	25667.50	0.310 1



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Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	$Ratio$ $P_u$
	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
					K=0.90				1/
T2	112.5 - 100	L3x3 1/2x1/4	17.91	9.51	162.7 K=0.90	1.5600	<b>-</b> 9936.95	16869.90	0.589
Т3	100 - 75	L4x3x1/4	20.97	11.00	182.5 K=0.90	1.6900	-12014.80	14521.10	0.827
T4	75 - 62.5	2L2 1/2x2 1/2x1/4x3/8	15.15	15.15	145.9 K=0.90	2.3800	-18614.50	31288.50	0.595
T5	62.5 - 50	2L2 1/2x2 1/2x1/4x3/8	15.79	15.79	151.4 K=0.90	2.3800	-18625.70	29110.90	0.640
Т6	50 - 25	2L2 1/2x2 1/2x1/4x3/8	17.15	17.15	163.2 K=0.90	2.3800	-18711.20	25143.30	0.744
T7	25 - 0	2L3x3 1/2x3/8x3/8	28.04	28.04	185.4 K=0.90	4.5900	-33182.60	37976.50	0.874

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

# **Horizontal Design Data (Compression)**

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio $P_u$
	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
Т3	100 - 75	2L3x2 1/2x1/4x3/8	18.13	18.13	230.2 K=1.00	2.6300	-1797.76	14210.10	0.127 1
		KL/R > 200 (C) - 59							•
T4	75 - 62.5	2L3x2 1/2x1/4x3/8	20.00	10.00	114.3 K=0.90	2.6300	-11988.90	55794.60	0.215 1
T5	62.5 - 50	2L2 1/2x2 1/2x1/4x3/8	21.88	10.94	153.6 K=0.90	2.3800	-12814.50	28869.80	0.444 1
T6	50 - 25	2L2 1/2x2 1/2x1/4x3/8	25.63	12.81	179.9 K=0.90	2.3800	-13773.40	21038.40	0.655 1
T7	25 - 0	2L3x2 1/2x5/16x3/8	27.50	13.75	158.5 K=0.90	3.2422	-15499.70	36926.20	0.420 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

# Secondary Horizontal Design Data (Compression)

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	lb	lb	$\frac{1}{\Phi P_n}$
T1	125 - 112.5	L2 1/2x2x1/4	13.37	13.37	270.9 K=1.00	1.0600	-1645.35	4135.04	0.398 1
T2	112.5 - 100	KL/R > 250 (C) - 18 C6x8.2	15.26	15.26	340.9 K=1.00	2.4000	-1256.05	4665.59	0.269 1
Т3	100 - 75	KL/R > 250 (C) - 39 L3x2 1/2x1/4	19.02	19.02	303.6	1.3100	-930.29	4067.34	0.229 1



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Section	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio
No.									$P_u$
	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
Non-transportation and a second					K=1.00				V

KL/R > 250 (C) - 61

# **Top Girt Design Data (Compression)**

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	lb	· lb	$\phi P_n$
T1	125 - 112.5	C9x13.4	12.50	12.50	224.2 K=1.00	3.9400	-200.73	17705.40	0.011
		KL/R > 200 (C) - 8							
T2	112.5 - 100	2L 3 1/2 x 3 x 7/16 x 3/8	14.38	14.38	159.6 K=1.00	5.3047	-562.07	59609.60	0.009
Т3	100 - 75	2L3x2 1/2x1/4x3/8	16.25	16.25	206.3 K=1.00	2.6300	-1063.63	17678.60	0.060
		KL/R > 200 (C) - 47							•

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

## Redundant Horizontal (1) Design Data (Compression)

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio $P_u$
	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
T4	75 - 62.5	L2x2x3/16	5.00	5.00	137.1 K=0.90	0.7150	-1133.82	10894.50	0.104 1
T5	62.5 - 50	L2x2x3/16	5.47	5.47	149.9 K=0.90	0.7150	-1424.38	9106.94	0.156 1
T6	50 - 25	L2x2x3/16	6.41	6.41	175.6 K=0.90	0.7150	-1992.19	6636.53	0.300 1
T7	25 - 0	L2 1/2x2x3/16	4.58	4.58	115.9 K=0.90	0.8090	-2117.03	12919.40	0.164 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

## Redundant Horizontal (2) Design Data (Compression)

Section	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio
No.									$P_u$
	ft		ft	ft		in <sup>2</sup>	lb	lb	$\Phi P_n$
T7	25 - 0	2L2 1/2x2 1/2x1/4x3/8	9.17	9.17	128.7	2.3800	-2117.03	32225.20	0.066 1
					K=0.90				

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls



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Section	Elevation	Size	L	<i>L</i>	Kl/r	A	$P_{"}$	$\phi P_n$	Ratio
37.	Bieraiion	2.20	~				- •	τ- π	P
NO.						. 2	11	**	
	ft		ft	ft		in²	lb	lb	$\phi P_n$
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<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

## Redundant Diagonal (1) Design Data (Compression)

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio $P_u$
	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
T4	75 - 62.5	L2 1/2x2x3/16	7.73	7.73	195.6 K=0.90	0.8090	-876.90	6051.30	0.145 1
T5	62.5 - 50	L2 1/2x2x3/16	8.02	8.02	202.8 K=0.90	0.8090	-1044.13	5630.72	0.185 1
T6	50 - 25	L2 1/2x2 1/2x3/16	8.63	8.63	188.4 K=0.90	0.9020	-1342.40	7276.13	0.184 1
T7	25 - 0	L2-1/2x2-1/2x3/16	9.25	9.25	128.2 K=0.90	0.9020	-2135.54	12295.70	0.174 1

 $<sup>{}^{1}</sup>P_{u}/\phi P_{n}$  controls

# Redundant Diagonal (2) Design Data (Compression)

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
T7	25 - 0	2L2 1/2x2x1/4x3/8	11.95	11.95	164.6 K=0.90	2.1300	-9287.09	17768.50	0.523 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

## **Redundant Sub-Horizontal Design Data (Compression)**

Section	Elevation	Size	L	$L_u$	Kl/r	А	$P_u$	$\phi P_n$	Katto
No.									$P_u$
	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
T7	25 - 0	2L3x2 1/2x1/4x3/8	10.42	10.42	132.3	2.6300	-10921.90	43022.60	0.254 1
					K=1.00				/

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

## **Inner Bracing Design Data (Compression)**



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Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio $P_u$
	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
T4	75 - 62.5	2L2 1/2x2x3/16x3/8	14.14	14.14	107.8 K=0.50	1.6200	-51.32	34620.20	0.001
T5	62.5 - 50	2L2 1/2x2x3/16x3/8	15.47	15.47	117.0 K=0.50	1.6200	-52.83	31664.10	0.002
T6	50 - 25	2L2 1/2x2 1/2x3/16x3/8	18.12	18.12	139.7 K=0.50	1.8000	-59.22	26383.30	0.002
T7	25 - 0	L2 1/2x2 1/2x1/4	19.45	19.45	237.6 K=0.50	1.1900	-103.22	6032.12	0.017

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

## **Tension Checks**

Leg Design Data (Tension)												
Section No.	Elevation	Size	L	$L_u$	Kl/r	Α	$P_u$	$\phi P_n$	Ratio $P_u$			
	ft		ft	ft		$in^2$	lb	lb	${\phi P_n}$			
T1	125 - 112.5	L6x6x1/2	12.57	6.72	43.4	5.7500	2979.17	186300.00	0.016			
T2	112.5 - 100	L6x6x1/2	12.57	6.67	43.0	5.7500	13934.30	186300.00	0.075			
Т3	100 - 75	L6x6x5/8	25.14	6.63	43.2	7.1100	43201.80	230364.00	0.188			
T4	75 - 62.5	L6x6x3/4	12.57	6.29	41.2	8.4400	51732.30	273456.00	0.189			
T5	62.5 - 50	L6x6x3/4	12.57	6.29	41.2	8.4400	68695.10	273456.00	0.251			
Т6	50 - 25	L6x6x7/8	25.14	6.29	41.7	9.7300	100777.00	315252.00	0.320			
T7	25 - 0	L8x8x7/8	25.14	8.38	41.0	13.2000	108510.00	427680.00	0.254			

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

Diagonal Design Data (Tension)											
Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P.,		
	ft		ft	ft		$in^2$	lb	lb	$\frac{\Pi}{\Phi P_n}$		
T1	125 - 112.5	L3 1/2x3 1/2x1/4	16.49	8.82	97.1	1.1034	6664.67	47999.50	0.139 1		
T2	112.5 - 100	L3x3 1/2x1/4	17.91	9.51	124.9	1.0059	8733.88	43758.30	0.200 1		
T3	100 - 75	L4x3x1/4	20.97	11.00	147.2	1.1034	11096.10	47999.50	0.231 1		



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Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	lb	lb	$\frac{1}{\phi P_n}$
T4	75 - 62.5	2L2 1/2x2 1/2x1/4x3/8	15.15	15.15	152.7	1.4569	18130.70	63374.10	0.286
T5	62.5 - 50	2L2 1/2x2 1/2x1/4x3/8	15.79	15.79	159.2	1.4569	18190.80	63374.10	0.287
Т6	50 - 25	2L2 1/2x2 1/2x1/4x3/8	16.46	16.46	166.0	1.4569	18223.70	63374.10	0.288
T7	25 - 0	2L3x3 1/2x3/8x3/8	28.04	28.04	201.5	2.9503	29354.50	128339.00	0.229

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

# Horizontal Design Data (Tension)

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	lb	lb	${\Phi P_n}$
Т3	100 - 75	2L3x2 1/2x1/4x3/8	18.13	18.13	230.2	1.6444	3247.03	71530.30	0.045 1
T4	75 - 62.5	2L3x2 1/2x1/4x3/8	20.00	10.00	127.0	1.6444	12467.20	71530.30	0.174 1
T5	62.5 - 50	2L2 1/2x2 1/2x1/4x3/8	21.88	10.94	170.7	1.4569	12877.50	63374.10	0.203 1
Т6	50 - 25	2L2 1/2x2 1/2x1/4x3/8	25.63	12.81	199.9	1.4569	13918.00	63374.10	0.220 1
Т7	25 - 0	2L3x2 1/2x5/16x3/8	27.50	13.75	176.1	2.0215	18167.50	87934.60	0.207 1

 $<sup>^{1}</sup> P_{u} / \phi P_{n}$  controls

# **Secondary Horizontal Design Data (Tension)**

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio
	fi		ſì	ft		$in^2$	lb	lb	$\frac{1}{\phi P_n}$
T1	125 - 112.5	L2 1/2x2x1/4	13.37	13.37	270.9	0.6309	1491.07	27445.80	0.054 1
T2	112.5 - 100	C6x8.2	15.26	15.26	340.9	1.6688	1060.03	72590.60	0.015 1
T3	100 - 75	L3x2 1/2x1/4	17.14	17.14	273.6	0.8184	930.29	35602.00	0.026 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls



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	Top Girt Design Data (Tension)									
Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>	
110.	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$	
T1	125 - 112.5	C9x13.4	12.50	12.50	224.2	2.8021	1153.68	121891.00	0.009 1	
T2	112.5 - 100	2L 3 1/2 x 3 x 7/16 x 3/8	14.38	14.38	159.6	3.4043	3524.21	148087.00	0.024 1	
Т3	100 - 75	2L3x2 1/2x1/4x3/8	16.25	16.25	206.3	1.6444	3314.80	71530.30	0.046 1	

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

Redundant Horizontal (1)	Design Data	(Tension)
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Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio $P_u$
1101	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
T4	75 - 62.5	L2x2x3/16	5.00	5.00	97.2	0.7150	1133.82	23166.00	0.049 1
T5	62.5 - 50	L2x2x3/16	5.47	5.47	106.4	0.7150	1424.38	23166.00	0.061 1
T6	50 - 25	L2x2x3/16	6.41	6.41	124.6	0.7150	1992.19	23166.00	0.086 1
Т7	25 - 0	L2 1/2x2x3/16	4.58	4.58	128.8	0.8090	2117.03	26211.60	0.081 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

# Redundant Horizontal (2) Design Data (Tension)

Section	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P.,
No.	ft		ft	ft		$in^2$	lb	lb	$\frac{u}{\phi P_n}$
T7	25 - 0	2L2 1/2x2 1/2x1/4x3/8	9.17	9.17	143.0	2.3800	2117.03	77112.00	0.027 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

# Redundant Diagonal (1) Design Data (Tension)

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio $P_u$
	ft		ft	ft		in <sup>2</sup>	lb	lb	$\phi P_n$
T4	75 - 62.5	L2 1/2x2x3/16	7.73	7.73	154.7	0.8090	876.90	26211.60	0.033 1



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Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	$Ratio$ $P_u$
	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
									V
T5	62.5 - 50	L2 1/2x2x3/16	8.02	8.02	160.4	0.8090	1044.13	26211.60	0.040 1
									1
T6	50 - 25	L2 1/2x2 1/2x3/16	8.32	8.32	128.3	0.9020	1395.46	29224.80	0.048 1
									V
T7	25 - 0	L2-1/2x2-1/2x3/16	9.25	9.25	142.5	0.9020	2135.54	29224.80	$0.073^{-1}$
									V

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

# Redundant Diagonal (2) Design Data (Tension)

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio $P_u$
	fŧ		ft	ft		$in^2$	lb	lb	$\phi P_n$
T7	25 - 0	2L2 1/2x2x1/4x3/8	11.95	11.95	182.8	2.1300	6623.25	69012.00	0.096 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

## Redundant Sub-Horizontal Design Data (Tension)

Section	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio
No.									$P_u$
	ft		ft	ft		in²	lb	lb	$\phi P_n$
T7	25 - 0	2L3x2 1/2x1/4x3/8	10.42	10.42	132.3	2.6300	7150.90	85212.00	0.084 1
									1.00

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

## **Section Capacity Table**

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	øP <sub>allow</sub> lb	% Capacity	Pass Fail
T1	125 - 112.5	Leg	L6x6x1/2	2	-10606.50	176569.00	6.0	Pass
T2	112.5 - 100	Leg	L6x6x1/2	22	-25981.40	177053.00	14.7	Pass
T3	100 - 75	Leg	L6x6x5/8	42	-61846.60	219395.00	28.2	Pass
							31.1 (b)	
T4	75 - 62.5	Leg	L6x6x3/4	78	-75377.60	264139.00	28.5	Pass
T5	62.5 - 50	Leg	L6x6x3/4	115	-94694.20	264139.00	35.9	Pass
		7 3					38.1 (b)	
T6	50 - 25	Leg	L6x6x7/8	152	-132443.00	304511.00	43.5	Pass
							44.4 (b)	
T7	25 - 0	Leg	L8x8x7/8	222	-140742.00	413897.00	34.0	Pass
							40.3 (b)	



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### Job 125' SQR SELF-SUPPORTING TOWER ANALYSIS .

Project
Dish Wireless BOHVN00204B West Peak/Meriden, CT

Client

Everest Infrastructure - 638281

Page

28 of 29

Date

08:21:04 09/22/21

Designed by PB

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	øP <sub>allow</sub> lb	% Capacity	Pas. Fai
T1	125 - 112.5		I 2 1/2-2 1/2 1/4					-
		Diagonal	L3 1/2x3 1/2x1/4	9	-7958.67	25667.50	31.0	Pass
T2	112.5 - 100	Diagonal	L3x3 1/2x1/4	32	-9936.95	16869.90	58.9	Pass
T3	100 - 75	Diagonal	L4x3x1/4	52	-12014.80	14521.10	82.7	Pass
T4	75 - 62.5	Diagonal	2L2 1/2x2 1/2x1/4x3/8	92	-18614.50	31288.50	59.5	Pass
T5	62.5 - 50	Diagonal	2L2 1/2x2 1/2x1/4x3/8	129	-18625.70	29110.90	64.0	Pass
T6	50 - 25	Diagonal	2L2 1/2x2 1/2x1/4x3/8	166	-18711.20	25143.30	74.4	Pas
T7	25 - 0	Diagonal	2L3x3 1/2x3/8x3/8	226	-33182.60	37976.50	87.4	Pas
Т3	100 - 75	Horizontal	2L3x2 1/2x1/4x3/8	59	-1797.76	14210.10	12.7	Pas
T4	75 - 62.5	Horizontal	2L3x2 1/2x1/4x3/8	88	-11988.90	55794.60	21.5	Pas
							28.0 (b)	
T5	62.5 - 50	Horizontal	2L2 1/2x2 1/2x1/4x3/8	125	-12814.50	28869.80	44.4	Pass
T6	50 - 25	Horizontal	2L2 1/2x2 1/2x1/4x3/8	162	-13773.40	21038.40	65.5	Pass
T7	25 - 0	Horizontal	2L3x2 1/2x5/16x3/8	225	-15499.70	36926.20	42.0	Pas
T1	125 - 112.5	Secondary Horizontal	L2 1/2x2x1/4	18	-1645.35	4135.04	39.8	Pass
T2	112.5 - 100	Secondary Horizontal	C6x8.2	39	-1256.05	4665.59	26.9	Pass
T3	100 - 75	Secondary Horizontal	L3x2 1/2x1/4	61	-930.29	4067.34	22.9	Pass
T1								
	125 - 112.5	Top Girt	C9x13.4	8	<b>-</b> 200.73	17705.40	1.1 2.3 (b)	Pas
T2	112.5 - 100	Top Girt	2L 3 1/2 x 3 x 7/16 x 3/8	25	3524.21	148087.00	2.4 4.7 (b)	Pas
T3	100 - 75	Top Girt	2L3x2 1/2x1/4x3/8	47	-1063.63	17678.60	6.0	Pass
T4	75 - 62.5	Redund Horz 1 Bracing	L2x2x3/16	86	-1133.82	10894.50	10.4	Pas
T5	62.5 - 50	Redund Horz 1 Bracing	L2x2x3/16	123	-1424.38	9106.94	15.6	Pas
T6	50 - 25	Redund Horz 1 Bracing	L2x2x3/16	160	-1992.19	6636.53	30.0	Pas
T7	25 - 0	Redund Horz 1 Bracing	L2 1/2x2x3/16	232	<b>-</b> 2117.03	12919.40	16.4	Pas
T7	25 - 0	Redund Horz 2 Bracing	2L2 1/2x2 1/2x1/4x3/8	233	-2117.03	32225.20	6.6	Pas
T4	75 - 62.5	Redund Diag 1 Bracing	L2 1/2x2x3/16	91	-876.90	6051.30	14.5	Pass
T5	62.5 - 50	Redund Diag 1 Bracing	L2 1/2x2x3/16	124	-1044.13	5630.72	18.5	Pass
Т6	50 - 25	Redund Diag 1 Bracing	L2 1/2x2 1/2x3/16	165	-1342.40	7276.13	18.4	Pass
T7	25 - 0	Redund Diag 1 Bracing	L2-1/2x2-1/2x3/16	241	-2135.54	12295.70	17.4	Pass
T7	25 - 0	Redund Diag 2 Bracing	2L2 1/2x2x1/4x3/8	230	-9287.09	17768.50	52.3	Pass
T7	25 - 0	Redund Sub Horz Bracing	2L3x2 1/2x1/4x3/8	236	-10921.90	43022.60	25.4	Pass
T4	75 - 62.5	Inner Bracing	2L2 1/2x2x3/16x3/8	113	-13.50	20248.70	1.0	Pass
T5	62.5 - 50	Inner Bracing	2L2 1/2x2x3/16x3/8	150	-14.65	16926.20	1.1	Pass
T6	50 - 25	Inner Bracing	2L2 1/2x2 1/2x3/16x3/8	187	-17.51	13191.70	1.3	Pass
T7	25 - 0	Inner Bracing	L2 1/2x2 1/2x3/10x3/8					
11	23 - 0	milei Diaenig	L4 1/484 1/481/4	273	-103.22	6032.12	1.7	Pass
						Lag (TC)	Summary	D
						Leg (T6)	44.4	Pass
						Diagonal (T7)	87.4	Pass
						Horizontal (T6)	65.5	Pass
						Secondary Horizontal (T1)	39.8	Pass
						Top Girt (T3)	6.0	Pass
						Redund Horz 1 Bracing (T6)	30.0	Pass



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Job 125' SQR SELF-SUPPORTING TOWER ANALYSIS .	Page 29 of 29
Project Dish Wireless BOHVN00204B West Peak/Meriden, CT	Date 08:21:04 09/22/21
Client Everest Infrastructure - 638281	Designed by PB

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	øP <sub>allow</sub> lb	% Capacity	Pass Fail
THE REAL PROPERTY OF THE PERSON NAMED IN COLUMN 1	COLUMN TO THE STATE OF THE STAT					Redund	6.6	Pass
						Horz 2		
						Bracing (T7)		-
						Redund	18.5	Pass
						Diag 1		
						Bracing (T5)		
						Redund	52.3	Pass
						Diag 2		
						Bracing (T7)		
						Redund Sub	25.4	Pass
						Horz		
						Bracing (T7)		
						Inner	1.7	Pass
						Bracing (T7)		
						Bolt Checks		Pass
						RATING =	87.4	Pass

Client: Everest Infrastructure

Project: DishWireless West PeakCT Calculated By: PB

ARMOR TOWER, INC. 9 N. Main St. 2nd Floor Cortland, NY 13045



Date: Sep 2021

### Check Foundation

Applied Load Factored:

Download := 190.0kip

Uplift := 147.0kip

 $LegShear_c := 40.5 \cdot kip$ 

 $LegShear_u := 33.9 \cdot kip$ 

Check Uplift Capacity:

Top radius of the anchor rods:

Slope of rod:

Length of rod from the base of the pier:

Depth of anchor rod to the bed rock:

Bottom radius of the anchor rod:

Frustum base length for top of the rock:

Frustum base length for bottom og the rock:

Unit wt. of rock:

Weight of inverted frustum of the rock:

Concrete foundation dimension:

Length of top base:

Length of the foundation:

Height of the foundation:

Unit wt. of concrete:

Concrete foundation weight:

Top base length of the soil:

Unit wt. of soil:

r := 41.5in

 $\theta := 14 \cdot \deg$ 

1 := 20ft

 $h_r := 1 \cdot \cos(\theta) = 19.4 \cdot ft$ 

 $R := r + 1 \cdot \sin(\theta) = 8.3 \cdot ft$ 

 $x := 2(R + 19.4ft \cdot tan(45deg)) = 55.4 \cdot ft$ 

 $y := 2 \cdot R = 16.6 \cdot ft$ 

 $\gamma_r := 150 pcf$ 

 $W_r := \frac{h_r}{3} \left( x^2 + x \cdot y + y^2 \right) \cdot \gamma_r = 4136.3 \cdot \text{kip}$ 

a := 5ft

b := 8.25 ft

 $h_f := 6.5ft$ 

 $\mu_c := 150 pcf$ 

 $W_c := \frac{1}{2} \cdot \left(a^2 + a \cdot b + b^2\right) h_f \cdot \mu_c = 43.7 \cdot \text{kip}$ 

 $x_s := x + 2 \cdot h_f \cdot tan (10deg) = 57.7 \cdot ft$ 

 $\gamma_s := 110 pcf$ 

Weight of soil fill above the rock:

 $W_{s} := \left[ \frac{1}{3} \cdot \left( x_{s}^{2} + x_{s} \cdot x + x^{2} \right) - \frac{1}{3} \left( a^{2} + a \cdot b + b^{2} \right) \right] \cdot h_{f} \cdot \gamma_{s} = 2254 \cdot \text{kip}$ 

Total uplift capacity:

 $\phi := 0.75$   $P_n := W_r + W_c + W_s = 6434 \cdot \text{kip}$ 

Uplift

Client: Everest Infrastructure

Project: DishWireless West PeakCT Calculated By: PB

Date: Sep 2021

ARMOR TOWER, INC.

9 N. Main St. 2nd Floor Cortland, NY 13045



## Check Bearing Capacity:

Area of the bottom footing:

Overturning moment for a single pier:

Overturning moment soil bearing:

Total ultimate Bearing Load:

Allowable bearing capacity of the Bedrock:

Safety factor:

Ultimate Bearing capacity of the Bedrock:

$$A := b^2 = 68.1 \cdot ft^2$$

 $OTM := LegShear_u \cdot h_f = 220.3 \cdot kip \cdot ft$ 

$$f_b := \frac{OTM}{\left(b^3 \cdot \frac{\sqrt{2}}{12}\right)} = 3329.8 \cdot psf$$

$$P_b := \frac{\text{Download} + 1.2W_c}{A} + f_b = 6891 \cdot psf$$

P. = 8ksf Very conservative

FS := 2.0

 $P_{ult} := FS \cdot P_n = 16 \cdot ksf$ 

Client: Everest Infrastructure Project: DishWireless West PeakCT

Calculated By: PB Date: Sep 2021

ARMOR TOWER, INC.

9 N. Main St. 2nd Floor Cortland, NY 13045



Chech Anchor Rods TIA 4.9.9:

Assuming F1554-36

Number of bolts:

n := 4

 $Fu_{bolt} := 58 \cdot ksi$ 

Bolt diameter: Bolt $\theta := 2$ in

 $Fy_{bolt} := 36 \cdot ksi$ 

$$A_g := \frac{\pi}{4} \cdot (Bolt\theta)^2$$

$$\mathbf{A_g} \coloneqq \frac{\pi}{4} \cdot (\mathbf{Bolt}\theta)^2 \qquad \qquad \mathbf{n} \coloneqq 4.5 \quad \mathrm{tpi} \qquad \qquad \mathbf{A_n} \coloneqq \frac{\pi}{4} \cdot \left(\mathbf{Bolt}\theta - \frac{0.9743 \cdot \mathrm{in}}{\mathbf{n}}\right)^2$$

 $l_{ar} := 0 \cdot in$  Leveling nut on top of pier

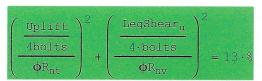
$$\phi_t := 0.75$$

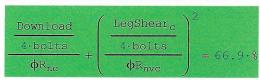
$$\phi_c := 1.00$$

$$\varphi_{\text{t}} \coloneqq \text{0.75} \qquad \varphi_{\text{c}} \coloneqq \text{1.00} \qquad \qquad \varphi \\ \mathsf{R}_{\text{nt}} \coloneqq \varphi_{\text{t}} \cdot \mathsf{Fu}_{\text{bolt}} \cdot \mathsf{A}_{\text{n}} \qquad \qquad \varphi \\ \mathsf{R}_{\text{nc}} \coloneqq \varphi_{\text{c}} \cdot \mathsf{Fy}_{\text{bolt}} \cdot \mathsf{A}_{\text{n}}$$

$$\Phi R_{nc} := \Phi_c \cdot Fy_{bolt} \cdot A_r$$

$$\phi_{v} := 0.75$$





#### Rock Anchors

Rock bolt QTY:

n := 6

Shear moment on the bolts:

 $M := LegShear_u \cdot h_f = 220.35 \cdot kip \cdot ft$ 

Bolt Are Bar $\theta := 2.0 \cdot in$ 

 $BarXArea := \frac{\pi}{4} \cdot Bar\theta^2 = 3.1 \cdot in^2$ 

Section modulus of bolt cluster:

 $S = 10.38 \frac{\text{ft}^3}{\text{ft}^2}$ 

Resultant load:

 $P_r := \frac{\text{Uplift}}{p} + \frac{M}{s} = 45.7 \cdot \text{kip}$ 

Steel grade: A306 Gr.80:

 $F_u := 80 \text{ksi}$ 

Nominal tensile capacity:  $\phi_t := 0.8$   $\phi_t := \phi_t \cdot F_u \cdot (0.75 \cdot A_\sigma) = 150.8 \cdot \text{kip}$ 



### Address:

No Address at This Location

# ASCE 7 Hazards Report

Standard:

ASCE/SEI 7-16

**Elevation:** 1004.55 ft (NAVD 88)

Risk Category: II

Latitude:

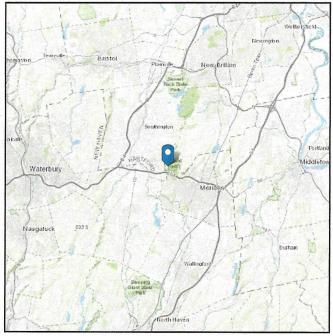
41.561168

Soil Class:

B - Rock

Longitude: -72.843646





### Wind

#### Results:

Wind Speed:

118 Vmph

10-year MRI

75 Vmph

25-year MRI

84 Vmph

50-year MRI 100-year MRI 90 Vmph 98 Vmph

**Data Source:** 

ASCE/SEI 7-16, Fig. 26.5-1B and Figs. CC.2-1-CC.2-4

**Date Accessed:** 

Fri Sep 25 2020

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-16 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

Site is in a hurricane-prone region as defined in ASCE/SEI 7-16 Section 26.2. Glazed openings need not be protected against wind-borne debris.

Mountainous terrain, gorges, ocean promontories, and special wind regions should be examined for unusual wind conditions.



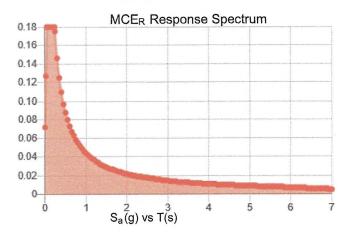
### Seismic

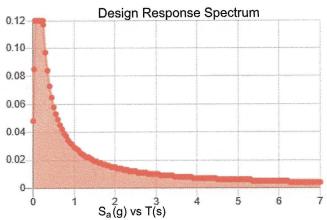
Site Soil Class:	B - Rock
------------------	----------

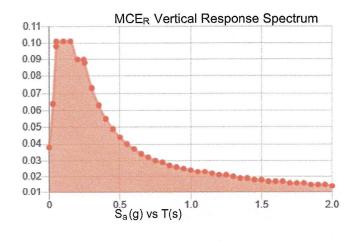
#### Results:

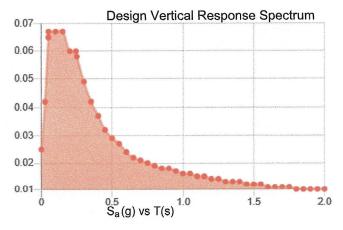
S <sub>s</sub> :	0.2	S <sub>D1</sub> :	0.029
S <sub>1</sub> :	0.055	T <sub>L</sub> :	6
F <sub>a</sub> :	0.9	PGA:	0.11
F <sub>v</sub> :	0.8	PGA <sub>M</sub> :	0.099
S <sub>MS</sub> :	0.18	F <sub>PGA</sub> :	0.9
S <sub>M1</sub> :	0.044	l <sub>e</sub> :	1
S <sub>DS</sub> :	0.12	C <sub>v</sub> :	0.7

### Seismic Design Category









Data Accessed:

Fri Sep 25 2020

USGS Seismic Design Maps based on ASCE/SEI 7-16 and ASCE/SEI 7-16 **Date Source:** Table 1.5-2. Additional data for site-specific ground motion procedures in

accordance with ASCE/SEI 7-16 Ch. 21 are available from USGS.



### Ice

Results:

Ice Thickness:

1.00 in.

Concurrent Temperature:

15 F

Gust Speed:

50 mph

**Data Source:** 

Standard ASCE/SEI 7-16, Figs. 10-2 through 10-8

Date Accessed:

Fri Sep 25 2020

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 500-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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In using this Tool, you expressly assume all risks associated with your use. Under no circumstances shall ASCE or its officers, directors, employees, members, affiliates, or agents be liable to you or any other person for any direct, indirect, special, incidental, or consequential damages arising from or related to your use of, or reliance on, the Tool or any information obtained therein. To the fullest extent permitted by law, you agree to release and hold harmless ASCE from any and all liability of any nature arising out of or resulting from any use of data provided by the ASCE 7 Hazard Tool.

# Exhibit E

**Mount Analysis** 



1033 WATERVLIET SHAKER RD, ALBANY, NY 12205

### **Mount Analysis Report**

**September 27, 2021** 

Dish Wireless Site Number	BOHVN00204B
Job Number	2039-Z5555C
Client	Northeast Site Solutions
Carrier	Dish Wireless
	West Peak Drive,
Site Location	Meriden, CT 06451
Site Education	41.5612 N NAD83
	72.8441 W NAD83
Mount Centerline EL.	117 ft
Mount Classification	Sector Frame
Structural Usage Ratio	61%
Overall Result	Pass

Upon reviewing the results of this analysis, it is our opinion that the structure meets the specified TIA and ASCE code requirements. The proposed mounts for the proposed carrier is therefore deemed **adequate** to support the final loading configuration as listed in this report.



Dmitriy Albul, P.E. Engineering Consultant to Infinigy

### Mount Analysis Report

### September 27, 2021

### **Contents**

Introduction	3
Supporting Documentation	3
Analysis Code Requirements	3
Conclusion	3
Final Configuration Loading	4
Structure Usages	4
Assumptions and Limitations	4
Calculations	Appended

September 27, 2021

### Introduction

Infinigy Engineering has been requested to perform a mount analysis of proposed antenna mount from the Dish Wireless equipment. All supporting documents have been obtained from the client and are assumed to be accurate and applicable to this site. The mount was analyzed using RISA-3D Version 19.0.1 analysis software.

### **Supporting Documentation**

<b>Construction Drawings</b>	Infinigy Engineering PLLC, Job No. 2039-Z5555C, dated June 22, 2021
RF Design Sheet	Dish Wireless, dated June 15, 2021

### **Analysis Code Requirements**

125 mph (3-second Gust, Vult.)
50 mph (3-Second Gust) w/ 0.75" ice
ANSI/TIA-222-G
II
В
Method 2
5
Flat Topped Hill
630
1088
40
Ss=0.183, S <sub>1</sub> =0.063
D – Default (Assumed)
1013.8 ft.

### Conclusion

Upon reviewing the results of this analysis, it is our opinion that the structure meets the specified TIA code requirements. The proposed mounts are therefore deemed adequate to support the final loading configuration as listed in this report.

If you have any questions, require additional information, or actual conditions differ from those as detailed in this report please contact me via the information below:

Dmitriy Albul, P.E.
Professional Engineer | Engineering Consultant to Infinigy
1033 Watervliet Shaker Road, Albany, NY 12205
(O) (518) 690-0790 | (M) (518) 699-4428

www.infinigy.com
BOHVN00204B

### September 27, 2021

### Final Configuration Loading

Mount CL (ft)	Rad. HT (ft)	Vert. O/S (ft)	Horiz. O/S (ft)*	Qty	Appurtenance	Carrier
			7.75	3	JMA MX08FRO665-21	
117.0	117.0		7.75	3	Fujitsu TA08025-B605	Dish
117.0	117.0	-	7.75	3	Fujitsu TA08025-B604	Wireless
*Uavi	00		-	1	Raycap RDIDC-9181-PF-48	

\*Horizontal Offset is defined as the distance from the left most edge of the mount face horizontal when viewed facing the tower.

### **Structure Usages**

Bracing	61%	Pass
Frame Rails	19%	Pass
Plates	19%	Pass
Arms	25%	Pass
Mount Pipes	28%	Pass
Stabilizer	7%	Pass
Rating	61%	Pass

### **Assumptions and Limitations**

Our structural calculations are completed assuming all information provided to Infinigy Engineering is accurate and applicable to this site. For the purposes of calculations, we assume an overall structure condition of "like new" and all members and connections to be free of corrosion and/or structural defects. The structure owner and/or contractor shall verify the structure's condition prior to installation of any proposed equipment. If actual conditions differ from those described in this report Infinigy Engineering should be notified immediately to complete a revised evaluation.

Our evaluation is completed using standard TIA, AISC, ACI, and ASCE methods and procedures. Our structural results are proprietary and should not be used by others as their own. Infinigy Engineering is not responsible for decisions made by others that are or are not based on our supplied assumptions and conclusions.

This report is an evaluation of the proposed carriers mount structure only and does not reflect adequacy of the existing tower, other mounts, or coax mounting attachments. These elements are assumed to be adequate for the purposes of this analysis and are assumed to have been installed per their manufacturer requirements.

### INFINIGY8

### FROM ZERO TO INFINIGY

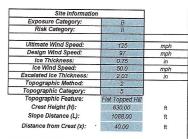
the solutions are endless

Date:	9/27/2021
Site Name:	BOHVN00204B
Project Engineer:	DVA
Infinigy Job No:	2039-Z5555C
Customer:	Northeast Site Solutions
Carrier:	Dish Wireless

Building Code:	2015	
ASCE Standard:	ASCE 7-10	2000.000
TIA Standard:	G	
Mount Type:	Sector Frame	
	Proposed	
Mount Centerline:	117	ft
Superstructure Height:	N/A	ft
Structure Type:	Tower	

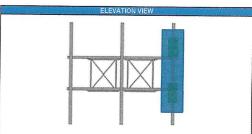
	Factors
Gh:	1.000
K <sub>zmin</sub> :	0.700
K <sub>z</sub> :	1.034
K <sub>d</sub> :	0.950
K <sub>at</sub> :	1.665
Ка:	0.900
I wind:	1,000
lice:	1.000

$q_z$ :	39.23	psf
Surface Wind Pressure:	0.00	nsf



Run Seismic?	Yes
Site Soil:	D (Default,
Short-Period Accel. (Ss):	0.1830
1-Second Accel. (S1):	0.0630
Short-Period Design (SDS):	0.1960
1-Second Design (SD1):	0.1010
Short-Period Coeff. (Fa):	1.6000
1-Second Coeff. (Fv):	2.4000
Cs	0.0980
Cs min	0.0300
Amplification Factor (ap):	1.00
Response Mod. (Rp):	2.50
Overstrength (Ωo):	1.00

	1	
Service Wind:	30.0	mph
Lm (man live load) =	500.0	lb
Lu (man live lead) =	050.0	



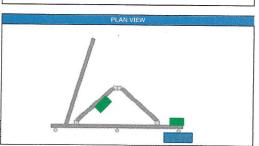


Table 1. Equipment Specifications and Wind Pressure

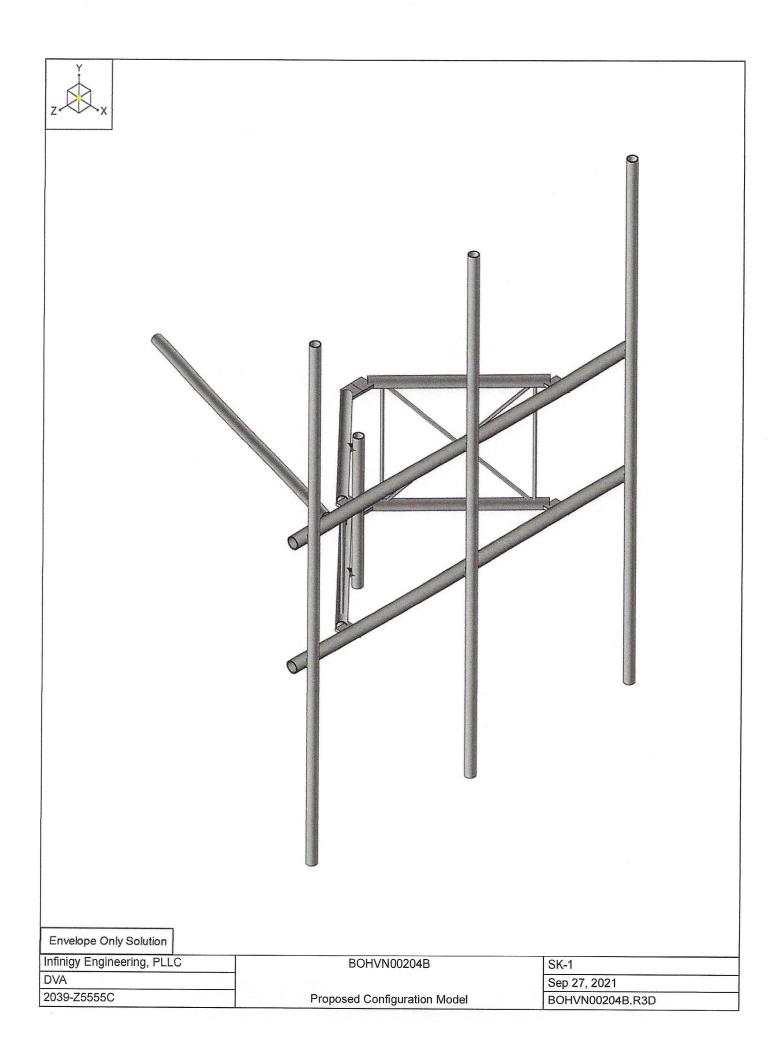
Manufacturer	Model	Elevation	Pipe Label	Weight (lb)	Height (in)	Width (in)	Donth (in)	EPA	EPA-	EDA	EPA-			
JMA	MX08FRO665-21	117	16	64.50	72	20	Depth (III)			L A N W los	A VI 105	$q_z$ :	q z ice	$q_{ziive}$
Fuiitsu	TA08025-B605	117	16	74,90	12	20	8	8.01	3,21	8,80	3.90	39.23	10.46	3.77
Fuiltsu	TA08025-B604	117			15.75	14.96	9.06	1.86	1.16	2.97	2.09	39,23	10,46	3.77
Raycap	RDIDC-9181-PF-48	117	16	63,90	15.75	14.96	7.87	1.86	1.01	2.97	1.91	39.23	10.46	3.77
2 Equipment Wind and S		1117	39	21.85	16	14	8	1.77	1.05	2.87	1,96	39.23	10.46	3.77

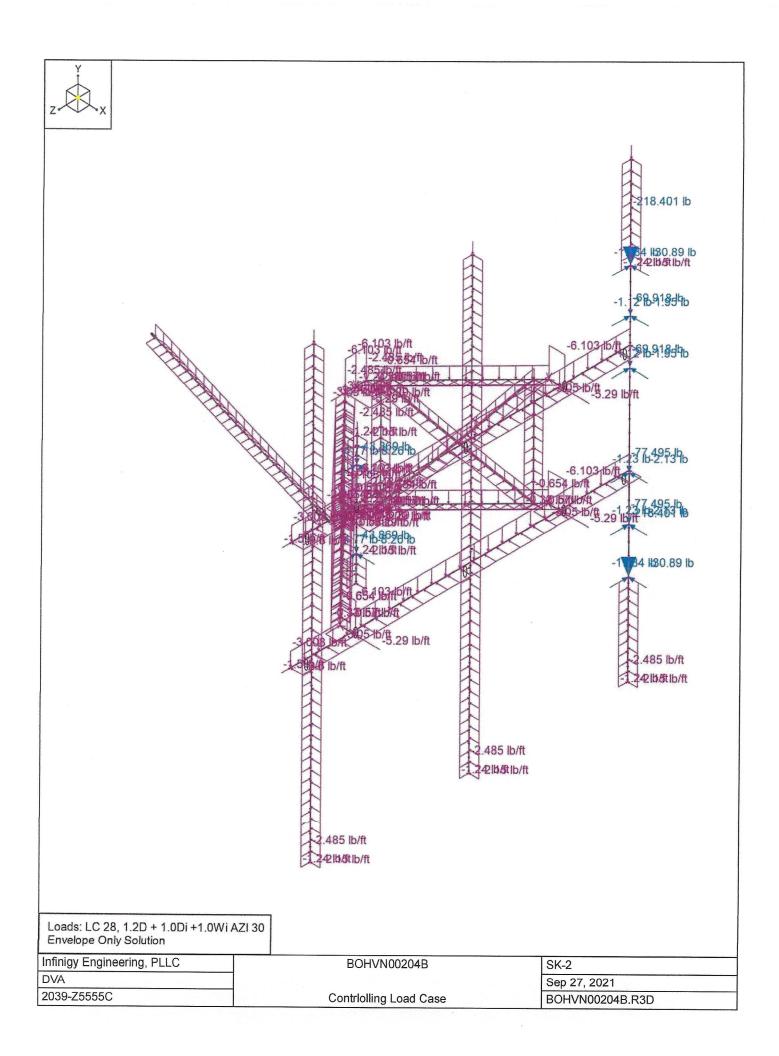
Table 2. Equipment Wind and Seismic Loads

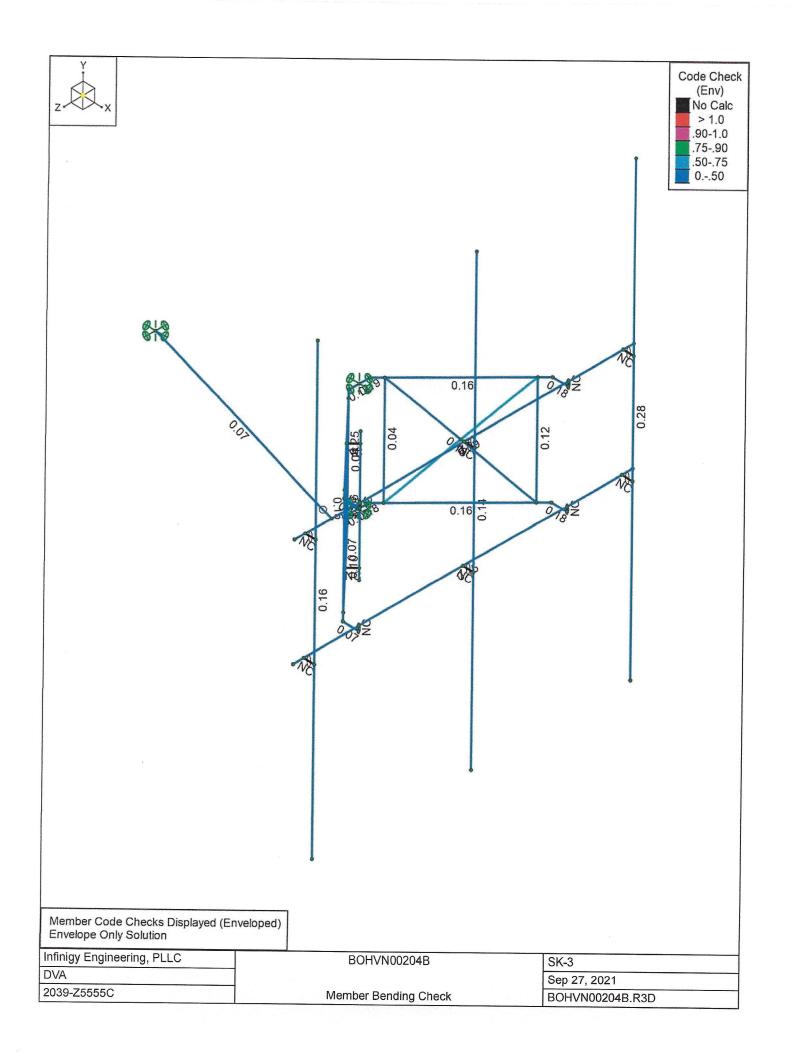
Manufacturer	Model	Wind Lo	ad (F <sub>A</sub> ). lb	Wind	Load Ice Case	(F.). lb	Wind Load	Service Case	Seismic
JMA	MX08FRO665-21	283	113	83	37	359	27	11	Seisimic
Fujitsu	TA08025-B605	66	41	28	20	65	21	- 11	0.3
Fujitsu	TA08025-B604	66	36	28	10	00	6	4	7.3
Raycap	RDIDC-9181-PF-48	62	27	20	18	63	6	3	6.3
	1101000101-11-40	03	3/	21	18	62	6	4	2.1

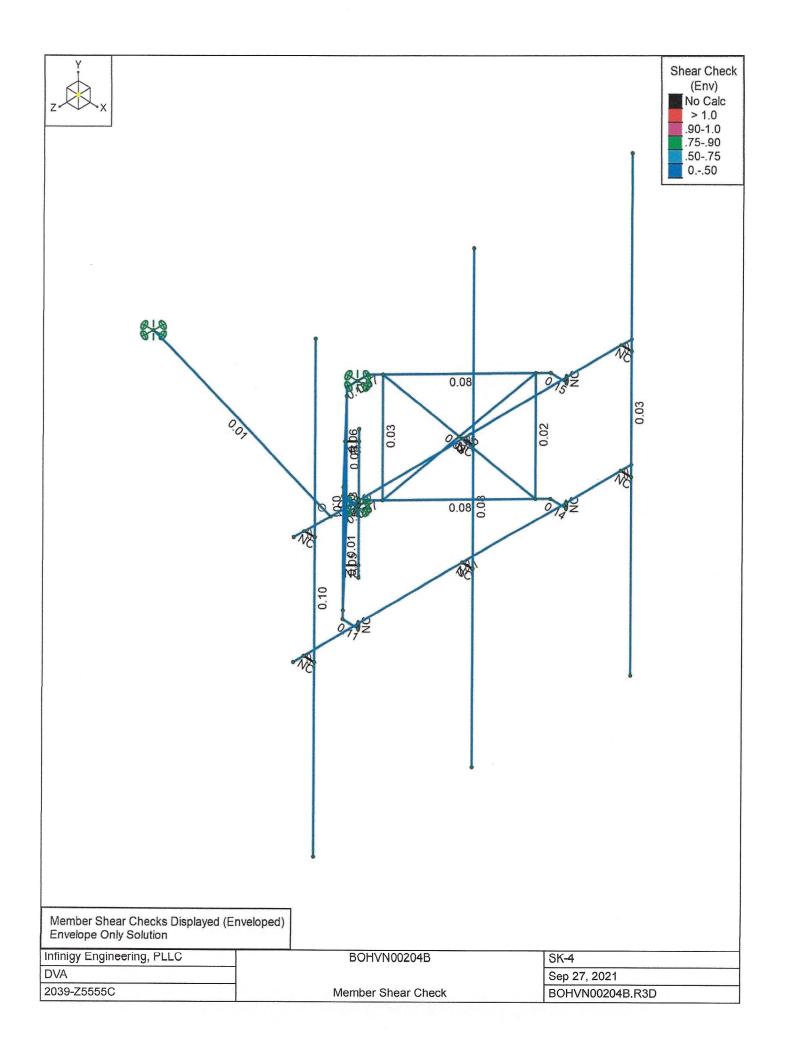
### Table 3, Member Capacities

Member Name	Member Shape	(plf)	Wind Load Ice (plf)	Weight Ice (plf)	Bending Check	Shear Check	Total Capacity	Controlling Capacity
Frame Rail	PIPE_2.5	11.28	3.01	1.47	19%	15%	19%	Jupating
Mount Pipe	PIPE_2.0	9.32	2.48	1.36	28%	10%	28%	
Arm	PIPE_2.0	9.32	2.48	1,36	25%	8%	25%	
Bracing	0.625" SR	2.45	0.65	0.99	61%	3%	61%	61%
Plate	3.5"x0,625" Plate	22.89	6.10	1,60	19%	15%		
Stabilizer	PIPE 2.0	9,32	2.48	1,36	7%	1%	19% 7%	









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Company Designer

: Infinigy Engineering, PLLC

Job Number : 2039-Z5555C Model Name: BOHVN00204B 9/27/2021 3:13:57 PM

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### Model Settings

### Solution

Members

Number of Reported Sections	5
Number of Internal Sections	100
Member Area Load Mesh Size (in²)	144
Consider Shear Deformation	Yes
Consider Torsional Warping	Yes

### Wall Panels

Approximate Mesh Size (in)	24
Transfer Forces Between Intersecting Wood Walls	Yes
Increase Wood Wall Nailing Capacity for Wind Loads	Yes
Include P-Delta for Walls	Yes
Optimize Masonry and Wood Walls	Yes
Maximum Number of Iterations	3

### **Processor Core Utilization**

Single	No
Multiple (Optimum)	Yes
Maximum	No

### Axis

Vertical Global Axis

Global Axis corresponding to vertical direction	Y
Convert Existing Data	Yes

### **Default Member Orientation**

Default Global Plane for z-axis	XZ

### Plate Axis

Plate Local Axis Orientation	Nodal

### Codes

Hot Rolled Steel	AISC 14th (360-10): LRFD
Stiffness Adjustment	Yes (Iterative)
Notional Annex	None
Connections	AISC 14th (360-10): LRFD
Cold Formed Steel	AISI S100-12: LRFD
Stiffness Adjustment	Yes (Iterative)
Wood	AWC NDS-12: ASD
Temperature	< 100F
Concrete	ACI 318-11
Masonry	ACI 530-11: Strength
Aluminum	AA ADM1-10: LRFD
Structure Type	Building
Stiffness Adjustment	Yes (Iterative)
Stainless	AISC 14th (360-10): LRFD
Stiffness Adjustment	Yes (Iterative)

### Concrete

Column Design

Analysis Methodology	Exact Integration Method
Parme Beta Factor	0.65

Compression Stress Block	Rectangular Stress Block
Analyze using Cracked Sections	Yes
Leave room for horizontal rebar splices (2*d bar spacing)	No

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Company : Infinigy Engineering, PLLC Designer : DVA

FROM ZERO TO INFINIGY Job Number : 2039-Z5555C the solutions are endless Model Name: BOHVN00204B 9/27/2021

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Model Settings (Continued)	
List forces which were ignored for design in the Detail Report	Yes
Rebar	
Column Min Steel	1
Column Max Steel	8
Rebar Material Spec	ASTM A615
Warn if beam-column framing arrangement is not understood	No
Shear Reinforcement	
Number of Shear Regions	4
Region 2 & 3 Spacing Increase Increment (in)	4
Seismic RISA-3D Seismic Load Options	
Code	ASCE 7-10
Risk Category	l or II
Drift Cat	Other
Base Elevation (ft)	
Include the weight of the structure in base shear calcs	Yes
Site Parameters	
S <sub>1</sub> (g)	1
SD <sub>1</sub> (g)	
SD <sub>s</sub> (g)	1
T <sub>L</sub> (sec)	5
Structure Characteristics	
TZ (sec)	
TX (sec)	
C,X	0.02
C₁Exp. Z	0.75
C₁Exp. X	0.75
RZ	3
RX	3
$\Omega_0 Z$	1
$\Omega_0 X$	1
$C_dZ$	4
$C_{o}X$	4
ρΖ	1
o X	



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### Member Primary Data

	Label	l Node	J Node	Rotate(deg)	Section/Shape	Туре	Design List	Material	Design Rule
1	M1	N1	N2		Frame Rail	Beam	Pipe	A53 Gr.B	Typical
2	M2	N11	N3	90	Plate	Beam	RECT	A36 Gr.36	Typical
3	M3	N13	N5	90	Plate	Beam	RECT	A36 Gr.36	Typical
4	M4	N29	N3		Arm	Beam	Pipe	A53 Gr.B	Typical
5	M5	N27	N5		Arm	Beam	Pipe	A53 Gr.B	Typical
6	M6	N6	N7		Frame Rail	Beam	Pipe	A53 Gr.B	Typical
7	M7	N17	N8	90	Plate	Beam	RECT	A36 Gr.36	Typical
8	M8	N15	N10	90	Plate	Beam	RECT	A36 Gr.36	Typical
9	M9	N30	N8		Arm	Beam	Pipe	A53 Gr.B	Typical
10	M10	N28	N10		Arm	Beam	Pipe	A53 Gr.B	Typical
11	M11	N11	N12		RIGID	None	None	RIGID	Typical
12	M12	N13	N14		RIGID	None	None	RIGID	Typical
13	M13	N15	N16		RIGID	None	None	RIGID	Typical
14	M14	N17	N18		RIGID	None	None	RIGID	Typical
15	M15	N20	N19		Mount Pipe	Column	Pipe	A53 Gr.B	Typical
16	M16	N22	N21		Mount Pipe	Column	Pipe	A53 Gr.B	Typical
17	M17	N30	N9	90	Plate	Beam	RECT	A36 Gr.36	Typical
18	M18	N29	N4	90	Plate	Beam	RECT	A36 Gr.36	Typical
19	M19	N9	N28	90	Plate	Beam	RECT	A36 Gr.36	Typical
20	M20	N4	N27	90	Plate	Beam	RECT	A36 Gr.36	Typical
21	M21	N25	N33		RIGID	None	None	RIGID	Typical
22	M22	N23	N31		RIGID	None	None	RIGID	Typical
23	M23	N24	N32		RIGID	None	None	RIGID	Typical
24	M24	N26	N34		RIGID	None	None	RIGID	Typical
25	M25	N39	N40		Bracing	VBrace	BAR	A36 Gr.36	Typical
26	M26	N38	N35		Bracing	VBrace	BAR	A36 Gr.36	Typical
27	M27	N37	N36		Bracing	VBrace	BAR	A36 Gr.36	Typical
28	M28	N41	N42		Bracing	VBrace	BAR	A36 Gr.36	Typical
29	M29	N39	N35		Bracing	VBrace	BAR	A36 Gr.36	Typical
30	M30	N40	N38		Bracing	VBrace	BAR	A36 Gr.36	Typical
31	M31	N37	N42		Bracing	VBrace	BAR	A36 Gr.36	Typical
32	M32	N36	N41		Bracing	VBrace	BAR	A36 Gr.36	Typical
33	M33	N51	N52		Stabilizer	HBrace	Pipe	A53 Gr.B	Typical
34	M34	N46	N45		Mount Pipe	Column	Pipe	A53 Gr.B	Typical
35	M35	N48	N50		RIGID	None	None	RIGID	Typical
36	M36	N47	N49		RIGID	None	None	RIGID	Typical
37	M37	N55	N53		RIGID	None	None	RIGID	Typical
38	M38	N56	N54		RIGID	None	None	RIGID	Typical
39	M39	N57	N58		Mount Pipe	Column	Pipe	A53 Gr.B	Typical

### Material Take-Off

	Material	Size	Pieces	Length[in]	Weight[LB]
1	General Members				
2	RIGID		12	28.7	0
3	Total General	and the state of t	12	28.7	0
4					
5	Hot Rolled Steel				and the second second second second
6	A36 Gr.36	0.625" SR	8	291.9	25.396
7	A36 Gr.36	3.5"x0.625" Plate	8	30	18.609
8	A53 Gr.B	PIPE 2.0	9	630.9	182,491
9	A53 Gr.B	PIPE_2.5	2	192	87.656
10	Total HR Steel		27	1144.9	314 151

### Basic Load Cases

	<b>BLC Description</b>	Category	X Gravity	Y Gravity	Z Gravity	Point	Distributed
1	Self Weight	DL		-1		8	
2	Wind Load AZI 0	WLX				16	82



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### Basic Load Cases (Continued)

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Point	Distributed
3	Wind Load AZI 30	None			100 No. 200 No	16	82
4	Wind Load AZI 60	None				16	82
5	Wind Load AZI 90	WLZ				16	82
6	Wind Load AZI 120	None				16	82
7	Wind Load AZI 150	None				16	82
8	Wind Load AZI 180	None				16	82
9	Wind Load AZI 210	None				16	82
10	Wind Load AZI 240	None				16	82
11	Wind Load AZI 270	None				16	82
12	Wind Load AZI 300	None				16	82
13	Wind Load AZI 330	None				16	82
14	Ice Weight	OL1				8	39
15	Ice Wind Load AZI 0	OL2				16	82
16	Ice Wind Load AZI 30	None				16	82
17	Ice Wind Load AZI 60	None				16	82
18	Ice Wind Load AZI 90	OL3				16	82
19	Ice Wind Load AZI 120	None				16	82
20	Ice Wind Load AZI 150	None				16	82
21	Ice Wind Load AZI 180	None				16	82
22	Ice Wind Load AZI 210	None				16	82
23	Ice Wind Load AZI 240	None	10 M			16	82
24	Ice Wind Load AZI 270	None				16	82
25	Ice Wind Load AZI 300	None				16	82
26	Ice Wind Load AZI 330	None				16	82
27	Seismic Load X	ELX			-0.098	8	
28	Seismic Load Z	ELZ	-0.098			8	
29	Service Live Loads	LL					
30	Maintenance Load 1	LL				1	
31	Maintenance Load 2	LL				1	
32	Maintenance Load 3	LL				1	
33	Maintenance Load 4	LL				1	
34	Maintenance Load 5	LL				1	
35	Maintenance Load 6	LL				1	

### **Load Combinations**

	Description	Solve	PDelta	BLC	Factor	BLC	Factor	BLC	Factor
1	1.4DL	Yes	Υ	1	1.4				
2	1.2DL + 1.6WL AZI 0	Yes	Y	1	1.2	2	1:6		
3	1.2DL + 1.6WL AZI 30	Yes	Υ	1	1.2	3	1.6		
4	1.2DL + 1.6WL AZI 60	Yes	Υ	1	1.2	4	1.6		
5	1.2DL + 1.6WL AZI 90	Yes	Υ	1	1.2	5	1.6		
6	1.2DL + 1.6WL AZI 120	Yes	Υ	1	1.2	6	1.6		
7	1.2DL + 1.6WL AZI 150	Yes	Υ	1	1.2	7	1.6		
8	1.2DL + 1.6WL AZI 180	Yes	Υ	1	1.2	8	1.6		
9	1.2DL + 1.6WL AZI 210	Yes	Υ	1	1.2	9	1.6		
10	1.2DL + 1.6WL AZI 240	Yes	Y	1	1.2	10	1.6		
11	1.2DL + 1.6WL AZI 270	Yes	Υ	1	1.2	11	1.6		
12	1.2DL + 1.6WL AZI 300	Yes	Υ	1	1.2	12	1.6		
13	1.2DL + 1.6WL AZI 330	Yes	Υ	1	1.2	13	1.6		
14	0.9DL + 1.6WL AZI 0	Yes	Υ	1	0.9	2	1.6		
15	0.9DL + 1.6WL AZI 30	Yes	Υ	1	0.9	3	1.6		
16	0.9DL + 1.6WL AZI 60	Yes	Υ	1	0.9	4	1.6		
17	0.9DL + 1.6WL AZI 90	Yes	Υ	1	0.9	5	1.6		
18	0.9DL + 1.6WL AZI 120	Yes	Υ	1	0.9	6	1.6		
19	0.9DL + 1.6WL AZI 150	Yes	Υ	1	0.9	7	1.6		
20	0.9DL + 1.6WL AZI 180	Yes	Υ	1	0.9	8	1.6		-
21	0.9DL + 1.6WL AZI 210	Yes	Υ	1	0.9	9	1.6		
22	0.9DL + 1.6WL AZI 240	Yes	Υ	1	0.9	10	1.6		



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### Load Combinations (Continued)

	ioda Gomminationo (Gomanaga)			***************************************					
	Description	Solve	PDelta	BLC	Factor	BLC	Factor	BLC	Factor
23	0.9DL + 1.6WL AZI 270	Yes	Υ	1	0.9	11	1.6		
24	0.9DL + 1.6WL AZI 300	Yes	Υ	1	0.9	12	1.6		
25	0.9DL + 1.6WL AZI 330	Yes	Υ	1	0.9	13	1.6		
26	1.2D + 1.0Di	Yes	Y	1	1.2	14	1		
27	1.2D + 1.0Di +1.0Wi AZI 0	Yes	Y	1	1.2	14	1	15	1
28	1.2D + 1.0Di +1.0Wi AZI 30	Yes	Y	1	1.2	14	1	16	1
29	1.2D + 1.0Di +1.0Wi AZI 60	Yes	Y	1	1.2	14	1	17	1
30	1.2D + 1.0Di +1.0Wi AZI 90	Yes	Y	1	1.2	14	1	18	1
31	1.2D + 1.0Di +1.0Wi AZI 120	Yes	Y	1	1.2	14	1	19	1
									Control of the Contro
32	1.2D + 1.0Di +1.0Wi AZI 150	Yes	Y	11	1.2	14	1	20	1
33	1.2D + 1.0Di +1.0Wi AZI 180	Yes	Υ	1	1.2	14	1	21	1
34	1.2D + 1.0Di +1.0Wi AZI 210	Yes	Y	11	1.2	14	1	22	1
35	1.2D + 1.0Di +1.0Wi AZI 240	Yes	Υ	1	1.2	14	1	23	1
36	1.2D + 1.0Di +1.0Wi AZI 270	Yes	Y	1	1.2	14	1	24	1
37	1.2D + 1.0Di +1.0Wi AZI 300	Yes	Υ	1	1.2	14	1	25	1
38	1.2D + 1.0Di +1.0Wi AZI 330	Yes	Υ	1	1.2	14	1	26	1
39	(1.2 + 0.2Sds)DL + 1.0E AZI 0	Yes	Υ	1	1.239	27	1	28	
40	(1.2 + 0.2Sds)DL + 1.0E AZI 30	Yes	Υ	1	1.239	27	0.866	28	0.5
41	(1.2 + 0.2Sds)DL + 1.0E AZI 60	Yes	Υ	1	1.239	27	0.5	28	0.866
42	(1.2 + 0.2Sds)DL + 1.0E AZI 90	Yes	Υ	1	1.239	27		28	1
43	(1.2 + 0.2Sds)DL + 1.0E AZI 120	Yes	Υ	1	1.239	27	-0.5	28	0.866
44	(1.2 + 0.2Sds)DL + 1.0E AZI 150	Yes	Y	1	1.239	27	-0.866	28	0.5
45	(1.2 + 0.25ds)DL + 1.0E AZI 180	Yes	Y	1	1.239	27	-0.000	28	0.5
							Committee of the Commit		0.5
46	(1.2 + 0.2Sds)DL + 1.0E AZI 210	Yes	Y	1	1.239	27	-0.866	28	-0.5
47	(1.2 + 0.2Sds)DL + 1.0E AZI 240	Yes	Y	1	1.239	27	-0.5	28	-0.866
48	(1.2 + 0.2Sds)DL + 1.0E AZI 270	Yes	Y	1	1.239	27		28	-1
49	(1.2 + 0.2Sds)DL + 1.0E AZI 300	Yes	Y	1	1.239	27	0.5	28	-0.866
50	(1.2 + 0.2Sds)DL + 1.0E AZI 330	Yes	Υ	1	1.239	27	0.866	28	-0.5
51	(0.9 - 0.2Sds)DL + 1.0E AZI 0	Yes	Υ	1	0.861	27	1	28	
52	(0.9 - 0.2Sds)DL + 1.0E AZI 30	Yes	Y	1	0.861	27	0.866	28	0.5
53	(0.9 - 0.2Sds)DL + 1.0E AZI 60	Yes	Υ	1	0.861	27	0.5	28	0.866
54	(0.9 - 0.2Sds)DL + 1.0E AZI 90	Yes	Υ	1	0.861	27		28	1
55	(0.9 - 0.2Sds)DL + 1.0E AZI 120	Yes	Υ	1	0.861	27	-0.5	28	0.866
56	(0.9 - 0.2Sds)DL + 1.0E AZI 150	Yes	Υ	1	0.861	27	-0.866	28	0.5
57	(0.9 - 0.2Sds)DL + 1.0E AZI 180	Yes	Υ	1	0.861	27	-1	28	
58	(0.9 - 0.2Sds)DL + 1.0E AZI 210	Yes	Υ	1	0.861	27	-0.866	28	-0.5
59	(0.9 - 0.2Sds)DL + 1.0E AZI 240	Yes	Y	1	0.861	27	-0.5	28	-0.866
60	(0.9 - 0.2Sds)DL + 1.0E AZI 270	Yes	Y	1	0.861	27	-0.0	28	-1
61	(0.9 - 0.2Sds)DL + 1.0E AZI 270	Yes	Υ	1	0.861	27	0.5	28	-0.866
62	(0.9 - 0.25ds)DL + 1.0E AZI 330		Y	the respective and the	0.861				
		Yes		1	0.001	27	0.866	28	-0.5
63	1.0DL + 1.5LL + 1.0SWL (30 mph) AZI 0	Yes	Y	1		2	0.096	29	1.5
64	1.0DL + 1.5LL + 1.0SWL (30 mph) AZI 30	Yes	Y	1	1	3	0.096	29	1.5
65	1.0DL + 1.5LL + 1.0SWL (30 mph) AZI 60	Yes	Υ	1	1	4	0.096	29	1.5
66	1.0DL + 1.5LL + 1.0SWL (30 mph) AZI 90	Yes	Υ	1	1	5	0.096	29	1.5
67	1.0DL + 1.5LL + 1.0SWL (30 mph) AZI 120	Yes	Υ	1	1	6	0.096	29	1.5
68	1.0DL + 1.5LL + 1.0SWL (30 mph) AZI 150	Yes	Υ	1	1	7	0.096	29	1.5
69	1.0DL + 1.5LL + 1.0SWL (30 mph) AZI 180	Yes	Υ	1	1	8	0.096	29	1.5
70	1.0DL + 1.5LL + 1.0SWL (30 mph) AZI 210	Yes	Υ	1	1	9	0.096	29	1.5
71	1.0DL + 1.5LL + 1.0SWL (30 mph) AZI 240	Yes	Υ	1	1	10	0.096	29	1.5
72	1.0DL + 1.5LL + 1.0SWL (30 mph) AZI 270	Yes	Y	1	1	11	0.096	29	1.5
73	1.0DL + 1.5LL + 1.0SWL (30 mph) AZI 300	Yes	Υ	1	1	12	0.096	29	1.5
74	1.0DL + 1.5LL + 1.0SWL (30 mph) AZI 330	Yes	Υ	1	1	13	0.096	29	1.5
75	1.2DL + 1.5LM1 + 1.6SWL (30 mph) AZI 0	Yes	Υ	1	1.2	34	1.5	2	0.154
76	1.2DL + 1.5LM1 + 1.6SWL (30 mph) AZI 30	Yes	Υ	1	1.2	34	1.5	3	0.154
77	1.2DL + 1.5LM1 + 1.6SWL (30 mph) AZI 60	Yes	Y	1	1.2	34	1.5	4	0.154
78	1.2DL + 1.5LM1 + 1.6SWL (30 mph) AZI 90	Yes	Y	1	1.2	34	1.5	5	0.154
79	1.2DL + 1.5LM1 + 1.6SWL (30 mph) AZI 120	Yes	Y	1	1.2	34	1.5	6	0.154
80	1.2DL + 1.5LM1 + 1.6SWL (30 mph) AZI 150	Yes	Y	1	1.2	34	1.5	7	0.154
001		1 100			1.4	UT	1.0		J. 10-T



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### Load Combinations (Continued)

	oad combinations (continued)								
	Description	Solve	<b>PDelta</b>	BLC	Factor	BLC	Factor	BLC	Factor
81	1.2DL + 1.5LM1 + 1.6SWL (30 mph) AZI 180	Yes	Υ	1	1.2	34	1.5	8	0.154
82	1.2DL + 1.5LM1 + 1.6SWL (30 mph) AZI 210	Yes	Y	1	1.2	34	1.5	9	0.154
83	1.2DL + 1.5LM1 + 1.6SWL (30 mph) AZI 240	Yes	Υ	1	1.2	34	1.5	10	0.154
84	1.2DL + 1.5LM1 + 1.6SWL (30 mph) AZI 270	Yes	Y	1	1.2	34	1.5	11	0.154
85	1.2DL + 1.5LM1 + 1.6SWL (30 mph) AZI 300	Yes	Y	1	1.2	34	1.5	12	0.154
86	1.2DL + 1.5LM1 + 1.6SWL (30 mph) AZI 330	Yes	Y	1	1.2	34	1.5	13	0.154
87	1.2DL + 1.5LM2 + 1.6SWL (30 mph) AZI 0	Yes	Y	1	1.2	35	1.5	2	0.154
88	1.2DL + 1.5LM2 + 1.6SWL (30 mph) AZI 30	Yes	Y	1	1.2	35	1.5	3	0.154
89	1.2DL + 1.5LM2 + 1.6SWL (30 mph) AZI 60	Yes	Y	1	1.2	35	1.5	4	0.154
90	1.2DL + 1.5LM2 + 1.6SWL (30 mph) AZI 90	Yes	Y	1	1.2	35		CONTRACTOR OF THE PARTY OF THE	
91	1.2DL + 1.5LM2 + 1.6SWL (30 mph) AZI 120		Y				1.5	5	0.154
92		Yes		1	1.2	35	1.5	6	0.154
	1.2DL + 1.5LM2 + 1.6SWL (30 mph) AZI 150	Yes	Y	1	1.2	35	1.5	7	0.154
93	1.2DL + 1.5LM2 + 1.6SWL (30 mph) AZI 180	Yes	Υ	1	1.2	35	1.5	8	0.154
94	1.2DL + 1.5LM2 + 1.6SWL (30 mph) AZI 210	Yes	Υ	1	1.2	35	1.5	9	0.154
95	1.2DL + 1.5LM2 + 1.6SWL (30 mph) AZI 240	Yes	Υ	1	1.2	35	1.5	10	0.154
96	1.2DL + 1.5LM2 + 1.6SWL (30 mph) AZI 270	Yes	Y	1	1.2	35	1.5	11	0.154
97	1.2DL + 1.5LM2 + 1.6SWL (30 mph) AZI 300	Yes	Υ	1	1.2	35	1.5	12	0.154
98	1.2DL + 1.5LM2 + 1.6SWL (30 mph) AZI 330	Yes	Υ	1	1.2	35	1.5	13	0.154
99	1.2DL + 1.5LM3 + 1.6SWL (30 mph) AZI 0	Yes	Υ	1	1.2	36	1.5	2	0.154
100	1.2DL + 1.5LM3 + 1.6SWL (30 mph) AZI 30	Yes	Υ	1	1.2	36	1.5	3	0.154
101	1.2DL + 1.5LM3 + 1.6SWL (30 mph) AZI 60	Yes	Υ	1	1.2	36	1.5	4	0.154
102	1.2DL + 1.5LM3 + 1.6SWL (30 mph) AZI 90	Yes	Υ	1	1.2	36	1.5	5	0.154
103	1.2DL + 1.5LM3 + 1.6SWL (30 mph) AZI 120	Yes	Υ	1	1.2	36	1.5	6	0.154
104	1.2DL + 1.5LM3 + 1.6SWL (30 mph) AZI 150	Yes	Y	1	1.2	36	1.5	7	0.154
105	1.2DL + 1.5LM3 + 1.6SWL (30 mph) AZI 180	Yes	Ý	1	1.2	36	1.5	8	0.154
106	1.2DL + 1.5LM3 + 1.6SWL (30 mph) AZI 210	Yes	Y	1	1.2	36	1.5	9	
107	1.2DL + 1.5LM3 + 1.6SWL (30 mph) AZI 240	Yes	Υ	1					0.154
108	1.2DL + 1.5LM3 + 1.6SWL (30 mph) AZI 240				1.2	36	1.5	10	0.154
109		Yes	Y	1	1.2	36	1.5	11	0.154
110	1.2DL + 1.5LM3 + 1.6SWL (30 mph) AZI 300	Yes	Υ	1	1.2	36	1.5	12	0.154
111	1.2DL + 1.5LM3 + 1.6SWL (30 mph) AZI 330	Yes	Y	1	1.2	36	1.5	13	0.154
	1.2DL + 1.5LM4 + 1.6SWL (30 mph) AZI 0	Yes	Υ	1	1.2	37	1.5	2	0.154
112	1.2DL + 1.5LM4 + 1.6SWL (30 mph) AZI 30	Yes	Υ	1	1.2	37	1.5	3	0.154
113	1.2DL + 1.5LM4 + 1.6SWL (30 mph) AZI 60	Yes	Υ	1	1.2	37	1.5	4	0.154
114	1.2DL + 1.5LM4 + 1.6SWL (30 mph) AZI 90	Yes	Υ	1	1.2	37	1.5	5	0.154
115	1.2DL + 1.5LM4 + 1.6SWL (30 mph) AZI 120	Yes	Υ	1	1.2	37	1.5	6	0.154
116	1.2DL + 1.5LM4 + 1.6SWL (30 mph) AZI 150	Yes	Υ	1	1.2	37	1.5	7	0.154
117	1.2DL + 1.5LM4 + 1.6SWL (30 mph) AZI 180	Yes	Υ	1	1.2	37	1.5	8	0.154
118	1.2DL + 1.5LM4 + 1.6SWL (30 mph) AZI 210	Yes	Υ	1	1.2	37	1.5	9	0.154
119	1.2DL + 1.5LM4 + 1.6SWL (30 mph) AZI 240	Yes	Υ	1	1.2	37	1.5	10	0.154
120	1.2DL + 1.5LM4 + 1.6SWL (30 mph) AZI 270	Yes	Y	1	1.2	37	1.5	11	0.154
121	1.2DL + 1.5LM4 + 1.6SWL (30 mph) AZI 300	Yes	Y	1	1.2	37	1.5	12	0.154
122	1.2DL + 1.5LM4 + 1.6SWL (30 mph) AZI 330	Yes	Y	1	1.2	37	1.5	13	0.154
123	1.2DL + 1.5LM5 + 1.6SWL (30 mph) AZI 0	Yes	Y	1	1.2	38	1.5	2	0.154
124	1.2DL + 1.5LM5 + 1.6SWL (30 mph) AZI 30	Yes	Y	1	1.2	38	1.5	3	0.154
125	1.2DL + 1.5LM5 + 1.6SWL (30 mph) AZI 60	Yes	Y	1	1.2	38	1.5		
126	1.2DL + 1.5LM5 + 1.6SWL (30 mph) AZI 90	Yes	Y	1				4	0.154
127	1.2DL + 1.5LM5 + 1.6SWL (30 mph) AZI 90	Yes	Y	1	1.2	38	1.5	5	0.154
128	1.2DL + 1.5LM5 + 1.6SWL (30 mph) AZI 120	Yes	Y	1	1.2	38 38	1.5	6	0.154
129	1.2DL + 1.5LM5 + 1.6SWL (30 mph) AZI 180	Yes	Y	1	1.2	38	1.5	7	0.154
130	1.2DL + 1.5LM5 + 1.6SWL (30 mph) AZI 210	Yes	Y				1.5	8	0.154
131	1.2DL + 1.5LM5 + 1.6SWL (30 mph) AZI 240			1	1.2	38	1.5	9	0.154
132	1.2DL + 1.3LM3 + 1.6SVVL (30 mph) AZI 240 1.2DL + 1.5LM5 + 1.6SVVL (30 mph) AZI 270	Yes	Y	1	1.2	38	1.5	10	0.154
133	1.2DL + 1.5LM5 + 1.6SWL (30 mph) AZI 270 1.2DL + 1.5LM5 + 1.6SWL (30 mph) AZI 300	Yes	Y		1.2	38	1.5	11	0.154
134		Yes	Y	1	1.2	38	1.5	12	0.154
135	1.2DL + 1.5LM5 + 1.6SWL (30 mph) AZI 330	Yes	Y	1	1.2	38	1.5	13	0.154
	1.2DL + 1.5LM6 + 1.6SWL (30 mph) AZI 0	Yes	Υ	1	1.2	39	1.5	2	0.154
136	1.2DL + 1.5LM6 + 1.6SWL (30 mph) AZI 30	Yes	Y	1	1.2	39	1.5	3	0.154
137 138	1.2DL + 1.5LM6 + 1.6SWL (30 mph) AZI 60	Yes	Υ	1	1.2	39	1.5	4	0.154
130	1.2DL + 1.5LM6 + 1.6SWL (30 mph) AZI 90	Yes	Υ	1	1.2	39	1.5	5	0.154



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### Load Combinations (Continued)

	Description	Solve	<b>PDelta</b>	BLC	Factor	BLC	Factor	BLC	Factor
139	1.2DL + 1.5LM6 + 1.6SWL (30 mph) AZI 120	Yes	Υ	1	1.2	39	1.5	6	0.154
140	1.2DL + 1.5LM6 + 1.6SWL (30 mph) AZI 150	Yes	Υ	1	1.2	39	1.5	7	0.154
141	1.2DL + 1.5LM6 + 1.6SWL (30 mph) AZI 180	Yes	Υ	1	1.2	39	1.5	8	0.154
142	1.2DL + 1.5LM6 + 1.6SWL (30 mph) AZI 210	Yes	- Y	1	1.2	39	1.5	9	0.154
143	1.2DL + 1.5LM6 + 1.6SWL (30 mph) AZI 240	Yes	Υ	1	1.2	39	1.5	10	0.154
144	1.2DL + 1.5LM6 + 1.6SWL (30 mph) AZI 270	Yes	Υ	1	1.2	39	1.5	11	0.154
145	1.2DL + 1.5LM6 + 1.6SWL (30 mph) AZI 300	Yes	Y	1	1.2	39	1.5	12	0.154
146	1.2DL + 1.5LM6 + 1.6SWL (30 mph) AZI 330	Yes	Y	1	1.2	39	1.5	13	0.154

### Envelope Node Reactions

Node Label		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-in]	LC	MY [lb-in]	LC	MZ [lb-in]	LC
1 N9	max	1393.416	15	698.186	38	1437.349	4	1080.61	35	0	146	529.703	10
2	min	-2413.19	9	211.033	19	-884.036	22	-182.514	16	0	1	-411.963	16
3 N4	max	1456.198	27	695.107	32	336.789	18	1103.72	35	0	146	574.964	10
4	min	-268.258	20	209.741	25	-1074.384	37	-300.513	16	0	1	-459.805	16
5 N52	max	1301.689	10	24.737	35	332.515	10	52.079	82	0	146	452.588	10
6	min	-1307.709	4	8.299	16	-330.759	4	-265.424	28	0	1	-50.002	16
7 Totals:	max	1616.941	14	1406.256	28	1365.61	18						
8	min	-1616.941	8	464.22	60	-1365.613	12						

### Envelope AISC 14TH (360-10): LRFD Member Steel Code Checks

	Membe	r Shape (	Code Chec	kLoc[in]LCS	Shear Chec	k Loc[in	Dir	LC	phi*Pnc [lb]	phi*Pnt [lb]	phi*Mn y-y [lb-in]	phi*Mn z-z [lb-ii	n Cb Egn
1	M31	0.625" SR	0.609	42.97928	0.022	42.979		3		9940.19	1242.501	1242.501	2.791H1-1a
2	M16	PIPE_2.0	0.277	78.75 8	0.034	77.438	3	35	8922.084	32130	22459.5	22459.5	1.868 H1-1b
3	M9	PIPE_2.0	0.254	36.776 10	0.055	2.682		10	30405.086	32130	22459.5	22459.5	1.836 H1-1b
4	M19	3.5"x0.625" Plate	0.193	0 34	0.067	0	У	32	69850.881	70875	11074.223	62015.641	1.822 H1-1b
5	M6	PIPE_2.5	0.185	19 10	0.151	18		4	30038.461	50715	43155	43155	1.287 H1-1b
6	M8	3.5"x0.625" Plate	0.184	0 2	0.148	4.5	У	31	68591.516	70875	11074.223	62015.641	1.667 H1-1b
7	M3	3.5"x0.625" Plate	0.183	0 8	0.143	0	У	29	68591.516	70875	11074.223	62015.641	1.667 H1-1b
8	M20	3.5"x0.625" Plate	0.182	0 38	0.067	0	У	27	69850.881	70875	11074.223	62015.641	2.156 H1-1b
9	M15	PIPE_2.0	0.162	48.563 4	0.096	48.563		4	8922.084	32130	22459.5	22459.5	3 H1-1b
10	M5	PIPE_2.0	0.159	33.71233	0.076	0		29	30405.086	32130	22459.5	22459.5	2.045 H1-1b
11	M10	PIPE_2.0	0.158	0 4	0.078	0		29	30405.086	32130	22459.5	22459.5	2.041 H1-1b
12	M1	PIPE_2.5	0.154	78 33	0.108	78		8	30038.461	50715	43155	43155	2.002 H1-1b
13	M30	0.625" SR	0.148	42.97977	0.007	42.979		12	1869.479	9940.19	1242.501	1242.501	1.835H1-1b*
14	M34	PIPE_2.0	0.143	77.438 9	0.078	77.438		4	8922.084	32130	22459.5	22459.5	2.433 H1-1b
15		3.5"x0.625" Plate	0.141	4.5 10	0.153	4.5	У	10	68591.516	70875	11074.223	62015.641	1.669 H1-1b
16	M28	0.625" SR	0.124	30 4	0.02	30	123	10	3836.923	9940.19	1242.501	1242.501	2.286 H1-1b
1/		3.5"x0.625" Plate	0.119	3 4	0.134	3	-		69850.881	70875	11074.223	62015.641	1.845 H1-1b
18		3.5"x0.625" Plate	0.111	3 3	0.106	3	-	4	69850.881	70875	11074.223	62015.641	1.355 H1-1b
19	M32	0.625" SR	0.104	0 4	0.024	21.489		4	1869.479	9940.19	1242.501	1242.501	2.354 H1-1b
20	M4	PIPE_2.0	0.098	18.388 4	0.052	0		4	30405.086	32130	22459.5	22459.5	1.172 H1-1b
21	M25	0.625" SR	0.075	30 4	0.014	30		4	3836.923	9940.19	1242.501	1242.501	2.325 H1-1b
22		3.5"x0.625" Plate	0.066	4.5 10	0.112	4.5	у	10	68591.516	70875	11074.223	62015.641	1.676 H1-1b
23	M33	PIPE_2.0	0.066	0 10	0.012	0		4	21406.935	32130	22459.5	22459.5	3 H1-1b*
24	M29	0.625" SR	0.058	21.48915	0.025	21.489		4	1869.479	9940.19	1242.501	1242.501	1.333 H1-1b
25	M39	PIPE_2.0	0.057	3 4	0.077	3		-	28843.414	32130	22459.5	22459.5	2.017 H1-1b
26	M26	0.625" SR	0.045	0 4	0.028	0			3836.923	9940.19	1242.501	1242.501	2.607 H1-1b
27	M27	0.625" SR	0.043	30 3	0.025	30		4	3836.923	9940.19	1242.501	1242.501	2.597 H1-1b



### FROM ZERO TO INFINIGY

the solutions are endless

### **BOLT CONNECTION CALCULATION**

### **BOLT PROPERTIES**

DOLIFICOFLICITLO		
Date:	9/27/2021	
Site:	BOHVN00204B	
Engineer:	DVA	
Project No:	2039-Z5555C	
Connection Location:	Mount to Tower	
	P	
Bolt Capacity Equation	TIA-222-H	
Connection Type	Steel	= -1
Bolt Size, d	5/8	in
Threads per Inch, n	11	
Steel Grade	A325	
Bolt Ultimate Tensile Stress, F <sub>u</sub>	120	ksi
Threads Exclusion	N	
Shear Plane	1	
Net Bolt Cross-Sectional Area, <b>An</b>	0.226	in <sup>2</sup>
Gross Bolt Cross-Sectional Area, Ag	0.307	in <sup>2</sup>
Tensile Steel Strength (per bolt), $\phi R_{nt}$	20340	lbs
Shear Steel Strength (per bolt), $\phi R_{nv}$	13806	lbs

# INFINIGY8

# FROM ZERO TO INFINIGY the solutions are endless

# **BOLT CONNECTION CALCULATION**

# BOLT GROUP CHECK

Date:	9/27/2021
Site:	BOHVN00204B
Engineer:	DVA
Project No:	2039-Z5555C
Connection Location:	Mount to Tower

**Bolt Group Pattern** 

	7	Loads Properties			
Controlling LC.	10		and the state of t		
Load Point Number:	6N				
X-Coordinate (in.)	4.50				
Y-Coordinate (in.)	1.50				
Z-Coordinate (in.)	5.00				
Shear Load, Fx (lbs)	805.000	0	0	0	0
Shear Load, Py (lbs)	2338,000	0	0	0	0
Axial Load, Pz (lbs)	-319.000	0	0	0	0
Moment, Mx (lb-in)	513.000	0	0	0	0
Moment, My (b-in)	-751.000	0	0	0	0
Moment, Mz (lb-in)	0.000	0	0	0	0

2	Member Properties	
	×	γ
Start Coordinates:	0.0	0.0
Dimentions:	9.0	3.0

Number of Bolts

		Bolt Coordinate	ordinates	Boh	3oft Loads		Steel Bolt Usage
No.	Bolt Type	Xo (in)	Yo (in)	Axial (Ibs)	bs) Shear (lbs)	Tension	Shear
-	Main Type	1.5	1.5	4111.83	1236.35	20.2%	9.0%
7	Main Type	7.5	1.5	3020.50	1236.35	14.8%	9.0%

Max. Capacity

**Combined** 20.2% 14.8%

8.0

6.0

4.0

2.0

0.0

0.0

1.0

3.0

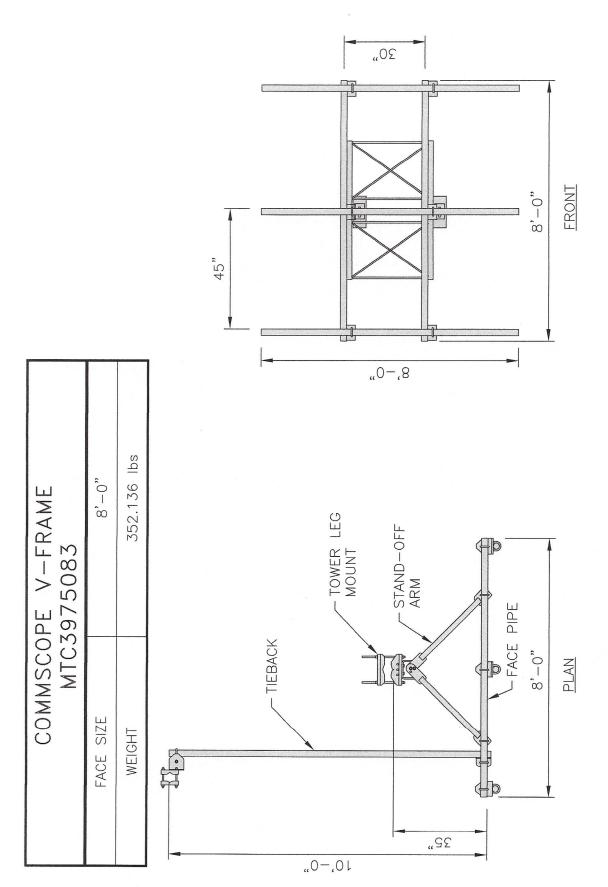
\*

4.50 1.50 5.52 0.00 5.53

Bolt Group:	sql	sql	sql	P-in	lb-in	lp-in
Loads at Center of Gravity of Bolt Group:	-319.00	805.00	2338.00	-11177.00	3274.00	0.00
Loads at Ce	Pz =	Px =	Py =	Mx =	My =	Mz =

Total Capacity of Bolt Group:

20.2%



ANTENNA FRAME DETAIL

## Exhibit F

**Power Density/RF Emissions Report** 



### Radio Frequency Emissions Analysis Report



Site ID: BOHVN00204B

EVE - West Peak Drive 11 West Peak Drive Meriden, CT 06451

October 12, 2021

Fox Hill Telecom Project Number: 210625

Site Compliance	Summary
Compliance Status:	COMPLIANT
Site total MPE% of FCC general population allowable limit:	15.83 %



October 12, 2021

Dish Wireless 5701 South Santa Fe Drive Littleton, CO 80120

Emissions Analysis for Site: **BOHVN00204B – EVE - West Peak Drive** 

Fox Hill Telecom, Inc ("Fox Hill") was directed to analyze the proposed radio installation for Dish Wireless, LLC (Dish) facility located at **11 West Peak Drive, Meriden, CT**, for the purpose of determining whether the emissions from the Proposed Dish radio and antenna installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu$ W/cm2). The number of  $\mu$ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu$ W/cm²). The general population exposure limits for the 600 MHz & 700 MHz bands are approximately 400  $\mu$ W/cm² and 467  $\mu$ W/cm² respectively. The general population exposure limit for the 1900 MHz (PCS) and 2100 MHz (AWS) bands is 1000  $\mu$ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.



### **CALCULATIONS**

Calculations were performed for the proposed radio system installation for **Dish** on the subject site located at **11 West Peak Drive**, **Meriden**, **CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since **Dish** is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. All power values expressed and analyzed are maximum power levels expected to be used on all radios.

All emissions values for additional carriers were taken from the Connecticut Siting Council (CSC) active MPE database. Values in this database are provided by the individual carriers themselves

For each sector the following channel counts, frequency bands and power levels were utilized as shown in *Table 1*:

Technology	Frequency Band	Channel Count	Transmit Power per Channel (W)
5G	600 MHz	4	61.5
5G	1900 MHz (PCS)	4	40
5G	2100 MHz (AWS)	4	40

Table 1: Channel Data Table



The following antennas listed in *Table 2* were used in the modeling for transmission in the 600 MHz, 1900 MHz (PCS) and 2100 MHz (AWS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

			Antenna
	Antenna		Centerline
Sector	Number	Antenna Make / Model	(ft)
A	1	JMA MX08FRO665-21	117
В	1	JMA MX08FRO665-21	117
C	1	JMA MX08FRO665-21	117

Table 2: Antenna Data

All calculations were done with respect to uncontrolled / general population threshold limits.



### **RESULTS**

Per the calculations completed for the proposed **Dish** configurations *Table 3* shows resulting emissions power levels and percentages of the FCC's allowable general population limit.

					Total TX		
Antenna	Antenna Make /		Antenna Gain	Channel	Power		
ID	Model	Frequency Bands	(dBd)	Count	(W)	ERP (W)	MPE %
		600 MHz/					
Antenna	JMA	1900 MHz (PCS) /	11.45 / 16.15 /				
A1	MX08FRO665-21	2100 MHz (AWS)	16.65	12	566	17,426.72	6.59
Sector A Composite MPE%							6.59
		600 MHz/					
Antenna	JMA	1900 MHz (PCS) /	11.45 / 16.15 /				
B1	MX08FRO665-21	2100 MHz (AWS)	16.65	12	566	17,426.72	6.59
Sector B Composite MPE%						6.59	
		600 MHz/					
Antenna	JMA	1900 MHz (PCS) /	11.45 / 16.15 /				
C1	MX08FRO665-21	2100 MHz (AWS)	16.65	12	566	17,426.72	6.59
Sector C Composite MPE%							6.59

Table 3: Dish Emissions Levels



The Following table (*table 4*) shows all additional carriers on site and their MPE% as recorded in the CSC active MPE database for this facility along with the newly calculated maximum **Dish** MPE contributions per this report. FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. For this site, all three sectors have the same configuration yielding the same results on all three sectors. *Table 5* below shows a summary for each **Dish** Sector as well as the composite MPE value for the site.

Site Composite MPE%				
Carrier	MPE%			
Dish – Max Per Sector Value	6.59 %			
T-Mobile	8.20 %			
PageNet	0.54 %			
SNET TMRS	0.31 %			
XM Satellite Radio	0.12 %			
Arrow Bus	0.07 %			
Site Total MPE %:	15.83 %			

Table 4: All Carrier MPE Contributions

Dish Sector A Total:	6.59 %
Dish Sector B Total:	6.59 %
Dish Sector C Total:	6.59 %
Site Total:	15.83 %

Table 5: Site MPE Summary



FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. *Table 6* below details a breakdown by frequency band and technology for the MPE power values for the maximum calculated **Dish** sector(s). For this site, all three sectors have the same configuration yielding the same results on all three sectors.

Dish _ Frequency Band / Technology Max Power Values (Per Sector)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm²)	Frequency (MHz)	Allowable MPE (µW/cm²)	Calculated % MPE
Dish 600 MHz 5G	4	858.77	117	10.02	600 MHz	400	2.51%
Dish 1900 MHz (PCS) 5G	4	1,648.39	117	19.24	1900 MHz (PCS)	1000	1.92%
Dish 2100 MHz (AWS) 5G	4	1,849.52	117	21.59	2100 MHz (AWS)	1000	2.16%
						Total:	6.59%

Table 6: Dish Maximum Sector MPE Power Values



### **Summary**

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the Dish facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

Dish Sector	Power Density Value (%)			
Sector A:	6.59 %			
Sector B:	6.59 %			
Sector C:	6.59 %			
Dish Maximum Total (per sector):	6.59 %			
Site Total:	15.83 %			
	_			
Site Compliance Status:	COMPLIANT			

The anticipated composite MPE value for this site assuming all carriers present is **15.83** % of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

Scott Heffernan Principal RF Engineer

**Fox Hill Telecom, Inc** Holden, MA 01520 (978)660-3998

## Exhibit G

### **Letter of Authorization**

### **LETTER OF AUTHORIZATION**

I, Michael Ashley Culbert, the owner representative for the telecommunications tower located at West Peak Dr, Meriden, New Haven County, Connecticut, as evidenced by the Memorandum of Lease recorded with the New Haven County Recorder of Deeds on 8/6/2019, Instrument Number 2019004809.

As owner of the above-referenced telecommunications tower, I hereby authorize DISH Wireless L.L.C., through its designated agent, Northeast Site Solutions, to apply for all necessary municipal, state, federal and other permits necessary to accommodate the installation of DISH Wireless L.L.C.'s antennas and ancillary equipment on the subject tower and base station equipment on the ground on our leasehold property.

EIP Communications I, LLC

Michael Ashley Culbert

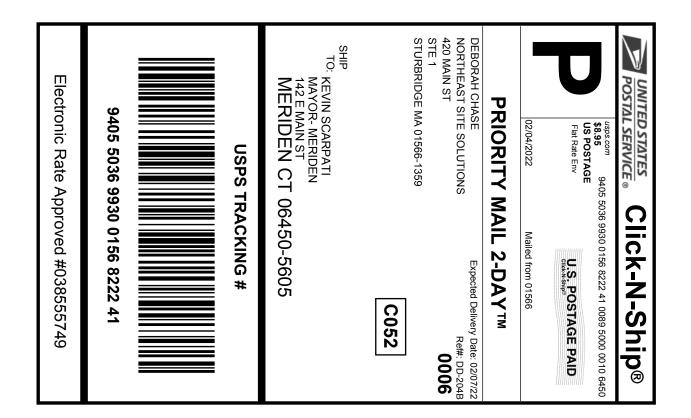
Vice President of Site Development

Michael ashly Culler

Date: December 17, 2021

# Exhibit H

**Recipient Mailings** 





### Instructions

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### **USPS TRACKING #:** 9405 5036 9930 0156 8222 41

555805814 02/04/2022 Trans. #: Print Date: Ship Date: 02/04/2022 02/07/2022 Delivery Date:

Priority Mail® Postage: \$8.95 \$8.95 Total:

Ref#: DD-204B From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

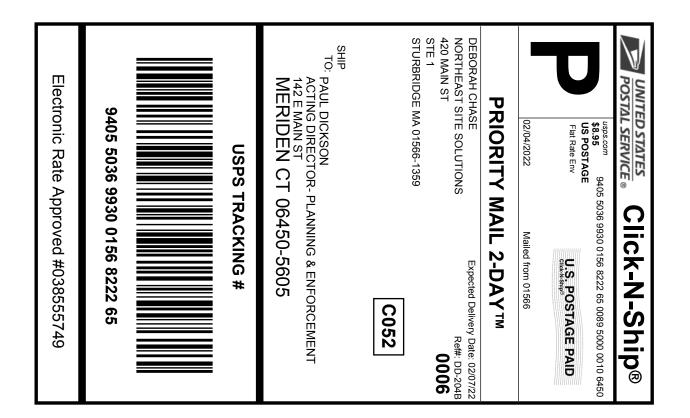
STE 1

**STURBRIDGE MA 01566-1359** 

KEVIN SCARPATI

MAYOR- MERIDEN 142 E MAIN ST

MERIDEN CT 06450-5605





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555805814 02/04/2022 Trans. #: Print Date: Ship Date: 02/04/2022 02/07/2022 Delivery Date:

Priority Mail® Postage: Total:

\$8.95 \$8.95

Ref#: DD-204B From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

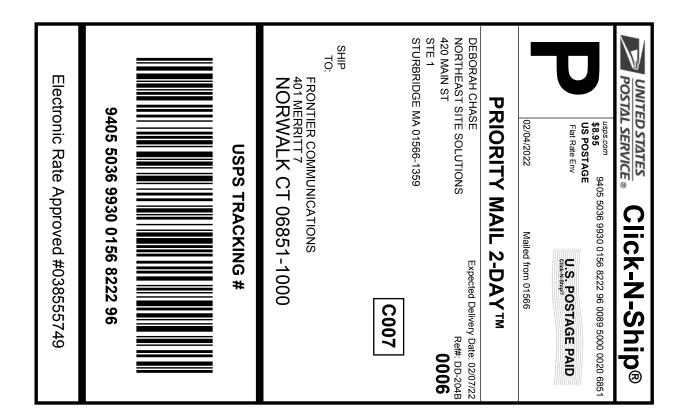
STURBRIDGE MA 01566-1359

PAUL DICKSON

**ACTING DIRECTOR- PLANNING & ENFORCEMENT** 

142 E MAIN ST

MERIDEN CT 06450-5605





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555805814 02/04/2022 Trans. #: Print Date: Ship Date: 02/04/2022 02/07/2022 Delivery Date:

Priority Mail® Postage: Total:

\$8.95 \$8.95

Ref#: DD-204B

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

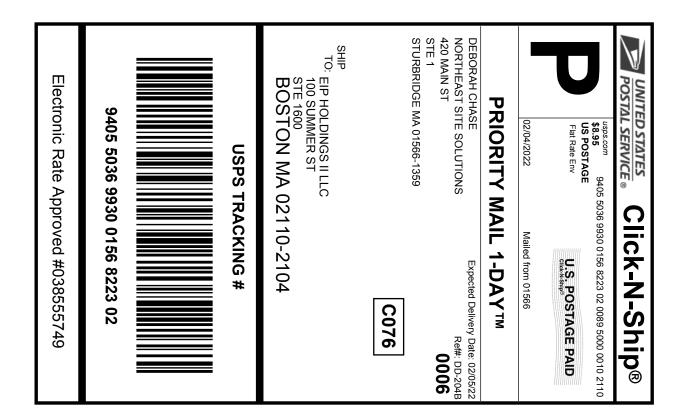
STE 1

**STURBRIDGE MA 01566-1359** 

FRONTIER COMMUNICATIONS

401 MFRRITT 7

NORWALK CT 06851-1000





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### Click-N-Ship® Label Record

### **USPS TRACKING #:** 9405 5036 9930 0156 8223 02

555805814 02/04/2022 Trans. #: Print Date: Ship Date: 02/04/2022 02/05/2022 Delivery Date:

Priority Mail® Postage: Total:

\$8.95 \$8.95

Ref#: DD-204B

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

**STURBRIDGE MA 01566-1359** 

**EIP HOLDINGS II LLC** 

100 SUMMER ST STE 1600

BOSTON MA 02110-2104

BOHVNOU ROYB



UNIONVILLE 24 MILL ST UNIONVILLE, CT 06085-9998 (800)275-8777

02/07/2022 11:10 AM Product Unit Price Price Prepaid Mail \$0.00 Meriden, CT 06450 Weight: 0 1b 0.30 oz Acceptance Date: Mon 02/07/2022 Tracking #: 9405 5036 9930 0156 8222 41 Prepaid Mail 1 Meriden, CT 06450 Weight: 0 lb 10.60 oz \$0,00 Acceptance Date: Mon 02/07/2022 Tracking #: 9405 5036 9930 0156 8222 65 Prepaid Mail 1 Norwalk, CT 06851 Weight: 0 lb 0.30 oz \$0.00 Acceptance Date: Mon 02/07/2022 Tracking #: 9405 5036 9930 0156 8222 96 Prepaid Mail \$0.00 Boston, MA 02110 Weight: 0 lb 10.60 oz Acceptance Date: Mon 02/07/2022 Tracking #: 9405 5036 9930 0156 8223 02 Grand Total: \$0.00

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