



STATE OF CONNECTICUT

CONNECTICUT SITING COUNCIL

Ten Franklin Square
New Britain, Connecticut 06051
Phone: (860) 827-2935
Fax: (860) 827-2950

March 8, 2002

Michelle Briggs
Manager of Real Estate Operations
SNET Mobility, LLC
500 Enterprise Drive
Rocky Hill, CT 06067-3900

Ronald C. Clark
Manager - Real Estate Operations
Nextel Communications
100 Corporate Place
Rocky Hill, CT 06067

RE: **EM-SNET/NEXTEL-078-020226** - SNET Mobility, LLC and Nextel Communications Inc.
notice of intent to modify an existing telecommunications facility located on the University of
Connecticut campus off North Eagleville Road, Storrs, Connecticut. (Docket No. 179)

Dear Ms. Briggs and Mr. Clark

At a public meeting held on March 7, 2002, the Connecticut Siting Council (Council) acknowledged your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies.


The proposed modifications are to be implemented as specified in your filings dated April 26, 2001 and February 26, 2002. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest

point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Any deviation from this format may result in the Council implementing enforcement proceedings pursuant to General Statutes § 16-50u including, without limitation, imposition of expenses resulting from such failure and of civil penalties in an amount not less than one thousand dollars per day for each day of construction or operation in material violation.

Thank you for your attention and cooperation.

Very truly yours,


Mortimer A. Gelston

Chairman

MAG/laf

- c: Honorable Elizabeth Patterson, Mayor, Town of Mansfield
- Gregory Padick, Town Planner, Town of Mansfield
- George Davis, UCONN Tower Site Manager



STATE OF CONNECTICUT
CONNECTICUT SITING COUNCIL

Ten Franklin Square
New Britain, Connecticut 06051
Phone: (860) 827-2935
Fax: (860) 827-2950

March 1, 2002

Honorable Elizabeth Patterson
Mayor
Town of Mansfield
4 South Eagleville Road
Mansfield, CT 06268

RE: **EM-SNET/NEXTEL-078-020226** - SNET Mobility, LLC and Nextel Communications Inc. notice of intent to modify an existing telecommunications facility located on the University of Connecticut campus off North Eagleville Road, Storrs, Connecticut. (Docket No. 179)

Dear Ms. Patterson:

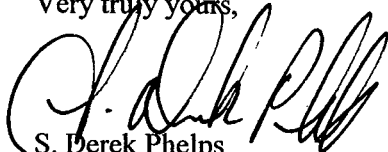
The Connecticut Siting Council (Council) received this request to modify an existing telecommunications facility, pursuant to Regulations of Connecticut State Agencies Section 16-50j-72.

The Council will consider this item at the next meeting scheduled for March 7, 2002, at 1:30 p.m. in Hearing Room One, Ten Franklin Square, New Britain, Connecticut.

Please call me or inform the Council if you have any questions or comments regarding this proposal.

Thank you for your cooperation and consideration.

Very truly yours,



S. Derek Phelps
Executive Director

SDP/RDM/grg

Enclosure: Notice of Intent

c: Gregory Padick, Town Planner, Town of Mansfield
Martin H. Berliner, Town Manager, Town of Mansfield

Built to a Higher Standard

*CINGULAR WIRELESS
REVISED STRUCTURAL ANALYSIS REPORT*

STORRS, CT

02-01020

*YOUR SABRE
REPRESENTATIVE IS
LORA KEITHLEY
1-800-369-6690 EXT. 217*



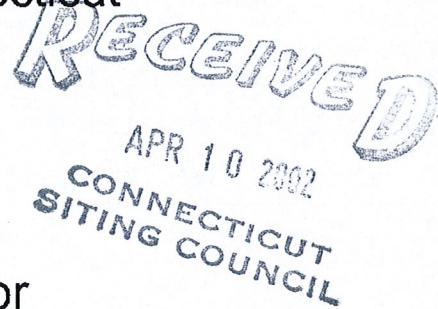
*2101 Murray Street • P.O. Box 658 • Sioux City, IA 51102
Phone 712-258-6690 • 1-800-369-6690 • FAX 712-258-8250*

Structural Analysis Report

Job #02-01020
Revision A

Existing 327' Sabre Communications Corporation
4400SRW Guyed Tower

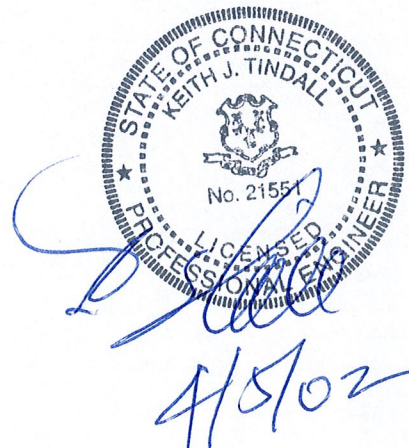
Located at Storrs, Connecticut



Report Completed for

Cingular Wireless

Westwood, MA



Prepared by

Sabre Communications Corporation

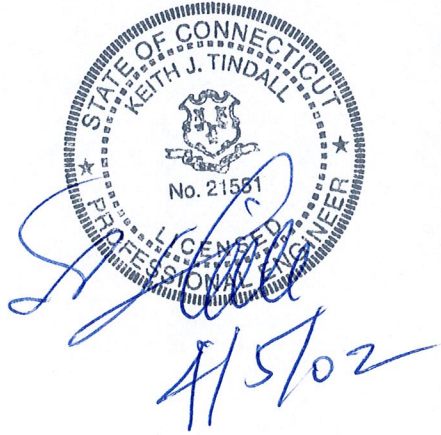
April 5, 2002

Structural Analysis Report
Existing 327' Sabre Communications Corporation
4400SRW Guyed Tower

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SUPPORTED EQUIPMENT..... 3-5
RESULTS..... 5
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DESCRIPTION OF GUYED TOWER PROGRAM..... 6-7
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Prepared by AMS
Checked by AKN
Approved by KJT



Introduction

The purpose of this analysis is to determine if the existing tower is in conformance with the requirements of ANSI/TIA/EIA 222-F, while supporting specified equipment. The tower is a 327' 4400SRW guyed tower and was originally manufactured by Sabre Communications Corporation. The tower is located in Storrs, Connecticut. The analysis is being performed for Cingular Wireless, Westwood, MA.

Method of Analysis

The computer program that was used for this analysis is described on the attached page. The analysis was performed using a basic wind speed of 90 mph concurrent with 1/2" ice, in accordance with ANSI/TIA/EIA 222-F. Allowable stresses, safety factors and load factors were also determined in accordance with this standard.

Supported Equipment

The analysis was performed for the tower, supporting the following equipment:

WHUS

1. One (1) Shively 6813 2-bay antenna on a pole from 290' to 327'
2. One (1) OMB-GP1 antenna from 209' to 211'
3. One (1) Telewave 450F6 antenna from 273' to 281'
4. One (1) Diamond x50 antenna from 273' to 267'
5. One (1) Cushcraft antenna at 92'

Connecticut State Police

6. Two (2) 6' dishes with radomes from 113' to 119'
7. One (1) 6' dish with radome from 101' to 107'
8. One (1) Scala AP14-850-105 antenna from 258' to 267'
9. One (1) DB-810K antenna from 265' to 273'
10. One (1) Scala AP14-850-105 antenna from 258' to 250'
11. One (1) DB-212 antenna from 74' to 94'
12. One (1) Scala OGT9-840 antenna from 258' to 247'

Verizon

- 13. Twelve (12) panel antennas on a 3T-Boom from 82' to 86'
- 14. One (1) PD-220 antenna from 154' to 174'
- 15. Three (3) Metawave panel antennas on a 3T-Boom at 80'
- 16. Twelve (12) 1-1/4" lines and three (3) 1/2" lines

PageNet

- 17. One (1) DB872H120 antenna from 158' to 160'
- 18. One (1) DB586-T3 antenna from 151' to 155'
- 19. One (1) DB589-T3 antenna from 156' to 165'
- 20. One (1) DB589-T3 antenna from 162' to 171'
- 21. One (1) Scala yagi antenna at 105'

Other Tenants

- 22. One (1) Shively 2-bay antenna from 193' to 206'
- 23. One (1) Celwave TDF6319A antenna from 111' to 121'
- 24. One (1) 8' whip antenna from 153' to 161'
- 25. One (1) Celwave PD10108 antenna at 29'
- 26. One (1) Sinclair SRL420NHD antenna from 124' to 133'

SNET(proposed)

- 27. Twelve (12) CSS DUO4-8670 antennas, one (1) MB100RR650200DPAL antenna and eight (8) ADC tovertop amplifiers on a 3T-Boom at 185', with sixteen (16) 1-5/8" lines
- 28. One (1) emergency 911 LMU approximately tree-line level plus 10'

Nextel (proposed)

- 29. Twelve (12) DB844H90(E)-XY antennas on a platform at 225', with fifteen (15) 1-5/8" lines
- 30. Two (2) Radiall/Larsen GPS0015 antennas at 65' with two (2) 1/2" lines

UConn Inventory Update

- 31. One (1) DB 801 at 180'
- 32. One (1) Cellinc DBD2702 Tower Amplifier at 180'
- 33. One (1) DB872H at 175'
- 34. Scala PR-950 Grid at 50'

Other

- 35. Two (2) Ice Shields at 125'
- 36. Two Equipment Boxes at 258'
- 37. Side Lights and J Box at 155'

Each tower face is assumed to be fully covered with feedlines.

Results

Tower Section Elevation (ft)	Percentage of Allowable Leg Capacity Used (%)	Percentage of Allowable Diagonal Capacity Used (%)
0-20	69.3	74.9
20-40	75.2	44.4
40-60	70.7	51.4
60-80	72.5	44.4
80-100	72.6	72.2
100-120	68.0	58.8
120-140	57.2	51.9
140-160	70.8	58.1
160-180	56.9	63.0
180-200	60.8	92.1
200-220	58.6	43.0
220-240	63.6	53.2
240-260	62.0	41.0
260-280	42.0	25.0
280-288	46.0	49.4
288-323	13.8	

Guy Elevation (ft)	Percentage of Allowable Guy Capacity Used (%)
57	89.7
107	92.2
167	84.0
217	74.6
257	73.5
284	70.2

The results of the analysis show no overstresses in any tower component.

In addition, the results of the analysis show that the foundations are adequate.

Conclusions

Based on the preceding results, the following conclusions have been made:

1. The tower with specified equipment is adequate to achieve a basic wind speed rating of 90 mph concurrent with 1/2" ice, in accordance with ANSI/TIA/EIA 222-F.
2. No modifications are required, in order to meet the structural criteria stated above.
3. The analysis is valid only for the equipment listed above. If the equipment is not as listed, an additional analysis should be performed.
4. The analysis assumes that the tower contains no structural defects, and that all components have been installed properly.

Description of Guyed Tower Computer Program

A guyed tower computer program called GUYMAST™, developed by Guymast Inc., is utilized by Sabre Communications Corporation to perform the structural analysis and design of guyed towers. This program is one of the most widely used programs in the tower industry.

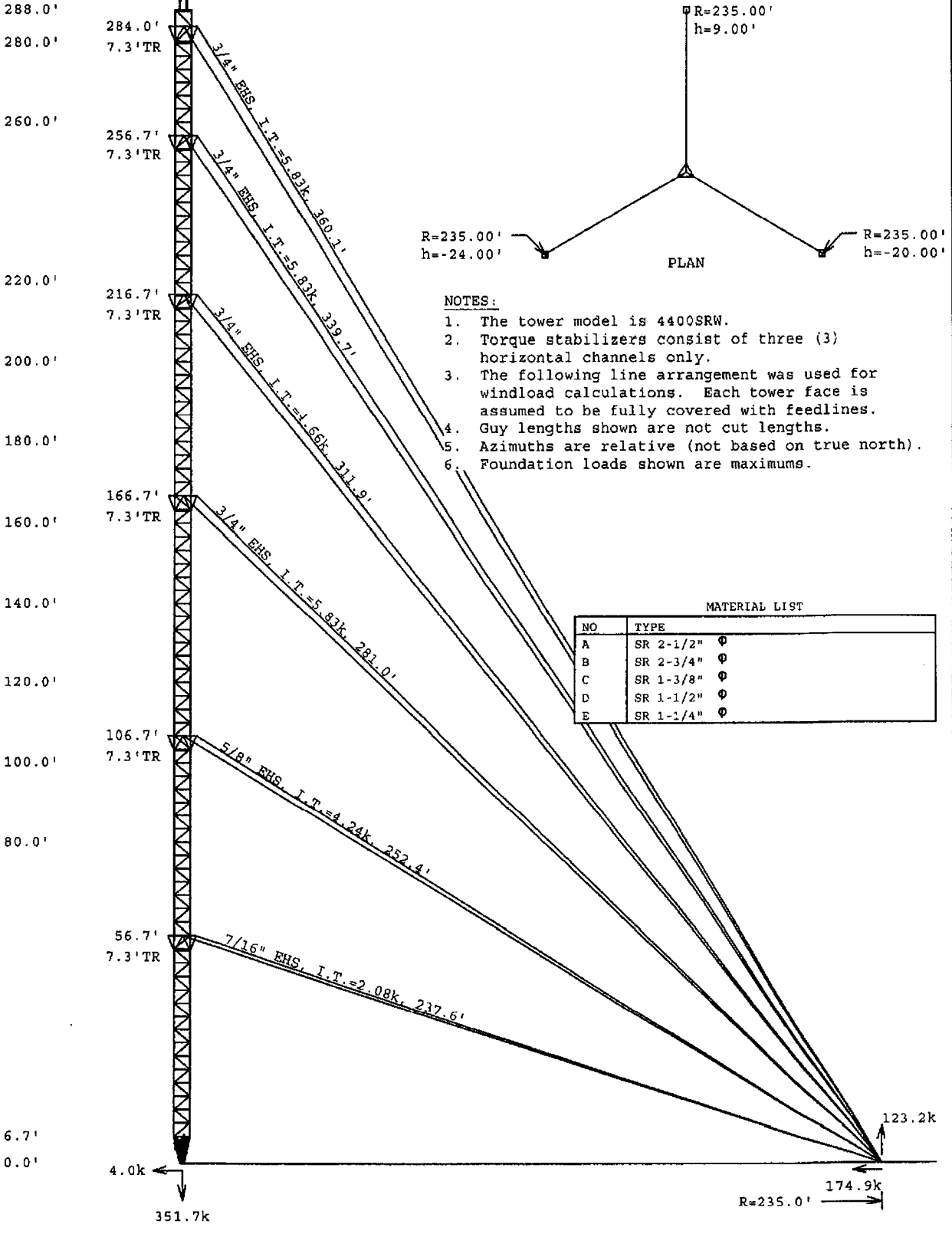
"GUYMAST performs the analysis of guyed towers using three separate models. The first consists of a set of guys connected at the same elevation on the mast, used independently for each guy level. The second is a continuous beam model using the stiffness method (including beam-column interaction), used to analyze the bending of the mast in the two orthogonal vertical planes independently. The third is a model of a shaft subjected to torsional loading, for the analysis of tower twist.

"With an arbitrary set of initial displacements, the guy model is used to obtain the guy stiffnesses and guy loads to be applied in the mast models. These guy stiffnesses and loads are then used in the two mast models to calculate a new set of mast displacements, which are then fed back into the guy model to obtain a better approximation of guy stiffnesses and loads.

“The process is repeated until displacements calculated in the beam model match, within a specified tolerance, those used by the guy model in determining the support stiffnesses.”¹

1. GUYMAST™ user manual, by Guymast Inc., © 1997.

Leg	50 ksi	SR 2-3/4" φ	SR 2-1/2" φ	A	B	SR 2-1/4" φ	SR 2" φ
Diagonal	36 ksi	SR 1-1/2" φ	SR 2-3/4" φ	C	D	SR 1-3/8" φ	SR 1-3/8" φ
Horizontal	36 ksi			E			
Brace Bolts							
Face Width	3.7'						
Panel Height#Panels							



- NOTES:**
1. The tower model is 4400SRW.
 2. Torque stabilizers consist of three (3) horizontal channels only.
 3. The following line arrangement was used for windload calculations. Each tower face is assumed to be fully covered with feedlines.
 4. Guy lengths shown are not cut lengths.
 5. Azimuths are relative (not based on true north).
 6. Foundation loads shown are maximums.

MATERIAL LIST

NO	TYPE
A	SR 2-1/2" φ
B	SR 2-3/4" φ
C	SR 1-3/8" φ
D	SR 1-1/2" φ
E	SR 1-1/4" φ

Sabre Communications Corporation
 2101 Murray Street, Sioux City, Iowa 51102
 Phone: (712) 258-6690 Fax: (712) 258-8250

Client: Cingular Wireless Job No: 02-01020 Date: 4 apr 2002
 Location: Storrs, CT Tower Height: 288.00'
 Standard: ANSI/TIA/EIA 222-F 1996 Design Wind & Ice: 90 mph + 1/2" ice (Concurrent)

Supported Equipment

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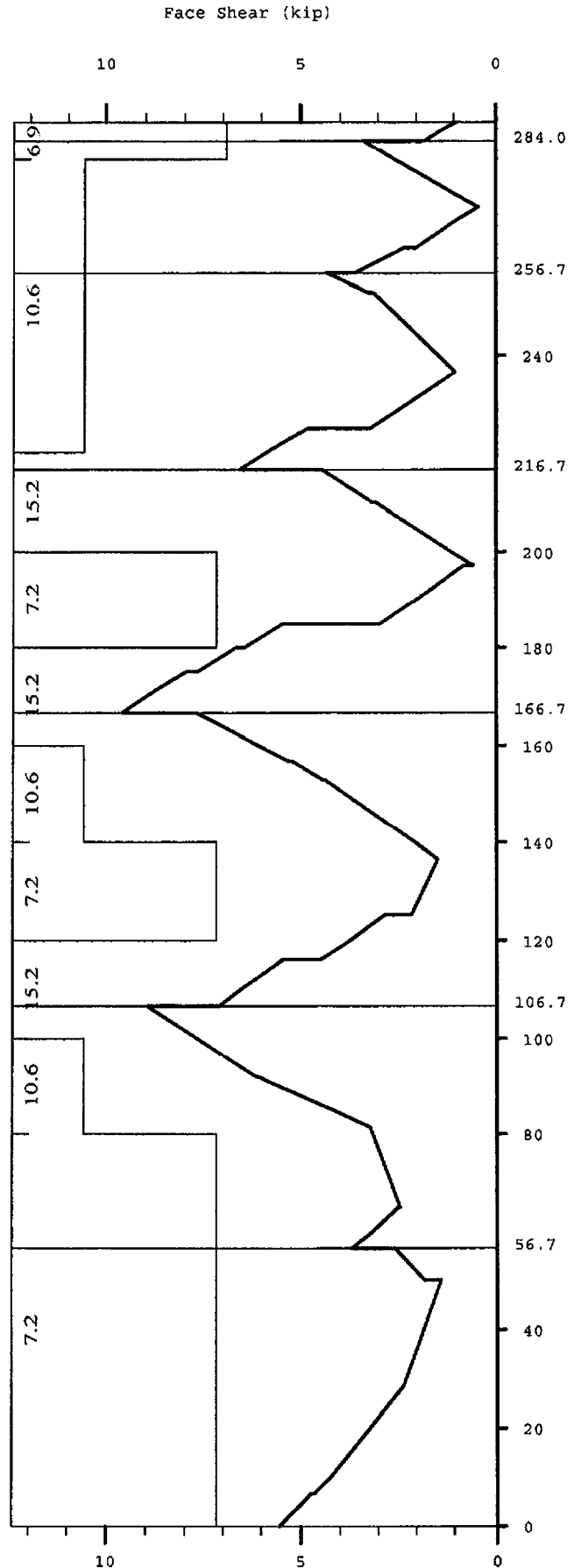
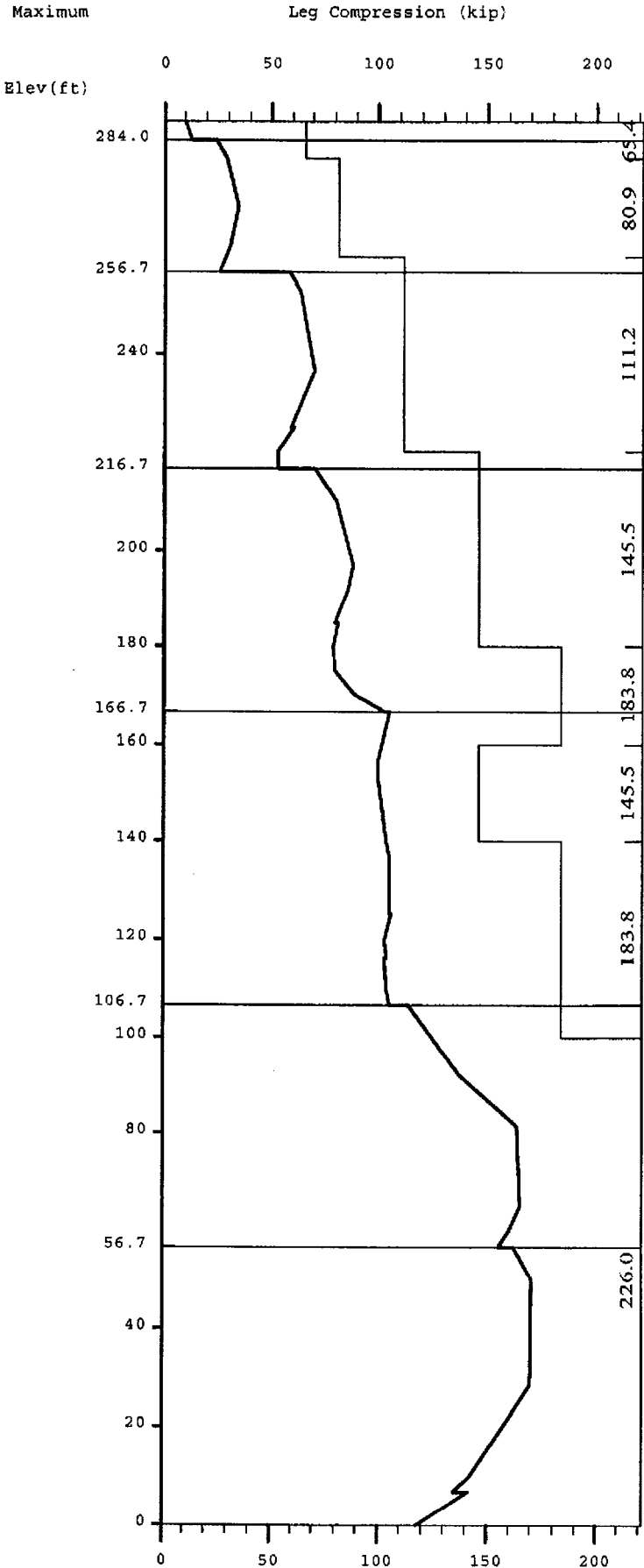
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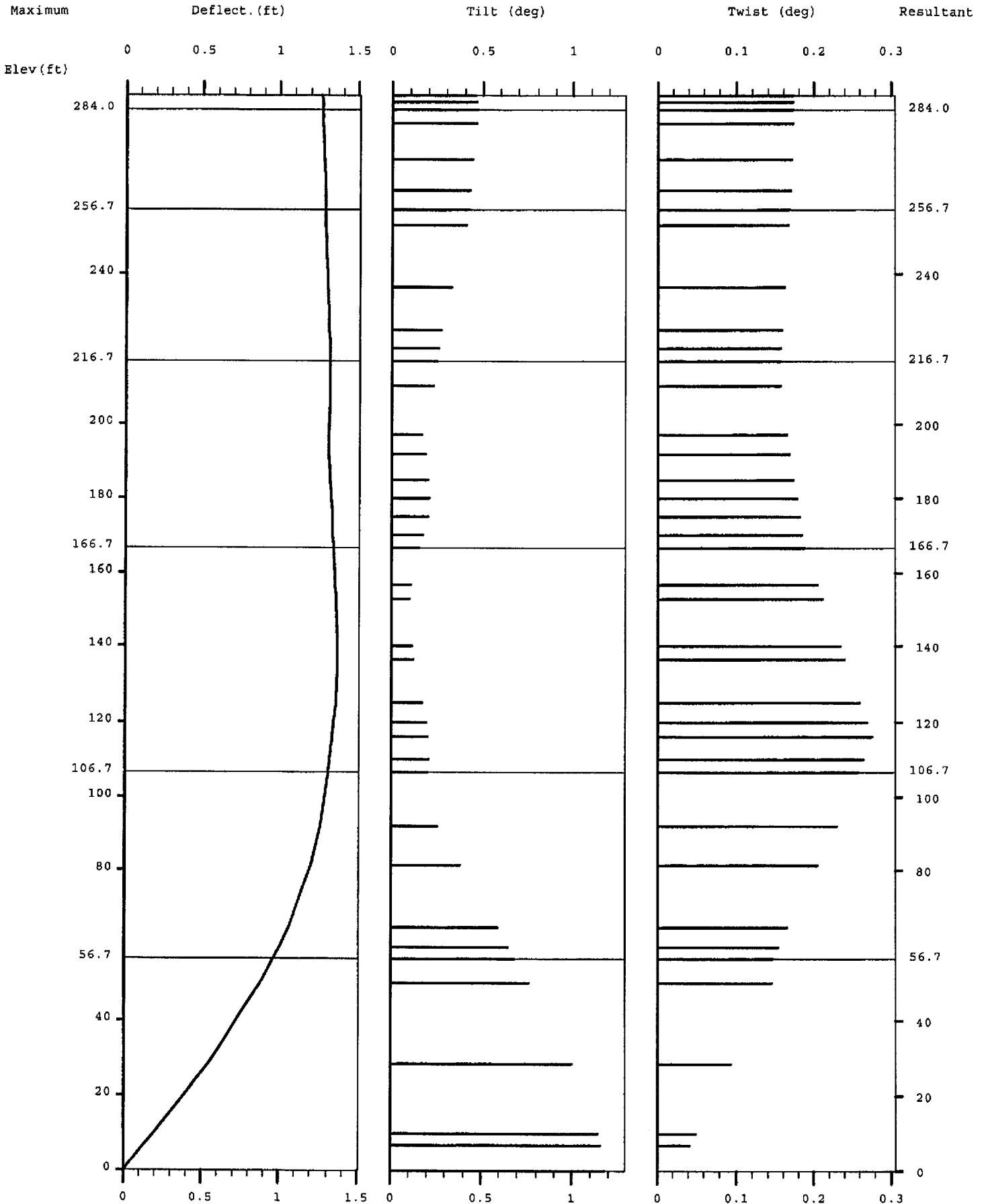
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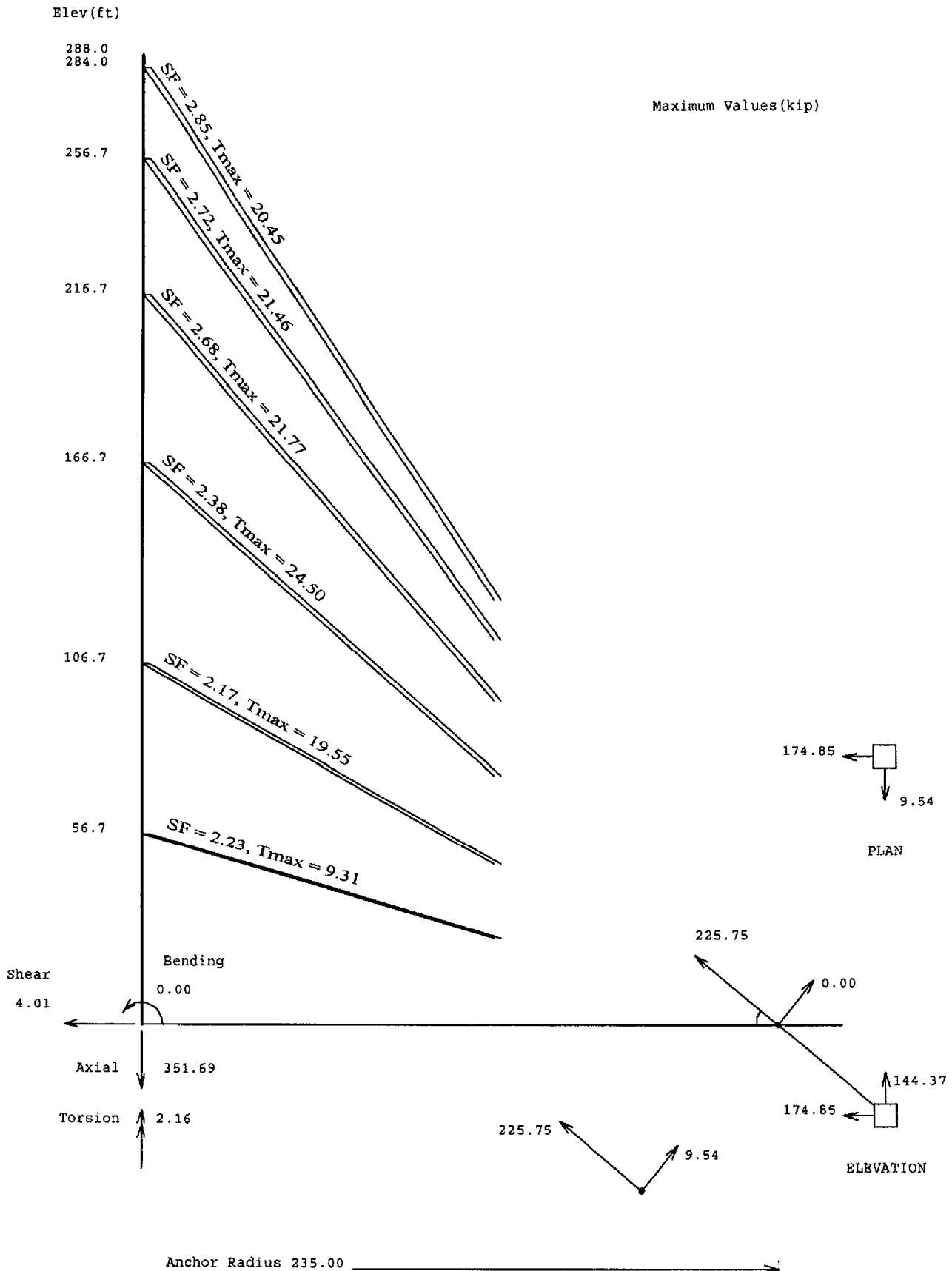
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327' 4400 CINGULAR WIRELESS STORRS, CT (02-01020A) 4-4-02 JMB

Guy Tensions, Anchor Loads and Base Loads



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GUYMAST (USA)-Guyed Tower Analysis (c)1997 Guymast Inc. 416-736-7453

Processed under license at:

Sabre Communications Corporation on: 4 apr 2002 at: 16:30:13

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327' 4400 CINGULAR WIRELESS STORRS, CT (02-01020A) 4-4-02 JMB

MAST DATA

=====

UPPER ELEV FT	MAST TYPE OF WEB	NO OF LEGS	FACE WIDTH FT	GEOM PANEL HEIGHT FT	X-SECTION-AREA ONE LEG IN.SQ.	X-SECTION-AREA ONE DIAG IN.SQ.	BARE WEIGHT K/FT.	ELASTIC MODULUS KIP/IN.SQ.	TEMP COEFF /DEG
288.0	4	3	3.667	4.000	3.140	1.480	0.061	29000.0	0.0000116
280.0	4	3	3.667	3.333	5.610	1.440	0.089	29000.0	0.0000116
6.7	4	3	2.688	3.333	7.070	1.230	0.098	29000.0	0.0000116

* If NO OF LEGS is 1 : that part of the mast is assumed to be Cylindrical

and : FACE WIDTH = outside diameter

PANEL HEIGHT = thickness

AREA OF DIAG = Poisson ratio

GUY GEOMETRY

=====

ELEV FT	GUY AZI DEG	DIAMETER IN.	HEIGHT FT.	RADIUS FT.	MAST ATTACH RADIUS FT.	ATTACH AZI DEG	INITIAL TENSION KIP
284.0	0.0	0.750	275.0	235.0	4.230	300.0	5.830
284.0	240.0	0.750	308.0	235.0	4.230	300.0	5.830
284.0	240.0	0.750	308.0	235.0	4.230	180.0	5.830
284.0	120.0	0.750	304.0	235.0	4.230	180.0	5.830
284.0	120.0	0.750	304.0	235.0	4.230	60.0	5.830
284.0	0.0	0.750	275.0	235.0	4.230	60.0	5.830
256.7	0.0	0.750	247.7	235.0	4.230	300.0	5.830
256.7	240.0	0.750	280.7	235.0	4.230	300.0	5.830
256.7	240.0	0.750	280.7	235.0	4.230	180.0	5.830
256.7	120.0	0.750	276.7	235.0	4.230	180.0	5.830
256.7	120.0	0.750	276.7	235.0	4.230	60.0	5.830
256.7	0.0	0.750	247.7	235.0	4.230	60.0	5.830
216.7	0.0	0.750	207.7	235.0	4.230	300.0	4.660
216.7	240.0	0.750	240.7	235.0	4.230	300.0	4.660
216.7	240.0	0.750	240.7	235.0	4.230	180.0	4.660
216.7	120.0	0.750	236.7	235.0	4.230	180.0	4.660
216.7	120.0	0.750	236.7	235.0	4.230	60.0	4.660
216.7	0.0	0.750	207.7	235.0	4.230	60.0	4.660
166.7	0.0	0.750	157.7	235.0	4.230	300.0	5.830
166.7	240.0	0.750	190.7	235.0	4.230	300.0	5.830
166.7	240.0	0.750	190.7	235.0	4.230	180.0	5.830
166.7	120.0	0.750	186.7	235.0	4.230	180.0	5.830
166.7	120.0	0.750	186.7	235.0	4.230	60.0	5.830
166.7	0.0	0.750	157.7	235.0	4.230	60.0	5.830
106.7	0.0	0.625	97.7	235.0	4.230	300.0	4.240

P.A6

106.7	240.0	0.625	130.7	235.0	4.230	300.0	4.240
106.7	240.0	0.625	130.7	235.0	4.230	180.0	4.240
106.7	120.0	0.625	126.7	235.0	4.230	180.0	4.240
106.7	120.0	0.625	126.7	235.0	4.230	60.0	4.240
106.7	0.0	0.625	97.7	235.0	4.230	60.0	4.240
56.7	0.0	0.438	47.7	235.0	4.230	300.0	2.080
56.7	240.0	0.438	80.7	235.0	4.230	300.0	2.080
56.7	240.0	0.438	80.7	235.0	4.230	180.0	2.080
56.7	120.0	0.438	76.7	235.0	4.230	180.0	2.080
56.7	120.0	0.438	76.7	235.0	4.230	60.0	2.080
56.7	0.0	0.438	47.7	235.0	4.230	60.0	2.080

GUY MATERIAL PROPERTIES

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ELEV FT	GUY AZI DEG	BREAKING STRENGTH KIP	GUY WEIGHT LBS/FT	GUY AREA IN. SQ	ELASTIC MODULUS KIP/IN. SQ	THERMAL COEFF /DEG	UNSTRESS LENGTH FT
284.0	0.0	58.300	1.180	0.338	19000.0	0.0000120	360.068
284.0	240.0	58.300	1.180	0.338	19000.0	0.0000120	385.813
284.0	240.0	58.300	1.180	0.338	19000.0	0.0000120	385.813
284.0	120.0	58.300	1.180	0.338	19000.0	0.0000120	382.632
284.0	120.0	58.300	1.180	0.338	19000.0	0.0000120	382.632
284.0	0.0	58.300	1.180	0.338	19000.0	0.0000120	360.068
256.7	0.0	58.300	1.180	0.338	19000.0	0.0000120	339.690
256.7	240.0	58.300	1.180	0.338	19000.0	0.0000120	364.408
256.7	240.0	58.300	1.180	0.338	19000.0	0.0000120	364.408
256.7	120.0	58.300	1.180	0.338	19000.0	0.0000120	361.341
256.7	120.0	58.300	1.180	0.338	19000.0	0.0000120	361.341
256.7	0.0	58.300	1.180	0.338	19000.0	0.0000120	339.690
216.7	0.0	58.300	1.180	0.338	19000.0	0.0000120	311.856
216.7	240.0	58.300	1.180	0.338	19000.0	0.0000120	334.707
216.7	240.0	58.300	1.180	0.338	19000.0	0.0000120	334.707
216.7	120.0	58.300	1.180	0.338	19000.0	0.0000120	331.846
216.7	120.0	58.300	1.180	0.338	19000.0	0.0000120	331.846
216.7	0.0	58.300	1.180	0.338	19000.0	0.0000120	311.856
166.7	0.0	58.300	1.180	0.338	19000.0	0.0000120	281.025
166.7	240.0	58.300	1.180	0.338	19000.0	0.0000120	300.749
166.7	240.0	58.300	1.180	0.338	19000.0	0.0000120	300.749
166.7	120.0	58.300	1.180	0.338	19000.0	0.0000120	298.232
166.7	120.0	58.300	1.180	0.338	19000.0	0.0000120	298.232
166.7	0.0	58.300	1.180	0.338	19000.0	0.0000120	281.025
106.7	0.0	42.400	0.819	0.234	20000.0	0.0000120	252.351
106.7	240.0	42.400	0.819	0.234	20000.0	0.0000120	266.838
106.7	240.0	42.400	0.819	0.234	20000.0	0.0000120	266.838
106.7	120.0	42.400	0.819	0.234	20000.0	0.0000120	264.907
106.7	120.0	42.400	0.819	0.234	20000.0	0.0000120	264.907
106.7	0.0	42.400	0.819	0.234	20000.0	0.0000120	252.351
56.7	0.0	20.800	0.388	0.115	21000.0	0.0000120	237.554
56.7	240.0	20.800	0.388	0.115	21000.0	0.0000120	246.292
56.7	240.0	20.800	0.388	0.115	21000.0	0.0000120	246.292
56.7	120.0	20.800	0.388	0.115	21000.0	0.0000120	245.014
56.7	120.0	20.800	0.388	0.115	21000.0	0.0000120	245.014
56.7	0.0	20.800	0.388	0.115	21000.0	0.0000120	237.554

FACTORED LEG AND FACE SHEAR RESISTANCE

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BOTTOM ELEV	TOP ELEV	LEG COMP	FACE SHEAR
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	ft	ft	kip	kip
	0.00	20.00	226.03	7.17
	20.00	40.00	226.03	7.17
	40.00	60.00	226.03	7.17
	60.00	80.00	226.03	7.17
	80.00	100.00	226.03	10.59
	100.00	120.00	183.79	15.24
	120.00	140.00	183.79	7.17
	140.00	160.00	145.52	10.59
	160.00	180.00	183.79	15.24
	180.00	200.00	145.52	7.17
	200.00	220.00	145.52	15.24
	220.00	240.00	111.22	10.59
	240.00	260.00	111.22	10.59
	260.00	280.00	80.89	10.59
	280.00	288.00	65.42	6.90

 *LOADING CONDITION A *****

90 MPH + 1/2" ICE WIND AZ 0 DEGREES

*12 wind directions were analyzed.

Only one condition is shown
 in full.

MAST LOADING

LOAD TYPE	ELEV FT	.FORCES (KIP & KIP/FT)			.MOMENTS (FT.K & FT.K/FT)			ANT-ORIENT	
		N	E	DOWN	N	E	TORSION	AZI DEG	VERT DEG
C	309.0	-0.530	0.000	0.380	0.00	0.00	0.00	0.0	0.00
C	306.0	-0.890	0.000	3.200	0.00	0.00	0.00	0.0	0.00
C	284.0	-0.790	0.000	1.110	0.00	0.00	0.00	0.0	0.00
C	277.0	-0.160	0.000	0.020	0.00	0.00	0.00	0.0	0.00
C	270.0	-0.120	0.000	0.020	0.00	0.00	0.00	0.0	0.00
C	265.0	-0.290	0.000	0.070	0.00	0.00	0.00	0.0	0.00
C	262.0	-0.490	0.000	0.060	0.00	0.00	0.00	0.0	0.00
C	258.0	-0.200	0.000	0.040	0.00	0.00	0.00	0.0	0.00
C	256.7	-0.770	0.000	1.110	0.00	0.00	0.00	0.0	0.00
C	254.0	-0.480	0.000	0.060	0.00	0.00	0.00	0.0	0.00
C	253.0	-0.160	0.000	0.040	0.00	0.00	0.00	0.0	0.00
C	225.0	-2.370	0.000	3.840	0.00	0.00	0.00	0.0	0.00
C	216.7	-0.730	0.000	1.110	0.00	0.00	0.00	0.0	0.00
C	210.0	-0.080	0.000	0.020	0.00	0.00	0.00	0.0	0.00
C	197.0	-0.470	0.000	0.380	0.00	0.00	0.00	0.0	0.00
C	185.0	-3.670	0.000	3.720	0.00	0.00	0.00	0.0	0.00
C	180.0	-0.150	0.000	0.050	0.00	0.00	0.00	0.0	0.00
C	180.0	-0.180	0.000	0.050	0.00	0.00	0.00	0.0	0.00
C	175.0	-0.470	0.000	0.040	0.00	0.00	0.00	0.0	0.00
C	166.7	-0.680	0.000	1.110	0.00	0.00	0.00	0.0	0.00
C	166.0	-0.110	0.000	0.030	0.00	0.00	0.00	0.0	0.00
C	164.0	-0.210	0.000	0.050	0.00	0.00	0.00	0.0	0.00
C	160.0	-0.110	0.000	0.030	0.00	0.00	0.00	0.0	0.00
C	159.0	-0.490	0.000	0.040	0.00	0.00	0.00	0.0	0.00
C	157.0	-0.140	0.000	0.050	0.00	0.00	0.00	0.0	0.00
C	155.0	-0.070	0.000	0.050	0.00	0.00	0.00	0.0	0.00
C	153.0	-0.070	0.000	0.020	0.00	0.00	0.00	0.0	0.00
C	128.0	-0.130	0.000	0.040	0.00	0.00	0.00	0.0	0.00
C	125.0	-1.050	0.000	1.000	0.00	0.00	0.00	0.0	0.00
C	116.0	-0.720	0.000	0.358	-0.65	-1.12	-1.69	180.0	0.00

C	116.0	-0.720	0.000	0.358	1.30	0.00	-1.69	0.0	0.00
C	116.0	-0.160	0.000	0.050	0.00	0.00	0.00	0.0	0.00
C	106.7	-0.600	0.000	1.110	0.00	0.00	0.00	0.0	0.00
C	105.0	-0.160	0.000	0.020	0.00	0.00	0.00	0.0	0.00
C	104.0	-0.815	0.000	0.358	1.30	0.00	-2.32	0.0	0.00
C	92.0	-0.030	0.000	0.010	0.00	0.00	0.00	0.0	0.00
C	84.0	-0.180	0.000	0.040	0.00	0.00	0.00	0.0	0.00
C	84.0	-1.700	0.000	3.480	0.00	0.00	0.00	0.0	0.00
C	80.0	-2.600	0.000	3.450	0.00	0.00	0.00	0.0	0.00
C	65.0	-0.050	0.000	0.020	0.00	0.00	0.00	0.0	0.00
C	56.7	-0.500	0.000	1.110	0.00	0.00	0.00	0.0	0.00
C	50.0	-0.630	0.000	0.080	0.00	0.00	0.00	0.0	0.00
C	29.0	-0.040	0.000	0.010	0.00	0.00	0.00	0.0	0.00

D	288.0	-0.325	0.000	0.082	0.00	0.00	0.00		
D	284.0	-0.325	0.000	0.082	0.00	0.00	0.00		
D	284.0	-0.320	0.000	0.091	0.00	0.00	0.00		
D	280.0	-0.320	0.000	0.091	0.00	0.00	0.00		
D	280.0	-0.322	0.000	0.096	0.00	0.00	0.00		
D	260.0	-0.321	0.000	0.096	0.00	0.00	0.00		
D	260.0	-0.322	0.000	0.109	0.00	0.00	0.00		
D	256.7	-0.322	0.000	0.109	0.00	0.00	0.00		
D	256.7	-0.313	0.000	0.146	0.00	0.00	0.00		
D	220.0	-0.310	0.000	0.146	0.00	0.00	0.00		
D	220.0	-0.311	0.000	0.168	0.00	0.00	0.00		
D	216.7	-0.311	0.000	0.168	0.00	0.00	0.00		
D	216.7	-0.298	0.000	0.235	0.00	0.00	0.00		
D	200.0	-0.297	0.000	0.235	0.00	0.00	0.00		
D	200.0	-0.296	0.000	0.226	0.00	0.00	0.00		
D	180.0	-0.294	0.000	0.226	0.00	0.00	0.00		
D	180.0	-0.297	0.000	0.246	0.00	0.00	0.00		
D	170.0	-0.296	0.000	0.246	0.00	0.00	0.00		
D	170.0	-0.294	0.000	0.249	0.00	0.00	0.00		
D	166.7	-0.294	0.000	0.249	0.00	0.00	0.00		
D	166.7	-0.273	0.000	0.274	0.00	0.00	0.00		
D	160.0	-0.273	0.000	0.274	0.00	0.00	0.00		
D	160.0	-0.271	0.000	0.258	0.00	0.00	0.00		
D	140.0	-0.269	0.000	0.258	0.00	0.00	0.00		
D	140.0	-0.270	0.000	0.265	0.00	0.00	0.00		
D	120.0	-0.268	0.000	0.265	0.00	0.00	0.00		
D	120.0	-0.268	0.000	0.274	0.00	0.00	0.00		
D	110.0	-0.268	0.000	0.274	0.00	0.00	0.00		
D	110.0	-0.264	0.000	0.276	0.00	0.00	0.00		
D	106.7	-0.264	0.000	0.276	0.00	0.00	0.00		
D	106.7	-0.237	0.000	0.300	0.00	0.00	0.00		
D	60.0	-0.231	0.000	0.303	0.00	0.00	0.00		
D	60.0	-0.226	0.000	0.300	0.00	0.00	0.00		
D	56.7	-0.226	0.000	0.300	0.00	0.00	0.00		
D	56.7	-0.185	0.000	0.305	0.00	0.00	0.00		
D	10.0	-0.179	0.000	0.305	0.00	0.00	0.00		
D	10.0	-0.181	0.000	0.306	0.00	0.00	0.00		
D	0.0	-0.175	0.000	0.299	0.00	0.00	0.00		

GUY LOADING

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.. WIND LOADING ..			TEMP	.ICE LOAD..		CONV	PROFILES.	.LOAD FACTORS.	
AZI	SPEED	REF	CHANGE	RAD	DENS	TOL	CAB	WIND	WIND DEAD ICE
DEG	MPH	PSF	DEG	IN	PCF				

0.0 90.0 0.00 -10.00 0.50 56.00 0.0100 2 1 1.00 1.00 1.00

CABLE PROFILE: 1 - CATENARY 2 - PARABOLIC

WIND PROFILE: 1 - EIA 222 F 2 - Kz = 1 ; Gh = 1
 3 - EIA 222 C 4 - Special Factors
 5 - Site Specific Wind Formula

MAXIMUM LEG LOADS AND FACE SHEARS (KIP - stress in KSI)

MAST ELEV FT	MAX LEG LOADS					MAX FACE SHEARS		
	AXIAL	BENDING TENS	COMP	TOTAL TENS	COMP	TORSN	BEAM	TOTAL
288.00	1.2A	8.5E	8.5C	7.4E	9.7C	0.0A	-0.9B	0.9B
	1.2A	9.6E	9.6C	8.4E	10.9C	0.0G	-1.4B	1.4B
286.00	1.2A	9.6E	9.6C	8.4E	10.9C	0.0G	-1.4B	1.4B
	1.3A	11.2E	11.2C	9.9E	12.5C	0.0G	1.8D	1.8D
284.00	22.5A	1.9G	1.6I	0.0A	24.1I	-0.1L	3.3B	3.4L
	22.6A	7.4G	6.6I	0.0A	29.2I	-0.1L	2.4B	2.6L
280.00	22.6A	7.4G	6.6I	0.0A	29.2I	-0.1L	2.4B	2.6L
	23.0A	13.4G	11.6I	0.0A	34.5I	-0.1L	0.3C	0.4K
270.34	23.0A	13.4G	11.6I	0.0A	34.5I	-0.1L	0.3C	0.4K
	23.2A	10.2G	7.5I	0.0A	30.7I	-0.1L	1.9H	2.0F
262.00	23.3A	10.2G	7.5I	0.0A	30.7I	-0.1L	2.3H	2.3F
	23.5A	3.2G	2.7L	0.0A	25.5L	-0.1L	3.5H	3.6F
256.67	43.4I	17.7G	14.3I	0.0A	57.7I	-0.3L	4.0B	4.3L
	43.6I	24.2G	20.0I	0.0A	63.6I	-0.3L	3.0B	3.3L
253.00	43.6I	24.2G	20.0I	0.0A	63.7I	-0.3L	2.9B	3.2L
	44.4I	33.7G	26.3I	0.0A	70.7I	-0.3L	0.8I	1.0E
236.67	44.4I	33.7G	26.3I	0.0A	70.7I	-0.3L	0.8I	1.0E
	45.0I	23.9G	14.2I	0.0A	59.2I	-0.3L	3.1H	3.2F
225.00	46.3I	23.9G	14.2I	0.0A	60.5I	-0.3L	4.7H	4.8F
	46.5I	11.8G	7.3H	0.0A	52.3H	-0.3L	5.7H	5.8F
220.01	46.5I	11.8G	7.3H	0.0A	52.3H	-0.3L	5.7H	5.8F
	46.7I	10.3A	7.7H	0.0A	52.9H	-0.3L	6.4H	6.5F
216.67	64.0I	17.0G	9.1K	0.0A	70.4F	-0.5L	-4.0L	4.5L

P. A10

210.00	64.6I	28.4G	16.3J	0.0A	80.7I	-0.5L	-2.7L	3.2L
	64.6I	28.4G	16.3J	0.0A	80.7I	-0.5L	-2.6L	3.1L
197.00	65.6I	37.7G	22.9H	0.0A	88.3I	-0.5L	0.3I	0.5L
	65.7I	37.7G	22.9H	0.0A	88.4I	-0.5L	0.5I	0.8E
191.67	66.1I	35.9G	20.8H	0.0A	86.0I	-0.5L	1.4I	1.7J
	66.1I	35.9G	20.8H	0.0A	86.0I	-0.5L	1.4I	1.7J
185.00	66.6I	29.6G	15.0G	0.0A	79.6H	-0.5L	-2.7J	3.0J
	67.8I	29.6G	15.0G	0.0A	80.9H	-0.5L	-5.1J	5.4J
180.00	68.2I	16.3G	12.6F	0.0A	79.1H	-0.5L	-6.1J	6.4J
	68.3I	16.3G	12.6F	0.0A	79.1H	-0.5L	-6.4J	6.6J
175.00	68.7I	19.7A	13.1H	0.0A	80.0H	-0.5L	-7.3J	7.6J
	68.7I	19.7A	13.1H	0.0A	80.0H	-0.5L	-7.7J	7.9J
170.01	69.1I	39.1A	22.7C	0.0A	88.6E	-0.5L	-8.6J	8.9J
	69.1I	39.1A	22.7C	0.0A	88.6E	-0.5L	-8.6J	8.9J
166.67	69.4I	53.3A	36.9C	0.0A	103.0C	-0.5L	-9.3J	9.6J
	85.0I	41.4A	22.8C	0.0A	104.6A	-0.7L	7.0B	7.7L
157.00	86.0I	14.8A	14.7F	0.0A	99.4F	-0.7L	4.6B	5.3L
	86.0I	14.8A	14.7F	0.0A	99.4F	-0.7L	4.5B	5.2L
153.00	86.3I	20.4G	14.9F	0.0A	100.0F	-0.7L	3.7B	4.4L
	86.3I	20.4G	14.9F	0.0A	100.0F	-0.7L	3.7B	4.4L
140.01	87.5I	38.6G	19.4G	0.0A	103.4G	-0.7L	1.4G	2.1L
	87.5I	38.6G	19.4G	0.0A	103.4G	-0.7L	1.4G	2.1L
136.67	87.8I	40.7G	20.5G	0.0A	104.8G	-0.7L	0.9G	1.5L
	87.8I	40.7G	20.5G	0.0A	104.8G	-0.7L	0.9G	1.5L
125.00	88.8I	39.8G	19.9G	0.0A	105.3G	-0.7L	-1.6J	2.2J
	89.1I	39.8G	19.9G	0.0A	105.6G	-0.7L	-2.3J	2.9J
120.02	89.6I	33.7G	16.9G	0.0A	103.1H	-0.7L	-3.1J	3.7J
	89.6I	33.7G	16.9G	0.0A	103.1H	-0.7L	-3.1J	3.7J
116.00	89.9I	27.1G	14.4H	0.0A	103.2H	-0.7L	-3.9J	4.5J
	90.2I	26.9G	14.0F	0.0A	103.0H	0.6B	-4.9J	5.4D

110.02	90.7I	18.2A	13.5H	0.0A	103.1H	0.6B	-6.0J	6.5D
	90.7I	18.2A	13.5H	0.0A	103.1H	0.6B	-6.0J	6.5D
106.67	91.1I	27.9A	14.0A	0.0A	104.7I	0.6B	-6.6J	7.1D
	100.4I	21.2A	13.3F	0.0A	113.1F	0.5F	8.4B	8.9B
92.00	102.0I	58.9G	35.3I	0.0A	137.3I	0.7F	5.5B	6.1B
	102.0I	58.9G	35.3I	0.0A	137.3I	0.7F	5.4B	6.1B
81.67	104.2I	79.4G	59.4I	0.0A	163.6I	0.7F	2.7A	3.2A
	104.2I	79.4G	59.4I	0.0A	163.6I	0.7F	2.7A	3.2A
65.00	107.0I	71.2G	58.0I	0.0A	165.1I	0.7F	-1.9J	2.4G
	107.0I	71.2G	58.0I	0.0A	165.1I	0.7F	-2.0J	2.5G
60.02	107.5I	63.2G	52.3A	0.0A	159.8I	0.7F	-2.7J	3.2D
	107.5I	63.2G	52.3A	0.0A	159.8I	0.7F	-2.7J	3.2D
56.67	107.9I	56.6G	47.5A	0.0A	155.0I	0.7F	-3.2J	3.7D
	111.5I	59.5G	50.3I	0.0A	161.7I	0.7F	2.1B	2.6F
50.00	112.1I	66.4G	58.0I	0.0A	170.1I	0.7F	1.3B	1.8F
	112.2I	66.4G	58.0I	0.0A	170.1I	0.7F	0.9B	1.4F
28.33	114.4I	60.4G	55.3I	0.0A	169.6I	0.7F	2.0H	2.3G
	114.4I	60.4G	55.3I	0.0A	169.6I	0.7F	2.0H	2.3G
10.03	116.2I	27.5G	25.7I	0.0A	141.9I	0.7F	4.2H	4.3H
	116.2I	27.5G	25.7I	0.0A	141.9I	0.7F	4.2H	4.3H
6.67	116.6I	19.0G	17.8I	0.0A	134.3I	0.7F	4.6H	4.7H
	116.6I	25.9G	24.2I	0.0A	140.8I	0.9F	4.6H	4.7D
0.00	117.2I	0.0J	0.0J	0.0A	117.2I	0.9F	5.4H	5.5D

MAXIMUM GUY FORCES AT MAST

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GUY LEVEL FT	GUY AZICOMPONENTS AT MAST.....				FACTOR OF SAFETY	...GUY ANGLES...	
		N KIP	E KIP	DOWN KIP	TOTAL KIP		VERT	HORIZ
284.0	0.0	12.0A	-1.0D	15.4A	19.5A	3.0A	-52.2B	8.0I
	120.0	-5.9D	10.0E	16.5E	20.1E	2.9E	-55.2F	-8.6I
	120.0	-5.8D	10.1E	16.6E	20.3E	2.9E	-55.1F	-8.6I
	240.0	-5.8J	-10.1I	16.8I	20.5I	2.9I	-55.5H	8.7E
	240.0	-5.8J	-10.0I	16.8I	20.4I	2.9I	-55.5H	8.6E
	0.0	12.0A	1.0J	15.3A	19.5A	3.0A	-52.2L	8.0I

P.A.R.

256.7	0.0	13.5A	-0.9D	15.4A	20.5A	2.8A	-48.8L	7.7I
	120.0	-6.5E	11.3E	16.7E	21.2E	2.8E	-52.1F	-8.2I
	120.0	-6.5E	11.3E	16.8E	21.3E	2.7E	-52.1F	-8.2I
	240.0	-6.5I	-11.3I	17.0I	21.5I	2.7I	-52.5H	8.3E
	240.0	-6.5I	-11.3I	17.0I	21.4I	2.7I	-52.5H	8.2E
	0.0	13.5A	0.9J	15.4A	20.5A	2.9A	-48.8L	7.7I
216.7	0.0	15.0A	-0.8D	14.1A	20.5A	2.8A	-43.4K	7.6H
	120.0	-7.3E	12.6E	15.7E	21.4E	2.7E	-47.3F	-8.0I
	120.0	-7.3E	12.6E	15.7E	21.4E	2.7E	-47.2F	-8.0I
	240.0	-7.3I	-12.6I	16.0I	21.6I	2.7I	-47.7H	8.0E
	240.0	-7.3I	-12.7I	16.1I	21.8I	2.7I	-47.8H	8.0E
	0.0	14.9A	-0.8D	14.0A	20.5A	2.8A	-43.4K	-7.6F
166.7	0.0	18.9A	-0.7D	13.3A	23.1A	2.5A	-35.3K	8.2H
	120.0	-9.2E	15.9E	15.4E	24.0E	2.4E	-40.0G	-7.7J
	120.0	-9.2E	16.0E	15.4E	24.1E	2.4E	-40.0G	-7.7J
	240.0	-9.2I	-15.9I	15.7I	24.2I	2.4I	-40.6G	7.6D
	240.0	-9.3I	-16.2I	15.9I	24.5I	2.4I	-40.6G	7.6D
	0.0	18.7A	-0.7D	13.2A	22.9A	2.5A	-35.3C	-8.2F
106.7	0.0	17.1A	-0.5D	7.4A	18.6A	2.3A	-30.4G	7.6H
	120.0	-8.2E	14.3E	9.3E	18.9E	2.2E	-34.2K	-7.8J
	120.0	-8.4E	14.5E	9.4E	19.2E	2.2E	-34.1K	-7.6J
	240.0	-8.3I	-14.3I	9.6I	19.1I	2.2I	-34.5C	7.7D
	240.0	-8.5I	-14.6I	9.8I	19.6I	2.2I	-34.4C	7.6D
	0.0	16.7A	-0.5D	7.3A	18.3A	2.3A	-30.5G	-7.7F
56.7	0.0	8.6A	-0.4D	1.9A	8.8A	2.4A	-19.3G	6.1H
	120.0	-4.3E	7.4E	3.0E	9.1E	2.3E	-24.9K	-6.5J
	120.0	-4.3E	7.4E	3.0E	9.1E	2.3E	-24.8K	-6.4J
	240.0	-4.3I	-7.4I	3.1I	9.1I	2.3I	-25.5C	6.5D
	240.0	-4.4I	-7.6I	3.2I	9.3I	2.2I	-25.5C	6.5D
	0.0	8.5A	-0.4D	1.9A	8.7A	2.4A	-19.3G	-6.1F

MAXIMUM GUY FORCES AT ANCHOR

=====

GUY LEVEL FT	GUY AZICOMPONENTS AT ANCHOR.....				TOTAL KIP	FACTOR OF SAFETY
		RAD KIP	LAT KIP	VERT KIP	TOTAL KIP		
284.0	0.0	13.0A	-1.2J	13.9A	19.0A	3.1A	
	120.0	12.7E	1.2H	14.9E	19.6E	3.0E	
	120.0	12.8E	-1.2B	15.1E	19.8E	2.9E	
	240.0	12.8I	1.3L	15.3I	19.9I	2.9I	
	240.0	12.8I	-1.2F	15.2I	19.8I	2.9I	
	0.0	12.9A	1.2D	13.9A	19.0A	3.1A	
256.7	0.0	14.3A	-1.1J	14.1A	20.1A	2.9A	
	120.0	14.0E	1.2H	15.2E	20.7E	2.8E	
	120.0	14.1E	-1.2B	15.3E	20.8E	2.8E	
	240.0	14.1I	1.2L	15.6I	21.0I	2.8I	
	240.0	14.1I	-1.2F	15.5I	21.0I	2.8I	
	0.0	14.3A	1.1D	14.0A	20.0A	2.9A	
216.7	0.0	15.5A	-1.0J	12.9A	20.2A	2.9A	
	120.0	15.3E	1.0H	14.4E	21.0E	2.8E	
	120.0	15.3E	-1.0B	14.4E	21.0E	2.8E	
	240.0	15.3I	1.0L	14.7I	21.2I	2.7I	
	240.0	15.4I	-1.0F	14.8I	21.4I	2.7I	

	0.0	15.5A	1.0D	12.9A	20.1A	2.9A
166.7	0.0	19.2A	-0.9J	12.4A	22.8A	2.6A
	120.0	18.8E	0.9H	14.3E	23.6E	2.5E
	120.0	18.9E	-0.9B	14.4E	23.7E	2.5E
	240.0	18.8I	0.9L	14.6I	23.9I	2.4I
	240.0	19.1I	-0.9F	14.8I	24.2I	2.4I
	0.0	19.0A	0.9D	12.3A	22.6A	2.6A
106.7	0.0	17.2A	-0.6J	6.9A	18.5A	2.3A
	120.0	16.6E	0.7H	8.7E	18.8E	2.3E
	120.0	16.9E	-0.7B	8.8E	19.1E	2.2E
	240.0	16.7I	0.7L	9.0I	19.0I	2.2I
	240.0	17.1I	-0.7F	9.2I	19.4I	2.2I
	0.0	16.8A	0.6D	6.7A	18.1A	2.3A
56.7	0.0	8.6A	-0.5J	1.6A	8.8A	2.4A
	120.0	8.6E	0.5H	2.7E	9.0E	2.3E
	120.0	8.6E	-0.5B	2.7E	9.0E	2.3E
	240.0	8.6I	0.5L	2.8I	9.0I	2.3I
	240.0	8.8I	-0.5F	2.9I	9.3I	2.2I
	0.0	8.5A	0.5D	1.6A	8.7A	2.4A

MAXIMUM ANCHOR LOADS

=====

AZI DEG	RADIUS FT	GUY TO ELEV FTANCHOR LOADS.....		SHAFT FORCES.....			ANGLE DEG
			HORIZ KIP	VERT KIP	LATER- AL KIP	AXIAL KIP	...LATERAL... VERT PLANE KIP	HORIZ PLANE KIP	
0.0	235.0	284.0	12.9A	13.9A	1.1D	18.6A	3.9A	1.1D	
		284.0	13.0A	13.9A	-1.1J	18.6A	3.9A	-1.1J	
		256.7	14.3A	14.0A	1.0D	19.8A	3.2A	1.0D	
		256.7	14.3A	14.1A	-1.0J	19.8A	3.2A	-1.0J	
		216.7	15.5A	12.9A	0.8D	20.1A	1.6A	0.8D	
		216.7	15.5A	12.9A	-0.8J	20.1A	1.6A	-0.8J	
		166.7	19.0A	12.3A	-0.7J	22.6A	-0.9A	-0.7J	
		166.7	19.2A	12.4A	0.7D	22.8A	-0.9A	0.7D	
		106.7	16.8A	6.7A	-0.5J	17.7A	-4.2A	-0.5J	
		106.7	17.2A	6.9A	0.5D	18.0A	-4.3A	0.5D	
		56.7	8.5A	1.6A	-0.4J	7.9A	-3.6A	-0.4J	
		56.7	8.6A	1.6A	0.4D	8.0A	-3.6A	0.4D	
					174.9A	123.2A	8.9D	213.9A	0.0A
120.0	235.0	284.0	12.8E	15.1E	1.1H	19.4E	3.6E	1.1H	
		284.0	12.7E	14.9E	-1.1B	19.3E	3.5E	-1.1B	
		256.7	14.1E	15.3E	1.0H	20.6E	3.0E	1.0H	
		256.7	14.0E	15.2E	-1.0B	20.5E	3.0E	-1.0B	
		216.7	15.3E	14.4E	0.9H	21.0E	1.5E	0.9H	
		216.7	15.3E	14.4E	-0.9B	20.9E	1.5E	-0.9B	
		166.7	18.9E	14.4E	-0.7B	23.7E	-0.8E	-0.7B	
		166.7	18.8E	14.3E	-0.7B	23.6E	-0.8E	-0.7B	
		106.7	16.9E	8.8E	-0.5B	18.7E	-3.9E	-0.5B	
		106.7	16.6E	8.7E	-0.5B	18.4E	-3.8E	-0.5B	
		56.7	8.6E	2.7E	-0.4B	8.4E	-3.4E	-0.4B	
		56.7	8.6E	2.7E	-0.4B	8.4E	-3.4E	-0.4B	
					172.6E	140.9E	9.5H	222.8E	0.0G

240.0	235.0	284.0	12.8I	15.2I	1.1L	19.5I	3.5I	1.1L
		284.0	12.8I	15.3I	-1.1F	19.6I	3.5I	-1.1F
		256.7	14.1I	15.5I	1.0L	20.7I	2.9I	1.0L
		256.7	14.1I	15.6I	-1.0F	20.8I	2.9I	-1.0F
		216.7	15.4I	14.8I	0.9L	21.3I	1.5I	0.9L
		216.7	15.3I	14.7I	-0.9F	21.2I	1.5I	-0.9F
		166.7	19.1I	14.8I	0.8L	24.2I	-0.8I	0.8L
		166.7	18.8I	14.6I	0.8L	23.9I	-0.8I	0.8L
		106.7	17.1I	9.2I	0.5L	19.0I	-3.9I	0.5L
		106.7	16.7I	9.0I	0.5L	18.6I	-3.8I	0.5L
		56.7	8.8I	2.9I	0.4L	8.6I	-3.4I	0.4L
		56.7	8.6I	2.8I	0.4L	8.4I	-3.3I	0.4L

		173.6I	144.4I	-9.5F	225.8I	0.0G	-9.5F	39.8I



SNET Mobility, LLC
500 Enterprise Drive
Rocky Hill, Connecticut 06067-3900
Phone: (860) 513-7700
Fax: (860) 513-7190

Michele G. Briggs
Manager of Real Estate

February 26, 2002

Mr. Mortimer A. Gelston, Chairman
Connecticut Siting Council
10 Franklin Square
New Britain, Connecticut 06051



Re: SNET Mobility, LLC and Nextel Communications Inc. notice of intent to modify an existing telecommunications facility located on the University of Connecticut campus off North Eagleville Road, Storrs, Connecticut. (Docket No. 179)

Dear Mr. Gelston:

SNET Mobility, LLC ("SNET") and Nextel Communications Inc. ("Nextel") intend to install telecommunications antennas and associated equipment at an existing multicarrier telecommunications tower facility at North Eagleville Road in Storrs, Connecticut. Please accept this letter as notification to the Connecticut Siting Council ("Council"), pursuant to R.C.S.A. Section 16-50j-73, of construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2). In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter is being sent to the Town Manager of the Town of Mansfield.

The North Eagleville Road facility is owned and operated by the University of Connecticut ("UCONN") and WHUS-FM Radio. UCONN's Tower Site Manager is Mr. George Davis, whose address is 1814 Route 171, Woodstock Valley, Connecticut 06282. The tower is located on the UCONN campus at coordinates N41° 48' 53" and W 72° 15' 36" (NAD 83).

UCONN has agreed to plans put forth by SNET and Nextel pursuant to mutually acceptable terms and conditions and has authorized them to obtain all necessary government approvals. Nextel has authorized SNET to provide this notice to the Council on its behalf.

On April 26, 2001, the Council approved EM-NEXTEL-078-000724 in which Nextel and SNET (then known as "Springwich Cellular Limited Partnership") proposed to co-locate telecommunications antennas and equipment at the WHUS tower facility. (The Council will recall that it was then promoting the consolidation of telecommunications equipment on three towers at the site onto the WHUS tower with the goal of eliminating the other two towers.) The present Notice of Exempt Modification concerns changes in antenna models and/or centerline

heights that the applicants have decided upon since April, 2001. Since the *previously approved* Nextel and SNET antenna configurations have not yet been implemented on the WHUS tower, the current application amends the record only, rather than conditions existing on the tower itself.

Attached to this notice are location maps, the revised tower profile, and a new structural analysis report incorporating the changed equipment information. The structural analysis indicates that the tower is structurally capable of supporting the revised antenna configuration.

SNET Modifications

SNET originally proposed in EM-NEXTEL-078-000724 to install nine (9) Allgon Model 7120.16 panel antennas on the WHUS tower. In Responses to Interrogatories filed on April 5, 2001 SNET amended its application to propose installing twelve (12) panel antennas as follows:

- Six (6) Decibel Products Model DB846H80(E)-SX
- Three (3) EMS Model RS90-12-000A-2
- Three (3) Allgon Model 7125.18.05.

SNET now proposes to mount the following twelve (12) antennas on the WHUS tower rather than those listed above:

- Eight (8) Decibel Products Model DB846H80(E)-SX
- Three (3) Allgon Model 7125.18.05
- One (1) EMS RS90-12-00DA-2.

Additionally, SNET first proposed in EM-NEXTEL-078-000724 to mount its antennas at a centerline height of 150 feet above ground level ("AGL"). It later amended the mounting height to 145 feet AGL in a letter to Analyst Paul Aresta dated April 20, 2001. By subsequent agreement with UCONN and WHUS, SNET now proposes to mount its antennas at a centerline height of 185 feet AGL.

Nextel Modification

The only modification proposed by Nextel is a change in antenna centerline height from 248 feet to 225 feet AGL.

Discussion

The proposed changes to the Storrs tower facility do not constitute a modification as defined in Connecticut General Statutes ("C.G.S.") Section 16-50i(d) because the general physical characteristics of the facility will not be significantly changed or altered. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in R.C.S.A.

Section 16-50j-72(b)(2) because they will not result in any substantial adverse environmental effect.

1. The height of the overall structure will be unaffected.
2. The proposed changes will not affect the property boundaries. No changes are proposed that would affect the facility site plan already approved by the Council in EM-NEXTEL-078-000724.
3. The proposed additions will not increase the noise level at the existing facility by six decibels or more. No changes are proposed that would alter noise emissions as already approved by the Council in EM-NEXTEL-078-000724.
4. Operation of the additional antennas will not increase the total radio frequency electromagnetic radiation power density, measured at the tower base, to or above the standard adopted by the State of Connecticut and the FCC. The "worst-case" exposure calculation in accordance with FCC OET Bulletin No. 65 (1997) for a point of interest at the base of the tower in relation to the operation of the currently proposed antenna array is as follows:

Company	Centerline Height (feet)	Frequency (MHz)	Number of Channels	Power Per Channel (Watts)	Power Density † (mW/cm ²)	Standard Limits (mW/cm ²)	Percent of Limit
Existing *	--	--	--	--	0.1530	--	31.6
Nextel	225	851	9	100	0.0064	0.5673	1.1
SNET	185	880-894	19	100	0.0199	0.5867	3.4
UCONN Police**	180	866-868	3	197	0.0066	0.5773	1.1
UCONN Police***	50	Not given in UCONN application.					0.0
Total							37.2 %

* Values taken from "Tower Consolidation Feasibility Study," RCC Consultants, February 2001.

** See, EM-UCONN-078-010525. Supplemented by data from G. Davis, UCONN Tower Manager.

*** See, EM-UCONN-078-010824.

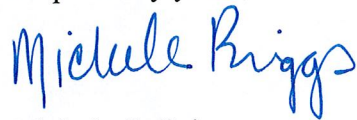
† Please note that the standard power density equation provided by the Council in its memo of January 22, 2001 incorporates a ground reflection factor of 2.56 (i.e., the square of 1.6) as described in FCC OET Bulletin No. 65.

As the table demonstrates, the cumulative "worst-case" exposure would be 37.2 % of the ANSI/IEEE standard, as calculated for mixed frequency sites. This is less than the exposure originally calculated with Nextel and SNET antennas at 248 feet and 145 feet, respectively. Total power density levels resulting from SNET's use of the tower facility would thus comply with applicable standards.

For the foregoing reasons, SNET respectfully submits that proposed changes to implement expanded shared use at the Storrs site constitute an exempt modification under R.C.S.A. Section 16-50j-72(b)(2).

Please feel free to call me at (860) 513-7700 with questions concerning this application. Thank you for your consideration in this matter.

Respectfully yours,



Michele G. Briggs
Manager of Real Estate

Enclosures

cc: Honorable Martin H. Berliner, Town Manager, Town of Mansfield

Storrs - WHUS UCONN Tower

WHUS Tower
Location of Proposed SNET Facility

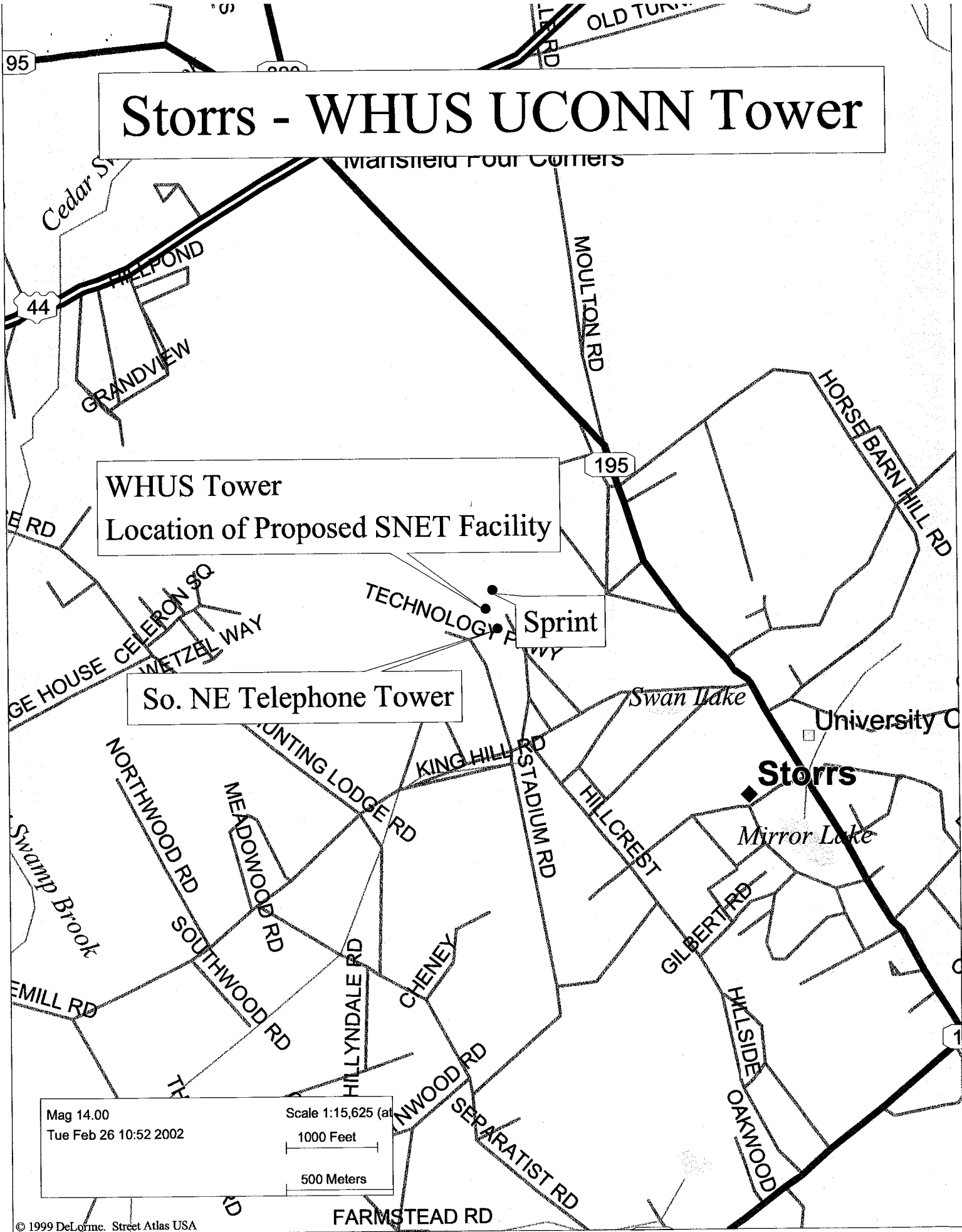
So. NE Telephone Tower

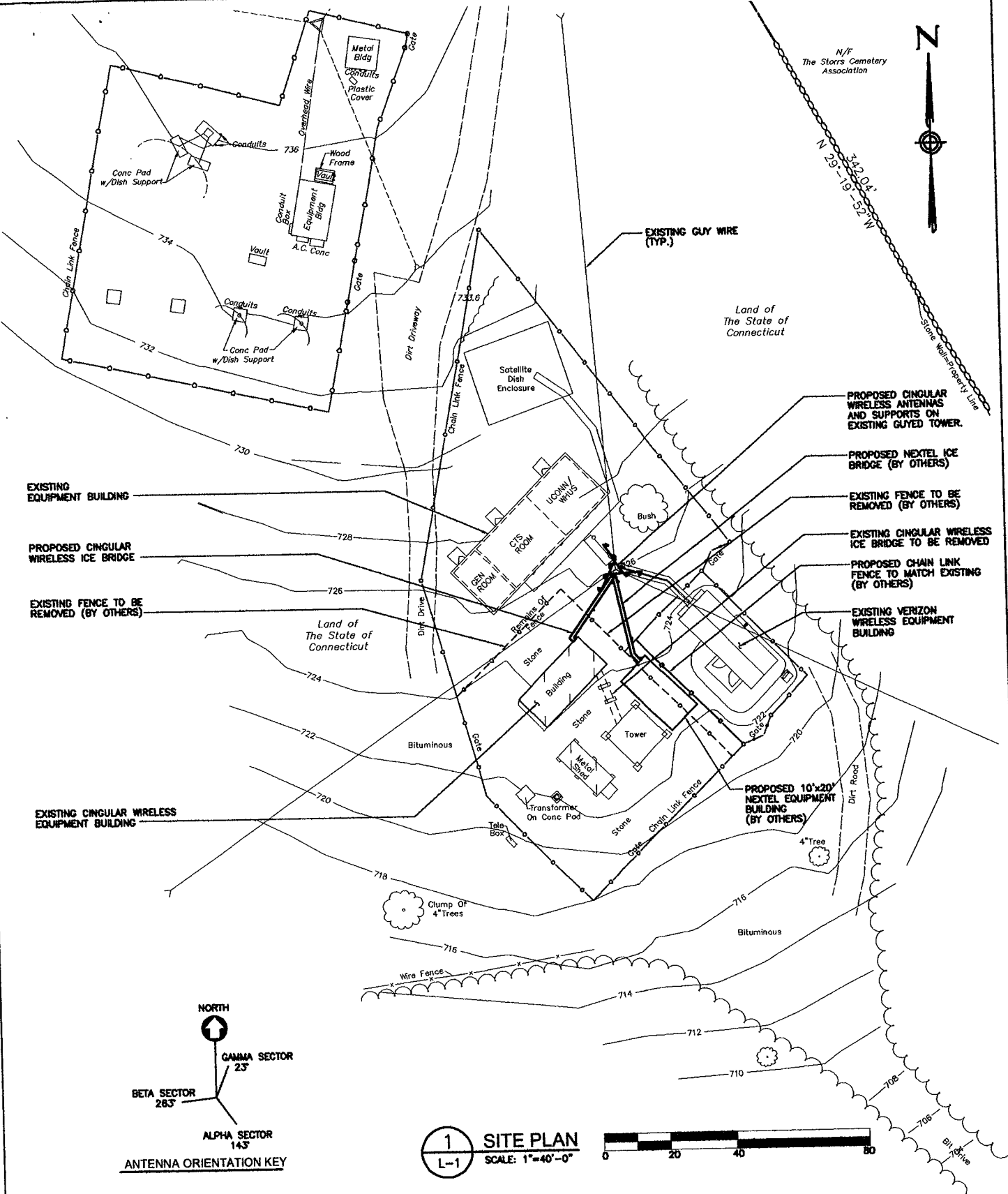
Sprint

Storrs

Mag 14.00
Tue Feb 26 10:52 2002

Scale 1:15,625 (at
1000 Feet
500 Meters





N/F
The Storrs Cemetery
Association



N 29°-19'-52" W
342.04'

Land of
The State of
Connecticut

PROPOSED CINGULAR
WIRELESS ANTENNAS
AND SUPPORTS ON
EXISTING GUYED TOWER.

PROPOSED NEXTEL ICE
BRIDGE (BY OTHERS)

EXISTING FENCE TO BE
REMOVED (BY OTHERS)

EXISTING CINGULAR WIRELESS
ICE BRIDGE TO BE REMOVED

PROPOSED CHAIN LINK
FENCE TO MATCH EXISTING
(BY OTHERS)

EXISTING VERIZON
WIRELESS EQUIPMENT
BUILDING

PROPOSED 10'x20'
NEXTEL EQUIPMENT
BUILDING
(BY OTHERS)

EXISTING
EQUIPMENT BUILDING

PROPOSED CINGULAR
WIRELESS ICE BRIDGE

EXISTING FENCE TO BE
REMOVED (BY OTHERS)

EXISTING CINGULAR WIRELESS
EQUIPMENT BUILDING

NORTH



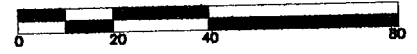
GAMMA SECTOR
23°

BETA SECTOR
283°

ALPHA SECTOR
143°

ANTENNA ORIENTATION KEY

1 SITE PLAN
L-1 SCALE: 1"=40'-0"



SITE ID NO:
Designed by:
Drawn by: CTJ
Checked by:
Approved by:

URS CORPORATION AES
795 BROOK STREET, BLDG 5
ROCKY HILL, CONNECTICUT
1-(880)-529-9882

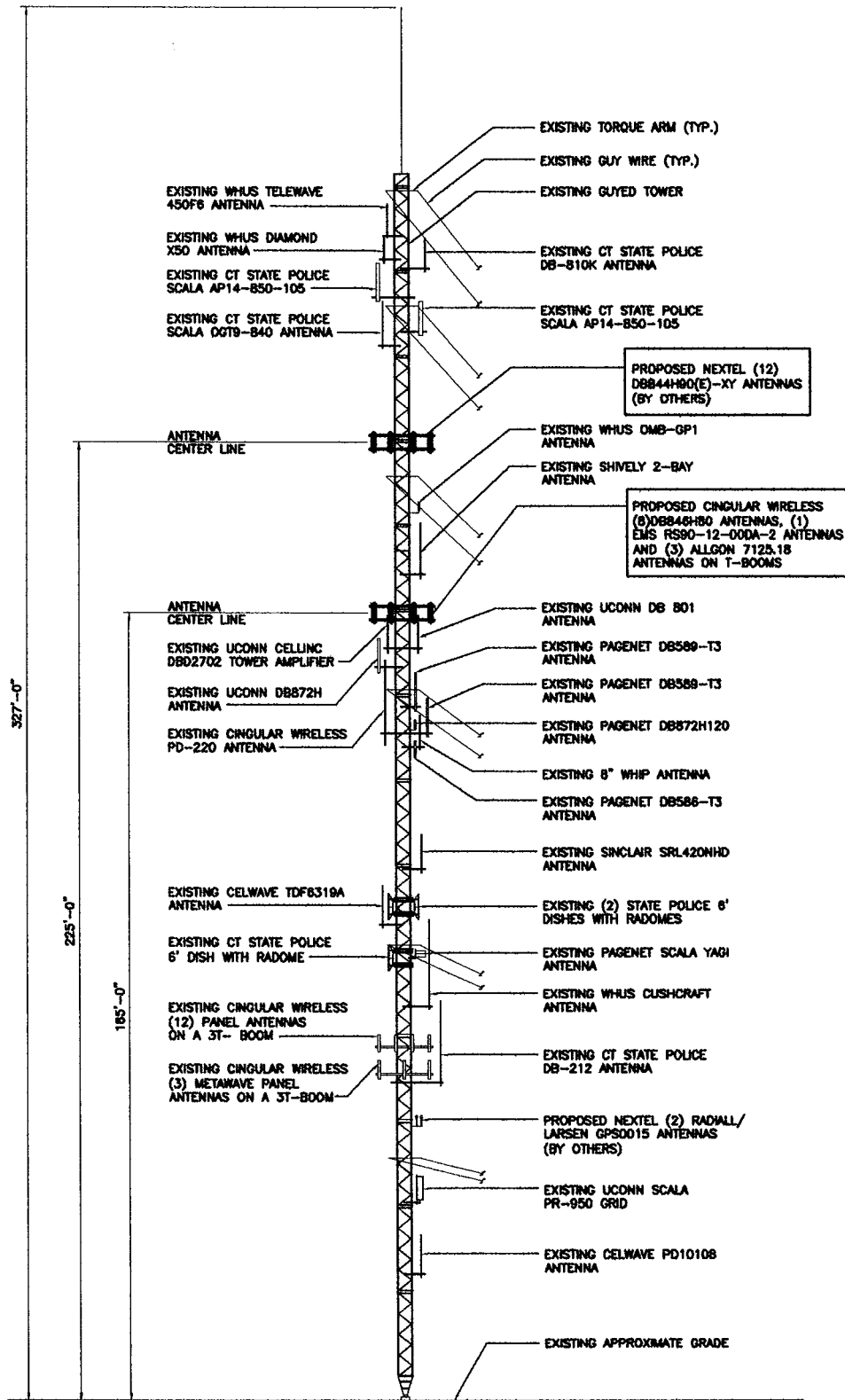
cingular
WIRELESS
WIRELESS COMMUNICATIONS FACILITY

WHUS RADIO TOWER
STORRS, CONNECTICUT

SITE ADDRESS:

02/25/02	REVISED
REV. DATE:	DESCRIPTION
Scale: AS NOTED	Date: 02-25-02
Job No. F301804.47	File No. L-1

Draw. No.
L-1
Draw. 1 of 2



327'-0"

225'-0"

185'-0"

1 TOWER ELEVATION
L-2 SCALE: 1"=40'-0"



SITE ID NO:
Designed by:
Drawn by: CTJ
Checked by:
Approved by:

URS CORPORATION AES
795 BROOK STREET, BLDG 5
ROCKY HILL, CONNECTICUT
1-(866)-529-8882

cingular
WIRELESS
WIRELESS COMMUNICATIONS FACILITY
WHUS RADIO TOWER
STORRS, CONNECTICUT
SITE ADDRESS:

REV.	DATE	DESCRIPTION
△	02/25/02	REVISED
Scale:	AS NOTED	Date: 01-31-00
Job No.	F301804.47	File No. L-2

Dwg. No.
L-2
Dwg. 2 of 2

Structural Analysis Report

Job #02-01020

Existing 327' Sabre Communications Corporation
4400SRW Guyed Tower

Located at Storrs, Connecticut

Report Completed for

Cingular Wireless

Westwood, MA

Prepared by

Sabre Communications Corporation

January 16, 2002



Introduction

The purpose of this analysis is to determine if the existing tower is in conformance with the requirements of ANSI/TIA/EIA 222-F, while supporting specified equipment. The tower is a 327' 4400SRW guyed tower and was originally manufactured by Sabre Communications Corporation. The tower is located in Storrs, Connecticut. The analysis is being performed for Cingular Wireless, Westwood, MA.

Method of Analysis

The computer program that was used for this analysis is described on the attached page. The analysis was performed using a basic wind speed of 90 mph concurrent with 1/2" ice, in accordance with ANSI/TIA/EIA 222-F. Allowable stresses, safety factors and load factors were also determined in accordance with this standard.

Supported Equipment

The analysis was performed for the tower, supporting the following equipment:

WHUS

1. One (1) Shively 6813 2-bay antenna on a pole from 290' to 327'
2. One (1) OMB-GP1 antenna from 209' to 211'
3. One (1) Telewave 450F6 antenna from 273' to 281'
4. One (1) Diamond x50 antenna from 273' to 267'
5. One (1) Cushcraft antenna at 92'

Connecticut State Police

6. Two (2) 6' dishes with radomes from 113' to 119'
7. One (1) 6' dish with radome from 101' to 107'
8. One (1) Scala AP14-850-105 antenna from 258' to 267'
9. One (1) DB-810K antenna from 265' to 273'
10. One (1) Scala AP14-850-105 antenna from 258' to 250'
11. One (1) DB-212 antenna from 74' to 94'
12. One (1) Scala OGT9-840 antenna from 258' to 247'

Verizon

- 13. Twelve (12) panel antennas on a 3T-Boom from 82' to 86'
- 14. One (1) PD-220 antenna from 154' to 174'
- 15. Three (3) Metawave panel antennas on a 3T-Boom at 80'
- 16. Twelve (12) 1-1/4" lines and three (3) 1/2" lines

PageNet

- 17. One (1) DB872H120 antenna from 158' to 160'
- 18. One (1) DB586-T3 antenna from 151' to 155'
- 19. One (1) DB589-T3 antenna from 156' to 165'
- 20. One (1) DB589-T3 antenna from 162' to 171'
- 21. One (1) Scala yagi antenna at 105'

Other Tenants

- 22. One (1) Shively 2-bay antenna from 193' to 206'
- 23. One (1) Celwave TDF6319A antenna from 111' to 121'
- 24. One (1) 8' whip antenna from 153' to 161'
- 25. One (1) Celwave PD10108 antenna at 29'
- 26. One (1) Sinclair SRL420NHD antenna from 124' to 133'

SNET(proposed)

- 27. Eight (8) DB846H80 antennas, one (1) EMS RS90-12-00DA-2 antennas and three (3) Allgon 7125.18 antennas on a 3T-Boom at 185', with fifteen (15) 1-1/4" lines

Nextel (proposed)

- 28. Twelve (12) DB844H90(E)-XY antennas on a platform at 225', with fifteen (15) 1-5/8" lines
- 29. Two (2) Radiall/Larsen GPS0015 antennas at 65' with two (2) 1/2" lines

UCONN Inventory Update

- 30. One (1) DB 801 at 180'
- 31. One (1) Cellinc DBD2702 Tower Amplifier at 180'
- 32. One (1) DB872H at 175'
- 33. Scala PR-950 Grid at 50'

Other

- 34. Two (2) Ice Shields at 125'
- 35. Two Equipment Boxes at 258'
- 36. Side Lights and J Box at 155'

Each tower face is assumed to be fully covered with feedlines.

Results

Tower Section Elevation (ft)	Percentage of Allowable Leg Capacity Used (%)	Percentage of Allowable Diagonal Capacity Used (%)
0-20	68.1	76.7
20-40	73.3	44.6
40-60	73.3	57.2
60-80	73.4	50.2
80-100	73.5	72.7
100-120	68.2	59.1
120-140	54.6	46.0
140-160	71.8	58.5
160-180	65.6	60.4
180-200	67.8	87.9
200-220	58.2	40.7
220-240	60.7	51.9
240-260	60.7	39.7
260-280	52.0	26.4
280-288	55.5	47.8
288-323	13.8	

Guy Elevation (ft)	Percentage of Allowable Guy Capacity Used (%)
57	84.7
107	88.1
167	80.0
217	71.1
257	70.6
284	66.9

The results of the analysis show no overstresses in any tower component.

In addition, the results of the analysis show that the foundations are adequate.

Conclusions

Based on the preceding results, the following conclusions have been made:

1. The tower with specified equipment is adequate to achieve a basic wind speed rating of 90 mph concurrent with 1/2" ice, in accordance with ANSI/TIA/EIA 222-F.
2. No modifications are required, in order to meet the structural criteria stated above.
3. The analysis is valid only for the equipment listed above. If the equipment is not as listed, an additional analysis should be performed.
4. The analysis assumes that the tower contains no structural defects, and that all components have been installed properly.

Description of Guyed Tower Computer Program

A guyed tower computer program called GUYMAST™, developed by Guymast Inc., is utilized by Sabre Communications Corporation to perform the structural analysis and design of guyed towers. This program is one of the most widely used programs in the tower industry.

"GUYMAST performs the analysis of guyed towers using three separate models. The first consists of a set of guys connected at the same elevation on the mast, used independently for each guy level. The second is a continuous beam model using the stiffness method (including beam-column interaction), used to analyze the bending of the mast in the two orthogonal vertical planes independently. The third is a model of a shaft subjected to torsional loading, for the analysis of tower twist.

"With an arbitrary set of initial displacements, the guy model is used to obtain the guy stiffnesses and guy loads to be applied in the mast models. These guy stiffnesses and loads are then used in the two mast models to calculate a new set of mast displacements, which are then fed back into the guy model to obtain a better approximation of guy stiffnesses and loads.

"The process is repeated until displacements calculated in the beam model match, within a specified tolerance, those used by the guy model in determining the support stiffnesses."¹

1. GUYMASTTM user manual, by Guymast Inc., © 1997.



SNET Mobility, LLC
500 Enterprise Drive
Rocky Hill, Connecticut 06067-3900
Phone: (860) 513-7700
Fax: (860) 513-7190

Michele G. Briggs
Manager of Real Estate

February 26, 2002

Honorable Martin H. Berliner, Town Manager
Mansfield Town Hall
Four South Eagleville Rd.
Mansfield, Connecticut 06268

Re: SNET Mobility, LLC and Nextel Communications Inc. notice of intent to modify an existing telecommunications facility located on the University of Connecticut campus off North Eagleville Road, Storrs, Connecticut. (Docket No. 179)

Dear Mr. Berliner:

SNET Mobility, LLC ("SNET") and Nextel Communications Inc. ("Nextel") intend to install telecommunications antennas and associated equipment at an existing multicarrier telecommunications tower facility at North Eagleville Road in Storrs, Connecticut.

The North Eagleville Road facility is owned and operated by the University of Connecticut ("UCONN") and WHUS-FM Radio. UCONN's Tower Site Manager is Mr. George Davis, whose address is 1814 Route 171, Woodstock Valley, Connecticut 06282.

A Notice of Exempt Modification has been filed with the Connecticut Siting Council as required by Regulations of Connecticut State Agencies ("R.C.S.A.") Section 16-50j-73. Please accept this letter as notification to the Town of Mansfield under Section 16-50j-73 of construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2).

The attached letter fully sets forth the SNET proposal. However, if you have any questions or require any further information on the plans for the site or the Siting Council's procedures, please contact the undersigned or Mr. Derek Phelps, Executive Director of the Connecticut Siting Council, at (860) 827-2935.

Sincerely,

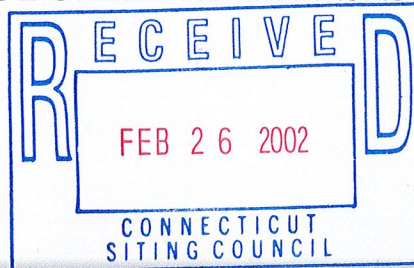
Michele G. Briggs
Manager of Real Estate

Enclosure

Built to a Higher Standard

CINGULAR WIRELESS STRUCTURAL ANALYSIS REPORT

STORRS, CT



02-01020

*YOUR SABRE
REPRESENTATIVE IS
LORA KEITHLEY
1-800-369-6690 EXT. 217*



2101 Murray Street • P.O. Box 658 • Sioux City, IA 51102
Phone 712-258-6690 • 1-800-369-6690 • FAX 712-258-8250

Structural Analysis Report

Job #02-01020

Existing 327' Sabre Communications Corporation
4400SRW Guyed Tower

Located at Storrs, Connecticut

Report Completed for

Cingular Wireless

Westwood, MA

Prepared by

Sabre Communications Corporation

January 16, 2002



Structural Analysis Report
Existing 327' Sabre Communications Corporation
4400SRW Guyed Tower

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INTRODUCTION 3
METHOD OF ANALYSIS 3
SUPPORTED EQUIPMENT 3-5
RESULTS 5
CONCLUSIONS 6
DESCRIPTION OF GUYED TOWER PROGRAM 6-7
CALCULATIONS A1-A19

Prepared by QJD
Checked by JMB
Approved by KJT



1/17/02

Introduction

The purpose of this analysis is to determine if the existing tower is in conformance with the requirements of ANSI/TIA/EIA 222-F, while supporting specified equipment. The tower is a 327' 4400SRW guyed tower and was originally manufactured by Sabre Communications Corporation. The tower is located in Storrs, Connecticut. The analysis is being performed for Cingular Wireless, Westwood, MA.

Method of Analysis

The computer program that was used for this analysis is described on the attached page. The analysis was performed using a basic wind speed of 90 mph concurrent with 1/2" ice, in accordance with ANSI/TIA/EIA 222-F. Allowable stresses, safety factors and load factors were also determined in accordance with this standard.

Supported Equipment

The analysis was performed for the tower, supporting the following equipment:

WHUS

1. One (1) Shively 6813 2-bay antenna on a pole from 290' to 327'
2. One (1) OMB-GP1 antenna from 209' to 211'
3. One (1) Telewave 450F6 antenna from 273' to 281'
4. One (1) Diamond x50 antenna from 273' to 267'
5. One (1) Cushcraft antenna at 92'

Connecticut State Police

6. Two (2) 6' dishes with radomes from 113' to 119'
7. One (1) 6' dish with radome from 101' to 107'
8. One (1) Scala AP14-850-105 antenna from 258' to 267'
9. One (1) DB-810K antenna from 265' to 273'
10. One (1) Scala AP14-850-105 antenna from 258' to 250'
11. One (1) DB-212 antenna from 74' to 94'
12. One (1) Scala OGT9-840 antenna from 258' to 247'

Verizon

- 13. Twelve (12) panel antennas on a 3T-Boom from 82' to 86'
- 14. One (1) PD-220 antenna from 154' to 174'
- 15. Three (3) Metawave panel antennas on a 3T-Boom at 80'
- 16. Twelve (12) 1-1/4" lines and three (3) 1/2" lines

PageNet

- 17. One (1) DB872H120 antenna from 158' to 160'
- 18. One (1) DB586-T3 antenna from 151' to 155'
- 19. One (1) DB589-T3 antenna from 156' to 165'
- 20. One (1) DB589-T3 antenna from 162' to 171'
- 21. One (1) Scala yagi antenna at 105'

Other Tenants

- 22. One (1) Shively 2-bay antenna from 193' to 206'
- 23. One (1) Celwave TDF6319A antenna from 111' to 121'
- 24. One (1) 8' whip antenna from 153' to 161'
- 25. One (1) Celwave PD10108 antenna at 29'
- 26. One (1) Sinclair SRL420NHD antenna from 124' to 133'

SNET(proposed)

- 27. Eight (8) DB846H80 antennas, one (1) EMS RS90-12-00DA-2 antennas and three (3) Allgon 7125.18 antennas on a 3T-Boom at 185', with fifteen (15) 1-1/4" lines

Nextel (proposed)

- 28. Twelve (12) DB844H90(E)-XY antennas on a platform at 225', with fifteen (15) 1-5/8" lines
- 29. Two (2) Radiall/Larsen GPS0015 antennas at 65' with two (2) 1/2" lines

UCONN Inventory Update

- 30. One (1) DB 801 at 180'
- 31. One (1) Cellinc DBD2702 Tower Amplifier at 180'
- 32. One (1) DB872H at 175'
- 33. Scala PR-950 Grid at 50'

Other

- 34. Two (2) Ice Shields at 125'
- 35. Two Equipment Boxes at 258'
- 36. Side Lights and J Box at 155'

Each tower face is assumed to be fully covered with feedlines.

Results

Tower Section Elevation (ft)	Percentage of Allowable Leg Capacity Used (%)	Percentage of Allowable Diagonal Capacity Used (%)
0-20	68.1	76.7
20-40	73.3	44.6
40-60	73.3	57.2
60-80	73.4	50.2
80-100	73.5	72.7
100-120	68.2	59.1
120-140	54.6	46.0
140-160	71.8	58.5
160-180	65.6	60.4
180-200	67.8	87.9
200-220	58.2	40.7
220-240	60.7	51.9
240-260	60.7	39.7
260-280	52.0	26.4
280-288	55.5	47.8
288-323	13.8	

Guy Elevation (ft)	Percentage of Allowable Guy Capacity Used (%)
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The results of the analysis show no overstresses in any tower component.

In addition, the results of the analysis show that the foundations are adequate.

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Based on the preceding results, the following conclusions have been made:

1. The tower with specified equipment is adequate to achieve a basic wind speed rating of 90 mph concurrent with 1/2" ice, in accordance with ANSI/TIA/EIA 222-F.
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A guyed tower computer program called GUYMAST™, developed by Guymast Inc., is utilized by Sabre Communications Corporation to perform the structural analysis and design of guyed towers. This program is one of the most widely used programs in the tower industry.

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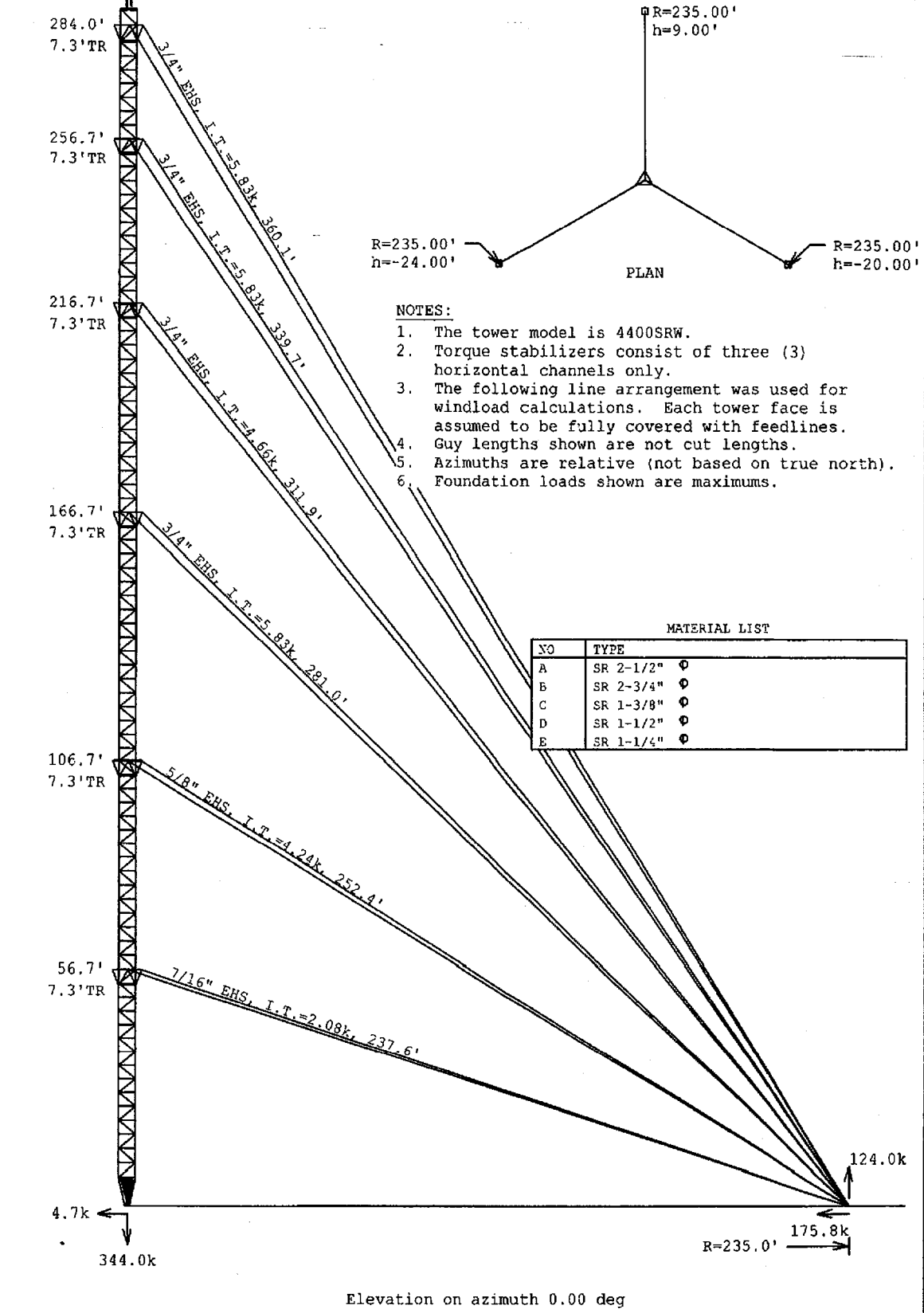
"With an arbitrary set of initial displacements, the guy model is used to obtain the guy stiffnesses and guy loads to be applied in the mast models. These guy stiffnesses and loads are then used in the two mast models to calculate a new set of mast displacements, which are then fed back into the guy model to obtain a better approximation of guy stiffnesses and loads.

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Leg	50 ksi	SR 3" ϕ	SR 2-3/4" ϕ	A	B	SR 2-1/2" ϕ	SR 2-1/4" ϕ	SR 2" ϕ
Diagonal	36 ksi	SR 1-1/4" ϕ	C	D	E	D	SR 1-3/8" ϕ	
Horizontal	36 ksi							
Brace Bolts								
Face Width	3.7'							
Panel Height#panels								


288.0' #2
280.0'
260.0'
220.0'
200.0'
180.0'
160.0'
140.0'
120.0'
100.0'
80.0'
6.7'
0.0'



- NOTES:**
1. The tower model is 4400SRW.
 2. Torque stabilizers consist of three (3) horizontal channels only.
 3. The following line arrangement was used for windload calculations. Each tower face is assumed to be fully covered with feedlines.
 4. Guy lengths shown are not cut lengths.
 5. Azimuths are relative (not based on true north).
 6. Foundation loads shown are maximums.

MATERIAL LIST

NO	TYPE
A	SR 2-1/2" ϕ
B	SR 2-3/4" ϕ
C	SR 1-3/8" ϕ
D	SR 1-1/2" ϕ
E	SR 1-1/4" ϕ

 <p>Sabre Communications Corporation 2101 Murray Street, Sioux City, Iowa 51102 Phone: (712) 258-6690</p>	Client: Cingular Wireless	Job No: 02-01020	Date: 16 jan 2002
	Location: Storrs, CT	Standard: ANSI/TIA/EIA 222-F 1996	Design Wind & Ice: 90 mph + 1/2" ice (Concurrent)
		Fax: (712) 258-8250	

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PageNet:

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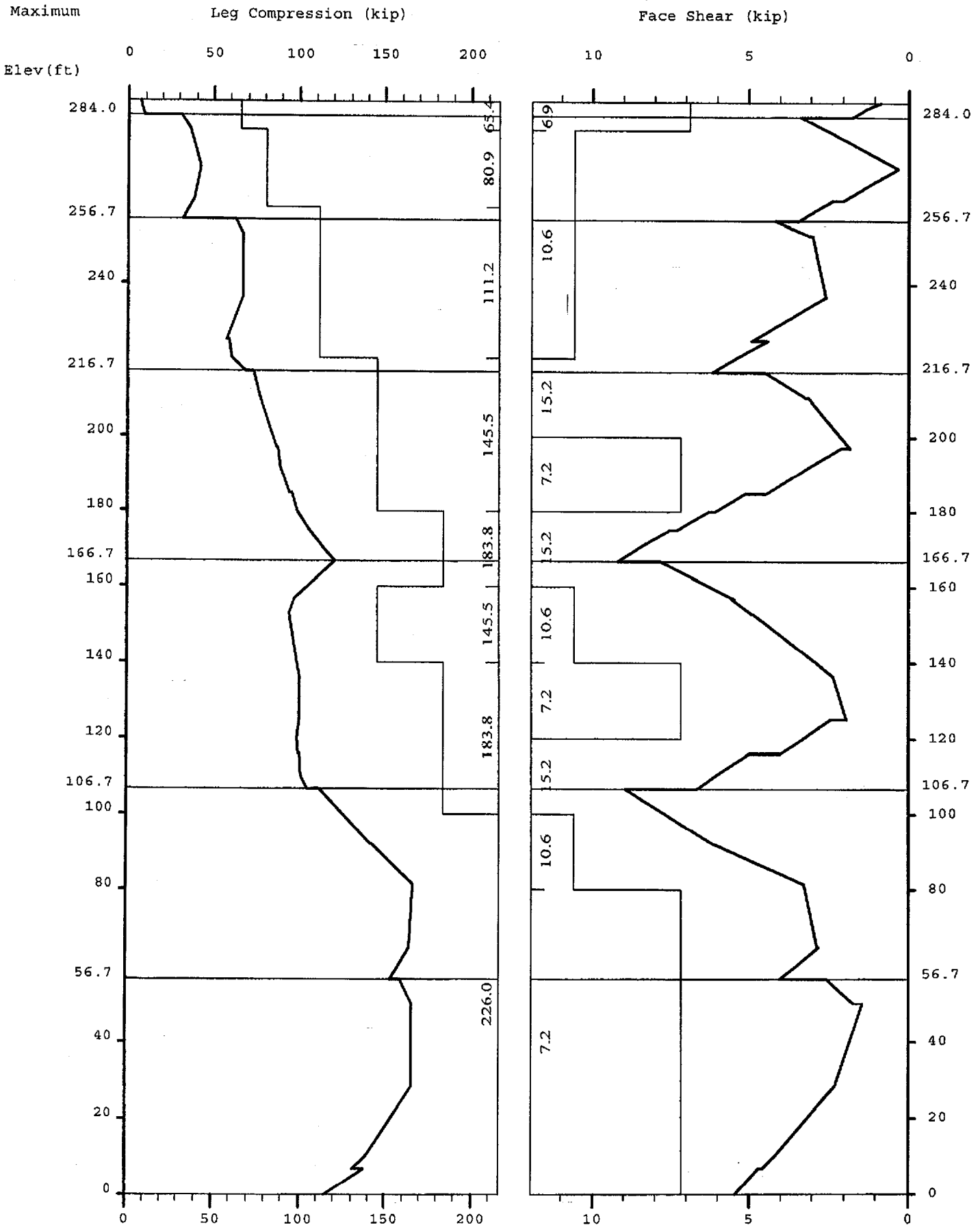
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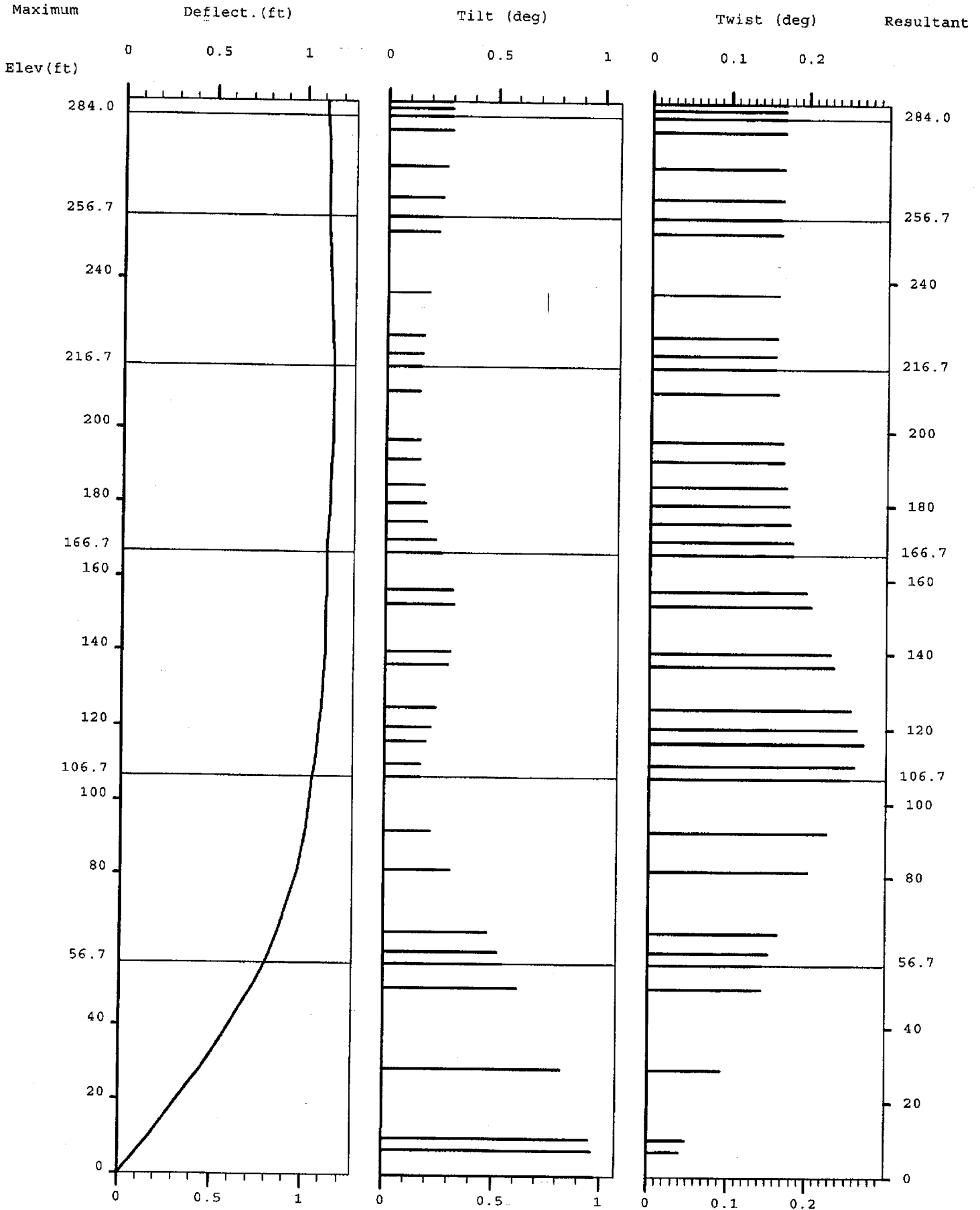
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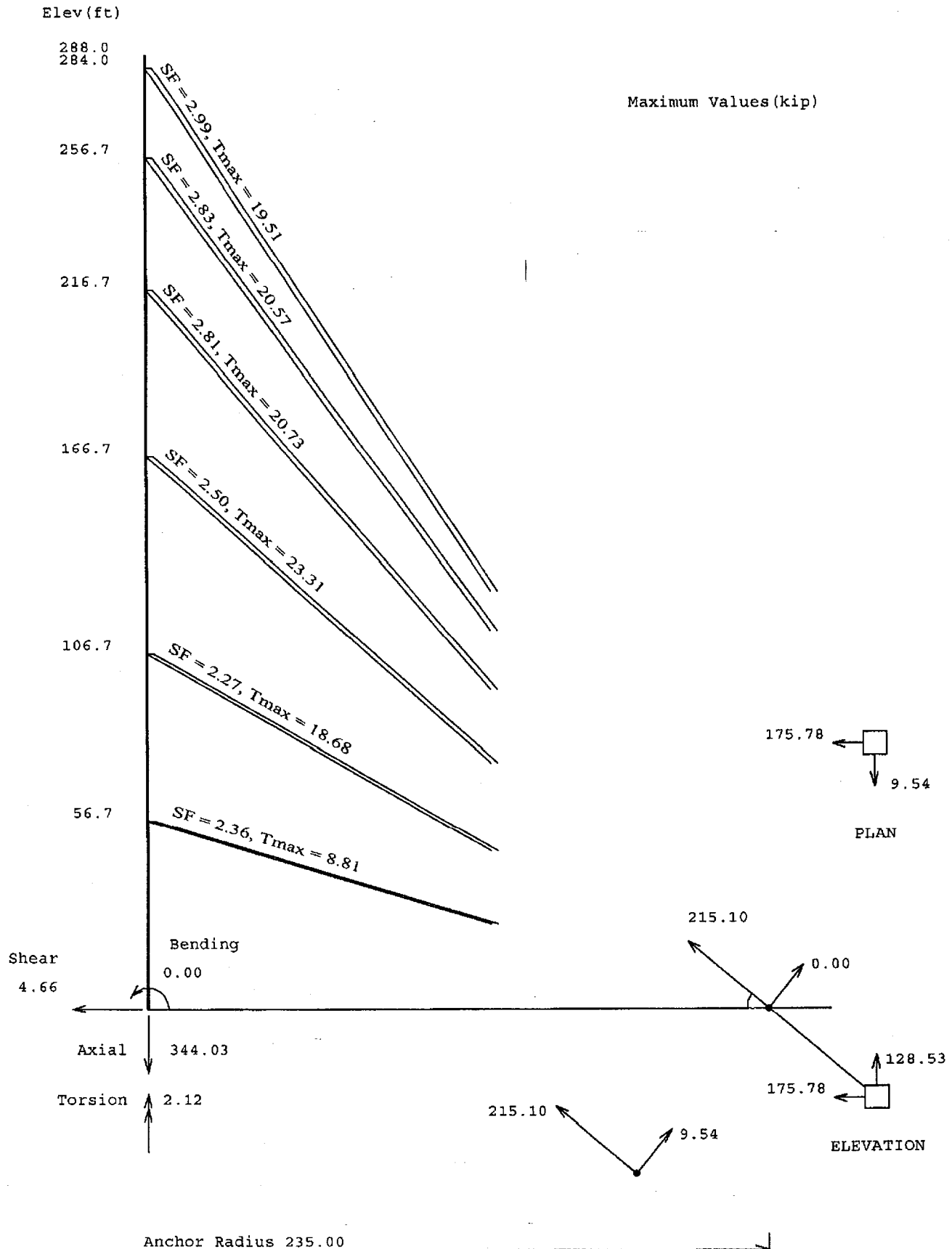
Other:

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35. Two Equipment Boxes at 258'
36. Side Lights and J Box at 155'





Guy Tensions, Anchor Loads and Base Loads



Sabre Communications Corporation

on: 16 jan 2002 at: 13:50:08

327' 4400 CINGULAR WIRELESS STORRS, CT (02-01020) 1-10-02 JDI

MAST DATA

UPPER ELEV FT	MAST TYPE OF WEB	NO OF LEGS *	FACE WIDTH FT *	GEOM PANEL HEIGHT FT *	X-SECTION-AREA		BARE WEIGHT K/FT.	ELASTIC MODULUS KIP/IN.SQ	TEMP COEFF /DEG
					ONE LEG IN.SQ.	ONE DIAG. IN.SQ.			
288.0	4	3	3.667	4.000	3.140	1.480	0.061	29000.0	0.0000116
280.0	4	3	3.667	3.333	5.610	1.440	0.089	29000.0	0.0000116
6.7	4	3	2.688	3.333	7.070	1.230	0.098	29000.0	0.0000116

* If NO OF LEGS is 1 : that part of the mast is assumed to be Cylindrical
 and : FACE WIDTH = outside diameter
 PANEL HEIGHT = thickness
 AREA OF DIAG = Poisson ratio

GUY GEOMETRY

ELEV FT	GUY AZI DEG	DIAMETER IN.	HEIGHT FT.	RADIUS FT.	MAST ATTACH RADIUS FT.	ATTACH AZI DEG	INITIAL TENSION KIP
284.0	0.0	0.750	275.0	235.0	4.230	300.0	5.830
284.0	240.0	0.750	308.0	235.0	4.230	300.0	5.830
284.0	240.0	0.750	308.0	235.0	4.230	180.0	5.830
284.0	120.0	0.750	304.0	235.0	4.230	180.0	5.830
284.0	120.0	0.750	304.0	235.0	4.230	60.0	5.830
284.0	0.0	0.750	275.0	235.0	4.230	60.0	5.830
256.7	0.0	0.750	247.7	235.0	4.230	300.0	5.830
256.7	240.0	0.750	280.7	235.0	4.230	300.0	5.830
256.7	240.0	0.750	280.7	235.0	4.230	180.0	5.830
256.7	120.0	0.750	276.7	235.0	4.230	180.0	5.830
256.7	120.0	0.750	276.7	235.0	4.230	60.0	5.830
256.7	0.0	0.750	247.7	235.0	4.230	60.0	5.830
216.7	0.0	0.750	207.7	235.0	4.230	300.0	4.660
216.7	240.0	0.750	240.7	235.0	4.230	300.0	4.660
216.7	240.0	0.750	240.7	235.0	4.230	180.0	4.660
216.7	120.0	0.750	236.7	235.0	4.230	180.0	4.660
216.7	120.0	0.750	236.7	235.0	4.230	60.0	4.660
216.7	0.0	0.750	207.7	235.0	4.230	60.0	4.660
166.7	0.0	0.750	157.7	235.0	4.230	300.0	5.830
166.7	240.0	0.750	190.7	235.0	4.230	300.0	5.830
166.7	240.0	0.750	190.7	235.0	4.230	180.0	5.830

166.7	120.0	0.750	186.7	235.0	4.230	180.0	5.830
166.7	120.0	0.750	186.7	235.0	4.230	60.0	5.830
166.7	0.0	0.750	157.7	235.0	4.230	60.0	5.830
106.7	0.0	0.625	97.7	235.0	4.230	300.0	4.240
106.7	240.0	0.625	130.7	235.0	4.230	300.0	4.240
106.7	240.0	0.625	130.7	235.0	4.230	180.0	4.240
106.7	120.0	0.625	126.7	235.0	4.230	180.0	4.240
106.7	120.0	0.625	126.7	235.0	4.230	60.0	4.240
106.7	0.0	0.625	97.7	235.0	4.230	60.0	4.240
56.7	0.0	0.438	47.7	235.0	4.230	300.0	2.080
56.7	240.0	0.438	80.7	235.0	4.230	300.0	2.080
56.7	240.0	0.438	80.7	235.0	4.230	180.0	2.080
56.7	120.0	0.438	76.7	235.0	4.230	180.0	2.080
56.7	120.0	0.438	76.7	235.0	4.230	60.0	2.080
56.7	0.0	0.438	47.7	235.0	4.230	60.0	2.080

GUY MATERIAL PROPERTIES
=====

ELEV FT	GUY AZI DEG	BREAKING STRENGTH KIP	GUY WEIGHT LBS/FT	GUY AREA IN. SQ	ELASTIC MODULUS KIP/IN. SQ	THERMAL COEFF /DEG	UNSTRESS LENGTH FT
284.0	0.0	58.300	1.180	0.338	19000.0	0.0000120	360.068
284.0	240.0	58.300	1.180	0.338	19000.0	0.0000120	385.813
284.0	240.0	58.300	1.180	0.338	19000.0	0.0000120	385.813
284.0	120.0	58.300	1.180	0.338	19000.0	0.0000120	382.632
284.0	120.0	58.300	1.180	0.338	19000.0	0.0000120	382.632
284.0	0.0	58.300	1.180	0.338	19000.0	0.0000120	360.068
256.7	0.0	58.300	1.180	0.338	19000.0	0.0000120	339.690
256.7	240.0	58.300	1.180	0.338	19000.0	0.0000120	364.408
256.7	240.0	58.300	1.180	0.338	19000.0	0.0000120	364.408
256.7	120.0	58.300	1.180	0.338	19000.0	0.0000120	361.341
256.7	120.0	58.300	1.180	0.338	19000.0	0.0000120	361.341
256.7	0.0	58.300	1.180	0.338	19000.0	0.0000120	339.690
216.7	0.0	58.300	1.180	0.338	19000.0	0.0000120	311.856
216.7	240.0	58.300	1.180	0.338	19000.0	0.0000120	334.707
216.7	240.0	58.300	1.180	0.338	19000.0	0.0000120	334.707
216.7	120.0	58.300	1.180	0.338	19000.0	0.0000120	331.846
216.7	120.0	58.300	1.180	0.338	19000.0	0.0000120	331.846
216.7	0.0	58.300	1.180	0.338	19000.0	0.0000120	311.856
166.7	0.0	58.300	1.180	0.338	19000.0	0.0000120	281.025
166.7	240.0	58.300	1.180	0.338	19000.0	0.0000120	300.749
166.7	240.0	58.300	1.180	0.338	19000.0	0.0000120	300.749
166.7	120.0	58.300	1.180	0.338	19000.0	0.0000120	298.232
166.7	120.0	58.300	1.180	0.338	19000.0	0.0000120	298.232
166.7	0.0	58.300	1.180	0.338	19000.0	0.0000120	281.025
106.7	0.0	42.400	0.819	0.234	20000.0	0.0000120	252.351
106.7	240.0	42.400	0.819	0.234	20000.0	0.0000120	266.838
106.7	240.0	42.400	0.819	0.234	20000.0	0.0000120	266.838
106.7	120.0	42.400	0.819	0.234	20000.0	0.0000120	264.907
106.7	120.0	42.400	0.819	0.234	20000.0	0.0000120	264.907
106.7	0.0	42.400	0.819	0.234	20000.0	0.0000120	252.351
56.7	0.0	20.800	0.388	0.115	21000.0	0.0000120	237.554
56.7	240.0	20.800	0.388	0.115	21000.0	0.0000120	246.292
56.7	240.0	20.800	0.388	0.115	21000.0	0.0000120	246.292
56.7	120.0	20.800	0.388	0.115	21000.0	0.0000120	245.014
56.7	120.0	20.800	0.388	0.115	21000.0	0.0000120	245.014
56.7	0.0	20.800	0.388	0.115	21000.0	0.0000120	237.554

FACTORED LEG AND FACE SHEAR RESISTANCE
=====

BOTTOM ELEV ft	TOP ELEV ft	LEG COMP kip	FACE SHEAR kip
0.00	20.00	226.03	7.17
20.00	40.00	226.03	7.17
40.00	60.00	226.03	7.17
60.00	80.00	226.03	7.17
80.00	100.00	226.03	10.59
100.00	120.00	183.79	15.24
120.00	140.00	183.79	7.17
140.00	160.00	145.52	10.59
160.00	180.00	183.79	15.24
180.00	200.00	145.52	7.17
200.00	220.00	145.52	15.24
220.00	240.00	111.22	10.59
240.00	260.00	111.22	10.59
260.00	280.00	80.89	10.59
280.00	288.00	65.42	6.90

=====
LOADING CONDITION A

90 MPH + 1/2" ICE WIND AZ 0 DEGREES

*12 wind directions were analyzed.
Only one condition is shown
in full.

MAST LOADING
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LOAD TYPE	ELEV FT	.FORCES (KIP & KIP/FT)			.MOMENTS (FT.K & FT.K/FT)			ANT-ORIENT	
		N	E	DOWN	N	E	TORSION	AZI DEG	VERT DEG
C	309.0	-0.530	0.000	0.380	0.00	0.00	0.00	0.0	0.00
C	306.0	-0.890	0.000	3.200	0.00	0.00	0.00	0.0	0.00
C	284.0	-0.790	0.000	1.110	0.00	0.00	0.00	0.0	0.00
C	277.0	-0.160	0.000	0.020	0.00	0.00	0.00	0.0	0.00
C	270.0	-0.120	0.000	0.020	0.00	0.00	0.00	0.0	0.00
C	265.0	-0.290	0.000	0.070	0.00	0.00	0.00	0.0	0.00
C	262.0	-0.490	0.000	0.060	0.00	0.00	0.00	0.0	0.00
C	258.0	-0.200	0.000	0.040	0.00	0.00	0.00	0.0	0.00
C	256.7	-0.770	0.000	1.110	0.00	0.00	0.00	0.0	0.00
C	254.0	-0.480	0.000	0.060	0.00	0.00	0.00	0.0	0.00
C	253.0	-0.160	0.000	0.040	0.00	0.00	0.00	0.0	0.00
C	225.0	-2.370	0.000	3.840	0.00	0.00	0.00	0.0	0.00
C	216.7	-0.730	0.000	1.110	0.00	0.00	0.00	0.0	0.00
C	210.0	-0.080	0.000	0.020	0.00	0.00	0.00	0.0	0.00
C	197.0	-0.470	0.000	0.380	0.00	0.00	0.00	0.0	0.00
C	185.0	-4.370	0.000	3.720	0.00	0.00	0.00	0.0	0.00
C	180.0	-0.150	0.000	0.050	0.00	0.00	0.00	0.0	0.00
C	180.0	-0.180	0.000	0.050	0.00	0.00	0.00	0.0	0.00
C	175.0	-0.470	0.000	0.040	0.00	0.00	0.00	0.0	0.00
C	166.7	-0.680	0.000	1.110	0.00	0.00	0.00	0.0	0.00
C	166.0	-0.110	0.000	0.030	0.00	0.00	0.00	0.0	0.00
C	164.0	-0.210	0.000	0.050	0.00	0.00	0.00	0.0	0.00
C	160.0	-0.110	0.000	0.030	0.00	0.00	0.00	0.0	0.00
C	159.0	-0.490	0.000	0.040	0.00	0.00	0.00	0.0	0.00
C	157.0	-0.140	0.000	0.050	0.00	0.00	0.00	0.0	0.00
C	155.0	-0.070	0.000	0.050	0.00	0.00	0.00	0.0	0.00

C	153.0	-0.070	0.000	0.020	0.00	0.00	0.00	0.0	0.00
C	128.0	-0.130	0.000	0.040	0.00	0.00	0.00	0.0	0.00
C	125.0	-1.050	0.000	1.000	0.00	0.00	0.00	0.0	0.00
C	116.0	-0.720	0.000	0.358	-0.65	-1.12	-1.69	180.0	0.00
C	116.0	-0.720	0.000	0.358	1.30	0.00	-1.69	0.0	0.00
C	116.0	-0.160	0.000	0.050	0.00	0.00	0.00	0.0	0.00
C	106.7	-0.600	0.000	1.110	0.00	0.00	0.00	0.0	0.00
C	105.0	-0.160	0.000	0.020	0.00	0.00	0.00	0.0	0.00
C	104.0	-0.815	0.000	0.358	1.30	0.00	-2.32	0.0	0.00
C	92.0	-0.030	0.000	0.010	0.00	0.00	0.00	0.0	0.00
C	84.0	-0.180	0.000	0.040	0.00	0.00	0.00	0.0	0.00
C	84.0	-1.700	0.000	3.480	0.00	0.00	0.00	0.0	0.00
C	80.0	-2.600	0.000	3.450	0.00	0.00	0.00	0.0	0.00
C	65.0	-0.050	0.000	0.020	0.00	0.00	0.00	0.0	0.00
C	56.7	-0.500	0.000	1.110	0.00	0.00	0.00	0.0	0.00
C	50.0	-0.630	0.000	0.080	0.00	0.00	0.00	0.0	0.00
C	29.0	-0.040	0.000	0.010	0.00	0.00	0.00	0.0	0.00
D	288.0	-0.325	0.000	0.082	0.00	0.00	0.00		
D	284.0	-0.325	0.000	0.082	0.00	0.00	0.00		
D	284.0	-0.320	0.000	0.091	0.00	0.00	0.00		
D	280.0	-0.320	0.000	0.091	0.00	0.00	0.00		
D	280.0	-0.322	0.000	0.096	0.00	0.00	0.00		
D	260.0	-0.321	0.000	0.096	0.00	0.00	0.00		
D	260.0	-0.322	0.000	0.109	0.00	0.00	0.00		
D	256.7	-0.322	0.000	0.109	0.00	0.00	0.00		
D	256.7	-0.313	0.000	0.146	0.00	0.00	0.00		
D	220.0	-0.310	0.000	0.146	0.00	0.00	0.00		
D	220.0	-0.311	0.000	0.168	0.00	0.00	0.00		
D	216.7	-0.311	0.000	0.168	0.00	0.00	0.00		
D	216.7	-0.298	0.000	0.235	0.00	0.00	0.00		
D	200.0	-0.297	0.000	0.235	0.00	0.00	0.00		
D	200.0	-0.296	0.000	0.226	0.00	0.00	0.00		
D	180.0	-0.294	0.000	0.226	0.00	0.00	0.00		
D	180.0	-0.297	0.000	0.246	0.00	0.00	0.00		
D	170.0	-0.296	0.000	0.246	0.00	0.00	0.00		
D	170.0	-0.294	0.000	0.249	0.00	0.00	0.00		
D	166.7	-0.294	0.000	0.249	0.00	0.00	0.00		
D	166.7	-0.273	0.000	0.274	0.00	0.00	0.00		
D	160.0	-0.273	0.000	0.274	0.00	0.00	0.00		
D	160.0	-0.271	0.000	0.258	0.00	0.00	0.00		
D	140.0	-0.269	0.000	0.258	0.00	0.00	0.00		
D	140.0	-0.270	0.000	0.265	0.00	0.00	0.00		
D	120.0	-0.268	0.000	0.265	0.00	0.00	0.00		
D	120.0	-0.268	0.000	0.274	0.00	0.00	0.00		
D	110.0	-0.268	0.000	0.274	0.00	0.00	0.00		
D	110.0	-0.264	0.000	0.276	0.00	0.00	0.00		
D	106.7	-0.264	0.000	0.276	0.00	0.00	0.00		
D	106.7	-0.237	0.000	0.300	0.00	0.00	0.00		
D	60.0	-0.231	0.000	0.303	0.00	0.00	0.00		
D	60.0	-0.226	0.000	0.300	0.00	0.00	0.00		
D	56.7	-0.226	0.000	0.300	0.00	0.00	0.00		
D	56.7	-0.185	0.000	0.305	0.00	0.00	0.00		
D	10.0	-0.179	0.000	0.305	0.00	0.00	0.00		
D	10.0	-0.181	0.000	0.306	0.00	0.00	0.00		
D	0.0	-0.175	0.000	0.299	0.00	0.00	0.00		

GUY LOADING

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.. WIND LOADING ..	TEMP	.ICE LOAD..	CONV	PROFILES.	.LOAD FACTORS.
AZI SPEED REF	CHANGE	RAD DENS	TOL	CAB WIND	WIND DEAD ICE
PRESS					

DEG MPH PSF DEG IN PCF
 0.0 90.0 0.00 -10.00 0.50 56.00 0.0100 2 1 1.00 1.00 1.00

CABLE PROFILE: 1 - CATENARY 2 - PARABOLIC
 WIND PROFILE: 1 - EIA 222 F 2 - Kz = 1 ; Gh = 1
 3 - EIA 222 C 4 - Special Factors
 5 - Site Specific Wind Formula

MAXIMUM LEG LOADS AND FACE SHEARS (KIP - stress in KSI)

MAST ELEV FT	MAX LEG LOADS					MAX FACE SHEARS		
	AXIAL	BENDING		TOTAL		TORSN	BEAM	TOTAL
		TENS	COMP	TENS	COMP			
288.00	1.2A	8.5A	6.0C	7.4A	7.2C	0.0A	-0.9B	0.9B
	1.2A	9.6A	6.8C	8.4A	8.1C	0.0A	-1.3B	1.3B
286.00	1.2A	9.6A	6.8C	8.4A	8.1C	0.0A	-1.3B	1.3B
	1.3A	11.2A	8.1C	9.9A	9.4C	0.0A	-1.7B	1.7B
284.00	22.5A	11.1G	8.9I	0.0A	30.9I	-0.1L	-3.3D	3.3L
	22.6A	16.1G	14.1I	0.0A	36.3I	-0.1L	-2.4D	2.5L
280.00	22.6A	16.1G	14.1I	0.0A	36.3I	-0.1L	-2.4D	2.5L
	23.0A	20.9G	19.6I	0.1G	42.1I	-0.1L	-0.3D	0.3K
270.34	23.0A	20.9G	19.6I	0.1G	42.1I	-0.1L	-0.3D	0.3K
	23.3A	16.6G	15.9I	0.0A	38.7I	-0.1L	2.0H	2.0F
262.00	23.3A	16.6G	15.9I	0.0A	38.7I	-0.1L	2.3H	2.3F
	23.5A	9.6F	8.7I	0.0A	31.8I	-0.1L	3.5L	3.4B
256.67	43.4A	19.7G	20.5I	0.0A	63.0I	-0.3L	3.9B	4.2L
	43.6A	22.0G	24.3I	0.0A	67.0I	-0.3L	2.8B	3.1L
253.00	43.7A	22.0G	24.3I	0.0A	67.0I	-0.3L	2.7B	3.0L
	44.5A	28.0C	23.1A	0.0A	67.5A	-0.3L	2.5H	2.6G
236.67	44.5A	28.0C	23.1A	0.0A	67.5A	-0.3L	2.5H	2.6G
	45.0A	16.8J	12.7B	0.0A	57.3A	-0.3L	4.9H	4.9F
225.00	46.3A	16.8J	12.7B	0.0A	58.6A	-0.3L	4.5L	4.4B
	46.5A	9.3G	18.6G	0.0A	60.2G	-0.3L	5.5L	5.5B
220.01								

	46.5A	9.3G	18.6G	0.0A	60.2G	-0.3L	5.5L	5.5B
216.67	46.7A	14.2E	26.5G	0.0A	68.3G	-0.3L	6.2L	6.2B
	63.8A	10.6J	16.1G	0.0A	73.2E	-0.4L	-4.1L	4.5L
210.00	64.3A	19.7J	15.0J	0.0A	77.1A	-0.4L	-2.7L	3.2L
	64.3A	19.7J	15.0J	0.0A	77.1A	-0.4L	-2.7L	3.1L
197.00	65.3A	26.4J	22.6C	0.0A	87.0A	-0.4L	1.7I	1.8E
	65.4A	26.4J	22.6C	0.0A	87.2A	-0.4L	2.0I	2.1F
191.67	65.8A	24.3J	25.4J	0.0A	88.9D	-0.4L	3.0H	3.2F
	65.8A	24.3J	25.4J	0.0A	88.9D	-0.4L	3.0H	3.2F
185.00	66.3A	18.3J	34.1G	0.0A	94.3E	-0.4L	4.3H	4.5F
	67.6A	18.3J	34.1G	0.0A	95.5E	-0.4L	-5.1B	5.1B
180.00	67.9A	22.7E	38.7G	0.0A	98.7G	-0.4L	-6.1B	6.1B
	68.0A	22.7E	38.7G	0.0A	98.7G	-0.4L	-6.3B	6.3B
175.00	68.4A	32.2E	45.1G	0.0A	105.4G	-0.4L	-7.2B	7.3B
	68.4A	32.2E	45.1G	0.0A	105.4G	-0.4L	-7.5B	7.6B
170.01	68.8A	43.4E	52.8G	0.0A	113.5G	-0.4L	-8.5B	8.5B
	68.8A	43.4E	52.8G	0.0A	113.5G	-0.4L	-8.5B	8.5B
166.67	69.1A	52.5A	59.1G	0.0A	120.2G	-0.4L	-9.2B	9.2B
	84.0A	40.5A	47.4G	0.0A	120.6G	-0.7L	7.2F	7.8H
157.00	85.0A	14.1A	15.6G	0.0A	97.4I	-0.7L	4.9G	5.5H
	85.0A	14.1A	15.6G	0.0A	97.4I	-0.7L	5.0G	5.5H
153.00	85.3A	11.1J	11.1J	0.0A	94.3J	-0.7L	4.4G	4.9G
	85.3A	11.1J	11.1J	0.0A	94.3J	-0.7L	4.4G	4.9G
140.01	86.5A	27.0K	15.0K	0.0A	98.4J	-0.7L	2.3G	2.9G
	86.5A	27.0K	15.0K	0.0A	98.4J	-0.7L	2.3G	2.9G
136.67	86.8A	28.9K	16.1K	0.0A	100.3J	-0.7L	1.8G	2.3G
	86.8A	28.9K	16.1K	0.0A	100.3J	-0.7L	1.8G	2.3G
125.00	87.8A	29.3G	15.7K	0.0A	100.0J	-0.7L	1.4L	2.0J
	88.1A	29.3G	15.7K	0.0A	100.3J	-0.7L	2.0L	2.4A
120.02	88.6A	29.8G	15.0G	0.0A	98.6B	-0.7L	2.9L	3.3B
	88.6A	29.8G	15.0G	0.0A	98.6B	-0.7L	2.9L	3.3B
	88.9A	28.7G	14.5G	0.0A	99.6B	-0.7L	3.6L	4.0B

116.00	89.2A	28.5G	14.4G	0.0A	100.0L	0.6B	4.7L	5.0L
	89.7A	21.0G	13.6L	0.0A	101.4L	0.6B	5.7L	6.1L
110.02	89.7A	21.0G	13.6L	0.0A	101.4L	0.6B	5.7L	6.1L
	90.0A	28.8A	14.5H	0.0A	104.5A	0.6B	6.3L	6.7L
106.67	98.4A	23.0G	15.0L	0.0A	111.6L	0.5F	8.4B	8.9B
	100.0A	67.9G	42.9I	0.0A	141.9I	0.7F	5.5B	6.1B
92.00	100.0A	67.9G	42.9I	0.0A	141.9I	0.7F	5.4B	6.1B
	102.2A	86.2G	64.8I	0.0A	166.1I	0.7F	2.7A	3.2A
81.67	102.2A	86.2G	64.8I	0.0A	166.1I	0.7F	2.7A	3.2A
	105.0A	74.6G	59.8I	0.0A	163.9I	0.7F	2.3H	2.9G
65.00	105.0A	74.6G	59.8I	0.0A	163.9I	0.7F	2.3H	2.8G
	105.5A	65.8G	53.1I	0.0A	157.6I	0.7F	3.0H	3.6H
60.02	105.5A	65.8G	53.1I	0.0A	157.6I	0.7F	3.0H	3.6H
	105.9A	58.6G	47.3I	0.0A	152.9A	0.7F	3.5H	4.1H
56.67	108.9A	61.2G	50.2I	0.0A	158.6I	0.7F	2.1B	2.6B
	109.6A	65.1G	55.9I	0.0A	165.5A	0.7F	1.2B	1.7B
50.00	109.6A	65.1G	55.9I	0.0A	165.5A	0.7F	-0.9D	1.4F
	111.8A	58.8G	53.9A	0.0A	165.7A	0.7F	1.9G	2.3G
28.33	111.8A	58.8G	53.9A	0.0A	165.7A	0.7F	1.9G	2.3G
	113.7A	26.8G	25.1A	0.0A	138.8A	0.7F	4.1H	4.2G
10.03	113.7A	26.8G	25.1A	0.0A	138.8A	0.7F	4.1H	4.2G
	114.0A	18.5G	17.4A	0.0A	131.4A	0.7F	4.5H	4.6L
6.67	114.0A	25.3G	23.7A	0.0A	137.8A	0.9F	4.5H	4.7G
	114.7A	0.0H	0.0H	0.0A	114.7A	0.9F	5.3H	5.5L
0.00								

MAXIMUM ANTENNA ROTATIONS

ELEV FT	ORIENTATION	 BEAM DEFLECTIONS (DEG)			TOTAL
	AZI DEG	ELEV DEG	ROLL	YAW	PITCH	
309.0	0.0	0.0	0.254 K	0.169 B	0.168 G	0.193 F
306.0	0.0	0.0	0.254 K	0.169 B	0.168 G	0.193 F

284.C	0.0	0.0	0.258 K	0.169 B	0.167 G	0.191 F
277.C	0.0	0.0	0.245 K	0.168 B	-0.174 J	0.185 B
270.0	0.0	0.0	0.229 K	0.167 B	-0.180 J	0.186 J
265.0	0.0	0.0	0.218 K	0.166 B	-0.184 J	0.190 J
262.0	0.0	0.0	0.210 K	0.166 B	-0.186 J	0.192 J
258.0	0.0	0.0	0.203 K	0.165 B	-0.190 J	0.196 J
256.7	0.0	0.0	0.200 K	0.165 B	-0.191 J	0.197 J
254.0	0.0	0.0	0.192 K	0.164 B	-0.192 J	0.199 J
253.0	0.0	0.0	0.189 K	0.164 B	-0.193 J	0.199 J
225.0	0.0	0.0	-0.109 H	0.159 B	-0.169 J	0.181 J
216.7	0.0	0.0	-0.110 H	0.158 F	-0.154 J	0.169 J
210.0	0.0	0.0	-0.113 H	0.161 F	-0.140 J	0.165 B
197.0	0.0	0.0	-0.129 I	0.167 F	0.136 A	0.183 B
185.0	0.0	0.0	-0.142 I	0.173 F	0.173 A	0.207 B
180.0	0.0	0.0	-0.138 I	0.175 F	0.179 A	0.213 B
180.0	0.0	0.0	-0.138 I	0.175 F	0.179 A	0.213 B
175.0	0.0	0.0	-0.127 I	0.177 F	0.186 G	0.211 B
166.7	0.0	0.0	-0.088 H	0.181 F	0.258 G	0.277 G
166.0	0.0	0.0	-0.087 H	0.182 F	0.261 G	0.280 G
164.0	0.0	0.0	-0.082 H	0.186 F	0.272 G	0.292 G
160.0	0.0	0.0	-0.073 H	0.193 F	0.293 G	0.314 G
159.0	0.0	0.0	-0.070 H	0.195 F	0.298 G	0.320 G
157.0	0.0	0.0	0.068 K	0.199 F	0.309 G	0.331 G
155.0	0.0	0.0	0.066 K	0.202 F	0.312 G	0.335 G
153.0	0.0	0.0	0.064 K	0.206 F	0.316 G	0.340 G
128.0	0.0	0.0	-0.095 I	0.250 F	0.246 G	0.302 G
125.0	0.0	0.0	-0.101 I	0.256 F	0.233 G	0.294 G
116.0	180.0	0.0	0.123 J	0.272 F	-0.188 G	0.291 F
116.0	0.0	0.0	-0.123 J	0.272 F	0.188 G	0.291 F
116.0	0.0	0.0	-0.123 J	0.272 F	0.188 G	0.291 F
106.7	0.0	0.0	-0.124 J	0.253 F	0.155 G	0.269 F
105.0	0.0	0.0	-0.131 J	0.250 F	0.142 G	0.263 F

104.0	0.0	0.0	-0.135 J	0.248 F	0.134 G	0.260 F
92.0	0.0	0.0	-0.183 J	0.226 F	0.108 D	0.226 F
84.0	0.0	0.0	-0.260 J	0.207 F	0.157 A	0.231 B
84.0	0.0	0.0	-0.260 J	0.207 F	0.157 A	0.231 B
80.0	0.0	0.0	-0.301 J	0.198 F	0.191 A	0.246 B
65.0	0.0	0.0	-0.464 J	0.164 F	0.339 A	0.352 A
56.7	0.0	0.0	-0.535 J	0.145 F	-0.416 G	0.425 G
50.0	0.0	0.0	-0.601 J	0.145 F	-0.497 G	0.504 G
29.0	0.0	0.0	-0.798 J	0.097 F	-0.723 G	0.725 G

MAXIMUM INTERNAL MAST FORCES

MAST ELEV FT	TOTAL AXIAL KIPSHEAR.....	 MOMENT.....		TORSION FT-KIP
		N - S KIP	E - W KIP	N - S FT-KIP	E - W FT-KIP	
288.0	3.58 A	-1.42 A	-0.53 D	27.15 A	11.13 D	0.00 A
	3.74 A	-2.07 A	-1.18 D	30.64 A	12.84 D	0.00 A
286.0	3.74 A	-2.07 A	-1.18 D	30.64 A	12.84 D	0.00 A
	3.91 A	-2.72 A	-1.83 D	35.43 A	15.85 D	0.00 A
284.0	* 62.57 A	+ 8.20 A	+ 6.57 D	& 31.62 G	& 34.27 J	@ 0.45 L
	67.58 A	4.64 A	4.94 D	35.35 G	18.41 J	0.44 L
	67.95 A	3.36 A	3.66 D	51.02 G	35.70 J	0.44 L
280.0	67.95 A	3.36 A	3.66 D	51.02 G	35.70 J	0.44 L
	68.90 A	0.21 J	0.39 D	66.32 G	54.97 J	0.44 L
270.3	68.90 A	0.21 J	0.39 D	66.32 G	54.97 J	0.44 L
	69.79 A	3.13 G	2.72 J	52.85 G	44.95 J	0.44 L
262.0	69.85 A	3.62 G	3.21 J	52.85 G	44.95 J	0.44 L
	70.44 A	-5.40 A	4.92 J	30.53 F	23.11 J	0.44 L
256.7	* 58.79 A	+ 12.21 A	+ 9.81 D	& -35.82 A	& 38.17 J	@ 0.45 L
	130.35 A	5.82 A	4.83 D	62.42 G	61.27 J	0.88 L
	130.94 A	4.20 A	3.20 D	70.01 G	76.91 J	0.88 L

253.0	130.98 A	4.04 A	3.04 D	70.01 G	76.91 J	0.88 L
	133.37 A	4.21 G	2.09 J	-73.29 A	85.57 J	0.88 L
236.7	133.37 A	4.21 G	2.09 J	-73.29 A	85.57 J	0.88 L
	135.07 A	7.84 G	5.72 J	-40.44 B	39.25 J	0.88 L
225.0	138.91 A	-7.06 A	5.72 J	-40.44 B	39.25 J	0.88 L
	139.64 A	-8.61 A	7.27 J	-58.92 G	-8.68 B	0.88 L
220.0	139.64 A	-8.61 A	7.27 J	-58.92 G	-8.68 B	0.88 L
	140.20 A	-9.65 A	8.31 J	-84.14 G	23.14 E	0.88 L
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216.7	49.98 A	16.91 A	-13.32 J	-36.99 A	38.97 J	0.63 I
	191.29 A	6.09 A	-4.77 J	-51.05 G	18.81 J	1.36 L
	192.85 A	4.10 A	-2.78 J	-47.60 J	44.88 J	1.36 L
210.0	192.87 A	4.02 A	-2.70 J	-47.60 J	44.88 J	1.36 L
	195.90 A	2.66 G	1.24 I	-71.70 C	55.55 J	1.36 L
197.0	196.28 A	3.13 G	1.65 I	-71.70 C	55.55 J	1.36 L
	197.48 A	4.70 G	-3.21 D	-80.80 J	42.47 J	1.36 L
191.7	197.48 A	4.70 G	-3.21 D	-80.80 J	42.47 J	1.36 L
	198.99 A	6.67 G	-5.17 D	-108.24 G	21.46 L	1.36 L
185.0	202.71 A	-8.22 A	-5.17 D	-108.24 G	21.46 L	1.36 L
	203.83 A	-9.69 A	-6.65 D	-123.04 G	28.93 E	1.36 L
180.0	203.93 A	-10.02 A	-6.65 D	-123.04 G	28.93 E	1.36 L
	205.16 A	-11.51 A	-8.13 D	-143.10 G	62.18 E	1.36 L
175.0	205.20 A	-11.98 A	-8.13 D	-143.10 G	62.18 E	1.36 L
	206.43 A	-13.46 A	-9.61 D	-167.59 G	101.50 E	1.36 L
170.0	206.43 A	-13.46 A	-9.61 D	-167.59 G	101.50 E	1.36 L
	207.26 A	-14.44 A	-10.59 D	-187.72 G	132.43 D	1.36 L
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166.7	44.61 J	25.48 A	20.86 D	-38.28 A	41.93 J	1.31 E
	252.12 A	-12.04 G	10.23 D	-150.55 G	95.48 E	2.28 L
	254.87 A	-8.48 G	6.68 D	-49.54 G	25.74 F	2.28 L
157.0	254.92 A	-8.63 G	6.68 D	-49.54 G	25.74 F	2.28 L
	256.01 A	-7.61 G	5.60 D	-35.26 J	31.55 L	2.28 L
153.0						

	256.03 A	-7.54 G	5.53 D	-35.26 J	31.56 L	2.28 L
140.0	259.38 A	-4.04 G	2.03 D	61.71 G	77.00 K	2.28 L
	259.38 A	-4.04 G	2.02 D	61.71 G	77.00 K	2.28 L
136.7	260.26 A	-3.14 G	-1.19 K	74.15 G	82.52 K	2.28 L
	260.26 A	-3.14 G	-1.19 K	74.14 G	82.52 K	2.28 L
125.0	263.39 A	-2.19 A	2.04 J	92.94 G	79.74 K	2.28 L
	264.39 A	-3.24 A	2.04 J	92.94 G	79.74 K	2.28 L
120.0	265.71 A	-4.57 A	3.37 J	94.75 G	68.16 K	2.28 L
	265.71 A	-4.57 A	3.37 J	94.76 G	68.16 K	2.28 L
116.0	266.81 A	-5.65 A	4.45 J	91.01 G	54.33 K	2.28 L
	267.57 A	-7.25 A	6.05 J	90.36 G	55.45 K	-2.05 B
110.0	269.21 A	-8.85 A	7.65 J	66.68 G	30.92 F	-2.05 B
	269.21 A	-8.85 A	7.65 J	66.68 G	30.92 F	-2.05 B
	270.14 A	-9.74 A	8.54 J	91.32 A	50.82 E	-2.05 B
106.7	* 26.37 J	+ 24.44 A	+ 21.70 D	& 23.88 G	& 28.50 J	@ 2.52 D
	295.19 A	13.43 A	11.98 D	72.89 G	31.36 F	-1.68 F
92.0	299.97 A	8.99 A	7.54 D	215.71 G	147.95 J	-2.10 F
	299.98 A	8.96 A	7.51 D	215.71 G	147.95 J	-2.10 F
81.7	306.61 A	4.65 A	3.34 E	273.86 G	211.98 J	-2.10 F
	306.61 A	4.65 A	3.34 E	273.86 G	211.98 J	-2.10 F
65.0	315.09 A	3.91 G	3.31 J	236.82 G	186.67 J	-2.10 F
	315.11 A	3.86 G	3.31 J	236.82 G	186.67 J	-2.10 F
60.0	316.61 A	5.01 G	4.46 J	208.82 G	162.36 J	-2.10 F
	316.61 A	5.01 G	4.46 J	208.81 G	162.36 J	-2.10 F
	317.62 A	5.77 G	5.21 J	186.01 G	142.16 J	-2.10 F
56.7	* 9.47 G	+ 10.19 A	+ -10.65 J	& 8.19 G	& 9.19 J	@ 1.26 H
	326.70 A	3.05 A	2.54 D	194.20 G	151.35 J	-2.12 F
50.0	328.73 A	1.82 A	1.31 D	206.63 G	172.20 J	-2.12 F
	328.81 A	1.19 A	1.31 D	206.63 G	172.20 J	-2.12 F
28.3	335.42 A	3.35 G	2.69 J	186.59 G	167.15 J	-2.12 F
	335.42 A	3.35 G	2.69 J	186.59 G	167.15 J	-2.12 F

10.0	341.00 A	6.65 G	5.99 J	85.20 G	78.32 J	-2.12 F	
	341.00 A	6.65 G	5.99 J	85.20 G	78.32 J	-2.12 F	
6.7	342.02 A	7.25 G	6.59 J	58.86 G	54.29 J	-2.12 F	
	342.02 A	7.25 G	6.59 J	58.86 G	54.29 J	-2.12 F	
	344.03 A	8.43 G	7.77 J	0.00 H	0.00 K	-2.12 F	

base	reaction	344.03 A	-4.66 G	3.25 D	0.00 A	0.00 D	2.12 F

* VERTICAL GUY LOAD & GUY ECCENTRIC MOMENT
+ HORIZONTAL REACTION @ TORSIONAL RESISTANCE

MAXIMUM GUY FORCES AT MAST
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GUY LEVEL FT	GUY AZICOMPONENTS AT MAST.....				FACTOR OF SAFETY	...GUY ANGLES...	
		N KIP	E KIP	DOWN KIP	TOTAL KIP		VERT	HORIZ
284.0	0.0	12.0A	-1.0D	15.4A	19.5A	3.0A	-52.2B	7.2I
	120.0	-5.5D	9.3E	15.5E	18.8E	3.1E	-55.4F	-9.0H
	120.0	-5.4D	9.3E	15.5E	18.9E	3.1E	-55.3F	-9.0H
	240.0	-5.4J	-9.3I	15.6I	18.9I	3.1I	-55.7H	9.0F
	240.0	-5.4J	-9.2I	15.6I	18.9I	3.1I	-55.8H	9.0F
	0.0	12.0A	1.0J	15.4A	19.5A	3.0A	-52.2L	7.2J
256.7	0.0	13.6A	-0.9D	15.5A	20.6A	2.8A	-48.8L	6.6I
	120.0	-5.9D	10.3E	15.3E	19.4E	3.0E	-52.4F	-8.6I
	120.0	-5.9E	10.3E	15.3E	19.4E	3.0E	-52.4F	-8.7I
	240.0	-5.9I	-10.2I	15.4I	19.4I	3.0I	-52.8H	8.7E
	240.0	-5.9J	-10.3I	15.5I	19.5I	3.0I	-52.8H	8.6E
	0.0	13.6A	0.9J	15.4A	20.5A	2.8A	-48.8L	6.6I
216.7	0.0	15.1A	-0.8D	14.2A	20.7A	2.8A	-43.4L	6.0I
	120.0	-6.3E	11.0E	13.8E	18.7E	3.1E	-47.7G	-8.6I
	120.0	-6.3E	10.9E	13.7E	18.6E	3.1E	-47.7G	-8.6I
	240.0	-6.2I	-10.8I	13.8I	18.6I	3.1I	-48.2G	8.7E
	240.0	-6.3I	-11.0I	14.0I	18.9I	3.1I	-48.2G	8.6E
	0.0	15.0A	-0.8D	14.2A	20.7A	2.8A	-43.4B	-6.0E
166.7	0.0	19.1A	-0.7D	13.4A	23.3A	2.5A	-35.3K	5.1H
	120.0	-7.9E	13.7E	13.3E	20.7E	2.8E	-40.6G	-7.6I
	120.0	-8.0E	13.9E	13.5E	20.9E	2.8E	-40.5G	-7.5I
	240.0	-7.9I	-13.7I	13.6I	20.8I	2.8I	-41.1G	7.6E
	240.0	-7.9I	-13.8I	13.7I	21.0I	2.8I	-41.1G	7.6E
	0.0	18.9A	-0.7D	13.3A	23.1A	2.5A	-35.3K	-5.1F
106.7	0.0	17.1A	-0.5D	7.4A	18.7A	2.3A	-28.6G	5.6H
	120.0	-7.6E	13.2E	8.6E	17.4E	2.4E	-33.9K	-7.6J
	120.0	-7.8E	13.5E	8.8E	17.8E	2.4E	-33.8K	-7.5J
	240.0	-7.6I	-13.2I	8.9I	17.6I	2.4I	-34.2C	7.5D
	240.0	-7.7I	-13.4I	9.0I	17.9I	2.4I	-34.2C	7.5D
	0.0	16.8A	-0.5D	7.3A	18.3A	2.3A	-28.7G	-5.7F

56.7	0.0	8.6A	-0.4D	1.9A	8.8A	2.4A	-18.0G	5.1I
	120.0	-4.1E	7.1E	2.9E	8.7E	2.4E	-24.6K	-6.4J
	120.0	-4.1E	7.1E	2.8E	8.7E	2.4E	-24.5K	-6.3J
	240.0	-4.0I	-7.0I	3.0I	8.6I	2.4I	-25.2C	6.4D
	240.0	-4.1I	-7.2I	3.0I	8.8I	2.4I	-25.3C	6.4D
	0.0	8.5A	-0.4D	1.9A	8.7A	2.4A	-18.1G	-5.1E

MAXIMUM GUY FORCES AT ANCHOR

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GUY LEVEL FT	GUY AZICOMPONENTS AT ANCHOR.....				TOTAL KIP	FACTOR OF SAFETY
		RAD KIP	LAT KIP	VERT KIP			
284.0	0.0	13.0A	-1.2J	13.9A	19.1A	3.1A	
	120.0	11.9E	1.2H	13.9E	18.3E	3.2E	
	120.0	11.9E	-1.2B	13.9E	18.4E	3.2E	
	240.0	11.9I	1.3L	14.1I	18.4I	3.2I	
	240.0	11.9I	-1.2F	14.0I	18.4I	3.2I	
	0.0	13.0A	1.2D	13.9A	19.0A	3.1A	
256.7	0.0	14.4A	-1.1J	14.1A	20.2A	2.9A	
	120.0	12.9E	1.1H	13.9E	18.9E	3.1E	
	120.0	12.8E	-1.2B	13.9E	18.9E	3.1E	
	240.0	12.8I	1.2L	14.0I	18.9I	3.1I	
	240.0	12.8I	-1.1F	14.1I	19.0I	3.1I	
	0.0	14.4A	1.1D	14.1A	20.1A	2.9A	
216.7	0.0	15.6A	-1.0J	13.1A	20.4A	2.9A	
	120.0	13.3E	1.0H	12.5E	18.3E	3.2E	
	120.0	13.3E	-1.0B	12.5E	18.2E	3.2E	
	240.0	13.2I	1.0L	12.6I	18.2I	3.2I	
	240.0	13.4I	-1.0F	12.7I	18.5I	3.2I	
	0.0	15.6A	1.0D	13.0A	20.3A	2.9A	
166.7	0.0	19.4A	-0.9J	12.5A	23.0A	2.5A	
	120.0	16.2E	0.9H	12.3E	20.4E	2.9E	
	120.0	16.4E	-0.9B	12.4E	20.6E	2.8E	
	240.0	16.2I	0.9L	12.5I	20.5I	2.8I	
	240.0	16.3I	-0.9F	12.6I	20.6I	2.8I	
	0.0	19.2A	0.9D	12.4A	22.9A	2.6A	
106.7	0.0	17.2A	-0.7J	6.9A	18.6A	2.3A	
	120.0	15.3E	0.6H	8.0E	17.3E	2.5E	
	120.0	15.7E	-0.7B	8.1E	17.7E	2.4E	
	240.0	15.4I	0.7L	8.2I	17.4I	2.4I	
	240.0	15.6I	-0.6F	8.3I	17.7I	2.4I	
	0.0	16.9A	0.6D	6.8A	18.2A	2.3A	
56.7	0.0	8.6A	-0.5J	1.6A	8.8A	2.4A	
	120.0	8.2E	0.5H	2.5E	8.6E	2.4E	
	120.0	8.2E	-0.5B	2.5E	8.6E	2.4E	
	240.0	8.1I	0.5L	2.6I	8.5I	2.4I	
	240.0	8.3I	-0.5F	2.7I	8.8I	2.4I	
	0.0	8.5A	0.5D	1.6A	8.7A	2.4A	

MAXIMUM ANCHOR LOADS

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AZI DEG	RADIUS FT	GUY TO ELEV FTANCHOR LOADS.....		SHAFT FORCES.....			ANGLE DEG
			HORIZ KIP	VERT KIP	LATER- AL KIP	AXIAL KIP	...LATERAL... VERT PLANE KIP	HORIZ PLANE KIP	
0.0	235.0	284.0	13.0A	13.9A	1.1D	18.6A	3.9A	1.1D	
		284.0	13.0A	13.9A	-1.1J	18.6A	3.9A	-1.1J	
		256.7	14.4A	14.1A	1.0D	19.9A	3.3A	1.0D	
		256.7	14.4A	14.1A	-1.0J	19.9A	3.3A	-1.0J	
		216.7	15.6A	13.0A	-0.8J	20.2A	1.7A	-0.8J	
		216.7	15.6A	13.1A	0.8D	20.3A	1.7A	0.8D	
		166.7	19.2A	12.4A	-0.7J	22.8A	-0.9A	-0.7J	
		166.7	19.4A	12.5A	0.7D	23.0A	-1.0A	0.7D	
		106.7	16.9A	6.8A	-0.5J	17.7A	-4.2A	-0.5J	
		106.7	17.2A	6.9A	0.5D	18.1A	-4.3A	0.5D	
		56.7	8.5A	1.6A	-0.4J	7.9A	-3.6A	-0.4J	
		56.7	8.6A	1.6A	0.4D	8.0A	-3.6A	0.4D	
					175.8A	124.0A	8.9D	215.1A	
120.0	235.0	284.0	11.9E	13.9E	1.1H	18.0E	3.3E	1.1H	
		284.0	11.9E	13.9E	1.1H	18.0E	3.3E	1.1H	
		256.7	12.8E	13.9E	1.0H	18.7E	2.7E	1.0H	
		256.7	12.9E	13.9E	1.0H	18.7E	2.7E	1.0H	
		216.7	13.3E	12.5E	0.9H	18.2E	1.3E	0.9H	
		216.7	13.4E	12.5E	0.9H	18.3E	1.3E	0.9H	
		166.7	16.4E	12.4E	0.7H	20.6E	-0.7E	0.7H	
		166.7	16.2E	12.3E	0.7H	20.4E	-0.7E	0.7H	
		106.7	15.7E	8.1E	0.5H	17.3E	-3.5E	0.5H	
		106.7	15.3E	8.0E	-0.5B	16.9E	-3.5E	-0.5B	
		56.7	8.2E	2.5E	-0.4B	8.0E	-3.2E	-0.4B	
		56.7	8.2E	2.5E	-0.4B	8.0E	-3.2E	-0.4B	
					156.3E	126.5E	9.5H	201.1E	
240.0	235.0	284.0	11.9I	14.0I	-1.1F	18.1I	3.3I	-1.1F	
		284.0	11.9I	14.1I	-1.1F	18.1I	3.3I	-1.1F	
		256.7	12.8I	14.1I	-1.0F	18.9I	2.7I	-1.0F	
		256.7	12.8I	14.0I	-1.0F	18.8I	2.7I	-1.0F	
		216.7	13.4I	12.7I	-0.9F	18.4I	1.3I	-0.9F	
		216.7	13.2I	12.6I	-0.9F	18.2I	1.3I	-0.9F	
		166.7	16.3I	12.6I	-0.8F	20.6I	-0.7I	-0.8F	
		166.7	16.2I	12.5I	-0.8F	20.5I	-0.7I	-0.8F	
		106.7	15.6I	8.3I	0.5L	17.3I	-3.5I	0.5L	
		106.7	15.4I	8.2I	-0.5F	17.1I	-3.4I	-0.5F	
		56.7	8.3I	2.7I	0.4L	8.1I	-3.2I	0.4L	
		56.7	8.1I	2.6I	0.4L	8.0I	-3.1I	0.4L	
					155.9I	128.5I	-9.5F	202.1I	