



Northeast Site Solutions
Denise Sabo
4 Angela's Way, Burlington CT 06013
203-435-3640
denise@northeastsitesolutions.com

September 17, 2021

Members of the Siting Council
Connecticut Siting Council
Ten Franklin Square
New Britain, CT 06051

RE: Tower Share Application
497 Middle Turnpike, Storrs Mansfield, CT 06268
Latitude: 41.82578056
Longitude: 72.28179444
Site# 842867_Crown_Dish

Dear Ms. Bachman:

This letter and attachments are submitted on behalf of Dish Wireless LLC. Dish Wireless LLC plans to install antennas and related equipment to the tower site located at 497 Middle Turnpike, Storrs Mansfield, Connecticut.

Dish Wireless LLC proposes to install three (3) 600/1900 5G MHz antenna and six (6) RRUs, at the 99-foot level of the existing 120-foot monopole tower, one (1) Fiber cables will also be installed. Dish Wireless LLC equipment cabinets will be placed within 7x5 lease area. Included are plans by Infinigy, dated July 26, 2021 Exhibit C. Also included is a structural analysis prepared by Crown Castle, dated May 28, 2021, confirming that the existing tower is structurally capable of supporting the proposed equipment. Attached as Exhibit D. This facility was approved by Connecticut Siting Council, Docket No. 247 on September 12, 2003. Please see attached Exhibit A.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies 16-50aa, of Dish Wireless LLC intent to share a telecommunications facility pursuant to R.C.S.A. 16-50j-88. In accordance with R.C.S.A., a copy of this letter is being sent to Toni Moran, Mayor, for the Town of Mansfield, Linda Painter – Director of Planning and Zoning, as well as the tower owner (Crown Castle) and property owner (CMC Storrs SPV LLC)

The planned modifications of the facility fall squarely within those activities explicitly provided for in R.C.S.A. 16-50j-89.

1. The proposed modification will not result in an increase in the height of the existing structure. The top of the tower is 120-feet; Dish Wireless LLC proposed antennas will be located at a center line height of 99-feet.
2. The proposed modifications will not result in the increase of the site boundary as depicted on the attached site plan.



NSS **NORTHEAST**
SITE SOLUTIONS

Turnkey Wireless Development

3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed local and state criteria. The incremental effect of the proposed changes will be negligent.

4. The operation of the proposed antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard. As indicated in the attached power density calculations, the combined site operations will result in a total power density of 10.83% as evidenced by Exhibit F.

Connecticut General Statutes 16-50aa indicates that the Council must approve the shared use of a telecommunications facility provided it finds the shared use is technically, legally, environmentally, and economically feasible and meets public safety concerns. As demonstrated in this letter, Dish Wireless LLC respectfully indicates that the shared use of this facility satisfies these criteria.

A. Technical Feasibility. The existing monopole has been deemed structurally capable of supporting Dish Wireless LLC proposed loading. The structural analysis is included as Exhibit D.

B. Legal Feasibility. As referenced above, C.G.S. 16-50aa has been authorized to issue orders approving the shared use of an existing tower such as this support tower in Storrs Mansfield. Under the authority granted to the Council, an order of the Council approving the requested shared use would permit Dish Wireless LLC to obtain a building permit for the proposed installation. Further, a Letter of Authorization is included as Exhibit G, authorizing Dish Wireless LLC to file this application for shared use.

C. Environmental Feasibility. The proposed shared use of this facility would have a minimal environmental impact. The installation of Dish Wireless LLC equipment at the 99-foot level of the existing 120-foot tower would have an insignificant visual impact on the area around the tower. Dish Wireless LLC ground equipment would be installed within the existing facility compound. Dish Wireless LLC shared use would therefore not cause any significant alteration in the physical or environmental characteristics of the existing site. Additionally, as evidenced by Exhibit F, the proposed antennas would not increase radio frequency emissions to a level at or above the Federal Communications Commission safety standard.

D. Economic Feasibility. Dish Wireless LLC will be entering into an agreement with the owner of this facility to mutually agreeable terms. As previously mentioned, the Letter of Authorization has been provided by the owner to assist Dish Wireless LLC with this tower sharing application.

E. Public Safety Concerns. As discussed above, the tower is structurally capable of supporting Dish Wireless LLC proposed loading. Dish Wireless LLC is not aware of any public safety concerns relative to the proposed sharing of the existing tower. Dish Wireless LLC intentions of providing new and improved wireless service through the shared use of this facility is expected to enhance the safety and welfare of local residents and individuals traveling through Storrs Mansfield.

Sincerely,

Denise Sabo

Denise Sabo

Mobile: 203-435-3640

Fax: 413-521-0558

Office: 4 Angela's Way, Burlington CT 06013

Email: denise@northeastsitesolutions.com



Attachments

cc: Toni Moran, Mayor

Town of Mansfield

4 South Eagleville Road, Storrs Mansfield, CT 06268

Attn: Mayor's Office

Linda Painter – Director of Planning and Zoning

Town of Mansfield

4 South Eagleville Road, Storrs Mansfield, CT 06268

Attn: Town Planning Office

CMC Storrs SPV LLC – Property Owner

1 Harbor Point Road Unit 1855, Stamford, CT 06902

Attn: JZ Investments Inc.

Crown Castle, Tower Owner

Exhibit A

Original Facility Approval

DOCKET NO. 247 – AT&T Wireless PCS, LLC d/b/a	}	Connecticut
AT&T Wireless application for a Certificate of		
Environmental Compatibility and Public Need for the	}	Siting
construction, maintenance and operation of a		
telecommunications facility in Mansfield, Connecticut.	}	Council

September 12, 2003

Decision and Order

Pursuant to the foregoing Findings of Fact and Opinion, the Connecticut Siting Council (Council) finds that the effects associated with the construction, operation, and maintenance of a telecommunications facility including effects on the natural environment; ecological integrity and balance; public health and safety; scenic, historic, and recreational values; forests and parks; air and water purity; and fish and wildlife are not disproportionate either alone or cumulatively with other effects when compared to need, are not in conflict with the policies of the State concerning such effects, and are not sufficient reason to deny the application and therefore directs that a Certificate of Environmental Compatibility and Public Need, as provided by General Statutes § 16-50k, be issued to AT&T Wireless PCS, LLC (AT&T) for the construction, maintenance and operation of a wireless telecommunications facility at proposed Site A-1 located at 497 Middle Turnpike, Mansfield, Connecticut. We deny certification of the proposed Site B located off Cedar Swamp Road, Mansfield, Connecticut.

The facility shall be constructed, operated, and maintained substantially as specified in the Council's record in this matter, and subject to the following conditions:

1. There shall be a minimal shift in the tower's location in a southerly direction to the extent necessary to keep the tower's setback radius within the host property's boundaries.
2. The tower shall be constructed as a monopole, no taller than necessary to provide the proposed telecommunications services, sufficient to accommodate the antennas of AT&T and other entities, both public and private, but such tower shall not exceed a height of 120 feet above ground level.
3. Construction activities shall be conducted between November 1 and April 1 in order to minimize possible disturbance of any *Clemmys insculpta* (wood turtles) in the vicinity of the site.
4. The Certificate Holder shall prepare a Development and Management (D&M) Plan for this site in compliance with Sections 16-50j-75 through 16-50j-77 of the Regulations of Connecticut State Agencies. The D&M Plan shall be submitted to and approved by the Council prior to the commencement of facility construction and shall include:
 - a. a detailed site development plan that depicts the location of the access road, compound, tower, and utility line;
 - b. specifications for the tower, tower foundation, antennas, equipment building, and security fence;
 - c. construction plans for site clearing, water drainage, and erosion and sedimentation control consistent with the 2002 Connecticut Guidelines for Soil Erosion and Sediment Control, as amended.
5. The Certificate Holder shall, prior to the commencement of operation, provide the Council worst-case modeling of electromagnetic radio frequency power densities of all proposed entities' antennas at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin No. 65, August 1997. The Certificate Holder shall provide a recalculated report of electromagnetic radio frequency power density if and when circumstances in operation cause a change in power density above the levels calculated and provided pursuant to this Decision and Order.
6. Upon the establishment of any new state or federal radio frequency standards applicable to frequencies of this facility, the facility granted herein shall be brought into compliance with such standards.

7. The Certificate Holder shall permit public or private entities to share space on the proposed tower for fair consideration, or shall provide any requesting entity with specific legal, technical, environmental, or economic reasons precluding such tower sharing. Should the local municipality have a need to locate antennas on this tower, the Certificate Holder shall provide appropriate space on the tower with no lease charges.

8. If the facility does not initially provide wireless services within one year of completion of construction or ceases to provide wireless services for a period of one year, this Decision and Order shall be void, and the Certificate Holder shall dismantle the tower and remove all associated equipment or reapply for any continued or new use to the Council before any such use is made.

9. Any antenna that becomes obsolete and ceases to function shall be removed within 60 days after such antennas become obsolete and cease to function.

10. Unless otherwise approved by the Council, this Decision and Order shall be void if the facility authorized herein is not operational within one year of the effective date of this Decision and Order or within one year after all appeals to this Decision and Order have been resolved.

Pursuant to General Statutes § 16-50p, we hereby direct that a copy of the Findings of Fact, Opinion, and Decision and Order be served on each person listed below, and notice of issuance shall be published in The Hartford Courant.

By this Decision and Order, the Council disposes of the legal rights, duties, and privileges of each party named or admitted to the proceeding in accordance with Section 16-50j-17 of the Regulations of Connecticut State Agencies.

The parties and intervenors to this proceeding are:

Applicant

AT&T Wireless PCS, LLC
d/b/a AT&T Wireless

Its Representative

Christopher B. Fisher, Esq.
Cuddy & Feder & Worby LLP
90 Maple Avenue
White Plains, NY 10601
(914) 761-1300
(914) 761-6405 - fax

Exhibit B

Property Card

497 MIDDLE TPKE

Location	497 MIDDLE TPKE	Mblu	8/ 14/ 19/ /
Acct#	8 14 19	Owner	CMC STORRS SPV LLC
PBN		Assessment	\$820,700
Appraisal	\$1,172,300	PID	5973
Building Count	2		

Current Value

Appraisal			
Valuation Year	Improvements	Land	Total
2019	\$574,600	\$597,700	\$1,172,300
Assessment			
Valuation Year	Improvements	Land	Total
2019	\$402,300	\$418,400	\$820,700

Owner of Record

Owner	CMC STORRS SPV LLC	Sale Price	\$1,100,000
Co-Owner	ATTN JZ INVESTMENTS INC	Certificate	
Address	1 HARBOR POINT RD UNIT 1855 STAMFORD, CT 06902	Book & Page	799/538
		Sale Date	12/21/2018
		Instrument	81

Ownership History

Ownership History					
Owner	Sale Price	Certificate	Book & Page	Instrument	Sale Date
CMC STORRS SPV LLC	\$1,100,000		799/538	81	12/21/2018
BRODIN ANN TRUSTEE OF THE	\$0		763/988		05/14/2014
BRODIN ANN TRUSTEE OF THE	\$0		757/131	01	09/06/2013
BRODIN BERNARD R	\$58,000		699/309	00	12/21/2010
BRODIN BERNARD R EST OF	\$0		185/259		12/12/1980

Building Information

Building 1 : Section 1

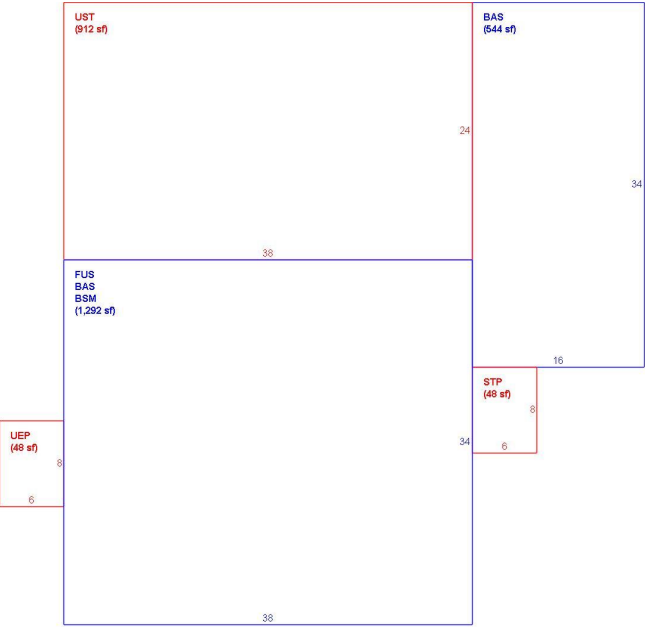
Year Built: 1928
Living Area: 3,128
Replacement Cost: \$268,286
Building Percent Good: 43
Replacement Cost
Less Depreciation: \$115,400

Building Attributes	
Field	Description
STYLE	Store
MODEL	Comm/Ind
Grade	D
Stories:	2
Occupancy	2.00
Exterior Wall 1	Stucco on Mas.
Exterior Wall 2	
Roof Structure	Gable
Roof Cover	Asphalt Shingl
Interior Wall 1	Drywall
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	
Heating Fuel	Gas
Heating Type	Hot Water
AC Type	Unit/AC
Struct Class	
Bldg Use	Commercial Improv
Usrflid 215	
Usrflid 216	
Usrflid 217	
Usrflid 218	
Usrflid 219	
1st Floor Use:	
Heat/AC	HEAT/AC SPLIT
Frame Type	MASONRY
Baths/Plumbing	AVERAGE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
Usrflid 214	

Building Photo

 Building Photo
(http://images.vgsi.com/photos/MansfieldCTPhotos/A00\00\92\52.JPG)

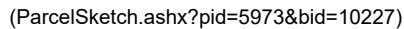
Building Layout



(ParcelSketch.ashx?pid=5973&bid=5973)

Building Sub-Areas (sq ft)			Legend
Code	Description	Gross Area	Living Area
BAS	First Floor	1,836	1,836
FUS	Finished Upper Story	1,292	1,292
BSM	Basement	1,292	0
STP	Stoop	48	0
UEP	Utility Enclosed Porch	48	0
UST	Utility Storage	912	0
		5,428	3,128

Building Attributes : Bldg 2 of 2	
Field	Description
STYLE	Store
MODEL	Comm/Ind
Grade	C-
Stories:	1
Occupancy	1.00
Exterior Wall 1	Vinyl
Exterior Wall 2	
Roof Structure	Gable
Roof Cover	Asphalt Shingl
Interior Wall 1	Drywall
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	
Heating Fuel	Oil
Heating Type	Forced Air
AC Type	Central
Struct Class	
Bldg Use	Commercial Improv
Usrflid 215	
Usrflid 216	
Usrflid 217	
Usrflid 218	
Usrflid 219	
1st Floor Use:	
Heat/AC	HEAT/AC PKGS
Frame Type	WOOD FRAME
Baths/Plumbing	AVERAGE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
Usrflid 214	



Building Sub-Areas (sq ft)			Legend
Code	Description	Gross Area	Living Area
BAS	First Floor	3,200	3,200
FOP	Framed Open Porch	1,176	0
		4,376	3,200

Extra Features	Legend
No Data for Extra Features	

Land

Land Use	Land Line Valuation
Use Code201	Size (Acres)30.88
DescriptionCommercial Improv	Frontage
ZoneRAR90	Depth
NeighborhoodC100	Assessed Value\$418,400
Alt Land ApprNo	Appraised Value\$597,700
Category	

Outbuildings

Outbuildings						Legend
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #
PAV1	Paving			9000.00 S.F.	\$8,100	1
SHD1	Shed			80.00 S.F.	\$700	2
FGR1	Garage			1024.00 S.F.	\$17,200	1
GLF2	Golf Course Fair			9.00 HOLES	\$226,800	2

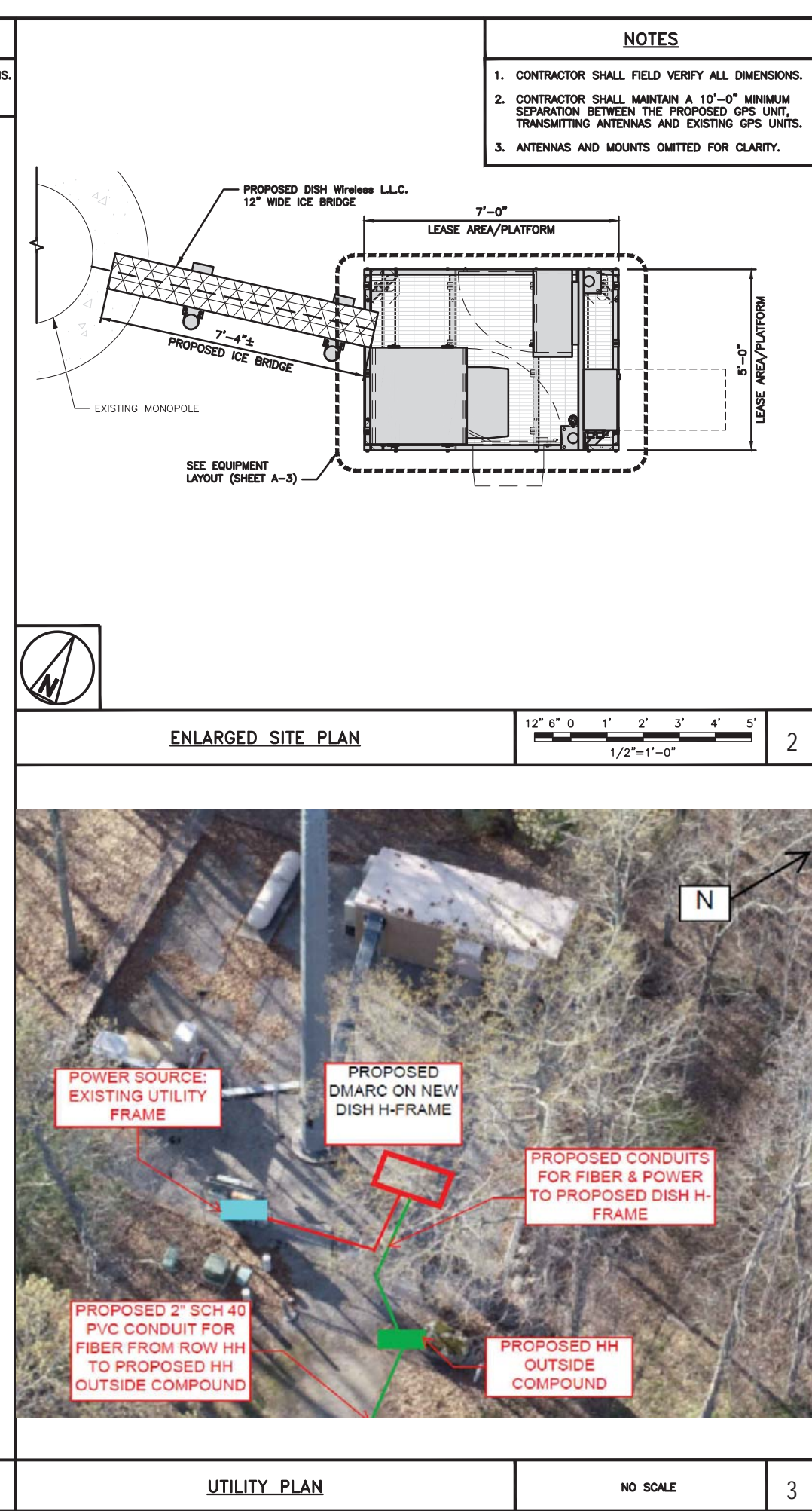
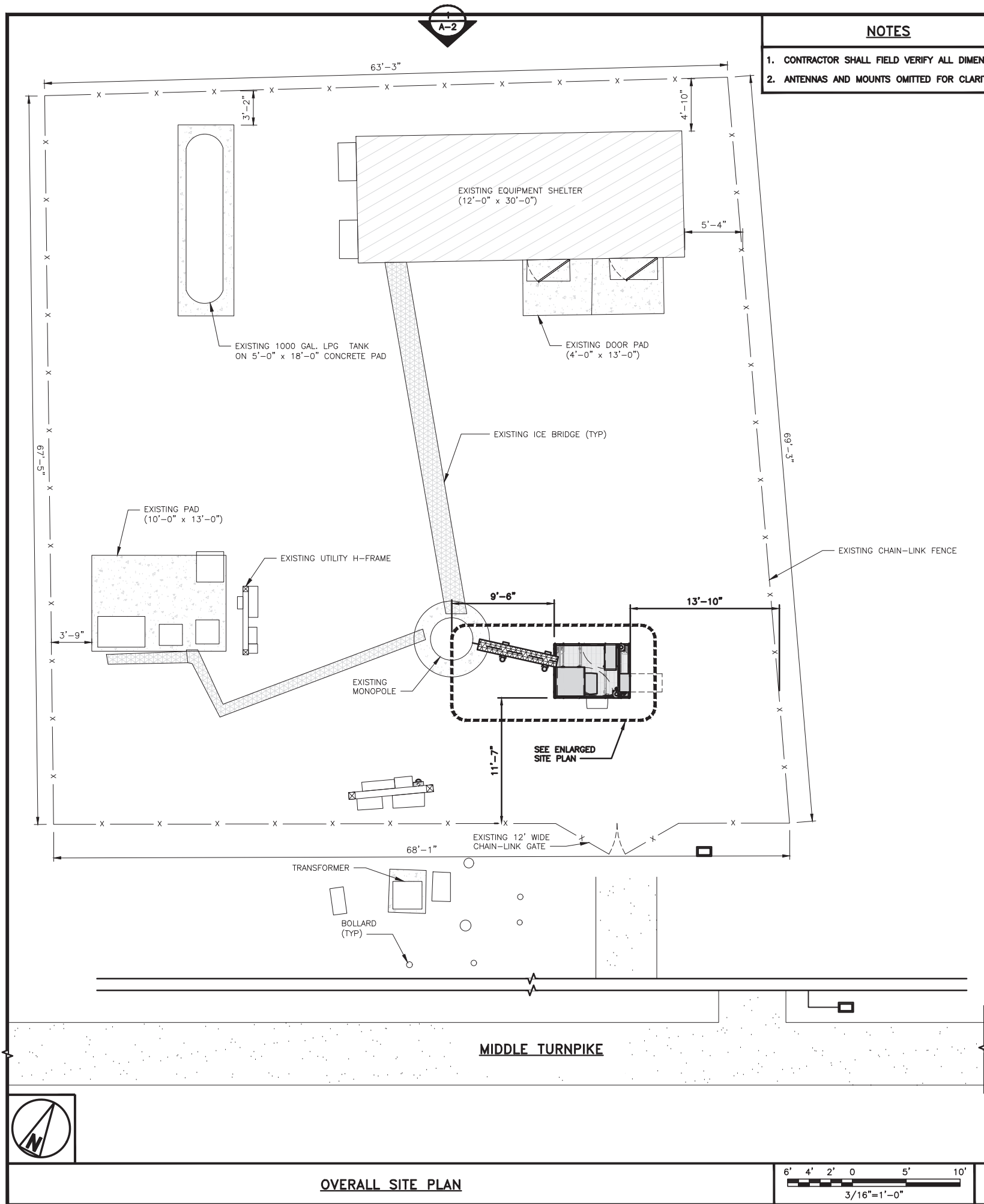
Valuation History

Appraisal			
Valuation Year	Improvements	Land	Total
2018	\$591,700	\$597,700	\$1,189,400
2017	\$591,700	\$597,700	\$1,189,400
2016	\$591,700	\$597,700	\$1,189,400

Assessment			
Valuation Year	Improvements	Land	Total
2018	\$414,200	\$418,400	\$832,600
2017	\$414,200	\$418,400	\$832,600
2016	\$414,200	\$418,400	\$832,600

Exhibit C

Construction Drawings



dish wireless.

5701 SOUTH SANTA FE DRIVE
LITTLETON, CO 80120

CROWN CASTLE

2000 CORPORATE DRIVE
CANONSBURG, PA 15317

B+T GRP

1717 S. BOULDER
SUITE 300
TULSA, OK 74119
PH: (918) 587-4630
www.btgrp.com

B&T ENGINEERING, INC.
PEC.0001564
Expires 2/10/22

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

DRAWN BY: CHECKED BY: APPROVED BY:

JJR JHW MDW

RFDS REV #: 0

CONSTRUCTION DOCUMENTS

REV	DATE	DESCRIPTION
A	6/17/21	ISSUED FOR REVIEW
0	7/26/21	ISSUED FOR CONSTRUCTION

A&E PROJECT NUMBER
151918.001.01

DISH Wireless L.L.C.
PROJECT INFORMATION
BOBDL00066A
497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT 06268

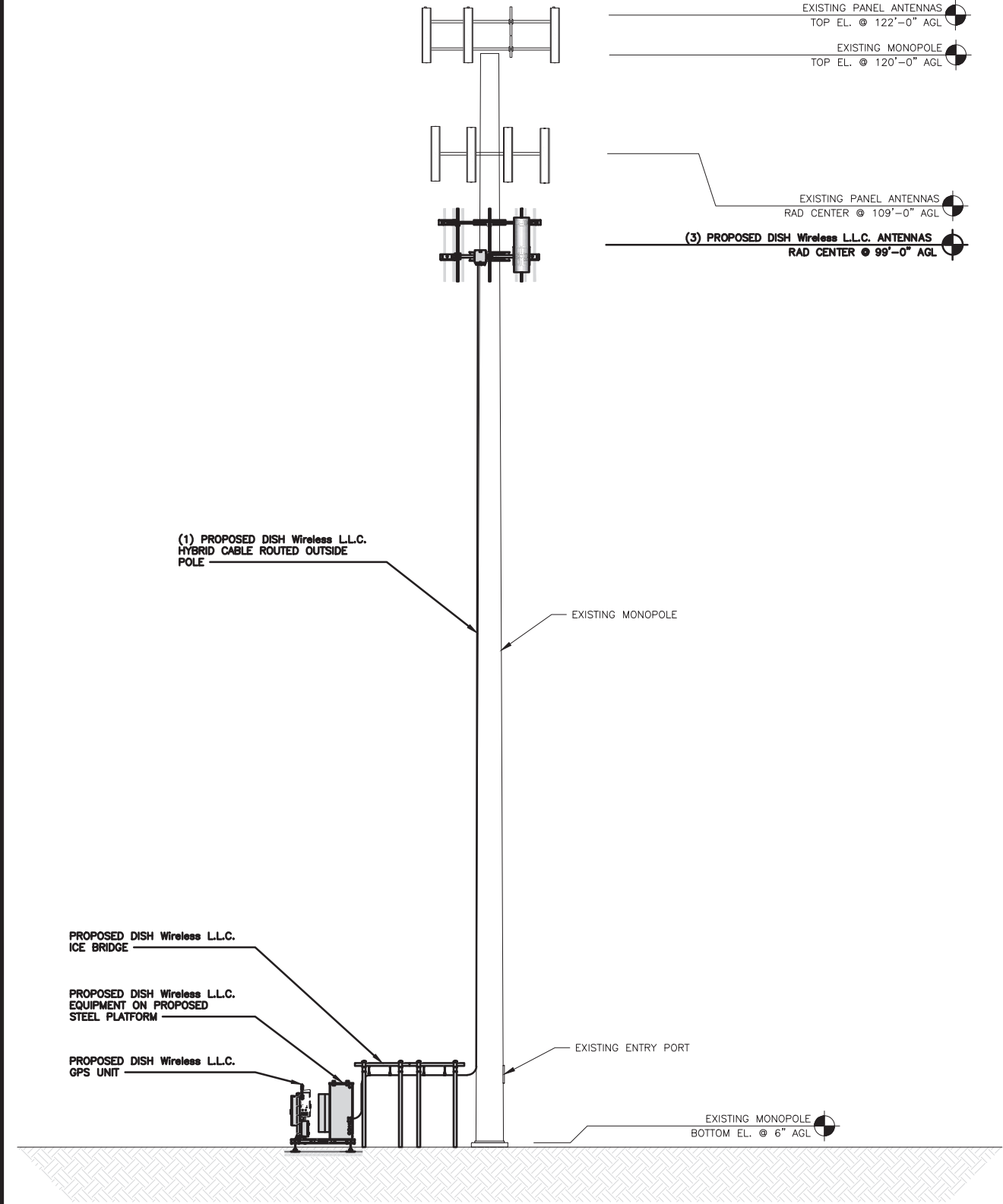
SHEET TITLE
OVERALL AND ENLARGED SITE PLAN

SHEET NUMBER
A-1

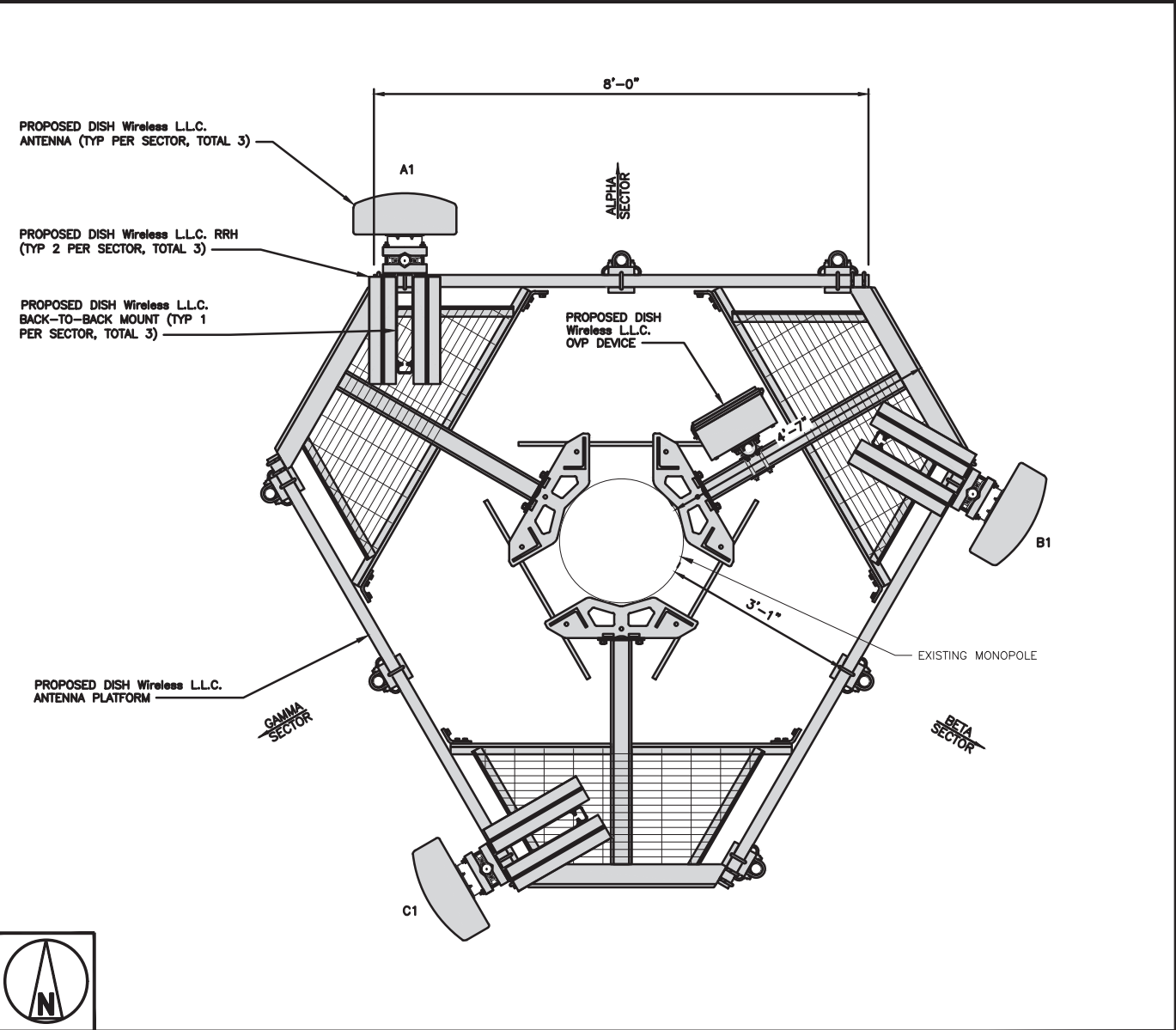
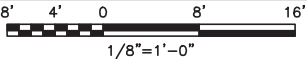
- NOTES
1. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS.

2. ANTENNA AND MW DISH SPECIFICATIONS REFER TO ANTENNA SCHEDULE AND TO FINAL CONSTRUCTION RFDS FOR ALL RF DETAILS

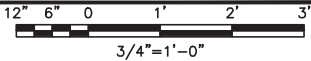
3. EXISTING EQUIPMENT AND FENCE OMITTED FOR CLARITY.



PROPOSED NORTH ELEVATION



ANTENNA LAYOUT



SECTOR	POSITION	ANTENNA						TRANSMISSION CABLE
		EXISTING OR PROPOSED	MANUFACTURER - MODEL NUMBER	TECHNOLOGY	SIZE (HxW)	AZIMUTH	RAD CENTER	
ALPHA	A1	PROPOSED	JMA - MX08FRO665-21	5G	72.0" x 20.0"	0°	99'-0"	(1) HIGH-CAPACITY HYBRID CABLE (129' LONG)
BETA	B1	PROPOSED	JMA - MX08FRO665-21	5G	72.0" x 20.0"	120°	99'-0"	
GAMMA	C1	PROPOSED	JMA - MX08FRO665-21	5G	72.0" x 20.0"	240°	99'-0"	

SECTOR	POSITION	RRH	
		MANUFACTURER - MODEL NUMBER	TECHNOLOGY
ALPHA	A1	FUJITSU - TA08025-B604	5G
	A1	FUJITSU - TA08025-B605	5G
BETA	B1	FUJITSU - TA08025-B604	5G
	B1	FUJITSU - TA08025-B605	5G
GAMMA	C1	FUJITSU - TA08025-B604	5G
	C1	FUJITSU - TA08025-B605	5G

- NOTES
1. CONTRACTOR TO REFER TO FINAL CONSTRUCTION RFDS FOR ALL RF DETAILS.

2. ANTENNA AND RRH MODELS MAY CHANGE DUE TO EQUIPMENT AVAILABILITY. ALL EQUIPMENT CHANGES MUST BE APPROVED AND REMAIN IN COMPLIANCE WITH THE PROPOSED DESIGN AND STRUCTURAL ANALYSES.

ANTENNA SCHEDULE



5701 SOUTH SANTA FE DRIVE
LITTLETON, CO 80120



2000 CORPORATE DRIVE
CANONSBURG, PA 15317



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RFDS REV #: 0

CONSTRUCTION
DOCUMENTS

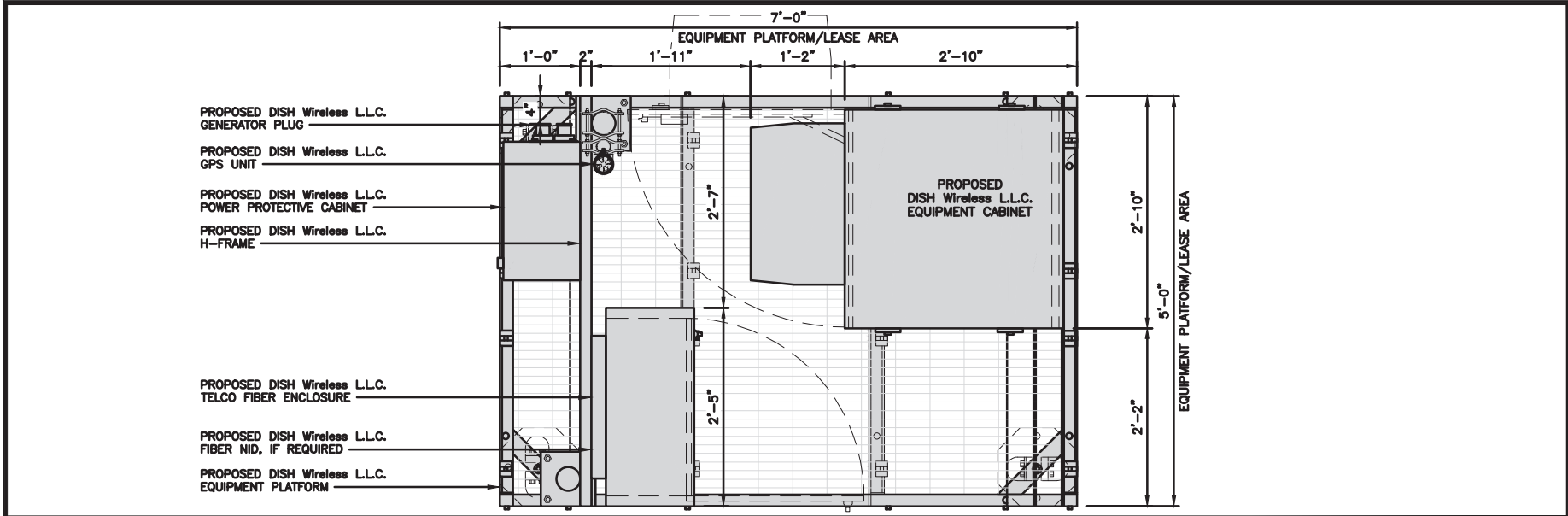
SUBMITTALS		
REV	DATE	DESCRIPTION
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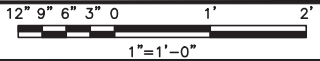
DISH Wireless L.L.C.
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497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT
06268

SHEET TITLE
ELEVATION, ANTENNA
LAYOUT AND SCHEDULE

SHEET NUMBER



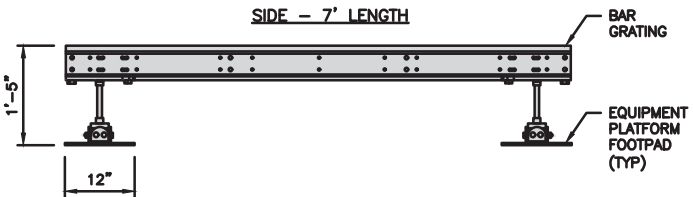
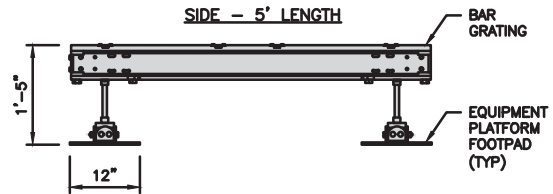
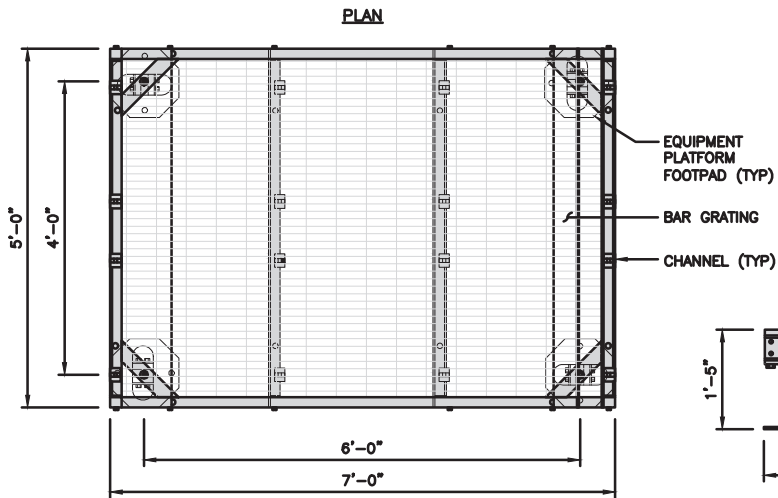
PLATFORM EQUIPMENT PLAN



1

COMMSCOPE MTC4045LP 5X7 PLATFORM	
DIMENSIONS (HxWxD)	16"x84"x60"
TOTAL WEIGHT	423 LBS

NOTE:
GC TO PROVIDE EXTENDED
THREAD FOR PLATFORM IF
REQUIRED HEIGHT EXCEEDS 17"

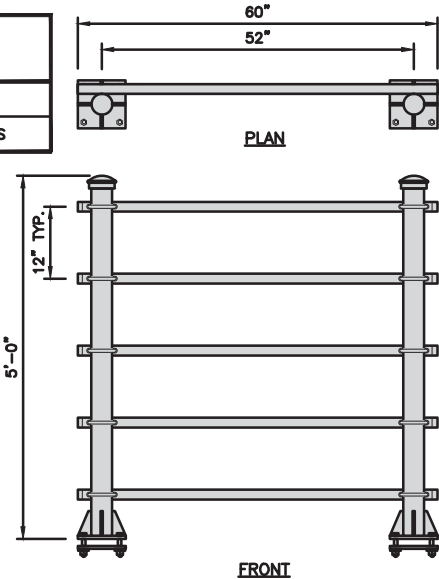
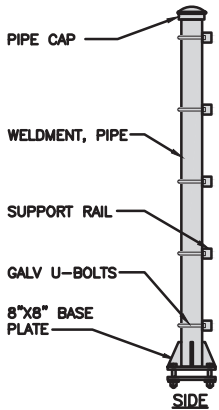


PLATFORM DETAIL

NO SCALE

2

KENWOOD T1701KT5-5S H-FRAME	
UNISTRUT/SUPPORT RAIL	5
WEIGHT/ VOLUME	173.6 LBS



H-FRAME DETAIL

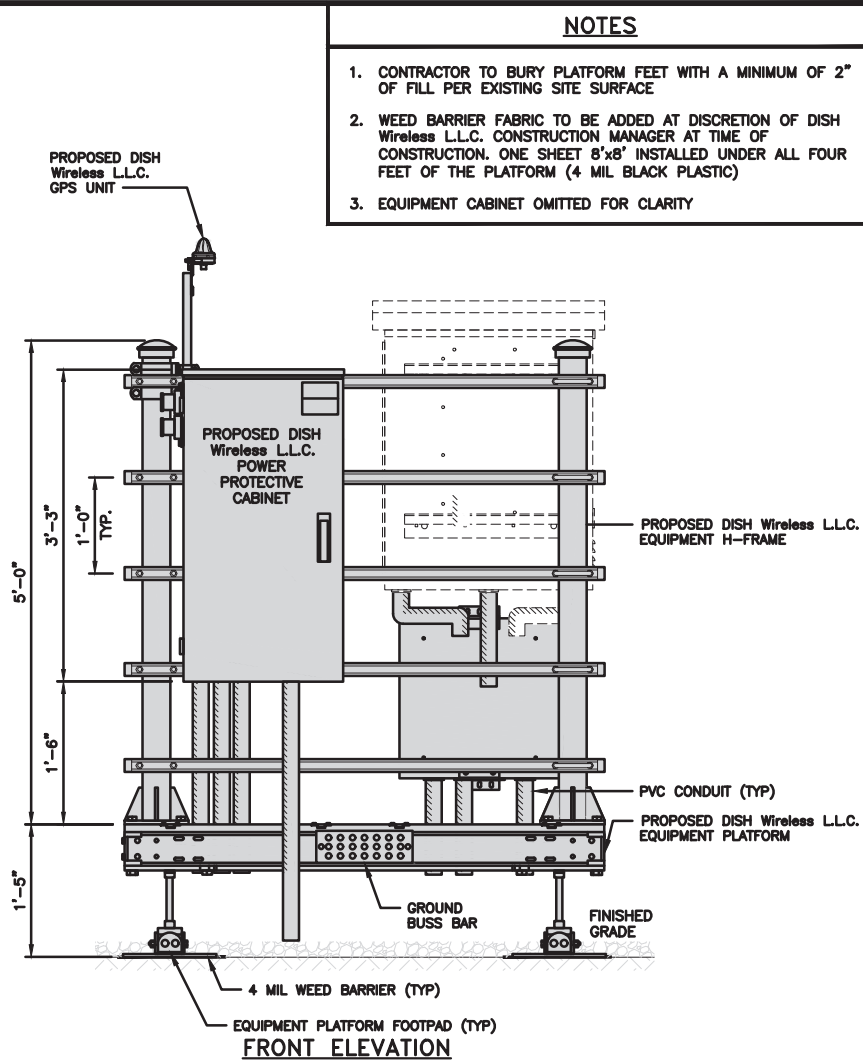
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3

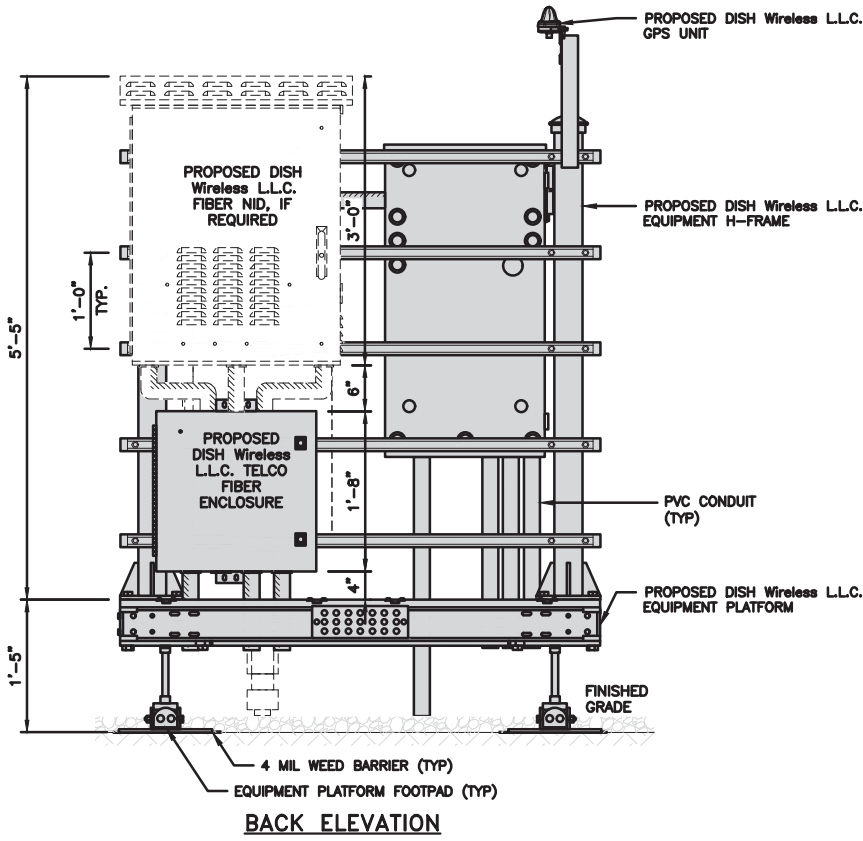
NOT USED

NO SCALE

4

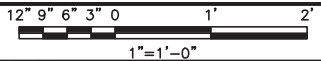


FRONT ELEVATION



BACK ELEVATION

H-FRAME EQUIPMENT ELEVATION



5

NOTES

1. CONTRACTOR TO BURY PLATFORM FEET WITH A MINIMUM OF 2" OF FILL PER EXISTING SITE SURFACE
2. WEED BARRIER FABRIC TO BE ADDED AT DISCRETION OF DISH Wireless L.L.C. CONSTRUCTION MANAGER AT TIME OF CONSTRUCTION. ONE SHEET 8'x8' INSTALLED UNDER ALL FOUR FEET OF THE PLATFORM (4 MIL BLACK PLASTIC)
3. EQUIPMENT CABINET OMITTED FOR CLARITY

dish
wireless.

5701 SOUTH SANTA FE DRIVE
LITTLETON, CO 80120

CROWN
CASTLE

2000 CORPORATE DRIVE
CANONSBURG, PA 15317

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06268

SHEET TITLE
EQUIPMENT PLATFORM AND
H-FRAME DETAILS

SHEET NUMBER

A-3

CHARLES INDUSTRY HEX
CUBE-PM639155N4

DIMENSIONS (HxWxD):	74"x32"x32"
POWER PLANT:	-48VDC ABB/600W
TOTAL WEIGHT (EMPTY)	408 LBS

PLAN

SIDE

BACK

SIDE

FRONT

CABINET DETAIL

NO SCALE

1

RAYCAP PPC
RDIAC-2465-P-240-MTS

ENCLOSURE DIMENSIONS (HxWxD):	39"x22.855"x12.593
WEIGHT:	80 lbs
OPERATING AC VOLTAGE	240/120 1 PHASE 3W+G

TOP

BACK

SIDE

FRONT

SIDE

POWER PROTECTION CABINET (PPC) DETAIL

NO SCALE

2

NOT USED

NO SCALE

3

ZAYO 5RU CABINET
LEFT SWING DOOR ("LIT" SITES)

DIMENSIONS (HxWxD)	36.115"x29"x12.9"
WEIGHT	85 LBS
POWER INPUT	20A, -48VDC

PLAN

FRONT

SIDE

BACK

NETWORK INTERFACE UNIT DETAIL

NO SCALE

5

CHARLES CFIT-PF2020DSH1
FIBER TELCO ENCLOSURE

ENCLOSURE DIMS (HxWxD)	20"x20"x9"
ENCLOSURE WEIGHT	20 lbs
MOUNTING	WALL
COMPLIANCE	TYPE 4

FRONT

SIDE

BACK

FRONT

FIBER TELCO ENCLOSURE DETAIL

NO SCALE

6

COMMSCOPE WB-K110-B
WAVEGUIDE BRIDGE KIT

DIMENSIONS (HxL)	160"x10"
WEIGHT/ VOLUME	325.0 LBS
CABLE RUN (QTY)	12

INCLUDED PRODUCTS:

WB-T12-3 TRAPEZE KIT,
3 RUNGS

WB-LB12-3 SUPPORT BRACKET

MF-130 DIRECT BURIAL PIPE
COLUMN, 13'-4"

TRAPEZE KIT
(WB-T12-3)

SUPPORT BRACKET
(WB-LB12-3)

3.5" DIA GALV SCH
40 PIPE (SPACED
9'-0" MAX)
(MF-130)

PLAN

FRONT

SIDE

ICE BRIDGE DETAIL

NO SCALE

7

FINISH SLOPE
TO DRAIN

A-A

A-A

PROPOSED 3.5" DIA.
SCH 40 PIPE
GALVANIZED

PROPOSED 1'-6"
DIA. CONCRETE
PIER (TYP)

CONCRETE PIER

3" DIA SCH 40 PIPE

18" DIA DRILLED
PIER FOUNDATION

A-A SECTION

1'-6"

TYPICAL ICE BRIDGE CONCRETE PIER DETAIL

NO SCALE

8

PROPOSED
ICE BRIDGE

PROPOSED X" DIA
HYBRID CABLE
(OPTION "A")

PROPOSED
CABLE CLAMP
@ 3'-0" O.C.

HYBRID SUPPORT
BRACKET AND BANDING
@ 4'-0" O.C.

EXISTING
MONOPOLE

HYBRID CABLE RUN

NO SCALE

9

5701 SOUTH SANTA FE DRIVE
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2000 CORPORATE DRIVE
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JJR	JHW	MDW

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A&E PROJECT NUMBER
151918.001.01

DISH Wireless L.L.C.
PROJECT INFORMATION
BOBDL00066A
497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT 06268

SHEET TITLE
EQUIPMENT DETAILS

SHEET NUMBER
A-4

<div>ROSENBERGER GPSGLONASS-36-N-S</div> <table><tr><td>DIMENSION (DIA x H)</td><td>69mm x 98.5mm</td></tr><tr><td>WEIGHT (WITH ACCESSORIES)</td><td>515.74g</td></tr><tr><td>CONNECTOR</td><td>N-FEMALE</td></tr><tr><td>FREQUENCY RANGE</td><td>1559 MHz ~ 1610.5MHz</td></tr></table> <div><div>GPS UNIT GROUNDING KIT MOUNTING BRACKET</div><div>GPS UNIT GROUNDING KIT MOUNTING BRACKET</div></div>			DIMENSION (DIA x H)	69mm x 98.5mm	WEIGHT (WITH ACCESSORIES)	515.74g	CONNECTOR	N-FEMALE	FREQUENCY RANGE	1559 MHz ~ 1610.5MHz	<div>TOP</div> <div>GPS UNIT GROUNDING KIT MOUNTING BRACKET</div> <div>GPS UNIT GROUNDING KIT MOUNTING BRACKET</div>			<div>MINIMUM OF 75% OR 270° IN ANY DIRECTION</div> <div>GPS</div> <div>GPS UNIT</div> <div>OBSTRUCTIONS MUST BE BELOW 10'</div>			<div>1.75"ø</div> <div>27" MIN BEND RADIUS</div> <div>CU12PSM6P4XXX (4 AWG CONDUCTORS)</div> <div>1.60"ø</div> <div>24" MIN BEND RADIUS</div> <div>CU12PSM9P6XXX (6 AWG CONDUCTORS)</div> <div>1.41"ø</div> <div>22" MIN BEND RADIUS</div> <div>CU12PSM9P8XXX (8 AWG CONDUCTORS)</div>									
DIMENSION (DIA x H)	69mm x 98.5mm																									
WEIGHT (WITH ACCESSORIES)	515.74g																									
CONNECTOR	N-FEMALE																									
FREQUENCY RANGE	1559 MHz ~ 1610.5MHz																									
GPS ANTENNA DETAIL			NO SCALE			1			GPS MINIMUM SKY VIEW REQUIREMENTS			NO SCALE			2			CABLES UNLIMITED HYBRID CABLE MINIMUM BEND RADIUSES			NO SCALE			3		
NOT USED			NO SCALE			4			NOT USED			NO SCALE			5			NOT USED			NO SCALE			6		
NOT USED			NO SCALE			7			NOT USED			NO SCALE			8			NOT USED			NO SCALE			9		



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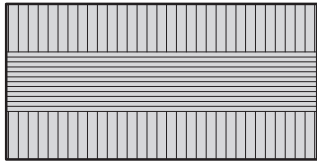
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151918.001.01

DISH Wireless L.L.C.
PROJECT INFORMATION
BOBDL00066A
497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT
06268

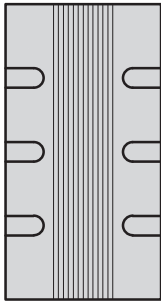
SHEET TITLE
EQUIPMENT DETAILS

SHEET NUMBER
A-5

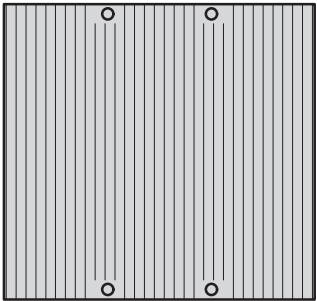
FUJITSU TA08025-B604 RRH	
DIMENSIONS (HxWxD) (KG/IN)	380x400x200/14.9"x15.7"x7.8"
WEIGHT(KG,LB)/ VOLUME	29kg,63.9lb/ 30L
POWER SUPPLY	DC-58~-36V



PLAN

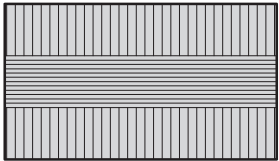


SIDE

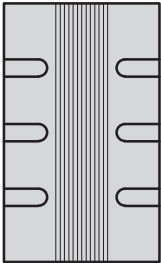


FRONT

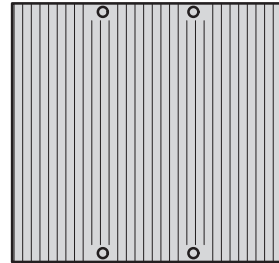
FUJITSU TA08025-B605 RRH	
DIMENSIONS (HxWxD) (KG/IN)	380x400x230/14.9"x15.7"x9.0"
WEIGHT(KG,LB)/ VOLUME	34kg,74.9lb/ 35L
POWER SUPPLY	DC-58~-36V



PLAN



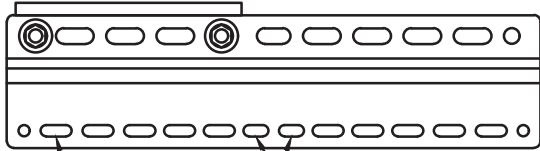
SIDE



FRONT

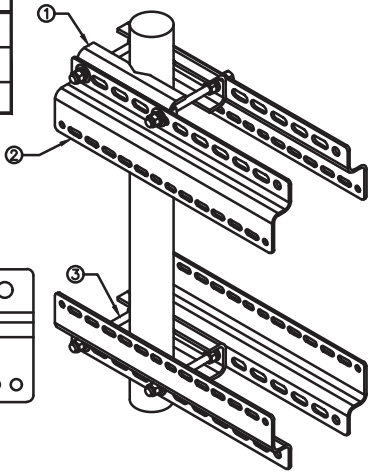
SABRE INDUSTRIES RRU BRACKET MOUNT C10123155	
DIMENSIONS (HxWxD) (1 BRACKET)	5"x20"x1-13/16"
WEIGHT (FULL ASSEMBLY)	35.79 lbs
PACKAGE QUANTITY	4

ITEM#	DESCRIPTION
1	PLATE, CHANNEL BRACKET
2	RRH Z BRACKET, 3/16"
3	THREADED ROD ASSEMBLY 1/2"x12"



11MM x 30MM SLOTS
40MM ON CENTER

11MM x 24MM SLOTS



REMOTE RADIO HEAD DETAIL

NO SCALE

1

REMOTE RADIO HEAD DETAIL

NO SCALE

2

REMOTE RADIO MOUNT DETAIL

NO SCALE

3

JMA WIRELESS MX08FRO665-21 ANTENNA	
DIMENSIONS (HxWxD)	72.0"x20.0"x8.0"
TOTAL WEIGHT	64.5 LB
RF PORTS, CONNECTOR TYPE	8 x 4.3-10 FEMALE



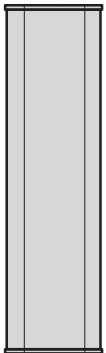
PLAN



BACK



SIDE



FRONT

ANTENNA DETAIL

NO SCALE

4

NOT USED

NO SCALE

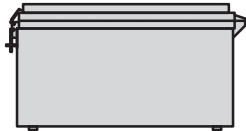
5

ANTENNA MOUNTING DETAIL

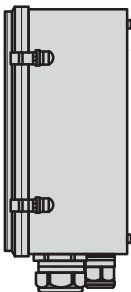
NO SCALE

6

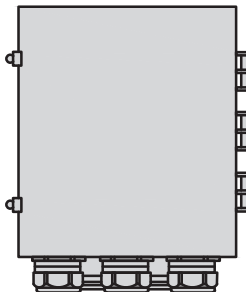
RAYCAP RDIDC-9181-PF-48 DC SURGE PROTECTION (OVP)	
DIMENSIONS (HxWxD)	18.98"x14.39"x8.15"
WEIGHT	21.82 LBS



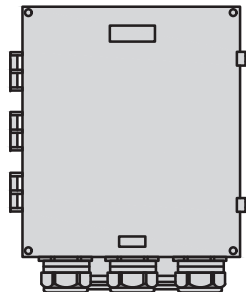
PLAN



SIDE



BACK



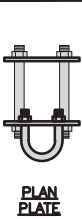
FRONT

SURGE SUPPRESSION DETAIL (OVP)

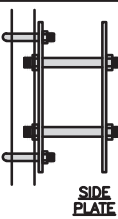
NO SCALE

7

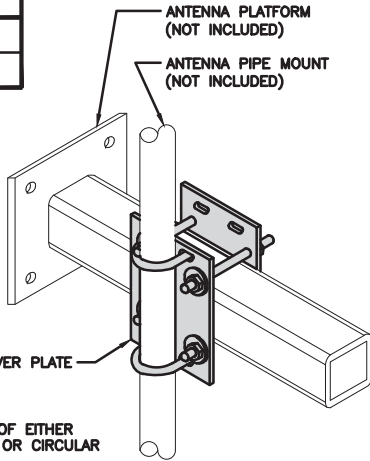
COMMSCOPE XP-2040 CROSSOVER PLATE	
DIMENSIONS (HxW)	10"x12"
WEIGHT	11.023 LBS



PLAN
U-BOLT



SIDE
U-BOLT



CROSSOVER PLATE

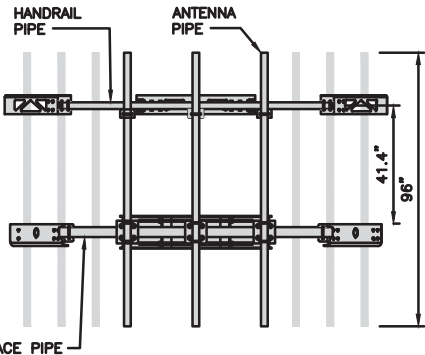
OPTION OF EITHER
SQUARE OR CIRCULAR
U-BOLT

RRH/OVP MOUNT DETAIL

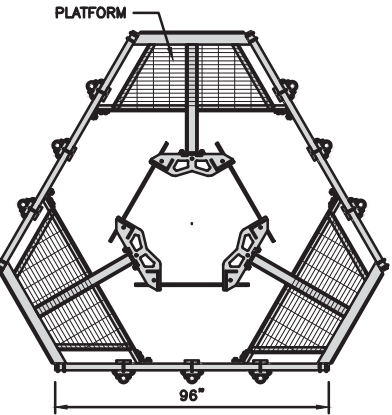
NO SCALE

8

COMMSCOPE MC-PK8-DSH	
FACE WIDTH	96"
WEIGHT	1373.08 lbs
NOTE: 15" TO 38" O.D.	



FACE PIPE



PLATFORM

ANTENNA PLATFORM DETAIL

NO SCALE

9

dish
wireless.

5701 SOUTH SANTA FE DRIVE
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CROWN
CASTLE

2000 CORPORATE DRIVE
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B+T GRP

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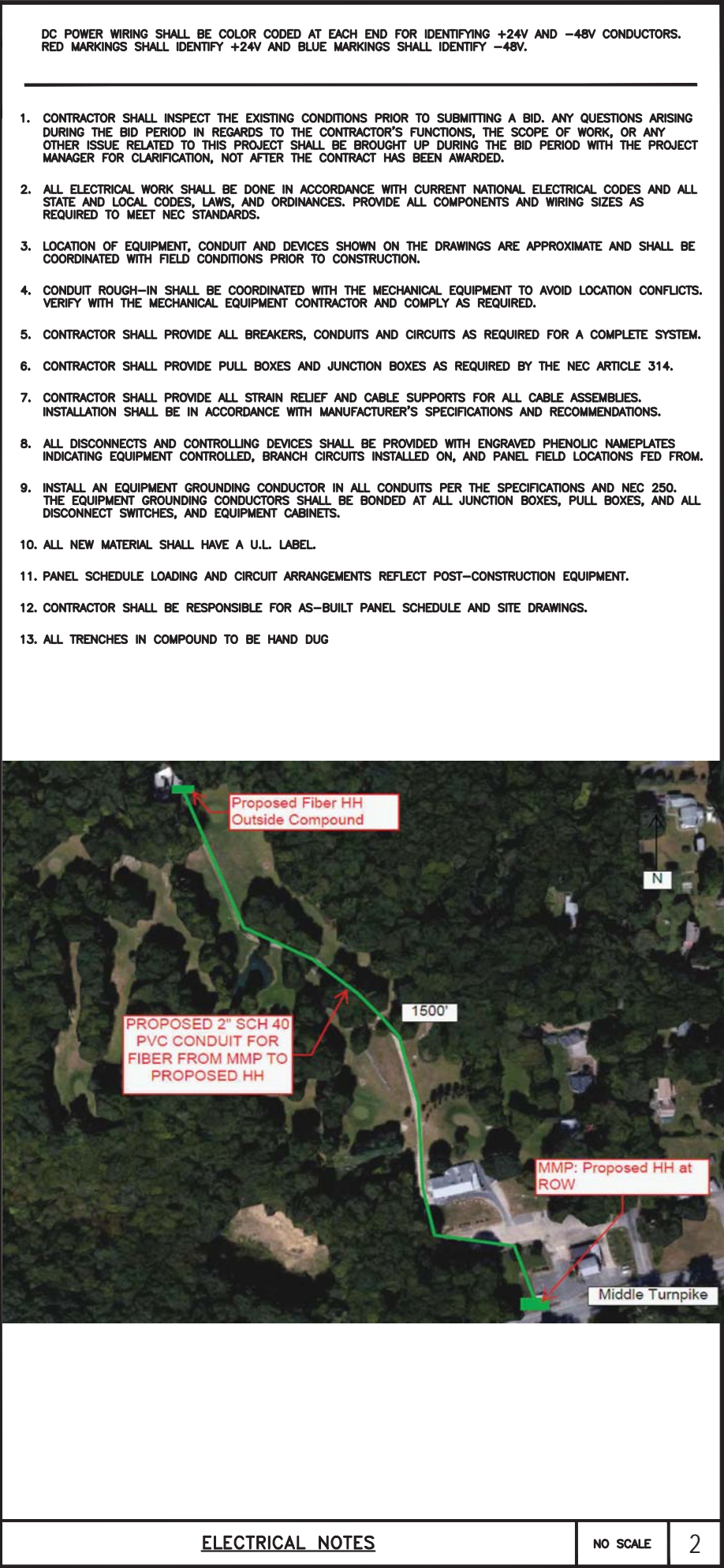
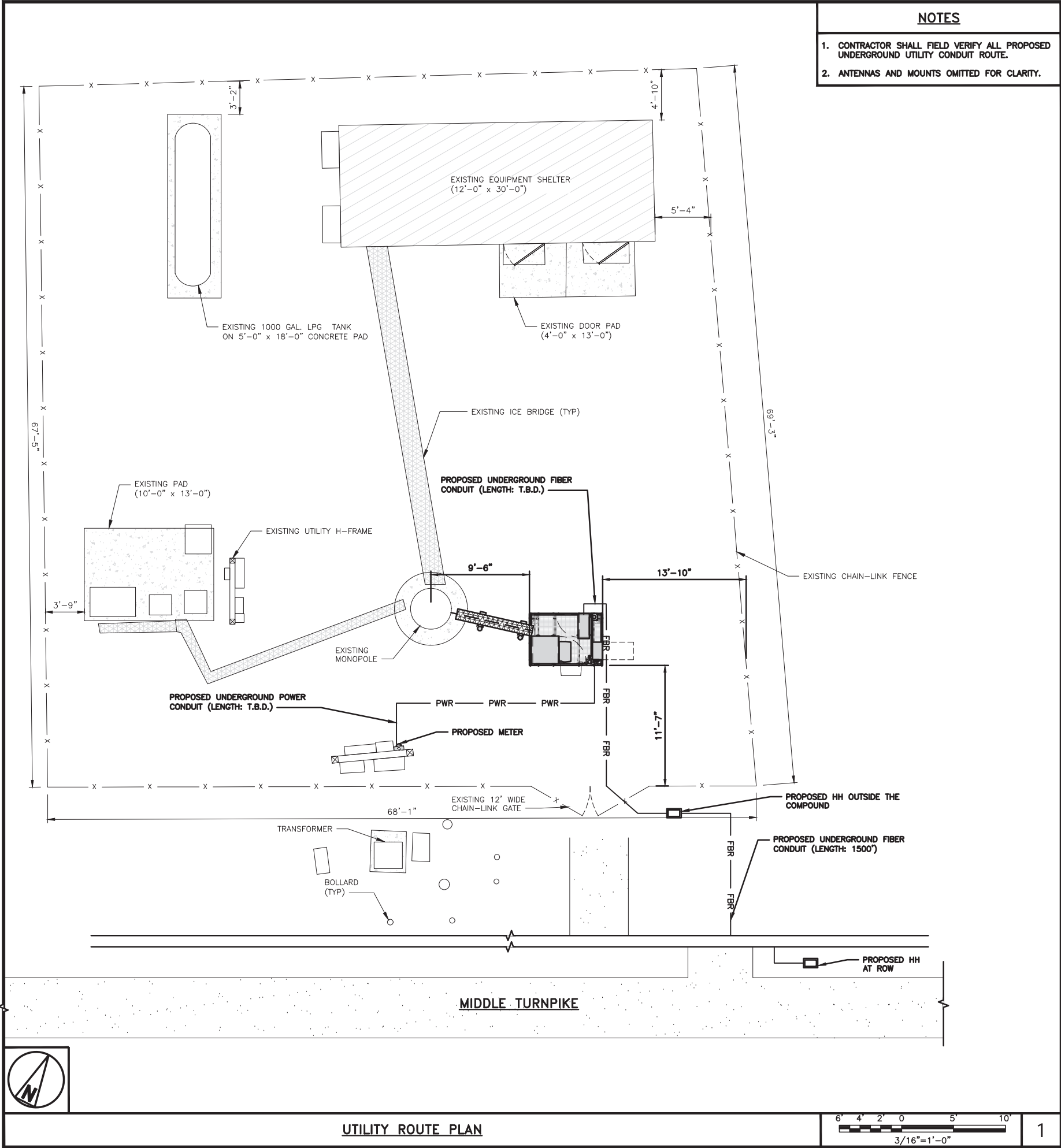
A&E PROJECT NUMBER
151918.001.01

DISH Wireless L.L.C.
PROJECT INFORMATION
BOBDL00066A
497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT
06268

SHEET TITLE
EQUIPMENT DETAILS

SHEET NUMBER

A-6



dish
wireless.

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LITTLETON, CO 80120

CROWN
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Professional Engineer
No. 23824
LICENSED
9/26/21

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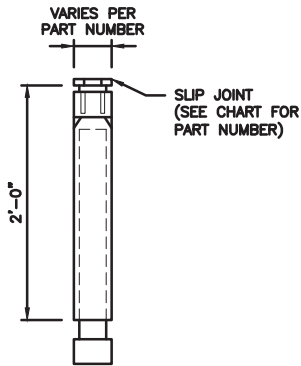
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497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT 06268

SHEET TITLE
ELECTRICAL/FIBER ROUTE
PLAN AND NOTES

SHEET NUMBER
E-1

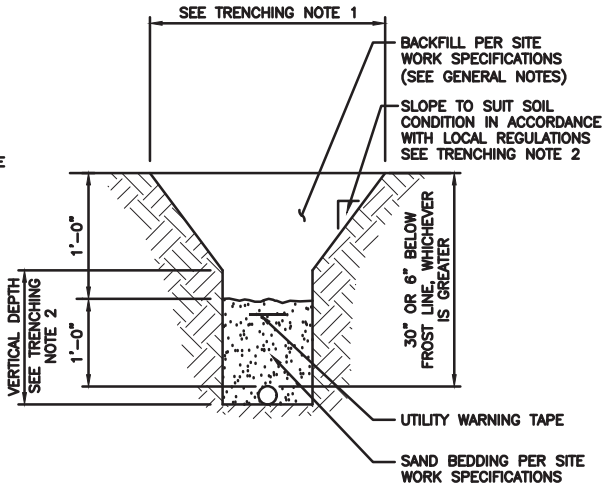
CARLON EXPANSION FITTINGS				
COUPLING END PART#	MALE TERMINAL ADAPTER END PART#	SIZE	STD CTN QTY.	TRAVEL LENGTH
E945D	E945DX	1/2"	20	4"
E945E	E945EX	3/4"	15	4"
E945F	E945FX	1"	10	4"
E945G	E945GX	1 1/4"	5	4"
E945H	E945HX	1 1/2"	5	4"
E945J	E945JX	2"	15	8"
E945K	E945KX	2 1/2"	10	8"
E945L	E945LX	3"	10	8"
E945M	E945MX	3 1/2"	5	8"
E945N	E945NX	4"	5	8"
E945P	E945PX	5"	1	8"
E945R	E945RX	6"	1	8"



NOTE:
CONTRACTOR TO INSTALL EXPANSION FITTING SLIP JOINT AT METER CENTER CONDUIT TERMINATION, AS PER LOCAL UTILITY POLICY, ORDINANCE AND/OR SPECIFIED REQUIREMENT.

TRENCHING NOTES

1. CONTRACTOR SHALL RESTORE THE TRENCH TO ITS ORIGINAL CONDITIONS BY EITHER SEEDING OR SODDING GRASS AREAS, OR REPLACING ASPHALT OR CONCRETE AREAS TO ITS ORIGINAL CROSS SECTION.
2. TRENCHING SAFETY; INCLUDING, BUT NOT LIMITED TO SOIL CLASSIFICATION, SLOPING, AND SHORING, SHALL BE GOVERNED BY THE CURRENT OSHA TRENCHING AND EXCAVATION SAFETY STANDARDS.
3. ALL CONDUITS SHALL BE INSTALLED IN COMPLIANCE WITH THE CURRENT NATIONAL ELECTRIC CODE (NEC) OR AS REQUIRED BY THE LOCAL JURISDICTION, WHICHEVER IS THE MOST STRINGENT.



EXPANSION JOINT DETAIL

NO SCALE

1

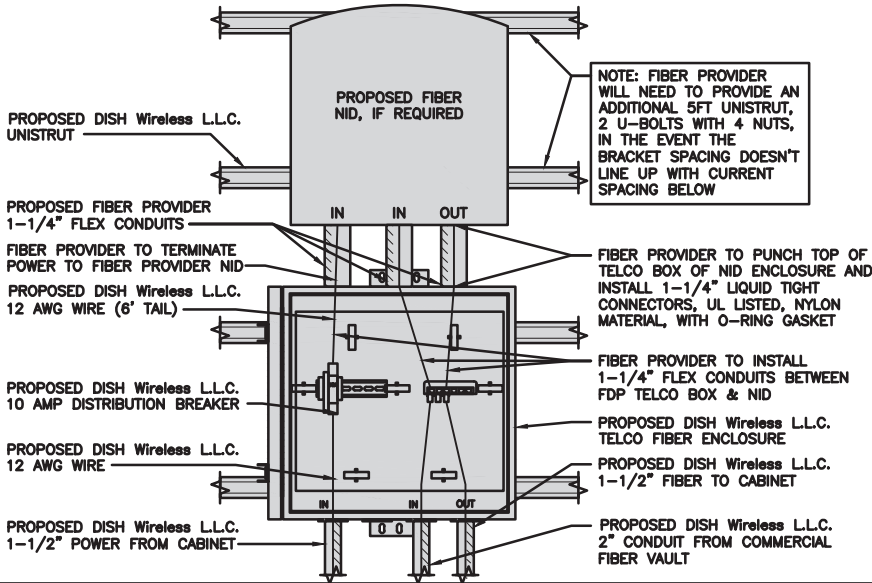
TYPICAL UNDERGROUND TRENCH DETAIL

NO SCALE

2

NO SCALE

3



LIT TELCO BOX – INTERIOR WIRING LAYOUT (OPTIONAL)

NO SCALE

4

NOT USED

NO SCALE

5

NOT USED

NO SCALE

6

NOT USED

NO SCALE

7

NOT USED

NO SCALE

8

NOT USED

NO SCALE

9



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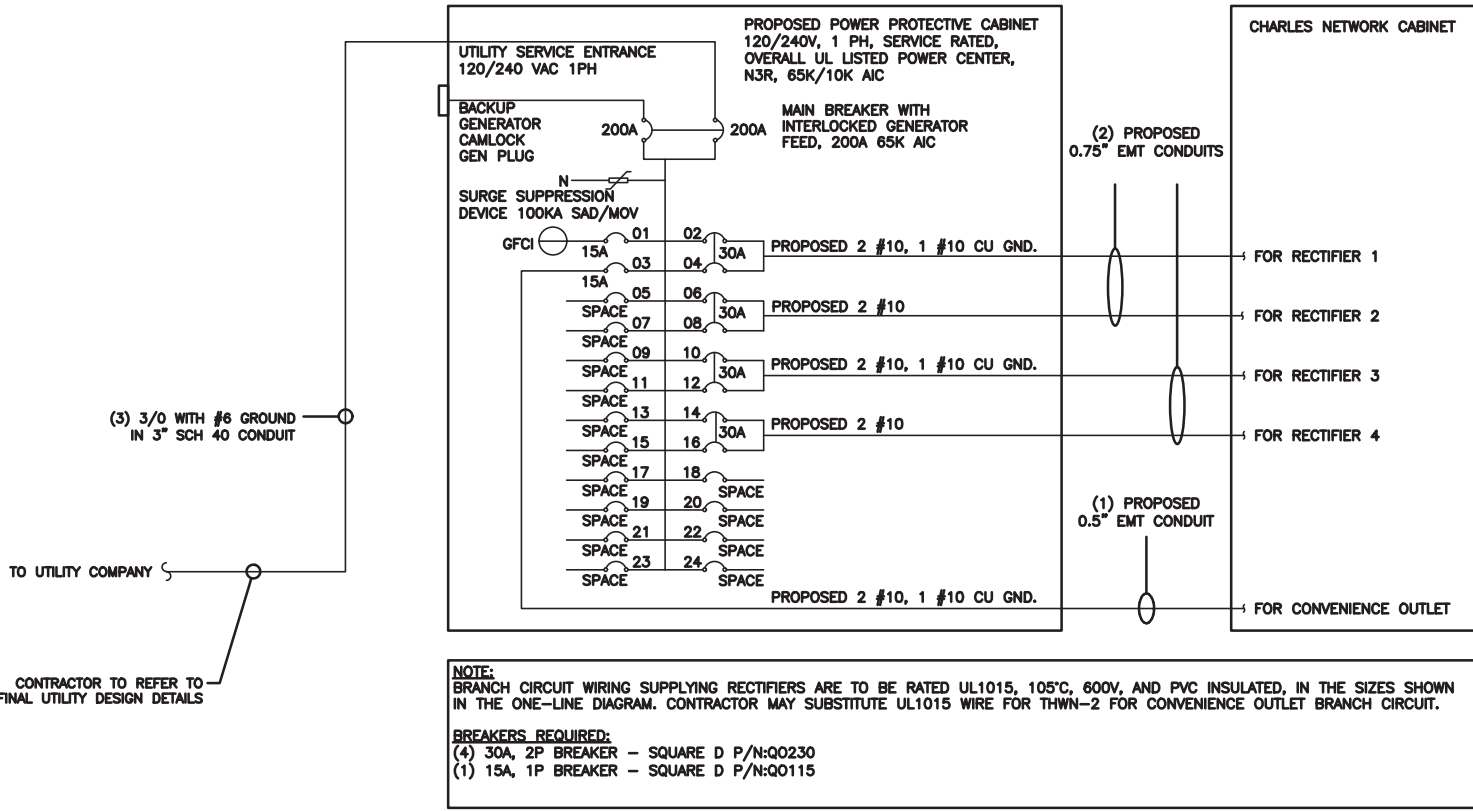
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PROJECT INFORMATION
BOBDL00066A
497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT
06268

SHEET TITLE
ELECTRICAL
DETAILS

SHEET NUMBER

E-2



NOTES

THE (2) CONDUITS WITH (4) CURRENT CARRYING CONDUCTORS EACH, SHALL APPLY THE ADJUSTMENT FACTOR OF 80% PER 2014/17 NEC TABLE 310.15(B)(3)(a) OR 2020 NEC TABLE 310.15(C)(1) FOR UL1015 WIRE.

#12 FOR 15A-20A/1P BREAKER: 0.8 x 30A = 24.0A
#10 FOR 25A-30A/2P BREAKER: 0.8 x 40A = 32.0A
#8 FOR 35A-40A/2P BREAKER: 0.8 x 55A = 44.0A
#6 FOR 45A-60A/2P BREAKER: 0.8 x 75A = 60.0A

CONDUIT SIZING: AT 40% FILL PER NEC CHAPTER 9, TABLE 4, ARTICLE 358.
0.5" CONDUIT - 0.122 SQ. IN AREA
0.75" CONDUIT - 0.213 SQ. IN AREA
2.0" CONDUIT - 1.316 SQ. IN AREA
3.0" CONDUIT - 2.907 SQ. IN AREA

CABINET CONVENIENCE OUTLET CONDUCTORS (1 CONDUIT): USING THWN-2, CU.
#10 - 0.0211 SQ. IN X 2 = 0.0422 SQ. IN
#10 - 0.0211 SQ. IN X 1 = 0.0211 SQ. IN <GROUND
TOTAL = 0.0633 SQ. IN

0.5" EMT CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (3) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

RECTIFIER CONDUCTORS (2 CONDUITS): USING UL1015, CU.
#10 - 0.0266 SQ. IN X 4 = 0.1064 SQ. IN
#10 - 0.0082 SQ. IN X 1 = 0.0082 SQ. IN <BARE GROUND
TOTAL = 0.1146 SQ. IN

0.75" EMT CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (5) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

PPC FEED CONDUCTORS (1 CONDUIT): USING THWN, CU.
3/0 - 0.2679 SQ. IN X 3 = 0.8037 SQ. IN
#6 - 0.0507 SQ. IN X 1 = 0.0507 SQ. IN <GROUND
TOTAL = 0.8544 SQ. IN

3.0" SCH 40 PVC CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (4) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.



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PROJECT INFORMATION
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497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT
06268

SHEET TITLE
ELECTRICAL ONE-LINE, FAULT
CALCS & PANEL SCHEDULE

SHEET NUMBER

E-3

PPC ONE-LINE DIAGRAM

NO SCALE

1

PROPOSED CHARLES PANEL SCHEDULE											
LOAD SERVED	VOLT AMPS (WATTS)		TRIP	CKT #	PHASE	CKT #	TRIP	VOLT AMPS (WATTS)		LOAD SERVED	
	L1	L2						L1	L2		
PPC GFCI OUTLET	180		15A	1	A	2	30A	2880		ABB/GE INFINITY RECTIFIER 1	
CHARLES GFCI OUTLET		180	15A	3	B	4			2880		
-SPACE-				5	A	6	30A	2880		ABB/GE INFINITY RECTIFIER 2	
-SPACE-				7	B	8			2880		
-SPACE-				9	A	10	30A	2880		ABB/GE INFINITY RECTIFIER 3	
-SPACE-				11	B	12			2880		
-SPACE-				13	A	14	30A	2880		ABB/GE INFINITY RECTIFIER 4	
-SPACE-				15	B	16			2880		
-SPACE-				17	A	18					
-SPACE-				19	A	20					
-SPACE-				21	B	22					
-SPACE-				23	B	24					
VOLTAGE AMPS	180	180						11520	11520		
200A MCB, 16, 24 SPACE, 120/240V				L1	L2						
MB RATING: 65,000 AIC				11700	11700			VOLTAGE AMPS			
				98	98			AMPS			
					98			MAX AMPS			
					123			MAX 125%			

PANEL SCHEDULE

NO SCALE

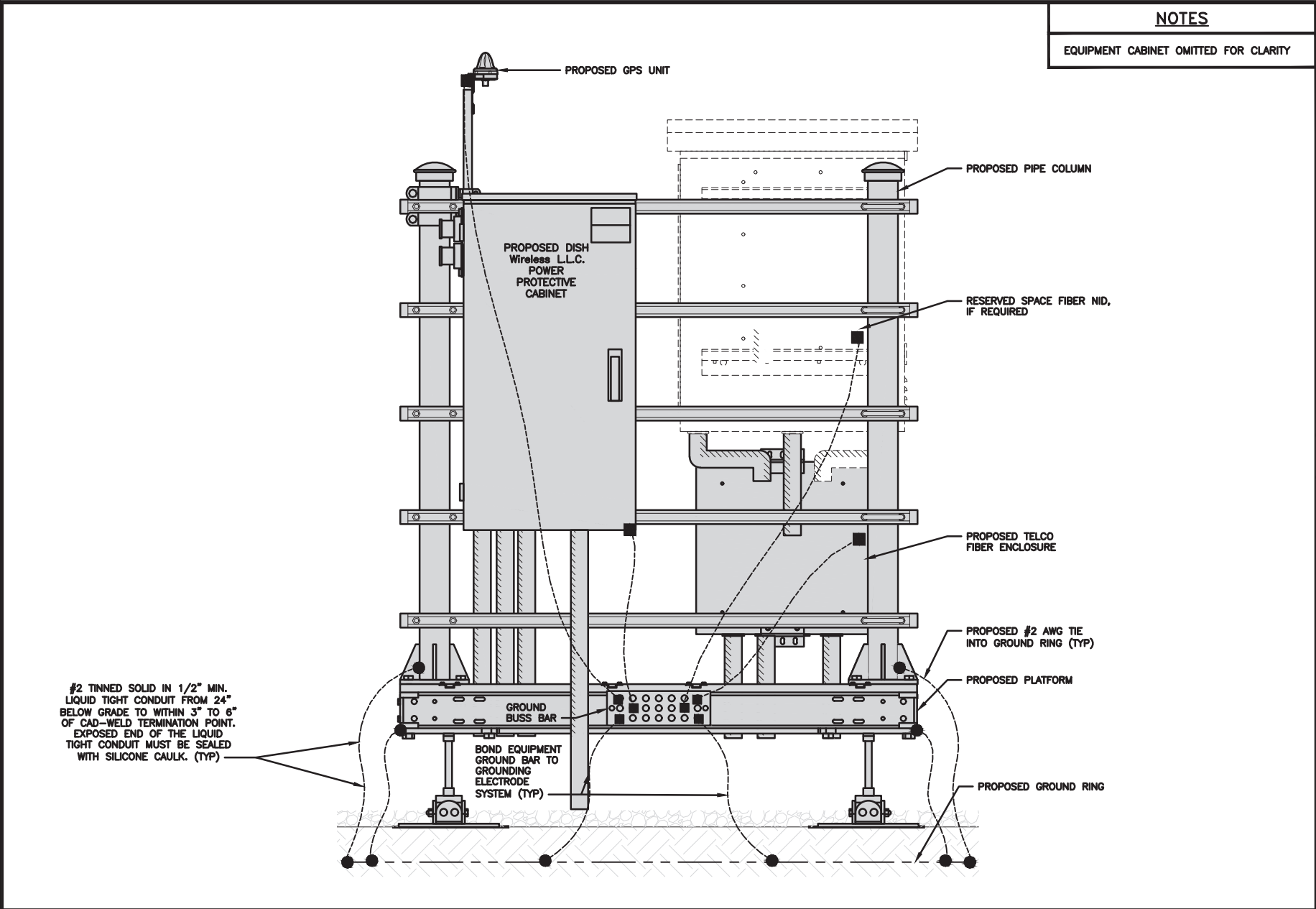
2

NOT USED

NO SCALE

3

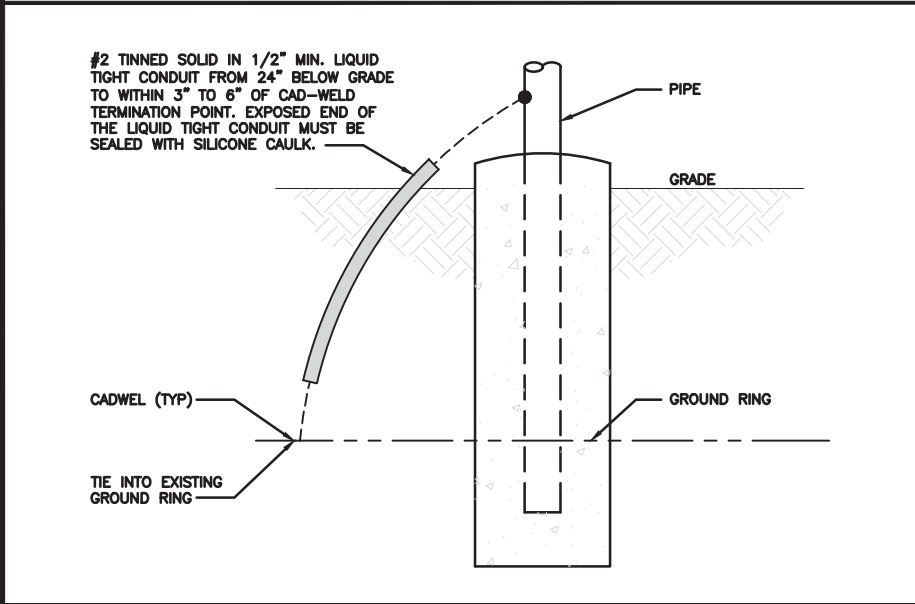
G-1



H-FRAME GROUNDING DETAIL

NO SCALE

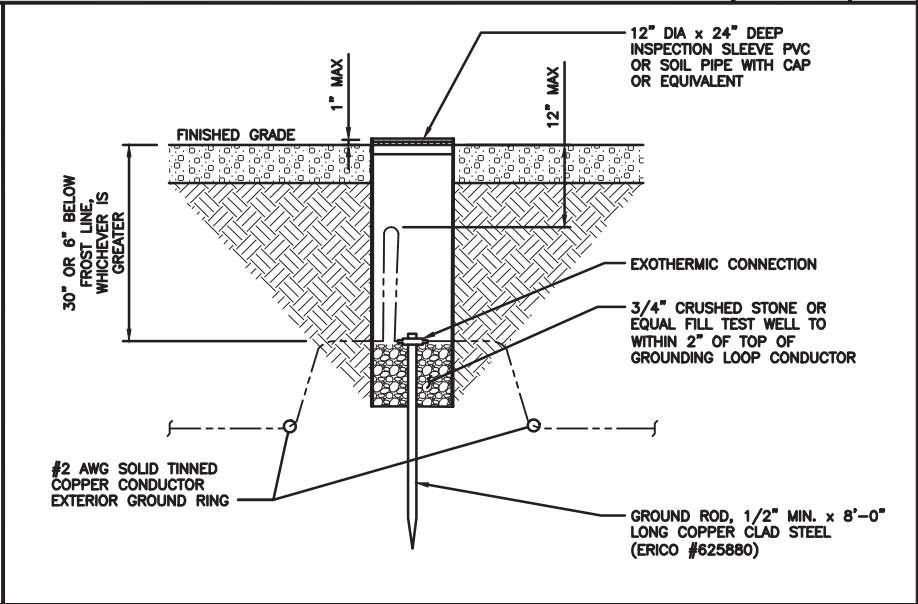
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TRANSITIONING GROUND DETAIL

NO SCALE

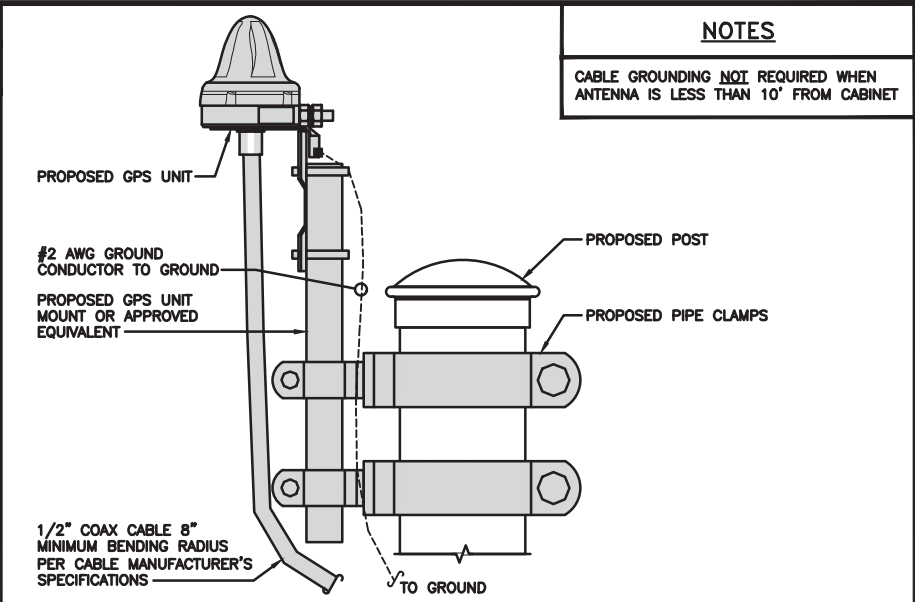
4



TYPICAL TEST GROUND ROD WITH INSPECTION SLEEVE

NO SCALE

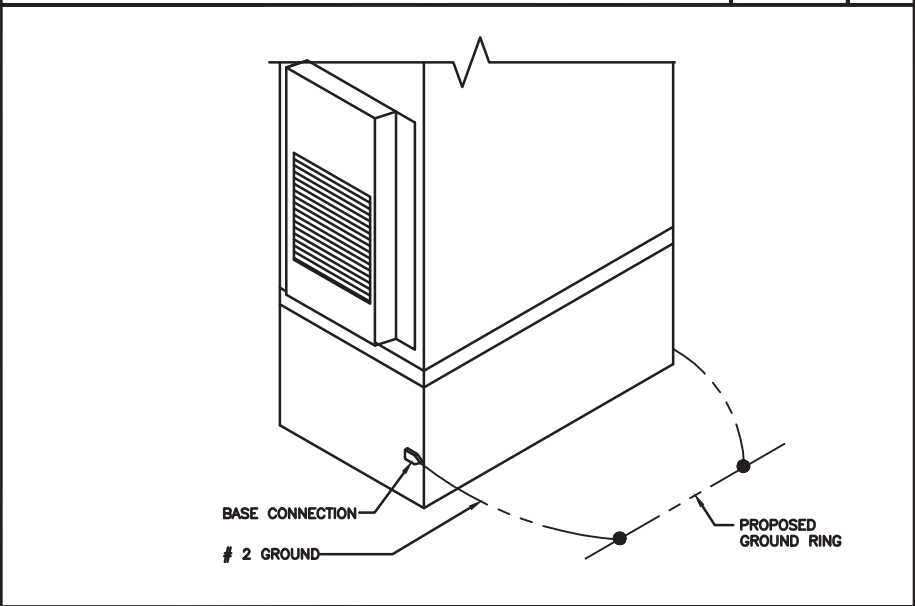
5



TYPICAL GPS UNIT GROUNDING

NO SCALE

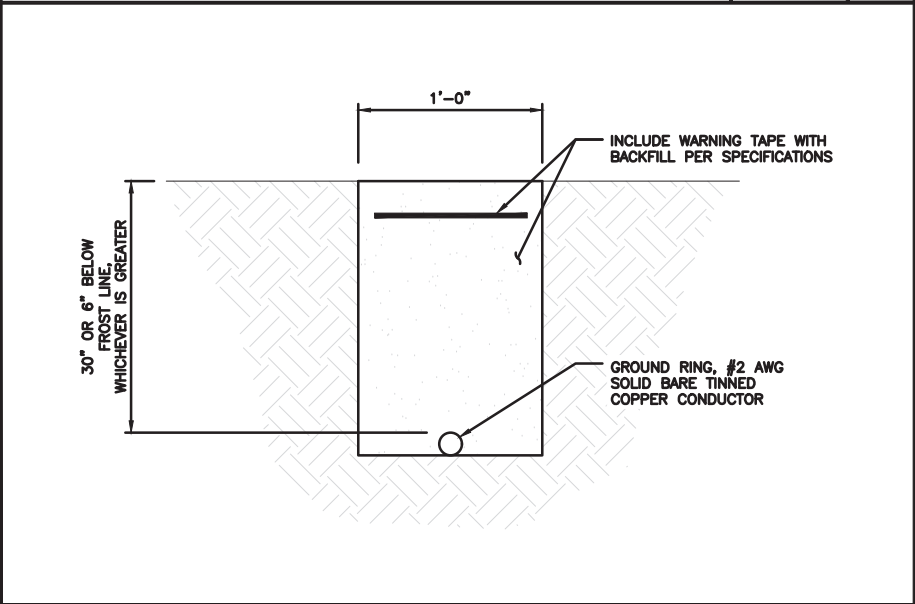
2



OUTDOOR CABINET GROUNDING

NO SCALE

3



TYPICAL GROUND RING TRENCH

NO SCALE

6

dish
wireless.

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CROWN
CASTLE

2000 CORPORATE DRIVE
CANONSBURG, PA 15317

B+T GRP

1717 S. BOULDER
SUITE 300
TULSA, OK 74119
PH: (918) 587-4630
www.btgrp.com



B&T ENGINEERING, INC.
PEC.0001564
Expires 2/10/22

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TO ALTER THIS DOCUMENT.

DRAWN BY: CHECKED BY: APPROVED BY:

JJR JHW MDW

RFDS REV #: 0

CONSTRUCTION
DOCUMENTS

SUBMITTALS		
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A&E PROJECT NUMBER
151918.001.01

DISH Wireless L.L.C.
PROJECT INFORMATION
BOBDL00066A
497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT
06268

SHEET TITLE
GROUNDING DETAILS

SHEET NUMBER

G-2

<div>1. EXOTHERMIC WELD (2) TWO, #2 AWG BARE TINNED SOLID COPPER CONDUCTORS TO GROUND BAR. ROUTE CONDUCTORS TO BURIED GROUND RING AND PROVIDE PARALLEL EXOTHERMIC WELD.</div> <div>2. ALL EXTERIOR GROUNDING HARDWARE SHALL BE STAINLESS STEEL 3/8" DIAMETER OR LARGER. ALL HARDWARE 18-8 STAINLESS STEEL INCLUDING LOCK WASHERS, COAT ALL SURFACES WITH AN ANTI-OXIDANT COMPOUND BEFORE MATING.</div> <div>3. FOR GROUND BOND TO STEEL ONLY: COAT ALL SURFACES WITH AN ANTI-OXIDANT COMPOUND BEFORE MATING.</div> <div>4. DO NOT INSTALL CABLE GROUNDING KIT AT A BEND AND ALWAYS DIRECT GROUND CONDUCTOR DOWN TO GROUNDING BUS.</div> <div>5. NUT & WASHER SHALL BE PLACED ON THE FRONT SIDE OF THE GROUND BAR AND BOLTED ON THE BACK SIDE.</div> <div>6. ALL GROUNDING PARTS AND EQUIPMENT TO BE SUPPLIED AND INSTALLED BY CONTRACTOR.</div> <div>7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING ADDITIONAL GROUND BAR AS REQUIRED.</div> <div>8. ENSURE THE WIRE INSULATION TERMINATION IS WITHIN 1/8" OF THE BARREL (NO SHINERS).</div>			<div></div>			<div></div>		
TYPICAL GROUNDING NOTES			TYPICAL EXTERIOR TWO HOLE LUG			TYPICAL INTERIOR TWO HOLE LUG		
<div></div>								
LUG DETAIL			NOT USED			NOT USED		
NOT USED			NOT USED			NOT USED		



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SHEET TITLE
GROUNDING DETAILS

SHEET NUMBER

G-3

RF JUMPER COLOR CODING				3/4" TAPE WIDTHS WITH 3/4" SPACING								
LOW-BAND RRH – (600MHz N71 BASEBAND) + (850MHz N26 BAND) + (700MHz N29 BAND) – OPTIONAL PER MARKET	ALPHA RRH				BETA RRH				GAMMA RRH			
	PORT 1 + SLANT	PORT 2 – SLANT	PORT 3 + SLANT	PORT 4 – SLANT	PORT 1 + SLANT	PORT 2 – SLANT	PORT 3 + SLANT	PORT 4 – SLANT	PORT 1 + SLANT	PORT 2 – SLANT	PORT 3 + SLANT	PORT 4 – SLANT
	RED	RED	RED	RED	BLUE	BLUE	BLUE	BLUE	GREEN	GREEN	GREEN	GREEN
	ORANGE	ORANGE	RED	RED	ORANGE	ORANGE	BLUE	BLUE	ORANGE	ORANGE	GREEN	GREEN
		WHITE (-) PORT	ORANGE	ORANGE		WHITE (-) PORT	ORANGE	ORANGE		WHITE (-) PORT	ORANGE	ORANGE
ADD FREQUENCY COLOR TO SECTOR BAND (CBRS WILL USE YELLOW BANDS)				WHITE (-) PORT				WHITE (-) PORT				WHITE (-) PORT
MID-BAND RRH – (AWS BANDS N66+N70)	RED	RED	RED	RED	BLUE	BLUE	BLUE	BLUE	GREEN	GREEN	GREEN	GREEN
	PURPLE	PURPLE	RED	RED	PURPLE	PURPLE	BLUE	BLUE	PURPLE	PURPLE	GREEN	GREEN
		WHITE (-) PORT	PURPLE	PURPLE		WHITE (-) PORT	PURPLE	PURPLE		WHITE (-) PORT	PURPLE	PURPLE
				WHITE (-) PORT				WHITE (-) PORT				WHITE (-) PORT
HYBRID/DISCREET CABLES	EXAMPLE 1		EXAMPLE 2		EXAMPLE 3							
	RED		RED		RED							
	BLUE		BLUE									
	GREEN		GREEN		ORANGE							
	ORANGE		YELLOW		PURPLE							
INCLUDE SECTOR BANDS BEING SUPPORTED ALONG WITH FREQUENCY BANDS												
EXAMPLE 1 – HYBRID, OR DISCREET, SUPPORTS ALL SECTORS, BOTH LOW-BANDS AND MID-BANDS												
EXAMPLE 2 – HYBRID, OR DISCREET, SUPPORTS CBRS ONLY, ALL SECTORS												
FIBER JUMPERS TO RRHs	LOW BAND RRH		HIGH BAND RRH		LOW BAND RRH		HIGH BAND RRH		LOW BAND RRH		HIGH BAND RRH	
	RED		RED		BLUE		BLUE		GREEN		GREEN	
			PURPLE				PURPLE				PURPLE	
POWER CABLES TO RRHs	LOW BAND RRH		HIGH BAND RRH		LOW BAND RRH		HIGH BAND RRH		LOW BAND RRH		HIGH BAND RRH	
	RED		RED		BLUE		BLUE		GREEN		GREEN	
			PURPLE				PURPLE				PURPLE	
RET MOTORS AT ANTENNAS	ANTENNA 1 LOW BAND/ "IN"		ANTENNA 1 HIGH BAND/ "IN"		ANTENNA 1 LOW BAND/ "IN"		ANTENNA 1 HIGH BAND/ "IN"		ANTENNA 1 LOW BAND/ "IN"		ANTENNA 1 HIGH BAND/ "IN"	
	RED		RED		BLUE		BLUE		GREEN		GREEN	
			PURPLE				PURPLE				PURPLE	
MICROWAVE RADIO LINKS	FORWARD AZIMUTH OF 0–120 DEGREES				FORWARD AZIMUTH OF 120–240 DEGREES				FORWARD AZIMUTH OF 240–360 DEGREES			
	PRIMARY		SECONDARY		PRIMARY		SECONDARY		PRIMARY		SECONDARY	
	WHITE		WHITE		WHITE		WHITE		WHITE		WHITE	
	RED		RED		BLUE		BLUE		GREEN		GREEN	
	WHITE		WHITE		WHITE		WHITE		WHITE		WHITE	
MICROWAVE CABLES WILL REQUIRE P-TOUCH LABELS INSIDE THE CABINET TO IDENTIFY THE LOCAL AND REMOTE SITE ID'S												

RF CABLE COLOR CODES

NO SCALE

1

NOT USED

NO SCALE

4

LOW BANDS (N71+N26)
OPTIONAL – (N29)

ORANGE

AWS
(N66+N70+H-BLOCK)

PURPLE

CBRS TECH
(3 GHz)

YELLOW

NEGATIVE SLANT PORT
ON ANT/RRH

WHITE

ALPHA SECTOR

RED

BETA SECTOR

BLUE

GAMMA SECTOR

GREEN

COLOR IDENTIFIER

NO SCALE

2

NOT USED

NO SCALE

3

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wireless.

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497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT
06268

SHEET TITLE
RF
CABLE COLOR CODES

SHEET NUMBER

RF-1

EXOTHERMIC CONNECTION
MECHANICAL CONNECTION
BUSS BAR INSULATOR
CHEMICAL ELECTROLYTIC GROUNDING SYSTEM
TEST CHEMICAL ELECTROLYTIC GROUNDING SYSTEM
EXOTHERMIC WITH INSPECTION SLEEVE
GROUNDING BAR
GROUND ROD
TEST GROUND ROD WITH INSPECTION SLEEVE

SINGLE POLE SWITCH

DUPLEX RECEPTACLE

DUPLEX GFCI RECEPTACLE

FLUORESCENT LIGHTING FIXTURE
(2) TWO LAMPS 48-T8

SMOKE DETECTION (DC)

EMERGENCY LIGHTING (DC)

SECURITY LIGHT W/PHOTOCELL LITHONIA ALXW
LED-1-25A400/51K-SR4-120-PE-DEBTD

CHAIN LINK FENCE

WOOD/WROUGHT IRON FENCE

WALL STRUCTURE

LEASE AREA

PROPERTY LINE (PL)

SETBACKS

ICE BRIDGE

CABLE TRAY

WATER LINE

UNDERGROUND POWER

UNDERGROUND TELCO

OVERHEAD POWER

OVERHEAD TELCO

UNDERGROUND TELCO/POWER

ABOVE GROUND POWER

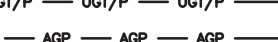
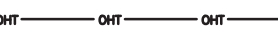
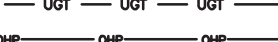
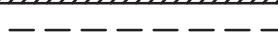
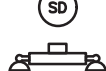
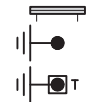
ABOVE GROUND TELCO

ABOVE GROUND TELCO/POWER

WORKPOINT

SECTION REFERENCE

DETAIL REFERENCE



AB ANCHOR BOLT
ABV ABOVE
AC ALTERNATING CURRENT
ADDL ADDITIONAL
AFF ABOVE FINISHED FLOOR
AFG ABOVE FINISHED GRADE
AGL ABOVE GROUND LEVEL
AIC AMPERAGE INTERRUPTION CAPACITY
ALUM ALUMINUM
ALT ALTERNATE
ANT ANTENNA
APPROX APPROXIMATE
ARCH ARCHITECTURAL
ATS AUTOMATIC TRANSFER SWITCH
AWG AMERICAN WIRE GAUGE
BATT BATTERY
BLDG BUILDING
BLK BLOCK
BLKG BLOCKING
BM BEAM
BTC BARE TINNED COPPER CONDUCTOR
BOF BOTTOM OF FOOTING
CAB CABINET
CANT CANTILEVERED
CHG CHARGING
CLG CEILING
CLR CLEAR
COL COLUMN
COMM COMMON
CONC CONCRETE
CONSTR CONSTRUCTION
DBL DOUBLE
DC DIRECT CURRENT
DEPT DEPARTMENT
DF DOUGLAS FIR
DIA DIAMETER
DIAG DIAGONAL
DIM DIMENSION
DWG DRAWING
DWL DOWEL
EA EACH
EC ELECTRICAL CONDUCTOR
EL ELEVATION
ELEC ELECTRICAL
EMT ELECTRICAL METALLIC TUBING
ENG ENGINEER
EQ EQUAL
EXP EXPANSION
EXT EXTERIOR
EW EACH WAY
FAB FABRICATION
FF FINISH FLOOR
FG FINISH GRADE
FIF FACILITY INTERFACE FRAME
FIN FINISH(ED)
FLR FLOOR
FDN FOUNDATION
FOC FACE OF CONCRETE
FOM FACE OF MASONRY
FOS FACE OF STUD
FOW FACE OF WALL
FS FINISH SURFACE
FT FOOT
FTG FOOTING
GA GAUGE
GEN GENERATOR
GFCI GROUND FAULT CIRCUIT INTERRUPTER
GLB GLUE LAMINATED BEAM
GLV GALVANIZED
GPS GLOBAL POSITIONING SYSTEM
GND GROUND
GSM GLOBAL SYSTEM FOR MOBILE
HDG HOT DIPPED GALVANIZED
HDR HEADER
HGR HANGER
HVAC HEAT/VENTILATION/AIR CONDITIONING
HT HEIGHT
IGR INTERIOR GROUND RING

IN INCH
INT INTERIOR
LB(S) POUND(S)
LF LINEAR FEET
LTE LONG TERM EVOLUTION
MAS MASONRY
MAX MAXIMUM
MB MACHINE BOLT
MECH MECHANICAL
MFR MANUFACTURER
MGB MASTER GROUND BAR
MIN MINIMUM
MISC MISCELLANEOUS
MTL METAL
MTS MANUAL TRANSFER SWITCH
MW MICROWAVE
NEC NATIONAL ELECTRIC CODE
NM NEWTON METERS
NO. NUMBER
NUMBER
NTS NOT TO SCALE
OC ON-CENTER
OSHA OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION
OPNG OPENING
P/C PRECAST CONCRETE
PCS PERSONAL COMMUNICATION SERVICES
PCU PRIMARY CONTROL UNIT
PRC PRIMARY RADIO CABINET
PP POLARIZING PRESERVING
PSF POUNDS PER SQUARE FOOT
PSI POUNDS PER SQUARE INCH
PT PRESSURE TREATED
PWR POWER CABINET
QTY QUANTITY
RAD RADIUS
RECT RECTIFIER
REF REFERENCE
REINF REINFORCEMENT
REQ'D REQUIRED
RET REMOTE ELECTRIC TILT
RF RADIO FREQUENCY
RMC RIGID METALLIC CONDUIT
RRH REMOTE RADIO HEAD
RRU REMOTE RADIO UNIT
RWY RACEWAY
SCH SCHEDULE
SHT SHEET
SIAD SMART INTEGRATED ACCESS DEVICE
SIM SIMILAR
SPEC SPECIFICATION
SQ SQUARE
SS STAINLESS STEEL
STD STANDARD
STL STEEL
TEMP TEMPORARY
THK THICKNESS
TMA TOWER MOUNTED AMPLIFIER
TN TOE NAIL
TOA TOP OF ANTENNA
TOC TOP OF CURB
TOF TOP OF FOUNDATION
TOP TOP OF PLATE (PARAPET)
TOS TOP OF STEEL
TOW TOP OF WALL
TVSS TRANSIENT VOLTAGE SURGE SUPPRESSION
TYP TYPICAL
UG UNDERGROUND
UL UNDERWRITERS LABORATORY
UNO UNLESS NOTED OTHERWISE
UMTS UNIVERSAL MOBILE TELECOMMUNICATIONS SYSTEM
UPS UNINTERRUPTIBLE POWER SYSTEM (DC POWER PLANT)
VIF VERIFIED IN FIELD
W WIDE
W/ WITH
WD WOOD
WP WEATHERPROOF
WT WEIGHT

dish
wireless.

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497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT
06268

SHEET TITLE
LEGEND AND
ABBREVIATIONS

SHEET NUMBER

GN-1

LEGEND

ABBREVIATIONS

SITE ACTIVITY REQUIREMENTS:

1. NOTICE TO PROCEED – NO WORK SHALL COMMENCE PRIOR TO CONTRACTOR RECEIVING A WRITTEN NOTICE TO PROCEED (NTP) AND THE ISSUANCE OF A PURCHASE ORDER. PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE DISH Wireless L.L.C. AND TOWER OWNER NOC & THE DISH Wireless L.L.C. AND TOWER OWNER CONSTRUCTION MANAGER.
2. "LOOK UP" – DISH Wireless L.L.C. AND TOWER OWNER SAFETY CLIMB REQUIREMENT:

THE INTEGRITY OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY ON THE STRUCTURE. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO: PINCHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM ITS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTIONAL WEAR, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO IMPEDE/BLOCK ITS INTENDED USE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGGED OUT AND REPORTED TO YOUR DISH Wireless L.L.C. AND DISH Wireless L.L.C. AND TOWER OWNER POC OR CALL THE NOC TO GENERATE A SAFETY CLIMB MAINTENANCE AND CONTRACTOR NOTICE TICKET.
3. PRIOR TO THE START OF CONSTRUCTION, ALL REQUIRED JURISDICTIONAL PERMITS SHALL BE OBTAINED. THIS INCLUDES, BUT IS NOT LIMITED TO, BUILDING, ELECTRICAL, MECHANICAL, FIRE, FLOOD ZONE, ENVIRONMENTAL, AND ZONING. AFTER ONSITE ACTIVITIES AND CONSTRUCTION ARE COMPLETED, ALL REQUIRED PERMITS SHALL BE SATISFIED AND CLOSED OUT ACCORDING TO LOCAL JURISDICTIONAL REQUIREMENTS.
4. ALL CONSTRUCTION MEANS AND METHODS; INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS, AND RESCUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONTAINED HEREIN, AND SHALL MEET ANSI/ASSE A10.48 (LATEST EDITION); FEDERAL, STATE, AND LOCAL REGULATIONS; AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL ADHERE TO ANSI/ASSE A10.48 (LATEST EDITION) AND DISH Wireless L.L.C. AND TOWER OWNER STANDARDS, INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION, TO CERTIFY THE SUPPORTING STRUCTURE(S) IN ACCORDANCE WITH ANSI/TIA–322 (LATEST EDITION).
5. ALL SITE WORK TO COMPLY WITH DISH Wireless L.L.C. AND TOWER OWNER INSTALLATION STANDARDS FOR CONSTRUCTION ACTIVITIES ON DISH Wireless L.L.C. AND TOWER OWNER TOWER SITE AND LATEST VERSION OF ANSI/TIA–1019–A–2012 "STANDARD FOR INSTALLATION, ALTERATION, AND MAINTENANCE OF ANTENNA SUPPORTING STRUCTURES AND ANTENNAS."
6. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY DISH Wireless L.L.C. AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
8. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER’S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
9. THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES INCLUDING PRIVATE LOCATES SERVICES PRIOR TO THE START OF CONSTRUCTION.
10. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION E) CONSTRUCTION SAFETY PROCEDURES.
11. ALL SITE WORK SHALL BE AS INDICATED ON THE STAMPED CONSTRUCTION DRAWINGS AND DISH PROJECT SPECIFICATIONS, LATEST APPROVED REVISION.
12. CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH AT THE COMPLETION OF THE WORK. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
13. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF DISH Wireless L.L.C. AND TOWER OWNER, AND/OR LOCAL UTILITIES.
14. THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE TECHNICAL SPECIFICATION FOR SITE SIGNAGE REQUIRED BY LOCAL JURISDICTION AND SIGNAGE REQUIRED ON INDIVIDUAL PIECES OF EQUIPMENT, ROOMS, AND SHELTERS.
15. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE CARRIER’S EQUIPMENT AND TOWER AREAS.
16. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
17. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION AS SPECIFIED ON THE CONSTRUCTION DRAWINGS AND/OR PROJECT SPECIFICATIONS.
18. CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.
19. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR’S EXPENSE TO THE SATISFACTION OF OWNER.
20. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS AND RADIOS REMOVED SHALL BE RETURNED TO THE OWNER’S DESIGNATED LOCATION.
21. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.
22. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.

GENERAL NOTES:

- 1.FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:

CONTRACTOR:GENERAL CONTRACTOR RESPONSIBLE FOR CONSTRUCTION

CARRIER:DISH Wireless L.L.C.

TOWER OWNER:TOWER OWNER
2. THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE ENGINEERS IN THIS OR SIMILAR LOCALITIES. IT IS ASSUMED THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKPEOPLE WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.
3. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY FOR PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, FORMWORK, SHORING, ETC. SITE VISITS BY THE ENGINEER OR HIS REPRESENTATIVE WILL NOT INCLUDE INSPECTION OF THESE ITEMS AND IS FOR STRUCTURAL OBSERVATION OF THE FINISHED STRUCTURE ONLY.
4. NOTES AND DETAILS IN THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE GREATER, MORE STRICT REQUIREMENTS, SHALL GOVERN. IF FURTHER CLARIFICATION IS REQUIRED CONTACT THE ENGINEER OF RECORD.
5. SUBSTANTIAL EFFORT HAS BEEN MADE TO PROVIDE ACCURATE DIMENSIONS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.
6. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CARRIER POC AND TOWER OWNER.
7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
8. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
9. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER’S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
10. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CARRIER AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
11. CONTRACTOR IS TO PERFORM A SITE INVESTIGATION, BEFORE SUBMITTING BIDS, TO DETERMINE THE BEST ROUTING OF ALL CONDUITS FOR POWER, AND TELCO AND FOR GROUNDING CABLES AS SHOWN IN THE POWER, TELCO, AND GROUNDING PLAN DRAWINGS.
12. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR’S EXPENSE TO THE SATISFACTION OF DISH Wireless L.L.C. AND TOWER OWNER
13. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER’S DESIGNATED LOCATION.
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JJR JHW MDW

RFDS REV #: 0

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DISH Wireless L.L.C.
PROJECT INFORMATION
BOBDL00066A
497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT
06268

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

GN-2

CONCRETE, FOUNDATIONS, AND REINFORCING STEEL:

1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST-IN-PLACE CONCRETE.
2. UNLESS NOTED OTHERWISE, SOIL BEARING PRESSURE USED FOR DESIGN OF SLABS AND FOUNDATIONS IS ASSUMED TO BE 1000 psf.
3. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f'c) OF 3000 psi AT 28 DAYS, UNLESS NOTED OTHERWISE. NO MORE THAN 90 MINUTES SHALL ELAPSE FROM BATCH TIME TO TIME OF PLACEMENT UNLESS APPROVED BY THE ENGINEER OF RECORD. TEMPERATURE OF CONCRETE SHALL NOT EXCEED 90°f AT TIME OF PLACEMENT.
4. CONCRETE EXPOSED TO FREEZE-THAW CYCLES SHALL CONTAIN AIR ENTRAINING ADMIXTURES. AMOUNT OF AIR ENTRAINMENT TO BE BASED ON SIZE OF AGGREGATE AND F3 CLASS EXPOSURE (VERY SEVERE). CEMENT USED TO BE TYPE II PORTLAND CEMENT WITH A MAXIMUM WATER-TO-CEMENT RATIO (W/C) OF 0.45.
5. ALL STEEL REINFORCING SHALL CONFORM TO ASTM A615. ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185. ALL SPLICES SHALL BE CLASS "B" TENSION SPLICES, UNLESS NOTED OTHERWISE. ALL HOOKS SHALL BE STANDARD 90 DEGREE HOOKS, UNLESS NOTED OTHERWISE. YIELD STRENGTH (Fy) OF STANDARD DEFORMED BARS ARE AS FOLLOWS:
#4 BARS AND SMALLER 40 ksi
#5 BARS AND LARGER 60 ksi
6. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:
 - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3"
 - CONCRETE EXPOSED TO EARTH OR WEATHER:
 - #6 BARS AND LARGER 2"
 - #5 BARS AND SMALLER 1-1/2"
 - CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
 - SLAB AND WALLS 3/4"
 - BEAMS AND COLUMNS 1-1/2"
7. A TOOLED EDGE OR A 3/4" CHAMFER SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNLESS NOTED OTHERWISE, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.

ELECTRICAL INSTALLATION NOTES:

1. ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES/ORDINANCES.
2. CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED AND TRIP HAZARDS ARE ELIMINATED.
3. WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC.
4. ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC.
- 4.1. ALL EQUIPMENT SHALL BEAR THE UNDERWRITERS LABORATORIES LABEL OF APPROVAL, AND SHALL CONFORM TO REQUIREMENT OF THE NATIONAL ELECTRICAL CODE.
- 4.2. ALL OVERCURRENT DEVICES SHALL HAVE AN INTERRUPTING CURRENT RATING THAT SHALL BE GREATER THAN THE SHORT CIRCUIT CURRENT TO WHICH THEY ARE SUBJECTED, 22,000 AIC MINIMUM. VERIFY AVAILABLE SHORT CIRCUIT CURRENT DOES NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRENT ADOPTED CODE PRE THE GOVERNING JURISDICTION.
5. EACH END OF EVERY POWER PHASE CONDUCTOR, GROUNDING CONDUCTOR, AND TELCO CONDUCTOR OR CABLE SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2" PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA.
6. ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH LAMICOID TAGS SHOWING THEIR RATED VOLTAGE, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING AND BRANCH CIRCUIT ID NUMBERS (i.e. PANEL BOARD AND CIRCUIT ID'S).
7. PANEL BOARDS (ID NUMBERS) SHALL BE CLEARLY LABELED WITH PLASTIC LABELS.
8. TIE WRAPS ARE NOT ALLOWED.
9. ALL POWER AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE COPPER CONDUCTOR (#14 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
10. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE COPPER CONDUCTOR (#6 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
11. POWER AND CONTROL WIRING IN FLEXIBLE CORD SHALL BE MULTI-CONDUCTOR, TYPE SOOW CORD (#14 OR LARGER) UNLESS OTHERWISE SPECIFIED.
12. POWER AND CONTROL WIRING FOR USE IN CABLE TRAY SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#14 OR LARGER), WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
13. ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRE NUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRE NUTS SHALL BE RATED FOR OPERATION NOT LESS THAN 75° C (90° C IF AVAILABLE).
14. RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.
15. ELECTRICAL METALLIC TUBING (EMT), INTERMEDIATE METAL CONDUIT (IMC), OR RIGID METAL CONDUIT (RMC) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.

16. ELECTRICAL METALLIC TUBING (EMT) OR METAL-CLAD CABLE (MC) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.
17. SCHEDULE 40 PVC UNDERGROUND ON STRAIGHTS AND SCHEDULE 80 PVC FOR ALL ELBOWS/90s AND ALL APPROVED ABOVE GRADE PVC CONDUIT.
18. LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.
19. CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION-TYPE AND APPROVED FOR THE LOCATION USED. SET SCREW FITTINGS ARE NOT ACCEPTABLE.
20. CABINETS, BOXES AND WIRE WAYS SHALL BE LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND THE NEC.
21. WIREWAYS SHALL BE METAL WITH AN ENAMEL FINISH AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARDS (WIREMOLD SPECMATE WIREWAY).
22. SLOTTED WIRING DUCT SHALL BE PVC AND INCLUDE COVER (PANDUIT TYPE E OR EQUAL).
23. CONDUITS SHALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-PERFORATED STRAPS AND HANGERS. EXPLOSIVE DEVICES (i.e. POWDER-ACTUATED) FOR ATTACHING HANGERS TO STRUCTURE WILL NOT BE PERMITTED. CLOSELY FOLLOW THE LINES OF THE STRUCTURE, MAINTAIN CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES. CHANGES IN DIRECTION TO ROUTE AROUND OBSTACLES SHALL BE MADE WITH CONDUIT OUTLET BODIES. CONDUIT SHALL BE INSTALLED IN A NEAT AND WORKMANLIKE MANNER. PARALLEL AND PERPENDICULAR TO STRUCTURE WALL AND CEILING LINES. ALL CONDUIT SHALL BE FISHED TO CLEAR OBSTRUCTIONS. ENDS OF CONDUITS SHALL BE TEMPORARILY CAPPED FLUSH TO FINISH GRADE TO PREVENT CONCRETE, PLASTER OR DIRT FROM ENTERING. CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED MALLEABLE IRON BUSHING ON INSIDE AND GALVANIZED MALLEABLE IRON LOCKNUT ON OUTSIDE AND INSIDE.
24. EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES AND PULL BOXES SHALL BE GALVANIZED OR EPOXY-COATED SHEET STEEL. SHALL MEET OR EXCEED UL 50 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND NEMA 3 (OR BETTER) FOR EXTERIOR LOCATIONS.
25. METAL RECEPTACLE, SWITCH AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY-COATED OR NON-CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
26. NONMETALLIC RECEPTACLE, SWITCH AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2 (NEWEST REVISION) AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
27. THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CARRIER AND/OR DISH Wireless L.L.C. AND TOWER OWNER BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.
28. THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD LIFE AND PROPERTY.
29. INSTALL LAMICOID LABEL ON THE METER CENTER TO SHOW "DISH Wireless L.L.C.".
30. ALL EMPTY/SPARE CONDUITS THAT ARE INSTALLED ARE TO HAVE A METERED MULE TAPE PULL CORD INSTALLED.



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DRAWN BY:	CHECKED BY:	APPROVED BY:
JJR	JHW	MDW

RFDS REV #: 0

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A&E PROJECT NUMBER
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DISH Wireless L.L.C.
PROJECT INFORMATION
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497 MIDDLE TURNPIKE
STORRS MANSFIELD, CT
06268

SHEET TITLE
GENERAL NOTES

SHEET NUMBER
GN-3

GROUNDING NOTES:

1. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION AND AC POWER GES’S) SHALL BE BONDED TOGETHER AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
2. THE CONTRACTOR SHALL PERFORM IEEE FALL–OF–POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS, THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
3. THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT AND PROVIDE TESTING RESULTS.
4. METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
5. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
6. EACH CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 STRANDED COPPER OR LARGER FOR INDOOR BTS; #2 BARE SOLID TINNED COPPER FOR OUTDOOR BTS.
7. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED BACK TO BACK CONNECTIONS ON OPPOSITE SIDE OF THE GROUND BUS ARE PERMITTED.
8. ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING SHALL BE #2 SOLID TINNED COPPER UNLESS OTHERWISE INDICATED.
9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
10. USE OF 90° BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45° BENDS CAN BE ADEQUATELY SUPPORTED.
11. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
12. ALL GROUND CONNECTIONS ABOVE GRADE (INTERIOR AND EXTERIOR) SHALL BE FORMED USING HIGH PRESS CRIMPS.
13. COMPRESSION GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.
14. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR.
15. APPROVED ANTIOXIDANT COATINGS (i.e. CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
16. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.
17. MISCELLANEOUS ELECTRICAL AND NON–ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
18. BOND ALL METALLIC OBJECTS WITHIN 6 ft OF MAIN GROUND RING WITH (1) #2 BARE SOLID TINNED COPPER GROUND CONDUCTOR.
19. GROUND CONDUCTORS USED FOR THE FACILITY GROUNDING AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON–METALLIC MATERIAL SUCH AS PVC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (i.e., NONMETALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT.
20. ALL GROUNDS THAT TRANSITION FROM BELOW GRADE TO ABOVE GRADE MUST BE #2 BARE SOLID TINNED COPPER IN 3/4" NON–METALLIC, FLEXIBLE CONDUIT FROM 24" BELOW GRADE TO WITHIN 3" TO 6" OF CAD–WELD TERMINATION POINT. THE EXPOSED END OF THE CONDUIT MUST BE SEALED WITH SILICONE CAULK. (ADD TRANSITIONING GROUND STANDARD DETAIL AS WELL).
21. BUILDINGS WHERE THE MAIN GROUNDING CONDUCTORS ARE REQUIRED TO BE ROUTED TO GRADE, THE CONTRACTOR SHALL ROUTE TWO GROUNDING CONDUCTORS FROM THE ROOFTOP, TOWERS, AND WATER TOWERS GROUNDING RING, TO THE EXISTING GROUNDING SYSTEM, THE GROUNDING CONDUCTORS SHALL NOT BE SMALLER THAN 2/0 COPPER. ROOFTOP GROUNDING RING SHALL BE BONDED TO THE EXISTING GROUNDING SYSTEM, THE BUILDING STEEL COLUMNS, LIGHTNING PROTECTION SYSTEM, AND BUILDING MAIN WATER LINE (FERROUS OR NONFERROUS METAL PIPING ONLY). DO NOT ATTACH GROUNDING TO FIRE SPRINKLER SYSTEM PIPES.



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SHEET TITLE

GENERAL NOTES

SHEET NUMBER

GN-4

Exhibit D

Structural Analysis Report

Date: **May 28, 2021**



Crown Castle
2000 Corporate Drive
Canonsburg, PA 15317
(724) 416-2000

Subject: **Structural Analysis Report**

Carrier Designation: **DISH Network Co-Locate**
Site Number: BOBDL00066A
Site Name: CT-CCI-T-842867

Crown Castle Designation: **BU Number:** 842867
Site Name: MANSFIELD FOUR CORNERS
JDE Job Number: 650055
Work Order Number: 1966176
Order Number: 556626 Rev. 1

Engineering Firm Designation: **Crown Castle Project Number:** 1966176

Site Data: **497 MIDDLE TURNPIKE, STORRS MANSFIELD,**
TOLLAND County, CT
Latitude 41° 49' 32.81", Longitude -72° 16' 54.46"
120 Foot - Monopole Tower

Crown Castle is pleased to submit this "**Structural Analysis Report**" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Proposed Equipment Configuration

Sufficient Capacity

This analysis utilizes an ultimate 3-second gust wind speed of 125 mph as required by the 2018 Connecticut Building Code. Applicable Standard references and design criteria are listed in Section 2 - "Analysis Criteria".

Structural analysis prepared by: Rohit Soni

Respectfully submitted by:

Maribel Dentinger

Maribel Dentinger, P.E.
Senior Project Engineer

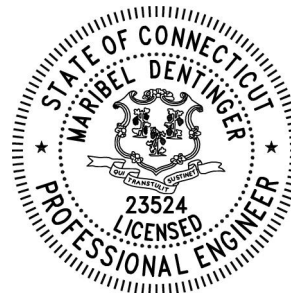


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1) INTRODUCTION

This tower is a 120 ft Monopole tower designed by PENNSUMMIT TUBULAR, LLC.

2) ANALYSIS CRITERIA

TIA-222 Revision:	TIA-222-H
Risk Category:	II
Wind Speed:	125 mph
Exposure Category:	B
Topographic Factor:	1
Ice Thickness:	2 in
Wind Speed with Ice:	50 mph
Seismic Ss:	0.175
Seismic S1:	0.064
Service Wind Speed:	60 mph

Table 1 - Proposed Equipment Configuration

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
99.0	99.0	3	fujitsu	TA08025-B604	1	1-1/2
		3	fujitsu	TA08025-B605		
		3	jma wireless	MX08FRO665-21 w/ Mount Pipe		
		1	raycap	RDIDC-9181-PF-48		
		1	tower mounts	Commscope MC-PK8-DSH		

Table 2 - Other Considered Equipment

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
120.0	122.0	1	andrew	SBNH-1D6565C w/ Mount Pipe	12 1 2	1-1/4 3/8 7/8
		2	cci antennas	HPA-65R-BUU-H6 w/ Mount Pipe		
		1	cci antennas	HPA-65R-BUU-H8 w/ Mount Pipe		
		3	ericsson	RRUS 11		
		3	ericsson	RRUS 32 B2		
		3	kathrein	78211056		
		2	kmw communications	AM-X-CD-16-65-00T-RET w/ Mount Pipe		
		6	powerwave technologies	7020.00		
		3	powerwave technologies	7770.00 w/ Mount Pipe		
		6	powerwave technologies	LGP 17201		
		1	raycap	DC6-48-60-18-8F		
	120.0	1	tower mounts	Platform Mount [LP 303-1]		

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
109.0	109.0	3	alcatel lucent	RRH 2x40-700 W/SOLAR	19	1-5/8
		3	alcatel lucent	RRH2X60-1900		
		3	alcatel lucent	RRH2X60-AWS		
		6	commscope	HBXX-6517DS-A2M w/ Mount Pipe		
		1	commscope	LNx-4514DS-A1M w/ Mount Pipe		
		2	commscope	LNx-6514DS-A1M w/ Mount Pipe		
		3	commscope	LNx-8513DS-VTM w/ Mount Pipe		
		2	rfs celwave	DB-T1-6Z-8AB-0Z		
		1	tower mounts	Platform Mount [LP 303-1]		

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Reference	Source
4-GEOTECHNICAL REPORTS	4713232	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	4858941	CCISITES
4-TOWER MANUFACTURER DRAWINGS	5214860	CCISITES

3.1) Analysis Method

tnxTower (version 8.0.9.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

3.2) Assumptions

- 1) Tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	120 - 70.75	Pole	TP32.28x18x0.1875	1	-12.94	1130.77	60.6	Pass
L2	70.75 - 34.75	Pole	TP42.35x30.7452x0.3125	2	-19.73	2468.37	40.7	Pass
L3	34.75 - 0	Pole	TP51.8x40.2019x0.375	3	-31.21	3759.73	37.0	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
							Summary	
						Pole (L1)	60.6	Pass
						Rating =	60.6	Pass

Table 5 - Tower Component Stresses vs. Capacity - LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	31.7	Pass
1	Base Plate	0	28.1	Pass
1	Base Foundation (Structure)	0	47.6	Pass
1	Base Foundation (Soil Interaction)	0	48.3	Pass

Structure Rating (max from all components) =	60.6%
---	--------------

Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

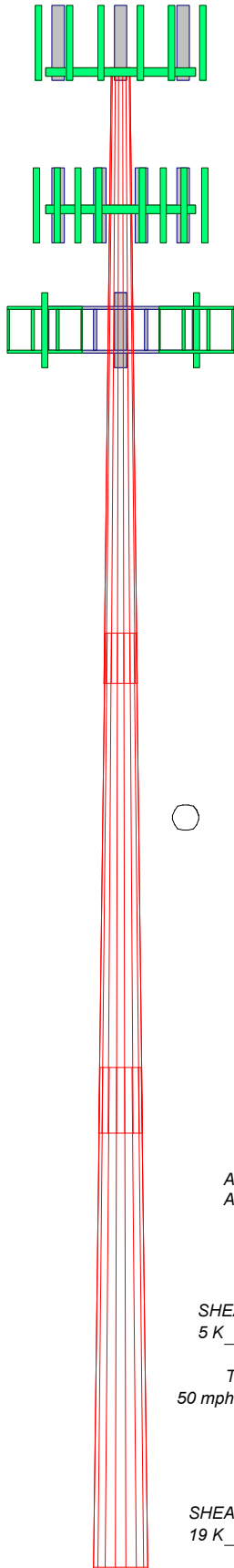
Section	1	2	3	
Length (ft)	49.25	40.00	40.00	
Number of Sides	18	18	18	
Thickness (in)	0.1875	0.3125	0.3750	
Socket Length (ft)	4.00	5.25	40.2019	
Top Dia (in)	18.0000	30.7452	51.8000	
Bot Dia (in)	32.2800	42.3500		
Grade		A607-65		
Weight (K)	2.5	4.9	7.4	14.8

120.0 ft

70.8 ft

34.8 ft

0.0 ft



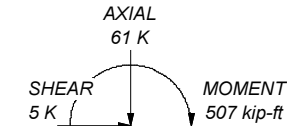
MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A607-65	65 ksi	80 ksi			

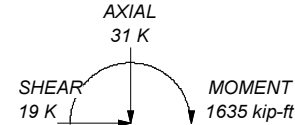
TOWER DESIGN NOTES

1. Tower is located in Tolland County, Connecticut.
2. Tower designed for Exposure B to the TIA-222-H Standard.
3. Tower designed for a 125 mph basic wind in accordance with the TIA-222-H Standard.
4. Tower is also designed for a 50 mph basic wind with 2.00 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Risk Category II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING: 60.6%

ALL REACTIONS
ARE FACTORED



TORQUE 0 kip-ft
50 mph WIND - 2.0000 in ICE



TORQUE 1 kip-ft
REACTIONS - 125 mph WIND

Crown Castle
2000 Corporate Drive
Canonsburg, PA 15317
Phone: (724) 416-2000
FAX:

Job:	842867		
Project:			
Client:	Crown Castle	Drawn by:	RSoni
Code:	TIA-222-H	Date:	05/28/21
Path:	C:\Work Area\842867\WO 1966176 - SAIProd\20171019_APP411405_842867.dwg		
		App'd:	
		Scale:	NTS
		Dwg No.	E-1

Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

- Tower is located in Tolland County, Connecticut.
- Tower base elevation above sea level: 0.00 ft.
- Basic wind speed of 125 mph.
- Risk Category II.
- Exposure Category B.
- Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- Topographic Category: 1.
- Crest Height: 0.00 ft.
- Nominal ice thickness of 2.0000 in.
- Ice thickness is considered to increase with height.
- Ice density of 56 pcf.
- A wind speed of 50 mph is used in combination with ice.
- Temperature drop of 50 °F.
- Deflections calculated using a wind speed of 60 mph.
- A non-linear (P-delta) analysis was used.
- Pressures are calculated at each section.
- Stress ratio used in pole design is 1.05.
- Tower analysis based on target reliabilities in accordance with Annex S.
- Load Modification Factors used: $K_{es}(F_w) = 0.95$, $K_{es}(t_i) = 0.85$.
- Maximum demand-capacity ratio is: 1.
- Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Horizontals	Assume Legs Pinned	Calculate Redundant Bracing Forces
Consider Moments - Diagonals	✓ Assume Rigid Index Plate	Ignore Redundant Members in FEA
Use Moment Magnification	✓ Use Clear Spans For Wind Area	SR Leg Bolts Resist Compression
Use Code Stress Ratios	Use Clear Spans For KL/r	All Leg Panels Have Same Allowable
✓ Use Code Safety Factors - Guys	Retention Guys To Initial Tension	Offset Girt At Foundation
Escalate Ice	✓ Bypass Mast Stability Checks	✓ Consider Feed Line Torque
Always Use Max Kz	✓ Use Azimuth Dish Coefficients	Include Angle Block Shear Check
Use Special Wind Profile	✓ Project Wind Area of Appurt.	Use TIA-222-H Bracing Resist.
Include Bolts In Member Capacity	Autocalc Torque Arm Areas	Exemption
Leg Bolts Are At Top Of Section	Add IBC .6D+W Combination	Use TIA-222-H Tension Splice
Secondary Horizontal Braces Leg	✓ Sort Capacity Reports By Component	Exemption
Use Diamond Inner Bracing (4 Sided)	Triangulate Diamond Inner Bracing	
SR Members Have Cut Ends	Treat Feed Line Bundles As Cylinder	
SR Members Are Concentric	Ignore KL/ry For 60 Deg. Angle Legs	

Poles

- ✓ Include Shear-Torsion Interaction
- Always Use Sub-Critical Flow
- Use Top Mounted Sockets
- Pole Without Linear Attachments
- Pole With Shroud Or No
- Appurtenances
- Outside and Inside Corner Radii Are Known

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	120.00-70.75	49.25	4.00	18	18.0000	32.2800	0.1875	0.7500	A607-65 (65 ksi)
L2	70.75-34.75	40.00	5.25	18	30.7452	42.3500	0.3125	1.2500	A607-65 (65 ksi)
L3	34.75-0.00	40.00		18	40.2019	51.8000	0.3750	1.5000	A607-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	It/Q in ²	w in	w/t
L1	18.2488	10.6007	424.9328	6.3234	9.1440	46.4712	850.4248	5.3013	2.8380	15.136
	32.7490	19.0990	2485.1711	11.3928	16.3982	151.5511	4973.6122	9.5513	5.3513	28.54
L2	32.3497	30.1854	3531.9616	10.8036	15.6186	226.1387	7068.5707	15.0956	4.8612	15.556
	42.9551	41.6959	9309.0430	14.9233	21.5138	432.7010	18630.335	20.8519	6.9036	22.092
L3	42.3099	47.4039	9499.5752	14.1385	20.4226	465.1513	19011.650	23.7065	6.4155	17.108
	52.5412	61.2086	20450.246	18.2559	26.3144	777.1504	40927.401	30.6101	8.4568	22.551

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _r	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontal in	Double Angle Stitch Bolt Spacing Redundants in
L1 120.00- 70.75				1	1	1			
L2 70.75- 34.75				1	1	1			
L3 34.75-0.00				1	1	1			

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Sector	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	Number Per Row	Start/End Position	Width or Diameter in	Perimeter in	Weight plf
HJ7-50A(1-5/8)	A	No	Surface Ar (CaAa)	109.00 - 0.00	7	7	-0.100 0.100	1.9800		1.04

CU12PSM9P6XXX(1- 1/2)	C	No	Surface Ar (CaAa)	99.00 - 0.00	1	1	-0.450 -0.400	1.6000		2.35

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		CaA _A ft²/ft	Weight plf

HJ7-50A(1-5/8)	A	No	No	Inside Pole	109.00 - 0.00	12	No Ice	0.00	1.04
							1/2" Ice	0.00	1.04
							1" Ice	0.00	1.04
							2" Ice	0.00	1.04

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		C _A A _A ft ² /ft	Weight plf
LDF6-50A(1-1/4)	C	No	No	Inside Pole	120.00 - 0.00	12	No Ice	0.00	0.60
							1/2" Ice	0.00	0.60
							1" Ice	0.00	0.60
							2" Ice	0.00	0.60
WR-VG86ST- BRDA(7/8)	C	No	No	Inside Pole	120.00 - 0.00	2	No Ice	0.00	0.68
							1/2" Ice	0.00	0.68
							1" Ice	0.00	0.68
							2" Ice	0.00	0.68
FB-L98B-002- XXX(3/8)	C	No	No	Inside Pole	120.00 - 0.00	1	No Ice	0.00	0.06
							1/2" Ice	0.00	0.06
							1" Ice	0.00	0.06
							2" Ice	0.00	0.06

Feed Line/Linear Appurtenances Section Areas

Tower Section n	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L1	120.00-70.75	A	0.000	0.000	53.014	0.000	0.76
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	4.520	0.000	0.49
L2	70.75-34.75	A	0.000	0.000	49.896	0.000	0.71
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	5.760	0.000	0.40
L3	34.75-0.00	A	0.000	0.000	48.163	0.000	0.69
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	5.560	0.000	0.38

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section n	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L1	120.00-70.75	A	1.887	0.000	0.000	84.308	0.000	1.84
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	15.179	0.000	0.72
L2	70.75-34.75	A	1.780	0.000	0.000	79.349	0.000	1.73
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	19.343	0.000	0.68
L3	34.75-0.00	A	1.589	0.000	0.000	75.671	0.000	1.61
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	17.933	0.000	0.64

Feed Line Center of Pressure

Section	Elevation ft	CP _x in	CP _z in	CP _x Ice in	CP _z Ice in
L1	120.00-70.75	-4.5312	-2.5253	-3.2180	-1.7074
L2	70.75-34.75	-5.4901	-3.0239	-3.8565	-1.9787
L3	34.75-0.00	-5.9232	-3.2620	-4.2931	-2.2118

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L1	3	HJ7-50A(1-5/8)	70.75 - 109.00	1.0000	1.0000
L1	14	CU12PSM9P6XXX(1-1/2)	70.75 - 99.00	1.0000	1.0000
L2	3	HJ7-50A(1-5/8)	34.75 - 70.75	1.0000	1.0000
L2	14	CU12PSM9P6XXX(1-1/2)	34.75 - 70.75	1.0000	1.0000
L3	3	HJ7-50A(1-5/8)	0.00 - 34.75	1.0000	1.0000
L3	14	CU12PSM9P6XXX(1-1/2)	0.00 - 34.75	1.0000	1.0000

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft

7770.00 w/ Mount Pipe	A	From Leg	4.00 0.00 2.00	0.0000	120.00
7770.00 w/ Mount Pipe	B	From Leg	4.00 0.00 2.00	0.0000	120.00
7770.00 w/ Mount Pipe	C	From Leg	4.00 0.00 2.00	0.0000	120.00
AM-X-CD-16-65-00T-RET w/ Mount Pipe	A	From Leg	4.00 0.00 2.00	0.0000	120.00
AM-X-CD-16-65-00T-RET w/ Mount Pipe	B	From Leg	4.00 0.00 2.00	0.0000	120.00
SBNH-1D6565C w/ Mount Pipe	C	From Leg	4.00 0.00 2.00	0.0000	120.00
RRUS 11	A	From Leg	4.00 0.00 2.00	0.0000	120.00
RRUS 11	B	From Leg	4.00 0.00 2.00	0.0000	120.00
RRUS 11	C	From Leg	4.00 0.00 2.00	0.0000	120.00
DC6-48-60-18-8F	A	From Leg	4.00 0.00 2.00	0.0000	120.00
HPA-65R-BUU-H6 w/ Mount Pipe	A	From Leg	4.00 0.00 2.00	0.0000	120.00

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft
HPA-65R-BUU-H6 w/ Mount Pipe	B	From Leg	4.00 0.00 2.00	0.0000	120.00
HPA-65R-BUU-H8 w/ Mount Pipe	C	From Leg	4.00 0.00 2.00	0.0000	120.00
(2) LGP 17201	A	From Leg	4.00 0.00 2.00	0.0000	120.00
(2) LGP 17201	B	From Leg	4.00 0.00 2.00	0.0000	120.00
(2) LGP 17201	C	From Leg	4.00 0.00 2.00	0.0000	120.00
(2) 7020.00	A	From Leg	4.00 0.00 2.00	0.0000	120.00
(2) 7020.00	B	From Leg	4.00 0.00 2.00	0.0000	120.00
(2) 7020.00	C	From Leg	4.00 0.00 2.00	0.0000	120.00
78211056	A	From Leg	4.00 0.00 2.00	0.0000	120.00
78211056	B	From Leg	4.00 0.00 2.00	0.0000	120.00
78211056	C	From Leg	4.00 0.00 2.00	0.0000	120.00
RRUS 32 B2	A	From Leg	4.00 0.00 2.00	0.0000	120.00
RRUS 32 B2	B	From Leg	4.00 0.00 2.00	0.0000	120.00
RRUS 32 B2	C	From Leg	4.00 0.00 2.00	0.0000	120.00
6' x 2" Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	120.00
6' x 2" Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	120.00
6' x 2" Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	120.00
Platform Mount [LP 303-1] ***	C	None		0.0000	120.00
LNx-6514DS-A1M w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	109.00
LNx-6514DS-A1M w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	109.00
LNx-8513DS-VTM w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	109.00
LNx-8513DS-VTM w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	109.00

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft
LNx-8513DS-VTM w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	109.00
(2) HBXX-6517DS-A2M w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	109.00
(2) HBXX-6517DS-A2M w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	109.00
(2) HBXX-6517DS-A2M w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	109.00
LNx-4514DS-A1M w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH 2x40-700 W/SOLAR	A	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH 2x40-700 W/SOLAR	B	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH 2x40-700 W/SOLAR	C	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH2X60-AWS	A	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH2X60-AWS	B	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH2X60-AWS	C	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH2X60-1900	A	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH2X60-1900	B	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH2X60-1900	C	From Leg	4.00 0.00 0.00	0.0000	109.00
DB-T1-6Z-8AB-0Z	B	From Leg	4.00 0.00 0.00	0.0000	109.00
DB-T1-6Z-8AB-0Z	C	From Leg	4.00 0.00 0.00	0.0000	109.00
Platform Mount [LP 303-1] ***	C	None		0.0000	109.00
Commscope MC-PK8-DSH	C	None		0.0000	99.00
(2) 8' x 2" Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	99.00
(2) 8' x 2" Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	99.00
(2) 8' x 2" Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	99.00
MX08FRO665-21 w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	99.00
MX08FRO665-21 w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	99.00

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft
MX08FRO665-21 w/ Mount Pipe	C	From Leg	0.00 4.00 0.00 0.00	0.0000	99.00
TA08025-B604	A	From Leg	4.00 0.00 0.00	0.0000	99.00
TA08025-B604	B	From Leg	4.00 0.00 0.00	0.0000	99.00
TA08025-B604	C	From Leg	4.00 0.00 0.00	0.0000	99.00
TA08025-B605	A	From Leg	4.00 0.00 0.00	0.0000	99.00
TA08025-B605	B	From Leg	4.00 0.00 0.00	0.0000	99.00
TA08025-B605	C	From Leg	4.00 0.00 0.00	0.0000	99.00
RDIDC-9181-PF-48	B	From Leg	4.00 0.00 0.00	0.0000	99.00

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp

Comb. No.	Description
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	120 - 70.75	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-33.09	2.43	-0.80
			Max. Mx	20	-12.94	447.10	0.63
			Max. My	14	-12.95	-0.36	-441.27
			Max. Vy	20	-13.29	447.10	0.63
			Max. Vx	14	13.13	-0.36	-441.27
			Max. Torque	22			0.63
			Max Tension	1	0.00	0.00	0.00
L2	70.75 - 34.75	Pole	Max. Compression	26	-44.09	4.27	-0.38
			Max. Mx	20	-19.73	950.54	1.84
			Max. My	14	-19.73	-1.06	-938.67
			Max. Vy	20	-15.68	950.54	1.84
			Max. Vx	14	15.52	-1.06	-938.67
			Max. Torque	10			-0.62
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-60.92	6.71	0.22
L3	34.75 - 0	Pole	Max. Mx	20	-31.21	1634.95	3.24
			Max. My	2	-31.21	4.72	1616.09
			Max. Vy	20	-18.55	1634.95	3.24
			Max. Vx	2	-18.40	4.72	1616.09
			Max. Torque	10			-0.62

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	36	60.92	5.48	0.01
	Max. H _x	20	31.21	18.54	0.03
	Max. H _z	2	31.21	0.03	18.38
	Max. M _x	2	1616.09	0.03	18.38
	Max. M _z	8	1631.96	-18.54	-0.03
	Max. Torsion	22	0.62	16.07	9.22
	Min. Vert	5	23.41	-9.24	15.91

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Min. H _x	8	31.21	-18.54	-0.03
	Min. H _z	14	31.21	-0.03	-18.38
	Min. M _x	14	-1616.07	-0.03	-18.38
	Min. M _z	20	-1634.95	18.54	0.03
	Min. Torsion	10	-0.62	-16.07	-9.22

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturing Moment, M _x kip-ft	Overturing Moment, M _z kip-ft	Torque kip-ft
Dead Only	26.01	0.00	0.00	-0.02	1.21	0.00
1.2 Dead+1.0 Wind 0 deg - No Ice	31.21	-0.03	-18.38	-1616.09	4.72	-0.26
0.9 Dead+1.0 Wind 0 deg - No Ice	23.41	-0.03	-18.38	-1604.76	4.31	-0.26
1.2 Dead+1.0 Wind 30 deg - No Ice	31.21	9.24	-15.91	-1397.96	-812.45	0.05
0.9 Dead+1.0 Wind 30 deg - No Ice	23.41	9.24	-15.91	-1388.16	-807.12	0.06
1.2 Dead+1.0 Wind 60 deg - No Ice	31.21	16.04	-9.16	-805.25	-1411.51	0.36
0.9 Dead+1.0 Wind 60 deg - No Ice	23.41	16.04	-9.16	-799.61	-1401.98	0.36
1.2 Dead+1.0 Wind 90 deg - No Ice	31.21	18.54	0.03	3.21	-1631.96	0.56
0.9 Dead+1.0 Wind 90 deg - No Ice	23.41	18.54	0.03	3.19	-1620.89	0.57
1.2 Dead+1.0 Wind 120 deg - No Ice	31.21	16.07	9.22	810.81	-1414.73	0.62
0.9 Dead+1.0 Wind 120 deg - No Ice	23.41	16.07	9.22	805.13	-1405.18	0.62
1.2 Dead+1.0 Wind 150 deg - No Ice	31.21	9.30	15.94	1401.16	-818.03	0.52
0.9 Dead+1.0 Wind 150 deg - No Ice	23.41	9.30	15.94	1391.34	-812.67	0.51
1.2 Dead+1.0 Wind 180 deg - No Ice	31.21	0.03	18.38	1616.07	-1.73	0.27
0.9 Dead+1.0 Wind 180 deg - No Ice	23.41	0.03	18.38	1604.74	-2.09	0.26
1.2 Dead+1.0 Wind 210 deg - No Ice	31.21	-9.24	15.91	1397.94	815.44	-0.05
0.9 Dead+1.0 Wind 210 deg - No Ice	23.41	-9.24	15.91	1388.14	809.35	-0.06
1.2 Dead+1.0 Wind 240 deg - No Ice	31.21	-16.04	9.16	805.23	1414.50	-0.36
0.9 Dead+1.0 Wind 240 deg - No Ice	23.41	-16.04	9.16	799.59	1404.21	-0.36
1.2 Dead+1.0 Wind 270 deg - No Ice	31.21	-18.54	-0.03	-3.24	1634.95	-0.57
0.9 Dead+1.0 Wind 270 deg - No Ice	23.41	-18.54	-0.03	-3.21	1623.11	-0.57
1.2 Dead+1.0 Wind 300 deg - No Ice	31.21	-16.07	-9.22	-810.83	1417.72	-0.62
0.9 Dead+1.0 Wind 300 deg - No Ice	23.41	-16.07	-9.22	-805.15	1407.41	-0.62
1.2 Dead+1.0 Wind 330 deg - No Ice	31.21	-9.30	-15.94	-1401.18	821.02	-0.51
0.9 Dead+1.0 Wind 330 deg - No Ice	23.41	-9.30	-15.94	-1391.36	814.89	-0.51
1.2 Dead+1.0 Ice+1.0 Temp	60.92	-0.00	0.00	-0.22	6.71	0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	60.92	-0.01	-5.45	-497.46	7.44	-0.06
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	60.92	2.73	-4.72	-430.53	-242.85	-0.00

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	60.92	4.74	-2.72	-248.30	-426.24	0.05
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	60.92	5.48	0.01	0.40	-493.59	0.10
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	60.92	4.75	2.73	248.94	-426.86	0.11
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	60.92	2.74	4.72	430.72	-243.92	0.10
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	60.92	0.01	5.45	497.02	6.20	0.06
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	60.92	-2.73	4.72	430.10	256.49	0.00
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	60.92	-4.74	2.72	247.86	439.88	-0.05
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	60.92	-5.48	-0.01	-0.84	507.23	-0.09
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	60.92	-4.75	-2.73	-249.38	440.50	-0.11
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	60.92	-2.74	-4.72	-431.15	257.56	-0.10
Dead+Wind 0 deg - Service	26.01	-0.01	-3.99	-349.45	1.94	-0.06
Dead+Wind 30 deg - Service	26.01	2.01	-3.45	-302.28	-174.75	0.01
Dead+Wind 60 deg - Service	26.01	3.48	-1.99	-174.13	-304.29	0.07
Dead+Wind 90 deg - Service	26.01	4.02	0.01	0.69	-351.96	0.11
Dead+Wind 120 deg - Service	26.01	3.49	2.00	175.31	-304.99	0.13
Dead+Wind 150 deg - Service	26.01	2.02	3.46	302.96	-175.96	0.11
Dead+Wind 180 deg - Service	26.01	0.01	3.99	349.43	0.54	0.06
Dead+Wind 210 deg - Service	26.01	-2.01	3.45	302.27	177.24	-0.01
Dead+Wind 240 deg - Service	26.01	-3.48	1.99	174.11	306.77	-0.07
Dead+Wind 270 deg - Service	26.01	-4.02	-0.01	-0.71	354.44	-0.11
Dead+Wind 300 deg - Service	26.01	-3.49	-2.00	-175.33	307.47	-0.13
Dead+Wind 330 deg - Service	26.01	-2.02	-3.46	-302.98	178.44	-0.11

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-26.01	0.00	0.00	26.01	0.00	0.000%
2	-0.03	-31.21	-18.38	0.03	31.21	18.38	0.000%
3	-0.03	-23.41	-18.38	0.03	23.41	18.38	0.000%
4	9.24	-31.21	-15.91	-9.24	31.21	15.91	0.000%
5	9.24	-23.41	-15.91	-9.24	23.41	15.91	0.000%
6	16.04	-31.21	-9.16	-16.04	31.21	9.16	0.000%
7	16.04	-23.41	-9.16	-16.04	23.41	9.16	0.000%
8	18.54	-31.21	0.03	-18.54	31.21	-0.03	0.000%
9	18.54	-23.41	0.03	-18.54	23.41	-0.03	0.000%
10	16.07	-31.21	9.22	-16.07	31.21	-9.22	0.000%
11	16.07	-23.41	9.22	-16.07	23.41	-9.22	0.000%
12	9.30	-31.21	15.94	-9.30	31.21	-15.94	0.000%
13	9.30	-23.41	15.94	-9.30	23.41	-15.94	0.000%
14	0.03	-31.21	18.38	-0.03	31.21	-18.38	0.000%
15	0.03	-23.41	18.38	-0.03	23.41	-18.38	0.000%
16	-9.24	-31.21	15.91	9.24	31.21	-15.91	0.000%
17	-9.24	-23.41	15.91	9.24	23.41	-15.91	0.000%
18	-16.04	-31.21	9.16	16.04	31.21	-9.16	0.000%
19	-16.04	-23.41	9.16	16.04	23.41	-9.16	0.000%
20	-18.54	-31.21	-0.03	18.54	31.21	0.03	0.000%

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
21	-18.54	-23.41	-0.03	18.54	23.41	0.03	0.000%
22	-16.07	-31.21	-9.22	16.07	31.21	9.22	0.000%
23	-16.07	-23.41	-9.22	16.07	23.41	9.22	0.000%
24	-9.30	-31.21	-15.94	9.30	31.21	15.94	0.000%
25	-9.30	-23.41	-15.94	9.30	23.41	15.94	0.000%
26	0.00	-60.92	0.00	0.00	60.92	-0.00	0.000%
27	-0.01	-60.92	-5.45	0.01	60.92	5.45	0.000%
28	2.73	-60.92	-4.72	-2.73	60.92	4.72	0.000%
29	4.74	-60.92	-2.72	-4.74	60.92	2.72	0.000%
30	5.48	-60.92	0.01	-5.48	60.92	-0.01	0.000%
31	4.75	-60.92	2.73	-4.75	60.92	-2.73	0.000%
32	2.74	-60.92	4.72	-2.74	60.92	-4.72	0.000%
33	0.01	-60.92	5.45	-0.01	60.92	-5.45	0.000%
34	-2.73	-60.92	4.72	2.73	60.92	-4.72	0.000%
35	-4.74	-60.92	2.72	4.74	60.92	-2.72	0.000%
36	-5.48	-60.92	-0.01	5.48	60.92	0.01	0.000%
37	-4.75	-60.92	-2.73	4.75	60.92	2.73	0.000%
38	-2.74	-60.92	-4.72	2.74	60.92	4.72	0.000%
39	-0.01	-26.01	-3.99	0.01	26.01	3.99	0.000%
40	2.01	-26.01	-3.45	-2.01	26.01	3.45	0.000%
41	3.48	-26.01	-1.99	-3.48	26.01	1.99	0.000%
42	4.02	-26.01	0.01	-4.02	26.01	-0.01	0.000%
43	3.49	-26.01	2.00	-3.49	26.01	-2.00	0.000%
44	2.02	-26.01	3.46	-2.02	26.01	-3.46	0.000%
45	0.01	-26.01	3.99	-0.01	26.01	-3.99	0.000%
46	-2.01	-26.01	3.45	2.01	26.01	-3.45	0.000%
47	-3.48	-26.01	1.99	3.48	26.01	-1.99	0.000%
48	-4.02	-26.01	-0.01	4.02	26.01	0.01	0.000%
49	-3.49	-26.01	-2.00	3.49	26.01	2.00	0.000%
50	-2.02	-26.01	-3.46	2.02	26.01	3.46	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	4	0.00000001	0.00008504
3	Yes	4	0.00000001	0.00004899
4	Yes	5	0.00000001	0.00005975
5	Yes	5	0.00000001	0.00002801
6	Yes	5	0.00000001	0.00005872
7	Yes	5	0.00000001	0.00002746
8	Yes	4	0.00000001	0.00012151
9	Yes	4	0.00000001	0.00007583
10	Yes	5	0.00000001	0.00006288
11	Yes	5	0.00000001	0.00002948
12	Yes	5	0.00000001	0.00005856
13	Yes	5	0.00000001	0.00002737
14	Yes	4	0.00000001	0.00007536
15	Yes	4	0.00000001	0.00004186
16	Yes	5	0.00000001	0.00005990
17	Yes	5	0.00000001	0.00002802
18	Yes	5	0.00000001	0.00006142
19	Yes	5	0.00000001	0.00002876
20	Yes	4	0.00000001	0.00010923
21	Yes	4	0.00000001	0.00006732
22	Yes	5	0.00000001	0.00005859
23	Yes	5	0.00000001	0.00002732
24	Yes	5	0.00000001	0.00006242
25	Yes	5	0.00000001	0.00002925
26	Yes	4	0.00000001	0.00002316
27	Yes	5	0.00000001	0.00014901
28	Yes	5	0.00000001	0.00015800
29	Yes	5	0.00000001	0.00015797
30	Yes	5	0.00000001	0.00014794

31	Yes	5	0.00000001	0.00015930
32	Yes	5	0.00000001	0.00015939
33	Yes	5	0.00000001	0.00015013
34	Yes	5	0.00000001	0.00016296
35	Yes	5	0.00000001	0.00016396
36	Yes	5	0.00000001	0.00015323
37	Yes	5	0.00000001	0.00016346
38	Yes	5	0.00000001	0.00016242
39	Yes	4	0.00000001	0.00001043
40	Yes	4	0.00000001	0.00002407
41	Yes	4	0.00000001	0.00002331
42	Yes	4	0.00000001	0.00001118
43	Yes	4	0.00000001	0.00002691
44	Yes	4	0.00000001	0.00002311
45	Yes	4	0.00000001	0.00001041
46	Yes	4	0.00000001	0.00002448
47	Yes	4	0.00000001	0.00002571
48	Yes	4	0.00000001	0.00001120
49	Yes	4	0.00000001	0.00002326
50	Yes	4	0.00000001	0.00002663

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	120 - 70.75	9.843	48	0.8332	0.0023
L2	74.75 - 34.75	3.313	48	0.4555	0.0004
L3	40 - 0	0.884	48	0.2062	0.0001

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
120.00	7770.00 w/ Mount Pipe	48	9.843	0.8332	0.0024	43485
109.00	LNx-6514DS-A1M w/ Mount Pipe	48	8.060	0.7393	0.0018	19766
99.00	Commscope MC-PK8-DSH	48	6.502	0.6545	0.0012	10353

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	120 - 70.75	45.427	20	3.8421	0.0112
L2	74.75 - 34.75	15.297	20	2.1040	0.0020
L3	40 - 0	4.078	20	0.9517	0.0006

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
120.00	7770.00 w/ Mount Pipe	20	45.427	3.8421	0.0113	9483
109.00	LNx-6514DS-A1M w/ Mount	20	37.203	3.4101	0.0084	4310

Elevation	Appurtenance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
ft			in	°	°	ft
99.00	Pipe Commscope MC-PK8-DSH	20	30.012	3.0204	0.0061	2256

Compression Checks

Pole Design Data

Section No.	Elevation	Size	L	L _u	Kl/r	A	P _u	φP _n	Ratio P _u /φP _n
	ft		ft	ft		in ²	K	K	φP _n
L1	120 - 70.75 (1)	TP32.28x18x0.1875	49.25	0.00	0.0	18.408	-12.94	1076.92	0.012
L2	70.75 - 34.75 (2)	TP42.35x30.7452x0.3125	40.00	0.00	0.0	40.185	-19.73	2350.83	0.008
L3	34.75 - 0 (3)	TP51.8x40.2019x0.375	40.00	0.00	0.0	61.208	-31.21	3580.70	0.009

Pole Bending Design Data

Section No.	Elevation	Size	M _{ux}	φM _{nx}	Ratio M _{ux} /φM _{nx}	M _{uy}	φM _{ny}	Ratio M _{uy} /φM _{ny}
	ft		kip-ft	kip-ft		kip-ft	kip-ft	
L1	120 - 70.75 (1)	TP32.28x18x0.1875	447.10	718.72	0.622	0.00	718.72	0.000
L2	70.75 - 34.75 (2)	TP42.35x30.7452x0.3125	950.54	2271.92	0.418	0.00	2271.92	0.000
L3	34.75 - 0 (3)	TP51.8x40.2019x0.375	1634.95	4303.93	0.380	0.00	4303.93	0.000

Pole Shear Design Data

Section No.	Elevation	Size	Actual V _u	φV _n	Ratio V _u /φV _n	Actual T _u	φT _n	Ratio T _u /φT _n
	ft		K	K		kip-ft	kip-ft	
L1	120 - 70.75 (1)	TP32.28x18x0.1875	13.29	323.07	0.041	0.57	875.18	0.001
L2	70.75 - 34.75 (2)	TP42.35x30.7452x0.3125	15.68	705.25	0.022	0.57	2502.26	0.000
L3	34.75 - 0 (3)	TP51.8x40.2019x0.375	18.55	1074.21	0.017	0.57	4837.76	0.000

Pole Interaction Design Data

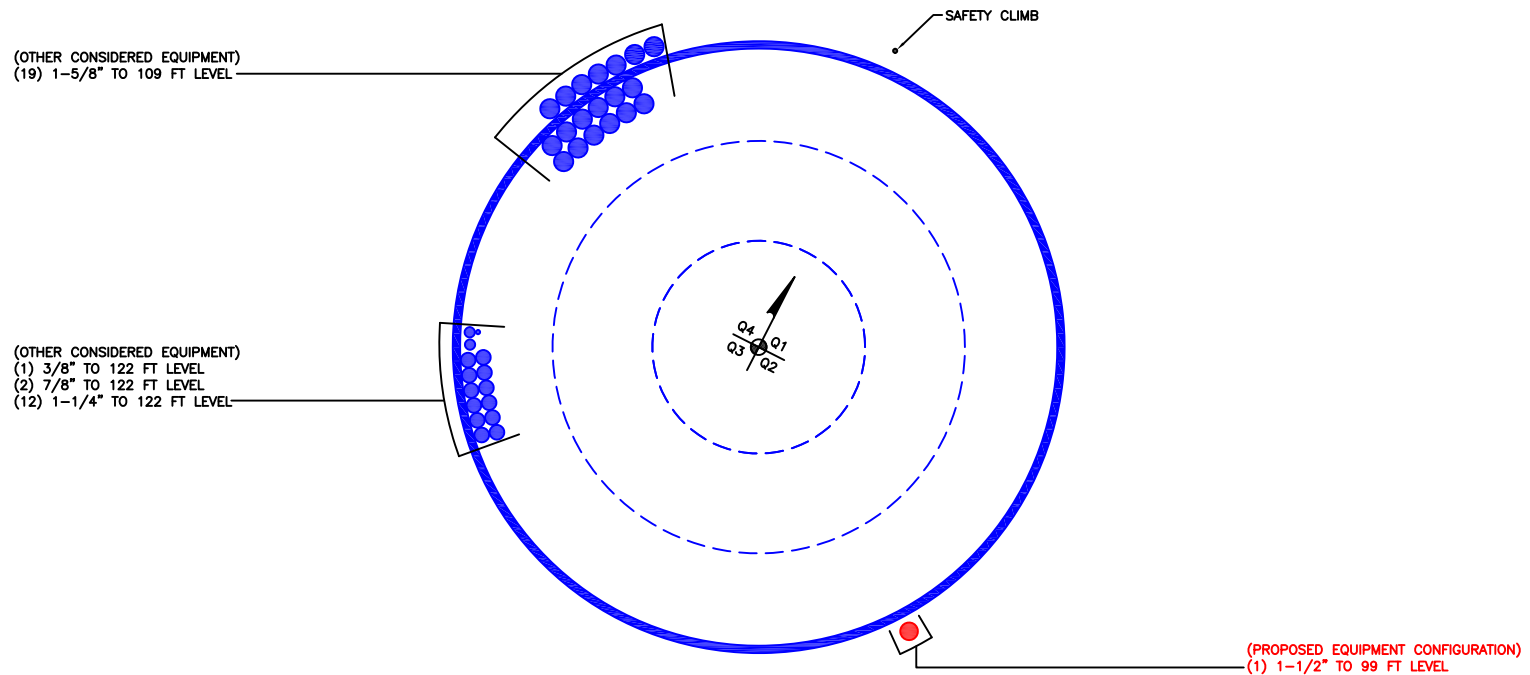
Section No.	Elevation	Ratio P _u /φP _n	Ratio M _{ux} /φM _{nx}	Ratio M _{uy} /φM _{ny}	Ratio V _u /φV _n	Ratio T _u /φT _n	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
	ft								
L1	120 - 70.75 (1)	0.012	0.622	0.000	0.041	0.001	0.636	1.050	4.8.2
L2	70.75 - 34.75 (2)	0.008	0.418	0.000	0.022	0.000	0.427	1.050	4.8.2

Section No.	Elevation ft	Ratio P_u	Ratio M_{ux}	Ratio M_{uy}	Ratio V_u	Ratio T_u	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L3	34.75 - 0 (3)	0.009	0.380	0.000	0.017	0.000	0.389	1.050	4.8.2

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail
L1	120 - 70.75	Pole	TP32.28x18x0.1875	1	-12.94	1130.77	60.6	Pass
L2	70.75 - 34.75	Pole	TP42.35x30.7452x0.3125	2	-19.73	2468.37	40.7	Pass
L3	34.75 - 0	Pole	TP51.8x40.2019x0.375	3	-31.21	3759.73	37.0	Pass
							Summary	
							Pole (L1)	60.6
							RATING =	60.6
								Pass

APPENDIX B
BASE LEVEL DRAWING



APPENDIX C

ADDITIONAL CALCULATIONS

Monopole Base Plate Connection

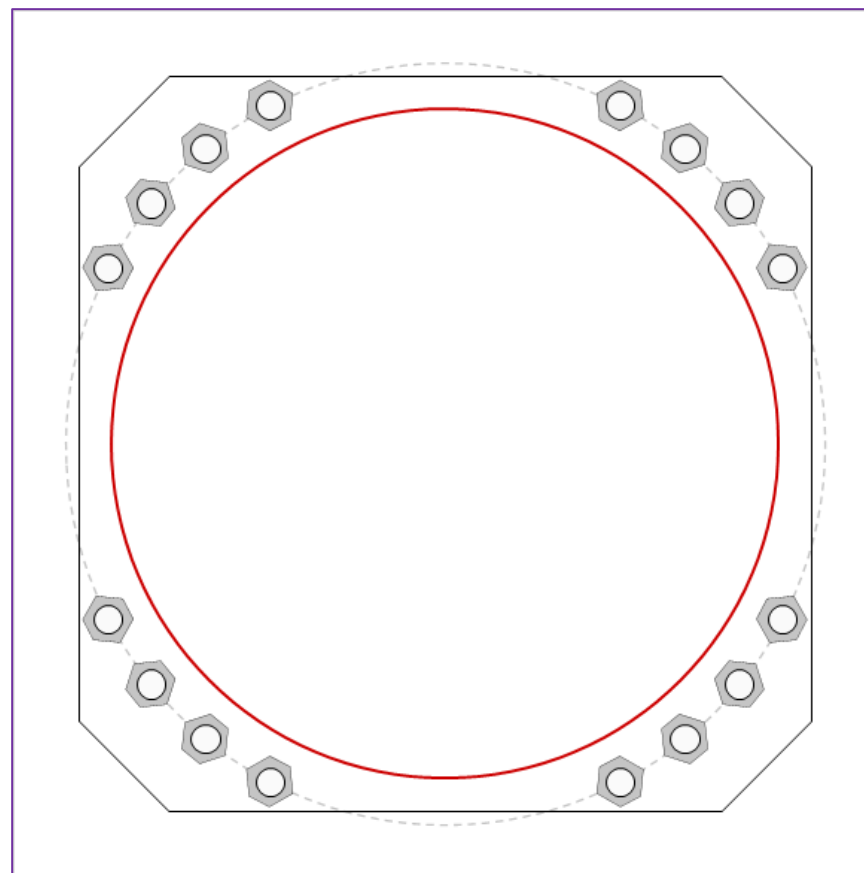


Site Info	
BU #	842867
Site Name	ANSFIELD FOUR CORNER
Order #	556626 Rev 1

Analysis Considerations	
TIA-222 Revision	H
Grout Considered:	No
l_{ar} (in)	1.25

Applied Loads	
Moment (kip-ft)	1634.95
Axial Force (kips)	31.21
Shear Force (kips)	18.55

*TIA-222-H Section 15.5 Applied



Connection Properties		Analysis Results	
Anchor Rod Data		Anchor Rod Summary <i>(units of kips, kip-in)</i>	
(16) 2-1/4" ϕ bolts (A615-75 N; $F_y=75$ ksi, $F_u=100$ ksi) on 59" BC <i>Anchor Spacing: 6 in</i>		$Pu_t = 81.13$	$\phi Pn_t = 243.75$ Stress Rating
		$Vu = 1.16$	$\phi Vn = 149.1$ 31.7%
		$Mu = n/a$	$\phi Mn = n/a$ Pass
Base Plate Data		Base Plate Summary	
57" W x 3" Plate (A572-55; $F_y=55$ ksi, $F_u=70$ ksi); Clip: 7 in		Max Stress (ksi):	14.59 (Flexural)
Stiffener Data		Allowable Stress (ksi):	49.5
N/A		Stress Rating:	28.1% Pass
Pole Data			
51.8" x 0.375" 18-sided pole (A607-65; $F_y=65$ ksi, $F_u=80$ ksi)			

Drilled Pier Foundation

BU # : 842867
 Site Name: MANSFIELD FOUR CORNER
 Order Number: 556626 Rev 1
 TIA-222 Revision: H
 Tower Type: Monopole



Applied Loads		
	Comp.	Uplift
Moment (kip-ft)	1634.95	
Axial Force (kips)	31.21	
Shear Force (kips)	18.54	

Material Properties		
Concrete Strength, f'c:	3	ksi
Rebar Strength, Fy:	60	ksi
Tie Yield Strength, Fyt:	40	ksi

Pier Design Data		
Depth	19	ft
Ext. Above Grade	0.5	ft
Pier Section 1		
From 0.5' above grade to 19' below grade		
Pier Diameter	7	ft
Rebar Quantity	20	
Rebar Size	11	
Clear Cover to Ties	4	in
Tie Size	4	
Tie Spacing		in

[Rebar & Pier Options](#)

[Embedded Pole Inputs](#)

[Belled Pier Inputs](#)

Analysis Results		
Soil Lateral Check	Compression	Uplift
D _{v=0} (ft from TOC)	5.31	-
Soil Safety Factor	2.62	-
Max Moment (kip-ft)	1763.58	-
Rating*	48.3%	-
Soil Vertical Check	Compression	Uplift
Skin Friction (kips)	364.19	-
End Bearing (kips)	115.45	-
Weight of Concrete (kips)	117.77	-
Total Capacity (kips)	479.65	-
Axial (kips)	148.98	-
Rating*	29.6%	-
Reinforced Concrete Flexure	Compression	Uplift
Critical Depth (ft from TOC)	5.23	-
Critical Moment (kip-ft)	1763.51	-
Critical Moment Capacity	4952.80	-
Rating*	33.9%	-
Reinforced Concrete Shear	Compression	Uplift
Critical Depth (ft from TOC)	14.05	-
Critical Shear (kip)	271.69	-
Critical Shear Capacity	543.58	-
Rating*	47.6%	-

Structural Foundation Rating*	47.6%
Soil Interaction Rating*	48.3%

*Rating per TIA-222-H Section 15.5

Check Limitation	
Apply TIA-222-H Section 15.5:	<input checked="" type="checkbox"/>
N/A	<input type="checkbox"/>
Additional Longitudinal Rebar	
Input Effective Depths (else Actual):	<input type="checkbox"/>
Shear Design Options	
Check Shear along Depth of Pier:	<input checked="" type="checkbox"/>
Utilize Shear-Friction Methodology:	<input type="checkbox"/>
Override Critical Depth:	<input type="checkbox"/>

[Go to Soil Calculations](#)

Soil Profile														
Groundwater Depth		13	# of Layers		6									
Layer	Top (ft)	Bottom (ft)	Thickness (ft)	Y _{soil} (pcf)	Y _{concrete} (pcf)	Cohesion (ksf)	Angle of Friction (degrees)	Calculated Ultimate Skin Friction Comp (ksf)	Calculated Ultimate Skin Friction Uplift (ksf)	Ultimate Skin Friction Comp Override (ksf)	Ultimate Skin Friction Uplift Override (ksf)	Ult. Gross Bearing Capacity (ksf)	SPT Blow Count	Soil Type
1	0	3.5	3.5	130	150	0	0	0.000	0.000					Cohesionless
2	3.5	5	1.5	130	150	0	32	0.675	0.675				72	Cohesionless
3	5	7	2	130	150	0	32	0.912	0.912				59	Cohesionless
4	7	10	3	130	150	0	32	1.223	1.223				52	Cohesionless
5	10	13	3	130	150	0	32	1.558	1.558				54	Cohesionless
6	13	19	6	67.6	87.6	0	32	1.817	1.817			4	41	Cohesionless



BU: 842867
WO: 1966176
Order: 556626

Structure: A
Rev: 0

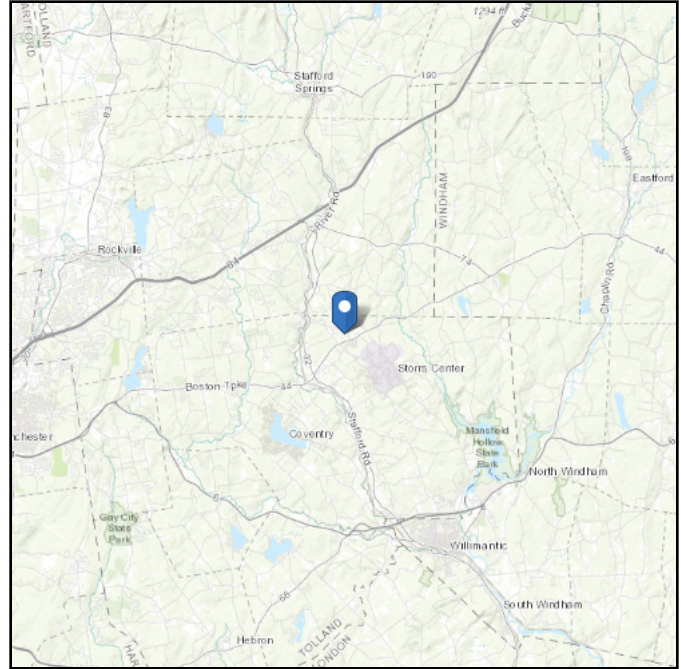
Location				
	Decimal Degrees	Deg	Min	Sec
Lat:	41.825778	+	41	49
Long:	-72.281778	-	72	16
Code and Site Parameters				
Seismic Design Code:		TIA-222-H-1		
Site Soil:		D (Default) Stiff Soil (Default)		
Risk Category:		II		
<u>USGS Seismic Reference</u>				
	S _s :	0.1750	g	
	S ₁ :	0.0640	g	
	T _L :	6	s	
Seismic Design Category Determination				
Importance Factor, I _e :		1		
Acceleration-based site coefficient, F _a :		1.6000		
Velocity-based site coefficient, F _v :		2.4000		
Design spectral response acceleration short period, S _{DS} :		0.1867	g	
Design spectral response acceleration 1 s period, S _{D1} :		0.1024	g	
T _s :		0.5486		
Seismic Design Category Based on S _{DS} :		B		
Seismic Design Category Based on S _{D1} :		B		
Seismic Design Category Based on S ₁ :		N/A		
Controlling Seismic Design Category:		B		

ASCE 7 Hazards Report

Address:
No Address at This
Location

Standard: ASCE/SEI 7-10
Risk Category: II
Soil Class: D - Stiff Soil

Elevation: 559.18 ft (NAVD 88)
Latitude: 41.825781
Longitude: -72.281794



Wind

Results:

Wind Speed:	126 Vmph	125 mph Per
10-year MRI	77 Vmph	Jurisdiction
25-year MRI	87 Vmph	
50-year MRI	95 Vmph	
100-year MRI	102 Vmph	

Data Source: ASCE/SEI 7-10, Fig. 26.5-1A and Figs. CC-1–CC-4, and Section 26.5.2, incorporating errata of March 12, 2014

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.

Results:

Ice Thickness: 1.00 in.

Concurrent Temperature: 5 F

Gust Speed: 50 mph

Data Source: Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Fri May 21 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

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Exhibit E

Mount Analysis

Date: **August 1, 2021**

Darcy Tarr
Crown Castle
3530 Toringdon Way, Suite 300
Charlotte, NC 28277
704-405-6589



Trylon
1825 W. Walnut Hill Lane,
Suite 302
Irving, TX 75038
214-930-1730

Subject: Mount Replacement Analysis Report

Carrier Designation: DISH Network Equipment Change-Out
Carrier Site Number: BOBDL00066A
Carrier Site Name: CT-CCI-T-842867

Crown Castle Designation: **Crown Castle BU Number:** 842867
Crown Castle Site Name: MANSFIELD FOUR CORNERS
Crown Castle JDE Job Number: 650055
Crown Castle Order Number: 556626 Rev. 1

Engineering Firm Designation: **Trylon Report Designation:** 189205

Site Data: 497 Middle Turnpike, Storrs Mansfield, Tolland County, CT, 06268
Latitude 41°49'32.81" Longitude -72°16'54.46"

Structure Information: **Tower Height & Type:** 120.0 ft Monopole
Mount Elevation: 99.0 ft
Mount Type: 8.0 ft Platform

Dear Darcy Tarr,

Trylon is pleased to submit this "**Mount Replacement Analysis Report**" to determine the structural integrity of DISH Network's antenna mounting system with the proposed appurtenance and equipment addition on the abovementioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

Platform

Sufficient

***Sufficient upon completion of the changes listed in the 'Recommendations' section of this report.**

This analysis utilizes an ultimate 3-second gust wind speed of 125 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount analysis prepared by: Jordan Everson, E.I.T.

Respectfully Submitted by:
Cliff Abernathy, P.E.



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Supplemental Drawings

1) INTRODUCTION

This is a proposed 3 sector 8.0 ft Platform designed by Commscope.

2) ANALYSIS CRITERIA

Building Code:	2015 IBC / 2018 Connecticut State Building Code
TIA-222 Revision:	TIA-222-H
Risk Category:	II
Ultimate Wind Speed:	130 mph
Exposure Category:	B
Topographic Factor at Base:	1.0
Topographic Factor at Mount:	1.0
Ice Thickness:	2.0 in
Wind Speed with Ice:	50 mph
Seismic S_s:	0.173
Seismic S_1:	0.062
Live Loading Wind Speed:	30 mph
Man Live Load at Mid/End-Points:	250 lb
Man Live Load at Mount Pipes:	500 lb

Table 1 - Proposed Equipment Configuration

Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount Details
99.0	99.0	3	JMA WIRELESS	MX08FRO665-21	8.0 ft Platform [CommScope MC-PK8-DSH]
		3	FUJITSU	TA08025-B604	
		3	FUJITSU	TA08025-B605	
		1	RAYCAP	RDIDC-9181-PF-48	

3) ANALYSIS PROCEDURE

Table 2 - Documents Provided

Document	Remarks	Reference	Source
Crown Application	DISH Network Application	556626 Rev. 1	CCI Sites
Mount Manufacturer Drawings	CommScope	MC-PK8-DSH	Trylon

3.1) Analysis Method

RISA-3D (Version 17.0.4), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases.

A tool internally developed, using Microsoft Excel, by Trylon was used to calculate wind loading on all appurtenances, dishes, and mount members for various load cases. Selected output from the analysis is included in Appendix B.

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 *Tower Mount Analysis* (Revision B).

3.2) Assumptions

- 1) The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- 2) The configuration of antennas, mounts, and other appurtenances are as specified in Table 1 and the referenced drawings.
- 3) All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- 4) The analysis will be required to be revised if the existing conditions in the field differ from those shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
- 5) Prior structural modifications to the tower mounting system are assumed to be installed as shown per available data.
- 6) Steel grades have been assumed as follows, unless noted otherwise:

Channel, Solid Round, Angle, Plate	ASTM A36 (GR 36)
HSS (Rectangular)	ASTM A500 (GR B-46)
Pipe	ASTM A53 (GR 35)
Connection Bolts	ASTM A325

This analysis may be affected if any assumptions are not valid or have been made in error. Tylon should be notified to determine the effect on the structural integrity of the antenna mounting system.

4) ANALYSIS RESULTS

Table 3- Mount Component Stresses vs. Capacity (Platform, All Sectors)

Notes	Component	Critical Member	Centerline (ft)	% Capacity	Pass / Fail
1,2	Mount Pipe(s)	MP2	120.0	29.0	Pass
	Horizontal(s)	H1		11.3	Pass
	Standoff(s)	M12		58.8	Pass
	Bracing(s)	M11		48.0	Pass
	Handrail(s)	M19		11.7	Pass
	Mount Connection(s)	-		24.1	Pass

Structure Rating (max from all components) =	58.8%
---	--------------

Notes:

- 1) See additional documentation in "Appendix C - Software Analysis Output" for calculations supporting the % capacity consumed.
- 2) Rating per TIA-222-H, Section 15.5

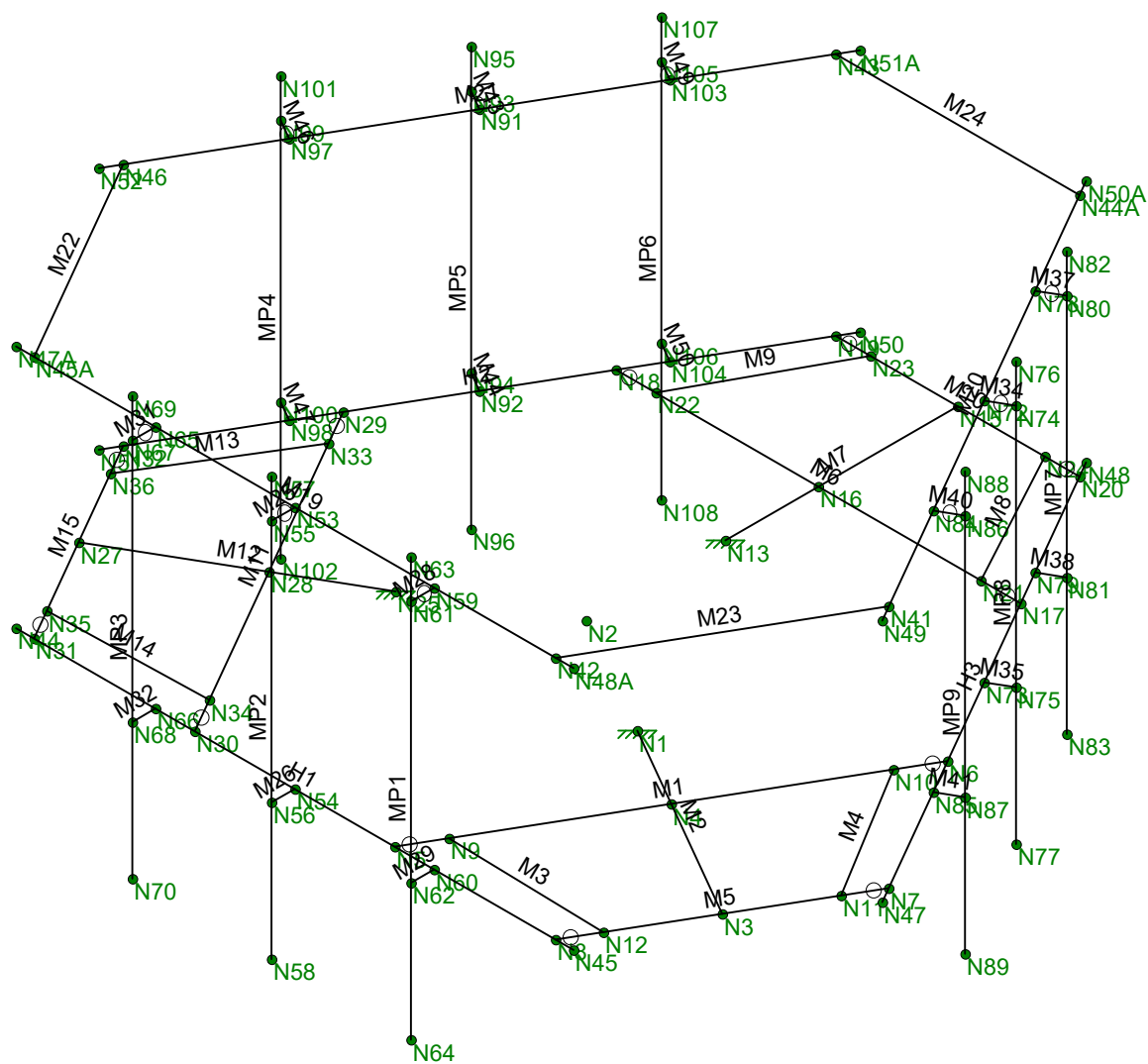
4.1) Recommendations

The mount has sufficient capacity to carry the proposed loading configuration. In order for the results of the analysis to be considered valid, the proposed mount listed below must be installed.

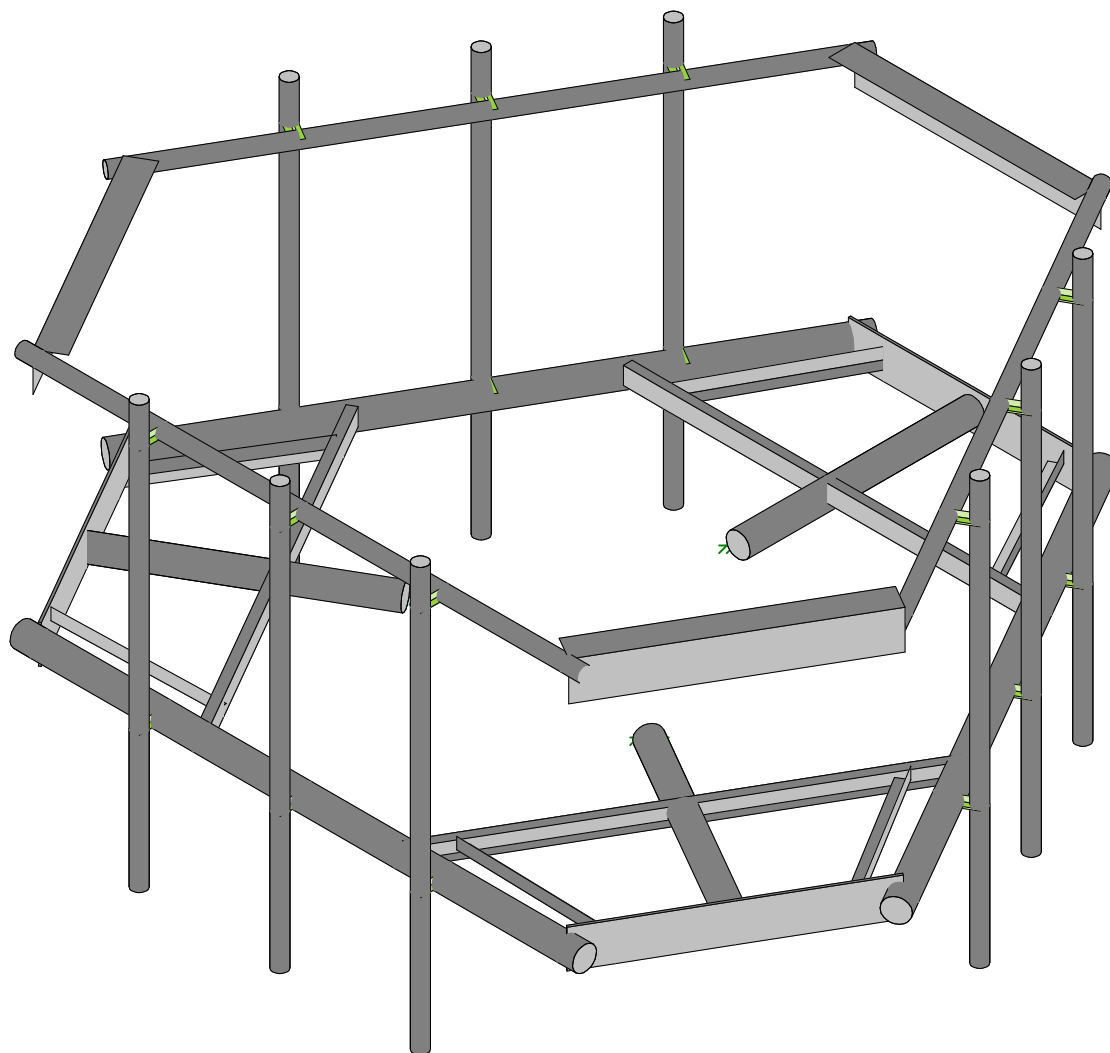
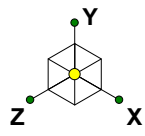
1. CommScope MC-PK8-DSH

No structural modifications are required at this time, provided that the above-listed changes are implemented.

APPENDIX A
WIRE FRAME AND RENDERED MODELS



Trylon	842867	Wireframe
JE		Aug 1, 2021 at 7:28 AM
		842867_loaded.r3d



Envelope Only Solution

Trylon	842867	Render
JE		Aug 1, 2021 at 7:28 AM
		842867_loaded.r3d

APPENDIX B
SOFTWARE INPUT CALCULATIONS

ASCE 7 Hazards Report

Address:

No Address at This
Location

Standard: ASCE/SEI 7-10

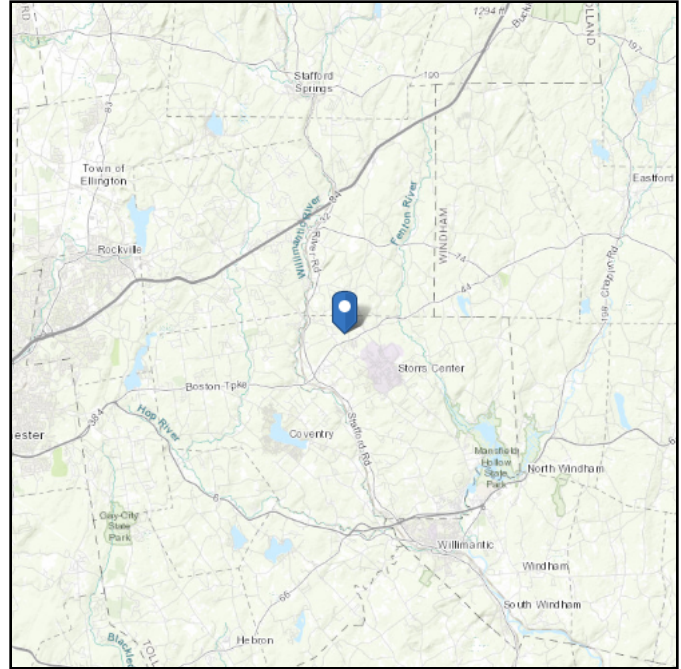
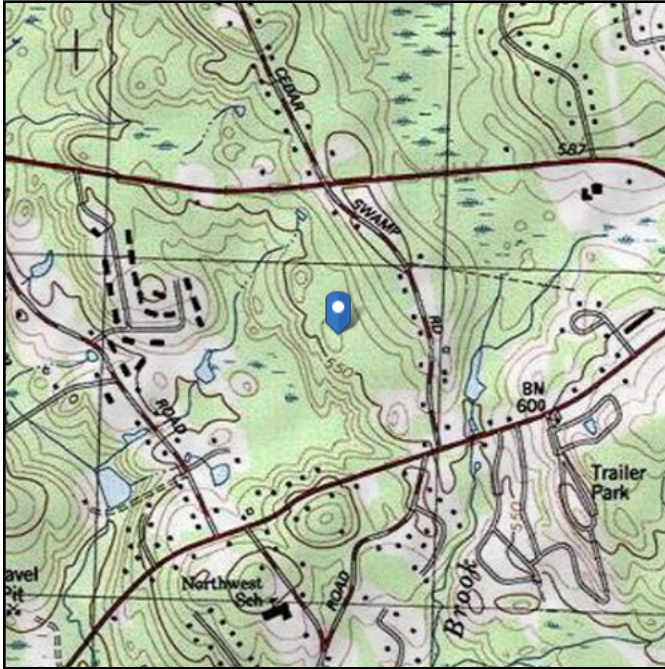
Risk Category: II

Soil Class: D - Stiff Soil

Elevation: 559.18 ft (NAVD 88)

Latitude: 41.825781

Longitude: -72.281794



Ice

Results:

Ice Thickness: 1.00 in.
Concurrent Temperature: 5 F
Gust Speed: 50 mph

Data Source: Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Mon Aug 02 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

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TIA LOAD CALCULATOR 2.0

PROJECT DATA		
Job Code:	189205	
Carrier Site ID:	BOBDL00066A	
Carrier Site Name:	CT-CCI-T-842867	

CODES AND STANDARDS		
Building Code:	2015 IBC	
Local Building Code:	2018 CTSBC	
Design Standard:	TIA-222-H	

STRUCTURE DETAILS		
Mount Type:	Platform	--
Mount Elevation:	99.0	ft.
Number of Sectors:	3	--
Structure Type:	Monopole	--
Structure Height:	120.0	ft.

ANALYSIS CRITERIA		
Structure Risk Category:	II	--
Exposure Category:	B	--
Site Class:	D - Default	--
Ground Elevation:	559.18	ft.

TOPOGRAPHIC DATA		
Topographic Category:	1.00	--
Topographic Feature:	N/A	--
Crest Point Elevation:	0.00	ft.
Base Point Elevation:	0.00	ft.
Crest to Mid-Height (L/2):	0.00	ft.
Distance from Crest (x):	0.00	ft.
Base Topo Factor (K_{zt}):	1.00	--
Mount Topo Factor (K_{zt}):	1.00	--

WIND PARAMETERS		
Design Wind Speed:	130	mph
Wind Escalation Factor (K_s):	1.00	--
Velocity Coefficient (K_z):	0.99	--
Directionality Factor (K_d):	0.95	--
Gust Effect Factor (G_h):	1.00	--
Shielding Factor (K_a):	0.90	--
Velocity Pressure (q_z):	39.69	psf

ICE PARAMETERS		
Design Ice Wind Speed:	50	mph
Design Ice Thickness (t_i):	2.00	in
Importance Factor (I_i):	1.00	--
Ice Velocity Pressure (q_{zi}):	39.69	psf
Mount Ice Thickness (t_{iz}):	2.23	in

WIND STRUCTURE CALCULATIONS		
Flat Member Pressure:	71.44	psf
Round Member Pressure:	42.86	psf
Ice Wind Pressure:	7.18	psf

SEISMIC PARAMETERS		
Importance Factor (I_e):	1.00	--
Short Period Accel. (S_s):	0.17	g
1 Second Accel. (S_1):	0.06	g
Short Period Des. (S_{DS}):	0.18	g
1 Second Des. (S_{D1}):	0.10	g
Short Period Coeff. (F_a):	1.60	--
1 Second Coeff. (F_v):	2.40	--
Response Coefficient (C_s):	0.09	--
Amplification Factor (A_S):	1.20	--

LOAD COMBINATIONS [LRFD]

#	Description
1	1.4DL
2	1.2DL + 1WL 0 AZI
3	1.2DL + 1WL 30 AZI
4	1.2DL + 1WL 45 AZI
5	1.2DL + 1WL 60 AZI
6	1.2DL + 1WL 90 AZI
7	1.2DL + 1WL 120 AZI
8	1.2DL + 1WL 135 AZI
9	1.2DL + 1WL 150 AZI
10	1.2DL + 1WL 180 AZI
11	1.2DL + 1WL 210 AZI
12	1.2DL + 1WL 225 AZI
13	1.2DL + 1WL 240 AZI
14	1.2DL + 1WL 270 AZI
15	1.2DL + 1WL 300 AZI
16	1.2DL + 1WL 315 AZI
17	1.2DL + 1WL 330 AZI
18	0.9DL + 1WL 0 AZI
19	0.9DL + 1WL 30 AZI
20	0.9DL + 1WL 45 AZI
21	0.9DL + 1WL 60 AZI
22	0.9DL + 1WL 90 AZI
23	0.9DL + 1WL 120 AZI
24	0.9DL + 1WL 135 AZI
25	0.9DL + 1WL 150 AZI
26	0.9DL + 1WL 180 AZI
27	0.9DL + 1WL 210 AZI
28	0.9DL + 1WL 225 AZI
29	0.9DL + 1WL 240 AZI
30	0.9DL + 1WL 270 AZI
31	0.9DL + 1WL 300 AZI
32	0.9DL + 1WL 315 AZI
33	0.9DL + 1WL 330 AZI
34	1.2DL + 1DLi + 1WLi 0 AZI
35	1.2DL + 1DLi + 1WLi 30 AZI
36	1.2DL + 1DLi + 1WLi 45 AZI
37	1.2DL + 1DLi + 1WLi 60 AZI
38	1.2DL + 1DLi + 1WLi 90 AZI
39	1.2DL + 1DLi + 1WLi 120 AZI
40	1.2DL + 1DLi + 1WLi 135 AZI
41	1.2DL + 1DLi + 1WLi 150 AZI

#	Description
42	1.2DL + 1DLi + 1WLi 180 AZI
43	1.2DL + 1DLi + 1WLi 210 AZI
44	1.2DL + 1DLi + 1WLi 225 AZI
45	1.2DL + 1DLi + 1WLi 240 AZI
46	1.2DL + 1DLi + 1WLi 270 AZI
47	1.2DL + 1DLi + 1WLi 300 AZI
48	1.2DL + 1DLi + 1WLi 315 AZI
49	1.2DL + 1DLi + 1WLi 330 AZI
50	(1.2+0.2Sds) + 1.0E 0 AZI
51	(1.2+0.2Sds) + 1.0E 30 AZI
52	(1.2+0.2Sds) + 1.0E 45 AZI
53	(1.2+0.2Sds) + 1.0E 60 AZI
54	(1.2+0.2Sds) + 1.0E 90 AZI
55	(1.2+0.2Sds) + 1.0E 120 AZI
56	(1.2+0.2Sds) + 1.0E 135 AZI
57	(1.2+0.2Sds) + 1.0E 150 AZI
58	(1.2+0.2Sds) + 1.0E 180 AZI
59	(1.2+0.2Sds) + 1.0E 210 AZI
60	(1.2+0.2Sds) + 1.0E 225 AZI
61	(1.2+0.2Sds) + 1.0E 240 AZI
62	(1.2+0.2Sds) + 1.0E 270 AZI
63	(1.2+0.2Sds) + 1.0E 300 AZI
64	(1.2+0.2Sds) + 1.0E 315 AZI
65	(1.2+0.2Sds) + 1.0E 330 AZI
66	(0.9-0.2Sds) + 1.0E 0 AZI
67	(0.9-0.2Sds) + 1.0E 30 AZI
68	(0.9-0.2Sds) + 1.0E 45 AZI
69	(0.9-0.2Sds) + 1.0E 60 AZI
70	(0.9-0.2Sds) + 1.0E 90 AZI
71	(0.9-0.2Sds) + 1.0E 120 AZI
72	(0.9-0.2Sds) + 1.0E 135 AZI
73	(0.9-0.2Sds) + 1.0E 150 AZI
74	(0.9-0.2Sds) + 1.0E 180 AZI
75	(0.9-0.2Sds) + 1.0E 210 AZI
76	(0.9-0.2Sds) + 1.0E 225 AZI
77	(0.9-0.2Sds) + 1.0E 240 AZI
78	(0.9-0.2Sds) + 1.0E 270 AZI
79	(0.9-0.2Sds) + 1.0E 300 AZI
80	(0.9-0.2Sds) + 1.0E 315 AZI
81	(0.9-0.2Sds) + 1.0E 330 AZI
82-88	1.2D + 1.5 Lv1

#	Description
89	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP1
90	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP1
91	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP1
92	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP1
93	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP1
94	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP1
95	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP1
96	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP1
97	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP1
98	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP1
99	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP1
100	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP1
101	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP1
102	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP1
103	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP1
104	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP1
105	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP2
106	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP2
107	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP2
108	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP2
109	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP2
110	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP2
111	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP2
112	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP2
113	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP2
114	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP2
115	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP2
116	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP2
117	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP2
118	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP2
119	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP2
120	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP2

#	Description
121	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP3
122	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP3
123	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP3
124	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP3
125	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP3
126	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP3
127	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP3
128	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP3
129	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP3
130	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP3
131	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP3
132	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP3
133	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP3
134	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP3
135	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP3
136	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP3
137	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP4
138	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP4
139	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP4
140	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP4
141	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP4
142	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP4
143	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP4
144	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP4
145	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP4
146	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP4
147	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP4
148	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP4
149	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP4
150	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP4
151	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP4
152	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP4

*This page shows an example of maintenance loads for (4) pipes, the number of mount pipe LCs may vary per site

EQUIPMENT LOADING

[illegible]

EQUIPMENT LOADING [CONT.]

<i>Appurtenance Name/Location</i>	<i>Qty.</i>	<i>Elevation [ft]</i>	<i>--</i>	<i>EPA_N (ft²)</i>	<i>EPA_T (ft²)</i>	<i>Weight (lbs)</i>
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			

EQUIPMENT WIND CALCULATIONS

[illegible]

EQUIPMENT LATERAL WIND FORCE CALCULATIONS

[illegible]

EQUIPMENT LATERAL WIND FORCE CALCULATIONS [CONT.]

[illegible]

EQUIPMENT SEISMIC FORCE CALCULATIONS

[illegible]

APPENDIX C
SOFTWARE ANALYSIS OUTPUT

(Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	24
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Y
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver

Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISACONNECTION CODE	AISC 15th(360-16): LRFD
Cold Formed Steel Code	AISI S100-12: LRFD
Wood Code	AWC NDS-15: ASD
Wood Temperature	< 100F
Concrete Code	ACI 318-14
Masonry Code	ACI 530-13: Strength
Aluminum Code	AA ADM1-10: LRFD - Building
Stainless Steel Code	AISC 14th(360-10): LRFD
Adjust Stiffness?	Yes(Iterative)

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parame Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	Yes
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR SET ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8

(Global) Model Settings, Continued

Seismic Code	ASCE 7-10
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	3
R Z	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	1
Cd X	1
Rho Z	1
Rho X	1

Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E...	Density[k/ft...	Yield[psi]	Ry	Fu[psi]	Rt
1	A992	29000	11154	.3	.65	.49	50000	1.1	65000	1.1
2	A36 Gr.36	29000	11154	.3	.65	.49	36000	1.5	58000	1.2
3	A572 Gr.50	29000	11154	.3	.65	.49	50000	1.1	65000	1.1
4	A500 Gr.B RND	29000	11154	.3	.65	.527	42000	1.4	58000	1.3
5	A500 Gr.B Rect	29000	11154	.3	.65	.527	46000	1.4	58000	1.3
6	A53 Gr.B	29000	11154	.3	.65	.49	35000	1.6	60000	1.2
7	A1085	29000	11154	.3	.65	.49	50000	1.4	65000	1.3

Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design R...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	Plates	6.5"x0.37" Plate	Beam	RECT	A53 Gr.B	Typical	2.405	.027	8.468	.106
2	Grating Bracing	L2x2x3	Beam	Single An...	A36 Gr.36	Typical	.722	.271	.271	.009
3	Standoffs	PIPE 3.5	Beam	Pipe	A53 Gr.B	Typical	2.5	4.52	4.52	9.04
4	Standoff Bracing	C3X5	Beam	Channel	A36 Gr.36	Typical	1.47	.241	1.85	.043
5	Handrails	PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical	1.02	.627	.627	1.25
6	Handrail Corners	L6 5/8x4 7/16x3/16	Beam	Single An...	A36 Gr.36	Typical	2.039	3.593	9.575	.023
7	Horizontals	PIPE 3.5	Beam	Pipe	A53 Gr.B	Typical	2.5	4.52	4.52	9.04
8	Mount Pipes	PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical	1.02	.627	.627	1.25

Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]
1	N25	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction
2	N1	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction
3	N13	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction

Basic Load Cases

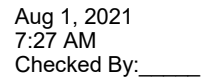
	BLC Description	Category	X Gra...	Y Gra...	Z Gra...	Joint	Point	Distributed	Area(Member)	Surface(Plate/Wall)
1	Self Weight	DL		-1			20		3	
2	Structure Wind Z	WLZ						51		
3	Structure Wind X	WLX						51		
4	Wind Load 0 AZI	WLZ					40			
5	Wind Load 30 AZI	None					40			
6	Wind Load 45 AZI	None					40			
7	Wind Load 60 AZI	None					40			
8	Wind Load 90 AZI	WLX					40			
9	Wind Load 120 AZI	None					40			
10	Wind Load 135 AZI	None					40			
11	Wind Load 150 AZI	None					40			
12	Ice Weight	OL1					20	51	3	
13	Ice Structure Wind Z	OL2						51		
14	Ice Structure Wind X	OL3						51		
15	Ice Wind Load 0 AZI	OL2					40			
16	Ice Wind Load 30 AZI	None					40			
17	Ice Wind Load 45 AZI	None					40			
18	Ice Wind Load 60 AZI	None					40			
19	Ice Wind Load 90 AZI	OL3					40			
20	Ice Wind Load 120 ...	None					40			
21	Ice Wind Load 135 ...	None					40			
22	Ice Wind Load 150 ...	None					40			
23	Seismic Load Z	ELZ			-111		20			
24	Seismic Load X	ELX	-111				20			
25	Live Load 1 (Lv)	None					1			
26	Live Load 2 (Lv)	None					1			
27	Live Load 3 (Lv)	None					1			
28	Live Load 4 (Lv)	None					1			
29	Live Load 5 (Lv)	None					1			
30	Live Load 6 (Lv)	None					1			
31	Live Load 7 (Lv)	None					1			
32	Live Load 8 (Lv)	None					1			
33	Live Load 9 (Lv)	None					1			
34	Maintenance Load 1...	None					1			
35	Maintenance Load 2...	None					1			
36	Maintenance Load 3...	None					1			
37	Maintenance Load 4...	None					1			
38	Maintenance Load 5...	None					1			
39	Maintenance Load 6...	None					1			
40	Maintenance Load 7...	None					1			
41	Maintenance Load 8...	None					1			
42	Maintenance Load 9...	None					1			
43	BLC 1 Transient Are...	None						9		
44	BLC 12 Transient Ar...	None						9		

Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	1.4DL	Yes	Y			DL	1.4													
2	1.2DL + 1WL 0 AZI	Yes	Y			DL	1.2	2	1	3	4	1								
3	1.2DL + 1WL 30 AZI	Yes	Y			DL	1.2	2	.866	3	.5	5	1							
4	1.2DL + 1WL 45 AZI	Yes	Y			DL	1.2	2	.707	3	.707	6	1							
5	1.2DL + 1WL 60 AZI	Yes	Y			DL	1.2	2	.5	3	.866	7	1							
6	1.2DL + 1WL 90 AZI	Yes	Y			DL	1.2	2		3	1	8	1							
7	1.2DL + 1WL 120 AZI	Yes	Y			DL	1.2	2	-.5	3	.866	9	1							

Load Combinations (Continued)

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
8	1.2DL + 1WL 135 AZI	Yes	Y		DL	1.2	2	-7...	3	.707	10	1								
9	1.2DL + 1WL 150 AZI	Yes	Y		DL	1.2	2	-8...	3	.5	11	1								
10	1.2DL + 1WL 180 AZI	Yes	Y		DL	1.2	2	-1	3		4	-1								
11	1.2DL + 1WL 210 AZI	Yes	Y		DL	1.2	2	-8...	3	-.5	5	-1								
12	1.2DL + 1WL 225 AZI	Yes	Y		DL	1.2	2	-7...	3	-.7...	6	-1								
13	1.2DL + 1WL 240 AZI	Yes	Y		DL	1.2	2	-.5	3	-.8...	7	-1								
14	1.2DL + 1WL 270 AZI	Yes	Y		DL	1.2	2		3	-1	8	-1								
15	1.2DL + 1WL 300 AZI	Yes	Y		DL	1.2	2	.5	3	-.8...	9	-1								
16	1.2DL + 1WL 315 AZI	Yes	Y		DL	1.2	2	.707	3	-.7...	10	-1								
17	1.2DL + 1WL 330 AZI	Yes	Y		DL	1.2	2	.866	3	-.5	11	-1								
18	0.9DL + 1WL 0 AZI	Yes	Y		DL	.9	2	1	3		4	1								
19	0.9DL + 1WL 30 AZI	Yes	Y		DL	.9	2	.866	3	.5	5	1								
20	0.9DL + 1WL 45 AZI	Yes	Y		DL	.9	2	.707	3	.707	6	1								
21	0.9DL + 1WL 60 AZI	Yes	Y		DL	.9	2	.5	3	.866	7	1								
22	0.9DL + 1WL 90 AZI	Yes	Y		DL	.9	2		3	1	8	1								
23	0.9DL + 1WL 120 AZI	Yes	Y		DL	.9	2	-.5	3	.866	9	1								
24	0.9DL + 1WL 135 AZI	Yes	Y		DL	.9	2	-7...	3	.707	10	1								
25	0.9DL + 1WL 150 AZI	Yes	Y		DL	.9	2	-8...	3	.5	11	1								
26	0.9DL + 1WL 180 AZI	Yes	Y		DL	.9	2	-1	3		4	-1								
27	0.9DL + 1WL 210 AZI	Yes	Y		DL	.9	2	-8...	3	-.5	5	-1								
28	0.9DL + 1WL 225 AZI	Yes	Y		DL	.9	2	-7...	3	-.7...	6	-1								
29	0.9DL + 1WL 240 AZI	Yes	Y		DL	.9	2	-.5	3	-.8...	7	-1								
30	0.9DL + 1WL 270 AZI	Yes	Y		DL	.9	2		3	-1	8	-1								
31	0.9DL + 1WL 300 AZI	Yes	Y		DL	.9	2	.5	3	-.8...	9	-1								
32	0.9DL + 1WL 315 AZI	Yes	Y		DL	.9	2	.707	3	-.7...	10	-1								
33	0.9DL + 1WL 330 AZI	Yes	Y		DL	.9	2	.866	3	-.5	11	-1								
34	1.2DL + 1DLi + 1WLi 0 AZI	Yes	Y		DL	1.2	O...	1	13	1	14		15	1						
35	1.2DL + 1DLi + 1WLi 30 AZI	Yes	Y		DL	1.2	O...	1	13	.866	14	.5	16	1						
36	1.2DL + 1DLi + 1WLi 45 AZI	Yes	Y		DL	1.2	O...	1	13	.707	14	.707	17	1						
37	1.2DL + 1DLi + 1WLi 60 AZI	Yes	Y		DL	1.2	O...	1	13	.5	14	.866	18	1						
38	1.2DL + 1DLi + 1WLi 90 AZI	Yes	Y		DL	1.2	O...	1	13		14	1	19	1						
39	1.2DL + 1DLi + 1WLi 120 AZI	Yes	Y		DL	1.2	O...	1	13	-.5	14	.866	20	1						
40	1.2DL + 1DLi + 1WLi 135 AZI	Yes	Y		DL	1.2	O...	1	13	-.7...	14	.707	21	1						
41	1.2DL + 1DLi + 1WLi 150 AZI	Yes	Y		DL	1.2	O...	1	13	-.8...	14	.5	22	1						
42	1.2DL + 1DLi + 1WLi 180 AZI	Yes	Y		DL	1.2	O...	1	13	-1	14		15	-1						
43	1.2DL + 1DLi + 1WLi 210 AZI	Yes	Y		DL	1.2	O...	1	13	-.8...	14	-.5	16	-1						
44	1.2DL + 1DLi + 1WLi 225 AZI	Yes	Y		DL	1.2	O...	1	13	-.7...	14	-.7...	17	-1						
45	1.2DL + 1DLi + 1WLi 240 AZI	Yes	Y		DL	1.2	O...	1	13	-.5	14	-.8...	18	-1						
46	1.2DL + 1DLi + 1WLi 270 AZI	Yes	Y		DL	1.2	O...	1	13		14	-1	19	-1						
47	1.2DL + 1DLi + 1WLi 300 AZI	Yes	Y		DL	1.2	O...	1	13	.5	14	-.8...	20	-1						
48	1.2DL + 1DLi + 1WLi 315 AZI	Yes	Y		DL	1.2	O...	1	13	.707	14	-.7...	21	-1						
49	1.2DL + 1DLi + 1WLi 330 AZI	Yes	Y		DL	1.2	O...	1	13	.866	14	-.5	22	-1						
50	(1.2+0.2Sds)DL + 1E 0 AZI	Yes	Y		DL	1.2...	23	1	24											
51	(1.2+0.2Sds)DL + 1E 30 AZI	Yes	Y		DL	1.2...	23	.866	24	.5										
52	(1.2+0.2Sds)DL + 1E 45 AZI	Yes	Y		DL	1.2...	23	.707	24	.707										
53	(1.2+0.2Sds)DL + 1E 60 AZI	Yes	Y		DL	1.2...	23	.5	24	.866										
54	(1.2+0.2Sds)DL + 1E 90 AZI	Yes	Y		DL	1.2...	23		24	1										
55	(1.2+0.2Sds)DL + 1E 120 AZI	Yes	Y		DL	1.2...	23	-.5	24	.866										
56	(1.2+0.2Sds)DL + 1E 135 AZI	Yes	Y		DL	1.2...	23	-.7...	24	.707										
57	(1.2+0.2Sds)DL + 1E 150 AZI	Yes	Y		DL	1.2...	23	-.8...	24	.5										
58	(1.2+0.2Sds)DL + 1E 180 AZI	Yes	Y		DL	1.2...	23	-1	24											
59	(1.2+0.2Sds)DL + 1E 210 AZI	Yes	Y		DL	1.2...	23	-.8...	24	-.5										
60	(1.2+0.2Sds)DL + 1E 225 AZI	Yes	Y		DL	1.2...	23	-.7...	24	-.7...										
61	(1.2+0.2Sds)DL + 1E 240 AZI	Yes	Y		DL	1.2...	23	-.5	24	-.8...										
62	(1.2+0.2Sds)DL + 1E 270 AZI	Yes	Y		DL	1.2...	23		24	-1										
63	(1.2+0.2Sds)DL + 1E 300 AZI	Yes	Y		DL	1.2...	23	.5	24	-.8...										
64	(1.2+0.2Sds)DL + 1E 315 AZI	Yes	Y		DL	1.2...	23	.707	24	-.7...										



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Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-ft]	LC	MY [lb-ft]	LC	MZ [lb-ft]	LC
1	N25	max	966.495	20	2412.551	39	1596.382	3	218.788	33	1806.122	3	70.389	30
2		min	-972.499	12	95.939	31	-1592.556	27	-2587.536	41	-1805.532	27	-4193.535	38
3	N1	max	972.5	8	2412.54	45	1596.383	17	218.785	19	1805.533	25	4193.513	46
4		min	-966.497	32	95.941	21	-1592.558	25	-2587.526	43	-1806.123	17	-70.39	22
5	N13	max	1622.95	22	2320.108	34	383.399	18	4707.398	34	1540.105	30	684.38	167
6		min	-1622.95	30	63.932	26	-391.133	10	-153.003	26	-1540.105	22	-684.384	223
7	Totals:	max	2966.256	22	6835.481	42	3146.648	18						
8		min	-2966.256	30	1427.994	66	-3146.649	10						

Envelope AISC 15th(360-16): LRFD Steel Code Checks

Member	Shape	Code Check	Loc[in]	LC	Shear Che...	Loc[...]	Dir	LC	phi*Pn...	phi*Pnt...	phi*Mn...	phi*Mn...	Cb	Eqn
1	M1	C3X5	.504	34.856	46	.176	6.536	y	34	11202....	47628	981.263	4104	1... H1-1b
2	M2	PIPE 3.5	.617	40	45	.165	40		9	75262....	78750	7953.75	7953.75	2... H1-1b
3	M3	L2x2x3	.135	0	12	.038	0	z	49	18051....	23392.8	557.717	1239.29	2... H2-1
4	M4	L2x2x3	.150	0	13	.037	0	y	41	18051....	23392.8	557.717	1239.29	2... H2-1
5	M5	6.5"x0.37"...	.229	21	12	.111	21	y	42	3513.8...	75757.5	583.963	6396.0...	1... H1-1b
6	M6	C3X5	.480	34.856	34	.168	63.1...	y	45	37027....	47628	981.263	4020.2...	1... H1-1b
7	M7	PIPE 3.5	.592	40	34	.150	40		2..	75262....	78750	7953.75	7953.75	2... H1-1b
8	M8	L2x2x3	.137	0	2	.036	0	z	38	18051....	23392.8	557.717	1239.29	2... H2-1
9	M9	L2x2x3	.137	0	2	.036	0	y	46	18051....	23392.8	557.717	1239.29	2... H2-1
10	M10	6.5"x0.37"...	.226	21	2	.107	21	y	47	3513.8...	75757.5	583.963	6175.5...	1... H1-1b
11	M11	C3X5	.504	34.856	38	.176	63.1...	y	34	11202....	47628	981.263	4104	1... H1-1b
12	M12	PIPE 3.5	.617	40	39	.165	40		11	75262....	78750	7953.75	7953.75	2... H1-1b
13	M13	L2x2x3	.150	0	7	.037	0	z	43	18051....	23392.8	557.717	1239.29	2... H2-1
14	M14	L2x2x3	.135	0	8	.038	0	y	35	18051....	23392.8	557.717	1239.29	2... H2-1
15	M15	6.5"x0.37"...	.229	21	8	.111	21	y	42	3513.8...	75757.5	583.963	6396.0...	1... H1-1b
16	H1	PIPE 3.5	.119	48	106	.081	24		10	60666....	78750	7953.75	7953.75	1... H1-1b
17	H3	PIPE 3.5	.116	48	146	.077	24		15	60666....	78750	7953.75	7953.75	1... H1-1b
18	H2	PIPE 3.5	.116	48	196	.077	72		5	60666....	78750	7953.75	7953.75	1... H1-1b
19	M19	PIPE 2.0	.123	72	10	.114	24		2	14916....	32130	1871.6...	1871.6...	1... H1-1b
20	M20	PIPE 2.0	.122	24	16	.111	72		8	14916....	32130	1871.6...	1871.6...	1... H1-1b
21	M21	PIPE 2.0	.122	72	4	.111	24		12	14916....	32130	1871.6...	1871.6...	1... H1-1b
22	M22	L6 5/8x4 7...	.097	.875	22	.031	0	y	3	15453....	66065...	1040.5...	3031.0...	1... H2-1
23	M23	L6 5/8x4 7...	.097	41.125	30	.031	42	y	17	15453....	66065...	1040.5...	3031.0...	1... H2-1
24	M24	L6 5/8x4 7...	.100	21	18	.030	0	y	14	15453....	66065...	1040.5...	3031.0...	1... H2-1
25	MP2	PIPE 2.0	.305	48	15	.049	48		5	20866....	32130	1871.6...	1871.6...	1... H1-1b
26	MP1	PIPE 2.0	.309	48	16	.042	48		10	20866....	32130	1871.6...	1871.6...	2... H1-1b
27	MP3	PIPE 2.0	.309	48	4	.042	48		10	20866....	32130	1871.6...	1871.6...	2... H1-1b
28	MP8	PIPE 2.0	.296	48	10	.044	48		10	20866....	32130	1871.6...	1871.6...	1... H1-1b
29	MP7	PIPE 2.0	.282	48	5	.042	48		16	20866....	32130	1871.6...	1871.6...	2... H1-1b
30	MP9	PIPE 2.0	.305	48	10	.036	48		15	20866....	32130	1871.6...	1871.6...	1... H1-1b
31	MP5	PIPE 2.0	.296	48	10	.044	48		10	20866....	32130	1871.6...	1871.6...	1... H1-1b
32	MP4	PIPE 2.0	.305	48	10	.036	48		5	20866....	32130	1871.6...	1871.6...	1... H1-1b
33	MP6	PIPE 2.0	.282	48	15	.042	48		4	20866....	32130	1871.6...	1871.6...	2... H1-1b

APPENDIX D
ADDITIONAL CALCUATIONS

BOLT TOOL 1.5.2

Project Data	
Job Code:	189205
Carrier Site ID:	BOBDL00066A
Carrier Site Name:	CT-CCI-T-842867

Code	
Design Standard:	TIA-222-H
Slip Check:	No
Pretension Standard:	TIA-222-H

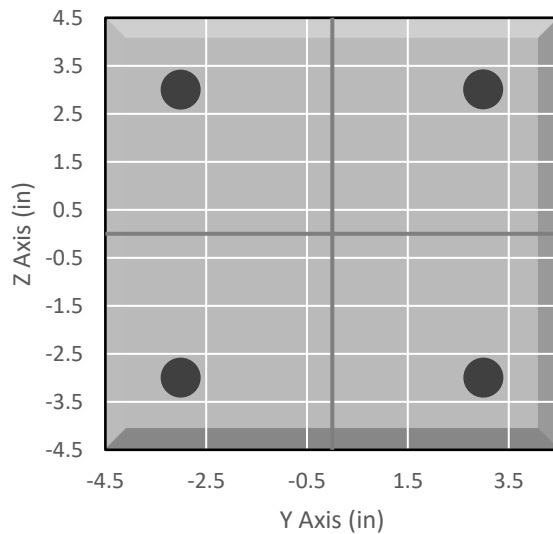
Bolt Properties		
Connection Type:	Bolt	
Diameter:	0.625	in
Grade:	A325	--
Yield Strength (Fy):	92	ksi
Ultimate Strength (Fu):	120	ksi
Number of Bolts:	4	--
Threads Included:	Yes	--
Double Shear:	No	--
Connection Pipe Size:	-	in

Connection Description
Mount to Tower

Bolt Check*		
Tensile Capacity (ϕT_n):	20340.1	lbs
Shear Capacity (ϕV_n):	13805.8	lbs
Tension Force (T_u):	5155.3	lbs
Shear Force (V_u):	735.7	lbs
Tension Usage:	24.1%	--
Shear Usage:	5.1%	--
Interaction:	24.1%	Pass
Controlling Member:	M12	--
Controlling LC:	42	--

*Rating per TIA-222-H Section 15.5

Bolt Layout



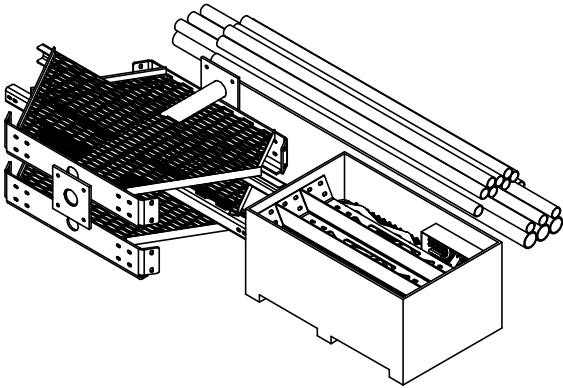
APPENDIX E
SUPPLEMENTAL DRAWINGS

ITEM	PART NO.	DESCRIPTION	QTY.	WEIGHT	NOTE NO.
1	MTC3006SB	STEEL BUNDLE FOR SNUB NOSE PLATFORM	1	402.64 LBS	
2	MCPK8CSB	PIPE STEEL BUNDLE FOR MC-PK8-C	1	464.27 LBS	
3	MCPK8CHWK	HARDWARE KIT FOR MC-PK8-C	1	543.22 LBS	




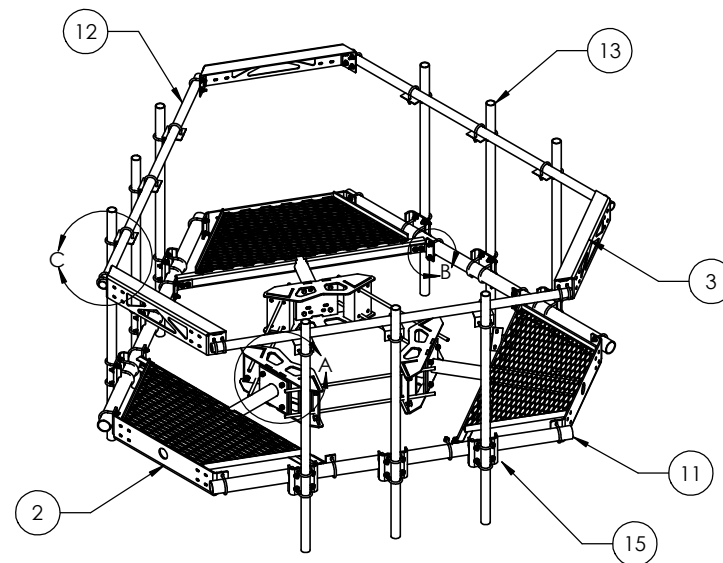
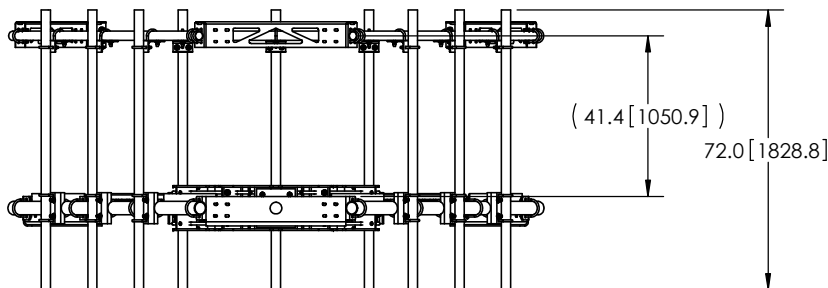
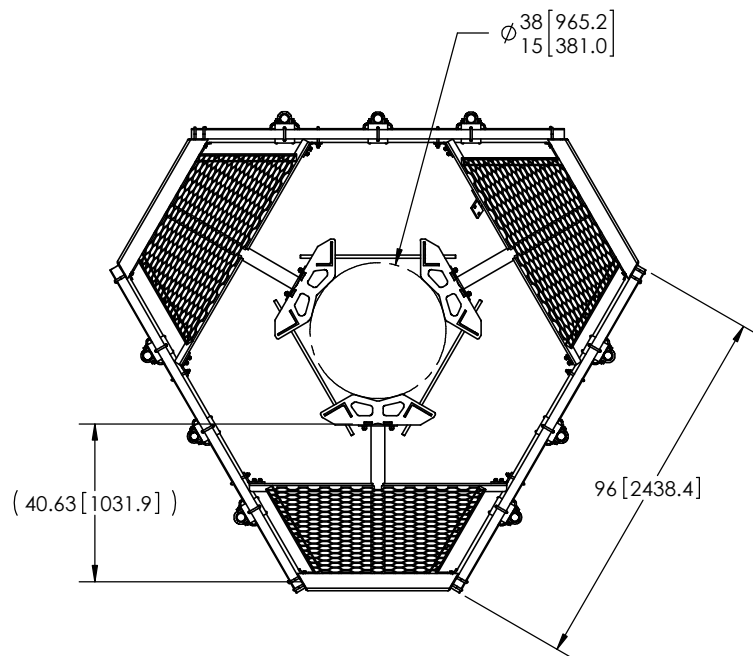
REVISIONS				
REV.	ECN	DESCRIPTION	BY	DATE
A		INITIAL RELEASE	DRR	12/27/11
B	8000005979	CHANGE NOSE CORNER BRKT, ADD GUB-4240	MSM	11/25/14
C	8000007579	NEW RINGMOUNT WELDMNT DESIGN	RJC	04/07/15

FOR BOM ENTRY ONLY




NOTES:
1. CUSTOMER ASSEMBLY SHEETS 2-3.

<small>These drawings and specifications are the proprietary property of ANDREW CORPORATION and may be used only for the specific purpose authorized in writing by Andrew Corporation.</small>				
DESIGNED BY: MSM	SHEET: 1 of 3	PART NUMBER: MC-PK8-C		
CHECKED BY: TP	SCALE: NTS	DESCRIPTION: LOW PROFILE PLATFORM KIT 8' FACE		
DATE: 10/18/11	MATERIAL: A36, A500	DRAWING TYPE: ASSEMBLY DRAWING		
REVISION: C	FINISH: GALV A123	WEIGHT: 1410.14 LBS		
DO NOT SCALE THIS PRINT			 WESTCHESTER, IL. 60154 U.S.A.	

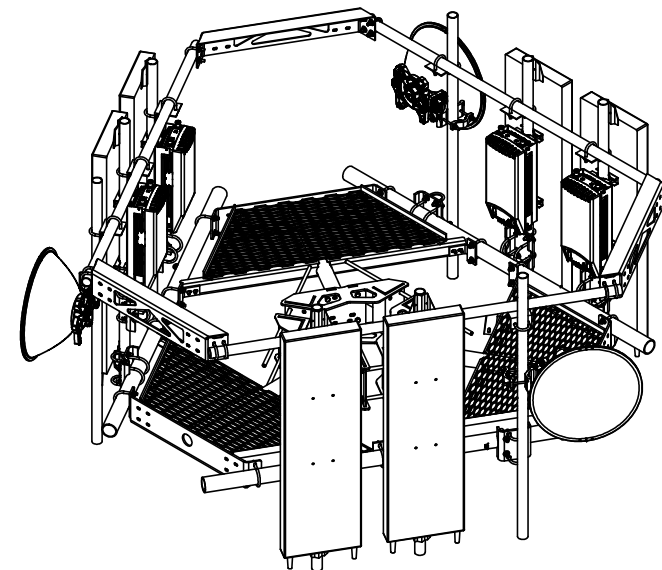


ITEM	PART NO.	DESCRIPTION	QTY.	WEIGHT
1	MC-RM1550-3	12" - 50" OD RINGMOUNT	1	230.42 LBS
2	MTC300601	Low Profile Co-Location Platform Snub Nose	3	134.21 LBS
3	MT195801	Corner Weldment Snub Nose Handrail	3	27.10 LBS
4	XA2020.01	CROSS OVER ANGLE	9	2.65 LBS
5	GUB-4356	1/2" X 3-5/8" X 6" GALV U-BOLT	18	0.82 LBS
6	GUB-4355	1/2" X 3-5/8" X 5" GALV U-BOLT	12	0.71 LBS
7	GUB-4240	1/2" X 2-1/2" X 4" GALV U-BOLT	48	0.56 LBS
8	GB-04145	1/2" X 1-1/2" GALV BOLT KIT	12	0.13 LBS
9	GWF-04	1/2" GALV FLAT WASHER	24	0.03 LBS
10	GB-0520A	5/8" X 2" GALV BOLT KIT (A325)	12	0.27 LBS
11	MT54796	3.50" OD X 96" GALV PIPE	3	60.28 LBS
12	MT-651-96	Ø2.375" OD X 96" PIPE	3	29.07 LBS
13	MT-651	2.375" OD x 72" PIPE	9	21.80 LBS
14	MT19617	MT196 Pipe Mount Plate	6	2.49 LBS
15	MT21701	PIPE MOUNT PLATE	9	7.93 LBS

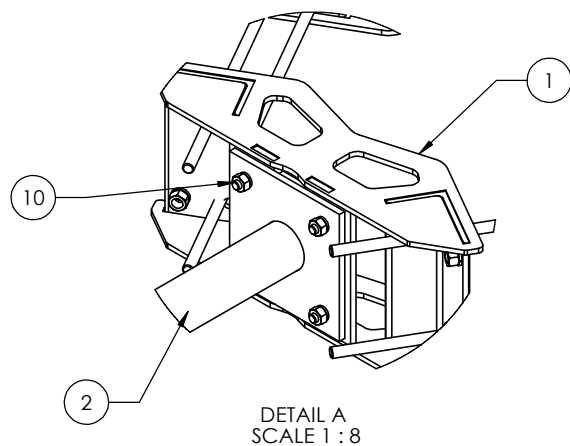
THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPRIETARY PROPERTY OF ANDREW CORPORATION AND MAY BE USED ONLY FOR THE SPECIFIC PURPOSES AUTHORIZED IN WRITING BY ANDREW CORPORATION.			
DESIGNED BY: MSM	DATE: 10/18/11	REVISED: C	WEIGHT: 1361.27 LBS
CHECKED BY: TP	DATE: 10/18/11	REVISION: C	WEIGHT: 1361.27 LBS
ALL DIMENSIONS ARE IN INCHES U.S.S. TOLERANCES UNLESS OTHERWISE SPECIFIED: .X = ± .12 ANGLES ±2° .XX = ± .06 FRACTIONS ±1/32 .XXX = ± .03 REMOVE BURRS AND BREAK EDGES .005 DO NOT SCALE THIS PRINT			
DRAWING NO: 25" OD Snub Nose MT-196 MATERIAL: A36, A53 FINISH: GALV A123		DRAWING TYPE: ASSEMBLY DRAWING  WESTCHESTER, IL 60154 U.S.A.	

NOTES:

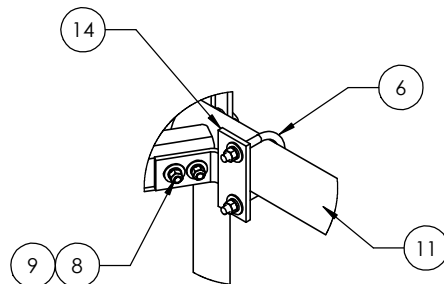
1. ALL METRIC DIMENSIONS ARE IN BRACKETS.
2. WILL FIT MONOPOLES 15"—38" OD.



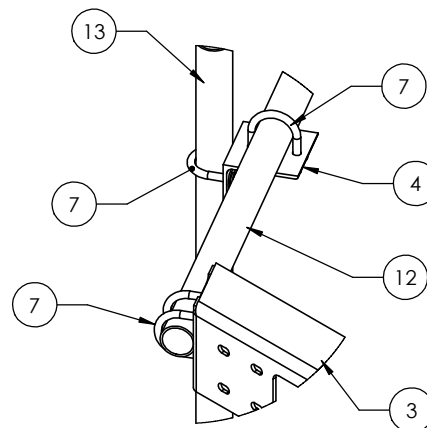
WITH ANTENNAS



**DETAIL A
SCALE 1:8**



**DETAIL B
SCALE 1:8**



**DETAIL C
SCALE 1:8**

NOTES:
1. ALL METRIC DIMENSIONS ARE IN BRACKETS.


These drawings and specifications are the proprietary property of ANDREW CORPORATION and may be used only for the specific purposes authorized in writing by Andrew Corporation.			DRAWN BY: MSM	SHEET: 3 of 3	PART NUMBER: MC-PK8-C
ALL DIMENSIONS ARE IN INCHES U.O.S. TOLERANCES UNLESS OTHERWISE SPECIFIED: .X = ± .12 ANGLES ±2° .XX = ± .06 FRACTIONS ±1/32 .XXX = ± .03 REMOVE BURRS AND BREAK EDGES .005			CHECKED BY: TP	SCALE: NTS	DESCRIPTION: 25" OD Snub Nose MT-196
DO NOT SCALE THIS PRINT			DATE: 10/18/11	MATERIAL: A36, A53	DRAWING TYPE: ASSEMBLY DRAWING
			REVISION: C	FINISH: GALV A123	WEIGHT: 1361.27 LBS
			 ANDREW ® WESTCHESTER, IL. 60154 U.S.A.		

Exhibit F

Power Density/RF Emissions Report



EBI Consulting

environmental | engineering | due diligence

RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

Dish Wireless Existing Facility

Site ID: BOBDL00066A

842867

497 Middle Turnpike

Storrs Mansfield, Connecticut 06268

August 31, 2021

EBI Project Number: 6221004797

Site Compliance Summary	
Compliance Status:	COMPLIANT
Site total MPE% of FCC general population allowable limit:	10.83%

August 31, 2021

Dish Wireless

Emissions Analysis for Site: BOBDL00066A - 842867

EBI Consulting was directed to analyze the proposed Dish Wireless facility located at **497 Middle Turnpike** in **Storrs Mansfield, Connecticut** for the purpose of determining whether the emissions from the Proposed Dish Wireless Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits; therefore, it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limits for the 600 MHz and 700 MHz frequency bands are approximately $400 \mu\text{W}/\text{cm}^2$ and $467 \mu\text{W}/\text{cm}^2$, respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 11 GHz frequency bands is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully



aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed Dish Wireless antenna facility located at 497 Middle Turnpike in Storrs Mansfield, Connecticut using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since Dish Wireless is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 20 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was focused at the base of the tower. For this report, the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 4 n71 channels (600 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) 4 n70 channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
- 3) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 4) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 20 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.



- 5) The antennas used in this modeling are the JMA MX08FRO665-2I for the 600 MHz / 1900 MHz channel(s) in Sector A, the JMA MX08FRO665-2I for the 600 MHz / 1900 MHz channel(s) in Sector B, the JMA MX08FRO665-2I for the 600 MHz / 1900 MHz channel(s) in Sector C. This is based on feedback from the carrier with regard to anticipated antenna selection. All Antenna gain values and associated transmit power levels are shown in the Site Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 20 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 6) The antenna mounting height centerline of the proposed antennas is 99 feet above ground level (AGL).
- 7) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 8) All calculations were done with respect to uncontrolled / general population threshold limits.



Dish Wireless Site Inventory and Power Data

Sector:	A	Sector:	B	Sector:	C
Antenna #:	I	Antenna #:	I	Antenna #:	I
Make / Model:	JMA MX08FRO665-2I	Make / Model:	JMA MX08FRO665-2I	Make / Model:	JMA MX08FRO665-2I
Frequency Bands:	600 MHz / 1900 MHz	Frequency Bands:	600 MHz / 1900 MHz	Frequency Bands:	600 MHz / 1900 MHz
Gain:	17.45 dBd / 22.65 dBd	Gain:	17.45 dBd / 22.65 dBd	Gain:	17.45 dBd / 22.65 dBd
Height (AGL):	99 feet	Height (AGL):	99 feet	Height (AGL):	99 feet
Channel Count:	8	Channel Count:	8	Channel Count:	8
Total TX Power (W):	280 Watts	Total TX Power (W):	280 Watts	Total TX Power (W):	280 Watts
ERP (W):	3,065.51	ERP (W):	3,065.51	ERP (W):	3,065.51
Antenna AI MPE %:	1.83%	Antenna BI MPE %:	1.83%	Antenna CI MPE %:	1.83%



EBI Consulting

environmental | engineering | due diligence

Site Composite MPE %	
Carrier	MPE %
Dish Wireless (Max at Sector A):	1.83%
AT&T	3.93%
Verizon	5.07%
Site Total MPE % :	10.83%

Dish Wireless MPE % Per Sector	
Dish Wireless Sector A Total:	1.83%
Dish Wireless Sector B Total:	1.83%
Dish Wireless Sector C Total:	1.83%
Site Total MPE % :	10.83%

Dish Wireless Maximum MPE Power Values (Sector A)							
Dish Wireless Frequency Band / Technology (Sector A)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ($\mu\text{W}/\text{cm}^2$)	Frequency (MHz)	Allowable MPE ($\mu\text{W}/\text{cm}^2$)	Calculated % MPE
Dish Wireless 600 MHz n71	4	223.68	99.0	3.72	600 MHz n71	400	0.93%
Dish Wireless 1900 MHz n70	4	542.70	99.0	9.02	1900 MHz n70	1000	0.90%
						Total:	1.83%

• NOTE: Totals may vary by approximately 0.01% due to summation of remainders in calculations.



Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the Dish Wireless facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

Dish Wireless Sector	Power Density Value (%)
Sector A:	1.83%
Sector B:	1.83%
Sector C:	1.83%
Dish Wireless Maximum MPE % (Sector A):	1.83%
Site Total:	10.83%
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **10.83%** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

Exhibit G

Letter of Authorization



4545 E River Rd, Suite 320
West Henrietta, NY 14586

Phone: (585) 445-5896
Fax: (724) 416-4461
www.crowncastle.com

Crown Castle Letter of Authorization

CT - CONNECTICUT SITING COUNCIL

Melanie A. Bachman
Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Re: Tower Share Application
Crown Castle telecommunications site at:
497 MIDDLE TURNPIKE, STORRS MANSFIELD, CT 06268

CCATT LLC ("Crown Castle") hereby authorizes DISH Wireless LLC, including their Agent, to act as our Agent in the processing of all zoning applications, building permits and approvals through the CT - CONNECTICUT SITING COUNCIL for the existing wireless communications site described below:

Crown Site ID/Name: 842867/MANSFIELD FOUR CORNERS
Customer Site ID: BOBDL00066A/CT-CCI-T-842867
Site Address: 497 MIDDLE TURNPIKE, STORRS MANSFIELD, CT 06268

Crown Castle

By: _____

A handwritten signature in black ink, appearing to read 'Richard Zajac'.


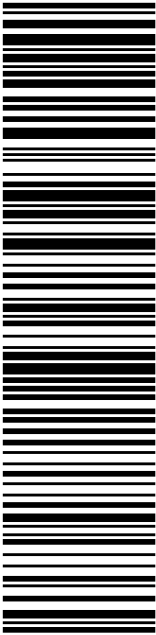
Richard Zajac
Site Acquisition Specialist

9/7/2021

Date: _____

Exhibit H

Recipient Mailings

 Click-N-Ship®	
P	usps.com US POSTAGE Flat Rate Env 09/21/2021 Mailed from 01566
PRIORITY MAIL 2-DAY™	
DEBORAH CHASE NORTHEAST SITE SOLUTIONS 420 MAIN ST STE 1 STURBRIDGE MA 01566-1359	Expected Delivery Date: 09/24/21 Re#: DS-842867 0006
SHIP TO: TONI MORAN MAYOR OF MANSFIELD 4 S EAGLEVILLE RD STORRS MANFLD CT 06268-2574	
USPS TRACKING #  9405 5036 9930 0010 1022 36	
Electronic Rate Approved #038555749	

✂ ————— Cut on dotted line.

Instructions


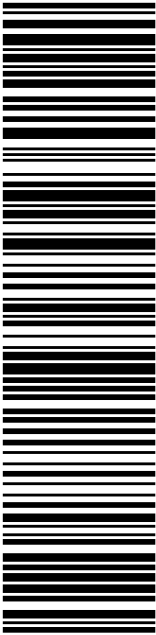
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USPS TRACKING # : 9405 5036 9930 0010 1022 36	
Trans. #: 544110718 Print Date: 09/21/2021 Ship Date: 09/21/2021 Expected Delivery Date: 09/24/2021	Priority Mail® Postage: \$7.95 Total: \$7.95
From: DEBORAH CHASE NORTHEAST SITE SOLUTIONS 420 MAIN ST STE 1 STURBRIDGE MA 01566-1359	
To: TONI MORAN MAYOR OF MANSFIELD 4 S EAGLEVILLE RD STORRS MANFLD CT 06268-2574	
Re#: DS-842867	
<small>* Retail Pricing Priority Mail rates apply. There is no fee for USPS Tracking® service on Priority Mail service with use of this electronic rate shipping label. Refunds for unused postage paid labels can be requested online 30 days from the print date.</small>	



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 Click-N-Ship®	
P	usps.com US POSTAGE Flat Rate Env \$7.95 9405 5036 9930 0010 1022 43 0000 0000 0010 6268
09/21/2021	Mailed from 01566
PRIORITY MAIL 2-DAY™	
DEBORAH CHASE NORTHEAST SITE SOLUTIONS 420 MAIN ST STE 1 STURBRIDGE MA 01566-1359	Expected Delivery Date: 09/24/21 Re#: DS-842867 0006
C002	
SHIP TO: LINDA PAINTER DIRECTOR-PLANNING & ZONING-TOWN OF 4 S EAGLEVILLE RD TOWN PLANNING OFFICE STORRS CT 06268-2574	
USPS TRACKING #	
	
9405 5036 9930 0010 1022 43	
Electronic Rate Approved #038555749	



Cut on dotted line.

Instructions


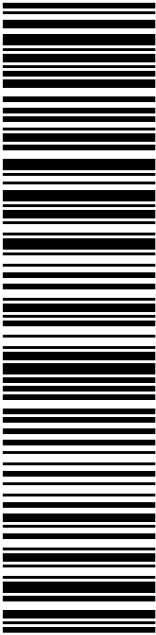
- Each Click-N-Ship® label is unique. Labels are to be used as printed and used only once. DO NOT PHOTO COPY OR ALTER LABEL.
- Place your label so it does not wrap around the edge of the package.
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Click-N-Ship® Label Record

USPS TRACKING # : 9405 5036 9930 0010 1022 43	
Trans. #: 544110718 Print Date: 09/21/2021 Ship Date: 09/21/2021 Expected Delivery Date: 09/24/2021	Priority Mail® Postage: \$7.95 Total: \$7.95
From: DEBORAH CHASE NORTHEAST SITE SOLUTIONS 420 MAIN ST STE 1 STURBRIDGE MA 01566-1359	
To: LINDA PAINTER DIRECTOR-PLANNING & ZONING-TOWN OF MANSFIELD 4 S EAGLEVILLE RD TOWN PLANNING OFFICE STORRS CT 06268-2574	
Re#: DS-842867	
<small>* Retail Pricing Priority Mail rates apply. There is no fee for USPS Tracking® service on Priority Mail service with use of this electronic rate shipping label. Refunds for unused postage paid labels can be requested online 30 days from the print date.</small>	



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 Click-N-Ship®	
P	usps.com US POSTAGE Flat Rate Env \$7.95 9405 5036 9930 0010 1022 50 0000 0000 0031 4586 U.S. POSTAGE PAID Click-N-Ship®
09/21/2021	Mailed from 01566
PRIORITY MAIL 2-DAY™	
DEBORAH CHASE NORTHEAST SITE SOLUTIONS 420 MAIN ST STE 1 STURBRIDGE MA 01566-1359	Expected Delivery Date: 09/24/21 Re#: DS-884867 0006
SHIP TO: RICH ZAJAC CROWN CASTLE 4545 E RIVER RD STE 320 W HENRIETTA NY 14586-9024	
USPS TRACKING #  9405 5036 9930 0010 1022 50	
Electronic Rate Approved #038555749	

R013



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Click-N-Ship® Label Record

USPS TRACKING # :
9405 5036 9930 0010 1022 50

Trans. #: 544110718
 Print Date: 09/21/2021
 Ship Date: 09/21/2021
 Expected Delivery Date: 09/24/2021

Priority Mail® Postage: **\$7.95**
 Total: **\$7.95**

From: DEBORAH CHASE
 NORTHEAST SITE SOLUTIONS
 420 MAIN ST
 STE 1
 STURBRIDGE MA 01566-1359


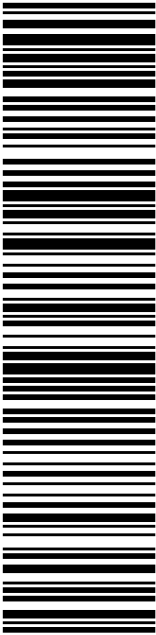
Re#: DS-884867

To: RICH ZAJAC
 CROWN CASTLE
 4545 E RIVER RD
 STE 320
 W HENRIETTA NY 14586-9024

* Retail Pricing Priority Mail rates apply. There is no fee for USPS Tracking® service on Priority Mail service with use of this electronic rate shipping label. Refunds for unused postage paid labels can be requested online 30 days from the print date.



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 Click-N-Ship®	
P	usps.com US POSTAGE Flat Rate Env 09/21/2021 Mailed from 01566
PRIORITY MAIL 2-DAY™	
DEBORAH CHASE NORTHEAST SITE SOLUTIONS 420 MAIN ST STE 1 STURBRIDGE MA 01566-1359	Expected Delivery Date: 09/24/21 Re#: DS-842867 0006
SHIP TO: JZ INVESTMENTS CMC STORRS SPV LLC 1 HARBOR POINT RD APT 1855 STAMFORD CT 06902-7351	
USPS TRACKING #  9405 5036 9930 0010 1022 67	
Electronic Rate Approved #038555749	

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Click-N-Ship® Label Record

USPS TRACKING # : 9405 5036 9930 0010 1022 67	
Trans. #: 544110718 Print Date: 09/21/2021 Ship Date: 09/21/2021 Expected Delivery Date: 09/24/2021	Priority Mail® Postage: \$7.95 Total: \$7.95
From: DEBORAH CHASE NORTHEAST SITE SOLUTIONS 420 MAIN ST STE 1 STURBRIDGE MA 01566-1359	
To: JZ INVESTMENTS CMC STORRS SPV LLC 1 HARBOR POINT RD APT 1855 STAMFORD CT 06902-7351	
Re#: DS-842867	
<small>* Retail Pricing Priority Mail rates apply. There is no fee for USPS Tracking® service on Priority Mail service with use of this electronic rate shipping label. Refunds for unused postage paid labels can be requested online 30 days from the print date.</small>	



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842867-MANSFIELD



FIKSDALE
458 MAIN ST
FIKSDALE, MA 01518-9998
(800)275-8777

09/23/2021

02:07 PM

Product	Qty	Unit Price	Price
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Prepaid Mail	1		\$0.00
West Henrietta, NY 14586			
Weight: 0 lb 2.00 oz			
Acceptance Date:			
Thu 09/23/2021			
Tracking #:			
9405 5036 9930 0010 1022 50			

Prepaid Mail	1		\$0.00
Storrs Mansfield, CT 06268			
Weight: 1 lb 2.90 oz			
Acceptance Date:			
Thu 09/23/2021			
Tracking #:			
9405 5036 9930 0010 1022 36			

Prepaid Mail	1		\$0.00
Stamford, CT 06902			
Weight: 1 lb 2.90 oz			
Acceptance Date:			
Thu 09/23/2021			
Tracking #:			
9405 5036 9930 0010 1022 67			

Prepaid Mail	1		\$0.00
Storrs Mansfield, CT 06268			
Weight: 1 lb 3.00 oz			
Acceptance Date:			
Thu 09/23/2021			
Tracking #:			
9405 5036 9930 0010 1022 43			

Grand Total:			\$0.00
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Date: **May 28, 2021**



Crown Castle
2000 Corporate Drive
Canonsburg, PA 15317
(724) 416-2000

Subject: **Structural Analysis Report**

Carrier Designation: **DISH Network Co-Locate**
Site Number: BOBDL00066A
Site Name: CT-CCI-T-842867

Crown Castle Designation: **BU Number:** 842867
Site Name: MANSFIELD FOUR CORNERS
JDE Job Number: 650055
Work Order Number: 1966176
Order Number: 556626 Rev. 1

Engineering Firm Designation: **Crown Castle Project Number:** 1966176

Site Data: **497 MIDDLE TURNPIKE, STORRS MANSFIELD,**
TOLLAND County, CT
Latitude 41° 49' 32.81", Longitude -72° 16' 54.46"
120 Foot - Monopole Tower

Crown Castle is pleased to submit this "**Structural Analysis Report**" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Proposed Equipment Configuration

Sufficient Capacity

This analysis utilizes an ultimate 3-second gust wind speed of 125 mph as required by the 2018 Connecticut Building Code. Applicable Standard references and design criteria are listed in Section 2 - "Analysis Criteria".

Structural analysis prepared by: Rohit Soni

Respectfully submitted by:

Maribel Dentinger

Maribel Dentinger, P.E.
Senior Project Engineer

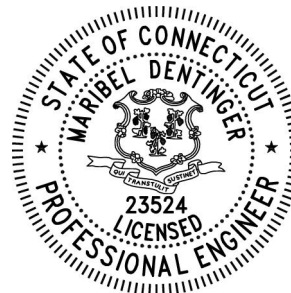


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1) INTRODUCTION

This tower is a 120 ft Monopole tower designed by PENNSUMMIT TUBULAR, LLC.

2) ANALYSIS CRITERIA

TIA-222 Revision:	TIA-222-H
Risk Category:	II
Wind Speed:	125 mph
Exposure Category:	B
Topographic Factor:	1
Ice Thickness:	2 in
Wind Speed with Ice:	50 mph
Seismic Ss:	0.175
Seismic S1:	0.064
Service Wind Speed:	60 mph

Table 1 - Proposed Equipment Configuration

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
99.0	99.0	3	fujitsu	TA08025-B604	1	1-1/2
		3	fujitsu	TA08025-B605		
		3	jma wireless	MX08FRO665-21 w/ Mount Pipe		
		1	raycap	RDIDC-9181-PF-48		
		1	tower mounts	Commscope MC-PK8-DSH		

Table 2 - Other Considered Equipment

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
120.0	122.0	1	andrew	SBNH-1D6565C w/ Mount Pipe	12 1 2	1-1/4 3/8 7/8
		2	cci antennas	HPA-65R-BUU-H6 w/ Mount Pipe		
		1	cci antennas	HPA-65R-BUU-H8 w/ Mount Pipe		
		3	ericsson	RRUS 11		
		3	ericsson	RRUS 32 B2		
		3	kathrein	78211056		
		2	kmw communications	AM-X-CD-16-65-00T-RET w/ Mount Pipe		
		6	powerwave technologies	7020.00		
		3	powerwave technologies	7770.00 w/ Mount Pipe		
		6	powerwave technologies	LGP 17201		
		1	raycap	DC6-48-60-18-8F		
	120.0	1	tower mounts	Platform Mount [LP 303-1]		

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
109.0	109.0	3	alcatel lucent	RRH 2x40-700 W/SOLAR	19	1-5/8
		3	alcatel lucent	RRH2X60-1900		
		3	alcatel lucent	RRH2X60-AWS		
		6	commscope	HBXX-6517DS-A2M w/ Mount Pipe		
		1	commscope	LNx-4514DS-A1M w/ Mount Pipe		
		2	commscope	LNx-6514DS-A1M w/ Mount Pipe		
		3	commscope	LNx-8513DS-VTM w/ Mount Pipe		
		2	rfs celwave	DB-T1-6Z-8AB-0Z		
		1	tower mounts	Platform Mount [LP 303-1]		

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Reference	Source
4-GEOTECHNICAL REPORTS	4713232	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	4858941	CCISITES
4-TOWER MANUFACTURER DRAWINGS	5214860	CCISITES

3.1) Analysis Method

tnxTower (version 8.0.9.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

3.2) Assumptions

- 1) Tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	120 - 70.75	Pole	TP32.28x18x0.1875	1	-12.94	1130.77	60.6	Pass
L2	70.75 - 34.75	Pole	TP42.35x30.7452x0.3125	2	-19.73	2468.37	40.7	Pass
L3	34.75 - 0	Pole	TP51.8x40.2019x0.375	3	-31.21	3759.73	37.0	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
							Summary	
						Pole (L1)	60.6	Pass
						Rating =	60.6	Pass

Table 5 - Tower Component Stresses vs. Capacity - LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	31.7	Pass
1	Base Plate	0	28.1	Pass
1	Base Foundation (Structure)	0	47.6	Pass
1	Base Foundation (Soil Interaction)	0	48.3	Pass

Structure Rating (max from all components) =	60.6%
---	--------------

Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

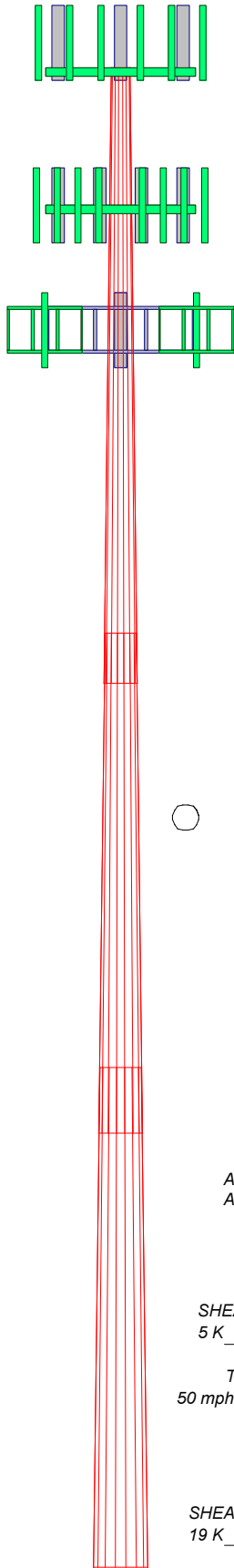
Section	1	2	3	
Length (ft)	49.25	40.00	40.00	
Number of Sides	18	18	18	
Thickness (in)	0.1875	0.3125	0.3750	
Socket Length (ft)	4.00	5.25	40.2019	
Top Dia (in)	18.0000	30.7452	51.8000	
Bot Dia (in)	32.2800	42.3500		
Grade		A607-65		
Weight (K)	2.5	4.9	7.4	14.8

120.0 ft

70.8 ft

34.8 ft

0.0 ft



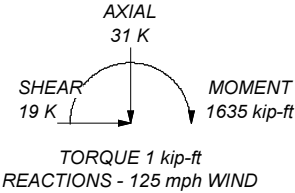
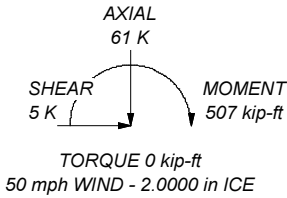
MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A607-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower is located in Tolland County, Connecticut.
2. Tower designed for Exposure B to the TIA-222-H Standard.
3. Tower designed for a 125 mph basic wind in accordance with the TIA-222-H Standard.
4. Tower is also designed for a 50 mph basic wind with 2.00 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Risk Category II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING: 60.6%

ALL REACTIONS
ARE FACTORED



Crown Castle
2000 Corporate Drive
Canonsburg, PA 15317
Phone: (724) 416-2000
FAX:

Job:	842867		
Project:			
Client:	Crown Castle	Drawn by:	RSoni
Code:	TIA-222-H	Date:	05/28/21
Path:	C:\Work Area\842867\WO 1966176 - SAIProd\20171019_APP411405_842867.dwg		
		App'd:	
		Scale:	NTS
		Dwg No.	E-1

Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

- Tower is located in Tolland County, Connecticut.
- Tower base elevation above sea level: 0.00 ft.
- Basic wind speed of 125 mph.
- Risk Category II.
- Exposure Category B.
- Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- Topographic Category: 1.
- Crest Height: 0.00 ft.
- Nominal ice thickness of 2.0000 in.
- Ice thickness is considered to increase with height.
- Ice density of 56 pcf.
- A wind speed of 50 mph is used in combination with ice.
- Temperature drop of 50 °F.
- Deflections calculated using a wind speed of 60 mph.
- A non-linear (P-delta) analysis was used.
- Pressures are calculated at each section.
- Stress ratio used in pole design is 1.05.
- Tower analysis based on target reliabilities in accordance with Annex S.
- Load Modification Factors used: $K_{es}(F_w) = 0.95$, $K_{es}(t_i) = 0.85$.
- Maximum demand-capacity ratio is: 1.
- Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Horizontals	Assume Legs Pinned	Calculate Redundant Bracing Forces
Consider Moments - Diagonals	✓ Assume Rigid Index Plate	Ignore Redundant Members in FEA
Use Moment Magnification	✓ Use Clear Spans For Wind Area	SR Leg Bolts Resist Compression
Use Code Stress Ratios	Use Clear Spans For KL/r	All Leg Panels Have Same Allowable
✓ Use Code Safety Factors - Guys	Retention Guys To Initial Tension	Offset Girt At Foundation
Escalate Ice	✓ Bypass Mast Stability Checks	✓ Consider Feed Line Torque
Always Use Max Kz	✓ Use Azimuth Dish Coefficients	Include Angle Block Shear Check
Use Special Wind Profile	✓ Project Wind Area of Appurt.	Use TIA-222-H Bracing Resist.
Include Bolts In Member Capacity	Autocalc Torque Arm Areas	Exemption
Leg Bolts Are At Top Of Section	Add IBC .6D+W Combination	Use TIA-222-H Tension Splice
Secondary Horizontal Braces Leg	✓ Sort Capacity Reports By Component	Exemption
Use Diamond Inner Bracing (4 Sided)	Triangulate Diamond Inner Bracing	
SR Members Have Cut Ends	Treat Feed Line Bundles As Cylinder	
SR Members Are Concentric	Ignore KL/ry For 60 Deg. Angle Legs	

Poles

- ✓ Include Shear-Torsion Interaction
- Always Use Sub-Critical Flow
- Use Top Mounted Sockets
- Pole Without Linear Attachments
- Pole With Shroud Or No
- Appurtenances
- Outside and Inside Corner Radii Are Known

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	120.00-70.75	49.25	4.00	18	18.0000	32.2800	0.1875	0.7500	A607-65 (65 ksi)
L2	70.75-34.75	40.00	5.25	18	30.7452	42.3500	0.3125	1.2500	A607-65 (65 ksi)
L3	34.75-0.00	40.00		18	40.2019	51.8000	0.3750	1.5000	A607-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	It/Q in ²	w in	w/t
L1	18.2488	10.6007	424.9328	6.3234	9.1440	46.4712	850.4248	5.3013	2.8380	15.136
	32.7490	19.0990	2485.1711	11.3928	16.3982	151.5511	4973.6122	9.5513	5.3513	28.54
L2	32.3497	30.1854	3531.9616	10.8036	15.6186	226.1387	7068.5707	15.0956	4.8612	15.556
	42.9551	41.6959	9309.0430	14.9233	21.5138	432.7010	18630.335	20.8519	6.9036	22.092
L3	42.3099	47.4039	9499.5752	14.1385	20.4226	465.1513	19011.650	23.7065	6.4155	17.108
	52.5412	61.2086	20450.246	18.2559	26.3144	777.1504	40927.401	30.6101	8.4568	22.551

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _r	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontal in	Double Angle Stitch Bolt Spacing Redundants in
L1 120.00- 70.75				1	1	1			
L2 70.75- 34.75				1	1	1			
L3 34.75-0.00				1	1	1			

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Sector	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	Number Per Row	Start/End Position	Width or Diameter in	Perimeter in	Weight plf
HJ7-50A(1-5/8)	A	No	Surface Ar (CaAa)	109.00 - 0.00	7	7	-0.100 0.100	1.9800		1.04

CU12PSM9P6XXX(1- 1/2)	C	No	Surface Ar (CaAa)	99.00 - 0.00	1	1	-0.450 -0.400	1.6000		2.35

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		CaA _A ft²/ft	Weight plf

HJ7-50A(1-5/8)	A	No	No	Inside Pole	109.00 - 0.00	12	No Ice	0.00	1.04
							1/2" Ice	0.00	1.04
							1" Ice	0.00	1.04
							2" Ice	0.00	1.04

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		C _A A _A ft ² /ft	Weight plf
LDF6-50A(1-1/4)	C	No	No	Inside Pole	120.00 - 0.00	12	No Ice	0.00	0.60
							1/2" Ice	0.00	0.60
							1" Ice	0.00	0.60
							2" Ice	0.00	0.60
WR-VG86ST- BRDA(7/8)	C	No	No	Inside Pole	120.00 - 0.00	2	No Ice	0.00	0.68
							1/2" Ice	0.00	0.68
							1" Ice	0.00	0.68
							2" Ice	0.00	0.68
FB-L98B-002- XXX(3/8)	C	No	No	Inside Pole	120.00 - 0.00	1	No Ice	0.00	0.06
							1/2" Ice	0.00	0.06
							1" Ice	0.00	0.06
							2" Ice	0.00	0.06

Feed Line/Linear Appurtenances Section Areas

Tower Section n	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L1	120.00-70.75	A	0.000	0.000	53.014	0.000	0.76
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	4.520	0.000	0.49
L2	70.75-34.75	A	0.000	0.000	49.896	0.000	0.71
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	5.760	0.000	0.40
L3	34.75-0.00	A	0.000	0.000	48.163	0.000	0.69
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	5.560	0.000	0.38

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section n	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L1	120.00-70.75	A	1.887	0.000	0.000	84.308	0.000	1.84
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	15.179	0.000	0.72
L2	70.75-34.75	A	1.780	0.000	0.000	79.349	0.000	1.73
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	19.343	0.000	0.68
L3	34.75-0.00	A	1.589	0.000	0.000	75.671	0.000	1.61
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	17.933	0.000	0.64

Feed Line Center of Pressure

Section	Elevation ft	CP _x in	CP _z in	CP _x Ice in	CP _z Ice in
L1	120.00-70.75	-4.5312	-2.5253	-3.2180	-1.7074
L2	70.75-34.75	-5.4901	-3.0239	-3.8565	-1.9787
L3	34.75-0.00	-5.9232	-3.2620	-4.2931	-2.2118

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L1	3	HJ7-50A(1-5/8)	70.75 - 109.00	1.0000	1.0000
L1	14	CU12PSM9P6XXX(1-1/2)	70.75 - 99.00	1.0000	1.0000
L2	3	HJ7-50A(1-5/8)	34.75 - 70.75	1.0000	1.0000
L2	14	CU12PSM9P6XXX(1-1/2)	34.75 - 70.75	1.0000	1.0000
L3	3	HJ7-50A(1-5/8)	0.00 - 34.75	1.0000	1.0000
L3	14	CU12PSM9P6XXX(1-1/2)	0.00 - 34.75	1.0000	1.0000

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft

7770.00 w/ Mount Pipe	A	From Leg	4.00 0.00 2.00	0.0000	120.00
7770.00 w/ Mount Pipe	B	From Leg	4.00 0.00 2.00	0.0000	120.00
7770.00 w/ Mount Pipe	C	From Leg	4.00 0.00 2.00	0.0000	120.00
AM-X-CD-16-65-00T-RET w/ Mount Pipe	A	From Leg	4.00 0.00 2.00	0.0000	120.00
AM-X-CD-16-65-00T-RET w/ Mount Pipe	B	From Leg	4.00 0.00 2.00	0.0000	120.00
SBNH-1D6565C w/ Mount Pipe	C	From Leg	4.00 0.00 2.00	0.0000	120.00
RRUS 11	A	From Leg	4.00 0.00 2.00	0.0000	120.00
RRUS 11	B	From Leg	4.00 0.00 2.00	0.0000	120.00
RRUS 11	C	From Leg	4.00 0.00 2.00	0.0000	120.00
DC6-48-60-18-8F	A	From Leg	4.00 0.00 2.00	0.0000	120.00
HPA-65R-BUU-H6 w/ Mount Pipe	A	From Leg	4.00 0.00 2.00	0.0000	120.00

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft
HPA-65R-BUU-H6 w/ Mount Pipe	B	From Leg	4.00 0.00 2.00	0.0000	120.00
HPA-65R-BUU-H8 w/ Mount Pipe	C	From Leg	4.00 0.00 2.00	0.0000	120.00
(2) LGP 17201	A	From Leg	4.00 0.00 2.00	0.0000	120.00
(2) LGP 17201	B	From Leg	4.00 0.00 2.00	0.0000	120.00
(2) LGP 17201	C	From Leg	4.00 0.00 2.00	0.0000	120.00
(2) 7020.00	A	From Leg	4.00 0.00 2.00	0.0000	120.00
(2) 7020.00	B	From Leg	4.00 0.00 2.00	0.0000	120.00
(2) 7020.00	C	From Leg	4.00 0.00 2.00	0.0000	120.00
78211056	A	From Leg	4.00 0.00 2.00	0.0000	120.00
78211056	B	From Leg	4.00 0.00 2.00	0.0000	120.00
78211056	C	From Leg	4.00 0.00 2.00	0.0000	120.00
RRUS 32 B2	A	From Leg	4.00 0.00 2.00	0.0000	120.00
RRUS 32 B2	B	From Leg	4.00 0.00 2.00	0.0000	120.00
RRUS 32 B2	C	From Leg	4.00 0.00 2.00	0.0000	120.00
6' x 2" Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	120.00
6' x 2" Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	120.00
6' x 2" Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	120.00
Platform Mount [LP 303-1] ***	C	None		0.0000	120.00
LNx-6514DS-A1M w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	109.00
LNx-6514DS-A1M w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	109.00
LNx-8513DS-VTM w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	109.00
LNx-8513DS-VTM w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	109.00

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft
LNx-8513DS-VTM w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	109.00
(2) HBXX-6517DS-A2M w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	109.00
(2) HBXX-6517DS-A2M w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	109.00
(2) HBXX-6517DS-A2M w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	109.00
LNx-4514DS-A1M w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH 2x40-700 W/SOLAR	A	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH 2x40-700 W/SOLAR	B	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH 2x40-700 W/SOLAR	C	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH2X60-AWS	A	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH2X60-AWS	B	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH2X60-AWS	C	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH2X60-1900	A	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH2X60-1900	B	From Leg	4.00 0.00 0.00	0.0000	109.00
RRH2X60-1900	C	From Leg	4.00 0.00 0.00	0.0000	109.00
DB-T1-6Z-8AB-0Z	B	From Leg	4.00 0.00 0.00	0.0000	109.00
DB-T1-6Z-8AB-0Z	C	From Leg	4.00 0.00 0.00	0.0000	109.00
Platform Mount [LP 303-1] ***	C	None		0.0000	109.00
Commscope MC-PK8-DSH	C	None		0.0000	99.00
(2) 8' x 2" Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	99.00
(2) 8' x 2" Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	99.00
(2) 8' x 2" Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	99.00
MX08FRO665-21 w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	99.00
MX08FRO665-21 w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	99.00

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft
MX08FRO665-21 w/ Mount Pipe	C	From Leg	0.00 4.00 0.00 0.00	0.0000	99.00
TA08025-B604	A	From Leg	4.00 0.00 0.00	0.0000	99.00
TA08025-B604	B	From Leg	4.00 0.00 0.00	0.0000	99.00
TA08025-B604	C	From Leg	4.00 0.00 0.00	0.0000	99.00
TA08025-B605	A	From Leg	4.00 0.00 0.00	0.0000	99.00
TA08025-B605	B	From Leg	4.00 0.00 0.00	0.0000	99.00
TA08025-B605	C	From Leg	4.00 0.00 0.00	0.0000	99.00
RDIDC-9181-PF-48	B	From Leg	4.00 0.00 0.00	0.0000	99.00

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp

Comb. No.	Description
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	120 - 70.75	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-33.09	2.43	-0.80
			Max. Mx	20	-12.94	447.10	0.63
			Max. My	14	-12.95	-0.36	-441.27
			Max. Vy	20	-13.29	447.10	0.63
			Max. Vx	14	13.13	-0.36	-441.27
			Max. Torque	22			0.63
			Max Tension	1	0.00	0.00	0.00
L2	70.75 - 34.75	Pole	Max. Compression	26	-44.09	4.27	-0.38
			Max. Mx	20	-19.73	950.54	1.84
			Max. My	14	-19.73	-1.06	-938.67
			Max. Vy	20	-15.68	950.54	1.84
			Max. Vx	14	15.52	-1.06	-938.67
			Max. Torque	10			-0.62
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-60.92	6.71	0.22
L3	34.75 - 0	Pole	Max. Mx	20	-31.21	1634.95	3.24
			Max. My	2	-31.21	4.72	1616.09
			Max. Vy	20	-18.55	1634.95	3.24
			Max. Vx	2	-18.40	4.72	1616.09
			Max. Torque	10			-0.62

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	36	60.92	5.48	0.01
	Max. H _x	20	31.21	18.54	0.03
	Max. H _z	2	31.21	0.03	18.38
	Max. M _x	2	1616.09	0.03	18.38
	Max. M _z	8	1631.96	-18.54	-0.03
	Max. Torsion	22	0.62	16.07	9.22
	Min. Vert	5	23.41	-9.24	15.91

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Min. H _x	8	31.21	-18.54	-0.03
	Min. H _z	14	31.21	-0.03	-18.38
	Min. M _x	14	-1616.07	-0.03	-18.38
	Min. M _z	20	-1634.95	18.54	0.03
	Min. Torsion	10	-0.62	-16.07	-9.22

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead Only	26.01	0.00	0.00	-0.02	1.21	0.00
1.2 Dead+1.0 Wind 0 deg - No Ice	31.21	-0.03	-18.38	-1616.09	4.72	-0.26
0.9 Dead+1.0 Wind 0 deg - No Ice	23.41	-0.03	-18.38	-1604.76	4.31	-0.26
1.2 Dead+1.0 Wind 30 deg - No Ice	31.21	9.24	-15.91	-1397.96	-812.45	0.05
0.9 Dead+1.0 Wind 30 deg - No Ice	23.41	9.24	-15.91	-1388.16	-807.12	0.06
1.2 Dead+1.0 Wind 60 deg - No Ice	31.21	16.04	-9.16	-805.25	-1411.51	0.36
0.9 Dead+1.0 Wind 60 deg - No Ice	23.41	16.04	-9.16	-799.61	-1401.98	0.36
1.2 Dead+1.0 Wind 90 deg - No Ice	31.21	18.54	0.03	3.21	-1631.96	0.56
0.9 Dead+1.0 Wind 90 deg - No Ice	23.41	18.54	0.03	3.19	-1620.89	0.57
1.2 Dead+1.0 Wind 120 deg - No Ice	31.21	16.07	9.22	810.81	-1414.73	0.62
0.9 Dead+1.0 Wind 120 deg - No Ice	23.41	16.07	9.22	805.13	-1405.18	0.62
1.2 Dead+1.0 Wind 150 deg - No Ice	31.21	9.30	15.94	1401.16	-818.03	0.52
0.9 Dead+1.0 Wind 150 deg - No Ice	23.41	9.30	15.94	1391.34	-812.67	0.51
1.2 Dead+1.0 Wind 180 deg - No Ice	31.21	0.03	18.38	1616.07	-1.73	0.27
0.9 Dead+1.0 Wind 180 deg - No Ice	23.41	0.03	18.38	1604.74	-2.09	0.26
1.2 Dead+1.0 Wind 210 deg - No Ice	31.21	-9.24	15.91	1397.94	815.44	-0.05
0.9 Dead+1.0 Wind 210 deg - No Ice	23.41	-9.24	15.91	1388.14	809.35	-0.06
1.2 Dead+1.0 Wind 240 deg - No Ice	31.21	-16.04	9.16	805.23	1414.50	-0.36
0.9 Dead+1.0 Wind 240 deg - No Ice	23.41	-16.04	9.16	799.59	1404.21	-0.36
1.2 Dead+1.0 Wind 270 deg - No Ice	31.21	-18.54	-0.03	-3.24	1634.95	-0.57
0.9 Dead+1.0 Wind 270 deg - No Ice	23.41	-18.54	-0.03	-3.21	1623.11	-0.57
1.2 Dead+1.0 Wind 300 deg - No Ice	31.21	-16.07	-9.22	-810.83	1417.72	-0.62
0.9 Dead+1.0 Wind 300 deg - No Ice	23.41	-16.07	-9.22	-805.15	1407.41	-0.62
1.2 Dead+1.0 Wind 330 deg - No Ice	31.21	-9.30	-15.94	-1401.18	821.02	-0.51
0.9 Dead+1.0 Wind 330 deg - No Ice	23.41	-9.30	-15.94	-1391.36	814.89	-0.51
1.2 Dead+1.0 Ice+1.0 Temp	60.92	-0.00	0.00	-0.22	6.71	0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	60.92	-0.01	-5.45	-497.46	7.44	-0.06
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	60.92	2.73	-4.72	-430.53	-242.85	-0.00

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	60.92	4.74	-2.72	-248.30	-426.24	0.05
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	60.92	5.48	0.01	0.40	-493.59	0.10
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	60.92	4.75	2.73	248.94	-426.86	0.11
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	60.92	2.74	4.72	430.72	-243.92	0.10
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	60.92	0.01	5.45	497.02	6.20	0.06
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	60.92	-2.73	4.72	430.10	256.49	0.00
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	60.92	-4.74	2.72	247.86	439.88	-0.05
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	60.92	-5.48	-0.01	-0.84	507.23	-0.09
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	60.92	-4.75	-2.73	-249.38	440.50	-0.11
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	60.92	-2.74	-4.72	-431.15	257.56	-0.10
Dead+Wind 0 deg - Service	26.01	-0.01	-3.99	-349.45	1.94	-0.06
Dead+Wind 30 deg - Service	26.01	2.01	-3.45	-302.28	-174.75	0.01
Dead+Wind 60 deg - Service	26.01	3.48	-1.99	-174.13	-304.29	0.07
Dead+Wind 90 deg - Service	26.01	4.02	0.01	0.69	-351.96	0.11
Dead+Wind 120 deg - Service	26.01	3.49	2.00	175.31	-304.99	0.13
Dead+Wind 150 deg - Service	26.01	2.02	3.46	302.96	-175.96	0.11
Dead+Wind 180 deg - Service	26.01	0.01	3.99	349.43	0.54	0.06
Dead+Wind 210 deg - Service	26.01	-2.01	3.45	302.27	177.24	-0.01
Dead+Wind 240 deg - Service	26.01	-3.48	1.99	174.11	306.77	-0.07
Dead+Wind 270 deg - Service	26.01	-4.02	-0.01	-0.71	354.44	-0.11
Dead+Wind 300 deg - Service	26.01	-3.49	-2.00	-175.33	307.47	-0.13
Dead+Wind 330 deg - Service	26.01	-2.02	-3.46	-302.98	178.44	-0.11

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-26.01	0.00	0.00	26.01	0.00	0.000%
2	-0.03	-31.21	-18.38	0.03	31.21	18.38	0.000%
3	-0.03	-23.41	-18.38	0.03	23.41	18.38	0.000%
4	9.24	-31.21	-15.91	-9.24	31.21	15.91	0.000%
5	9.24	-23.41	-15.91	-9.24	23.41	15.91	0.000%
6	16.04	-31.21	-9.16	-16.04	31.21	9.16	0.000%
7	16.04	-23.41	-9.16	-16.04	23.41	9.16	0.000%
8	18.54	-31.21	0.03	-18.54	31.21	-0.03	0.000%
9	18.54	-23.41	0.03	-18.54	23.41	-0.03	0.000%
10	16.07	-31.21	9.22	-16.07	31.21	-9.22	0.000%
11	16.07	-23.41	9.22	-16.07	23.41	-9.22	0.000%
12	9.30	-31.21	15.94	-9.30	31.21	-15.94	0.000%
13	9.30	-23.41	15.94	-9.30	23.41	-15.94	0.000%
14	0.03	-31.21	18.38	-0.03	31.21	-18.38	0.000%
15	0.03	-23.41	18.38	-0.03	23.41	-18.38	0.000%
16	-9.24	-31.21	15.91	9.24	31.21	-15.91	0.000%
17	-9.24	-23.41	15.91	9.24	23.41	-15.91	0.000%
18	-16.04	-31.21	9.16	16.04	31.21	-9.16	0.000%
19	-16.04	-23.41	9.16	16.04	23.41	-9.16	0.000%
20	-18.54	-31.21	-0.03	18.54	31.21	0.03	0.000%

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
21	-18.54	-23.41	-0.03	18.54	23.41	0.03	0.000%
22	-16.07	-31.21	-9.22	16.07	31.21	9.22	0.000%
23	-16.07	-23.41	-9.22	16.07	23.41	9.22	0.000%
24	-9.30	-31.21	-15.94	9.30	31.21	15.94	0.000%
25	-9.30	-23.41	-15.94	9.30	23.41	15.94	0.000%
26	0.00	-60.92	0.00	0.00	60.92	-0.00	0.000%
27	-0.01	-60.92	-5.45	0.01	60.92	5.45	0.000%
28	2.73	-60.92	-4.72	-2.73	60.92	4.72	0.000%
29	4.74	-60.92	-2.72	-4.74	60.92	2.72	0.000%
30	5.48	-60.92	0.01	-5.48	60.92	-0.01	0.000%
31	4.75	-60.92	2.73	-4.75	60.92	-2.73	0.000%
32	2.74	-60.92	4.72	-2.74	60.92	-4.72	0.000%
33	0.01	-60.92	5.45	-0.01	60.92	-5.45	0.000%
34	-2.73	-60.92	4.72	2.73	60.92	-4.72	0.000%
35	-4.74	-60.92	2.72	4.74	60.92	-2.72	0.000%
36	-5.48	-60.92	-0.01	5.48	60.92	0.01	0.000%
37	-4.75	-60.92	-2.73	4.75	60.92	2.73	0.000%
38	-2.74	-60.92	-4.72	2.74	60.92	4.72	0.000%
39	-0.01	-26.01	-3.99	0.01	26.01	3.99	0.000%
40	2.01	-26.01	-3.45	-2.01	26.01	3.45	0.000%
41	3.48	-26.01	-1.99	-3.48	26.01	1.99	0.000%
42	4.02	-26.01	0.01	-4.02	26.01	-0.01	0.000%
43	3.49	-26.01	2.00	-3.49	26.01	-2.00	0.000%
44	2.02	-26.01	3.46	-2.02	26.01	-3.46	0.000%
45	0.01	-26.01	3.99	-0.01	26.01	-3.99	0.000%
46	-2.01	-26.01	3.45	2.01	26.01	-3.45	0.000%
47	-3.48	-26.01	1.99	3.48	26.01	-1.99	0.000%
48	-4.02	-26.01	-0.01	4.02	26.01	0.01	0.000%
49	-3.49	-26.01	-2.00	3.49	26.01	2.00	0.000%
50	-2.02	-26.01	-3.46	2.02	26.01	3.46	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	4	0.00000001	0.00008504
3	Yes	4	0.00000001	0.00004899
4	Yes	5	0.00000001	0.00005975
5	Yes	5	0.00000001	0.00002801
6	Yes	5	0.00000001	0.00005872
7	Yes	5	0.00000001	0.00002746
8	Yes	4	0.00000001	0.00012151
9	Yes	4	0.00000001	0.00007583
10	Yes	5	0.00000001	0.00006288
11	Yes	5	0.00000001	0.00002948
12	Yes	5	0.00000001	0.00005856
13	Yes	5	0.00000001	0.00002737
14	Yes	4	0.00000001	0.00007536
15	Yes	4	0.00000001	0.00004186
16	Yes	5	0.00000001	0.00005990
17	Yes	5	0.00000001	0.00002802
18	Yes	5	0.00000001	0.00006142
19	Yes	5	0.00000001	0.00002876
20	Yes	4	0.00000001	0.00010923
21	Yes	4	0.00000001	0.00006732
22	Yes	5	0.00000001	0.00005859
23	Yes	5	0.00000001	0.00002732
24	Yes	5	0.00000001	0.00006242
25	Yes	5	0.00000001	0.00002925
26	Yes	4	0.00000001	0.00002316
27	Yes	5	0.00000001	0.00014901
28	Yes	5	0.00000001	0.00015800
29	Yes	5	0.00000001	0.00015797
30	Yes	5	0.00000001	0.00014794

31	Yes	5	0.00000001	0.00015930
32	Yes	5	0.00000001	0.00015939
33	Yes	5	0.00000001	0.00015013
34	Yes	5	0.00000001	0.00016296
35	Yes	5	0.00000001	0.00016396
36	Yes	5	0.00000001	0.00015323
37	Yes	5	0.00000001	0.00016346
38	Yes	5	0.00000001	0.00016242
39	Yes	4	0.00000001	0.00001043
40	Yes	4	0.00000001	0.00002407
41	Yes	4	0.00000001	0.00002331
42	Yes	4	0.00000001	0.00001118
43	Yes	4	0.00000001	0.00002691
44	Yes	4	0.00000001	0.00002311
45	Yes	4	0.00000001	0.00001041
46	Yes	4	0.00000001	0.00002448
47	Yes	4	0.00000001	0.00002571
48	Yes	4	0.00000001	0.00001120
49	Yes	4	0.00000001	0.00002326
50	Yes	4	0.00000001	0.00002663

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	120 - 70.75	9.843	48	0.8332	0.0023
L2	74.75 - 34.75	3.313	48	0.4555	0.0004
L3	40 - 0	0.884	48	0.2062	0.0001

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
120.00	7770.00 w/ Mount Pipe	48	9.843	0.8332	0.0024	43485
109.00	LNx-6514DS-A1M w/ Mount Pipe	48	8.060	0.7393	0.0018	19766
99.00	Commscope MC-PK8-DSH	48	6.502	0.6545	0.0012	10353

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	120 - 70.75	45.427	20	3.8421	0.0112
L2	74.75 - 34.75	15.297	20	2.1040	0.0020
L3	40 - 0	4.078	20	0.9517	0.0006

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
120.00	7770.00 w/ Mount Pipe	20	45.427	3.8421	0.0113	9483
109.00	LNx-6514DS-A1M w/ Mount	20	37.203	3.4101	0.0084	4310

Elevation	Appurtenance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
ft			in	°	°	ft
99.00	Pipe Commscope MC-PK8-DSH	20	30.012	3.0204	0.0061	2256

Compression Checks

Pole Design Data

Section No.	Elevation	Size	L	L _u	Kl/r	A	P _u	φP _n	Ratio P _u φP _n
	ft		ft	ft		in ²	K	K	
L1	120 - 70.75 (1)	TP32.28x18x0.1875	49.25	0.00	0.0	18.408 8	-12.94	1076.92	0.012
L2	70.75 - 34.75 (2)	TP42.35x30.7452x0.3125	40.00	0.00	0.0	40.185 2	-19.73	2350.83	0.008
L3	34.75 - 0 (3)	TP51.8x40.2019x0.375	40.00	0.00	0.0	61.208 6	-31.21	3580.70	0.009

Pole Bending Design Data

Section No.	Elevation	Size	M _{ux}	φM _{nx}	Ratio M _{ux} φM _{nx}	M _{uy}	φM _{ny}	Ratio M _{uy} φM _{ny}
	ft		kip-ft	kip-ft		kip-ft	kip-ft	
L1	120 - 70.75 (1)	TP32.28x18x0.1875	447.10	718.72	0.622	0.00	718.72	0.000
L2	70.75 - 34.75 (2)	TP42.35x30.7452x0.3125	950.54	2271.92	0.418	0.00	2271.92	0.000
L3	34.75 - 0 (3)	TP51.8x40.2019x0.375	1634.95	4303.93	0.380	0.00	4303.93	0.000

Pole Shear Design Data

Section No.	Elevation	Size	Actual V _u	φV _n	Ratio V _u φV _n	Actual T _u	φT _n	Ratio T _u φT _n
	ft		K	K		kip-ft	kip-ft	
L1	120 - 70.75 (1)	TP32.28x18x0.1875	13.29	323.07	0.041	0.57	875.18	0.001
L2	70.75 - 34.75 (2)	TP42.35x30.7452x0.3125	15.68	705.25	0.022	0.57	2502.26	0.000
L3	34.75 - 0 (3)	TP51.8x40.2019x0.375	18.55	1074.21	0.017	0.57	4837.76	0.000

Pole Interaction Design Data

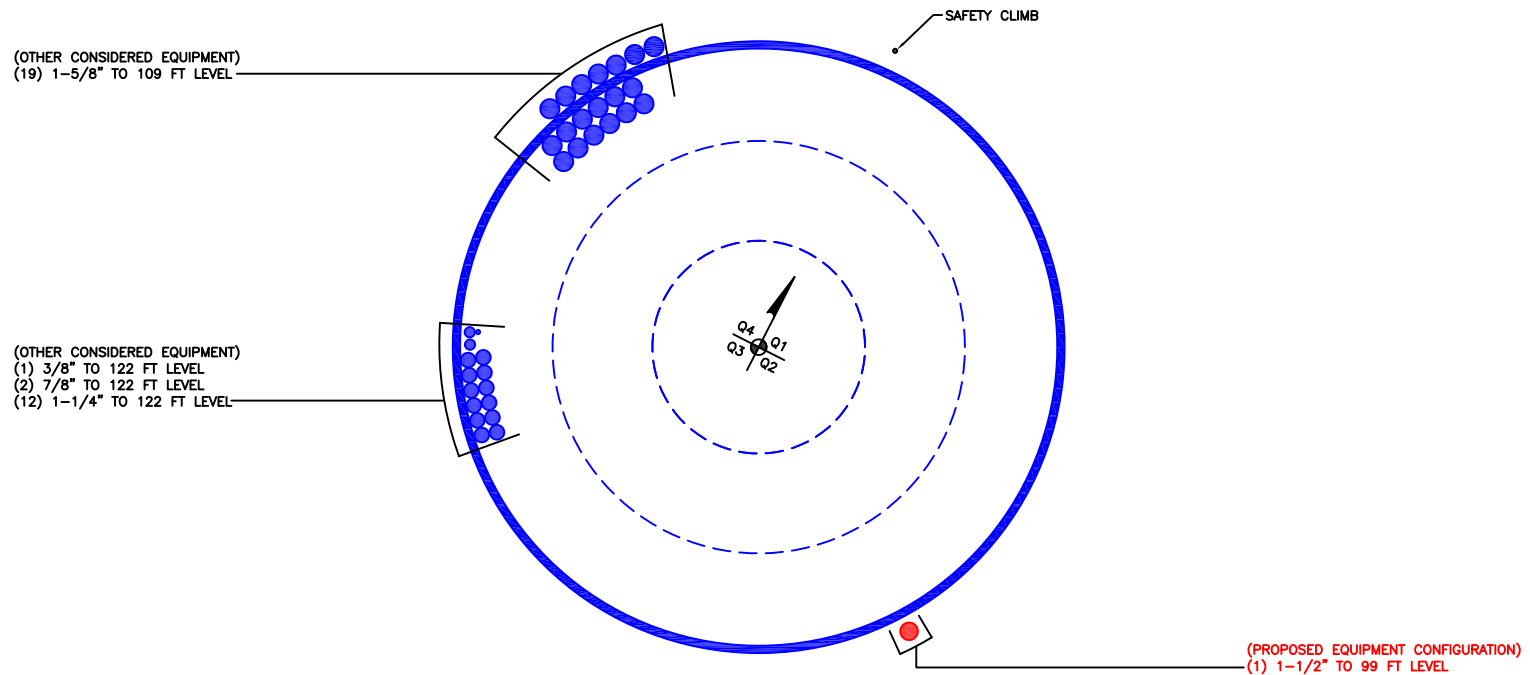
Section No.	Elevation	Ratio P _u φP _n	Ratio M _{ux} φM _{nx}	Ratio M _{uy} φM _{ny}	Ratio V _u φV _n	Ratio T _u φT _n	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
	ft								
L1	120 - 70.75 (1)	0.012	0.622	0.000	0.041	0.001	0.636	1.050	4.8.2
L2	70.75 - 34.75 (2)	0.008	0.418	0.000	0.022	0.000	0.427	1.050	4.8.2

Section No.	Elevation ft	Ratio P_u	Ratio M_{ux}	Ratio M_{uy}	Ratio V_u	Ratio T_u	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L3	34.75 - 0 (3)	0.009	0.380	0.000	0.017	0.000	0.389	1.050	4.8.2

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail
L1	120 - 70.75	Pole	TP32.28x18x0.1875	1	-12.94	1130.77	60.6	Pass
L2	70.75 - 34.75	Pole	TP42.35x30.7452x0.3125	2	-19.73	2468.37	40.7	Pass
L3	34.75 - 0	Pole	TP51.8x40.2019x0.375	3	-31.21	3759.73	37.0	Pass
							Summary	
							Pole (L1)	60.6
							RATING =	60.6
								Pass

APPENDIX B
BASE LEVEL DRAWING



APPENDIX C

ADDITIONAL CALCULATIONS

Monopole Base Plate Connection

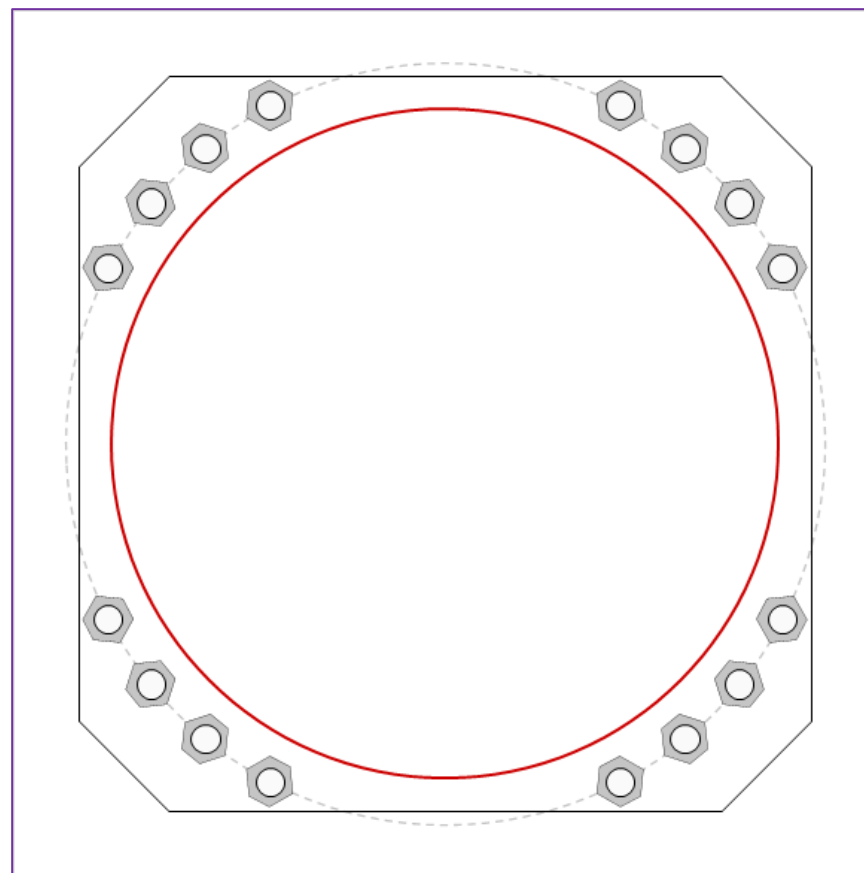


Site Info	
BU #	842867
Site Name	ANSFIELD FOUR CORNER
Order #	556626 Rev 1

Analysis Considerations	
TIA-222 Revision	H
Grout Considered:	No
l_{ar} (in)	1.25

Applied Loads	
Moment (kip-ft)	1634.95
Axial Force (kips)	31.21
Shear Force (kips)	18.55

*TIA-222-H Section 15.5 Applied



Connection Properties		Analysis Results	
Anchor Rod Data		Anchor Rod Summary <i>(units of kips, kip-in)</i>	
(16) 2-1/4" ϕ bolts (A615-75 N; $F_y=75$ ksi, $F_u=100$ ksi) on 59" BC <i>Anchor Spacing: 6 in</i>		$Pu_t = 81.13$	$\phi Pn_t = 243.75$ Stress Rating
		$Vu = 1.16$	$\phi Vn = 149.1$ 31.7%
		$Mu = n/a$	$\phi Mn = n/a$ Pass
Base Plate Data		Base Plate Summary	
57" W x 3" Plate (A572-55; $F_y=55$ ksi, $F_u=70$ ksi); Clip: 7 in		Max Stress (ksi):	14.59 (Flexural)
Stiffener Data		Allowable Stress (ksi):	49.5
N/A		Stress Rating:	28.1% Pass
Pole Data			
51.8" x 0.375" 18-sided pole (A607-65; $F_y=65$ ksi, $F_u=80$ ksi)			

Drilled Pier Foundation

BU # : 842867
 Site Name: MANSFIELD FOUR CORNER
 Order Number: 556626 Rev 1
 TIA-222 Revision: H
 Tower Type: Monopole



Applied Loads		
	Comp.	Uplift
Moment (kip-ft)	1634.95	
Axial Force (kips)	31.21	
Shear Force (kips)	18.54	

Material Properties		
Concrete Strength, f'c:	3	ksi
Rebar Strength, Fy:	60	ksi
Tie Yield Strength, Fyt:	40	ksi

Pier Design Data		
Depth	19	ft
Ext. Above Grade	0.5	ft
Pier Section 1		
From 0.5' above grade to 19' below grade		
Pier Diameter	7	ft
Rebar Quantity	20	
Rebar Size	11	
Clear Cover to Ties	4	in
Tie Size	4	
Tie Spacing		in

[Rebar & Pier Options](#)

[Embedded Pole Inputs](#)

[Belled Pier Inputs](#)

Analysis Results		
Soil Lateral Check	Compression	Uplift
D _{v=0} (ft from TOC)	5.31	-
Soil Safety Factor	2.62	-
Max Moment (kip-ft)	1763.58	-
Rating*	48.3%	-
Soil Vertical Check	Compression	Uplift
Skin Friction (kips)	364.19	-
End Bearing (kips)	115.45	-
Weight of Concrete (kips)	117.77	-
Total Capacity (kips)	479.65	-
Axial (kips)	148.98	-
Rating*	29.6%	-
Reinforced Concrete Flexure	Compression	Uplift
Critical Depth (ft from TOC)	5.23	-
Critical Moment (kip-ft)	1763.51	-
Critical Moment Capacity	4952.80	-
Rating*	33.9%	-
Reinforced Concrete Shear	Compression	Uplift
Critical Depth (ft from TOC)	14.05	-
Critical Shear (kip)	271.69	-
Critical Shear Capacity	543.58	-
Rating*	47.6%	-

Structural Foundation Rating*	47.6%
Soil Interaction Rating*	48.3%

*Rating per TIA-222-H Section 15.5

Check Limitation	
Apply TIA-222-H Section 15.5:	<input checked="" type="checkbox"/>
N/A	<input type="checkbox"/>
Additional Longitudinal Rebar	
Input Effective Depths (else Actual):	<input type="checkbox"/>
Shear Design Options	
Check Shear along Depth of Pier:	<input checked="" type="checkbox"/>
Utilize Shear-Friction Methodology:	<input type="checkbox"/>
Override Critical Depth:	<input type="checkbox"/>

[Go to Soil Calculations](#)

Soil Profile														
Groundwater Depth		13	# of Layers		6									
Layer	Top (ft)	Bottom (ft)	Thickness (ft)	Y _{soil} (pcf)	Y _{concrete} (pcf)	Cohesion (ksf)	Angle of Friction (degrees)	Calculated Ultimate Skin Friction Comp (ksf)	Calculated Ultimate Skin Friction Uplift (ksf)	Ultimate Skin Friction Comp Override (ksf)	Ultimate Skin Friction Uplift Override (ksf)	Ult. Gross Bearing Capacity (ksf)	SPT Blow Count	Soil Type
1	0	3.5	3.5	130	150	0	0	0.000	0.000					Cohesionless
2	3.5	5	1.5	130	150	0	32	0.675	0.675				72	Cohesionless
3	5	7	2	130	150	0	32	0.912	0.912				59	Cohesionless
4	7	10	3	130	150	0	32	1.223	1.223				52	Cohesionless
5	10	13	3	130	150	0	32	1.558	1.558				54	Cohesionless
6	13	19	6	67.6	87.6	0	32	1.817	1.817			4	41	Cohesionless



BU: 842867
WO: 1966176
Order: 556626

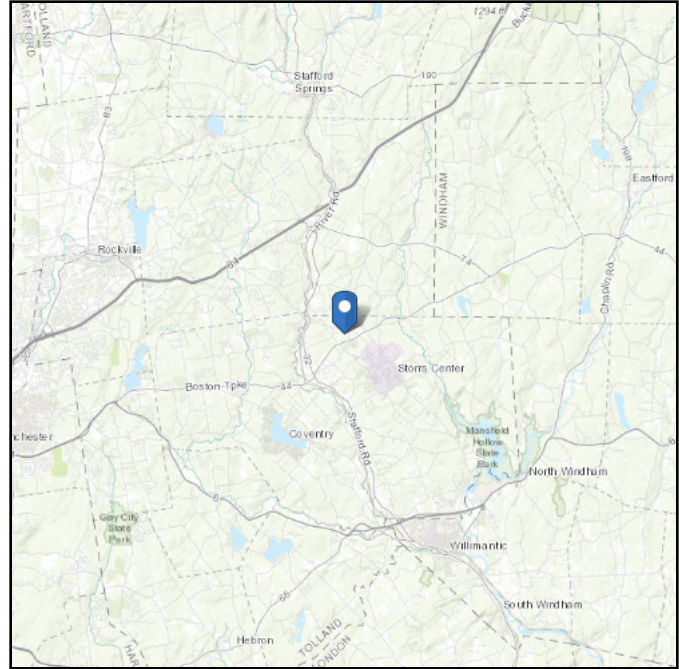
Structure: A
Rev: 0

Location				
	Decimal Degrees	Deg	Min	Sec
Lat:	41.825778	+	41	49
Long:	-72.281778	-	72	16
Code and Site Parameters				
Seismic Design Code:		TIA-222-H-1		
Site Soil:		D (Default) Stiff Soil (Default)		
Risk Category:		II		
<u>USGS Seismic Reference</u>				
	S _s :	0.1750	g	
	S ₁ :	0.0640	g	
	T _L :	6	s	
Seismic Design Category Determination				
Importance Factor, I _e :		1		
Acceleration-based site coefficient, F _a :		1.6000		
Velocity-based site coefficient, F _v :		2.4000		
Design spectral response acceleration short period, S _{DS} :		0.1867	g	
Design spectral response acceleration 1 s period, S _{D1} :		0.1024	g	
T _s :		0.5486		
Seismic Design Category Based on S _{DS} :		B		
Seismic Design Category Based on S _{D1} :		B		
Seismic Design Category Based on S ₁ :		N/A		
Controlling Seismic Design Category:		B		

ASCE 7 Hazards Report

Address:

No Address at This
Location

Standard: ASCE/SEI 7-10**Risk Category:** II**Soil Class:** D - Stiff Soil**Elevation:** 559.18 ft (NAVD 88)**Latitude:** 41.825781**Longitude:** -72.281794

Wind

Results:

Wind Speed:	126 Vmph	125 mph Per
10-year MRI	77 Vmph	Jurisdiction
25-year MRI	87 Vmph	
50-year MRI	95 Vmph	
100-year MRI	102 Vmph	

Data Source: ASCE/SEI 7-10, Fig. 26.5-1A and Figs. CC-1–CC-4, and Section 26.5.2, incorporating errata of March 12, 2014

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.

Results:

Ice Thickness: 1.00 in.

Concurrent Temperature: 5 F

Gust Speed: 50 mph

Data Source: Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Fri May 21 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

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