

Northeast Site Solutions Victoria Masse 420 Main St Unit 1 Box 2 Sturbridge, MA 01566 victoria@northeastsitesolutions.com

March 30, 2022

Members of the Siting Council Connecticut Siting Council Ten Franklin Square New Britain, CT 06051

RE: Tower Share Application 33 Mitchell Drive, Manchester, CT Latitude: 41.797333 N Longitude: 72.512056 W

Longitude: 72.512056 W Site#: BOBDL00110C

#### Dear Ms. Bachman:

This letter and attachments are submitted on behalf of Dish Wireless LLC. Dish Wireless LLC plans to install antennas and related equipment to the tower site located at 33 Mitchell Drive, Manchester, Connecticut.

Dish Wireless LLC proposes to install three (3) 600/1900/2100 5G MHz antenna and six (6) RRUs, at the 170-foot level of the existing 170-foot lattice tower, one (1) Fiber cable will also be installed. Dish Wireless LLC equipment cabinets will be placed within 7x5 lease area. Included are plans by Infinigy, dated March 29, 2022, Exhibit C. Also included is a structural analysis prepared by Infinigy, dated March 19, 2022 confirming that the existing tower is structurally capable of supporting the proposed equipment. Attached as Exhibit D. This facility was approved by the Town of Manchester. Please see attached Exhibit A.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies 16-50aa, of Dish Wireless LLC intent to share a telecommunications facility pursuant to R.C.S.A. 16-50j-88. In accordance with R.C.S.A., a copy of this letter is being sent to Jay Moran, Mayor for the Town of Manchester, Gary Anderson, Director of Planning and Economic Development, as well as the property owner and tower owner.

The planned modifications of the facility fall squarely within those activities explicitly provided for in R.C.S.A. 16-50j-89.

- 1. The proposed modifications will not result in an increase in the height of the existing structure. The top of the tower is 170-feet; Dish Wireless LLC proposed antennas will be located at a center line height of 170-feet.
- 2. The proposed modification will not result in the increase of the site boundary as depicted on the attached site plan.
- 3. The proposed modification will not increase the noise levels at the facility by six decibels or more, or to levels that exceed local and state criteria. The incremental effect of the proposed changes will be negligent.



4.The operation of the proposed antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard. As indicated in the attached power density calculations, the combined site operations will result in a total density of 21.16% as evidenced by Exhibit F.

Connecticut General Statutes 16-50-aa indicates that the Council must approve the shared use of a telecommunications facility provided it finds the shared use is technically, legally, environmentally, and economically feasible and meets public safety concerns. As demonstrated in this letter, Dish Wireless LLC respectfully indicates that the shared use of this facility satisfies these criteria.

A. Technical Feasibility. The existing lattice tower has been deemed structurally capable of supporting Dish Wireless LLC proposed loading. The structural analysis is included in Exhibit D.

B. Legal Feasibility. As referenced above, C.G.S. 16-50aa has been authorized to issue orders approving the shared use of an existing tower such as this lattice tower in Manchester. Under the authority granted to the Council, an order of the Council approving the requested shared use would permit Dish Wireless LLC to obtain a building permit for the proposed installation. Further, a letter of Authorization is included as Exhibit G, authorizing Dish Wireless LLC to file this application for shared use.

C. Environmental Feasibility. The proposed shared use of this facility would have a minimal environmental impact. The installation of Dish Wireless LLC equipment at the 170-foot level of the existing 170-foot tower would have an insignificant visual impact on the area around the lattice tower. Dish Wireless LLC ground equipment would be installed within the existing facility compound. Dish Wireless LLC shared use would therefore not cause any significant alteration in the physical or environmental characteristics of the existing site. Additionally, as evidenced by Exhibit F, the proposed antennas would not increase radio frequency emissions to a level at or above the Federal Communications Commission safety standard.

D. Economic Feasibility. Dish Wireless LLC will be entering into an agreement with the owner of this facility to mutually agreeable terms. As previously mentioned, the Letter of Authorization has been provided by the owner to assist Dish Wireless LLC with this tower share application.

E. Public Safety Concerns. As discussed above, the tower is structurally capable of supporting Dish Wireless LLC proposed loading. Dish Wireless LLC is not aware of any public safety concerns relative to the proposed sharing of the existing tower. Dish Wireless LLC intentions of providing new and improved wireless service through the shared use of this facility is expected to enhance the safety and welfare of local residents and individuals traveling through Manchester.

Sincerely,

Victoria Masse Mobile: 860-306-2326 Fax: 413-521-0558

Office: 420 Main Street, Unit 1 Box 2, Sturbridge, MA 01566

Email: victoria@northeastsitesolutions.com



Attachments

Cc: Jay Moran, Mayor Lincoln Center, 2nd Floor 494 Main Street, PO Box 191

Gary Anderson, Director of Planning and Economic Development Lincoln Center, 2nd Floor 494 Main Street, PO Box 191

Manchester, CT 06045

Manchester, CT 06045

Mitchell Drive LLC, Property Owner PO BOX 1498 Manchester, CT 06045

American Tower Corp, Tower Owners 10 Presidential Way Woburn, MA 01801

## Exhibit A

**Original Facility Approval** 



## Town of Manchester

41 Center Street • P.O. Box 191

Manchester, Connecticut 06045-0191

www.manchesterct.gov

LEO V. DIANA, MAYOR JAY MORAN, DEPUTY MAYOR LISA P. O'NEILL, SECRETARY

> DIRL'CTORS STEVE GATES SUSAN HOLMES RUDY C. KISSMANN CHERI A. PELLETIER JOHN D. TOPPING MARK D. TWEEDIE

CERTIFIED LETTER November 21, 2012

Todd Stacy Marcus Communications, LLC 33 Mitchell Drive Manchester, CT 06045

Re: Marcus Communication, LLC – 33 Mitchell Drive & 184 Sheldon Road Special Exception (2012-106)

Dear Mr. Stacy:

As agent for the applicant, please be advised that at its meeting of November 19, 2012, the Planning and Zoning Commission approved with modifications the above referenced application. The approval is for activities as shown on plans entitled, "SITE PLAN #33 MITCHELL DRIVE & #184 SHELDON ROAD, PREPARED FOR MARCUS COMMUNICATIONS, MANCHESTER, CONN", by Aeschliman Land Surveying, PC, Map No. 212007-1A, dated 08/27/2012, revised 11/09/2012.

The specific approval granted is as follows:

Special Exception (2012-106) – approved the special exception under Article II Section 16.15.02(o) to construct a 170' self-supporting telecommunications tower and related appurtenances with modifications as outlined in a memorandum from Derrick Gregor, Assistant Town Engineer to Renata Bertotti, Senior Planner, dated November 19, 2012.

All site work related to the above approvals must be completed by November 19, 2017 in accordance with the Connecticut General Statutes, Section 8-3. Failure to complete all work within the specified time period will result in automatic expiration of the approval.

Please submit one set of sealed and signed washoff or fixed line mylar plans and five (5) paper copies of the plans incorporating the modifications listed above, sealed and signed, to this office for stamping and signature.

To speed the endorsement of final plans staff requests the following block be added to the lower right of each page of the plans above or to the left of the title block: Please do not reduce this block to less than 2" X 3".

#### APPROVED

PLANNING AND ZONING COMMISSION MANCHESTER, CT

DATE:	
SIGNED:	-

You are also required to submit a fee of \$50.00 for the above referenced plan to cover digital GIS conversion costs incurred by the Town. The payment for GIS conversion should accompany the final mylar and paper copies. A copy of the GIS Conversion Fee Requirement is enclosed.

A Certificate of Approval of this decision will be forwarded to you immediately following the expiration of the Superior Court appeal period (i.e., 15 days after the legal decision notice is published in the newspaper). Upon receipt, you must file the Certificate of Approval with the Town Clerk and pay the required recording fee. The approval of your petition by the Planning and Zoning Commission will not be effective until the Certificate has been recorded on the land records in the Town Clerk's office. You are hereby advised not to engage in any activity concerning your petition until the Certificate has been recorded. We also remind you to obtain a building permit for this work before you start construction.

NOTE: No changes to the approved site plans, or to the building elevations, materials or colors, are to be made until the proposed changes are submitted to the Planning Department and it is determined whether the changes can be approved administratively or will need Planning and Zoning Commission approval.

Sincerely,

Renata Bertotti, AICP Senior Planner

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 $RB/lg \\ \text{R:PLANNINGPZC} \\ \text{20}12\text{-}11 - \text{NOVEMBER } 19\text{-}DECISION LETTERS} \\ \text{20}12-106 \text{ MARCUS COMMUNICATIONS.DOC } \\ \text{Encls.}$ 

cc: Engineering Department (w/out encl.)

John Rainaldi, Director of Assessment & Collection (w/out encl.)

James A. Davis, Zoning Enforcement Officer (w/out encl.)

Richard Gallacher, GIS Coordinator (w/out encl.)

Greg Smith, Chief Building Official (w/out encl.)

# Exhibit B

**Property Card** 

#### 33 MITCHELL DRIVE

Location 33 MITCHELL DRIVE Mblu 90/4000/33//

Acct# 400000033 Owner MITCHELL DRIVE LLC

**Assessment** \$951,600 **Appraisal** \$1,359,400

PID 11439 Building Count 1

DISTRICT T CONCRETE

#### **Current Value**

Appraisal				
Valuation Year	Improvements	Land	Total	
2016	\$1,051,100	\$308,300	\$1,359,400	
	Assessment			
Valuation Year	Improvements	Land	Total	
2016	\$735,800	\$215,800	\$951,600	

#### **Owner of Record**

Owner MITCHELL DRIVE LLC Sale Price \$710,000

Address PO BOX 1498 Certificate

MANCHESTER, CT 06045-1498 Book & Page 3918/ 222

**Sale Date** 12/30/2011

Instrument 33

#### **Ownership History**

Ownership History					
Owner	Sale Price	Certificate	Book & Page	Instrument	Sale Date
MITCHELL DRIVE LLC	\$710,000		3918/ 222	33	12/30/2011
GREAT OAK WAY INC	\$0		3815/ 185	31	11/12/2010
MITCHELL DRIVE ASSOC LLC	\$0	С	1865/ 337		12/13/1996

#### **Building Information**

#### **Building 1: Section 1**

Year Built: 1967

 Living Area:
 33,898

 Replacement Cost:
 \$1,539,539

**Replacement Cost** 

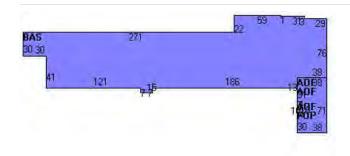
Less Depreciation: \$785,200

Building A	Attributes
Field	Description
STYLE	Light Indust
MODEL	Ind/Comm
Grade	Average
Stories:	1
Occupancy	4
Exterior Wall 1	Pre-finsh Metl
Exterior Wall 2	Concr/Cinder
Roof Structure	Flat
Roof Cover	Tar + Gravel
Interior Wall 1	Minim/Masonry
Interior Wall 2	
Interior Floor 1	Concr-Finished
Interior Floor 2	
Heating Fuel	Solar Assisted
Heating Type	Hot Air-no Duc
AC Type	Partial
Bldg Use	Industrial 96
Total Rooms	
Total Bedrms	00
Total Baths	0
1st Floor Use:	300
Heat/AC	Heat AC Split
Frame Type	Steel
Baths/Plumbing	Average
Ceiling/Wall	Ceil & Min WI
Rooms/Prtns	Average
Wall Height	20
% Comn Wall	0



(http://images.vgsi.com/photos2/ManchesterCTPhotos/\\00\\03\\31/69.jpg)

#### **Building Layout**



 $(http://images.vgsi.com/photos2/ManchesterCTPhotos//Sketches/11439\_11$ 

Building Sub-Areas (sq ft) <u>L</u> e			Legend
Code	Description	Gross Area	Living Area
BAS	First Floor	28,542	28,542
AOF	Office, (Average)	5,356	5,356
FOP	Porch, Open	40	0
		33,938	33,898

#### **Extra Features**

	E	xtra Features		<u>Legend</u>
Code	Description	Size	Value	Bldg #
SPR1	Sprinklers-Wet	2700 S.F.	\$2,100	1
A/C	Partial AC	5356 S.F.	\$5,500	1
SOL	Solar Panels	750 UNIT	\$150,000	1

#### Land

#### **Land Use Land Line Valuation Use Code** 300 Size (Acres) 4.49 Description Industrial 96 Frontage 0 Zone Depth 0 Neighborhood 4000 Assessed Value \$215,800 Alt Land Appr No Appraised Value \$308,300

Category

#### Outbuildings

		C	Outbuildings			<u>Legend</u>
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #
PAV1	Paving Asphalt			24000 S.F.	\$30,000	1
PAV2	Paving Concrete			48 S.F.	\$100	1
SHD1	Shed			200 S.F.	\$1,800	1
SHD1	Shed			240 S.F.	\$2,200	1
SHD1	Shed			240 S.F.	\$2,200	1
SPNL	Solar Pan Comm			72 EACH	\$72,000	1

#### **Valuation History**

Appraisal			
Valuation Year	Improvements	Land	Total
2015	\$686,400	\$308,300	\$994,700
2010	\$1,083,000	\$495,000	\$1,578,000
2005	\$781,800	\$538,800	\$1,320,600

Assessment			
Valuation Year	Improvements	Land	Total
2015	\$480,500	\$215,800	\$696,300
2010	\$758,100	\$346,500	\$1,104,600
2005	\$547,400	\$377,200	\$924,600



# TOWN OF MANCHESTER PROPERTY MAP

## TILE 90

## Legend

Property Line
Road Right of Way
Building
Road, Driveway or
Parking Lot

Bridge
Railroad Right
Access Easem

River

Parcel Hook

02517
0477

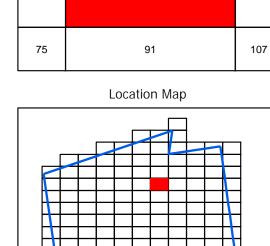
RPKEY

02517 0477 RPKEY 12.98 Ac Acreage

127 Street Address
155 Developer Lot Number
127.6(P) Dimension (Plan Source)
(127.6) Dimension (Scaled)

73 89 105 74 90 106

127.6(D) Dimension (Deed Source)



#### DISCLAIMER:

Please be advised that all information presented in this map is provided "as-is" without warranty of any kind, either expressed or implied. Real property is compiled from recorded deeds, subdivision plans, and other public records and data. Users of these map data are hereby notified that the aforementioned public primary information sources should be consulted for verification of the information contained in these map data. This map is intended for informational purposes only and does not meet the accuracy requirements of survey data.

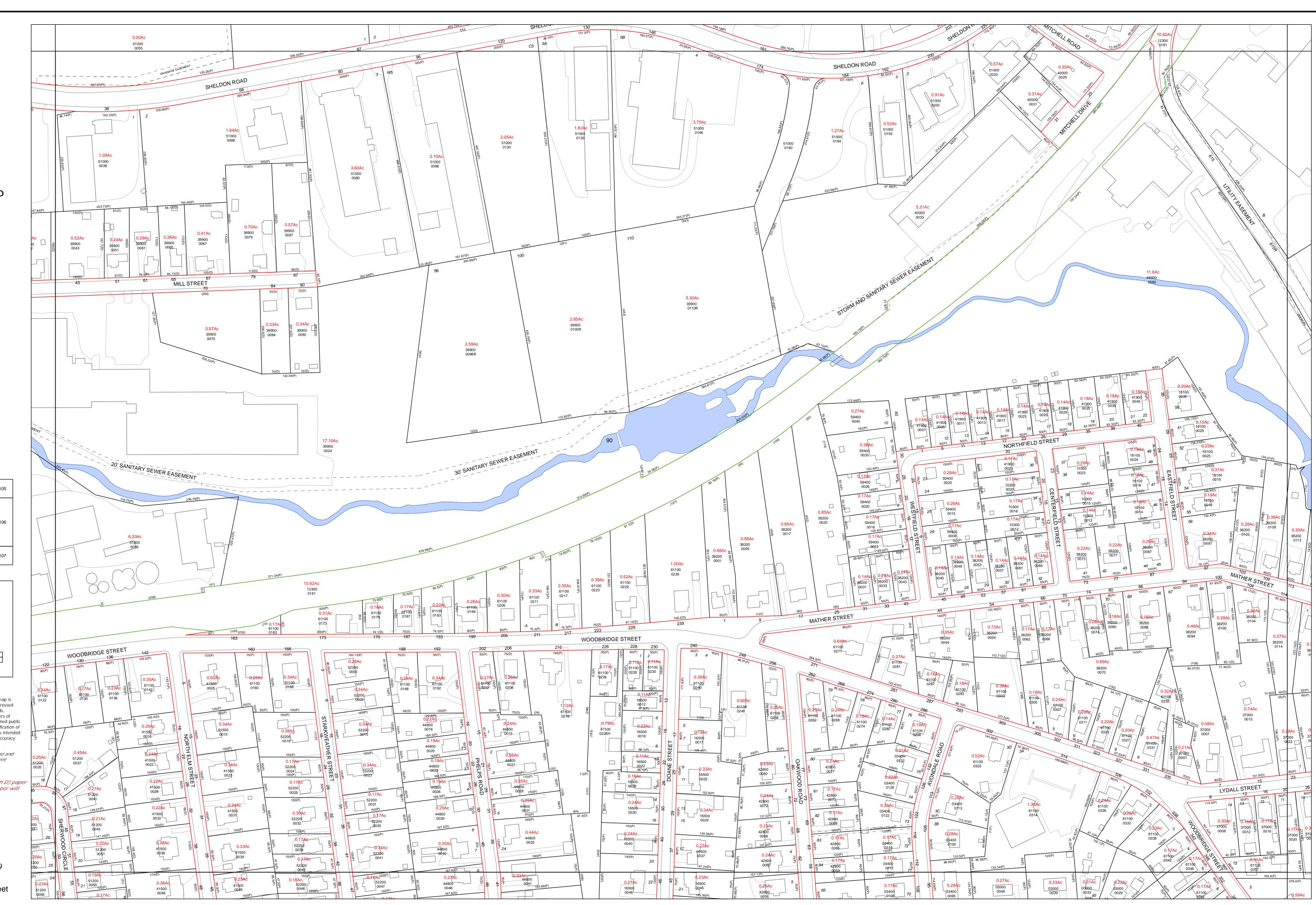
NOTE: The GIS parcel layer acreages are NOT used and should NOT be used for property assessment purposes.

This tax map is formatted for 24" x 36" (Arch D) paper size only. Printing these maps on smaller paper will render the map scale (1"=100') inaccurate.



DATE PRINTED: 14-Jun-19
0 50 100 200

1 Inch = 100 Feet



## Exhibit C

**Construction Drawings** 

# wireless

DISH Wireless L.L.C. SITE ID:

BOBDL00110C

DISH Wireless L.L.C. SITE ADDRESS:

# 33 MITCHELL DRIVE MANCHESTER, CT 06042

#### CONNECTICUT CODE OF COMPLIANCE

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES

2018 CT STATE BUILDING CODE/2015 IBC W/ CT AMENDMENTS 2018 CT STATE BUILDING CODE/2015 IMC W/ CT AMENDMENTS 2018 CT STATE BUILDING CODE/2017 NEC W/ CT AMENDMENTS

	SHEET INDEX
SHEET NO.	SHEET TITLE
T-1	TITLE SHEET
A-1	OVERALL AND ENLARGED SITE PLAN
A-2	ELEVATION, ANTENNA LAYOUT AND SCHEDULE
A-3	EQUIPMENT PLATFORM AND H-FRAME DETAILS
A-4	EQUIPMENT DETAILS
A-5	EQUIPMENT DETAILS
A-6	EQUIPMENT DETAILS
E-1	ELECTRICAL/FIBER ROUTE PLAN AND NOTES
E-2	ELECTRICAL DETAILS
E-3	ELECTRICAL ONE-LINE, FAULT CALCS & PANEL SCHEDULE
G-1	GROUNDING PLANS AND NOTES
G-2	GROUNDING DETAILS
G-3	GROUNDING DETAILS
RF-1	RF CABLE COLOR CODE
GN-1	LEGEND AND ABBREVIATIONS
GN-2	RF SIGNAGE
GN-3	GENERAL NOTES
GN-4	GENERAL NOTES
GN-5	GENERAL NOTES

### SCOPE OF WORK

THIS IS NOT AN ALL INCLUSIVE LIST. CONTRACTOR SHALL UTILIZE SPECIFIED EQUIPMENT PART OR ENGINEER APPROVED EQUIVALENT. CONTRACTOR SHALL VERIFY ALL NEEDED EQUIPMENT TO PROVIDE A FUNCTIONAL SITE. THE PROJECT GENERALLY CONSISTS OF THE FOLLOWING:

#### TOWER SCOPE OF WORK:

- INSTALL (3) PROPOSED PANEL ANTENNAS (1 PER SECTOR)
- INSTALL (3) PROPOSED SECTOR FRAMES INSTALL PROPOSED JUMPERS
- INSTALL (6) PROPOSED RRUS (2 PER SECTOR)
- INSTALL (1) PROPOSED OVER VOLTAGE PROTECTION DEVICE (OVP)
- INSTALL (1) PROPOSED HYBRID CABLE

#### **GROUND SCOPE OF WORK:**

- INSTALL (1) PROPOSED METAL PLATFORM
- INSTALL (1) PROPOSED ICE BRIDGE
- INSTALL (1) PROPOSED PPC CABINET
- INSTALL (1) PROPOSED EQUIPMENT CABINET
- INSTALL (1) PROPOSED POWER CONDUIT INSTALL (1) PROPOSED TELCO CONDUIT
- INSTALL (1) PROPOSED TELCO-FIBER BOX
- INSTALL (1) PROPOSED GPS UNIT
- INSTALL (1) PROPOSED SAFETY SWITCH (IF REQUIRED)
- INSTALL (1) PROPOSED FIBER NID (IF REQUIRED)
- UTILIZE EXISTING METER SOCKET

### SITE PHOTO





UNDERGROUND SERVICE ALERT CBYD 811 UTILITY NOTIFICATION CENTER OF CONNECTICUT (800) 922-4455 WWW.CBYD.COM

CALL 2 WORKING DAYS UTILITY NOTIFICATION PRIOR TO CONSTRUCTION

#### **GENERAL NOTES**

THE FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION. A TECHNICIAN WILL VISIT THE SITE AS REQUIRED FOR ROUTINE MAINTENANCE. THE PROJECT WILL NOT RESULT IN ANY SIGNIFICANT DISTURBANCE OR EFFECT ON DRAINAGE. NO SANITARY SEWER SERVICE, POTABLE WATER, OR TRASH DISPOSAL IS REQUIRED AND NO COMMERCIAL SIGNAGE IS PROPOSED.

#### 11"x17" PLOT WILL BE HALF SCALE UNLESS OTHERWISE NOTED

CONTRACTOR SHALL VERIFY ALL PLANS, EXISTING DIMENSIONS, AND CONDITIONS ON THE JOB SITE, AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK.

## **DIRECTIONS**

PROJECT DIRECTORY

TOWER OWNER: AMERICAN TOWER CORPORATION

**APPLICANT:** 

SITE DESIGNER:

SITE ACQUISITION:

**RF ENGINEER:** 

DISH Wireless L.L.C.

LITTLETON, CO 80120

10 PRESIDENTIAL WAY

WOBURN, MA 01801

(781) 926-4500

ALBANY, NY 12205

(518) 690-0790

CONSTRUCTION MANAGER: JAVIER SOTO

5701 SOUTH SANTA FE DRIVE

1033 WATERVLIET SHAKER RD

APRIL PARROTT

SYED ZAIDI

(203) 927-4317

JAVIER.SOTO DISH.COM

SYED.ZAIDIODISH.COM

#### DIRECTIONS FROM HARTFORD BRAINARD AIRPORT:

SITE INFORMATION

PROPERTY OWNER:

TOWER CO SITE ID:

TOWER APP NUMBER:

LATITUDE (NAD 83):

ZONING JURISDICTION:

**ZONING DISTRICT:** 

PARCEL NUMBER:

OCCUPANCY GROUP:

CONSTRUCTION TYPE:

TELEPHONE COMPANY:

POWER COMPANY:

ADDRESS:

TOWER TYPE:

COUNTY:

TOWN OF EAST HARTFORD

**SELF-SUPPORT TOWER** 

EAST HARTFORD

CT 06108

**HARTFORD** 

41.797333° N

72.512056° W

LONGITUDE (NAD 83): 72° 30′ 42.4016″ W

R2

13740

41° 47' 50.3988" N

CONNECTICUT SITING COUNCIL

CONNECTICUT LIGHT & POWER

HEAD NORTH ON LINDBERGH DR TOWARD MAXIM RD. TURN LEFT ONTO MAXIM RD. CONTINUE ONTO BRAINARD RD. TURN RIGHT ONTO THE RAMP TO I-91 N/I-84/SPRINGFIELD/BOSTON. MERGE ONTO US-5 N. CONTINUE ONTO CT-15N. TAKE THE EXIT ON THE LEFT ONTO I-84 E TOWARD BOSTON. TAKE EXIT 63 FOR CT-30/CT-83 TOWARD SOUTH WINDSOR. USE THE 2ND FROM THE LEFT LANE TO TURN LEFT ONTO CT-30 N/CT-83 N. TURN RIGHT ONTO PARKER ST. CONTINUE ONTO SHELDON RD. TURN LEFT ONTO MITCHELL DR. DESTINATION WILL BE ON THE RIGHT







5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120



the solutions are endless HOFFMAN ESTATES, IL 60169 PHONE: 847-648-4068 | FAX: 518-690-0793



IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

DRAWN BY:	CHECKED BY:	APPROVED BY:
HL	AL	SS
RFDS REV ;	#:0	1/6/2022

### CONSTRUCTION **DOCUMENTS**

	SUBMITTALS		
REV	DATE	DESCRIPTION	
A	03/02/2022	ISSUED FOR REVIEW	
В	03/07/2022	ISSUED FOR REVIEW	
0	03/29/2022	ISSUED FOR CONSTRUCTION	
A&E PROJECT NUMBER			
1197-F0001-C			

DISH Wireless L.L.C.

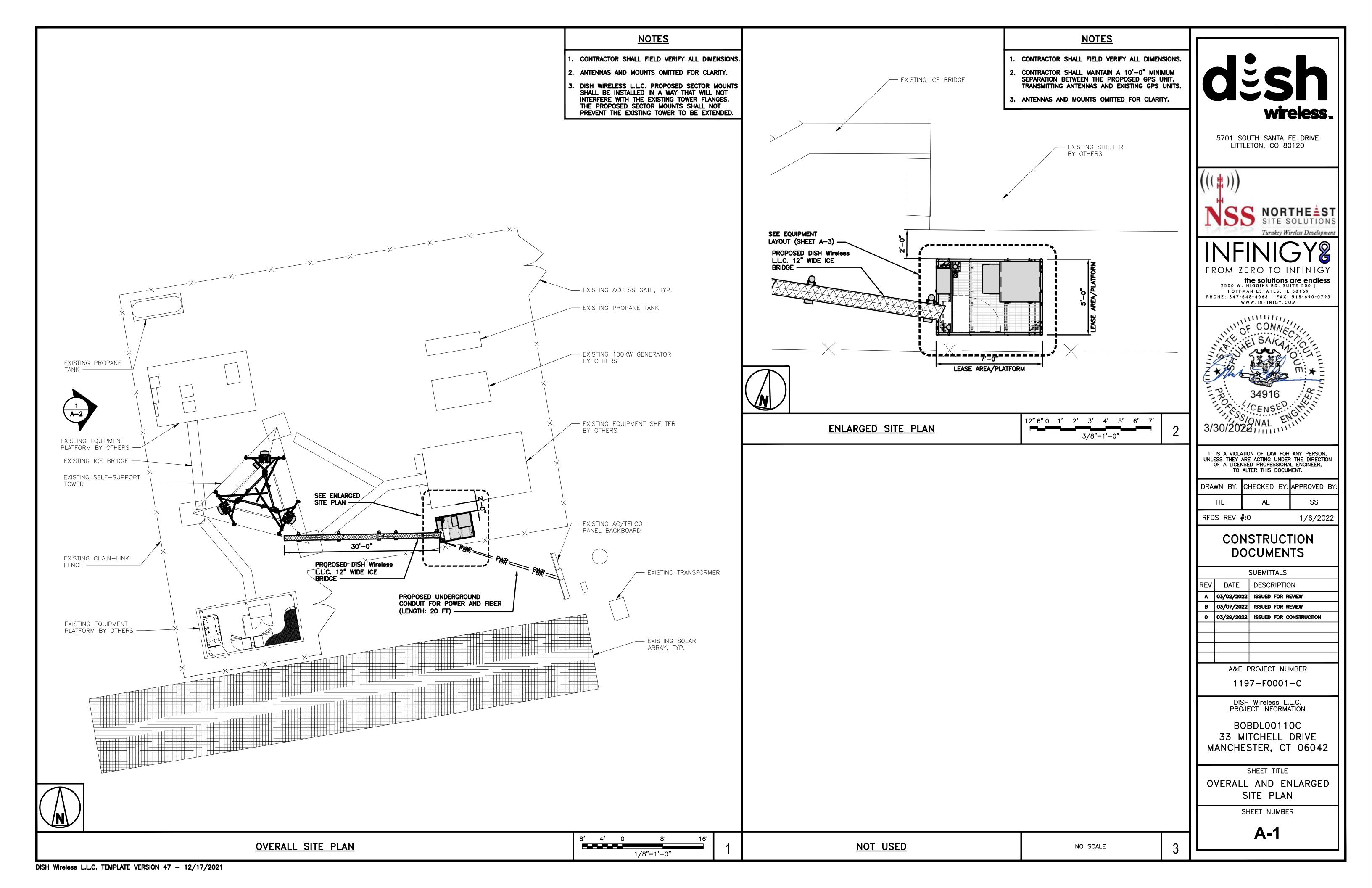
PROJECT INFORMATION

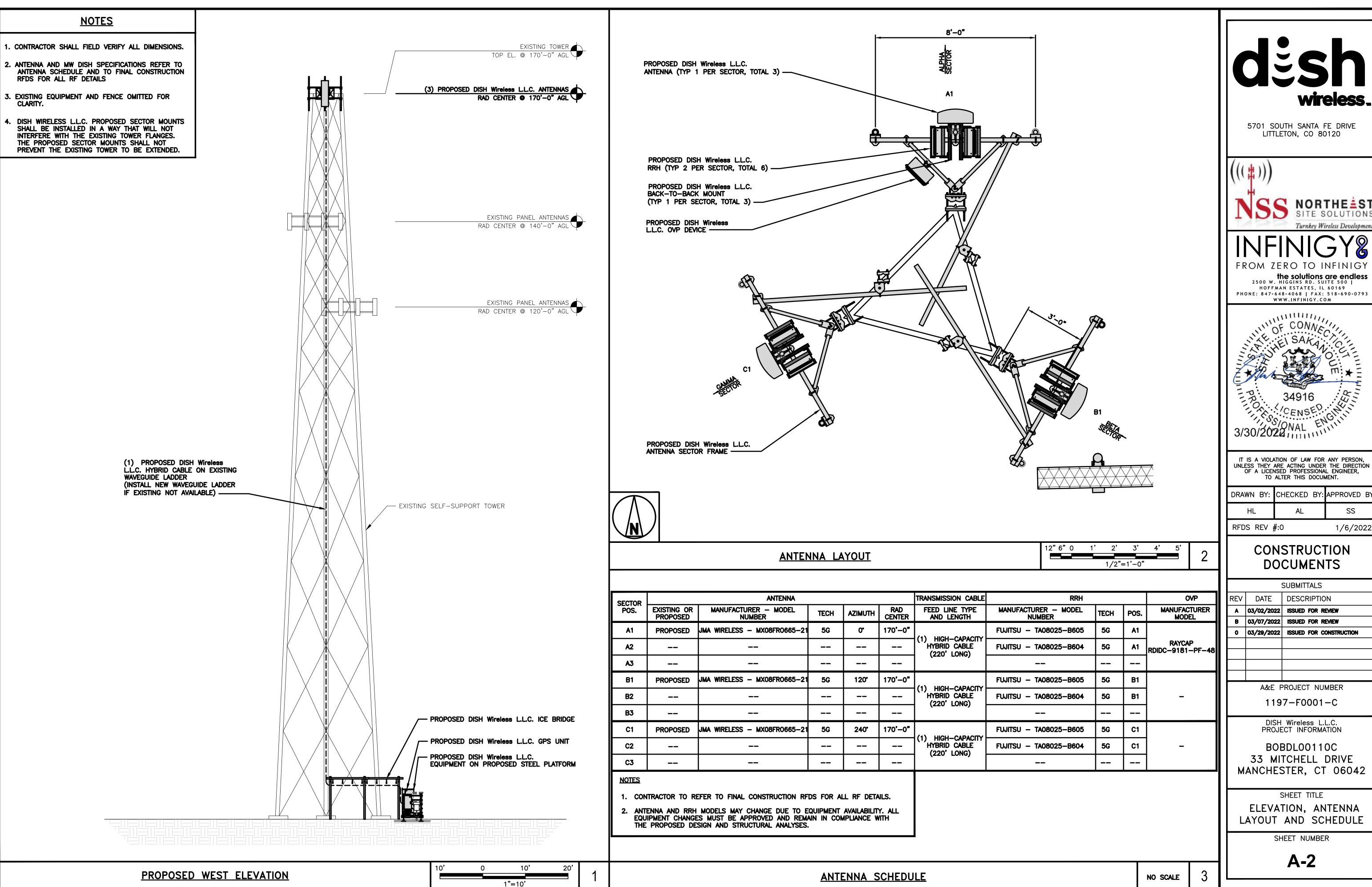
BOBDL00110C 33 MITCHELL DRIVE MANCHESTER, CT 06042

> SHEET TITLE TITLE SHEET

SHEET NUMBER

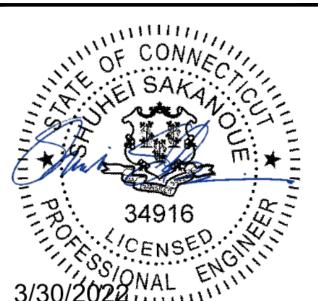
**T-1** 





5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120





IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

DRAWN BY:	CHECKED B	3Y:	APPROVED	BY:
HL	AL		SS	
RFDS REV	#:0		1/6/202	22

## CONSTRUCTION **DOCUMENTS**

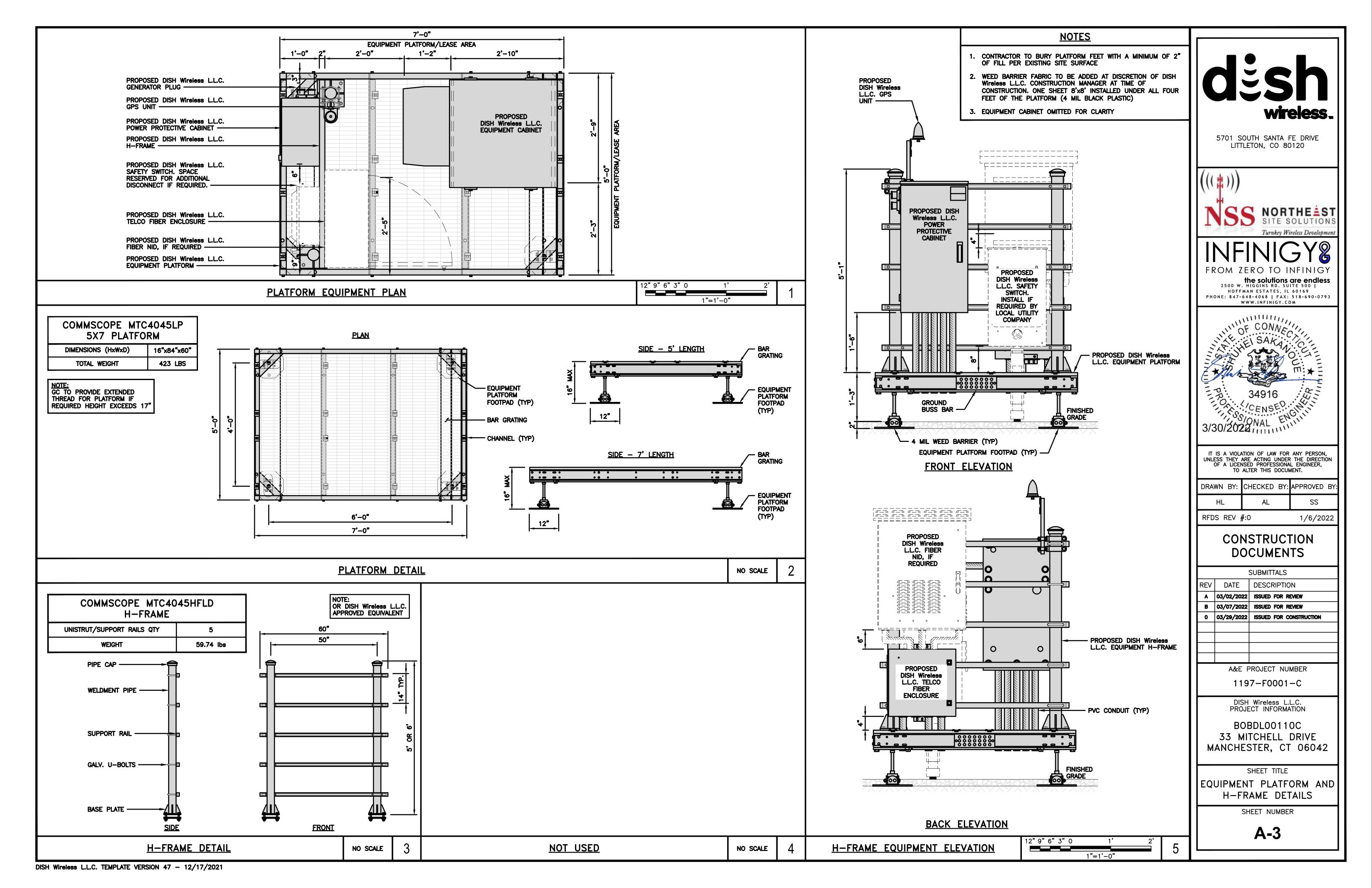
	SUBMITTALS						
REV	DESCRIPTION						
A	03/02/2022	ISSUED FOR REVIEW					
В	03/07/2022	ISSUED FOR REVIEW					
0	03/29/2022	ISSUED FOR CONSTRUCTION					
A&E PROJECT NUMBER							
1197-F0001-C							

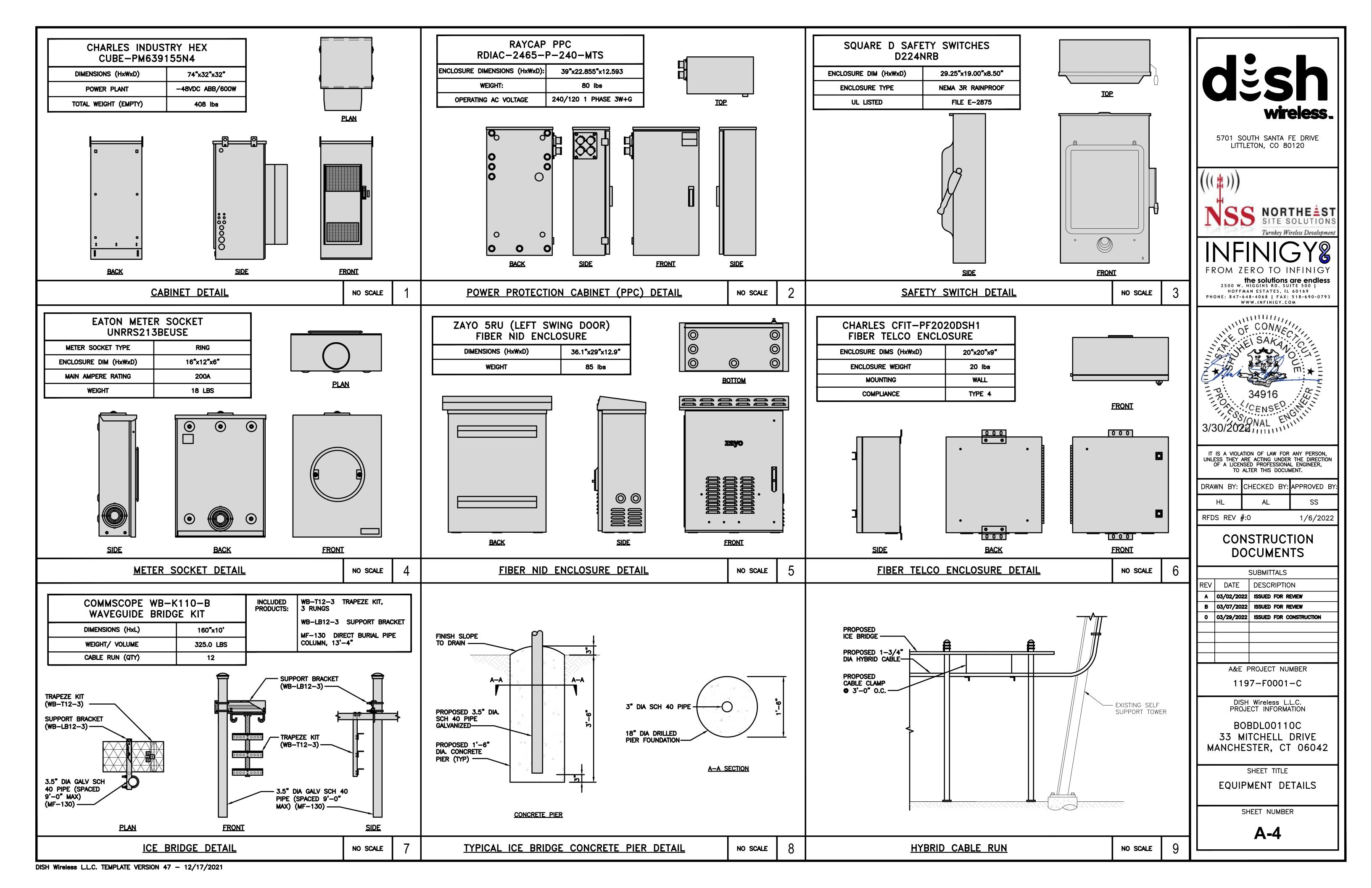
DISH Wireless L.L.C. PROJECT INFORMATION

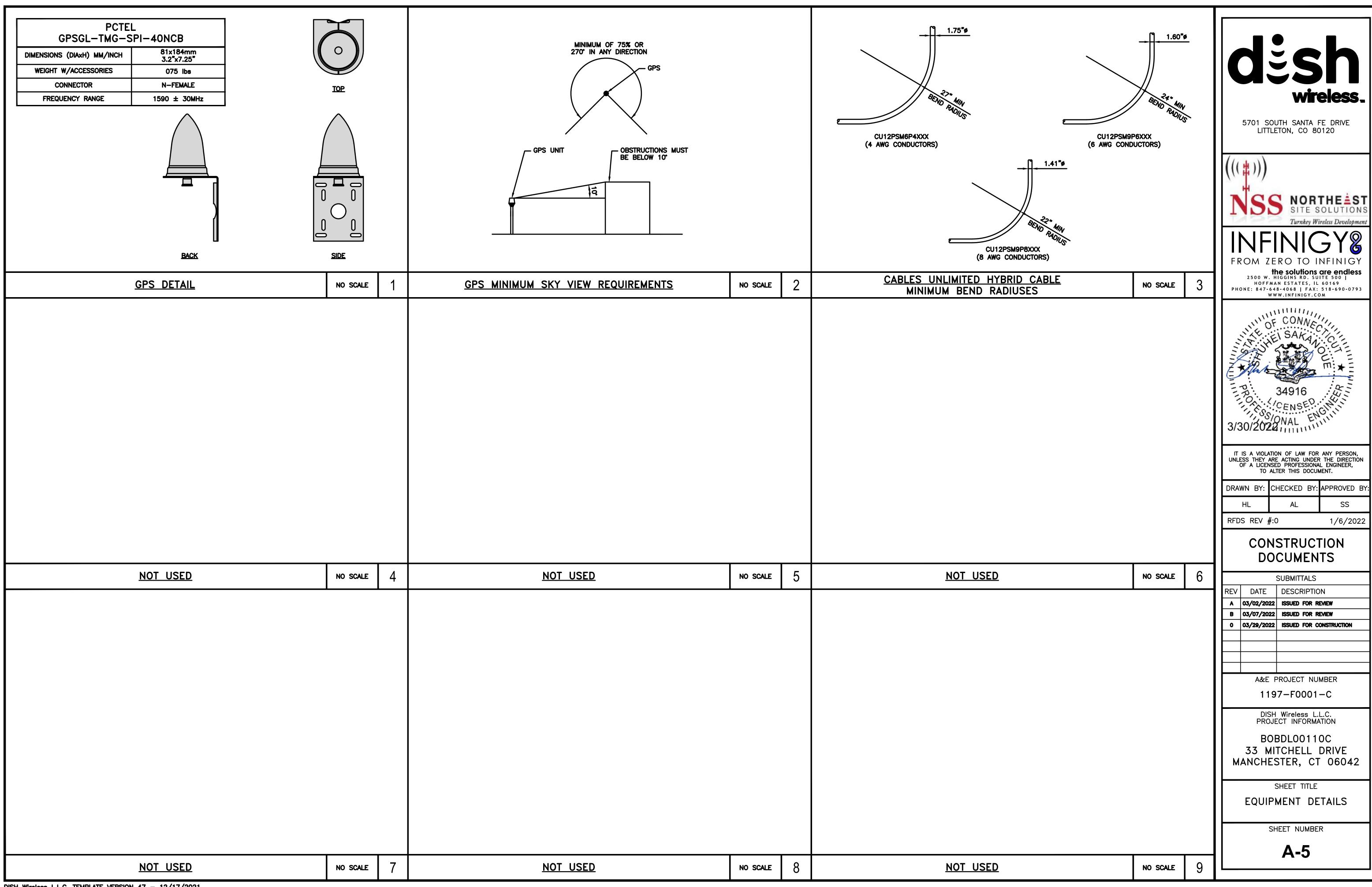
BOBDL00110C 33 MITCHELL DRIVE MANCHESTER, CT 06042

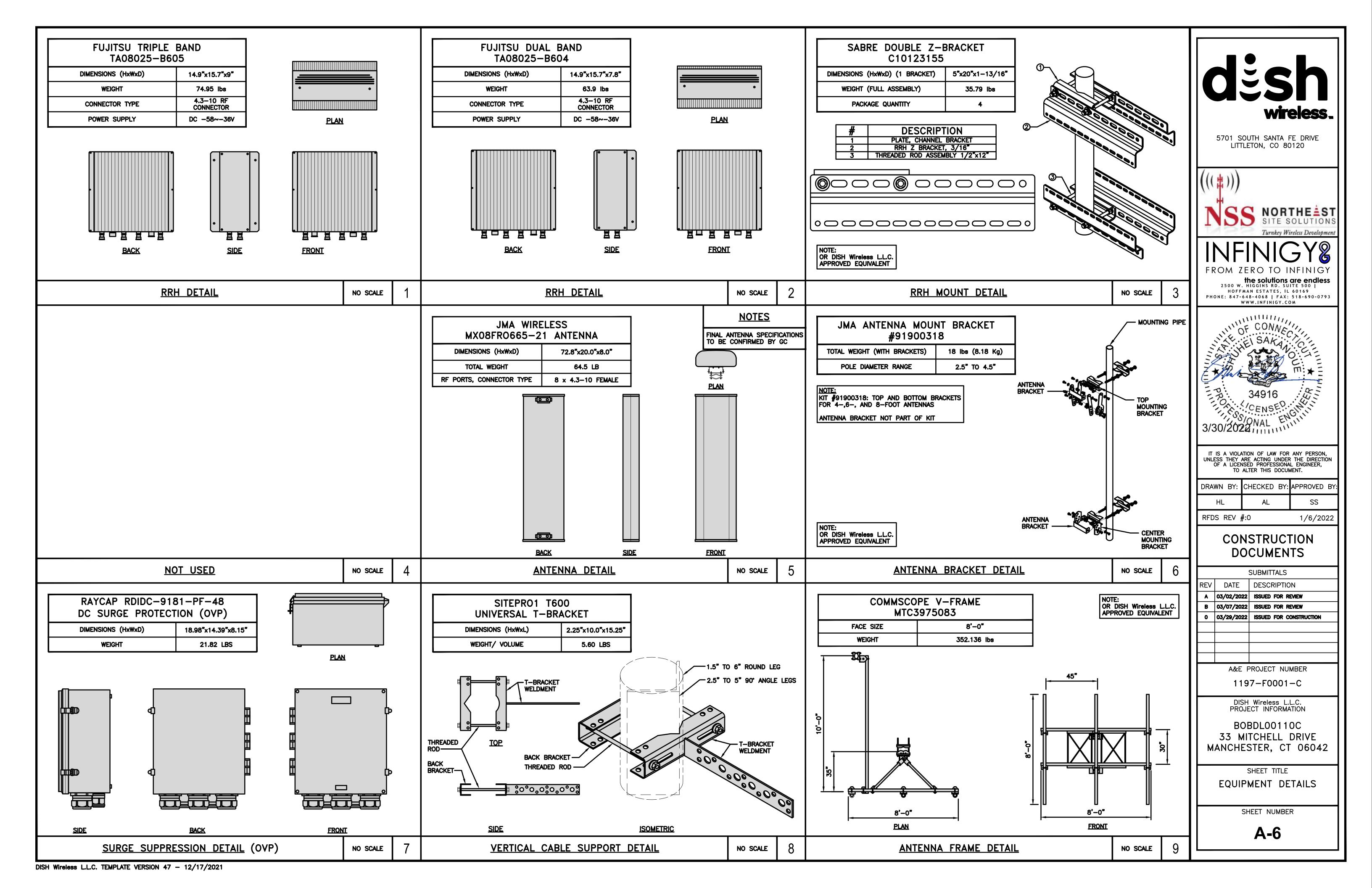
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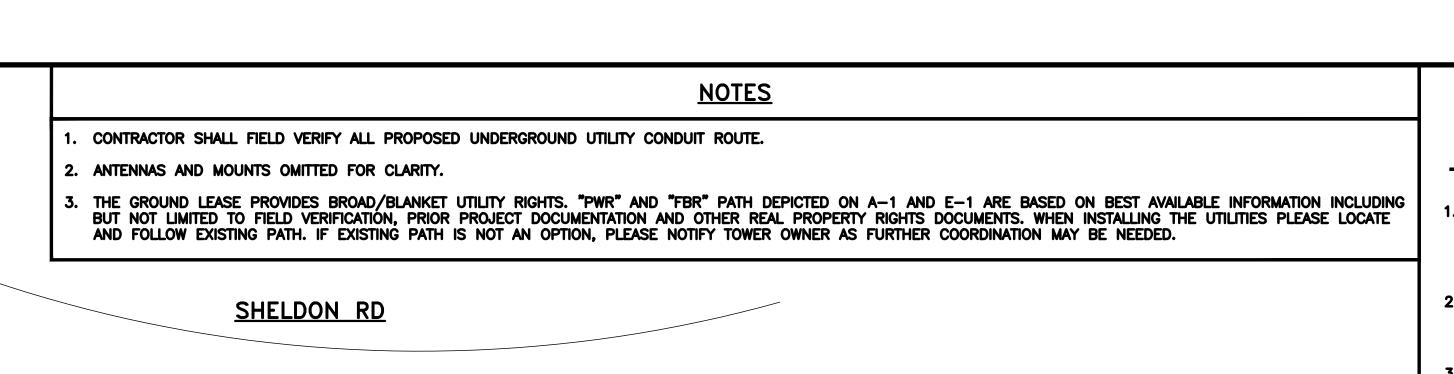
SHEET NUMBER











- EXISTING ACCESS GATE, TYP.

EXISTING 100KW GENERATOR

BY OTHERS

- EXISTING PROPANE TANK

DC POWER WIRING SHALL BE COLOR CODED AT EACH END FOR IDENTIFYING +24V AND -48V CONDUCTORS. RED MARKINGS SHALL IDENTIFY +24V AND BLUE MARKINGS SHALL IDENTIFY -48V.

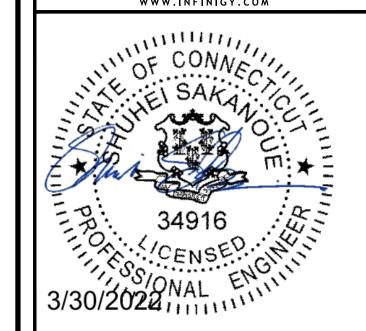
- I. CONTRACTOR SHALL INSPECT THE EXISTING CONDITIONS PRIOR TO SUBMITTING A BID. ANY QUESTIONS ARISING DURING THE BID PERIOD IN REGARDS TO THE CONTRACTOR'S FUNCTIONS, THE SCOPE OF WORK, OR ANY OTHER ISSUE RELATED TO THIS PROJECT SHALL BE BROUGHT UP DURING THE BID PERIOD WITH THE PROJECT MANAGER FOR CLARIFICATION, NOT AFTER THE CONTRACT HAS BEEN AWARDED.
- 2. ALL ELECTRICAL WORK SHALL BE DONE IN ACCORDANCE WITH CURRENT NATIONAL ELECTRICAL CODES AND ALL STATE AND LOCAL CODES, LAWS, AND ORDINANCES. PROVIDE ALL COMPONENTS AND WIRING SIZES AS REQUIRED TO MEET NEC STANDARDS.
- 3. LOCATION OF EQUIPMENT, CONDUIT AND DEVICES SHOWN ON THE DRAWINGS ARE APPROXIMATE AND SHALL BE COORDINATED WITH FIELD CONDITIONS PRIOR TO CONSTRUCTION.
- 4. CONDUIT ROUGH-IN SHALL BE COORDINATED WITH THE MECHANICAL EQUIPMENT TO AVOID LOCATION CONFLICTS. VERIFY WITH THE MECHANICAL EQUIPMENT CONTRACTOR AND COMPLY AS REQUIRED.
- 5. CONTRACTOR SHALL PROVIDE ALL BREAKERS, CONDUITS AND CIRCUITS AS REQUIRED FOR A COMPLETE SYSTEM.
- 6. CONTRACTOR SHALL PROVIDE PULL BOXES AND JUNCTION BOXES AS REQUIRED BY THE NEC ARTICLE 314.
- 7. CONTRACTOR SHALL PROVIDE ALL STRAIN RELIEF AND CABLE SUPPORTS FOR ALL CABLE ASSEMBLIES. INSTALLATION SHALL BE IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS.
- 8. ALL DISCONNECTS AND CONTROLLING DEVICES SHALL BE PROVIDED WITH ENGRAVED PHENOLIC NAMEPLATES INDICATING EQUIPMENT CONTROLLED, BRANCH CIRCUITS INSTALLED ON, AND PANEL FIELD LOCATIONS FED FROM.
- 9. INSTALL AN EQUIPMENT GROUNDING CONDUCTOR IN ALL CONDUITS PER THE SPECIFICATIONS AND NEC 250. THE EQUIPMENT GROUNDING CONDUCTORS SHALL BE BONDED AT ALL JUNCTION BOXES, PULL BOXES, AND ALL DISCONNECT SWITCHES, AND EQUIPMENT CABINETS.
- 10. ALL NEW MATERIAL SHALL HAVE A U.L. LABEL.
- 11. PANEL SCHEDULE LOADING AND CIRCUIT ARRANGEMENTS REFLECT POST—CONSTRUCTION EQUIPMENT.
- 12. CONTRACTOR SHALL BE RESPONSIBLE FOR AS-BUILT PANEL SCHEDULE AND SITE DRAWINGS.
- 13. ALL TRENCHES IN COMPOUND TO BE HAND DUG



5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120



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DRAWN BY:	CHECKED BY:	APPROVED BY:
HL	AL	SS
RFDS REV ;	<b>#</b> :0	1/6/2022

## CONSTRUCTION **DOCUMENTS**

	SUBMITTALS							
REV	DATE DESCRIPTION							
A	03/02/2022	ISSUED FOR REVIEW						
В	03/07/2022	ISSUED FOR REVIEW						
0	03/29/2022	ISSUED FOR CONSTRUCTION						
	A&E F	PROJECT NUMBER						

1197-F0001-C

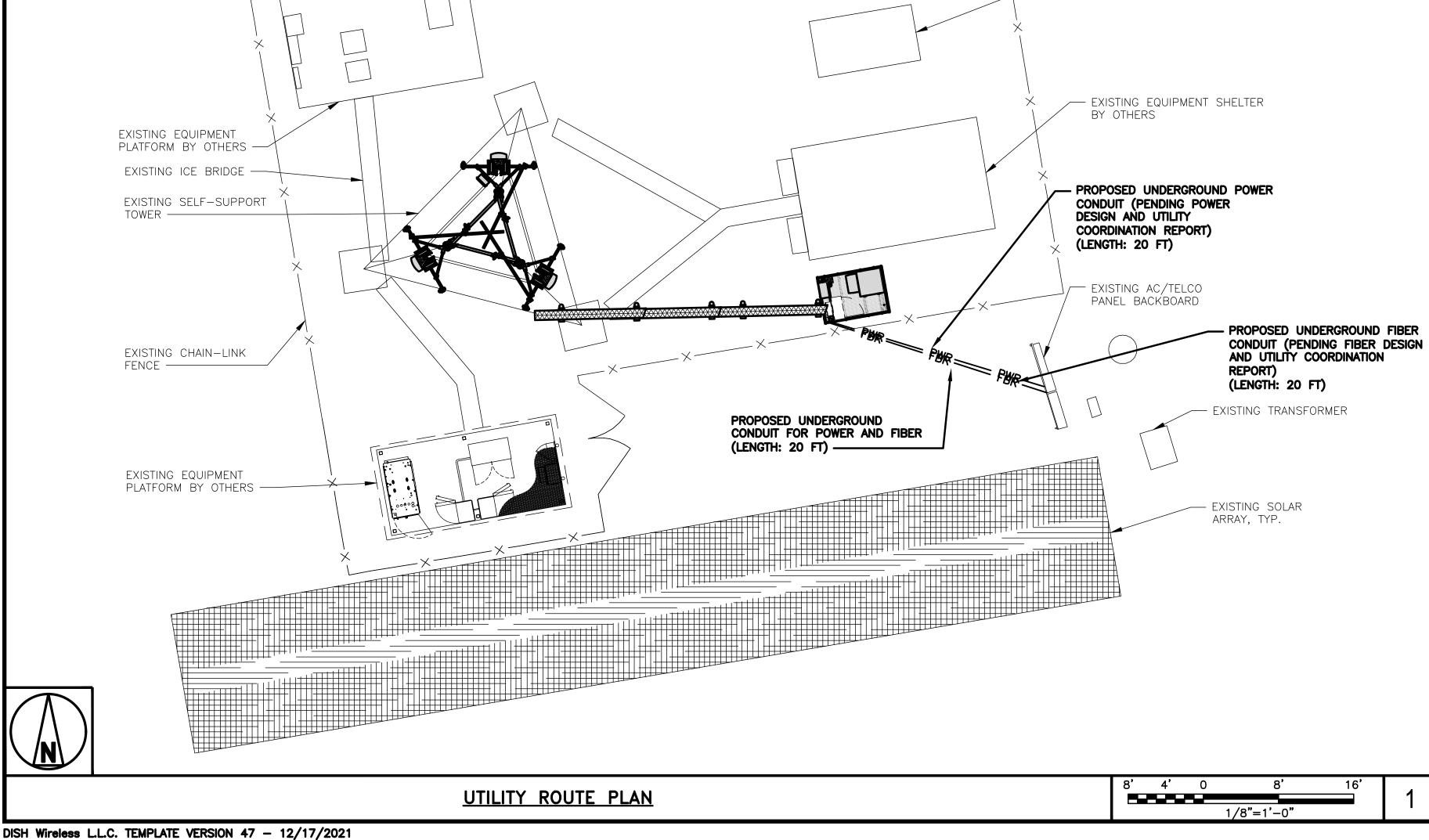
DISH Wireless L.L.C. PROJECT INFORMATION

BOBDL00110C 33 MITCHELL DRIVE MANCHESTER, CT 06042

SHEET TITLE ELECTRICAL/FIBER ROUTE PLAN AND NOTES

SHEET NUMBER

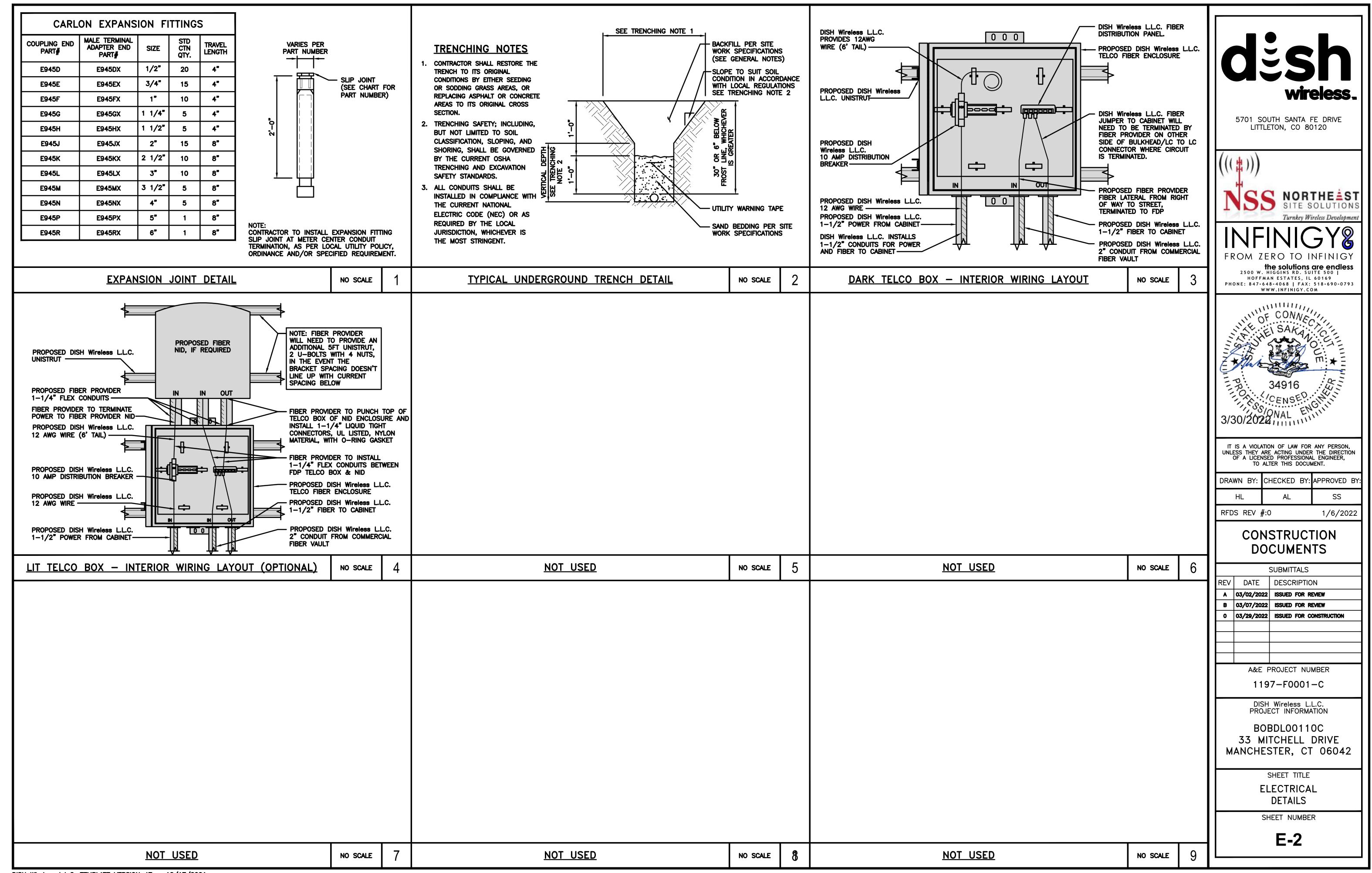
E-1

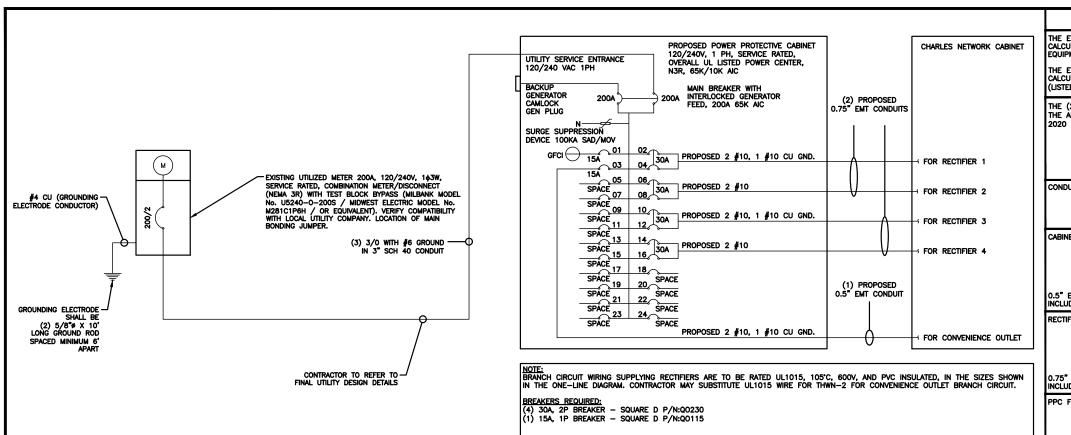


**ELECTRICAL NOTES** 

NO SCALE

EXISTING PROPANE





<u>NOTES</u>

THE ENGINEER OF RECORD HAS PERFORMED ALL REQUIRED SHORT CIRCUIT CALCULATIONS AND THE ALC RATINGS FOR EACH DEVICE IS ADEQUATE TO PROTECT THE EQUIPMENT AND THE ELECTRICAL SYSTEM.

THE ENGINEER OF RECORD HAS PERFORMED ALL REQUIRED VOLTAGE DROP CALCULATIONS AND ALL BRANCH CIRCUIT AND FEEDERS COMPLY WITH THE NEC (LISTED ON T-1) ARTICLE 210.19(A)(1) FPN NO. 4.

THE (2) CONDUITS WITH (4) CURRENT CARRYING CONDUCTORS EACH, SHALL APPLY THE ADJUSTMENT FACTOR OF 80% PER 2014/17 NEC TABLE 310.15(B)(3)(a) OR 2020 NEC TABLE 310.15(C)(1) FOR UL1015 WIRE.

#12 FOR 15A-20A/1P BREAKER: 0.8 x 30A = 24.0A #10 FOR 25A-30A/2P BREAKER: 0.8 x 40A = 32.0A #8 FOR 35A-40A/2P BREAKER: 0.8 x 55A = 44.0A #6 FOR 45A-60A/2P BREAKER: 0.8 x 75A = 60.0A

CONDUIT SIZING: AT 40% FILL PER NEC CHAPTER 9, TABLE 4, ARTICLE 358. 0.5" CONDUIT - 0.122 SQ. IN AREA

0.75" CONDUIT - 0.213 SQ. IN AREA 2.0" CONDUIT - 1.316 SQ. IN AREA 3.0" CONDUIT - 2.907 SQ. IN AREA

CABINET CONVENIENCE OUTLET CONDUCTORS (1 CONDUIT): USING THWN-2, CU.

#10 - 0.0211 SQ. IN X 2 = 0.0422 SQ. IN #10 - 0.0211 SQ. IN X 1 = 0.0211 SQ. IN <GROUND

 $0.5{\rm "}$  EMT CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (3) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

RECTIFIER CONDUCTORS (2 CONDUITS): USING UL1015, CU.

#10 - 0.0266 SQ. IN X 4 = 0.1064 SQ. IN #10 - 0.0082 SQ. IN X 1 = 0.0082 SQ. IN <BARE GROUND

0.75" EMT CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (5) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

PPC FEED CONDUCTORS (1 CONDUIT): USING THWN, CU.

3/0 - 0.2679 SQ. IN X 3 = 0.8037 SQ. IN #6 - 0.0507 SQ. IN X 1 = 0.0507 SQ. IN <GROUND

TOTAL = 0.8544 SQ. IN

3.0" SCH 40 PVC CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (4) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

PPC ONE-LINE DIAGRAM

NO

NO SCALE

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OF CONNECTION OF

DRAWN BY: CHECKED BY: APPROVED BY
HL AL SS

RFDS REV #:0

3/30/2022 NAL ENGINE

### CONSTRUCTION DOCUMENTS

1/6/2022

SUBMITTALS							
DATE DESCRIPTION							
03/02/2022	ISSUED FOR REVIEW						
03/07/2022	ISSUED FOR REVIEW						
03/29/2022	ISSUED FOR CONSTRUCTION						
	DATE 03/02/2022 03/07/2022						

A&E PROJECT NUMBER

1197-F0001-C

DISH Wireless L.L.C. PROJECT INFORMATION

BOBDL00110C 33 MITCHELL DRIVE MANCHESTER, CT 06042

SHEET TITLE

ELECTRICAL ONE-LINE, FAULT CALCS & PANEL SCHEDULE

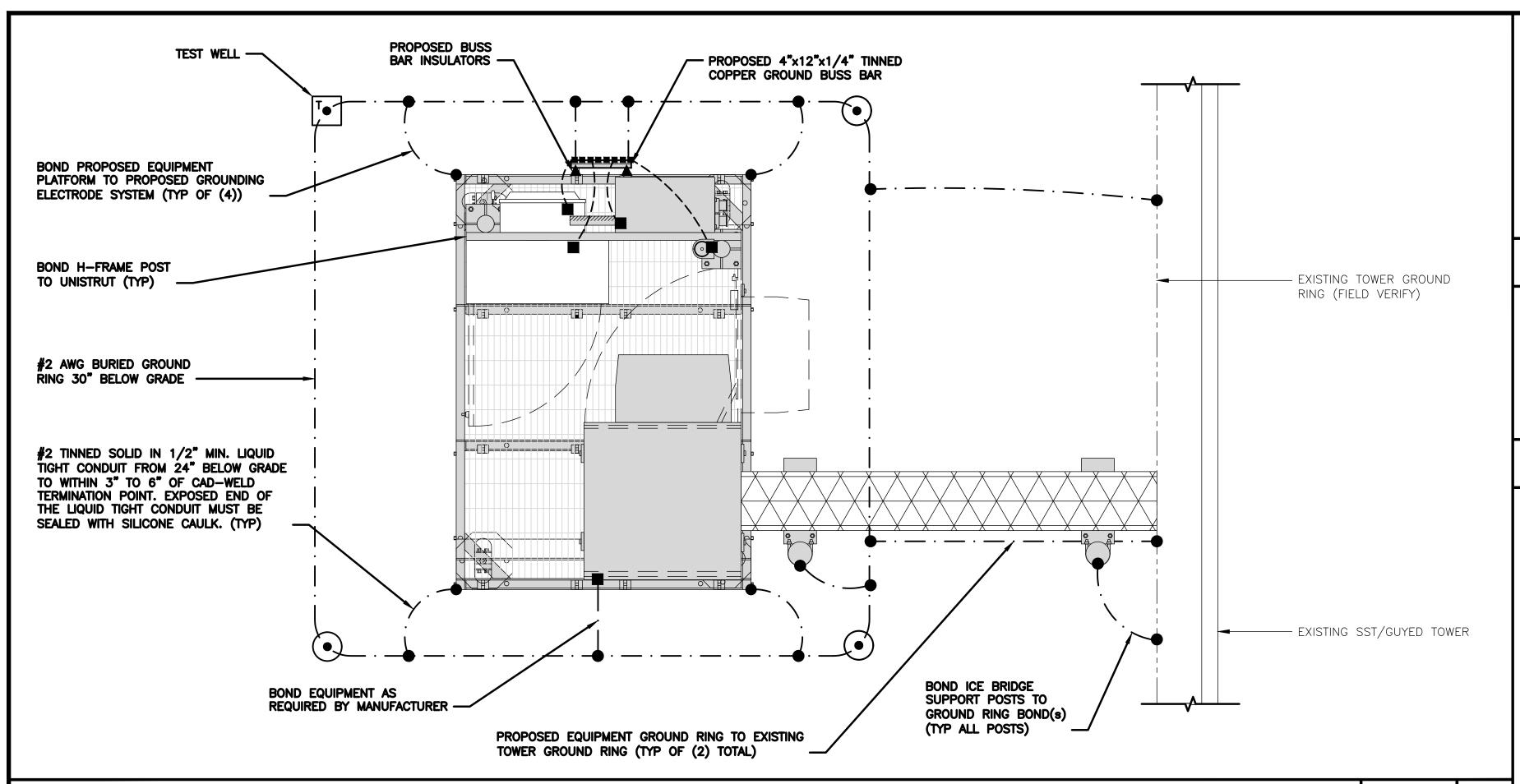
SHEET NUMBER

E-3

LOAD SERVED		AMPS TTS)			TRIP CKT										T PHASE CKT				TRIP		AMPS TTS)	LOAD SERVED
	L1	L2								L1	L2											
PPC GFCI OUTLET CHARLES GFCI OUTLET	180	180	15A 15A	1 3	쉱	B	光	4	30A	2880	2880	ABB/GE INFINITY RECTIFIER 1										
-SPACE- -SPACE-				5	$\leq$	A	*	6	30A	2880	2880	ABB/GE INFINITY RECTIFIER 2										
-SPACE- -SPACE-				9	$\leq$	Ā	4	10	30A	2880	2880	ABB/GE INFINITY RECTIFIER 3										
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/OLTAGE AMPS	180	180								11520	11520											
200A MCB, 1¢, 24 SPA	CE, 120,	240V	L1			L2																
MB RATING: 65,000 AIC			1170	0	1	170	0		TAGE AM	PS												
			98		<u></u>	98		AMI														
			l		9 <u>8</u> 23				( AMPS ( 125%													

PANEL SCHEDULE

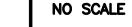
NO SCALE 2 NOT USED NO SCALE 3



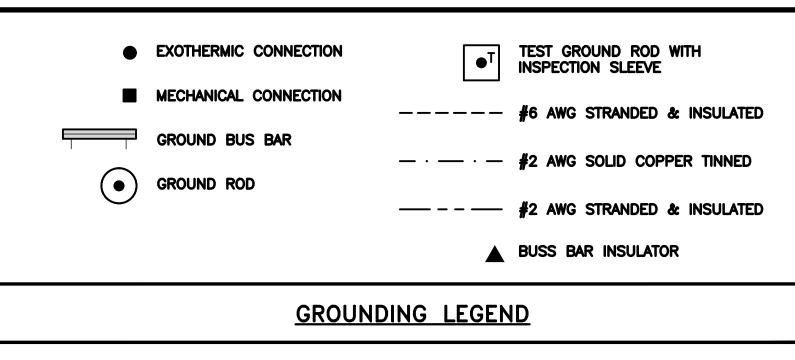
TYPICAL EQUIPMENT GROUNDING PLAN

### **NOTES**

ANTENNAS AND OVP SHOWN ARE GENERIC AND NOT REFERENCING TO A SPECIFIC MANUFACTURER. THIS LAYOUT IS FOR REFERENCE PURPOSES ONLY



NO SCALE



- 1. GROUNDING IS SHOWN DIAGRAMMATICALLY ONLY.
- 2. CONTRACTOR SHALL GROUND ALL EQUIPMENT AS A COMPLETE SYSTEM. GROUNDING SHALL BE IN COMPLIANCE WITH NEC SECTION 250 AND DISH Wireless L.L.C. GROUNDING AND BONDING REQUIREMENTS AND MANUFACTURER'S SPECIFICATIONS.
- 3. ALL GROUND CONDUCTORS SHALL BE COPPER; NO ALUMINUM CONDUCTORS SHALL BE USED.

#### **GROUNDING KEY NOTES**

- EXTERIOR GROUND RING: #2 AWG SOLID COPPER, BURIED AT A DEPTH OF AT LEAST 30 INCHES BELOW A EXTERIOR GROUND RING: #2 AWG SOLID COPPER, BURIED AT A DEFIT OF AT LESS SO THE EXTERIOR WALL GRADE, OR 6 INCHES BELOW THE FROST LINE AND APPROXIMATELY 24 INCHES FROM THE EXTERIOR WALL OR FOOTING.
- TOWER GROUND RING: THE GROUND RING SYSTEM SHALL BE INSTALLED AROUND AN ANTENNA TOWER'S LEGS, AND/OR GUY ANCHORS. WHERE SEPARATE SYSTEMS HAVE BEEN PROVIDED FOR THE TOWER AND THE BUILDING, AT LEAST TWO BONDS SHALL BE MADE BETWEEN THE TOWER RING GROUND SYSTEM AND THE BUILDING RING GROUND SYSTEM USING MINIMUM #2 AWG SOLID COPPER CONDUCTORS.
- INTERIOR GROUND RING: #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTOR EXTENDED AROUND THE PERIMETER OF THE EQUIPMENT AREA ALL NON-TELECOMMUNICATIONS OF ATTEMPT AND AREA ALL NON-TELECOMMUNICATIONS OF ATTEMPT. PERIMETER OF THE EQUIPMENT AREA. ALL NON-TELECOMMUNICATIONS RELATED METALLIC OBJECTS FOUND WITHIN A SITE SHALL BE GROUNDED TO THE INTERIOR GROUND RING WITH #6 AWG STRANDED GREEN INSULATED CONDUCTOR.
- BOND TO INTERIOR GROUND RING: #2 AWG SOLID TINNED COPPER WIRE PRIMARY BONDS SHALL BE PROVIDED AT LEAST AT FOUR POINTS ON THE INTERIOR GROUND RING, LOCATED AT THE CORNERS OF THE
- E GROUND ROD: UL LISTED COPPER CLAD STEEL. MINIMUM 1/2" DIAMETER BY EIGHT FEET LONG. GROUND RODS SHALL BE INSTALLED WITH INSPECTION SLEEVES. GROUND RODS SHALL BE DRIVEN TO THE DEPTH OF GROUND ROD: UL LISTED COPPER CLAD STEEL. MINIMUM 1/2" DIAMETER BY EIGHT FEET LONG. GROUND GROUND RING CONDUCTOR.
- F CELL REFERENCE GROUND BAR: POINT OF GROUND REFERENCE FOR ALL COMMUNICATIONS EQUIPMENT FRAMES. ALL BONDS ARE MADE WITH #2 AWG UNLESS NOTED OTHERWISE STRANDED GREEN INSULATED COPPER CONDUCTORS. BOND TO GROUND RING WITH (2) #2 SOLID TINNED COPPER CONDUCTORS.
- HATCH PLATE GROUND BAR: BOND TO THE INTERIOR GROUND RING WITH TWO #2 AWG STRANDED GREEN G HATCH PLATE GROUND BAR: BOND TO THE INTERIOR GROUND RING WITH TWO #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTORS. WHEN A HATCH-PLATE AND A CELL REFERENCE GROUND BAR ARE BOTH PRESENT, THE CRGB MUST BE CONNECTED TO THE HATCH-PLATE AND TO THE INTERIOR GROUND RING USING (2) TWO #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTORS EACH.
- EXTERIOR CABLE ENTRY PORT GROUND BARS: LOCATED AT THE ENTRANCE TO THE CELL SITE BUILDING. BOND TO GROUND RING WITH A #2 AWG SOLID TINNED COPPER CONDUCTORS WITH AN EXOTHERMIC WELD AND INSPECTION SLEEVE.
- TELCO GROUND BAR: BOND TO BOTH CELL REFERENCE GROUND BAR OR EXTERIOR GROUND RING.
- FRAME BONDING: THE BONDING POINT FOR TELECOM EQUIPMENT FRAMES SHALL BE THE GROUND BUS THAT IS NOT ISOLATED FROM THE EQUIPMENTS METAL FRAMEWORK.
- INTERIOR UNIT BONDS: METAL FRAMES, CABINETS AND INDIVIDUAL METALLIC UNITS LOCATED WITH THE AREA
  OF THE INTERIOR GROUND RING REQUIRE A #6 AWG STRANDED GREEN INSULATED COPPER BOND TO THE OF THE INTERIOR GROUND RING REQUIRE A #6 AWG STRANDED GREEN INSULATED COPPER BOND TO THE INTERIOR GROUND RING.
- FENCE AND GATE GROUNDING: METAL FENCES WITHIN 7 FEET OF THE EXTERIOR GROUND RING OR OBJECTS
  RONDED TO THE EXTERIOR GROUND RING SHALL BE RONDED TO THE CROUND RING WITH A #2 AWC SOLD BONDED TO THE EXTERIOR GROUND RING SHALL BE BONDED TO THE GROUND RING WITH A #2 AWG SOLID TINNED COPPER CONDUCTOR AT AN INTERVAL NOT EXCEEDING 25 FEET. BONDS SHALL BE MADE AT EACH GATE POST AND ACROSS GATE OPENINGS.
- EXTERIOR UNIT BONDS: METALLIC OBJECTS, EXTERNAL TO OR MOUNTED TO THE BUILDING, SHALL BE BONDED TO THE EXTERIOR GROUND RING. USING #2 TINNED SOLID COPPER WIRE TO THE EXTERIOR GROUND RING. USING #2 TINNED SOLID COPPER WIRE
- N ICE BRIDGE SUPPORTS: EACH ICE BRIDGE LEG SHALL BE BONDED TO THE GROUND RING WITH #2 AWG BARE TINNED COPPER CONDUCTOR. PROVIDE EXOTHERMIC WELDS AT BOTH THE ICE BRIDGE LEG AND BURIED GROUND RING.
- DURING ALL DC POWER SYSTEM CHANGES INCLUDING DC SYSTEM CHANGE OUTS, RECTIFIER REPLACEMENTS OR ADDITIONS, BREAKER DISTRIBUTION CHANGES, BATTERY ADDITIONS, BATTERY REPLACEMENTS AND INSTALLATIONS OR CHANGES TO DC CONVERTER SYSTEMS IT SHALL BE REQUIRED THAT SERVICE CONTRACTORS VERIFY ALL DC POWER SYSTEMS ARE EQUIPPED WITH A MASTER DC SYSTEM RETURN GROUND CONDUCTOR FROM THE DC POWER SYSTEM COMMON RETURN BUS DIRECTLY CONNECTED TO THE CELL SITE REFERENCE GROUND BAR
- (P) TOWER TOP COLLECTOR BUSS BAR IS TO BE MECHANICALLY BONDED TO TOWER STEEL.

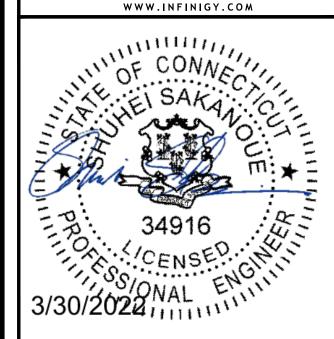
REFER TO DISH Wireless L.L.C. GROUNDING NOTES.

wireless.

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HL	AL	SS
RFDS REV ;	<b>#</b> :0	1/6/2022

### CONSTRUCTION **DOCUMENTS**

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REV	DATE	DESCRIPTION					
A	03/02/2022	ISSUED FOR REVIEW					
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							A&E PROJECT NUMBER  1197-F0001-C

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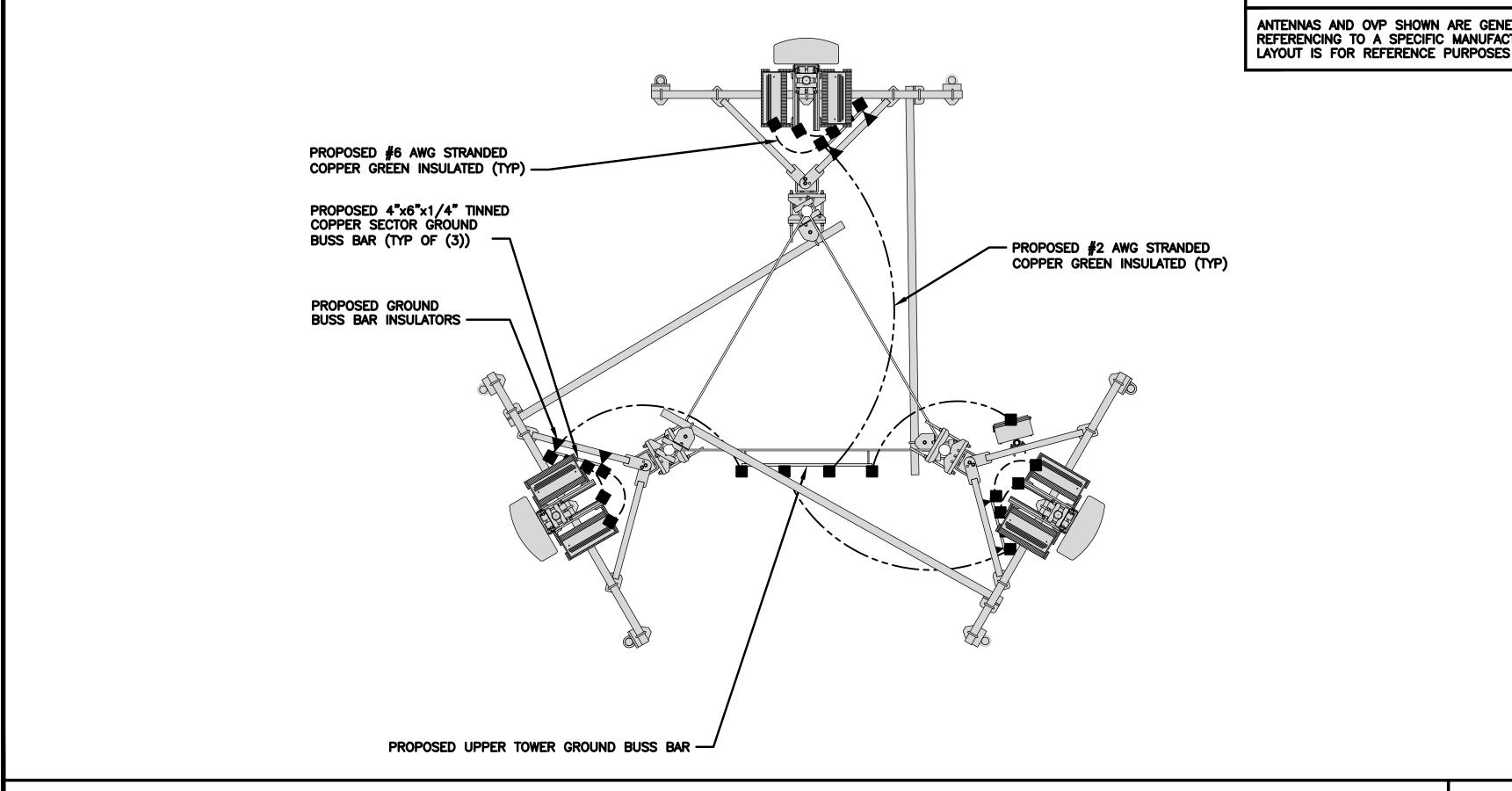
DISH Wireless L.L.C. PROJECT INFORMATION

BOBDL00110C 33 MITCHELL DRIVE MANCHESTER, CT 06042

SHEET TITLE GROUNDING PLANS AND NOTES

SHEET NUMBER

**G-1** 

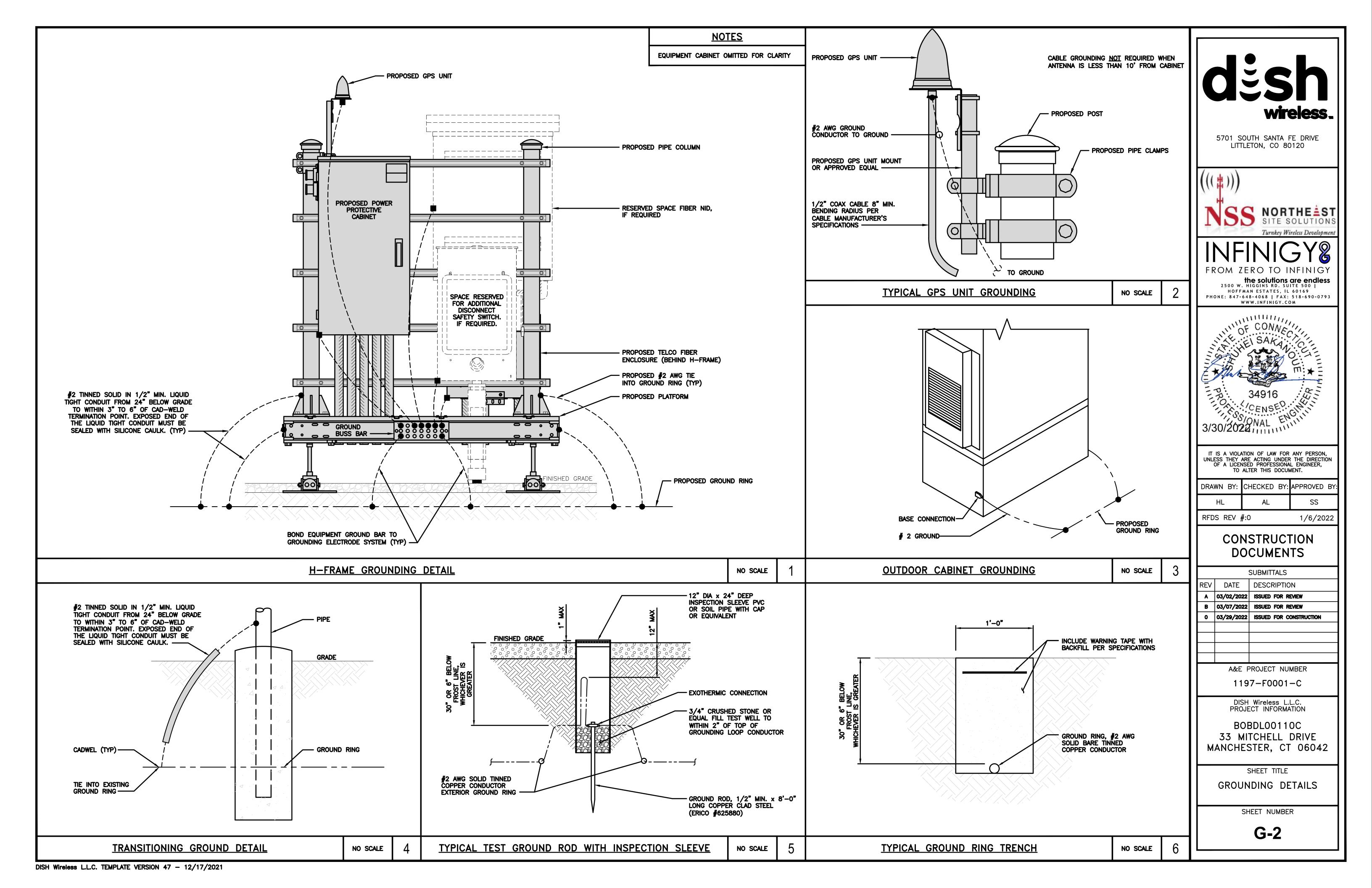


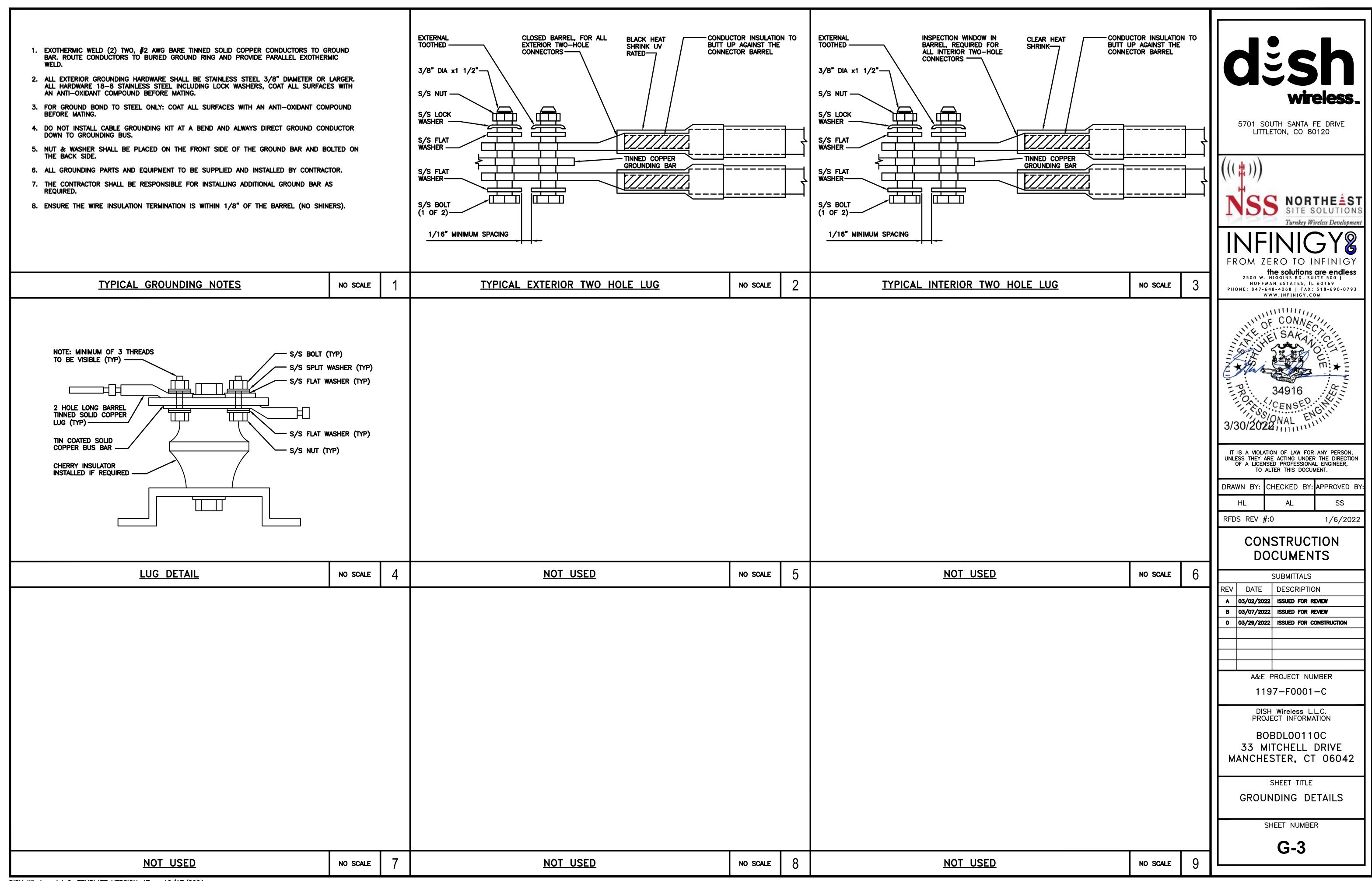
TYPICAL ANTENNA GROUNDING PLAN

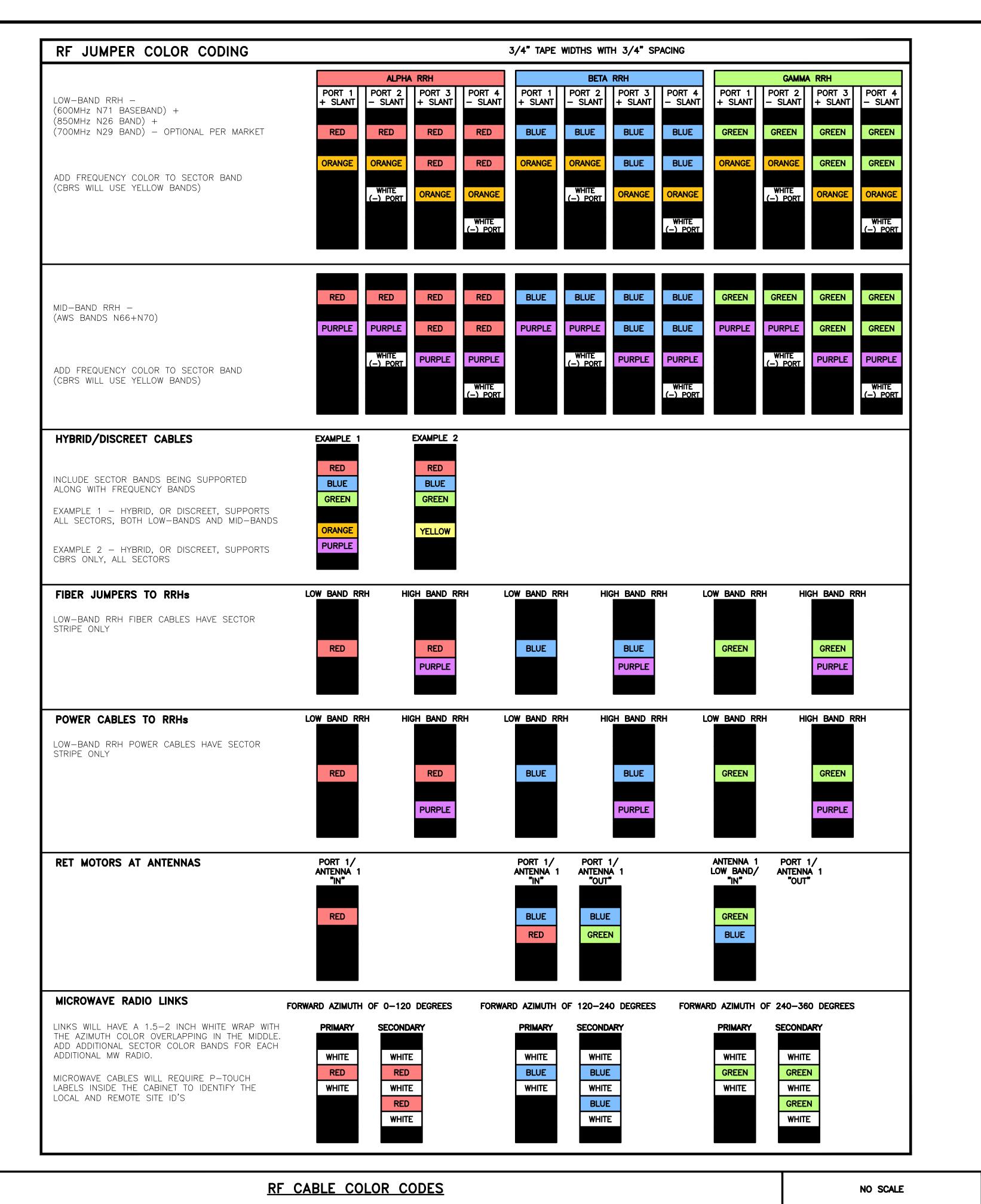
**GROUNDING KEY NOTES** 

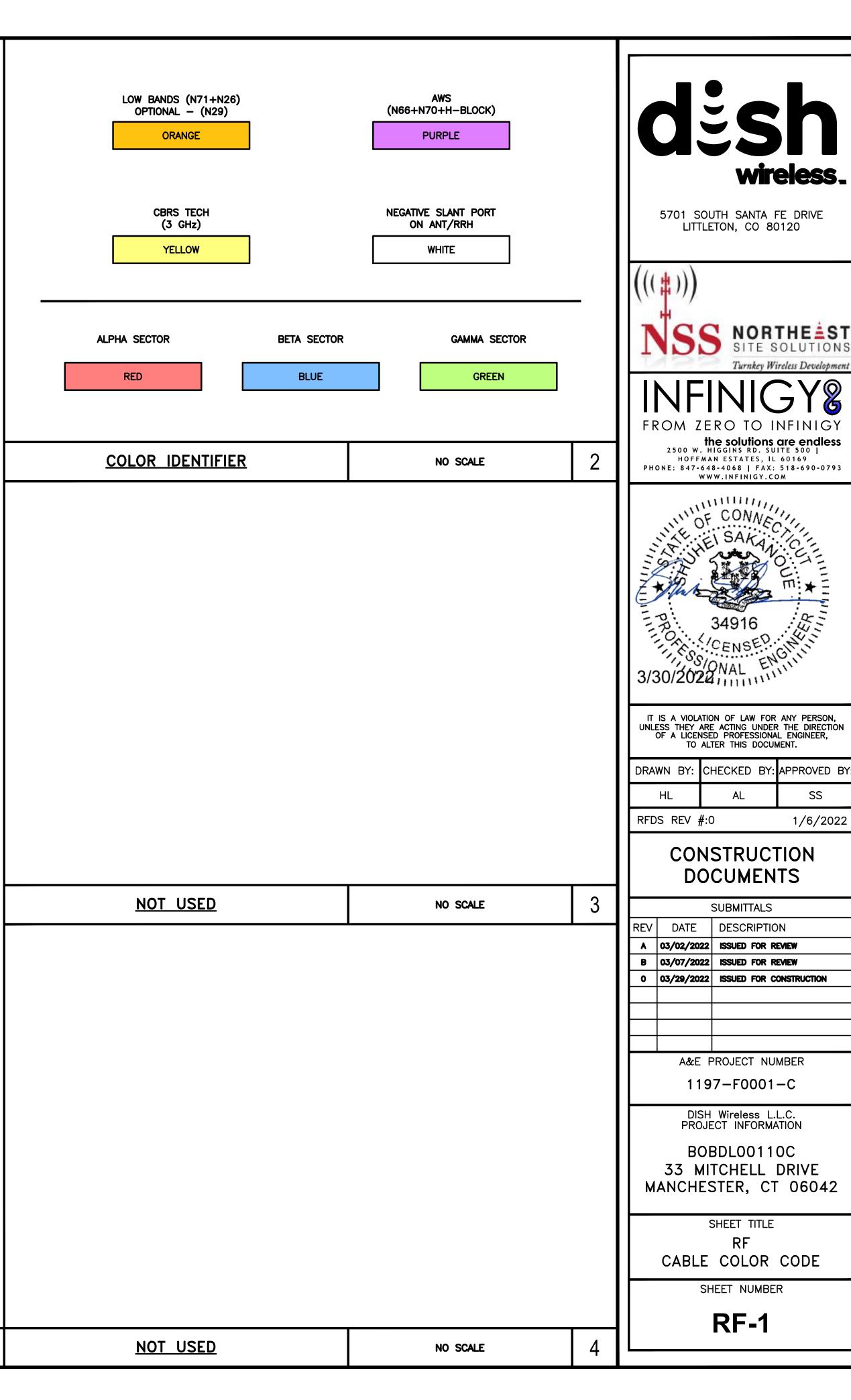
NO SCALE

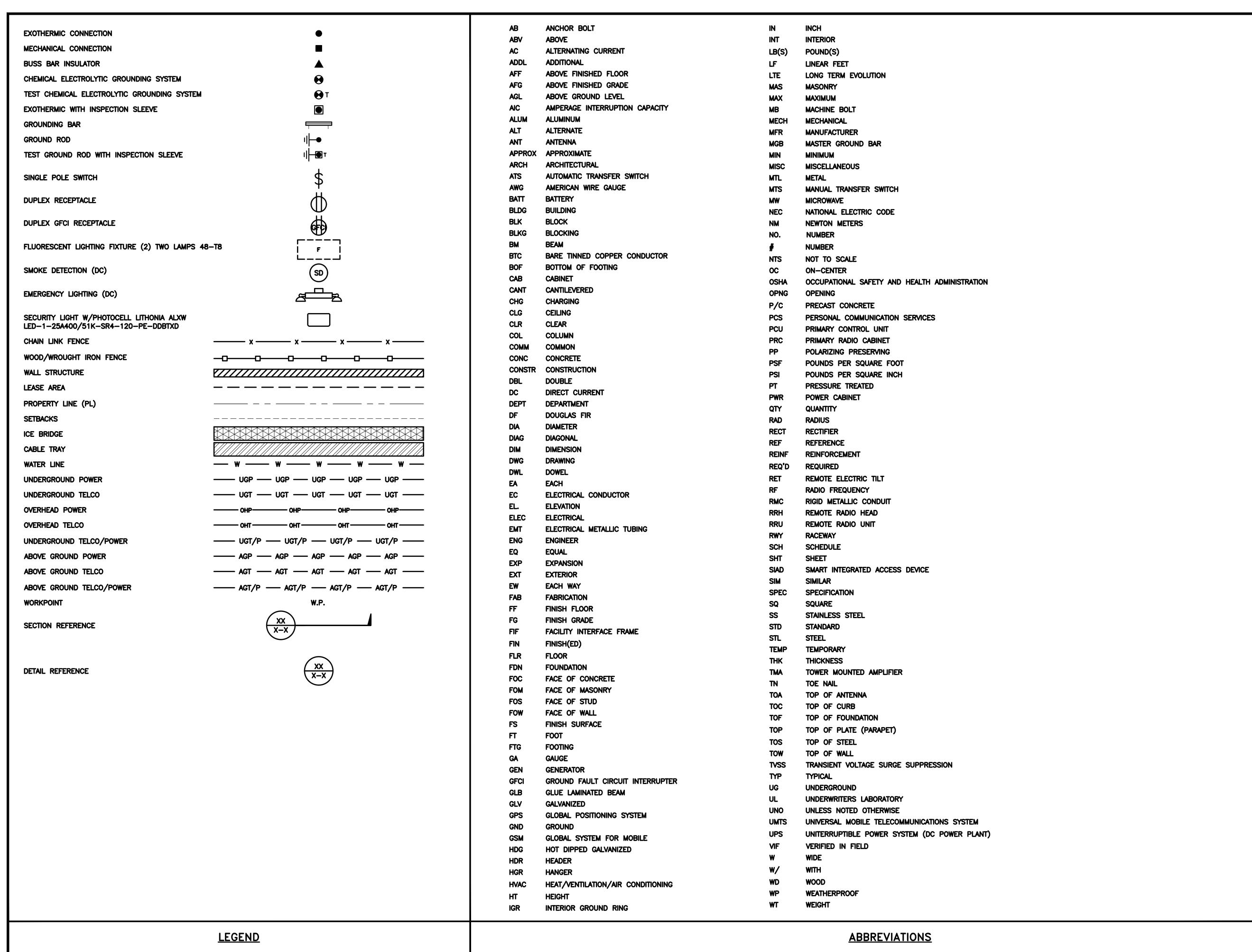
DISH Wireless L.L.C. TEMPLATE VERSION 47 - 12/17/2021











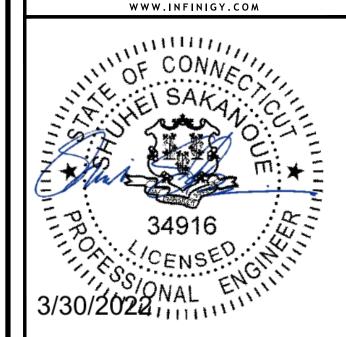


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	В	03/07/2022	ISSUED FOR REVIEW								
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1197-F0001-C

DISH Wireless L.L.C. PROJECT INFORMATION

BOBDL00110C 33 MITCHELL DRIVE MANCHESTER, CT 06042

SHEET TITLE

LEGEND AND
ABBREVIATIONS

SHEET NUMBER

·	SIGN TYPES					
TYPE	COLOR	COLOR CODE PURPOSE				
INFORMATION	GREEN	"INFORMATIONAL SIGN" TO NOTIFY OTHERS OF SITE OWNERSHIP & CONTACT NUMBER AND POTENTIAL RF EXPOSURE.				
NOTICE	BLUE	"NOTICE BEYOND THIS POINT" RF FIELDS BEYOND THIS POINT MAY EXCEED THE FCC GENERAL PUBLIC EXPOSURE LIMIT. OBEY ALL POSTED SIGNS AND SITE GUIDELINES FOR WORKING IN RF ENVIRONMENTS. IN ACCORDANCE WITH FEDERAL COMMUNICATIONS COMMISSION RULES ON RADIO FREQUENCY EMISSIONS 47 CFR-1.1307(b)				
CAUTION	YELLOW	"CAUTION BEYOND THIS POINT" RF FIELDS BEYOND THIS POINT MAY EXCEED THE FCC GENERAL PUBLIC EXPOSURE LIMIT. OBEY ALL POSTED SIGNS AND SITE GUIDELINES FOR WORKING IN RF ENVIRONMENTS. IN ACCORDANCE WITH FEDERAL COMMUNICATIONS COMMISSION RULES ON RADIO FREQUENCY EMISSIONS 47 CFR-1.1307(b)				
WARNING	ORANGE/RED	"WARNING BEYOND THIS POINT" RF FIELDS AT THIS SITE EXCEED FCC RULES FOR HUMAN EXPOSURE. FAILURE TO OBEY ALL POSTED SIGNS AND SITE GUIDELINES FOR WORKING IN RF ENVIRONMENTS COULD RESULT IN SERIOUS INJURY. IN ACCORDANCE WITH FEDERAL COMMUNICATIONS COMMISSION RULES ON RADIO FREQUENCY EMISSIONS 47 CFR-1.1307(b)				

#### **SIGN PLACEMENT:**

- IF EME REPORT IS NOT AVAILABLE AT THE TIME OF CREATION OF CONSTRUCTION DOCUMENTS; PLEASE CONTACT DISH Wireless L.L.C. CONSTRUCTION MANAGER FOR

- 1. FOR DISH Wireless L.L.C. LOGO, SEE DISH Wireless L.L.C. DESIGN SPECIFICATIONS (PROVIDED BY DISH Wireless L.L.C.)

- 5. ALL SIGNS WILL BE SECURED WITH EITHER STAINLESS STEEL ZIP TIES OR STAINLESS STEEL TECH SCREWS
- 6. ALL SIGNS TO BE 8.5"x11" AND MADE WITH 0.04" OF ALUMINUM MATERIAL

# INFORMATION

This is an access point to an area with transmitting antennas.

Obey all signs and barriers beyond this point. Call the DISH Wireless L.L.C. NOC at 1-866-624-6874

Site ID:



THIS SIGN IS FOR REFERENCE PURPOSES ONLY

# NOTICE



#### **Transmitting Antenna(s)**

Radio frequency fields beyond this point MAY **EXCEED** the FCC Occupational exposure limit.

Obey all posted signs and site guidelines for working in radio frequency environments.

Call the DISH Wireless L.L.C. NOC at 1-866-624-6874 prior to working beyond this point.

# A CAUTION



Transmitting Antenna(s)

Radio frequency fields beyond this point MAY **EXCEED** the FCC Occupational exposure limit.

Obey all posted signs and site guidelines for working in radio frequency environments.

Call the DISH Wireless L.L.C. NOC at 1-866-624-6874 prior to working beyond this point.

Site ID:

dish



Transmitting Antenna(s)

Radio frequency fields beyond this point **EXCEED** the FCC Occupational exposure limit.

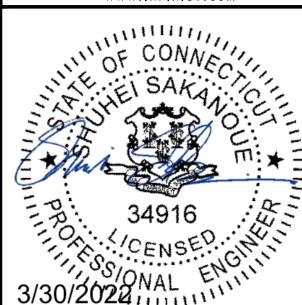
Obey all posted signs and site guidelines for working in radio frequency environments.

Call the DISH Wireless L.L.C. NOC at 1-866-624-6874 prior to working beyond this point.

dish

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1/6/2022

RFDS REV #:0

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**								

A&E PROJECT NUMBER 1197-F0001-C

DISH Wireless L.L.C. PROJECT INFORMATION

BOBDL00110C 33 MITCHELL DRIVE MANCHESTER, CT 06042

> SHEET TITLE RF SIGNAGE

SHEET NUMBER

GN-2

RF SIGNAGE

#### SITE ACTIVITY REQUIREMENTS:

- 1. NOTICE TO PROCEED NO WORK SHALL COMMENCE PRIOR TO CONTRACTOR RECEIVING A WRITTEN NOTICE TO PROCEED (NTP) AND THE ISSUANCE OF A PURCHASE ORDER. PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE DISH Wireless L.L.C. AND TOWER OWNER NOC & THE DISH Wireless L.L.C. AND TOWER CONSTRUCTION MANAGER.
- 2. "LOOK UP" DISH Wireless L.L.C. AND TOWER OWNER SAFETY CLIMB REQUIREMENT:

THE INTEGRITY OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY ON THE STRUCTURE. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO: PINCHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM ITS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTIONAL WEAR, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO IMPEDE/BLOCK ITS INTENDED USE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGGED OUT AND REPORTED TO YOUR DISH Wireless L.L.C. AND DISH Wireless L.L.C. AND TOWER OWNER POC OR CALL THE NOC TO GENERATE A SAFETY CLIMB MAINTENANCE AND CONTRACTOR NOTICE TICKET.

- 3. PRIOR TO THE START OF CONSTRUCTION, ALL REQUIRED JURISDICTIONAL PERMITS SHALL BE OBTAINED. THIS INCLUDES, BUT IS NOT LIMITED TO, BUILDING, ELECTRICAL, MECHANICAL, FIRE, FLOOD ZONE, ENVIRONMENTAL, AND ZONING. AFTER ONSITE ACTIVITIES AND CONSTRUCTION ARE COMPLETED, ALL REQUIRED PERMITS SHALL BE SATISFIED AND CLOSED OUT ACCORDING TO LOCAL JURISDICTIONAL REQUIREMENTS.
- 4. ALL CONSTRUCTION MEANS AND METHODS; INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS, AND RESCUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONTAINED HEREIN, AND SHALL MEET ANSI/ASSE A10.48 (LATEST EDITION); FEDERAL, STATE, AND LOCAL REGULATIONS; AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL ADHERE TO ANSI/ASSE A10.48 (LATEST EDITION) AND DISH Wireless L.L.C. AND TOWER OWNER STANDARDS, INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION, TO CERTIFY THE SUPPORTING STRUCTURE(S) IN ACCORDANCE WITH ANSI/TIA—322 (LATEST EDITION).
- 5. ALL SITE WORK TO COMPLY WITH DISH Wireless L.L.C. AND TOWER OWNER INSTALLATION STANDARDS FOR CONSTRUCTION ACTIVITIES ON DISH Wireless L.L.C. AND TOWER OWNER TOWER SITE AND LATEST VERSION OF ANSI/TIA-1019-A-2012 "STANDARD FOR INSTALLATION, ALTERATION, AND MAINTENANCE OF ANTENNA SUPPORTING STRUCTURES AND ANTENNAS."
- 6. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY DISH Wireless L.L.C. AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- 7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 8. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 9. THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES INCLUDING PRIVATE LOCATES SERVICES PRIOR TO THE START OF CONSTRUCTION.
- 10. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION E) CONSTRUCTION SAFETY PROCEDURES.
- 11. ALL SITE WORK SHALL BE AS INDICATED ON THE STAMPED CONSTRUCTION DRAWINGS AND DISH PROJECT SPECIFICATIONS, LATEST APPROVED REVISION.
- 12. CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH AT THE COMPLETION OF THE WORK. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
- 13. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF DISH Wireless L.L.C. AND TOWER OWNER, AND/OR LOCAL UTILITIES.
- 14. THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE TECHNICAL SPECIFICATION FOR SITE SIGNAGE REQUIRED BY LOCAL JURISDICTION AND SIGNAGE REQUIRED ON INDIVIDUAL PIECES OF EQUIPMENT, ROOMS, AND SHELTERS.
- 15. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE CARRIER'S EQUIPMENT AND TOWER AREAS.
- 16. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
- 17. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION AS SPECIFIED ON THE CONSTRUCTION DRAWINGS AND/OR PROJECT SPECIFICATIONS.
- 18. CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.
- 19. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.
- 20. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS AND RADIOS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 21. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.
- 22. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.

#### **GENERAL NOTES:**

1.FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:

CONTRACTOR: GENERAL CONTRACTOR RESPONSIBLE FOR CONSTRUCTION

CARRIER:DISH Wireless L.L.C.

TOWER OWNER:TOWER OWNER

- 2. THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE ENGINEERS IN THIS OR SIMILAR LOCALITIES. IT IS ASSUMED THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKPEOPLE WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.
- 3. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY FOR PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, FORMWORK, SHORING, ETC. SITE VISITS BY THE ENGINEER OR HIS REPRESENTATIVE WILL NOT INCLUDE INSPECTION OF THESE ITEMS AND IS FOR STRUCTURAL OBSERVATION OF THE FINISHED STRUCTURE ONLY.
- 4. NOTES AND DETAILS IN THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE GREATER, MORE STRICT REQUIREMENTS, SHALL GOVERN. IF FURTHER CLARIFICATION IS REQUIRED CONTACT THE ENGINEER OF RECORD.
- 5. SUBSTANTIAL EFFORT HAS BEEN MADE TO PROVIDE ACCURATE DIMENSIONS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.
- 6. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CARRIER POC AND TOWER OWNER.
- 7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 8. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
- 9. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 10. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CARRIER AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- 11. CONTRACTOR IS TO PERFORM A SITE INVESTIGATION, BEFORE SUBMITTING BIDS, TO DETERMINE THE BEST ROUTING OF ALL CONDUITS FOR POWER, AND TELCO AND FOR GROUNDING CABLES AS SHOWN IN THE POWER, TELCO, AND GROUNDING PLAN DRAWINGS.
- 12. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF DISH Wireless L.L.C. AND TOWER OWNER
- 13. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 14. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.

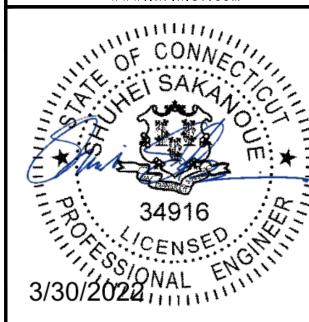


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DRAWN BY:	CHECKED BY:	APPROVED BY:
HL	AL	SS
RFDS REV	#:O	1/6/2022

# CONSTRUCTION DOCUMENTS

	SUBMITTALS							
REV	DATE	DESCRIPTION						
A	03/02/2022	ISSUED FOR REVIEW						
В	03/07/2022	ISSUED FOR REVIEW						
0	03/29/2022	ISSUED FOR CONSTRUCTION						
	A&E PROJECT NUMBER							
	1197-F0001-C							

DISH Wireless L.L.C. PROJECT INFORMATION

BOBDL00110C 33 MITCHELL DRIVE MANCHESTER, CT 06042

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

#### CONCRETE, FOUNDATIONS, AND REINFORCING STEEL:

- 1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST—IN—PLACE CONCRETE.
- 2. UNLESS NOTED OTHERWISE, SOIL BEARING PRESSURE USED FOR DESIGN OF SLABS AND FOUNDATIONS IS ASSUMED TO BE 1000
- 3. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f'c) OF 3000 psi AT 28 DAYS, UNLESS NOTED OTHERWISE. NO MORE THAN 90 MINUTES SHALL ELAPSE FROM BATCH TIME TO TIME OF PLACEMENT UNLESS APPROVED BY THE ENGINEER OF RECORD. TEMPERATURE OF CONCRETE SHALL NOT EXCEED 90°f AT TIME OF PLACEMENT.
- 4. CONCRETE EXPOSED TO FREEZE—THAW CYCLES SHALL CONTAIN AIR ENTRAINING ADMIXTURES. AMOUNT OF AIR ENTRAINMENT TO BE BASED ON SIZE OF AGGREGATE AND F3 CLASS EXPOSURE (VERY SEVERE). CEMENT USED TO BE TYPE II PORTLAND CEMENT WITH A MAXIMUM WATER—TO—CEMENT RATIO (W/C) OF 0.45.
- 5. ALL STEEL REINFORCING SHALL CONFORM TO ASTM A615. ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185. ALL SPLICES SHALL BE CLASS "B" TENSION SPLICES, UNLESS NOTED OTHERWISE. ALL HOOKS SHALL BE STANDARD 90 DEGREE HOOKS, UNLESS NOTED OTHERWISE. YIELD STRENGTH (Fy) OF STANDARD DEFORMED BARS ARE AS FOLLOWS:

#4 BARS AND SMALLER 40 ksi

#5 BARS AND LARGER 60 ksi

- 6. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:
- CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3"
- CONCRETE EXPOSED TO EARTH OR WEATHER:
- #6 BARS AND LARGER 2"
- #5 BARS AND SMALLER 1-1/2"
- CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
- SLAB AND WALLS 3/4"
- BEAMS AND COLUMNS 1-1/2"
- 7. A TOOLED EDGE OR A 3/4" CHAMFER SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNLESS NOTED OTHERWISE, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.

#### **ELECTRICAL INSTALLATION NOTES:**

- 1. ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES/ORDINANCES.
- 2. CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED AND TRIP HAZARDS ARE ELIMINATED.
- 3. WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC.
- 4. ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC.
- 4.1. ALL EQUIPMENT SHALL BEAR THE UNDERWRITERS LABORATORIES LABEL OF APPROVAL, AND SHALL CONFORM TO REQUIREMENT OF THE NATIONAL ELECTRICAL CODE.
- 4.2. ALL OVERCURRENT DEVICES SHALL HAVE AN INTERRUPTING CURRENT RATING THAT SHALL BE GREATER THAN THE SHORT CIRCUIT CURRENT TO WHICH THEY ARE SUBJECTED, 22,000 AIC MINIMUM. VERIFY AVAILABLE SHORT CIRCUIT CURRENT DOES NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRENT ADOPTED CODE PRE THE GOVERNING JURISDICTION.
- 5. EACH END OF EVERY POWER PHASE CONDUCTOR, GROUNDING CONDUCTOR, AND TELCO CONDUCTOR OR CABLE SHALL BE LABELED WITH COLOR—CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2" PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA.
- 6. ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH LAMICOID TAGS SHOWING THEIR RATED VOLTAGE, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING AND BRANCH CIRCUIT ID NUMBERS (i.e. PANEL BOARD AND CIRCUIT ID'S).
- 7. PANEL BOARDS (ID NUMBERS) SHALL BE CLEARLY LABELED WITH PLASTIC LABELS.
- 8. TIE WRAPS ARE NOT ALLOWED.
- 9. ALL POWER AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE COPPER CONDUCTOR (#14 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 10. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE COPPER CONDUCTOR (#6 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 11. POWER AND CONTROL WIRING IN FLEXIBLE CORD SHALL BE MULTI-CONDUCTOR, TYPE SOOW CORD (#14 OR LARGER) UNLESS OTHERWISE SPECIFIED.
- 12. POWER AND CONTROL WIRING FOR USE IN CABLE TRAY SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#14 OR LARGER), WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 13. ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP—STYLE, COMPRESSION WIRE LUGS AND WIRE NUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRE NUTS SHALL BE RATED FOR OPERATION NOT LESS THAN 75° C (90° C IF AVAILABLE).
- 14. RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.
- 15. ELECTRICAL METALLIC TUBING (EMT), INTERMEDIATE METAL CONDUIT (IMC), OR RIGID METAL CONDUIT (RMC) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.

- ELECTRICAL METALLIC TUBING (EMT) OR METAL—CLAD CABLE (MC) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.
- 17. SCHEDULE 40 PVC UNDERGROUND ON STRAIGHTS AND SCHEDULE 80 PVC FOR ALL ELBOWS/90s AND ALL APPROVED ABOVE GRADE PVC CONDUIT.
- 18. LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.
- 19. CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION—TYPE AND APPROVED FOR THE LOCATION USED. SET SCREW FITTINGS ARE NOT ACCEPTABLE.
- 20. CABINETS, BOXES AND WIRE WAYS SHALL BE LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND THE NEC.
- 21. WIREWAYS SHALL BE METAL WITH AN ENAMEL FINISH AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARDS (WIREMOLD SPECMATE WIREWAY).
- 22. SLOTTED WIRING DUCT SHALL BE PVC AND INCLUDE COVER (PANDUIT TYPE E OR EQUAL).
- 23. CONDUITS SHALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-PERFORATED STRAPS AND HANGERS. EXPLOSIVE DEVICES (i.e. POWDER-ACTUATED) FOR ATTACHING HANGERS TO STRUCTURE WILL NOT BE PERMITTED. CLOSELY FOLLOW THE LINES OF THE STRUCTURE, MAINTAIN CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES. CHANGES IN DIRECTION TO ROUTE AROUND OBSTACLES SHALL BE MADE WITH CONDUIT OUTLET BODIES. CONDUIT SHALL BE INSTALLED IN A NEAT AND WORKMANLIKE MANNER. PARALLEL AND PERPENDICULAR TO STRUCTURE WALL AND CEILING LINES. ALL CONDUIT SHALL BE FISHED TO CLEAR OBSTRUCTIONS. ENDS OF CONDUITS SHALL BE TEMPORARILY CAPPED FLUSH TO FINISH GRADE TO PREVENT CONCRETE, PLASTER OR DIRT FROM ENTERING. CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED MALLEABLE IRON BUSHING ON INSIDE AND GALVANIZED MALLEABLE IRON LOCKNUT ON OUTSIDE AND INSIDE.
- 24. EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES AND PULL BOXES SHALL BE GALVANIZED OR EPOXY—COATED SHEET STEEL. SHALL MEET OR EXCEED UL 50 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND NEMA 3 (OR BETTER) FOR EXTERIOR LOCATIONS.
- 25. METAL RECEPTACLE, SWITCH AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY—COATED OR NON—CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- 26. NONMETALLIC RECEPTACLE, SWITCH AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2 (NEWEST REVISION) AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- 27. THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CARRIER AND/OR DISH Wireless L.L.C. AND TOWER OWNER BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.
- 28. THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD LIFE AND PROPERTY.
- 29. INSTALL LAMICOID LABEL ON THE METER CENTER TO SHOW "DISH Wireless L.L.C.".
- 30. ALL EMPTY/SPARE CONDUITS THAT ARE INSTALLED ARE TO HAVE A METERED MULE TAPE PULL CORD INSTALLED.



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HL	AL	SS
RFDS REV	#:0	1/6/2022

# CONSTRUCTION DOCUMENTS

SUBMITTALS							
REV	DATE	DESCRIPTION					
A	03/02/2022	ISSUED FOR REVIEW					
В	03/07/2022	ISSUED FOR REVIEW					
0	03/29/2022	ISSUED FOR CONSTRUCTION					
	A&E F	PROJECT NUMBER					
	110	7 50001 6					

1197-F0001-C

DISH Wireless L.L.C.

PROJECT INFORMATION

BOBDL00110C 33 MITCHELL DRIVE MANCHESTER, CT 06042

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

#### **GROUNDING NOTES:**

- 1. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION AND AC POWER GES'S) SHALL BE BONDED TOGETHER AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
- 2. THE CONTRACTOR SHALL PERFORM IEEE FALL—OF—POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS, THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT AND PROVIDE TESTING RESULTS.
- 4. METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- 5. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
- 6. EACH CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 STRANDED COPPER OR LARGER FOR INDOOR BTS; #2 BARE SOLID TINNED COPPER FOR OUTDOOR BTS.
- 7. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED BACK TO BACK CONNECTIONS ON OPPOSITE SIDE OF THE GROUND BUS ARE PERMITTED.
- 8. ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING SHALL BE #2 SOLID TINNED COPPER UNLESS OTHERWISE INDICATED.
- 9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
- 10. USE OF 90° BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45° BENDS CAN BE ADEQUATELY SUPPORTED.
- 11. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
- 12. ALL GROUND CONNECTIONS ABOVE GRADE (INTERIOR AND EXTERIOR) SHALL BE FORMED USING HIGH PRESS CRIMPS.
- 13. COMPRESSION GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.
- 14. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR.
- 15. APPROVED ANTIOXIDANT COATINGS (i.e. CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- 16. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.
- 17. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
- 18. BOND ALL METALLIC OBJECTS WITHIN 6 ft OF MAIN GROUND RING WITH (1) #2 BARE SOLID TINNED COPPER GROUND CONDUCTOR.
- 19. GROUND CONDUCTORS USED FOR THE FACILITY GROUNDING AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDUITIONS, NON-METALLIC MATERIAL SUCH AS PVC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (i.e., NONMETALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT
- 20. ALL GROUNDS THAT TRANSITION FROM BELOW GRADE TO ABOVE GRADE MUST BE #2 BARE SOLID TINNED COPPER IN 3/4" NON-METALLIC, FLEXIBLE CONDUIT FROM 24" BELOW GRADE TO WITHIN 3" TO 6" OF CAD-WELD TERMINATION POINT. THE EXPOSED END OF THE CONDUIT MUST BE SEALED WITH SILICONE CAULK. (ADD TRANSITIONING GROUND STANDARD DETAIL AS WELL).
- 21. BUILDINGS WHERE THE MAIN GROUNDING CONDUCTORS ARE REQUIRED TO BE ROUTED TO GRADE, THE CONTRACTOR SHALL ROUTE TWO GROUNDING CONDUCTORS FROM THE ROOFTOP, TOWERS, AND WATER TOWERS GROUNDING RING, TO THE EXISTING GROUNDING SYSTEM, THE GROUNDING CONDUCTORS SHALL NOT BE SMALLER THAN 2/O COPPER. ROOFTOP GROUNDING RING SHALL BE BONDED TO THE EXISTING GROUNDING SYSTEM, THE BUILDING STEEL COLUMNS, LIGHTNING PROTECTION SYSTEM, AND BUILDING MAIN WATER LINE (FERROUS OR NONFERROUS METAL PIPING ONLY). DO NOT ATTACH GROUNDING TO FIRE SPRINKLER SYSTEM PIPES.



5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120



# INFINIGY &

the solutions are endless
2500 W. HIGGINS RD. SUITE 500 |
HOFFMAN ESTATES, IL 60169



IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

DRAWN BY:	CHECKED BY:	APPROVED BY:
HL	AL	SS
RFDS REV ;	1/6/2022	

# CONSTRUCTION DOCUMENTS

	SUBMITTALS								
REV	DATE	DESCRIPTION							
A	03/02/2022	ISSUED FOR REVIEW							
В	03/07/2022	ISSUED FOR REVIEW							
0	03/29/2022	ISSUED FOR CONSTRUCTION							
	A&E PROJECT NUMBER								

1197-F0001-C

DISH Wireless L.L.C. PROJECT INFORMATION

BOBDL00110C 33 MITCHELL DRIVE MANCHESTER, CT 06042

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

## Exhibit D

**Structural Analysis Report** 

# INFINIGY &

#### TOWER STRUCTURAL ANALYSIS REPORT

March 19, 2022

Dish Site Name	BOBDL00110C
Dish Site Number	BOBDL00110C
Infinigy Job Number	1197-F0001-B
Client	Northeast Site Solutions
Carrier	Dish Wireless
	33 Mitchell Drive
	Manchester, CT 06042
Site Location	Hartford County
	41° 47′ 50.3988″ N NAD83
	72° 30′ 42.4016″ W NAD83
Structure Type	Self-Support Tower
Structure Height	170.0 ft
Structural Usage Ratio	87.5%
Overall Result	Pass

The enclosed structural analysis has been performed in accordance with the 2018 Connecticut State Building Code based on an ultimate 3-second gust wind speed of 125 mph. The evaluation criteria and applicable standards are presented in the next section of this report.



#### Tower Structural Analysis Report

#### March 19, 2022

#### CONTENTS

- 1. Introduction
- 2. Design/Analysis Parameters
- 3. Proposed Loading Configuration
- 4. Other Considered Loading
- 5. Supporting Documentation
- 6. Results
- 7. Recommendations
- 8. Assumptions
- 9. Liability Waiver and Limitations
- 10. Calculations

#### March 19, 2022

#### 1. INTRODUCTION

Infinigy performed a structural analysis on the existing Self-Support Tower. All referenced supporting documents have been obtained from the client and are assumed to be accurate and applicable to this site. The structure was analyzed using tnxTower version 8.1.1.0 analysis software.

#### 2. DESIGN/ANALYSIS PARAMETERS

Ultimate Wind Speed	125 mph (3-Second Gust)
Nominal Wind Speed	97 mph (3-Second Gust)
Wind Speed w/ ice	50 mph (3-Second Gust) w/ 1 ice / No Ice Loading Considered
Adopted Code	2018 Connecticut State Building Code
Standard(s)	TIA-222-G
Risk Category	
Exposure Category	C
Topographic Factor	1.0
Seismic Site Class	D
Seismic Spectral Response	$S_s = 0.187 \text{ g/} S_{1} = 0.064 \text{ g}$
Live Load Wind Speed	60 mph

#### 3. PROPOSED LOADING CONFIGURATION

Mount Center (ft)	RAD Center (ft)	Qty.	Appurtenance	Mount Type	Coax & Lines	Carrier
		3	JMA Wireless MX08FRO665-21	(2)		
170.0	170.0	3	FUJITSU TA08025-B605	(3) Commscope	(1) Hybrid	Dish
170.0	170.0	3	FUJITSU TA08025-B604	MTC3975083	(1) Hybrid	DISII
		1	Raycap RDIDC-9181-PF-48			

#### 4. OTHER CONSIDERED LOADING

Mount Center (ft)	RAD Center (ft)	Qty.	Appurtenance	Mount Type	Coax & Lines	Carrier
165.0	165.0	3	6' Dish	Leg Mounted	(3) 1-5/8	-
160.0	160.0	1	Shively Labs 6812	Leg Mounted	(1) 1-5/8	1
155.0	155.0	1	Shively Labs 6812	Leg Mounted	(1) 1-5/8	-
150.0	150.0	1	2' Dish	Leg	(3) 1-5/8	
130.0	130.0	2	4' Dish	Mounted	(3) 1-3/0	_
		1	2' MW Dish	(1) Custom	(8) 1-5/8" (8) 6X12	TMO
	_	4	AIR21 KRC118023-1_B2A_B4P			
		4	AIR32 KRD901146-1_B66A_B2A			
140.0	140.0	4	APXVAARR24_43-U-NA20			
140.0	140.0	4	Ericsson AIR6449 B41	Square Mount		
		4	Ericsson Radio 4449 B71+B85	Mount HCS	псэ	
		4	Ericsson Radio 4415 B25			
		4	Generic Twin Style 1B – AWS			
110.0	110.0	1	2' Dish	Leg Mounted	-	-

#### **Tower Structural Analysis Report**

March 19, 2022

			Maion 17, 2022			
Mount Center (ft)	RAD Center (ft)	Qty.	Appurtenance	Mount Type	Coax & Lines	Carrier
		12	Andrew SBNHH-1D65B		(2) 1-5/8 Verize	
	100.0	3	Alcatel Lucent RRH2x60-07-U	(3) Sector		Verizon
100.0		3	Alcatel Lucent RRH2x60-PCS			
100.0		Alcatel Lucent RRH4x45/2x90-	Mounts	(2) 1-3/6	VEHZOH	
		3	AWS			
		2	Commscope DB-T1-6Z-8AB-0Z			

#### 5. SUPPORTING DOCUMENTATION

Construction Drawings	Infinigy dated March 7, 2022
Structural Analysis Report	EFI Global, dated June 15, 2020

#### 6. RESULTS

Structural Components	Capacity	Pass/Fail
Legs	58.9%	Pass
Diagonals	87.5%	Pass
Secondary Horizontals	5.7%	Pass
Top Girts	6.2%	Pass
Flange Bolts	65.6%	Pass
Anchor Bolts	47.9%	Pass
Soil Interaction	53.8%	Pass
Structural Foundation	44.1%	Pass
RATING =	87.5%	Pass

#### 7. RECOMMENDATIONS

Infinigy recommends installing Dish's proposed equipment loading configuration on the mount at 170 ft on this structure. The installation shall be performed in accordance with the construction documents issued for this site.

If you have any questions, require additional information, or believe the actual conditions differ from those detailed in this report, please contact us immediately.

Luis Mendoza
Director of Structural Engineering | INFINIGY

March 19, 2022

#### 8. ASSUMPTIONS

The structure, its foundation system and related structures were built and maintained in accordance with the manufacturer's specifications and instructions.

The structure condition is essentially as erected and does not have corrosion, damages or defects that would affect its structural integrity. The structure is plumb and all members and their connections are sound and can fully develop their structural capacities.

The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in the loading configuration tables.

Some of the antennas and mounts used in the structure model are similar in size and weight to the actual appurtenances mounted on the structure.

Steel grades have been assumed as follows, unless noted otherwise:

Channel, Solid Round, Angle, Plate ASTM A36

HSS (Rectangular)
HSS (Circular)
ASTM A500-B GR 46
ASTM A500-B GR 42
Pipe
ASTM A53-B GR 35

Connection Bolts ASTM A325 U-Bolts ASTM A307

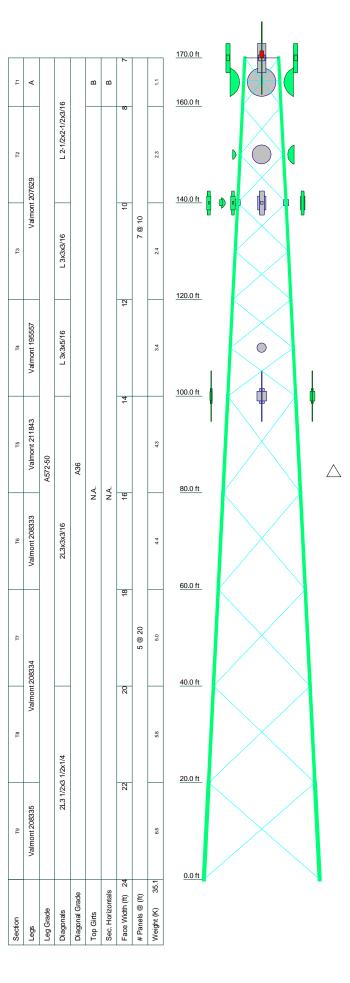
All bolted connections are pretensioned in accordance with Table 8.2 of the RCSC 2014 Standard.

#### 9. LIABILITY WAIVER AND LIMITATIONS

Our structural calculations are completed assuming all information provided to Infinigy is accurate and applicable to this site. For the purposes of calculations, we assume an overall structure condition as erected and all members and connections to be free of corrosion and/or structural defects. The structure owner and/or contractor shall verify the structure's condition prior to installation of any proposed equipment. If actual conditions differ from those described in this report, Infinigy Engineering should be notified immediately to assess the impact on the results of this report.

Our evaluation is completed using industry standard methods and procedures. The structural results, conclusions and recommendations contained in this report are proprietary and should not be used by others as their own. Infinigy is not responsible for decisions made by others that are or are not based on the stated assumptions and conclusions in this report.

This report is an evaluation of the tower structure only and does not reflect adequacy of any existing antenna mounts, mount connections, or cable mounting attachments. The analysis of these elements is outside the scope of this analysis and are assumed to be adequate for the purposes of this report and are assumed to have been installed per their manufacturer requirements. This document is not for construction purposes.



#### **DESIGNED APPURTENANCE LOADING**

	DESIGNED AFFOR I ENANCE LOADING								
TYPE	ELEVATION	TYPE	ELEVATION						
15' lighting rod	170	(2) RADIO 4415 B25 (TMO)	140						
Beacon	170	KRY 112 144/1 (TMO)	140						
MX08FRO665-21 w/ Mount Pipe (Dish)	170	KRY 112 144/1 (TMO)	140						
TA08025-B604 (Dish)	170	(2) KRY 112 144/1 (TMO)	140						
TA08025-B605 (Dish)	170	custom 4 sided sector mount	140						
MX08FRO665-21 w/ Mount Pipe (Dish)	170	SC2-W100AB	140						
TA08025-B604 (Dish)	170	AIR 21 B2A/B4P w/ Mount Pipe (TMO)	140						
TA08025-B605 (Dish)	170	(2) AIR 21 B2A/B4P w/ Mount Pipe	140						
MX08FRO665-21 w/ Mount Pipe (Dish)	170	(TMO)							
TA08025-B604 (Dish)	170	AIR 21 B2A/B4P w/ Mount Pipe (TMO)	140						
TA08025-B605 (Dish)	170	AIR 32 B2A B66AA w/ Mount Pipe	140						
RDIDC-9181-PF-48 (Dish)	170	(TMO)							
(3) Commscope MTC3975083 (Dish)	170	AIR 32 B2A B66AA w/ Mount Pipe (TMO)	140						
6 FT DISH	165	(2) AIR 32 B2A B66AA w/ Mount Pipe	140						
6 FT DISH	165	(TMO)	140						
6 FT DISH	165	2' Dish	110						
6812	160	RRH2X60-PCS (Verizon)	100						
6812	155	RRH4x45/2x90-AWS (Verizon)	100						
4 FT DISH	150	RRH4x45/2x90-AWS (Verizon)	100						
4 FT DISH	150	RRH4x45/2x90-AWS (Verizon)	100						
2' Dish	150	DB-T1-6Z-8AB-0Z (Verizon)	100						
(2) APXVAARR24_43-U-NA20 w/	140	DB-T1-6Z-8AB-0Z (Verizon)	100						
Mount Pipe (TMO)		10'6"x2-3/8" Pipe Mount (Verizon)	100						
APXVAARR24_43-U-NA20 w/ Mount	140	10'6"x2-3/8" Pipe Mount (Verizon)	100						
Pipe (TMO)		10'6"x2-3/8" Pipe Mount (Verizon)	100						
APXVAARR24_43-U-NA20 w/ Mount Pipe (TMO)	140	Sector Mount [SM 502-3] (Verizon)	100						
AIR 6449 B41 (MASSIVE MIMO) w/ MP (TMO)	140	(4) SBNHH-1D65B w/ Mount Pipe (Verizon)	100						
AIR 6449 B41 (MASSIVE MIMO) w/ MP (TMO)	140	(4) SBNHH-1D65B w/ Mount Pipe (Verizon)	100						
(2) AIR 6449 B41 (MASSIVE MIMO) w/ MP (TMO)	140	(4) SBNHH-1D65B w/ Mount Pipe (Verizon)	100						
(2) Radio 4449 B71+B85 (TMO)	140	RRH2x60-700 (Verizon)	100						
Radio 4449 B71+B85 (TMO)	140	RRH2x60-700 (Verizon)	100						
Radio 4449 B71+B85 (TMO)	140	RRH2x60-700 (Verizon)	100						
RADIO 4415 B25 (TMO)	140	RRH2X60-PCS (Verizon)	100						
RADIO 4415 B25 (TMO)	140	RRH2X60-PCS (Verizon)	100						

#### SYMBOL LIST

MARK	SIZE	MARK	SIZE
Α	Valmont 207628	В	L 2-1/2x2-1/2x3/16

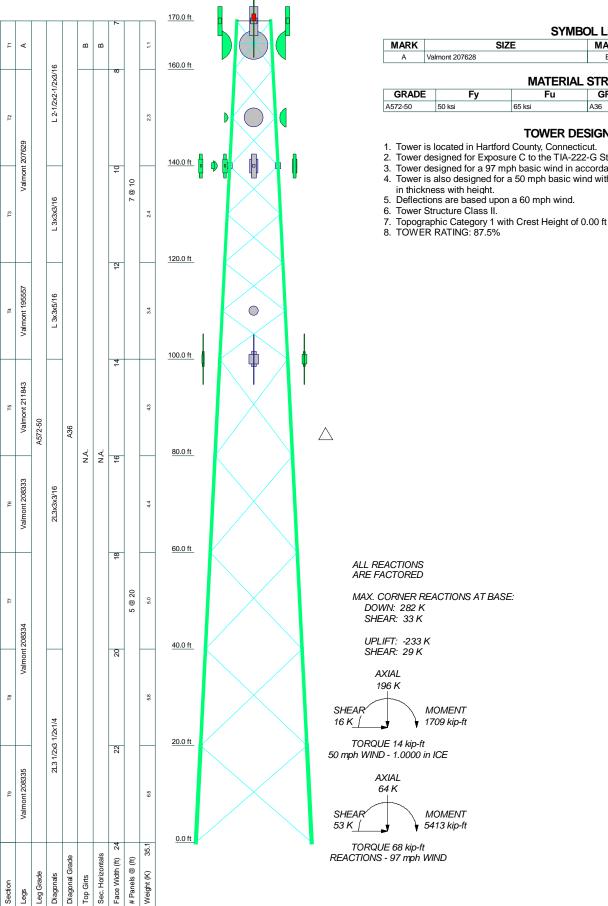
#### **MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

#### **TOWER DESIGN NOTES**

- Tower is located in Hartford County, Connecticut.
   Tower designed for Exposure C to the TIA-222-G Standard.
   Tower designed for a 97 mph basic wind in accordance with the TIA-222-G Standard.
- Tower is also designed for a 50 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
- 5. Deflections are based upon a 60 mph wind.
- Tower Structure Class II.
   Topographic Category 1 with Crest Height of 0.00 ft

Infinigy Engineering, LLP	<sup>Job:</sup> <b>B0BDL00110C</b>		
26455 Rancho Parkway S.	Project: <b>B0BDL00110C</b>		
Lake Forest, CA 92630	Client: Northeast Site Solutions	Drawn by: L. Mendoza	App'd:
Phone: (518) 690-0790	Code: TIA-222-G	Date: 03/23/22	Scale: NTS
FAX: (518) 690-0790	Path: C:\Users\Public\pdf\BOBDI 00110C.eri		Dwg No. E-1



	SYMBOL LIST							
MARK	•	SIZE	MARK	SIZ	E			
Α	Valmont 207628		В	L 2-1/2x2-1/2x3/16				
		MATERIAL		_				
GRADE	Fy	Fu	GRADE	Ē	Fu			

#### **TOWER DESIGN NOTES**

A36

36 ksi

58 ksi

- 2. Tower designed for Exposure C to the TIA-222-G Standard.
- Tower designed for a 97 mph basic wind in accordance with the TIA-222-G Standard.
   Tower is also designed for a 50 mph basic wind with 1.00 in ice. Ice is considered to increase

ob: B0BDL00110C Infinigy Engineering, LLP Project: BOBDL00110C 26455 Rancho Parkway S. Client: Northeast Site Solutions Drawn by: L. Mendoza App'd: Lake Forest, CA 92630 Scale: NTS Date: 03/23/22 Code: TIA-222-G Phone: (518) 690-0790 Dwg No. E-1 Path:
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26455 Rancho Parkway S. Lake Forest, CA 92630 Phone: (518) 690-0790 FAX: (518) 690-0790

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Client	Northeast Site Solutions	Designed by L. Mendoza

### **Tower Input Data**

The main tower is a 3x free standing tower with an overall height of 170.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 7.00 ft at the top and 24.00 ft at the base.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Hartford County, Connecticut.

Basic wind speed of 97 mph.

Structure Class II.

Exposure Category C.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

### **Options**

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

- √ Use Code Stress Ratios
- √ Use Code Safety Factors Guys Escalate Ice

Always Use Max Kz Use Special Wind Profile

- √ Include Bolts In Member Capacity Leg Bolts Are At Top Of Section
- √ Secondary Horizontal Braces Leg
  Use Diamond Inner Bracing (4 Sided)
- √ SR Members Have Cut Ends SR Members Are Concentric

Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area
- √ Use Clear Spans For KL/r
- √ Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
- ✓ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt.
- ✓ Autocalc Torque Arm Areas
   Add IBC .6D+W Combination
- √ Sort Capacity Reports By Component
- √ Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs

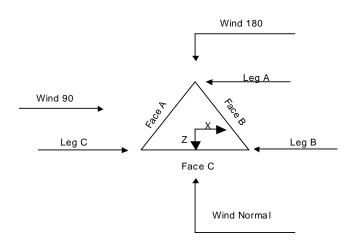
Use ASCE 10 X-Brace Ly Rules

- √ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA
- √ SR Leg Bolts Resist Compression
  All Leg Panels Have Same Allowable
  Offset Girt At Foundation
- √ Consider Feed Line Torque
- √ Include Angle Block Shear Check Use TIA-222-G Bracing Resist. Exemption Use TIA-222-G Tension Splice Exemption Poles

Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known

# Infinigy Engineering, LLP 26455 Rancho Parkway S.

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Triangular Tower

	Tower Section Geometry						
Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length	
	ft			ft	sections	ft	
T1	170.00-160.00			7.00	1	10.00	
T2	160.00-140.00			8.00	1	20.00	
T3	140.00-120.00			10.00	1	20.00	
T4	120.00-100.00			12.00	1	20.00	
T5	100.00-80.00			14.00	1	20.00	
Т6	80.00-60.00			16.00	1	20.00	
T7	60.00-40.00			18.00	1	20.00	
Т8	40.00-20.00			20.00	1	20.00	
Т9	20.00-0.00			22.00	1	20.00	

	Tower Section Geometry (cont'd)						
Tower	Tower	Diagonal	Bracing	Has	Has	Top Girt	Bottom Girt
Section	Elevation	Spacing	Type	K Brace End	Horizontals	Offset	Offset
	ft	ft		Panels		in	in
T1	170.00-160.00	10.00	X Brace	No	Yes	0.0000	0.0000
T2	160.00-140.00	10.00	X Brace	No	No	0.0000	0.0000
T3	140.00-120.00	10.00	X Brace	No	No	0.0000	0.0000
T4	120.00-100.00	10.00	X Brace	No	No	0.0000	0.0000
T5	100.00-80.00	20.00	X Brace	No	No	0.0000	0.0000
T6	80.00-60.00	20.00	X Brace	No	No	0.0000	0.0000

### Infinity Engineering, LLP

26455 Rancho Parkway S. Lake Forest, CA 92630 Phone: (518) 690-0790 FAX: (518) 690-0790

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Client Northeast Site Solutions	Designed by L. Mendoza

Tower	Tower	Diagonal	Bracing	Has	Has	Top Girt	Bottom Girt
Section	Elevation	Spacing	Type	K Brace	Horizontals	Offset	Offset
				End			
	ft	ft		Panels		in	in
T7	60.00-40.00	20.00	X Brace	No	No	0.0000	0.0000
T8	40.00-20.00	20.00	X Brace	No	No	0.0000	0.0000
T9	20.00-0.00	20.00	X Brace	No	No	0.0000	0.0000

## **Tower Section Geometry** (cont'd)

Tower	Leg	Leg	Leg	Diagonal	Diagonal	Diagonal
Elevation	Type	Size	Grade	Type	Size	Grade
ft						
T1 170.00-160.00	Truss Leg	Valmont 207628	A572-50	Single Angle	L 2-1/2x2-1/2x3/16	A36
			(50 ksi)			(36 ksi)
T2 160.00-140.00	Truss Leg	Valmont 207629	A572-50	Single Angle	L 2-1/2x2-1/2x3/16	A36
			(50 ksi)			(36 ksi)
T3 140.00-120.00	Truss Leg	Valmont 207629	A572-50	Single Angle	L 3x3x3/16	A36
	_		(50 ksi)			(36 ksi)
T4 120.00-100.00	Truss Leg	Valmont 195557	A572-50	Single Angle	L 3x3x5/16	A36
			(50 ksi)			(36 ksi)
T5 100.00-80.00	Truss Leg	Valmont 211843	A572-50	Double Angle	2L3x3x3/16	A36
			(50 ksi)			(36 ksi)
T6 80.00-60.00	Truss Leg	Valmont 208333	A572-50	Double Angle	2L3x3x3/16	A36
			(50 ksi)			(36 ksi)
T7 60.00-40.00	Truss Leg	Valmont 208334	A572-50	Double Angle	2L3x3x3/16	A36
			(50 ksi)			(36 ksi)
T8 40.00-20.00	Truss Leg	Valmont 208334	A572-50	Double Angle	2L3 1/2x3 1/2x1/4	A36
			(50 ksi)			(36 ksi)
T9 20.00-0.00	Truss Leg	Valmont 208335	A572-50	Double Angle	2L3 1/2x3 1/2x1/4	A36
			(50 ksi)			(36 ksi)

## **Tower Section Geometry** (cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 170.00-160.00	Single Angle	L 2-1/2x2-1/2x3/16	A36	Flat Bar		A36
			(36 ksi)			(36 ksi)

### **Tower Section Geometry** (cont'd)

Tower Elevation	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
ft						
T1 170.00-160.0	0 Single Angle	L 2-1/2x2-1/2x3/16	A36	Solid Round		A572-50
			(36 ksi)			(50 ksi)

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Client Northeast Site Solutions	Designed by L. Mendoza

# **Tower Section Geometry** (cont'd)

Tower	Gusset	Gusset	Gusset Grade	Adjust. Factor	Adjust.	Weight Mult.	Double Angle	0	0
Elevation	Area	Thickness		$A_f$	Factor		Stitch Bolt	Stitch Bolt	Stitch Bolt
	(per face)				$A_r$		Spacing	Spacing	Spacing
							Diagonals	Horizontals	Redundants
ft	ft <sup>2</sup>	in					in	in	in
T1	0.00	0.0000	A36	1.03	1	1.05	30.0000	30.0000	36.0000
170.00-160.00			(36 ksi)						
T2	0.00	0.0000	A36	1.03	1	1.05	36.0000	36.0000	36.0000
160.00-140.00			(36 ksi)						
T3	0.00	0.0000	A36	1.03	1	1.05	36.0000	0.0000	36.0000
140.00-120.00			(36 ksi)						
T4	0.00	0.0000	A36	1.03	1	1.05	36.0000	0.0000	36.0000
120.00-100.00			(36 ksi)						
T5	0.00	0.0000	A36	1.03	1	1.05	36.0000	0.0000	36.0000
100.00-80.00			(36 ksi)						
T6 80.00-60.00	0.00	0.0000	A36	1.03	1	1.05	36.0000	0.0000	36.0000
			(36 ksi)						
T7 60.00-40.00	0.00	0.0000	A36	1.03	1	1.05	36.0000	0.0000	36.0000
			(36 ksi)						
T8 40.00-20.00	0.00	0.0000	A36	1.03	1	1.05	36.0000	0.0000	36.0000
			(36 ksi)						
T9 20.00-0.00	0.00	0.0000	A36	1.03	1	1.05	36.0000	0.0000	36.0000
			(36 ksi)						

# Tower Section Geometry (cont'd)

						K Fac	ctors1			
Tower Elevation	Calc K Single	Calc K Solid	Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace
	Angles	Rounds		X	X	X	X	X	X	X
ft				Y	Y	Y	Y	Y	Y	Y
T1	Yes	Yes	1	1	1	1	1	1	1	1
170.00-160.00				1	1	1	1	1	0.5	1
T2	Yes	Yes	1	1	1	1	1	1	1	1
160.00-140.00				1	1	1	1	1	1	1
T3	Yes	Yes	1	1	1	1	1	1	1	1
40.00-120.00				1	1	1	1	1	1	1
T4	Yes	Yes	1	1	1	1	1	1	1	1
120.00-100.00				1	1	1	1	1	1	1
T5	Yes	Yes	1	1	1	1	1	1	1	1
100.00-80.00				1	1	1	1	1	1	1
T6	Yes	Yes	1	1	1	1	1	1	1	1
80.00-60.00				1	1	1	1	1	1	1
T7	Yes	Yes	1	1	1	1	1	1	1	1
60.00-40.00				1	1	1	1	1	1	1
T8	Yes	Yes	1	1	1	1	1	1	1	1
40.00-20.00				1	1	1	1	1	1	1
Т9 20.00-0.00	Yes	Yes	1	1	1	1	1	1	1	1
				1	1	1	1	1	1	1

<sup>&</sup>lt;sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

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## **Tower Section Geometry** (cont'd)

			Truss-Leg	K Factors				
	Trus	s-Legs Used As Leg Me	mbers	Truss	-Legs Used As Inner Me	iner Members		
Tower	Leg	X	Z	Leg	X	Z		
Elevation	Panels	Brace	Brace	Panels	Brace	Brace		
ft		Diagonals	Diagonals		Diagonals	Diagonals		
T1	1	0.5	0.85	1	0.5	0.85		
170.00-160.00								
T2	1	0.5	0.85	1	0.5	0.85		
160.00-140.00								
T3	1	0.5	0.85	1	0.5	0.85		
140.00-120.00								
T4	1	0.5	0.85	1	0.5	0.85		
120.00-100.00								
T5	1	0.5	0.85	1	0.5	0.85		
100.00-80.00								
T6	1	0.5	0.85	1	0.5	0.85		
80.00-60.00								
T7	1	0.5	0.85	1	0.5	0.85		
60.00-40.00								
T8	1	0.5	0.85	1	0.5	0.85		
40.00-20.00								
Т9 20.00-0.00	1	0.5	0.85	1	0.5	0.85		

## **Tower Section Geometry** (cont'd)

Tower Elevation ft	Leg		Diagon	al	Top Gi	rt	Bottom	Bottom Girt		Mid Girt		rizontal	Short Ho	rizontal
	Net Width	U	Net Width	U	Net Width	U	Net	U	Net	U	Net	U	Net	U
	Deduct		Deduct		Deduct		Width		Width		Width		Width	
	in		in		in		Deduct		Deduct		Deduct		Deduct	
							in		in		in		in	
T1	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	1
170.00-160.00														
T2	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	1
160.00-140.00														
T3	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	1
140.00-120.00														
T4	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75
120.00-100.00														
T5	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75
100.00-80.00														
T6 80.00-60.00	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 60.00-40.00		1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 40.00-20.00		1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75
T9 20.00-0.00	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75

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Tower Elevation	Reduna Horizo		Redund Diago		Redundant Sub-Diagonal		Redun Sub-Hor		Redundant Vertical		Redundant Hip		Redundant Hip Diagonal	
ft														
	Net Width	U	Net Width	U	Net Width	U	Net	U	Net	U	Net	U	Net	U
	Deduct		Deduct		Deduct		Width		Width		Width		Width	
	in		in		in		Deduct		Deduct		Deduct		Deduct	
							in		in		in		in	
T1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
170.00-160.00														
T2	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
160.00-140.00														
T3	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
140.00-120.00														
T4	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
120.00-100.00														
T5	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
100.00-80.00														
T6 80.00-60.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 60.00-40.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 40.00-20.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T9 20.00-0.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

## **Tower Section Geometry** (cont'd)

Tower	Leg	Leg		Diagoi	ıal	Top G	irt	Bottom	Girt	Mid Girt		Long Horizontal		Short Horizontal	
Elevation	Connection														
ft	Type														
		Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.						
		in		in		in		in		in		in		in	
T1	Flange	1.0000	6	1.0000	1	1.0000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0
170.00-160.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T2	Flange	1.0000	6	1.0000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
160.00-140.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T3	Flange	1.0000	6	1.0000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
140.00-120.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T4	Flange	1.0000	6	1.0000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
120.00-100.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T5	Flange	1.0000	12	0.8750	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
100.00-80.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T6 80.00-60.00	Flange	1.0000	12	0.8750	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T7 60.00-40.00	Flange	1.0000	12	0.8750	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T8 40.00-20.00	Flange	1.0000	12	0.8750	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T9 20.00-0.00	Flange	0.7500	0	0.8750	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	

# Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face	Allow	Exclude	Component	Placement	Face	Lateral	#	#	Clear	Width or	Perimeter	Weight
	or	Shield	From	Type		Offset	Offset		Per	Spacing	Diameter		
	Leg		Torque		ft	in	(Frac FW)		Row	in	in	in	plf
			Calculation										
LDF7-50A(1	В	No	No	Ar (CaAa)	160.00 -	-6.0000	0.45	3	3	1.9800	1.9800		0.82

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Description	Face or	Allow Shield	Exclude From	Component Type	Placement	Face Offset	Lateral Offset	#	# Per	Clear Spacing	Width or Diameter	Perimeter	Weight
	Leg		Torque Calculation	21	ft	in	(Frac FW)		Row	in	in	in	plf
5/8)					165.00								
LDF7-50A(1 5/8)	В	No	No	Ar (CaAa)	155.00 - 160.00	-6.0000	0.45	4	4	1.9800	1.9800		0.82
LDF7-50A(1 5/8)	В	No	No	Ar (CaAa)	150.00 - 155.00	-6.0000	0.45	5	5	1.9800	1.9800		0.82
LDF7-50A(1 5/8)	В	No	No	Ar (CaAa)	0.00 - 150.00	-6.0000	0.45	8	4	1.9800	1.9800		0.82
LDF7-50A(1 5/8)	C	No	No	Ar (CaAa)	0.00 - 100.00	-6.0000	0.47	2	2	1.9800	1.9800		0.82
LDF7-50A(1 5/8) ****	С	No	No	Ar (CaAa)	0.00 - 140.00	0.0000	-0.4	16	8	1.9800	1.9800		0.82
HYBRID( 1-1/4") (Dish)	A	No	No	Ar (CaAa)	0.00 - 170.00	-6.0000	0.45	1	1	1.2500	1.2500		1.00
T-Brackets (Af)	A	No	No	Af (CaAa)	0.00 - 170.00	-6.0000	0.45	1	1	1.0000	1.0000		8.40
T-Brackets (Af)	В	No	No	Af (CaAa)	0.00 - 170.00	-6.0000	0.45	1	1	1.0000	1.0000		8.40
T-Brackets (Af) ***	С	No	No	Af (CaAa)	0.00 - 170.00	-6.0000	0.45	1	1	1.0000	1.0000		8.40

		Fee	d Line	/Linear	Appurte	enances	- Entered As	s Area
Description	Face	Allow	Exclude	Component	Placement	Total	$C_A A_A$	Weight
•	or	Shield	From	Type	C.	Number	c2 /c	10
	Leg		Torque Calculation		ſt		ft²/ft	plf
***								

# Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	$A_R$	$A_F$	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation				In Face	Out Face	
	ft		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	$ft^2$	K
T1	170.00-160.00	A	0.000	0.000	2.917	0.000	0
		В	0.000	0.000	4.637	0.000	0
		C	0.000	0.000	1.667	0.000	0
T2	160.00-140.00	Α	0.000	0.000	5.833	0.000	0
		В	0.000	0.000	28.083	0.000	0
		C	0.000	0.000	3.333	0.000	0
T3	140.00-120.00	A	0.000	0.000	5.833	0.000	0
		В	0.000	0.000	35.013	0.000	0
		C	0.000	0.000	66.693	0.000	0
T4	120.00-100.00	A	0.000	0.000	5.833	0.000	0
		В	0.000	0.000	35.013	0.000	0
		C	0.000	0.000	66.693	0.000	0
T5	100.00-80.00	A	0.000	0.000	5.833	0.000	0
		В	0.000	0.000	35.013	0.000	0

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Tower	Tower	Face	$A_R$	$A_F$	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation				In Face	Out Face	
	ft		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	K
		C	0.000	0.000	74.613	0.000	0
T6	80.00-60.00	A	0.000	0.000	5.833	0.000	0
		В	0.000	0.000	35.013	0.000	0
		C	0.000	0.000	74.613	0.000	0
T7	60.00-40.00	A	0.000	0.000	5.833	0.000	0
		В	0.000	0.000	35.013	0.000	0
		C	0.000	0.000	74.613	0.000	0
T8	40.00-20.00	A	0.000	0.000	5.833	0.000	0
		В	0.000	0.000	35.013	0.000	0
		C	0.000	0.000	74.613	0.000	0
T9	20.00-0.00	A	0.000	0.000	5.833	0.000	0
		В	0.000	0.000	35.013	0.000	0
		C	0.000	0.000	74.613	0.000	0

## Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	$A_R$	$A_F$	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation	or	Thickness			In Face	Out Face	
	ft	Leg	in	$ft^2$	$ft^2$	ft <sup>2</sup>	ft <sup>2</sup>	K
T1	170.00-160.00	A	2.349	0.000	0.000	12.314	0.000	0
		В		0.000	0.000	16.399	0.000	0
		C		0.000	0.000	6.365	0.000	0
T2	160.00-140.00	A	2.327	0.000	0.000	24.449	0.000	1
		В		0.000	0.000	66.241	0.000	2
		C		0.000	0.000	12.641	0.000	0
T3	140.00-120.00	A	2.294	0.000	0.000	24.184	0.000	1
		В		0.000	0.000	65.610	0.000	2
		C		0.000	0.000	102.178	0.000	2 3
T4	120.00-100.00	A	2.256	0.000	0.000	23.880	0.000	1
		В		0.000	0.000	65.199	0.000	2
		C		0.000	0.000	101.789	0.000	3
T5	100.00-80.00	A	2.211	0.000	0.000	23.522	0.000	1
		В		0.000	0.000	64.714	0.000	2
		C		0.000	0.000	131.781	0.000	3
T6	80.00-60.00	A	2.156	0.000	0.000	23.083	0.000	1
		В		0.000	0.000	64.121	0.000	2
		C		0.000	0.000	130.842	0.000	3
T7	60.00-40.00	A	2.085	0.000	0.000	22.512	0.000	1
		В		0.000	0.000	63.349	0.000	2
		C		0.000	0.000	129.622	0.000	3
T8	40.00-20.00	A	1.981	0.000	0.000	21.682	0.000	1
		В		0.000	0.000	62.228	0.000	2
		C		0.000	0.000	127.848	0.000	2 3
T9	20.00-0.00	A	1.775	0.000	0.000	20.033	0.000	0
		В		0.000	0.000	60.004	0.000	1
		C		0.000	0.000	124.336	0.000	3

### **Feed Line Center of Pressure**

Section	Elevation	$CP_X$	$CP_Z$	$CP_X$	$CP_Z$
				Ice	Ice
	ft	in	in	in	in
T1	170.00-160.00	1.4462	0.3461	0.8267	-0.2162
T2	160.00-140.00	5.6662	4.0917	3.6877	1.5901

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Section	Elevation	$CP_X$	$CP_Z$	$CP_X$	$CP_Z$
				Ice	Ice
	ft	in	in	in	in
T3	140.00-120.00	20.0850	8.3830	14.3642	4.4769
T4	120.00-100.00	22.4445	9.1445	17.1876	5.2683
T5	100.00-80.00	23.1312	11.6800	14.4020	7.8114
T6	80.00-60.00	25.6543	12.8354	16.0337	8.6086
T7	60.00-40.00	27.7045	13.7061	17.5724	9.3375
T8	40.00-20.00	28.8324	14.1449	18.8360	9.9099
T9	20.00-0.00	30.4824	14.8204	20.4499	10.5947

# **Shielding Factor Ka**

Tower	Feed Line	Description	Feed Line	$K_a$	$K_a$
Section	Record No.		Segment Elev.	No Ice	Ice
T1	2	LDF7-50A(1 5/8)	160.00 -	0.6000	0.3461
			165.00		
T1	9	HYBRID( 1-1/4")	160.00 -	0.6000	0.3461
			170.00		
T1	10	T-Brackets (Af)	160.00 -	0.6000	0.3461
T.1		T.D. 1 . (4.6)	170.00	0.6000	0.0461
T1	11	T-Brackets (Af)	160.00 - 170.00	0.6000	0.3461
T1	12	T-Brackets (Af)	160.00 -	0.6000	0.3461
11	12	I-Blackets (Al)	170.00	0.0000	0.3401
T2	3	LDF7-50A(1 5/8)	155.00 -	0.6000	0.4970
12	3	EDI / 3011(1 3/0)	160.00	0.0000	0.4770
T2	4	LDF7-50A(1 5/8)		0.6000	0.4970
	-		155.00		*****
T2	5	LDF7-50A(1 5/8)	140.00 -	0.6000	0.4970
		` '	150.00		
T2	9	HYBRID( 1-1/4")	140.00 -	0.6000	0.4970
			160.00		
T2	10	T-Brackets (Af)	140.00 -	0.6000	0.4970
			160.00		
T2	11	T-Brackets (Af)	140.00 -	0.6000	0.4970
			160.00		
T2	12	T-Brackets (Af)	140.00 -	0.6000	0.4970
TO .	_	I DET 50 4 (1.5/0)	160.00	0.6000	0.5550
Т3	5	LDF7-50A(1 5/8)	120.00 -	0.6000	0.5553
Т3	7	LDF7-50A(1 5/8)	140.00	0.6000	0.5553
13	,	LDF7-30A(1 3/8)	120.00 - 140.00	0.6000	0.5555
Т3	9	HYBRID( 1-1/4")	120.00 -	0.6000	0.5553
13		mibidb(1 i, i )	140.00	0.0000	0.5555
Т3	10	T-Brackets (Af)	120.00 -	0.6000	0.5553
		()	140.00		
Т3	11	T-Brackets (Af)	120.00 -	0.6000	0.5553
		. ,	140.00		
T3	12	T-Brackets (Af)	120.00 -	0.6000	0.5553
			140.00		
T4	5	LDF7-50A(1 5/8)	100.00 -	0.6000	0.6000
			120.00		
T4	7	LDF7-50A(1 5/8)	100.00 -	0.6000	0.6000
		III/DDID(1.1/III)	120.00	0.6000	0.6000
T4	9	HYBRID( 1-1/4")	100.00 -	0.6000	0.6000
T4	10	T Products (Af)	120.00 100.00 -	0.6000	0.6000
14	10	T-Brackets (Af)	100.00 -	0.0000	0.0000

### Infinigy Engineering, LLP 26455 Rancho Parkway S.

26455 Rancho Parkway S. Lake Forest, CA 92630 Phone: (518) 690-0790 FAX: (518) 690-0790

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Client	Northeast Site Solutions	Designed by L. Mendoza

Tower	Feed Line	Description	Feed Line	$K_a$	$K_a$
Section	Record No.		Segment Elev.	No Ice	Ice
			120.00		
T4	11	T-Brackets (Af)	100.00 -	0.6000	0.6000
			120.00		
T4	12	T-Brackets (Af)	100.00 -	0.6000	0.6000
			120.00		
T5	5	LDF7-50A(1 5/8)	80.00 - 100.00	0.6000	0.6000
T5	6	LDF7-50A(1 5/8)	80.00 - 100.00	0.6000	0.6000
T5	7	LDF7-50A(1 5/8)	80.00 - 100.00	0.6000	0.6000
T5	9	HYBRID( 1-1/4")	80.00 - 100.00	0.6000	0.6000
T5	10	T-Brackets (Af)	80.00 - 100.00	0.6000	0.6000
T5	11	T-Brackets (Af)	80.00 - 100.00	0.6000	0.6000
T5	12	T-Brackets (Af)	80.00 - 100.00	0.6000	0.6000
T6	5	LDF7-50A(1 5/8)	60.00 - 80.00	0.6000	0.6000
T6	6	LDF7-50A(1 5/8)	60.00 - 80.00	0.6000	0.6000
T6	7	LDF7-50A(1 5/8)	60.00 - 80.00	0.6000	0.6000
Т6	9	HYBRID( 1-1/4")	60.00 - 80.00	0.6000	0.6000
T6	10	T-Brackets (Af)	60.00 - 80.00	0.6000	0.6000
Т6	11	T-Brackets (Af)	60.00 - 80.00	0.6000	0.6000
T6	12	T-Brackets (Af)	60.00 - 80.00	0.6000	0.6000
T7	5	LDF7-50A(1 5/8)	40.00 - 60.00	0.6000	0.6000
T7	6	LDF7-50A(1 5/8)	40.00 - 60.00	0.6000	0.6000
T7	7	LDF7-50A(1 5/8)	40.00 - 60.00	0.6000	0.6000
T7	9	HYBRID( 1-1/4")	40.00 - 60.00	0.6000	0.6000
T7	10	T-Brackets (Af)	40.00 - 60.00	0.6000	0.6000
T7	11	T-Brackets (Af)	40.00 - 60.00	0.6000	0.6000
T7	12	T-Brackets (Af)	40.00 - 60.00	0.6000	0.6000
T8	5	LDF7-50A(1 5/8)	20.00 - 40.00	0.6000	0.6000
T8	6	LDF7-50A(1 5/8)	20.00 - 40.00	0.6000	0.6000
T8	7	LDF7-50A(1 5/8)	20.00 - 40.00	0.6000	0.6000
T8	9	HYBRID( 1-1/4")	20.00 - 40.00	0.6000	0.6000
T8	10	T-Brackets (Af)	20.00 - 40.00	0.6000	0.6000
T8	11	T-Brackets (Af)	20.00 - 40.00	0.6000	0.6000
Т8	12	T-Brackets (Af)	20.00 - 40.00	0.6000	0.6000
Т9	5	LDF7-50A(1 5/8)	0.00 - 20.00	0.6000	0.6000
Т9	6	LDF7-50A(1 5/8)	0.00 - 20.00	0.6000	0.6000
Т9	7	LDF7-50A(1 5/8)	0.00 - 20.00	0.6000	0.6000
Т9	9	HYBRID( 1-1/4")	0.00 - 20.00	0.6000	0.6000
Т9	10	T-Brackets (Af)	0.00 - 20.00	0.6000	0.6000
Т9	11	T-Brackets (Af)	0.00 - 20.00	0.6000	0.6000
Т9	12	T-Brackets (Af)	0.00 - 20.00	0.6000	0.6000

## **Discrete Tower Loads**

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	$C_A A_A$ Side	Weigh
			Vert ft ft ft	0	ft		ft <sup>2</sup>	ft²	K
****									
15' lighting rod	C	None		0.0000	170.00	No Ice	4.50	4.50	0
						1/2" Ice	6.03	6.03	0
						1" Ice	7.58	7.58	0
Beacon	C	None		0.0000	170.00	No Ice	2.70	2.70	0

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Client	Northeast Site Solutions	Designed by L. Mendoza

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		$C_AA_A$ Front	$C_AA_A$ Side	Weigh
	Leg		Vert ft	0	ft		ft²	ft²	K
			ft ft						
			<i>J</i> •			1/2" Ice 1" Ice	3.10 3.50	3.10	0
***						1 ICE	3.30	3.50	U
MX08FRO665-21 w/ Mount	A	From Leg	4.00	0.0000	170.00	No Ice	13.10	7.85	0
Pipe		S	0.00			1/2" Ice	13.81	9.14	0
(Dish)			0.00			1" Ice	14.48	10.28	0
TA08025-B604	A	From Leg	4.00	0.0000	170.00	No Ice	1.96	0.98	0
(Dish)			0.00			1/2" Ice	2.14	1.11	0
TA 00025 DC05		E I	0.00	0.0000	170.00	1" Ice	2.32	1.25	0
TA08025-B605 (Dish)	A	From Leg	4.00 0.00	0.0000	170.00	No Ice 1/2" Ice	1.96 2.14	1.13 1.27	0 0
(DISII)			0.00			1" Ice	2.14	1.41	0
MX08FRO665-21 w/ Mount	В	From Leg	4.00	0.0000	170.00	No Ice	13.10	7.85	0
Pipe	Ь	1 Tom Leg	0.00	0.0000	170.00	1/2" Ice	13.81	9.14	0
(Dish)			0.00			1" Ice	14.48	10.28	0
TA08025-B604	В	From Leg	4.00	0.0000	170.00	No Ice	1.96	0.98	0
(Dish)			0.00			1/2" Ice	2.14	1.11	0
, ,			0.00			1" Ice	2.32	1.25	0
TA08025-B605	В	From Leg	4.00	0.0000	170.00	No Ice	1.96	1.13	0
(Dish)			0.00			1/2" Ice	2.14	1.27	0
			0.00			1" Ice	2.32	1.41	0
MX08FRO665-21 w/ Mount	C	From Leg	4.00	0.0000	170.00	No Ice	13.10	7.85	0
Pipe			0.00			1/2" Ice	13.81	9.14	0
(Dish)			0.00			1" Ice	14.48	10.28	0
TA08025-B604	C	From Leg	4.00	0.0000	170.00	No Ice	1.96	0.98	0
(Dish)			0.00			1/2" Ice	2.14	1.11	0
TA 09025 DC05	C	F I	0.00	0.0000	170.00	1" Ice	2.32	1.25	0
TA08025-B605	С	From Leg	4.00	0.0000	170.00	No Ice	1.96	1.13	0
(Dish)			0.00			1/2" Ice 1" Ice	2.14 2.32	1.27 1.41	0
RDIDC-9181-PF-48	Α	From Leg	4.00	0.0000	170.00	No Ice	2.32	1.41	0
(Dish)	А	110iii Leg	0.00	0.0000	170.00	1/2" Ice	2.19	1.17	0
(Disii)			0.00			1" Ice	2.37	1.46	0
(3) Commscope	C	None	0.00	0.0000	170.00	No Ice	22.34	22.34	0
MTC3975083	C	TTORIC		0.0000	170.00	1/2" Ice	31.70	31.70	1
(Dish) *****						1" Ice	41.06	41.06	1
6812	В	From Leg	2.00	0.0000	160.00	No Ice	0.20	0.20	0
5512	~	205	0.00	0.0000	100.00	1/2" Ice	0.36	0.36	0
			0.00			1" Ice	0.52	0.52	0
6812	В	From Leg	2.00	0.0000	155.00	No Ice	0.20	0.20	0
		Ü	0.00			1/2" Ice	0.36	0.36	0
destructive to			0.00			1" Ice	0.52	0.52	0
***** AID 21 B2A/B4D w/ Mount	٨	From I ac	4.00	0.0000	140.00	No Ioo	6.16	5 5 5	0
AIR 21 B2A/B4P w/ Mount Pipe	A	From Leg	0.00	0.0000	140.00	No Ice 1/2" Ice	6.16 6.60	5.55 6.30	0 0
(TMO)			0.00			1" Ice	7.03	7.00	0
AIR 21 B2A/B4P w/ Mount	В	From Leg	4.00	0.0000	140.00	No Ice	6.16	5.55	0
Pipe	~	205	0.00	0.0000	1.5.00	1/2" Ice	6.60	6.30	0
(TMO)			0.00			1" Ice	7.03	7.00	0
(2) AIR 21 B2A/B4P w/	C	From Leg	4.00	0.0000	140.00	No Ice	6.16	5.55	0
Mount Pipe		3	0.00			1/2" Ice	6.60	6.30	0
(TMO)			0.00			1" Ice	7.03	7.00	0
AIR 32 B2A B66AA w/	Α	From Leg	4.00	0.0000	140.00	No Ice	7.09	6.37	0
Mount Pipe			0.00			1/2" Ice	7.56	7.23	0
(TMO)			0.00			1" Ice	8.02	7.97	0

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	Northeast Site Solutions	L. Mendoza

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		$C_AA_A$ Front	$C_AA_A$ Side	Weigh
	Leg	¥X .	Lateral Vert	•					
			veri ft	0	ft		ft <sup>2</sup>	ft <sup>2</sup>	K
			ft ft		Ji		Ji	Ji	K
AIR 32 B2A B66AA w/	В	From Leg	4.00	0.0000	140.00	No Ice	7.09	6.37	0
Mount Pipe			0.00			1/2" Ice	7.56	7.23	0
(TMO)			0.00			1" Ice	8.02	7.97	0
(2) AIR 32 B2A B66AA w/	C	From Leg	4.00	0.0000	140.00	No Ice	7.09	6.37	0
Mount Pipe			0.00			1/2" Ice	7.56	7.23	0
(TMO)			0.00			1" Ice	8.02	7.97	0
(2)	Α	From Leg	4.00	0.0000	140.00	No Ice	14.69	6.87	0
.PXVAARR24_43-U-NA20 w/ Mount Pipe			$0.00 \\ 0.00$			1/2" Ice 1" Ice	15.46 16.23	7.55 8.25	0
(TMO)									
APXVAARR24_43-U-NA20	В	From Leg	4.00	0.0000	140.00	No Ice	14.69	6.87	0
w/ Mount Pipe			0.00			1/2" Ice	15.46	7.55	0
(TMO)	C	Enoug I	0.00	0.0000	140.00	1" Ice	16.23	8.25	0
APXVAARR24_43-U-NA20	C	From Leg	4.00	0.0000	140.00	No Ice	14.69	6.87	0
w/ Mount Pipe			0.00			1/2" Ice	15.46	7.55	0
(TMO) AIR 6449 B41 (MASSIVE	A	From Leg	0.00 4.00	0.0000	140.00	1" Ice No Ice	16.23 5.98	8.25 3.46	0 0
MIMO) w/ MP	Α	From Leg	0.00	0.0000	140.00	1/2" Ice	6.36	3.46	0
(TMO)			0.00			1" Ice	6.75	4.43	0
AIR 6449 B41 (MASSIVE	В	From Leg	4.00	0.0000	140.00	No Ice	5.98	3.46	0
MIMO) w/ MP	ь	110III Leg	0.00	0.0000	140.00	1/2" Ice	6.36	3.40	0
(TMO)			0.00			1" Ice	6.75	4.43	0
(2) AIR 6449 B41	C	From Leg	4.00	0.0000	140.00	No Ice	5.98	3.46	0
(MASSIVE MIMO) w/ MP	_	Trom Leg	0.00	0.0000	110.00	1/2" Ice	6.36	3.94	0
(TMO)			0.00			1" Ice	6.75	4.43	0
(2) Radio 4449 B71+B85	Α	From Leg	4.00	0.0000	140.00	No Ice	1.75	1.31	0
(TMO)			0.00			1/2" Ice	1.91	1.46	0
,			0.00			1" Ice	2.09	1.61	0
Radio 4449 B71+B85	В	From Leg	4.00	0.0000	140.00	No Ice	1.75	1.31	0
(TMO)		_	0.00			1/2" Ice	1.91	1.46	0
			0.00			1" Ice	2.09	1.61	0
Radio 4449 B71+B85	C	From Leg	4.00	0.0000	140.00	No Ice	1.75	1.31	0
(TMO)			0.00			1/2" Ice	1.91	1.46	0
			0.00			1" Ice	2.09	1.61	0
RADIO 4415 B25	Α	From Leg	4.00	0.0000	140.00	No Ice	1.86	0.87	0
(TMO)			0.00			1/2" Ice	2.03	1.00	0
			0.00			1" Ice	2.20	1.14	0
RADIO 4415 B25	В	From Leg	4.00	0.0000	140.00	No Ice	1.86	0.87	0
(TMO)			0.00			1/2" Ice	2.03	1.00	0
(A) D   D   0   1   1   5   5			0.00	0.0000	4.40.00	1" Ice	2.20	1.14	0
(2) RADIO 4415 B25	C	From Leg	4.00	0.0000	140.00	No Ice	1.86	0.87	0
(TMO)			0.00			1/2" Ice	2.03	1.00	0
VDV 112 144/1		Enom Loo	0.00	0.0000	140.00	1" Ice	2.20	1.14	0
KRY 112 144/1	A	From Leg	4.00	0.0000	140.00	No Ice 1/2" Ice	0.35	0.17	0
(TMO)			0.00 0.00			1/2 Ice 1" Ice	0.43 0.51	0.23 0.30	0
KRY 112 144/1	В	From Leg	4.00	0.0000	140.00	No Ice	0.31	0.30	0
(TMO)	ъ	1 Ioni Leg	0.00	0.0000	140.00	1/2" Ice	0.33	0.17	0
(11/10)			0.00			1" Ice	0.43	0.23	0
(2) KRY 112 144/1	C	From Leg	4.00	0.0000	140.00	No Ice	0.35	0.30	0
(Z) KKT 112 144/1 (TMO)		1 Ioni Leg	0.00	0.0000	170.00	1/2" Ice	0.33	0.17	0
(1110)			0.00			1" Ice	0.43	0.23	0
custom 4 sided sector mount	C	None	0.00	0.0000	140.00	No Ice	36.00	36.00	3
	_					1/2" Ice	42.00	42.00	3
ند چه چه چه						1" Ice	48.00	48.00	4
**** (4) SBNHH-1D65B w/	A	From Leg	4.00	0.0000	100.00	No Ice	4.09	3.30	0

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Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		$C_AA_A$ Front	$C_A A_A$ Side	Weigh
	Leg		Lateral Vert						
			ft ft	٥	ft		$ft^2$	ft <sup>2</sup>	K
			ft						
Mount Pipe			0.00			1/2" Ice	4.49	3.68	0
(Verizon)			0.00			1" Ice	4.89	4.07	0
(4) SBNHH-1D65B w/	В	From Leg	4.00	0.0000	100.00	No Ice	4.09	3.30	0
Mount Pipe			0.00			1/2" Ice	4.49	3.68	0
(Verizon)	C	F I	0.00	0.0000	100.00	1" Ice	4.89	4.07	0
(4) SBNHH-1D65B w/	С	From Leg	4.00 0.00	0.0000	100.00	No Ice 1/2" Ice	4.09 4.49	3.30 3.68	0
Mount Pipe (Verizon)			0.00			1" Ice	4.49	4.07	0
RRH2x60-700	A	From Leg	4.00	0.0000	100.00	No Ice	3.50	1.82	0
(Verizon)		Trom Beg	0.00	0.0000	100.00	1/2" Ice	3.76	2.05	0
( :)			0.00			1" Ice	4.03	2.29	0
RRH2x60-700	В	From Leg	4.00	0.0000	100.00	No Ice	3.50	1.82	0
(Verizon)		_	0.00			1/2" Ice	3.76	2.05	0
			0.00			1" Ice	4.03	2.29	0
RRH2x60-700	C	From Leg	4.00	0.0000	100.00	No Ice	3.50	1.82	0
(Verizon)			0.00			1/2" Ice	3.76	2.05	0
DDHAM CO DCG		Б. т	0.00	0.0000	100.00	1" Ice	4.03	2.29	0
RRH2X60-PCS	A	From Leg	4.00	0.0000	100.00	No Ice	2.20	1.72	0
(Verizon)			0.00			1/2" Ice	2.39	1.90	0
RRH2X60-PCS	В	From Leg	0.00 4.00	0.0000	100.00	1" Ice No Ice	2.59 2.20	2.09 1.72	0
(Verizon)	ь	From Leg	0.00	0.0000	100.00	1/2" Ice	2.39	1.72	0
(VCHZOII)			0.00			1" Ice	2.59	2.09	0
RRH2X60-PCS	C	From Leg	4.00	0.0000	100.00	No Ice	2.20	1.72	0
(Verizon)			0.00			1/2" Ice	2.39	1.90	0
, ,			0.00			1" Ice	2.59	2.09	0
RRH4x45/2x90-AWS	A	From Leg	4.00	0.0000	100.00	No Ice	2.66	1.59	0
(Verizon)			0.00			1/2" Ice	2.88	1.77	0
			0.00			1" Ice	3.10	1.96	0
RRH4x45/2x90-AWS	В	From Leg	4.00	0.0000	100.00	No Ice	2.66	1.59	0
(Verizon)			0.00			1/2" Ice	2.88	1.77	0
RRH4x45/2x90-AWS	С	From Leg	0.00 4.00	0.0000	100.00	1" Ice No Ice	3.10 2.66	1.96 1.59	0
(Verizon)	C	From Leg	0.00	0.0000	100.00	1/2" Ice	2.88	1.77	0
(VCIIZOII)			0.00			1" Ice	3.10	1.96	0
DB-T1-6Z-8AB-0Z	A	From Leg	4.00	0.0000	100.00	No Ice	4.80	2.00	0
(Verizon)			0.00			1/2" Ice	5.07	2.19	0
, ,			0.00			1" Ice	5.35	2.39	0
DB-T1-6Z-8AB-0Z	В	From Leg	4.00	0.0000	100.00	No Ice	4.80	2.00	0
(Verizon)			0.00			1/2" Ice	5.07	2.19	0
			0.00			1" Ice	5.35	2.39	0
10'6"x2-3/8" Pipe Mount	Α	From Leg	4.00	0.0000	100.00	No Ice	2.49	2.49	0
(Verizon)			0.00			1/2" Ice	3.57	3.57	0
10'6"''' 2 2/9" Dina Mayor	D	Enom Loo	0.00	0.0000	100.00	1" Ice	4.67	4.67	0
10'6"x2-3/8" Pipe Mount	В	From Leg	4.00	0.0000	100.00	No Ice 1/2" Ice	2.49	2.49	0
(Verizon)			0.00 0.00			1/2 Ice 1" Ice	3.57 4.67	3.57 4.67	$0 \\ 0$
10'6"x2-3/8" Pipe Mount	C	From Leg	4.00	0.0000	100.00	No Ice	2.49	2.49	0
(Verizon)		1.0m Ecg	0.00	0.0000	100.00	1/2" Ice	3.57	3.57	0
( ) ( ) ( )			0.00			1" Ice	4.67	4.67	0
Sector Mount [SM 502-3]	C	None		0.0000	100.00	No Ice	33.02	33.02	2
(Verizon)						1/2" Ice	47.36	47.36	2
						1" Ice	61.70	61.70	3
****									

# Infinigy Engineering, LLP 26455 Rancho Parkway S.

26455 Rancho Parkway S. Lake Forest, CA 92630 Phone: (518) 690-0790 FAX: (518) 690-0790

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_			_		Dis	shes	-				
Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter		Aperture Area	Weight
				ft	0	0	ft	ft		ft <sup>2</sup>	K
SC2-W100AB	C	Paraboloid w/o	From	3.00	0.0000		140.00	2.20	No Ice	3.80	0
		Radome	Leg	0.00					1/2" Ice	4.10	0
			_	0.00					1" Ice	4.39	0
6 FT DISH	A	Paraboloid w/o	From	1.00	0.0000		165.00	6.00	No Ice	28.27	0
		Radome	Leg	0.00					1/2" Ice	29.05	0
				0.00					1" Ice	29.83	0
6 FT DISH	В	Paraboloid w/o	From	1.00	0.0000		165.00	6.00	No Ice	28.27	0
		Radome	Leg	0.00					1/2" Ice	29.05	0
				0.00					1" Ice	29.83	0
6 FT DISH	C	Paraboloid w/o	From	1.00	0.0000		165.00	6.00	No Ice	28.27	0
		Radome	Leg	0.00					1/2" Ice	29.05	0
				0.00					1" Ice	29.83	0
4 FT DISH	A	Paraboloid w/o	From	1.00	0.0000		150.00	4.00	No Ice	12.56	0
		Radome	Leg	0.00					1/2" Ice	13.09	0
				0.00					1" Ice	13.62	0
4 FT DISH	В	Paraboloid w/o	From	1.00	0.0000		150.00	4.00	No Ice	12.56	0
		Radome	Leg	0.00					1/2" Ice	13.09	0
				0.00					1" Ice	13.62	0
2' Dish	C	Paraboloid w/o	From	1.00	0.0000		150.00	2.00	No Ice	3.14	0
		Radome	Leg	0.00					1/2" Ice	3.41	0
				0.00					1" Ice	3.68	0
2' Dish	Α	Paraboloid	From	1.00	0.0000		110.00	2.00	No Ice	3.14	0
		w/Shroud (HP)	Leg	0.00					1/2" Ice	3.41	0
****				0.00					1" Ice	3.68	0

## **Truss-Leg Properties**

Section Designation	Area	Area Ice	Self Weight	Ice Weight	Equiv. Diameter	Equiv. Diameter	Leg Area
Designation		100	,, ,,	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Diameter	Ice	717 CG
	$in^2$	$in^2$	K	K	in	in	$in^2$
Valmont 207628	2164.5223	6580.3363	0	2	7.5157	22.8484	3.6816
Valmont 207629	2291.7722	6636.4847	1	2	7.9575	23.0433	5.3014
Valmont 207629	2291.7722	6612.9668	1	2	7.9575	22.9617	5.3014
Valmont 195557	2415.8323	6657.9346	1	2	8.3883	23.1178	7.2158
Valmont 211843	2556.2356	6698.0542	1	2	8.8758	23.2571	9.4248
Valmont 208333	2556.2356	6659.0197	1	2	8.8758	23.1216	9.4248
Valmont 208334	2683.9340	6680.2711	1	2	9.3192	23.1954	11.9282
Valmont 208334	2683.9340	6606.4167	1	2	9.3192	22.9389	11.9282
Valmont 208335	2824.1561	6531.8043	1	1	9.8061	22.6799	14.7262

### **Tower Pressures - No Ice**

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Section	Z	$K_Z$	$q_z$	$A_G$	F	$A_F$	$A_R$	$A_{leg}$	Leg	$C_A A_A$	$C_A A_A$
Elevation					а			-	%	In	Out
					С					Face	Face
ft	ft		psf	$ft^2$	e	$ft^2$	$ft^2$	$ft^2$		ft <sup>2</sup>	ft <sup>2</sup>
T1	165.00	1.406	29	86.055	Α	7.326	12.547	12.547	63.14	2.917	0.000
170.00-160.00					В	7.326	12.547		63.14	4.637	0.000
					C	7.326	12.547		63.14	1.667	0.000
T2	150.00	1.378	28	202.528	Α	10.269	26.569	26.569	72.12	5.833	0.000
160.00-140.00					В	10.269	26.569		72.12	28.083	0.000
					C	10.269	26.569		72.12	3.333	0.000
Т3	130.00	1.337	27	242.528	Α	13.925	26.569	26.569	65.61	5.833	0.000
140.00-120.00					В	13.925	26.569		65.61	35.013	0.000
					C	13.925	26.569		65.61	66.693	0.000
T4	110.00	1.291	26	282.945	Α	15.598	28.008	28.008	64.23	5.833	0.000
120.00-100.00					В	15.598	28.008		64.23	35.013	0.000
					C	15.598	28.008		64.23	66.693	0.000
T5	90.00	1.238	25	323.362	Α	12.021	29.635	29.635	71.14	5.833	0.000
100.00-80.00					В	12.021	29.635		71.14	35.013	0.000
					C	12.021	29.635		71.14	74.613	0.000
T6 80.00-60.00	70.00	1.174	24	363.362	Α	12.726	29.635	29.635	69.96	5.833	0.000
					В	12.726	29.635		69.96	35.013	0.000
					C	12.726	29.635		69.96	74.613	0.000
T7 60.00-40.00	50.00	1.094	22	403.780	Α	13.462	31.116	31.116	69.80	5.833	0.000
					В	13.462	31.116		69.80	35.013	0.000
					C	13.462	31.116		69.80	74.613	0.000
T8 40.00-20.00	30.00	0.982	20	443.780	Α	16.598	31.116	31.116	65.21	5.833	0.000
					В	16.598	31.116		65.21	35.013	0.000
					C	16.598	31.116		65.21	74.613	0.000
T9 20.00-0.00	10.00	0.85	17	484.197	Α	17.520	32.741	32.741	65.14	5.833	0.000
					В	17.520	32.741		65.14	35.013	0.000
					C	17.520	32.741		65.14	74.613	0.000

### **Tower Pressure - With Ice**

 $G_H = 0.850$ 

Section	z	$K_Z$	$q_z$	$t_Z$	$A_G$	F	$A_F$	$A_R$	$A_{leg}$	Leg	$C_A A_A$	$C_A A_A$
Elevation						a				%	In	Out
					_	c	_	_	_		Face	Face
ft	ft		psf	in	$ft^2$	e	$ft^2$	$ft^2$	$ft^2$		$ft^2$	$ft^2$
T1	165.00	1.406	8	2.3492	89.976	Α	7.326	51.512	38.144	64.83	12.314	0.000
170.00-160.00						В	7.326	51.512		64.83	16.399	0.000
						C	7.326	51.512		64.83	6.365	0.000
T2	150.00	1.378	7	2.3270	210.294	Α	10.269	95.499	76.939	72.74	24.449	0.000
160.00-140.00						В	10.269	95.499		72.74	66.241	0.000
						C	10.269	95.499		72.74	12.641	0.000
T3	130.00	1.337	7	2.2939	250.184	Α	13.925	97.342	76.666	68.90	24.184	0.000
140.00-120.00						В	13.925	97.342		68.90	65.610	0.000
						C	13.925	97.342		68.90	102.178	0.000
T4	110.00	1.291	7	2.2559	290.474	Α	15.598	99.963	77.188	66.79	23.880	0.000
120.00-100.00						В	15.598	99.963		66.79	65.199	0.000
						C	15.598	99.963		66.79	101.789	0.000
T5 100.00-80.00	90.00	1.238	7	2.2111	330.742	Α	12.021	94.856	77.653	72.66	23.522	0.000
						В	12.021	94.856		72.66	64.714	0.000
						C	12.021	94.856		72.66	131.781	0.000
T6 80.00-60.00	70.00	1.174	6	2.1562	370.559	Α	12.726	94.961	77.200	71.69	23.083	0.000
						В	12.726	94.961		71.69	64.121	0.000
						C	12.726	94.961		71.69	130.842	0.000
T7 60.00-40.00	50.00	1.094	6	2.0849	410.738	A	13.462	95.613	77.447	71.00	22.512	0.000

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Section Elevation	z	$K_Z$	$q_z$	$t_Z$	$A_G$	F	$A_F$	$A_R$	$A_{leg}$	Leg %	$C_A A_A$	$C_AA_A$ Out
Elevation						а				70	In	
					_	c	_	_	_		Face	Face
ft	ft		psf	in	ft <sup>2</sup>	e	$ft^2$	$ft^2$	$ft^2$		$ft^2$	ft <sup>2</sup>
						В	13.462	95.613		71.00	63.349	0.000
						C	13.462	95.613		71.00	129.622	0.000
T8 40.00-20.00	30.00	0.982	5	1.9810	450.391	Α	16.598	94.832	76.590	68.73	21.682	0.000
						В	16.598	94.832		68.73	62.228	0.000
						C	16.598	94.832		68.73	127.848	0.000
T9 20.00-0.00	10.00	0.85	5	1.7749	490.121	Α	17.520	92.978	75.725	68.53	20.033	0.000
						В	17.520	92.978		68.53	60.004	0.000
						C	17.520	92.978		68.53	124.336	0.000

### **Tower Pressure - Service**

 $G_H = 0.850$ 

Section	z	$K_Z$	$q_z$	$A_G$	F	$A_F$	$A_R$	$A_{leg}$	Leg	$C_A A_A$	$C_A A_A$
Elevation					a				%	In	Out
					c					Face	Face
ft	ft		psf	$ft^2$	e	$ft^2$	$ft^2$	$ft^2$		$ft^2$	$ft^2$
T1	165.00	1.406	11	86.055	Α	7.326	12.547	12.547	63.14	2.917	0.000
170.00-160.00					В	7.326	12.547		63.14	4.637	0.000
					C	7.326	12.547		63.14	1.667	0.000
T2	150.00	1.378	11	202.528	Α	10.269	26.569	26.569	72.12	5.833	0.000
160.00-140.00					В	10.269	26.569		72.12	28.083	0.000
					C	10.269	26.569		72.12	3.333	0.000
T3	130.00	1.337	10	242.528	Α	13.925	26.569	26.569	65.61	5.833	0.000
140.00-120.00					В	13.925	26.569		65.61	35.013	0.000
					C	13.925	26.569		65.61	66.693	0.000
T4	110.00	1.291	10	282.945	Α	15.598	28.008	28.008	64.23	5.833	0.000
120.00-100.00					В	15.598	28.008		64.23	35.013	0.000
					C	15.598	28.008		64.23	66.693	0.000
T5	90.00	1.238	10	323.362	Α	12.021	29.635	29.635	71.14	5.833	0.000
100.00-80.00					В	12.021	29.635		71.14	35.013	0.000
					C	12.021	29.635		71.14	74.613	0.000
T6 80.00-60.00	70.00	1.174	9	363.362	Α	12.726	29.635	29.635	69.96	5.833	0.000
					В	12.726	29.635		69.96	35.013	0.000
					C	12.726	29.635		69.96	74.613	0.000
T7 60.00-40.00	50.00	1.094	9	403.780	Α	13.462	31.116	31.116	69.80	5.833	0.000
					В	13.462	31.116		69.80	35.013	0.000
					C	13.462	31.116		69.80	74.613	0.000
T8 40.00-20.00	30.00	0.982	8	443.780	Α	16.598	31.116	31.116	65.21	5.833	0.000
					В	16.598	31.116		65.21	35.013	0.000
					C	16.598	31.116		65.21	74.613	0.000
T9 20.00-0.00	10.00	0.85	7	484.197	Α	17.520	32.741	32.741	65.14	5.833	0.000
					В	17.520	32.741		65.14	35.013	0.000
					C	17.520	32.741		65.14	74.613	0.000

## **Tower Forces - No Ice - Wind Normal To Face**

Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			С			psf						
ft	K	K	e						$ft^2$	K	plf	
	1 0	1	Α	0.231	2.496	29	1	1	14.611	1	100.95	В

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Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	a			_						Face
	_	_	c			psf						
ft	K	K	e						ft <sup>2</sup>	K	plf	
170.00-160.00			В	0.231	2.496		1	1	14.611			
			C	0.231	2.496		1	1	14.611			
T2	1	2	Α	0.182	2.658	28	1	1	25.455	2	106.15	В
160.00-140.00			В	0.182	2.658		1	1	25.455			
			C	0.182	2.658		1	1	25.455			
T3	1	2	Α	0.167	2.71	27	1	1	29.056	3	156.69	C
140.00-120.00			В	0.167	2.71		1	1	29.056			
			C	0.167	2.71		1	1	29.056			
T4	1	3	Α	0.154	2.756	26	1	1	31.506	3	160.38	C
120.00-100.00			В	0.154	2.756		1	1	31.506			
			C	0.154	2.756		1	1	31.506			
T5	1	4	Α	0.129	2.851	25	1	1	28.786	3	153.72	C
100.00-80.00			В	0.129	2.851		1	1	28.786			
			C	0.129	2.851		1	1	28.786			
T6	1	4	Α	0.117	2.898	24	1	1	29.470	3	149.22	C
80.00-60.00			В	0.117	2.898		1	1	29.470			
			C	0.117	2.898		1	1	29.470			
T7	1	5	Α	0.11	2.923	22	1	1	31.034	3	144.04	C
60.00-40.00			В	0.11	2.923		1	1	31.034			
			C	0.11	2.923		1	1	31.034			
T8	1	6	Α	0.108	2.934	20	1	1	34.166	3	137.51	C
40.00-20.00			В	0.108	2.934		1	1	34.166			
			C	0.108	2.934		1	1	34.166			
T9 20.00-0.00	1	6	Α	0.104	2.949	17	1	1	36.002	2	123.38	C
			В	0.104	2.949		1	1	36.002			
			C	0.104	2.949		1	1	36.002			
Sum Weight:	7	35						OTM	1982 kip-ft	24		

## **Tower Forces - No Ice - Wind 60 To Face**

Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			С			psf						
ft	K	K	e						$ft^2$	K	plf	
T1	0	1	Α	0.231	2.496	29	0.8	1	13.146	1	92.00	C
170.00-160.00			В	0.231	2.496		0.8	1	13.146			
			C	0.231	2.496		0.8	1	13.146			
T2	1	2	Α	0.182	2.658	28	0.8	1	23.401	2	99.60	C
160.00-140.00			В	0.182	2.658		0.8	1	23.401			
			C	0.182	2.658		0.8	1	23.401			
T3	1	2	Α	0.167	2.71	27	0.8	1	26.271	3	147.91	Α
140.00-120.00			В	0.167	2.71		0.8	1	26.271			
			C	0.167	2.71		0.8	1	26.271			
T4	1	3	Α	0.154	2.756	26	0.8	1	28.386	3	150.72	Α
120.00-100.00			В	0.154	2.756		0.8	1	28.386			
			C	0.154	2.756		0.8	1	28.386			
T5	1	4	Α	0.129	2.851	25	0.8	1	26.382	3	146.34	Α
100.00-80.00			В	0.129	2.851		0.8	1	26.382			
			C	0.129	2.851		0.8	1	26.382			
T6	1	4	Α	0.117	2.898	24	0.8	1	26.925	3	141.68	Α
80.00-60.00			В	0.117	2.898		0.8	1	26.925			
			C	0.117	2.898		0.8	1	26.925			
T7	1	5	Α	0.11	2.923	22	0.8	1	28.341	3	136.55	Α
60.00-40.00			В	0.11	2.923		0.8	1	28.341			

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Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	K	K	e						$ft^2$	K	plf	
			C	0.11	2.923		0.8	1	28.341			
T8	1	6	Α	0.108	2.934	20	0.8	1	30.846	3	129.19	Α
40.00-20.00			В	0.108	2.934		0.8	1	30.846			
			C	0.108	2.934		0.8	1	30.846			
T9 20.00-0.00	1	6	Α	0.104	2.949	17	0.8	1	32.498	2	115.73	Α
			В	0.104	2.949		0.8	1	32.498			
			C	0.104	2.949		0.8	1	32.498			
Sum Weight:	7	35						OTM	1866 kip-ft	22		

### **Tower Forces - No Ice - Wind 90 To Face**

Section	Add	Self	F	е	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			c			psf						
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1	0	1	Α	0.231	2.496	29	0.85	1	13.512	1	94.24	C
170.00-160.00			В	0.231	2.496		0.85	1	13.512			
			C	0.231	2.496		0.85	1	13.512			
T2	1	2	Α	0.182	2.658	28	0.85	1	23.914	2	100.52	C
160.00-140.00			В	0.182	2.658		0.85	1	23.914			
			C	0.182	2.658		0.85	1	23.914			
T3	1	2	Α	0.167	2.71	27	0.85	1	26.967	3	154.25	A
140.00-120.00			В	0.167	2.71		0.85	1	26.967			
			C	0.167	2.71		0.85	1	26.967			
T4	1	3	Α	0.154	2.756	26	0.85	1	29.166	3	157.14	A
120.00-100.00			В	0.154	2.756		0.85	1	29.166			
			C	0.154	2.756		0.85	1	29.166			
T5	1	4	Α	0.129	2.851	25	0.85	1	26.983	3	152.02	A
100.00-80.00			В	0.129	2.851		0.85	1	26.983			
			C	0.129	2.851		0.85	1	26.983			
T6	1	4	Α	0.117	2.898	24	0.85	1	27.561	3	147.20	A
80.00-60.00			В	0.117	2.898		0.85	1	27.561			
			C	0.117	2.898		0.85	1	27.561			
T7	1	5	Α	0.11	2.923	22	0.85	1	29.014	3	141.82	A
60.00-40.00			В	0.11	2.923		0.85	1	29.014			
			C	0.11	2.923		0.85	1	29.014			
T8	1	6	Α	0.108	2.934	20	0.85	1	31.676	3	134.32	A
40.00-20.00			В	0.108	2.934		0.85	1	31.676			
			C	0.108	2.934		0.85	1	31.676			
T9 20.00-0.00	1	6	Α	0.104	2.949	17	0.85	1	33.374	2	120.28	A
			В	0.104	2.949		0.85	1	33.374			
			C	0.104	2.949		0.85	1	33.374			
Sum Weight:	7	35						OTM	1930 kip-ft	23		

## **Tower Forces - With Ice - Wind Normal To Face**

Infinigy Engineering, LLP 26455 Rancho Parkway S.

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Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а			-						Face
		_	c			psf						
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1	1	6	Α	0.654	1.78	8	1	1	47.459	1	62.64	В
170.00-160.00			В	0.654	1.78		1	1	47.459			
			C	0.654	1.78		1	1	47.459			
T2	3	10	Α	0.503	1.896	7	1	1	75.839	1	61.43	В
160.00-140.00			В	0.503	1.896		1	1	75.839			
			C	0.503	1.896		1	1	75.839			
Т3	5	11	Α	0.445	1.982	7	1	1	77.852	2	76.26	C
140.00-120.00			В	0.445	1.982		1	1	77.852			
			C	0.445	1.982		1	1	77.852			
T4	5	12	Α	0.398	2.068	7	1	1	79.106	2	78.44	C
120.00-100.00			В	0.398	2.068		1	1	79.106			
			C	0.398	2.068		1	1	79.106			
T5	6	13	Α	0.323	2.236	7	1	1	69.547	2	77.13	C
100.00-80.00			В	0.323	2.236		1	1	69.547			
			C	0.323	2.236		1	1	69.547			
T6	6	13	Α	0.291	2.321	6	1	1	69.329	1	74.34	C
80.00-60.00			В	0.291	2.321		1	1	69.329			
			C	0.291	2.321		1	1	69.329			
T7	5	13	Α	0.266	2.392	6	1	1	69.776	1	70.43	C
60.00-40.00			В	0.266	2.392		1	1	69.776			
			C	0.266	2.392		1	1	69.776			
T8	5	14	Α	0.247	2.445	5	1	1	72.015	1	64.90	C
40.00-20.00			В	0.247	2.445		1	1	72.015			
			C	0.247	2.445		1	1	72.015			
T9 20.00-0.00	5	14	Α	0.225	2.513	5	1	1	71.391	1	56.06	C
			В	0.225	2.513		1	1	71.391			
			C	0.225	2.513		1	1	71.391			
Sum Weight:	41	107						OTM	1022 kip-ft	12		

## **Tower Forces - With Ice - Wind 60 To Face**

Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1	1	6	Α	0.654	1.78	8	0.8	1	45.994	1	60.94	C
170.00-160.00			В	0.654	1.78		0.8	1	45.994			
			C	0.654	1.78		0.8	1	45.994			
T2	3	10	Α	0.503	1.896	7	0.8	1	73.785	1	60.19	C
160.00-140.00			В	0.503	1.896		0.8	1	73.785			
			C	0.503	1.896		0.8	1	73.785			
T3	5	11	Α	0.445	1.982	7	0.8	1	75.067	1	74.55	A
140.00-120.00			В	0.445	1.982		0.8	1	75.067			
			C	0.445	1.982		0.8	1	75.067			
T4	5	12	Α	0.398	2.068	7	0.8	1	75.986	2	76.51	Α
120.00-100.00			В	0.398	2.068		0.8	1	75.986			
			C	0.398	2.068		0.8	1	75.986			
T5	6	13	Α	0.323	2.236	7	0.8	1	67.143	2	75.59	A
100.00-80.00			В	0.323	2.236		0.8	1	67.143			
			C	0.323	2.236		0.8	1	67.143			
T6	6	13	Α	0.291	2.321	6	0.8	1	66.783	1	72.74	Α
80.00-60.00			В	0.291	2.321		0.8	1	66.783			
			C	0.291	2.321		0.8	1	66.783			
T7	5	13	Α	0.266	2.392	6	0.8	1	67.084	1	68.80	Α

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	Northeast Site Solutions	L. Mendoza

Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	K	K	e						ft <sup>2</sup>	K	plf	
60.00-40.00			В	0.266	2.392		0.8	1	67.084			
			C	0.266	2.392		0.8	1	67.084			
Т8	5	14	Α	0.247	2.445	5	0.8	1	68.696	1	63.06	Α
40.00-20.00			В	0.247	2.445		0.8	1	68.696			
			C	0.247	2.445		0.8	1	68.696			
T9 20.00-0.00	5	14	Α	0.225	2.513	5	0.8	1	67.887	1	54.33	Α
			В	0.225	2.513		0.8	1	67.887			
			C	0.225	2.513		0.8	1	67.887			
Sum Weight:	41	107						OTM	999 kip-ft	12		

### **Tower Forces - With Ice - Wind 90 To Face**

Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			с			psf						
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1	1	6	Α	0.654	1.78	8	0.85	1	46.360	1	61.09	C
170.00-160.00			В	0.654	1.78		0.85	1	46.360			
			C	0.654	1.78		0.85	1	46.360			
T2	3	10	Α	0.503	1.896	7	0.85	1	74.299	1	59.42	C
160.00-140.00			В	0.503	1.896		0.85	1	74.299			
			C	0.503	1.896		0.85	1	74.299			
T3	5	11	Α	0.445	1.982	7	0.85	1	75.764	1	74.13	A
140.00-120.00			В	0.445	1.982		0.85	1	75.764			
			C	0.445	1.982		0.85	1	75.764			
T4	5	12	Α	0.398	2.068	7	0.85	1	76.766	2	76.11	Α
120.00-100.00			В	0.398	2.068		0.85	1	76.766			
			C	0.398	2.068		0.85	1	76.766			
T5	6	13	Α	0.323	2.236	7	0.85	1	67.744	1	74.70	Α
100.00-80.00			В	0.323	2.236		0.85	1	67.744			
			C	0.323	2.236		0.85	1	67.744			
T6	6	13	Α	0.291	2.321	6	0.85	1	67.420	1	71.93	A
80.00-60.00			В	0.291	2.321		0.85	1	67.420			
			C	0.291	2.321		0.85	1	67.420			
T7	5	13	Α	0.266	2.392	6	0.85	1	67.757	1	68.08	A
60.00-40.00			В	0.266	2.392		0.85	1	67.757			
			C	0.266	2.392		0.85	1	67.757			
Т8	5	14	Α	0.247	2.445	5	0.85	1	69.526	1	62.51	A
40.00-20.00			В	0.247	2.445		0.85	1	69.526			
			C	0.247	2.445		0.85	1	69.526			
T9 20.00-0.00	5	14	Α	0.225	2.513	5	0.85	1	68.763	1	53.89	A
			В	0.225	2.513		0.85	1	68.763			
			C	0.225	2.513		0.85	1	68.763			
Sum Weight:	41	107						OTM	991 kip-ft	11		

### **Tower Forces - Service - Wind Normal To Face**

Infinigy Engineering, LLP 26455 Rancho Parkway S.

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Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1	0	1	A	0.231	2.496	11	1	1	14.611	0	38.63	В
170.00-160.00			В	0.231	2.496		1	1	14.611			
			C	0.231	2.496		1	1	14.611			
T2	1	2	Α	0.182	2.658	11	1	1	25.455	1	40.61	В
160.00-140.00			В	0.182	2.658		1	1	25.455			
			C	0.182	2.658		1	1	25.455			
T3	1	2	Α	0.167	2.71	10	1	1	29.056	1	59.95	C
140.00-120.00			В	0.167	2.71		1	1	29.056			
			C	0.167	2.71		1	1	29.056			
T4	1	3	Α	0.154	2.756	10	1	1	31.506	1	61.36	C
120.00-100.00			В	0.154	2.756		1	1	31.506			
			C	0.154	2.756		1	1	31.506			
T5	1	4	Α	0.129	2.851	10	1	1	28.786	1	58.82	C
100.00-80.00			В	0.129	2.851		1	1	28.786			
			C	0.129	2.851		1	1	28.786			
Т6	1	4	Α	0.117	2.898	9	1	1	29.470	1	57.09	C
80.00-60.00			В	0.117	2.898		1	1	29.470			
			C	0.117	2.898		1	1	29.470			
T7	1	5	Α	0.11	2.923	9	1	1	31.034	1	55.11	C
60.00-40.00			В	0.11	2.923		1	1	31.034			
			C	0.11	2.923		1	1	31.034			
T8	1	6	Α	0.108	2.934	8	1	1	34.166	1	52.61	C
40.00-20.00			В	0.108	2.934		1	1	34.166			
			C	0.108	2.934		1	1	34.166			
T9 20.00-0.00	1	6	Α	0.104	2.949	7	1	1	36.002	1	47.20	C
			В	0.104	2.949		1	1	36.002			
			C	0.104	2.949		1	1	36.002			
Sum Weight:	7	35						OTM	758 kip-ft	9		

### **Tower Forces - Service - Wind 60 To Face**

Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	K	K	e						$ft^2$	K	plf	
T1	0	1	Α	0.231	2.496	11	0.8	1	13.146	0	35.20	C
170.00-160.00			В	0.231	2.496		0.8	1	13.146			
			C	0.231	2.496		0.8	1	13.146			
T2	1	2	Α	0.182	2.658	11	0.8	1	23.401	1	38.11	C
160.00-140.00			В	0.182	2.658		0.8	1	23.401			
			C	0.182	2.658		0.8	1	23.401			
T3	1	2	Α	0.167	2.71	10	0.8	1	26.271	1	56.59	A
140.00-120.00			В	0.167	2.71		0.8	1	26.271			
			C	0.167	2.71		0.8	1	26.271			
T4	1	3	Α	0.154	2.756	10	0.8	1	28.386	1	57.67	A
120.00-100.00			В	0.154	2.756		0.8	1	28.386			
			C	0.154	2.756		0.8	1	28.386			
T5	1	4	Α	0.129	2.851	10	0.8	1	26.382	1	55.99	Α
100.00-80.00			В	0.129	2.851		0.8	1	26.382			
			C	0.129	2.851		0.8	1	26.382			
T6	1	4	Α	0.117	2.898	9	0.8	1	26.925	1	54.21	Α
80.00-60.00			В	0.117	2.898		0.8	1	26.925			
			C	0.117	2.898		0.8	1	26.925			
T7	1	5	Α	0.11	2.923	9	0.8	1	28.341	1	52.25	A

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Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	K	K	e						$ft^2$	K	plf	
60.00-40.00			В	0.11	2.923		0.8	1	28.341			
			C	0.11	2.923		0.8	1	28.341			
Т8	1	6	Α	0.108	2.934	8	0.8	1	30.846	1	49.43	A
40.00-20.00			В	0.108	2.934		0.8	1	30.846			
			C	0.108	2.934		0.8	1	30.846			
T9 20.00-0.00	1	6	Α	0.104	2.949	7	0.8	1	32.498	1	44.28	A
			В	0.104	2.949		0.8	1	32.498			
			C	0.104	2.949		0.8	1	32.498			
Sum Weight:	7	35						OTM	714 kip-ft	9		

### **Tower Forces - Service - Wind 90 To Face**

Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			c			psf						
ft	K	K	e						$ft^2$	K	plf	
T1	0	1	Α	0.231	2.496	11	0.85	1	13.512	0	36.06	C
170.00-160.00			В	0.231	2.496		0.85	1	13.512			
			C	0.231	2.496		0.85	1	13.512			
T2	1	2	Α	0.182	2.658	11	0.85	1	23.914	1	38.46	C
160.00-140.00			В	0.182	2.658		0.85	1	23.914			
			C	0.182	2.658		0.85	1	23.914			
T3	1	2	Α	0.167	2.71	10	0.85	1	26.967	1	59.02	A
140.00-120.00			В	0.167	2.71		0.85	1	26.967			
			C	0.167	2.71		0.85	1	26.967			
T4	1	3	Α	0.154	2.756	10	0.85	1	29.166	1	60.12	A
120.00-100.00			В	0.154	2.756		0.85	1	29.166			
			C	0.154	2.756		0.85	1	29.166			
T5	1	4	Α	0.129	2.851	10	0.85	1	26.983	1	58.17	A
100.00-80.00			В	0.129	2.851		0.85	1	26.983			
			C	0.129	2.851		0.85	1	26.983			
T6	1	4	Α	0.117	2.898	9	0.85	1	27.561	1	56.32	Α
80.00-60.00			В	0.117	2.898		0.85	1	27.561			
			C	0.117	2.898		0.85	1	27.561			
T7	1	5	Α	0.11	2.923	9	0.85	1	29.014	1	54.26	A
60.00-40.00			В	0.11	2.923		0.85	1	29.014			
			C	0.11	2.923		0.85	1	29.014			
Т8	1	6	Α	0.108	2.934	8	0.85	1	31.676	1	51.39	Α
40.00-20.00			В	0.108	2.934		0.85	1	31.676			
			C	0.108	2.934		0.85	1	31.676			
T9 20.00-0.00	1	6	Α	0.104	2.949	7	0.85	1	33.374	1	46.02	A
			В	0.104	2.949		0.85	1	33.374			
			C	0.104	2.949		0.85	1	33.374			
Sum Weight:	7	35						OTM	738 kip-ft	9		

### **Force Totals**

# Infinigy Engineering, LLP 26455 Rancho Parkway S.

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Load	Vertical	Sum of	Sum of	Sum of	Sum of	Sum of Torques
Case	Forces	Forces	Forces	Overturning	Overturning	
		X	Z	Moments, $M_x$	Moments, $M_z$	
	K	K	K	kip-ft	kip-ft	kip-ft
Leg Weight	24					
Bracing Weight	11					
Total Member Self-Weight	35			12	-16	
Total Weight	54			12	-16	
Wind 0 deg - No Ice		0	-33	-3370	8	42
Wind 90 deg - No Ice		28	-1	-98	-2952	16
Wind 180 deg - No Ice		0	32	3237	-16	-42
Member Ice	72					
Total Weight Ice	185			121	-165	
Wind 0 deg - Ice		0	-16	-1476	-158	13
Wind 90 deg - Ice		14	0	88	-1625	5
Wind 180 deg - Ice		0	16	1683	-166	-13
Total Weight	54			12	-16	
Wind 0 deg - Service		0	-13	-1295	12	16
Wind 90 deg - Service		11	0	-43	-1121	6
Wind 180 deg - Service		0	12	1233	3	-16

### **Load Combinations**

Comb.	Description
No.	
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 90 deg - No Ice
5	0.9 Dead+1.6 Wind 90 deg - No Ice
6	1.2 Dead+1.6 Wind 180 deg - No Ice
7	0.9 Dead+1.6 Wind 180 deg - No Ice
8	1.2 Dead+1.0 Ice+1.0 Temp
9	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
10	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
11	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
12	Dead+Wind 0 deg - Service
13	Dead+Wind 90 deg - Service
14	Dead+Wind 180 deg - Service

## **Maximum Member Forces**

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	170 - 160	Leg	Max Tension	7	2.	0	0
••	1,0 100	205	Max. Compression	9	-5	0	0
			Max. Mx	6	1	1	0
			Max. My	2	1	0	1
			Max. Vy	6	-1	0	0
			Max. Vx	4	1	0	0
		Diagonal	Max Tension	5	3	0	0
			Max. Compression	2	-3	0	0
			Max. Mx	11	-1	0	0
			Max. My	10	-1	0	0
			Max. Vy	11	0	0	0
			Max. Vx	10	0	0	0

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Section No.	Elevation ft	Component Type	Condition	Gov. Load	Axial	Major Axis Moment	Minor Axi Moment
		Secondary Horizontal	Max Tension	<u>Comb.</u> 6	1 1	<u>kip-ft</u> 0	kip-ft 0
		поптоппан	Max. Compression	3	-1	0	0
			Max. Mx	10	0	0	0
			Max. My	10	0	0	0
			Max. Vy	10	0	0	0
			Max. Vx	10	0	0	0
		Top Girt	Max Tension	3	1	0	0
		•	Max. Compression	6	-1	0	0
			Max. Mx	8	0	0	0
			Max. My	11	0	0	0
			Max. Vy	8	0	0	0
			Max. Vx	11	0	0	0
T2	160 - 140	Leg	Max Tension	7	20	0	0
			Max. Compression	2	-26	1	0
			Max. Mx	2	-26	1	0
			Max. My	4	-3	0	1
			Max. Vy	6	0	0	0
		D' 1	Max. Vx	6	1	0	1
		Diagonal	Max Tension	6	4	0	0
			Max. Compression Max. Mx	2 10	-5 1	0	0
			Max. My	10	1	0	0
			Max. Wy	10	0	0	0
			Max. Vx	9	0	0	0
Т3	140 - 120	Leg	Max Tension	7	47	-1	0
13	140 120	Leg	Max. Compression	2	-60	2	0
			Max. Mx	3	-59	2	0
			Max. My	2	22	-1	-2
			Max. Vy	6	-2	-1	0
			Max. Vx	4	2	0	0
		Diagonal	Max Tension	4	7	0	0
		· ·	Max. Compression	4	-7	0	0
			Max. Mx	11	2	0	0
			Max. My	9	0	0	0
			Max. Vy	11	0	0	0
			Max. Vx	9	0	0	0
T4	120 - 100	Leg	Max Tension	7	77	-2	0
			Max. Compression	2	-95	2	0
			Max. Mx	3	-93	3	0
			Max. My	2	30	-1	-2
			Max. Vy	6	0	-2	0
		D' 1	Max. Vx	2	0	-1	-2
		Diagonal	Max Tension	2	8	0	0
			Max. Compression	2	-8	0	0
			Max. Mx	11 11	2 -2	0	0
			Max. My Max. Vy	11	0	0	0
			Max. Vx	11	0	0	0
T5	100 - 80	Leg	Max Tension	7	100	-2	0
1.5	100 .00	LCg	Max. Compression	2	-122	3	-1
			Max. Mx	6	96	-3	1
			Max. My	2	46	-2	-5
			Max. Vy	6	-1	-2	0
			Max. Vx	2	-1	-1	-2
		Diagonal	Max Tension	7	13	0	0
		J	Max. Compression	2	-14	0	0
			Max. Mx	11	3	0	0
			Max. My	11	-4	0	0
			Max. Vy	11	0	0	0
			Max. Vx	11	0	0	0

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Section No.	Elevation ft	Component Type	Condition	Gov. Load	Axial	Major Axis Moment	Minor Axi: Moment
	v	- 1		Comb.	K	kip-ft	kip-ft
T6	80 - 60	Leg	Max Tension	7	133	-3	1
		Č	Max. Compression	2	-161	3	0
			Max. Mx	6	129	-3	0
			Max. My	2	63	-2	-5
			Max. Vy	9	0	-3	-1
			Max. Vx	2	-1	-2	-5
		Diagonal	Max Tension	2	13	0	0
			Max. Compression	2	-13	0	0
			Max. Mx	11	3	0	0
			Max. My	9	-1	0	Õ
			Max. Vy	11	0	0	0
			Max. Vx	9	0	0	0
T7	60 - 40	Leg	Max Tension	7	163	-3	0
- '	00 .0	206	Max. Compression	2	-196	5	-1
			Max. Mx	2	-196	5	-1
			Max. My	2	78	-2	-3
			Max. Vy	9	0	-4	-1
			Max. Vx	2	0	-2	-3
		Diagonal	Max Tension	7	13	0	0
		Diagonai	Max. Compression	2	-14	0	0
			Max. Mx	11	-14 4	-1	0
				11	-1	-1 -1	0
			Max. My			-1 -1	0
			Max. Vy	11 9	0	0	
Т8	40 - 20	T	Max. Vx Max Tension	9 7	0 193	-5	0 1
18	40 - 20	Leg					
			Max. Compression	2	-232	3	0
			Max. Mx	2	-230	5	-1
			Max. My	2	90	-2	-7
			Max. Vy	9	0	-4	-1
			Max. Vx	2	1	-2	-7
		Diagonal	Max Tension	2	14	0	0
			Max. Compression	2	-14	0	0
			Max. Mx	11	1	-1	0
			Max. My	11	-4	-1	0
			Max. Vy	11	0	-1	0
			Max. Vx	11	0	0	0
T9	20 - 0	Leg	Max Tension	7	220	-4	0
			Max. Compression	2	-264	0	0
			Max. Mx	9	-129	4	0
			Max. My	2	104	-2	-7
			Max. Vy	6	0	-4	0
			Max. Vx	2	-1	-2	-7
		Diagonal	Max Tension	6	14	0	0
		-	Max. Compression	2	-16	0	0
			Max. Mx	10	4	-1	0
			Max. My	9	4	-1	0
			Max. Vy	10	0	-1	0
			Max. Vx	9	0	0	0

## **Maximum Reactions**

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg C	Max. Vert	6	146	14	-10
	Max. H <sub>x</sub>	6	146	14	-10
	Max. H <sub>z</sub>	5	-185	-21	9

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Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, Z
		Load	K	K	K
		Comb.			
	Min. Vert	5	-185	-21	9
	Min. H <sub>x</sub>	5	-185	-21	9
	Min. H <sub>z</sub>	6	146	14	-10
Leg B	Max. Vert	4	215	-22	-10
C	Max. H <sub>x</sub>	3	-115	10	12
	Max. H <sub>z</sub>	3	-115	10	12
	Min. Vert	3	-115	10	12
	Min. H <sub>x</sub>	4	215	-22	-10
	Min. Hz	6	147	-12	-13
Leg A	Max. Vert	2	282	2	33
	Max. H <sub>x</sub>	2	282	2	33
	Max. H <sub>z</sub>	2	282	2	33
	Min. Vert	7	-233	-2	-29
	Min. H <sub>x</sub>	5	24	-3	2
	Min. Hz	7	-233	-2	-29

## **Tower Mast Reaction Summary**

Load Combination	Vertical	$Shear_x$	$Shear_z$	Overturning Moment, $M_x$	Overturning Moment, M <sub>z</sub>	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Dead Only	54	0	0	12	-16	0
1.2 Dead+1.6 Wind 0 deg - No	64	0	-53	-5413	20	67
Ice						
0.9 Dead+1.6 Wind 0 deg - No	48	0	-53	-5413	25	67
Ice						
1.2 Dead+1.6 Wind 90 deg - No	64	45	-1	-162	-4731	26
Ice						
0.9 Dead+1.6 Wind 90 deg - No	48	45	-1	-165	-4723	25
Ice						
1.2 Dead+1.6 Wind 180 deg -	64	0	51	5189	-19	-68
No Ice						
0.9 Dead+1.6 Wind 180 deg -	48	0	51	5182	-14	-68
No Ice						
1.2 Dead+1.0 Ice+1.0 Temp	196	0	0	125	-170	0
1.2 Dead+1.0 Wind 0 deg+1.0	196	0	-16	-1488	-162	14
Ice+1.0 Temp						
1.2 Dead+1.0 Wind 90 deg+1.0	196	14	0	92	-1644	6
Ice+1.0 Temp						
1.2 Dead+1.0 Wind 180	196	0	16	1701	-171	-14
deg+1.0 Ice+1.0 Temp						
Dead+Wind 0 deg - Service	54	0	-13	-1285	-7	16
Dead+Wind 90 deg - Service	54	11	0	-30	-1142	6
Dead+Wind 180 deg - Service	54	0	12	1249	-16	-16

## **Solution Summary**

	Sui	m of Applied Forces	ï		Sum of Reaction	ıs	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
1	0	-54	0	0	54	0	0.000%
2	0	-64	-53	0	64	53	0.001%
3	0	-48	-53	0	48	53	0.000%
4	45	-64	-1	-45	64	1	0.001%

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	Sui	m of Applied Forces	,		Sum of Reaction	ıs	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
5	45	-48	-1	-45	48	1	0.001%
6	0	-64	51	0	64	-51	0.001%
7	0	-48	51	0	48	-51	0.001%
8	0	-196	0	0	196	0	0.000%
9	0	-196	-16	0	196	16	0.000%
10	14	-196	0	-14	196	0	0.000%
11	0	-196	16	0	196	-16	0.000%
12	0	-54	-13	0	54	13	0.000%
13	11	-54	0	-11	54	0	0.000%
14	0	-54	12	0	54	-12	0.000%

### **Non-Linear Convergence Results**

Load	Converged?	Number	Displacement	Force
Combination		of Cycles	Tolerance	Tolerance
1	Yes	6	0.00000001	0.00000001
2	Yes	7	0.00000001	0.00006977
3	Yes	7	0.00000001	0.00004764
4	Yes	7	0.00000001	0.00009331
5	Yes	7	0.00000001	0.00007138
6	Yes	7	0.00000001	0.00010745
7	Yes	7	0.00000001	0.00008486
8	Yes	6	0.00000001	0.00012524
9	Yes	8	0.00000001	0.00003261
10	Yes	8	0.00000001	0.00003558
11	Yes	8	0.00000001	0.00003654
12	Yes	7	0.00000001	0.00006467
13	Yes	7	0.00000001	0.00006847
14	Yes	7	0.00000001	0.00007259

### **Maximum Tower Deflections - Service Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
T1	170 - 160	2.055	12	0.0944	0.0161
T2	160 - 140	1.851	12	0.0940	0.0161
T3	140 - 120	1.448	12	0.0889	0.0150
T4	120 - 100	1.072	12	0.0771	0.0142
T5	100 - 80	0.757	12	0.0641	0.0124
T6	80 - 60	0.487	12	0.0518	0.0095
T7	60 - 40	0.279	12	0.0374	0.0066
T8	40 - 20	0.126	12	0.0250	0.0037
Т9	20 - 0	0.033	12	0.0113	0.0018

### **Critical Deflections and Radius of Curvature - Service Wind**

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Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	۰	ft
170.00	15' lighting rod	12	2.055	0.0944	0.0161	Inf
165.00	6 FT DISH	12	1.953	0.0943	0.0161	Inf
160.00	6812	12	1.851	0.0940	0.0161	Inf
155.00	6812	12	1.749	0.0933	0.0159	686044
150.00	4 FT DISH	12	1.648	0.0923	0.0156	430825
140.00	SC2-W100AB	12	1.448	0.0889	0.0150	219327
110.00	2' Dish	12	0.908	0.0705	0.0134	88241
100.00	(4) SBNHH-1D65B w/ Mount Pipe	12	0.757	0.0641	0.0124	144515

## **Maximum Tower Deflections - Design Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
T1	170 - 160	8.635	2	0.3962	0.0675
T2	160 - 140	7.779	2	0.3946	0.0673
T3	140 - 120	6.087	2	0.3734	0.0629
T4	120 - 100	4.507	2	0.3243	0.0594
T5	100 - 80	3.184	2	0.2695	0.0518
T6	80 - 60	2.049	2	0.2179	0.0400
T7	60 - 40	1.172	2	0.1574	0.0275
T8	40 - 20	0.531	2	0.1051	0.0153
T9	20 - 0	0.137	2	0.0475	0.0075

## **Critical Deflections and Radius of Curvature - Design Wind**

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	٥	0	ft
170.00	15' lighting rod	2	8.635	0.3962	0.0675	699867
165.00	6 FT DISH	2	8.207	0.3958	0.0675	699867
160.00	6812	2	7.779	0.3946	0.0673	319885
155.00	6812	2	7.353	0.3920	0.0665	169575
150.00	4 FT DISH	2	6.927	0.3878	0.0654	104799
140.00	SC2-W100AB	2	6.087	0.3734	0.0629	52872
110.00	2' Dish	2	3.817	0.2964	0.0563	20993
100.00	(4) SBNHH-1D65B w/ Mount Pipe	2	3.184	0.2695	0.0518	34191

### **Bolt Design Data**

Section	Elevation	Component	Bolt	Bolt Size	Number	Maximum	Allowable	Ratio	Allowable	Criteria
No.	ft	Туре	Grade	in	Of Bolts	Load per Bolt	Load per Bolt	Load Allowable	Ratio	
T1	170	Leg	A325N	1.0000	6	<i>K</i> 0	K 53	/	1	Bolt Tension
11	1,0	Diagonal	A325N	1.0000	1	3	9	0.006	1	Member Block Shear

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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load per Bolt K	Ratio Load Allowable	Allowable Ratio	Criteria
		Top Girt	A325N	1.0000	1	1	9	0.062	1	Member Block Shear
T2	160	Leg	A325N	1.0000	6	3	53	0.064	1	Bolt Tension
		Diagonal	A325N	1.0000	1	4	9	0.473	1	Member Block Shear
T3	140	Leg	A325N	1.0000	6	8	53	0.148	1	Bolt Tension
		Diagonal	A325N	1.0000	1	7	10	0.656	1	Member Block Shear
T4	120	Leg	A325N	1.0000	6	13	53	0.243	1	Bolt Tension
		Diagonal	A325N	1.0000	1	8	17	0.443	1	Member Block Shear
T5	100	Leg	A325N	1.0000	12	8	53	0.158	1	Bolt Tension
		Diagonal	A325N	0.8750	1	13	20	0.635	1	Member Block Shear
T6	80	Leg	A325N	1.0000	12	11	53	0.209	1	Bolt Tension
		Diagonal	A325N	0.8750	1	13	20	0.647	1	Member Block Shear
T7	60	Leg	A325N	1.0000	12	14	53	0.256	1	Bolt Tension
		Diagonal	A325N	0.8750	1	13	20	0.649	1	Member Block Shear
Т8	40	Leg	A325N	1.0000	12	16	53	0.303	1	Bolt Tension
		Diagonal	A325N	0.8750	1	14	30	0.461	1	Member Bearing
Т9	20	Diagonal	A325N	0.8750	1	14	30	0.482	1	Member Bearing

## **Compression Checks**

## Leg Design Data (Compression)

Section No.	Elevation	Size	L	$L_u$	Kl/r	Α	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	K	K	$\phi P_n$
T1	170 - 160	Valmont 207628	10.02	5.34	45.0 K=1.00	3.6816	-5	143	0.038 1
T2	160 - 140	Valmont 207629	20.03	10.02	37.5 K=1.00	5.3014	-26	215	0.119 1
Т3	140 - 120	Valmont 207629	20.03	10.02	37.5 K=1.00	5.3014	-60	215	0.280 1
T4	120 - 100	Valmont 195557	20.03	10.02	31.9 K=1.00	7.2158	-95	301	0.314 1
T5	100 - 80	Valmont 211843	20.03	20.03	48.8 K=1.00	9.4248	-122	356	0.342 1
T6	80 - 60	Valmont 208333	20.03	20.03	48.8 K=1.00	9.4248	-161	356	0.451 1
Т7	60 - 40	Valmont 208334	20.03	20.03	48.8 K=1.00	11.9282	-196	451	0.434 1

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Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	K	K	$\phi P_n$
Т8	40 - 20	Valmont 208334	20.03	20.03	48.8 K=1.00	11.9282	-232	451	0.514 1
Т9	20 - 0	Valmont 208335	20.03	20.03	48.7 K=1.00	14.7262	-264	557	0.475 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

		Truss-Leg Diagonal Data									
Section No.	Elevation ft	Diagonal Size	$L_d$ ft	Kl/r	$\phi P_n$ $K$	A in <sup>2</sup>	$V_u$ $K$	$\phi V_n \ K$	Stress Ratio		
T1	170 - 160	0.5	1.48	120.4	166	0.1963	1	3	0.358		
T2	160 - 140	0.5	1.46	119.3	239	0.1963	0	3	0.087		
Т3	140 - 120	0.5	1.46	119.3	239	0.1963	2	3	0.589		
T4	120 - 100	0.5	1.44	117.6	325	0.1963	0	3	0.111		
T5	100 - 80	0.5	1.39	113.2	424	0.1963	1	4	0.398		
Т6	80 - 60	0.5	1.39	113.2	424	0.1963	1	4	0.148		
Т7	60 - 40	0.5	1.38	112.2	537	0.1963	1	4	0.138		
Т8	40 - 20	0.5	1.38	112.2	537	0.1963	1	4	0.168		
Т9	20 - 0	0.5	1.36	111.2	663	0.1963	1	4	0.202		

		Diagor	nal Des	sign [	Data (C	Compr	ession	)	
Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	K	K	$\Phi P_n$
T1	170 - 160	L 2-1/2x2-1/2x3/16	12.50	5.67	137.5 K=1.00	0.9023	-3	11	0.298 1
T2	160 - 140	L 2-1/2x2-1/2x3/16	13.80	6.37	154.4 K=1.00	0.9023	-5	9	0.543 1
Т3	140 - 120	L 3x3x3/16	15.24	7.12	143.3 K=1.00	1.0898	-7	12	0.556 <sup>1</sup>
T4	120 - 100	L 3x3x5/16	16.80	7.92	161.4 K=1.00	1.7773	-8	15	0.490 1
T5	100 - 80	2L3x3x3/16	25.01	12.35	157.9 K=1.00	2.1800	-14	20	0.701 1
Т6	80 - 60	2L3x3x3/16	26.26	12.98	165.8 K=1.00	2.1800	-13	18	0.744 ¹

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Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio $P_u$
	ft		ft	ft		$in^2$	K	K	$\phi P_n$
Т7	60 - 40	2L3x3x3/16	27.59	13.65	174.4 K=1.00	2.1800	-14	16	0.875 1
Т8	40 - 20	2L3 1/2x3 1/2x1/4	29.01	14.35	158.0 K=1.00	3.3800	-14	31	0.448 1
Т9	20 - 0	2L3 1/2x3 1/2x1/4	30.49	15.09	166.1 K=1.00	3.3800	-16	28	0.562 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

		Secondary H	orizon	tal De	esign	Data (0	Compr	ession)	
Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	K	K	$\phi P_n$
T1	170 - 160	L 2-1/2x2-1/2x3/16	7.47	3.23	99.2	0.9023	-1	17	0.057 1

K=1.27

		Top G	Top Girt Design Data (Compression)								
Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P.,		
1,0,	ft		ft	ft		$in^2$	K	K	$\frac{-1}{\phi P_n}$		
T1	170 - 160	L 2-1/2x2-1/2x3/16	7.00	5.67	137.4 K=1.00	0.9023	-1	11	0.054 1		

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

## Tension Checks

		L	eg Des	sign L	vata (	rensio	n)		
Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	K	K	$\phi P_n$
T1	170 - 160	Valmont 207628	10.02	4.67	45.0	3.6816	2	166	0.014 1
T2	160 - 140	Valmont 207629	20.03	10.02	37.5	5.3014	20	239	0.085 1
Т3	140 - 120	Valmont 207629	20.03	10.02	37.5	5.3014	47	239	0.197 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

# Infinigy Engineering, LLP 26455 Rancho Parkway S.

Lake Forest, CA 92630 Phone: (518) 690-0790 FAX: (518) 690-0790

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Client	Northeast Site Solutions	Designed by L. Mendoza

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	K	K	$\phi P_n$
T4	120 - 100	Valmont 195557	20.03	10.02	31.9	7.2158	77	325	0.238 1
T5	100 - 80	Valmont 211843	20.03	20.03	48.8	9.4248	100	424	0.237 1
Т6	80 - 60	Valmont 208333	20.03	20.03	48.8	9.4248	133	424	0.313 1
T7	60 - 40	Valmont 208334	20.03	20.03	48.8	11.9282	163	537	0.304 1
Т8	40 - 20	Valmont 208334	20.03	20.03	48.8	11.9282	193	537	0.359 1
Т9	20 - 0	Valmont 208335	20.03	20.03	48.7	14.7262	220	663	0.331 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

	Truss-Leg Diagonal Data									
Section No.	Elevation ft	Diagonal Size	$L_d$ ft	Kl/r	$\phi P_n$ $K$	A in <sup>2</sup>	V <sub>u</sub> K	$\phi V_n \ K$	Stress Ratio	
T1	170 - 160	0.5	1.48	120.4	166	0.1963	1	3	0.358	
T2	160 - 140	0.5	1.46	119.3	239	0.1963	0	3	0.087	
Т3	140 - 120	0.5	1.46	119.3	239	0.1963	2	3	0.589	
T4	120 - 100	0.5	1.44	117.6	325	0.1963	0	3	0.111	
T5	100 - 80	0.5	1.39	113.2	424	0.1963	1	4	0.398	
Т6	80 - 60	0.5	1.39	113.2	424	0.1963	1	4	0.148	
T7	60 - 40	0.5	1.38	112.2	537	0.1963	1	4	0.138	
Т8	40 - 20	0.5	1.38	112.2	537	0.1963	1	4	0.168	
T9	20 - 0	0.5	1.36	111.2	663	0.1963	1	4	0.202	

Diagonal Design Data (Tension)									
Section No.	Elevation	Size	L	$L_u$	Kl/r	Α	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	K	K	$\phi P_n$
T1	170 - 160	L 2-1/2x2-1/2x3/16	12.50	5.67	90.0	0.9023	3	29	0.103 1
T2	160 - 140	L 2-1/2x2-1/2x3/16	13.80	6.37	100.8	0.9023	4	29	0.148 1

# Infinigy Engineering, LLP 26455 Rancho Parkway S.

6455 Rancho Parkway S. Lake Forest, CA 92630 Phone: (518) 690-0790 FAX: (518) 690-0790

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Client	Northeast Site Solutions	Designed by L. Mendoza

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	K	K	$\phi P_n$
Т3	140 - 120	L 3x3x3/16	15.24	7.12	93.1	1.0898	7	35	0.189 1
T4	120 - 100	L 3x3x5/16	16.80	7.92	105.3	1.7773	8	58	0.130 1
T5	100 - 80	2L3x3x3/16	25.01	12.35	159.8	2.1800	13	71	0.181 1
Т6	80 - 60	2L3x3x3/16	26.26	12.98	167.8	2.1800	13	71	0.185 1
T7	60 - 40	2L3x3x3/16	27.59	13.65	176.3	2.1800	13	71	0.185 1
Т8	40 - 20	2L3 1/2x3 1/2x1/4	29.01	14.35	159.7	3.3800	14	110	0.125 1
Т9	20 - 0	2L3 1/2x3 1/2x1/4	30.49	15.09	167.8	3.3800	14	110	0.130 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

Secondar	y Horizontal	<b>Design D</b>	ata (To	ension)

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	K	K	$\phi P_n$
T1	170 - 160	L 2-1/2x2-1/2x3/16	7.47	3.23	99.7	0.9023	1	29	0.031 1
									./

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

## **Top Girt Design Data (Tension)**

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P <sub>u</sub>
	ft		ft	ft		$in^2$	K	K	$\phi P_n$
T1	170 - 160	L 2-1/2x2-1/2x3/16	7.00	5.67	92.5	0.9023	1	29	0.020 1

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

## **Section Capacity Table**

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow} \ K$	% Capacity	Pass Fail
T1	170 - 160	Leg	Valmont 207628	2	-4	143	35.8	Pass
T2	160 - 140	Leg	Valmont 207629	18	-26	215	11.9	Pass
T3	140 - 120	Leg	Valmont 207629	33	-43	215	58.9	Pass

# tnxTower

#### Infinigy Engineering, LLP 26455 Rancho Parkway S.

26455 Rancho Parkway S. Lake Forest, CA 92630 Phone: (518) 690-0790 FAX: (518) 690-0790

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Client	Northeast Site Solutions	Designed by L. Mendoza

Section	Elevation	Component	Size	Critical	P	$\phi P_{allow}$	%	Pass
No.	ft	Туре		Element	K	K	Capacity	Fail
T4	120 - 100	Leg	Valmont 195557	48	-95	301	31.4	Pass
T5	100 - 80	Leg	Valmont 211843	62	-94	356	39.8	Pass
T6	80 - 60	Leg	Valmont 208333	72	-161	356	45.1	Pass
T7	60 - 40	Leg	Valmont 208334	81	-196	451	43.4	Pass
T8	40 - 20	Leg	Valmont 208334	90	-232	451	51.4	Pass
T9	20 - 0	Leg	Valmont 208335	99	-264	557	47.5	Pass
T1	170 - 160	Diagonal	L 2-1/2x2-1/2x3/16	10	-3	11	29.8	Pass
		_					33.0 (b)	
T2	160 - 140	Diagonal	L 2-1/2x2-1/2x3/16	22	-5	9	54.3	Pass
T3	140 - 120	Diagonal	L 3x3x3/16	35	-7	12	55.6	Pass
		-					65.6 (b)	
T4	120 - 100	Diagonal	L 3x3x5/16	52	-8	15	49.0	Pass
T5	100 - 80	Diagonal	2L3x3x3/16	67	-14	20	70.1	Pass
T6	80 - 60	Diagonal	2L3x3x3/16	76	-13	18	74.4	Pass
T7	60 - 40	Diagonal	2L3x3x3/16	85	-14	16	87.5	Pass
T8	40 - 20	Diagonal	2L3 1/2x3 1/2x1/4	94	-14	31	44.8	Pass
		_					46.1 (b)	
T9	20 - 0	Diagonal	2L3 1/2x3 1/2x1/4	103	-16	28	56.2	Pass
T1	170 - 160	Secondary Horizontal	L 2-1/2x2-1/2x3/16	14	-1	17	5.7	Pass
T1	170 - 160	Top Girt	L 2-1/2x2-1/2x3/16	4	-1	11	5.4	Pass
							6.2 (b)	
							Summary	
						Leg (T3)	58.9	Pass
						Diagonal (T7)	87.5	Pass
						Secondary Horizontal (T1)	5.7	Pass
						Top Girt (T1)	6.2	Pass
						Bolt Checks	65.6	Pass
						RATING =	87.5	Pass

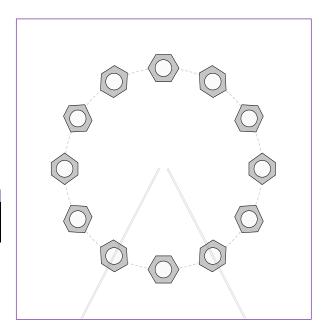
# **Self Support Anchor Rod Capacity**

Analysis Considerations				
TIA-222 Revision	G			
Grout Considered:	No			
I <sub>ar</sub> (in)	1			
Eta Factor, η	0.5			

Applied Loads						
	Comp.	Uplift				
Axial Force (kips)	282.00	233.00				
Shear Force (kips)	33.00	29.00				

Considered Eccentricity	
Leg Mod Eccentricity (in)	0.000
Anchor Rod N.A Shift (in)	0.000
Total Eccentricity (in)	0.000

<sup>\*</sup>Anchor Rod Eccentricity Applied



Connection Properties	Analysis Results			
Anchor Rod Data	Anchor Rod Summary		(units of kips, kip-in)	
(12) 1" ø bolts (F1554-105 N; Fy=105 ksi, Fu=125 ksi)	Pu_c = 23.5	φPn_t = 60.6	Stress Rating	
I <sub>ar</sub> (in): 1	Vu = 2.75	φVn = n/a	47.9%	
	Mu = n/a	φMn = n/a	Pass	

# **SST Unit Base Foundation**

TIA-222 Revision: G

Top & Bot. Pad Rein. Different?:	
Tower Centroid Offset?:	<b>~</b>
Block Foundation?:	
Rectangular Pad?:	

Superstructure Analysis Reactions					
Global Moment, M:	5413	ft-kips			
Global Axial, P:	64	kips			
Global Shear, V:	53	kips			
Leg Compression, P <sub>comp</sub> :	282	kips			
Leg Comp. Shear, V <sub>u_comp</sub> :	33	kips			
Leg Uplift, P <sub>uplift</sub> :		kips			
Leg Uplift. Shear, V <sub>u_uplift</sub> :	29	kips			
Tower Height, H:	170	ft			
Base Face Width, <b>BW</b> :	24	ft			
BP Dist. Above Fdn, <b>bp</b> <sub>dist</sub> :	3	in			

Foundation Analysis Checks					
	Capacity	Demand	Rating	Check	
Lateral (Sliding) (kips)	309.67	53.00	17.1%	Pass	
Bearing Pressure (ksf)	9.00	1.70	18.9%	Pass	
Overturning (kip*ft)	11222.84	6043.01	53.8%	Pass	
Pier Flexure (Comp.) (kip*ft)	1843.42	156.75	8.5%	Pass	
Pier Flexure (Tension) (kip*ft)	1088.09	137.75	12.7%	Pass	
Pier Compression (kip)	8998.02	295.60	3.3%	Pass	
Pad Flexure (kip*ft)	2609.92	227.73	8.7%	Pass	
Pad Shear - 1-way (kips)	607.92	80.24	13.2%	Pass	
Pad Shear - Comp 2-way (ksi)	0.190	0.084	44.1%	Pass	

Pier Properties					
Pier Shape:	Circular				
Pier Diameter, <b>dpier</b> :	4.5	ft			
Ext. Above Grade, E:	0.50	ft			
Pier Rebar Size, Sc:	7				
Pier Rebar Quantity, mc:	24				
Pier Tie/Spiral Size, <b>St</b> :	4				
Pier Tie/Spiral Quantity, mt:	5				
Pier Reinforcement Type:	Tie				
Pier Clear Cover. cc	3	in			

Structural Rating:	44.1%
Soil Rating:	53.8%

Pad Properties				
Depth, D:	6.00	ft		
Pad Width, <b>W</b> <sub>1</sub> :	32.00	ft		
Pad Thickness, T:	1.75	ft		
Pad Rebar Size (Bottom dir. 2), Sp <sub>2</sub> :	7			
Pad Rebar Quantity (Bottom dir. 2), mp <sub>2</sub> :	61			
Pad Clear Cover, cc <sub>pad</sub> :	3	in		

Material Properties			
Rebar Grade, Fy:	60	ksi	
Concrete Compressive Strength, F'c:	4	ksi	
Dry Concrete Density, δc:	150	ncf	

Soil Properties		
Total Soil Unit Weight, γ:	100	pcf
Ultimate Gross Bearing, Qult:	12.000	ksf
Cohesion, Cu:	0.000	ksf
Friction Angle, <b>ф</b> :	30	degrees
SPT Blow Count, N <sub>blows</sub> :		
Base Friction, <b>µ</b> :	0.45	
Neglected Depth, N:	3.3	ft
Foundation Bearing on Rock?	No	
Groundwater Depth, gw:	N/A	ft

<-- Toggle between Gross and Net

# Exhibit E

**Mount Analysis** 

# INFINIGY &

# MOUNT ANALYSIS REPORT

March 19, 2022

Dish Wireless Site Name	BOBDL00110C
Dish Wireless Site Number	BOBDL00110C
Infinigy Job Number	1197-F0001-B
Client	Northeast Site Solutions
Carrier	Dish Wireless
	33 Mitchell Drive
	Manchester, CT 06042
Site Location	Hartford County
	41° 47′ 50.3988″ N NAD83
	72° 30′ 42.4016″ W NAD83
Structure Type	Self-Support Tower
Structure Height	170.0 ft
Mount Type	8.0 ft Sector Frame
Mount Elevation	170.0 ft AGL
Structural Usage Ratio	41.7%
Overall Result	Pass

The enclosed structural analysis has been performed in accordance with the 2018 Connecticut State Building Code based on an ultimate 3-second gust wind speed of 125 mph. The evaluation criteria and applicable standards are presented in the next section of this report.



# Mount Analysis Report

# March 19, 2022

# CONTENTS

- 1. Introduction
- 2. Design/Analysis Parameters
- 3. Proposed Loading Configuration
- 4. Supporting Documentation
- 5. Results
- 6. Recommendations
- 7. Assumptions
- 8. Liability Waiver and Limitations
- 9. Calculations

#### March 19, 2022

#### 1. INTRODUCTION

Infinigy performed a structural analysis on the Dish Wireless proposed telecommunication equipment supporting Sector Frame mounted to the existing structure located at the aforementioned address. All referenced supporting documents have been obtained from the client and are assumed to be accurate and applicable to this site. The mount was analyzed using RISA 3D version 17.0.4 analysis software.

#### 2. DESIGN/ANALYSIS PARAMETERS

Ultimate Wind Speed	125 mph (3-Second Gust)
Nominal Wind Speed	97 mph (3-Second Gust)
Wind Speed w/ ice	50 mph (3-Second Gust) w/ 1 ice / No Ice Loading Considered
Adopted Code	2018 Connecticut State Building Code
Standard(s)	TIA-222-G
Risk Category	
Exposure Category	C
Topographic Factor	1.0
Seismic Spectral Response	$S_s = 0.187 \text{ g/} S_{1} = 0.064 \text{ g}$

#### 3. PROPOSED LOADING CONFIGURATION - 170.0 ft. AGL Sector Frame

Centerline (ft)	Qty.	Appurtenance Manufacturers	Appurtenance Models
	3	JMA Wireless	MX08FRO665-21
170.0	3	FUJITSU	TA08025-B605
170.0	3	FUJITSU	TA08025-B604
	1	RAYCAP	RDIDC-9181-PF-48

#### 4. SUPPORTING DOCUMENTATION

Construction Drawings	Infinigy dated March 7, 2022
Design Drawings	Commscope MTC3975083

#### 5. RESULTS

Components	Capacity	Pass/Fail
Mount Pipe	8.1%	Pass
Tieback	9.1%	Pass
Standoff Horizontal	41.7%	Pass
Bracing Member	21.4%	Pass
Face Horizontal	13.2%	Pass
RATING =	41.7%	Pass

#### Notes:

- 1. See additional documentation in Appendix for calculations supporting the capacity consumed and detailed mount connection calculations.
- 2. Results table usages reflect worst case sector mount.

# Mount Analysis Report

March 19, 2022

#### 6. RECOMMENDATIONS

Infinigy recommends installing Dish Wireless's proposed equipment loading configuration on the Sector Frame at 170.0 ft. The installation shall be performed in accordance with the construction documents issued for this site.

Luis Mendoza
Director of Structural Engineering | INFINIGY

March 19, 2022

#### 7. ASSUMPTIONS

The antenna mounting system was properly fabricated, installed and maintained in accordance with its original design and manufacturer's specifications.

The configuration of antennas, mounts, and other appurtenances are as specified in the proposed loading configuration table.

All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.

The analysis will require revisions if the existing conditions in the field differ from those shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.

Steel grades have been assumed as follows, unless noted otherwise: Channel, Solid Round, Angle, Plate ASTM A36

HSS (Rectangular)

ASTM A500-B GR 46

HSS (Circular)

ASTM A500-B GR 42

Pipe

ASTM A53-B GR 35

Connection Bolts ASTM A325 U-Bolts ASTM A307

All bolted connections are pretensioned in accordance with Table 8.2 of the RCSC 2014 Standard.

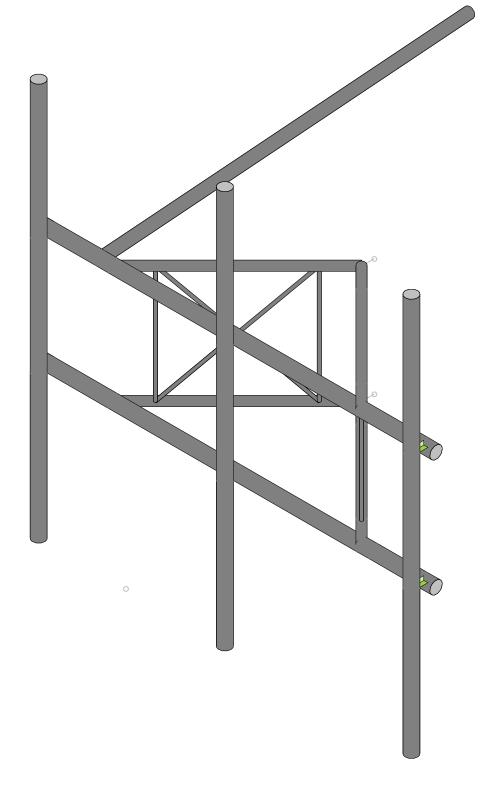
#### 8. LIABILITY WAIVER AND LIMITATIONS

Our structural calculations are completed assuming all information provided to Infinigy is accurate and applicable to this site. For the purposes of calculations, we assume an overall structure condition as erected and all members and connections to be free of corrosion and/or structural defects. The structure owner and/or contractor shall verify the structure's condition prior to installation of any proposed equipment. If actual conditions differ from those described in this report, Infinigy should be notified immediately to assess the impact on the results of this report.

Our evaluation is completed using industry standard methods and procedures. The structural results, conclusions and recommendations contained in this report are proprietary and should not be used by others as their own. Infinigy is not responsible for decisions made by others that are or are not based on the stated assumptions and conclusions in this report.

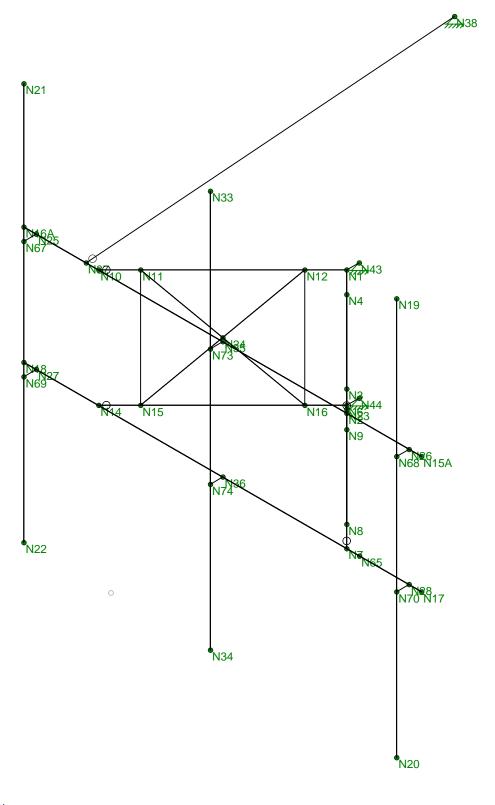
This report is an evaluation of the mount structure only and does not determine the adequacy of the supporting structure, other carrier mounts or cable mounting attachments. The analysis of these elements is outside the scope of this analysis, are assumed to be adequate for the purpose of this report and to have been installed per their manufacturer requirements. This document is not for construction purposes.





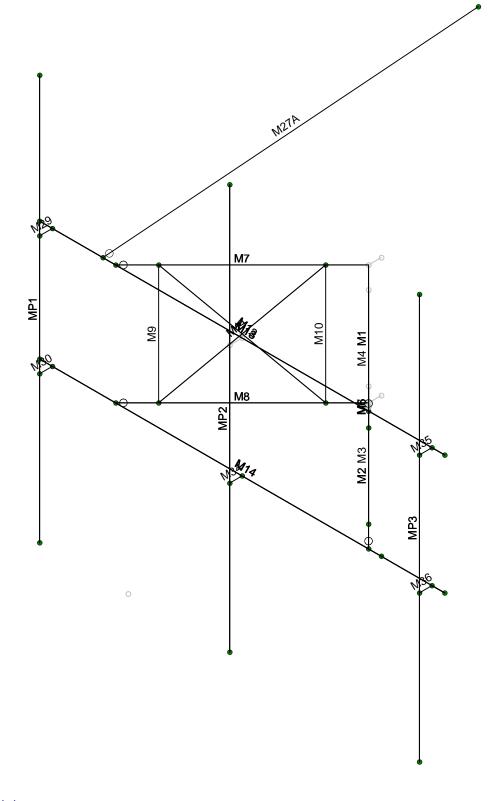
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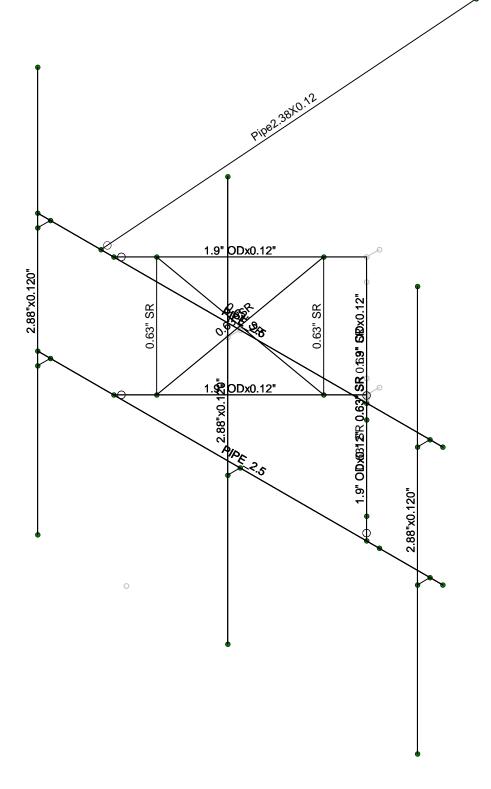
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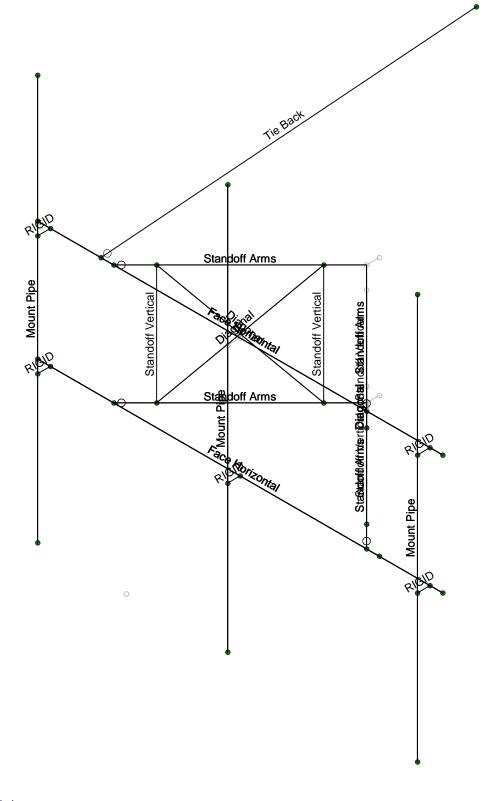
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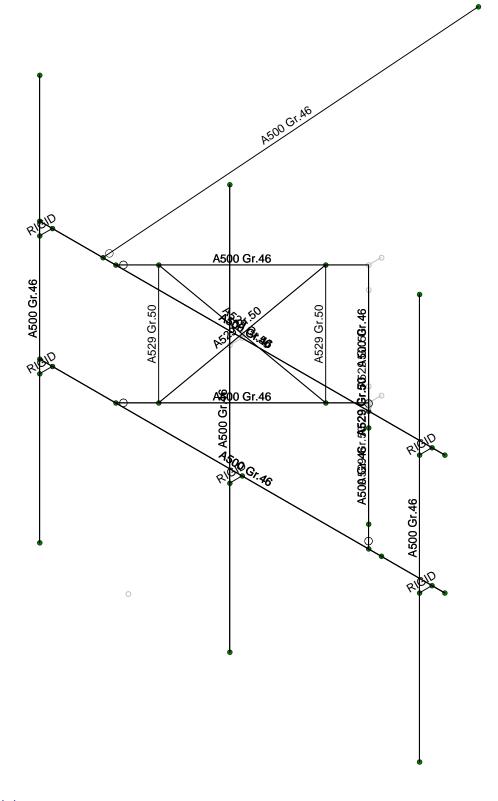
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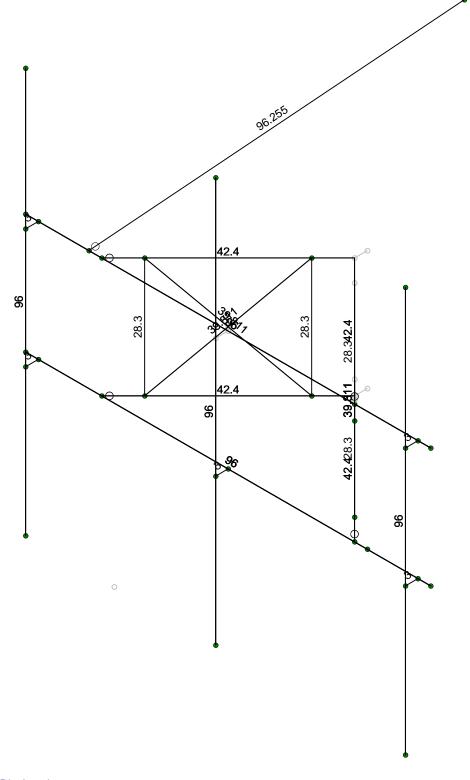
Infinigy		SK - 12
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		BOBDL00110C_loaded.r3d





Infinigy		SK - 5
	BOBDL00110C	Mar 23, 2022 at 10:40 PM
		BOBDL00110C_loaded.r3d

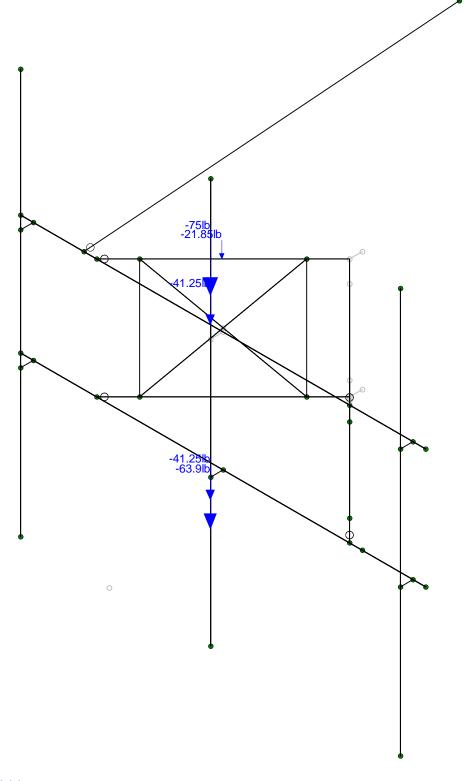




Member Length (in) Displayed Envelope Only Solution

Infinigy		SK - 6
	BOBDL00110C	Mar 23, 2022 at 10:40 PM
		BOBDL00110C_loaded.r3d

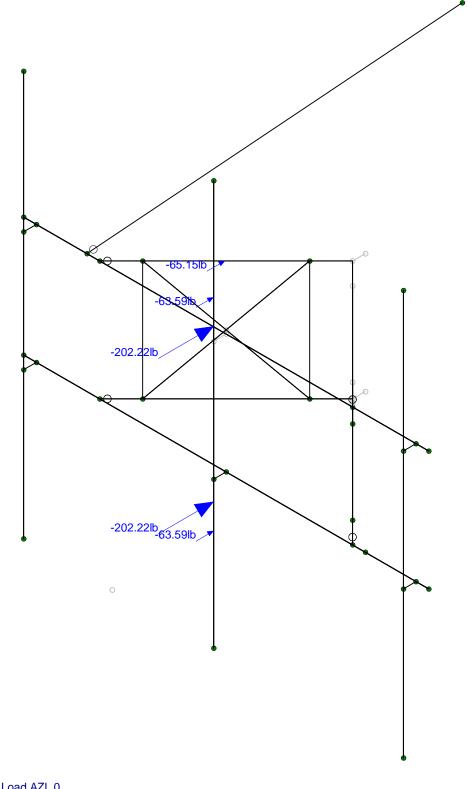




Loads: BLC 1, Self Weight Envelope Only Solution

Infinigy		SK - 7
	BOBDL00110C	Mar 23, 2022 at 10:40 PM
		BOBDL00110C_loaded.r3d

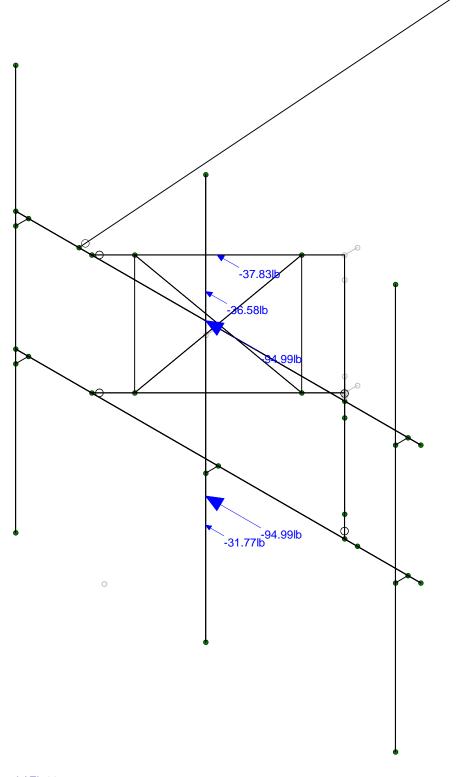




Loads: BLC 2, Wind Load AZI 0 Envelope Only Solution

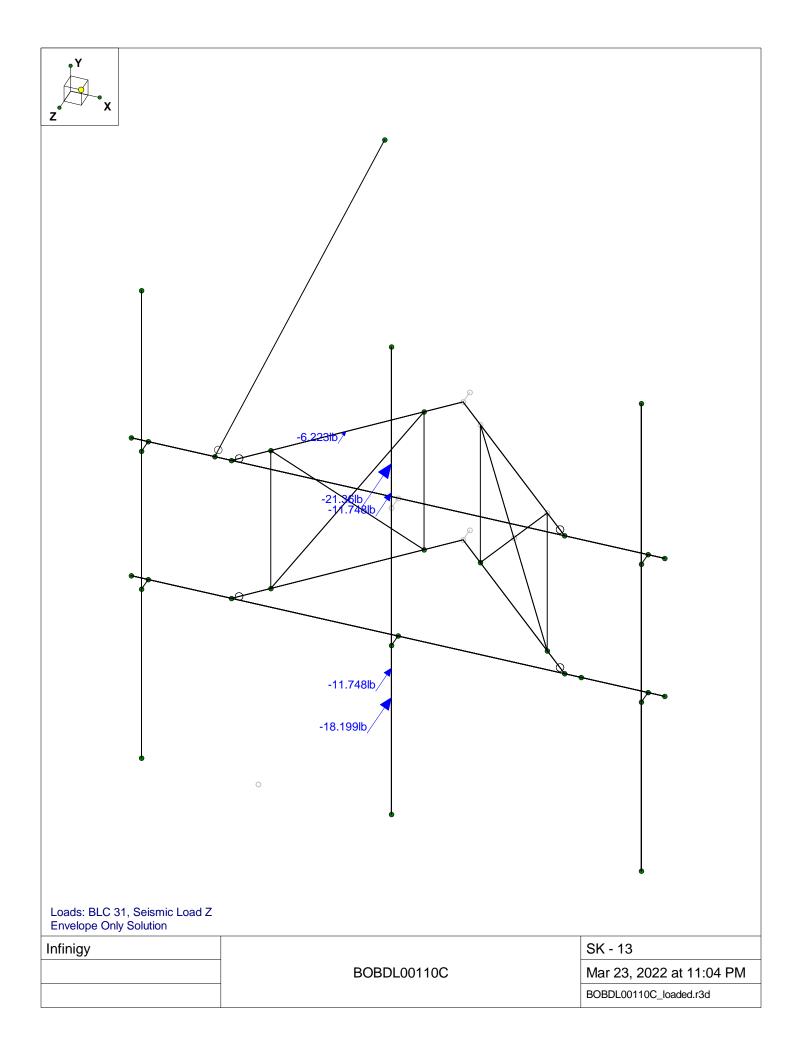
Infinigy		SK - 8
	BOBDL00110C	Mar 23, 2022 at 10:41 PM
		BOBDL00110C_loaded.r3d



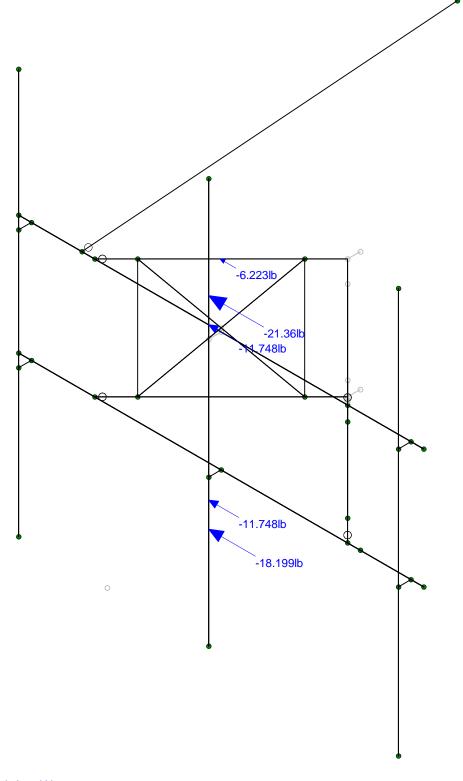


Loads: BLC 5, Wind Load AZI 90 Envelope Only Solution

Infinigy		SK - 9
	BOBDL00110C	Mar 23, 2022 at 10:41 PM
		BOBDL00110C_loaded.r3d







Loads: BLC 32, Seismic Load X Envelope Only Solution

Infinigy		SK - 11
	BOBDL00110C	Mar 23, 2022 at 10:42 PM
		BOBDL00110C_loaded.r3d

# **Program Inputs**

PROJECT INFORMATION		
Client:	Northeast Site Solutions	
Carrier:	Dish Wireless	
Engineer:	Luis Mendoza, PE	

SITE INFORMATION			
Risk Category:	II		
Exposure Category:	С		
Topo Category:	: 1		
Site Class:	D - Stiff Soil (Assumed)		
Ground Elevation:	N/A	ft *Rev H	

MOUNT INFORMATION			
Mount Type: Sector Frame			
Num Sectors:	3		
Centerline AGL:	170.00	ft	
Tower Height AGL:	170.00	ft	

TOPOGRAPHIC DATA			
Topo Feature:	N,	/A	
Slope Distance:	N/A	ft	
Crest Distance:	N/A	ft	
Crest Height:	N/A	ft	

FACTORS		
Directionality Fact. (K <sub>d</sub> ):	0.950	
Ground Ele. Factor (K <sub>e</sub> ):	N/A	*Rev H Only
Rooftop Speed-Up (K <sub>s</sub> ):	N/A	*Rev H Only
Topographic Factor (K <sub>zt</sub> ):	1.000	
Gust Effect Factor (G <sub>h</sub> ):	1.000	

CODE STANDARDS		
Building Code:	2015 IBC	
TIA Standard:	TIA-222-G	
ASCE Standard:	ASCE 7-10	

WIND AND	ICE DATA	
Ultimate Wind (V <sub>ult</sub> ):	125	mph
Design Wind (V):	97	mph
Ice Wind (V <sub>ice</sub> ):	50	mph
Base Ice Thickness (t <sub>i</sub> ):	1	in
Flat Pressure:	64.767	psf
Round Pressure:	38.860	psf
Ice Wind Pressure:	10.325	psf

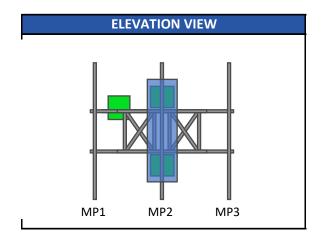
SEISMIC	CDATA	
Short-Period Accel. (S <sub>s</sub> ):	0.178	g
1-Second Accel. (S <sub>1</sub> ):	0.064	g
Short-Period Design (S <sub>DS</sub> ):	0.190	
1-Second Design (S <sub>D1</sub> ):	0.102	
Short-Period Coeff. (F <sub>a</sub> ):	1.600	
1-Second Coeff. (F <sub>v</sub> ):	2.400	
Amplification Factor (A <sub>s</sub> ):	3.000	
Response Mod. Coeff. (R):	2.000	

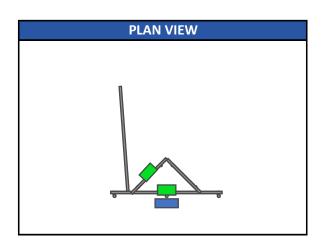


Infinigy Load Calculator V2.1.7

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# **Program Inputs**



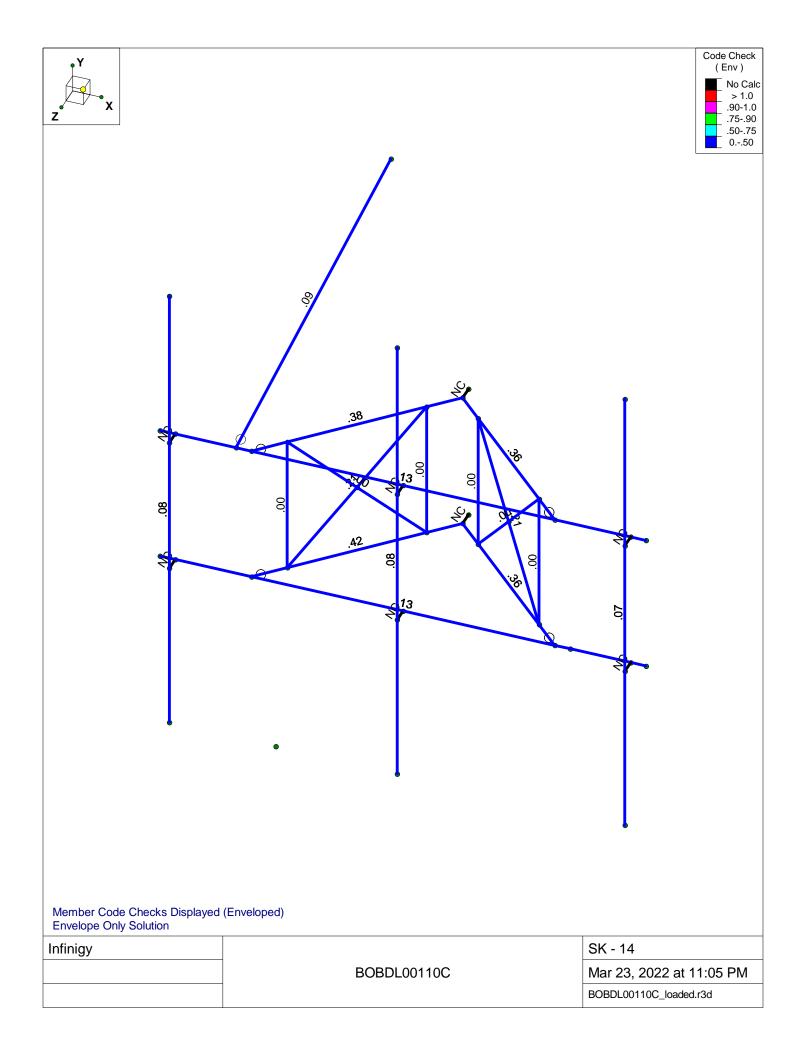


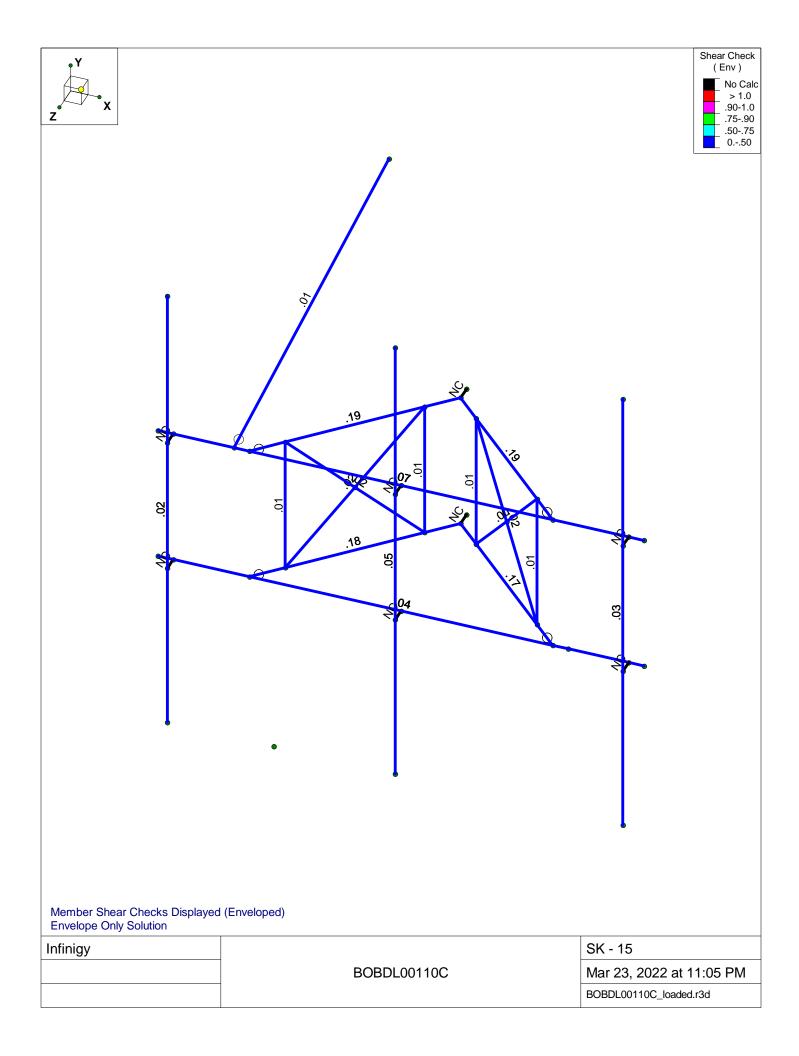


Infinigy Load Calculator V2.1.7

			APPURT	ENANCE IN	FORMATION						
Appurtenance Name	Elevation	Qty.	K <sub>a</sub>	q <sub>z</sub> (psf)	EPA <sub>N</sub> (ft <sup>2</sup> )	EPA <sub>T</sub> (ft <sup>2</sup> )	Wind F <sub>z</sub>	Wind F <sub>x</sub>	Weight	Seismic	Member
	2.000.0	_	a				(lbs)	(lbs)	(lbs)	F (lbs)	(α sector)
JMA WIRELESS MX08FRO665-21	170.0	3	1.00	32.38	12.49	5.87	404.44	189.98	82.50	23.50	MP2
FUJITSU TA08025-B604	170.0	3	1.00	32.38	1.96	0.98	63.59	31.77	63.90	18.20	MP2
FUJITSU TA08025-B605	170.0	3	1.00	32.38	1.96	1.13	63.59	36.58	75.00	21.36	MP2
RAYCAP RDIDC-9181-PF-48	170.0	3	1.00	32.38	2.01	1.17	65.15	37.83	21.85	6.22	M7

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# A Ya VYf Df Ja Ufmi8 UHU

	Šæèn^∣	OÁR[ãjc	RÁR[ã}c	SÁR[ã]c	Ü[ ææ^ <b>(ﷺ</b>		V^]^	Ö^∙ãt}ÆŠãc	Tæc∿¦ãæ	Ö^• ã} ÁÜÈÈ
F	TF	ÞG	ÞF			Ùœ); å[~ÁŒ{ •	Þ[}^	Þ[}^	OÉÆÃÕ¦ÈÌÎ	V^]ã&æ
G	TG	ÞÏ	ÞÎ			Ùœ)å[~ÁŒ{•	þ[}^	þ[}^	OÉ €€ÁÕ¦ÈÌÎ	V^] ã&æ
Н	TH	ÞH	ÞÌ			Ùca); å[~-ÁK^¦cã&a	Þ[}^	þ[}^	OÉ GJÁÕ¦Ě€	V^]
1	TI	ÞI	ÞJ			Ùcæ)å[~-ÁK^¦cã&æ)	Þ[}^	þ[}^	OÉGJÁÕ¦Ě€	V^] ã&æ
ĺ	Τĺ	ÞI	ÞÌ			Öãet [}æ	Þ[}^	Þ[}^	OÉGJÁզɀ	V^] ã&æ
Î	ΤÎ	ÞH	ÞJ			Öãæ* [ } æ∳	Þ[}^	þ[}^	OÉGJÁզɀ	V^] ã&æ
Ϊ	ΤÏ	ÞF€	ÞF			Ùœ) å[~ÁŒ{ •	Þ[}^	þ[}^	OÉÆÃÕ¦ÈÌÎ	V^] ã&æ
ì	ΤÌ	ÞFI	ÞÎ			Ùœ)å[~ÁŒ{•	Þ[}^	þ[}^	OÉ €€ÁÕ¦ÈÌÎ	V^] ã&æ
J	TJ	ÞFF	ÞFÍ			Ùcæ)å[~-ÁK^¦cã&æ)	Þ[}^	þ[}^	OÉGJÁÕ¦Ě€	V^] ã&æ
F€	TF€	ÞFG	ÞFÎ			Ùcæ)å[~-ÁK^¦cã&æ)	Þ[}^	þ[}^	OÉGJÁÕ¦Ě€	V^] ã&æ
FF	T FF	ÞFG	ÞFÍ			Öãeť[}æ	Þ[}^	þ[}^	OÉGJÁÕ¦Ě€	V^] ã&æ
FG	T FG	ÞFF	ÞFÎ			Öãæ≛[}æ⊧	Þ[}^	þ[}^	OÉGJÁÕ¦Ě€	V^] ã&æ
FH	T FH	ÞFÎŒ				Øæ&∧ÁP[¦ã[}œe	Þ[}^	þ[}^	OÉÆÃÕ¦ÈÌÎ	V^] ã&æ
FI	T FI	ÞFÌ	ÞFÏ			Øæ&∧ÁP[¦ã[}œe;	Þ[}^	þ[}^	OÉ €€ÁÕ¦ÈÌÎ	V^] ã&æ
FÍ	T ÚF	ÞŒ	ÞŒ			T [ ˇ } cÁÚą] ^	PÓ¦æ&^	Úą^	OÉÆÃÕ¦ÈÌÎ	V^] ã&æ
FÎ	ΤÚΗ	ÞŒ	ÞFJ			T[~}cÁÚą]^	PÓ¦æ&^	Úą^	OÉ €€ÁÕ¦ÈÌÎ	V^] ã&æ
FΪ	TÚG	ÞH	ÞН			T[~}cÁÚą]^	PÓ¦æ&^	Úą^	OÉÆÃÕ¦ÈÌÎ	V^] ã&æ
FÌ	ΤĠŒ	ÞHÏ	ÞĤ			Va\ÁÓa&\	þ[}^	Þ[}^	OÉÆÃÕ¦ÈÌÎ	V^]
FJ	TGJ	ÞĞ	ÞÎÏ			ÜÕÖÖ	Þ[}^	þ[}^	ÜÕÖÖ	V^] ã&æ
G€	TH€	ÞĞ	ÞÎJ			ÜÕÖÖ	Þ[}^	þ[}^	ÜÕÖÖ	V^] ã&æ
Œ	THH	ÞÁ	ÞÏH			ÜÕÕ	Þ[}^	þ[}^	ÜÕÖÖ	V^] ã&æ
GG	ΤH	ÞĤ	ÞÏI			ÜÕÖÖ	þ[}^	þ[}^	ÜÕÖÖ	V^]
GH	ΤHÍ	ÞĜ	ÞÎÌ			ÜÕÖÖ	þ[}^	Þ[}^	ÜÕÖÖ	V^]
G	ΤHÎ	ÞĠ	ÞÏ€			ÜÕÖÖ	þ[}^	Þ[}^	ÜÕÖÖ	V^]
GÍ GÎ	ΤĠ	ÞF	ÞН			ÜÕÖÖ	þ[}^	Þ[}^	ÜÕÖÖ	V^]
Ĝ	ΤĠ	ÞÎ	ÞH			ÜÕÖ	þ[}^	Þ[}^	ÜÕÖÖ	V^]

# A Unyf]U HU\_YcZZ

	Tæe^¦ãæ‡	Ùã^	Úã\&^•	Š^}*c@Ž <b>a</b> já	Y^ãt@žŠá
F	Õ^} ^¦æ				
G	ÜÕÖÖ		Ì	G	€
Н	V[œ(ÁÕ^}^¦æ(		Ì	G	€
1					
ĺ	P[cÁÜ[∥^åÁÛc^^				
Î	OÉ €ÉÁÕ¦ÈÎ	FÈÄŰÖ¢€ÈGÄ		FÎJĒ	È€HG
Ϊ	OÉ €€ÁÕ¦ÈÌÎ	ÚQÚÒ′ GĚ	G	FJG	Èì
Ì	OÉ €€ÁÕ¦ÈÌÎ	GÈÌÄ¢€ÈG€Ä	Н	Ġì	ÈÌÍ
J	OÉ €€ÁÕ¦ÈÌÎ	Ú∄, ^GÌHÌ Ý€ÌFG	F	JÎ ÈH	ÈEGH
F€	OÉGJÁÕ¦Ě€	€Ē HÄĄĴÜ	Ì	GÏ GÈ	ÈEG
FF	V[œ∳ÁPÜÁÛc^^		FÌ	F€FÌÈH	ÈGÍ G

# 6 Ug]W@: UX'7 UgYg

	ÓŠÔÁÖ^∙&¦ą[a[{}]	Ôæe^*[¦^	ÝÁÕ¦æçãcî	ŸÁŐ¦æçãcî	ZÁŐ¦æçãcî	R[ã]c	Ú[ặc	Öã dãa čdĚ	ÊCE^æÇT ÈË	:Ù`¦æ&^∰
F	Ù^ -ÁY ^	ÖŠ		Ë			ĺ			
G	YajåÁŠ[æåÁOEZOÁK€	Y ŠZ					F€			
Н	YājåÁŠ[æåÁOEZOÁÁH€	Þ[}^					F€			
1	YāļåÁŠ[æåÁOEZOÁÁÍ€	Þ[}^					F€			
ĺ	YājåÁĞ[æåÁOEZŒÁJ€	Y ŠÝ					F€			

#### 6 Ug]W@ UX'7 UgYg'ff cbhjbi YXŁ

	ÓŠÔÁÖ^•&¦ājcāj}	Ôæz^*[¦^	ÝÁÕ¦æçãcî	ŸÁÕ¦æçãcî	ZÁÕ¦æçãcî	R[ã]c	Ú[ặc	Öãrdaã d	₩OH^æQT HH	Ù`¦æ&∧ÈÈ
Î	YāļåÁŠ[æåÁOEZOÁÁFG€	Þ[}^					F€			
Ï	YajåÁŠ[æåÁOZOÁÁFÍ€	Þ[}^					F€			
Ì	YājåÁŠ[æåÁOEZOÁÁFÌ€	Þ[}^					F€			
J	YajåÁŠjæåÁOEZOÁKGF€	Þ[}^					F€			
F€	YājåÁŠ[æåÁOEZOÁKGI€	Þ[}^					F€			
FF	YajåÁŠjæåÁOEZOÁKGi€	Þ[}^					F€			
FG	YājåÁŠ[æåÁOEZOÁÁH€€	Þ[}^					F€			
FH	YājåÁŠ[æåÁOEZOÁÁHH€	Þ[}^					F€			
FI	Öãrd ÉÁY ã) åÁŠ[æåÁZ	Y ŠZ						G		
FÍ	Öãrd ĐÁY đị ả ÁŠĮ anà ÁÝ	Y ŠÝ UŠF						G		
FÎ	<b>@</b> ∧ÁY ^ãt @c	UŠF					ĺ	G		
FΪ	O&^ÁY ajáÁŠ[æåÁOEZOÁK€	UŠG					F€			
FÌ	O&AÁY ajáÁŠ[æáÁÐEZOÁÁH€	Þ[}^					F€			
FJ	O&∧ÁYajakŠ[ænák0EZOÁÁ€	Þ[}^					F€			
G€	O&∧ÁYajåÁŠ[æåÁOEZOÁÁJ€	UŠH					F€			
GF	O&^ÁYa}åÁŠ[æåÁOEZOÁÁFG€	Þ[}^					F€			
GG	O&∧ÁYa}åÁŠ[æåÁOEZOÁNFÍ€	Þ[}^					F€			
GH	O&^ÁYa}åÁŠ[æåÁOEZOÁNFÌ€	Þ[}^					F€			
G	O&^ÁY ∄åÁŠ[æåÁOEZOÁÁGF€	Þ[}^					F€			
GÍ	O&^ÁYa}åÁŠ[æåÁOEZOÁÁGI€	Þ[}^					F€			
Ĝ	Qa∧ÁY ajåÁŠ[æåÁOEZQÁÁGÏ€	Þ[}^					F€			
ĞÏ	O&^ÁY ā}åÁŠ[æåÁOEZOÁÁH€€	Þ[}^					F€			
GÌ	O&^ÁY ∄,åÁŠ[æåÁOEZOÁÁHH€	Þ[}^					F€			
GJ	ÖãrdÉAQA∧ÁY ãjáÁS[æáÁZ	UŠG						G		
H€	Öãrd ÈÁO&∧ÁY ãja ÁŠ[æåÁÝ	UŠH						G		
HF	Ù^ãr{ 88√ÁŠ[æåÁZ	ÒŠZ			ËĠÍ		ĺ			
HG	Ù^ãr{ ã&ÁŠ[æåÁÝ	ÒŠÝ	⊞GÌÍ				ĺ			
HH	Ù^¦çã&^ÆŠãç^ÆŠ[æå•	ŠŠ								

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	Ö^• &¦āj cāi}	Ù[ È	Ě	ÙЩα	ŠÔ	Øæ&H	ĎŠÔ	Øæ&À	<b>T</b> ĎŠĈ	Øæ&H	ĎŠĆ	)Øæ&H	ĎŠÔ	Øæ&H	ĎŠÔ	Øæ&H	<b>İ</b> ŠÔ	Øæ&H	<b>T</b> ĎŠĈ	Øæ&H	ĎŠÔ	Øæ&È	ĎŠÔ	Øæ&III
F	FÈ ÖŠ	Ÿ^•	Ϋ		F	FÈ																		
G	FÉGÖŠÁÉÁFÉ Y ŠÁOZ ÓÆ		Ϋ		F	Ħ	G	FÈ	FI	F₿	FÍ													
Н	FÉGÖŠÆÆÆFÉÌYŠÆOZOÁH€	Ÿ^•	Ϋ		F	ÈΘ	Н	FÈ	FI	FÈHÈ	FÍ	È												
1		Ÿ^•	Ϋ		F	É	1	FÈ	FI	È	Fĺ	FÈHÈ												
ĺ	I EDOOMEN E I ONGE GENE	Ÿ^•	Ϋ		F	É	ĺ	FÈ	FI		Fĺ	ΓÈ												
Î	FÈGÖŠÆÆFERYŠÆOZØFG€	1	Y		F	É	Î	FÈ	FI	Ë	Fĺ	FÈHÈ												
Ï	FÈGÖŠÆÁFÉÌYŠÁOZÓÁFÍ€		Ϋ		F	FÈG	Ϊ	FÈ	FI	ËÈÀ	Ŧĺ	È												
Ì	FÉGÖŠÆÁFÉÌYŠÁOZÓÁFÌ€		Ϋ		F	FÈG	Ì	FÈ	FI	ËÈ	FÍ													
J	FÈGÖŠÆÆÆFĒÌYŠÁOZÓÁGF€		Ϋ		F	FÈG	J	FÈ	FI	<del>Ë È À</del>	¥Í	Ë												
F€	FÈGÖŠÆÁFÉÌYŠÁOZQÁGI€				F	FÈG	F€	FÈ	FI	Ë	FÍ	ËÈÀ	È											
FF	FÈGÖŠÆÉÁFÉÌYŠÁOZQÁGÏ€		Ϋ		F	FÈG	FF	FÈ	FI		FÍ	ËÈ												
FG	FÉGÖŠÆÁFÉÌY ŠÁOZOÁH€€		Ϋ		F	FÈG	FG	FÈ	FI	È	FÍ	ËÈÀ	È											
FH	FÈGÖŠÆÁFÉÌYŠÁOZOÁHH€	Ÿ^•	Ϋ		F	FÈG	FΗ	FÈ	FI	FÈHÈ	FÍ	Ë												
FI	€ÈÖŠÁÉÁFĒY ŠÁOEZØÆ		Ϋ		F	Á	G	FÈ	FI	FÈ	FÍ													
FÍ	€ÈIÖŠÆÉÁFEÌYŠÁOZOÁH€	Ÿ^•	Ϋ		F	Á	Н	FÈ	FI	FÈHÈ	FÍ	È												
FÎ	€ÈÖŠÆÆÆFĒYŠÆOZOÂS€	Ÿ^•	Ϋ		F	Á	Τ	FÈ	FI	È	FÍ	FÈHÈ												
FΪ	€ÈIÖŠÆÉÁFEÌYŠÁOZOÁJ€	Ϋ́^•	Ϋ		F	Á	ĺ	FÈ	FI		FÍ	FÈ												
FÌ	€ÈÖŠÆÆÆFÈÈYŠÆOZØÆG€	Ÿ^•	Ϋ		F	Á	Î	FÈ	FI	H	FÍ	FÈHÈ												
FJ	(E)ÖŠÆÆÆÆÆ ÝŠÆOZÓÆFÍ€	Ÿ^•	Ϋ		F	Á	Ϊ	FÈ	FI	ËÈÀ	ŧří	È												

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			ĎĖ ÙĖ		•	DSC					Øæ&⊞	DSO	Øæ&III	980	Øæ&Œ	DSO <sub></sub>	Øæ&Œ	DSO	Øæ&H	<b>E</b> SO	Øæ&⊞	<u> DSO</u>	Øæ&III
G€	EDÖŠÆÆÆFĒYŠÁOZÓÆÌ€Ÿ		Y	F	É		ΕĒ		ËĒ,														
GF	eÈ ÖŠÆÆÆBÈΥŠÆOZÓAGF€Ÿ		Ÿ	F	Ð	J	FÊ	FI	ËÈÒ														
GG	(E)ÖŠÆÆÆFĒYŠÁOZÓAGE Ÿ		Ÿ	F	À		FÈ	FI	Ħ		ËÈÀ	Ë											
GH	(HE)ÖŠÁÉÁFÉÍYŠÁOZÓÁGÍ€ Ÿ		Ϋ	F	È			FI		Fĺ	ËΕÈ												
G	<del>€È</del> )ÖŠÆÉÁFEÌYŠÁOZOÁH€€  Ÿ	<b>′</b> ∧•	Ϋ	F	Ą	FG	FÈ	FI	È	FÍ	ËÈÀ	Щ											
GÍ	(€E)ÖŠÆÉÁFEÌYŠÁOZOÁHH€ Ÿ	<b>′</b> ∧•	Ϋ	F	À		FÈ	FI	FÈHÈ	FÍ	Ë												
GÎ	F <b>Ì</b> EÇÖÆÆÆFÈEÖã Ÿ	<b>/∧•</b>	Ϋ	F	FÈG	FÎ	F																
	FÈCOÁÉÁFÈEÖÁÁFÈEY ÁOZOÈÍ	<b>′</b> ∧•	Ϋ	F	FÈG		F	FΪ	F	GJ	FÈ	H€											
	FEGÖÆÆFEGÆÆFEY ÆOZO	<b>′</b> ∧•	Ϋ	F	FÈG		F	FÌ	F		FÈHÈ												
	FEÇÖÁÉÁFÉEÖZÁÉFÉEY ZÁOZODÁ		Ÿ	F	FÈG		F	FJ	F	GJ			FÈHÈ										
	FEGÖÆÆFEGÆFEY ÆOZO	<b>7∧</b> •	Ÿ	F	FÈG		F	G€		GJ			FÈ										
	FEÇÖÁÉÁFÉEÖÁÉFÉEY ÁOZOM		Ÿ	F	FEG	_	F	Œ	F	GJ			FÈIÈ	:									
	FÉGÖÆÆFEGÆÆFEY ÆÆZDE		Ÿ	F	FÈG		F	Œ	_		ËÈÈ												
	FÉGÖÆÆFEGÆÆFEY ÆØZØ		Ÿ	F	FEG		F	GH GH			ËÈ											-	
H	FÈGÖÆÆFÈGÖÆFÈEY ÆOZOÈ		Ÿ	F	FEG		F	G	F		ËÈÀ												
	FÈCÖÆÆFÈCÖÆFÈCY ÆOZOÈ		Ÿ	F	FEG		F	ď	F	GJ			ËÈÀ	È								$\rightarrow$	
	FECOAÉAFECOAÉFEY AOZOM		Ÿ	<u> </u>	FEG		_																
	FEGÜÄEAFEGÄÆFEY ÄOZOE		Ÿ	F	FEG		F	Ĝ	F	GJ			ËË	<u>.</u>									
	FEGÜÄEAFEGÄÆFEY ÄOZON		Ÿ	F			F	Ġ	F	GJ				-									
HÌ	(FIE)ÁEÁFIE)Úª DÖSÁEÁFIEÓHÍ		Ÿ	F	FÈG		F	G	F	لک	FÈHÈ	-HE	ш										
	UFICAÉÁRICO DO SÁEÁFIE O DISTA ÁFIE O DESTA ÓFIE O DESTA O DESTA O DES		Ϋ		FÈGH			HG															
	3		Ÿ	F	FÌŒ	_	_	HG															
	ŢĖÆÆÆĠŮå•DÖŠÆÆÆÈÒĦŠ		Ÿ	F	FÌŒ				ÈÎÎ														
	ŢĖÆÆŒŮå•DÖŠÆÆÆÈÒĦŠ		Ÿ		FÈGH			HG															
	ŢĒÆÆŒŮª•DÖŠÆÆÆĒÒ		Ÿ		FÈGH																		
	ŢĒÁĖÁĒĎŮå•DÖŠÆÆFĒÒĒŠ		Ÿ	+	FEGIL			_	_														
	ŢĒÁĖÁĒĎŮå•DÖŠÆÆFĒÒĒŠ		Ÿ	F	FÌŒ			HG															
	ŢĒÁĖÁĒĎŮå•DÖŠÆÆFĒÒĒŠ		Ÿ		FİĞİİ																		
	ŢĒÁÉÁĒĒŮå•DÖŠÆÆFĒÈÒĒŠ		Ÿ		FİGİİ				ĦÎÎÎ														
	ŢĒÁÉÁÐÐŮå•DÖŠÆÆFÐÖÐ		Ϋ	F	FÌGH				Ë														
	ŢĒÁÉÁÐÐŮå•DÖŠÆÆFÐÖÐ		Ϋ	F	FÌŒ	_	_		ĤÎÎ														
	ŢĒÆÆŒĠŮå•DÖŠÆÆŒĠ		Ϋ	F	FİGİİ			_	_														
	Ç€ÈJÆÆECÌå•DÖŠÆÆFECÒÆ		Ϋ	F	ÈÎG			HG															
	ŒÐÆÆŒŨå•ØŠÆÆÆŒÔÆ		Ϋ	F	ÈÎG			HG	Ě														
ĺΗ	ÇEÐÆÆÐÚå•DÖŠÆÆFEÐÆ		Ϋ	F	ÈÎG		Ě	HG	ÈÎÎ														
ÍΙ	ŒÐÆÆÐÚå•ÞÖŠÆÆÆÐÓÆ		Ϋ	F	ÈÎG	HF		HG	F														
ĺĺ	ÇEÐÆÆÐÙå•DÖŠÆÆFEÐÆ		Ϋ	F	ÈÎG				Èîî													$\Box$	
ĺÎ	ÇĒDÆÆŒÙå•DÖŠÆÆÆĒÒÆ		Ϋ		ÈÎG																		
	ÇĒDÁËÆĒDÙå•DÖŠÆÆÆĒÒÆĒ		Ϋ	F	ÈÎG	HF	Ë	HG															
ĺÌ	ÇEÈJÁËÆÐÐÙå•DÖŠÆÆÆÐÆÐÁ		Ϋ	F	ÈÎG	HF	ĦÎÎ	HG	Η̈́														
ÍJ	ÇEÈ ÆÆEÒÀ	<b>′</b> ∧•	Ÿ		ÈÎG																		
΀	Ç€ÈJÆÆÈCÌJå•DÖŠÆÆÆÈCÒÆ	<b>7∧•</b>	Ϋ́		ÈÎG				Ë														
	ÇEÈJÆÆÈCÌå•DÖŠÆÆÆÈCÒÆ		Ϋ́		ÈÎG			HG	ĦÎÎÎ													$\neg$	
	ÇEÈJÆÆÈÈÙå•DÖŠÆÆÆÈÈÒÆ		Ÿ		ÈÎG																		
	FREOŠÆÆFĚŠŠÆÆÆÐUY		Ÿ	F	F				ĖΗ	FÍ		НН	FĚ										
	FÈEÖŠÆÆFĚŠŠÆÆÆÈÐY			F	F						ÈJF												
	FREOSÁEÁFIE SSÁEÁFREUY EETY			F	F						ÈHF												
	FIEÖŠÆÆFĚŠŠÆÆÆÐY			F	F		ĖΗΉ				È H												
	FREOSÁEÁFIL SSÁEÁFIEÚY EELY		Ÿ	F	F						ÈHF												
	FIECOŠÁEÁFIĚ ŠŠÁEÁFIEÚY III		Ÿ	F	F						ÈJF												
	FIECUS ÁEÁFIE ŠŠÁEÁFIEÚ Y ÈÉÝ		Ÿ	F	F				⊞HÌH				ΓŒ										
	FIECUS ÁLÁFIL SSÁLÁFILOV III.		Ÿ	_							∰JF												
	FIEOSÁEÁFIE SSÁEÁFIEÙY III		Ÿ	F	F						#HF												
LIF	FEEUS/REAFEL SSAEAFEEUY EETY	/ \•	Y	F	F	F€	шн	H	шIJГ	FI	шП	HH	F性										



#### @UX'7ca V]bUh]cbg'ff'cbh]bi YXŁ

Ö^• &¦āj cāj}	Ù[ ÈĚÈÈ	Ù <b>È</b> ÓŠÔØ	<b>ŒŬ</b>	ŠÔØ&##</th><th>ÓŠÔØæ&l</th><th><b>HĎ</b>ŠĆ</th><th>)Øæ&HH</th><th>)ŠÔØæ&E</th><th>HĎŠÔØ</th><th><b>z&HHÖ</b>ŠĈ</th><th>)Øæ&∰</th><th>ŠŠÔØ</th><th>SÔØ#SE</th><th><b>H</b>ĎŠĈ</th><th>)Øæ&<del>III</del></th></tr><tr><td>ÏG FÉEÖŠÆÆÆÉÉŠŠÆÆÆÉEÙYÈ</td><td><b>Ξ</b>Ϋ́^• Ϋ́</td><td>F</td><td>FF</td><td>F H</td><td>FI</td><td>FÍ</td><td>⊞ìH</td><td>HH FĚ</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td>ΪΗ FÈEÖŠÆÆÆĒĖŠŠÆÆÆĒŪΥĖ</td><td><b>Ϋ</b>γ^• Ϋ</td><td>F</td><td>F F</td><td>-G ÈHÌ H</td><td>FI ÈJF</td><td>FÍ</td><td>⊞HF</td><td>HH FĚ</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td>                                     </td><td><b>Ξ</b>Ϋ́^• Ϋ́</td><td>F</td><td>F F</td><td>H ÉH H</td><td>FI ÈH</td><td>Fĺ</td><td>∰JF</td><td>HH FĚ</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr></tbody></table>
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#### 9bj YcdY'>c]bhFYUM/jcbg

	R[ã]c		ÝÆjaá	ŠÔ	ŸÆjàá	ŠÔ	Z <i>Ä</i> Žjàá	ŠÔ	ΤÝΑӁΕ̈σά	ŠÔ	ΤΫ́ΑӁΕ̈́ά	ŠÔ	TZÁŽË-cá	ŠÔ
F	ÞĤ	{ æ¢	FFÎ 🖺 Ì Ì	FΪ	ÎJÈHÌÍ	Ĥ,	JĜĖÏI	FΪ	€	ΪŦ	€	ΪΤ	€	ΪΙ
G		{ <b>a</b> }	ËFFÏ ÈEÏ I	ŒН	Ϊ́ĠL	ÍΙ	ËΘĖÎΗ	£	€	F	€	F	€	F
Н	ÞIH	{ æ¢	Í€ÎÈEGJ	ĺ	FGGGÊH	B	Ì∉ÍÈII	Ğ	€	ΪΙ	€	ΪΙ	€	ΪΙ
- 1		{ <b>a</b> }	ËÍHÈ€ĴJ	<u>₽</u>	G€ÍÈÈÎI	$\oplus$	ËHFGÏ È€H	Ŧ	€	F	€	F	€	F
ĺ	ÞH	{ æ¢	ΙΙΙÈÌG	FΪ	FFHÏ ÈÌG	I,	GIÍĚÌH	Ð.	€	ΪΙ	€	ΪΤ	€	ΪΙ
Î		{ <b>a</b> }	ËJÏÈGJ		FJÌ ÈÌÏ	FI	ËHFÈÏH	⊛	€	F	€	F	€	F
Ϊ	V[œ <b>;</b> K		F€JÈÎF		GFÎĦH	₩	FÏ FÌ ÈHF	FI						
Ì		{ <b>a</b>	ËF€ÍJĒÎF	FF	IĞŒÌÏ	ĺF	ËÏFÌÈHF	Ì						

# 9bj YcdY5=G7 % h fl \*\$!%\$L @F: 8 GhYY 7cXY7\ YWg

	T^{ à^¦	Ù@ <b>≱</b> ^	Ô[ å^ÆÔÈ	EŠ[&޶áŠĆ	Ù@ælÆE	ÈŠ[&ާjá(	Öã ŠÔ	]@AEÚ}&ÁŽÜÉ		á]@a⊑⊺}Á⊞É	Ή <u>j</u> @aET}Á.É	
F	ΤÌ	FÈÄÜÖ¢€ÈGÄ	ÈFÏ	H HH H		ΙŒÌ	ΗÏ	ŒI JJ <u>È</u> JI	GÏÏJÈ	FÈHFI	FÈHFI	F#PF#à
G	ΤÏ	FÈÄUÖ¢€ÈGÄ	ÈÏÌ	H EHHH		I ŒÌ	HÍ	ŒI JJÈEJI	GÏÏJÈ	FÈHFI	FÈHFI	F∰PFË à
Н	TG	FÈÄÜÖ¢€ÈGÄ	ÈĤΗ	H HHG		ΙŒÌ	Ġ	ŒI JJ <u>È</u> JI	GÏÏJÈ	FÈHFI	FÈHFI	F∰PFËà
1	TF	FÈÄUÖ¢€ÈGÄ	ÈÚJ	H EHH		I ŒÌ	HF	ŒI JJÈEJI	GÏÏJÈ	FÈHFI	FÈHFI	F#PF#à
ĺ	T FF	€Ē HÄÂÛÜ	ÈGFI	HUÈFFH		HUÈFF	HÎ	FF€GÈÖ G	l	ÈΠΪ	ÈΠΪ	ŒËPFË à
Î	Τĺ	€Ē HÄÂÛÜ	ÈG€Ï	HIÈFFG	ÈEG€	HUÈFF	H€			ÈΠΪ	ÈlÏ	ŒËPFË à
Ϊ	T FI	ÚQÚÒ´GĚ	ÈHG	lì G		FJ	FH	HHÌÏÈHGG	ÎÎÎÎI	ΙĖ̈́Ğ	ΙĖ̈́Ğ	F∰PFË à
ì	T FH	ÚQÚÒ´GĚ	ÈH€		ÈÊÎÍ	FÌ	FF	HHÌÏÈHGG	ÎÎÎÎ	ΙĖ̈́Ğ	ΙĖ̈́Ğ	F#PF#à
J	ΤĠŒ	Ú∄, ^GÈHÌ Ý€ÈFG	È€JF	IÌÈEGÏH	€ È€Ë	JÎÈĞÍ	HÎ	FHGÌÈÍÌ	HÍ GÏ ŒÌ	GÈFÍ	GÈFÍ	F⊞PFErà
F€	T ÚG	GÈÌÄ¢€ÈG€Ä	ÈÈÌF	H Ì	ÈÎ	ÎΗ		GGI JHÈHFI	IH€ÍÎ	HÈÍÏ	HÈĖÏ	I∰PFËà
FF	T ÚF	GÈÌÄ¢€ÈG€Ä	ÈEÏÎ	ÎΗΗ	È€GI	HÍ	GJ	GGI JHÈHFI	IH€ÍÎ	HÈÍÏ	HÈĖÏ	I∰PFËà
FG	T ÚH	GÈÌÄ¢€ÈEG€Ä	ÈEÏH	Î H H	ÈEGÏ	ÎΗ	H€	GGI JHÈHFI	IH€ÍÎ	HÈÍÏ	HÈĖÏ	I∰PFËà
FH	TFG	€ĒĖHÄÄÛÜ	È€H	HUÈFFF	È€FÌ	₩ÈFF	FΪ	FF€GÈĞ G	FI€GÏÈÏĞ	ÈΕΙΪ	ÈΠΪ	F∰PFË àE
FI	TI	€ĒĖHÄÄÛÜ	È€€€	€ ÏI	È€FG	GÌÈH	HÏ	ŒÌ FÈGJÌ	FI€GÏÈÏĞ	ÈΕΙΪ	ÈΠΪ	ŒËPFËæ
FÍ	TF€	€ĒĖHÄÄÛÜ	È€€€	l € ÏI	È€J	GÌÈH	HÏ	ŒÌ FÉGJÌ	FI€GÏÈÏĞ	ÈΕΙΪ	ÈΠΪ	ŒËPFËæ
FÎ	TJ	€Ē HÄĄÛÜ	È€€€	€ÏI	È€J	€	F€	ŒÌ FÈGJÌ	FI€GÏÈÏGÍ	ÈΙΪ	ÈΙΪ	Œ PFËæ
FΪ	TH	€ĒÌHÄÄÛÜ	È€€€	€ÏI	È€Î	GÌÈH	I	ŒÎ FÉGJÎ	FI€GÏÈÏĞ	ÈΙΪ	ÈΙΪ	ŒËPFËæ
FÌ	ΤÎ	€Ē HÄÂÛÜ	È€€€	€ÏI	È€€€	€	ΪΙ	FF€GÈĞ G	FI€GÏÈÏĞ	ÈΙΪ	ÈΠΪ	F PF#æ

#### <chFc``YX'GhYY`GYWfjcb'GYhg</pre>

	Šæà^∣	Ù@ <b>≱</b> ^	V^]^	Ö^∙ãt}ÁŠãac		Ö^• ã } ÁÜ	OEÁŽ)AGá	Q^ÃŽ[lá	Q:Æãjlá	RÁŽájIá
F	Øæ&∧ÁP[¦ã[⊞⊞		Þ[}^	Þ[}^	ŒÁÕ¦ÈÌÎ	V^]	FÈF	FÈÍ	FÈÍ	GÈJ
G	Ùœ)å[~ÁŒ{•	FÈJÄÄUÖ¢ <del>CÈ</del> GÄ	Þ[}^	Þ[}^	ŒÁÕ¦ÈÎ	V^]	ĒΪF	ÈĠΪ	ÈĠΪ	ĚΗ
Н	Öãæ* [ } æ	€Ē HÄÁÙÜ	Þ[}^	Þ[}^	OÉ GJÁÕ¦Ě€	V^]a&ae	ÈFG	È€ÈÌ	È€ÈÌ	È€FÍ
- 1	T[*} œÁÚa] ^	OEÈÌÄ¢€ÈFG€Ä	PÓ¦æ&^	Úą^	ŒÁÕ¦ÈÎ	V^]	FÈ€l	ÐЈН	ÐЈН	FÀÌÍ
ĺ		Ú∄^GÈÌ Ý€ÈÈ	Þ[}^	Þ[}^	ŒÁÕ¦ÈÎ	V^]	ÈÍG	Ělĺ	Ělĺ	FÈ€JF
Î		ÈHLĚÄ¢€ÈFG€	Þ[}^	Þ[}^	ŒÁÕ¦ÈÎ		FÈĞİ	FÈŒ	FÈŒ	HĒII
Ϊ	Ùœ)å[~ÁX^¦ÈÈ	È €ÉÈ HÄÄÛÜ	Þ[}^	þ[}^	OÉ GJÁÕ¦Ě€	V^]a8ee	ÈFG	È€ÈÌ	È€ÈÌ	È€FÍ

# <chFc``YX'GhYY`8 Yg][ b'DUfUa YhYfg</pre>

	Šæà^	Ù@a}^ Š^}*c@Žajá	Šà^^Žajá	Šà∷Žajá	Š&[{]Áq[]ŽĄ á	Š&[{]Áa[cŽā)á	áŠËą¦~~ÈÈ	È S^^	S::	Ôà	Ø"}&da[}
F	TF	Ù@a}^ Š^}*c@Žajá Ùcaà;å[~ÁOEÈÈ   CEÈ			Šà^^						Šæe^¦æ
G	TG	Ùœ≱å[~ÁŒ⊞   ŒÌ			Šà^^						Šæe^\a
Н	ΤH	Ùœ)å[~ÁK^È GÌÈH			Šà^^						Šæe^læ¢
	ΤI	Ùœ≱å[~Áx^i∰ GÌÈH			Šà^^						Šæe^\a\
ĺĺ	Τĺ	Öãæ≛[}æ; HJEÌFF			Šà^^						Šæe^læ¢
Î	ΤÎ	Öãe [}æ HJĒFF			Šà^^						Šæe^\a\
Ϊ	ΤÏ	Ùœ; å[~ÁŒ Ì ☐ ☐ Ùœ; å[~ÁŒ Ì ☐ ☐			Šà^^						Šæe^\læ¢
Ì	ΤÌ	Ùœ≱å[~ÁŒ⊞ IGH			Šà^^						Šæe^\læ
J	TJ	Ùœ)å[~Á×∧ĖĖ GÌÈH			Šà^^						Šæe^\læ¢
F€	TF€	Ùæ)å[~ÁK^誰 Gì主			Šà^^						Šæe^\læ
FF	TFF	Öãe≛[}æ; HJÈÌFF			Šà^^ Šà^^ Šà^^ Šà^^ Šà^^ Šà^^ Šà^^ Šà^^						Šæe^\a
FG	TFG	Öæt [}æ HJE FF			Šà^^						Šæe^\a
FH	T FH	Ø#&^ÁP[¦ã[## JÎ Ø#&^ÁP[¦ã[## JÎ									Šæe^\a
FI	T FI	Øæ&∧ÁP[¦ã[⊞ JÎ			Šà^^						Šæe^\a
FÍ	T ÚF	T[ˇ} đÁÚāj^ JÎ			Šà^^						Šæe^¦æ
FÎ	T ÚH	T[ * } œ\Úaj^ JÎ			Šà^^						Šæe^\læ¢
FΪ	TÚG	T[ˇ} ơÁÚą̄^ Jĵ			Šà^^ Šà^^ Šà^^ Šà^^						Šæe^\ła¢
FÌ	ΤĠŒ	VaNÁÓæ&N JÎÈGÍÍ									Šæe^\a\

>c]bhi6 ci bXUf mi7 cbX]hjcbg

	R[ãjoÁŠæàn^	ÝÃŽ Đặa	ŸÃŽĐĄjá	ZÁŽ Đặa	ÝÁÜ[dĚŽËdĐæåá	ŸÁÜ[œŽČĒœůá	ZÁÜ[dŽŽËdĐæåá
F	ÞF						
G	ÞÎ						
Н	ÞĤ	Ü^æ <b>\$</b> æ <b>1</b> }	Ü^æ <b>&amp;</b> æ <b>i</b> }	Ü^æ&aãi}			
1	ÞIH	Ü^æ <b>\$</b> æ <b>[</b> ]}	Ü^æ <b>&amp;</b> æ <b>i</b> }	Ü^æ <b>&amp;</b> æ <b>i</b> }			
ĺ	ÞH	Ü^æ&æ1}	Ü^æ&æ1}	Ü^æ&æ1}			

# A Ya VYf 5 Xj UbWYX 8 UHJ

	Šæà^	QÄÜ^ /æe^	RÁÜ^ ^æ•^	OÁJ~•^cŽajá	RÁU⊶^cŽajá	VĐÔÁU} ^	Ú@• <b>&amp;</b> æ	Ö^- ÁÜææ⊞CB; 梕ã ÁEE	Q1 2886 Cái; ^	Ù^ãa{ã&ÈÈ
F	TF	Ó^}ÚŒ					Ϋ́Λ∙	HEÁÞ CEÁH		Þ[}^
G	TG	Ó^}ÚŒ					Ϋ́Λ∙	HEÁÞ CEÁH		Þ[}^
Н	ΤH					Ò   \		HEÁÞ CEÁH		Þ[}^
	TI					Ò "   '   ÁÓ " & À	<u></u> Ϋ́Λ•	HEÁÞ CEÁH		Þ[}^
ĺ	Τĺ						Ϋ́Λ∙	HEÁÞ CEÁH		Þ[}^
Î	ΤÎ					V^}• <b>ā</b> }Æ	Ϋ́Λ∙	HEÁÞ CEÁH		Þ[}^
Ϊ	ΤÏ	Ó^} ÚŒ					Ϋ́Λ∙	HEÁÞ CEÁH		Þ[}^
ì	ΤÌ	Ó^} ÚŒ					Ϋ́Λ∙	HEÁÞ OFÁH		Þ[}^
J	TJ					Ò  ^   ÁÓ &À		HEÁÞ CEÁH		Þ[}^
F€	TF€					Ò  ^   ÁÓ &À	<b>₽</b> Ϋ́Λ•	HEÁÞ OÐÁH		Þ[}^
FF	T FF						Ϋ́Λ∙	HEÁÞ CEÁH		Þ[}^
FG	T FG					V^}• <b>ā</b> }Æ	Ϋ́Λ∙	HEÁÞ OFÁH		Þ[}^
FH	T FH						Ϋ́Λ∙	HEÁÞ CEÁH		Þ[}^
FI	T FI						Ϋ́Λ∙	HEÁÞ OEÁH		Þ[}^
FÍ	T ÚF						Ϋ́Λ∙	HEÁÞ CEÁH		Þ[}^
FÎ	T ÚH						Ϋ́Λ∙	HEÁÞ OEÁH		Þ[}^
FΪ	T ÚG					_	Ϋ́Λ∙	EEÁÞOEÁEE		þ[}^
FÌ	TGŒ	Ó^} ÚŒ					Ϋ́Λ∙	HEÁÞOEÁH		þ[}^
FJ	ΤGJ						Ϋ́Λ∙	HEÁÞ CEÁH		Þ[}^

A Ya VYf 5 X j UbWYX 8 UHU f17 c bh]bi YXŁ

	Šæà^	QÄÜ^ ^æe•^	RÁÜ^ ^æ•^	OÁJ⊶-^cŽajá	RÁU⊶^cŽajá	VÐÔÁU} ^	Ú@• <b>&amp;</b> æ	Ö^- ÁÜæd⊞CB; æ∳•ã ÁEÈ	Q) æ&cãç^	Ù^ãa{ã&ÈÈ
G€	TH€						Ÿ∧•	HEÁÞ OFÁH		þ[}^
GF	THH						Ÿ۸۰	EEÁÞOEÁEE		Þ[}^
GG	TH						Ÿ^•	EEÁDOEÁEE		Þ[}^
GH	ΤHÍ						Ϋ́Λ•	EEÁÞOEÁEE		Þ[}^
G	ΤHÎ						Ϋ́Λ•	EEÁDOEÁEE		Þ[}^
GÍ	Τď						Ϋ́Λ•	EEÁÞOEÁEE		Þ[}^
Ĝ	ΤĠ						Ÿ∧•	EEÁÞOEÁEE		þ[}^

A Ya VYf 'Dc]bh'@:UXg'f6 @ '% 'GY'ZK Y][\H\_

	T^{ à^ ÁŠæà^	Öã^&cã}	Tæ*}ããå^ŽàÉsËeá	Š[&aeda[}ŽājEÃá
F	T ÚG	Ϋ	Ë FÈĞ	H€
G	T ÚG	Ÿ	Ë FÈĠ	ÎÎ
Н	T ÚG	Ÿ	ËHÈ	G
- 1	T ÚG	Ÿ	Ëί	ΪG
ĺ	ТΪ	Ϋ	ËGFË	Œ

A Ya VYf 'Dc]bh'@cUXg'f6 @ '&. 'K ]bX'@cUX'5 N="\$Ł

	T^{ à^¦Æeeà^	Öã^&cã}}	Tæ*}ããå^ŽjàÉsËeá	Š[&æqā[}ŽājĒĀá
F	T ÚG	Ý	€	H€
G	T ÚG	Z	ËG€GÈG	H€
Н	T ÚG	Ý	€	ÎÎ
- 1	T ÚG	Z	ËG€GÈG	ÎÎ
ĺ	T ÚG	Ý	€	G
Î	T ÚG	Z	ËΉťJ	G
Ï	T ÚG	Ý	€	ΪG
Ì	T ÚG	Z	ËΉťJ	ΪG
J	ΤÏ	Ý	€	G
F€	ΤÏ	Z	ËÍÈ	Œ

A Ya VYf 'Dc]bh'@:UXg'f6 @' ' . 'K]bX'@:UX'5 N='' \$Ł

	T^{à^¦Ásaaà^	Öã^&cã}}	Tæ*}ããå^ŽjàÉsËeá	Š[&andaa]}ŽAjEÃá
F	T ÚG	Ý	ĤÏĤF	H€
G	T ÚG	Z	ËFÍFÐF	H€
Н	T ÚG	Ý	ĤÏĤF	ÎÎ
1	T ÚG	Z	ËFÍFÐF	ÎÎ
ĺ	T ÚG	Ý	ËĞİÈG	G
Î	T ÚG	Z	ËÌÈÌ	G
Ï	T ÚG	Ý	ËĠÈG	ΪG
Ì	T ÚG	Z	ËJÈG	ΪG
J	ΤÏ	Ý	ËJĖ	G
F€	ΤΪ	Z	Ë́€ĚF	æ

A Ya VYf 'Dc]bh'@cUXg'f6 @ ('.'K]bX'@cUX'5 N="\*\$Ł

	T^{à^¦ÁŠæà^	Öã^&cã;}	Tæ*}ããå^ŽjàÉÈëá	Š[&andai}ŽājEÃá
F	T ÚG	Ý	ËF€Í ÈÌ	H€
G	T ÚG	Z	Ë €È	H€
Н	T ÚG	Ý	ËF€ÍÈÌ	ÎÎ
1	T ÚG	Z	Ë €È	ÎÎ
ĺ	T ÚG	Ý	ËHÈ	G

#### A Ya VYf 'Dc]bh'@cUXg'f6 @ ( . 'K]bX'@cUX'5 N="\*\$L'f7 c bhjbi YXL

	T^{ à^¦ÁŠæà^	Öã^&cã[}	Tæ*}ãc°å^Ž∤àÉcËeá	Š[∧ā[}ŽājĒĀá
Î	T ÚG	Z	ËJËÎ	G
Ϊ	T ÚG	Ý	ËHÏ Ě G	ΪG
Ì	T ÚG	Z	ËŒĨÎ	ΪG
J	ΤΪ	Ý	ËHÌËÌ	æ
F€	ΤΪ	Z	ËGÀH	Œ

#### A Ya VYf 'Dc]bh'@cUXg'f6 @' ) . K ]bX'@cUX'5 N="-\$L

	T^{ à^¦ÁŠæà^	Öã^&cã}	Tæ*} <u>a</u> ãc°a^ŽàÉcËeá	Š[&aedā]}ŽŽ[JĒĀá
F	TÚG	Ý	ËI ÈJ	H€
G	T ÚG	Z	€	H€
Н	T ÚG	Ý	ËIÈJ	ÎÎ
	T ÚG	Z	€	ÎÎ
ĺ	T ÚG	Ý	ËÆËÏ	G
Î	T ÚG	Z	€	G
Ϊ	T ÚG	Ý	ËĤĚÌ	ΪG
Ì	T ÚG	Z	€	ΪG
J	ΤÏ	Ý	ËHËH	GF
F€	ΤÏ	Z	€	GF

# A Ya VYf 'Dc]bh'@:UXg'f6 @' '\* . 'K ]bX'@:UX'5 N="%&\$L

	T^{ à^¦ÁŠæà^	Öã^&cã}}	Tæ*}ãã å^ŽàÉÈŒá Ё∺€ÍÈÌ	Š[&æqā[}ŽājĒĀá
F	T ÚG	Ý	ËF€ÍÈÌ	H€
G	T ÚG	Z	Î <b>€</b> Ē	H€
Н	T ÚG	Ý	ËF€ÍÈÌ	ÎÎ
1	T ÚG	Z	΀È	ÎÎ
ĺ	T ÚG	Ý	ËHÈ	G
Î	T ÚG	Z	FJ∄Î	G
Ï	T ÚG	Ý	ËHÏ Ě G	ΪG
Ì	T ÚG	Z	GF <u>H</u> ÎÎ	ΪG
J	ΤΪ	Ý	ËHÌËÌ	GF
F€	ΤÏ	Z	GGÏH	GF

#### A Ya VYf 'Dc]bh'@cUXg'f6 @ '+'. 'K]bX'@cUX'5 N="%) \$Ł

	T^{ à^¦ÆŠæà^	Öã^&cã}	Tæ*}ããå^Ž∤àÉsÉeá	Š[&æqā[}ŽājĒĀá
F	T ÚG	Ý	ΪΪΪF	H€
G	T ÚG	Z	FÍ FÐF	H€
Н	T ÚG	Ý	ËÏËF	ÎÎ
1	T ÚG	Z	FÍ FÐF	ÎÎ
ĺ	T ÚG	Ý	ËĞİÈG	G
Î	T ÚG	Z	lì Èì	G
Ï	T ÚG	Ý	ËĠÈG	ΪG
Ì	T ÚG	Z	DÉG I	ΪG
J	ΤÏ	Ý	ËJĖ	GF
F€	ΤÏ	Z	Í∰F	<b>G</b> F

### A Ya VYf 'Dc]bh'@cUXg'f6 @r', . 'K]bX'@cUX'5 N="% \$Ł

	T^{ à^¦ÁŠæà^	Öã^&cã;}	Tæ*}ããå^ŽàÉãËeá	Š[&æqā[}Žā]ÉÃá
F	T ÚG	Ý	€	H€
G	T ÚG	Z	G€GÌÈG	H€
Н	T ÚG	Ý	€	ÎÎ

#### A Ya VYf 'Dc]bh'@:UXg'f6 @' , . K JbX'@:UX'5 N="% \$L'f7 cbh]bi YXL

	T^{ à^¦ASamà^	Öã^&cã}	Tæt"}ãoc°å^ŽàÉcËeá G€GÈGG	Š[∧ā]}ŽājEÃá
	T ÚG	Z	G€GÌÈG	ÎÎ
ĺ	T ÚG	Ý	€	G
Î	T ÚG	Z	ÎHĚJ	G
Ï	T ÚG	Ý	€	ΪG
Ì	T ÚG	Z	ÎHĚJ	ΪG
J	ΤÏ	Ý	€	GF .
F€	ΤÏ	Z	ÎÍĦ	GF .

#### A Ya VYf 'Dc]bh@UXg'f6 @ - . K ]bX'@UX'5 N='&%\$Ł

	T^{ à^ ÁŠæà^	Öã^&cã}	Tæt}ããå^ŽjàÉËeá ÌÏËËF	Š[&andaj}ŽājĒĀá
F	TÚG	Ý		H€
G	T ÚG	Z	FÍ FÐF	H€
Н	T ÚG	Ý	ÌÏËF	ÎÎ
1	T ÚG	Z	FÍ FÐF	ÎÎ
ĺ	T ÚG	Ý	GÏĒG	G
Î	T ÚG	Z	l Ì ÈÌ	G
Ï	T ÚG	Ý	GÌÈG	ΪG
Ì	TÚG	Z	I J <b>È</b> G	ΪG
J	ΤÏ	Ý	GIÈÎ	G <del>F</del>
F€	ΤÏ	Z	Í∉ŬF	G <del>F</del>

#### A Ya VYf 'Dc]bh'@cUXg'f6 @ '%\$'. 'K]bX'@cUX'5 N='&(\$L

	T^{ à^¦ÁŠæà^	Öã^&cã}	Tæ*}ããå^ŽàÉËæá F€ÍÈÌ	Š[&aedā]}ŽĀJĒĀá
F	TÚG	Ý	F€ÍÈÌ	H€
G	T ÚG	Z	΀È	H€
Н	T ÚG	Ý	F€ÍÈÌ	ÎÎ
	T ÚG	Z	Î⊕	ÎÎ
ĺ	T ÚG	Ý	ΗĖ	G
Î	T ÚG	Z	FJÈÌÎ	G
Ï	T ÚG	Ý	HÏĖĞ	ΪG
Ì	T ÚG	Z	GF <u>H</u> ÎÎ	ΪG
J	ΤÏ	Ý	ΗÌĒÌ	G
F€	ΤÏ	Z	GŒÌH	GF

# A Ya VYf 'Dc ]bh'@:UXg 'f6 @ '%% 'K ]bX' @ UX '5 N='&+\$L

	T^{ à^¦ASamà^	Öã^&cã}}	Tæ*}ããå^ŽàÉsÉeá	Š[&aedā[}Žā]ÉĀá
F	T ÚG	Ý	JI ÈJ	H€
G	T ÚG	Z	€	H€
Н	T ÚG	Ý	JI ÈJ	ÎÎ
1	T ÚG	Z	€	ÎÎ
ĺ	T ÚG	Ý	HFÈ Ï	G
Î	T ÚG	Z	€	G
Ϊ	T ÚG	Ý	HÎĚÌ	ΪG
ì	T ÚG	Z	€	ΪG
J	ΤÏ	Ý	ΗÏĖΉ	GF
F€	ΤÏ	Z	€	GF

#### A Ya VYf 'Dc]bh'@:UXg'f6 @' '%&'. 'K]bX'@:UX'5 N=" \$\$Ł

	T^{à^¦ÆSeeà^	Öã^&cã[}	Tæ*}ããå^ŽàÉsÉeá	Š[&aea[]}Žā]ĒÃá
F	T ÚG	Ý	F€ÍÈÌ	H€

#### A Ya VYf Dc]bh@cUXg'f6 @ '%&. K JbX'@cUX'5 N=" \$\$L'ff cbljbi YXL

	T^{ à^¦ÁŠæà^	Öã^&cã}	Tæ*}ãočå^ŽàÉcËeá	Š[&aedā[}Žā]EĀ á
G	T ÚG	Z	Ë €Đ	H€
Н	T ÚG	Ý	F€ÍÈÌ	ÎÎ
1	T ÚG	Z	Ë €È	ÎÎ
ĺ	T ÚG	Ý	ΗĖ	G
Î	T ÚG	Z	ËJĒÎ	G
Ï	T ÚG	Ý	HÏĚG	ΪG
Ì	T ÚG	Z	ËGFËÎÎ	ΪG
J	ΤÏ	Ý	ΗÌËÌ	GF
F€	ΤΪ	Z	ËGGÌH	GF

#### A Ya VYf 'Dc]bh'@cUXg'f6 @' '%' . K]bX'@cUX'5 N='' '\$Ł

	T^{ à^¦ÆŠæà^	Öã^&cã}	Tæt}ããå^ŽjàÉĒ⊛á ÌÏĒF	Š[&ænā[}ŽājĒĀá
F	T ÚG	Ý	ÌÏËF	H€
G	T ÚG	Z	ËTÍ FÐF	H€
Н	T ÚG	Ý	ÌÏËF	ÎÎ
1	T ÚG	Z	ËTÍ FÐF	ÎÎ
ĺ	T ÚG	Ý	GÏĖG	G
Î	T ÚG	Z	ËÌÈÌ	G
Ï	T ÚG	Ý	ĠĖG	ΪG
Ì	T ÚG	Z	ËJÈG	ΪG
J	ΤÏ	Ý	GIÈÎ	GF .
F€	ΤÏ	Z	ŰŰF	GF .

#### A Ya VYf 'Dc]bh'@:UXg'f6 @ '%' . '\W'K Y][\ HL

	T^{ à^¦ÁŠæà^	Öã^&cã[}	Tæ*}ããå^ŽàÉĒeá	Š[&anca]}ŽāJĒÃá
F	T ÚG	Ϋ	ËG€HÈÌF	H€
G	T ÚG	Ϋ́	ËGHÈIF	ÎÎ
Н	T ÚG	Ϋ́	ËF€IÈEÍG	G
1	T ÚG	Ϋ́	ËFF€ÈJ	ΪG
ĺ	ΤÏ	Ÿ	ËF€Ì ĎJÎ	Œ

# A Ya VYf 'Dc]bh'@:UXg'f6 @ '%+'. '\\W'K]bX'@:UX'5 N\\\\\\

	T^{ à^¦ÆSeeà^	Öã^&cã}	Tæ*}ããå^ŽjàÉsËeá	Š[&æqā[}ŽājĒĀá
F	T ÚG	Ý	€	H€
G	T ÚG	Z	ËGIÈÈÌ	H€
Н	T ÚG	Ý	€	ÎÎ
1	T ÚG	Z	ËGÈÈÌ	ÎÎ
ĺ	T ÚG	Ý	€	G
Î	T ÚG	Z	ËF C <del>ÈC</del> H	G
Ϊ	T ÚG	Ý	€	ΪG
Ì	T ÚG	Z	ËF C <del>ÈC</del> H	ΪG
J	ΤÏ	Ý	€	G <del>F</del>
F€	ΤΪ	Z	Ë Œ	Œ

#### A Ya VYf 'Dc]bh'@cUXg'f6 @' '% '. \\W'K]bX'@cUX'5 N='' \\L

	T^{ à^¦ÁŠæà^	Öã^&cã[}	Tæ*}ããå^ŽàÉcËeá	Š[&ændā[}Žā[bÃāá
F	T ÚG	Ý	ËHÈH	H€
G	T ÚG	Z	ËGĂÎ	H€
Н	T ÚG	Ý	ËFHÈCH	ÎÎ
- 1	T ÚG	Z	ËŒÎÎ	ÎÎ

#### A Ya VYf 'Dc]bh'@cUXg'f6 @ '% : `#W'K ]bX'@cUX'5 N='' \$L'ff cbljbi YXL

	T^{ à^¦ÁŠæà^	Öã^&cã[}	Tæ*}ãćå^ŽàÉcëá HŤĤÍ	Š[&ændā[}Žā]ÉĀá
ĺ	T ÚG	Ý	ĔÈÍ	G
Î	T ÚG	Z	ËÈH	G
Ϊ	T ÚG	Ý	Ε̈́Ε̈́G	ΪG
Ì	T ÚG	Z	ËĚÎ	ΪG
J	ΤΪ	Ý	Ĕ Ē I	G <del>F</del>
F€	ΤÏ	Z	ËËÌ	Œ

#### 

	T^{ à^;ÁŠæà^	Öå^&d}}	Tæt}ãcå^ŽàÉĒeá ËFÏÈHG	Š[&anca[}ŽājEĀá H€
F	T ÚG	Ý		H€
G	T ÚG	Z	ËF€	H€
Н	T ÚG	Ý	ËFÏÈG	ÎÎ
1	T ÚG	Z	ËF€	ÎÎ
ĺ	T ÚG	Ý	ËÈÏ	G
Î	T ÚG	Z	ËÈF	G
Ϊ	T ÚG	Ý	ËÈÎ	ΪG
ì	T ÚG	Z	ËĚI	ΪG
J	ΤÏ	Ý	ÊEJ	GF
F€	ΤÏ	Z	ËÈÏ	GF .

#### A Ya VYf 'Dc]bh'@:UXg'f6 @' '&\$'. '\W'K]bX'@:UX'5 N\='-\\$L

	T^{ à^¦ÆŠæà^	Öã^&cã}	Tæt}ãcå^ŽjàÉEæá ËFÎÈÜ	Š[&æna[}ŽājĒĀá
F	T ÚG	Ý	ËÎÈÏ	H€
G	T ÚG	Z	€	H€
Н	T ÚG	Ý	ËFÎ ÈJÏ	ÎÎ
1	T ÚG	Z	€	ÎÎ
ĺ	T ÚG	Ý	ËĖJ	G
Î	T ÚG	Z	€	G
Ï	T ÚG	Ý	ÊÈEJ	ΪG
Ì	T ÚG	Z	€	ΪG
J	ΤÏ	Ý	ĖĖ	GF
F€	ΤÏ	Z	€	GF

#### A Ya VYf 'Dc]bh'@cUXg'f6 @' '&%. '\\W'K]bX'@cUX'5 N\='%\\$\L

	T^{ à^¦Æsæà^	Öā^&cā[}	Tæ*}ããå^ŽjàÉsÉeá	Š[&aeqā[}ŽājEÃá
F	T ÚG	Ý	ËFÏÈHG	H€
G	T ÚG	Z	F€	H€
Н	T ÚG	Ý	ËFÏÈHG	ÎÎ
1	T ÚG	Z	F€	ÎÎ
ĺ	T ÚG	Ý	ËÈÏ	G
Î	T ÚG	Z	l ÈF	G
Ϊ	T ÚG	Ý	ËÈÎ	ΪG
Ì	T ÚG	Z	ΙĚΙ	ΪG
J	ΤΪ	Ý	ÊÈEJ	GF .
F€	ΤΪ	Z	ΙĒΪ	GF

#### A Ya VYf 'Dc]bh'@cUXg'f6 @' '&&'. '¥NY'K]bX'@cUX'5 N='%) \$Ł

	T^{ à^¦ASasà^	Öã^&cã[}	Tæ*}ããå^ŽàÉãËeá	Š[&æqā[}Žā]EÃá
F	T ÚG	Ý	ËFH <del>ÈC</del> H	H€
G	T ÚG	Z	GŒ <u>I</u> Î	H€

#### A Ya VYf Dc]bh@cUXg f6 @ 8&. =\WK]bX @UX 5 N="% \$Lff cbh]bi YXL

	T^{ à^¦ASasà^	Öã^&cã}	Tæ*}ãã å^ŽjàÉ Ëeá	Š[&aedā]}ŽāJĒĀá
Н	T ÚG	Ý	ËFH <del>EG</del> H	ÎÎ
1	T ÚG	Z	GŒĬÎ	ÎÎ
ĺ	T ÚG	Ý	ĔĖĺ	G
Î	T ÚG	Z	JÈH	G
Ï	T ÚG	Ý	<u> ĔĚG</u>	ΪG
Ì	T ÚG	Z	JĚÎ	ΪG
J	ΤÏ	Ý	Η̈́ĒΙ	GF
F€	ΤΪ	Z	JĚÌ	GF .

#### 

	T^{à^¦ÁŠæà^	Öã^&dã}	Tæ*}ããå^ŽjàÉsËeá	Š[&æqā[}ŽājĒĀá
F	T ÚG	Ý	€	H€
G	T ÚG	Z	GÆ	H€
Н	T ÚG	Ý	€	ÎÎ
- 1	T ÚG	Z	GÆ	ÎÎ
ĺ	T ÚG	Ý	€	G
Î	T ÚG	Z	FŒH	G
Ï	T ÚG	Ý	€	ΪG
Ì	T ÚG	Z	FŒH	ΪG
J	ΤΪ	Ý	€	Œ
F€	ΤΪ	Z	FŒĠ	<b>G</b> F

# 

	T^{ à^¦ÁŠæà^	Öã^&cã}	Tæ*}ãćå^ŽàÉÈëá FHÈEH	Š[&aedā[}Žā[bĒĀ á
F	TÚG	Ý	FHÈ	H€
G	T ÚG	Z	GŒ <u>I</u> Î	H€
Н	T ÚG	Ý	FH <del>Ì€</del> H	ÎÎ
1	T ÚG	Z	GŒĬÎ	ÎÎ
ĺ	T ÚG	Ý	ÍÈÍ	G
Î	T ÚG	Z	JÈH	G
Ϊ	T ÚG	Ý	ĺĚG	ΪG
ì	T ÚG	Z	JĚÎ	ΪG
J	ΤÏ	Ý	ÍÈÌ	G <del>F</del>
F€	ΤΪ	Z	JËÌ	GF GF

# A Ya VYf 'Dc]bh'@:UXg'f6 @7 '&) '. '\\YK]bX'@:UX'5 N\\\' &(\$\!

	T^{ à^¦ÁŠæà^	Öã^&cã[}	Tæ*}ãćå^ŽàÉĒcá FÏÈHG	Š[&aea[]}Ž3jĒÃá
F	T ÚG	Ý	FÏ ÈHG	H€
G	T ÚG	Z	F€	H€
Н	T ÚG	Ý	FÏ ÈG	ÎÎ
1	T ÚG	Z	F€	ÎÎ
ĺ	T ÚG	Ý	ÏÈÏ	G
Î	T ÚG	Z	l ÈF	G
Ï	T ÚG	Ý	ÏÈÎ	ΪG
Ì	T ÚG	Z	ΙĚΙ	ΪG
J	ΤÏ	Ý	ÌÈ	GF
F€	ΤÏ	Z	ΙĒΪ	Œ

#### 

 $\mathsf{T}^{\hat{a}}$   $\mathsf{T}^{\hat{b}}$ 

#### 

	T^{ à^¦ÁŠæà^	Öã^&cã}	Tæ*}ãc°å^ŽjàÈĒeá	Š[&aedā[}Ž5[bã(á
F	T ÚG	Ý	FÎ ÈÏ	H€
G	T ÚG	Z	€	H€
Н	T ÚG	Ý	fî ÈÏ	ÎÎ
1	T ÚG	Z	€	ÎÎ
ĺ	T ÚG	Ý	ΪÈIJ	G
Î	T ÚG	Z	€	G
Ï	T ÚG	Ý	ÌÈ	ΪG
Ì	T ÚG	Z	€	ΪG
J	ΤÏ	Ý	ìÈì	GF .
F€	ΤÏ	Z	€	GF .

#### A Ya VYf 'Dc]bh'@cUXg'f6 @7 '&+'. '±\W'K]bX'@cUX'5 N=" \$\$Ł

	T^{ à^¦ÁŠæà^	Öã^&cã}}	Tæ*}ãã å^ŽàÉÈŒá FÜÈHG	Š[&æqā[}ŽājĒĀá
F	T ÚG	Ý	FÏ ÈHG	H€
G	T ÚG	Z	ËF€	H€
Н	T ÚG	Ý	FÏ ÈG	ÎÎ
1	T ÚG	Z	ËF€	ÎÎ
ĺ	T ÚG	Ý	ÏÈÏ	G
Î	T ÚG	Z	ËÈF	G
Ï	T ÚG	Ý	ÏÈÎ	ΪG
Ì	T ÚG	Z	ËĚI	ΪG
J	ТΪ	Ý	ÌÈ€J	GF .
F€	ТΪ	Z	ËĒÏ	GF GF

### A Ya VYf 'Dc]bh'@cUXg'f6 @7 '&, . `₩YK]bX'@cUX'5 N=" ' \$Ł

	T^{ à^¦Æsæò^  TÚG	Öã^&cã}	Tæt}ãcå^ŽjàÉcëá FHÈEH	Š[&ænā[}Žā]EÃá
F	T ÚG	Ý	FI <del>-È</del>	H€
G	T ÚG	Z	EGGHÎÎ	H€
Н	T ÚG	Ý	FHÈ€H	ÎÎ
1	T ÚG	Z	ËGEĞÎ	ÎÎ
ĺ	T ÚG	Ý	ĺÈĺ	G
Î	T ÚG	Z	ËĖH	G
Ï	T ÚG	Ý	ĺĚG	ΪG
Ì	T ÚG	Z	ËĚÎ	ΪG
J	ΤÏ	Ý	ÍÈI	GF
F€	ΤÏ	Z	ËËÌ	Œ

## A Ya VYf 'Dc ]bh'@cUXg 'f6 @' ' % 'GY]ga ]W@cUX 'NL

	T^{ à^¦Æsæà^	Öā^&cā[}	Tæ*}ããå^ŽàÉÈ∈á	Š[∧ā[}Žā[bÃá
F	T ÚG	Z	ËFËIÌ	H€
G	T ÚG	Z	ËFËIÌ	ÎÎ
Н	T ÚG	Z	ËÌĖJJ	G
1	T ÚG	Z	ËŒĤ	ΪG
ĺ	ΤÏ	Z	HĐĐÌ ÎÌ	GF

# A Ya VYf Dc]bh@cUXg f6 @ ' & GY]ga JW@cUX LŁ

	T^{ à^¦ÁŠæà^	Öã^&cã}	Tæ*}ããå^ŽàÉsÉeá	Š[&æeā[}Žā[£Ãá
F	T ÚG	Ý	ËFËIÌ	H€
G	T ÚG	Ý	ËFËIÌ	ÎÎ
Н	T ÚG	Ý	ËÌĖJJ	G

TælÁGHÉÍG€GG F€NÍÄÁÚT Ô@^&\^åÆÍÓ^K ′′′′

### A Ya VYf 'Dc]bh'@cUXg'f6 @ " & GY[ga ]W@cUX'LLff7 cbh]bi YXL

	T^{à^¦ÆŠæà^	Öã^&cã[}	Tæ*}ããå^ŽàÉÈËæá	Š[∧ā]}Žā]ÉÃá
- 1	T ÚG	Ý	ËŒĤ	ΪG
ĺ	ΤÏ	Ý	HECCH	Œ

#### A Ya VYf'5 f YU @cUXg'

 R[ā]oÁŒ	RĮ ã cÁÓ	RĮ ã cÁÔ	RĮ ã, cÁÖ	Öã^&cã}}	Öãrdãa ča[}	Tæ*}ããå^Ž[∙-á
•	• •	Þ[ÁÖæga	Ar Alla Artr		-	

## >c]bh'@UXg'UbX'9 bZcfWYX'8 ]gd`UWYa Ybhg'

R[ã]oÁŠænà∧	ŠÉÖĤE	Öã^&cã[}	Tæt}ãc å^ŽQàÉEdDÁQàÉæåDÁQàEaCHÈ
	Þ[ÁÖæææÁtÁÚ¦ð]cÁEE		

# INFINIGY8

#### **Bolt Calculation Tool, V1.6.1**

Boit Calculation 1001, VI.0.1				
PROJECT DATA				
Site Name:	BOBDL00110C			
Site Number:	BOBDL00110C			
Connection Description:	Clamps (Frame to Tower Leg)			

MAXIMUM BOLT LOADS					
Bolt Tension: 1565.83 lbs					
Bolt Shear: 629.78 lbs					

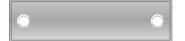
WORST CASE BOLT LOADS <sup>1</sup>					
Bolt Tension: 1565.83 lbs					
Bolt Shear:	603.83	lbs			

BOLT PROPERTIES				
Bolt Type:	Threaded Rod	-		
Bolt Diameter:	0.625	in		
Bolt Grade:	A449	-		
# of Threaded Rods:	2	-		
Threads Excluded?	No	-		

 $<sup>^{1}</sup>$  Worst case bolt loads correspond to Load combination #31 on member M25 in RISA-3D, which causes the maximum demand on the bolts.

# Member Information I nodes of M25, M26,

BOLT CHECK		
Tensile Strength	20340.15	
Shear Strength	12425.24	
Max Tensile Usage	7.7%	
Max Shear Usage	5.1%	
Interaction Check (Worst Case)	0.01	≤1.05
Result	Pass	





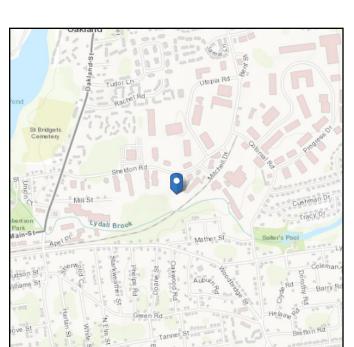
#### Address:

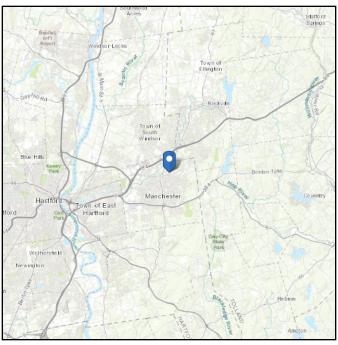
No Address at This Location

# **ASCE 7 Hazards Report**

Standard: ASCE/SEI 7-10 Elevation: 231.75 ft (NAVD 88)

Risk Category: || Latitude: 41.797333 Soil Class: D - Stiff Soil Longitude: -72.512056





#### Wind

#### Results:

Wind Speed

10-year MRI 77 Vmph

25-year MRI 87 Vmph 50-year MRI 93 Vmph 100-year MRI 101 Vmph 125 mph per Jurisdiction

Data Source: ASCE/SEI 7-10, Fig. 26.5-1A and Figs. CC-1–CC-4, and Section 26.5.2,

incorporating errata of March 12, 2014

Date Accessed: Wed Mar 16 2022

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

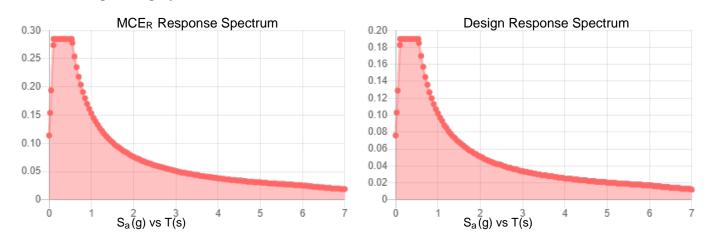
Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.



### **Seismic**

Site Soil Class: Results:	D - Stiff Soil			
S <sub>s</sub> :	0.178	S <sub>DS</sub> :	0.19	
$S_1$ :	0.064	$S_{D1}$ :	0.102	
Fa:	1.6	T <sub>L</sub> :	6	
F <sub>v</sub> :	2.4	PGA:	0.089	
S <sub>MS</sub> :	0.285	PGA <sub>M</sub> :	0.143	
S <sub>M1</sub> :	0.153	F <sub>PGA</sub> :	1.6	
		l <sub>e</sub> :	1	

#### Seismic Design Category B



Data Accessed: Wed Mar 16 2022

#### **Date Source:**

USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-10 Ch. 21 are available from USGS.



#### **Ice**

#### Results:

Ice Thickness: 1.00 in.

Concurrent Temperature: 5 F

Gust Speed 50 mph

Data Source: Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Wed Mar 16 2022

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

ASCE does not intend, nor should anyone interpret, the results provided by this Tool to replace the sound judgment of a competent professional, having knowledge and experience in the appropriate field(s) of practice, nor to substitute for the standard of care required of such professionals in interpreting and applying the contents of this Tool or the ASCE 7 standard.

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# Exhibit F

**Power Density/RF Emissions Report** 



# Radio Frequency Emissions Analysis Report



Site ID: BOBDL00110C

33 Mitchell Drive Manchester, CT 06042

March 28, 2022

Fox Hill Telecom Project Number: 220821

Site Compliance Summary					
Compliance Status:	COMPLIANT				
Site total MPE% of FCC general population allowable limit:	21.16 %				



March 28, 2022

Dish Wireless 5701 South Santa Fe Drive Littleton, CO 80120

Emissions Analysis for Site: BOBDL00110C

Fox Hill Telecom, Inc ("Fox Hill") was directed to analyze the proposed radio installation for Dish Wireless, LLC (Dish) facility located at **33 Mitchell Drive, Manchester, CT**, for the purpose of determining whether the emissions from the Proposed Dish radio and antenna installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu$ W/cm2). The number of  $\mu$ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu$ W/cm²). The general population exposure limits for the 600 MHz & 700 MHz bands are approximately 400  $\mu$ W/cm² and 467  $\mu$ W/cm² respectively. The general population exposure limit for the 1900 MHz (PCS) and 2100 MHz (AWS / AWS-4) bands is 1000  $\mu$ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.



#### **CALCULATIONS**

Calculations were performed for the proposed radio system installation for **Dish** on the subject site located at **33 Mitchell Drive, Manchester, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since **Dish** is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. All power values expressed and analyzed are maximum power levels expected to be used on all radios.

All emissions values for additional carriers were taken from the Connecticut Siting Council (CSC) active MPE database. Values in this database are provided by the individual carriers themselves

For each sector the following channel counts, frequency bands and power levels were utilized as shown in *Table 1*:

Technology	Frequency Band	Channel Count	Transmit Power per Channel (W)
5G	n71 (600 MHz)	4	61.5
5G	n70 (AWS-4 / 1995-2020)	4	40
5G	n66 (AWS-4 / 2180-2200)	4	40

Table 1: Channel Data Table



The following antennas listed in *Table 2* were used in the modeling for transmission in the 600 MHz (n71) frequency band and the 2100 MHz (AWS 4) frequency bands at 1995-2020 MHz (n70) and 2180-2200 MHz (n66). This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

			Antenna
	Antenna		Centerline
Sector	Number	Antenna Make / Model	(ft)
A	1	JMA MX08FRO665-21	170
В	1	JMA MX08FRO665-21	170
С	1	JMA MX08FRO665-21	170

Table 2: Antenna Data

All calculations were done with respect to uncontrolled / general population threshold limits.



# **RESULTS**

Per the calculations completed for the proposed **Dish** configurations *Table 3* shows resulting emissions power levels and percentages of the FCC's allowable general population limit.

					Total TX		
	Antenna Make /		Antenna Gain	Channel	Power		
Antenna ID	Model	Frequency Bands	(dBd)	Count	(W)	ERP (W)	MPE %
		n71 (600 MHz)/					
	JMA	n70 (AWS-4 / 1995-2020) /	11.45 / 16.15 /				
Antenna A1	MX08FRO665-21	n66 (AWS-4 / 2180-2200)	16.65	12	566	17,426.72	3.02
				Sec	ctor A Compo	site MPE%	3.02
		n71 (600 MHz)/					
	JMA	n70 (AWS-4 / 1995-2020) /	11.45 / 16.15 /				
Antenna B1	MX08FRO665-21	n66 (AWS-4 / 2180-2200)	16.65	12	566	17,426.72	3.02
Sector B Composite MPE%					3.02		
		n71 (600 MHz)/					
	JMA	n70 (AWS-4 / 1995-2020) /	11.45 / 16.15 /				
Antenna C1	MX08FRO665-21	n66 (AWS-4 / 2180-2200)	16.65	12	566	17,426.72	3.02
Sector C Composite MPE%						3.02	

Table 3: Dish Emissions Levels



The Following table (*table 4*) shows all additional carriers on site and their MPE% as recorded in the CSC active MPE database for this facility along with the newly calculated maximum **Dish** MPE contributions per this report. FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. For this site, all three sectors have the same configuration yielding the same results on all three sectors. *Table 5* below shows a summary for each **Dish** Sector as well as the composite MPE value for the site.

Site Composite MPE%					
Carrier	MPE%				
Dish – Max Per Sector Value	3.02 %				
Verizon Wireless	6.85 %				
T-Mobile	11.29 %				
Site Total MPE %:	21.16 %				

Table 4: All Carrier MPE Contributions

Dish Sector A Total:	3.02 %
Dish Sector B Total:	3.02 %
Dish Sector C Total:	3.02 %
Site Total:	21.16 %

Table 5: Site MPE Summary



FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. *Table 6* below details a breakdown by frequency band and technology for the MPE power values for the maximum calculated **Dish** sector(s). For this site, all three sectors have the same configuration yielding the same results on all three sectors.

Dish _ Frequency Band / Technology  Max Power Values  (Per Sector)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm²)	Frequency (MHz)	Allowable MPE (µW/cm²)	Calculated % MPE
Dish n71 (600 MHz) 5G	4	858.77	170	4.59	n71 (600 MHz)	400	1.15%
					n70 (AWS-4 / 1995-		
Dish n70 (AWS-4 / 1995-2020) 5G	4	1,648.39	170	8.81	2020)	1000	0.88%
					n66 (AWS-4 / 2180-		
Dish n66 (AWS-4 / 2180-2200) 5G	4	1,849.52	170	9.89	2200)	1000	0.99%
						Total:	3.02%

Table 6: Dish Maximum Sector MPE Power Values



### **Summary**

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the Dish facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

Dish Sector	Power Density Value (%)
Sector A:	3.02 %
Sector B:	3.02 %
Sector C:	3.02 %
Dish Maximum Total	3.02 %
(per sector):	3.02 %
Site Total:	21.16 %
	_
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **21.16** % of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

Scott Heffernan Principal RF Engineer

Fox Hill Telecom, Inc Holden, MA 01520 (978)660-3998

# Exhibit G

# **Letter of Authorization**

#### Mitchell Drive LLC

February 28, 2022

Chuck Regulbuto
Director of Operations
Northeast Site Solutions

RE:

Dish Wireless, LLC

Proposed Telecommunications facility at 33 Mitchell Drive, Manchester CT 06045

Chuck,

Mitchell Drive, LLC is the owner of the existing tower and property at 33 Mitchell Drive, Manchester Connecticut (the "Property"). This letter authorizes Dish Wireless and/or its authorized agent to file all necessary federal, state or local permits and approvals for the proposed wireless telecommunications facility at the property.

Please let us know if you have any questions or need anything further.

Sincerely,

Michael Bula, CPA

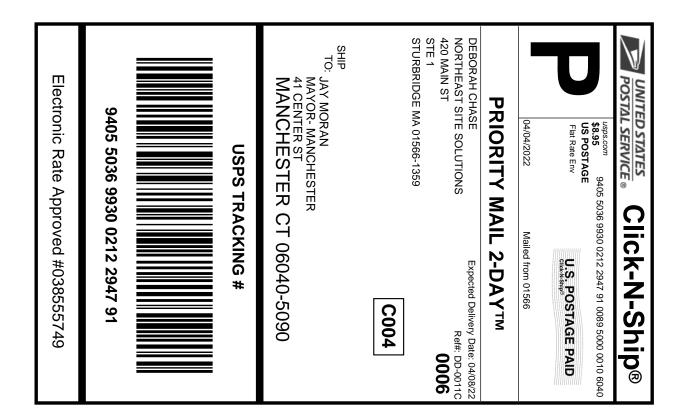
Chief Financial Officer

mulat

Mitchell Drive, LLC

# Exhibit H

**Recipient Mailings** 





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560440586 04/04/2022 Trans. #: Print Date: Ship Date: 04/04/2022 Delivery Date: 04/08/2022 Priority Mail® Postage: Total:

\$8.95 \$8.95

Ref#: DD-0011C

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

**STURBRIDGE MA 01566-1359** 

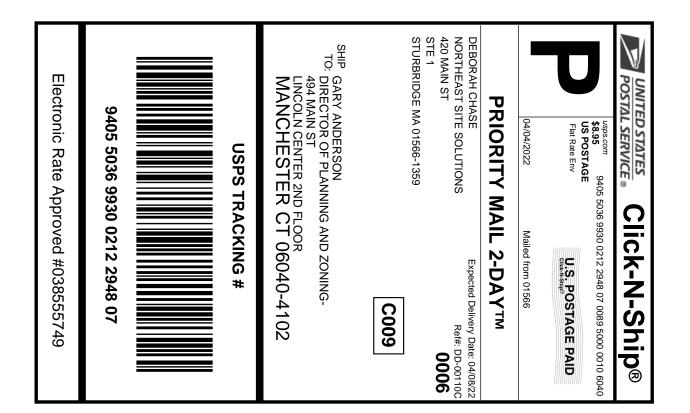
JAY MORAN

MAYOR- MANCHESTER

41 CENTER ST

MANCHESTER CT 06040-5090

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\$8.95 \$8.95

Ref#: DD-00110C

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

STURBRIDGE MA 01566-1359

**GARY ANDERSON** 

DIRECTOR OF PLANNING AND ZONING-MANCHESTER

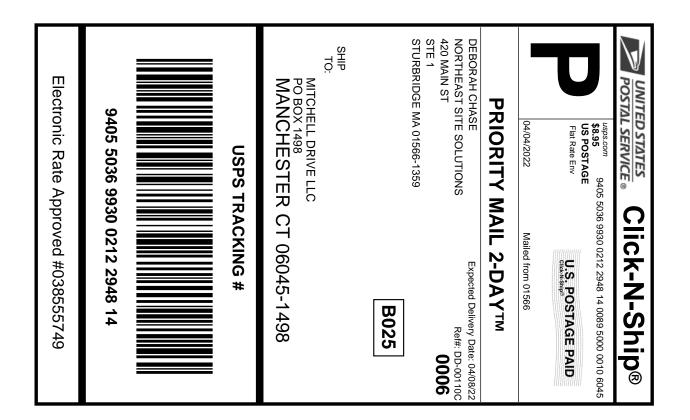
494 MAIN ST

LINCOLN CENTER 2ND FLOOR MANCHESTER CT 06040-4102

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560440586 04/04/2022 Trans. #: Print Date: Ship Date: 04/04/2022 Delivery Date: 04/08/2022 Priority Mail® Postage: \$8.95 \$8.95 Total:

Ref#: DD-00110C From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

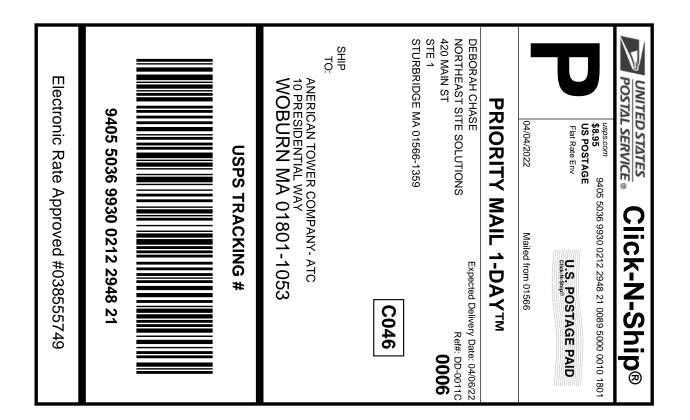
**STURBRIDGE MA 01566-1359** 

MITCHELL DRIVE LLC

PO BOX 1498

MANCHESTER CT 06045-1498

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\$8.95 \$8.95

Ref#: DD-0011C

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

STURBRIDGE MA 01566-1359

ANERICAN TOWER COMPANY- ATC

10 PRESIDENTIAL WAY WOBURN MA 01801-1053

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300 00110C



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03:21 PM

Product

Qty

Unit Price Price

\$0.00

\$0.00

\$0.00

\$0.00

Prepaid Mail

Woburn, MA 01801 Weight: 0 lb 12.00 oz

Acceptance Date: Tue 04/05/2022

Tracking #: 9405 5036 9930 0212 2948 21

Prepaid Mail

Manchester, CT 06040 Weight: O Ib 12.00 oz

Acceptance Date:

Tue 04/05/2022

Tracking #: 9405 5036 9930 0212 2948 07

Prepaid Mail

Manchester, CT 06045 Weight: 0 lb 12.00 oz

Acceptance Date:

Tue 04/05/2022

Tracking #: 9405 5036 9930 0212 2948 14

Prepaid Mail

Manchester, CT 06040 Weight: 0 lb 12.00 oz

Acceptance Date:

Tue 04/05/2022

Tracking #: 9405 5036 9930 0212 2947 91