



STATE OF CONNECTICUT

CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: siting.council@po.state.ct.us

Web Site: www.state.ct.us/csc/index.htm

March 12, 2003

Kenneth C. Baldwin
Robinson & Cole
280 Trumbull Street
Hartford, CT 06103-3597

RE: **EM-VER-077-030225** - Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at 266 Center Street, Manchester, Connecticut.

Dear Attorney Baldwin:

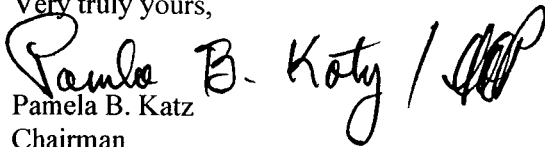
At a public meeting held on March 11, 2003, the Connecticut Siting Council (Council) acknowledged your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies.

The proposed modifications are to be implemented as specified here and in your notice dated February 25, 2003. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Any deviation from this format may result in the Council implementing enforcement proceedings pursuant to General Statutes § 16-50u including, without limitation, imposition of expenses resulting from such failure and of civil penalties in an amount not less than one thousand dollars per day for each day of construction or operation in material violation.

Thank you for your attention and cooperation.

Very truly yours,


Pamela B. Katz
Chairman

PBK/laf

c: Honorable Stephen T. Cassano, Mayor, Town of Manchester
Steven R. Werbner, General Manager, Town of Manchester
Thomas R. O'Marra, Zoning Enforcement Officer, Town of Manchester
Robert Stanford, Crown Atlantic Company, LLC

280 Trumbull Street
Hartford, CT 06103-3597
Main (860) 275-8200
Fax (860) 275-8299
kbaldwin@rc.com
Direct (860) 275-8345

February 25, 2003

Via Hand Delivery

RECEIVED

FEB 25 2003

**CONNECTICUT
SITING COUNCIL**

S. Derek Phelps
Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

**Re: Notice of Exempt Modification
Crown Atlantic Company, LLC Tower
266 Center Street
Manchester, Connecticut**

Dear Mr. Phelps:

Cellco Partnership d/b/a Verizon Wireless ("Cellco") intends to modify its existing antenna configuration on the Crown Atlantic Company, LLC ("Crown") tower at 266 Center Street in Manchester, Connecticut. Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for activity that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j 72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to the Manchester General Manager, Steven R. Werbner.

This tower is owned and operated by Crown and was approved by the Siting Council (the "Council") in Docket No. 129. Cellco's existing facility consists of fifteen (15) antennas attached to the tower at the 113-foot level and a single-story equipment shelter near the base of the tower. To improve system performance, Cellco now intends to replace the fifteen (15) antennas with twelve (12) antennas at the same 113-foot level on the tower. Eight of the twelve will be Antel RWB-80015/90LS antennas and the remaining four will be Antel RWA-80017LS antennas (see attached specifications). There are no changes proposed to any ground mounted structures or equipment.

The planned modifications to the Center Street facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).



Law Offices

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S. Derek Phelps
February 25, 2003
Page 2

1. The proposed modification will not increase the overall height of the existing tower. Cellco's replacement antennas will be mounted at the 113-foot level on the 115-foot tower.
2. The modifications to Cellco's antenna configuration does not effect any ground level equipment or structure and therefore will not require an extension of facility boundaries.
3. The proposed antenna modification will not increase the noise levels at the facility by six decibels or more.
4. The operation of the replacement antennas will not change radio frequency (RF) power density levels at the facility. Updated power density calculations are therefore not provided.

Also attached is the engineer's certification that the tower can support Cellco's proposed modifications. Please note that the full structural report, prepared by All-Points Technology, dated January 24, 2003 depicts the installation of twelve (12) DB844H80 panel antennas. Following the issuance of the report, Cellco decided that it would install eight (8) Antel RWB-80015/90LS and four (4) Antel RWA-80017LS antennas on the tower, in lieu of the DB844H80 antennas. A subsequent structural letter, prepared by All-Points Technology, dated February 3, 2003 is attached and verifies that the tower is capable of supporting the Antel antennas. Also attached are antenna specifications for the Antel antennas.

For the foregoing reasons, Cellco respectfully submits that the proposed modification of its antenna configuration at the Manchester facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,


Kenneth C. Baldwin

KCB:cag

cc: Steven R. Werbner, Manchester General Manager
Sandy M. Carter





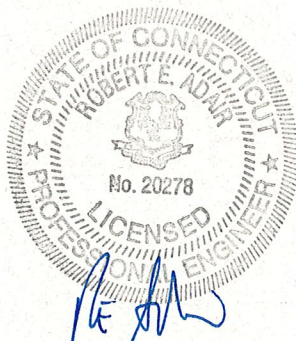
ALL-POINTS TECHNOLOGY CORPORATION, P.C.

**STRUCTURAL ANALYSIS REPORT
115' MONOPOLE TOWER
MANCHESTER, CONNECTICUT**

Prepared for
Verizon Wireless

Verizon Site: Manchester

January 24, 2003



APT Project #CT141230

**STRUCTURAL ANALYSIS REPORT
115' MONOPOLE TOWER
MANCHESTER, CONNECTICUT
prepared for
Verizon Wireless**

EXECUTIVE SUMMARY:

All-Points Technology Corporation, P.C. (APT) performed a structural analysis of this 115-foot monopole tower located in Manchester, Connecticut. The analysis was performed for Verizon Wireless' proposed replacement of their existing fifteen ALP7130.16 panel antennas with twelve DB844 panel antennas on the existing platform at 113'. Waveguide cables are to be twelve 1-5/8" cables.

Our analysis indicates the tower is capable of supporting the proposed antennas.

INTRODUCTION:

A structural analysis of this communications tower was performed by All-Points Technology Corporation, P.C. (APT) for Verizon Wireless. The tower is located off Route 6 in Manchester, Connecticut.

APT visited the tower site on January 15, 2003. Robert E. Adair, P.E. climbed the tower in its entirety to compile data necessary to perform the structural analysis. The analysis also used Valmont design drawings provided by Crown Castle Atlantic.

The structure is a 115-foot galvanized steel, three section monopole manufactured by Valmont Industries. The analysis was conducted using the following antenna inventory:

Antenna	Elev.	Mount	Coax.
(12) DB844H80 panels	113'	14' platform w/rails	(12) 1-5/8"
None	100'	14' low-profile platform	N.A.

STRUCTURAL ANALYSIS:

Methodology:

The structural analysis was done in accordance with TIA/EIA-222-F (EIA), Structural Standards for Steel Antenna Towers and Antenna Supporting Structures; and the American

All-Points Technology Corporation

150 Old Westside Road
North Conway, NH 03860
(603) 356-5214

711 North Mountain Road
Newington, CT 06111
(860) 1153-4444

Institute of Steel Construction (AISC), Manual of Steel Construction, Allowable Stress Design, Ninth Edition. The analysis was conducted using a wind speed of 80 miles per hour and one-half inch of radial ice over the entire structure and all appurtenances. The TIA/EIA Standard requires a minimum of 80-mph wind load for Hartford County, Connecticut.

Two analytical methods were used to evaluate the structure: a two-dimensional linear computer model developed by APT, and a P-delta analysis using CSTRAD finite element software distributed by Digital Canal Software. The 2-D model was used to generate dead loads of the tower and all of its appurtenances, radial ice loads and the resultant wind loading. The maximum bending moments and axial loads were used to calculate combined axial and bending stresses on each section of the monopole, which were compared to allowable stresses according to AISC and TIA/EIA.

Loads generated in the 2-D model were input into the CSTRAD program to evaluate secondary bending moments induced during deflection of the structure under load and to independently evaluate stresses. Evaluation of secondary bending moments is required by EIA paragraph 3.1.15. Our analysis indicates that the secondary moments exceed those of the linear analysis, and therefore govern in determining the capacity of the structure.

EIA requires two loading conditions to be evaluated to determine the tower's capacity. The higher stresses resulting from the two cases is used to calculate the tower capacity:

- Case 1 = Wind Load (without ice) + Tower Dead Load (controls)
- Case 2 = 0.75 Wind Load (with ice) + Ice Load + Tower Dead Load

EIA permits a one-third increase in allowable stresses for towers less than 700-feet tall. Allowable stresses of tower members were increased by one-third in computing the load capacity values indicated herein.

ANALYSIS RESULTS:

Our analysis determined the tower will support the proposed antenna array. The following table summarizes the capacity of the tower based on combined axial and bending stresses:

Elevation	Capacity
0'-30'	38%
30'-72'	32%
72'-115'	23%

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The capability of the existing foundation to support the proposed load was evaluated by comparing design reactions with those imposed by the proposed loading. We calculated reactions to be less than design reactions, indicating the existing foundation is adequate to support the proposed loads, provided it was designed and constructed to support original design reactions.

Base reactions imposed with the proposed antennas were calculated to be as follows:

Compression:	19.8 kips
Total Shear:	13.3 kips
Overturning Moment:	905 ft-kips

CONCLUSIONS AND SUGGESTIONS:

As detailed above, our analysis indicates that the existing 115' Valmont monopole tower and foundation in Manchester, Connecticut are capable of supporting Verizon Wireless' proposed antenna changes.

LIMITATIONS:

This report is based on the following:

1. Tower is properly installed and maintained.
2. All members are in new condition.
3. All bolts are in place and are properly tightened.
4. Tower is in plumb condition.
5. All tower members were properly designed, detailed, fabricated, and installed and have been properly maintained since erection.

All-Points Technology Corporation, P.C. (APT) is not responsible for any modifications completed prior to or hereafter which APT is not or was not directly involved. Modifications include but are not limited to:

1. Adding or relocating antennas.
2. Installing antenna mounting gates or side arms.
3. Extending tower.

APT hereby states that this document represents the entire report and that it assumes no liability for any factual changes that may occur after the date of this report. All representations, recommendations, and conclusions are based upon the information contained and set forth

All-Points Technology Corporation

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711 North Mountain Road
Newington, CT 06111
(860) 1153-4444

Verizon Wireless
115' Monopole, Manchester, MA
Crown BU #806372

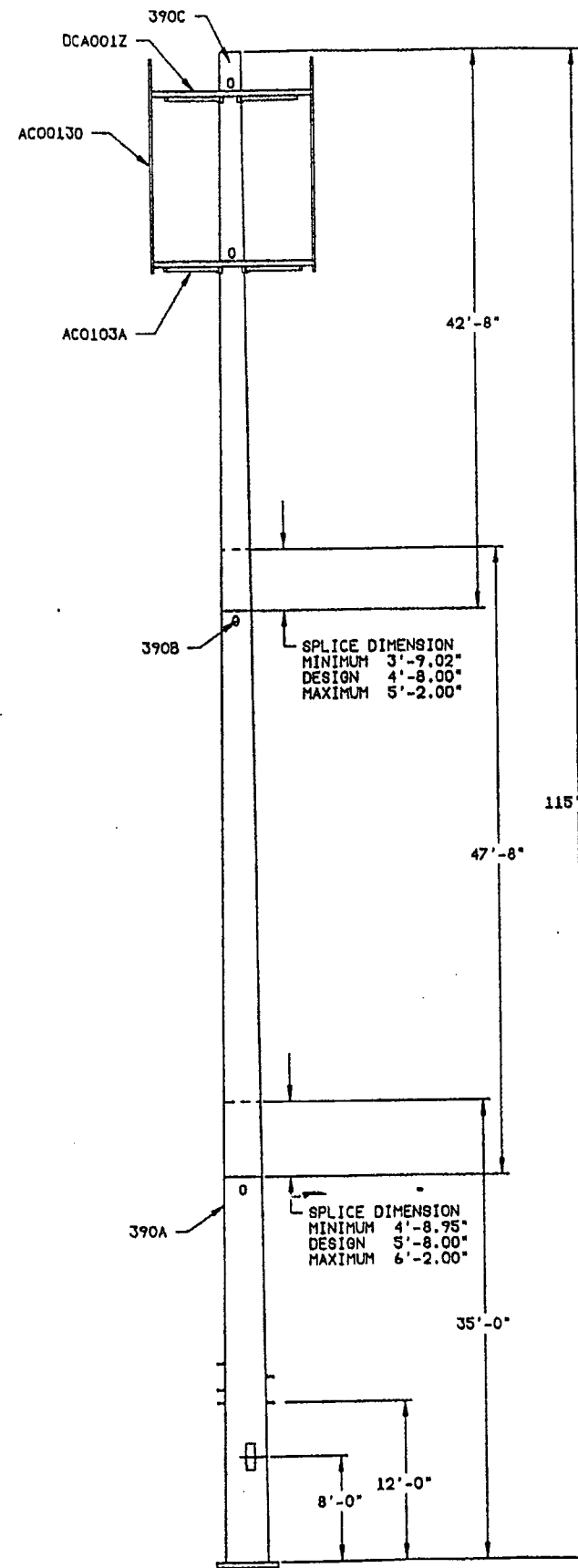
January 24, 2003
Page 4
APT Project #CT141230

herein. If you are aware of any information which conflicts with that which is contained herein, or you are aware of any defects arising from original design, material, fabrication, or erection deficiencies, you should disregard this report and immediately contact APT. APT disclaims all liability for any representation, recommendation, or conclusion not expressly stated herein.

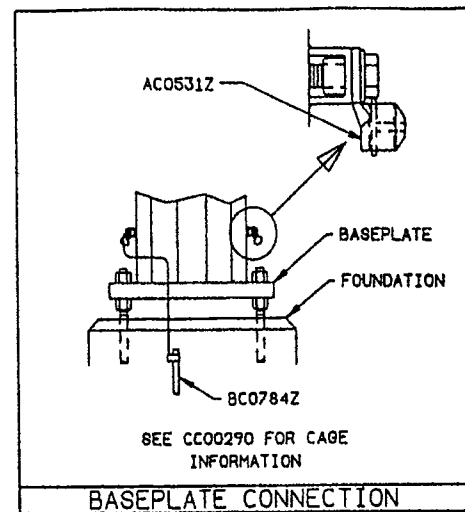
All-Points Technology Corporation

150 Old Westside Road
North Conway, NH 03860
(603) 356-5214

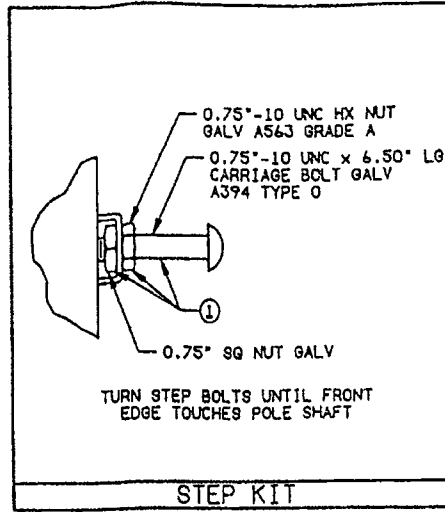
711 North Mountain Road
Newington, CT 06111
(860) 1153-4444



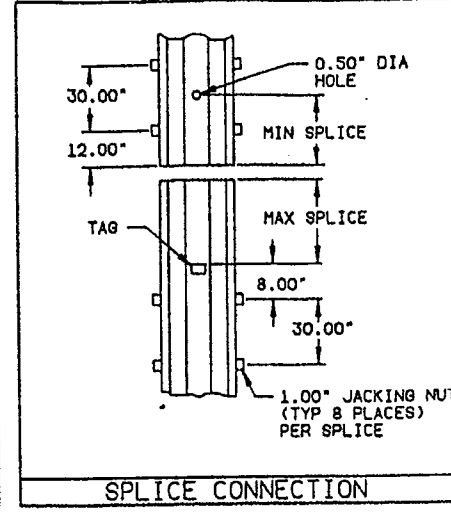
ERECTION DRAWING
SEE FABRICATION DRAWINGS
FOR ADDITIONAL DETAILS



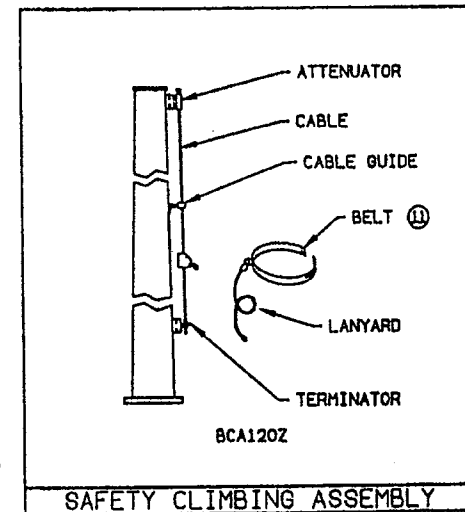
BASEPLATE CONNECTION



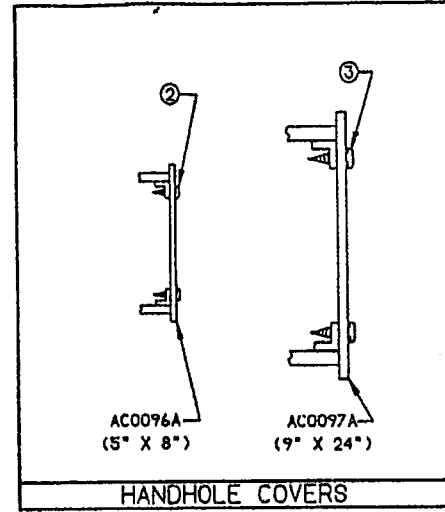
STEP KIT



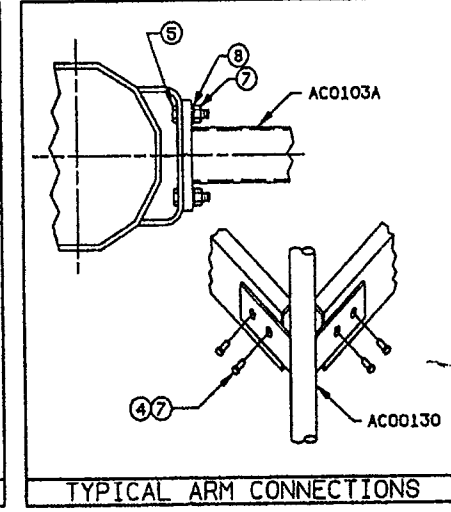
SPLICE CONNECTION



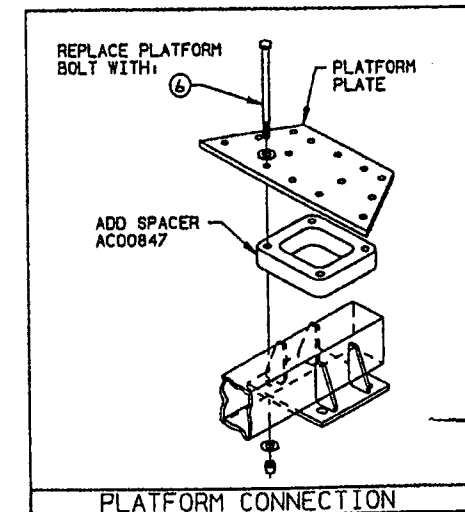
SAFETY CLIMBING ASSEMBLY



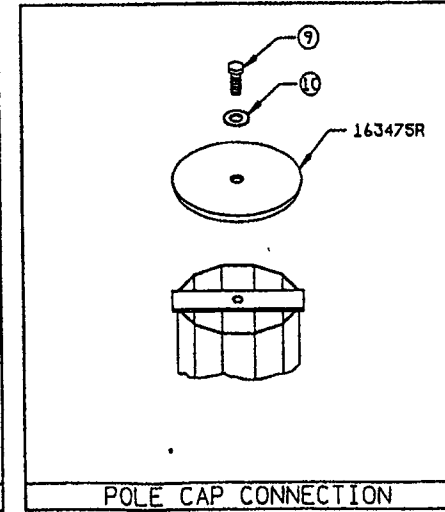
HANDHOLE COVERS



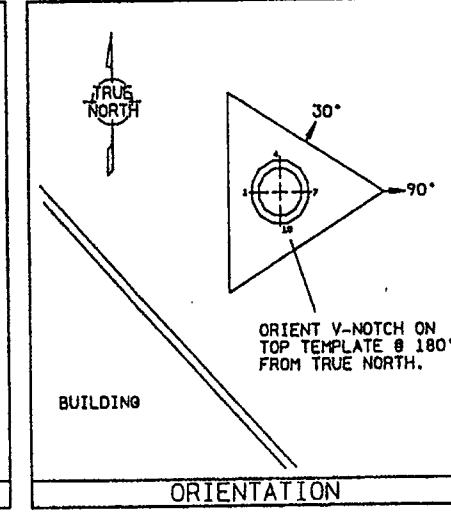
TYPICAL ARM CONNECTIONS



PLATFORM CONNECTION



POLE CAP CONNECTION



ORIENTATION

NOTES:

- POLE SHAFT-GOVERNING REACTIONS:
MOMENT = 28,352 IN-KIPS
SHEAR = 27,932 #
VERTICAL = 23,750 #
- COMPONENT IDENTIFICATION: TAG LOCATIONS ARE INDICATED BY CALLOUTS ON DRAWING. TAG MARK IN BILL OF MATERIAL SHOWS FIRST 4 DIGITS ON EACH TAG. SUBSEQUENT DIGITS WILL INDICATE SEQUENCE OF MANUFACTURER.
- ASSEMBLY AND ERECTION GUIDELINES: SEE VALMONT TRANSMISSION INSTALLATION GUIDELINE 1012.
- SLIP JOINT JACKING FORCE:
MINIMUM-25000#
MAXIMUM-90000#
- WEIGHT: WEIGHT IN TITLE BLOCK IS TOTAL STRUCTURE WEIGHT EXCLUDING ANCHORAGE.
- FINISH: GALVANIZED PER ASTM A-123.
- SITE: MANCHESTER, CT
- POLE DESIGNED FOR:
A. EIA-222-D, 90 MPH, 1/2" ICE.
B. 1-PLATFORM @ 112' ELEV.
C. 4-PD10017'S ON TOP PLATFORM.
D. 12-PD1132'S BETWEEN PLATFORMS,
6-WITH DOWN-TILTS.
E. 1-PLATFORM @ 99' ELEV.

BILL OF MATERIAL (SHIPPING SEQ.=1 FOR ALL)			
VALMONT PART NUMBER PREFIX TAG MARK	DESCRIPTION	UNIT WEIGHT (LBS)	QTY PER STR
DCO 390A	SECTION ASSEMBLY	7,825	1
DCO 390B	SECTION ASSEMBLY	5,700	1
DCO 390C	SECTION ASSEMBLY	2,800	1
CCO 0290	CAGE ASSEMBLY	2,040	1
ACO 0130	CORNER POST	57	3
ACO 103A	PLATFORM ARM	85	6
ACO 0847	SPACER PLATE	18	6
---	NOTE DOUBLE PLATFORM (DCA001Z)	2,186	1
---	NOTE E.I.A. GROUNDING (BCA784Z)	20	1
---	NOTE GROUND LUG KIT (AC0531Z)	1	2
---	NOTE SAFETY CLIMBING ASBY (BCA120Z)	44	1
MISCELLANEOUS			
ACO096A	HANDHOLE COVER (5 X 8)		3 8
ACO097A	HANDHOLE COVER (9 X 24)		9 1
163475R	POLE CAP (23" DIA)		30 1

ITEM NO	VALMONT PART NUMBER	DESCRIPTION				QTY PER STR
		HARDWARE SIZE (IN)	GENERAL	FINISH	ASTM SPEC	
		BOLT DIA	NUT WSHR			
1	2136A				STEP KIT	HQGV - 86
2	164035	0.25	0.75		SCREW	S.S. A410 18
3	161647	0.38	1.00		SCREW	S.S. A410 4
4	161147	0.63	1.50			HQGV A325 26
5	161170	0.63	2.50			HQGV A325 26
6	161678	0.63	8.00			HQGV A325 26
7	333014		0.63		LOCK	HQGV A563 52
8	142070		0.63		FLAT	HQGV F436 26
9	161171	0.75	1.75			HQGV A325 1
10	142056		0.75		FLAT	HQGV F436 1
11	ABLBELT				SAFETYBELT	- 1

* EXCEPT 46.2 & 8.5

DUPLICATE DRAWING DISTRIBUTION

REV ID		DATE		REV BY	CHK BY	REVISION DESCRIPTION
1		05-01-90		MAL		05-01-90 MAL
MATERIAL THICKNESS SPECIFICATIONS						
SIZE	WALL	THICKNESS	E.A. BK			
19,621#	NONE					
VALMONT						
VALMONT TRANSMISSION, INC.						
110645-90 METRO MOBILE-CT 115' CELLULAR DC0390Z						

All-Points Technology Corp., P.C.

150 Old Westside Road
 North Conway, NH 03860
 (603) 356-5214

Client: **Verizon Wireless**
 Job: **Manchester, CT**
 Calculated By: **R. Adair**

Job No.: **CT141230**
 Date: **16-Jan-03**

General Information

Tower Manufacturer Valmont
 Tower Type Monopole
 Total Height of Tower 115 ft.
 Wind Speed EIA Hartford Cty 80 mph.
 Radial Ice 0.5 in.
 25% Reduction for ice yes (yes or no)
 1/3 increase for allowable loads yes (yes or no)
 Number of faces 12 faces

Calculations based on EIA/TIA-222-F, using the following formulas:

Force on discrete appurtenance: $F=Qz*Gh*Ca*A$

Force on microwave antennae: $F=Cr*A*Gh*Kz*V^2$, where $Cr=((Ca^2)+(Cs^2))^(1/2)$

Gust response factor Gh= 1.69

V as specified EIA-222-F

E (Modulus of Elasticity) 29000 ksi
 Fb 0.6
 K 1
 Min. Width = 21.91 in
 Max. Width = 43.85 in
 Slope of Tower = 0.0159 in/in

Tower Information

Section	Length (ft.)	Midpt Elev.	Base Width (in.)	Top Width (in.)	Area (sf) w/o Ice	Area (sf) w/ Ice	Wall Thknss	Wt. (lbs) Tower	Wt. (lbs) Ice	
11	10.00	110.00	23.82	21.91	19.05	19.89	0.219	689.20	142.72	
10	11.00	99.50	25.92	23.82	22.79	23.71	0.219	825.18	170.46	
9	11.00	88.50	28.02	25.92	24.72	25.64	0.219	895.44	184.56	
8	11.00	77.50	30.11	28.02	26.64	27.56	0.219	854.27	198.66	
7	10.00	67.00	32.02	30.11	25.89	26.72	0.313	1183.41	192.84	
6	10.00	57.00	33.93	32.02	27.48	28.31	0.313	1256.82	204.49	
5	11.00	46.50	36.03	33.93	32.06	32.98	0.313	1467.30	238.40	
4	11.00	35.50	38.13	36.03	33.99	34.90	0.313	1556.13	252.50	
3	10.00	25.00	40.03	38.13	32.57	33.40	0.375	1784.37	241.78	
2	10.00	15.00	41.94	40.03	34.16	34.99	0.375	1872.33	253.44	
1	10.00	5.00	43.85	41.94	35.75	36.58	0.375	1960.28	265.09	
								Total	14345	2345

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Client: **Verizon Wireless**
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Job No.: **CT141230**
 Date: **16-Jan-03**

Monopole Properties

Section	I in ⁴	Area in ²	Area mid	I mid	J mid	r in	S in ³	L / side in
Top	919.91	15.27						
11	1184.59	16.62	15.9	1052	2104	8.44	99.47	6.38
10	1529.54	18.10	17.4	1357	2714	9.19	118.04	6.94
9	1935.69	19.57	18.8	1733	3465	9.94	138.19	7.51
8	2408.04	21.05	20.3	2172	4344	10.70	159.93	8.07
7	4107.04	31.91	26.5	3258	6515	11.34	256.52	8.58
6	4893.82	33.83	32.9	4500	9001	12.03	288.47	9.09
5	5868.71	35.94	34.9	5381	10763	12.78	325.79	9.65
4	6965.17	38.06	37.0	6417	12834	13.53	365.37	10.22
3	9627.71	47.82	42.9	8296	16593	14.19	480.97	10.73
2	11084.96	50.12	49.0	10356	20713	14.87	528.58	11.24
1	12682.31	52.42	51.3	11884	23767	15.55	578.44	11.75

Tower Dead Load Summary

Elev.	Dead load	Dead load
	Tower (lbs)	Ice (lbs)
94.0	1514	313
83.0	2410	498
72.0	3264	696
62.0	4447	889
52.0	5704	1094
41.0	7172	1332
30.0	8728	1585
20.0	10512	1826
10.0	12384	2080
0.0	14345	2345

All-Points Technology Corp., P.C.

150 Old Westside Road
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(603) 356-5214

Client: **Verizon Wireless**
Job: **Manchester, CT**
Calculated By: **R. Adair**

Job No.: **CT141230**
Date: **16-Jan-03**

Antenna Information

Wind Velocity= 80 mph
Tower Hgt= 115 ft.

ANTENNAS

Type	Elev. (z)	Coeff. (C)	Kz	Qz	Area (no ice)	Force (no ice)	Weight	EI/Dist
(12) DB844 on 14' platform w/rails	113	1.4	1.42	23.29	41.9	2310	1400	11-8
14' LP platform	100	2.0	1.37	22.49	7.0	532	1000	10-6

LINEAR APPURTENANCES

Section	Area w/o Ice	Area w/ Ice	Weight w/o Ice	Weight w/ Ice
11	0.31	1.15	130	140
10	0.34	1.26	143	154
9	0.34	1.26	143	154
8	0.34	1.26	143	154
7	0.31	1.15	130	140
6	0.31	1.15	130	140
5	0.34	1.26	143	154
4	0.34	1.26	143	154
3	0.31	1.15	130	140
2	0.31	1.15	130	140
1	0.31	1.15	130	140

All-Points Technology Corp., P.C.

150 Old Westside Road
 North Conway, NH 03860
 (603) 356-5214

Client: **Verizon Wireless**
 Job: **Manchester, CT**
 Calculated By: **R. Adair**

Job No.: **CT141230**
 Date: **16-Jan-03**

Wind Velocity = 80 mph
 Height of Tower = 115 feet

$K_z = \text{Exposure coefficient} = (z/33)^{2/7}; 1.00 \leq K_z \leq 2.58$

$Q_z = \text{Velocity pressure} = .00256 * K_z * V^2$

$G_h = \text{Gust response factor} = 1.69$

$C_f = \text{Structure force coefficient from Table 1 of TIA/EIA}$

$\text{Force} = Q_z * G_h * (C_f * A_e + C_a * A_a)$

Wind Load Without Ice

Section	Midpoint Height	Areas		Kz	Qz	Gh	Cf	Wind Load	Wind Load
		Ae	Aa						
11	110.00	19.1	0.31	1.41	23.11	1.69	1.03	781 lbs.	78 plf.
10	99.50	22.8	0.34	1.37	22.46	1.69	1.03	907 lbs.	82 plf.
9	88.50	24.7	0.34	1.33	21.72	1.69	1.03	950 lbs.	86 plf.
8	77.50	26.6	0.34	1.28	20.91	1.69	1.03	984 lbs.	89 plf.
7	67.00	25.9	0.31	1.22	20.06	1.69	1.03	917 lbs.	92 plf.
6	57.00	27.5	0.31	1.17	19.15	1.69	1.03	928 lbs.	93 plf.
5	46.50	32.1	0.34	1.10	18.07	1.69	1.03	1021 lbs.	93 plf.
4	35.50	34.0	0.34	1.02	16.73	1.69	1.03	1001 lbs.	91 plf.
3	25.00	32.6	0.31	1.00	16.38	1.69	1.03	939 lbs.	94 plf.
2	15.00	34.2	0.31	1.00	16.38	1.69	1.03	985 lbs.	98 plf.
1	5.00	35.7	0.31	1.00	16.38	1.69	1.03	1030 lbs.	103 plf.

All-Points Technology Corp., P.C.

150 Old Westside Road
North Conway, NH 03860
(603) 356-5214

Client: **Verizon Wireless**
Job: **Manchester, CT**
Calculated By: **R. Adair**

Job No.: **CT141230**
Date: **16-Jan-03**

Wind Load With Ice

Section	Midpoint Height	Areas		Kz	Qz	Gh	Cf	Wind Load	75% Wind Load
		Ae	Ai						
11	110.00	19.9	1.15	1.41	23.11	1.69	1.03	854 lbs.	64 plf.
10	99.50	23.7	1.26	1.37	22.46	1.69	1.03	984 lbs.	67 plf.
9	88.50	25.6	1.26	1.33	21.72	1.69	1.03	1025 lbs.	70 plf.
8	77.50	27.6	1.26	1.28	20.91	1.69	1.03	1057 lbs.	72 plf.
7	67.00	26.7	1.15	1.22	20.06	1.69	1.03	980 lbs.	73 plf.
6	57.00	28.3	1.15	1.17	19.15	1.69	1.03	988 lbs.	74 plf.
5	46.50	33.0	1.26	1.10	18.07	1.69	1.03	1084 lbs.	74 plf.
4	35.50	34.9	1.26	1.02	16.73	1.69	1.03	1059 lbs.	72 plf.
3	25.00	33.4	1.15	1.00	16.38	1.69	1.03	991 lbs.	74 plf.
2	15.00	35.0	1.15	1.00	16.38	1.69	1.03	1036 lbs.	78 plf.
1	5.00	36.6	1.15	1.00	16.38	1.69	1.03	1081 lbs.	81 plf.

Wind Loads

Frame Static Analysis Report

Project: CT141230 Manchester
 Description: 115' Valmont Monopole
 Date: 01/21/2003 10:15 AM

Company: All-Points Technology Corporation
 User: Robert Adair, P.E.
 Software: Digital Canal Frame Analysis & Design

NODAL COORDINATES			BOUNDARY CONDITIONS (F=FIX, S=SUP, M=MASTER/SLAVE)								
NODE NO	REBAND NO	X	Y	Z	NODE TEMP	ALPHA	BETA	GAMMA	DIR	DDDOOO XYZXYZ	STIFFNESS
Units:		Ft	Ft	Ft	F	Deg	Deg	Deg			K /In /Deg
1	1	0.00	0.00	0.00	0.00	0.00	0.00	0.00		FFFFFF	
2	2	0.00	10.00	0.00	0.00	0.00	0.00	0.00			
3	3	0.00	20.00	0.00	0.00	0.00	0.00	0.00			
4	4	0.00	30.00	0.00	0.00	0.00	0.00	0.00			
5	5	0.00	41.00	0.00	0.00	0.00	0.00	0.00			
6	6	0.00	52.00	0.00	0.00	0.00	0.00	0.00			
7	7	0.00	62.00	0.00	0.00	0.00	0.00	0.00			
8	8	0.00	72.00	0.00	0.00	0.00	0.00	0.00			
9	9	0.00	83.00	0.00	0.00	0.00	0.00	0.00			
10	10	0.00	94.00	0.00	0.00	0.00	0.00	0.00			
11	11	0.00	105.00	0.00	0.00	0.00	0.00	0.00			
12	12	0.00	115.00	0.00	0.00	0.00	0.00	0.00			

2 NODE PRISMATIC BEAM ELEMENT														
E	NE NO	PE NO	ALPHA	BETA	GAMMA	LENGTH	MAT TYPE	PROP TYPE	RELEASE NE PE	REF TEMP	DIR	OFFSET NE PE	STIFFNESS NE PE	
Units:			Deg	Deg	Deg	Ft				F		Ft	Ft	K /In /Deg K /In /Deg
1	1	2	90.00	-90.00	0.00	10.00	1	1						
2	2	3	90.00	-90.00	0.00	10.00	1	2						
3	3	4	90.00	-90.00	0.00	10.00	1	3						
4	4	5	90.00	-90.00	0.00	11.00	1	4						
5	5	6	90.00	-90.00	0.00	11.00	1	5						
6	6	7	90.00	-90.00	0.00	10.00	1	6						
7	7	8	90.00	-90.00	0.00	10.00	1	7						
8	8	9	90.00	-90.00	0.00	11.00	1	8						
9	9	10	90.00	-90.00	0.00	11.00	1	9						
10	10	11	90.00	-90.00	0.00	11.00	1	10						
11	11	12	90.00	-90.00	0.00	10.00	1	11						

MATERIAL PROPERTIES						
MATL NO	DESIGNATION	YOUNG'S MODULUS	POISSON'S RATIO	THERMAL COEFF	MASS DENSITY	WEIGHT DENSITY
Units:		K /In ^2		F	Slug/Ft^3	Lb/Ft ^3
1	Steel	2.9e+004	0.295	6.5e-006	15.2	490
2	Cable	9e+004	0.151	6.5e-006	11.9	382

2 NODE PRISMATIC BEAM ELEMENT PROPERTIES									
PROP	DESIGNATION	A	IXX	IYY	J	IXY	SFY	SFX	CW
Units:									
		In^2	In^4	In^4	In^4	In^4			In^6
1	MANCH1	51.3	1.19e+004	1.19e+004	2.38e+004	0	1.000	1.000	0
2	MANCH2	49	1.04e+004	1.04e+004	2.07e+004	0	1.000	1.000	0
3	MANCH3	42.9	8.3e+003	8.3e+003	1.66e+004	0	1.000	1.000	0
4	MANCH4	37	6.42e+003	6.42e+003	1.28e+004	0	1.000	1.000	0
5	MANCH5	34.9	5.38e+003	5.38e+003	1.08e+004	0	1.000	1.000	0
6	MANCH6	32.9	4.5e+003	4.5e+003	9e+003	0	1.000	1.000	0
7	MANCH7	26.5	3.26e+003	3.26e+003	6.52e+003	0	1.000	1.000	0
8	MANCH8	20.3	2.17e+003	2.17e+003	4.34e+003	0	1.000	1.000	0
9	MANCH9	18.8	1.73e+003	1.73e+003	3.47e+003	0	1.000	1.000	0
10	MANCH10	17.4	1.36e+003	1.36e+003	2.71e+003	0	1.000	1.000	0
11	MANCH11	15.9	1.05e+003	1.05e+003	2.1e+003	0	1.000	1.000	0

2 NODE PRISMATIC BEAM ELEMENT LOAD INFORMATION											
REC	LOAD	LOAD	DIST								
NO	TYPE	SYS	SPEC	DIST	PX	PY	PZ	MX	MY	MZ	
Units:											
				Ft	K	K	K	Ft-K	Ft-K	Ft-K	
DESCRIPTION : Wind on section 1											
LOAD CASES : 1											
ELEMENT LIST : 1											
1	LINR	GLO	FRAC	B	0.000	0.103	0.000	0.000	0.000	0.000	0.000
				E	1.000	0.103	0.000	0.000	0.000	0.000	0.000
DESCRIPTION : Wind on section 2											
LOAD CASES : 1											
ELEMENT LIST : 2											
2	LINR	GLO	FRAC	B	0.000	0.098	0.000	0.000	0.000	0.000	0.000
				E	1.000	0.098	0.000	0.000	0.000	0.000	0.000
DESCRIPTION : Wind on section 3											
LOAD CASES : 1											
ELEMENT LIST : 3											
3	LINR	GLO	FRAC	B	0.000	0.094	0.000	0.000	0.000	0.000	0.000
				E	1.000	0.094	0.000	0.000	0.000	0.000	0.000
DESCRIPTION : Wind on section 4											
LOAD CASES : 1											
ELEMENT LIST : 4											
4	LINR	GLO	FRAC	B	0.000	0.091	0.000	0.000	0.000	0.000	0.000
				E	1.000	0.091	0.000	0.000	0.000	0.000	0.000
DESCRIPTION : Wind on section 5 & 6											
LOAD CASES : 1											
ELEMENT LIST : 5											
5	LINR	GLO	FRAC	B	0.000	0.093	0.000	0.000	0.000	0.000	0.000
				E	1.000	0.093	0.000	0.000	0.000	0.000	0.000
DESCRIPTION : Wind on section 5 & 6											
LOAD CASES : 1											
ELEMENT LIST : 6											
	LINR	GLO	FRAC	B	0.000	0.093	0.000	0.000	0.000	0.000	0.000
				E	1.000	0.093	0.000	0.000	0.000	0.000	0.000
DESCRIPTION : Wind on section 7											

LOAD CASES : 1
ELEMENT LIST : 7

7	LINR	GLO	FRAC	B	0.000	0.092	0.000	0.000	0.000	0.000	0.000
				E	1.000	0.092	0.000	0.000	0.000	0.000	0.000

DESCRIPTION : Wind on section 8
LOAD CASES : 1
ELEMENT LIST : 8

8	LINR	GLO	FRAC	B	0.000	0.089	0.000	0.000	0.000	0.000	0.000
				E	1.000	0.089	0.000	0.000	0.000	0.000	0.000

DESCRIPTION : Wind on section 9
LOAD CASES : 1
ELEMENT LIST : 9

9	LINR	GLO	FRAC	B	0.000	0.086	0.000	0.000	0.000	0.000	0.000
				E	1.000	0.086	0.000	0.000	0.000	0.000	0.000

DESCRIPTION : Wind on section 10
LOAD CASES : 1
ELEMENT LIST : 10

10	LINR	GLO	FRAC	B	0.000	0.082	0.000	0.000	0.000	0.000	0.000
				E	1.000	0.082	0.000	0.000	0.000	0.000	0.000

DESCRIPTION : Low-profile platform @ 100'
LOAD CASES : 1
ELEMENT LIST : 10
DISTANCES : 6

11	CONC	GLO	DIST			0.532	-1.000	0.000	0.000	0.000	0.000
----	------	-----	------	--	--	-------	--------	-------	-------	-------	-------

DESCRIPTION : Wind on section 11
LOAD CASES : 1
ELEMENT LIST : 11

12	LINR	GLO	FRAC	B	0.000	0.078	0.000	0.000	0.000	0.000	0.000
				E	1.000	0.078	0.000	0.000	0.000	0.000	0.000

DESCRIPTION : (12) DB844 on platform @ 113'
LOAD CASES : 1
ELEMENT LIST : 11
DISTANCES : 8

13	CONC	GLO	DIST			2.310	-1.400	0.000	0.000	0.000	0.000
----	------	-----	------	--	--	-------	--------	-------	-------	-------	-------

GRAVITY LOAD MULTIPLIERS

REC NO	PX	PY	PZ
DESCRIPTION : Self Weight			
LOAD CASES : 1			
ELEMENT LIST : 1			
1	0.000	-1.300	0.000
DESCRIPTION : Self Weight			
LOAD CASES : 1			
ELEMENT LIST : 2			
2	0.000	-1.300	0.000
DESCRIPTION : Self Weight			
LOAD CASES : 1			
ELEMENT LIST : 3			
3	0.000	-1.300	0.000

DESCRIPTION : Self Weight

LOAD CASES : 1

ELEMENT LIST : 4

0.000 -1.300 0.000

DESCRIPTION : Self Weight

LOAD CASES : 1

ELEMENT LIST : 5

5 0.000 -1.300 0.000

DESCRIPTION : Self Weight

LOAD CASES : 1

ELEMENT LIST : 6

6 0.000 -1.300 0.000

DESCRIPTION : Self Weight

LOAD CASES : 1

ELEMENT LIST : 7

7 0.000 -1.300 0.000

DESCRIPTION : Self Weight

LOAD CASES : 1

ELEMENT LIST : 8

8 0.000 -1.300 0.000

DESCRIPTION : Self Weight

LOAD CASES : 1

ELEMENT LIST : 9

0.000 -1.300 0.000

DESCRIPTION : Self Weight

LOAD CASES : 1

ELEMENT LIST : 10

10 0.000 -1.300 0.000

DESCRIPTION : Self Weight

LOAD CASES : 1

ELEMENT LIST : 11

11 0.000 -1.300 0.000

REC NO	ALPHA	BETA	GAMMA	PX	N O D A L L O A D S			MX	MY	MZ
					PY	PZ				
Units:	Deg	Deg	Deg	K	K	K	Ft-K	Ft-K	Ft-K	

DESCRIPTION : Waveguide d.l.

LOAD CASES : 1

NODE LIST : 2

1 0.00 0.00 0.00 0.000 -0.130 0.000 0.000 0.000 0.000

DESCRIPTION : Waveguide d.l.

LOAD CASES : 1

NODE LIST : 3

1 0.00 0.00 0.00 0.000 -0.130 0.000 0.000 0.000 0.000

DESCRIPTION : Waveguide d.l.

LOAD CASES : 1

NODE LIST : 4

3	0.00	0.00	0.00	0.000	-0.130	0.000	0.000	0.000	0.000	
DESCRIPTION : Waveguide d.l.										
LOAD CASES : 1										
N. LIST : 5										
4	0.00	0.00	0.00	0.000	-0.143	0.000	0.000	0.000	0.000	
DESCRIPTION : Waveguide d.l.										
LOAD CASES : 1										
NODE LIST : 6										
5	0.00	0.00	0.00	0.000	-0.143	0.000	0.000	0.000	0.000	
DESCRIPTION : Waveguide d.l.										
LOAD CASES : 1										
NODE LIST : 7										
6	0.00	0.00	0.00	0.000	-0.130	0.000	0.000	0.000	0.000	
DESCRIPTION : Waveguide d.l.										
LOAD CASES : 1										
NODE LIST : 8										
7	0.00	0.00	0.00	0.000	-0.130	0.000	0.000	0.000	0.000	
DESCRIPTION : Waveguide d.l.										
LOAD CASES : 1										
NODE LIST : 9										
8	0.00	0.00	0.00	0.000	-0.143	0.000	0.000	0.000	0.000	
DESCRIPTION : Waveguide d.l.										
LOAD CASES : 1										
N. LIST : 10										
9	0.00	0.00	0.00	0.000	-0.143	0.000	0.000	0.000	0.000	
DESCRIPTION : Waveguide d.l.										
LOAD CASES : 1										
NODE LIST : 11										
10	0.00	0.00	0.00	0.000	-0.143	0.000	0.000	0.000	0.000	
DESCRIPTION : Waveguide d.l.										
LOAD CASES : 1										
NODE LIST : 12										
11	0.00	0.00	0.00	0.000	-0.130	0.000	0.000	0.000	0.000	

L I N E A R A N A L Y S I S R E S U L T S

NODAL NO	LOAD COMB	N O D A L D I S P L A C E M E N T S					
		DX	DY	DZ	OX	OY	OZ
		(* Indicates Displacements Occur in Nodal Local System)					
Units:		In	In	In	Deg	Deg	Deg
2	1	0.2150	-0.0015	0.0000	0.0000	0.0000	-0.1977
3	1	0.8396	-0.0029	0.0000	0.0000	0.0000	-0.3909
4	1	1.8796	-0.0043	0.0000	0.0000	0.0000	-0.5935
5	1	3.5312	-0.0057	0.0000	0.0000	0.0000	-0.8288
6	1	5.7089	-0.0071	0.0000	0.0000	0.0000	-1.0498
7	1	8.1125	-0.0082	0.0000	0.0000	0.0000	-1.2347
8	1	10.9119	-0.0093	0.0000	0.0000	0.0000	-1.4258
9	1	14.4696	-0.0106	0.0000	0.0000	0.0000	-1.6439
10	1	18.4686	-0.0118	0.0000	0.0000	0.0000	-1.8085
11	1	22.7677	-0.0127	0.0000	0.0000	0.0000	-1.9051
12	1	26.7956	-0.0131	0.0000	0.0000	0.0000	-1.9286

2 NODE PRISMATIC BEAM ELEMENT -- ELEMENT REPORTS

E_N LOAD NODE SIGN CONVENTION : BEAM DESIGNERS
 No COMB NO AXIAL TORSION SHEAR X MOMENT Y MAX MOM/DEFL DIST SHEAR Y MOMENT X MAX MOM/DEFL DIST

Units: K K -Ft K K -Ft K -Ft / In Ft K K -Ft K -Ft / In Ft

REPORT TYPE : Max Forces Full
 ELEMENT LIST : 1-11

E _N No	LOAD COMB	NODE NO	AXIAL	TORSION	SHEAR X	MOMENT Y	MAX MOM/DEFL	DIST	SHEAR Y	MOMENT X	MAX MOM/DEFL	DIST
1	1	1	-19.8085	0.0000	0.0000	0.0000			13.2730	-890.4775		
		2	-17.5392	0.0000	0.0000	0.0000			12.2430	-762.8975	0.0517	4.94
2	1	2	-17.4092	0.0000	0.0000	0.0000			12.2430	-762.8975		
		3	-15.2416	0.0000	0.0000	0.0000			11.2630	-645.3675	0.0506	4.93
3	1	3	-15.1116	0.0000	0.0000	0.0000			11.2630	-645.3675		
		4	-13.2139	0.0000	0.0000	0.0000			10.3230	-537.4375	0.0530	4.92
4	1	4	-13.0839	0.0000	0.0000	0.0000			10.3230	-537.4375		
		5	-11.2835	0.0000	0.0000	0.0000			9.3220	-429.3900	0.0677	5.40
5	1	5	-11.1405	0.0000	0.0000	0.0000			9.3220	-429.3900		
		6	-9.4423	0.0000	0.0000	0.0000			8.2990	-332.4745	0.0636	5.38
6	1	6	-9.2993	0.0000	0.0000	0.0000			8.2990	-332.4745		
		7	-7.8439	0.0000	0.0000	0.0000			7.3690	-254.1345	0.0484	4.89
7	1	7	-7.7139	0.0000	0.0000	0.0000			7.3690	-254.1345		
		8	-6.5416	0.0000	0.0000	0.0000			6.4490	-185.0445	0.0500	4.87
8	1	8	-6.4116	0.0000	0.0000	0.0000			6.4490	-185.0445		
		9	-5.4238	0.0000	0.0000	0.0000			5.4700	-119.4900	0.0627	5.30
9	1	9	-5.2808	0.0000	0.0000	0.0000			5.4700	-119.4900		
		10	-4.3660	0.0000	0.0000	0.0000			4.5240	-64.5230	0.0473	5.22
10	1	10	-4.2230	0.0000	0.0000	0.0000			4.5240	-64.5230		
		11	-2.3764	0.0000	0.0000	0.0000			3.0900	-22.3800	0.0276	5.03
11	1	11	-2.2334	0.0000	0.0000	0.0000			3.0900	-22.3800		
		12	-0.1300	0.0000	0.0000	0.0000			0.0000	0.0000	-4.0278	10.00

R E A C T I O N S

(* Indicates Reactions Occur in Nodal Local System)

NODE NO LOAD COMB PX PY PZ MX MY MZ

Units: K K K K -Ft K -Ft K -Ft

1	1		-13.2730	19.8085	0.0000	0.0000	0.0000	890.4775
---	---	--	----------	---------	--------	--------	--------	----------

P-D E L T A A N A L Y S I S R E S U L T S

NODE NO	LOAD COMB	N O D A L D I S P L A C E M E N T S					
		DX	DY	DZ	OX	OY	OZ
		(* Indicates Displacements Occur in Nodal Local System)					
Units:		In	In	In	Deg	Deg	Deg
2	1	0.2187	-0.0017	0.0000	0.0000	0.0000	-0.2012
3	1	0.8546	-0.0048	0.0000	0.0000	0.0000	-0.3982
4	1	1.9145	-0.0108	0.0000	0.0000	0.0000	-0.6051
5	1	3.5994	-0.0230	0.0000	0.0000	0.0000	-0.8460
6	1	5.8233	-0.0431	0.0000	0.0000	0.0000	-1.0725
7	1	8.2795	-0.0693	0.0000	0.0000	0.0000	-1.2623
8	1	11.1421	-0.1045	0.0000	0.0000	0.0000	-1.4586
9	1	14.7824	-0.1560	0.0000	0.0000	0.0000	-1.6828
10	1	18.8763	-0.2207	0.0000	0.0000	0.0000	-1.8520
11	1	23.2782	-0.2949	0.0000	0.0000	0.0000	-1.9511
12	1	27.4023	-0.3662	0.0000	0.0000	0.0000	-1.9752

2 NODE PRISMATIC BEAM ELEMENT -- ELEMENT REPORTS

E^v LOAD NODE SIGN CONVENTION : BEAM DESIGNERS
 N. COMB NO AXIAL TORSION SHEAR X MOMENT Y MAX MOM/DEFL DIST SHEAR Y MOMENT X MAX MOM/DEFL DIST

Units: K K -Ft K K -Ft K -Ft / In Ft K K -Ft K -Ft / In Ft

REPORT TYPE : Max Forces Full
 ELEMENT LIST : 1-11

E ^v N.	LOAD COMB	NODE NO	AXIAL	TORSION	SHEAR X	MOMENT Y	MAX MOM/DEFL	DIST	SHEAR Y	MOMENT X	MAX MOM/DEFL	DIST
1	1	1	-19.7843	0.0000	0.0000	0.0000			13.3091	-905.0724		
		2	-17.5169	0.0000	0.0000	0.0000			12.2750	-777.1522	0.0526	4.94
2	1	2	-17.3441	0.0000	0.0000	0.0000			12.3350	-777.1520		
		3	-15.1817	0.0000	0.0000	0.0000			11.3436	-658.7590	0.0515	4.93
3	1	3	-15.0115	0.0000	0.0000	0.0000			11.3962	-658.7595		
		4	-13.1222	0.0000	0.0000	0.0000			10.4395	-549.5811	0.0541	4.92
4	1	4	-12.9511	0.0000	0.0000	0.0000			10.4890	-549.5814		
		5	-11.1636	0.0000	0.0000	0.0000			9.4651	-439.8333	0.0693	5.40
5	1	5	-10.9818	0.0000	0.0000	0.0000			9.5084	-439.8324		
		6	-9.3011	0.0000	0.0000	0.0000			8.4569	-341.0234	0.0652	5.38
6	1	6	-9.1274	0.0000	0.0000	0.0000			8.4879	-341.0253		
		7	-7.6914	0.0000	0.0000	0.0000			7.5283	-260.9439	0.0496	4.89
7	1	7	-7.5359	0.0000	0.0000	0.0000			7.5511	-260.9465		
		8	-6.3859	0.0000	0.0000	0.0000			6.6034	-190.1736	0.0513	4.87
8	1	8	-6.2315	0.0000	0.0000	0.0000			6.6229	-190.1721		
		9	-5.2711	0.0000	0.0000	0.0000			5.6170	-122.8525	0.0645	5.30
9	1	9	-5.1085	0.0000	0.0000	0.0000			5.6315	-122.8516		
		10	-4.2235	0.0000	0.0000	0.0000			4.6575	-66.2620	0.0487	5.23
10	1	10	-4.0696	0.0000	0.0000	0.0000			4.6624	-66.2644		
		11	-2.2718	0.0000	0.0000	0.0000			3.1676	-22.9166	0.0283	5.03
11	1	11	-2.1262	0.0000	0.0000	0.0000			3.1646	-22.9157		
		12	-0.1303	0.0000	0.0000	0.0000			0.0042	-0.0014	0.0061	3.84

R E A C T I O N S

(* Indicates Reactions Occur in Nodal Local System)

NODE NO LOAD COMB PX PY PZ MX MY MZ

Units: K K K K -Ft K -Ft K -Ft

1	1	-13.2730	19.8085	0.0000	0.0000	0.0000	905.0724
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All-Points Technology Corp., P.C.

150 Old Westside Road
 North Conway, NH 03860
 (603) 356-5214

Client: **Verizon Wireless**
 Job: **Manchester, CT**
 Calculated By: **R. Adair**

Job No.: **CT141230**
 Date: **16-Jan-03**

Total Moment (ft-kips)

Axial Loads (kips)

Shear (kips)

Elevation	Total Moment (ft-kips)			Secondary	Axial Loads (kips)			Tower (lbs.)	Antenna (lbs)	Shear (kips)	
	Mom. w/o Ice	75% Mom w/ Ice	100% Mom w/ Ice		D+A Force	D+A+I Force	Secondary			Total (kips)	Secondary
0	891.4	465.1	620.1	905	16.9	19.2	19.8	11138	2842	13.98	13.3
10	763.7	385.6	514.1	777	14.9	17.0	17.3	10057	2842	12.90	12.3
20	646.1	314.1	418.7	659	13.0	14.9	15.0	9021	2842	11.86	11.4
30	538.1	250.1	333.5	550	11.3	12.9	13.0	8030	2842	10.87	10.5
41	430.0	188.2	251.0	440	9.7	11.1	11.0	6971	2842	9.81	9.5
52	333.0	135.2	180.3	341	8.2	9.3	9.1	5887	2842	8.73	8.5
62	254.5	94.7	126.3	261	7.0	7.9	7.5	4899	2842	7.74	7.6
72	185.3	61.7	82.2	190	5.8	6.5	6.2	3919	2842	6.76	6.6
83	119.6	33.7	44.9	123	5.0	5.5	5.1	2863	2842	5.70	5.6
94	64.6	14.3	19.1	66	4.1	4.4	4.1	1838	2842	4.68	4.7
105	22.4	3.2	4.3	23	2.2	2.4	2.1	854	2310	3.16	3.2

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150 Old Westside Road
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Client: **Verizon Wireless**
 Job: **Manchester, CT**
 Calculated By: **R. Adair**

Job No.: **CT141230**
 Date: **16-Jan-03**

Axial Stresses

Elev.	w/o ice	Area	Fy	Stress Ratio	
					w/o ice
0	19.8	52.42	65		0.38
10	17.3	50.12	65		0.35
20	15.0	47.82	65		0.31
30	13.0	38.06	65		0.34
41	11.0	35.94	65		0.31
52	9.1	33.83	65		0.27
62	7.5	31.91	65		0.24
72	6.2	21.05	65		0.29
83	5.1	19.57	65		0.26
94	4.1	18.10	65		0.23
105	2.1	16.62	65		0.13

Bending Stresses

Elev.	fb= Moment/Section Modulus w/o ice	S	$(F_y)^{1/2}$ w/t	Allowable		Actual
				F _b	1.33 F _b	w/o ice
0	905.0	578.44	252.6	38.12	50.70	18.77
10	777.0	528.58	241.6	38.92	51.77	17.64
20	659.0	480.97	240.0	39.04	51.93	16.44
30	550.0	365.37	263.1	37.35	49.68	18.06
41	440.0	325.79	248.7	38.41	51.09	16.21
52	341.0	288.47	240.0	39.04	51.93	14.19
62	261.0	256.52	240.0	39.04	51.93	12.21
72	190.0	159.93	297.0	34.88	46.39	14.26
83	123.0	138.19	276.3	36.39	48.40	10.68
94	66.0	118.04	255.6	37.90	50.41	6.71
105	23.0	99.47	240.0	39.04	51.93	2.77

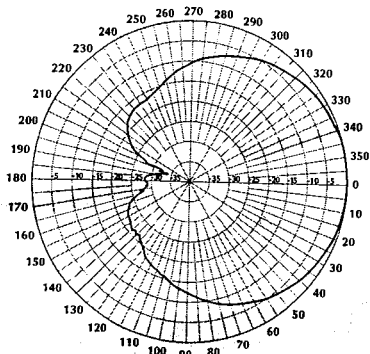
Tower Capacity

Elev.	Comb. Str. Ratio	Capacity
0	0.378	38%
10	0.347	35%
20	0.323	32%
30	0.370	37%
41	0.323	32%
52	0.278	28%
62	0.240	24%
72	0.314	31%
83	0.226	23%
94	0.138	14%
105	0.056	6%

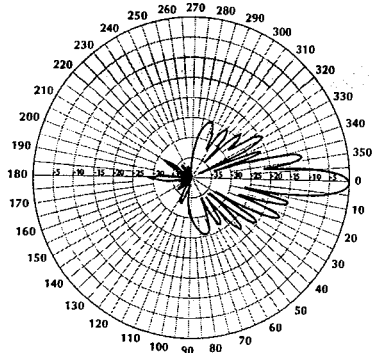
Panel 90°/14, 15 dBd

RWB-80015/90, RWB-80015/90LS

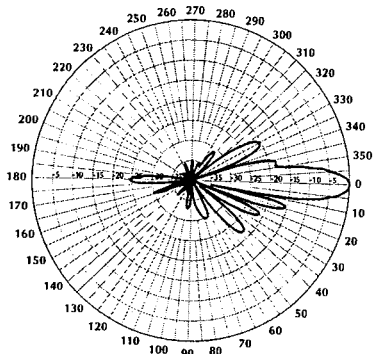
Radiation-pattern (at mid-band)



Horizontal



RWB-80015/90 Vertical

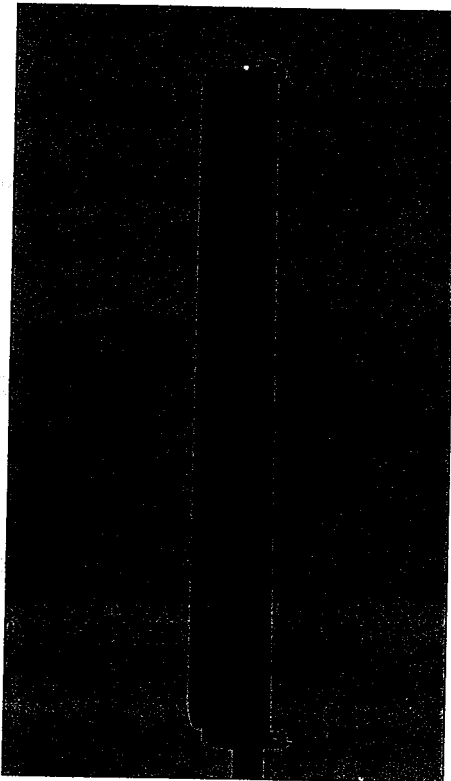


RWB-80015/90LS Vertical

RWB-80015/90LS specially designed for enhanced upper side lobe suppression.

Radiation patterns for all Antel antennas are measured with the antenna mounted on a fiberglass pole.

Mounting on a metal pole will typically improve the front-to-back ratio.



Antel's Exclusive 3T (True Transmission Line Technology) Antenna Design:

- A 1 1/4" four-channel extrusion running the entire length of the antenna for unmatched strength and rigidity.
- Durable brass feedline design that eliminates the need for solder joints in the signal path.
- A non-collinear system with access to every radiating element for broad band width and superior performance.
- Air as insulation for virtually no internal signal loss.

Every Antel antenna is under a five-year limited warranty for repair or replacement.

Antenna can be ordered with bottom-fed or center-fed connector. For center-fed connector, order model number RWB-80015/90CF or RWB-80015/90LSCF connector (N, NE, DIN, E-DIN).

Example: RWB-80015/90LSCF-E-DIN

Mechanical specifications

Length	2450 mm	96.5 in
Width	295 mm	11.6 in
Depth	160 mm	6.3 in
Weight	14 kg	31 lbs
Wind area	0.73 m ²	7.87 ft ²
Wind load at 50m/s	1140 N	256 lbs

Mounting

Through two pair of clamps to pipe diameter Ø50-160 mm (2.0-6.3 in), or by U-clamps to a 2" pipe.

Antenna consisting of aluminum alloy with brass feedlines covered by a UV safe fiberglass radome.

Mounting Bracket #36210002

Downtilt Bracket #36114003

Electrical specifications

Frequency range	806-960 MHz
Impedance	50Ω
3) Connector	N, NE, DIN, E-DIN or EIA
1) VSWR	≤1.4:1
Polarization	Vertical
1) Gain	RWB-80015/90 15 dBd RWB-80015/90LS 14 dBd
2) Power rating	500 W
1) Half power angle	H-plane 90° E-plane 7°
1) Lobe tilt	RWB-80015/90 1.25° RWB-80015/90LS 0°
1) Null fill	5%
Lightning protection	Direct ground

For a list of models, including electrical downtilt and/or null fill combinations, see page 141.

- 1) Typical values.
- 2) Power Rating limited by connector only.
- 3) NE indicates an elongated N Connector. E-DIN indicates an elongated DIN Connector. EIA indicates a 7/8" EIA Flange.

Improvements to mechanical and/or electrical performance of the antenna may be made without notice.

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INTERNATIONAL, INC.

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RWA-80017, RWA-80017LS

Mechanical specifications

Length	2450 mm	96.5 in
Width	295 mm	11.6 in
Depth	160 mm	6.3 in
Weight	14 kg	31.0 lbs
Wind area	0.73 m ²	7.8 ft ²
Wind load at 50m/s	1140 N	256 lbs

Mounting

Through two pair of clamps to pipe diameter Ø50-160 mm (2.0-6.3 in), or by U-clamps to a 2" pipe.

Antenna consisting of aluminum alloy with brass feedlines covered by a UV safe fiberglass radome.

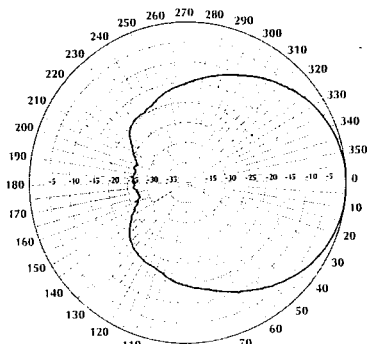
Mounting Bracket #36210002

Downtilt Bracket #36114003

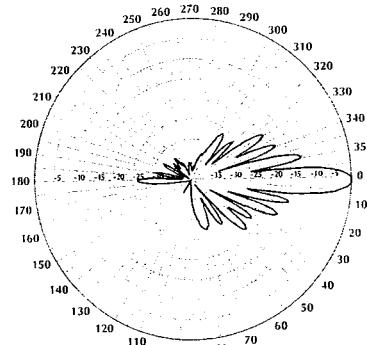
Electrical specifications

Frequency range	806-960 MHz
Impedance	50Ω
3) Connector	N, NE, DIN, E-DIN or EIA
1) VSWR	≤1.4:1
Polarization	Vertical
1) Gain	
RWA-80017	17 dBd
RWA-80017LS	16 dBd
2) Power rating	500 W
1) Half power angle	
H-plane	62°
E-plane	7°
1) Lobe tilt	
RWA-80017	1.25°
RWA-80017LS	0°
1) Null fill	5%
Lightning protection	Direct ground

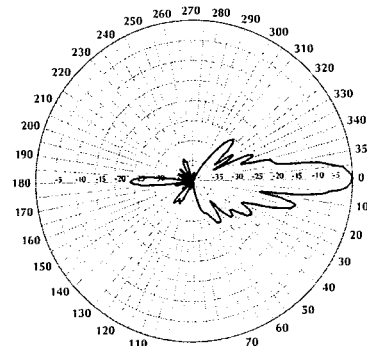
Radiation-pattern (at mid-band)



Horizontal



RWA-80017 Vertical

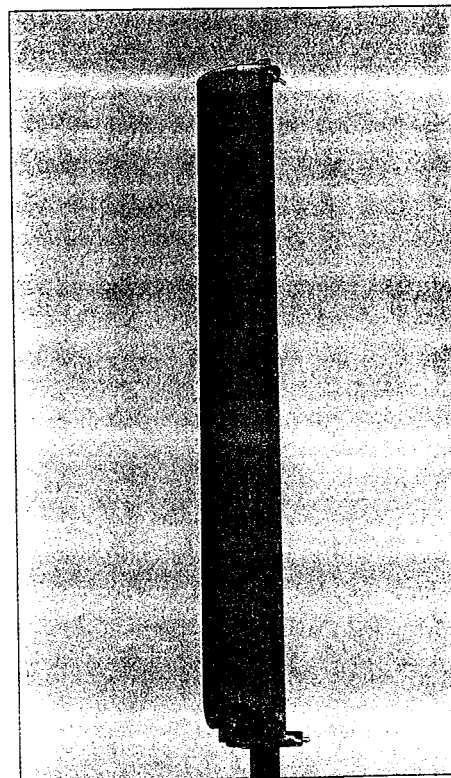


RWA-80017LS Vertical

RWA-80017LS specially designed for enhanced upper side lobe suppression.

Radiation patterns for all Antel antennas are measured with the antenna mounted on a fiberglass pole.

Mounting on a metal pole will typically improve the Front-to-Back Ratio.



806-960 MHz



Antel's Exclusive 3T (True Transmission Line Technology) Antenna Design:

- A 1 1/4" four-channel extrusion running the entire length of the antenna for unmatched strength and rigidity.
- Durable brass feedline design that eliminates the need for solder joints in the signal path.
- A non-collinear system with access to every radiating element for broad band width and superior performance.
- Air as insulation for virtually no internal signal loss.

Every Antel antenna is under a five-year limited warranty for repair or replacement.

Antenna can be ordered with bottom-fed or center-fed connector. For center-fed connector, order model number RWA-80017CF + connector or RWA-80017LSCF + connector (N, NE, DIN, E-DIN).

Example: RWA-80017LSCF E-DIN

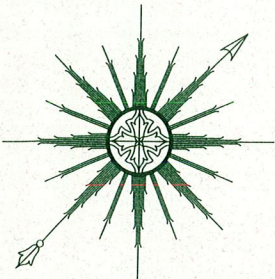
For a list of models, including electrical downtilt and/or null fill combinations, see page 139.

- 1) Typical values.
- 2) Power Rating limited by connector only.
- 3) NE indicates an elongated N Connector. E-DIN indicates an elongated DIN Connector. EIA indicates a 7/8" EIA Flange.

Improvements to mechanical and/or electrical performance of the antenna may be made without notice.

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Antel®
INTERNATIONAL, INC. **53**



ALL-POINTS TECHNOLOGY CORPORATION, P.C.

February 3, 2003

Verizon Wireless
99 East River Drive, 9th Floor
East Hartford, CT 06108

Attn: Wayne Lukachek
Re: 115' Valmont Monopole Tower
Manchester, Connecticut
Crown Castle BU #806372

Dear Wayne,

All-Points Technology Corporation, P.C. analyzed Crown Castle's 115' Valmont monopole tower located off Route 6 in Manchester, Connecticut. The analysis was conducted in accordance with EIA/TIA-222-F, Structural Standards for Steel Antenna Towers and Antenna Supporting Structures for replacement of Verizon Wireless' existing fifteen panel antennas with twelve DB844 panel antennas. Our results were submitted in a structural analysis report dated January 24, 2003.

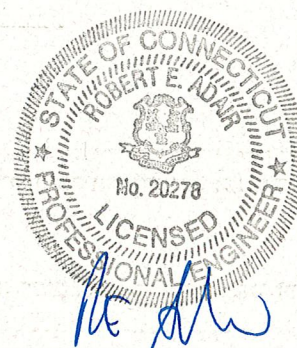
Subsequent to the analysis, Verizon Wireless requested the structure be evaluated for installation of eight Antel RWB-80015/90LS and four Antel RWA-80017LS panel antennas in lieu of the DB844 antennas. Our analysis indicates the tower and foundation are capable of supporting Verizon's proposed antenna installation. Results are as follows:

Elevation	Capacity
0'-30'	56%
30'-72'	54%
72'-115'	41%

We appreciate this opportunity to provide you with our services. Please call if you have any questions.

Sincerely,
All-Points Technology Corporation, P.C.

Robert E. Adair, P.E.
Principal



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