

#### JULIE D. KOHLER

PLEASE REPLY TO: Bridgeport
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E-Mail Address: jkohler@cohenandwolf.com

February 27, 2014

Attorney Melanie Bachman Acting Executive Director Connecticut Siting Council Ten Franklin Square New Britain, CT 06051

Re: Notice of Exempt Modification

Tarpon Tower/Florida Tower Partners, LLC/ T-Mobile co-location

Site ID CTNH808A

15 Orchard Park Road, Madison

Dear Attorney Bachman:

This office represents T-Mobile Northeast LLC ("T-Mobile") and has been retained to file exempt modification filings with the Connecticut Siting Council on its behalf.

In this case, Tarpon Tower/Florida Tower Partners, LLC owns the existing monopole telecommunications tower and related facility at 15 Orchard Park Road, Madison, Connecticut (Coordinates: 41° 16′ 59″ -72° 37′ 23″). T-Mobile intends to replace three and add six new antennas and related equipment at this existing telecommunications facility in Madison ("Madison Facility"). Please accept this letter as notification, pursuant to R.C.S.A. § 16-50j-73, of construction which constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to the First Selectman Fillmore McPherson, and the property owner, 15 Orchard Park, LLC.

The existing Madison Facility consists of a 99 foot tall monopole tower, approved by the Council in Docket No. 390. T-Mobile plans to replace three antennas and add six new antennas and three TMAs (tower mounted amplifiers) at a centerline of 100 feet. (See the plans revised to February 14, 2014 attached hereto as Exhibit A). T-Mobile will also install fiber cable along existing coax routing. The existing Madison Facility is structurally capable of supporting T-Mobile's proposed modifications, as indicated in the structural analysis dated February 9, 2014 and attached hereto as Exhibit B.

The planned modifications to the Madison Facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

<sup>&</sup>lt;sup>1</sup> The Decision and Order in this Docket (dated March 26, 2010) contains no relevant requirements or limitations on the configuration of the Madison Facility.



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- 1. The proposed modification will not increase the height of the tower. T-Mobile's replacement and additional antennas will be installed at a centerline of 100 feet, merely replacing existing antennas located at the same 100 foot elevation. The enclosed tower drawing confirms that the proposed modification will not increase the height of the tower.
- 2. The proposed modifications will not require an extension of the site boundaries. T-Mobile's equipment will be located entirely within the existing compound area.
- 3. The proposed modification to the Madison Facility will not increase the noise levels at the existing facility by six decibels or more.
- 4. The operation of the replacement antennas will not increase the total radio frequency (RF) power density, measured at the base of the tower, to a level at or above the applicable standard. According to a Radio Frequency Emissions Analysis Report prepared by EBI dated February 24, 2014, T-Mobile's operations would add 1.180% of the FCC Standard. Therefore, the calculated "worst case" power density for the planned combined operation at the site including all of the proposed antennas would be 17.940% of the FCC Standard as calculated for a mixed frequency site as evidenced by the engineering exhibit attached hereto as Exhibit C.

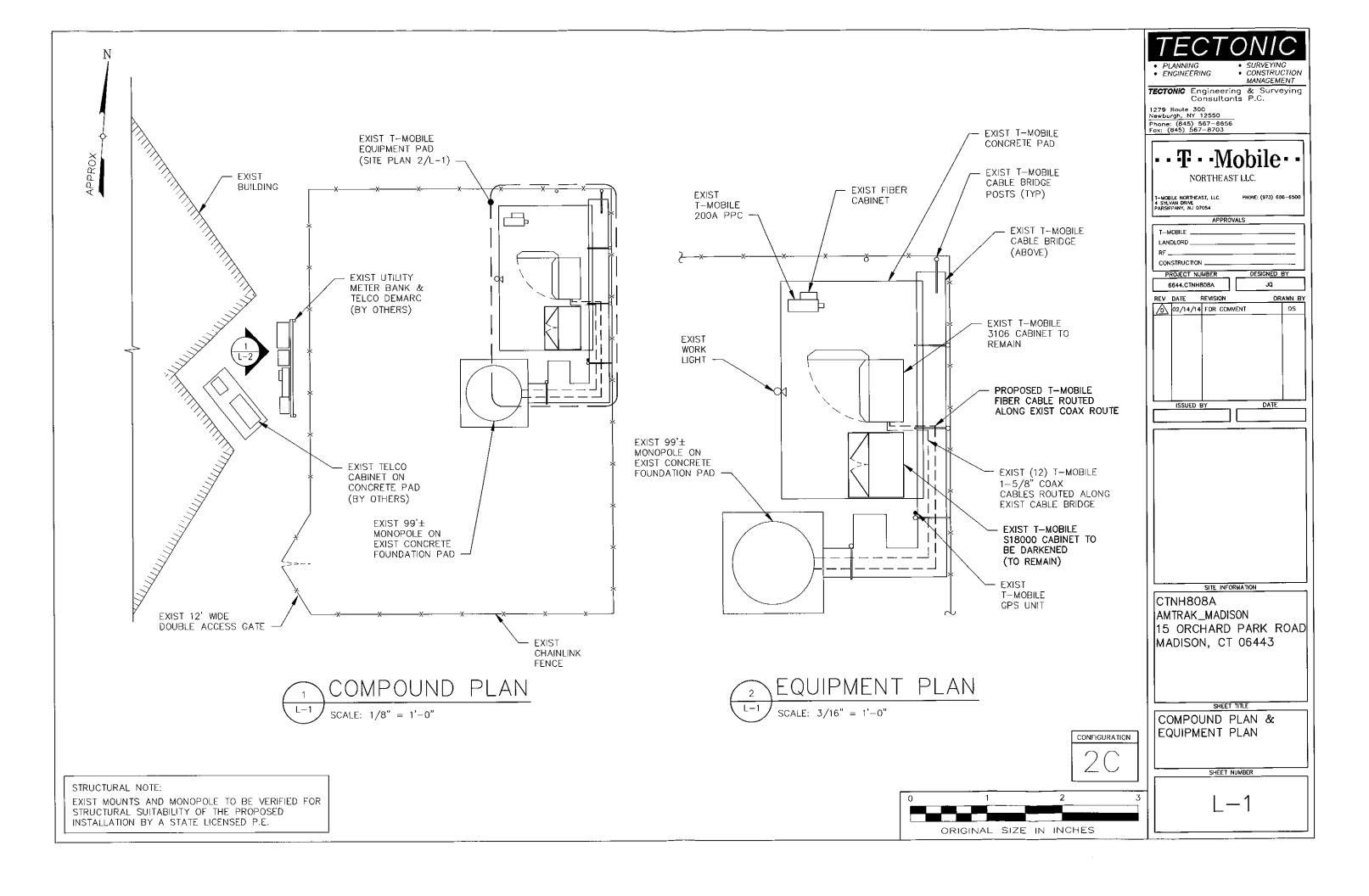
For the foregoing reasons, T-Mobile respectfully submits that the proposed replacement antennas and equipment at the Madison Facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Upon acknowledgement by the Council of this proposed exempt modification, T-Mobile shall commence construction approximately sixty days from the date of the Council's notice of acknowledgement.

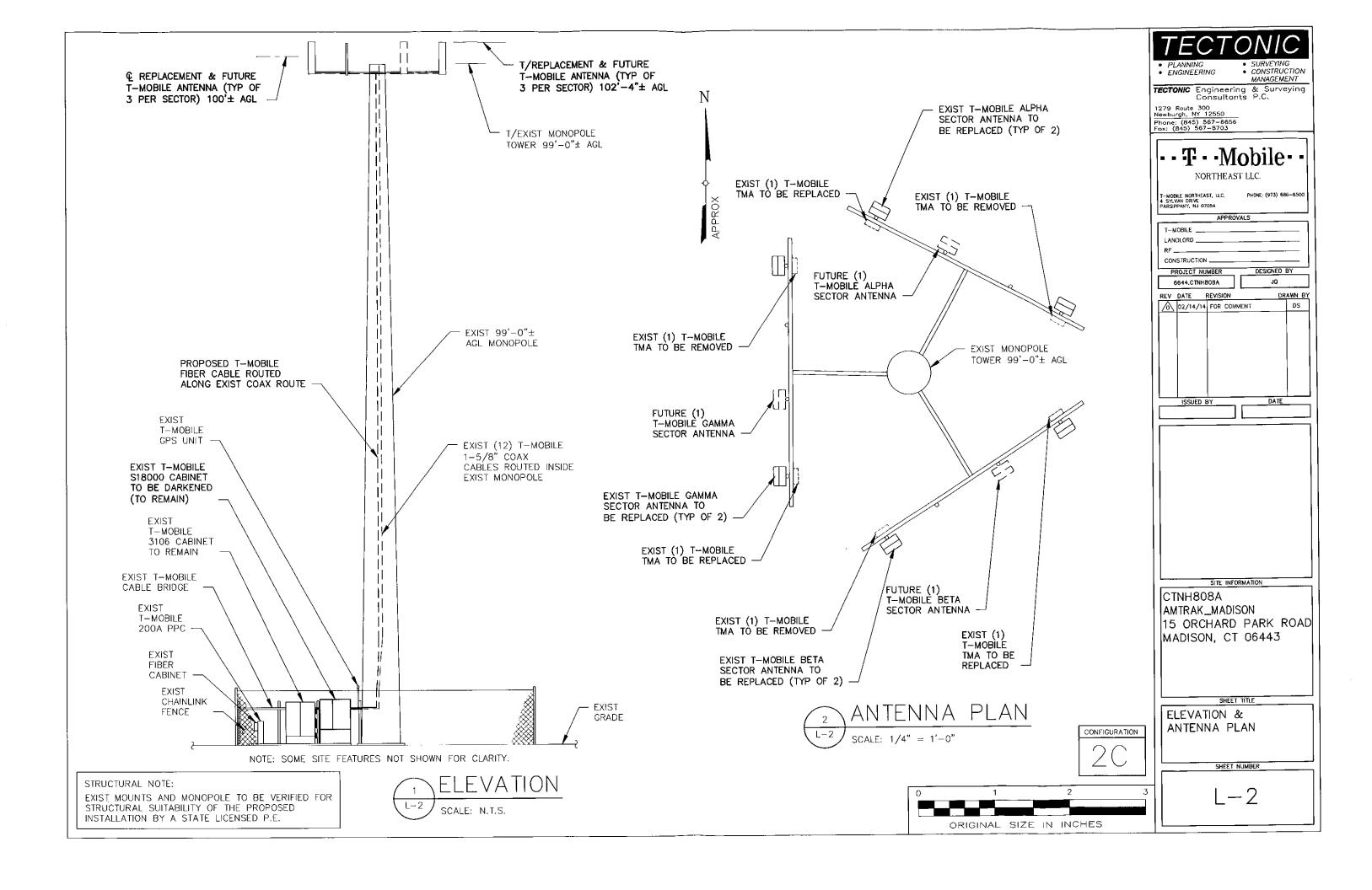
Sincerely,

Julie D. Kohler, Esq

cc: Town of Madison, First Selectman Fillmore McPherson Tarpon Tower\Florida Tower Partners, LLC 15 Orchard Park Road, LLC Halene Fujimoto, HPC Wireless Solutions

# **EXHIBIT A**





# EXHIBIT B

# Structural Analysis 99-ft Monopole

Prepared For: Florida Tower Partners, LLC 1001 3<sup>rd</sup> Ave. West, Suite 420 Bradenton, FL 34205

MFP Project #40913-039 r1

Site Location: CT-1014 / Madison New Haven Co., Connecticut Lat/Long: 41°16'59", -72°37'23"

Analysis Type: ANSI/TIA-222-G

February 9, 2014



Michael F. Plahovinsak, P.E. 1830| State Route 161 W, Plain City, OH 43064 614-398-6250 - mike@mfpeng.com

#### Project Summary:

I have completed a structural analysis of the existing monopole for the following new configuration:

- 100' T-Mobile:
  - o Install (9) Ericsson AIR-21 Panel and (3) Ericsson KRY-112-144/1 TMA's. (13) 15/8" Cables.

The pole has been analyzed in accordance with the requirements of the International Building Code per IBC section 3108.4, and the recommendations of the Telecommunications Industry Association "Structural Standard for Steel Antenna Supporting Structures" ANSI/TIA-222-G.

This analysis may be considered a "Rigorous Structural Analysis" as defined in ANSI/TIA-222-G 15.5.2.

As indicated in the conclusions of this analysis, I have determined that the existing pole and foundation *have sufficient capacity* to support the existing, reserved and proposed antenna loads as detailed herein. Based on the results of my analysis, no structural modifications are required at this time.

#### Source of Data:

Resource	Source	Job Number	Date
Pole and Foundation Drawings	Sabre Towers	11-30257	03/23/11
Geotechnical Report	Terracon	J2095225	12/21/09

# Analysis Criteria:

International Building Code (All Versions) Section 3108.4 Structural Standards for Steel Antenna Supporting Structures **ANSI/TIA-222-G 2** 

•	Basic Wind Speed	115 mph (3-Sec Gust)
•	Basic Wind Speed w/ 3/4" Ice	50 mph (3-Sec Gust)
•	Operational Wind Speed	60 mph (3-Sec Gust)

Structure Class	Exposure Category	Topographic Category
II $(I = 1.0)$	В	I

#### Appurtenance Listing:

Status	Elev.	Antenna / Mounting	Coax	Owner		
Proposed	100'	(9) Ericsson AIR-21 Panel + (3) KRY-112-144/1 TMA's T-Arm Mounts	(13) 1 5/8"	T-Mobile		
		(3) Andrew SBNH-1D6565C + (9) SBNHH-1A65C Panel				
Existing	90'	(18) RRUS-11 RRUs	(8) 3/4" + (2)	АТ&Т		
Existing	50	(4) Raycap DC6-48-60-18-8-F Suppressors	1/2" + (3) 3/8"	Aidi		
		T-Arm Mounts				

All antenna lines assumed internally mounted, not exposed to the wind.

#### Foundation Analysis:

Two foundation alternatives were designed for this site. Both foundation alternatives have sufficient capacity to support the loads from this analysis.

#### Conclusion:

I have completed a structural analysis of the existing monopole and foundation in accordance with the project specifics outlined above. My analysis indicates that the existing monopole and foundation is stressed to a maximum of 49.9% of its usable capacity when considering the existing plus proposed loading. Please refer to the attached calculations for an itemized listing of all member stress ratios. The existing pole and foundation have sufficient capacity to support the proposed configuration, and structural modifications are not required.

If you have any questions about the contents of this structural report or require any additional information, please feel free to contact my office.

Sincerely,

Michael F. Plahovinsak, P.E.

mike@mfpeng.com - 614.398-6250

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# Standard Conditions for Providing Structural Consulting Services on Existing Structures

1. The following standard conditions are a general overview of key issues regarding the work product supplied.

- 2. If the existing conditions are not as represented in this structural report or attached sketches, I should be contacted to evaluate the significance of the deviation and revise the structural assessment accordingly.
- 3. The structural analysis has been performed assuming that the structure is in "like new" condition. No allowance was made for excessive corrosion, damaged or missing structural members, loose bolts, etc. If there are any known deficiencies in the structure that potentially compromise structural integrity, I should be made aware of the deficiencies. If I am aware of a deficiency that exists in a structure at the time of my analysis, a general explanation of the structural concern due to the deficiency will be included in the structural report, but the deficiency will not be reflected in capacity calculations.
- 4. The structural analysis provided is an assessment of the primary load carrying capacity of the structure. I provide a limited scope of service in that I have not verified the capacity of every weld, plate, connection detail, etc. In most cases, structural fabrication details are unknown at the time of my analysis, and the detailed field measurement of this information is beyond the scope of my services. In instances where I have not performed connection capacity calculations, it is assumed that existing manufactured connections develop the full capacity of the primary members being connected.
- 5. The structural integrity of the existing foundation system can only be verified if exact foundation sizes and soils conditions are known. I will not accept any responsibility for the adequacy of the existing foundations unless this site-specific data is supplied.
- 6. Miscellaneous items such as antenna mounts, coax supports, etc. have not been designed, detailed, or specified as part of my work. It is assumed that material of adequate size and strength will be purchased from a reputable component manufacturer. The attached report and sketches are schematic in nature and should not be used to fabricate or purchase hardware and accessories to be attached to the structure. I recommend field measurement of the structure before fabricating or purchasing new hardware and accessories. I am not responsible for proper fit and clearance of hardware and accessory items in the field.
- 7. The structural analysis has been performed considering minimum code requirements or recommendations. If alternate wind, ice, or deflection criteria are to be considered, then I shall be made aware of the alternate criteria.

# 0.3125 38.3900 4.75 49.50 5.4 18 49.5 ft 0.4375 10.6 18 ALL REACTIONS ARE FACTORED AXIAL 41 K SHEAR MOMENT 4K / 309 kip-ft 50 mph WIND - 0.7500 in ICE AXIAL 27 K MOMENT SHEAR/ 20 K 1489 kip-ft 1.0 ft REACTIONS - 115 mph WIND 16.1 Socket Length Thickness (in) Top Dia (in) Length (ft) Weight (K) Bot Dia (in) Grade

#### **DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION
(3) Ericsson AIR 21 w/ mount pipe (T-Mobile)	100	(6) Ericsson RRUS11 Dual PA RRU (ATT)	90
Ericsson KRY 112 144-1 TMA (T-Mobile)	100	Andrew SBNH-1D6565C w/ mount pipe (ATT)	90
(3) Ericsson AIR 21 w/ mount pipe (T-Mobile)	100	(3) Andrew SBNHH-1A65C w/ mount pipe (ATT)	90
Ericsson KRY 112 144-1 TMA (T-Mobile)	100	(6) Ericsson RRUS11 Dual PA RRU (ATT)	90
(3) Ericsson AIR 21 w/ mount pipe (T-Mobile)	100	Andrew SBNH-1D6565C w/ mount pipe (ATT)	90
Ericsson KRY 112 144-1 TMA (T-Mobile)	100	(3) Andrew SBNHH-1A65C w/ mount pipe (ATT)	90
T-Arm Mounts (T-Mobile)	100	(6) Ericsson RRUS11 Dual PA RRU	90
Andrew SBNH-1D6565C w/ mount	90	(ATT)	- LEASTANNE
pipe (ATT)		(4) Raycap DC6-48-60-18-8F	90
(3) Andrew SBNHH-1A65C w/ mount	90	Supressor (ATT)	
pipe (ATT)		T-Arm Mounts (ATT)	90

**MATERIAL STRENGTH** 

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

#### **TOWER DESIGN NOTES**

- Tower is located in New Haven County, Connecticut.
   Tower designed for Exposure B to the TIA-222-G Standard.
   Tower designed for a 115 mph basic wind in accordance with the TIA-222-G Standard.
- Tower is also designed for a 50 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
- Deflections are based upon a 60 mph wind.
- 6. Tower Structure Class II.
- Topographic Category 1 with Crest Height of 0.00 ft
   TOWER RATING: 31.6%

Michael F. Plahovinsak, P.E. <sup>ob:</sup> 99-ft Monopole / MFP #40913-039 r1 Project: CT1014, Madison 18301 State Route 161 W Client: Florida Tower Partners Drawn by: Mike App'd: Plain City, OH 43064 Code: TIA-222-G Date: 02/09/14 Scale: N Phone: 614-398-6250 FAX: mike@mfpeng.com J:\Projects\409-Misc\40913-039\40913-039 r1.e

Michael F. Plahovinsak, P.E.

18301 State Route 161 W Plain City, OH 43064 Phone: 614-398-6250 FAX; mike@mfpeng.com

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Client	Florida Tower Partners	Designed by Mike

# **Tower Input Data**

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in New Haven County, Connecticut.

Basic wind speed of 115 mph.

Structure Class II.

Exposure Category B.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 0.7500 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

# **Tapered Pole Section Geometry**

Section	Elevation	Section	Splice	Number	Top	Bottom	Wall	Bend D = disse	Pole Grade
	ft	Length ft	Length ft	of Sides	Diameter in	Diameter in	Thickness in	Radius in	
L1	99.00-49.50	49.50	4.75	18	27.2500	38.3900	0.3125	1.2500	A572-65 (65 ksi)
L2	49.50-1.00	53.25		18	36.6960	48.6800	0.4375	1.7500	À572-65 (65 ksi)

# **Tapered Pole Properties**

Section	Tip Dia. in	Area in²	I in¹	r in	C in	I/C in³	J in⁴	It/Q in²	w in	w/t
L1	27.6704	26.7186	2449.4369	9.5628	13.8430	176.9441	4902.0968	13.3619	4.2460	13.587
	38.9822	37.7681	6918.3045	13.5175	19.5021	354.7463	13845.7123	18.8876	6.2066	19.861
L2	38.3476	50.3495	8362.8112	12.8718	18.6416	448.6108	16736.6263	25,1795	5.6885	13.002
	49.4310	66.9907	19697.5334	17.1261	24.7294	796.5216	39420.9854	33.5017	7.7977	17.823

Michael F. Plahovinsak, P.E. 18301 State Route 161 W

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	Florida Tower Partners	Mike

# Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Component Type	Placement	Total Number		$C_AA_A$	Weight
	Leg	Smela	1940	fi	711177001		ft²/ft	plf
1 5/8"	$\overline{c}$	No	Inside Pole	99.00 - 1.00	13	No Ice	0.00	0.92
(T-Mobile)						1/2" Ice	0.00	0.92
						I" Ice	0.00	0.92
***								
3/4"	C	No	Inside Pole	90.00 - 1.00	8	No Ice	0.00	0.33
(ATT)						1/2" Ice	0.00	0.33
` ′						1" Ice	0.00	0.33
1/2"	C	No	Inside Pole	90.00 - 1.00	2	No Ice	0.00	0.15
(ATT)						1/2" Ice	0.00	0.15
, ,						1" Ice	0.00	0.15
3/8"	C	No	Inside Pole	90.00 - 1.00	3	No Ice	0.00	0.08
(ATT)						1/2" Ice	0.00	0.08
` ,						1" Ice	0.00	0.08

# **Discrete Tower Loads**

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ft ft ft	o	ft		ft²	ft²	K
(3) Ericsson AIR 21 w/	A	From Face	3.00	0.0000	100.00	No Ice	6.61	5.50	0.11
mount pipe			0.00			1/2" Ice	7.08	6.22	0.16
(T-Mobile)			0.00			1" Ice	7.55	6.95	0.22
Ericsson KRY 112 144-1	$\boldsymbol{A}$	From Face	3.00	0.0000	100.00	No Ice	0.56	0.25	0.00
TMA			0.00			1/2" Ice	0.66	0.32	0.01
(T-Mobile)	_		0.00			1" Ice	0.78	0.41	0.01
(3) Ericsson AIR 21 w/	В	From Face	3.00	0.0000	100.00	No Ice	6.61	5.50	0.11
mount pipe			0.00			1/2" Ice	7.08	6.22	0.16
(T-Mobile)	_		0.00	0.0000	****	1" Ice	7.55	6.95	0.22
Ericsson KRY 112 144-1	В	From Face	3.00	0.0000	100.00	No Ice	0.56	0.25	0.00
TMA			0.00			1/2" Ice	0.66	0.32	0.01
(T-Mobile)	~		0.00	0.0000	100.00	1" Ice	0.78	0.41	0.01
(3) Ericsson AIR 21 w/	C	From Face	3.00	0.0000	100.00	No Ice	6.61	5.50	0.11
mount pipe			0.00 0.00			1/2" Ice 1" Ice	7.08 7.55	6.22 6.95	0.16 0.22
(T-Mobile)	C	From Face	0.00 3.00	0.0000	100.00	No Ice	7.33 0.56	0.93	0.22
Ericsson KRY 112 144-1 TMA	Č	rrom race	0.00	0.0000	100.00	1/2" Ice	0.56	0.23	0.00
TMA (T-Mobile)			0.00			1" Ice	0.00	0.32	0.01
(1-mobile) T-Arm Mounts	C	None	0.00	0.0000	100.00	No Ice	14.00	14.00	1,14
T-Arm Wounis (T-Mobile)	C	None		0.0000	100.00	1/2" Ice	16.00	16.00	1.14
(1-moone)						1" Ice	18.00	18.00	0.47
***						1 100	10.00	10.00	0.47
Andrew SBNH-1D6565C w/	Α	From Face	3.00	0.0000	90.00	No Ice	11.45	9.60	0.09
mount pipe	• •		0.00	0.0000	7 0.00	1/2" Ice	12.06	11.02	0.17
(ATT)			0.00			1" Ice	12.69	12.29	0.27
(3) Andrew SBNHH-1A65C	Α	From Face	3.00	0.0000	90.00	No Ice	11.45	9.60	0.08
w/ mount pipe			0.00			1/2" Ice	12.06	11.02	0.16
(ATT)			0.00			1" Ice	12.69	12.29	0.26
(6) Ericsson RRUS11 Dual	Α	From Face	3.00	0.0000	90.00	No Ice	2.55	0.92	0.05
PA RRU			0.00			1/2" Ice	2.77	1.07	0.06
(ATT)			0.00			l" Ice	2.99	1.23	0.08
Andrew SBNH-1D6565C w/	В	From Face	3.00	0.0000	90.00	No Ice	11.45	9.60	0.09
mount pipe			0.00			1/2" Ice	12.06	11.02	0.17

Michael F. Plahovinsak, P.E. 18301 State Route 161 W

Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ft ft ft	ō	ft		ft²	ft²	K
(ATT)			0.00			1" Ice	12.69	12.29	0.27
(3) Andrew SBNHH-1A65C	В	From Face	3.00	0.0000	90.00	No Ice	11.45	9.60	0.08
w/ mount pipe			0.00			1/2" Ice	12.06	11.02	0.16
(ATT)			0.00			l" Ice	12.69	12,29	0.26
(6) Ericsson RRUS11 Dual	В	From Face	3.00	0.0000	90.00	No Ice	2.55	0.92	0.05
PA RRU			0.00			1/2" Ice	2.77	1.07	0.06
(ATT)			0.00			1" Ice	2.99	1.23	0.08
Andrew SBNH-1D6565C w/	C	From Face	3.00	0.0000	90.00	No Ice	11.45	9.60	0.09
mount pipe			0.00			1/2" Ice	12.06	11.02	0.17
(ATT)			0.00			1" Ice	12.69	12.29	0.27
(3) Andrew SBNHH-1A65C	С	From Face	3.00	0.0000	90.00	No Ice	11.45	9.60	0.08
w/ mount pipe			0.00			1/2" Ice	12.06	11.02	0.16
(ATT)			0.00			l" Ice	12.69	12.29	0.26
(6) Ericsson RRUS11 Dual	C	From Face	3.00	0.0000	90.00	No Ice	2.55	0.92	0.05
PA RRU			0.00			1/2" Ice	2.77	1.07	0.06
(ATT)			0.00			1" Ice	2.99	1.23	0.08
(4) Raycap DC6-48-60-18-8F	C	None		0.0000	90.00	No Ice	1.47	1.47	0.03
Supressor						1/2" Ice	1.67	1.67	0.05
(ATT)						1" Ice	1.88	1.88	0.07
T-Arm Mounts	C	None		0.0000	90.00	No Ice	14.00	14.00	1.14
(ATT)						1/2" Ice	16.00	16.00	1.27
						1" Ice	18.00	18.00	0.47

# **Load Combinations**

Comb.	Description	
No.		
1	Dead Only	
2	1.2 Dead+1.6 Wind 0 deg - No Ice	
3	0.9 Dead+1.6 Wind 0 deg - No Ice	
4	1.2 Dead+1.6 Wind 90 deg - No Ice	
5	0.9 Dead+1.6 Wind 90 deg - No Ice	
6	1.2 Dead+1.6 Wind 180 deg - No Ice	
7	0.9 Dead+1.6 Wind 180 deg - No Ice	
8	1.2 Dead+1.0 Ice+1.0 Temp	
9	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	
10	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	
11	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	
12	Dead+Wind 0 deg - Service	
13	Dead+Wind 90 deg - Service	
14	Dead+Wind 180 deg - Service	

Michael F. Plahovinsak, P.E. 18301 State Route 161 W

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	Florida Tower Partners	Mike

# **Maximum Member Forces**

Section No.	Elevation ft	Component Type	4		Axial	Major Axis Moment	Minor Axis Moment
				Comb.	. K	kip-ft	kip-ft
LÏ	99 - 49.5	Pole	Max Tension	12	0.00	0.00	-0.00
			Max. Compression	8	-22.12	0.00	0.00
			Max. Mx	4	-12.63	-530.86	0.00
			Max. My	2	-12.63	0.00	530.86
			Max. Vy	4	15.52	-530.86	0.00
			Max. Vx	2	-15.52	0.00	530.86
L2	49.5 - 1	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-41.17	0.00	0.00
			Max, Mx	4	-27.36	-1488.62	0.00
			Max. My	6	-27.36	0.00	-1488.62
			Max. Vy	4	20.47	-1488.62	0.00
			Max. Vx	6	20.47	0.00	-1488.62

# **Maximum Tower Deflections - Service Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	٥	0
Li	99 - 49.5	3.014	12	0.2499	0.0000
L2	54.25 - 1	0.951	12	0.1609	0.0000

# Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	٥	0	ft
100.00	(3) Ericsson AIR 21 w/ mount pipe	12	3.014	0.2499	0.0000	124096
90.00	Andrew SBNH-1D6565C w/ mount	12	2.544	0.2341	0.0000	68942
	pipe					

# **Maximum Tower Deflections - Design Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	٥	•
L1	99 - 49.5	19.851	4	1.6461	0.0000
L2	54.25 - 1	6.259	4	1.0597	0.0000

# Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	•	ft
100.00	(3) Ericsson AIR 21 w/ mount pipe	4	19.851	1.6461	0.0000	18865
90.00	Andrew SBNH-1D6565C w/ mount	4	16.750	1.5416	0.0000	10480
	pipe					

Michael F. Plahovinsak, P.E.

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Job		Page
	99-ft Monopole / MFP #40913-039 r1	5 of 5
Project		Date
	CT1014, Madison	03:56:05 02/09/14
Client		Designed by
	Florida Tower Partners	Mike

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Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio P.,
	ft		ft	ft		in <sup>2</sup>	K	K	$\phi P_n$
L1	99 - 49.5 (1)	TP38.39x27.25x0.3125	49.50	0.00	0.0	36.7078	-12.63	2601.61	0.005
L2	49.5 - 1 (2)	TP48.68x36.696x0.4375	53.25	0.00	0.0	66.9907	-27.36	4849.69	0.006

# Pole Bending Design Data

Section	Elevation	Size	$M_{ux}$	$\phi M_{nx}$	Ratio	$M_{uy}$	$\phi M_{nv}$	Ratio
No.					$M_{ux}$	77	0. <b>▼</b> -0.0 (0.000 (1.000)	$M_{uy}$
	ft		kip-ft	kip-ft	$\phi M_{nx}$	kip-ft	kip-ft	$\phi M_{ny}$
L1	99 - 49.5 (1)	TP38.39x27.25x0.3125	530.86	1978.72	0.268	0.00	1978.72	0.000
L2	49.5 - 1 (2)	TP48.68x36.696x0.4375	1488.63	4805.25	0.310	0.00	4805.25	0.000

# Pole Shear Design Data

Section	Elevation	Size	Actual	$\phi V_n$	Ratio	Actual	$\phi T_n$	Ratio
No.			$V_u$		$V_u$	$T_u$		$T_u$
	ft		K	K	$\phi V_n$	kip-ft	kip-ft	$\phi T_n$
L1	99 - 49.5 (1)	TP38.39x27.25x0.3125	15.52	1300.81	0.012	0.00	3962.28	0.000
L2	49.5 - 1 (2)	TP48.68x36.696x0.4375	20.47	2424.85	0.008	0.00	9622.25	0.000

# **Pole Interaction Design Data**

Section No.	Elevation	Ratio P <sub>u</sub>	Ratio $M_{ux}$	Ratio $M_{uv}$	Ratio $V_u$	Ratio $T_u$	Comb. Stress	Allow. Stress	Criteria
	ft	 $\phi P_n$	$\phi M_{nx}$	$\phi M_{ny}$	$\overline{\qquad}$ $\phi V_n$	$\phi T_n$	Ratio	Ratio	
Ll	99 - 49.5 (1)	0.005	0.268	0.000	0.012	0.000	0.273	1.000	4.8.2
L2	49.5 - 1 (2)	0.006	0.310	0.000	0.008	0.000	0.316	1.000	4.8.2

# **Section Capacity Table**

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$ otag P_{allow} $ $ otag K$	% Capacity	Pass Fail
L1	99 - 49.5	Pole	TP38.39x27.25x0.3125	1	-12.63	2601.61	27.3	Pass
L2	49.5 - 1	Pole	TP48.68x36.696x0.4375	2	-27.36	4849.69	31.6	Pass
							Summary	
						Pole (L2)	31.6	Pass
						RATING =	31.6	Pass

### Michael F. Plahovinsak, P.E.

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dot	99-ft monopole - MFP #40913-039	Page BP-G
Project	CT1014, Madison	Date 2/9/2014
Client	FLORIDA TOWER PARTNERS	Designed by Mike

# **Anchor Rod and Base Plate Calculation**

#### ANSI/TIA-222-G-2

Factored Base Reactions:

Pole Shape:

Anchor Rods:

Base Plate:

Moment:

1489 ft-kips

18-Sided

(16) 2.25 in. A615 GR. 75

3 in. x 55.5 in. Square

Shear:

20 kips

Pole Dia. (D<sub>f</sub>):

Anchor Rods in Quadrants

fy = 50 ksi

Axial:

27 kips

48.68 in

On a 55 in Bolt Circle

#### Anchor Rod Calculation According to TIA-222-G section 4.9.9

0.80 TIA 4.9.9

6050.00 in Momet of Inertia

 $P_u =$ 

81 kips Tension Force

 $V_u =$ 

1 kips Shear Force

 $\mathbf{R}_{nt} =$ 

325.00 kips Nominal Tensile Strength

 $\eta =$ 

0.50 for detail type (d)

The following Interation Equation Shall Be Satisfied:

$$\left(\frac{\mathbf{P_u} + \frac{\mathbf{V_u}}{\eta}}{\phi \mathbf{R_{nt}}}\right) \leq 1.0$$

$$0.322 \leq 1$$

#### Base Plate Calculation According to TIA-222-G

0.90 TIA 4.7

 $M_{PL} =$ 

674.9 in-kip Plate Moment

L = $\mathbf{Z} =$  29.8 in Section Length

67.1 Plastic Section Modulus

674.85 in-kip ≤

Calculated Moment vs Factored Resistance 3018 in-kip

 $M_P =$ 

3353.5 in-kip Plastic Moment

 $\phi M_n =$ 

3018.1 in-kip Factored Resistance

**Anchor Rods Are Adequate** 

32.2% 🗹

Base Plate is Adequate

22.4% 🗹

#### Page 99-ft monopole - MFP #40913-039 Michael F. Plahovinsak, P.E. **FND** 18301 State Route 161 W Project Date Plain City, OH 43064 CT1014, Madison 2/9/2014 Phone: 614-398-6250 Client Designed by email: mike@mfpeng.com FLORIDA TOWER PARTNERS Mike

#### **Caisson Calculation**

#### According to ANSI/TIA-222-G-2

- 1. Foundation overturning resistance calculated with PLS Caisson, for Brom's method for rigid piles. Soil layers modeled after recommendations from the geotechnical report.
- 2. Cohesion strength for the upper 15.5 ft has been reduced by 50%
- 3. In lieu of a soil resistance factor fs = 0.75 (TIA-9.4.1) an additional safey fator against soil failure of 1.33 has been applied.
- 4. Foundation is designed with a minimum safety factor resisting overturning of 2.0
- 5. Foundation has been designed with factored loads per TIA-222-G.
- 6. Design water table = 8 ft below grade

\*\*\* PIER PROPERTIES CONCRETE STRENGTH (ksi) = 4.00

STEEL STRENGTH (ksi) = 60.00

DIAMETER (ft) = 7.000 DISTANCE FROM TOP OF PIER TO GROUND LEVEL (ft) = 1.00

*** SOIL PROPERTIES	LAYER	TYPE THICK	NESS D	EPTH AT TO	OP OF LAYE	R DENSITY	cυ	KР	PH
	(ft)	(ft) (p	cf) (psi	f) (degr	recs)				
1 S	4.00	0.00	0.0	1.000	-0.00				
2 S	2.00	4.00	100.0	1.698	14.99				
3 S	2.00	6.00	110.0	3.000	30.00				
4 S	7,50	8,00	47.6	3.000	30.00				
5 C	30.00	15.50	67.6	6000.0					

- ••• DESIGN (FACTORED) LOADS AT TOP OF PIER MOMENT (fi-k) = 1489.0 VERTICAL (k) = 27.0 SHEAR (k) = 20.0 ADDITIONAL SAFETY FACTOR AGAINST SOIL FAILURE = 1.33
- \*\*\* CALCULATED PIER LENGTH (ft) = 19.500
- \*\*\* CHECK OF SOILS PROPERTIES AND ULTIMATE RESISTING FORCES ALONG PIER

TYPE TOP OF	LAYER BE	LOW T	OP OF P	IER THICKNES	S DENS	ŧΤΥ	CU	KP	FORCE	ARM
	(ft)	(ft)	(pcf)	(psf)	(k) (I	ft)				
S	1.00	4.00	0.0	1.000	0.00	3.67				
S	5.00	2.00	100.0	1.698	7.13	6.33				
S	7.00	2.00	110.0	3.000	39.06	8.12				
S	9.00	7.50	47.6	3.000	282.79	13.12				
C	16.50	1.05	67.6	6000.0	353.12	17.03				
C	17,55	1,95	67.6	0,000	-654.88	18.53				

#### \*\*\* SHEAR AND MOMENTS ALONG PIER

	WITH THE ADDI	HONAL SAFELY	FACIO	K WITHOUT	ADDITIONA	AL SAFETY FACTOR
DISTANCE BELOW 1	TOP OF PIER (ft)	SHEAR (k)	MOME	NT (ft-k)	SHEAR (k)	MOMENT (ft-k)
0.00	27.2	2046.6	20.4	1535.4		
1.95	27.2	2099.7	20.4	1575.2		
3.90	27.2	2152.8	20.4	1615.0		
5.85	25.9	2205.5	19.5	1654.6		
7.80	7.8	2243.9	5.8	1683.3		
9.75	-39.7	2216.3	-29.8	1662.6		
11,70	-101.3	2080.7	-76,0	1560,9		
13.65	-174.4	1813.7	-130.9	1360.6		
15.60	-258.9	1393.0	-194.2	1045.0		
17.55	-654.6	638.8	-491.0	479.2		
19.50	-0.0	-0.0	-0.0	-0.0		

- \*\*\* TOTAL REINFORCEMENT PCT = 0.42 REINFORCEMENT AREA (in^2) = 23.28
- \*\*\* USABLE AXIAL CAP. (k) = 27.0 USABLE MOMENT CAP. (ft-k) = 3661.6

Est. Foundation Usage 49.98

Minimum Steel Per ACI-318

17.85 in

<sup>7-</sup>ft Diameter caisson x 22-ft long (21-ft Embeded with 1-ft above grade) W/(36) #8 Vertical Rebar. Concrete strength =4000 PSI @ 28 days. Estimated Concrete Volume = 31 cubic yards.

# **EXHIBIT C**



# RADIO FREQUENCY EMISSIONS ANALYSIS REPORT **EVALUATION OF HUMAN EXPOSURE POTENTIAL** TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CTNH808A

**Amtrak Madison** 15 Orchard Park Road Madison, CT 06443

February 24, 2014



February 24, 2014

T-Mobile USA Attn: Jason Overbey, RF Manager 35 Griffin Road South Bloomfield, CT 06002

Re: Emissions Values for Site: CTNH808A - Amtrak Madison

EBI Consulting was directed to analyze the proposed T-Mobile facility located at 15 Orchard Park Road, Madison, CT, for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu$ W/cm2). The number of  $\mu$ W/cm2 calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu$ W/cm2). The general population exposure limit for the cellular band is 567  $\mu$ W/cm2, and the general population exposure limit for the PCS and AWS bands is 1000  $\mu$ W/cm2. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

#### **CALCULATIONS**

Calculations were done for the proposed T-Mobile Wireless antenna facility located at 15 Orchard Park Road, Madison, CT, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, the actual antenna pattern gain value in the direction of the sample area was used. For this report the sample point is a 6 foot person standing at the base of the tower

For all calculations, all equipment was calculated using the following assumptions:

- 2 GSM channels (1935.000 MHz—to 1945.000 MHz / 1980.000 MHz—to 1985.000 MHz) were considered for each sector of the proposed installation.
- 2) 2 UMTS channels (2110.000 to 2120.000 MHz / 2140.000 MHz to 2145.000 MHz) were considered for each sector of the proposed installation.
- 3) 2 LTE channels (2110.000 to 2120.000 MHz / 2140.000 MHz to 2145.000 MHz) were considered for each sector of the proposed installation.
- 4) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 5) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The actual gain in this direction was used per the manufactures supplied specifications.
- 6) The antenna used in this modeling is the Ericsson AIR21 for LTE, UMTS and GSM. This is based on feedback from the carrier with regards to anticipated antenna selection. This antenna has a 15.6 dBd gain value at its main lobe. Actual antenna gain values were used for all calculations as per the manufacturers specifications



- 7) The antenna mounting height centerline of the proposed antennas is **100 feet** above ground level (AGL)
- 8) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculation were done with respect to uncontrolled / general public threshold limits

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Site ID	CTNH808A - Amtrak Madison
site Addresss	15 Orchard Park Road, Madison, CT 06443
ite Type	Monopole

	Power Density Percentage	0.19662%	0.00000%	0.09831%	0.09831%			Power	Percentage	0.19662%	0.00000%	0.09831%	0.09831%			Power	Density Percentage	0.19662%		200	0.09831%	がはないのでは、
	Power Density Value	1.966217	0	0.983109	0.983109	SECTION SECTIO		Power	Value	1.966217	0	0.983109	0.983109			Power	Density Value	1.966217	0	0.983109	0.983109	AND DESCRIPTION OF THE PERSON
	ERP	48.326044	0	24.163022 0.983109	24.163022	0.393%			ERP	48.326044	0	24.163022 0.983109	2	0.393%			ERP	48.326044	0	24.163022 0.983109	24.163022 0.983109	0.393%
	Additional Loss	0	0	0	0	Sector total Power Density Value:		Cable Lock Additional	Loss	0	0	0	0	Sector total Power Density Value:			Cable Loss Additional (dB) Loss	0	0	0	0	Sector total Power Density Value: 0.393%
	Cable Loss (dB)	0	0	0	0	al Power De		Cable Loss		0	0	0	0	al Power De			-	0	0	0	0	al Power De
	Cable Size	None	None	1-5/8"	1-5/8"	Sector tot			Cable Size	None	None	1-5/8"	1-5/8"	Sector tot			Cable Size	None	None	1-5/8"	1-5/8"	Sector tot
	analysis height	94	94	94	94			analveic	STREET,	94	94	94	94	THE REAL PROPERTY.			analysis	350	94	94	94	STATISTICS OF STREET
	Antenna Height (ft)	100	100	100	100			Antenna	Height (ft)	100	100	100	100	STREET, STREET			Antenna Height (ft)	100	100	100	100	CHARLES CONTROL
	Antenna Gain in direction of sample point (dBd)	-3.95	-3.95	-3.95	-3.95			Antenna Gain in direction	point (dBd)	-3.95	-3.95	-3.95	-3.95			Antenna Gain In direction	of sample point (dBd)	-3.95	-3.95	-3.95	-3.95	
Sector 1	Composite Power	120	0	09	09		Sector 2	Hoome	Power	120	0	09	09	THE REAL PROPERTY.	Sector 3		Number of Composite Channels Power	120	0	09	09	ON CONTRACTOR OF THE PERSONS ASSESSED.
Š	Number of Channels	2	ST. SECTION	2	2		Sec		Channels	2	State of the	2	2		Sei		Number of Channels	2		2	2	AUTOMOST PROPERTY.
	Power Out Per Channel (Watts)	09		30	30			Power Out Per	21705	09	DESCRIPTION OF THE PERSON OF T	30	30			Power Out Per	Channel (Watts)	. 09		30	8	Appropriate property of the second
	Technology	LTE	18 S. F. S. S. S.	GSM / UMTS	UMTS				Technology	LTE		GSM / UMTS	UMTS				Technology	LTE		GSM / UMTS	UMTS	Physical Inches Company of the Compa
	Frequency Band	AWS - 2100 MHz		PCS - 1950 MHz	AWS - 2100 MHz				Frequency Band	AWS - 2100 MHz		PCS - 1950 MHz	AWS - 2100 MHz	NATIONAL PROPERTY OF THE PARTY			Frequency Band	AWS - 2100 MHz		PCS - 1950 MHz	AWS - 2100 MHz	STATES OF THE PARTY OF THE PART
	Status	Active	Not Used	Active	Passive	ANIMATE PARTY AND ADDRESS OF THE PARTY AND ADD			Status	Active	Not Used	Active	Passive	Separate Sep			Status	Active	Not Used	Active	Passive	
	Antenna Model	AIR21 84A/82P	AIR21 B4A/B2P	AIR21 B2A / B4P	AIR21 B2A / B4P				Antenna Model	AIR21 B4A/B2P	AIR21 B4A/B2P	AIR21 B2A / B4P	AIR21 B2A / B4P				Antenna Model	AIR21 R44/R2P	AIR21 B4A/B2P	AIR21 B2A / B4P	AIR21 B2A / B4P	
	Antenna Make	Fricsson	Fricsson	Frirson	Fricson	DESCRIPTION OF STREET			Antenna Number Antenna Make	Fricson	Fricsson	Fricsson	Fricsson	SCHOOL SECTION			Antenna Number Antenna Make	Friceon	Fricsson	Ericsson	Fricson	LI ICOCCII
	Antenna	19	19	23	2R				Antenna	12	4	23	28				Antenna	13	14	29	38	77

Carrier	MPE %
T-Mobile	1.180%
AT&T	16.760%
Fotal Site MPE %	17.940%



### Summary

All calculations performed for this analysis yielded results that were well within the allowable limits for general public exposure to RF Emissions.

The anticipated Maximum Composite contributions from the T-Mobile facility are 1.180% (0.393% from each sector) of the allowable FCC established general public limit considering all three sectors simultaneously sampled at the ground level.

The anticipated composite MPE value for this site assuming all carriers present is 17.940% of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were within the allowable 100% threshold standard per the federal government.

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