



Crown Castle
3 Corporate Park Drive, Suite 101
Clifton Park, NY 12065

July 27, 2018

Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

RE: Notice of Exempt Modification for Sprint DO Macro: 876383
Sprint Site ID: CT33XC543
7 Sherwood Forest Lane, Killingworth, CT 06419
Latitude: 41° 20' 17.24"/ Longitude: -72° 33' 23.44"

Dear Ms. Bachman:

Sprint currently maintains six (6) antennas at the 150-foot level of the existing 150-foot monopole tower at 7 Sherwood Forest Lane in Killingworth, CT. The tower is owned by Crown Castle. The property is owned by Anderson Sherwood & Dian Trustees, Global Signal ACQ II LLC (Crown Castle entity). Sprint now intends to replace six (6) antennas with six (6) new antennas. These antennas would be installed at the 150-foot level of the tower. Sprint also intends to install twelve (12) RRH's and four (4) hybrid cables.

This facility was approved by the Town of Killingworth, CT Certified # Z 455 232 287 on February 22, 2000. This approval was given without conditions.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.S.C.A. § 16-50j-73, a copy of this letter is being sent to First-Selectwomen Catherine Lino, Town of Killingworth, property owner stated on property card is Global Signal which is a Crown subsidiary, and Crown Castle is the tower owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.

Melanie A. Bachman

July 27, 2018

Page 2

5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, Sprint respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Jeffrey Barbadora.

Sincerely,



Jeffrey Barbadora

Real Estate Specialist

12 Gill Street, Suite 5800, Woburn, MA 01801

781-729-0053

Jeff.Barbadora@crowncastle.com

Attachments:

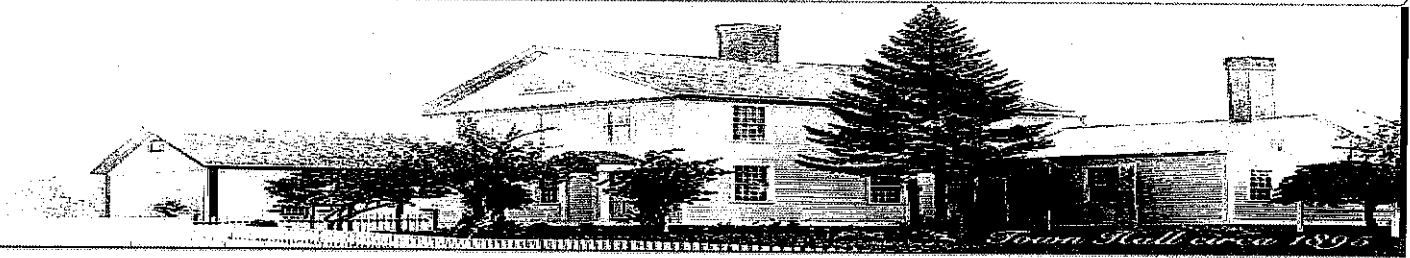
Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes

Tab 2: Exhibit-2: Structural Modification Report

Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: First-Selectwomen Catherine Lino
Town of Killingworth
323 Route 81
Killingworth, CT 06419

The Assessor's office is responsible for the maintenance of records on the ownership of properties. Assessments are computed at 70% of the estimated market value of real property at the time of the last revaluation which was 2016.



Information on the Property Records for the Municipality of Killingworth was last updated on 7/21/2018.

Property Summary Information

Parcel Data And Values

Sales

Permits

Google Map

Parcel Information

Location:	7 SHERWOOD FOREST LANE	Property Use:	Residential	Primary Use:	Residential
Unique ID:	00247300	Map Block Lot:	37-03	Acres:	2.14
490 Acres:	0.00	Zone:	R-2	Volume / Page:	0218/0491
Developers Map / Lot:	SM1902/SPRINT E	Census:	6401		

Value Information

	Appraised Value	Assessed Value
Land	247,499	173,250
Buildings	0	0

	Appraised Value	Assessed Value
Detached Outbuildings	0	0
Total	247,499	173,250

Owner's Information

Owner's Data

ANDERSON SHERWOOD & DIAN TRUSTEES
 GLOBAL SIGNAL ACQ II LLC
 PMB 331, 4017 WASHINGTON RD
 MCMURRAY PA 15317

[Back To Search \(JavaScript:window.history.back\(1\);\)](#)

[Print View \(PrintPage.aspx?towncode=070&uniqueid=00247300\)](#)

Information Published With Permission From The Assessor

7 sherwood

Search Results

Parcel Details

7 SHERWOOD FOREST LANE**ANDERSON SHERWOOD & DIAN TRUSTEES**

PMB 331, 4017 WASHINGTON RD
MCMURRAY, PA 15317

Parcel ID: 37-03
Lot Size (ac): 2.14
Sale Price: \$0

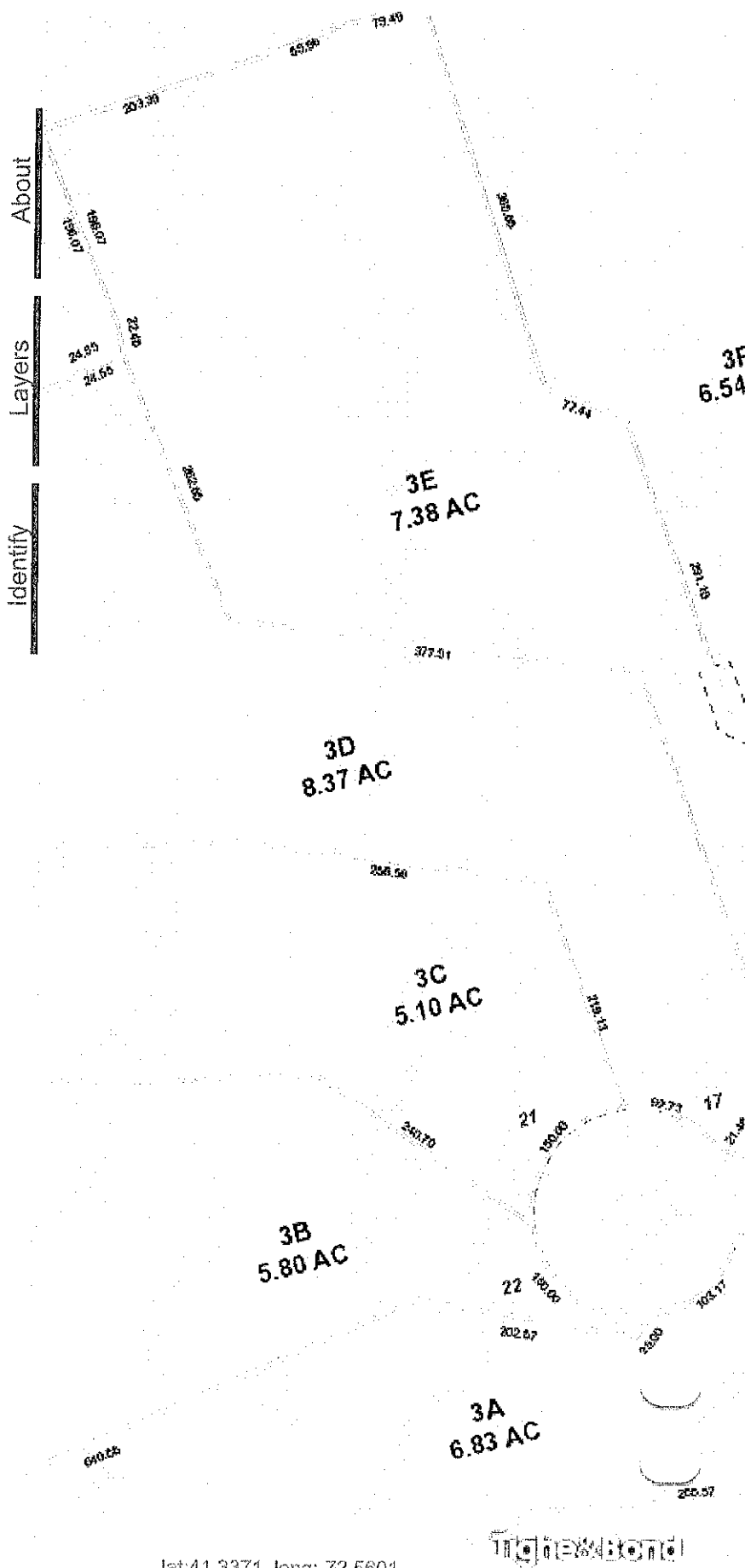
Links	Abutters
Parcel Details	Bing Bird's Eye
Photo	Add Parcel
Google Map	Remove Parcel
Abutter Distance:	Print Labels
Adjacent	Export List
Adjacent	Parcel_ID 37-03
50 ft	StreetAddr 7 SHERW
100 ft	
200 ft	
300 ft	
400 ft	
500 ft	ERWOOD & DIAN TRU
Find Abutters	
Clear Abutters	331, 4017 WASHINGTO

City MCMURRAY

State PA

Scroll

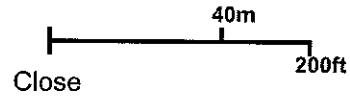
Zipcode 15317



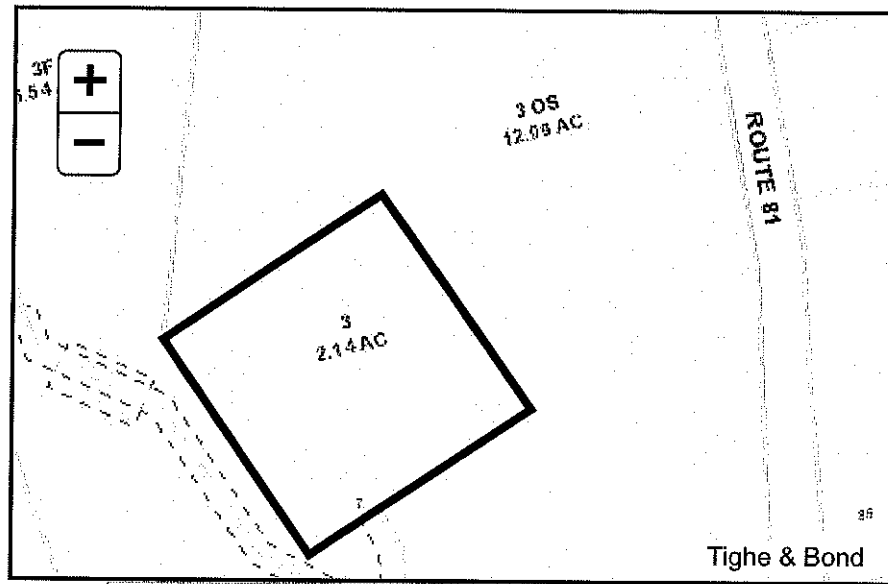
lat:41.3371, long:-72.5601

Email Map Link

Copy and paste the following string into an email to link to the current map view:



Print Map



Size: 8 1/2 X 11 Landscape

Scale: 1" = 100 ft. Title: 7 Sherwood Forest Lane

Close Print

*Town of Killingworth
Inland Wetlands and Watercourses Commission
Killingworth, Connecticut 06419*

Certified # Z 455 232 287

Memorandum of Decision

At its Regular Meeting of February 22, 2000, the Commission approved the application of Sherwood R. Anderson (applicant, Sprint Spectrum, L.P.) for construction of a telecommunication facility, a 150 foot monopole, and upgrading of an existing woods road on Route 81, Map 37, Parcel 3.

This permit is granted subject to the following conditions:

1. The work is to be done exactly as shown on the revised plan "Sprint PCS, Anderson Property, Clinton Road (Connecticut Route 81), Killingworth, CT CT33XC543" prepared by Goodkind & O'Dea, Inc. Consulting Engineers and Planners, dated 11/19/99, with two revisions (12/15/99 and 2/22/00), consisting of 10 sheets.
2. All construction is to be carried out in a workman-like manner.
3. During construction, appropriate and effective measures must be taken to prevent silting and water discoloration downstream.
4. If during construction, it becomes apparent that rain and surface drainage runoff will cause silting or water discoloration of the adjacent wetlands and/or watercourse, then appropriate and effective protection against these conditions must be taken.
5. No excavation may commence until all erosion and sedimentation controls, as defined on the approved plans, are in place and have been inspected and approved by the Killingworth IWWC or its agent.
6. The Commission is to be notified when this project begins and when it is completed.
7. The Commission, or its representative, shall be free to make interim inspections of the site as it deems necessary.
8. In no way is it to be construed that this permit allows the applicant to perform any other activity than that which is stated above. Any additional activity requires an additional permit or an amendment to this permit.

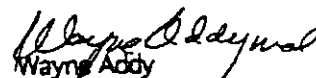
page 2 - Memorandum of Decision # Z 455 232 287

9. This permit shall be valid for five (5) years from the date of the legal notice publication. However, the regulated activity or use authorized by the permit must be completed within two (2) years from the time such activity is commenced, unless otherwise specified.

10. Any application to renew this permit should be submitted at least sixty-five (65) days prior to the expiration date.

This permit is granted in the belief that there will be minimal adverse impact on the environment.

For the Commission,


Wayne Addy
Chairman

IN RE APPLICATION
SPECIAL EXCEPTION
SPRINT SPECTRUM L.P.

KILLINGWORTH PLANNING
AND ZONING COMMISSION
MARCH 21, 2000

MEMORANDUM OF DECISION

An application #149 of SPRINT SPECTRUM L.P. for a Special Exception under Section 120 of the Zoning Regulations for Communication Tower was submitted to the Planning & Zoning Commission at its meeting of February 1, 2000. The property is located at Route 81 and is shown on Tax Map 37, Parcel 3. The owner as recorded in the Killingworth Land Records (Volume 87, Page 642) is Sherwood R. Anderson. The application was considered under the standards prescribed in Section 120G of the Zoning Regulations.

The Commission, at its meeting of March 21, 2000, voted to approve the application for Special Exception with the following conditions:

1. Leased area must encompass the fall zone of the tower.
2. Lighting be limited to time of service repairs.

Charles E. Martens
Charles E. Martens, Chairman

Dated at Killingworth, Connecticut this 3rd day of April, 2000.

Certified a true and correct copy of that
which is on file with the Killingworth
Planning & Zoning Commission

Date: 4/5/00 Time: 12:50 PM

Clerk of KP&ZC: Susith R. Brown

Rec'd. 4/5/00 at 12:50 PM
James G. Garry Town Clerk

IN RE APPLICATION
SPECIAL EXCEPTION
SPRINT SPECTRUM L.P.

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AND ZONING COMMISSION
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Certified a true and correct copy of that
which is on file with the Killingworth
Planning & Zoning Commission

Date: 4/5/00 Time: 12:50 PM

Clerk of KP&ZC: Judith R. Brown

Received for record April 6, 2000
at 10 h 00 m A M and recorded by
James J. Dwyer Town Clerk

08
KILLINGWORTH PLANNING & ZONING COMMISSION

TOWN OFFICE BUILDING
323 ROUTE 81
KILLINGWORTH, CONNECTICUT 06419-1298

April 5, 2000

Sprint Spectrum LP
1 International Boulevard
Mahwah, NJ 07495

RE: Memorandum of Decision
March 21, 2000

Gentlemen:

Please be advised that your application for Special Exception for Telecommunications Tower dated 1/18/00 was approved with conditions by the Planning & Zoning Commission at its March 21, 2000 meeting. A copy of the Memorandum of Decision is enclosed. The Notice of Decision was published in The Hartford Courant on Friday, March 31, 2000.

Sincerely,

Charles E. Martens Jr.

Charles E. Martens, Jr. (jr)
Chairman

CEM/jrb

CERTIFIED MAIL, RRR
Z 285 870 490



NORTHERN CONNECTICUT

AREA MAJ

DRAWING INDEX	
SHEET NO.	SHEET TITLE

1-1	TITLE SHEET & PROJECT DATA	1
2-1	STRUCTURAL FOUNDATIONS	2
3-2	SPRINKLER SYSTEMS	3
3-3	SPRINKLER SCHEDULE	3
4-1	SOIL PLAIN	4
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THESE PLANS HAVE BEEN DEVELOPED FOR THE OPERATION OF AN EXISTING UNMANNED TELECOMMUNICATIONS FACILITY OWNED OR LEASED BY GOVERNMENT IN ACCORDANCE WITH THE SCOPE OF WORK PROVIDED BY SPECIFIC INQUIRY HAS INDICATED THAT THE SCOPE OF WORK IN THE PLANS, THESE PLANS ARE NOT FOR CONSTRUCTION UNLESS ASSUMED BY A PRESSING STRUCTURAL, STABILITY ANALYSIS PREPARED BY A LICENSED STRUCTURAL ENGINEER. STRUCTURAL ANALYSIS MUST INCLUDE BOTH STRUCTURE AND MOUNT.

APPLICABLE CODES

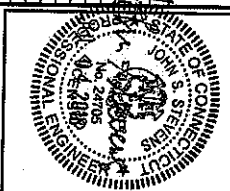
1. RESIDENTIAL BUILDING CODE (2015 IBC)
2. 2012 IBC
3. 2012 IBC OR LATEST EDITION
4. 2011 NATIONAL ELECTRICAL CODE
5. 2011 NATIONAL FIRE PROTECTION CODE
6. 2011 NATIONAL MECHANICAL CODE
7. ANY OTHER NATIONAL OR LOCAL APPLICABLE CODES,
8. MOST RECENT EDITIONS
9. LOCAL BUILDING CODE
10. CITY/COUNT ORDINANCES

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RELATIONS		DATE	BY	REMARKS
DESCRIPTION				
ISSUED FOR CONSTRUCTION	09/01/82	PAI	2	
LAND RESERVATION	26/02/83	SHB	1	
ISSUED FOR CONSTRUCTION	05/04/83	PAI	0	
ISSUED FOR REVIEW	11/07/87	PAI	1	

**CLINTON /
ANDERSON PROPERTY**

CT33XC543

SHEET DESCRIPTION: / SHERWOOD FOREST
KILLINGWORTH, CT 06419

& PROJECT DATA

CONTINUE FROM SP-1

1. PERFORM ALL REQUIRED SITE ENVIRONMENTAL MONITORING.
2. PREPARE GROUND STRESS, PROVIDE DE-DRILLING, AND ROUGH AND FINAL UNDERGROUND FACILITIES INCLUDING UNDERGROUND POWER AND COMMUNICATIONS CONDUITS, AND UNDERGROUND GROUNDING SYSTEM.
3. INSTALL ABOVE GROUND DRAINING SYSTEMS.
4. PROVIDE NEW HANG INSTALLATIONS AND MODIFICATIONS.
5. INSTALL BODIES, ACCESS WAYS, CABLES AND BOWNS AS INDICATED.
6. ACCOMPLISH REQUIRED MODIFICATION OF EXISTING FACILITIES.
7. PROVIDE ANTENNA SUPPORT STRUCTURE FOUNDATIONS.
8. PROVIDE STABLE AND EQUIPMENT PLATFORMS.
9. INSTALL COMPOUND FLOORING, SHIRT SHELVING, LANDSCAPING AND ACCESS BARRIERS.
10. PERFORM INSPECTION AND MONITORING, TESTING AS REQUIRED HEREINAFTER.
11. CONDUCT SITE RESISTANCE TO EARTH TESTING AS REQUIRED HEREINAFTER.
12. INSTALL TOWER, ANTENNA SUPPORT STRUCTURES AND PLATFORMS OR EXISTING TOWERS AS REQUIRED.
13. INSTALL CELL SITE RADIOS, WIRELESS, GPRS, CDMA, UML, AIRPORTS, RELATED EQUIPMENT.
14. PERFORM, DOCUMENT, AND CLOSE OUT ANY CONSTRUCTION CONTROL DOCUMENTS THAT MAY BE REQUIRED BY GOVERNMENT AGENCIES AND LANDLORDS.
15. PERFORM AIRPANEL, AND CABLE SNIP TESTING AND WARE ANY AND ALL NECESSARY CONNECTIONS.
16. REMAIN ON SITE UNTIL THE PROJECT IS COMPLETED AND INTERFERED TO BE PLACED ON AVE.

3.2 GENERAL REQUIREMENTS FOR CIL CONSTRUCTION:

- A. CONSTRUCTION SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIALS, DEBRIS, AND TRASH, AT THE COMPLETION OF THE WORK, CONSTRUCTION SHALL REMOVE FROM THE SITE ALL REMAINING RUBBER, IMPEDIMENTS, TEMPORARY FENCING, AND SHIELDING MATERIALS.
- B. EQUIPMENT SHALL BE AT ALL TIMES BE MAINTAINED THROUGH CLEAN AND CLEAN OF DEBRIS.
- C. CONSTRUCTION SHALL TAKE ALL NECESSARY PRECAUTIONS TO PROTECT AND LOCATE ANY HAZARDOUS MATERIALS.
1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY HAZARDOUS MATERIALS WHICH ARE NOT IDENTIFIED BY THE CONTRACTOR OR ANY OTHER PERSONS IN THE PROJECT AREA AND SHALL BE RESPONSIBLE FOR THE WORK IN THE AFFECTED AREA, SHALL NOT BE RESPONSIBLE FOR ANY OTHER PERSONS IN THE PROJECT AREA, SHALL NOT BE RESPONSIBLE FOR ANY OTHER PERSONS IN THE PROJECT AREA, SHALL NOT BE RESPONSIBLE FOR ANY OTHER PERSONS IN THE PROJECT AREA.
2. CONSTRUCTION SHALL USE CARE WHILE ON THE SITE AND SHALL NOT TAKE ANY ACTION WHICH COULD BE CONSIDERED AS A VIOLATION OF ANY FEDERAL, STATE, OR LOCAL LAWS, ORDINANCES, OR REGULATIONS, OR TO FURNISH EVIDENCE INDICATING TO THE AGENCY.
- D. CONSTRUCTION SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROTECT DOCUMENTATION.
- E. CONDUCT TESTING AS REQUIRED HEREIN.

3.3 REQUIREMENTS:

- A. CONDUCTOR SHALL REVIEW APPROVE AND SIGN TO SIGN AND SIGNATURES, PROJECT DATA, SURVEYS, AND SIGNATURES AS REQUIRED HEREINAFTER.
- B. DOCUMENTATION SHALL BE FORWARDED IN ORIGINAL FORM AND/OR ELECTRONIC FORM.
1. ALL CORRESPONDENCE AND PRELIMINARY CONSTRUCTION REVIEWS.
2. PROJECT PROGRESS REPORTS.
3. CIL CONSTRUCTION START DATE (POPULATE FIELD IN SWS AND/OR FORWARD INFORMATION).
4. ELECTRICAL SERVICE COMPLETION DATE (POPULATE FIELD IN SWS AND/OR FORWARD INFORMATION).

SECTION 01.400 - SUBMITTALS & TESTS

PART 1 - GENERAL

- 1.1 THE WORK, THESE STANDARD CONSTRUCTION SPECIFICATIONS, IN CONJUNCTION WITH THE OTHER CONTRACT DOCUMENTS AND THE CONSTRUCTION DRAWINGS, DESCRIBE THE WORK TO BE PERFORMED BY THE CONTRACTOR.
- 1.2 RELATED DOCUMENTS:
 - A. THE REQUIREMENTS OF THIS SECTION APPLY TO ALL SECTIONS IN THIS SPECIFICATION.
 - B. SPENT STANDARD CONSTRUCTION DETAILS FOR WIRELESS SITES ARE INCLUDED IN THE WORK OF THIS SECTION.
 - C. THE WORK IN ALL SECTIONS SHALL COMPLY WITH THE CONSTRUCTION DRAWINGS AND THE OTHER CONTRACT DOCUMENTS.
- 1.3 SUBMITTALS:
 - A. THE WORK IN ALL SECTIONS SHALL COMPLY WITH THE CONSTRUCTION DRAWINGS AND THE OTHER CONTRACT DOCUMENTS.
 - B. SUBMIT THE FOLLOWING TO COMPANY REPRESENTATIVE FOR APPROVAL:
 1. CONCRETE MIX DESIGNS FOR TOWER FOUNDATIONS, ANCHORS PIER, AND CONCRETE PILING.
 2. CONCRETE BLOCK TESTS AS SPECIFIED HEREIN.
 3. SPECIAL FINISHES FOR ANTENNA SPACES, IF ANY.
 4. ALL EQUIPMENT AND MATERIALS TO BE INSTALLED ON THE CONSTRUCTION DRAWINGS.
 5. CRITICAL CRACKING DESIGN.
 - C. ATTEMPTS AT THE COMPANY'S REQUEST, ANY ATTEMPTS TO THE MATERIALS OR METHODS SPECIFIED SHALL BE SUBMITTED TO SPRINT'S CONSTRUCTION REVIEW AND APPROVAL. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE WORK IN THE AFFECTED AREA, SHALL NOT BE RESPONSIBLE FOR ANY OTHER PERSONS IN THE PROJECT AREA, SHALL NOT BE RESPONSIBLE FOR ANY OTHER PERSONS IN THE PROJECT AREA, SHALL NOT BE RESPONSIBLE FOR ANY OTHER PERSONS IN THE PROJECT AREA.
- 1.4 TESTS AND INSPECTIONS:
 - A. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROTECT DOCUMENTATION.
 - B. CONSTRUCTION SHALL ACCOMPLISH TESTING INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
 1. CIL CONSTRUCTION START DATE (POPULATE FIELD IN SWS AND/OR FORWARD INFORMATION).
 2. ELECTRICAL SERVICE COMPLETION DATE (POPULATE FIELD IN SWS AND/OR FORWARD INFORMATION).
 3. CONSTRUCTION SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROTECT DOCUMENTATION.
 4. CONSTRUCTION SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROTECT DOCUMENTATION.
 - C. REQUIRED CIL CONSTRUCTION DOCUMENTATION INCLUDES, BUT IS NOT LIMITED TO THE FOLLOWING:
 1. CIL CONSTRUCTION START DATE (POPULATE FIELD IN SWS AND/OR FORWARD INFORMATION).
 2. ELECTRICAL SERVICE COMPLETION DATE (POPULATE FIELD IN SWS AND/OR FORWARD INFORMATION).
 3. CONSTRUCTION SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROTECT DOCUMENTATION.
 4. CONSTRUCTION SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROTECT DOCUMENTATION.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION

- 3.1 REQUIREMENTS FOR TESTING:
 - A. THIRD PARTY TESTING AGENCY:
 1. WHEN THE USE OF A THIRD PARTY INDEPENDENT TESTING AGENCY IS REQUIRED, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE WORK IN THE AFFECTED AREA, SHALL NOT BE RESPONSIBLE FOR ANY OTHER PERSONS IN THE PROJECT AREA, SHALL NOT BE RESPONSIBLE FOR ANY OTHER PERSONS IN THE PROJECT AREA, SHALL NOT BE RESPONSIBLE FOR ANY OTHER PERSONS IN THE PROJECT AREA.
 2. THE THIRD PARTY TESTING AGENCY IS TO BE SELECTION WITH THE APPLICABLE ASSOCIATED HEALTH AND SAFETY ISSUES.
 3. EXPERIENCE IN SOILS, CONCRETE, WOODWORK, ASBESTOS, AND ASPHALT TESTING USING SON, AGING, AND OTHER METHODS IS REQUIRED.
 4. EXPERIENCE IN SOILS, CONCRETE, WOODWORK, ASBESTOS, AND ASPHALT TESTING USING SON, AGING, AND OTHER METHODS IS REQUIRED.
- 3.2 REQUIRED TESTS:
 - A. CONSTRUCTION SHALL ACCOMPLISH TESTING INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
 1. CONCRETE CURED BREAK TESTS FOR THE TOWER AND ANCHOR FOUNDATIONS AS SPECIFIED IN SECTION PORTLAND CEMENT CONCRETE PILING.
 2. SPECIAL FINISHES FOR ANTENNA SPACES, IF ANY.
 3. SPECIAL FINISHES FOR ANTENNA SPACES, IF ANY.
 4. ALL EQUIPMENT AND MATERIALS TO BE INSTALLED ON THE CONSTRUCTION DRAWINGS.
 5. CRITICAL CRACKING DESIGN.
 6. TESTING REQUIRED UNDER SECTION: APPROVED BASE FOR ACCESS ROADS, PAVES AND ANCHOR LOCATIONS.
 7. STRUCTURAL, SKEWAL, COMPLETION TESTS FOR THE TOWER FOUNDATION.
 8. SITE RESISTANCE TO EARTH TESTING FOR EXHIBIT: CIL SITE GRADING AND SPECIAL FINISHES.
 9. ANTENNA AND CIL SWEEP TESTS FOR PORTLAND CEMENT TRANSMISSION LINE ACCEPTANCE STANDARDS.
 10. GEOTECHNICAL AT ANTENNA LOCATIONS FOR GRS AND ANTENNAS.
 11. ALL OTHER TESTS REQUIRED BY COMPANY OR JURISDICTION.
- 3.3 REQUIRED INSPECTIONS:
 - A. SCHEDULE INSPECTIONS WITH COMPANY REPRESENTATIVE.
 - B. CONDUCT INSPECTIONS INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
 1. CONSTRUCTION SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROTECT DOCUMENTATION.
 2. CONSTRUCTION SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROTECT DOCUMENTATION.
 3. CONSTRUCTION SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROTECT DOCUMENTATION.
 4. CONSTRUCTION SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROTECT DOCUMENTATION.
 - C. CONSTRUCTION SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROTECT DOCUMENTATION.

PLAN REQUIRED FOR:



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REVISION	DATE	BY	REV
1	07/02/03	DA	1
2	07/02/03	DA	2
3	07/02/03	DA	3
4	07/02/03	DA	4
5	07/02/03	DA	5

CLINTON /
ANDERSON PROPERTY
CT33XC543

7 SHERWOOD FOREST
KILLINGWORTH, CT 06419

SPRINT SPECIFICATIONS
SP-2

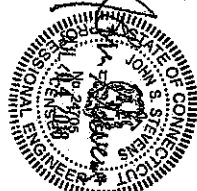


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**CROWN
CASTLE**



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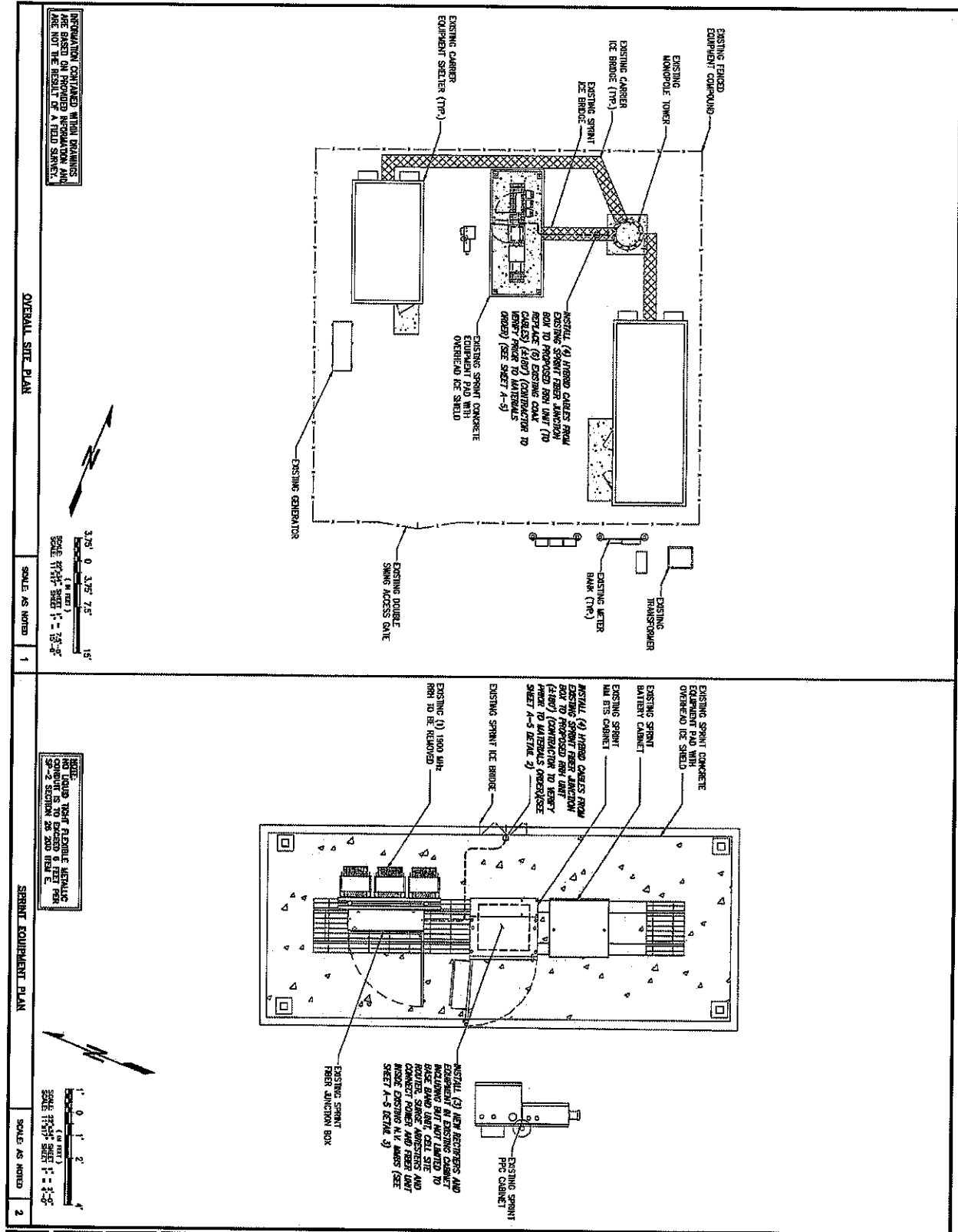
REVISION	DATE	BY	REV
ISSUED FOR CONSTRUCTION	07/24/11	AK	1
ISSUED FOR CONSTRUCTION	07/24/11	AK	2
ISSUED FOR CONSTRUCTION	07/27/11	AK	3

SITE NAME:
CLINTON /
ANDERSON PROPERTY

SITE OFFICER:
CT33XC543

SITE ADDRESS:
7 SHERWOOD FOREST
KILLINGWORTH, CT 06419

SHEET NUMBER:
A-1



INFORMATION CONTAINED WITHIN DRAWINGS ARE BASED ON PROVIDED INFORMATION AND ARE NOT THE RESULT OF A FIELD SURVEY.

NOTE:
SEE DETAIL 2 ON A-3
FOR ANTENNA LAYOUT

INSTALL (1) SPOT PANEL
ANTENNA TO REPLACE EXISTING
ANTENNA EACH SECTION (SEE
SHEET A-4 DETAIL 3)

TOP OF EXISTING TOWER
ELEV. = 2159'-0" A.S.L.

EXISTING (1) 1800 AMP
RHW TO BE RELOCATED
BEHIND EXISTING PANEL
ANTENNA EACH SECTION

EXISTING CARRIER
PANEL ANTENNA (TYP.)

INSTALL (2) 800 AMP
RHW EACH SECTION (SEE
SHEET A-4 DETAIL 2)

INSTALL (1) RHW-2.5
EACH SECTION (SEE
SHEET A-4 DETAIL 1)

NOTE:
STRUCTURAL ANALYSIS COMPLETED BY:
CROWN CASTLE FOR ADDITIONAL INFORMATION
SEE REPORT TITLED "STRUCTURAL ANALYSIS
REPORT, CARRIER SITE NUMBER C133XCS43"
CONDUCTED BY CROWN CASTLE. ACCORDING TO
RESULTS OF STRUCTURAL ANALYSIS, CARRIER SITE
STRUCTURE HAS SUFFICIENT CAPACITY TO
SUPPORT THE PROPOSED LOADING.

WIND ANALYSIS COMPLETED BY WINDPROY
FOR ADDITIONAL INFORMATION SEE REPORT
COMPLETED BY WINDPROY TITLED "WIND
STRUCTURAL ANALYSIS, CARRIER SITE
NUMBER C133XCS43 DATED MAY 14,
2016. ACCORDING TO THE RESULTS OF THE
ANALYSIS, THE EXISTING STRUCTURE HAS
SUFFICIENT CAPACITY TO SUPPORT THE
PROPOSED LOADING.

EXISTING MONOPOLE TOWER

INSTALL (4) HYBRID CABLE ROW
EXISTING SPOT PANEL JUNCTION BOX
TO PROPOSED RHW LIFT (RHW)
(CONTRACTOR TO VERIFY PRIOR TO
INSTALLATION) (SEE SHEET A-5
DETAIL 2)

GROUND LEVEL

LOWER ELEVATION

NO SCALE

1

NOTE:
CONTRACTOR TO LINE UP WHITE
MARKING ON EXISTING TOWER
W/ALSO CONNECTORS AND STAKE THE
BAYONET CONNECTOR TO THE LUMBER
ON THE LOWER CABLE ROW LIFT
AGAIN THE RED SEAL ON THE ROSS
CONNECTION.

TRAIL-LINE TO LUMBER CONNECTION
W/ALSO TO BE INSTALLED IN ACCORDANCE
WITH THE MANUFACTURER'S RECOMMENDATIONS
NOTE:
CONTRACTOR TO USE
INSERTS FOR
INSTALLATION OF
CONNECTIONS

BREAK OUT CYLINDER

TEAR BREAK
POINTS TO RHW

INSTALL (1) TEAR CABLE ROW EXISTING
SPOT PANEL JUNCTION BOX
TO PROPOSED RHW LIFT (CONTRACTOR TO
VERIFY PRIOR TO MATERIALS ORDER)

HYBRID BREAKOUT DETAIL

NO SCALE

2

ANTENNA: COMSCOPE NUVV-658-R4
DIMENSIONS (HxWxD): 79.1"x23.9"x13.9"
WEIGHT: 108 LBS.

PANEL ANTENNA DETAIL

NO SCALE

3



PLAN PROVIDED BY:
INFINICITY
FROM ZERO TO INFINICITY
FOR ADDITIONAL SEE ENDUSER
003 Westville Road
Phone: 910-940-0222 Fax: 910-940-0223
www.infinity-nc.com

CROWN CASTLE



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REVISION	DATE	BY	REV
ISSUED FOR CONSTRUCTION	01/24/16	MM	2
AND DESIGN	02/03/16	SM	1
ISSUED FOR CONSTRUCTION	04/01/16	MM	3
ISSUED FOR REVIEW	01/17/17	ETC	A

SITE NAME:
**CLINTON /
ANDERSON PROPERTY**

SITE CARRIER:
CT133XCS43

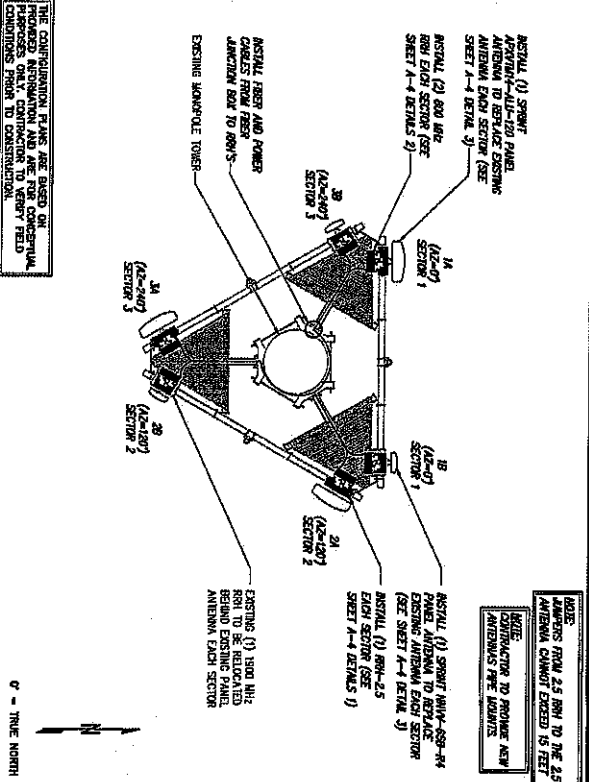
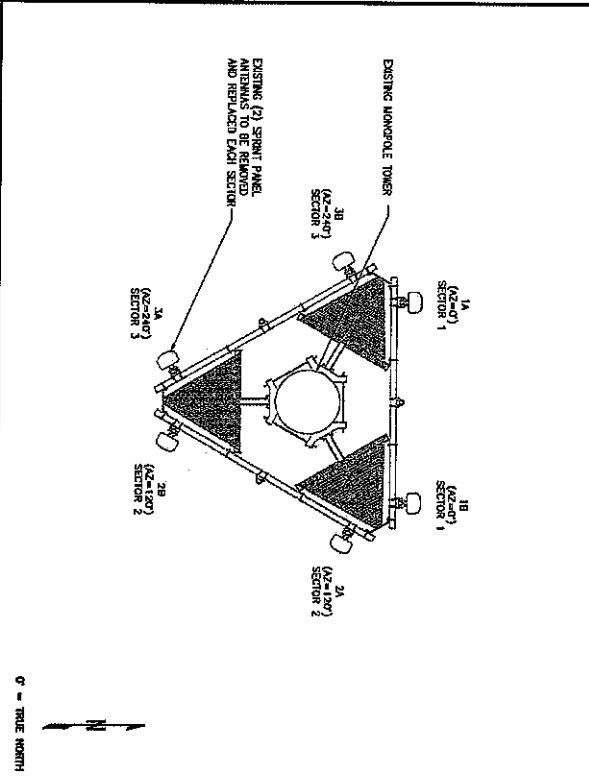
SITE ADDRESS:
**7 SHERWOOD FOREST
KILLINGWORTH, CT 06419**

SHEET DESIGNATION:
**TOWER ELEVATION
& CABLE PLAN**

SHEET NUMBER:
A-2

DATE	BY	DESCRIPTION	REMARKS
10/10/76	LM	RECEIVED THE CONSTRUCTION	THE CONSTRUCTION OF CONCRETE AND AS
10/20/76	LM	AND RECEIVED THE CONSTRUCTION	THE SOILS REPORT OF 10/10/76 AND AS
11/14/76	LM	RECEIVED THE CONSTRUCTION	REPRODUCED, DISSEMINATED FOR REBID
11/17/76	LM	RECEIVED THE CONSTRUCTION	WITHOUT THE EXPRESS WRITTEN CONSENT OF
			SPRINT.

ANDERSON PROPERTY
SITE NUMBER: CT33XC543
SITE ADDRESS: 7 SHREWOOD FOREST
KILLINGWORTH, CT 06419
AGENT INFORMATION: ANTENNA LAYOUT
& MOUNTING DETAILS
PICT NUMBER: A-3



EXISTING ANTENNA LAYOUT

NO SCALE

FINAL ANTENNA & RRH LAYOUT

NO SCALE

G = TRUE NORTH

Q - TRUE NOF

1. ALL ANTENNA HEIGHTS ARE TO CENTER OF HORIZONTAL ANTENNA

2. VERIFY AZADUTH AND CA HEIGHT WITH AS-BUILT DRAWINGS IF AVAILABLE.

3. NO OBJECT IS TO BE WITHIN 4.0 DEGREES OF BEAST-SHIRT OF THE END OF ANY OTHER TOWER ANTENNA. IF NECESSARY, 1.50 ANTENNA CAN BE PLACED AT THE END OF AN HORIZONTAL TOWER. NOON READERS FOR CLEAR LINE OF SITE OR EVEN ON ANOTHER SECTOR FOR CLEAR LINE OF SITE.

4. 2.50 ANTENNA MUST BE AT LEAST 8' FROM 1500MHZ ANTENNA, 30" FROM BROADBAND ANTENNA AND 30" FROM DUAL BAND 1500MHZ AND DUALBAND ANTENNA.

5. IF ANTENNAS ARE LOCATED ON A FREE STANDING SUCH AS A BUILDING WALL BROADBAND WILL, OR WIDER TOWER WALL, THIS TOWER MUST BE ACCOMPANIED BY A SIGNAGE PROVIDED BY ITS OPERATING COMPANY. THE SIGNAGE MUST BE PLACED ON THE SIDE OF THE TOWER OR WHERE ANTENNA IS TO BE LOCATED. CONTRACT SHALL BE PROVIDED BY THE SIGNAGE IS RESISTANT.

6. ELECTRICAL CONNECTIONS TO BUILD, VERIFY AZADUTH AND CA HEIGHT AND MECHANICAL DOWNWELL. IF ELECTRICAL CONNECTIONS TO BUILD, VERIFY AZADUTH AND CA HEIGHT AND MECHANICAL DOWNWELL TO BE DOWNWELL OR JAWWAKER IF IF DOWNWELL DOES NOT ANSWER, BUT STILL LEAVE DOWNWELL TO BE DOWNWELL USING DOWNWELL REPAIRING ABOVE FOR FURTHER INSTITUTIONS. IF SPRINT DOES NOT CONNECT WITHIN ONE HOUR, 2.50 ANTENNA AT SAME CA HEIGHT AS 1.50 ANTENNA AND DUAL CORRECTED TO HEIGHT. ALSO EVAL CORRECT 1900 MHZ AND 800 MHZ ANTENNA CA HEIGHT, AZADUTH AND MECHANICAL DOWNWELL TO BE DOWNWELL.

7. USE TEST TO VERIFY OPERATION IS TO BE PERFORMED AFTER FINAL INSTALLATION OF ANTENNAS AND ALSO CHARGES HAVE BEEN CONNECTED. VERIFY OPERATION OF ALL EXISTING SPRINT AND DOWNWELL INCLUDING BROADBAND, 1.50KHZ AND 2.50K. TEST INCLUDE COMPLETE DOWNWELL, AZADUTH (IF SHARP TEST SENSITIVITY).

8. A CRITICAL CONNECTIONS HAVE SITES (IF NOT LOCATED) IN FRONT OF ANTENNA. THIS MEANS NO OBJECT IS TO BE LOCATED 4.0 DEGREES OF BEAST-SHIRT OF THE END OF ANY OTHER TOWER ANTENNA. IF NECESSARY, 1.50 ANTENNA CAN BE PLACED AT THE END OF AN HORIZONTAL TOWER. NOON READERS FOR CLEAR LINE OF SITE OR EVEN ON ANOTHER SECTOR FOR CLEAR LINE OF SITE.

9. GENERAL CONTRACT IS REQUIRED TO USE A DUAL BANDBAND TOOL TO SET AZADUTH, TOLL AND DOWNWELL. AZADUTH ACCURACY IS TO BE WITHIN 0.1 DEGREES. IF FOR SOME REASON THIS ACCURACY CANNOT BE ACHIEVED, SPRINT AS-BUILT DRAWINGS AND DUAL SPRINT IF DOWNWELL WITH AS-BUILT SETTINGS. USE 22 DEGREES DOWNWELL TOOL OR EQUIVALENT TOOL. <http://www.sprint.com/antenna-aligner-tool/>

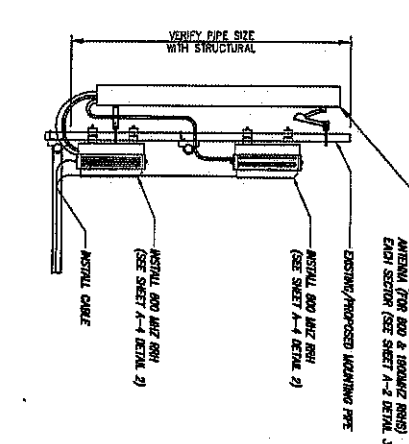
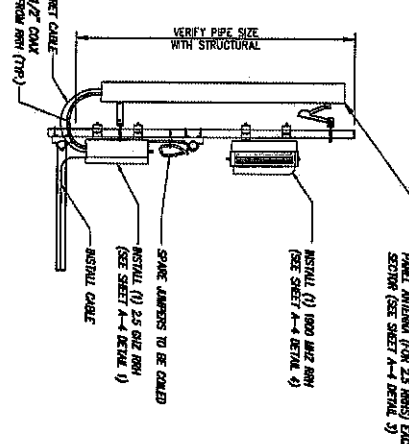
NOTES	NO SCALE
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[illegible]

TYPICAL ANTENNA & RRL MOUNTING DETAILS

NO SCALE

A-3



NOTES

1. CUT DC CONDUCTORS TO LENGTH
2. COIL FIBER CABLE AND SECURE AT SIDE OF RHT
3. DO NO EXCEED BEND RADIUS.

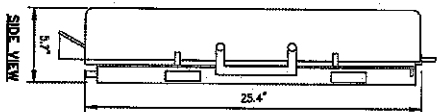
NOTE: CONTRACTOR TO POSITION RSH ON MOUNT BEHIND ANTENNA SUCH THAT THE RSH DOES NOT INTERFERE WITH THE EXISTING PLATFORM-ARM MOUNTING HARDWARE.

NOTE: DC CABLES ARE COILED UP ON NY RIGS AT SPUNT ARRAK. THESE ARE TO BE USED TO POWER UP THE 2.5 RIGS AND TIED INTO EXISTING DC BREAKERS ACROSS THE FREER JUNCTION BOX LOCATED AT EQUIPMENT

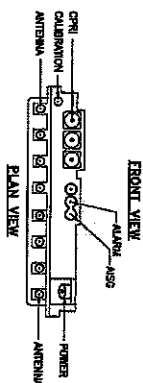
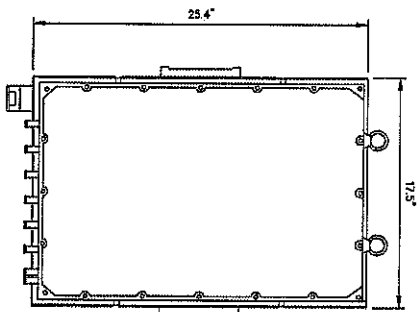
NOTE: THE PROGRAM IS FOR CONCEPTUAL PURPOSES ONLY. CONTRACTOR IS TO REFER TO PASSING STRUCTURAL ANALYSIS FOR ANCHORAGE AND BRIT MOMENTING DESIGN.

RRH: ALCATEL LUCENT 7D-RRH6X20

COLOR: LIGHT GREY
WEIGHT: 70 LBS.



NOTES
COMPLY WITH MANUFACTURER'S INSTRUCTIONS TO ENSURE
THAT ALL RRH'S RECEIVE ELECTRICAL POWER WITHIN 24
HOURS OF ARRIVAL. DO NOT OPEN RRH PACKAGES IN THE RRH
PACKAGING. DO NOT OPEN RRH PACKAGES IN THE RRH



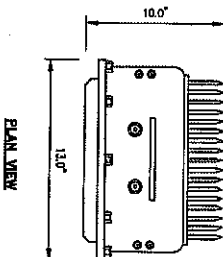
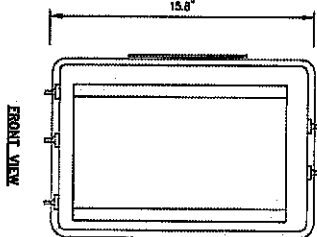
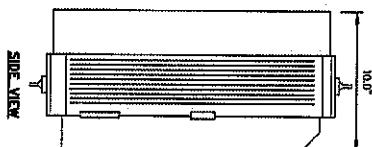
2.5. RRH'S

NO SCALE

1

RRH: ALCATEL LUCENT RRH 800 MHz 2x50W

COLOR: LIGHT GREY
WEIGHT: 53 LBS.



NOTES
COMPLY WITH MANUFACTURER'S INSTRUCTIONS TO ENSURE
THAT ALL RRH'S RECEIVE ELECTRICAL POWER WITHIN 24
HOURS OF ARRIVAL. DO NOT OPEN RRH PACKAGES IN THE RRH
PACKAGING. DO NOT OPEN RRH PACKAGES IN THE RRH

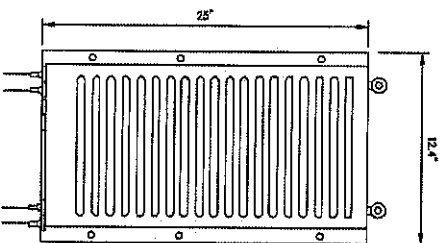
800 MHz RRH

NO SCALE

2

RRH: ALCATEL LUCENT 1900 MHz

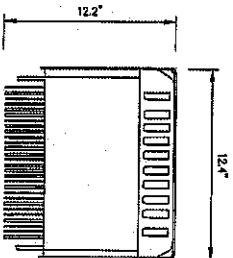
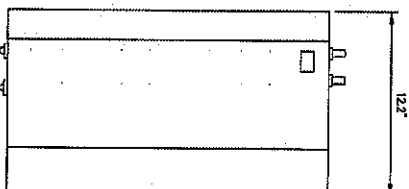
COLOR: LIGHT GREY
WEIGHT: 70 LBS.
(INCLUDING OPTIONAL SOLAR SHIELD)



FRONT VIEW

SIDE VIEW

TOP VIEW



2.5. ANTENNA DETAIL

NO SCALE

3

1900 MHz RRH

NO SCALE

4



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REVISION:	DATE	BY	REV
1.0	6/20/11	LRG	1
2.0	6/20/11	LRG	2
3.0	6/20/11	LRG	3
4.0	6/20/11	LRG	4
5.0	6/20/11	LRG	5
6.0	6/20/11	LRG	6
7.0	6/20/11	LRG	7
8.0	6/20/11	LRG	8
9.0	6/20/11	LRG	9
10.0	6/20/11	LRG	10

SITE NAME:
CLINTON /
ANDERSON PROPERTY

SITE ADDRESS:
CT33XC543

SHEET DESCRIPTION:
EQUIPMENT &
MOUNTING DETAILS

SHEET NUMBER:
A-4

[illegible]

Fiber Only	
High-Tensile Aramid rope, white 90 ft. (109.012-2-M1-501)	5 ft.
2 ft. 3-in. woven aramid fiber plate, (Amibond & LC Composites 5/27 cable)	
10 ft. (109.012-2-M1-500)	10 ft.
15 ft. (109.012-2-M1-501)	15 ft.
20 ft. (109.012-2-M1-502)	20 ft.
25 ft. (109.012-2-M1-503)	25 ft.
30 ft. (109.012-2-M1-504)	30 ft.

A cross-sectional diagram of a three-layer fiber. The outermost layer is labeled "CHAMBER FIBER DIST." and "DWT. 3". The middle layer is labeled "RED" and "BLACK". The innermost layer is labeled "BLACK CORE". The outer layer has a thickness of "0.110". The middle layer has a thickness of "0.034". The innermost layer has a thickness of "0.004". The outer layer is labeled "RED" and "BLACK". The middle layer is labeled "RED" and "BLACK". The innermost layer is labeled "BLACK CORE". The outer layer has a thickness of "0.110". The middle layer has a thickness of "0.034". The innermost layer has a thickness of "0.004".

[illegible]

FIBER ONLY

Diagram illustrating the assembly of a liquidtight straight connector. The assembly consists of a 2" liquidtight straight connector (top) and a 2" metal threaded hub with locknut and neoprene O-ring seal (middle). The hub is secured to a 105°C plastic bushing (bottom) using a 3" max length of liquidtight conduit. The entire assembly is shown connected to an existing junction box.

NO SCALE

Exploded view diagram of a 2004 Honda CRF 250L motorcycle. The diagram shows the following components and their assembly order:

- 1. OIL SINK PROTECTOR
- 2. POWER INJECTOR 1-6
- 3. POWER INJECTOR 1-4
- 4. 250 SIS-M 2
- 5. 250 SIS-M 1
- 6. 2205 SIS-M 6
- 7. 2205 SIS-M 5
- 8. 2205 SIS-M 4
- 9. 2205 SIS-M 3
- 10. 2205 SIS-M 2
- 11. 2205 SIS-M 1
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CIVIL DETAILS



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Overland Park, Kansas 66251

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REVENUE		DATE	BY	REMARKS
DESCRIPTION				
RECEIVED FROM CONSTRUCTION	66/04/74	RS.	2	
WATER REVENUE	66/06/18	SRG	1	
CONSTRUCTION FOR CONSTRUCTION	66/04/28	RS.	0	
RECEIVED FOR REVENUE	67/01/17	RS.	0	

CLINTON /
ANDERSON PROPERTY

CT133XC543

SITE ADDRESS: **7 SHERWOOD FOREST**
KILLINGWORTH, CT 06419

CIVIL DETAILS

[illegible]

code

6560 Spirit Parkway
Overland Park, Kansas 66251

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DESCRIPTION		DATE	BY	REV
ISSUED FOR CONSTRUCTION		07/05/01	KOL	2
AND MODIFICATIONS		04/24/03	SWT	1
ISSUED FOR CONSTRUCTION		07/07/03	KOL	0
ISSUED FOR REVIEW		11/07/02	AW	1
		ETC.	A	

CLINTON /
ANDERSON PROPERTY
SITE CADD: _____
CT33XC543

7 SHERWOOD FOREST
KILLINGWORTH, CT 06419

PLUMBING DIAGRAM

A-6



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Overland Park, Kansas 66251

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REVISION	DATE	BY	REV
1. INITIAL DESIGN	05/20/08	WJS	1
2. REVISED DESIGN	06/20/08	WJS	2
3. REVISED DESIGN	07/20/08	WJS	3
4. REVISED DESIGN	08/20/08	WJS	4
5. REVISED DESIGN	09/20/08	WJS	5

CLINTON/
ANDERSON PROPERTY
SITE ADDRESS:
CT33XC543

7 SHERWOOD FOREST
KILLINGWORTH, CT 06419
ELECTRICAL &
GROUNDING DETAILS

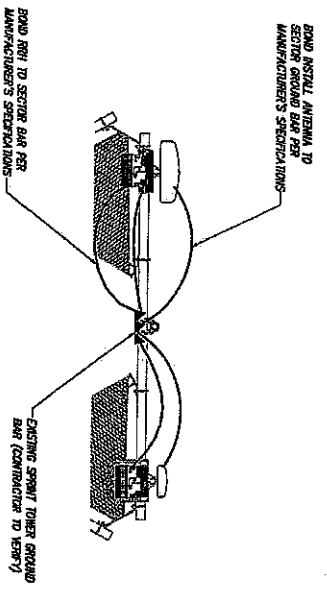
E-1

FINAL EQUIPMENT CONFIGURATION				
SECTION	ANTENNA MANUFACTURER	ANTENNA MODEL	RAD CENTER	BRACKET MAKE AND MODEL
1	RF5	APVTH4AU20	15"	(1) AU20SGR BRACKET 25
	COMSCOPE	MMV4SGM	15"	(1) AU20SGR BRACKET 25
2	RF5	APVTH4AU20	15"	(1) AU20SGR BRACKET 25
	COMSCOPE	MMV4SGM	15"	(1) AU20SGR BRACKET 25
3	RF5	APVTH4AU20	15"	(1) AU20SGR BRACKET 25
	COMSCOPE	MMV4SGM	15"	(1) AU20SGR BRACKET 25
FEEDER CABLES				
MANUFACTURER	MODEL	LENGTH	QTY	
RF5	APVTH4AU20	25'	10	
COMSCOPE	MMV4SGM	25'	10	

NOTES:
1. CONNECTION TO EXISTING PROPOSED ANTENNA STRUCTURE IS THE MOST CURRENT DATA AT TIME OF
2. CONNECTION TO EXISTING PROPOSED ANTENNA STRUCTURE IS THE MOST CURRENT DATA AT TIME OF
3. CONNECTION TO EXISTING PROPOSED ANTENNA STRUCTURE IS THE MOST CURRENT DATA AT TIME OF

ANTENNA/CABLE SCHEDULE

- LEGEND:
- G — EXISTING GROUND ROD
 - COMBIO CONNECTION (ELECTROMAGNETIC WELD)
 - ▲ MECHANICAL CONNECTION
 - GROUND ROD
 - CABLE GROUND KIT



TYPICAL ANTENNA GROUNDING PLAN

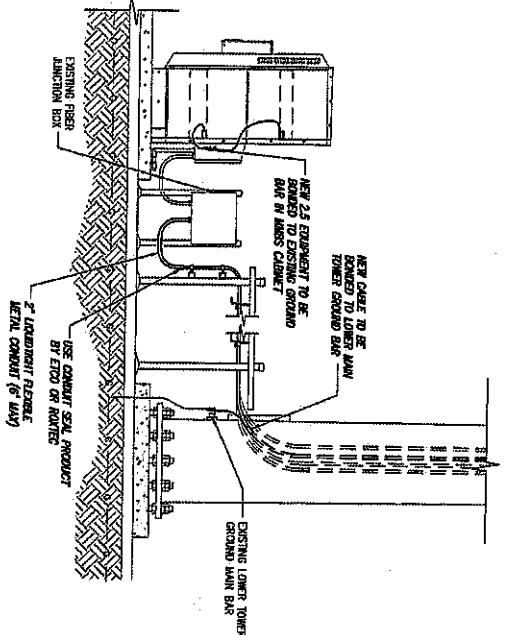
NO SCALE

2

TYPICAL EQUIPMENT GROUNDING PLAN (ELEVATION)

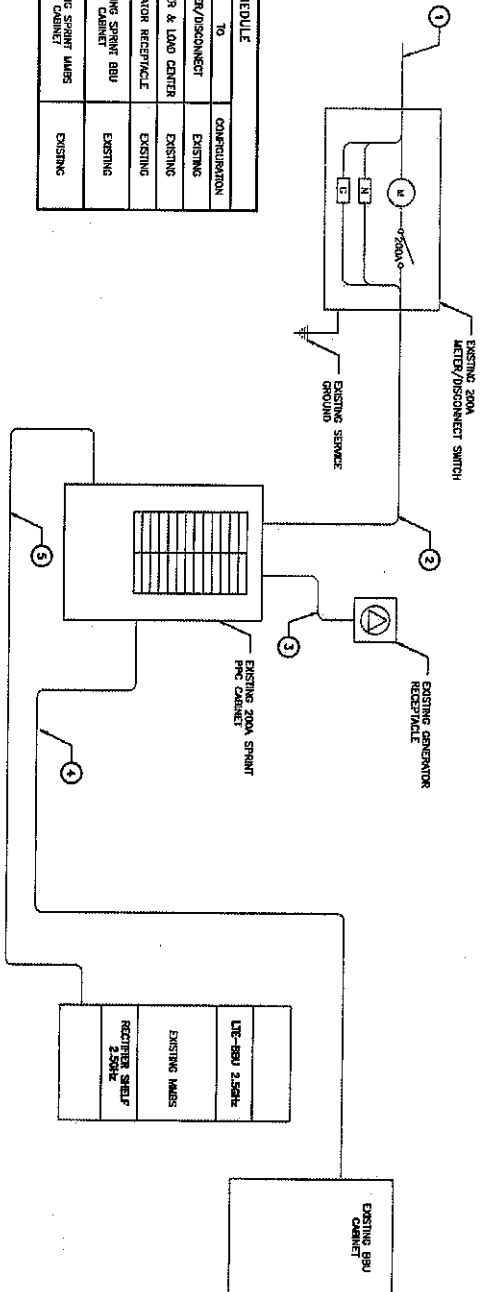
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3



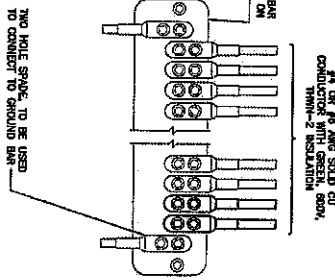
NOTES
 1. SEE REFERENCE ALL SPECS FOR
 2. CONNECTING THE POWER SUPPLY
 3. TO THE CONSTRUCTION EQUIPMENT
 4. FOR ALL CONSTRUCTION EQUIPMENT

CIRCUIT SCHEDULE		
NO	FROM	TO
1	UTILITY SOURCE	METER/DISCONNECT
2	METER/DISCONNECT	TRANSFER & LOAD CENTER
3	TRANSFER & LOAD CENTER	GENERATOR RECEPTACLE
4	TRANSFER & LOAD CENTER	EXISTING SPRING BBU CABINET
5	TRANSFER & LOAD CENTER	EXISTING SPRING LINES

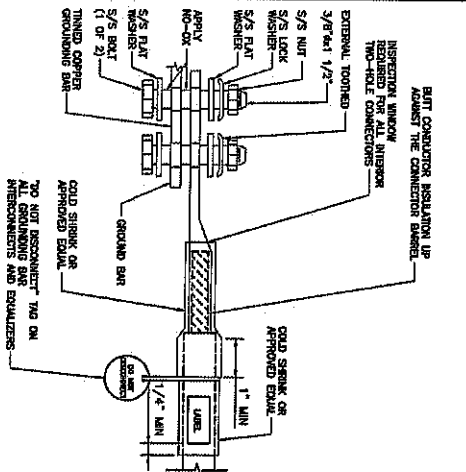


ELECTRICAL ONE-LINE DIAGRAM

NO SCALE 1



NOTES
 1. APPLY NO-ON TO LUGS AND BAR CONTACT SURFACE. DO NOT COAT RISE LUG.
 2. IF STOLEN GROUND BARS ARE ENCOUNTERED, CONTACT SPRING ON FOR REPLACEMENT THREADED ROD KIT.



INSTALLATION OF GROUNDING CONDUCTOR TO GROUNDING BAR

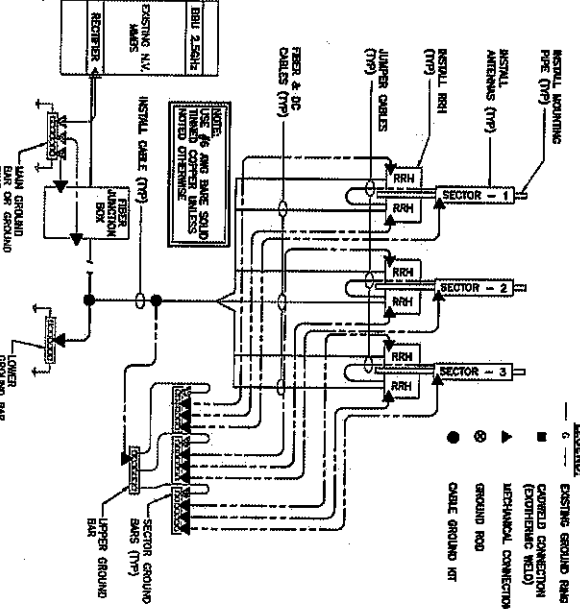
NO SCALE 2

TWO-HOLE LUG

NO SCALE 3

GROUNDING RISER DIAGRAM

NO SCALE 4



LEGEND

- EXISTING GROUND RING
- CABLED CONNECTION (EXHIBITING WELD)
- MECHANICAL CONNECTION
- GROUND ROD
- CABLE GROUND KIT

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REVISION	DATE	BY	REV
1.0	01/01/01	JD	1
2.0	01/01/01	JD	2
3.0	01/01/01	JD	3
4.0	01/01/01	JD	4
5.0	01/01/01	JD	5
6.0	01/01/01	JD	6
7.0	01/01/01	JD	7
8.0	01/01/01	JD	8
9.0	01/01/01	JD	9
10.0	01/01/01	JD	10

CLIENT NAME:
 CLINTON/
 ANDERSON PROPERTY

DATE OF ORDER:
 CT33XC543

DATE OF ORDER:
 7 SHERWOOD FOREST
 KILLINGWORTH, CT 06419

DATE OF ORDER:
 ELECTRICAL &
 GROUNDING DETAILS

DATE OF ORDER:
 E-2

Date: May 30, 2018

Denice Nicholson
Crown Castle
3 Corporate Park Drive Suite 101
Clifton Park, NY 12065



Crown Castle
2000 Corporate Drive
Canonsburg
724-416-2000

Subject: Structural Analysis Report

Carrier Designation: Sprint PCS Co-Locate
Carrier Site Number: CT33XC543
Carrier Site Name: CT33XC543

Crown Castle Designation: Crown Castle BU Number: 876383
Crown Castle Site Name: CLINTON / ANDERSON'S PROPERTY
Crown Castle JDE Job Number: 505983
Crown Castle Work Order Number: 1580562
Crown Castle Order Number: 441479 Rev. 0

Engineering Firm Designation: Crown Castle Project Number: 1580562

Site Data: 7 Sherwood Forest Lane, KILLINGWORTH, Middlesex County, CT
Latitude 41° 20' 17.24", Longitude -72° 33' 23.44"
149.9 Foot - Monopole Tower

Dear Denice Nicholson,

Crown Castle is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 1580562, in accordance with order 441479, revision 0.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment

Sufficient Capacity

Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

This analysis has been performed in accordance with the 2016 Connecticut State Building Code based upon an ultimate 3-second gust wind speed of 130 mph converted to a nominal 3-second gust wind speed of 101 mph per Section 1609.3 and Appendix N as required for use in the TIA-222-G Standard per Exception #5 of Section 1609.1.1. Exposure Category B and Risk Category II were used in this analysis.

All modifications and equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

We at Crown Castle appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Structural analysis prepared by: Benjamin McMurray / KB

Respectfully submitted by:

Terry P. Styran, P.E.
Senior Project Engineer

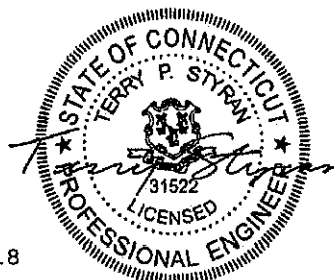


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1) INTRODUCTION

This tower is a 149.9 ft Monopole tower designed by ENGINEERED ENDEAVORS, INC. in April of 2000. The tower was originally designed for a wind speed of 90 mph per TIA/EIA-222-F.

The tower has been modified per reinforcement drawings prepared by Paul J. Ford, in August of 2009. Reinforcement consists of addition of baseplate stiffeners. The modification is effective and was considered in this analysis.

2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA-222-G Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a 3-second gust wind speed of 101 mph with no ice, 50 mph with 0.75 inch ice thickness and 60 mph under service loads, exposure category B.

Table 1 - Proposed Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
150.0	152.0	3	alcatel lucent	PCS 1900MHZ 4X45W-65MHZ	4	1-1/4	-
		6	alcatel lucent	RRH2X50-800			
		3	alcatel lucent	TD-RRH8X20-25			
		3	commscope	NNVV-65B-R4 w/ Mount Pipe			
		3	rfs celwave	APXV/TM14-ALU-I20 w/ Mount Pipe			

Table 2 - Existing and Reserved Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
150.0	152.0	6	decibel	DB980H90A-M w/ Mount Pipe	6	1-5/8	3
	150.0	1	tower mounts	Platform Mount [LP 601-1]	-	-	1
140.0	143.0	1	cci antennas	HPA-65R-BUU-H6 w/ Mount Pipe	-	-	2
		2	cci antennas	HPA-65R-BUU-H8 w/ Mount Pipe			
		3	ericsson	RRUS 11			
		3	ericsson	RRUS 32 B2			
		6	powerwave technologies	7770.00 w/ Mount Pipe	1 2 12 1	3/8 7/16 1-5/8 2" Conduit	1
		6	powerwave technologies	LGP21401			
		4	powerwave technologies	LGP21901			
		1	raycap	DC6-48-60-18-8F			
	140.0	1	tower mounts	Platform Mount [LP 304-1]			

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
128.0	128.0	3	alcatel lucent	RRH2X40-AWS	12	1-5/8	1
		6	andrew	DB846F65ZAXY w/ Mount Pipe			
		3	antel	BXA-70063/6CF w/ Mount Pipe			
		1	tower mounts	T-Arm Mount [TA 602-3]			
		6	kathrein	742 213 w/ Mount Pipe			
		1	rfs celwave	DB-T1-6Z-8AB-0Z			
50.0	51.0	1	lucent	KS24019-L112A	-	-	1
	50.0	1	tower mounts	Side Arm Mount [SO 701-1]			

Notes:

- 1) Existing Equipment
- 2) Reserved Equipment
- 3) Equipment to be removed; not considered in this analysis

Table 3 - Design Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
150.0	150.0	12	dapa	48000	-	-
140.0	140.0	12	dapa	48000	-	-
130.0	130.0	12	dapa	48000	-	-

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Goodkind & O'Dea, Inc.	2122536	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Engineered Endeavors, Inc.	1440547	CCISITES
4-TOWER MANUFACTURER DRAWINGS	Engineered Endeavors, Inc.	1613582	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	Semaan Engineering Solutions, Inc.	1595940	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	Paul J. Ford and Company.	2418226	CCISITES
4-POST-MODIFICATION INSPECTION	Paul J. Ford and Company.	2471721	CCISITES

3.1) Analysis Method

tnxTower (version 7.0.5.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) The existing base plate grout was not considered in this analysis.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	149.854 - 111.934	Pole	TP26.59x18x0.1875	1	-8.83	1026.75	74.1	Pass
L2	111.934 - 76.434	Pole	TP34.14x25.3474x0.3125	2	-14.69	2384.60	63.1	Pass
L3	76.434 - 43.014	Pole	TP40.97x32.4352x0.3125	3	-21.87	2713.66	76.3	Pass
L4	43.014 - 0	Pole	TP50x39.0798x0.375	4	-35.80	4033.20	68.1	Pass
							Summary	
						Pole (L3)	76.3	Pass
						Rating =	76.3	Pass

Table 6 - Tower Component Stresses vs. Capacity – LC7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	74.8	Pass
1	Base Plate	0	73.3	Pass
1	Base Foundation Structure	0	61.2	Pass
1	Base Foundation Soil Interaction	0	63.5	Pass

Structure Rating (max from all components) =	76.3%
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Notes:

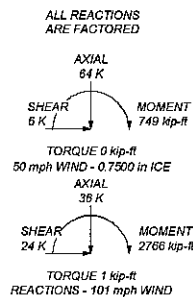
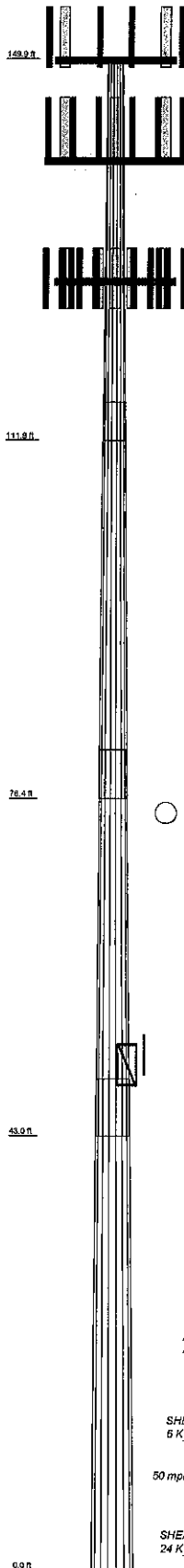
- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

Section	1	2	3	4
Length (ft)	37.02	35.33	36.26	44.88
Number of Sides	18	18	15	18
Thickness (in)	0.1975	0.3125	0.3125	0.3150
Section Length (ft)	3.80	4.83	5.67	38.0198
Top Dia (in)	15.0000	25.3474	32.4051	38.0198
Bot Dia (in)	25.5000	34.1420	40.3700	50.0000
Grade			A572-65	
Weight (lb)	1.7	3.9	4.7	6.7



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
APXVT1M14-ALL-120 w/ Mount Pipe	150	(2) LGP21901	140
APXVT1M14-ALL-120 w/ Mount Pipe	150	(2) LGP21901	140
APXVT1M14-ALL-120 w/ Mount Pipe	150	DCB-48-63-18-8P	140
NOVA45B-R4 w/ Mount Pipe	150	RRUS 11	140
NOVA45B-R4 w/ Mount Pipe	150	RRUS 11	140
NOVA45B-R4 w/ Mount Pipe	150	RRUS 32 B2	140
(2) RRH2X50-400	150	RRUS 32 B2	140
(2) RRH2X50-400	150	RRUS 32 B2	140
TD-RRH2X50-25	150	6 x 2" Mount Pipe	140
TD-RRH2X50-25	150	Platform Mount (LP 304-1)	140
TD-RRH2X50-25	150	(2) DBB48F65ZARY w/ Mount Pipe	128
PCS 1500MH-Z 4X45W-65W-Z	150	(2) DBB48F65ZARY w/ Mount Pipe	128
PCS 1500MH-Z 4X45W-65W-Z	150	(2) DBB48F65ZARY w/ Mount Pipe	128
PCS 1500MH-Z 4X45W-65W-Z	150	(2) 742 213 w/ Mount Pipe	128
6 x 2" Mount Pipe	150	(2) 742 213 w/ Mount Pipe	128
(2) 6 x 2" Mount Pipe	150	(2) 742 213 w/ Mount Pipe	128
6 x 2" Mount Pipe	150	BXA-700B3GCF w/ Mount Pipe	128
Transition Ladder	150	BXA-700B3GCF w/ Mount Pipe	128
Platform Mount (LP 801-1)	150	BXA-700B3GCF w/ Mount Pipe	128
(2) 7770.00 w/ Mount Pipe	140	RRH2X40-AWS	128
(2) 7770.00 w/ Mount Pipe	140	RRH2X40-AWS	128
(2) 7770.00 w/ Mount Pipe	140	RRH2X40-AWS	128
HPA-65B-BULH48 w/ Mount Pipe	140	DS-1 L-22-BAB-2	128
HPA-65B-BULH48 w/ Mount Pipe	140	T-Arm Mount (TA 912-3)	128
HPA-65B-BULH48 w/ Mount Pipe	140	K24-019-L11A	50
(2) LGP21401	140	Side Arm Mount (SO 701-1)	50
(2) LGP21401	140	2 x 2" Pipe Mount	50
(2) LGP21401	140		

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower is located in Middlesex County, Connecticut.
2. Tower designed for Exposure B to the TIA-222-G Standard.
3. Tower designed for a 101 mph basic wind in accordance with the TIA-222-G Standard.
4. Tower is also designed for a 50 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 80 mph wind.
6. Tower Structure Class II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING: 76.3%

Crown Castle		Sub: BU# 876383	
2000 Corporate Drive Canonsburg Phone: 724-416-2000 FAX:		Project: Crown Castle	App'd: BMcMurray
The Pathway to Possible		Code: TIA-222-G	Date: 05/30/18
		Scale: NTS	Dwg No: E-1

Tower Input Data

There is a pole section.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

- 1) Tower is located in Middlesex County, Connecticut.
- 2) Basic wind speed of 101 mph.
- 3) Structure Class II.
- 4) Exposure Category B.
- 5) Topographic Category 1.
- 6) Crest Height 0.00 ft.
- 7) Nominal ice thickness of 0.7500 in.
- 8) Ice thickness is considered to increase with height.
- 9) Ice density of 56 pcf.
- 10) A wind speed of 50 mph is used in combination with ice.
- 11) Temperature drop of 50 °F.
- 12) Deflections calculated using a wind speed of 60 mph.
- 13) A non-linear (P-delta) analysis was used.
- 14) Pressures are calculated at each section.
- 15) Stress ratio used in pole design is 1.
- 16) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Horizontals	Assume Legs Pinned	Calculate Redundant Bracing Forces
Consider Moments - Diagonals	✓ Assume Rigid Index Plate	Ignore Redundant Members in FEA
Use Moment Magnification	✓ Use Clear Spans For Wind Area	SR Leg Bolts Resist Compression
✓ Use Code Stress Ratios	Use Clear Spans For KL/r	All Leg Panels Have Same Allowable
✓ Use Code Safety Factors - Guys	Retension Guys To Initial Tension	Offset Girt At Foundation
Escalate Ice	✓ Bypass Mast Stability Checks	✓ Consider Feed Line Torque
Always Use Max Kz	✓ Use Azimuth Dish Coefficients	Include Angle Block Shear Check
Use Special Wind Profile	✓ Project Wind Area of Appurt.	Use TIA-222-G Bracing Resist.
Include Bolts In Member Capacity	Autocalc Torque Arm Areas	Exemption
Leg Bolts Are At Top Of Section	Add IBC .6D+W Combination	Use TIA-222-G Tension Splice
Secondary Horizontal Braces Leg	✓ Sort Capacity Reports By Component	Exemption
Use Diamond Inner Bracing (4 Sided)	Triangulate Diamond Inner Bracing	Poles
SR Members Have Cut Ends	Treat Feed Line Bundles As Cylinder	✓ Include Shear-Torsion Interaction
SR Members Are Concentric		Always Use Sub-Critical Flow
		Use Top Mounted Sockets

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	149.85-111.93	37.92	3.83	18	18.0000	26.5900	0.1875	0.7500	A572-65 (65 ksi)
L2	111.93-76.43	39.33	4.83	18	25.3474	34.1400	0.3125	1.2500	A572-65 (65 ksi)
L3	76.43-43.01	38.25	5.67	18	32.4352	40.9700	0.3125	1.2500	A572-65 (65 ksi)
L4	43.01-0.00	48.68		18	39.0798	50.0000	0.3750	1.5000	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	It/Q in ²	w in	w/t
L1	18.2777	10.6007	424.9328	6.3234	9.1440	46.4712	850.4248	5.3013	2.8380	15.136
	27.0002	15.7128	1383.8238	9.3729	13.5077	102.4469	2769.4685	7.8579	4.3498	23.199
L2	26.6079	24.8315	1966.2175	8.8874	12.8765	152.6984	3935.0222	12.4181	3.9111	12.516
	34.6667	33.5527	4850.6965	12.0088	17.3431	279.6900	9707.7757	16.7795	5.4586	17.468
L3	34.0299	31.8617	4153.6583	11.4036	16.4771	252.0870	8312.7820	15.9339	5.1586	16.508
	41.6020	40.3272	8422.0227	14.4334	20.8128	404.6567	16855.127	20.1674	6.6607	21.314
							3			
L4	40.9742	46.0684	8719.0978	13.7402	19.8526	439.1926	17449.668	23.0386	6.2181	16.581
							3			
	50.7713	59.0662	18377.109	17.6169	25.4000	723.5082	36778.399	29.5387	8.1400	21.707
			4				8			

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A _r	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft ²	in					in	in	in
L1 149.85- 111.93				1	1	1			
L2 111.93- 76.43				1	1	1			
L3 76.43- 43.01				1	1	1			
L4 43.01-0.00				1	1	1			

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Sector	Component Type	Placement ft	Total Number	Number Per Row	Start/End Position	Width or Diameter r in	Perimeter r in	Weight plf
AVA7-50(1-5/8)	C	Surface Ar (CaAa)	128.00 - 0.00	7	6	-0.500 -0.350	2.0100		0.70

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	C _A A _A ft ² /ft	Weight plf
HB114-13U3M12- XXXF(1-1/4)	C	No	Inside Pole	149.85 - 0.00	2	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00
HB114-13U3M12- XXXF(1-1/4)	C	No	Inside Pole	149.85 - 0.00	2	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00

LDF7-50A(1-5/8)	A	No	Inside Pole	140.00 - 0.00	10	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00
LCF158-50A(1-5/8)	A	No	Inside Pole	140.00 - 0.00	2	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00
FB-L98B-002- 75000(3/8)	A	No	Inside Pole	140.00 - 0.00	1	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number		C _A A _A ft ² /ft	Weight plf
WR-VG122ST- BRDA(7/16)	A	No	Inside Pole	140.00 - 0.00	2	No Ice	0.00	0.14
						1/2" Ice	0.00	0.14
						1" Ice	0.00	0.14
2" Rigid Conduit	A	No	Inside Pole	140.00 - 0.00	1	No Ice	0.00	2.80
						1/2" Ice	0.00	2.80
						1" Ice	0.00	2.80

AVA7-50(1-5/8)	C	No	Inside Pole	128.00 - 0.00	12	No Ice	0.00	0.70
						1/2" Ice	0.00	0.70
						1" Ice	0.00	0.70
**								

Feed Line/Linear Appurtenances Section Areas

Tower Section n	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L1	149.85-111.93	A	0.000	0.000	0.000	0.000	0.36
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	19.376	0.000	0.36
L2	111.93-76.43	A	0.000	0.000	0.000	0.000	0.46
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	42.813	0.000	0.61
L3	76.43-43.01	A	0.000	0.000	0.000	0.000	0.43
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	40.305	0.000	0.58
L4	43.01-0.00	A	0.000	0.000	0.000	0.000	0.56
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	51.875	0.000	0.74

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section n	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L1	149.85-111.93	A	1.720	0.000	0.000	0.000	0.000	0.36
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	31.129	0.000	0.77
L2	111.93-76.43	A	1.665	0.000	0.000	0.000	0.000	0.46
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	68.783	0.000	1.51
L3	76.43-43.01	A	1.591	0.000	0.000	0.000	0.000	0.43
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	64.291	0.000	1.40
L4	43.01-0.00	A	1.433	0.000	0.000	0.000	0.000	0.56
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	81.953	0.000	1.75

Feed Line Center of Pressure

Section	Elevation ft	CP _X in	CP _Z in	CP _X Ice in	CP _Z Ice in
L1	149.85-111.93	0.5960	0.4827	0.6994	0.5663
L2	111.93-76.43	1.0656	0.8629	1.1790	0.9547
L3	76.43-43.01	1.1094	0.8984	1.2809	1.0372
L4	43.01-0.00	1.1461	0.9281	1.3708	1.1101

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L1	13	AVA7-50(1-5/8)	111.93 - 128.00	1.0000	1.0000
L2	13	AVA7-50(1-5/8)	76.43 - 111.93	1.0000	1.0000
L3	13	AVA7-50(1-5/8)	43.01 - 76.43	1.0000	1.0000

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment t °	Placement ft		C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K

APXVTM14-ALU-I20 w/ Mount Pipe	A	From Leg	4.00	0.0000	150.00	No Ice	6.58	4.96	0.08
			0.00			1/2"	7.03	5.75	0.13
			2.00			Ice	7.47	6.47	0.19
APXVTM14-ALU-I20 w/ Mount Pipe	B	From Leg	4.00	0.0000	150.00	1" Ice			
			0.00			No Ice	6.58	4.96	0.08
			2.00			1/2"	7.03	5.75	0.13
APXVTM14-ALU-I20 w/ Mount Pipe	C	From Leg	4.00	0.0000	150.00	Ice	7.47	6.47	0.19
			0.00			1" Ice			
			2.00			No Ice	6.58	4.96	0.08
NNVV-65B-R4 w/ Mount Pipe	A	From Leg	4.00	0.0000	150.00	1/2"	7.03	5.75	0.13
			0.00			Ice	7.47	6.47	0.19
			2.00			1" Ice			
NNVV-65B-R4 w/ Mount Pipe	B	From Leg	4.00	0.0000	150.00	No Ice	12.51	7.41	0.10
			0.00			1/2"	13.11	8.60	0.19
			2.00			Ice	13.67	9.50	0.29
NNVV-65B-R4 w/ Mount Pipe	C	From Leg	4.00	0.0000	150.00	1" Ice			
			0.00			No Ice	12.51	7.41	0.10
			2.00			1/2"	13.11	8.60	0.19
(2) RRH2X50-800	A	From Leg	4.00	0.0000	150.00	Ice	13.67	9.50	0.29
			0.00			1" Ice			
			2.00			No Ice	1.70	1.28	0.05
(2) RRH2X50-800	B	From Leg	4.00	0.0000	150.00	1/2"	1.86	1.43	0.07
			0.00			Ice	2.03	1.58	0.09
			2.00			1" Ice			
(2) RRH2X50-800	C	From Leg	4.00	0.0000	150.00	No Ice	1.70	1.28	0.05
			0.00			1/2"	1.86	1.43	0.07
			2.00			Ice	2.03	1.58	0.09
						1" Ice			

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustmen t °	Placement ft		C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K
TD-RRH8X20-25	A	From Leg	4.00 0.00 2.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	4.05 4.30 4.56	1.53 1.71 1.90	0.07 0.10 0.13
TD-RRH8X20-25	B	From Leg	4.00 0.00 2.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	4.05 4.30 4.56	1.53 1.71 1.90	0.07 0.10 0.13
TD-RRH8X20-25	C	From Leg	4.00 0.00 2.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	4.05 4.30 4.56	1.53 1.71 1.90	0.07 0.10 0.13
PCS 1900MHZ 4X45W- 65MHZ	A	From Leg	4.00 0.00 2.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	2.32 2.53 2.74	2.24 2.44 2.65	0.06 0.08 0.11
PCS 1900MHZ 4X45W- 65MHZ	B	From Leg	4.00 0.00 2.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	2.32 2.53 2.74	2.24 2.44 2.65	0.06 0.08 0.11
PCS 1900MHZ 4X45W- 65MHZ	C	From Leg	4.00 0.00 2.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	2.32 2.53 2.74	2.24 2.44 2.65	0.06 0.08 0.11
6' x 2" Mount Pipe	A	From Face	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	1.43 1.92 2.29	1.43 1.92 2.29	0.02 0.03 0.05
(2) 6' x 2" Mount Pipe	B	From Face	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	1.43 1.92 2.29	1.43 1.92 2.29	0.02 0.03 0.05
6' x 2" Mount Pipe	C	From Face	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	1.43 1.92 2.29	1.43 1.92 2.29	0.02 0.03 0.05
Transition Ladder	C	None		0.0000	150.00	No Ice 1/2" Ice 1" Ice	6.00 8.00 10.00	6.00 8.00 10.00	0.16 0.24 0.32
Platform Mount [LP 601-1]	C	None		0.0000	150.00	No Ice 1/2" Ice 1" Ice	28.47 33.59 38.71	28.47 33.59 38.71	1.12 1.51 1.91
**** *****									
(2) 7770.00 w/ Mount Pipe	A	From Leg	4.00 0.00 3.00	0.0000	140.00	No Ice 1/2" Ice 1" Ice	5.75 6.18 6.61	4.25 5.01 5.71	0.06 0.10 0.16
(2) 7770.00 w/ Mount Pipe	B	From Leg	4.00 0.00 3.00	0.0000	140.00	No Ice 1/2" Ice 1" Ice	5.75 6.18 6.61	4.25 5.01 5.71	0.06 0.10 0.16
(2) 7770.00 w/ Mount Pipe	C	From Leg	4.00 0.00 3.00	0.0000	140.00	No Ice 1/2" Ice 1" Ice	5.75 6.18 6.61	4.25 5.01 5.71	0.06 0.10 0.16
HPA-65R-BUU-H6 w/ Mount Pipe	A	From Leg	4.00 0.00 3.00	0.0000	140.00	No Ice 1/2" Ice 1" Ice	9.90 10.47 11.01	8.11 9.30 10.21	0.08 0.16 0.25
HPA-65R-BUU-H8 w/ Mount Pipe	C	From Leg	4.00 0.00 3.00	0.0000	140.00	No Ice 1/2" Ice	13.21 13.90 14.59	9.58 11.05 12.50	0.10 0.20 0.30

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustmen t °	Placement ft		C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K
HPA-65R-BUU-H8 w/ Mount Pipe	B	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	13.21 13.90 14.59	9.58 11.05 12.50	0.10 0.20 0.30
(2) LGP21401	A	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	1.10 1.24 1.38	0.21 0.27 0.35	0.01 0.02 0.03
(2) LGP21401	B	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	1.10 1.24 1.38	0.21 0.27 0.35	0.01 0.02 0.03
(2) LGP21401	C	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	1.10 1.24 1.38	0.21 0.27 0.35	0.01 0.02 0.03
(2) LGP21901	A	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	0.23 0.29 0.36	0.16 0.21 0.28	0.01 0.01 0.01
(2) LGP21901	B	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	0.23 0.29 0.36	0.16 0.21 0.28	0.01 0.01 0.01
DC6-48-60-18-8F	B	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	0.79 1.27 1.45	0.79 1.27 1.45	0.02 0.04 0.05
RRUS 11	A	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	2.78 2.99 3.21	1.19 1.33 1.49	0.05 0.07 0.09
RRUS 11	B	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	2.78 2.99 3.21	1.19 1.33 1.49	0.05 0.07 0.09
RRUS 11	C	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	2.78 2.99 3.21	1.19 1.33 1.49	0.05 0.07 0.09
RRUS 32 B2	A	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	2.73 2.95 3.18	1.67 1.86 2.05	0.05 0.07 0.10
RRUS 32 B2	B	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	2.73 2.95 3.18	1.67 1.86 2.05	0.05 0.07 0.10
RRUS 32 B2	C	From Leg	4.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	2.73 2.95 3.18	1.67 1.86 2.05	0.05 0.07 0.10
6' x 2" Mount Pipe	B	From Leg	1.00 0.00 3.00	0.0000	140.00	1" Ice No Ice 1/2" Ice	1.43 1.92 2.29	1.43 1.92 2.29	0.02 0.03 0.05
Platform Mount [LP 304-1]	C	None		0.0000	140.00	1" Ice No Ice 1/2" Ice	17.46 22.44 27.42	17.46 22.44 27.42	1.35 1.62 1.90
***						1" Ice			
(2) DB846F65ZAXY w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	128.00	No Ice 1/2" Ice	7.27 7.83 8.35	7.82 9.01 9.91	0.05 0.11 0.19

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustmen t °	Placement ft		C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K
(2) DB846F65ZAXY w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	128.00	1" Ice No Ice 1/2" Ice	7.27 7.83 8.35	7.82 9.01 9.91	0.05 0.11 0.19
(2) DB846F65ZAXY w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	128.00	1" Ice No Ice 1/2" Ice	7.27 7.83 8.35	7.82 9.01 9.91	0.05 0.11 0.19
(2) 742 213 w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	128.00	1" Ice No Ice 1/2" Ice	5.37 5.95 6.50	4.62 6.00 6.98	0.05 0.09 0.15
(2) 742 213 w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	128.00	1" Ice No Ice 1/2" Ice	5.37 5.95 6.50	4.62 6.00 6.98	0.05 0.09 0.15
(2) 742 213 w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	128.00	1" Ice No Ice 1/2" Ice	5.37 5.95 6.50	4.62 6.00 6.98	0.05 0.09 0.15
BXA-70063/6CF w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	128.00	1" Ice No Ice 1/2" Ice	7.82 8.37 8.89	5.70 6.85 7.71	0.04 0.10 0.17
BXA-70063/6CF w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	128.00	1" Ice No Ice 1/2" Ice	7.82 8.37 8.89	5.70 6.85 7.71	0.04 0.10 0.17
BXA-70063/6CF w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	128.00	1" Ice No Ice 1/2" Ice	7.82 8.37 8.89	5.70 6.85 7.71	0.04 0.10 0.17
RRH2X40-AWS	A	From Leg	4.00 0.00 0.00	0.0000	128.00	1" Ice No Ice 1/2" Ice	2.16 2.36 2.57	1.42 1.59 1.77	0.04 0.06 0.08
RRH2X40-AWS	B	From Leg	4.00 0.00 0.00	0.0000	128.00	1" Ice No Ice 1/2" Ice	2.16 2.36 2.57	1.42 1.59 1.77	0.04 0.06 0.08
RRH2X40-AWS	C	From Leg	4.00 0.00 0.00	0.0000	128.00	1" Ice No Ice 1/2" Ice	2.16 2.36 2.57	1.42 1.59 1.77	0.04 0.06 0.08
DB-T1-6Z-8AB-0Z	A	From Leg	4.00 0.00 0.00	0.0000	128.00	1" Ice No Ice 1/2" Ice	4.80 5.07 5.35	2.00 2.19 2.39	0.04 0.08 0.12
T-Arm Mount [TA 602-3]	C	None		0.0000	128.00	1" Ice No Ice 1/2" Ice	11.59 15.44 19.29	11.59 15.44 19.29	0.77 0.99 1.21
*** KS24019-L112A	B	From Leg	2.00 0.00 1.00	0.0000	50.00	1" Ice No Ice 1/2" Ice	0.10 0.18 0.26	0.10 0.18 0.26	0.01 0.01 0.01
Side Arm Mount [SO 701- 1]	B	From Leg	0.00 0.00 0.00	0.0000	50.00	1" Ice No Ice 1/2" Ice	0.85 1.14 1.43	1.67 2.34 3.01	0.07 0.08 0.09
2' x 2" Pipe Mount	B	From Leg	2.00 0.00 0.00	0.0000	50.00	1" Ice No Ice 1/2" Ice	0.02 0.05 0.09	0.02 0.05 0.09	0.01 0.01 0.01

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustmen t °	Placement ft	C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K

1" Ice								

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 30 deg - No Ice
5	0.9 Dead+1.6 Wind 30 deg - No Ice
6	1.2 Dead+1.6 Wind 60 deg - No Ice
7	0.9 Dead+1.6 Wind 60 deg - No Ice
8	1.2 Dead+1.6 Wind 90 deg - No Ice
9	0.9 Dead+1.6 Wind 90 deg - No Ice
10	1.2 Dead+1.6 Wind 120 deg - No Ice
11	0.9 Dead+1.6 Wind 120 deg - No Ice
12	1.2 Dead+1.6 Wind 150 deg - No Ice
13	0.9 Dead+1.6 Wind 150 deg - No Ice
14	1.2 Dead+1.6 Wind 180 deg - No Ice
15	0.9 Dead+1.6 Wind 180 deg - No Ice
16	1.2 Dead+1.6 Wind 210 deg - No Ice
17	0.9 Dead+1.6 Wind 210 deg - No Ice
18	1.2 Dead+1.6 Wind 240 deg - No Ice
19	0.9 Dead+1.6 Wind 240 deg - No Ice
20	1.2 Dead+1.6 Wind 270 deg - No Ice
21	0.9 Dead+1.6 Wind 270 deg - No Ice
22	1.2 Dead+1.6 Wind 300 deg - No Ice
23	0.9 Dead+1.6 Wind 300 deg - No Ice
24	1.2 Dead+1.6 Wind 330 deg - No Ice
25	0.9 Dead+1.6 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	149.854 - 111.934	Pole	Max Tension	26	0.00	0.00	0.00
			Max. Compression	26	-26.12	-1.69	0.14
			Max. Mx	8	-8.83	-394.49	-0.02
			Max. My	2	-8.83	-0.29	393.61
			Max. Vy	8	16.28	-394.49	-0.02
			Max. Vx	14	16.32	-0.43	-393.48
L2	111.934 - 76.434	Pole	Max. Torque	17			0.55
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-34.87	-3.08	-0.86
			Max. Mx	8	-14.72	-998.43	-0.27
			Max. My	14	-14.71	-0.77	-998.63
			Max. Vy	8	18.71	-998.43	-0.27
L3	76.434 - 43.014	Pole	Max. Vx	14	18.75	-0.77	-998.63
			Max. Torque	24			-0.53
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-45.17	-4.83	-2.15
			Max. Mx	8	-21.89	-1645.76	-0.64
			Max. My	14	-21.89	-1.26	-1647.07
L4	43.014 - 0	Pole	Max. Vy	8	20.96	-1645.76	-0.64
			Max. Vx	14	21.01	-1.26	-1647.07
			Max. Torque	2			-0.61
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-63.90	-7.10	-3.98
			Max. Mx	8	-35.80	-2738.83	-0.51
			Max. My	14	-35.80	-1.23	-2742.54
			Max. Vy	8	23.86	-2738.83	-0.51
			Max. Vx	14	23.91	-1.23	-2742.54
			Max. Torque	2			-0.61

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	30	63.90	-6.33	0.00
	Max. H _x	21	26.87	23.83	-0.01
	Max. H _z	3	26.87	-0.01	23.88
	Max. M _x	2	2740.97	-0.01	23.88
	Max. M _z	8	2738.83	-23.83	0.01
	Max. Torsion	14	0.61	0.01	-23.88
	Min. Vert	11	26.87	-20.63	-11.93
	Min. H _x	9	26.87	-23.83	0.01
	Min. H _z	15	26.87	0.01	-23.88
	Min. M _x	14	-2742.54	0.01	-23.88
	Min. M _z	20	-2735.83	23.83	-0.01
	Min. Torsion	2	-0.61	-0.01	23.88

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overtuning Moment, M _x kip-ft	Overtuning Moment, M _z kip-ft	Torque kip-ft
Dead Only	29.85	0.00	0.00	0.63	-1.20	-0.00

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturing Moment, M _x kip-ft	Overturing Moment, M _z kip-ft	Torque kip-ft
1.2 Dead+1.6 Wind 0 deg - No Ice	35.82	0.01	-23.88	-2740.97	-1.78	0.61
0.9 Dead+1.6 Wind 0 deg - No Ice	26.87	0.01	-23.88	-2706.61	-1.39	0.61
1.2 Dead+1.6 Wind 30 deg - No Ice	35.82	12.06	-20.93	-2395.85	-1383.14	0.47
0.9 Dead+1.6 Wind 30 deg - No Ice	26.87	12.06	-20.93	-2365.93	-1365.38	0.47
1.2 Dead+1.6 Wind 60 deg - No Ice	35.82	20.74	-12.01	-1375.90	-2381.88	0.20
0.9 Dead+1.6 Wind 60 deg - No Ice	26.87	20.74	-12.01	-1358.77	-2351.50	0.21
1.2 Dead+1.6 Wind 90 deg - No Ice	35.82	23.83	-0.01	0.51	-2738.83	-0.11
0.9 Dead+1.6 Wind 90 deg - No Ice	26.87	23.83	-0.01	0.31	-2703.91	-0.11
1.2 Dead+1.6 Wind 120 deg - No Ice	35.82	20.63	11.93	1371.43	-2371.96	-0.39
0.9 Dead+1.6 Wind 120 deg - No Ice	26.87	20.63	11.93	1353.94	-2341.67	-0.39
1.2 Dead+1.6 Wind 150 deg - No Ice	35.82	11.91	20.68	2375.08	-1369.93	-0.58
0.9 Dead+1.6 Wind 150 deg - No Ice	26.87	11.91	20.68	2344.94	-1352.27	-0.57
1.2 Dead+1.6 Wind 180 deg - No Ice	35.82	-0.01	23.88	2742.54	-1.23	-0.61
0.9 Dead+1.6 Wind 180 deg - No Ice	26.87	-0.01	23.88	2707.77	-0.84	-0.61
1.2 Dead+1.6 Wind 210 deg - No Ice	35.82	-12.06	20.93	2397.42	1380.13	-0.47
0.9 Dead+1.6 Wind 210 deg - No Ice	26.87	-12.06	20.93	2367.09	1363.15	-0.48
1.2 Dead+1.6 Wind 240 deg - No Ice	35.82	-20.74	12.01	1377.47	2378.87	-0.21
0.9 Dead+1.6 Wind 240 deg - No Ice	26.87	-20.74	12.01	1359.94	2349.27	-0.22
1.2 Dead+1.6 Wind 270 deg - No Ice	35.82	-23.83	0.01	1.06	2735.83	0.11
0.9 Dead+1.6 Wind 270 deg - No Ice	26.87	-23.83	0.01	0.86	2701.68	0.10
1.2 Dead+1.6 Wind 300 deg - No Ice	35.82	-20.63	-11.93	-1369.86	2368.96	0.40
0.9 Dead+1.6 Wind 300 deg - No Ice	26.87	-20.63	-11.93	-1352.78	2339.44	0.40
1.2 Dead+1.6 Wind 330 deg - No Ice	35.82	-11.91	-20.68	-2373.51	1366.92	0.59
0.9 Dead+1.6 Wind 330 deg - No Ice	26.87	-11.91	-20.68	-2343.78	1350.05	0.58
1.2 Dead+1.0 Ice+1.0 Temp	63.90	0.00	0.00	3.98	-7.10	-0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	63.90	0.00	-6.34	-738.63	-7.36	0.20
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	63.90	3.17	-5.50	-639.21	-377.76	0.17
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	63.90	5.48	-3.18	-367.44	-648.87	0.09
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	63.90	6.33	-0.00	3.87	-748.04	-0.02
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	63.90	5.48	3.17	375.22	-648.71	-0.12
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	63.90	3.16	5.49	647.11	-377.48	-0.19
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	63.90	-0.00	6.34	746.69	-7.03	-0.21
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	63.90	-3.17	5.50	647.27	363.37	-0.17
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	63.90	-5.48	3.18	375.50	634.48	-0.09
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	63.90	-6.33	0.00	4.19	733.65	0.02

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	63.90	-5.48	-3.17	-367.16	634.32	0.12
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	63.90	-3.16	-5.49	-639.05	363.09	0.19
Dead+Wind 0 deg - Service	29.85	0.00	-4.71	-537.18	-1.31	0.12
Dead+Wind 30 deg - Service	29.85	2.38	-4.13	-469.49	-272.29	0.09
Dead+Wind 60 deg - Service	29.85	4.09	-2.37	-269.41	-468.20	0.04
Dead+Wind 90 deg - Service	29.85	4.70	-0.00	0.60	-538.21	-0.02
Dead+Wind 120 deg - Service	29.85	4.07	2.35	269.52	-466.24	-0.08
Dead+Wind 150 deg - Service	29.85	2.35	4.08	466.40	-269.68	-0.12
Dead+Wind 180 deg - Service	29.85	-0.00	4.71	538.48	-1.20	-0.12
Dead+Wind 210 deg - Service	29.85	-2.38	4.13	470.80	269.78	-0.10
Dead+Wind 240 deg - Service	29.85	-4.09	2.37	270.71	465.69	-0.04
Dead+Wind 270 deg - Service	29.85	-4.70	0.00	0.70	535.70	0.02
Dead+Wind 300 deg - Service	29.85	-4.07	-2.35	-268.22	463.74	0.08
Dead+Wind 330 deg - Service	29.85	-2.35	-4.08	-465.09	267.18	0.12

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-29.85	0.00	0.00	29.85	0.00	0.000%
2	0.01	-35.82	-23.88	-0.01	35.82	23.88	0.000%
3	0.01	-26.87	-23.88	-0.01	26.87	23.88	0.000%
4	12.06	-35.82	-20.93	-12.06	35.82	20.93	0.000%
5	12.06	-26.87	-20.93	-12.06	26.87	20.93	0.000%
6	20.74	-35.82	-12.01	-20.74	35.82	12.01	0.000%
7	20.74	-26.87	-12.01	-20.74	26.87	12.01	0.000%
8	23.83	-35.82	-0.01	-23.83	35.82	0.01	0.000%
9	23.83	-26.87	-0.01	-23.83	26.87	0.01	0.000%
10	20.63	-35.82	11.93	-20.63	35.82	-11.93	0.000%
11	20.63	-26.87	11.93	-20.63	26.87	-11.93	0.000%
12	11.91	-35.82	20.68	-11.91	35.82	-20.68	0.000%
13	11.91	-26.87	20.68	-11.91	26.87	-20.68	0.000%
14	-0.01	-35.82	23.88	0.01	35.82	-23.88	0.000%
15	-0.01	-26.87	23.88	0.01	26.87	-23.88	0.000%
16	-12.06	-35.82	20.93	12.06	35.82	-20.93	0.000%
17	-12.06	-26.87	20.93	12.06	26.87	-20.93	0.000%
18	-20.74	-35.82	12.01	20.74	35.82	-12.01	0.000%
19	-20.74	-26.87	12.01	20.74	26.87	-12.01	0.000%
20	-23.83	-35.82	0.01	23.83	35.82	-0.01	0.000%
21	-23.83	-26.87	0.01	23.83	26.87	-0.01	0.000%
22	-20.63	-35.82	-11.93	20.63	35.82	11.93	0.000%
23	-20.63	-26.87	-11.93	20.63	26.87	11.93	0.000%
24	-11.91	-35.82	-20.68	11.91	35.82	20.68	0.000%
25	-11.91	-26.87	-20.68	11.91	26.87	20.68	0.000%
26	0.00	-63.90	0.00	-0.00	63.90	-0.00	0.000%
27	0.00	-63.90	-6.34	-0.00	63.90	6.34	0.000%
28	3.17	-63.90	-5.50	-3.17	63.90	5.50	0.000%
29	5.48	-63.90	-3.18	-5.48	63.90	3.18	0.000%
30	6.33	-63.90	-0.00	-6.33	63.90	0.00	0.000%
31	5.48	-63.90	3.17	-5.48	63.90	-3.17	0.000%
32	3.16	-63.90	5.49	-3.16	63.90	-5.49	0.000%
33	-0.00	-63.90	6.34	0.00	63.90	-6.34	0.000%
34	-3.17	-63.90	5.50	3.17	63.90	-5.50	0.000%
35	-5.48	-63.90	3.18	5.48	63.90	-3.18	0.000%
36	-6.33	-63.90	0.00	6.33	63.90	-0.00	0.000%

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
37	-5.48	-63.90	-3.17	5.48	63.90	3.17	0.000%
38	-3.16	-63.90	-5.49	3.16	63.90	5.49	0.000%
39	0.00	-29.85	-4.71	-0.00	29.85	4.71	0.000%
40	2.38	-29.85	-4.13	-2.38	29.85	4.13	0.000%
41	4.09	-29.85	-2.37	-4.09	29.85	2.37	0.000%
42	4.70	-29.85	-0.00	-4.70	29.85	0.00	0.000%
43	4.07	-29.85	2.35	-4.07	29.85	-2.35	0.000%
44	2.35	-29.85	4.08	-2.35	29.85	-4.08	0.000%
45	-0.00	-29.85	4.71	0.00	29.85	-4.71	0.000%
46	-2.38	-29.85	4.13	2.38	29.85	-4.13	0.000%
47	-4.09	-29.85	2.37	4.09	29.85	-2.37	0.000%
48	-4.70	-29.85	0.00	4.70	29.85	-0.00	0.000%
49	-4.07	-29.85	-2.35	4.07	29.85	2.35	0.000%
50	-2.35	-29.85	-4.08	2.35	29.85	4.08	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	5	0.00000001	0.00005836
3	Yes	4	0.00000001	0.00067757
4	Yes	6	0.00000001	0.00016574
5	Yes	6	0.00000001	0.00004783
6	Yes	6	0.00000001	0.00016331
7	Yes	6	0.00000001	0.00004710
8	Yes	5	0.00000001	0.00002068
9	Yes	4	0.00000001	0.00032468
10	Yes	6	0.00000001	0.00016209
11	Yes	6	0.00000001	0.00004677
12	Yes	6	0.00000001	0.00016504
13	Yes	6	0.00000001	0.00004779
14	Yes	5	0.00000001	0.00005893
15	Yes	4	0.00000001	0.00068327
16	Yes	6	0.00000001	0.00016290
17	Yes	6	0.00000001	0.00004688
18	Yes	6	0.00000001	0.00016420
19	Yes	6	0.00000001	0.00004745
20	Yes	5	0.00000001	0.00002098
21	Yes	4	0.00000001	0.00032683
22	Yes	6	0.00000001	0.00016435
23	Yes	6	0.00000001	0.00004761
24	Yes	6	0.00000001	0.00016125
25	Yes	6	0.00000001	0.00004654
26	Yes	4	0.00000001	0.00010080
27	Yes	6	0.00000001	0.00016462
28	Yes	6	0.00000001	0.00024577
29	Yes	6	0.00000001	0.00024336
30	Yes	6	0.00000001	0.00016651
31	Yes	6	0.00000001	0.00024576
32	Yes	6	0.00000001	0.00024823
33	Yes	6	0.00000001	0.00016587
34	Yes	6	0.00000001	0.00023857
35	Yes	6	0.00000001	0.00024065
36	Yes	6	0.00000001	0.00016280
37	Yes	6	0.00000001	0.00023837
38	Yes	6	0.00000001	0.00023631
39	Yes	4	0.00000001	0.00009095
40	Yes	4	0.00000001	0.00067691
41	Yes	4	0.00000001	0.00064717
42	Yes	4	0.00000001	0.00007675
43	Yes	4	0.00000001	0.00063666
44	Yes	4	0.00000001	0.00067406
45	Yes	4	0.00000001	0.00009117
46	Yes	4	0.00000001	0.00063801

47	Yes	4	0.00000001	0.00065539
48	Yes	4	0.00000001	0.00007625
49	Yes	4	0.00000001	0.00065779
50	Yes	4	0.00000001	0.00062148

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	149.854 - 111.934	24.417	40	1.5748	0.0022
L2	115.764 - 76.434	14.060	40	1.2299	0.0008
L3	81.264 - 43.014	6.581	40	0.8176	0.0004
L4	48.684 - 0	2.243	40	0.4331	0.0002

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
150.00	APXVTM14-ALU-I20 w/ Mount Pipe	40	24.417	1.5748	0.0022	25114
140.00	(2) 7770.00 w/ Mount Pipe	40	21.257	1.4792	0.0017	12743
128.00	(2) DB846F65ZAXY w/ Mount Pipe	40	17.540	1.3596	0.0012	5745
50.00	KS24019-L112A	40	2.365	0.4472	0.0002	4411

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	149.854 - 111.934	124.146	4	8.0240	0.0109
L2	115.764 - 76.434	71.601	4	6.2721	0.0040
L3	81.264 - 43.014	33.545	4	4.1713	0.0018
L4	48.684 - 0	11.436	4	2.2095	0.0008

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
150.00	APXVTM14-ALU-I20 w/ Mount Pipe	4	124.146	8.0240	0.0109	5114
140.00	(2) 7770.00 w/ Mount Pipe	4	108.121	7.5390	0.0085	2594
128.00	(2) DB846F65ZAXY w/ Mount Pipe	4	89.268	6.9314	0.0060	1166
50.00	KS24019-L112A	4	12.058	2.2813	0.0008	867

Compression Checks

Pole Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
L1	149.854 - 111.934 (1)	TP26.59x18x0.1875	37.92	0.00	0.0	15.196 5	-8.83	1026.75	0.009
L2	111.934 - 76.434 (2)	TP34.14x25.3474x0.3125	39.33	0.00	0.0	32.481 6	-14.69	2384.60	0.006
L3	76.434 - 43.014 (3)	TP40.97x32.4352x0.3125	38.25	0.00	0.0	39.072 3	-21.87	2713.66	0.008
L4	43.014 - 0 (4)	TP50x39.0798x0.375	48.68	0.00	0.0	59.066 2	-35.80	4033.20	0.009

Pole Bending Design Data

Section No.	Elevation ft	Size	M _{ux} kip-ft	φM _{nx} kip-ft	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	M _{uy} kip-ft	φM _{ny} kip-ft	Ratio $\frac{M_{uy}}{\phi M_{ny}}$
L1	149.854 - 111.934 (1)	TP26.59x18x0.1875	394.49	539.40	0.731	0.00	539.40	0.000
L2	111.934 - 76.434 (2)	TP34.14x25.3474x0.3125	1001.71	1603.11	0.625	0.00	1603.11	0.000
L3	76.434 - 43.014 (3)	TP40.97x32.4352x0.3125	1657.74	2198.00	0.754	0.00	2198.00	0.000
L4	43.014 - 0 (4)	TP50x39.0798x0.375	2766.44	4116.93	0.672	0.00	4116.93	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V _u K	φV _n K	Ratio $\frac{V_u}{\phi V_n}$	Actual T _u kip-ft	φT _n kip-ft	Ratio $\frac{T_u}{\phi T_n}$
L1	149.854 - 111.934 (1)	TP26.59x18x0.1875	16.28	513.37	0.032	0.17	1080.13	0.000
L2	111.934 - 76.434 (2)	TP34.14x25.3474x0.3125	18.95	1192.30	0.016	0.35	3210.14	0.000
L3	76.434 - 43.014 (3)	TP40.97x32.4352x0.3125	21.29	1356.83	0.016	0.47	4401.38	0.000
L4	43.014 - 0 (4)	TP50x39.0798x0.375	24.19	2016.60	0.012	0.47	8243.93	0.000

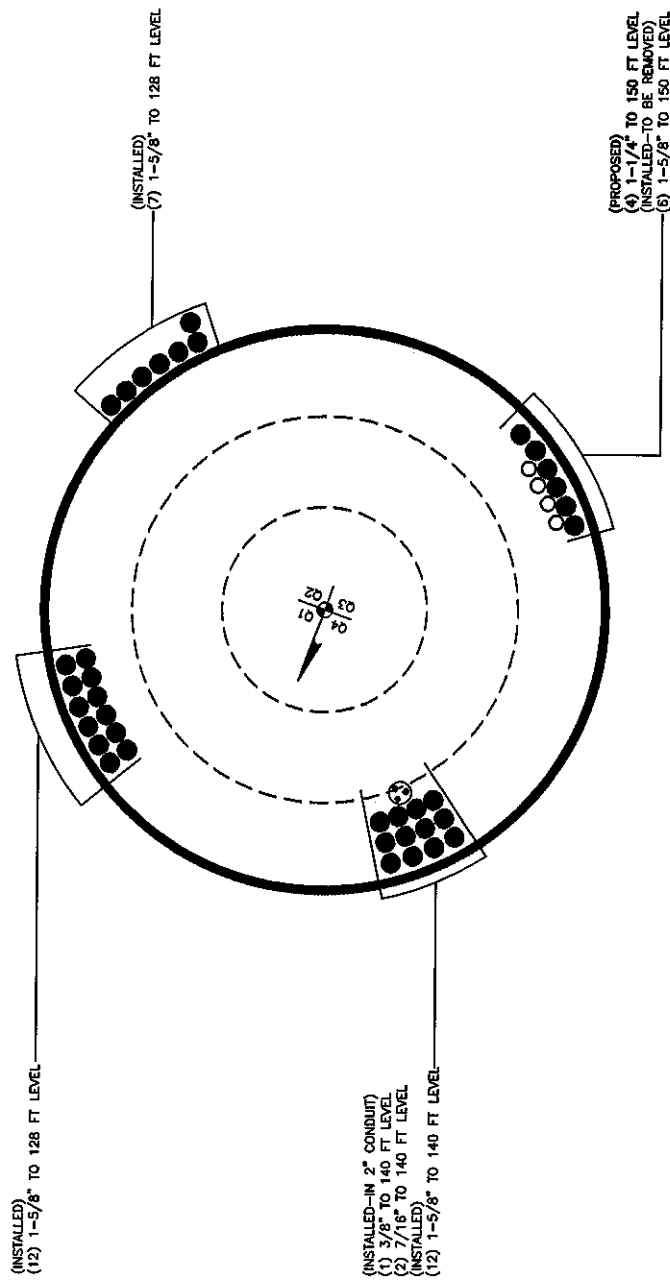
Pole Interaction Design Data

Section No.	Elevation ft	Ratio P _u φP _n	Ratio M _{ux} φM _{nx}	Ratio M _{uy} φM _{ny}	Ratio V _u φV _n	Ratio T _u φT _n	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	149.854 - 111.934 (1)	0.009	0.731	0.000	0.032	0.000	0.741	1.000	4.8.2
L2	111.934 - 76.434 (2)	0.006	0.625	0.000	0.016	0.000	0.631	1.000	4.8.2
L3	76.434 - 43.014 (3)	0.008	0.754	0.000	0.016	0.000	0.763	1.000	4.8.2
L4	43.014 - 0 (4)	0.009	0.672	0.000	0.012	0.000	0.681	1.000	4.8.2

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail
L1	149.854 - 111.934	Pole	TP26.59x18x0.1875	1	-8.83	1026.75	74.1	Pass
L2	111.934 - 76.434	Pole	TP34.14x25.3474x0.3125	2	-14.69	2384.60	63.1	Pass
L3	76.434 - 43.014	Pole	TP40.97x32.4352x0.3125	3	-21.87	2713.66	76.3	Pass
L4	43.014 - 0	Pole	TP50x39.0798x0.375	4	-35.80	4033.20	68.1	Pass
							Summary	
							Pole (L3)	Pass
							RATING =	Pass

APPENDIX B
BASE LEVEL DRAWING



APPENDIX C

ADDITIONAL CALCULATIONS

Stiffened or Unstiffened, UngROUTed, Circular Base Plate - Any Rod Material

TIA Rev G

Assumption: Clear space between bottom of leveling nut and top of concrete not exceeding (1)*(Rod Diameter)

Site Data

BU#: 876383
Site Name: CLINTON / ANDERSON'S PROPER
App #: 441479 Rev. 0
Pole Manufacturer: Other

Anchor Rod Data

Qty:	12	
Diam:	2.25	in
Rod Material:	A615-J	
Strength (Fu):	100	ksi
Yield (Fy):	75	ksi
Bolt Circle:	59	in

Plate Data

Diam:	65	in
Thick:	1.75	in
Grade:	60	ksi
Single-Rod B-eff:	13.22	in

Stiffener Data (Welding at both sides)

Config:	3	*
Weld Type:	Both	
Groove Depth:	0.5	in **
Groove Angle:	45	degrees
Fillet H. Weld:	0.5	in
Fillet V. Weld:	0.375	in
Width:	6	in
Height:	18	in
Thick:	1.25	in
Notch:	0.75	in
Grade:	50	ksi
Weld str.:	70	ksi
Clear Space between	6	in

Pole Data

Diam:	50	in
Thick:	0.375	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None

Reactions

Mu:	2766	ft-kips
Axial, Pu:	36	kips
Shear, Vu:	24	kips
Eta Factor, η	0.5	TIA G (Fig. 4-4)

If No stiffeners, Criteria: AISC LRFD <-Only Applicable to Unstiffened Cases

Anchor Rod Results

Max Rod (Cu+ Vu/η): 194.6 Kips
 Allowable Axial, Φ^*Fu^*Anet : 260.0 Kips
 Anchor Rod Stress Ratio: 74.8% Pass

Stiffened
AISC LRFD
ϕ^*Tn

Base Plate Results

Base Plate Stress: 39.6 ksi
 Allowable Plate Stress: 54.0 ksi
 Base Plate Stress Ratio: 73.3% Pass

Flexural Check

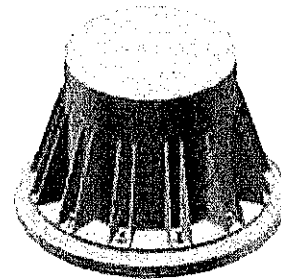
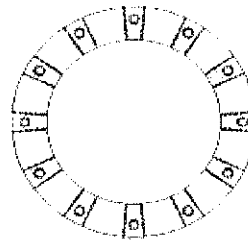
Stiffened
AISC LRFD
ϕ^*Fy
Y.L. Length:
N/A, Roark

Stiffener Results

Horizontal Weld : 45.3% Pass
 Vertical Weld: 30.2% Pass
 Plate Flex+Shear, $f_b/F_b+(f_v/F_v)^2$: 6.0% Pass
 Plate Tension+Shear, $f_t/F_t+(f_v/F_v)^2$: 27.4% Pass
 Plate Comp. (AISC Bracket): 28.7% Pass

Pole Results

Pole Punching Shear Check: 9.1% Pass



* 0 = none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

** Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

Pier and Pad Foundation



BU #: 876383
 Site Name: CLINTON / ANDERSON
 App. Number: 441479 Rev. 0

TIA-222 Revision: G
 Tower Type: Monopole

Block Foundation?: ☐

Superstructure Analysis Reactions		
Compression, P_{comp} :	36	kips
Base Shear, V_u_{comp} :	24	kips
Moment, M_u :	2766	ft-kips
Tower Height, H :	150	ft
BP Dist. Above Fdn, bp_{dist} :	4	in

Pier Properties		
Pier Shape:	Square	
Pier Diameter, $dpier$:	6.5	ft
Ext. Above Grade, E :	0.50	ft
Pier Rebar Size, Sc :	8	
Pier Rebar Quantity, mc :	39	
Pier Tie/Spiral Size, St :	4	
Pier Tie/Spiral Quantity, mt :	7	
Pier Reinforcement Type:	Tie	
Pier Clear Cover, cc_{pier} :	3	in

Pad Properties		
Depth, D :	5.0	ft
Pad Width, W :	24.5	ft
Pad Thickness, T :	3.0	ft
Pad Rebar Size, Sp :	8	
Pad Rebar Quantity, mp :	40	
Pad Clear Cover, cc_{pad} :	3	in

Material Properties		
Rebar Grade, F_y :	60000	psi
Concrete Compressive Strength, F'_c :	4000	psi
Dry Concrete Density, δ_c :	150	pcf

Soil Properties		
Total Soil Unit Weight, γ :	100	pcf
Ultimate Gross Bearing, Q_{ult} :	20.000	ksf
Cohesion, C_u :		ksf
Friction Angle, ϕ :	30	degrees
SPT Blow Count, N_{blows} :	6	
Base Friction, μ :		
Neglected Depth, N :	3.30	ft
Foundation Bearing on Rock?	Yes	
Groundwater Depth, gw :	n/a	ft

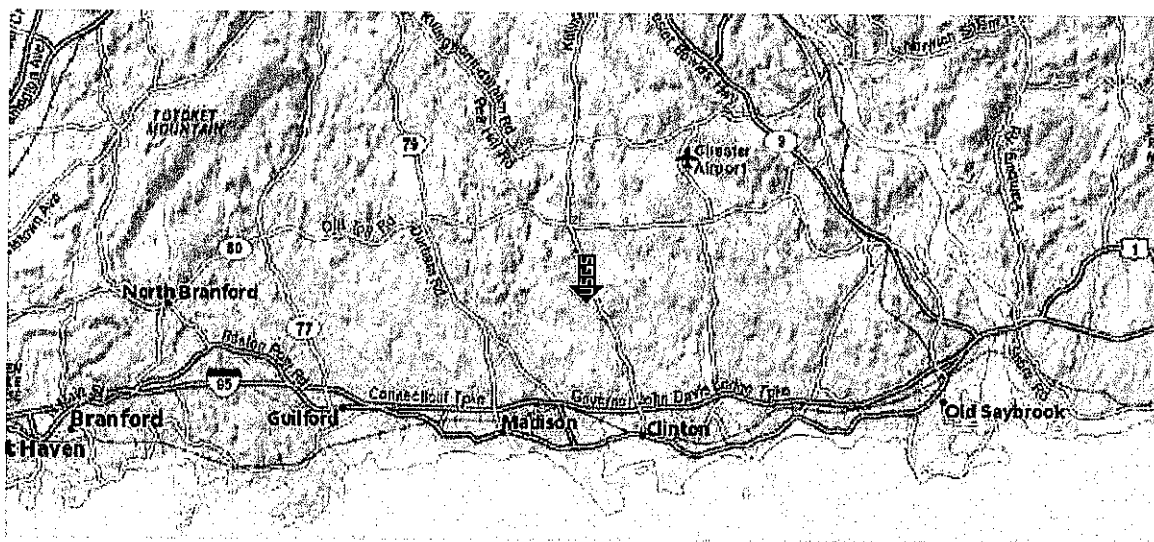
Foundation Analysis Checks				
	Capacity	Demand	Rating	Check
Lateral (Sliding) (kips)	143.93	24.00	16.7%	Pass
Bearing Pressure (ksf)	15.00	2.90	19.3%	Pass
Overtuming (kip*ft)	4578.32	2906.00	63.5%	Pass
Pier Flexure (Comp.) (kip*ft)	4616.29	2826.00	61.2%	Pass
Pier Compression (kip)	26891.28	55.01	0.2%	Pass
Pad Flexure (kip*ft)	4344.44	1160.29	26.7%	Pass
Pad Shear - 1-way (kips)	878.58	178.01	20.3%	Pass
Pad Shear - 2-way (ksf)	0.19	0.03	15.6%	Pass

Soil Rating: 63.5%
 Structural Rating: 61.2%

<--Toggle between Gross and Net

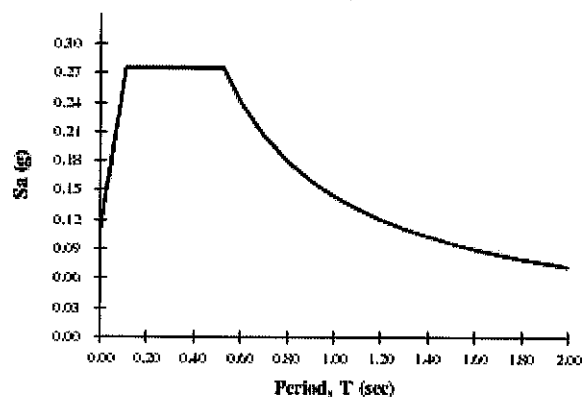
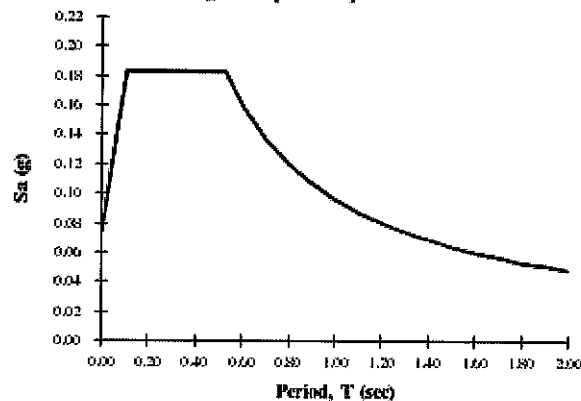
USGS Design Maps Summary Report**User-Specified Input****Report Title** 876383

Fri June 1, 2018 21:07:37 UTC

Building Code Reference Document 2012/2015 International Building Code
(which utilizes USGS hazard data available in 2008)**Site Coordinates** 41.3381°N, 72.5565°W**Site Soil Classification** Site Class D - "Stiff Soil"**Risk Category** I/II/III**USGS-Provided Output**

$S_s = 0.172 \text{ g}$	$S_{MS} = 0.275 \text{ g}$	$S_{DS} = 0.183 \text{ g}$
$S_1 = 0.060 \text{ g}$	$S_{M1} = 0.144 \text{ g}$	$S_{D1} = 0.096 \text{ g}$

For information on how the S_s and S_1 values above have been calculated from probabilistic (risk-targeted) and deterministic ground motions in the direction of maximum horizontal response, please return to the application and select the "2009 NEHRP" building code reference document.

MCE_R Response Spectrum**Design Response Spectrum**

CCISeismic - Design Category

Per 2012/2015 IBC

Site BU: 876383
Work Order: 1580562
Application: 441479 Rev. 0



	Degrees	Minutes	Seconds	
Site Latitude =	41	20	17.23	41.3381 degrees
Site Longitude =	-72	33	23.43	-72.5565 degrees
Ground Supported Structure =	Yes			
Structure Class =	II			(Table 2-1)
Site Class =	D - Stiff Soil			(Table 2-11)

Spectral response acceleration short periods, S_s =	0.172	USGS Seismic Tool
Spectral response acceleration 1 s period, S_1 =	0.060	

Importance Factor, I =	1.0	(Table 2-3)
Acceleration-based site coefficient, F_a =	1.6	(Table 2-12)
Velocity-based site coefficient, F_v =	2.4	(Table 2-13)

Design spectral response acceleration short period, S_{DS} =	0.183	(2.7.6)
Design spectral response acceleration 1 s period, S_{D1} =	0.096	(2.7.6)

Seismic Design Category - Short Period Response =	B	ASCE 7-05 Table 11.6-1
Seismic Design Category - 1s Period Response =	B	ASCE 7-05 Table 11.6-2

Worst Case Seismic Design Category =	B	ASCE 7-05 Tables 11.6-1 and 6-2
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RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

SPRINT Existing Facility

Site ID: CT33XC543

Clinton / Anderson Property
7 Sherwood Forest Lane
Killingworth, CT 06419

July 26, 2018

EBI Project Number: 6218005224

Site Compliance Summary	
Compliance Status:	COMPLIANT
Site total MPE% of FCC general population allowable limit:	8.84 %



July 26, 2018

SPRINT

Attn: RF Engineering Manager
1 International Boulevard, Suite 800
Mahwah, NJ 07495

Emissions Analysis for Site: **CT33XC543 – Clinton / Anderson Property**

EBI Consulting was directed to analyze the proposed SPRINT facility located at **7 Sherwood Forest Lane, Killingworth, CT**, for the purpose of determining whether the emissions from the Proposed SPRINT Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

General population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limits for the 850 MHz Band is approximately $567 \mu\text{W}/\text{cm}^2$. The general population exposure limit for the 1900 MHz (PCS) and 2500 MHz (BRS) bands is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed SPRINT Wireless antenna facility located at **7 Sherwood Forest Lane, Killingworth, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since SPRINT is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 1 CDMA channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.
- 2) 2 LTE channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 50 Watts per Channel.
- 3) 5 CDMA channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 16 Watts per Channel.
- 4) 2 LTE channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
- 5) 8 LTE channels (2500 MHz (BRS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.



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- 6) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 7) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antennas used in this modeling are the **Commscope NNVV-65B-R4** and the **RFS APXVTM14-ALU-I20** for transmission in the 850 MHz, 1900 MHz (PCS) and 2500 MHz (BRS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 9) The antenna mounting height centerlines of the proposed antennas are **152 feet** above ground level (AGL) for **Sector A**, **152 feet** above ground level (AGL) for **Sector B** and **152 feet** above ground level (AGL) for Sector C.
- 10) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general population threshold limits.



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SPRINT Site Inventory and Power Data by Antenna

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model	Commscope NNVV-65B-R4	Make / Model	Commscope NNVV-65B-R4	Make / Model	Commscope NNVV-65B-R4
Gain	12.75 / 15.05 dBd	Gain	12.75 / 15.05 dBd	Gain	12.75 / 15.05 dBd
Height (AGL)	152 feet	Height (AGL)	152 feet	Height (AGL)	152 feet
Frequency Bands	850 MHz / 1900 MHz (PCS)	Frequency Bands	850 MHz / 1900 MHz (PCS)	Frequency Bands	850 MHz / 1900 MHz (PCS)
Channel Count	10	Channel Count	10	Channel Count	10
Total TX Power(W)	280 Watts	Total TX Power(W)	280 Watts	Total TX Power(W)	280 Watts
ERP(W)	7,378.61	ERP(W)	7,378.61	ERP(W)	7,378.61
Antenna A1 MPE%	1.54 %	Antenna B1 MPE%	1.54 %	Antenna C1 MPE%	1.54 %
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model	RFS APXVTM14-ALU- I20	Make / Model	RFS APXVTM14-ALU- I20	Make / Model	RFS APXVTM14-ALU- I20
Gain	15.9 dBd	Gain	15.9 dBd	Gain	15.9 dBd
Height (AGL)	152 feet	Height (AGL)	152 feet	Height (AGL)	152 feet
Frequency Bands	2500 MHz (BRS)	Frequency Bands	2500 MHz (BRS)	Frequency Bands	2500 MHz (BRS)
Channel Count	8	Channel Count	8	Channel Count	8
Total TX Power(W)	160 Watts	Total TX Power(W)	160 Watts	Total TX Power(W)	160 Watts
ERP(W)	6,224.72	ERP(W)	6,224.72	ERP(W)	6,224.72
Antenna A2 MPE%	1.05 %	Antenna B2 MPE%	1.05 %	Antenna C2 MPE%	1.05 %

Site Composite MPE%	
Carrier	MPE%
SPRINT – Max per sector	2.59 %
AT&T	2.38 %
Verizon Wireless	3.87 %
Site Total MPE %:	8.84 %

SPRINT Sector A Total:	2.59 %
SPRINT Sector B Total:	2.59 %
SPRINT Sector C Total:	2.59 %
Site Total:	8.84 %

SPRINT Frequency Band / Technology (All Sectors)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (μW/cm ²)	Frequency (MHz)	Allowable MPE (μW/cm ²)	Calculated % MPE
Sprint 850 MHz CDMA	1	376.73	152	0.64	850 MHz	567	0.12%
Sprint 850 MHz LTE	2	941.82	152	3.18	850 MHz	567	0.56%
Sprint 1900 MHz (PCS) CDMA	5	511.82	152	4.32	1900 MHz (PCS)	1000	0.43%
Sprint 1900 MHz (PCS) LTE	2	1,279.56	152	4.32	1900 MHz (PCS)	1000	0.43%
Sprint 2500 MHz (BRS) LTE	8	778.09	152	10.50	2500 MHz (BRS)	1000	1.05%
						Total:	2.59%



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Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the SPRINT facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

SPRINT Sector	Power Density Value (%)
Sector A:	2.59 %
Sector B:	2.59 %
Sector C:	2.59 %
SPRINT Maximum MPE % (per sector):	2.59 %
Site Total:	8.84 %
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **8.84 %** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

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 WOBURN, MA 01801
 UNITED STATES US

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 TOWN OF KILLINGWORTH
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KILLINGWORTH CT 06419
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