

Northeast Site Solutions Denise Sabo 4 Angela's Way, Burlington CT 06013 203-435-3640 denise@northeastsitesolutions.com

August 27, 2021

Members of the Siting Council Connecticut Siting Council Ten Franklin Square New Britain, CT 06051

RE: Exempt Modification Application

197 North Chestnut Hill Road, Killingworth CT 06419

Latitude: 41.386139 Longitude: -72.604056 Site#: 807134\_ Crown\_DISH

Dear Ms. Bachman:

Based on the 2020 merger between T-Mobile and Sprint, and as part of the agreement, the DOJ required T-Mobile to divest some sites to Dish in order to create an additional wireless provider. This site is part of the agreement.

Dish Wireless LLC is requesting to file an exempt modification for an existing tower located at 197 North Chestnut Hill Road, Killingworth CT 06419. Dish Wireless LLC proposes to install three (3) antennas at the 148-foot level of the existing 170-foot tower. The property and the tower are both owned by Crown Castle. This modification includes hardware that is 5G capable.

#### **Dish Wireless LLC Planned Modifications:**

Remove: (At 146-147' level) Antenna mount (12) DB844H90 Antenna

(12) Coax Lines

Remove and Replace: NONE

Install New:

Commscope MC3975083 Mount (3) JMA MX08FRO665-20 Antenna

(3) TA08025-B604 RRU

(3) TA08025-B605 RRU

(1) Raycap

(1) 1-1/2" Hybrid

Existing to Remain:

NONE



Ground Work: (within existing compound)
New H-Frame
Equipment Cabinet
Power/Telco Cabinet
Ice Bridge
7'x5' Steel Platform

The facility was approved by the CT Siting Council Docket No. 164 on December 5, 1994. Please see attached.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16- SOj-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-SOj-73, a copy of this letter is being sent to The Honorable Catherine Iino, First Selectwoman, and Cathie S. Jefferson, Zoning Enforcement Officer, for the Town of Killingworth. A copy is also being sent to the tower owner, and property owner.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

- 1. The proposed modifications will not result in an increase in the height of the existing structure.
- 2. The proposed modifications will not require the extension of the site boundary.
- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
- 4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard.
- 5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
- 6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, DISH Wireless LLC respectfully submits that the proposed modifications to the above referenced telecommunications facility constitute an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

#### Denise Sabo

Denise Sabo

Mobile: 203-435-3640 Fax: 413-521-0558

Office: 4 Angela's Way, Burlington CT 06013 Email: denise@northeastsitesolutions.com



#### Attachments

cc: The Honorable Catherine Iino, First Selectwoman Town of Killingworth 323 Route 81 Killingworth, CT 06419

Cathie S. Jefferson, Zoning Enforcement Officer Town of Killingworth 323 Route 81 Killingworth, CT 06419

Crown Castle, Property and Tower Owner

# Exhibit A

**Original Facility Approval** 

**DOCKET NO. 164** - An Application of Metro Mobile CTS of Hartford, Inc. and Metro Mobile CTS of New Haven, Inc., for a Certificate of Environmental Compatibility and Public Need for the construction, maintenance, and operation of a cellular telecommunications facility. The proposed prime site is located approximately 700 feet northeast from the end of the North Chestnut Hill Road cul-de-sac (Lot No. 7), Killingworth. The proposed alternate site is located approximately 350 feet east of 828 Summer hill Road, Madison, Connecticut.

#### **Connecticut Siting Council**

December 5, 1994

#### **DECISION AND ORDER**

Pursuant to the foregoing Findings of Fact, and Opinion, the Connecticut Siting Council (Council) finds that the effects associated with the construction, operation, and maintenance of a cellular telecommunications tower and equipment building at the proposed prime site in Killingworth, Connecticut, including effects on the natural environment; ecological integrity and balance; public health and safety; scenic, historic, and recreational values; forests and parks; air and water purity; and fish and wildlife are not disproportionate either alone or cumulatively with other effects when compared to need, are not in conflict with the policies of the State concerning such effects, and are not sufficient reason to deny the application and therefore directs that a Certificate of Environmental Compatibility and Public Need, as provided by section 16-50k of the Connecticut General Statutes (CGS), be issued to Metro Mobile CTS of Hartford, Inc., (Metro Mobile), for the construction, operation, and maintenance of a cellular telecommunications tower, associated equipment, and building at the proposed prime site located off of North Chestnut Hill Road, Killingworth, Connecticut. We find the effects on scenic resources and adjacent landuses of the alternative site to be significant, and therefore deny certification of this site.

The facility shall be constructed, operated, and maintained substantially as specified in the Council's record in this matter, and subject to the following conditions:

1. The self-supporting lattice tower shall be no taller than necessary to provide the proposed telecommunications service, and the tower shall not exceed a total height of 170 feet above ground level (AGL).

- 2. The Certificate Holder shall prepare a Development and Management (D&M) plan for this site in compliance with sections 16-50j-75 through 16-50j-77 of the Regulations of Connecticut State Agencies. The D&M plan shall include detailed plans for the tower location and tower foundation; the placement of all antennas to be attached to this tower; placement of the emergency generator, equipment building, fuel storage tank, access road, utility line, and security fence; site and accessway clearing and tree trimming; and water drainage and erosion and sedimentation controls consistent with the Connecticut Guidelines for Soil Erosion and Sedimentation Control (as amended.)
- 3. The Certificate Holder shall acquire all regulatory permits and approvals prior to the operation of the facility and submit copies upon receipt to the Council.
- 4. The Certificate Holder shall comply with any existing and future radio frequency (RF) standard promulgated by State or federal regulatory agencies. Upon the establishment of any new government RF standards, the facility granted herein shall be brought into compliance with such standards.
- 5. The Certificate Holder shall provide the Council a recalculated report of electromagnetic radio frequency power density if and when circumstances in operation cause a change in power density above the levels originally calculated and provided in the application.
- 6. The Certificate Holder shall permit public or private entities to share space on the proposed tower for fair consideration, or shall provide any requesting entity with specific legal, technical, environmental, or economic reasons precluding such tower sharing.
- 7. If the facility does not initially provide, or permanently ceases to provide cellular services following completion of construction, this Decision and Order shall be void, and the Certificate Holder shall dismantle the tower and remove all associated equipment or reapplication for any continued or new use shall be made to the Council before any such use is made.
- 8. Unless otherwise approved by the Council, this Decision and Order shall be void if all construction authorized herein is not completed within three years of the effective date of this Decision and Order or within three years after all appeals to this Decision and Order have been resolved.
- 9. The Certificate Holder shall notify the Council upon completion of construction.

Pursuant to CGS section 16-50p, we hereby direct that a copy of the Findings of Fact, Opinion, and Decision and Order be served on each person listed below, and notice of issuance shall be published in The Hartford Courant, The Middletown Press, and New Haven Register.

By this Decision and Order, the Council disposes of the legal rights, duties, and privileges of each party named or admitted to the proceeding in accordance with section 16-50j-17 of the Regulations of Connecticut State Agencies.

The parties and intervenors of this proceeding are:

<u>APPLICANT</u>	ITS REPRESENTATIVE
Metro Mobile CTS of Hartford, Inc.	Brian C.S. Freeman, Esq. Robinson & Cole One Commercial Plaza Hartford, CT 06103-3597
	David S. Malko, P.E.,General Manager - Engineering Sandy M. Ranciato, Manager Metro Mobile CTS of New Haven, Inc. 20 Alexander Drive Wallingford, CT 06492
INTERVENOR	ITS REPRESENTATIVE
Springwich Cellular Limited Partnership	Peter J. Tyrrell, Esq., Senior Attorney Springwich Cellular Limited Partnership 227 Church Street New Haven, CT 06506

# Exhibit B

**Property Card** 

The Assessor's office is responsible for the maintenance of records on the ownership of properties. Assessments are computed at 70% of the estimated market value of real property at the time of the last revaluation which was 2016.



Information on the Property Records for the Municipality of Killingworth was last updated on 7/31/2021.



#### **Parcel Information**

Location:	197 N CHESTNUT HILL ROAD	Property Use:	Industrial	Primary Use:	Utility Building
Unique ID:	00082800	Map Block Lot:	14-12G	Acres:	6.60
490 Acres:	0.00	Zone:	R-2	Volume / Page:	0154/0542
Developers Map / Lot:	TC 1179/LOT 7	Census:	6401		

#### Value Information

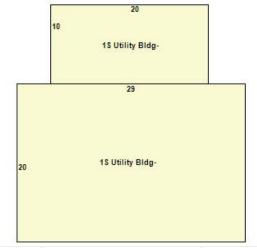
	Appraised Value	Assessed Value
Land	246,240	172,360
Buildings	42,152	29,510
Detached Outbuildings	222,525	155,770
Total	510,917	357,640

#### **Owner's Information**

#### Owner's Data

CROWN ATLANTIC COMPANY LLC C/O PMB 353 4017 WASHINGTON ROAD MCMURRAY PA 15317-2520





Category:	Industrial	Use:	Utility Building	GLA:	780
Stories:	1.00	Construction:	Average	Year Built:	1995
Heating:		Fuel:		Cooling Percent:	100%
Siding:	Pre-Cast Concrete	Roof Material:		Beds/Units:	0

#### **Special Features**

#### **Attached Components**

### **Detached Outbuildings**

Туре:	Year Built:	Length:	Width:	Area:
Fencing	1995	280	8	2,240
Concrete/Masonry Patio	1995	34	34	1,156
Paving	1995			2,000
Cell Tower	1995			1

#### Owner History - Sales

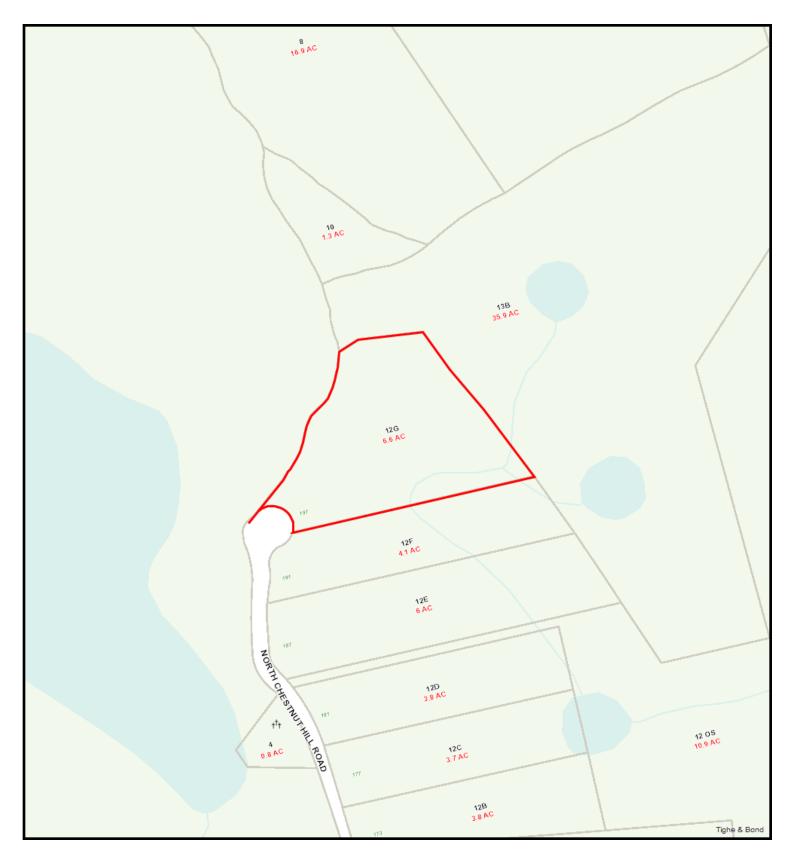
Owner Name	Volume	Page	Sale Date	Deed Type	Sale Price
CROWN ATLANTIC COMPANY LLC	0154	0542	04/19/1999	Warranty Deed	\$83,000

#### **Building Permits**

Permit Number	Permit Type	Date Opened	Reason
12-411	Commercial	12/27/2012	CELL TOWER MAINTENANCE

Permit Number	Permit Type	Date Opened	Reason
12-066	Commercial	03/03/2012	REPLACE 6 ANTENNAS ON EXISTING TOWER

Information Published With Permission From The Assessor



8/6/2021 9:54:37 PM

Scale: 1"=300'

Scale is approximate





# Exhibit C

**Construction Drawings** 

# wireless.

DISH WIRELESS, LLC, SITE ID:

### BOBDL00054A

DISH WIRELESS, LLC. SITE ADDRESS:

### 197 NORTH CHESTNUT HILL ROAD **KILLINGWORTH, CT 06419**

#### CONNECTICUT CODE COMPLIANCE

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:

2018 CT STATE BUILDING CODE/2015 IBC W/ CT AMENDMENTS MECHANICAL ELECTRICAL 2018 CT STATE BUILDING CODE/2015 IMC W/ CT AMENDMENTS 2018 CT STATE BUILDING CODE/2017 NEC W/ CT AMENDMENTS

	SHEET INDEX
SHEET NO.	SHEET TITLE
T-1	TITLE SHEET
A-1	OVERALL AND ENLARGED SITE PLAN
A-2	ELEVATION, ANTENNA LAYOUT AND SCHEDULE
A-3	EQUIPMENT PLATFORM AND H-FRAME DETAILS
	FOURNITY DETAILS
A-4	EQUIPMENT DETAILS
A-5 A-6	EQUIPMENT DETAILS EQUIPMENT DETAILS
E-1	ELECTRICAL ROUTE PLAN AND NOTES
E-2	ELECTRICAL DETAILS
E-3	ELECTRICAL ONE-LINE, FAULT CALCS & PANEL SCHEDULE
G-1	GROUNDING PLANS AND NOTES
G-2	GROUNDING DETAILS
G-3	GROUNDING DETAILS
RF-1	RF CABLE COLOR CODE
RF-2	RF PLUMBING DIAGRAM
GN-1	LEGEND AND ABBREVIATIONS
GN-2	GENERAL NOTES
GN-3	GENERAL NOTES
GN-4	GENERAL NOTES

#### SCOPE OF WORK

THIS IS NOT AN ALL INCLUSIVE LIST. CONTRACTOR SHALL UTILIZE SPECIFIED EQUIPMENT PART OR ENGINEER APPROVED EQUIPMENT. CONTRACTOR SHALL VERIFY ALL NEEDED EQUIPMENT TO PROVIDE A FUNCTIONAL SITE. THE PROJECT GENERALLY CONSISTS OF THE FOLLOWING:

#### TOWER SCOPE OF WORK:

- ABANDONED EQUIPMENT AT 146' AGL TBR INSTALL (3) PROPOSED PANEL ANTENNAS (1 PER SECTOR)
- INSTALL (3) PROPOSED ANTENNA MOUNTS (1 PER SECTOR) INSTALL PROPOSED JUMPERS
- INSTALL (6) PROPOSED RRUS (2 PER SECTOR)
- INSTALL (1) PROPOSED OVER VOLTAGE PROTECTION DEVICE (OVP)
- INSTALL (1) PROPOSED HYBRID CABLE

#### GROUND SCOPE OF WORKS

- INSTALL (1) PROPOSED METAL PLATFORM
- INSTALL (1) PROPOSED ICE BRIDGE INSTALL (1) PROPOSED PPC CABINET
- INSTALL PROPOSED EQUIPMENT CABINET INSTALL (1) PROPOSED POWER CONDUIT
- INSTALL (1) PROPOSED TELCO-FIBER BOX
- PROPOSED GPS UNIT

- INSTALL (1) PROPOSED SAFETY SWITCH (IF REQUIRED)
  INSTALL (1) PROPOSED CIENA BOX (IF REQUIRED)
  EXISTING METER SOCKET ON EXISTING H-FRAME TO BE UTILIZED

#### SITE PHOTO





**UNDERGROUND SERVICE ALERT CBYD 811** UTILITY NOTIFICATION CENTER OF CONNECTICUT (800) 922-4455 WWW.CBYD.COM

CALL 2 WORKING DAYS UTILITY NOTIFICATION PRIOR TO CONSTRUCTION

#### **GENERAL NOTES**

THE FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION. A TECHNICIAN WILL VISIT THE SITE AS REQUIRED FOR ROUTINE MAINTENANCE. THE PROJECT WILL NOT RESULT IN ANY SIGNIFICANT DISTURBANCE OR EFFECT ON DRAINAGE. NO SANITARY SEWER SERVICE, POTABLE WATER, OR TRASH DISPOSAL IS REQUIRED AND NO COMMERCIAL SIGNAGE IS PROPOSED.

#### 11"x17" PLOT WILL BE HALF SCALE UNLESS OTHERWISE NOTED

CONTRACTOR SHALL VERIFY ALL PLANS, EXISTING DIMENSIONS, AND CONDITIONS ON THE JOB SITE, AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCFEDING WITH THE WORK.

#### DISH WIRELESS, LLC. ADDRESS: POST OFFICE BOX 203112 5701 SOUTH SANTA FE DRIVE HOUSTON, TX 77216-3112 LITTLETON, CO 80120 TOWER TYPE: SELF SUPPORT TOWER TOWER CO SITE ID: 807134 TOWER OWNER: CROWN CASTLE 2000 CORPORATE DRIVE TOWER APP NUMBER: 553397 CANONSBURG, PA 15317 (877) 486-9377 COUNTY: MIDDLESEX SITE DESIGNER: INFINIGY 2500 W. HIGGINS RD. STE. 500 LATITUDE (NAD 83): 41° 23' 9.93" N 41.386083 N HOFFMAN ESTATES, IL 60169 LONGITUDE (NAD 83): -72° 36' 14.39" W (847) 648-4068 -72,604000 W SITE ACQUISITION: NICHOLAS CURRY ZONING JURISDICTION: CT - CONNECTICUT SITING COUNCIL TBD ZONING DISTRICT: CONSTRUCTION MANAGER: JAVIER SOTO PARCEL NUMBER: KILL-000008-002800 TRD BOSSENER CHARLES RF ENGINEER: OCCUPANCY GROUP: CONSTRUCTION TYPE: CONNECTICUT LIGHT & POWER CO TELEPHONE COMPANY: CROWN CASTLE

PROJECT DIRECTORY

#### **DIRECTIONS**

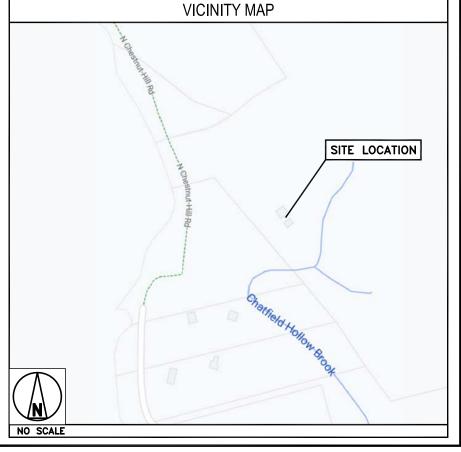
#### DIRECTIONS FROM CHESTER CHARTER, INC.:

SITE INFORMATION

CROWN CASTLE USA INC

PROPERTY OWNER:

HEAD NORTHWEST ON CHESTER AIRPORT TOWARD CROSS RD, TURN RIGHT ONTO CT-145 / WINTHROP RD, TURN LEFT ONTO CT-148 / W MAIN ST, TURN LEFT ONTO BECKWITH RD, KEEP RIGHT TO GET ONTO ABNER LN, TURN RIGHT ONTO N CHESTNUT HILL RD, ARRIVE AT 197 NORTH CHESTNUT HILL ROAD, KILLINGWORTH, CT 06419.





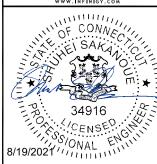
5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120



2000 CORPORATE DRIVE CANONSBURG PA 15317

# **INFINIGY8**

2500 W. HIGGINS RD. SUITE 500 | HOFFMAN ESTATES, IL 60169 PHONE: 847-648-4068 | FAX: 518-690-0793 WWW.INFINIGY.COM



IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

DRAWN	BY:	CHECKED	BY:	APPROVED	BY:
RCE	)	SS		CJW	

RFDS REV #: N/A

#### CONSTRUCTION DOCUMENTS

		SUBMITTALS
REV	DATE	DESCRIPTION
0	05/11/2021	FINAL
1	06/24/2021	FINAL
2	8/19/2021	TOWER ELEV UPDATED
	A&E F	PROJECT NUMBER

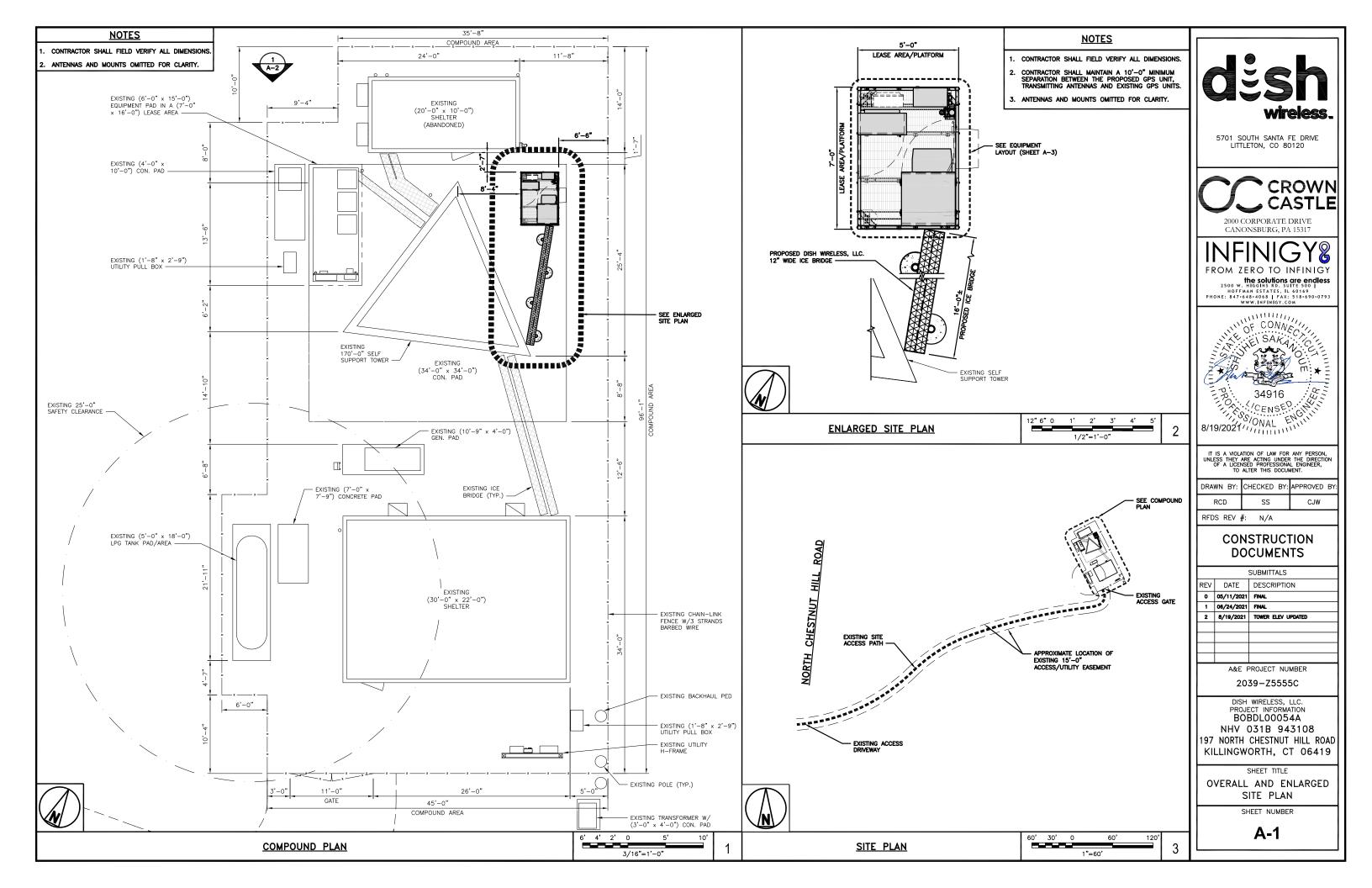
2039-Z5555C

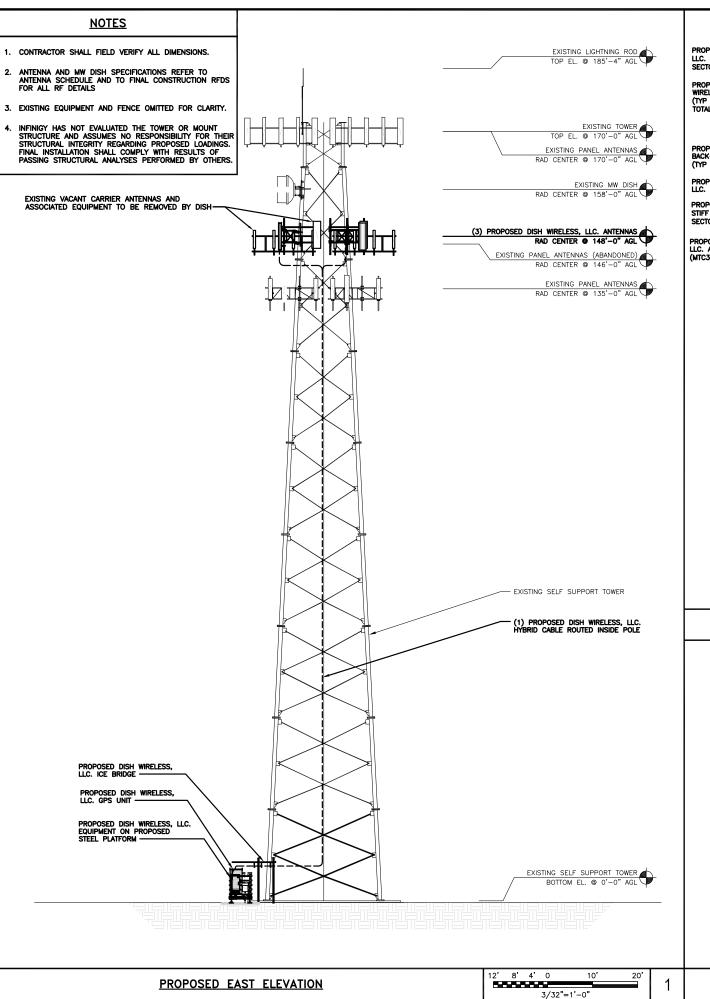
DISH WIRELESS, LLC. BOBDL00054A NHV 031B 943108 197 NORTH CHESTNUT HILL ROAD KILLINGWORTH, CT 06419

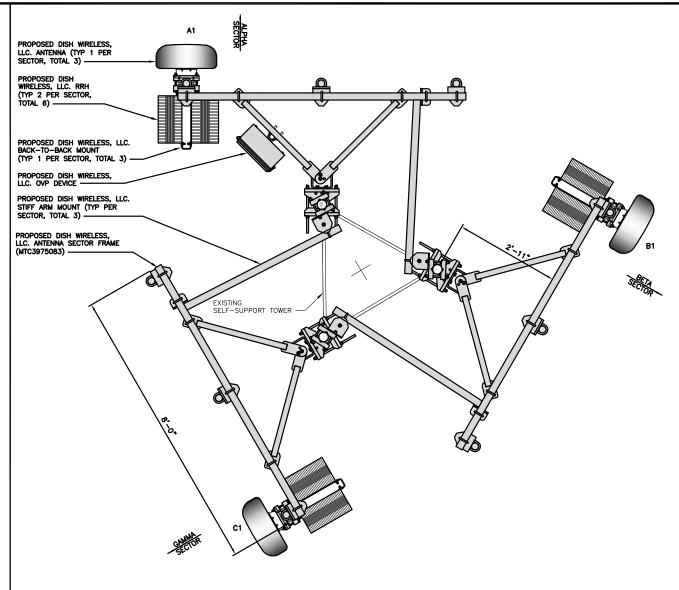
> SHEET TITLE TITLE SHEET

SHEET NUMBER

T-1







TRANSMISSION CABLE ANTENNA POSITION SECTOR FEED LINE TYPE AND LENGTH EXISTING OR PROPOSED MANUFACTURER - MODEL NUMBER TECHNOLOGY SIZE (HxW) AZMUITH RAD CENTER ALPHA A1 JMA WIRELESS - MX08FR0665-: 72.0" x 20.0 0. 148'-0" (1) HIGH-CAPACITY
HYBRID CABLE BETA 120° 148'-0" B1 72.0" x 20.0 C1 5G 240° 148'-0" GAMMA JMA WIRELESS - MX08FR0665-20 72.0" x 20.0" PROPOSED

#### NOTES

1. CONTRACTOR TO REFER TO FINAL CONSTRUCTION RFDS FOR ALL RF DETAILS.

**ANTENNA LAYOUT** 

2. ANTENNA OR RRH MODELS MAY CHANGE DUE TO EQUIPMENT AVAILABILITY. ALL EQUIPMENT CHANGES MUST BE APPROVED AND REMAIN IN COMPLIANCE WITH THE PROPOSED DESIGN AND STRUCTURAL ANALYSES.

		RRH		1
SECTOR	POSITION	MANUFACTURER — MODEL NUMBER	TECHNOLOGY	
ALPHA	A1	FUJITSU - TA08025-B604	5G	
ALPHA	A1	FUJITSU - TA08025-B605	5G	
BETA	B1	FUJITSU - TA08025-B604	5G	
	B1	FUJITSU - TA08025-B605	5G	
GAMMA	C1	FUJITSU - TA08025-B604	5G	
	C1	FUJITSU - TA08025-B605	5G	

- CONTRACTOR TO REFER TO FINAL CONSTRUCTION RFDS FOR ALL RF DETAILS.
- ANTENNA AND RRH MODELS MAY CHANGE DUE TO EQUIPMENT AVAILABILITY. ALL EQUIPMENT CHANGES MUST BE APPROVED AND REMAIN IN COMPLIANCE WITH THE PROPOSED DESIGN AND STRUCTURAL ANALYSES.

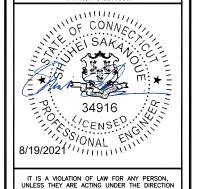
5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120



2000 CORPORATE DRIVE CANONSBURG, PA 15317

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HOFFANN ESTATES, IL 60169
PHONE: 847-648-4086 | FAX: 518-690-0793
WWW.INFINIGY.COM



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	CHECKED DI.	APPROVED BY
RCD	SS	CJM

RFDS REV #: N/A

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REV	DATE	DESCRIPTION			
0	05/11/2021	FINAL			
1	06/24/2021	FINAL			
2	8/19/2021	TOWER ELEV UPDATED			
	∧ 9a⊑ □	DO IECT NILIMPED			

A&E PROJECT NUMBER

2039-Z5555C

DISH WIRELESS, LLC. PROJECT INFORMATION BOBDL00054A NHV 031B 943108 197 NORTH CHESTNUT HILL ROAD KILLINGWORTH, CT 06419

SHEET TITLE

ELEVATION, ANTENNA LAYOUT AND SCHEDULE

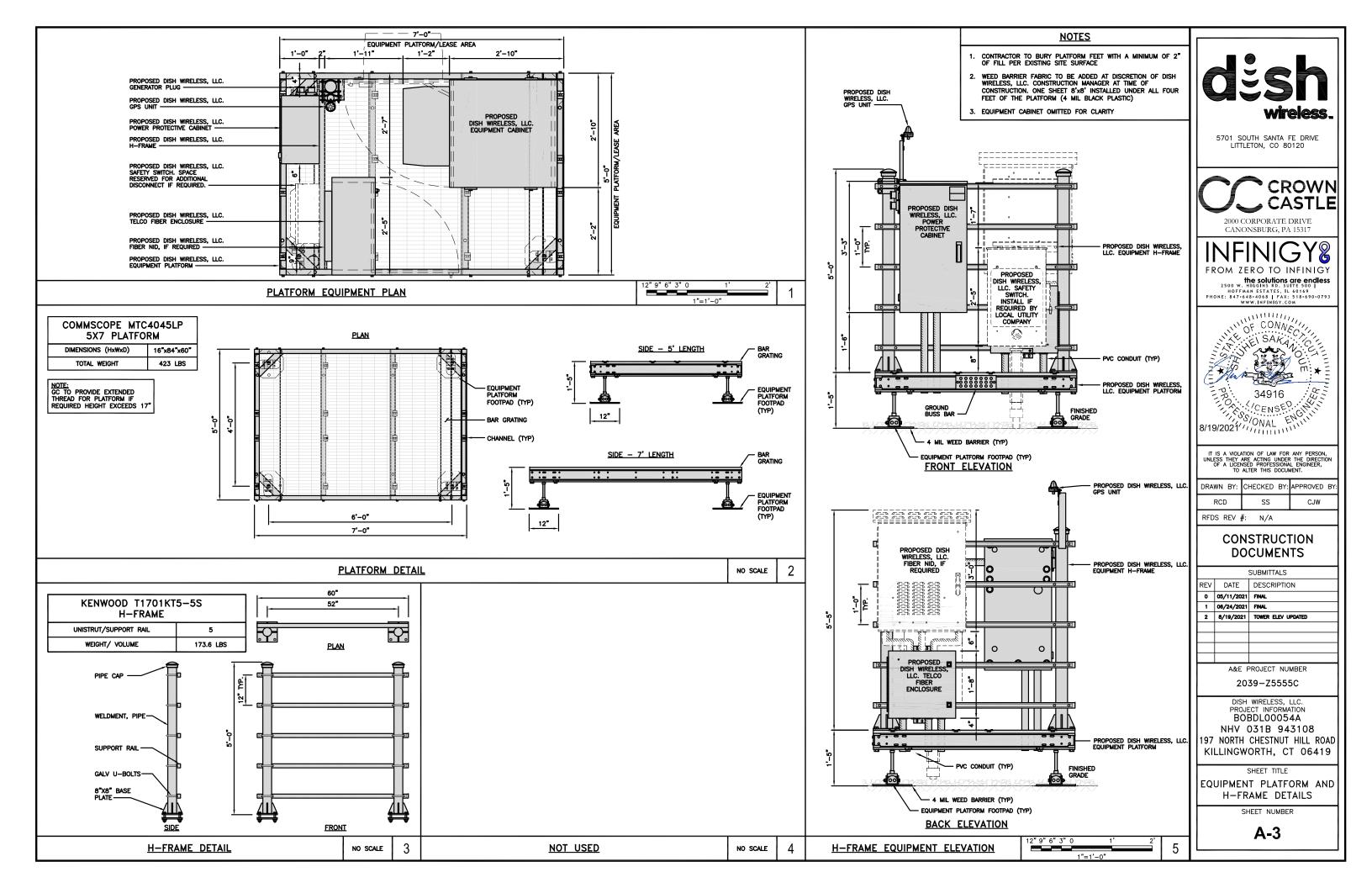
SHEET NUMBER

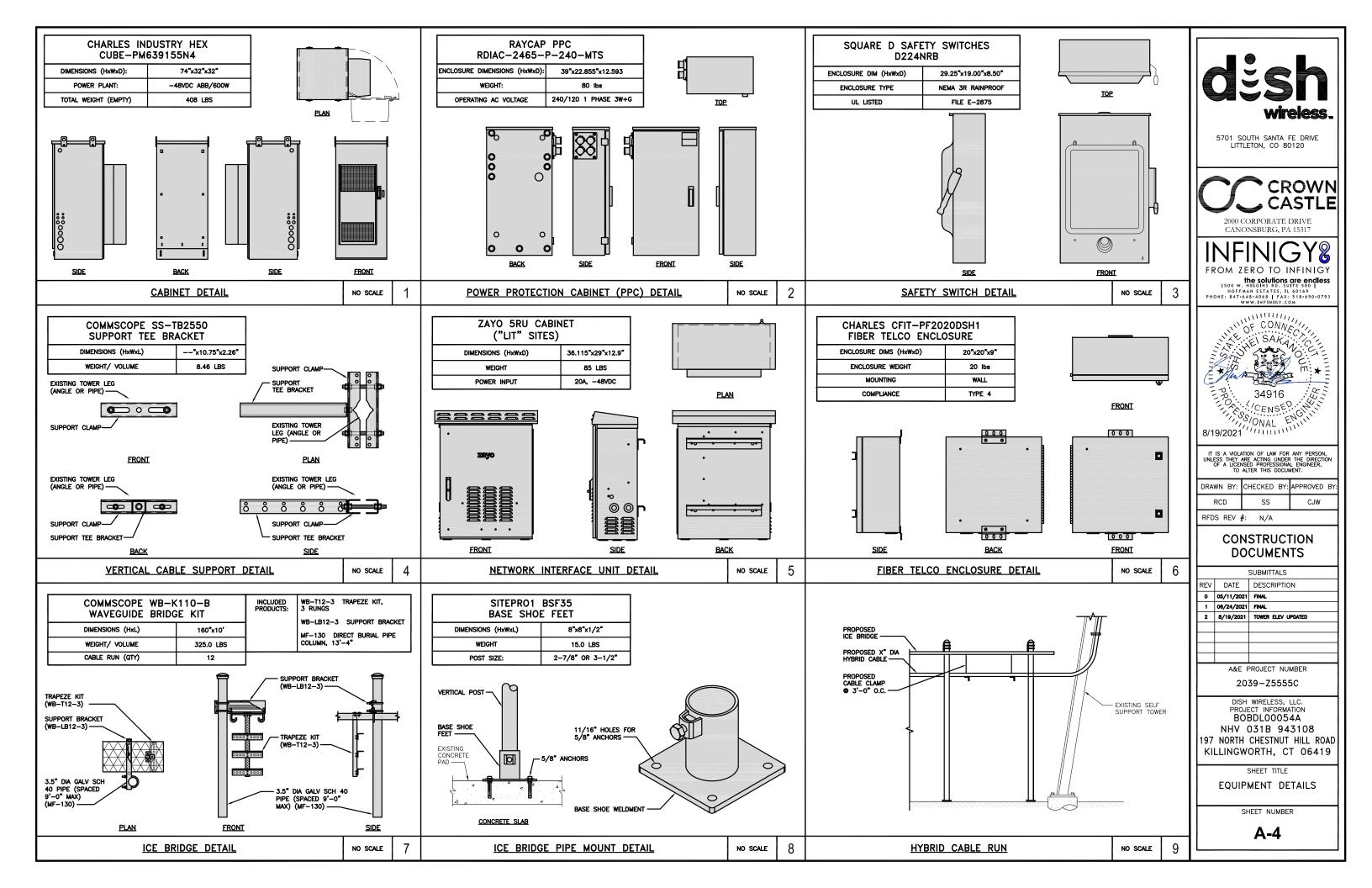
**A-2** 

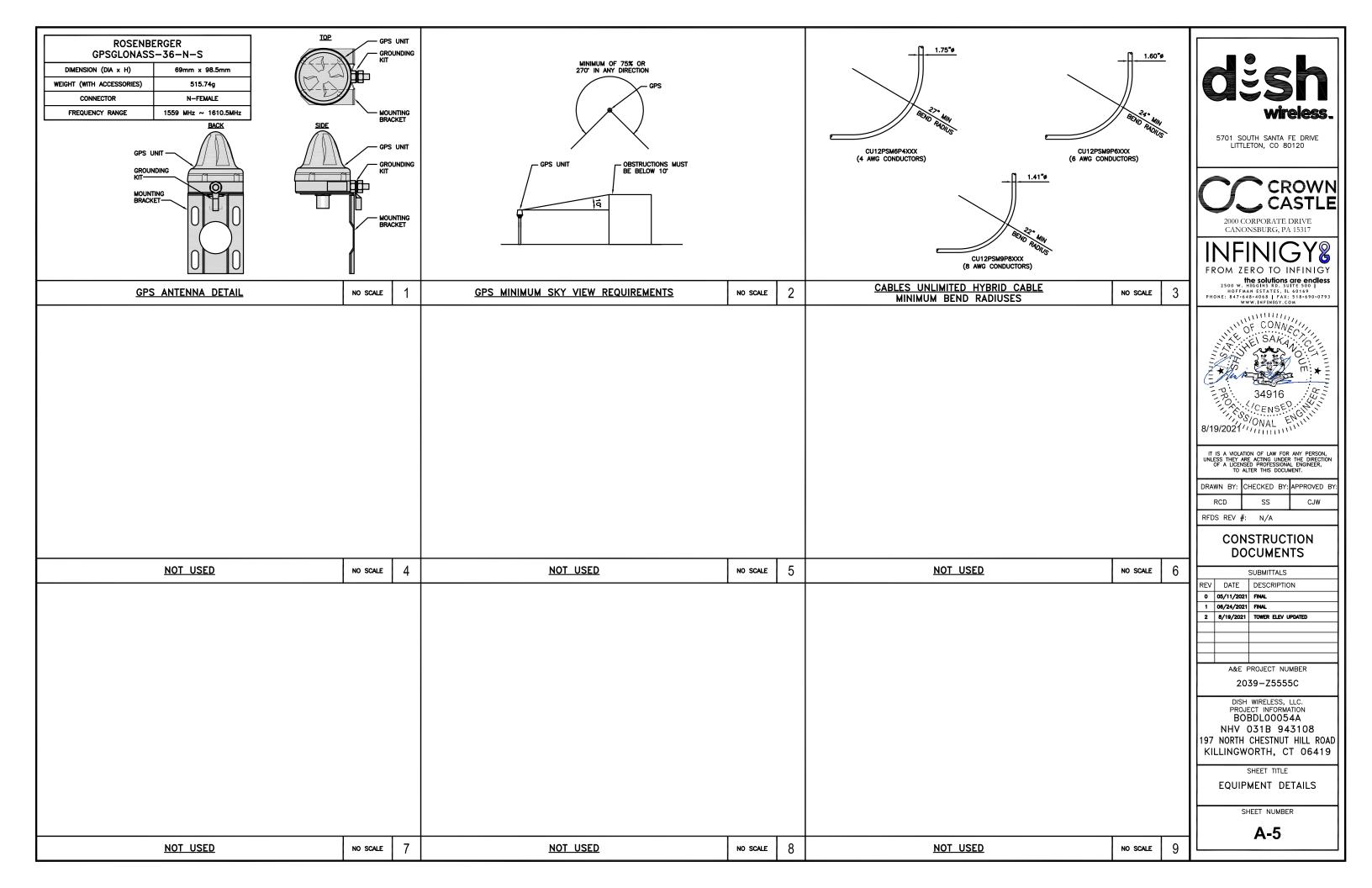
**ANTENNA SCHEDULE** 

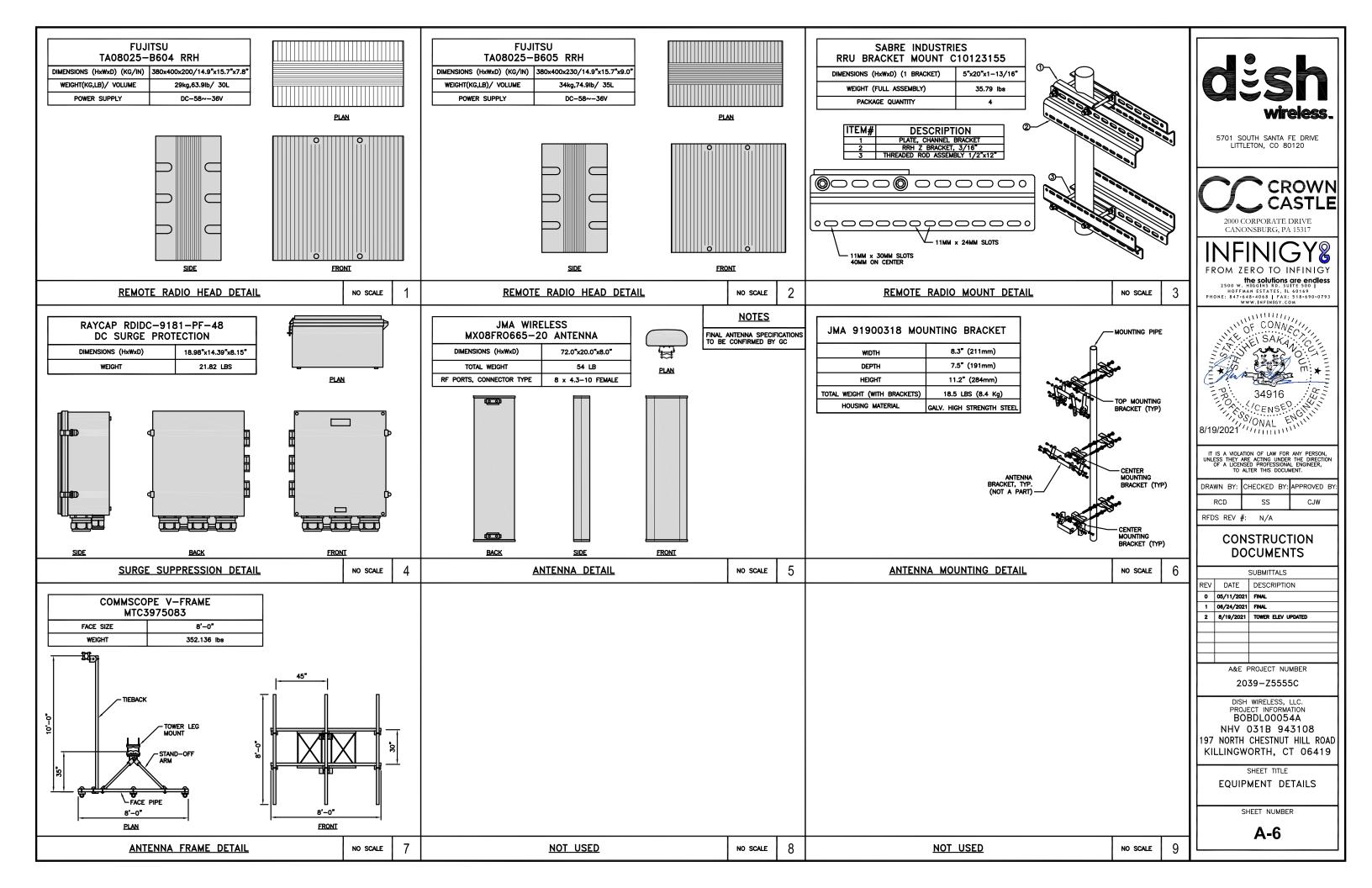
NO SCALE

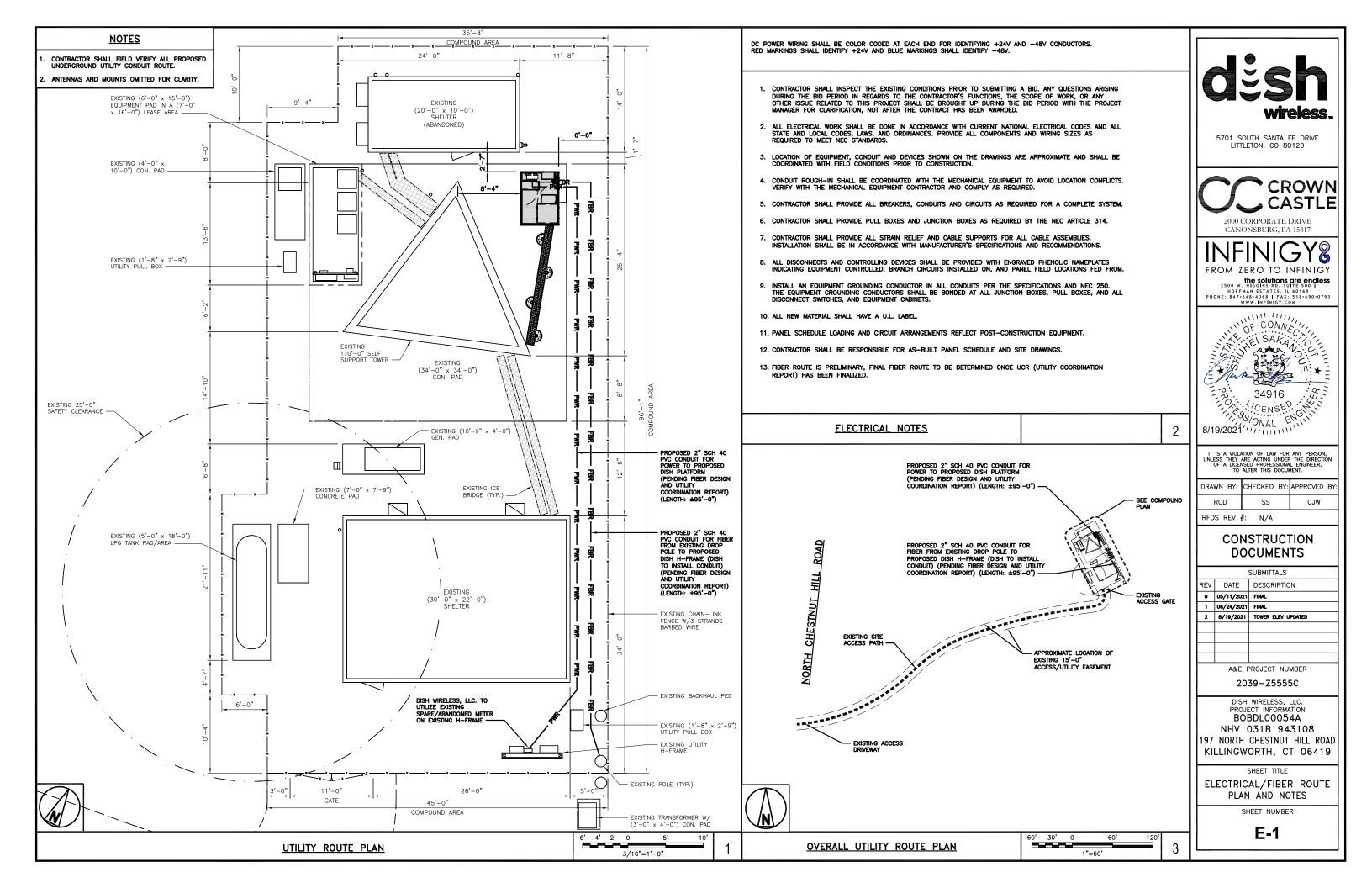
3/4"=1'-0"

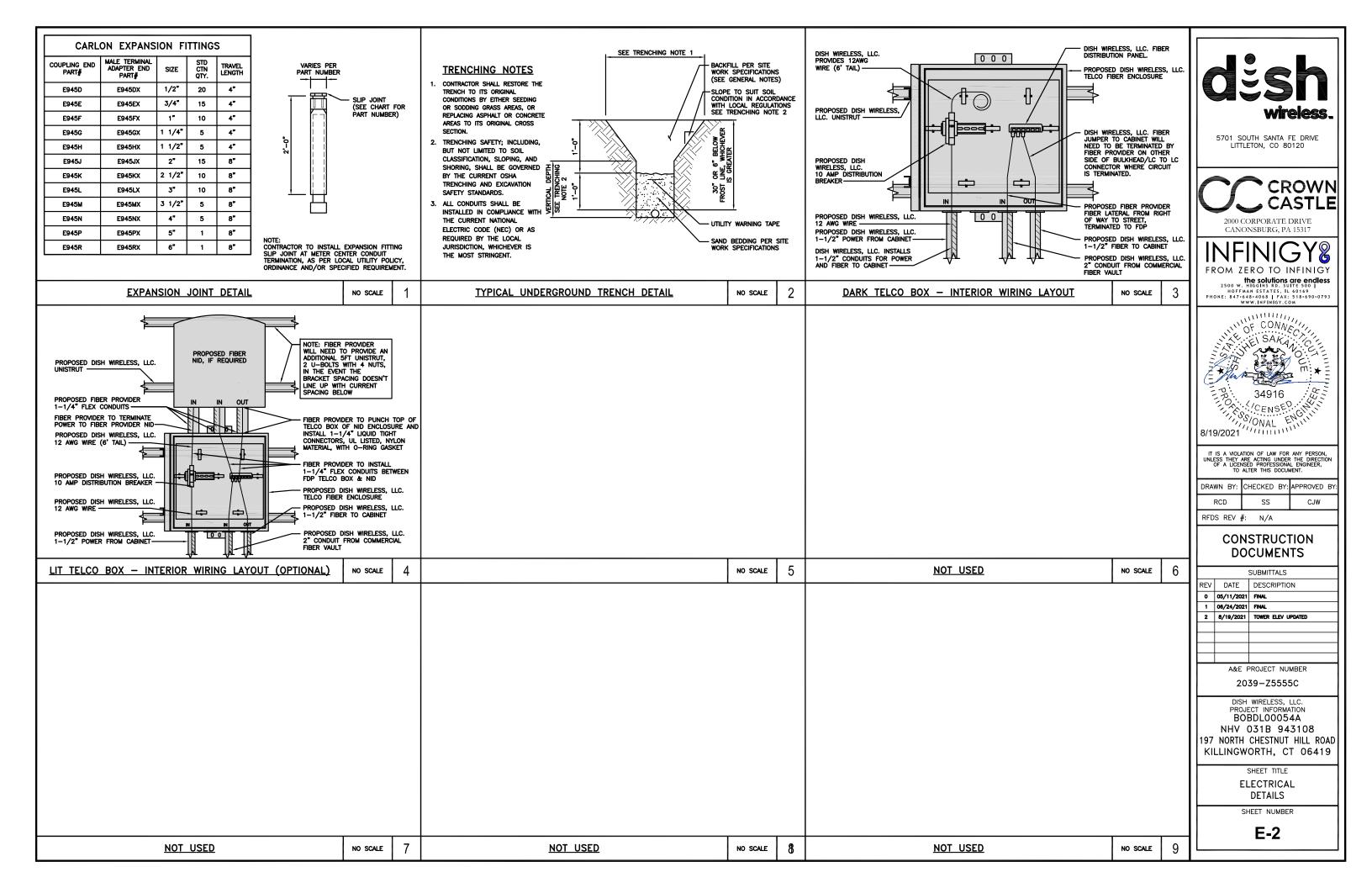


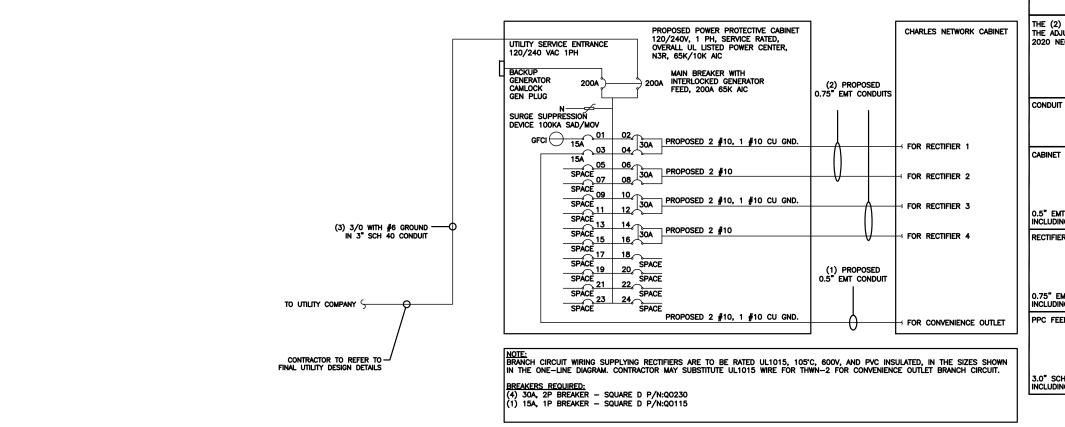












#### **NOTES**

THE (2) CONDUITS WITH (4) CURRENT CARRYING CONDUCTORS EACH, SHALL APPLY THE ADJUSTMENT FACTOR OF 80% PER 2014/17 NEC TABLE 310.15(B)(3)(a) OR 2020 NEC TABLE 310.15(C)(1) FOR UL1015 WIRE.

#12 FOR 15A-20A/1P BREAKER: 0.8 x 30A = 24.0A #10 FOR 25A-30A/2P BREAKER: 0.8 x 40A = 32.0A #8 FOR 35A-40A/2P BREAKER: 0.8 x 55A = 44.0A #6 FOR 45A-60A/2P BREAKER: 0.8 x 75A = 60.0A

CONDUIT SIZING: AT 40% FILL PER NEC CHAPTER 9, TABLE 4, ARTICLE 358. 0.5" CONDUIT - 0.122 SQ. IN AREA 0.75" CONDUIT - 0.213 SQ. IN AREA

2.0" CONDUIT - 1.316 SQ. IN AREA 3.0" CONDUIT - 2.907 SQ. IN AREA

CABINET CONVENIENCE OUTLET CONDUCTORS (1 CONDUIT): USING THWN-2, CU.

#10 - 0.0211 SQ. IN X 2 = 0.0422 SQ. IN #10 - 0.0211 SQ. IN X 1 = 0.0211 SQ. IN <GROUND = 0.0633 SQ. IN

0.5" EMT CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (3) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

RECTIFIER CONDUCTORS (2 CONDUITS): USING UL1015, CU.

#10 - 0.0266 SQ. IN X 4 = 0.1064 SQ. IN #10 - 0.0082 SQ. IN X 1 = 0.0082 SQ. IN <BARE GROUND

= 0.1146 SQ. IN

0.75" EMT CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (5) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

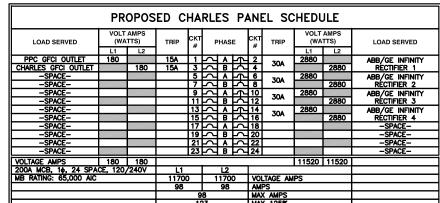
PPC FEED CONDUCTORS (1 CONDUIT): USING THWN, CU.

3/0 - 0.2679 SQ. IN X 3 = 0.8037 SQ. IN #6 - 0.0507 SQ. IN X 1 = 0.0507 SQ. IN <GROUND

TOTAL = 0.8544 SQ. IN

3.0" SCH 40 PVC CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (4) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

PPC ONE-LINE DIAGRAM NO SCALE



PANEL SCHEDULE

5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120



2000 CORPORATE DRIVE CANONSBURG PA 15317

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DOD 22 0.1W	ı	DRAWN BY:	CHECKED BY:	APPROVED BY
RCD 55 CJW		RCD	SS	CJW

RFDS REV #: N/A

#### CONSTRUCTION DOCUMENTS

	SUBMITTALS						
REV	DESCRIPTION						
0 05/11/2021 FINAL		FINAL					
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2039-Z5555C

DISH WIRELESS, LLC. BOBDL00054A NHV 031B 943108 197 NORTH CHESTNUT HILL ROAD KILLINGWORTH, CT 06419

SHEET TITLE

ELECTRICAL ONE-LINE, FAULT CALCS & PANEL SCHEDULE

SHEET NUMBER

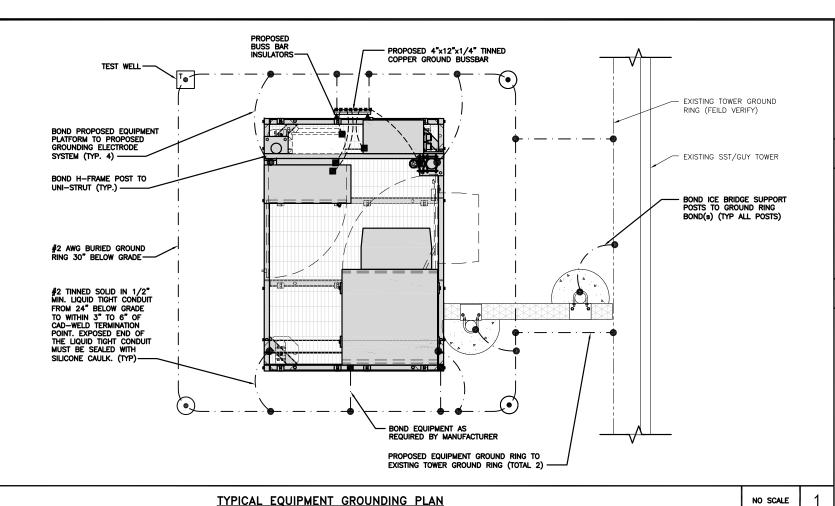
E-3

NO SCALE

2

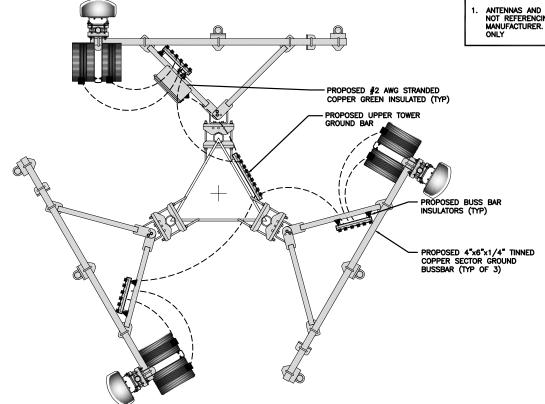
NOT USED

NO SCALE



**NOTES** 

ANTENNAS AND OVP SHOWN ARE GENERIC AND NOT REFERENCING TO A SPECIFIC MANUFACTURER. THIS LAYOUT IS FOR REFERENCE



EXOTHERMIC CONNECTION TEST GROUND ROD WITH MECHANICAL CONNECTION ---- #2 AWG STRANDED & INSULATED GROUND BUS BAR

- · - #2 AWG SOLID COPPER TINNED

#### **GROUNDING LEGEND**

1. GROUNDING IS SHOWN DIAGRAMMATICALLY ONLY.

GROUND ROD

- CONTRACTOR SHALL GROUND ALL EQUIPMENT AS A COMPLETE SYSTEM. GROUNDING SHALL BE IN COMPLIANCE WITH NEC SECTION 250 AND DISH WIRELESS, LLC. GROUNDING AND BONDING REQUIREMENTS AND MANUFACTURER'S SPECIFICATIONS.
- 3. ALL GROUND CONDUCTORS SHALL BE COPPER; NO ALUMINUM CONDUCTORS SHALL BE USED.

#### **GROUNDING KEY NOTES**

- (A) EXTERIOR GROUND RING: #2 AWG SOLID COPPER, BURIED AT A DEPTH OF AT LEAST 30 INCHES BELOW GRADE, OR 6 INCHES BELOW THE FROST LINE AND APPROXIMATELY 24 INCHES FROM THE EXTERIOR WALL OR FOOTING.
- B TOWER GROUND RING: THE GROUND RING SYSTEM SHALL BE INSTALLED AROUND AN ANTENNA TOWER'S LEGS, AND/OR GUY ANCHORS. WHERE SEPARATE SYSTEMS HAVE BEEN BROWNER FOR THE FOUNDATION OF THE FOUNDATION AND/OR GUY ANCHORS. WHERE SEPARATE SYSTEMS HAVE BEEN PROVIDED FOR THE TOWER AND THE BUILDING, AT LEAST TWO BONDS SHALL BE MADE BETWEEN THE TOWER RING GROUND SYSTEM AND THE BUILDING RING GROUND SYSTEM USING MINIMUM #2 AWG SOLID COPPER CONDUCTORS.
- © Interior ground ring: #2 awg stranded green insulated copper conductor extended around the perimeter of the equipment area. All non-telecommunications related metallic objects found within a site shall be grounded to the interior ground ring with #6 awg stranded green
- D BOND TO INTERIOR GROUND RING: #2 AWG SOLID TINNED COPPER WIRE PRIMARY BONDS SHALL BE PROVIDED AT LEAST AT FOUR POINTS ON THE INTERIOR GROUND RING, LOCATED AT THE CORNERS OF THE
- (E) GROUND ROD: UL LISTED COPPER CLAD STEEL. MINIMUM 1/2" DIAMETER BY EIGHT FEET LONG. GROUND RODS SHALL BE INSTALLED WITH INSPECTION SLEEVES. GROUND RODS SHALL BE DRIVEN TO THE DEPTH OF GROUND RING CONDUCTOR.
- F CELL REFERENCE GROUND BAR: POINT OF GROUND REFERENCE FOR ALL COMMUNICATIONS EQUIPMENT FRAMES. ALL BONDS ARE MADE WITH #2 AWG UNLESS NOTED OTHERWISE STRANDED GREEN INSULATED COPPER CONDUCTORS. BOND TO GROUND RING WITH (2) #2 SOLID TINNED COPPER CONDUCTORS.
- G HATCH PLATE GROUND BAR: BOND TO THE INTERIOR GROUND RING WITH TWO #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTORS. WHEN A HATCH-PLATE AND A CELL REFERENCE GROUND BAR ARE BOTH PRESENT, THE CRGB MUST BE CONNECTED TO THE HATCH-PLATE AND TO THE INTERIOR GROUND RING USING (2) TWO #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTORS EACH.
- (H) EXTERIOR CABLE ENTRY PORT GROUND BARS; LOCATED AT THE ENTRANCE TO THE CELL SITE BUILDING, BOND TO GROUND RING WITH A #2 AWG SOLID TINNED COPPER CONDUCTORS WITH AN EXOTHERMIC WELD AND INSPECTION SLEEVE.
- J TELCO GROUND BAR: BOND TO BOTH CELL REFERENCE GROUND BAR OR EXTERIOR GROUND RING.
- K FRAME BONDING: THE BONDING POINT FOR TELECOM EQUIPMENT FRAMES SHALL BE THE GROUND BUS THAT IS NOT ISOLATED FROM THE EQUIPMENTS METAL FRAMEWORK.
- INTERIOR UNIT BONDS: METAL FRAMES, CABINETS AND INDIVIDUAL METALLIC UNITS LOCATED WITH THE AREA OF THE INTERIOR GROUND RING REQUIRE A #6 AWG STRANDED GREEN INSULATED COPPER BOND TO THE
- M FENCE AND GATE GROUNDING: METAL FENCES WITHIN 7 FEET OF THE EXTERIOR GROUND RING OR OBJECTS BONDED TO THE EXTERIOR GROUND RING SHALL BE BONDED TO THE GROUND RING WITH A #2 AWG SOLID TINNED COPPER CONDUCTOR AT AN INTERVAL NOT EXCEEDING 25 FEET. BONDS SHALL BE MADE AT EACH
- N EXTERIOR UNIT BONDS: METALLIC OBJECTS, EXTERNAL TO OR MOUNTED TO THE BUILDING, SHALL BE BONDED TO THE EXTERIOR GROUND RING. USING #2 TINNED SOLID COPPER WIRE
- P ICE BRIDGE SUPPORTS: EACH ICE BRIDGE LEG SHALL BE BONDED TO THE GROUND RING WITH #2 AWG BARE TINNED COPPER CONDUCTOR. PROVIDE EXOTHERMIC WELDS AT BOTH THE ICE BRIDGE LEG AND BURIED
- Q DURING ALL DC POWER SYSTEM CHANGES INCLUDING DC SYSTEM CHANGE OUTS, RECTIFIER REPLACEMENTS OR ADDITIONS, BREAKER DISTRIBUTION CHANGES, BATTERY ADDITIONS, BATTERY REPLACEMENTS AND INSTALLATIONS OR CHANGES TO DC CONVERTER SYSTEMS IT SHALL BE REQUIRED THAT SERVICE CONTRACTORS VERIFY ALL DC POWER SYSTEMS ARE EQUIPPED WITH A MASTER DC SYSTEM RETURN GROUND CONDUCTOR FROM THE DC POWER SYSTEM COMMON RETURN BUS DIRECTLY CONNECTED TO THE CELL SITE REFERENCE (COLUMN) BAR
- (R) TOWER TOP COLLECTOR BUSS BAR IS TO BE MECHANICALLY BONDED TO PROPOSED ANTENNA MOUNT COLLAR. REFER TO DISH WIRELESS, LLC. GROUNDING NOTES.

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DRAWN	BY:	CHECKED	BY:	APPROVED	BY:
RCD		SS		CJW	

RFDS REV #: N/A

#### CONSTRUCTION **DOCUMENTS**

	SUBMITTALS						
REV	/ DATE	DESCRIPTION					
0	05/11/2021	FINAL					
1	06/24/2021	FINAL					
2	8/19/2021	TOWER ELEV UPDATED					
	A&E F	PROJECT NUMBER					

2039-Z5555C

DISH WIRELESS, LLC. PROJECT INFORMATION BOBDL00054A NHV 031B 943108 197 NORTH CHESTNUT HILL ROAD KILLINGWORTH, CT 06419

SHEET TITLE

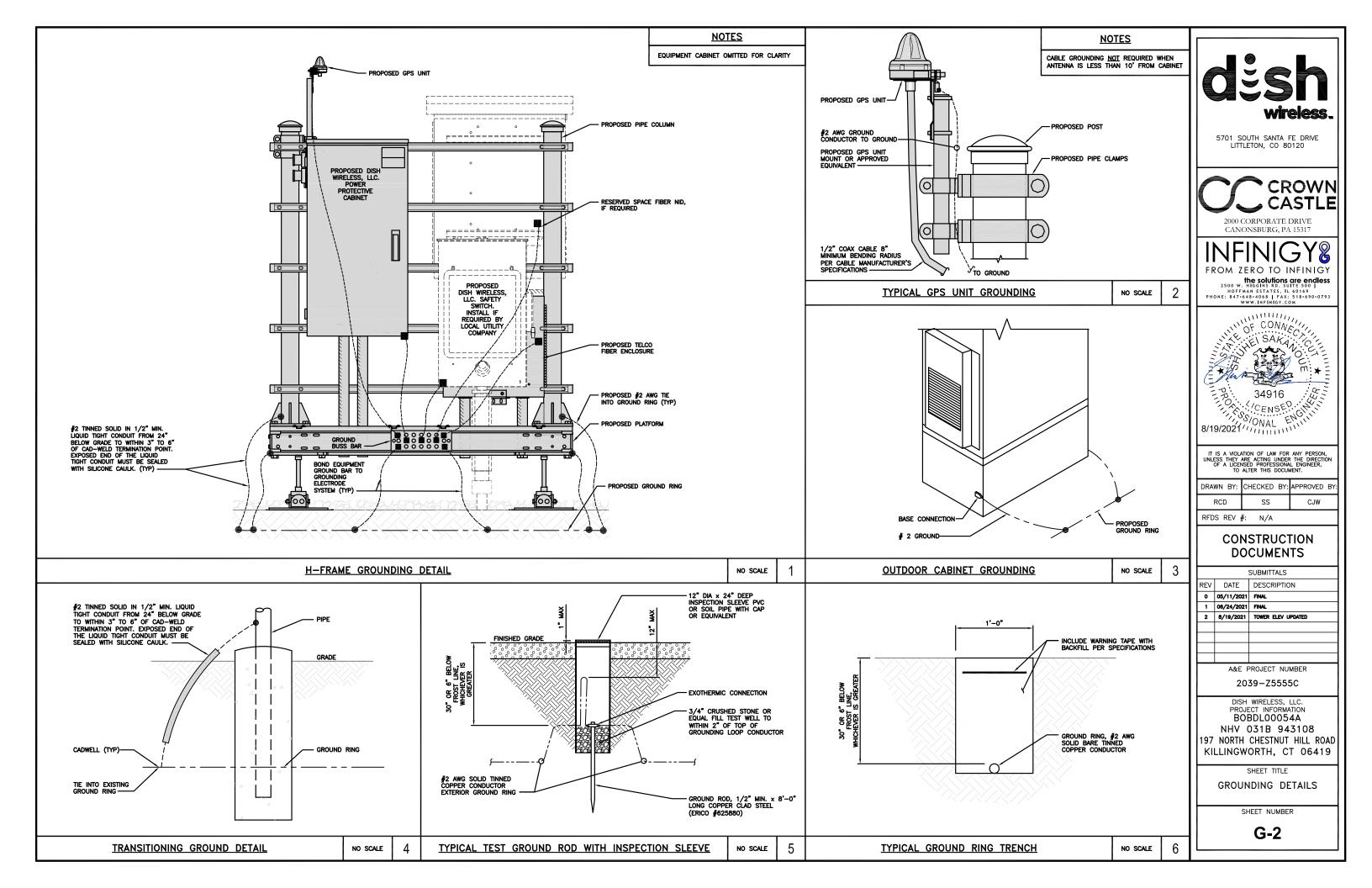
GROUNDING PLANS AND NOTES

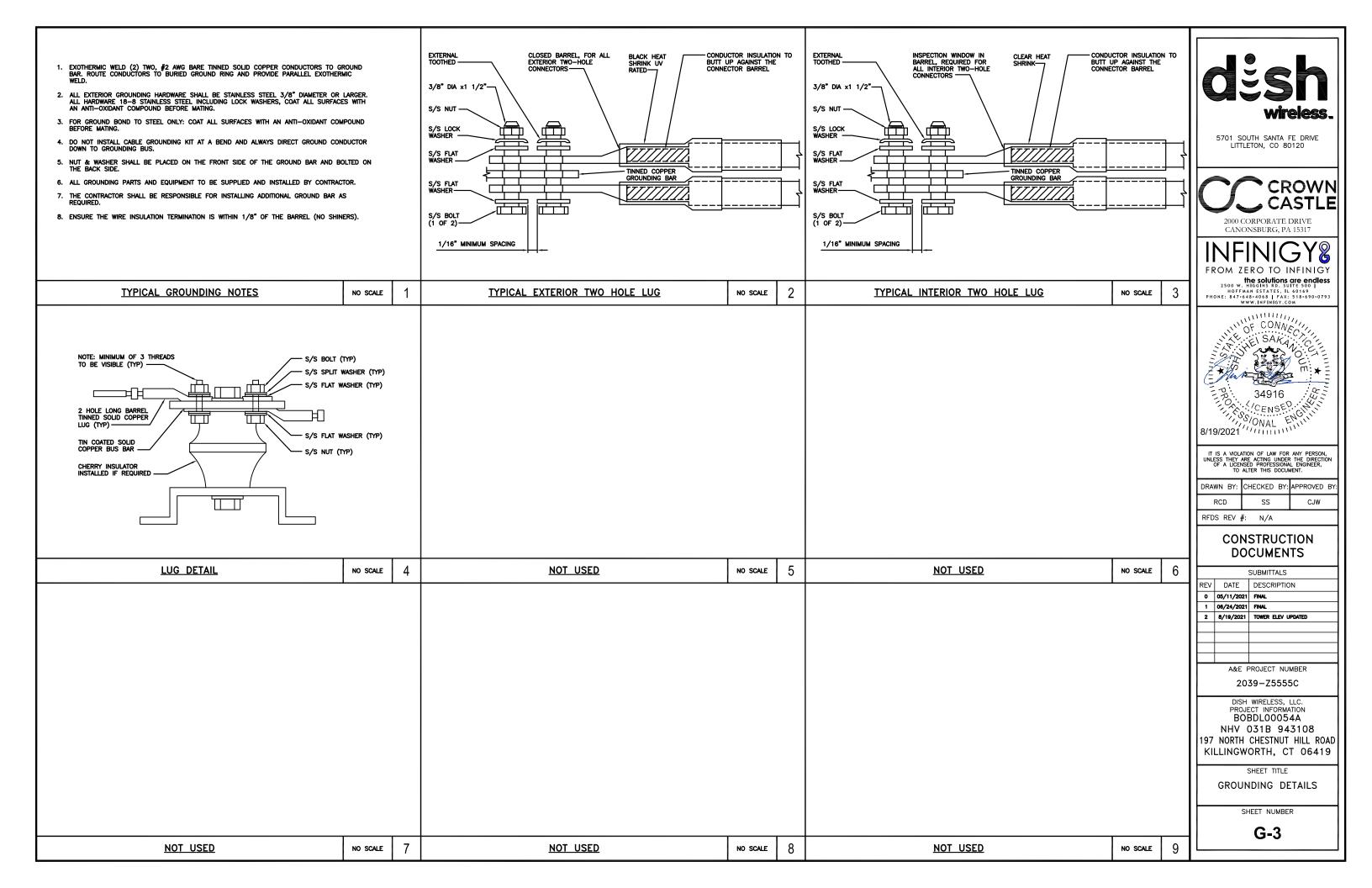
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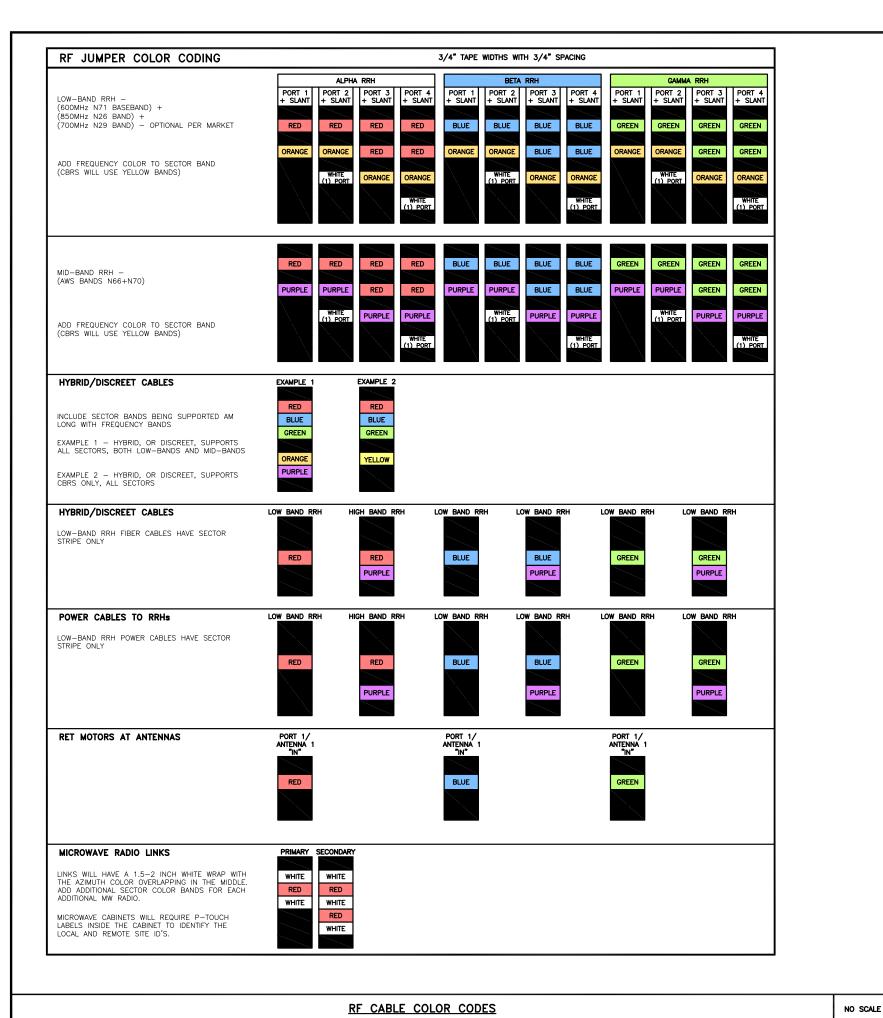
G-1

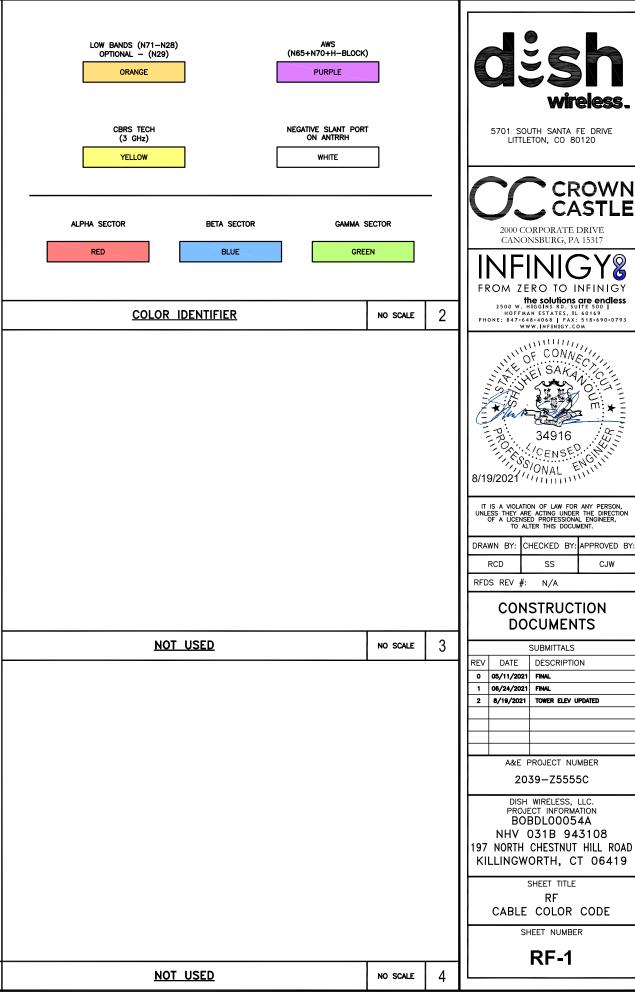
**GROUNDING KEY NOTES** 

NO SCALE





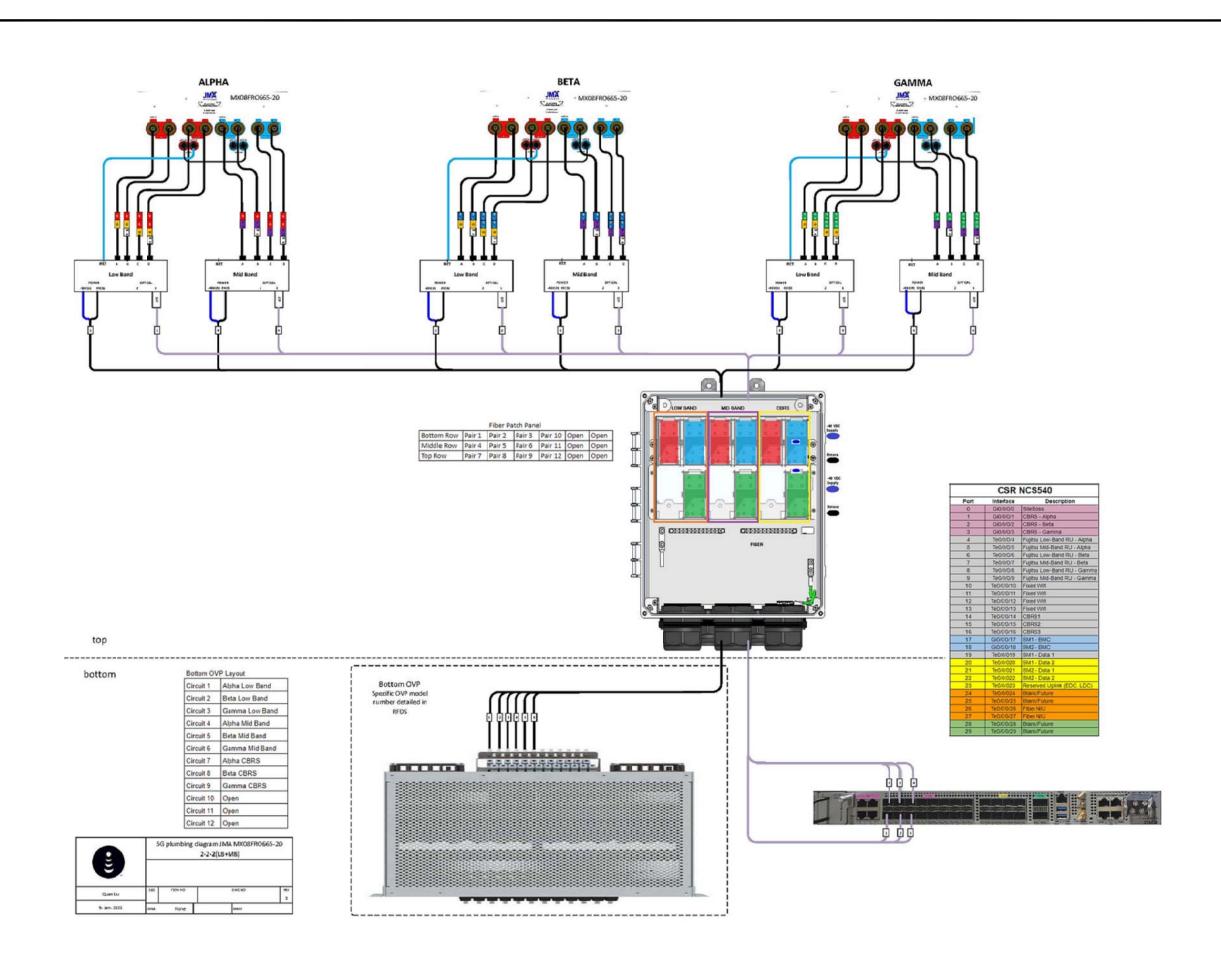




**CROWN** 

CJW

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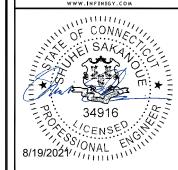
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SHEET TITLE

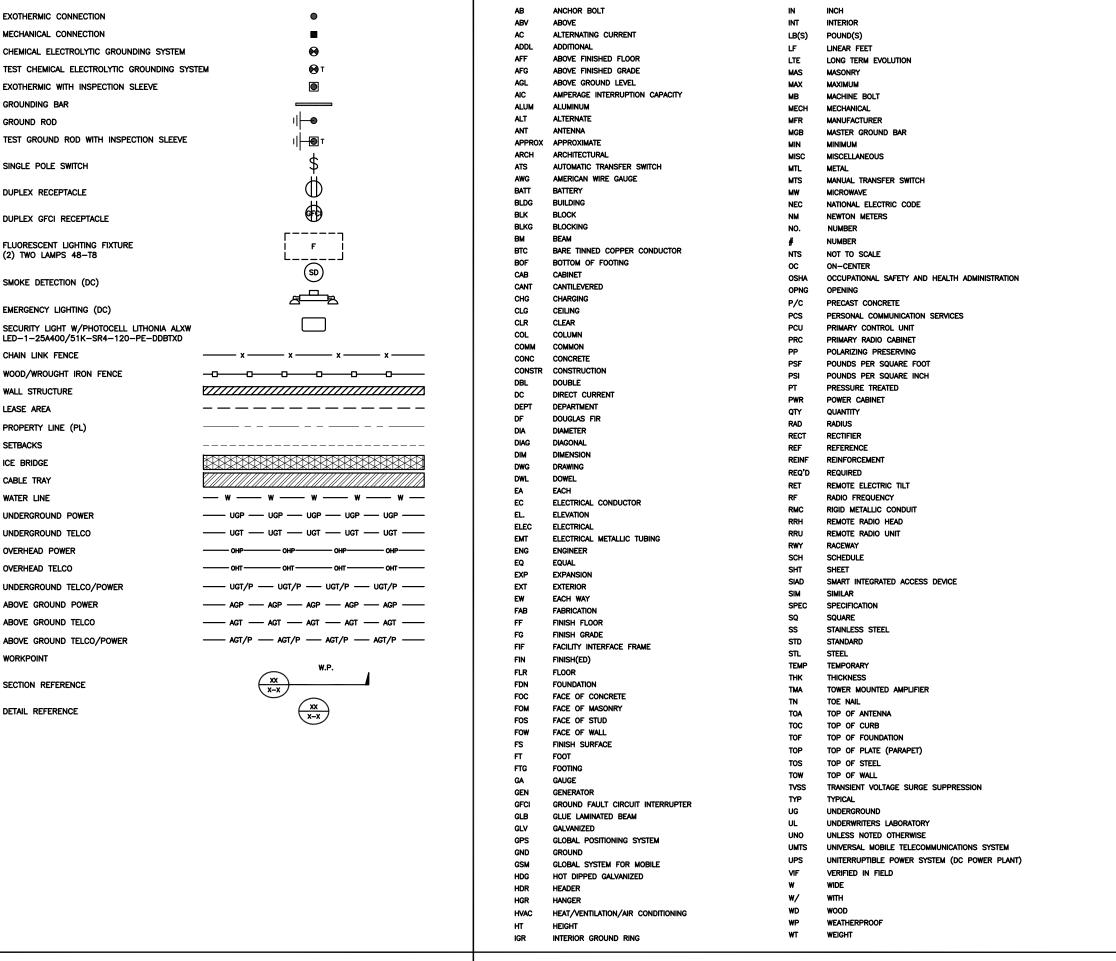
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SHEET NUMBER

RF-2

PLUMBING DIAGRAM

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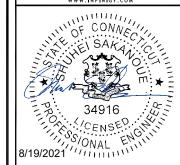
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DRAWN	BY:	CHECKED	BY:	APPROVED	BY:
RCD		SS		CJW	

RFDS REV #: N/A

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DISH WIRELESS, LLC.
PROJECT INFORMATION
BOBDLO0054A
NHV 031B 943108
197 NORTH CHESTNUT HILL ROAD
KILLINGWORTH, CT 06419

SHEET TITLE

LEGEND AND ABBREVIATIONS

SHEET NUMBER

GN-1

**LEGEND** 

**ABBREVIATIONS** 

#### SITE ACTIVITY REQUIREMENTS:

- 1. NOTICE TO PROCEED NO WORK SHALL COMMENCE PRIOR TO CONTRACTOR RECEIVING A WRITTEN NOTICE TO PROCEED (NTP) AND THE ISSUANCE OF A PURCHASE ORDER. PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE DISH WIRELESS, LLC, AND TOWER OWNER NOC & THE DISH WIRELESS, LLC, AND TOWER CONSTRUCTION MANAGER.
- 2. "LOOK UP" DISH WIRELESS, LLC. AND TOWER OWNER SAFETY CLIMB REQUIREMENT:

THE INTEGRITY OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY ON THE STRUCTURE. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO: PINCHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM TS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTIONAL WEAR, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO IMPEDE/BLOCK ITS INTENDED USE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGGED OUT AND REPORTED TO YOUR DISH WIRELESS, LLC. AND DISH WIRELESS, LLC. AND TOWER OWNER POC OR CALL THE NOC TO GENERATE A SAFETY CLIMB MAINTENANCE AND CONTRACTOR NOTICE TICKET.

- 3. PRIOR TO THE START OF CONSTRUCTION, ALL REQUIRED JURISDICTIONAL PERMITS SHALL BE OBTAINED. THIS INCLUDES, BUT IS NOT LIMITED TO, BUILDING, ELECTRICAL, MECHANICAL, FIRE, FLOOD ZONE, ENVIRONMENTAL, AND ZONING. AFTER ONSITE ACTIVITIES AND CONSTRUCTION ARE COMPLETED, ALL REQUIRED PERMITS SHALL BE SATISFIED AND CLOSED OUT ACCORDING TO LOCAL JURISDICTIONAL REQUIREMENTS.
- 4. ALL CONSTRUCTION MEANS AND METHODS; INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS, AND RESCUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONTAINED HEREIN, AND SHALL MEET ANSI/ASSE A10.48 (LATEST EDITION); FEDERAL, STATE, AND LOCAL REGULATIONS; AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL ADHERE TO ANSI/ASSE A10.48 (LATEST EDITION) AND DISH WIRELESS, LLC. AND TOWER OWNER STANDARDS, INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION, TO CERTIFY THE SUPPORTING STRUCTURE(S) IN ACCORDANCE WITH ANSI/TIA-322 (LATEST EDITION).
- 5. ALL SITE WORK TO COMPLY WITH DISH WIRELESS, LLC. AND TOWER OWNER INSTALLATION STANDARDS FOR CONSTRUCTION ACTIVITIES ON DISH WIRELESS, LLC. AND TOWER OWNER TOWER SITE AND LATEST VERSION OF ANSI/TIA-1019-A-2012 "STANDARD FOR INSTALLATION, ALTERATION, AND MAINTENANCE OF ANTENNA SUPPORTING STRUCTURES AND ANTENNAS."
- 6. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY DISH WIRELESS, LLC. AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- 7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 8. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 9. THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES INCLUDING PRIVATE LOCATES SERVICES PRIOR TO THE START OF CONSTRUCTION.
- 10. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION E) CONSTRUCTION SAFETY PROCEDURES.
- 11. ALL SITE WORK SHALL BE AS INDICATED ON THE STAMPED CONSTRUCTION DRAWINGS AND DISH PROJECT SPECIFICATIONS, LATEST APPROVED REVISION.
- 12. CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH AT THE COMPLETION OF THE WORK. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
- 13. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF DISH WIRELESS, LLC. AND TOWER OWNER, AND/OR LOCAL UTILITIES.
- 14. THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE TECHNICAL SPECIFICATION FOR SITE SIGNAGE REQUIRED BY LOCAL JURISDICTION AND SIGNAGE REQUIRED ON INDIVIDUAL PIECES OF EQUIPMENT, ROOMS, AND SHELTERS.
- 15. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE CARRIER'S EQUIPMENT AND TOWER AREAS.
- 16. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
- 17. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION AS SPECIFIED ON THE CONSTRUCTION DRAWINGS AND/OR PROJECT SPECIFICATIONS.
- 18. CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.
- 19. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.
- 20. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS AND RADIOS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 21. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.
- 22. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.

#### **GENERAL NOTES:**

1.FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:

CONTRACTOR: GENERAL CONTRACTOR RESPONSIBLE FOR CONSTRUCTION

CARRIER:DISH WIRELESS, LLC.

TOWER OWNER:TOWER OWNER

- 2. THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE ENGINEERS IN THIS OR SIMILAR LOCALITIES. IT IS ASSUMED THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKPEOPLE WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY STANDARD GOOD PRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.
- 3. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY FOR PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, FORMWORK, SHORING, ETC. SITE VISITS BY THE ENGINEER OR HIS REPRESENTATIVE WILL NOT INCLUDE INSPECTION OF THESE ITEMS AND IS FOR STRUCTURAL OBSERVATION OF THE FINISHED STRUCTURE ONLY.
- 4. NOTES AND DETAILS IN THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE GREATER, MORE STRICT REQUIREMENTS, SHALL GOVERN. IF FURTHER CLARIFICATION IS REQUIRED CONTACT THE ENGINEER OF RECORD.
- 5. SUBSTANTIAL EFFORT HAS BEEN MADE TO PROVIDE ACCURATE DIMENSIONS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.
- 6. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CARRIER POC AND TOWER OWNER.
- 7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 8. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
- 9. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 10. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CARRIER AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION
- 11. CONTRACTOR IS TO PERFORM A SITE INVESTIGATION, BEFORE SUBMITTING BIDS, TO DETERMINE THE BEST ROUTING OF ALL CONDUITS FOR POWER, AND TELCO AND FOR GROUNDING CABLES AS SHOWN IN THE POWER, TELCO, AND GROUNDING PLAN DRAWINGS.
- 12. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF DISH WIRELESS, LLC. AND TOWER OWNER
- 13. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
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ı	DRAWN BY:	CHECKED BY	: APPROVED	BY:
	RCD	SS	CJW	
ı	RFDS REV	#: N/A		

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PROJECT INFORMATION
BOBDL00054A
NHV 031B 943108
197 NORTH CHESTNUT HILL ROAD
KILLINGWORTH, CT 06419

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

GN-2

#### CONCRETE, FOUNDATIONS, AND REINFORCING STEEL:

- 1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST—IN—PLACE CONCRETE.
- 2. UNLESS NOTED OTHERWISE, SOIL BEARING PRESSURE USED FOR DESIGN OF SLABS AND FOUNDATIONS IS ASSUMED TO BE 1000 psf.
- 3. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f'c) OF 3000 psi AT 28 DAYS, UNLESS NOTED OTHERWISE. NO MORE THAN 90 MINUTES SHALL ELAPSE FROM BATCH TIME TO TIME OF PLACEMENT UNLESS APPROVED BY THE ENGINEER OF RECORD. TEMPERATURE OF CONCRETE SHALL NOT EXCEED 90'f AT TIME OF PLACEMENT.
- 4. CONCRETE EXPOSED TO FREEZE-THAW CYCLES SHALL CONTAIN AIR ENTRAINING ADMIXTURES. AMOUNT OF AIR ENTRAINMENT TO BE BASED ON SIZE OF AGGREGATE AND F3 CLASS EXPOSURE (VERY SEVERE). CEMENT USED TO BE TYPE II PORTLAND CEMENT WITH A MAXIMUM WATER-TO-CEMENT RATIO (W/C) OF 0.45.
- 5. ALL STEEL REINFORCING SHALL CONFORM TO ASTM A615. ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185. ALL SPLICES SHALL BE CLASS "B" TENSION SPLICES, UNLESS NOTED OTHERWISE. ALL HOOKS SHALL BE STANDARD 90 DEGREE HOOKS, UNLESS NOTED OTHERWISE. YIELD STRENGTH (Fy) OF STANDARD DEFORMED BARS ARE AS FOLLOWS:

#4 BARS AND SMALLER 40 ksi

#5 BARS AND LARGER 60 ksi

- 6. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:
- CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3"
- · CONCRETE EXPOSED TO EARTH OR WEATHER:
- #6 BARS AND LARGER 2"
- #5 BARS AND SMALLER 1-1/2"
- · CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
- SLAB AND WALLS 3/4"
- BEAMS AND COLUMNS 1-1/2\*
- 7. A TOOLED EDGE OR A 3/4" CHAMFER SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNLESS NOTED OTHERWISE, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.

#### **ELECTRICAL INSTALLATION NOTES:**

- 1. ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES/ORDINANCES.
- 2. CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED AND TRIP HAZARDS ARE ELIMINATED.
- 3. WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC.
- . ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC.
- 4.1. ALL EQUIPMENT SHALL BEAR THE UNDERWRITERS LABORATORIES LABEL OF APPROVAL, AND SHALL CONFORM TO REQUIREMENT OF THE NATIONAL ELECTRICAL CODE.
- 4.2. ALL OVERCURRENT DEVICES SHALL HAVE AN INTERRUPTING CURRENT RATING THAT SHALL BE GREATER THAN THE SHORT CIRCUIT CURRENT TO WHICH THEY ARE SUBJECTED, 22,000 AIC MINIMUM. VERIFY AVAILABLE SHORT CIRCUIT CURRENT DOES NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRENT ADOPTED CODE PRE THE GOVERNING JURISDICTION.
- 5. EACH END OF EVERY POWER PHASE CONDUCTOR, GROUNDING CONDUCTOR, AND TELCO CONDUCTOR OR CABLE SHALL BE LABELED WITH COLOR—CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2" PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA.
- 6. ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH LAMICOID TAGS SHOWING THEIR RATED VOLTAGE, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING AND BRANCH CIRCUIT ID NUMBERS (i.e. PANEL BOARD AND CIRCUIT ID'S).
- 7. PANEL BOARDS (ID NUMBERS) SHALL BE CLEARLY LABELED WITH PLASTIC LABELS.
- 8. TIE WRAPS ARE NOT ALLOWED.
- 9. ALL POWER AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE COPPER CONDUCTOR (#14 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 10. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE COPPER CONDUCTOR (#6 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 11. POWER AND CONTROL WIRING IN FLEXIBLE CORD SHALL BE MULTI-CONDUCTOR, TYPE SOOW CORD (#14 OR LARGER) UNLESS OTHERWISE SPECIFIED.
- 12. POWER AND CONTROL WIRING FOR USE IN CABLE TRAY SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#14 OR LARGER), WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 13. ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRE NUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRE NUTS SHALL BE RATED FOR OPERATION NOT LESS THAN 75° C (90° C IF AVAILABLE).
- 14. RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.
- 15. ELECTRICAL METALLIC TUBING (EMT), INTERMEDIATE METAL CONDUIT (IMC), OR RIGID METAL CONDUIT (RMC) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.

- ELECTRICAL METALLIC TUBING (EMT) OR METAL-CLAD CABLE (MC) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.
- 17. SCHEDULE 40 PVC UNDERGROUND ON STRAIGHTS AND SCHEDULE 80 PVC FOR ALL ELBOWS/90s AND ALL APPROVED ABOVE GRADE PVC CONDUIT.
- 18. LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.
- 19. CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION-TYPE AND APPROVED FOR THE LOCATION USED. SET SCREW FITTINGS ARE NOT ACCEPTABLE.
- 20. CABINETS, BOXES AND WIRE WAYS SHALL BE LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND THE NEC.
- 21. WIREWAYS SHALL BE METAL WITH AN ENAMEL FINISH AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARDS (WIREMOLD SPECMATE WIREWAY).
- 22. SLOTTED WIRING DUCT SHALL BE PVC AND INCLUDE COVER (PANDUIT TYPE E OR EQUAL).
- 23. CONDUITS SHALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-PERFORATED STRAPS AND HANGERS. EXPLOSIVE DEVICES (i.e. POWDER-ACTUATED) FOR ATTACHING HANGERS TO STRUCTURE WILL NOT BE PERMITTED. CLOSELY FOLLOW THE LINES OF THE STRUCTURE, MAINTAIN CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES. CHANGES IN DIRECTION TO ROUTE AROUND OBSTACLES SHALL BE MADE WITH CONDUIT OUTLET BODIES. CONDUIT SHALL BE INSTALLED IN A NEAT AND WORKMANLIKE MANNER. PARALLEL AND PERPENDICULAR TO STRUCTURE WALL AND CEILING LINES. ALL CONDUIT SHALL BE FISHED TO CLEAR OBSTRUCTIONS. ENDS OF CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED MALLEABLE IRON BUSHING ON INSIDE AND GALVANIZED MALLEABLE IRON LOCKNUT ON OUTSIDE AND INSIDE.
- 24. EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES AND PULL BOXES SHALL BE GALVANIZED OR EPOXY—COATED SHEET STEEL. SHALL MEET OR EXCEED UL 50 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND NEMA 3 (OR BETTER) FOR EXTERIOR LOCATIONS.
- 25. METAL RECEPTACLE, SWITCH AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY-COATED OR NON-CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- 26. NONMETALLIC RECEPTACLE, SWITCH AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2 (NEWEST REVISION) AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- 27. THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CARRIER AND/OR DISH WIRELESS, LLC. AND TOWER OWNER BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.
- 28. THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD LIFE AND PROPERTY.
- 29. INSTALL LAMICOID LABEL ON THE METER CENTER TO SHOW "DISH WIRELESS, LLC.".
- 30. ALL EMPTY/SPARE CONDUITS THAT ARE INSTALLED ARE TO HAVE A METERED MULE TAPE PULL CORD INSTALLED.



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DRAWN BY: CHECKED BY: APPROVED BY

RCD SS CJW

RFDS REV #: N/A

### CONSTRUCTION DOCUMENTS

2039-Z5555C

DISH WIRELESS, LLC.
PROJECT INFORMATION
BOBDLO0054A
NHV 031B 943108
197 NORTH CHESTNUT HILL ROAD
KILLINGWORTH, CT 06419

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

GN-3

#### **GROUNDING NOTES:**

- 1. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION AND AC POWER GES'S) SHALL BE BONDED TOGETHER AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
- 2. THE CONTRACTOR SHALL PERFORM IEEE FALL—OF—POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS, THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT AND PROVIDE TESTING RESULTS.
- 4. METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- 5. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
- 6. EACH CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 STRANDED COPPER OR LARGER FOR INDOOR BTS; #2 BARE SOLID TINNED COPPER FOR OUTDOOR BTS.
- 7. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED BACK TO BACK CONNECTIONS ON OPPOSITE SIDE OF THE GROUND BUS ARE PERMITTED.
- 8. ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING SHALL BE #2 SOLID TINNED COPPER UNLESS OTHERWISE INDICATED.
- 9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
- 10. USE OF 90° BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45° BENDS CAN BE ADEQUATELY SUPPORTED.
- 11. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
- 12. ALL GROUND CONNECTIONS ABOVE GRADE (INTERIOR AND EXTERIOR) SHALL BE FORMED USING HIGH PRESS CRIMPS.
- 13. COMPRESSION GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.
- 14. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR.
- 15. APPROVED ANTIOXIDANT COATINGS (i.e. CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- 16. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.
- 17. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
- 18. BOND ALL METALLIC OBJECTS WITHIN 6 ft OF MAIN GROUND RING WITH (1) #2 BARE SOLID TINNED COPPER GROUND CONDUCTOR.
- 19. GROUND CONDUCTORS USED FOR THE FACILITY GROUNDING AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (i.e., NONMETALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT.
- 20. ALL GROUNDS THAT TRANSITION FROM BELOW GRADE TO ABOVE GRADE MUST BE #2 BARE SOLID TINNED COPPER IN 3/4" NON-METALLIC, FLEXIBLE CONDUIT FROM 24" BELOW GRADE TO WITHIN 3" TO 6" OF CAD-WELD TERMINATION POINT. THE EXPOSED END OF THE CONDUIT MUST BE SEALED WITH SILICONE CAULK. (ADD TRANSITIONING GROUND STANDARD DETAIL AS WELL).
- 21. BUILDINGS WHERE THE MAIN GROUNDING CONDUCTORS ARE REQUIRED TO BE ROUTED TO GRADE, THE CONTRACTOR SHALL ROUTE TWO GROUNDING CONDUCTORS FROM THE ROOFTOP, TOWERS, AND WATER TOWERS GROUNDING RING, TO THE EXISTING GROUNDING SYSTEM, THE GROUNDING CONDUCTORS SHALL NOT BE SMALLER THAN 2/O COPPER. ROOFTOP GROUNDING RING SHALL BE BONDED TO THE EXISTING GROUNDING SYSTEM, THE BUILDING STEEL COLUMNS, LIGHTNING PROTECTION SYSTEM, AND BUILDING MAIN WATER LINE (FERROUS OR NONFERROUS METAL PIPING ONLY). DO NOT ATTACH GROUNDING TO FIRE SPRINKLER SYSTEM PIPES.



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2000 CORPORATE DRIVE CANONSBURG, PA 15317

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П			•
	RCD	SS	CJW
	DRAWN BY:	CHECKED B	r: APPROVED B

RFDS REV #: N/A

## CONSTRUCTION DOCUMENTS

	:	SUBMITTALS		
REV	DATE	DESCRIPTION		
0	05/11/2021	FINAL		
1	06/24/2021	FINAL		
2	8/19/2021	TOWER ELEV UPDATED		
	A SAE DECLECT NUMBER			

A&E PROJECT NUMBER

2039-Z5555C

DISH WIRELESS, LLC.
PROJECT INFORMATION
BOBDL00054A
NHV 031B 943108
197 NORTH CHESTNUT HILL ROAD
KILLINGWORTH, CT 06419

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

GN-4

# Exhibit D

**Structural Analysis Report** 

Date: April 08, 2021



Crown Castle 2000 Corporate Drive Canonsburg, PA 15317 (724) 416-2000

Subject: Structural Analysis Report

Carrier Designation: DISH Network Co-Locate

Site Number: BOBDL00054A Site Name: CT-CCI-T-807134

Crown Castle Designation: BU Number: 807134

**Site Name:** NHV 031B 943108

 JDE Job Number:
 645664

 Work Order Number:
 1945874

 Order Number:
 553397 Rev. 0

Engineering Firm Designation: Crown Castle Project Number: 1945874

Site Data: 197 North Chestnut Hill Road, KILLINGWORTH, Middlesex County, CT

Latitude 41° 23′ 9.93″, Longitude -72° 36′ 14.39″

170 Foot - Self Support Tower

Crown Castle is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Proposed Equipment Configuration

Sufficient Capacity - 58.4%

This analysis utilizes an ultimate 3-second gust wind speed of 130 mph as required by the 2018 Connecticut State Building Code and Appendix N. Applicable Standard references and design criteria are listed in Section 2 - "Analysis Criteria".

Structural analysis prepared by: Jared Koski

Respectfully submitted by:

Jamal A. Huwel, P.E. Director Engineering

Date Signed: 04/12/2021

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#### 1) INTRODUCTION

This tower is a 170 ft Self Support tower designed by Rohn.

The tower has been modified per reinforcement drawings prepared by GPD. Reinforcement consists of replacing diagonal bolts between elevations 0' and 20' with higher grade bolts.

#### 2) ANALYSIS CRITERIA

TIA-222 Revision: TIA-222-H

Risk Category:

Wind Speed: 130 mph

Exposure Category:BTopographic Factor:1Ice Thickness:1.5 inWind Speed with Ice:50 mphService Wind Speed:60 mph

**Table 1 - Proposed Equipment Configuration** 

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
	148.0	3	jma wireless	MX08FRO665-20 w/ Mount Pipe	1	1-1/2
		3	fujitsu	TA08025-B604		
148.0		3	fujitsu	TA08025-B605		
		1	raycap	RDIDC-9181-PF-48		
		1	tower mounts	Commscope MTC3975083 (3)		

Table 2 - Non-Carrier Equipment To Be Removed

	unting vel (ft)	Flevation	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
1	146.0	147.0	12	decibel	DB844H90E-SX w/Mount Pipe	12	1-5/8
1		146.0	1	tower mounts	Sector Mount [SM 602-3]		

**Table 3 - Other Considered Equipment** 

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
	171.0	3	antel	BXA-70063-6CF-EDIN-0 w/ Mount Pipe		
170.0		6	antel	LPA-171063-12CF-EDIN-2 w/ Mount Pipe	18	1-5/8
		6	antel	LPA-80063/6CF w/ Mount Pipe		
	170.0	1	tower mounts	Sector Mount [SM 514-3]		
135.0	136.0	136.0	kmw communications	AM-X-CD-16-65-00T-RET w/ Mount Pipe	1 2	3/8 7/16
135.0		6	powerwave technologies	7770.00	12 1	1-1/4 Conduit

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
		1	powerwave technologies	P65-16-XLH-RR w/ Mount Pipe		
		1	powerwave technologies	P65-17-XLH-RR w/ Mount Pipe		
		3	ericsson	RRUS-11		
		6	powerwave technologies	LGP21401		
	135.0	6	powerwave technologies	LGP21901		
		1	raycap	DC6-48-60-18-8F		
		1	tower mounts	Sector Mount [SM 402-3]		

#### 3) ANALYSIS PROCEDURE

**Table 4 - Documents Provided** 

Document	Reference	Source
4-GEOTECHNICAL REPORTS	258466	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	301178	CCISITES
4-TOWER MANUFACTURER DRAWINGS	258477	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	2303423	CCISITES
4-POST-MODIFICATION INSPECTION	2354059	CCISITES

#### 3.1) Analysis Method

tnxTower (version 8.0.7.5), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

#### 3.2) Assumptions

- 1) Tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

#### 4) ANALYSIS RESULTS

**Table 5 - Section Capacity (Summary)** 

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	170 - 160	Leg	ROHN 2.5 STD	2	-8.57	60.05	14.3	Pass
T2	160 - 140	Leg	ROHN 2.5 EH	21	-29.67	61.44	48.3	Pass
Т3	140 - 120	Leg	ROHN 3 EH	45	-57.86	99.05	58.4	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T4	120 - 100	Leg	ROHN 4 EH	66	-86.73	167.91	51.7	Pass
T5	100 - 80	Leg	ROHN 5 EH	87	-113.21	211.29	53.6	Pass
Т6	80 - 60	Leg	ROHN 6 EHS	102	-142.54	256.27	55.6	Pass
T7	60 - 40	Leg	ROHN 6 EH	117	-171.64	318.90	53.8	Pass
Т8	40 - 20	Leg	ROHN 8 EHS	132	-200.74	405.72	49.5	Pass
Т9	20 - 0	Leg	ROHN 8 EHS	147	-230.01	405.72	56.7	Pass
T1	170 - 160	Diagonal	L 2 x 2 x 3/16	10	-2.95	10.33	28.6 40.9 (b)	Pass
T2	160 - 140	Diagonal	L2 1/2x2 1/2x1/4	28	-4.50	16.20	27.8 35.0 (b)	Pass
Т3	140 - 120	Diagonal	L2 1/2x2 1/2x1/4	49	-5.90	12.43	47.5	Pass
T4	120 - 100	Diagonal	L3x3x1/4	70	-6.92	17.34	39.9 46.1 (b)	Pass
T5	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	91	-8.33	18.92	44.0 55.1 (b)	Pass
T6	T6 80 - 60 Diagonal		L4x4x5/16	106	-9.38	29.62	31.7 49.7 (b)	Pass
Т7	60 - 40	Diagonal	L4x4x5/16	121	-9.77	24.78	39.4 51.9 (b)	Pass
Т8	40 - 20	Diagonal	L4x4x5/16	136	-10.86	21.56	50.4 57.1 (b)	Pass
Т9			L4x4x3/8	151	-11.75	22.00	53.4	Pass
T1	T1 170 - 160 Top Girt		L2 1/2x2 1/2x3/16	4	-1.15	6.92	16.7	Pass
T2	T2 160 - 140 Top Girt		L2 1/2x2 1/2x3/16	22	-0.23	6.92	3.3	Pass
							Summary	
						Leg (T3)	58.4	Pass
						Diagonal (T8)	57.1	Pass
						Top Girt (T1)	16.7	Pass
						Bolt Checks	57.1	Pass
						Rating =	58.4	Pass

Table 6 - Tower Component Stresses vs. Capacity - LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	18.3	Pass
1	Base Foundation (Structure)	0	19.7	Pass
1	Base Foundation (Soil Interaction)	0	44.9	Pass

Structure Rating (max from all components) =	58.4%
--	-------

Notes:

#### 4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

<sup>1)</sup> See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

# APPENDIX A TNXTOWER OUTPUT

		T8		Т6	15	T4	Т3	12	1
ROHN 8 EHS	S EHS		ROHN 6 EH	ROHN 6 EHS	ROHN 5 EH	ROHN 4 EH	ROHN 3 EH	ROHN 2.5 EH	∢
					A572-50				
L4x4x3/8			L4x4x5/16		L3 1/2x3 1/2x1/4	L3x3x1/4	L2 1/2x2 1/2x1/4	1/2x1/4	L2x2x3/16
			A572-50	50				A36	
				N.A.				L2 1/2x2 1/2x3/16	3/16
23	8		21 18.8542	16.8542	14.7708	12.7604	10.6875		8.64583
	-		10 @ 10				9 @ 6.66667		2 @ 5
5.3	4.6		4.1	3.5	2.7	2.4	1.7	1.5	0.6
20.0 ft	20.0 ft		60.0 ft	80.0 ft	100.0 ft			140.0 ft_	170.0 ft

SYMBOL LIST

MARK	SIZE	MARK	SIZE
Α	ROHN 2.5 STD		

**MATERIAL STRENGTH** 

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

#### **TOWER DESIGN NOTES**

- Tower is located in Middlesex County, Connecticut.
   Tower designed for Exposure B to the TIA-222-H Standard.
   Tower designed for a 130 mph basic wind in accordance with the TIA-222-H Standard.
   Tower is also designed for a 50 mph basic wind with 1.50 in ice. Ice is considered to increase in thickness with height.
- 5. Deflections are based upon a 60 mph wind.
- 6. Tower Risk Category II.7. Topographic Category 1 with Crest Height of 0.00 ft8. TOWER RATING: 58.4%

ALL REACTIONS ARE FACTORED

 $\triangle$ 

MAX. CORNER REACTIONS AT BASE:

DOWN: 237 K SHEAR: 29 K

UPLIFT: -195 K SHEAR: 25 K

AXIAL 125 K SHEAR MOMENT 12 K ∫ 1241 kip-ft

TORQUE 21 kip-ft 50 mph WIND - 1.5000 in ICE

AXIAL 50 K SHEAR MOMENT 48 K ∫ 4778 kip-ft

TORQUE 67 kip-ft REACTIONS - 130 mph WIND



BU# 807134		
oject:		
ent: Crown Castle	Drawn by: JKoski	App'd:
ode: TIA-222-H	Date: 04/08/21	Scale: NTS
ith: C:\Users\jkoski\Desktop\Work Area\807	7134\WO 1945874 - SA\Prod\807134.eri	Dwg No. E-1

#### **Tower Input Data**

The main tower is a 3x free standing tower with an overall height of 170.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 8.65 ft at the top and 25.00 ft at the base.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

- 3) Tower is located in Middlesex County, Connecticut.
- 4) Tower base elevation above sea level: 382.00 ft.
- 5) Basic wind speed of 130 mph.
- 6) Risk Category II.
- 7) Exposure Category B.
- 8) Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- 9) Topographic Category: 1.
- 10) Crest Height: 0.00 ft.
- 11) Nominal ice thickness of 1.5000 in.
- 12) Ice thickness is considered to increase with height.
- 13) Ice density of 56 pcf.
- 14) A wind speed of 50 mph is used in combination with ice.
- 15) Temperature drop of 50 °F.
- 16) Deflections calculated using a wind speed of 60 mph.
- 17) A non-linear (P-delta) analysis was used.
- 18) Pressures are calculated at each section.
- 19) Tower analysis based on target reliabilities in accordance with Annex S.
- 20) Load Modification Factors used:  $K_{es}(F_w) = 0.95$ ,  $K_{es}(t_i) = 0.85$ .
- 21) Stress ratio used in tower member design is 1.05.
- 22) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

#### **Options**

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification Use Code Stress Ratios

- ✓ Use Code Safety Factors Guys Escalate Ice
   Always Use Max Kz
   Use Special Wind Profile
- √ Include Bolts In Member Capacity

Leg Bolts Are At Top Of Section

√ Secondary Horizontal Braces Leg
Use Diamond Inner Bracing (4 Sided)
SR Members Have Cut Ends
SR Members Are Concentric

Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area
- √ Use Clear Spans For KL/r
  Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt.

Autocalc Torque Arm Areas

Add IBC .6D+W Combination

✓ Sort Capacity Reports By Component
Triangulate Diamond Inner Bracing
Treat Feed Line Bundles As Cylinder
Ignore KL/ry For 60 Deg. Angle Legs

Use ASCE 10 X-Brace Ly Rules

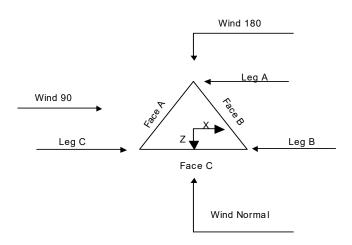
- √ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA
- √ ŠR Leg Bolts Resist Compression
  All Leg Panels Have Same Allowable
  Offset Girt At Foundation
- √ Consider Feed Line Torque

Exemption

√ Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice

#### Poles

Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known



Triangular Tower

Tower	Section	Geometry
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Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of	Section Length
					Sections	J
	ft			ft		ft
T1	170.00-160.00			8.65	1	10.00
T2	160.00-140.00			8.65	1	20.00
T3	140.00-120.00			10.69	1	20.00
T4	120.00-100.00			12.76	1	20.00
T5	100.00-80.00			14.77	1	20.00
T6	80.00-60.00			16.85	1	20.00
T7	60.00-40.00			18.85	1	20.00
T8	40.00-20.00			21.00	1	20.00
Т9	20.00-0.00			23.00	1	20.00

### **Tower Section Geometry** (cont'd)

Tower	Tower	Diagonal	Bracing	Has	Has	Top Girt	Bottom Girt
Section	Elevation	Spacing	Type	K Brace End	Horizontals	Offset	Offset
	ft	ft		Panels		in	in
T1	170.00-160.00	5.00	X Brace	No	Yes	0.0000	0.0000
T2	160.00-140.00	6.67	X Brace	No	Yes	0.0000	0.0000
Т3	140.00-120.00	6.67	X Brace	No	No	0.0000	0.0000
T4	120.00-100.00	6.67	X Brace	No	No	0.0000	0.0000
T5	100.00-80.00	10.00	X Brace	No	No	0.0000	0.0000
T6	80.00-60.00	10.00	X Brace	No	No	0.0000	0.0000
T7	60.00-40.00	10.00	X Brace	No	No	0.0000	0.0000
T8	40.00-20.00	10.00	X Brace	No	No	0.0000	0.0000
Т9	20.00-0.00	10.00	X Brace	No	No	0.0000	0.0000

# Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 170.00-	Pipe	ROHN 2.5 STD	A572-50	Single Angle	L 2 x 2 x 3/16	A36

Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
ft						
160.00			(50 ksi)			(36 ksi)
T2 160.00-	Pipe	ROHN 2.5 EH	A572-50	Single Angle	L2 1/2x2 1/2x1/4	A36
140.00			(50 ksi)			(36 ksi)
T3 140.00-	Pipe	ROHN 3 EH	A572-50	Single Angle	L2 1/2x2 1/2x1/4	A36
120.00			(50 ksi)			(36 ksi)
T4 120.00-	Pipe	ROHN 4 EH	A572-50	Single Angle	L3x3x1/4	A572-50
100.00			(50 ksi)			(50 ksi)
T5 100.00-	Pipe	ROHN 5 EH	A572-50	Single Angle	L3 1/2x3 1/2x1/4	A572-50
80.00			(50 ksi)			(50 ksi)
T6 80.00-60.00	Pipe	ROHN 6 EHS	A572-50	Single Angle	L4x4x5/16	A572-50
			(50 ksi)			(50 ksi)
T7 60.00-40.00	Pipe	ROHN 6 EH	A572-50	Single Angle	L4x4x5/16	A572-50
			(50 ksi)			(50 ksi)
T8 40.00-20.00	Pipe	ROHN 8 EHS	A572-50	Single Angle	L4x4x5/16	A572-50
			(50 ksi)			(50 ksi)
T9 20.00-0.00	Pipe	ROHN 8 EHS	A572-50	Single Angle	L4x4x3/8	A572-50
			(50 ksi)			(50 ksi)

Tower Section Geometry (cont'd)											
Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade					
T1 170.00- 160.00	Single Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)					
T2 160.00- 140.00	Single Angle	L2 1/2x2 1/2x3/16	`A36 <sup>′</sup> (36 ksi)	Solid Round		`A36 <sup>′</sup> (36 ksi)					

		1	Tower Se	ection Ge	ometi	<b>ry</b> (cont'c	<u>()</u>		
Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A <sub>f</sub>	Adjust. Factor Ar	Weight Mult.	Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Stitch Bolt Spacing Redundants
ft	ft <sup>2</sup>	in					in	in	in
T1 170.00- 160.00	0.00	0.0000	A36 (36 ksi)	1.05	1	1.05	36.0000	36.0000	36.0000
T2 160.00- 140.00	0.00	0.0000	A36 (36 ksi)	1.05	1	1.05	36.0000	36.0000	36.0000
T3 140.00- 120.00	0.00	0.0000	`A36 ´ (36 ksi)	1.05	1	1.05	36.0000	36.0000	36.0000
T4 120.00- 100.00	0.00	0.0000	`A36 ´ (36 ksi)	1.05	1	1.05	36.0000	36.0000	36.0000
T5 100.00- 80.00	0.00	0.0000	A36 (36 ksi)	1.05	1	1.05	36.0000	36.0000	36.0000
T6 80.00- 60.00	0.00	0.0000	`A36 ´ (36 ksi)	1.05	1	1.05	36.0000	36.0000	36.0000
T7 60.00- 40.00	0.00	0.0000	` A36 <sup>′</sup> (36 ksi)	1.05	1	1.05	36.0000	36.0000	36.0000
T8 40.00- 20.00	0.00	0.0000	A36 (36 ksi)	1.05	1	1.05	36.0000	36.0000	36.0000
T9 20.00-0.00	0.00	0.0000	A36 (36 ksi)	1.05	1	1.05	36.0000	36.0000	36.0000

### **Tower Section Geometry** (cont'd)

						K Fad	ctors¹			
Tower Elevation	Calc K Single	Calc K Solid	Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace
	Angles	Rounds		X	X	X	X	X	X	X
ft	3			Y	Y	Y	Υ	Y	Y	Υ
T1 170.00-	Yes	No	1	1	1	1	1	1	1	1
160.00				1	1	1	1	1	1	1
T2 160.00-	Yes	No	1	1	1	1	1	1	1	1
140.00				1	1	1	1	1	1	1
T3 140.00-	Yes	No	1	1	1	1	1	1	1	1
120.00				1	1	1	1	1	1	1
T4 120.00-	Yes	No	1	1	1	1	1	1	1	1
100.00				1	1	1	1	1	1	1
T5 100.00-	Yes	No	1	1	1	1	1	1	1	1
80.00				1	1	1	1	1	1	1
T6 80.00-	Yes	No	1	1	1	1	1	1	1	1
60.00				1	1	1	1	1	1	1
T7 60.00-	Yes	No	1	1	1	1	1	1	1	1
40.00				1	1	1	1	1	1	1
T8 40.00-	Yes	No	1	1	1	1	1	1	1	1
20.00				1	1	1	1	1	1	1
T9 20.00-	Yes	No	1	1	1	1	1	1	1	1
0.00				1	1	1	1	1	1	1

<sup>&</sup>lt;sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

### **Tower Section Geometry** (cont'd)

Tower Elevation	Leg		Diago	nal	Top G	irt	Botton	n Girt	Mid	Girt	Long Ho	rizontal	Short Ho	rizontal
ft														
	Net Width Deduct in	U	Net Width Deduct	U	Net Width Deduct in	U	Net Width Deduct	U	Net Width Deduct	U	Net Width Deduct	U	Net Width Deduct	U
			in				in		in		in		in	
T1 170.00- 160.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T2 160.00- 140.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T3 140.00- 120.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T4 120.00- 100.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T5 100.00- 80.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 80.00- 60.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 60.00- 40.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 40.00- 20.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T9 20.00-0.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

### **Tower Section Geometry** (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagor	nal	Top G	irt	Bottom	Girt	Mid G	irt	Long Horiz	zontal	Shor Horizor	-
		Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.						
		in		in		in		in		in		in		in	
T1 170.00- 160.00	Flange	0.6250 A325N	4	0.6250 A325N	1	0.6250 A325N	1	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0

Tower	Leg	Leg		Diagor	nal	Top G	irt	Bottom	Girt	Mid G	irt	Long Hori	zontal		
Elevation	Connection													Horizor	ntal
ft	Туре	D - # O'	A / -	D - # O'	A / -	D - # O:	A / -	D - # O:	A / -	D-# 0:	A / -	D-# 0:	A / -	D - # O'	- A / -
		Bolt Size	No.	Bolt Size	No.										
		in		in		in		in		in		in		in	
T2 160.00-	Flange	0.7500	4	0.7500	1	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0
140.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T3 140.00-	Flange	0.8750	4	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
120.00	_	A325N		A325N											
T4 120.00-	Flange	1.0000	4	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
100.00	-	A325N		A325N											
T5 100.00-	Flange	1.0000	6	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
80.00	-	A325N		A325N											
T6 80.00-	Flange	1.0000	6	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
60.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T7 60.00-	Flange	1.0000	8	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
40.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T8 40.00-	Flange	1.0000	8	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
20.00	_	A325N		A325N											
T9 20.00-0.00	Flange	1.0000	0	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A354-BC		A490N		A325N		A325N		A325N		A325N		A325N	

## Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Exclude From Torque	Componen t Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	g	Width or Diameter in		Weight plf
			Calculation							in		in	
*Face A* Feedline Ladder (Af) *Leg A*	Α	No	No	Af (CaAa)	170.00 - 0.00	0.0000	0.4	1	1	3.0000	3.0000		8.40
2" Rigid Conduit	Α	No	No	Ar (CaAa)	135.00 - 0.00	5.0000	0.42	1	1	2.0000	2.0000		2.80
FB-L98B- 002-75000( 3/8")	Α	No	No	Ar (CaAa)	135.00 - 0.00	5.0000	0.42	1	1	0.3937	0.0000		0.06
WR- VG122ST- BRDA(7/16)	Α	No	No	Ar (CaAa)	135.00 - 0.00	5.0000	0.42	2	2	0.4600	0.0000		0.14
LDF6-50A(1- 1/4")	Α	No	No	Ar (CaAa)	135.00 - 0.00	5.0000	0.45	8	5	1.5500	1.5500		0.66
LDF6-50A(1- 1/4")	В	No	No	Ar (CaAa)	135.00 - 0.00	3.0000	-0.43	4	3	1.5500	1.5500		0.66
T-Brackets (Af) *Face C*	Α	No	No	Af (CaAa)	135.00 - 0.00	3.0000	0.48	1	1	1.0000	1.0000		8.40
HJ7-50A(1- 5/8")	С	No	No	Ar (CaAa)	170.00 - 0.00	0.0000	-0.36	18	15	1.9800	1.9800		1.04
Feedline Ladder (Af) *Leg B*	С	No	No	Af (CaAa)	170.00 - 0.00	0.0000	-0.36	1	1	3.0000	3.0000		8.40
Thin Flat Bar Climbing Ladder	В	No	No	Af (CaAa)	170.00 - 0.00	12.000 0	0.4	1	1	2.0000	2.0000		4.00
CU12PSM9P 6XXX(1-1/2) ***	В	No	No	Ar (CaAa)	148.00 - 0.00	0.0000	0.48	1	1	1.6000	1.6000		2.35

# Feed Line/Linear Appurtenances - Entered As Area

Description	Face	Allow	Exclude	Componen	Placement	Total	$C_A A_A$	Weight
	or	Shield	From	t		Number		
	Leg		Torque	Type	ft		ft²/ft	plf
			Calculation	)				
***								

# Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	$A_R$	$A_F$	C <sub>A</sub> A <sub>A</sub>	C <sub>A</sub> A <sub>A</sub>	Weight
Sectio	Elevation		642	612	In Face	Out Face	
n	ft		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	K
T1	170.00-160.00	Α	0.000	0.000	5.000	0.000	0.08
		В	0.000	0.000	3.333	0.000	0.04
		С	0.000	0.000	40.640	0.000	0.27
T2	160.00-140.00	Α	0.000	0.000	10.000	0.000	0.17
		В	0.000	0.000	7.947	0.000	0.10
		С	0.000	0.000	81.280	0.000	0.54
T3	140.00-120.00	Α	0.000	0.000	34.100	0.000	0.42
		В	0.000	0.000	19.167	0.000	0.17
		С	0.000	0.000	81.280	0.000	0.54
T4	120.00-100.00	Α	0.000	0.000	42.133	0.000	0.50
		В	0.000	0.000	22.267	0.000	0.18
		С	0.000	0.000	81.280	0.000	0.54
T5	100.00-80.00	Α	0.000	0.000	42.133	0.000	0.50
		В	0.000	0.000	22.267	0.000	0.18
		С	0.000	0.000	81.280	0.000	0.54
T6	80.00-60.00	Α	0.000	0.000	42.133	0.000	0.50
		В	0.000	0.000	22.267	0.000	0.18
		С	0.000	0.000	81.280	0.000	0.54
T7	60.00-40.00	Α	0.000	0.000	42.133	0.000	0.50
		В	0.000	0.000	22.267	0.000	0.18
		С	0.000	0.000	81.280	0.000	0.54
T8	40.00-20.00	Α	0.000	0.000	42.133	0.000	0.50
		В	0.000	0.000	22.267	0.000	0.18
		С	0.000	0.000	81.280	0.000	0.54
T9	20.00-0.00	Ā	0.000	0.000	42.133	0.000	0.50
		В	0.000	0.000	22.267	0.000	0.18
		Č	0.000	0.000	81.280	0.000	0.54

## Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	A <sub>R</sub>	AF	C <sub>A</sub> A <sub>A</sub>	C <sub>A</sub> A <sub>A</sub>	Weight
Sectio	Elevation	or	Thickness			In Face	Out Face	
n	ft	Leg	in	ft²	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	K
T1	170.00-160.00	Α	1.498	0.000	0.000	7.995	0.000	0.19
		В		0.000	0.000	6.329	0.000	0.12
		С		0.000	0.000	84.336	0.000	1.50
T2	160.00-140.00	Α	1.483	0.000	0.000	15.934	0.000	0.37
		В		0.000	0.000	16.254	0.000	0.30
		С		0.000	0.000	168.534	0.000	2.98
T3	140.00-120.00	Α	1.462	0.000	0.000	77.580	0.000	1.39
		В		0.000	0.000	46.108	0.000	0.71
		С		0.000	0.000	168.330	0.000	2.96
T4	120.00-100.00	Α	1.438	0.000	0.000	97.438	0.000	1.71
		В		0.000	0.000	53.926	0.000	0.81
		С		0.000	0.000	168.096	0.000	2.93
T5	100.00-80.00	Α	1.410	0.000	0.000	96.590	0.000	1.69
		В		0.000	0.000	53.498	0.000	0.80
		С		0.000	0.000	167.820	0.000	2.89
T6	80.00-60.00	Α	1.375	0.000	0.000	95.553	0.000	1.66
		В		0.000	0.000	52.974	0.000	0.78
		С		0.000	0.000	167.482	0.000	2.85
T7	60.00-40.00	Α	1.329	0.000	0.000	94.205	0.000	1.62
		В		0.000	0.000	52.294	0.000	0.76
		С		0.000	0.000	167.042	0.000	2.80
T8	40.00-20.00	Α	1.263	0.000	0.000	92.244	0.000	1.56
		В		0.000	0.000	51.303	0.000	0.72
		С		0.000	0.000	166.403	0.000	2.72
T9	20.00-0.00	Α	1.132	0.000	0.000	88.352	0.000	1.45
		В		0.000	0.000	49.338	0.000	0.66
		С		0.000	0.000	165.136	0.000	2.56

## **Feed Line Center of Pressure**

Section	Elevation	CPx	CPz	CP <sub>X</sub> Ice	CPz Ice
	ft	in	in	in	in
T1	170.00-160.00	15.6650	3.7176	18.4673	3.5863
T2	160.00-140.00	17.7855	4.3490	21.3604	4.4945
T3	140.00-120.00	18.7852	-9.7782	22.0948	-13.5765
T4	120.00-100.00	18.7745	-13.5410	22.8999	-18.8509
T5	100.00-80.00	21.4346	-15.5018	26.2382	-21.6368
T6	80.00-60.00	21.9468	-15.7638	27.7002	-22.5294
T7	60.00-40.00	23.3490	-16.7592	29.9462	-24.1429
T8	40.00-20.00	24.4938	-17.4990	31.4571	-24.8497
Т9	20.00-0.00	25.7004	-18.3595	33.5395	-25.8649

# **Shielding Factor Ka**

Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.		Segment	No Ice	Ice
			Elev.		
T1	3	Feedline Ladder (Af)	160.00 -	0.6000	0.6000
			170.00		
T1	12	HJ7-50A(1-5/8")	160.00 -	0.6000	0.6000
T4	4.4	Facilia da ada (AA)	170.00	0.0000	0.0000
T1	14	Feedline Ladder (Af)	160.00 - 170.00	0.6000	0.6000
T1	16	Thin Flat Bar Climbing	160.00 -	0.6000	0.6000
		Ladder	170.00	0.0000	0.0000
T2	3	Feedline Ladder (Af)	140.00 -	0.6000	0.6000
		, ,	160.00		
T2	12	HJ7-50A(1-5/8")	140.00 -	0.6000	0.6000
т.	4.4	- III	160.00	0.0000	0.0000
T2	14	Feedline Ladder (Af)	140.00 -	0.6000	0.6000
T2	16	Thin Flat Bar Climbing	160.00 140.00 -	0.6000	0.6000
12	10	Ladder	160.00	0.0000	0.0000
T2	18	CU12PSM9P6XXX(1-1/2)	140.00 -	0.6000	0.6000
		,	148.00		
T3	3	Feedline Ladder (Af)	120.00 -	0.6000	0.6000
			140.00		
T3	5	2" Rigid Conduit	120.00 -	0.6000	0.6000
T3	6	ED 1 00D 002 75000( 3/0")	135.00	0.6000	0.6000
13	О	FB-L98B-002-75000( 3/8")	120.00 - 135.00	0.6000	0.6000
Т3	7	WR-VG122ST-BRDA(7/16)	120.00 -	1.0000	1.0000
	·		135.00		
T3	8	LDF6-50A(1-1/4")	120.00 -	0.6000	0.6000
			135.00		
Т3	9	LDF6-50A(1-1/4")	120.00 -	0.6000	0.6000
Т3	10	T Brackets (Af)	135.00	0.6000	0.6000
13	10	T-Brackets (Af)	120.00 - 135.00	0.6000	0.6000
Т3	12	HJ7-50A(1-5/8")	120.00 -	0.6000	0.6000
		,	140.00	0.0000	0.000
Т3	14	Feedline Ladder (Af)	120.00 -	0.6000	0.6000
			140.00		
T3	16	Thin Flat Bar Climbing	120.00 -	0.6000	0.6000
То	40	Ladder	140.00	0.0000	0.0000
T3	18	CU12PSM9P6XXX(1-1/2)	120.00 - 140.00	0.6000	0.6000
T4	3	Feedline Ladder (Af)	100.00 -	0.6000	0.6000
'7		i odamio Laddoi (Ai)	120.00	0.0000	0.0000
T4	5	2" Rigid Conduit	100.00 -	0.6000	0.6000
		_	120.00		
T4	6	FB-L98B-002-75000( 3/8")	100.00 -	0.6000	0.6000
		MID MOADOOT DDD 4/7/40	120.00	4 0000	4 0000
T4		WR-VG122ST-BRDA(7/16)	100.00 -	1.0000	1.0000
1	1	l	120.00		I

Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.	4	Segment Elev.	No Ice	Ice
T4	8	LDF6-50A(1-1/4")	100.00 -	0.6000	0.6000
T4	9	LDF6-50A(1-1/4")	120.00 100.00 -	0.6000	0.6000
T4	10	T-Brackets (Af)	120.00 100.00 -	0.6000	0.6000
T4	12	HJ7-50A(1-5/8")	120.00 100.00 -	0.6000	0.6000
T4	14	Feedline Ladder (Af)	120.00 100.00 -	0.6000	0.6000
T4	16	Thin Flat Bar Climbing	120.00 100.00 - 120.00	0.6000	0.6000
Т4	18	Ladder CU12PSM9P6XXX(1-1/2)	100.00 - 120.00	0.6000	0.6000
T5	3	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T5	5	2" Rigid Conduit	80.00 - 100.00	0.6000	0.6000
Т5	6	FB-L98B-002-75000( 3/8")	80.00 - 100.00	0.6000	0.6000
Т5	7	WR-VG122ST-BRDA(7/16)	80.00 - 100.00	1.0000	1.0000
T5	8	LDF6-50A(1-1/4")	80.00 - 100.00	0.6000	0.6000
T5	9	LDF6-50A(1-1/4")	80.00 - 100.00	0.6000	0.6000
T5	10	T-Brackets (Af)	80.00 - 100.00	0.6000	0.6000
T5	12	HJ7-50A(1-5/8")	80.00 - 100.00	0.6000	0.6000
T5	14	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T5	16	Thin Flat Bar Climbing Ladder	80.00 - 100.00	0.6000	0.6000
T5	18	CU12PSM9P6XXX(1-1/2)	80.00 - 100.00	0.6000	0.6000
Т6	3	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
Т6	5	2" Rigid Conduit	60.00 - 80.00	0.6000	0.6000
Т6	6	FB-L98B-002-75000( 3/8")	60.00 - 80.00	0.6000	0.6000
Т6	7	WR-VG122ST-BRDA(7/16)	60.00 - 80.00	1.0000	1.0000
Т6	8	LDF6-50A(1-1/4")	60.00 - 80.00	0.6000	0.6000
Т6	9	LDF6-50A(1-1/4")	60.00 - 80.00	0.6000	0.6000
Т6	10	T-Brackets (Af)	60.00 - 80.00	0.6000	0.6000
Т6	12	HJ7-50A(1-5/8")	60.00 - 80.00	0.6000	0.6000
Т6	14	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
Т6	16	Thin Flat Bar Climbing Ladder	60.00 - 80.00	0.6000	0.6000
Т6	18	CU12PSM9P6XXX(1-1/2)	60.00 - 80.00	0.6000	0.6000
Т7	3	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
Т7	5	2" Rigid Conduit	40.00 - 60.00	0.6000	0.6000
T7	6	FB-L98B-002-75000( 3/8")	40.00 - 60.00	0.6000	0.6000
T7	7	WR-VG122ST-BRDA(7/16)	40.00 - 60.00	1.0000	1.0000
T7	8	LDF6-50A(1-1/4")	40.00 - 60.00	0.6000	0.6000
T7	9	LDF6-50A(1-1/4")	40.00 -	0.6000	0.6000

Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.	•	Segment	No Ice	Ice
			Elev.		
_			60.00		
T7	10	T-Brackets (Af)	40.00 -	0.6000	0.6000
	40	11.17 50.4 (4.5 (01))	60.00	0.0000	0.0000
T7	12	HJ7-50A(1-5/8")	40.00 - 60.00	0.6000	0.6000
T7	14	Feedline Ladder (Af)	40.00 -	0.6000	0.6000
1 '	14	r eediirle Ladder (Ar)	60.00	0.0000	0.0000
T7	16	Thin Flat Bar Climbing	40.00 -	0.6000	0.6000
	. •	Ladder	60.00	0.000	0.000
T7	18	CU12PSM9P6XXX(1-1/2)	40.00 -	0.6000	0.6000
			60.00		
T8	3	Feedline Ladder (Af)	20.00 -	0.6000	0.6000
	_	0,5,1,0	40.00		
Т8	5	2" Rigid Conduit	20.00 -	0.6000	0.6000
Т8	6	FB-L98B-002-75000( 3/8")	40.00 20.00 -	0.6000	0.6000
10	Ü	FB-L96B-002-75000( 5/6 )	40.00	0.0000	0.0000
Т8	7	WR-VG122ST-BRDA(7/16)	20.00 -	1.0000	1.0000
	·		40.00		
T8	8	LDF6-50A(1-1/4")	20.00 -	0.6000	0.6000
			40.00		
T8	9	LDF6-50A(1-1/4")	20.00 -	0.6000	0.6000
т.	40	T.D. 1. (A.C.	40.00	0.0000	0.0000
Т8	10	T-Brackets (Af)	20.00 -	0.6000	0.6000
Т8	12	HJ7-50A(1-5/8")	40.00 20.00 -	0.6000	0.6000
10	12	1107-30A(1-3/0 )	40.00	0.0000	0.0000
Т8	14	Feedline Ladder (Af)	20.00 -	0.6000	0.6000
		( )	40.00		
T8	16	Thin Flat Bar Climbing	20.00 -	0.6000	0.6000
		Ladder	40.00		
Т8	18	CU12PSM9P6XXX(1-1/2)	20.00 -	0.6000	0.6000
T-0	•	Foodling Ladden (AS)	40.00	0.0000	0.0000
T9 T9	3 5	Feedline Ladder (Af) 2" Rigid Conduit	0.00 - 20.00 0.00 - 20.00	0.6000	0.6000
T9	5 6		0.00 - 20.00	0.6000 0.6000	0.6000 0.6000
T9	7	WR-VG122ST-BRDA(7/16)	0.00 - 20.00	1.0000	1.0000
T9	8	LDF6-50A(1-1/4")	0.00 - 20.00	0.6000	0.6000
T9	9	LDF6-50A(1-1/4")	0.00 - 20.00	0.6000	0.6000
Т9	10	T-Brackets (Af)	0.00 - 20.00	0.6000	0.6000
Т9	12	HJ7-50A(1-5/8")	0.00 - 20.00	0.6000	0.6000
T9	14	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T9	16	Thin Flat Bar Climbing	0.00 - 20.00	0.6000	0.6000
	40	Ladder	0.00 00.00	0.0000	0.0000
T9	18	CU12PSM9P6XXX(1-1/2)	0.00 - 20.00	0.6000	0.6000

## Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ft ft ft	o	ft		ft²	ft²	K
Lightning Rod 1" x 5' on 5' Pole	С	From Leg	0.00 0.00 5.00	0.0000	170.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.94 2.96 3.68 4.99	1.94 2.96 3.68 4.99	0.08 0.10 0.13 0.21
(2) LPA-80063/6CF w/ Mount Pipe	Α	From Face	4.00 0.00 1.00	0.0000	170.00	No Ice 1/2" Ice 1" Ice 2" Ice	9.83 10.40 10.93 12.03	10.22 11.38 12.27 14.09	0.05 0.14 0.25 0.48

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	۰	ft		ft²	ft²	κ
(2) LPA-80063/6CF w/	В	From Face	4.00	0.0000	170.00	No Ice	9.83	10.22	0.05
Mount Pipe			0.00			1/2"	10.40	11.38	0.14
			1.00			Ice	10.93	12.27	0.25
						1" Ice 2" Ice	12.03	14.09	0.48
(2) LPA-80063/6CF w/	С	From Face	4.00	0.0000	170.00	No Ice	9.83	10.22	0.05
Mount Pipe			0.00			1/2"	10.40	11.38	0.14
			1.00			lce 1" lce	10.93 12.03	12.27 14.09	0.25 0.48
						2" Ice			
(2) LPA-171063-12CF-	Α	From Face	4.00	0.0000	170.00	No Ice	6.23	7.75	0.04
EDIN-2 w/ Mount Pipe			0.00			1/2"	6.80	8.97	0.10
			1.00			Ice	7.35	9.91	0.17
						1" Ice 2" Ice	8.44	11.79	0.34
(2) LPA-171063-12CF-	В	From Face	4.00	0.0000	170.00	No Ice	6.23	7.75	0.04
EDIN-2 w/ Mount Pipe			0.00			1/2"	6.80	8.97	0.10
			1.00			Ice	7.35	9.91	0.17
						1" Ice 2" Ice	8.44	11.79	0.34
(2) LPA-171063-12CF-	С	From Face	4.00	0.0000	170.00	No Ice	6.23	7.75	0.04
EDIN-2 w/ Mount Pipe			0.00			1/2"	6.80	8.97	0.10
			1.00			Ice	7.35	9.91	0.17
						1" Ice 2" Ice	8.44	11.79	0.34
BXA-70063-6CF-EDIN-0	Α	From Face	4.00	0.0000	170.00	No Ice	7.40	5.39	0.04
w/ Mount Pipe			0.00			1/2"	8.14	6.10	0.10
			1.00			Ice	8.90	6.83	0.16
						1" Ice 2" Ice	10.46	8.34	0.33
BXA-70063-6CF-EDIN-0	В	From Face	4.00	0.0000	170.00	No Ice	7.40	5.39	0.04
w/ Mount Pipe	Ь	1 TOTAL ACC	0.00	0.0000	170.00	1/2"	8.14	6.10	0.10
<del>.</del> .			1.00			Ice	8.90	6.83	0.16
						1" Ice 2" Ice	10.46	8.34	0.33
BXA-70063-6CF-EDIN-0	С	From Face	4.00	0.0000	170.00	No Ice	7.40	5.39	0.04
w/ Mount Pipe			0.00			1/2"	8.14	6.10	0.10
			1.00			Ice	8.90	6.83	0.16
						1" Ice	10.46	8.34	0.33
Costor Mount ICM E14 21	0	None		0.0000	170.00	2" Ice No Ice	55.50	EE E0	1 24
Sector Mount [SM 514-3]	С	None		0.0000	170.00	1/2"	80.55	55.50 80.55	1.34 2.26
						Ice	105.00	105.00	3.50
						1" Ice	152.52	152.52	6.87
***						2" Ice			
***									
MX08FRO665-20 w/	Α	From Leg	4.00	0.0000	148.00	No Ice	8.01	4.23	0.10
Mount Pipe			0.00	0.000		1/2"	8.52	4.69	0.18
·			0.00			Ice	9.04	5.16	0.28
						1" Ice 2" Ice	10.11	6.12	0.51
MX08FRO665-20 w/	В	From Leg	4.00	0.0000	148.00	No Ice	8.01	4.23	0.10
Mount Pipe		Ü	0.00			1/2"	8.52	4.69	0.18
			0.00			Ice	9.04	5.16	0.28
						1" Ice 2" Ice	10.11	6.12	0.51
MX08FRO665-20 w/	С	From Leg	4.00	0.0000	148.00	No Ice	8.01	4.23	0.10
Mount Pipe	-	3	0.00			1/2"	8.52	4.69	0.18
·			0.00			Ice	9.04	5.16	0.28
						1" Ice	10.11	6.12	0.51
DDIDC 0404 DE 40	^	Гиона I	4.00	0.0000	140.00	2" Ice	0.04	4.00	0.00
RDIDC-9181-PF-48	Α	From Leg	4.00 0.00	0.0000	148.00	No Ice 1/2"	2.31 2.50	1.29 1.45	0.02 0.04
			0.00			lce	2.50	1.45	0.04
			3.00			100	5	1.01	3.00

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	۰	ft		ft²	ft²	К
						1" Ice 2" Ice	3.12	1.96	0.12
TA08025-B604	Α	From Leg	4.00	0.0000	148.00	No Ice	1.96	0.98	0.06
17.00020 2001	,,	r rom Log	0.00	0.0000	1 10.00	1/2"	2.14	1.11	0.08
			0.00			Ice	2.32	1.25	0.10
	_					1" Ice 2" Ice	2.71	1.55	0.15
TA08025-B604	В	From Leg	4.00	0.0000	148.00	No Ice	1.96	0.98	0.06
			0.00			1/2" Ice	2.14 2.32	1.11 1.25	0.08 0.10
			0.00			1" Ice	2.71	1.55	0.10
						2" Ice	2.7 1	1.00	0.10
TA08025-B604	С	From Leg	4.00	0.0000	148.00	No Ice	1.96	0.98	0.06
			0.00			1/2"	2.14	1.11	0.08
			0.00			Ice	2.32	1.25	0.10
						1" Ice	2.71	1.55	0.15
TA08025-B605	Α	From Leg	4.00	0.0000	148.00	2" Ice No Ice	1.96	1.13	0.08
1A00023-B003	^	From Leg	0.00	0.0000	140.00	1/2"	2.14	1.13	0.00
			0.00			lce	2.32	1.41	0.11
						1" Ice	2.71	1.72	0.16
						2" Ice			
TA08025-B605	В	From Leg	4.00	0.0000	148.00	No Ice	1.96	1.13	0.08
			0.00			1/2"	2.14	1.27	0.09
			0.00			Ice 1" Ice	2.32 2.71	1.41 1.72	0.11 0.16
						2" Ice	2.7 1	1.72	0.10
TA08025-B605	С	From Leg	4.00	0.0000	148.00	No Ice	1.96	1.13	0.08
		ū	0.00			1/2"	2.14	1.27	0.09
			0.00			Ice	2.32	1.41	0.11
						1" Ice 2" Ice	2.71	1.72	0.16
Commscope MTC3975083	С	None		0.0000	148.00	No Ice	23.85	23.85	1.26
(3)	Ü	140110		0.0000	140.00	1/2"	34.12	34.12	1.80
(-)						Ice	44.39	44.39	2.35
						1" Ice	64.93	64.93	3.43
(0) 01 0111 (0)					4.40.00	2" Ice			
(2) 8' x 2" Mount Pipe	Α	From Leg	4.00	0.0000	148.00	No Ice 1/2"	1.90	1.90	0.03
			0.00			lce	2.73 3.40	2.73 3.40	0.04 0.06
			0.00			1" Ice	4.40	4.40	0.12
						2" Ice			
(2) 8' x 2" Mount Pipe	В	From Leg	4.00	0.0000	148.00	No Ice	1.90	1.90	0.03
			0.00			1/2"	2.73	2.73	0.04
			0.00			Ice	3.40	3.40	0.06
						1" Ice 2" Ice	4.40	4.40	0.12
(2) 8' x 2" Mount Pipe	С	From Leg	4.00	0.0000	148.00	No Ice	1.90	1.90	0.03
(2) 6 % 26	•		0.00	0.000		1/2"	2.73	2.73	0.04
			0.00			Ice	3.40	3.40	0.06
						1" Ice	4.40	4.40	0.12
***						2" Ice			
***									
(2) 7770.00	Α	From Leg	4.00	0.0000	135.00	No Ice	5.51	2.93	0.04
(=,	- •	<b></b> -9	0.00			1/2"	5.87	3.27	0.07
			1.00			Ice	6.23	3.63	0.11
						1" Ice	6.99	4.35	0.20
(0) 7770 00	Б	Francis:	4.00	0.0000	125.00	2" Ice	E F4	2.02	0.04
(2) 7770.00	В	From Leg	4.00 0.00	0.0000	135.00	No Ice 1/2"	5.51 5.87	2.93 3.27	0.04 0.07
			1.00			lce	6.23	3.63	0.07
						1" Ice	6.99	4.35	0.20
						2" Ice			
(2) 7770.00	С	From Leg	4.00	0.0000	135.00	No Ice	5.51	2.93	0.04

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	•	ft		ft²	ft²	K
			0.00 1.00			1/2" Ice 1" Ice	5.87 6.23 6.99	3.27 3.63 4.35	0.07 0.11 0.20
P65-17-XLH-RR w/ Mount Pipe	Α	From Leg	4.00 0.00 1.00	0.0000	135.00	2" Ice No Ice 1/2" Ice 1" Ice	7.48 8.17 8.88 10.33	5.29 5.96 6.64 8.05	0.09 0.17 0.26 0.49
AM-X-CD-16-65-00T-RET w/ Mount Pipe	В	From Leg	4.00 0.00 1.00	0.0000	135.00	2" Ice No Ice 1/2" Ice	4.63 5.06 5.51	3.27 3.69 4.12	0.07 0.13 0.20
P65-16-XLH-RR w/ Mount Pipe	С	From Leg	4.00 0.00	0.0000	135.00	1" Ice 2" Ice No Ice 1/2"	6.43 5.66 6.21	5.00 4.01 4.53	0.38 0.08 0.14
(2) LGP21401	A	From Leg	1.00 4.00	0.0000	135.00	Ice 1" Ice 2" Ice No Ice	6.76 7.90 1.10	5.06 6.15 0.21	0.21 0.38 0.01
.,		Ū	0.00 0.00			1/2" Ice 1" Ice 2" Ice	1.24 1.38 1.69	0.27 0.35 0.52	0.02 0.03 0.05
(2) LGP21401	В	From Leg	4.00 0.00 0.00	0.0000	135.00	No Ice 1/2" Ice 1" Ice	1.10 1.24 1.38 1.69	0.21 0.27 0.35 0.52	0.01 0.02 0.03 0.05
(2) LGP21401	С	From Leg	4.00 0.00 0.00	0.0000	135.00	2" Ice No Ice 1/2" Ice 1" Ice	1.10 1.24 1.38 1.69	0.21 0.27 0.35 0.52	0.01 0.02 0.03 0.05
(2) LGP21901	Α	From Leg	4.00 0.00 0.00	0.0000	135.00	2" Ice No Ice 1/2" Ice 1" Ice	0.23 0.29 0.36 0.53	0.16 0.21 0.28 0.42	0.01 0.01 0.01 0.02
(2) LGP21901	В	From Leg	4.00 0.00 0.00	0.0000	135.00	2" Ice No Ice 1/2" Ice 1" Ice	0.23 0.29 0.36 0.53	0.16 0.21 0.28 0.42	0.01 0.01 0.01 0.02
(2) LGP21901	С	From Leg	4.00 0.00 0.00	0.0000	135.00	2" Ice No Ice 1/2" Ice 1" Ice	0.23 0.29 0.36 0.53	0.16 0.21 0.28 0.42	0.01 0.01 0.01 0.02
RRUS-11	Α	From Leg	1.00 0.00 0.00	0.0000	135.00	2" Ice No Ice 1/2" Ice 1" Ice	2.78 2.99 3.21 3.66	1.19 1.33 1.49 1.83	0.05 0.07 0.09 0.15
RRUS-11	В	From Leg	1.00 0.00 0.00	0.0000	135.00	2" Ice No Ice 1/2" Ice 1" Ice	2.78 2.99 3.21 3.66	1.19 1.33 1.49 1.83	0.05 0.07 0.09 0.15
RRUS-11	С	From Leg	1.00 0.00 0.00	0.0000	135.00	2" Ice No Ice 1/2" Ice	2.78 2.99 3.21	1.19 1.33 1.49	0.05 0.07 0.09
DC6-48-60-18-8F	В	From Leg	1.00	0.0000	135.00	1" Ice 2" Ice No Ice	3.66 1.21	1.83 1.21	0.15 0.02

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	۰	ft		ft²	ft²	К
			0.00 0.00			1/2" Ice 1" Ice 2" Ice	1.89 2.11 2.57	1.89 2.11 2.57	0.04 0.07 0.13
Sector Mount [SM 402-3]	С	None		0.0000	135.00	No Ice 1/2" Ice 1" Ice 2" Ice	18.87 26.47 33.99 48.84	18.87 26.47 33.99 48.84	0.85 1.21 1.70 3.04
***									

Dishes										
Description	Face or Leg	Dish Type	Offset Type		Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weigh
				ft	۰	۰	ft	ft	ft²	K

# **Load Combinations**

Comb.	Description
No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25 26	0.9 Dead+1.0 Wind 330 deg - No Ice
26 27	1.2 Dead+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp

Comb. No.	Description
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

### **Maximum Member Forces**

Sectio n No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	170 - 160	Log	Max Tension	15	6.59		-0.03
11	170 - 160	Leg	Max. Compression	10	-8.57	-0.06 -0.04	-0.03 -0.02
			Max. Mx	4	-0.37 -7.38	0.08	0.02
				4	-7.36 -1.14		0.04
			Max. My	20	-1.14 2.05	-0.05 0.00	0.10
			Max. Vy Max. Vx	14	-2.05	0.00	0.00
		Diagonal	Max Tension	12			0.00
		Diagonal			2.93	0.00	
			Max. Compression	24	-2.95	0.00	0.00
			Max. Mx	32	0.34	0.03	-0.00
			Max. My	8	-2.47	0.01	-0.00
			Max. Vy	31	-0.03	0.03	-0.00
		- 0	Max. Vx	8	-0.00	0.00	0.00
		Top Girt	Max Tension	3	1.11	0.00	0.00
			Max. Compression	14	-1.15	0.00	0.00
			Max. Mx	31	0.20	-0.12	0.00
			Max. My	4	-0.01	0.00	0.00
			Max. Vy	31	0.06	0.00	0.00
			Max. Vx	4	-0.00	0.00	0.00
T2	160 - 140	Leg	Max Tension	15	24.47	-0.34	0.06
		-	Max. Compression	2	-29.67	0.06	-0.06
			Max. Mx	14	16.84	0.44	0.06
			Max. My	20	-1.49	-0.02	0.47
			Max. Vý	14	0.60	-0.35	0.06
			Max. Vx	4	0.59	-0.02	-0.32
		Diagonal	Max Tension	24	4.40	0.00	0.00
		3	Max. Compression	24	-4.50	0.00	0.00
			Max. Mx	32	0.32	0.06	0.01
			Max. My	14	-3.98	0.01	0.01
			Max. Vy	33	0.05	0.06	0.01
			Max. Vx	33	-0.00	0.00	0.00
		Top Girt	Max Tension	3	0.17	0.00	0.00
		TOP OIL	Max. Compression	14	-0.23	0.00	0.00
			Max. Mx	31	-0.03	-0.12	0.00
			Max. My	27	-0.03	0.00	0.00
			Max. Vy	31	0.06	0.00	0.00
			Max. Vx	27	-0.00	0.00	0.00
Т3	140 100	Lon		27 15			
13	140 - 120	Leg	Max Tension		48.45	0.04	0.06
			Max. Compression	2	-57.86	0.24	-0.06
			Max. Mx	14	38.77	-0.52	0.06
			Max. My	20	-3.70	-0.01	0.60
			Max. Vy	14	0.62	-0.52	0.06
		D: .	Max. Vx	4	0.60	-0.01	-0.37
		Diagonal	Max Tension	24	5.89	0.00	0.00
			Max. Compression	24	-5.90	0.00	0.00
			Max. Mx	32	0.62	0.09	0.01
			Max. My	14	-5.03	0.02	0.01

Sectio	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
n	ft	Type		Load	K	Moment	Moment
No.			M-14 1/14	Comb.	K	kip-ft	kip-ft
			Max. Vy Max. Vx	32 33	0.06 -0.00	0.08 0.00	-0.01 0.00
T4	120 - 100	Leg	Max Tension	15	73.46	-0.17	0.05
	120 100	Log	Max. Compression	2	-86.73	0.34	-0.10
			Max. Mx	3	-85.11	0.34	-0.10
			Max. My	4	-6.72	-0.01	-0.42
			Max. Vy	19	-0.08	0.33	0.03
			Max. Vx	4	0.15	-0.01	-0.42
		Diagonal	Max Tension	24	6.84	0.00	0.00
			Max. Compression	24	-6.92	0.00	0.00
			Max. Mx	31	1.09	0.12	0.01
			Max. My	33 33	-1.43	0.10	0.02
			Max. Vy Max. Vx	33 27	0.07 0.00	0.12 0.00	0.02 0.00
T5	100 - 80	Leg	Max Tension	15	95.97	-0.34	0.00
10	100 - 00	Log	Max. Compression	2	-113.21	0.61	-0.14
			Max. Mx	2	-113.21	0.61	-0.14
			Max. My	4	-7.51	-0.04	-0.57
			Max. Vý	18	-0.12	0.59	0.04
			Max. Vx	4	0.18	-0.04	-0.57
		Diagonal	Max Tension	24	8.19	0.00	0.00
			Max. Compression	24	-8.33	0.00	0.00
			Max. Mx	33	1.83	0.19	-0.02
			Max. My	27	0.00	0.16	-0.03
			Max. Vy	33	0.10	0.19	-0.02
TC	00 00	1	Max. Vx	27	-0.01	0.00	0.00
T6	80 - 60	Leg	Max Tension Max. Compression	15 2	120.27 -142.54	-0.52 0.65	0.11 -0.11
			Max. Mx	2	-142.54	0.65	-0.11
			Max. My	4	-9.26	-0.06	-0.80
			Max. Vy	18	-0.12	0.63	0.03
			Max. Vx	4	0.19	-0.06	-0.80
		Diagonal	Max Tension	24	9.23	0.00	0.00
		_	Max. Compression	24	-9.38	0.00	0.00
			Max. Mx	31	2.00	0.28	-0.03
			Max. My	27	0.19	0.24	-0.04
			Max. Vy	33	0.13	0.27	-0.03
T-7	00 40	1	Max. Vx	27	-0.01	0.00	0.00
T7	60 - 40	Leg	Max Tension	15	143.78	-0.54	0.10
			Max. Compression Max. Mx	2 33	-171.64 9.40	0.70 -1.48	-0.09 0.05
			Max. My	4	-12.49	0.02	-0.69
			Max. Vy	37	0.25	-1.47	-0.03
			Max. Vx	4	0.16	-0.06	-0.66
		Diagonal	Max Tension	24	9.63	0.00	0.00
		•	Max. Compression	24	-9.77	0.00	0.00
			Max. Mx	33	1.50	0.32	-0.04
			Max. My	33	-1.44	0.27	0.05
			Max. Vy	33	0.14	0.31	0.04
TO	4000	1	Max. Vx	27	-0.01	0.00	0.00
Т8	40 - 20	Leg	Max Tension	15	166.94	-0.93	0.07
			Max. Compression Max. Mx	2 33	-200.74 10.24	1.38 -4.12	-0.13 0.05
			Max. My	4	-13.72	-4.12 -0.11	-1.20
			Max. Vy	37	0.70	-4.08	-0.03
			Max. Vx	4	-0.21	-0.11	-1.20
		Diagonal	Max Tension	24	10.59	0.00	0.00
		3	Max. Compression	24	-10.86	0.00	0.00
			Max. Mx	33	1.20	0.39	-0.04
			Max. My	27	-0.83	0.35	-0.05
			Max. Vy	33	0.15	0.39	-0.04
			Max. Vx	27	-0.01	0.00	0.00
Т9	20 - 0	Leg	Max Tension	15	189.69	-0.96	0.10
			Max. Compression	2	-230.01	-0.00	0.00
			Max. Mx	27	-97.20 16.21	4.27	0.05
			Max. My Max. Vy	4 37	-16.21 -0.84	-0.16 -4.08	-2.09 -0.03
			Max. Vx	4	-0.64 -0.35	-4.06 -0.16	-0.03 -2.09
		Diagonal	Max Tension	24	11.42	0.00	0.00
						0.00	0.00

Sectio	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
n	ft	Type		Load		Moment	Moment
No.				Comb.	K	kip-ft	kip-ft
			Max. Compression	24	-11.75	0.00	0.00
			Max. Mx	33	-1.08	0.52	0.06
			Max. My	33	-5.19	0.49	0.07
			Max. Vy	33	0.17	0.52	0.06
			Max. Vx	33	-0.01	0.00	0.00

	<b>D</b> (:
Mavimiim	Reactions
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Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, 2
		Load	K	K	K
		Comb.			
Leg C	Max. Vert	18	220.41	23.99	-13.27
•	Max. H <sub>x</sub>	18	220.41	23.99	-13.27
	Max. H <sub>z</sub>	7	-182.52	-20.25	11.12
	Min. Vert	7	-182.52	-20.25	11.12
	Min. H <sub>x</sub>	7	-182.52	-20.25	11.12
	$Min. H_z$	18	220.41	23.99	-13.27
Leg B	Max. Vert	10	226.14	-23.77	-14.62
•	Max. H <sub>x</sub>	23	-182.70	19.96	12.38
	Max. H <sub>z</sub>	25	-171.21	17.66	13.84
	Min. Vert	23	-182.70	19.96	12.38
	Min. H <sub>x</sub>	10	226.14	-23.77	-14.62
	$Min. H_z$	12	202.90	-19.93	-15.14
Leg A	Max. Vert	2	237.39	1.56	29.40
_	Max. H <sub>x</sub>	20	17.25	3.83	1.52
	Max. H <sub>z</sub>	2	237.39	1.56	29.40
	Min. Vert	15	-195.27	-1.54	-25.02
	Min. H <sub>x</sub>	11	-91.12	-3.96	-12.08
	Min. H₂	15	-195.27	-1.54	-25.02

# **Tower Mast Reaction Summary**

Load Combination	Vertical	Shearx	Shearz	Overturning Moment, M <sub>x</sub>	Overturning Moment, M <sub>z</sub>	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Dead Only	41.80	0.00	0.00	-9.92	-32.98	0.00
1.2 Dead+1.0 Wind 0 deg -	50.16	0.00	-47.75	<b>-</b> 4777.68	-40.25	67.04
No Ice						
0.9 Dead+1.0 Wind 0 deg -	37.62	0.00	-47.75	-4771.30	-30.31	67.03
No Ice						
1.2 Dead+1.0 Wind 30 deg -	50.16	22.38	-38.79	-3935.83	-2302.41	67.31
No Ice						
0.9 Dead+1.0 Wind 30 deg -	37.62	22.38	-38.79	-3930.02	-2290.84	67.28
No Ice	=- 1-			0.1.0.1.1	00=0.44	0.4.00
1.2 Dead+1.0 Wind 60 deg -	50.16	35.71	-20.64	-2112.44	-3672.11	21.62
No Ice	07.00	05.74	00.04	0407.04	0050 55	04.04
0.9 Dead+1.0 Wind 60 deg -	37.62	35.71	-20.64	-2107.94	-3659.55	21.61
No Ice	E0 40	20.25	0.00	40.00	2040.70	20.00
1.2 Dead+1.0 Wind 90 deg - No Ice	50.16	38.35	-0.00	-12.38	-3940.76	-30.90
0.9 Dead+1.0 Wind 90 deg -	37.62	38.35	-0.00	-9.40	-3928.00	-30.88
No Ice	37.02	30.33	-0.00	-9.40	-3920.00	-30.00
1.2 Dead+1.0 Wind 120 deg	50.16	39.09	22.59	2243.05	-3940.48	-32.75
- No Ice	30.10	00.00	22.00	2240.00	-00+0.+0	-02.70
0.9 Dead+1.0 Wind 120 deg	37.62	39.09	22.59	2244.42	-3927.77	-32.74
- No Ice	002	00.00			002	02
1.2 Dead+1.0 Wind 150 deg	50.16	23.01	39.90	3993.43	-2348.90	-54.66
- No Ice						
0.9 Dead+1.0 Wind 150 deg	37.62	23.01	39.90	3993.55	-2337.31	-54.63
- No Ice						
1.2 Dead+1.0 Wind 180 deg	50.16	-0.00	44.61	4499.59	-39.39	-67.05
· ·						

Load Combination	Vertical	Shearx	Shear₂	Overturning Moment, M <sub>x</sub>	Overturning Moment, M₂	Torque
- No Ice	K	K	K	kip-ft	kip-ft	kip-ft
- No Ice 0.9 Dead+1.0 Wind 180 deg - No Ice	37.62	-0.00	44.61	4499.33	-29.45	-67.01
1.2 Dead+1.0 Wind 210 deg	50.16	-22.38	38.79	3912.13	2222.79	-67.31
- No Ice 0.9 Dead+1.0 Wind 210 deg	37.62	-22.38	38.79	3912.29	2231.11	-67.28
- No Ice 1.2 Dead+1.0 Wind 240 deg	50.16	-38.43	22.21	2215.81	3812.87	-21.62
- No Ice 0.9 Dead+1.0 Wind 240 deg	37.62	-38.43	22.21	2217.21	3820.07	-21.62
- No Ice 1.2 Dead+1.0 Wind 270 deg	50.16	-38.35	0.00	-11.54	3861.38	30.90
- No Ice 0.9 Dead+1.0 Wind 270 deg	37.62	-38.35	0.00	-8.56	3868.51	30.88
- No Ice 1.2 Dead+1.0 Wind 300 deg	50.16	-36.36	-21.01	-2139.84	3640.87	32.76
- No Ice 0.9 Dead+1.0 Wind 300 deg	37.62	-36.36	-21.01	-2135.33	3648.16	32.73
- No Ice 1.2 Dead+1.0 Wind 330 deg	50.16	-23.01	-39.90	-4017.37	2269.35	54.66
- No Ice 0.9 Dead+1.0 Wind 330 deg	37.62	-23.01	-39.90	-4011.52	2277.64	54.63
- No Ice	404.00				400 =0	
1.2 Dead+1.0 Ice+1.0 Temp	124.90	-0.00	0.00	-2.25 -1227.79	-166.59	0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	124.90	0.00	-12.07	-1227.79	-166.67	21.20
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	124.90	5.53	-9.58	-986.80	-734.47	15.91
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	124.90	8.46	-4.89	-509.92	-1044.78	-0.03
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	124.90	9.38	-0.00	-2.33	-1139.53	-10.53
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	124.90	8.99	5.20	529.09	-1085.99	-11.82
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	124.90	5.63	9.75	994.99	-741.74	-15.37
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	124.90	-0.00	11.65	1190.39	-166.53	-21.20
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	124.90	-5.53	9.58	982.26	401.27	-15.91
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	124.90	-8.82	5.10	521.82	740.12	0.03
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	124.90	-9.38	0.00	-2.22	806.34	10.53
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	124.90	-8.63	-4.99	-517.21	724.33	11.82
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	124.90	-5.63	-9.75	-999.55	408.54	15.37
Dead+Wind 0 deg - Service	41.80	0.00	-10.71	-1078.06	-33.17	15.03
Dead+Wind 30 deg - Service	41.80	5.02	-8.70	-889.38	-540.17	15.09
Dead+Wind 60 deg - Service	41.80	8.01	-4.63	-480.69	-847.15	4.85
Dead+Wind 90 deg - Service	41.80	8.60	-0.00	-10.03	-907.39	-6.93
Dead+Wind 120 deg - Service	41.80	8.76	5.06	495.47	-907.33	-7.34
Dead+Wind 150 deg - Service	41.80	5.16	8.95	887.77	-550.59	-12.25
Dead+Wind 180 deg - Service	41.80	-0.00	10.00	1001.18	-32.99	-15.03
Dead+Wind 210 deg - Service	41.80	-5.02	8.70	869.53	474.03	-15.09
Dead+Wind 240 deg - Service	41.80	-8.62	4.98	489.36	830.41	-4.85
Dead+Wind 270 deg - Service	41.80	-8.60	0.00	-9.84	841.26	6.93
Dead+Wind 300 deg - Service	41.80	-8.15	-4.71	-486.84	791.81	7.34
Dead+Wind 330 deg - Service	41.80	-5.16	-8.95	-907.63	484.46	12.25

## **Solution Summary**

	Sur	n of Applied Force			Sum of Reaction	ne	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	Ŕ	ĸ	K	ĸ	ĸ	K	70 LITOI
1	0.00	-41.80	0.00	0.00	41.80	0.00	0.000%
2	0.00	-50.16	-47.75	-0.00	50.16	47.75	0.000%
3	0.00	-37.62	-47.75 -47.75	-0.00	37.62	47.75	0.000%
4	22.38	-57.02 -50.16	-47.75 -38.79	-22.38	50.16	38.79	0.000%
5	22.38	-37.62	-38.79 -38.79	-22.38 -22.38	37.62	38.79	0.000%
5 6							
	35.71	-50.16	-20.64	-35.71	50.16	20.64	0.000%
7	35.71	-37.62	-20.64	-35.71	37.62	20.64	0.000%
8	38.35	-50.16	-0.00	-38.35	50.16	0.00	0.000%
9	38.35	-37.62	-0.00	-38.35	37.62	0.00	0.000%
10	39.09	-50.16	22.59	-39.09	50.16	-22.59	0.001%
11	39.09	-37.62	22.59	-39.09	37.62	-22.59	0.000%
12	23.01	-50.16	39.90	-23.01	50.16	-39.90	0.000%
13	23.01	-37.62	39.90	-23.01	37.62	-39.90	0.000%
14	-0.00	-50.16	44.61	0.00	50.16	-44.61	0.000%
15	-0.00	-37.62	44.61	0.00	37.62	-44.61	0.000%
16	-22.38	-50.16	38.79	22.38	50.16	-38.79	0.000%
17	-22.38	-37.62	38.79	22.38	37.62	-38.79	0.000%
18	-38.43	-50.16	22.21	38.43	50.16	-22.21	0.000%
19	-38.43	-37.62	22.21	38.43	37.62	-22.21	0.000%
20	-38.35	<b>-</b> 50.16	0.00	38.35	50.16	-0.00	0.000%
21	-38.35	-37.62	0.00	38.35	37.62	-0.00	0.000%
22	-36.36	-50.16	-21.01	36.36	50.16	21.01	0.000%
23	-36.36	-37.62	-21.01	36.36	37.62	21.01	0.000%
24	-23.01	-50.16	-39.90	23.01	50.16	39.90	0.000%
25	-23.01	-37.62	-39.90	23.01	37.62	39.90	0.000%
26	0.00	-124.90	0.00	0.00	124.90	0.00	0.000%
27	0.00	-124.90	-12.07	-0.00	124.90	12.07	0.000%
28	5.53	-124.90	-9.58	-5.53	124.90	9.58	0.000%
29	8.46	-124.90	-4.89	-8.46	124.90	4.89	0.000%
30	9.38	-124.90	-0.00	-9.38	124.90	0.00	0.000%
31	8.99	-124.90	5.20	-8.99	124.90	-5.20	0.000%
32	5.63	-124.90	9.75	-5.63	124.90	-9.75	0.000%
33	-0.00	-124.90	11.65	0.00	124.90	-11.65	0.000%
34	-5.53	-124.90	9.58	5.53	124.90	-9.58	0.000%
35	-8.82	-124.90	5.10	8.82	124.90	-5.10	0.000%
36	-9.38	-124.90	0.00	9.38	124.90	-0.00	0.000%
37	-8.63	-124.90	-4.99	8.63	124.90	4.99	0.000%
38	-5.63	-124.90	-9.75	5.63	124.90	9.75	0.000%
39	0.00	-41.80	-10.71	-0.00	41.80	10.71	0.000%
40	5.02	-41.80	-8.70	<b>-</b> 5.02	41.80	8.70	0.000%
41	8.01	-41.80	-4.63	-8.01	41.80	4.63	0.000%
42	8.60	-41.80	-0.00	-8.60	41.80	0.00	0.000%
43	8.76	-41.80	5.06	-8.76	41.80	-5.06	0.000%
44	5.16	-41.80	8.95	-5.16	41.80	-8.95	0.000%
45	-0.00	-41.80	10.00	0.00	41.80	-10.00	0.000%
46	-5.02	-41.80	8.70	5.02	41.80	-8.70	0.000%
47	-8.62	-41.80	4.98	8.62	41.80	-4.98	0.000%
48	-8.60	-41.80	0.00	8.60	41.80	-0.00	0.000%
49	-8.15	-41.80	-4.71	8.15	41.80	4.71	0.000%
50	-5.16	-41.80	-8.95	5.16	41.80	8.95	0.000%
	-3.10	-41.00	-0.95	J. 10	41.00	0.80	0.000 /0

# **Non-Linear Convergence Results**

Load	Converged?	Number	Displacement	Force
Combination		of Cycles	Tolerance	Tolerance
1	Yes	4	0.0000001	0.0000001
2	Yes	4	0.0000001	0.0000001
3	Yes	4	0.0000001	0.00000001
4	Yes	4	0.0000001	0.0000001
5	Yes	4	0.0000001	0.00000001
6	Yes	4	0.0000001	0.00000001
7	Yes	4	0.0000001	0.00000001
8	Yes	4	0.0000001	0.00000001

9	Yes	4	0.0000001	0.0000001
10	Yes	4	0.0000001	0.0000001
11	Yes	4	0.0000001	0.0000001
12	Yes	4	0.0000001	0.0000001
13	Yes	4	0.0000001	0.0000001
14	Yes	4	0.0000001	0.00000001
15	Yes	4	0.0000001	0.0000001
16	Yes	4	0.0000001	0.00000247
17	Yes	4	0.0000001	0.00000001
18	Yes	4	0.0000001	0.0000001
19	Yes	4	0.0000001	0.0000001
20	Yes	4	0.0000001	0.0000001
21	Yes	4	0.0000001	0.0000001
22	Yes	4	0.0000001	0.0000001
23	Yes	4	0.0000001	0.0000001
24	Yes	4	0.0000001	0.0000001
25	Yes	4	0.0000001	0.0000001
26	Yes	4	0.0000001	0.0000001
27	Yes	4	0.0000001	0.0000001
28	Yes	4	0.0000001	0.0000001
29	Yes	4	0.0000001	0.0000001
30	Yes	4	0.0000001	0.00000324
31	Yes	4	0.0000001	0.0000001
32	Yes	4	0.0000001	0.0000001
33	Yes	4	0.0000001	0.0000001
34	Yes	4	0.0000001	0.0000001
35	Yes	4	0.0000001	0.0000001
36	Yes	4	0.0000001	0.0000001
37	Yes	4	0.0000001	0.0000001
38	Yes	4	0.0000001	0.0000001
39	Yes	4	0.0000001	0.0000001
40	Yes	4	0.0000001	0.0000001
41	Yes	4	0.0000001	0.0000001
42	Yes	4	0.0000001	0.0000001
43	Yes	4	0.0000001	0.0000001
44	Yes	4	0.0000001	0.0000001
45	Yes	4	0.0000001	0.0000001
46	Yes	4	0.0000001	0.0000001
47	Yes	4	0.0000001	0.0000001
48	Yes	4	0.0000001	0.0000001
49	Yes	4	0.0000001	0.0000001
50	Yes	4	0.0000001	0.0000001

#### **Maximum Tower Deflections - Service Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
T1	170 - 160	2.335	39	0.1219	0.0231
T2	160 - 140	2.074	39	0.1194	0.0225
T3	140 - 120	1.584	39	0.1065	0.0199
T4	120 - 100	1.155	39	0.0884	0.0161
T5	100 - 80	0.800	39	0.0710	0.0123
T6	80 - 60	0.516	39	0.0557	0.0092
T7	60 - 40	0.301	39	0.0398	0.0068
T8	40 - 20	0.147	39	0.0260	0.0045
T9	20 - 0	0.045	39	0.0134	0.0021

### **Critical Deflections and Radius of Curvature - Service Wind**

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	۰	۰	ft
170.00	Lightning Rod 1" x 5' on 5' Pole	39	2.335	0.1219	0.0231	189505
148.00	MX08FRO665-20 w/ Mount Pipe	39	1.773	0.1127	0.0212	80772

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	•	ft
135.00	(2) 7770.00	39	1.470	0.1021	0.0190	70419

## **Maximum Tower Deflections - Design Wind**

Section No.	Elevation	Horz. Deflection	Gov. Load	Tilt	Twist
	ft	in	Comb.	0	۰
T1	170 - 160	10.385	2	0.5447	0.1032
T2	160 - 140	9.217	2	0.5331	0.1006
T3	140 - 120	7.033	2	0.4740	0.0890
T4	120 - 100	5.127	2	0.3926	0.0718
T5	100 - 80	3.549	2	0.3149	0.0549
T6	80 - 60	2.290	2	0.2471	0.0409
T7	60 - 40	1.337	2	0.1765	0.0306
T8	40 - 20	0.653	2	0.1154	0.0200
T9	20 - 0	0.202	2	0.0593	0.0092

## Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	٥	۰	ft
170.00	Lightning Rod 1" x 5' on 5' Pole	2	10.385	0.5447	0.1032	43919
148.00	MX08FRO665-20 w/ Mount Pipe	2	7.876	0.5025	0.0946	18770
135.00	(2) 7770.00	2	6.528	0.4545	0.0850	16297

#### **Bolt Design Data**

Section	Elevation	Component	Bolt	Bolt Size	Number	Maximum	Allowable	Ratio	Allowable	Criteria
No.		Type	Grade		Of	Load	Load	Load	Ratio	
	ft			in	Bolts	per Bolt K	per Bolt K	Allowable		
T1	170	Leg	A325N	0.6250	4	1.65	20.34	0.081	1.05	<b>Bolt Tension</b>
		Diagonal	A325N	0.6250	1	2.93	6.83	0.429	1.05	Member Block Shear
		Top Girt	A325N	0.6250	1	1.11	7.83	0.141	1.05	Member Bearing
T2	160	Leg	A325N	0.7500	4	6.12	30.10	0.203	1.05	Bolt Tension
		Diagonal	A325N	0.7500	1	4.40	11.96	0.368	1.05	Member Block Shear
		Top Girt	A325N	0.6250	1	0.17	7.83	0.021	1.05	Member Bearing
T3	140	Leg	A325N	0.8750	4	12.11	41.56	0.291	1.05	Bolt Tension
		Diagonal	A325N	0.7500	1	5.89	11.96	0.493	1.05	Member Block Shear
T4	120	Leg	A325N	1.0000	4	18.36	54.52	0.337	1.05	<b>Bolt Tension</b>
		Diagonal	A325N	0.7500	1	6.84	14.14	0.484	1.05	Member Bearing
T5	100	Leg	A325N	1.0000	6	15.99	54.52	0.293	1.05	Bolt Tension
		Diagonal	A325N	0.7500	1	8.19	14.14	0.579	1.05	Member Bearing
T6	80	Leg	A325N	1.0000	6	20.04	54.52	0.368	1.05	Bolt Tension
		Diagonal	A325N	0.7500	1	9.23	17.67	0.522	1.05	Member Bearing
T7	60	Leg	A325N	1.0000	8	17.97	54.52	0.330	1.05	Bolt Tension
		Diagonal	A325N	0.7500	1	9.63	17.67	0.545	1.05	Member Bearing
T8	40	Leg	A325N	1.0000	8	20.87	54.52	0.383	1.05	Bolt Tension
		Diagonal	A325N	0.7500	1	10.59	17.67	0.600	1.05	Member

Section	Elevation	Component	Bolt	Bolt Size	Number	Maximum	Allowable	Ratio	Allowable	Criteria
No.		Type	Grade		Of	Load	Load	Load	Ratio	
	ft			in	Bolts	per Bolt	per Bolt	Allowable	<u>-</u>	
						K	K			
Т9	20	Diagonal	A490N	0.7500	1	11.42	21.21	0.538	1.05	Bearing Member Bearing

# Compression Checks

Leg Design Data (Compression)										
Section No.	Elevation	Size	L	Lu	KI/r	Α	Pu	φ <b>P</b> n	Ratio Pu	
	ft		ft	ft		in²	K	K	$\phi P_n$	
T1	170 - 160	ROHN 2.5 STD	10.00	5.00	63.3 K=1.00	1.7040	-8.57	57.19	0.150 <sup>1</sup>	
T2	160 - 140	ROHN 2.5 EH	20.03	6.68	86.7 K=1.00	2.2535	-29.67	58.51	0.507 <sup>1</sup>	
Т3	140 - 120	ROHN 3 EH	20.04	6.68	70.5 K=1.00	3.0159	-57.86	94.34	0.613 <sup>1</sup>	
T4	120 - 100	ROHN 4 EH	20.03	6.68	54.3 K=1.00	4.4074	-86.73	159.91	0.542 <sup>1</sup>	
T5	100 - 80	ROHN 5 EH	20.04	10.02	65.4 K=1.00	6.1120	-113.21	201.23	0.563 <sup>1</sup>	
T6	80 - 60	ROHN 6 EHS	20.03	10.02	54.0 K=1.00	6.7133	-142.54	244.06	0.584 <sup>1</sup>	
T7	60 - 40	ROHN 6 EH	20.04	10.02	54.8 K=1.00	8.4049	-171.64	303.71	0.565 <sup>1</sup>	
Т8	40 - 20	ROHN 8 EHS	20.03	10.02	41.2 K=1.00	9.7193	-200.74	386.40	0.520 <sup>1</sup>	
Т9	20 - 0	ROHN 8 EHS	20.03	10.02	41.2 K=1.00	9.7193	-230.01	386.40	0.595 <sup>1</sup>	

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

Section No.	Elevation	Size	L	Lu	KI/r	Α	$P_u$	$\phi P_n$	Ratio Pu
	ft		ft	ft		in²	K	K	$\phi P_n$
T1	170 - 160	L 2 x 2 x 3/16	9.99	4.74	144.2 K=1.00	0.7150	-2.95	9.84	0.300 1
T2	160 - 140	L2 1/2x2 1/2x1/4	12.31	6.08	148.6 K=1.00	1.1900	-4.50	15.43	0.291 1
Т3	140 - 120	L2 1/2x2 1/2x1/4	14.09	6.94	169.7 K=1.00	1.1900	-5.90	11.83	0.499 1
T4	120 - 100	L3x3x1/4	15.90	7.79	158.0 K=1.00	1.4400	-6.92	16.51	0.419 <sup>1</sup>
T5	100 - 80	L3 1/2x3 1/2x1/4	19.15	9.48	163.8 K=1.00	1.6900	-8.33	18.02	0.462 1
Т6	80 - 60	L4x4x5/16	20.90	10.29	156.1 K=1.00	2.4000	-9.38	28.21	0.332 1
T7	60 - 40	L4x4x5/16	22.78	11.25	170.6 K=1.00	2.4000	-9.77	23.60	0.414 1
T8	40 - 20	L4x4x5/16	24.62	12.06	182.9 K=1.00	2.4000	-10.86	20.53	0.529 <sup>1</sup>
T9	20 - 0	L4x4x3/8	26.46	12.98	197.6 K=1.00	2.8600	-11.75	20.96	0.561 <sup>1</sup>

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

		Top Girt	Desig	Top Girt Design Data (Compression)									
Section No.	Elevation	Size	L	Lu	KI/r	Α	Pu	фРп	Ratio Pu				
	ft		ft	ft		in <sup>2</sup>	K	K	$\Phi P_n$				
T1	170 - 160	L2 1/2x2 1/2x3/16	8.65	8.17	198.0 K=1.00	0.9020	-1.15	6.59	0.175 <sup>1</sup>				
T2	160 - 140	L2 1/2x2 1/2x3/16	8.65	8.17	198.0 K=1.00	0.9020	-0.23	6.59	0.035 1				

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

### **Tension Checks**

Section No.	Elevation	Size	L	Lu	KI/r	Α	$P_u$	$\phi P_n$	Ratio Pu
	ft		ft	ft		in <sup>2</sup>	K	Κ	$\phi P_n$
T1	170 - 160	ROHN 2.5 STD	10.00	5.00	63.3	1.7040	6.59	76.68	0.086
T2	160 - 140	ROHN 2.5 EH	20.03	6.68	86.7	2.2535	24.47	101.41	0.241
T3	140 - 120	ROHN 3 EH	20.04	6.68	70.5	3.0159	48.45	135.72	0.357
T4	120 - 100	ROHN 4 EH	20.03	6.68	54.3	4.4074	73.46	198.34	0.370
T5	100 - 80	ROHN 5 EH	20.04	10.02	65.4	6.1120	95.97	275.04	0.349
T6	80 - 60	ROHN 6 EHS	20.03	10.02	54.0	6.7133	120.27	302.10	0.398
T7	60 - 40	ROHN 6 EH	20.04	10.02	54.8	8.4049	143.78	378.22	0.380
T8	40 - 20	ROHN 8 EHS	20.03	10.02	41.2	9.7193	166.94	437.37	0.382
T9	20 - 0	ROHN 8 EHS	20.03	10.02	41.2	9.7193	189.69	437.37	0.434

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

# Diagonal Design Data (Tension)

Section No.	Elevation	Size	L	Lu	KI/r	Α	Pu	φ <b>P</b> <sub>n</sub>	Ratio Pu
	ft		ft	ft		in²	K	K	<b>φP</b> <sub>n</sub>
T1	170 - 160	L 2 x 2 x 3/16	9.99	4.74	94.4	0.4308	2.93	18.74	0.156 <sup>1</sup>
T2	160 - 140	L2 1/2x2 1/2x1/4	12.31	6.08	97.0	0.7284	4.40	31.69	0.139 <sup>1</sup>
T3	140 - 120	L2 1/2x2 1/2x1/4	14.09	6.94	110.4	0.7284	5.89	31.69	0.186 <sup>1</sup>
T4	120 - 100	L3x3x1/4	15.90	7.79	102.3	0.9159	6.84	44.65	0.153 <sup>1</sup>
T5	100 - 80	L3 1/2x3 1/2x1/4	19.15	9.48	105.8	1.1034	8.19	53.79	0.152 <sup>1</sup>
T6	80 - 60	L4x4x5/16	20.90	10.29	100.9	1.5949	9.23	77.75	0.119 <sup>1</sup>
T7	60 - 40	L4x4x5/16	22.78	11.25	110.1	1.5949	9.63	77.75	0.124 1
T8	40 - 20	L4x4x5/16	24.62	12.06	118.0	1.5949	10.59	77.75	0.136 <sup>1</sup>
Т9	20 - 0	L4x4x3/8	26.46	12.98	127.9	1.8989	11.42	92.57	0.123 <sup>1</sup>

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

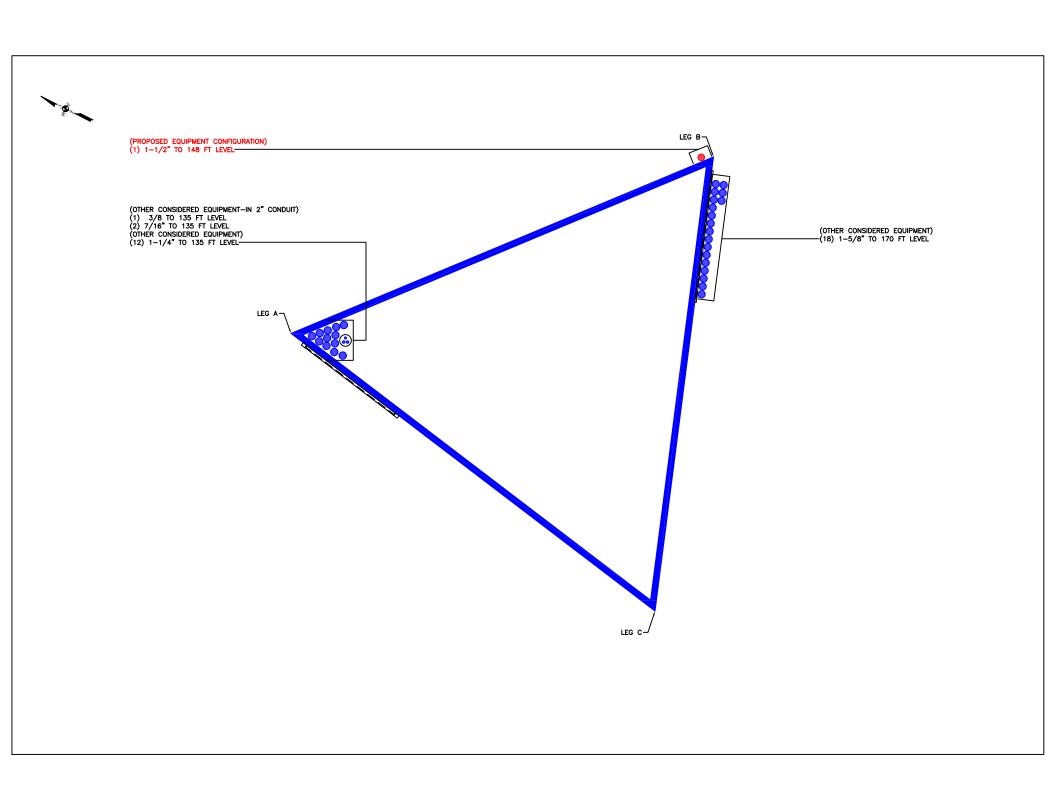
Top Girt Design Data (Tension)									
Section No.	Elevation	Size	L	Lu	KI/r	Α	Pu	φPn	Ratio P <sub>u</sub>
	ft		ft	ft		in²	K	K	$\overline{\phi P_n}$
T1 T2	170 - 160 160 - 140	L2 1/2x2 1/2x3/16 L2 1/2x2 1/2x3/16	8.65 8.65	8.17 8.17	129.7 129.7	0.5710 0.5710	1.11 0.17	24.84 24.84	0.045 <sup>1</sup> 0.007 <sup>1</sup>

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

Section Capacity Tab	le
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Section	Elevation	Component	Size	Critical	Р	øP <sub>allow</sub>	%	Pass
No.	ft	Type		Element	K	K	Capacity	Fail
T1	170 - 160	Leg	ROHN 2.5 STD	2	-8.57	60.05	14.3	Pass
T2	160 - 140	Leg	ROHN 2.5 EH	21	-29.67	61.44	48.3	Pass
T3	140 - 120	Leg	ROHN 3 EH	45	-57.86	99.05	58.4	Pass
T4	120 - 100	Leg	ROHN 4 EH	66	-86.73	167.91	51.7	Pass
T5	100 - 80	Leg	ROHN 5 EH	87	-113.21	211.29	53.6	Pass
T6	80 - 60	Leg	ROHN 6 EHS	102	-142.54	256.27	55.6	Pass
T7	60 - 40	Leg	ROHN 6 EH	117	-171.64	318.90	53.8	Pass
T8	40 - 20	Leg	ROHN 8 EHS	132	-200.74	405.72	49.5	Pass
Т9	20 - 0	Leg	ROHN 8 EHS	147	-230.01	405.72	56.7	Pass
T1	170 - 160	Diagonal	L 2 x 2 x 3/16	10	-2.95	10.33	28.6 40.9 (b)	Pass
T2	160 - 140	Diagonal	L2 1/2x2 1/2x1/4	28	-4.50	16.20	27.8 ´ 35.0 (b)	Pass
T3	140 - 120	Diagonal	L2 1/2x2 1/2x1/4	49	-5.90	12.43	47.Š <sup>′</sup>	Pass
T4	120 - 100	Diagonal	L3x3x1/4	70	-6.92	17.34	39.9 46.1 (b)	Pass
T5	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	91	-8.33	18.92	44.0 ´ 55.1 (b)	Pass
T6	80 - 60	Diagonal	L4x4x5/16	106	-9.38	29.62	31.7 ´ 49.7 (b)	Pass
T7	60 - 40	Diagonal	L4x4x5/16	121	-9.77	24.78	39.4 51.9 (b)	Pass
T8	40 - 20	Diagonal	L4x4x5/16	136	-10.86	21.56	50.4 57.1 (b)	Pass
Т9	20 - 0	Diagonal	L4x4x3/8	151	-11.75	22.00	53.4	Pass
T1	170 - 160	Top Girt	L2 1/2x2 1/2x3/16	4	-1.15	6.92	16.7	Pass
T2	160 - 140	Top Girt	L2 1/2x2 1/2x3/16	22	-0.23	6.92	3.3	Pass
		•					Summary	
						Leg (T3)	58.4	Pass
						Diagonal (T8)	57.1	Pass
						Top Girt (T1)	16.7	Pass
						Bolt Checks	57.1	Pass
						RATING =	58.4	Pass

# APPENDIX B BASE LEVEL DRAWING



# APPENDIX C ADDITIONAL CALCULATIONS

#### **Self Support Anchor Rod Capacity**



Site Info	
BU	# 807134
Site Nam	e NHV 031B 943108
Order	# 553397 Rev 0

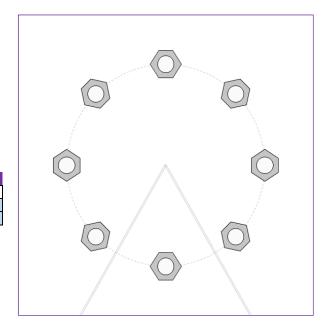
Analysis Considerations	
TIA-222 Revision	Н
Grout Considered:	Yes
I <sub>ar</sub> (in)	3

Applied Loads						
	Comp.	Uplift				
Axial Force (kips)	237.39	195.27				
Shear Force (kips)	29.44	25.07				

<sup>\*</sup>TIA-222-H Section 15.5 Applied

<b>Considered Eccentricity</b>	
Leg Mod Eccentricity (in)	0.000
Anchor Rod N.A Shift (in)	0.000
Total Eccentricity (in)	0.000

<sup>\*</sup>Anchor Rod Eccentricity Applied



Connec	tion	Prop	erti	e
--------	------	------	------	---

Connection Properties	Analysis Results			
Anchor Rod Data	Anchor Rod Summary		(units of kips, kip-in)	
(8) 1" ø bolts (A354-BC N; Fy=109 ksi, Fu=125 ksi)	Pu_t = 24.41	φPn_t = 56.81	Stress Rating	
1 (1)		111 0000		

(8) 1" ø bolts (A35 I<sub>ar</sub> (in): 3

ng Vu = 3.13 φVn = 36.82 Mu = n/a $\phi$ Mn = n/a Pass

CCIplate - Version 4.0.1 Analysis Date: 4/8/2021

#### **SST Unit Base Foundation**

BU # : 807134 Site Name: NHV 031B App. Number: 553397 Rev 0

TIA-222 Revision:



Top & Bot. Pad Rein. Different?:	
Tower Centroid Offset?:	
Block Foundation?:	<b>✓</b>
Rectangular Pad?:	

Superstructure Analysis Reactions				
Global Moment, M:	4777.85	ft-kips		
Global Axial, P:	50.16	kips		
Global Shear, V:	47.75	kips		
Leg Compression, P <sub>comp</sub> :	237.39	kips		
Leg Comp. Shear, <b>V</b> <sub>u_comp</sub> :	29.44	kips		
Leg Uplift, <b>P<sub>uplift</sub></b> :	195.27	kips		
Leg Uplift. Shear, <b>V</b> <sub>u_uplift</sub> :	25.07	kips		
Tower Height, <b>H</b> :	170	ft		
Base Face Width, <b>BW</b> :	25	ft		
BP Dist. Above Fdn, <b>bp</b> <sub>dist</sub> :	4	in		
Anchor Bolt Circle, <b>BC</b> :	14	in		

Foundation Analysis Checks				
	Capacity	Demand	Rating*	Check
Lateral (Sliding) (kips)	325.60	47.75	14.0%	Pass
Bearing Pressure (ksf)	60.00	1.83	2.9%	Pass
Overturning (kip*ft)	11108.76	4984.77	44.9%	Pass
Pad Flexure (kip*ft)	4757.68	981.66	19.7%	Pass
Pad Shear - 1-way (kips)	1464.43	131.64	8.6%	Pass
Pad Shear - Comp 2-way (ksi)	0.164	0.029	17.1%	Pass
Flexural 2-way (Comp) (kip*ft)	2499.77	0.00	0.0%	Pass
Pad Shear - Tension 2-way (ksi)	0.164	0.024	14.0%	Pass
Flexural 2-way (Tension) (kip*ft)	2499.77	0.00	0.0%	Pass

\*Rating per TIA-222-H Section 15.5

Soil Rating*:	44.9%
Structural Rating*:	19.7%

Pad Properties				
Depth, <b>D</b> :	3.50	ft		
Pad Width, <b>W</b> <sub>1</sub> :	34.00	ft		
Pad Thickness, T:	4.00	ft		
Pad Rebar Size (Bottom dir. 2), Sp <sub>2</sub> :	7			
Pad Rebar Quantity (Bottom dir. 2), mp <sub>2</sub> :	41			
Pad Clear Cover, <b>cc</b> <sub>pad</sub> :	3	in		

Material Properties				
Rebar Grade, <b>Fy</b> :	60	ksi		
Concrete Compressive Strength, F'c:	3	ksi		
Dry Concrete Density, δ <b>c</b> :	150	pcf		

Soil Properties					
Total Soil Unit Weight, $\gamma$ :	170	pcf			
Ultimate Gross Bearing, Qult:	80.000	ksf			
Cohesion, Cu:		ksf			
Friction Angle, $oldsymbol{arphi}$ :		degrees			
SPT Blow Count, N <sub>blows</sub> :					
Base Friction, $\mu$ :	0.65				
Neglected Depth, N:	3.3	ft			
Foundation Bearing on Rock?	Yes				
Groundwater Depth, <b>gw</b> :	N/A	ft			

<-- Toggle between Gross and Net



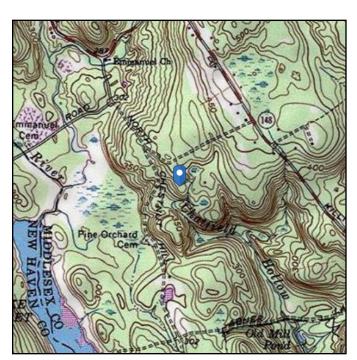
#### Address:

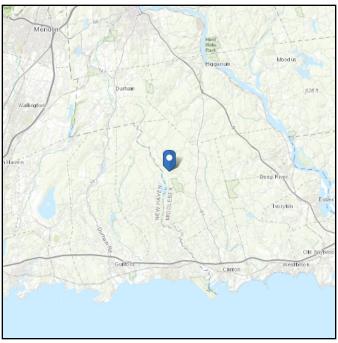
No Address at This Location

# **ASCE 7 Hazards Report**

Standard: ASCE/SEI 7-10 Elevation: 381.75 ft (NAVD 88)

Risk Category: || Latitude: 41.386092 Soil Class: D - Stiff Soil Longitude: -72.603997





#### Wind

#### Results:

Wind Speed: 130 Vmph per Appendix N of 2018 Connecticut State Building Code

10-year MRI 78 Vmph 25-year MRI 88 Vmph 50-year MRI 95 Vmph 100-year MRI 105 Vmph

Date Somessed: Was devise Fig. 26.5-1A and Figs. CC-1–CC-4, and Section 26.5.2, incorporating errata of March 12, 2014

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

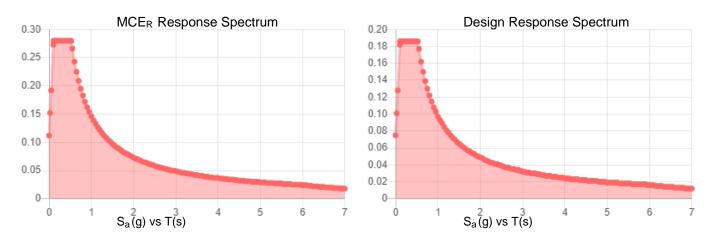
Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.



#### Seismic

Site Soil Class: Results:	D - Stiff Soil			
S <sub>S</sub> :	0.175	S <sub>DS</sub> :	0.186	
$S_1$ :	0.061	S <sub>D1</sub> :	0.097	
$F_a$ :	1.6	T <sub>L</sub> :	6	
F <sub>v</sub> :	2.4	PGA:	0.089	
$S_{MS}$ :	0.28	PGA <sub>M</sub> :	0.142	
S <sub>M1</sub> :	0.146	F <sub>PGA</sub> :	1.6	
		1 .	1	

#### Seismic Design Category B



Data Accessed: Wed Mar 31 2021

Date Source: USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating

Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with

ASCE/SEI 7-10 Ch. 21 are available from USGS.



#### **Ice**

#### Results:

Ice Thickness: 0.75 in.

Concurrent Temperature: 15 F

Gust Speed: 50 mph

**Data Source:** Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Wed Mar 31 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

ASCE does not intend, nor should anyone interpret, the results provided by this Tool to replace the sound judgment of a competent professional, having knowledge and experience in the appropriate field(s) of practice, nor to substitute for the standard of care required of such professionals in interpreting and applying the contents of this Tool or the ASCE 7 standard.

In using this Tool, you expressly assume all risks associated with your use. Under no circumstances shall ASCE or its officers, directors, employees, members, affiliates, or agents be liable to you or any other person for any direct, indirect, special, incidental, or consequential damages arising from or related to your use of, or reliance on, the Tool or any information obtained therein. To the fullest extent permitted by law, you agree to release and hold harmless ASCE from any and all liability of any nature arising out of or resulting from any use of data provided by the ASCE 7 Hazard Tool.

# Exhibit E

**Mount Analysis** 

Date: August 3, 2021

Darcy Tarr Crown Castle 3530 Toringdon Way, Suite 300 Charlotte. NC 28277 (704) 405-6589



Trylon 1825 W. Walnut Hill Lane. Suite 302 Irving, TX 75038 214-930-1730

Subject: Mount Replacement Analysis Report

Carrier Designation: **DISH Network Equipment Change-Out** 

> **Carrier Site Number:** BOBDL00054A Carrier Site Name: CT-CCI-T-807134

**Crown Castle BU Number:** Crown Castle Designation: 807134

**Crown Castle Site Name:** NHV 031B 943108

**Crown Castle JDE Job Number:** 645664 553397 Rev. 2 **Crown Castle Order Number:** 

Engineering Firm Designation: **Trylon Report Designation:** 189059

Site Data: 197 North Chestnut Hill Road, Killingworth, Middlesex County, CT, 06419

Latitude 41°23'9.93" Longitude -72°36'14.39"

Structure Information: **Tower Height & Type:** 170.0 ft Self-Support

> **Mount Elevation:** 148.0 ft

**Mount Type:** 8.0 ft Sector Frame

Dear Darcy Tarr,

Trylon is pleased to submit this "Mount Replacement Analysis Report" to determine the structural integrity of DISH Network's antenna mounting system with the proposed appurtenance and equipment addition on the abovementioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

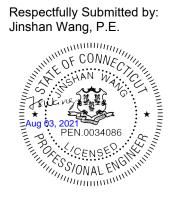
The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

**Sector Frame** Sufficient \*Sufficient upon completion of the changes listed in the 'Recommendations' section of this report.

This analysis utilizes an ultimate 3-second gust wind speed of 130 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount analysis prepared by: Trevor Leahy, E.I.T.

Respectfully Submitted by:



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- 3.2) Assumptions

### 4) ANALYSIS RESULTS

Table 3 - Mount Component Stresses vs. Capacity

Table 4 - Tieback End Reactions

4.1) Recommendations

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Wire Frame and Rendered Models

### 6) APPENDIX B

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#### 7) APPENDIX C

Software Analysis Output

### 8) APPENDIX D

**Additional Calculations** 

#### 9) APPENDIX E

**Supplemental Drawings** 

#### 1) INTRODUCTION

This is a proposed 3 sector 8.0 ft Sector Frame Mount, designed by Commscope.

### 2) ANALYSIS CRITERIA

Building Code: 2015 IBC / 2018 CTSBC

TIA-222 Revision: TIA-222-H

Risk Category:

Ultimate Wind Speed: 130 mph

**Exposure Category:** В Topographic Factor at Base: 1.0 **Topographic Factor at Mount:** 1.0 Ice Thickness: 1.50 in Wind Speed with Ice: 50 mph Seismic Ss: 0.173 Seismic S<sub>1</sub>: 0.061 Live Loading Wind Speed: 30 mph Man Live Load at Mid/End-Points: 250 lb Man Live Load at Mount Pipes: 500 lb

**Table 1 - Proposed Equipment Configuration** 

	Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount / Modification Details
	148.0	.0 148.0	3	JMA Wireless	MX08FRO665-20	0.0 ft Cooter France
			3	Fujitsu	TA08025-B604	8.0 ft Sector Frame
			140.0	3	Fujitsu	TA08025-B605
			1	Raycap	RDIDC-9181-PF-48	WITC3973063]

#### 3) ANALYSIS PROCEDURE

**Table 2 - Documents Provided** 

December 4	Damada	Defenses	0	
Document	Remarks	Reference	Source	
Crown Application	DISH Network Application	553397 Rev. 2	CCI Sites	
Mount Manufacturer Drawings	Commscope	MTC3975083	Trylon	
Tower Analysis	Crown Castle	9742351	CCI Sites	

### 3.1) Analysis Method

RISA-3D (Version 17.0.4), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases.

A tool internally developed, using Microsoft Excel, by Trylon was used to calculate wind loading on all appurtenances, dishes, and mount members for various load cases. Selected output from the analysis is included in Appendix B.

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 *Tower Mount Analysis* (Revision B).

### 3.2) Assumptions

- 1) The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- 2) The configuration of antennas, mounts, and other appurtenances are as specified in Table 1 and the referenced drawings.
- 3) All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- 4) The analysis will be required to be revised if the existing conditions in the field differ from those shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
- 5) Prior structural modifications to the tower mounting system are assumed to be installed as shown per available data.
- 6) Steel grades have been assumed as follows, unless noted otherwise:

Channel, Solid Round, Angle, Plate

HSS (Rectangular)

Pipe

ASTM A36 (GR 36)

ASTM A500 (GR B-46)

ASTM A53 (GR 35)

Connection Bolts

ASTM A325

This analysis may be affected if any assumptions are not valid or have been made in error. Trylon should be notified to determine the effect on the structural integrity of the antenna mounting system.

#### 4) ANALYSIS RESULTS

Table 3 - Mount Component Stresses vs. Capacity (Sector Frame, All Sectors)

Notes	Component	Critical Member	Centerline (ft)	% Capacity	Pass / Fail
	Mount Pipe(s)	MP1		29.4	Pass
	Horizontal(s)	M5		20.4	Pass
1.2	Standoff(s)	M4	148.0	20.3	Pass
1, 2	Bracing(s)	M24	140.0	37.2	Pass
	Tieback(s)	M31A		10.3	Pass
	Mount Connection(s)	-		18.2	Pass

Structure Rating (max from all components) =	29.4%
--	-------

Notes:

- See additional documentation in "Appendix C Software Analysis Output" for calculations supporting the % capacity consumed.
- 2) Rating per TIA-222-H, Section 15.5

#### **Table 4 - Tieback Connection Data Table**

Tower Connection Node No.	Existing / Proposed	Resultant End Reaction (lb)	Connected Member Type	Connected Member Size	Member Compressive Capacity (lb) <sup>3</sup>	Notes
N52A	Proposed	1,016.5	Leg	Rohn 2.5 EH	2,925.6	1

Notes:

1)

- Tieback connection point is within 25% of either end of the connected tower member
- 2) Tieback connection point is NOT within 25% of either end of the connected tower member
- 3) Reduced member compressive capacity according to CED-STD-10294 Standard for Installation of Mounts and Appurtenances

### 4.1) Recommendations

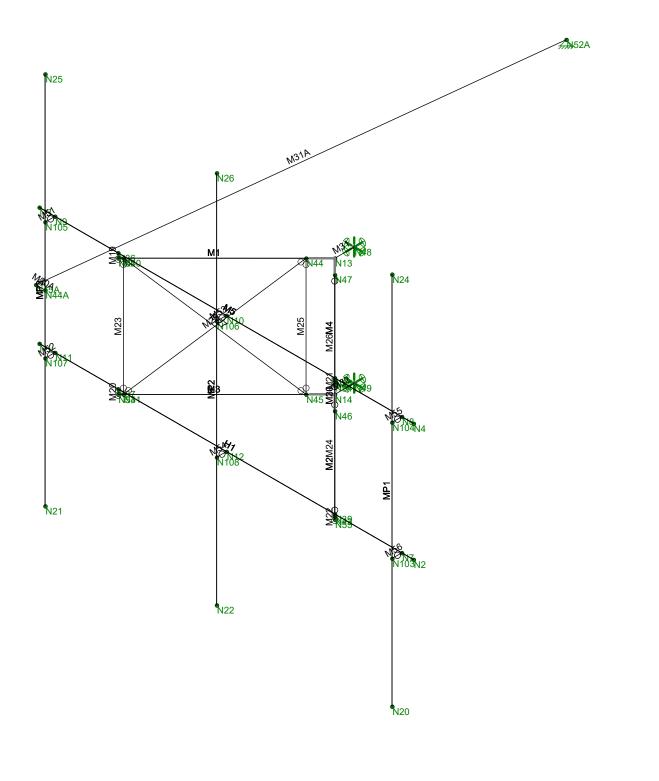
The mount has sufficient capacity to carry the proposed loading configuration. In order for the results of the analysis to be considered valid, the proposed mount listed below must be installed.

1. Commscope MTC3975083. Install tieback connection point within 25% of either end of tower leg.

No structural modifications are required at this time, provided that the above-listed changes are implemented.

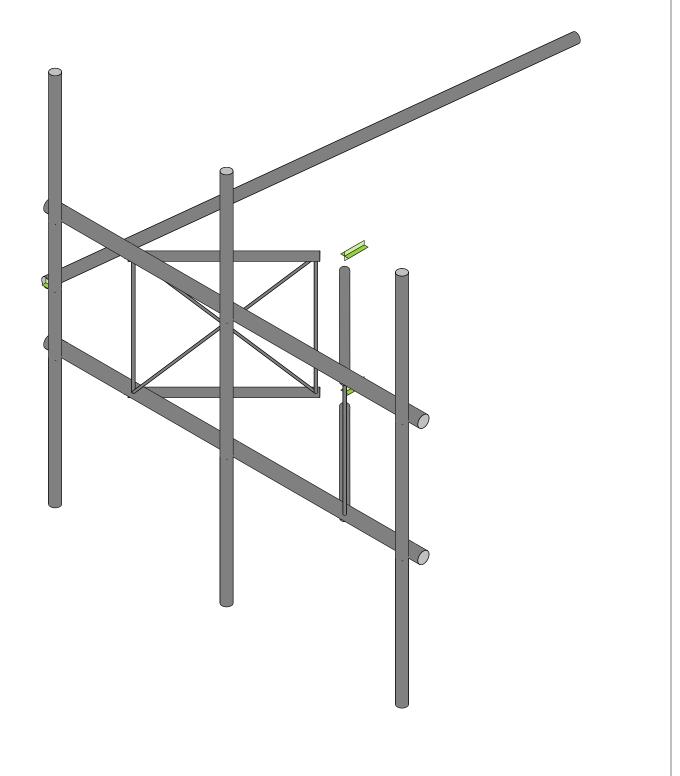
# APPENDIX A WIRE FRAME AND RENDERED MODELS





Trylon		SK - 1
TL	NHV 031B 943108 (BU 807134 Order 553397)	Aug 3, 2021 at 9:24 AM
189059		MTC3975083_loaded.r3d





Trylon	
TL	
189059	

NHV 031B 943108 (BU 807134 Order 553397)

SK - 2
Aug 3, 2021 at 9:25 AM
MTC3975083 loaded.r3d

# APPENDIX B SOFTWARE INPUT CALCULATIONS



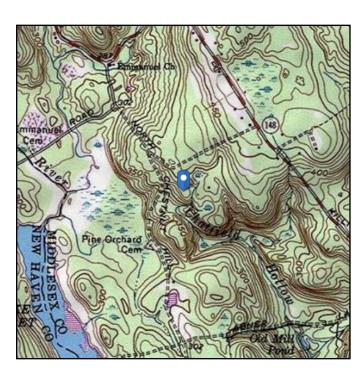
#### Address:

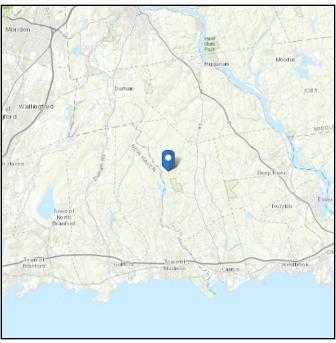
No Address at This Location

# **ASCE 7 Hazards Report**

Standard: ASCE/SEI 7-10 Elevation: 381.75 ft (NAVD 88)

Risk Category: II Latitude: 41.386092 Soil Class: D - Stiff Soil Longitude: -72.603997





Tue Aug 03 2021

### **Ice**

#### Results:

Ice Thickness: 0.75 in.

Concurrent Temperature: 15 F

Gust Speed: 50 mph

**Data Source:** Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

**Date Accessed:** Tue Aug 03 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.



### **TIA LOAD CALCULATOR 2.0**

PROJECT DATA				
Job Code:	189059			
Carrier Site ID:	BOBDL00054A			
Carrier Site Name:	CT-CCI-T-807134			

CODES AND STANDARDS				
Building Code:	2015 IBC			
Local Building Code:	2018 CTSBC			
Design Standard:	TIA-222-H			

STRUCTURE DETAILS				
Mount Type:	Sector Frame			
Mount Elevation:	148.0	ft.		
Number of Sectors:	3			
Structure Type:	Self Support Tower			
Structure Height:	170.0	ft.		

ANALYSIS CRITERIA				
Structure Risk Category:	II			
Exposure Category:	В			
Site Class:	D - Stiff Soil			
Ground Elevation:	381.75	ft.		

TOPOGRAPHIC DATA								
Topographic Category:	1.00							
Topographic Feature:	N/A							
Crest Point Elevation:	0.00	ft.						
Base Point Elevation:	0.00	ft.						
Crest to Mid-Height (L/2):	0.00	ft.						
Distance from Crest (x):	0.00	ft.						
Base Topo Factor (K <sub>zt</sub> ):	1.00							
Mount Topo Factor (K <sub>zt</sub> ):	1.00							

WIND PARAMETERS							
Design Wind Speed:	130	mph					
Wind Escalation Factor (K <sub>s</sub> ):	1.00						
Velocity Coefficient (K <sub>z</sub> ):	1.11						
Directionality Factor (K <sub>d</sub> ):	0.95						
Gust Effect Factor (Gh):	1.00						
Shielding Factor (K <sub>a</sub> ):	0.90						
Velocity Pressure (q <sub>z</sub> ):	44.81	psf					

ICE PARAMETERS							
Design Ice Wind Speed:	50	mph					
Design Ice Thickness (t <sub>i</sub> ):	1.50	in					
Importance Factor (I <sub>i</sub> ):	1.00						
Ice Velocity Pressure (qzi):	44.81	psf					
Mount Ice Thickness (t <sub>iz</sub> ):	1.74	in					

WIND STRUCTURE CALCULATIONS							
Flat Member Pressure:	80.65	psf					
Round Member Pressure:	48.39	psf					
Ice Wind Pressure:	7.52	psf					

SEISMIC PARA	METERS	
Importance Factor (I <sub>e</sub> ):	1.00	
Short Period Accel .(S <sub>s</sub> ):	0.17	g
1 Second Accel (S <sub>1</sub> ):	0.06	g
Short Period Des. $(S_{DS})$ :	0.18	g
1 Second Des. (S <sub>D1</sub> ):	0.10	g
Short Period Coeff. (F <sub>a</sub> ):	1.60	
1 Second Coeff. (F <sub>v</sub> ):	2.40	
Response Coefficient (Cs):	0.09	
Amplification Factor (A <sub>S</sub> ):	1.20	

# **LOAD COMBINATIONS [LRFD]**

#	Description
1	1.4DL
2	1.2DL + 1WL 0 AZI
3	1.2DL + 1WL 30 AZI
4	1.2DL + 1WL 45 AZI
5	1.2DL + 1WL 60 AZI
6	1.2DL + 1WL 90 AZI
7	1.2DL + 1WL 120 AZI
8	1.2DL + 1WL 135 AZI
9	1.2DL + 1WL 150 AZI
10	1.2DL + 1WL 180 AZI
11	1.2DL + 1WL 210 AZI
12	1.2DL + 1WL 225 AZI
13	1.2DL + 1WL 240 AZI
14	1.2DL + 1WL 270 AZI
15	1.2DL + 1WL 300 AZI
16	1.2DL + 1WL 315 AZI
17	1.2DL + 1WL 330 AZI
18	0.9DL + 1WL 0 AZI
19	0.9DL + 1WL 30 AZI
20	0.9DL + 1WL 45 AZI
21	0.9DL + 1WL 60 AZI
22	0.9DL + 1WL 90 AZI
23	0.9DL + 1WL 120 AZI
24	0.9DL + 1WL 135 AZI
25	0.9DL + 1WL 150 AZI
26	0.9DL + 1WL 180 AZI
27	0.9DL + 1WL 210 AZI
28	0.9DL + 1WL 225 AZI
29	0.9DL + 1WL 240 AZI
30	0.9DL + 1WL 270 AZI
31	0.9DL + 1WL 300 AZI
32	0.9DL + 1WL 315 AZI
33	0.9DL + 1WL 330 AZI
34	1.2DL + 1DLi + 1WLi 0 AZI
35	1.2DL + 1DLi + 1WLi 30 AZI
36	1.2DL + 1DLi + 1WLi 45 AZI
37	1.2DL + 1DLi + 1WLi 60 AZI
38	1.2DL + 1DLi + 1WLi 90 AZI
39	1.2DL + 1DLi + 1WLi 120 AZI
40	1.2DL + 1DLi + 1WLi 135 AZI
41	1.2DL + 1DLi + 1WLi 150 AZI

#	Description
42	1.2DL + 1DLi + 1WLi 180 AZI
43	1.2DL + 1DLi + 1WLi 210 AZI
44	1.2DL + 1DLi + 1WLi 225 AZI
45	1.2DL + 1DLi + 1WLi 240 AZI
46	1.2DL + 1DLi + 1WLi 270 AZI
47	1.2DL + 1DLi + 1WLi 300 AZI
48	1.2DL + 1DLi + 1WLi 315 AZI
49	1.2DL + 1DLi + 1WLi 330 AZI
50	(1.2+0.2Sds) + 1.0E 0 AZI
51	(1.2+0.2Sds) + 1.0E 30 AZI
52	(1.2+0.2Sds) + 1.0E 45 AZI
53	(1.2+0.2Sds) + 1.0E 60 AZI
54	(1.2+0.2Sds) + 1.0E 90 AZI
55	(1.2+0.2Sds) + 1.0E 120 AZI
56	(1.2+0.2Sds) + 1.0E 135 AZI
57	(1.2+0.2Sds) + 1.0E 150 AZI
58	(1.2+0.2Sds) + 1.0E 180 AZI
59	(1.2+0.2Sds) + 1.0E 210 AZI
60	(1.2+0.2Sds) + 1.0E 225 AZI
61	(1.2+0.2Sds) + 1.0E 240 AZI
62	(1.2+0.2Sds) + 1.0E 270 AZI
63	(1.2+0.2Sds) + 1.0E 300 AZI
64	(1.2+0.2Sds) + 1.0E 315 AZI
65	(1.2+0.2Sds) + 1.0E 330 AZI
66	(0.9-0.2Sds) + 1.0E 0 AZI
67	(0.9-0.2Sds) + 1.0E 30 AZI
68	(0.9-0.2Sds) + 1.0E 45 AZI
69	(0.9-0.2Sds) + 1.0E 60 AZI
70	(0.9-0.2Sds) + 1.0E 90 AZI
71	(0.9-0.2Sds) + 1.0E 120 AZI
72	(0.9-0.2Sds) + 1.0E 135 AZI
73	(0.9-0.2Sds) + 1.0E 150 AZI
74	(0.9-0.2Sds) + 1.0E 180 AZI
75	(0.9-0.2Sds) + 1.0E 210 AZI
76	(0.9-0.2Sds) + 1.0E 225 AZI
77	(0.9-0.2Sds) + 1.0E 240 AZI
78	(0.9-0.2Sds) + 1.0E 270 AZI
79	(0.9-0.2Sds) + 1.0E 300 AZI
80	(0.9-0.2Sds) + 1.0E 315 AZI
81	(0.9-0.2Sds) + 1.0E 330 AZI
82-88	

#	Description
89	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP1
90	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP1
91	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP1
92	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP1
93	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP1
94	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP1
95	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP1
96	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP1
97	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP1
98	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP1
99	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP1
100	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP1
101	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP1
102	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP1
103	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP1
104	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP1
105	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP2
106	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP2
107	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP2
108	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP2
109	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP2
110	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP2
111	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP2
112	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP2
113	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP2
114	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP2
115	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP2
116	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP2
117	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP2
118	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP2
119	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP2
120	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP2

#	Description
121	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP3
122	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP3
123	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP3
124	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP3
125	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP3
126	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP3
127	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP3
128	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP3
129	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP3
130	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP3
131	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP3
132	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP3
133	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP3
134	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP3
135	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP3
136	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP3
137	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP4
138	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP4
139	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP4
140	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP4
141	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP4
142	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP4
143	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP4
144	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP4
145	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP4
146	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP4
147	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP4
148	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP4
149	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP4
150	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP4
151	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP4
152	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP4

<sup>\*</sup>This page shows an example of maintenance loads for (4) pipes, the number of mount pipe LCs may vary per site

# **EQUIPMENT LOADING**

Appurtenance Name/Location	Qty.	Elevation [ft]		EPA <sub>N</sub> (ft2)	EPA <sub>T</sub> (ft2)	Weight (lbs)
MX08FRO665-20	1	148	No Ice	12.49	5.87	72.00
MP1/5/8, 0/140			w/ Ice	13.72	7.00	286.56
TA08025-B604	1	148	No Ice	1.96	0.98	63.90
MP1/5/8, 0/140			w/ Ice	2.39	1.31	70.48
TA08025-B605	1	148	No Ice	1.96	1.13	75.00
MP1/5/8, 0/140			w/ Ice	2.39	1.48	75.08
RDIDC-9181-PF-48	1	148	No Ice	2.01	1.17	21.85
MP1, 0			w/ Ice	2.45	1.53	74.00
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
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			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
-			No Ice			
			w/ Ice			

# **EQUIPMENT LOADING [CONT.]**

Appurtenance Name/Location	Qty.	Elevation [ft]		EPA <sub>N</sub> (ft2)	EPA <sub>T</sub> (ft2)	Weight (lbs)
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			

# **EQUIPMENT WIND CALCULATIONS**

Appurtenance Name	Qty.	Elevation [ft]	K <sub>zt</sub>	K <sub>z</sub>	<b>K</b> <sub>d</sub>	t <sub>d</sub>	<b>q</b> <sub>z</sub> [psf]	<b>q</b> <sub>zi</sub> [psf]
MX08FRO665-20	1	148	1.00	1.11	0.95	1.74	44.81	6.63
TA08025-B604	1	148	1.00	1.11	0.95	1.74	44.81	6.63
TA08025-B605	1	148	1.00	1.11	0.95	1.74	44.81	6.63
RDIDC-9181-PF-48	1	148	1.00	1.11	0.95	1.74	44.81	6.63

# **EQUIPMENT LATERAL WIND FORCE CALCULATIONS**

Appurtenance Name	Qty.		0° 180°	30° 210°	60° 240°	90° 270°	120° 300°	150° 330°
MX08FRO665-20	1	No Ice	503.64	303.35	436.88	236.59	436.88	303.35
MP1/5/8, 0/140		w/ Ice	81.87	51.79	71.84	41.76	71.84	51.79
TA08025-B604	1	No Ice	79.18	49.47	69.28	39.57	69.28	49.47
MP1/5/8, 0/140		w/ Ice	14.26	9.44	12.66	7.84	12.66	9.44
TA08025-B605	1	No Ice	79.18	53.96	70.77	45.55	70.77	53.96
MP1/5/8, 0/140		w/ Ice	14.26	10.17	12.90	8.80	12.90	10.17
RDIDC-9181-PF-48	1	No Ice	81.13	55.62	72.63	47.11	72.63	55.62
MP1, 0		w/ Ice	14.59	10.49	13.22	9.12	13.22	10.49
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
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		No Ice						
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		No Ice w/ Ice						
		No Ice						
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		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						

# **EQUIPMENT LATERAL WIND FORCE CALCULATIONS [CONT.]**

Appurtenance Name	Qty.		0° 180°	30° 210°	60° 240°	90° 270°	120° 300°	150° 330°
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						

# **EQUIPMENT SEISMIC FORCE CALCULATIONS**

Appurtenance Name	Qty.	Elevation [ft]	Weight [lbs]	<b>F</b> <sub>p</sub> [lbs]
MX08FRO665-20	1	148	72	7.97
TA08025-B604	1	148	63.9	7.08
TA08025-B605	1	148	75	8.30
RDIDC-9181-PF-48	1	148	21.85	2.42

# APPENDIX C SOFTWARE ANALYSIS OUTPUT

Company :
Designer :
Job Number :
Model Name :

: Trylon : TL : 189059

: NHV 031B 943108 (BU 807134 Order 553397)

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## (Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	24
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Υ
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver

Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISAConnection Code	AISC 15th(360-16): LRFD
Cold Formed Steel Code	AISI S100-16: LRFD
Wood Code	AWC NDS-18: ASD
Wood Temperature	< 100F
Concrete Code	ACI 318-14
Masonry Code	TMS 402-16: Strength
Aluminum Code	AA ADM1-15: LRFD - Building
Stainless Steel Code	AISC 14th(360-10): LRFD
Adjust Stiffness?	Yes(Iterative)

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parme Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	Yes
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR_SET_ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8



Company : Trylon Designer : TL Job Number : 189059

: Trylon : TL

Model Name : NHV 031B 943108 (BU 807134 Order 553397)

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# (Global) Model Settings, Continued

Seismic Code	ASCE 7-10
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
RX	3
RZ	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	1
Cd X	1
Rho Z	1
Rho X	1

# **Hot Rolled Steel Properties**

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E	.Density[k/ft	Yield[ksi]	Ry	Fu[ksi]	Rt
1	A992	29000	11154	.3	.65	.49	50	1.1	65	1.1
2	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
3	A572 Gr.50	29000	11154	.3	.65	.49	50	1.1	65	1.1
4	A500 Gr.B RND	29000	11154	.3	.65	.527	42	1.4	58	1.3
5	A500 Gr.B Rect	29000	11154	.3	.65	.527	46	1.4	58	1.3
6	A53 Gr.B	29000	11154	.3	.65	.49	35	1.6	60	1.2
7	A1085	29000	11154	.3	.65	.49	50	1.4	65	1.3
8	A500 Gr. C - 46	29000	11154	.3	.65	.49	46	1.3	62	1.4
9	A529 Gr. 50	29000	11154	.3	.65	.49	50	1.3	65	1.4

# **Cold Formed Steel Properties**

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E5 F)	Density[k/ft^3]	Yield[ksi]	Fu[ksi]
1	A653 SS Gr33	29500	11346	.3	.65	.49	33	45
2	A653 SS Gr50/1	29500	11346	.3	.65	.49	50	65

### **Hot Rolled Steel Section Sets**

	Label	Shape	Type	Design List	Material	Design	A [in2]	lyy [in4]	Izz [in4]	J [in4]
1	Horizontals	PIPE 2.5	Beam	None	A500 Gr. C - 46	Typical	1.61	1.45	1.45	2.89
2	Standoffs	PIPE 1.5	Beam	None	A500 Gr. C - 46	Typical	.749	.293	.293	.586
3	Tie Backs	PIPE 2.0	Beam	None	A500 Gr. C - 46	Typical	1.02	.627	.627	1.25
4	Mount Pipes	PIPE 2.0	Beam	None	A500 Gr. C - 46	Typical	1.02	.627	.627	1.25
5	Standoff Bracing (V	SR 5/8_HRA	Beam	None	A529 Gr. 50	Typical	.307	.007	.007	.015
6	Vertical pipes	PIPE 3.0	Beam	None	A500 Gr. C - 46	Typical	2.07	2.85	2.85	5.69
7	Standoff Bracing (D	SR 1/2"	Beam	None	A529 Gr. 50	Typical	.196	.003	.003	.006



 Company
 : Trylon

 Designer
 : TL

 Job Number
 : 189059

 Model Name
 : NHV 031B 943108 (BU 807134 Order 553397)

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## **Cold Formed Steel Section Sets**

	Label	Shape	Type	Design List	Material	Design Rul	A [in2]	lyy [in4]	Izz [in4]	J [in4]
1	CF1A	8CU1.25X057	Beam	None	A653 SS Gr33	Typical	.581	.057	4.41	.00063

# **Joint Boundary Conditions**

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]
1	N13						
2	N14						
3	N48	Reaction	Reaction	Reaction	Reaction		Reaction
4	N49	Reaction	Reaction	Reaction	Reaction		Reaction
5	N52A	Reaction	Reaction	Reaction			

# **Basic Load Cases**

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed	Area(Me	Surface(
1	Self Weight	DL		-1			7			
2	Structure Wind Z	WLZ						31		
3	Structure Wind X	WLX						31		
4	Wind Load 0 AZI	WLZ					14			
5	Wind Load 30 AZI	None					14			
6	Wind Load 45 AZI	None					14			
7	Wind Load 60 AZI	None					14			
8	Wind Load 90 AZI	WLX					14			
9	Wind Load 120 AZI	None					14			
10	Wind Load 135 AZI	None					14			
11	Wind Load 150 AZI	None					14			
12	Ice Weight	OL1					7	31		
13	Ice Structure Wind Z	OL2						31		
14	Ice Structure Wind X	OL3						31		
15	Ice Wind Load 0 AZI	OL2					14			
16	Ice Wind Load 30 AZI	None					14			
17	Ice Wind Load 45 AZI	None					14			
18	Ice Wind Load 60 AZI	None					14			
19	Ice Wind Load 90 AZI	OL3					14			
20	Ice Wind Load 120 AZI	None					14			
21	Ice Wind Load 135 AZI	None					14			
22	Ice Wind Load 150 AZI	None					14			
23	Seismic Load Z	ELZ			111		7			
24	Seismic Load X	ELX	111				7			
25	Live Load 1 (Lv)	None					1			
26	Live Load 2 (Lv)	None					1			
27	Live Load 3 (Lv)	None					1			
28	Maintenance Load 1 (Lm)	None					1			
29	Maintenance Load 2 (Lm)	None					1			
30	Maintenance Load 3 (Lm)	None					1			

### **Load Combinations**

	Description	S P	S B Fa	. B	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa
1	1.4DL	Yes Y	DL 1.4																		
2	1.2DL + 1WL 0 AZI	Yes Y	DL 1.2	2	1	3		4	1												
3	1.2DL + 1WL 30 AZI	Yes Y	DL 1.2	2	.866	3	.5	5	1												
4	1.2DL + 1WL 45 AZI	Yes Y	DL 1.2	2	.707	3	.707	6	1												
5	1.2DL + 1WL 60 AZI	Yes Y	DL 1.2	2	.5	3	.866	7	1												
6	1.2DL + 1WL 90 AZI	Yes Y	DL 1.2	2		3	1	8	1												



Company Designer Job Number Model Name

: Trylon : TL : 189059

: NHV 031B 943108 (BU 807134 Order 553397)

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# **Load Combinations (Continued)**

	Description	S	P S	R	Fa	R	Fa	R	Fa	R	Fa	R	Fa	R	Fa	R	Fa	R	Fa	R	Fa	B	Fa
7	1.2DL + 1WL 120 AZI	Yes			1.2		5		.866		1	J	<u> </u>	J	<u> </u>	J	<u> </u>	J	. u	J	<u> </u>	J	u
8	1.2DL + 1WL 135 AZI	Yes		DL			7		.707														
9	1.2DL + 1WL 150 AZI	Yes		DL			8	3	.5	_	_												
10	1.2DL + 1WL 180 AZI	Yes		DL			-1	3		4	-1												
11	1.2DL + 1WL 210 AZI	Yes		DL			8	3	5		-1												
12	1.2DL + 1WL 225 AZI	Yes		DL		2	7		7		-1												
13	1.2DL + 1WL 240 AZI	Yes		DL			5		8	7	-1												
14	1.2DL + 1WL 270 AZI	Yes		DL			.0	3	-1	8	-1												
15	1.2DL + 1WL 300 AZI	Yes		DL			.5		8		-1												
16	1.2DL + 1WL 315 AZI	Yes		DL			.707		7														
17	1.2DL + 1WL 330 AZI	Yes		DL	1.2	2	.866		5		-1												
18	0.9DL + 1WL 0 AZI	Yes		DL		2	1	3	.0	4	1												
19	0.9DL + 1WL 30 AZI	Yes		DL	.9		.866		.5	5	1												
20	0.9DL + 1WL 45 AZI	Yes		DL	.9		.707				1												
21	0.9DL + 1WL 60 AZI	Yes		DL	.9	2	.5	3	.866		1												
22	0.9DL + 1WL 90 AZI	Yes		DL	.9	2		3	1	8	1												
23	0.9DL + 1WL 120 AZI	Yes		DL	.9	2	5		.866		1												
24	0.9DL + 1WL 135 AZI	Yes		DL	.9		7		.707		1												
25	0.9DL + 1WL 150 AZI	Yes		DL	a	2	- 8	3	.5		1												
26	0.9DL + 1WL 180 AZI	Yes		DL	.9	2	-1	3	.0	4	-1												
27	0.9DL + 1WL 100 AZI	Yes		DL	.9	2	8	3	5		-1												
28	0.9DL + 1WL 225 AZI	Yes		DL			7		7		-1												
29	0.9DL + 1WL 240 AZI	Yes		DL	.9	2	5		8	7	-1												
30	0.9DL + 1WL 270 AZI	Yes		DL	.9	2	5	3	-1	8	-1												
31	0.9DL + 1WL 300 AZI	Yes		DL		2	.5		8		-1												
32	0.9DL + 1WL 300 AZI	Yes		DL	.9	2	.707	3	7	10													
33	0.9DL + 1WL 330 AZI	Yes		DL	.9		.866																
34	1.2DL + 1DLi + 1WLi 0 AZI	Yes		DL		0		13		14		15	1										
35	1.2DL + 1DLi + 1WLi 30 AZI	Yes		DL		0			.866		.5	16											
36	1.2DL + 1DLi + 1WLi 45 AZI				1.2				.707			17	1										
37	1.2DL + 1DLi + 1WLi 60 AZI	Yes			1.2			13			.866												
38	1.2DL + 1DLi + 1WLi 90 AZI				1.2			13		14		19											
39	1.2DL + 1DLi + 1WLi 120 AZI				1.2				5		866												
40	1.2DL + 1DLi + 1WLi 135 AZI				1.2				7														
41	1.2DL + 1DLi + 1WLi 150 AZI	Ves	V		1.2				8			22											
42	1.2DL + 1DLi + 1WLi 180 AZI			DL		0	1	13				15											
43	1.2DL + 1DLi + 1WLi 210 AZI			DL					8														
44	1.2DL + 1DLi + 1WLi 225 AZI								7														
45	1.2DL + 1DLi + 1WLi 240 AZI			DL				13			8												
46	1.2DL + 1DLi + 1WLi 270 AZI				1.2			13				19											
47	1.2DL + 1DLi + 1WLi 300 AZI			DI	1.2	0	1	13	5	1/1	- 8	20	_1										
48	1.2DL + 1DLi + 1WLi 315 AZI						1																
49	1.2DL + 1DLi + 1WLi 330 AZI				1.2				.866														
50					1.2			24		14	5	22											
51	(1.2+0.2Sds)DL + 1E 30 AZI						.866																
52	(1.2+0.2Sds)DL + 1E 45 AZI						.707																
53	(1.2+0.2Sds)DL + 1E 60 AZI						.5																
54	(1.2+0.2Sds)DL + 1E 00 AZI			DI	1.2	22	.ü	<del>24</del> 24															
	(1.2+0.2Sds)DL + 1E 90 AZI			DI	1.2	22	5																
<u>55</u>	(1.2+0.2Sds)DL + 1E 120 AZI						5 7																
<u>56</u>	(1.2+0.2Sds)DL + 1E 155 AZI						8																
57	(1.2+0.2Sds)DL + 1E 180 AZI			DL	1.2	23	o -1	24	.၁														
58	(1.2+0.2Sds)DL + 1E 180 AZI																						
59	(1.2+0.2Sds)DL + 1E 210 AZI						8 7																
60	(1.2+0.2Sds)DL + 1E 225 AZI (1.2+0.2Sds)DL + 1E 240 AZI																						
61							5																
62	(1.2+0.2Sds)DL + 1E 270 AZI	_			1.2				-1														
63	(1.2+0.2Sds)DL + 1E 300 AZI	res	Υ	IJL	1.Z	23	.5	<u> 24</u>	o														<u> </u>



Company Designer Job Number Model Name

: Trylon : TL : 189059

: NHV 031B 943108 (BU 807134 Order 553397)

Aug 3, 2021 9:24 AM Checked By:\_

# **Load Combinations (Continued)**

	Description	S P	S B	Fa	B	Fa	В	Fa	R	Fa	R	Fa	R	Fa	В	Fa	R	Fa	R	Fa	R	Fa
64	(1.2+0.2Sds)DL + 1E 315 AZI	Yes Y		1.2.					J	1 a		1 a	<u>ا</u>	1 a		1 a	<u>ں۔۔۔</u>	a	J	1 a	D	1 a
65	(1.2+0.2Sds)DL + 1E 330 AZI			1.2.																		
66	,	Yes Y		863			24	.0														
67		Yes Y		.863	23	.866	24	.5														
68		Yes Y		863																		
69	( )	Yes Y	D		23			.866														
70	(0.9-0.2Sds)DL + 1E 90 AZI			863			24	1														
71	(0.9-0.2Sds)DL + 1E 120 AZI		D					.866														
72	(0.9-0.2Sds)DL + 1E 135 AZI			863																		
73	(0.9-0.2Sds)DL + 1E 150 AZI	1 1		.863																		
74	(0.9-0.2Sds)DL + 1E 180 AZI			.863			24															
75	(0.9-0.2Sds)DL + 1E 210 AZI			863				- 5														
76	(0.9-0.2Sds)DL + 1E 225 AZI			.863																		
77	(0.9-0.2Sds)DL + 1E 240 AZI			.863																		
78	(0.9-0.2Sds)DL + 1E 270 AZI			863			24	-1														
79	(0.9-0.2Sds)DL + 1E 300 AZI			.863																		
80	(0.9-0.2Sds)DL + 1E 315 AZI			L .863																		
81	(0.9-0.2Sds)DL + 1E 330 AZI		D			.866																
82	1.2DL + 1Lv1	Yes Y		L 1.2																		
83	1.2DL + 1Lv2	Yes Y		L 1.2																		
84	1.2DL + 1Lv3	Yes Y		L 1.2																		
	1.2DL + 1.5Lm + 1Wm 0 AZI	Yes Y		L 1.2				.053	3		4	.053										
86	1.2DL + 1.5Lm + 1Wm 30 AZI	Yes Y		L 1.2						.027	5	.053										
87	1.2DL + 1.5Lm + 1Wm 45 AZI	Yes Y		L 1.2				.038				.053										
88	1.2DL + 1.5Lm + 1Wm 60 AZI	Yes Y	D	L 1.2	28	1.5	2	.027	3	.046	7	.053										
	1.2DL + 1.5Lm + 1Wm 90 AZI		D	L 1.2	28	1.5	2		3	.053	8	.053										
90	1.2DL + 1.5Lm + 1Wm 120 A	Yes Y		L 1.2				0	3	.046	9	.053										
	1.2DL + 1.5Lm + 1Wm 135 A		D	L 1.2	28	1.5	2	0	3	.038	10	.053										
92	1.2DL + 1.5Lm + 1Wm 150 A	Yes Y	D	L 1.2	28	1.5	2	0	3	.027	11	.053										
93	1.2DL + 1.5Lm + 1Wm 180 A	Yes Y	D	L 1.2	28	1.5	2	0	3		4	0										
	1.2DL + 1.5Lm + 1Wm 210 A		D	L 1.2	28	1.5	2	0		0	5	0										
	1.2DL + 1.5Lm + 1Wm 225 A			L 1.2	28	1.5		0		0		0										
	1.2DL + 1.5Lm + 1Wm 240 A			L 1.2				0		0		0										
	1.2DL + 1.5Lm + 1Wm 270 A			L 1.2						0		0										
	1.2DL + 1.5Lm + 1Wm 300 A			L 1.2						0												
	1.2DL + 1.5Lm + 1Wm 315 A			L 1.2				.038		0												
	1.2DL + 1.5Lm + 1Wm 330 A			L 1.2				.046		0												
	1.2DL + 1.5Lm + 1Wm 0 AZI			L 1.2				.053			4										$oxed{oxed}$	
	1.2DL + 1.5Lm + 1Wm 30 AZI			L 1.2								.053										
	1.2DL + 1.5Lm + 1Wm 45 AZI			L 1.2						.038		.053									<u> </u>	
	1.2DL + 1.5Lm + 1Wm 60 AZI			L 1.2				.027														
	1.2DL + 1.5Lm + 1Wm 90 AZI.			L 1.2				_				.053										
	1.2DL + 1.5Lm + 1Wm 120 A			L 1.2							_	.053										
	1.2DL + 1.5Lm + 1Wm 135 A			L 1.2				0	_			.053										
	1.2DL + 1.5Lm + 1Wm 150 A			L 1.2				0	_	.027		.053										
	1.2DL + 1.5Lm + 1Wm 180 A			L 1.2				0	3		_	0										
	1.2DL + 1.5Lm + 1Wm 210 A			L 1.2				0	_	0		0										
	1.2DL + 1.5Lm + 1Wm 225 A			L 1.2				0	_	0		0										
	1.2DL + 1.5Lm + 1Wm 240 A			L 1.2				0	_	0		0										
	1.2DL + 1.5Lm + 1Wm 270 A			L 1.2				007	_	0		0										
	1.2DL + 1.5Lm + 1Wm 300 A			L 1.2				.027	_	0		0										
	1.2DL + 1.5Lm + 1Wm 315 A			L 1.2				.038	_	0												
	1.2DL + 1.5Lm + 1Wm 330 A			L 1.2				.046		0												
	1.2DL + 1.5Lm + 1Wm 0 AZI			L 1.2				.053		007		.053										
	1.2DL + 1.5Lm + 1Wm 30 AZI			L 1.2								.053										
	1.2DL + 1.5Lm + 1Wm 45 AZI			L 1.2				.038				.053										
120	1.2DL + 1.5Lm + 1Wm 60 AZI	res Y	l D	L 1.2	30	1.5	2	.027	3	.046	/	.053										



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# **Load Combinations (Continued)**

Description	S	P	S	B Fa	. B	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa
121 1.2DL + 1.5Lm + 1Wm 90 AZI.	.Yes	Υ		DL 1.2	30	1.5	2		3	.053	8	.053										
122 1.2DL + 1.5Lm + 1Wm 120 A	Yes	Υ		DL 1.2	30	1.5	2	0	3	.046	9	.053										
123 1.2DL + 1.5Lm + 1Wm 135 A	Yes	Υ		DL 1.2	30	1.5	2	0	3	.038	10	.053										
124 1.2DL + 1.5Lm + 1Wm 150 A	Yes	Υ		DL 1.2	30	1.5	2	0	3	.027	11	.053										
125 1.2DL + 1.5Lm + 1Wm 180 A	Yes	Υ		DL 1.2	30	1.5	2	0	3		4	0										
126 1.2DL + 1.5Lm + 1Wm 210 A	Yes	Υ		DL 1.2	30	1.5	2	0	3	0	5	0										
127 1.2DL + 1.5Lm + 1Wm 225 A	Yes	Υ		DL 1.2	30	1.5	2	0	3	0	6	0										
128 1.2DL + 1.5Lm + 1Wm 240 A	Yes	Υ		DL 1.2	30	1.5	2	0	3	0	7	0										
129 1.2DL + 1.5Lm + 1Wm 270 A	Yes	Υ		DL 1.2	30	1.5	2		3	0	8	0										
130 1.2DL + 1.5Lm + 1Wm 300 A	Yes	Υ		DL 1.2	30	1.5	2	.027	3	0	9	0										
131 1.2DL + 1.5Lm + 1Wm 315 A	Yes	Υ		DL 1.2	30	1.5	2	.038	3	0	10	0										
132 1.2DL + 1.5Lm + 1Wm 330 A	Yes	Υ		DL 1.2	30	1.5	2	.046	3	0	11	0										

# **Envelope Joint Reactions**

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-ft]	LC	MY [lb-ft]	LC	MZ [lb-ft]	LC
1	N48	max	582.946	89	1536.374	41	923.737	18	-79.262	18	0	132	645.717	132
2		min	-1183.74	129	110.033	18	-1758.52	10	-1062.458	41	0	1	-192.158	92
3	N49	max	1176.59	122	662.814	101	1586.144	49	112.981	124	0	132	277.301	84
4		min	-575.301	98	-53.404	124	-437.516	25	-519.317	101	0	1	-193.703	92
5	N52A	max		2	66.47		989.455		0	132	0	132	0	132
6		min	-87.047	9	15.358	33	-998.509	17	0	1	0	1	0	1
7	Totals:	max	780.625	22	1763.519	41	1278.891	2						
8		min	-780.634	14	422.438	71	-1278.887	26						

### Envelope AISC 15th(360-16): LRFD Steel Code Checks

	Member	Shape	Code	Loc[in]	LC	Shear	.Loc[in]Di	rLC	phi*Pnc	phi*Pnt	phi*Mn	phi*Mn	Cb	Eqn
1	M24	SR 5/8_HRA_H	.391	24.8	125	.029	0	17	1849.145	13805.82	143.808	143.808	1	H1-1a
2	M29	SR 1/2"	.350	22.4	37	.013	44.8	17	1432.022	8835.75	73.632	73.632	1	H1-1a
3	MP1	PIPE 2.0	.309	33	2	.141	33	2	15369.6	42228	2459.85	2459.85	1.363	H1-1b
4	MP3	PIPE 2.0	.250	48	17	.170	33	17	15369.6	42228	2459.85	2459.85	1.712	H1-1b
5	M5	PIPE 2.5	.214	21	17	.052	76	1	45255.2	66654	4726.5	4726.5	1	H1-1b
6	M4	PIPE 1.5	.213	34.81	130	.196	34.81	1	23485.28	31008.6	1452.45	1452.45	1	H1-1b
7	M2	PIPE 1.5	.191	34.81	120	.122	.725	1	23485.28	31008.6	1452.45	1452.45	1.334	H1-1b
8	M3	PIPE 1.5	.181	34.81	122	.084	34.81	1	23485.28	31008.6	1452.45	1452.45	1	H1-1b
9	H1	PIPE 2.5	.170	76	117	.056	76	1	45255.2	66654	4726.5	4726.5	1.975	H1-1b
10	M1	PIPE 1.5	.170	34.81	131	.083	34.81	91	23485.28	31008.6	1452.45	1452.45	1	H1-1b
11	M28	SR 1/2"	.139	22.4	48	.017	44.8	17	1432.022	8835.75	73.632	73.632	1	H1-1b
12	M31A	PIPE 2.0	.108	0	9	.005	123	46	9324.69	42228	2459.85	2459.85	1.136	H1-1b*
13	M26	SR 5/8_HRA_H	.035	30.25	41	.056	0	1	1849.145	13805.82	143.808	143.808	1	H1-1b*
14	M23	SR 5/8_HRA_H	.034	30.25	9	.029	0	17	1849.145	13805.82	143.808	143.808	1	H1-1b*
15	MP2	PIPE 2.0	.025	63	109	.137	33	17	15369.6	42228	2459.85	2459.85	1.149	H1-1b*
16	M25	SR 5/8_HRA_H	.015	15.44	10	.056	0	1	1849.145	13805.82	143.808	143.808	1	H1-1b
17	M27	SR 1/2"	.003	44.8	25	.013	44.8	2	1432.022	8835.75	73.632	73.632	1	H1-1b*
18	M30	SR 1/2"	.000	0	132	.009	0	2	1432.022	8835.75	73.632	73.632	1	H1-1a

## Envelope AISI S100-16: LRFD Cold Formed Steel Code Checks

Member	Shape	Code Check	Loc[in] LC SheaLoc	[iDirLC phi*Pn[	phi*Tn[.	phi*Mn	phi*Mnphi*.	phi*	Cb	Egn
	•		No Data to	o Print		•				

# APPENDIX D ADDITIONAL CALCULATIONS

Analysis date: 8/3/2021

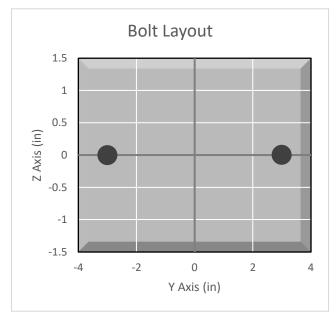


### **BOLT TOOL 1.5.2**

Projec	t Data
Job Code:	189059
Carrier Site ID:	BOBDL00054A
Carrier Site Name:	CT-CCI-T-807134

Co	de
Design Standard:	TIA-222-H
Slip Check:	Yes
Pretension Standard:	TIA-222-H

Bolt Pro	perties	
Connection Type:	Thread	led Rod
Diameter:	0.75	in
Grade:	A529	
Yield Strength (Fy):	50	ksi
Ultimate Strength (Fu):	65	ksi
Number of Bolts:	2	
Threads Included:	Yes	
Double Shear:	No	
Distance Between Rods:	6	in



Connection Description
Mount Standoff to Tower Leg

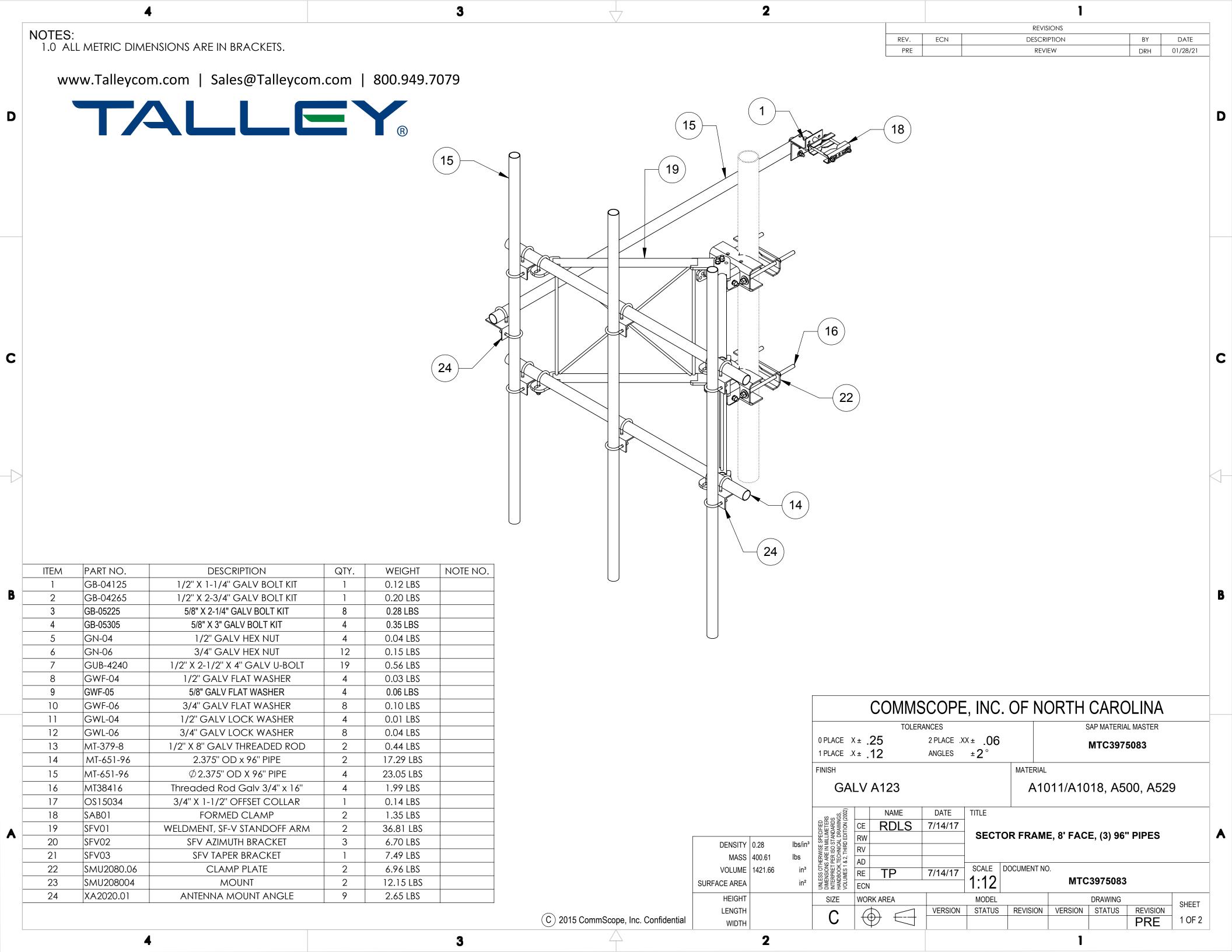
Bolt Check*		
Tensile Capacity $(\phi T_n)$ :		lbs
Shear Capacity $(\phi V_n)$ :		lbs
Tension Force (T <sub>u</sub> ):		lbs
Shear Force (V <sub>u</sub> ):	2055.3	lbs
Tension Usage:	3.2%	
Shear Usage:	18.2%	
Interaction:	18.2%	Pass
Controlling Member:	M31	
Controlling LC:	132	

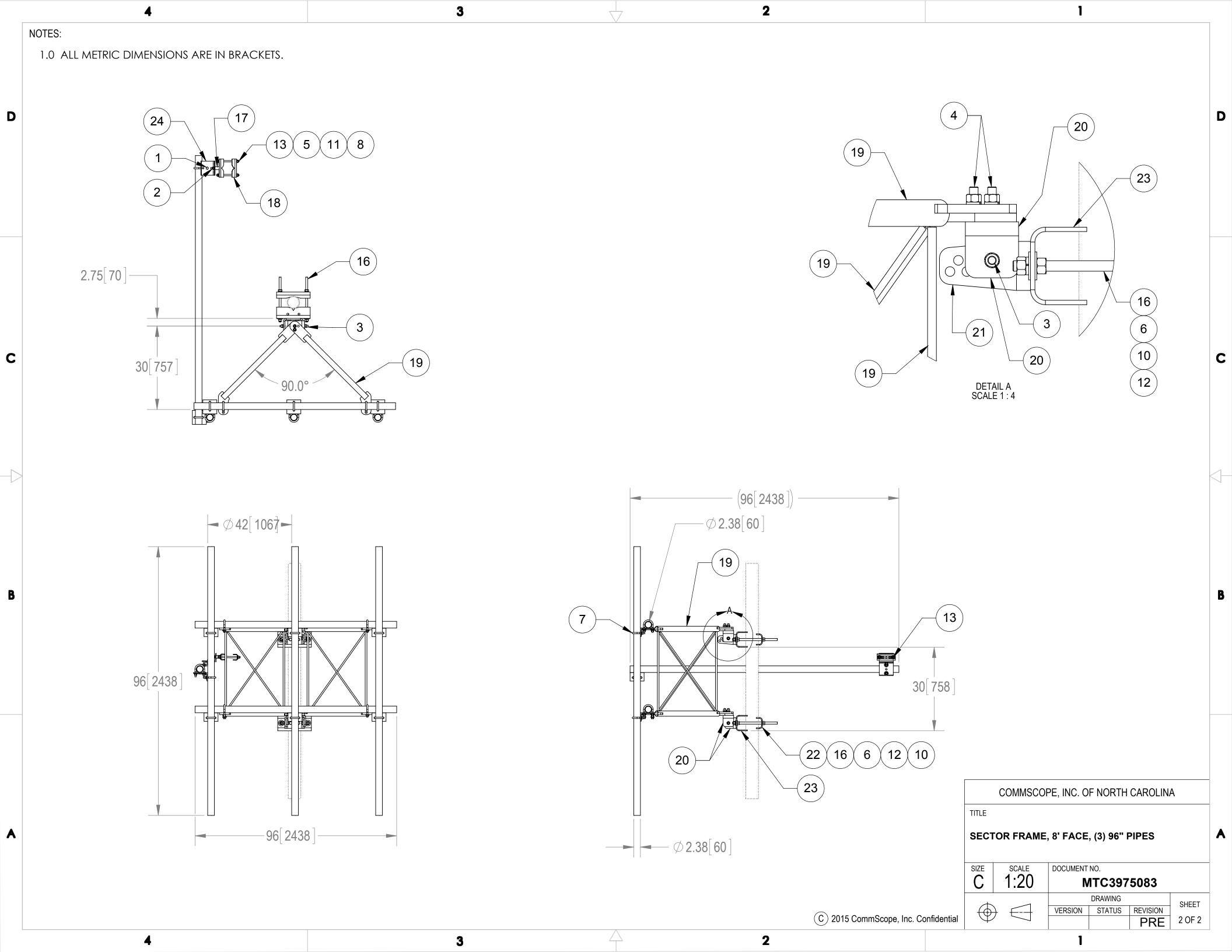
\*Rating per TIA-222-H Section 15.5

Slip Check*		
Sliding Capacity ( $\phi R_{ns}$ ):		lbs
Torsion Capacity (φR <sub>nr</sub> ):		lb-ft
Sliding Force (V <sub>us</sub> ):		lbs
Torsional Force (T <sub>ur</sub> ):	0.0	lb-ft
Sliding Usage:	15.2%	
Torsion Usage:	0.0%	
Interaction:	15.2%	Pass
Controlling Member:	M31	
Controlling LC:	41	

<sup>\*</sup>Rating per TIA-222-H Section 15.5

# APPENDIX E SUPPLEMENTAL DRAWINGS





# Exhibit F

**Power Density/RF Emissions Report** 



# RF EMISSIONS COMPLIANCE REPORT

# **Crown Castle on behalf of Dish Wireless**

Crown Castle Site Name: NHV 031B 943108
Crown Castle Site BU Number: 807134
Dish Wireless Site Name: CT-CCI-T-807134
Dish Wireless Site ID: BOBDL00054A
Application ID: 553397
197 N. Chestnut Hill Road
Killingworth, CT
5/27/2021

# **Report Status:**

**Dish Wireless is Compliant** 

Signed 27 May 2021

**Prepared By:** 

Site Safe, LLC

8618 Westwood Center Drive Suite 315

Vienna, VA 22182

Voice: 703-276-1100 Fax: 703-276-1169

### Engineering Statement in Re: Electromagnetic Energy Analysis Crown Castle Killingworth, CT

My signature on the cover of this document indicates:

That I am registered as a Professional Engineer in the jurisdiction indicated; and

That I have extensive professional experience in the wireless communications engineering industry; and

That I am an employee of Site Safe, LLC in Vienna, Virginia; and

That I am thoroughly familiar with the Rules and Regulations of the Federal Communications Commission ("the FCC" and "the FCC Rules") both in general and specifically as they apply to the FCC's Guidelines for Human Exposure to Radio Frequency Electromagnetic Fields; and

That the technical information serving as the basis for this report was supplied by Crown Castle on behalf of Dish Wireless (see attached Site Summary and Carrier documents) and that Dish Wireless' installation involves communications equipment, antennas and associated technical equipment at a location referred to as "NHV 031B 943108" ("the site"); and

That Dish Wireless proposes to operate at the site with transmit antennas listed in the carrier summary and with a maximum effective radiated power as specified by Dish Wireless and shown on the worksheet and that worst-case 100% duty cycle has been assumed; and

That this analysis has been performed with the assumption that the ground immediately surrounding the tower is primarily flat or falling; and

That at this time, the FCC requires that certain licensees address specific levels of radio frequency energy to which workers or members of the public might possibly be exposed (at §1.1307(b) of the FCC Rules); and

That such consideration of possible exposure of humans to radio frequency energy must utilize the standards set by the FCC, which is the federal agency having jurisdiction over communications facilities; and

That the FCC rules define two tiers of permissible exposure guidelines: 1) "uncontrolled environments," which defines situations in which persons may not be aware of (the "general public"), or may not be able to control their exposure to a transmission facility; and 2) "controlled environments," which defines situations in which persons are aware of their potential for exposure (industry personnel); and

That this statement specifically addresses the uncontrolled environment (which is more conservative than the controlled environment) and the limit set forth in the FCC rules for licensees of Dish Wireless' operating frequencies as shown on the attached antenna worksheet; and

That when applying the uncontrolled environment standards, the predicted Maximum Power Density at two meters above ground level from the proposed T-Mobile operation is no more than 0.881% of the maximum permissible exposure limits in any accessible area on the ground; and

That it is understood per FCC Guidelines and OET 65 Appendix A, that regardless of the existent radio frequency environment, only those licensees whose contributions exceed 5% of the exposure limit pertinent to their operation(s) bear any responsibility for bringing any non-compliant area(s) into compliance; and

That when applying the uncontrolled environment standards, the cumulative predicted energy density from the proposed operation is no more than 4.972% of the maximum in any accessible area up to two meters above the ground per OET 65; and

That the calculations provided in this report are based on data provided by the client and antenna pattern data supplied by the antenna manufacturer, in accordance with FCC guidelines listed in OET 65. Horizontal and vertical antenna patterns are combined for modeling purposes to accurately reflect the energy two meters above ground level where on-axis energy refers to maximum energy two meters above the ground along the azimuth of the antenna and where area energy refers to the maximum energy anywhere two meters above the ground regardless of the antenna azimuth, accounting for cumulative energy from multiple antennas for the carrier(s) and frequency range(s) indicated; and

That the Occupational Safety and Health Administration has policies in place which address worker safety in and around communications sites, thus individual companies will be responsible for their employees' training regarding radio frequency safety; and

In summary, it is stated here that the proposed operation at the site will not result in exposure of the public to excessive levels of radio frequency energy as defined in the FCC Rules and Regulations, specifically 47 CFR 1.1307(b), and that Dish Wireless' proposed operation is completely compliant.

Finally, it is stated that access to the tower should be restricted to communication industry professionals and approved contractor personnel trained in radio frequency safety and that this instant analysis addresses exposure levels at two meters above ground level and does not address exposure levels on the tower or in the immediate proximity of the antennas.

Note: Sprint currently has (4) 850 MHz panel antennas (Decibel DB844H90E-SX) on each of three sectors ((12) total antennas) spaced with azimuths of 50/170/290 degrees at a centerline of 147' above ground level. These antennas have an "abandoned" status in the CCI database and are currently inactive and were therefore not included in this analysis.

# Crown Castle NHV 031B 943108 Site Summary

Carrier	Area Maximum Percentage MPE	
AT&T Mobility, LLC	0.395 %	
AT&T Mobility, LLC	0.463 %	
AT&T Mobility, LLC	0.107 %	
AT&T Mobility, LLC (Not in service)	0.000 %	
Dish Wireless (Proposed)	0.236 %	
Dish Wireless (Proposed)	0.236 %	
Dish Wireless (Proposed)	0.409 %	
Verizon Wireless	1.671 %	
Verizon Wireless	0.352 %	
Verizon Wireless	0.390 %	
Verizon Wireless	0.297 %	
Verizon Wireless	0.416 %	
Composite Site MPE:	4.972 %	

## AT&T Mobility, LLC NHV 031B 943108 Carrier Summary

Antenna Make					On A	Max Power Density (μW/cm²)         Percent of MPE         Max Power Density (μW/cm²)           0.913067         0.091307         1.522591           2.147790         0.214779         3.906048		
	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	Density			Percent of MPE
KMW	AM-X-CD-16-65-00T	136	115	3666	0.913067	0.091307	1.522591	0.152259
Powerwave	P65-16-XLH-RR	136	235	5130	2.147790	0.214779	3.906048	0.390605
KMW	AM-X-CD-16-65-00T	136	345	3666	0.913067	0.091307	1.522591	0.152259

## AT&T Mobility, LLC NHV 031B 943108 Carrier Summary

					On Axis		Area	
Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	Max Power Density (μW/cm²)	Percent of MPE	Max Power Density (μW/cm²)	Percent of MPE
KMW	AM-X-CD-16-65-00T	136	115	1239	0.704321	0.143349	0.738210	0.150246
Powerwave	P65-16-XLH-RR	136	235	2952	1.235923	0.251545	2.247694	0.457468
KMW	AM-X-CD-16-65-00T	136	345	1239	0.704321	0.143349	0.738210	0.150246

## AT&T Mobility, LLC NHV 031B 943108 Carrier Summary

Antenna Make					On A	Axis	Area		
	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	Max Power Density (μW/cm²)	Percent of MPE	Max Power Density (μW/cm²)	Percent of MPE	
Powerwave	7770	136	115	547	0.283263	0.049988	0.443577	0.078278	
Powerwave	7770	136	235	547	0.283263	0.049988	0.443577	0.078278	
Powerwave	7770	136	345	547	0.283263	0.049988	0.443577	0.078278	

# AT&T Mobility, LLC (Not in service) NHV 031B 943108 Carrier Summary

 $\begin{tabular}{lllll} \textbf{Frequency:} & 850 & MHz \\ \textbf{Maximum Permissible Exposure (MPE):} & 566.67 & $\mu W/cm^2$ \\ \textbf{Maximum power density at ground level:} & 0.00000 & $\mu W/cm^2$ \\ \textbf{Highest percentage of Maximum Permissible Exposure:} & 0.00000 & \% \\ \end{tabular}$ 

					On A	Axis	Area		
Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	Max Power Density (μW/cm²)	Percent of MPE	Max Power Density (μW/cm²)	Percent of MPE	
Powerwave	7770	136	115	0	0.000000	0.000000	0.000000	0.000000	
Powerwave	7770	136	235	0	0.000000	0.000000	0.000000	0.000000	
Powerwave	7770	136	345	0	0.000000	0.000000	0.000000	0.000000	

## Dish Wireless (Proposed) NHV 031B 943108 Carrier Summary

 $\begin{tabular}{lllll} Frequency: & 2100 & MHz \\ Maximum Permissible Exposure (MPE): & 1000 & $\mu W/cm^2$ \\ Maximum power density at ground level: & 2.35554 & $\mu W/cm^2$ \\ Highest percentage of Maximum Permissible Exposure: & 0.23555 & % \\ \end{tabular}$ 

					On /	Axis	Ar	ea
Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	Max Power Density (μW/cm²)	Percent of MPE	Max Power Density (µW/cm²)	Percent of MPE
JMA Wireless	MX08FRO665-20	148	0	6904	1.300512	0.130051	2.325472	0.232547
JMA Wireless	MX08FRO665-20	148	120	6904	1.300512	0.130051	2.325472	0.232547
JMA Wireless	MX08FRO665-20	148	240	6904	1.300512	0.130051	2.325472	0.232547

## Dish Wireless (Proposed) NHV 031B 943108 Carrier Summary

					On <i>I</i>	Axis	Ar	Area		
Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	Max Power Density (μW/cm²)	Percent of MPE	Max Power Density (μW/cm²)	Percent of MPE		
JMA Wireless	MX08FRO665-20	148	0	6904	1.300512	0.130051	2.325472	0.232547		
JMA Wireless	MX08FRO665-20	148	120	6904	1.300512	0.130051	2.325472	0.232547		
JMA Wireless	MX08FRO665-20	148	240	6904	1.300512	0.130051	2.325472	0.232547		

## Dish Wireless (Proposed) NHV 031B 943108 Carrier Summary

					On <i>I</i>	Axis	Area		
Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	Max Power Density (μW/cm²)	Percent of MPE	Max Power Density (μW/cm²)	Percent of MPE	
JMA Wireless	MX08FRO665-20	148	0	3229	0.964082	0.241021	1.559502	0.389876	
JMA Wireless	MX08FRO665-20	148	120	3229	0.964082	0.241021	1.559502	0.389876	
JMA Wireless	MX08FRO665-20	148	240	3229	0.964082	0.241021	1.559502	0.389876	

 $\begin{tabular}{lllll} Frequency: & 3700 & MHz \\ Maximum Permissible Exposure (MPE): & 1000 & $\mu W/cm^2$ \\ Maximum power density at ground level: & 16.71235 & $\mu W/cm^2$ \\ Highest percentage of Maximum Permissible Exposure: & 1.67123 & $\%$ \\ \end{tabular}$ 

Antenna Make					On /	Axis	Are	ea
	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	Max Power Density (μW/cm²)	Percent of MPE	Max Power Density (μW/cm²)	Percent of MPE
Samsung	MT6407-77A	170	0	43155	6.188689	0.618869	13.543024	1.354302
Samsung	MT6407-77A	170	120	43155	6.188689	0.618869	13.543024	1.354302
Samsung	MT6407-77A	170	210	43155	6.188689	0.618869	13.543024	1.354302

					On /	On Axis Are		
Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	Max Power Density (μW/cm²)	Percent of MPE	Max Power Density (μW/cm²)	Percent of MPE
Commscope	JAHH-65B-R3B	170	0	6069	1.519253	0.151925	2.678591	0.267859
Commscope	JAHH-65B-R3B	170	120	6069	1.519253	0.151925	2.678591	0.267859
Commscope	JAHH-65B-R3B	170	210	6069	1.519253	0.151925	2.678591	0.267859

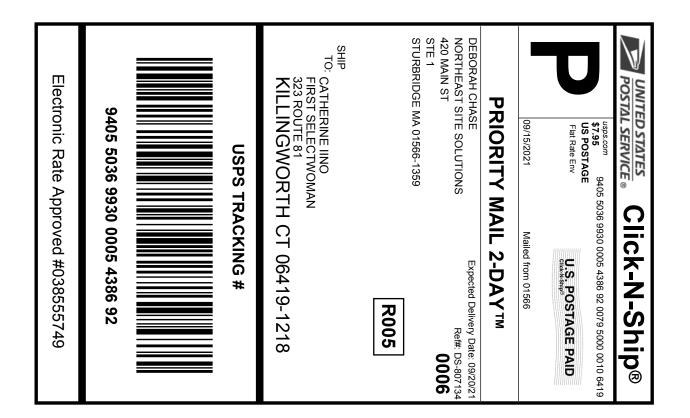
					On /	Axis	Ar	ea
Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	Max Power Density (μW/cm²)	Percent of MPE	Max Power Density (μW/cm²)	Percent of MPE
Commscope	JAHH-65B-R3B	170	0	5890	1.762965	0.176297	2.776985	0.277698
Commscope	JAHH-65B-R3B	170	120	5890	1.762965	0.176297	2.776985	0.277698
Commscope	JAHH-65B-R3B	170	210	5890	1.762965	0.176297	2.776985	0.277698

					On A	Axis	Arc	ea
Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	Max Power Density (μW/cm²)	Percent of MPE	Max Power Density (μW/cm²)	Percent of MPE
Commscope	JAHH-65B-R3B	170	0	3120	0.640968	0.128023	1.033496	0.206424
Commscope	JAHH-65B-R3B	170	120	3120	0.640968	0.128023	1.033496	0.206424
Commscope	JAHH-65B-R3B	170	210	3120	0.640968	0.128023	1.033496	0.206424

					On /	Axis	Ar	ea
Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	Max Power Density (μW/cm²)	Percent of MPE	Max Power Density (µW/cm²)	Percent of MPE  0.048122 0.048122 0.160902 0.048122 0.160902 0.048122 0.160902 0.048122
Antel	LPA-80063-6CF	170	0	1127	0.265397	0.046835	0.272691	0.048122
Antel	LPA-80063-6CF	170	0	1127	0.265397	0.046835	0.272691	0.048122
Commscope	JAHH-65B-R3B	170	0	2661	0.640685	0.113062	0.911780	0.160902
Antel	LPA-80063-6CF	170	120	1127	0.265397	0.046835	0.272691	0.048122
Antel	LPA-80063-6CF	170	120	1127	0.265397	0.046835	0.272691	0.048122
Commscope	JAHH-65B-R3B	170	120	2661	0.640685	0.113062	0.911780	0.160902
Antel	LPA-80063-6CF	170	210	1127	0.265397	0.046835	0.272691	0.048122
Antel	LPA-80063-6CF	170	210	1127	0.265397	0.046835	0.272691	0.048122
Commscope	JAHH-65B-R3B	170	210	2661	0.640685	0.113062	0.911780	0.160902

## Exhibit G

**DW/b**[Wf? S[/[`Ye





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### Instructions

- 1. Each Click-N-Ship® label is unique. Labels are to be used as printed and used only once. DO NOT PHOTO **COPY OR ALTER LABEL.**
- 2. Place your label so it does not wrap around the edge of the package.
- 3. Adhere your label to the package. A self-adhesive label is recommended. If tape or glue is used, DO NOT TAPE OVER BARCODE. Be sure all edges are secure.
- 4. To mail your package with PC Postage®, you may schedule a Package Pickup online, hand to your letter carrier, take to a Post Office™, or drop in a USPS collection box.
- 5. Mail your package on the "Ship Date" you selected when creating this label.

## Click-N-Ship® Label Record

### **USPS TRACKING #:** 9405 5036 9930 0005 4386 92

Trans. #: 543720552 Print Date: 09/15/2021 Ship Date: 09/15/2021 09/20/2021 Delivery Date:

Priority Mail® Postage: \$7.95 \$7.95 Total:

Ref#: DS-807134 From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

**STURBRIDGE MA 01566-1359** 

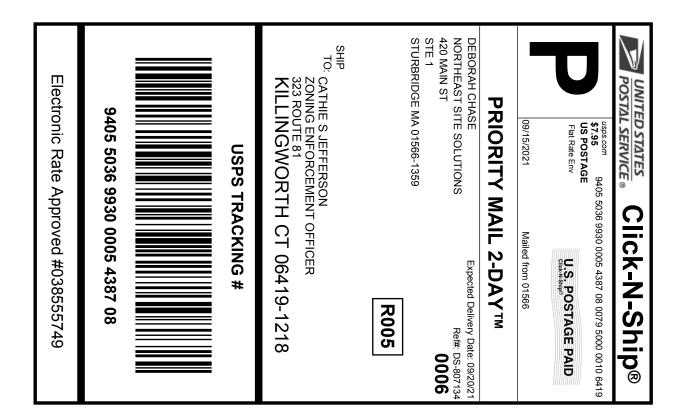
**CATHERINE IINO** 

FIRST SELECTWOMAN

323 ROUTE 81

KILLINGWORTH CT 06419-1218

Retail Pricing Priority Mail rates apply. There is no fee for USPS Tracking® service on Priority Mail service with use of this electronic rate shipping label. Refunds for unused postage paid labels can be requested online 30 days from the print date.





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### Instructions

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- 3. Adhere your label to the package. A self-adhesive label is recommended. If tape or glue is used, DO NOT TAPE OVER BARCODE. Be sure all edges are secure.
- 4. To mail your package with PC Postage®, you may schedule a Package Pickup online, hand to your letter carrier, take to a Post Office™, or drop in a USPS collection box.
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## Click-N-Ship® Label Record

### **USPS TRACKING #:** 9405 5036 9930 0005 4387 08

Trans. #: 543720552 Print Date: 09/15/2021 Ship Date: 09/15/2021 09/20/2021 Delivery Date:

Priority Mail® Postage: Total:

\$7.95 \$7.95

Ref#: DS-807134

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

STURBRIDGE MA 01566-1359

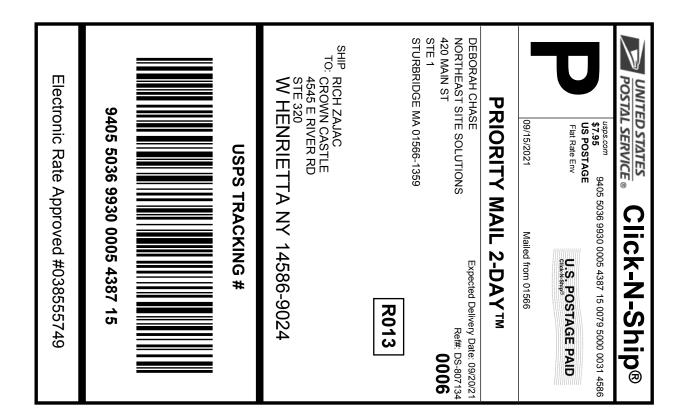
**CATHIE S JEFFERSON** 

ZONING ENFORCEMENT OFFICER

323 ROUTE 81

KILLINGWORTH CT 06419-1218

Retail Pricing Priority Mail rates apply. There is no fee for USPS Tracking® service on Priority Mail service with use of this electronic rate shipping label. Refunds for unused postage paid labels can be requested online 30 days from the print date.





Cut on dotted line.

### Instructions

- 1. Each Click-N-Ship® label is unique. Labels are to be used as printed and used only once. DO NOT PHOTO **COPY OR ALTER LABEL.**
- 2. Place your label so it does not wrap around the edge of the package.
- 3. Adhere your label to the package. A self-adhesive label is recommended. If tape or glue is used, DO NOT TAPE OVER BARCODE. Be sure all edges are secure.
- 4. To mail your package with PC Postage®, you may schedule a Package Pickup online, hand to your letter carrier, take to a Post Office™, or drop in a USPS collection box.
- 5. Mail your package on the "Ship Date" you selected when creating this label.

## Click-N-Ship® Label Record

### **USPS TRACKING #:** 9405 5036 9930 0005 4387 15

Trans. #: 543720552 Print Date: 09/15/2021 Ship Date: 09/15/2021 09/20/2021 Delivery Date:

Priority Mail® Postage: \$7.95 \$7.95 Total:

Ref#: DS-807134

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

STURBRIDGE MA 01566-1359

**RICH ZAJAC** 

**CROWN CASTLE** 4545 E RIVER RD

**STE 320** 

W HENRIETTA NY 14586-9024

\* Retail Pricing Priority Mail rates apply. There is no fee for USPS Tracking® service on Priority Mail service with use of this electronic rate shipping label. Refunds for unused postage paid labels can be requested online 30 days from the print date.



FISKDALE 458 MATN ST

458 MAIN 51 FISKDALE, MA 01518-9998 (800)275-8777 03:16 PM			
09/16/2021			
Product	Qty	Unit Price	Price
Prepaid Mail 1 \$0.00  West Henrietta, NY 14586  Weight: 0 lb 2.10 oz  Acceptance Date:  Thu 09/16/2021  Tracking #:  9405 5036 9930 0005 4387 15			
Prepaid Mail Killingworth Weight: 1 lk Acceptance [ Thu 09/ Tracking #: 9405 50	o 6.10 02 Date:	!	\$0.00 92
Tracking #: 9405 50	h, C1 064 b 5.70 o Date: (16/2021 036 9930 0	z 1005 4387	
Grand Total:	and while these was sold took took while the	THE REP CHE SHE SHE SHE SHE SHE SHE	\$0.00
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\*\*\*\*\*\*\*\*\*\*\*\* USPS is experiencing unprecedented volume increases and limited employee