



1 Cityplace Dr, Suite 490
Creve Coeur, MO 63141

Phone: (314) 513-0147
www.crowncastle.com

April 29, 2021

Melanie A. Bachman
Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

**RE: Notice of Exempt Modification for T-Mobile
Crown Site ID# 876369; T-Mobile Site ID# CTNH558A
64 Hungerford Lane Harwinton, CT 06791
Latitude: 41.75726000/ Longitude: -73.05260000**

Dear Ms. Bachman:

T-Mobile currently maintains six (6) antennas at the 180-foot mount on the existing 178-foot Monopole Tower located at 64 Hungerford Lane in Harwinton. The property is owned by Red Wolf Broadcasting Corporation and the Tower by Crown Castle. T-Mobile now intends to replace six (6) existing antennas and add three (3) antennas. This modification/proposal includes hardware that is both 4G(LTE) and 5G capable through remote software configuration and either or both services may be turned on or off at various times.

Planned Modifications:

Tower:

Remove and Replace:

(3) RFS – APXVTM14-C-120 Antennas (**REMOVE**) – (3) RFS-APX16DWV-16-DWV-S-E-A20 Antennas (**REPLACE**)

(3) RFS – APXVSPP18C-A20 Antennas (**REMOVE**) - (3) RFS APXVAALL24_43-U-NA20 Antennas (**REPLACE**)

(3) Sprint RRUs Radios (**REMOVE**) – (3) Ericsson 4449 B71+B85 Radios (**REPLACE**)

(3) Sprint RRUs Radios (**REMOVE**) – (3) Ericsson 4415 B66A Radios (**REPLACE**)

(1) Sprint Antenna Platform (**REMOVE**) – (1) T-Mobile Antenna Platform (**REPLACE**)

(3) Hybrid Cables (1 5/8”) (**REMOVE**) – (3) Hybrid Cables (1 5/8”) (**REPLACE**)

Install New:

(3) Ericsson-AIR6449 B41 Antennas

(3) Ericsson 4424 B25 Radios



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(1) Hybrid Cables (1 5/8")

Remove:

- (9) Sprint TMAs
- (1) RRH Mount

Ground:

Remove and Replace:

- (1) MMBS Cabinet (**REMOVE**) – (1) 6160 Equipment Cabinet (**REPLACE**)
- (1) BBU Cabinet (**REMOVE**) – (1) B160 Battery Cabinet (**REPLACE**)

Install New:

- (1) PSU 4813 Booster in 6160 Cabinet
- (3) BB6630 in 6160 Cabinet
- (1) BB6648 in 6160 Cabinet
- (1) IXRE Router in 6160 Cabinet

The facility was approved by the Town of Harwinton Building Official by way of a Building Permit on May 29th, 2001.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. §16-50j-72(b)(2). In accordance with R.C.S.A. §16-50j-73, a copy of this letter is being sent to Michael R. Criss, First Selectman of the Town of Harwinton, Jeffrey Neumann, Building Official for the Town of Harwinton and Polly Redmond, Land Use Coordinator for the Town of Harwinton. A copy will also be sent to the property owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.



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For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. §16-50j-72(b)(2).

Sincerely,

Colin Robinson

Colin Robinson
Project Manager
NETWORK BUILDING + CONSULTING
100 Apollo Drive Suite 303
Chelmsford, MA 01824
crobinson@nbcllc.com
(360) 561-3311

cc:

Michael R. Criss, First Selectman (*via email only to mcriss@harwinton.us*)
Town of Harwinton
100 Bentley Drive
Harwinton CT, 06791
(860) 485-9051

Jeffrey Neumann, Building Official (*via email only to building@harwinton.us*) and
Polly Redmond, Land Use Coordinator (*via email only to landuse@harwinton.us*)
Town of Harwinton
100 Bentley Drive
Harwinton CT, 06791
(860) 485-2784

Red Wolf Broadcasting Corporation (*via FedEx*)
758 Colonel Ledyard Highway
Ledyard, CT 06339
(860) 464-1065

Colin Robinson

From: Colin Robinson
Sent: Thursday, April 29, 2021 3:07 PM
To: mcriss@harwinton.us
Cc: Colin Robinson
Subject: CSC Exempt Modification Application 64 Hungerford Ln Harwinton CT 876369
Attachments: CSC Exempt Modification Application 64 Hungerford Ln Harwinton CT 876369
04292021.pdf

Good Afternoon First Selectman Criss,

Please see the attached application to the Connecticut Siting Council regarding antenna work on the existing cell tower located at 64 Hungerford Ln Harwinton, CT.

Should you have any questions/comments/concerns regarding this application, please do not hesitate to contact me.

Thank you,

Colin

Colin Robinson

Project Manager

NETWORK BUILDING + CONSULTING

100 Apollo Drive | Suite 303 | Chelmsford, MA | 01824
M 360.561.3311



Colin Robinson

From: Colin Robinson
Sent: Thursday, April 29, 2021 3:06 PM
To: building@harwinton.us
Cc: Colin Robinson
Subject: CSC Exempt Modification Application 64 Hungerford Ln Harwinton CT 876369
Attachments: CSC Exempt Modification Application 64 Hungerford Ln Harwinton CT 876369 04292021.pdf

Good Afternoon Mr. Neumann,

Please see the attached application to the Connecticut Siting Council regarding antenna work on the existing cell tower located at 64 Hungerford Ln Harwinton, CT.

Should you have any questions/comments/concerns regarding this application, please do not hesitate to contact me.

Thank you,

Colin

Colin Robinson

Project Manager

NETWORK BUILDING + CONSULTING

100 Apollo Drive | Suite 303 | Chelmsford, MA | 01824
M 360.561.3311



Colin Robinson

From: Colin Robinson
Sent: Thursday, April 29, 2021 3:06 PM
To: landuse@harwinton.us
Cc: Colin Robinson
Subject: CSC Exempt Modification Application 64 Hungerford Ln Harwinton CT 876369
Attachments: CSC Exempt Modification Application 64 Hungerford Ln Harwinton CT 876369
04292021.pdf

Good Afternoon Ms. Redmond,

Please see the attached application to the Connecticut Siting Council regarding antenna work on the existing cell tower located at 64 Hungerford Ln Harwinton, CT.

Should you have any questions/comments/concerns regarding this application, please do not hesitate to contact me.

Thank you,

Colin

Colin Robinson

Project Manager

NETWORK BUILDING + CONSULTING

100 Apollo Drive | Suite 303 | Chelmsford, MA | 01824
M 360.561.3311



ORIGIN ID:FOXA (360) 561-3311
COLIN ROBINSON
NB+C
100 APOLLO DR.
SUITE 303
CHELMSFORD, MA 01824
UNITED STATES US

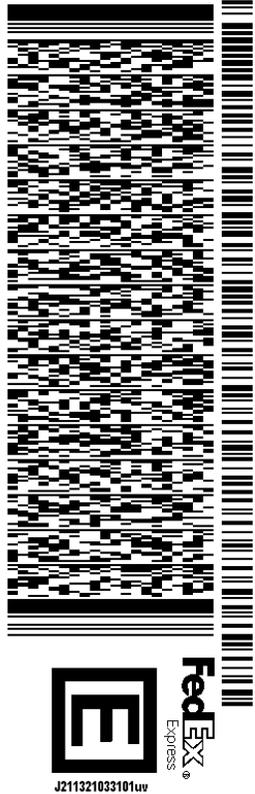
SHIP DATE: 29APR21
ACTWGT: 1.00 LB
CAD: 108980334IN/ET4340
BILL SENDER

TO **RED WOLF BROADCASTING CORPORATION**

758 COLONEL LEDYARD HIGHWAY

LEDYARD CT 06339

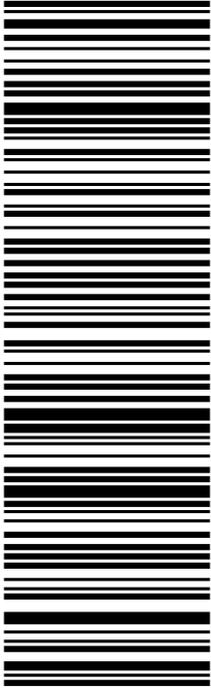
(860) 464-1065 REF: 100789876369 HARRINGTON, CT
INV/ PO: DEPT:



56DJ3/F9A6/FE4A

TRK# 7735 9004 3065 FRI - 30 APR 4:30P
0201 STANDARD OVERNIGHT

EB GONA 06339
CT-US BDL



After printing this label:

- 1. Use the 'Print' button on this page to print your label to your laser or inkjet printer.
- 2. Fold the printed page along the horizontal line.
- 3. Place label in shipping pouch and affix it to your shipment so that the barcode portion of the label can be read and scanned.

Warning: Use only the printed original label for shipping. Using a photocopy of this label for shipping purposes is fraudulent and could result in additional billing charges, along with the cancellation of your FedEx account number.

Use of this system constitutes your agreement to the service conditions in the current FedEx Service Guide, available on fedex.com. FedEx will not be responsible for any claim in excess of \$100 per package, whether the result of loss, damage, delay, non-delivery, misdelivery, or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim. Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including intrinsic value of the package, loss of sales, income interest, profit, attorney's fees, costs, and other forms of damage whether direct, incidental, consequential, or special is limited to the greater of \$100 or the authorized declared value. Recovery cannot exceed actual documented loss. Maximum for items of extraordinary value is \$1,000, e.g. jewelry, precious metals, negotiable instruments and other items listed in our ServiceGuide. Written claims must be filed within strict time limits, see current FedEx Service Guide.

Exhibit A

Original Facility Approval

Building Permit

09

TOWN OF HARWINTON

MINIMUM FEE: \$ _____

DATE: 5/29/01

TYPE OF INSTALLATION

ELECTRICAL SERVICE-NEW OR CHANGE _____	WATER HEATER _____
ELECTRICAL INSTALLATION _____	ROOFING _____
ELECTRICAL-SWIMMING POOL _____	RE-ROOFING: _____
PLUMBING INSTALLATION _____	HOW MANY LAYERS? _____
HEATING INSTALLATION _____	SIDING _____
AIR CONDITIONING _____	OTHER _____
OIL BURNER-GAS BURNER _____	
WOODBURNING STOVE _____	
CHIMNEY INSTALLATION _____	

Steve Florent 485-

NAME OF CONTRACTOR Baron M.H. Corporation
 ADDRESS OF CONTRACTOR 24 Corporate Circle Albany NY 12203
 LICENSE # 009-00-019 EXPIRATION DATE _____ TELE. # 518-486-8114
 REQUEST PERMISSION TO PERFORM _____
 AT: LOT # 21A-21-B STREET ADDRESS Hungerford Lane
 ESTIMATED COST \$272,000.00

REMARKS: _____

OWNER Tower owner: Sprint Spectrum LP
 ADDRESS One International Blvd.
3rd Floor
Mechuck NS 02495

VALUATION OF WORK 972,000.00

FEE \$4 per \$1000.00

PERMIT # 4587

BUILDING OFFICIAL Frank Rybak 4

PAID DATE 5-29-01

10-15-2001

BUILDING INSPECTION DIVISION HARWINTON, CONN.
CERTIFICATE OF OCCUPANCY

Sprint Spectrum LP

This is to certify that the ~~new house~~ *Tele Communication Tower* at Lot as constructed under Permit No conforms substantially to the requirements of the State Building Code and is hereby approved for occupancy as indicated below. At the date and time this Certificate is issued the house is owned by

Approved for occupancy: *Sprint Spectrum LP*

Basement

•

First Floor

•

Second Floor

•

Towers ok

Use Group:

Type of Construction:

Frank Lyjak

This certificate is VOID unless signed by the Building Official

Exhibit B

Property Card

Summary

ParcelId 341
Account Number 595
Location Address 64 HUNGERFORD LA
Map-Block-Lot D5 /02 /0032

Use Class/Description 2-1 COMM LAND
Assessing Neighborhood 0001A
Census Tract 2984
Acreage 40.28
Utilities



Owner

RED WOLF BROADCASTING CORPORATION
 758 COLONEL LEDYARD HIGHWAY
 LEDYARD, CT 06339

Current Appraised Value

	2019	2018	2017
+ Building Value	\$76,360	\$76,360	\$35,280
+ XF Value	\$0	\$0	\$0
+ OB Value	\$2,950	\$2,950	\$3,950
+ Land Value	\$3,139,960	\$3,139,960	\$367,850
+ Special Land Value			
+ Total Appraised Value	\$3,219,270	\$3,219,270	\$407,080
+ Net Appraised Value	\$3,219,270	\$3,219,270	\$407,080
+ Current Assessment	\$293,590	\$293,590	\$284,960

Assessment History

	2018	2017	2016	2015
+ Building Value	\$53,450	\$24,700	\$24,700	\$24,700
+ OB/Misc	\$2,060	\$2,760	\$2,760	\$2,760
+ Land	\$238,080	\$257,500	\$257,500	\$257,500
+ Total Assessment	\$293,590	\$284,960	\$284,960	\$284,960

Land

Use	Class	Zoning	Area	Value
2-1 COMM LAND	C	CR2	1 AC	\$74,330
5-2V EX COMM V	C		10.97 AC	\$78,980
3-1 IND LAND	I		1 BL	\$180,000
6-2 FOREST LD	R		28.31 AC	\$2,806,650

Commercial Building

Building # 1
Style Office Bldg
Actual Year Built 1964
Effective Year Built 1965
Gross Area 1230
Stories 1
Grade Below Average
Exterior Wall Brick/Masonry
Interior Wall Drywall/Sheet
Wall Height 8
Units 1
Roof Cover Asph/F Gls/Cmp
Roof Structure Gable/Hip
Floor Type Quarry Tile
Heat Type Oil
Heat Fuel Forced Air-Duc
AC Type HEAT/AC PKGS
Sprinkler 03

Construction MASONRY
 Plumbing AVERAGE
 Comm Walls 0

Building Sub Areas

Code	Description	Living Area	Gross Area	Effective Area
BAS		1200	1200	1200
FEP	Enclosed Porch	0	30	20
Totals		1200	1230	1220

Out Buildings\Extra Features

Description	Sub Description	Area	Year Built	Value
SHED FRAME AVE		360S.F.	2004	\$2,790
PATIO GOOD		36S.F.	2000	\$160

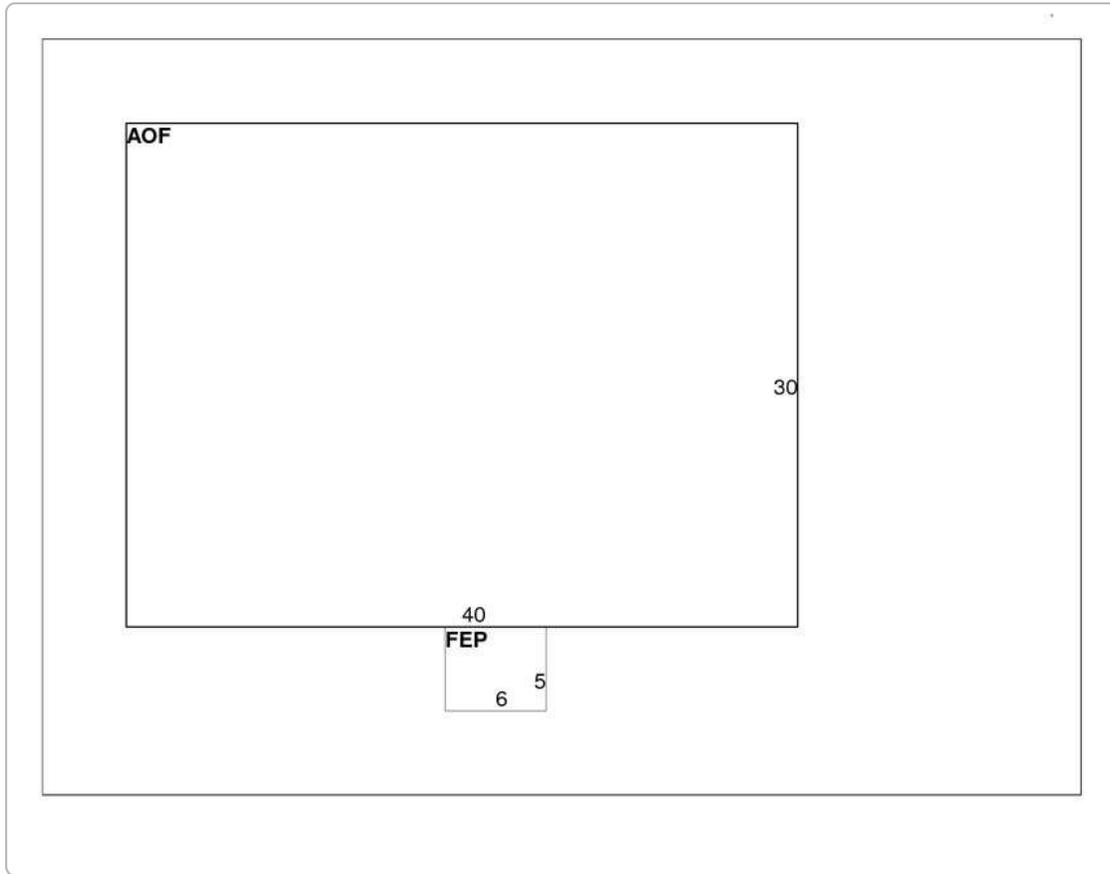
Sales History

Sales Date	Type of Document	Grantee	Vacant/Improved	Book/Page	Amount
04-04-2018		RED WOLF BROADCASTING CORPORATION	Improved	0256/0776	\$407,080
07-09-2014		CONNOISSEUR MEDIA OF CONNECTICUT LLC	Improved	0243/1029	\$407,080
07-23-1997		BUCKLEY BROADCASTING CORP OF CT	Improved	0145/0372	\$0
01-09-1997		USA	Improved	0145/0216	\$0
07-24-1985		CONSUMER SERVICE RADIO INC	Improved	0101/0665	\$0

Permit Information

Permit ID	Issue Date	Type	Description	Amount	Inspection Date	% Complete	Date Complete	Comments
17126B	09-11-2017		3 ANTENNAS	\$20,000		100		
	11-30-2015		CERTIFICATE OF APPROV	\$0		0		
9417	10-24-2014		MODIFICATIONS	\$20,000		0		
8721	11-29-2012		CELL TOWER MODIFICAT	\$25,000		0		
8703	11-21-2012		ANTENNAS	\$12,000		0		
8619	10-02-2012		REPLACE 6 ANTENNAS O	\$10,000		0		
CO	04-17-2006		CO ISSUED	\$0		0		
6239	01-17-2006			\$50,000		0		PREFAB CONCRETE SHELTER

Sketch



Photos



No data available for the following modules: Building Data.

The Town of Harwinton Assessor makes every effort to produce the most accurate information possible. No warranties, expressed or implied are provided for the data herein, its use or interpretation. The assessment information is from the last certified tax roll. All other data is subject to change.

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[Last Data Upload: 4/26/2021, 8:21:54 PM](#)

Developed by
 Schneider
GEO SPATIAL

Version 2.3.118

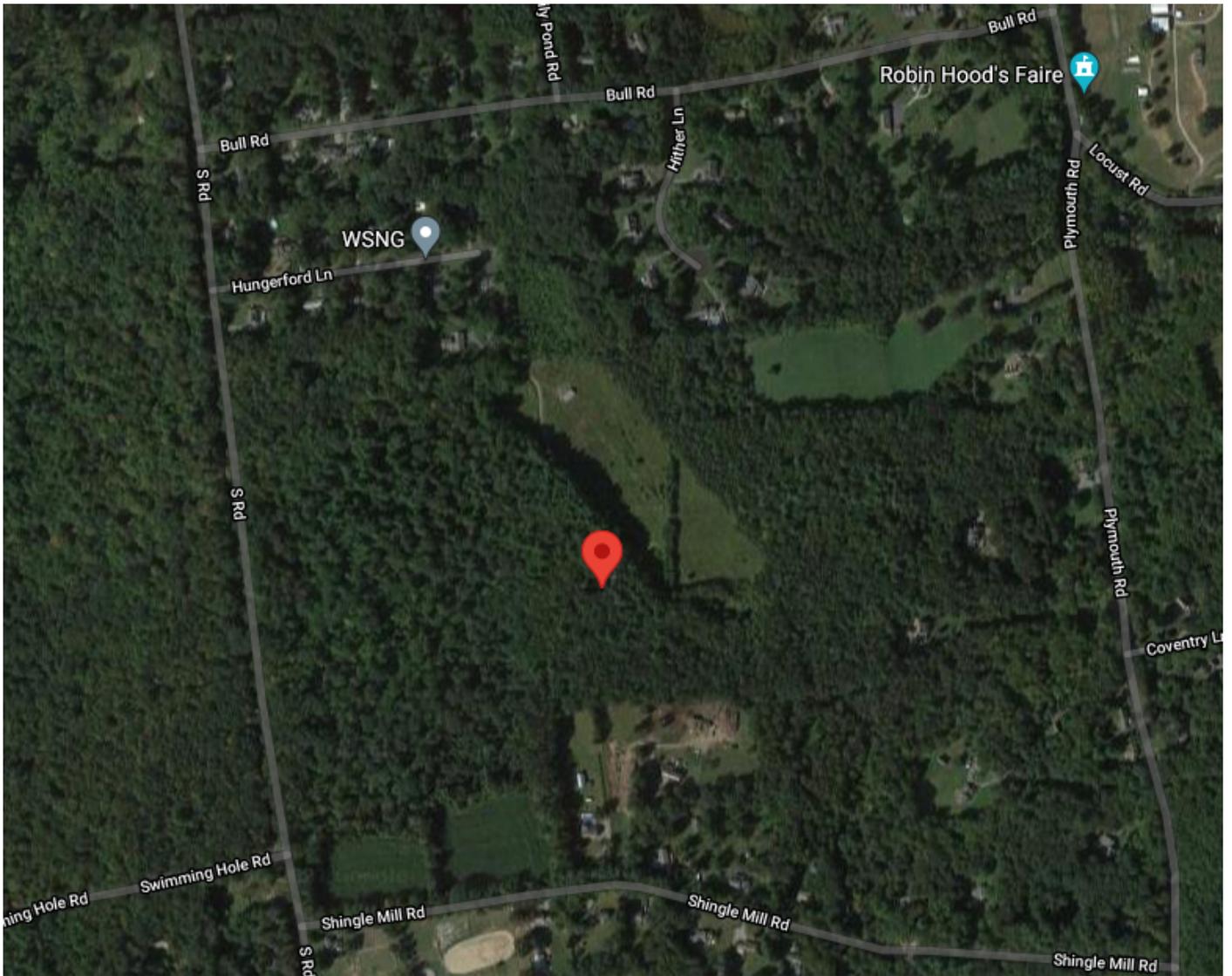


Exhibit C

Construction Drawings

T-Mobile

T-MOBILE SITE NUMBER: CTNH558A

T-MOBILE SITE NAME: CTNH558A

SITE TYPE: MONOPOLE

TOWER HEIGHT: 178'-0"

T-MOBILE SPRINT RETAIN SITE CONFIGURATION: 67D5998C_1xAIR+1QP (GSM ONLY)

BUSINESS UNIT #: 876369

**SITE ADDRESS: 64 HUNGERFORD LANE
HARWINTON, CT 06791**

COUNTY: LITCHFIELD

JURISDICTION: TOWN OF HARWINTON

T-Mobile

35 GRIFFIN ROAD
BLOOMFIELD, CT 06002

CROWN CASTLE

1500 CORPORATE DRIVE
CANONSBURG, PA 15317

INFINIGY

FROM ZERO TO INFINIGY
the solutions are endless
1033 Watervliet Shaker Rd | Albany, NY 12205
Phone: 518-690-0790 | Fax: 518-690-0793
www.infinigy.com

T-MOBILE SITE NUMBER:
CTNH558A

BU #: 876369
HARWINTON / BUCKLEY BROADCASTI

64 HUNGERFORD LANE
HARWINTON, CT 06791

EXISTING 178'-0" MONOPOLE

ISSUED FOR:

REV	DATE	DRWN	DESCRIPTION	DES/QA
A	03/23/2021	RCD	PRELIMINARY	SS
0	04/22/2021	BMM	FINAL	SS

SITE INFORMATION

CROWN CASTLE USA INC. HARWINTON / BUCKLEY BROADCASTI
SITE NAME:
SITE ADDRESS: 64 HUNGERFORD LANE
HARWINTON, CT 06791
COUNTY: LITCHFIELD
MAP/PARCEL #: D5/02/0032
AREA OF CONSTRUCTION: EXISTING
LATITUDE: 41.75726000° (41° 45' 26.15")
LONGITUDE: -73.05260000° (-73° 3' 9.20")
LAT/LONG TYPE: NAD83
GROUND ELEVATION: 857.0 FT
CURRENT ZONING: 2-1 COMM LAND
JURISDICTION: TOWN OF HARWINTON
OCCUPANCY CLASSIFICATION: U
TYPE OF CONSTRUCTION: IIB
A.D.A. COMPLIANCE: FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION
PROPERTY OWNER: CROWN CASTLE USA
2000 CORPORATE DRIVE
CANONSBURG, PA
TOWER OWNER: CROWN CASTLE
2000 CORPORATE DRIVE
CANONSBURG, PA 15317
CARRIER/APPLICANT: T-MOBILE
35 GRIFFIN ROAD
BLOOMFIELD, CT 06002
ELECTRIC PROVIDER: CONNECTICUT LIGHT AND POWER
(800) 286-2000
TELCO PROVIDER: TBD

DRAWING INDEX

SHEET #	SHEET DESCRIPTION
T-1	TITLE SHEET
T-2	GENERAL NOTES
C-1	SITE PLAN & ENLARGED SITE PLAN
C-2	FINAL ELEVATION & ANTENNA PLANS
C-3	ANTENNA & CABLE SCHEDULE
C-4	PLUMBING DIAGRAM
C-5	EQUIPMENT SPECS
C-6	EQUIPMENT SPECS
E-1	AC PANEL SCHEDULES & ONE LINE DIAGRAM
G-1	ANTENNA GROUNDING DIAGRAM
G-2	GROUNDING DETAILS

ALL DRAWINGS CONTAINED HEREIN ARE FORMATTED FOR ----. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

PROJECT DESCRIPTION

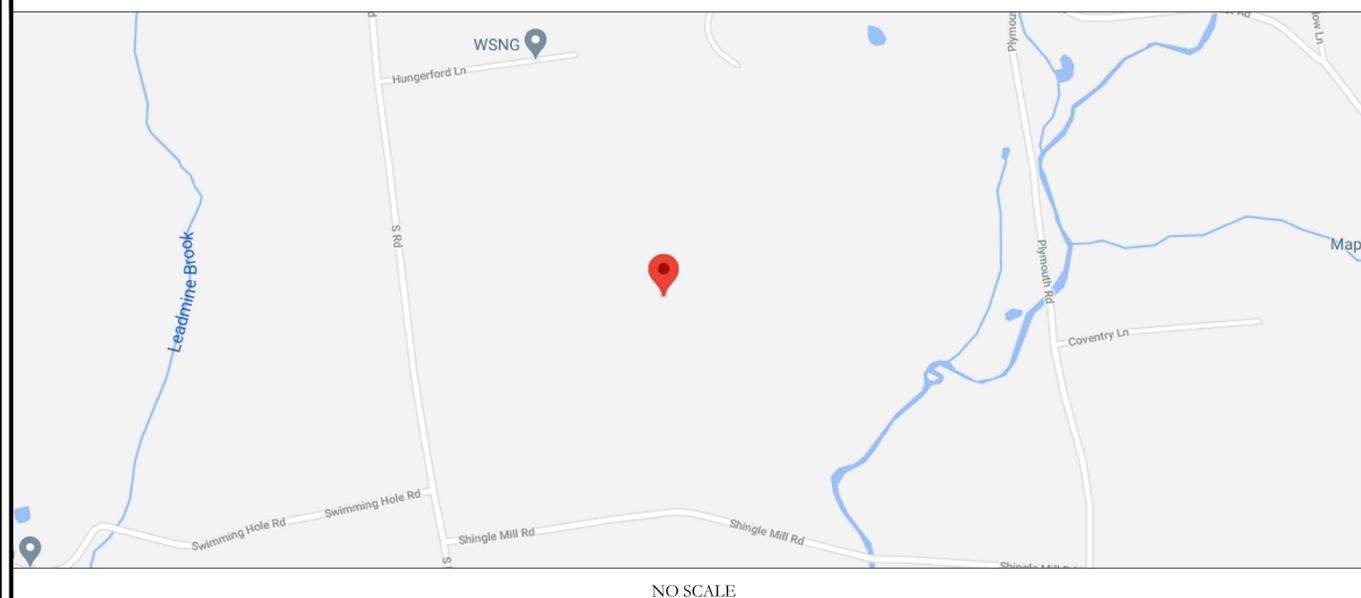
THE PURPOSE OF THIS PROJECT IS TO ENHANCE BROADBAND CONNECTIVITY AND CAPACITY TO THE EXISTING ELIGIBLE WIRELESS FACILITY.

- TOWER SCOPE OF WORK:
- REMOVE (6) ANTENNAS
 - REMOVE (9) TMAS
 - REMOVE (6) RRHs
 - REMOVE (1) RRH MOUNT
 - REMOVE (3) HYBRID CABLES
 - INSTALL (9) ANTENNAS
 - INSTALL (9) RRHs
 - INSTALL (4) HYBRID CABLES
 - REMOVE (1) PLATFORM
 - INSTALL (1) PLATFORM

- GROUND SCOPE OF WORK:
- REMOVE (1) MMBS EQUIPMENT CABINET
 - REMOVE (1) BBU EQUIPMENT CABINET
 - INSTALL (1) 6160 & (1) B160 BATTERY CABINETS
 - INSTALL (1) PSU4813 BOOSTER IN (P) CABINET
 - INSTALL (3) BB6630 IN (P) CABINET
 - INSTALL (1) BB6648 IN (P) CABINET
 - INSTALL (1) IXRE ROUTER IN (P) CABINET

NOTE:
PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE CROWN NOC AT (800) 788-7011 & CROWN CONSTRUCTION MANAGER.

LOCATION MAP



APPLICABLE CODES/REFERENCE DOCUMENTS

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:

CODE TYPE	CODE
BUILDING	2018 CT STATE BUILDING CODE
MECHANICAL	2015 IMC
ELECTRICAL	2017 NEC

REFERENCE DOCUMENTS:

STRUCTURAL ANALYSIS: BY OTHERS
DATED:
MOUNT ANALYSIS: GPD ENGINEERING
DATED: 03/11/2021
RFDS REVISION: 1
DATED: 01/15/2021
ORDER ID: 538773
REVISION: 0

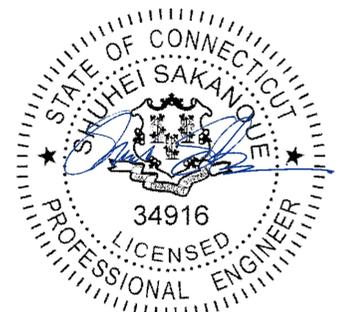
APPROVALS

APPROVAL	SIGNATURE	DATE
PROPERTY OWNER OR REP.	_____	_____
LAND USE PLANNER	_____	_____
T-MOBILE	_____	_____
OPERATIONS	_____	_____
RF	_____	_____
NETWORK	_____	_____
BACKHAUL	_____	_____
CONSTRUCTION MANAGER	_____	_____

THE PARTIES ABOVE HEREBY APPROVE AND ACCEPT THESE DOCUMENTS AND AUTHORIZE THE CONTRACTOR TO PROCEED WITH THE CONSTRUCTION DESCRIBED HEREIN. ALL CONSTRUCTION DOCUMENTS ARE SUBJECT TO REVIEW BY THE LOCAL BUILDING DEPARTMENT AND ANY CHANGES AND MODIFICATIONS THEY MAY IMPOSE.

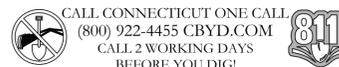
PROJECT TEAM

A&E FIRM: INFINIGY
1033 WATERVLIET SHAKER RD.
ALBANY, NY 12205
CROWN CASTLE USA INC. DISTRICT CONTACTS:
1500 CORPORATE DRIVE
CANONSBURG, PA 15317
TRICIA PELON - PROJECT MANAGER
(518) 373-3507
JASON D'AMICO - CONSTRUCTION MANAGER
(860) 209-0104

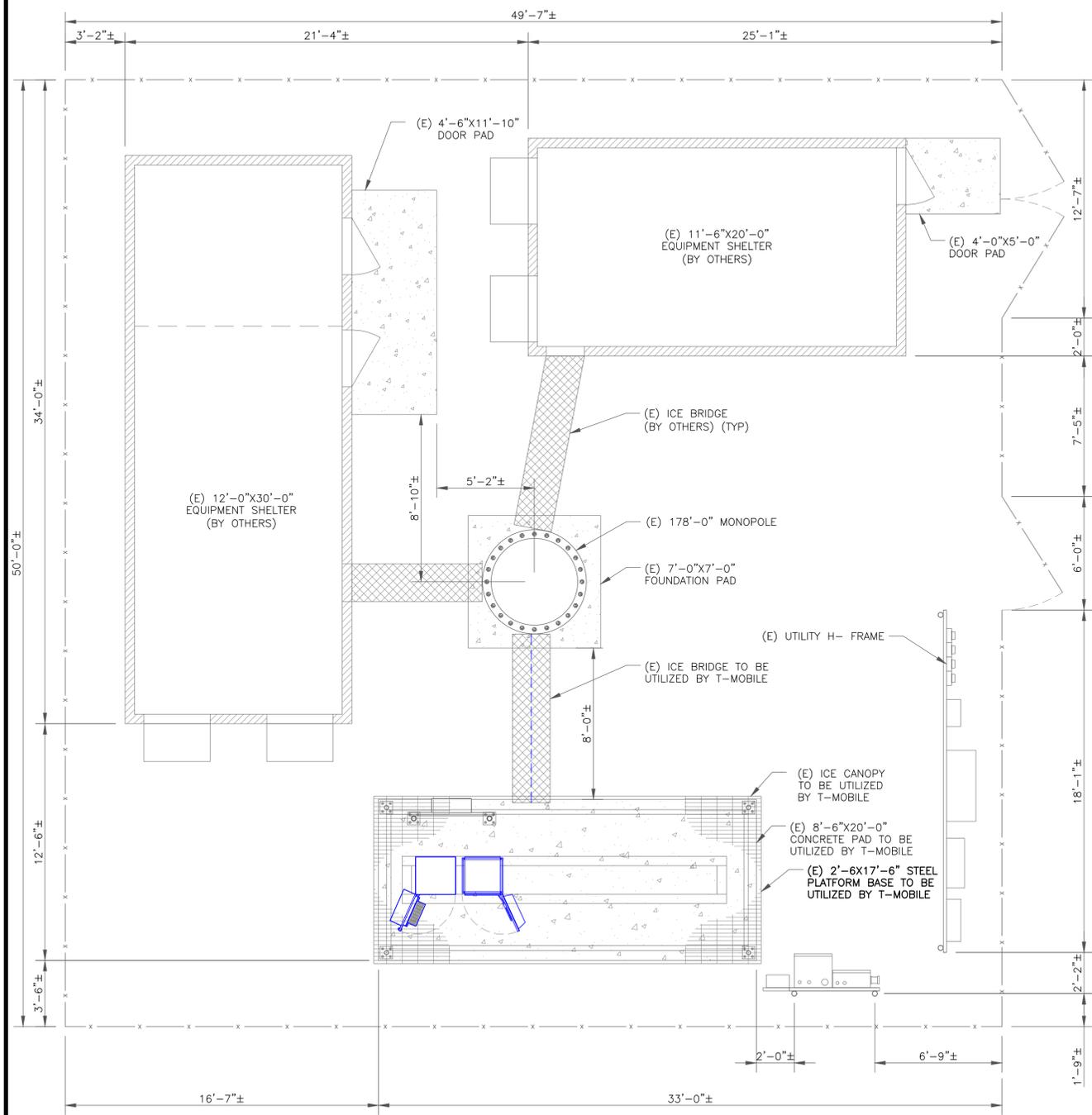


IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

SHEET NUMBER: **T-1** REVISION: **0**



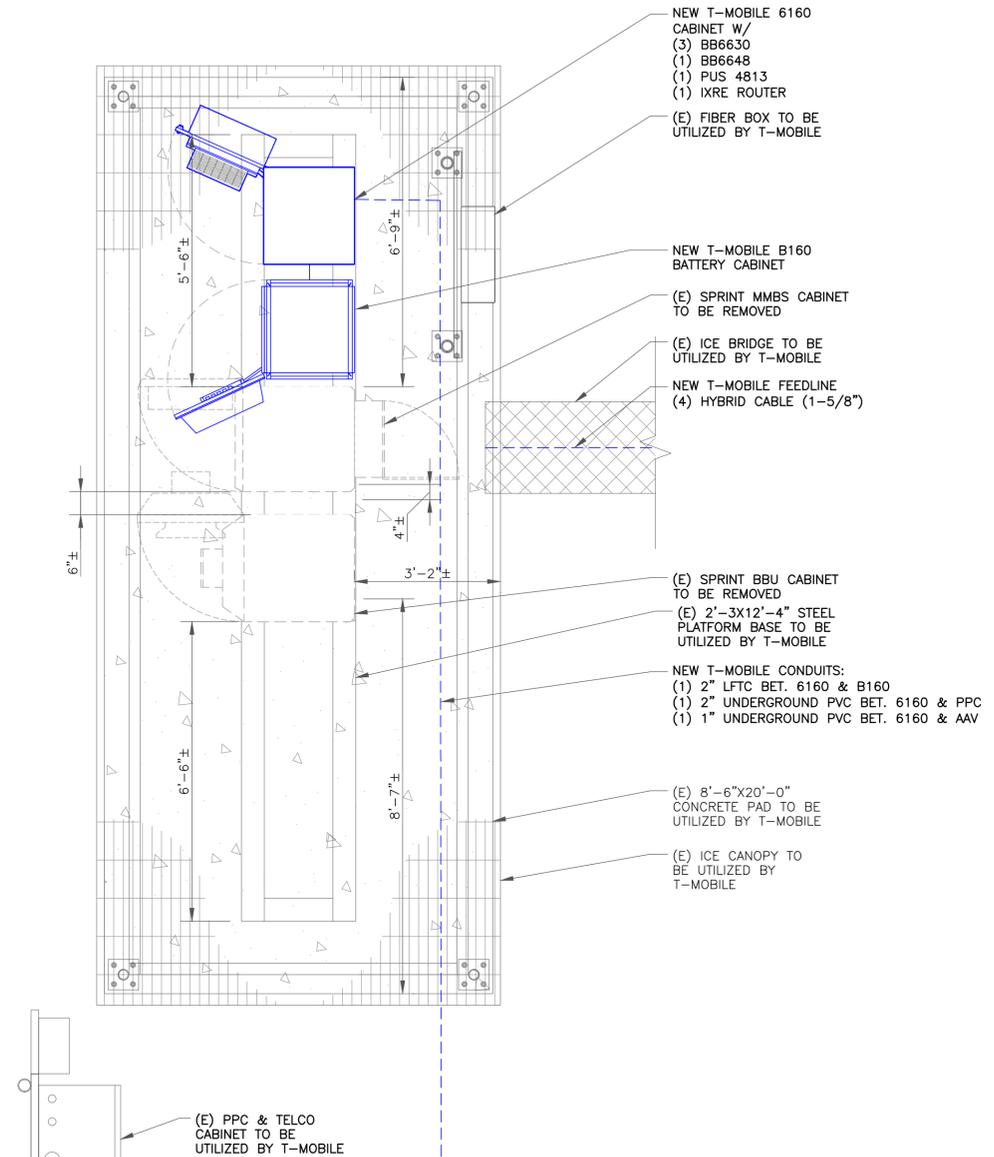
NOTE:
 1. PLANS BASED ON SITE PLAN PROVIDED BY TOWER OWNER AND SITE VISIT PERFORMED BY INFINIGY. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS AND LOCATION/ORIENTATION OF EXISTING T-MOBILE EQUIPMENT.



1 SITE PLAN
 SCALE: 1/2"=1'-0" (FULL SIZE)
 1/4"=1'-0" (11x17)



NOTES:
 THE POWER DESIGN FOR ANY AC ELECTRICAL POWER CHANGES IS TO BE PERFORMED BY OTHERS AND IS SHOWN HERE FOR REFERENCE PURPOSES ONLY. T-MOBILE IS SOLELY RESPONSIBLE FOR THE ELECTRICAL POWER DESIGN.



2 ENLARGED SITE PLAN
 SCALE: 1"=1'-0" (FULL SIZE)
 1/2"=1'-0" (11x17)



T-Mobile
 35 GRIFFIN ROAD
 BLOOMFIELD, CT 06002

CROWN CASTLE
 1500 CORPORATE DRIVE
 CANONSBURG, PA 15317

INFINIGY
 FROM ZERO TO INFINIGY
 the solutions are endless
 1033 Watervliet Shaker Rd | Albany, NY 12205
 Phone: 518-690-0790 | Fax: 518-690-0793
 www.infinigy.com

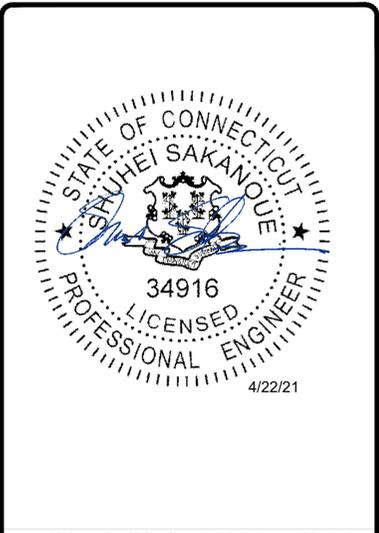
T-MOBILE SITE NUMBER:
CTNH558A
 BU #: **876369**
HARWINTON / BUCKLEY BROADCASTI

64 HUNGERFORD LANE
 HARWINTON, CT 06791

EXISTING 178'-0" MONOPOLE

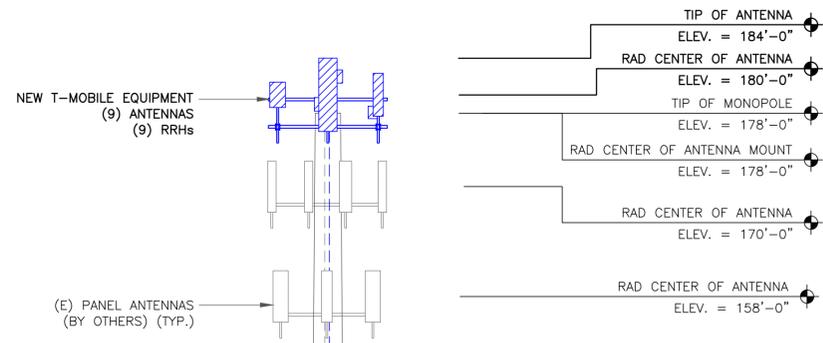
ISSUED FOR:

REV	DATE	DRWN	DESCRIPTION	DWG./QA
A	05/23/2021	RCD	PRELIMINARY	SS
0	04/22/2021	BMM	FINAL	SS



IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

SHEET NUMBER: **C-1** REVISION: **0**



T-MOBILE EQUIPMENT

ANTENNA CL: 180'-0"
MOUNT CL: 178'-0"
ANTENNA CL: 76'-0"
MOUNT CL: 75'-0"

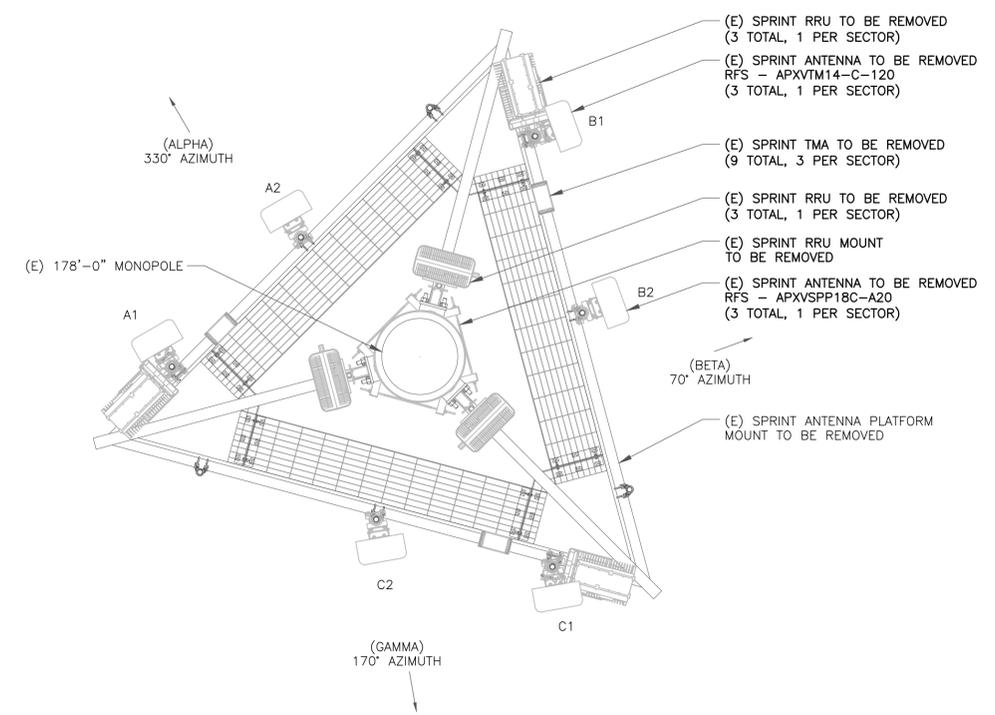
ANY AND ALL TOWER MOUNTED EQUIPMENT MUST NOT TRAP OR INTERFERE W/ EXISTING SAFETY CLIMB

- REMOVE SPRINT FEEDLINE (3) HYBRID CABLE (1-5/8")
- NEW T-MOBILE FEEDLINE (4) HYBRID CABLE (1-5/8")
- EXISTING SPRINT FEEDLINE TO REMAIN (1) COAX CABLE (1-1/2")
- (E) 178'-0" MONOPOLE

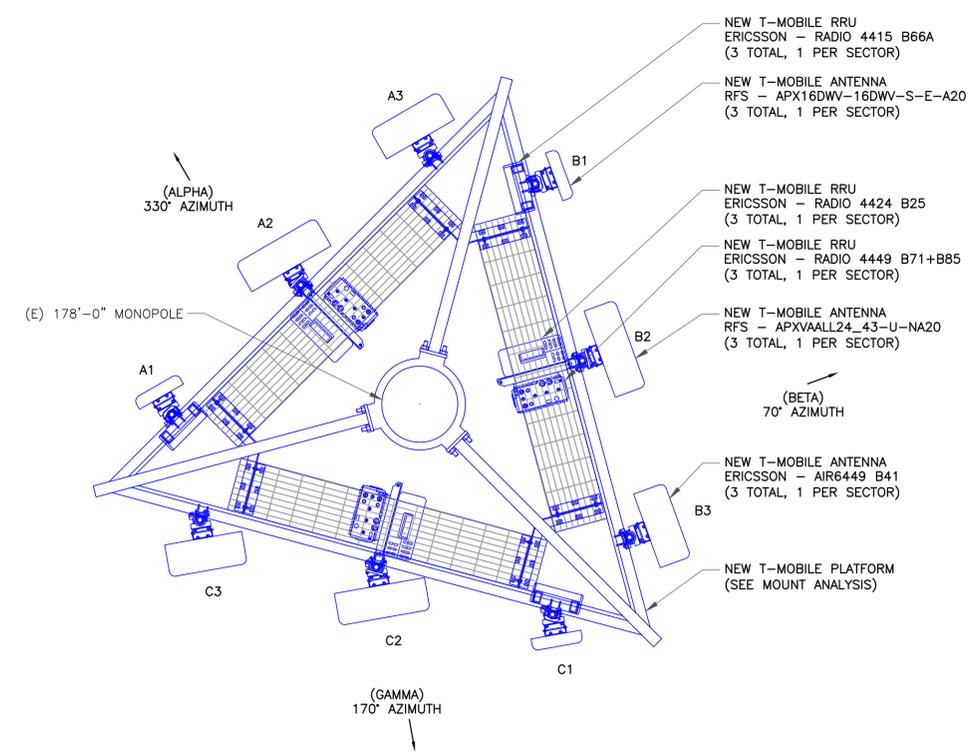


- NOTES:**
- ELEVATION BASED ON DRAWING PROVIDED BY TOWER OWNER. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS AND LOCATION/ORIENTATION OF EXISTING EQUIPMENT.
 - INFINIGY HAS NOT EVALUATED THE TOWER OR MOUNT STRUCTURE AND ASSUMES NO RESPONSIBILITY FOR THEIR STRUCTURAL INTEGRITY REGARDING PROPOSED LOADINGS. FINAL INSTALLATION SHALL COMPLY WITH RESULTS OF PASSING STRUCTURAL ANALYSES PERFORMED BY OTHERS.

1 FINAL ELEVATION SCALE: NOT TO SCALE



2 EXISTING ANTENNA LAYOUT SCALE: NOT TO SCALE



NOTE:
A STRUCTURAL EVALUATION OF THE T-MOBILE ANTENNA MOUNTS HAS BEEN PERFORMED BY GPD ENGINEERING. REFER TO ANTENNA MOUNT STRUCTURAL ANALYSIS DATED TBD PRIOR TO CONSTRUCTION.

INFINIGY HAS NOT EVALUATED THE TOWER FOR THIS SITE AND ASSUMES NO RESPONSIBILITY FOR ITS STRUCTURAL INTEGRITY. CONTRACTOR TO COORDINATE LOADING WITH RF ENGINEER. REFER TO STRUCTURAL ANALYSIS PERFORMED BY OTHERS PRIOR TO CONSTRUCTION.

3 FINAL ANTENNA LAYOUT SCALE: NOT TO SCALE



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EXISTING 178'-0" MONOPOLE

ISSUED FOR:

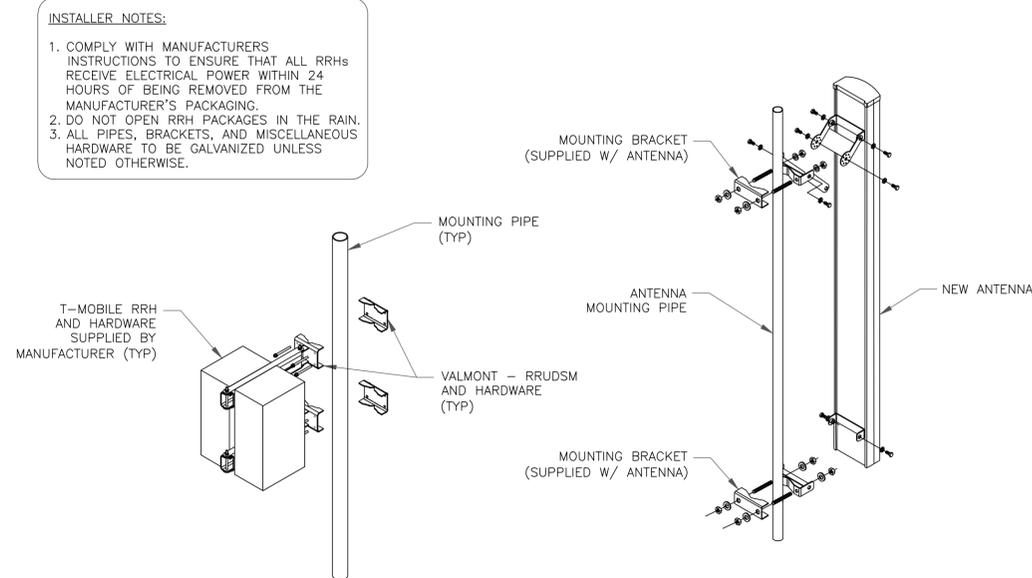
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SHEET NUMBER: **C-2** REVISION: **0**

ANTENNA SCHEDULE										
SECTOR	POS.	TECHNOLOGY	RAD CENTER	AZIMUTH	ANTENNA MANUFACTURER	ANTENNA MODEL	MECH. TILT	ELECT. TILT	TOWER MOUNTED EQUIPMENT	FEEDLINE TYPE
ALPHA	A1	L2100	180'-0"	330°	RFS	APX16DWV-16DWV-S-E-A20	0°	0°	(1) ERICSSON - RRUS 4415 B66A	(1) 6X24 HCS HYBRID (SHARED)
ALPHA	A2	L700, L600, N600, L1900, G1900	180'-0"	330°	RFS	APXVAALL24_43-U-NA20	0°	0°	(1) ERICSSON - RRUS 4449 B71+B85 (1) ERICSSON - RRUS 4424 B25	(2) 6X24 HCS HYBRID (SHARED)
ALPHA	A3	L2500, N2500	180'-0"	330°	ERICSSON	AIR6449 B41	0°	0°	--	(1) 6X24 HCS HYBRID (SHARED)
BETA	B1	L2100	180'-0"	70°	RFS	APX16DWV-16DWV-S-E-A20	0°	0°	(1) ERICSSON - RRUS 4415 B66A	(1) 6X24 HCS HYBRID (SHARED)
BETA	B2	L700, L600, N600, L1900, G1900	180'-0"	70°	RFS	APXVAALL24_43-U-NA20	0°	0°	(1) ERICSSON - RRUS 4449 B71+B85 (1) ERICSSON - RRUS 4424 B25	(2) 6X24 HCS HYBRID (SHARED)
BETA	B3	L2500, N2500	180'-0"	70°	ERICSSON	AIR6449 B41	0°	0°	--	(1) 6X24 HCS HYBRID (SHARED)
GAMMA	C1	L2100	180'-0"	170°	RFS	APX16DWV-16DWV-S-E-A20	0°	0°	(1) ERICSSON - RRUS 4415 B66A	(1) 6X24 HCS HYBRID (SHARED)
GAMMA	C2	L700, L600, N600, L1900, G1900	180'-0"	170°	RFS	APXVAALL24_43-U-NA20	0°	0°	(1) ERICSSON - RRUS 4449 B71+B85 (1) ERICSSON - RRUS 4424 B25	(2) 6X24 HCS HYBRID (SHARED)
GAMMA	C3	L2500, N2500	180'-0"	170°	ERICSSON	AIR6449 B41	0°	0°	--	(1) 6X24 HCS HYBRID (SHARED)

1 ANTENNA AND CABLE SCHEDULE
SCALE: NOT TO SCALE



2 ANTENNA WITH RRHs MOUNTING DETAIL
SCALE: NOT TO SCALE

NOTE:
1. CONTRACTOR SHALL INSTALL 3RD DUAL RRH MOUNT TO ACCOMMODATE ALL RRH BRACKETS HOLES IF NECESSARY.

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EXISTING 178'-0" MONOPOLE

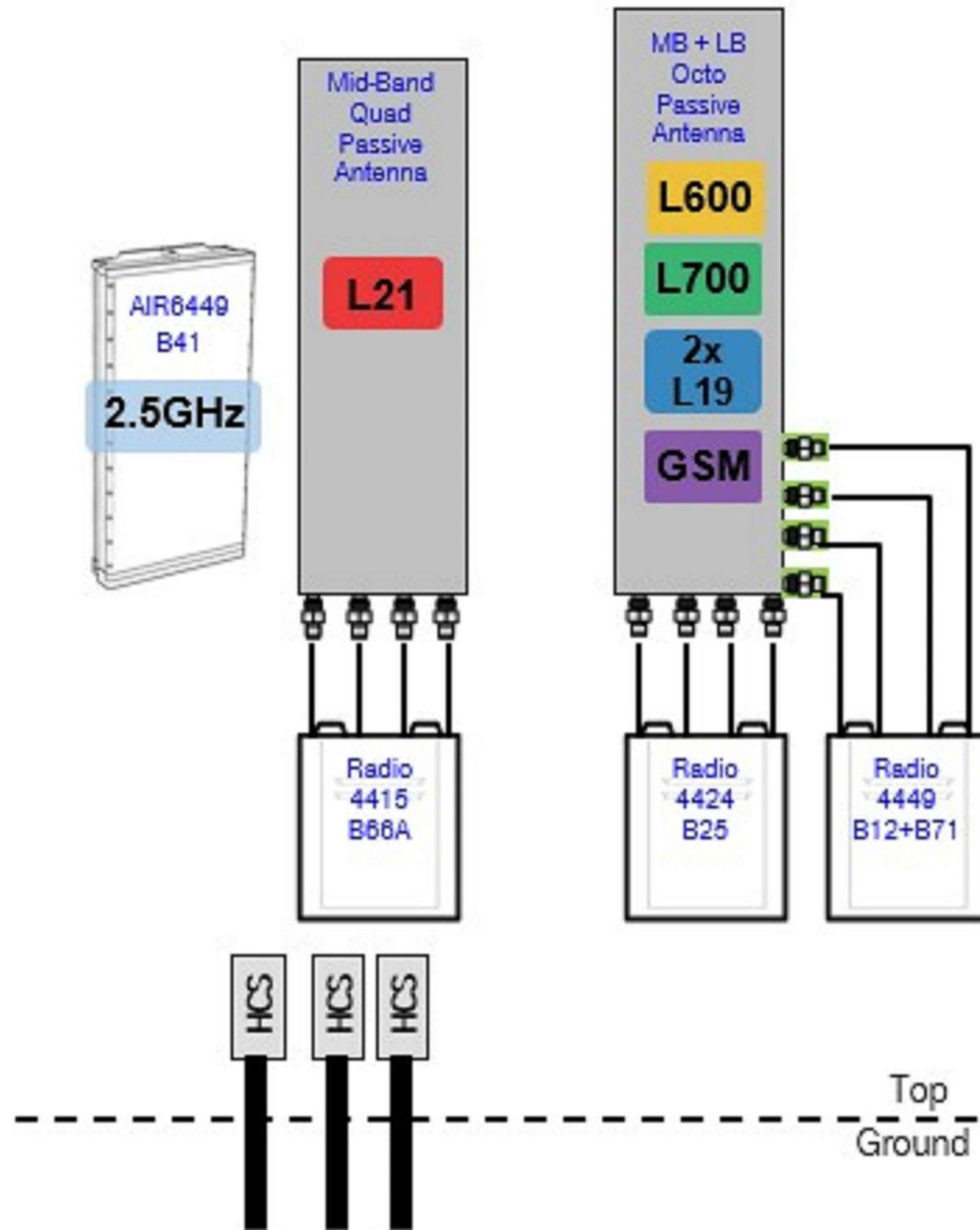
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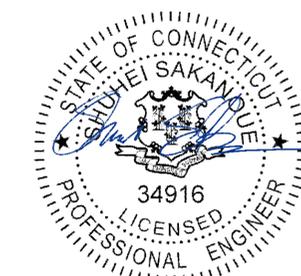
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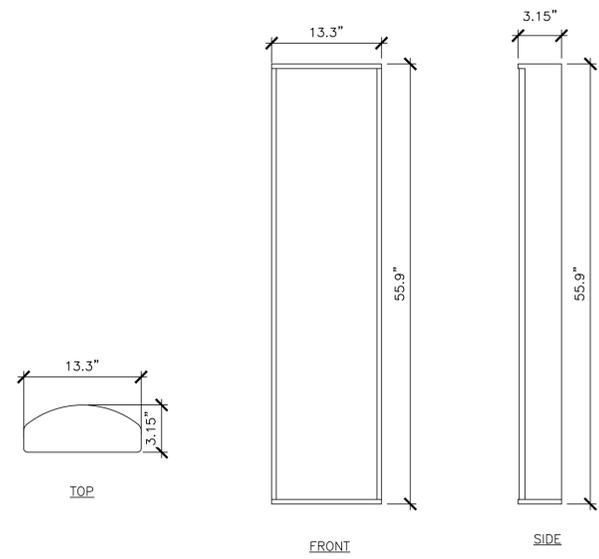


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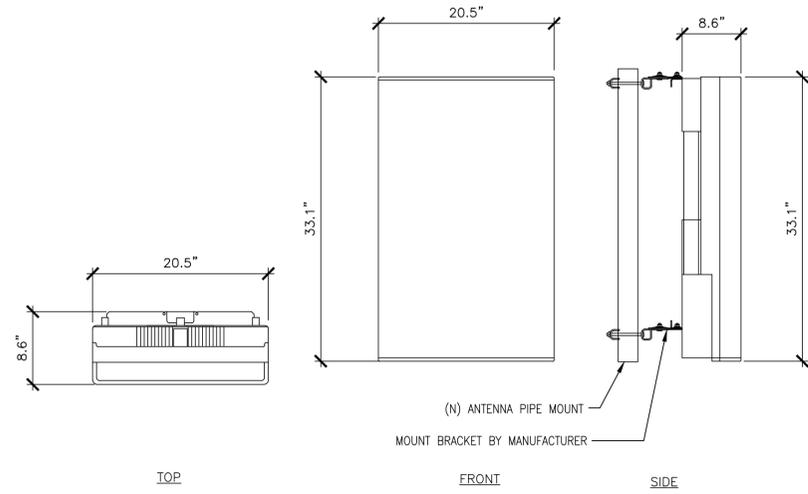
C-4 0

MANUFACTURER: RFS
 MODEL: APX16DW-16DW-S-E-A20
 WEIGHT: 40.7 LBS
 DIMENSIONS: 55.9"H. X 13.3"W. X 3.15"D.
 FREQUENCY: REFER TO RF DATA SHEET



② (N) APX16DW-16DW-S-E-A20 ANTENNA SPEC
 SCALE: NOT TO SCALE

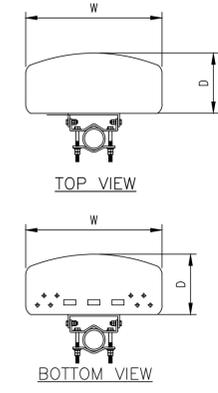
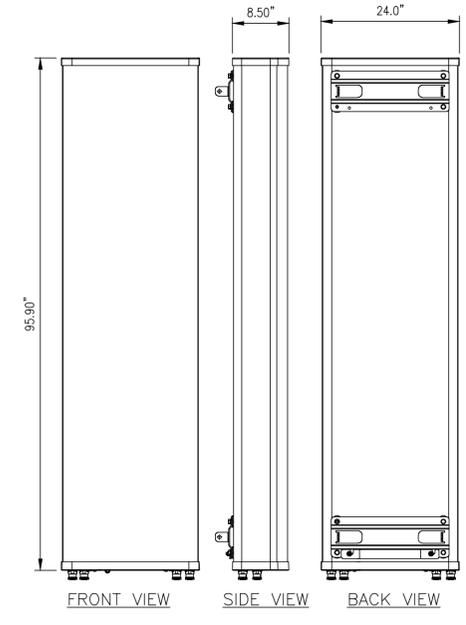
MANUFACTURER: ERICSSON
 MODEL: AIR6449 B41
 WEIGHT: 104 LBS (W/ MOUNT BRACKET 113)
 DIMENSIONS: 33.1"H. X 20.5"W. X 8.6"D.
 FREQUENCY: REFER TO RF DATA SHEET



① (N) AIR6449 B41 ANTENNA SPEC
 SCALE: NOT TO SCALE

700MHz RFS ANTENNAS

MODEL	WEIGHT (lb)
(8') APXVAALL24_43-UNA20	149.90
WEIGHT W/ MOUNTING BRACKET (lb):	154



③ (N) APXVAALL24_43-UNA20 ANTENNA SPEC
 SCALE: NOT TO SCALE

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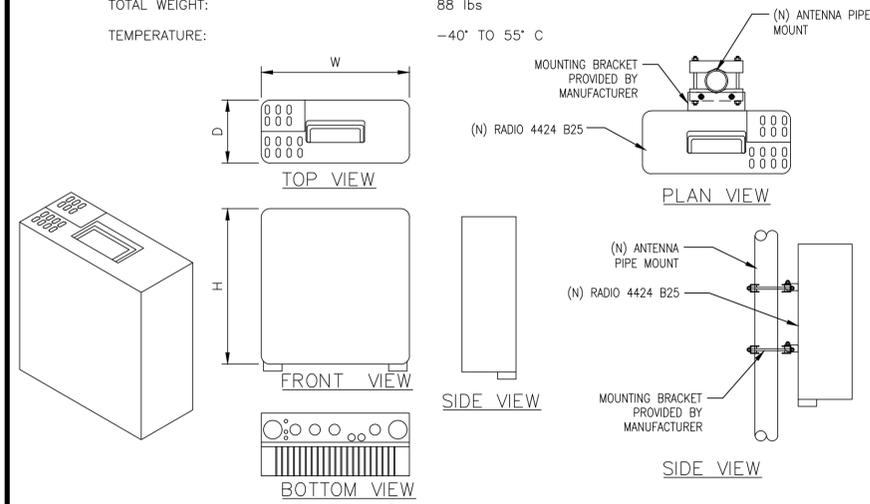
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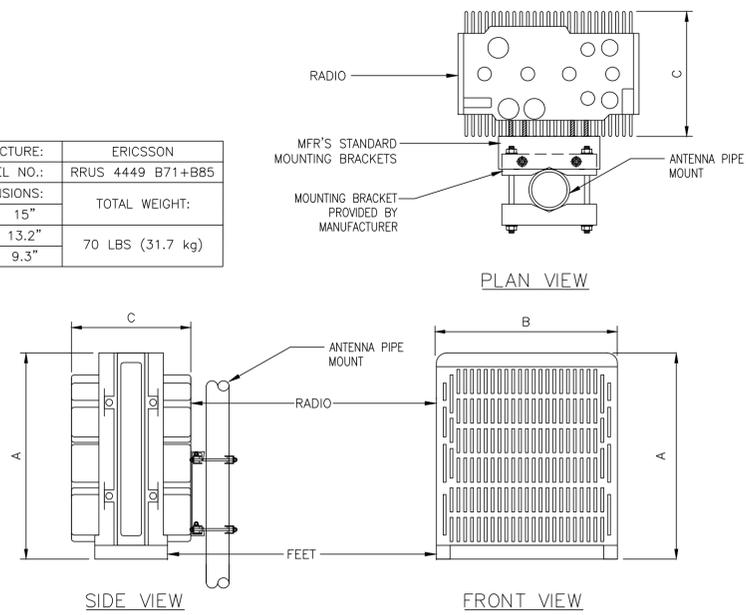
ERICSSON RADIO-4424 B25

DIMENSIONS, WxDxH: 13.5"x9.6"x16.5"
 MAX OUTPUT POWER: 4x80W (2x(2x80W))
 TOTAL WEIGHT: 88 lbs
 TEMPERATURE: -40° TO 55° C



④ (N) RADIO 4424 B25 SPEC
 SCALE: NOT TO SCALE

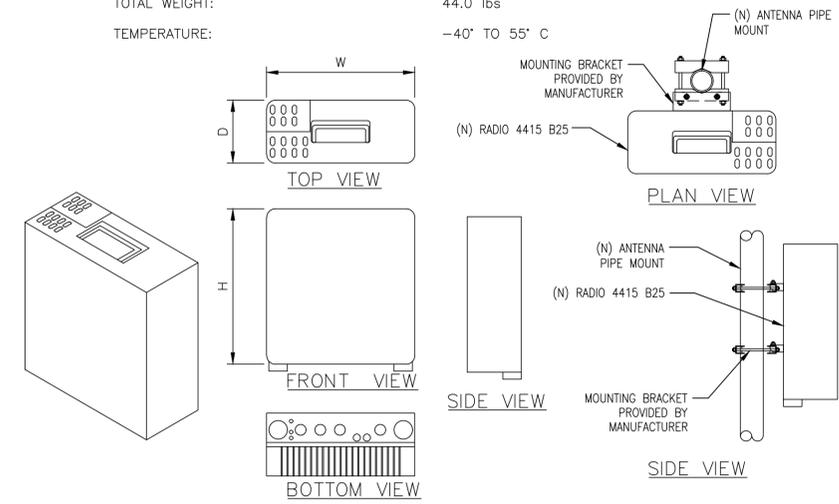
MANUFACTURE:		ERICSSON	
MODEL NO.:		RRUS 4449 B71+B85	
DIMENSIONS:		TOTAL WEIGHT:	
A	15"	70 LBS (31.7 kg)	
B	13.2"		
C	9.3"		



⑤ (N) RADIO 4449 B71+B85 SPEC
 SCALE: NOT TO SCALE

ERICSSON RADIO-4415 B25

DIMENSIONS, WxDxH: 13.19"x5.39"x14.96"
 POWER CONSUMPTION: 660 WATTS
 TOTAL WEIGHT: 44.0 lbs
 TEMPERATURE: -40° TO 55° C

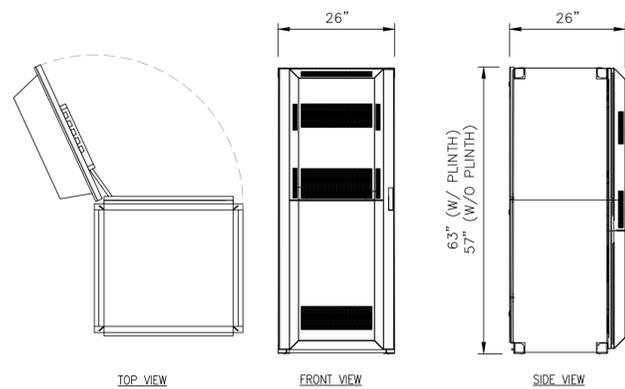


⑥ (N) RADIO 4415 B25 SPEC
 SCALE: NOT TO SCALE

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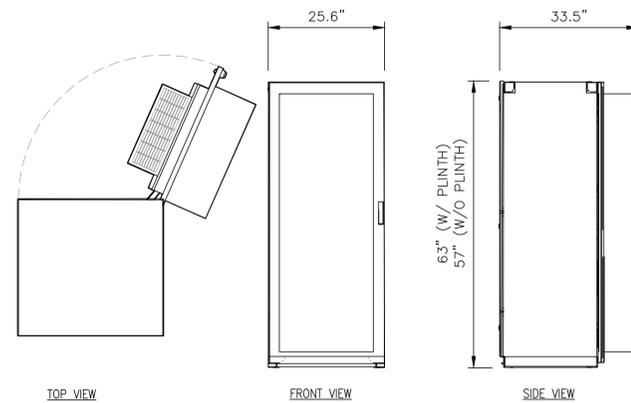
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SHEET NUMBER: C-5	REVISION: 0
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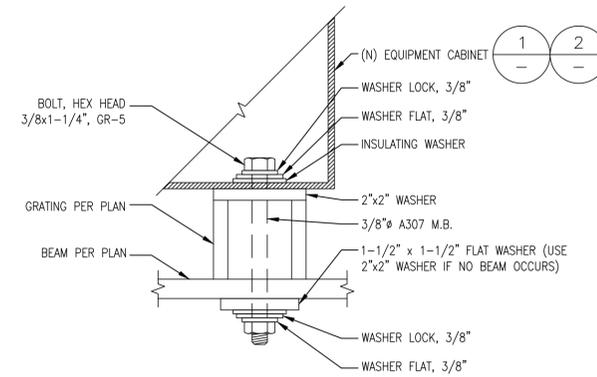
ERICSSON MODEL NO.:	B160
RACK SPACE:	19U
DIMENSIONS, HxWxD:	63"x26"x26" (W/ 6" PLINTH)
CABINET WEIGHT, EMPTY:	485 LBS
MAXIMUM WEIGHT:	2100± LBS

1 (N) B160 CABINET DETAIL
SCALE: NOT TO SCALE

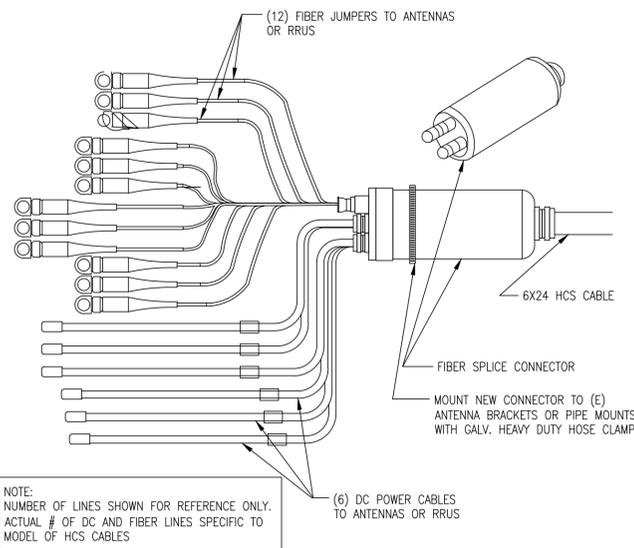


ERICSSON MODEL NO.:	6160
RACK SPACE:	19U
DIMENSIONS, HxWxD:	63"x25.6"x25.6" (W/ 6" PLINTH)
CABINET WEIGHT, EMPTY:	410 LBS
MAXIMUM WEIGHT:	770± LBS

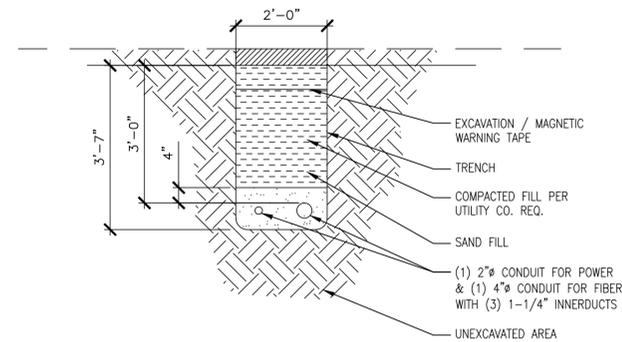
2 (N) 6160 CABINET DETAIL
SCALE: NOT TO SCALE



3 (N) EQUIPMENT CABINET MOUNTING DETAIL
SCALE: NOT TO SCALE



4 (N) 6X24 HCS CABLE DETAIL
SCALE: NOT TO SCALE



5 (N) CONDUIT TRENCH DETAIL
SCALE: NOT TO SCALE

6 NOT USED
SCALE: NOT TO SCALE

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EXISTING 178'-0" MONOPOLE

ISSUED FOR:

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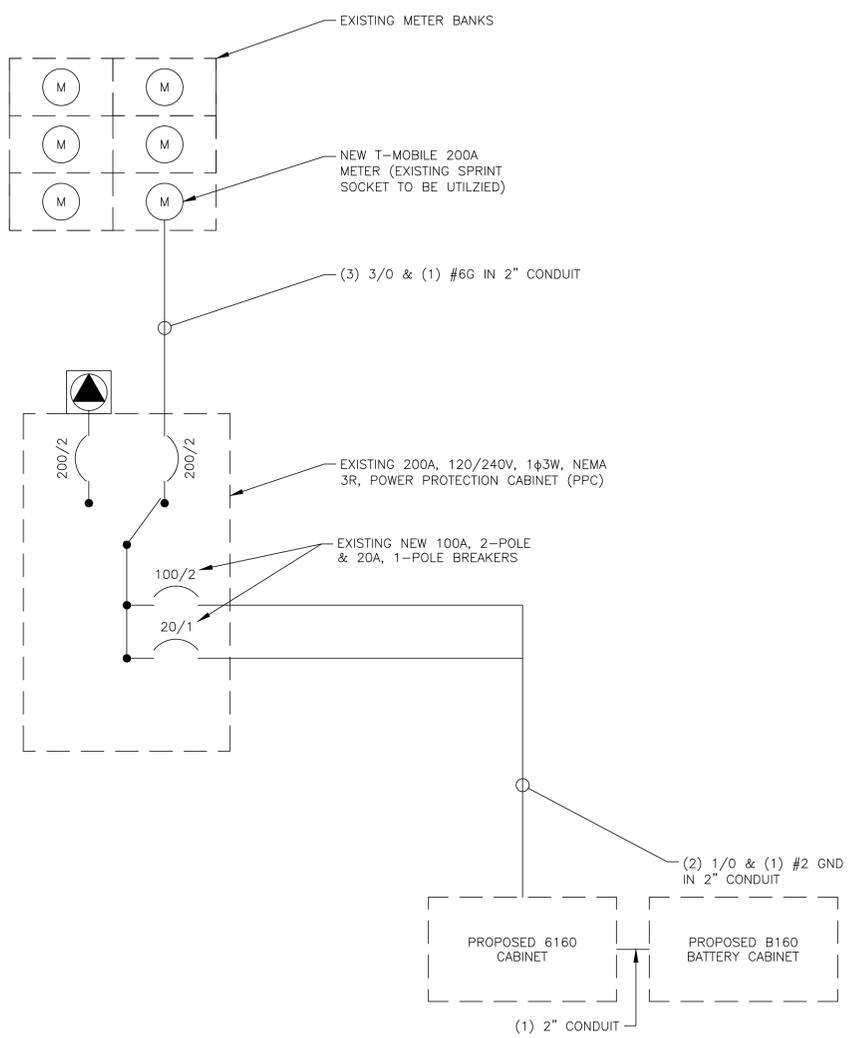
SHEET NUMBER: **C-6** REVISION: **0**

T-MOBILE PANEL SCHEDULE											
MAIN: 200A MAIN BREAKER			VOTAGE/PHASE: 120/240V, 1-PHASE, 3-WIRE				SHORT CIRCUIT CURRENT RATING: ---				
MOUNTING: INSIDE PPC ENCLOSURE			ENCLOSURE: NEMA 3R				SURGE PROTECTION DEVICE: YES				
DESCRIPTION	LOAD (VA)	C or NC	C/B	CIR No.	PHASE LOADS (VA)		CIR No.	C/B	C or NC	LOAD (VA)	DESCRIPTION
					A	B					
6160**	7000	C	100	1	7000		7	60	NC	0	(NOT CONNECTED)
	7000	C		2		7000	8		NC	0	
outside lights	200	NC	20	3	200		9		NC	0	Blank
6160 GFI**	180	NC	20	4		180	10		NC	0	Blank
NID Outlet	180	NC	15	5	1260		11	15	NC	1080	Not Labeled
Telco Fan (OFF)	0	NC	10	6		0	12		NC	0	BLANK
BASE LOAD (VA) =					8640	7180	C = CONTINUOUS LOAD; NC = NON-CONTINUOUS LOAD				
25% OF CONTINUOUS LOAD (VA) =					2115	1795	*INDICATES NEW LOAD. ALL OTHER LOADS ARE EXISTING.				
TOTAL LOAD (VA) =					10755	8975	NEW BREAKER TO BE SAME TYPE AND HAVE SAME AIC RATING AS EXISTING. CUSTOMER HAS NOT PROVIDED LOADS FOR EQUIPMENT CABINETS THEREFORE THE CABINET LOADS SHOWN ARE ESTIMATED				
TOTAL LOAD (A) =					89.6	74.8					

1 AC PANEL SCHEDULE
SCALE: NOT TO SCALE

NOTES:

- ALL NEW CONDUCTORS TO BE INSTALLED SHALL BE COPPER. ALL CONDUCTORS SHALL BE THHW, THWN, THWN-2, XHHW, OR XHHW-2 UNLESS NOTED OTHERWISE.
- CONTRACTOR IS TO FIELD VERIFY ALL EXISTING ITEMS SHOWN ON THE ELECTRICAL ONE-LINE DIAGRAM AND NOTIFY THE ENGINEER OF ANY DISCREPANCIES.
- ALL GROUNDING AND BONDING PER THE NEC.



2 ONE LINE DIAGRAM
SCALE: NOT TO SCALE

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STATE OF CONNECTICUT
DHMEI SAKAMOTO
34916
LICENSED PROFESSIONAL ENGINEER
4/22/21

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SHEET NUMBER: **E-1** REVISION: **0**

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HARWINTON, CT 06791

EXISTING 178'-0" MONOPOLE

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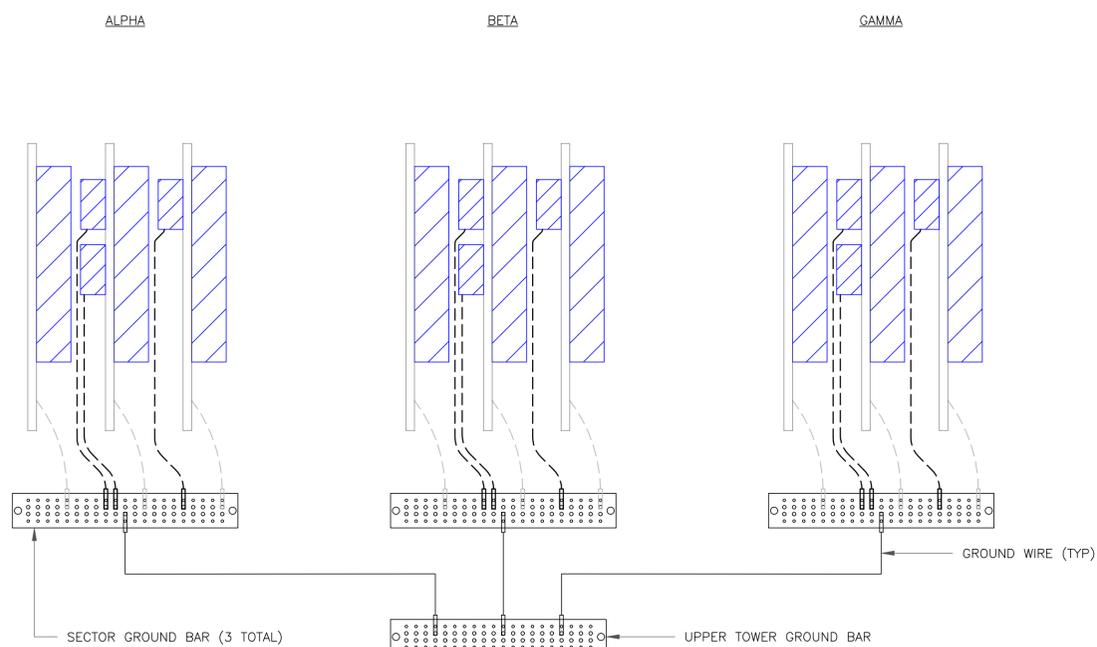
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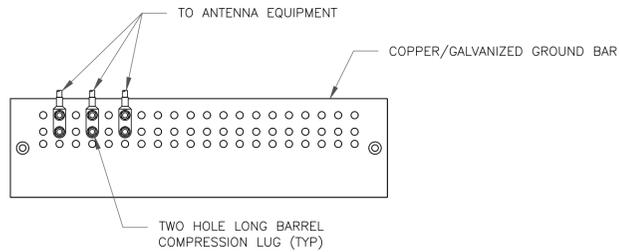
SHEET NUMBER: REVISION:

G-1 0



NOTE:
ALL NEW GROUNDS TO BE #6 STRANDED
COPPER WITH GREEN INSULATION UNLESS
NOTED OTHERWISE.

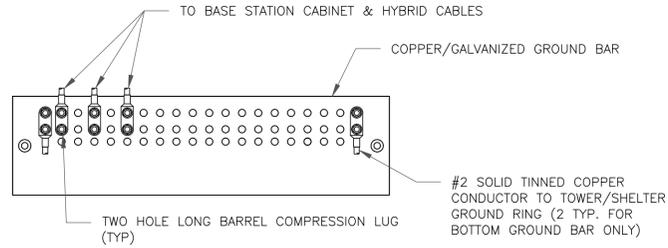
1 ANTENNA GROUNDING DIAGRAM
SCALE: NOT TO SCALE



NOTES:

1. DOUBLING UP "OR STACKING" OF CONNECTIONS IS NOT PERMITTED.
2. EXTERIOR ANTIOXIDANT JOINT COMPOUND TO BE USED ON ALL EXTERIOR CONNECTIONS.
3. GROUND BAR SHALL NOT BE ISOLATED FROM TOWER. MOUNT DIRECTLY TO ANTENNA MOUNT STEEL.

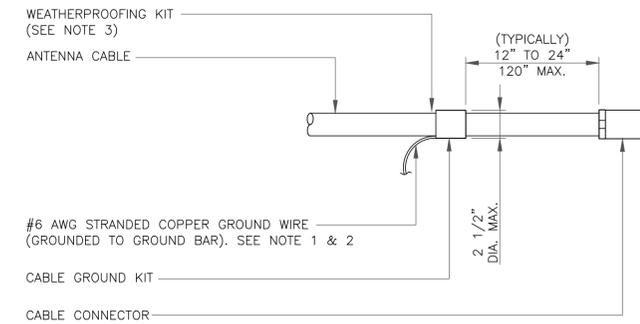
1 ANTENNA SECTOR GROUND BAR DETAIL
SCALE: NOT TO SCALE



NOTES:

1. EXTERIOR ANTIOXIDANT JOINT COMPOUND TO BE USED ON ALL EXTERIOR CONNECTIONS.
2. GROUND BAR SHALL NOT BE ISOLATED FROM TOWER. MOUNT DIRECTLY TO TOWER STEEL (TOWER ONLY).
3. GROUND BAR SHALL BE ISOLATED FROM BUILDING OR SHELTER.

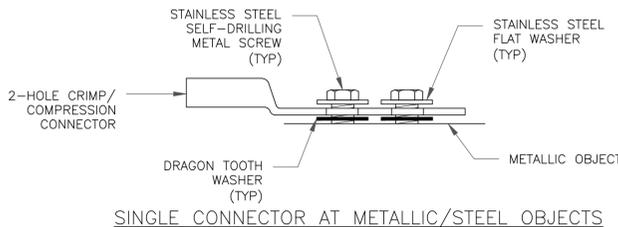
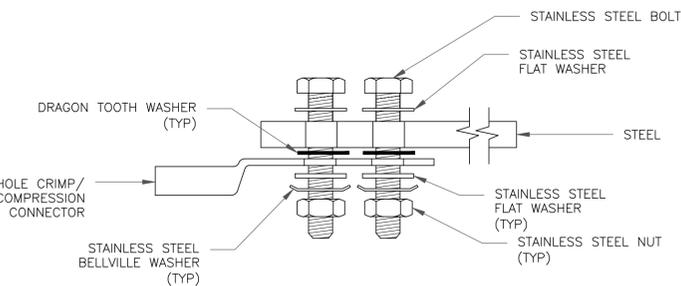
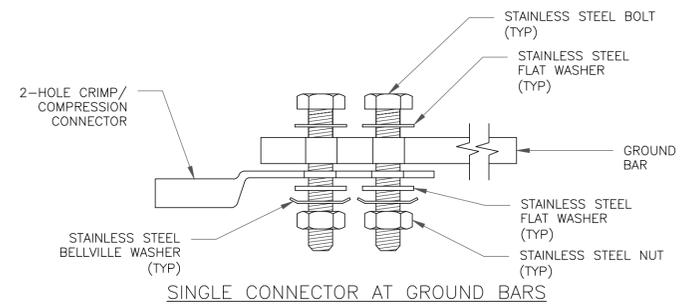
2 TOWER/SHELTER GROUND BAR DETAIL
SCALE: NOT TO SCALE



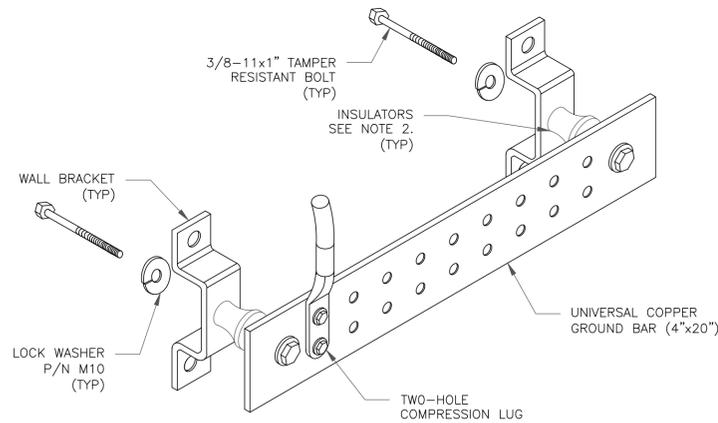
NOTES:

1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.
2. GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.
3. WEATHER PROOFING SHALL BE TWO-PART TAPE KIT, COLD SHRINK SHALL NOT BE USED.

3 CABLE GROUND KIT CONNECTION
SCALE: NOT TO SCALE



4 HARDWARE DETAIL FOR EXTERIOR CONNECTIONS
SCALE: NOT TO SCALE



NOTES:

1. DOWN LEAD (HOME RUN) CONDUCTORS ARE NOT TO BE INSTALLED ON CROWN CASTLE USA INC. TOWER, PER THE GROUNDING DOWN CONDUCTOR POLICY OAS-STD-10091. NO MODIFICATION OR DRILLING TO TOWER STEEL IS ALLOWED IN ANY FORM OR FASHION, CAD-WELDING ON THE TOWER AND/OR IN THE AIR ARE NOT PERMITTED.
2. OMIT INSULATOR WHEN MOUNTING TO TOWER STEEL OR PLATFORM STEEL USE INSULATORS WHEN ATTACHING TO BUILDING OR SHELTERS.

5 GROUND BAR DETAIL
SCALE: NOT TO SCALE

6 NOT USED
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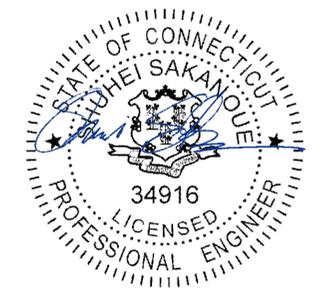
BU #: 876369
HARWINTON / BUCKLEY BROADCASTI

64 HUNGERFORD LANE
HARWINTON, CT 06791

EXISTING 178'-0" MONOPOLE

ISSUED FOR:

REV	DATE	DRWN	DESCRIPTION	DES./QA
A	05/23/2021	RCD	PRELIMINARY	SS
0	04/22/2021	BMM	FINAL	SS



4/22/21

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

SHEET NUMBER: **G-2** REVISION: **0**

Exhibit D

Structural Analysis Report

Date: **March 16, 2021**



Black & Veatch Corp.
6800 W. 115th St., Suite 2292
Overland Park, KS 66211
(913) 458-6909

Subject: **Structural Analysis Report**

Carrier Designation:

Site Number: CTNH558A
Site Name: CTNH558A

Crown Castle Designation:

BU Number: 876369
Site Name: HARWINTON / BUCKLEY
BROADCASTI
JDE Job Number: 628849
Work Order Number: 1918906
Order Number: 538773 Rev. 1

Engineering Firm Designation:

Black & Veatch Corp. Project Number: 406642

Site Data:

64 Hungerford Lane, Harwinton, Litchfield County, CT
Latitude 41° 45' 26.15", Longitude -73° 3' 9.2"
178 Foot - Monopole Tower

Black & Veatch Corp. is pleased to submit this “**Structural Analysis Report**” to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Proposed Equipment Configuration

Sufficient Capacity - 74.5%

This analysis utilizes an ultimate 3-second gust wind speed of 120 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: Thanyaporn Yakhasem / Kanchanaporn Rattanachan

Respectfully submitted by:

Joshua J. Riley, P.E.
Professional Engineer



03/16/2021

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1) INTRODUCTION

This tower is a 178 ft Monopole tower designed by Engineered Endeavors, Inc.

2) ANALYSIS CRITERIA

TIA-222 Revision:	TIA-222-H
Risk Category:	II
Wind Speed:	120 mph
Exposure Category:	C
Topographic Factor:	1
Ice Thickness:	1.500 in
Wind Speed with Ice:	50 mph
Service Wind Speed:	60 mph

Table 1 - Proposed Equipment Configuration

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
178.0	180.0	3	ericsson	AIR6449 B41_T-MOBILE w/ Mount Pipe	4	1-5/8
		3	ericsson	RADIO 4415 B66A_CCIV3		
		3	ericsson	RADIO 4424 B25_TMO		
		3	ericsson	RADIO 4449 B71 B85A_T-MOBILE		
		3	rfs celwave	APX16DWV-16DWV-S-E-A20 w/ Mount Pipe		
	3	rfs celwave	APXVAALL24_43-U-NA20_TMO w/ Mount Pipe			
	178.0	1	site pro 1	RMQP-496-HK + PRK-1245 + HRK12 12.5' Platform with Handrails and Kickers		
75.0	76.0	1	lucent	KS24019-L112A	1	1/2

Table 2 - Other Considered Equipment

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
168.0	170.0	3	antel	BXA-171085-12BF-2 w/ Mount Pipe	12	1-5/8
		3	antel	BXA-70063-6CF-2 w/ Mount Pipe		
		6	antel	LPA-80080/6CF w/ Mount Pipe		
		6	rfs celwave	FD9R6004/2C-3L		
		168.0	1	cci tower mounts (v2.1)		
156.0	158.0	1	cci antennas	HPA65R-BU6A w/ Mount Pipe		
		2	commscope	SBNHH-1D65A w/ Mount Pipe		
		3	ericsson	RRUS 4449 B5/B12		
		3	ericsson	RRUS 8843 B2/B66A		

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
		3	kathrein	80010964 w/ Mount Pipe	12 2 2 2 1	1-5/8 3/4 7/16 3/8 2"conduit
		3	powerwave technologies	7770.00 w/ Mount Pipe		
		6	powerwave technologies	LGP21401		
		1	raycap	DC6-48-60-18-8C-EV		
		1	raycap	DC6-48-60-18-8F		
	156.0	1	cci tower mounts (v2.1)	Platform Mount [LP 303-1]		

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Reference	Source
4-GEOTECHNICAL REPORTS	1532983	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	2150286	CCISITES
4-TOWER MANUFACTURER DRAWINGS	2150280	CCISITES

3.1) Analysis Method

tnxTower (version 8.0.7.5), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

3.2) Assumptions

- 1) Tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Black & Veatch Corp. should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary) (Monopole Tower)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	178 - 129.87	Pole	TP29.64x19.5x0.25	1	-13.30	1388.85	60.7	Pass
L2	129.87 - 84.83	Pole	TP38.5x28.2446x0.375	2	-22.70	2706.23	57.1	Pass
L3	84.83 - 41.2833	Pole	TP46.8x36.6403x0.4375	3	-35.66	3840.78	56.8	Pass
L4	41.2833 - 0	Pole	TP54.5x44.5913x0.5	4	-55.03	5264.00	54.0	Pass
							Summary	
						Pole (L1)	60.7	Pass
						Rating =	60.7	Pass

Table 5 - Tower Component Stresses vs. Capacity (Monopole Tower) - LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	52.3	Pass
	Base Plate		61.1	Pass
1	Base Foundation	0	74.5	Pass
	Base Foundation Soil Interaction		66.6	Pass

Structure Rating (max from all components) =	74.5%
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Notes:

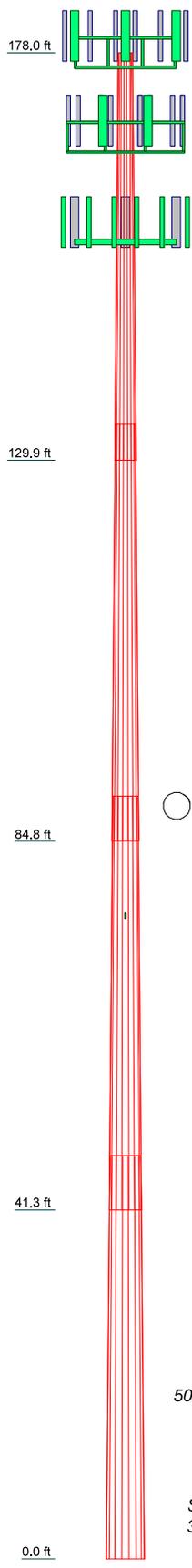
- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity. Rating per TIA-222-H Section 15.5.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

Section	1	2	3	4	
Length (ft)	48.13	45.29	48.88	47.70	
Number of Sides	18	18	18	18	
Thickness (in)	0.2500	0.3750	0.4375	0.5000	
Socket Length (ft)	4.25	5.33	6.42		
Top Dia (in)	19.5000	28.2446	36.6403	44.5913	
Bot Dia (in)	29.6400	38.5000	46.8000	54.5000	
Grade			A572-65		
Weight (K)	3.2	6.6	9.5	12.6	31.9



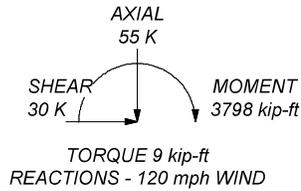
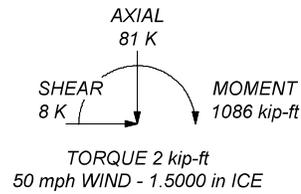
MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower is located in Litchfield County, Connecticut.
2. Tower designed for Exposure C to the TIA-222-H Standard.
3. Tower designed for a 120 mph basic wind in accordance with the TIA-222-H Standard.
4. Tower is also designed for a 50 mph basic wind with 1.50 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Risk Category II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING: 60.7%

ALL REACTIONS ARE FACTORED



Black & Veatch Corp.
 6800 W. 115th St., Suite 2292
 Overland Park, KS 66211
 Phone: (913) 458-6909
 FAX:

Job: HARWINTON / BUCKLEY BROADCASTI (BU# 87636)		
Project: 406642 (876369.1918906)	Drawn by: Kanchanaporn Rattanachan	App'd:
Client: Crown Castle	Date: 03/16/21	Scale: NTS
Code: TIA-222-H	Path:	Dwg No. E-1

Tower Input Data

The tower is a monopole.
 This tower is designed using the TIA-222-H standard.
 The following design criteria apply:

- 1) Tower is located in Litchfield County, Connecticut.
- 2) Tower base elevation above sea level: 841.00 ft.
- 3) Basic wind speed of 120 mph.
- 4) Risk Category II.
- 5) Exposure Category C.
- 6) Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- 7) Topographic Category: 1.
- 8) Crest Height: 0.00 ft.
- 9) Nominal ice thickness of 1.5000 in.
- 10) Ice thickness is considered to increase with height.
- 11) Ice density of 56 pcf.
- 12) A wind speed of 50 mph is used in combination with ice.
- 13) Temperature drop of 50 °F.
- 14) Deflections calculated using a wind speed of 60 mph.
- 15) A non-linear (P-delta) analysis was used.
- 16) Pressures are calculated at each section.
- 17) Stress ratio used in pole design is 1.05.
- 18) Tower analysis based on target reliabilities in accordance with Annex S.
- 19) Load Modification Factors used: $K_{es}(F_w) = 0.95$, $K_{es}(t_i) = 0.85$.
- 20) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification Use Code Stress Ratios Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric	Distribute Leg Loads As Uniform Assume Legs Pinned ✓ Assume Rigid Index Plate ✓ Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension ✓ Bypass Mast Stability Checks ✓ Use Azimuth Dish Coefficients ✓ Project Wind Area of Appurt. Autocalc Torque Arm Areas Add IBC .6D+W Combination Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs	Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation ✓ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice Exemption <div style="text-align: center; background-color: #e0e0e0; padding: 2px;">Poles</div> ✓ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known
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Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft		in	in	in	in	
L1	178.00-129.87	48.13	4.25	18	19.5000	29.6400	0.2500	1.0000	A572-65 (65 ksi)

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L2	129.87-84.83	49.29	5.33	18	28.2446	38.5000	0.3750	1.5000	A572-65 (65 ksi)
L3	84.83-41.28	48.88	6.42	18	36.6403	46.8000	0.4375	1.7500	A572-65 (65 ksi)
L4	41.28-0.00	47.70		18	44.5913	54.5000	0.5000	2.0000	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	I/Q in ²	w in	w/t
L1	19.7623	15.2749	715.1161	6.8338	9.9060	72.1902	1431.1733	7.6389	2.9920	11.968
	30.0587	23.3210	2544.9728	10.4335	15.0571	169.0212	5093.2943	11.6627	4.7766	19.107
L2	29.5204	33.1718	3255.1307	9.8937	14.3483	226.8658	6514.5446	16.5891	4.3111	11.496
	39.0361	45.3783	8333.0732	13.5344	19.5580	426.0698	16677.111	22.6935	6.1160	16.309
L3	38.2637	50.2722	8324.3516	12.8520	18.6133	447.2261	16659.656	25.1409	5.6787	12.98
	47.4545	64.3801	17483.282	16.4587	23.7744	735.3827	34989.569	32.1962	7.4668	17.067
L4	46.5556	69.9729	17185.926	15.6524	22.6524	758.6809	34394.467	34.9931	6.9681	13.936
	55.2636	85.6980	31571.532	19.1700	27.6860	1140.3428	63184.606	42.8571	8.7120	17.424

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _r	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontal in	Double Angle Stitch Bolt Spacing Redundants in
L1 178.00- 129.87				1	1	1			
L2 129.87- 84.83				1	1	1			
L3 84.83- 41.28				1	1	1			
L4 41.28-0.00				1	1	1			

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Sector	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	Number Per Row	Start/End Position	Width or Diameter in	Perimeter in	Weight plf
Safety Line 3/8 ***	B	No	Surface Ar (CaAa)	178.00 - 10.00	1	1	-0.400 -0.390	0.3750		0.22
LDF4-50A(1/2) ***	C	No	Surface Ar (CaAa)	75.00 - 8.00	1	1	0.170 0.183	0.6250		0.15

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	C _A A _A ft ² /ft	Weight plf
-------------	-------------------	-----------------	--	-------------------	-----------------	-----------------	--	---------------

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		C _A A _A ft ² /ft	Weight plf

HB158-21U6S24-xxM_TMO(1-5/8)	C	No	No	Inside Pole	178.00 - 0.00	4	No Ice	0.00	2.50
							1/2" Ice	0.00	2.50
							1" Ice	0.00	2.50
							2" Ice	0.00	2.50

LDF7-50A(1-5/8)	C	No	No	Inside Pole	168.00 - 0.00	12	No Ice	0.00	0.82
							1/2" Ice	0.00	0.82
							1" Ice	0.00	0.82
							2" Ice	0.00	0.82

2" innerduct conduit	C	No	No	Inside Pole	156.00 - 0.00	1	No Ice	0.00	0.20
							1/2" Ice	0.00	0.20
							1" Ice	0.00	0.20
							2" Ice	0.00	0.20
LDF7-50A(1-5/8)	C	No	No	Inside Pole	156.00 - 0.00	12	No Ice	0.00	0.82
							1/2" Ice	0.00	0.82
							1" Ice	0.00	0.82
							2" Ice	0.00	0.82
FB-L98B-002-75000(3/8)	C	No	No	Inside Pole	156.00 - 0.00	2	No Ice	0.00	0.06
							1/2" Ice	0.00	0.06
							1" Ice	0.00	0.06
							2" Ice	0.00	0.06
WR-VG122ST-BRDA(7/16)	C	No	No	Inside Pole	156.00 - 0.00	2	No Ice	0.00	0.14
							1/2" Ice	0.00	0.14
							1" Ice	0.00	0.14
							2" Ice	0.00	0.14
WR-VG86ST-BRD(3/4)	C	No	No	Inside Pole	156.00 - 0.00	2	No Ice	0.00	0.58
							1/2" Ice	0.00	0.58
							1" Ice	0.00	0.58
							2" Ice	0.00	0.58

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L1	178.00-129.87	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	1.805	0.000	0.01
		C	0.000	0.000	0.000	0.000	1.16
L2	129.87-84.83	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	1.689	0.000	0.01
		C	0.000	0.000	0.000	0.000	1.42
L3	84.83-41.28	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	1.633	0.000	0.01
		C	0.000	0.000	2.107	0.000	1.37
L4	41.28-0.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	1.173	0.000	0.01
		C	0.000	0.000	2.080	0.000	1.30

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L1	178.00-129.87	A	1.486	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	16.108	0.000	0.17
		C		0.000	0.000	0.000	0.000	1.16
L2	129.87-84.83	A	1.434	0.000	0.000	0.000	0.000	0.00

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L3	84.83-41.28	B	1.359	0.000	0.000	15.074	0.000	0.16
		C		0.000	0.000	0.000	0.000	1.42
		A		0.000	0.000	0.000	0.000	0.00
L4	41.28-0.00	B	1.218	0.000	0.000	14.118	0.000	0.15
		C		0.000	0.000	11.774	0.000	1.50
		A		0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	9.679	0.000	0.10
		C		0.000	0.000	11.130	0.000	1.41

Feed Line Center of Pressure

Section	Elevation ft	CP _x in	CP _z in	CP _x Ice in	CP _z Ice in
L1	178.00-129.87	0.0657	-0.2939	0.2916	-1.3045
L2	129.87-84.83	0.0659	-0.2947	0.3082	-1.3789
L3	84.83-41.28	-0.0778	0.0782	-0.1237	-0.2350
L4	41.28-0.00	-0.0950	0.1536	-0.2001	0.1057

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L1	1	Safety Line 3/8	129.87 - 178.00	1.0000	1.0000
L2	1	Safety Line 3/8	84.83 - 129.87	1.0000	1.0000
L3	1	Safety Line 3/8	41.28 - 84.83	1.0000	1.0000
L3	15	LDF4-50A(1/2)	41.28 - 75.00	1.0000	1.0000
L4	1	Safety Line 3/8	10.00 - 41.28	1.0000	1.0000
L4	15	LDF4-50A(1/2)	8.00 - 41.28	1.0000	1.0000

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral ft	Azimuth Adjustmen t °	Placement ft	C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K	
Site Pro 1 RMQP-496-HK + PRK-1245 + HRK12	C	None		0.0000	178.00	No Ice	23.14	21.40	1.95
12.5' Platform with Handrails and Kickers						1/2"	28.17	26.44	2.34
						Ice	33.23	31.60	2.85
						1" Ice	43.26	41.56	3.50

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A _{Front}	C _A A _{Side}	Weight
			Horz	Lateral					
APX16DWV-16DWV-S-E-A20 w/ Mount Pipe	A	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	6.29	2.76	0.06
						1/2"	6.86	3.27	0.11
						Ice	7.45	3.79	0.16
APX16DWV-16DWV-S-E-A20 w/ Mount Pipe	B	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	6.29	2.76	0.06
						1/2"	6.86	3.27	0.11
						Ice	7.45	3.79	0.16
APX16DWV-16DWV-S-E-A20 w/ Mount Pipe	C	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	6.29	2.76	0.06
						1/2"	6.86	3.27	0.11
						Ice	7.45	3.79	0.16
APXVAALL24_43-U-NA20_TMO w/ Mount Pipe	A	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	14.69	6.87	0.18
						1/2"	15.46	7.55	0.31
						Ice	16.23	8.25	0.45
APXVAALL24_43-U-NA20_TMO w/ Mount Pipe	B	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	14.69	6.87	0.18
						1/2"	15.46	7.55	0.31
						Ice	16.23	8.25	0.45
APXVAALL24_43-U-NA20_TMO w/ Mount Pipe	C	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	14.69	6.87	0.18
						1/2"	15.46	7.55	0.31
						Ice	16.23	8.25	0.45
AIR6449 B41_T-MOBILE w/ Mount Pipe	A	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	5.87	3.27	0.13
						1/2"	6.23	3.73	0.18
						Ice	6.61	4.20	0.23
AIR6449 B41_T-MOBILE w/ Mount Pipe	B	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	5.87	3.27	0.13
						1/2"	6.23	3.73	0.18
						Ice	6.61	4.20	0.23
AIR6449 B41_T-MOBILE w/ Mount Pipe	C	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	5.87	3.27	0.13
						1/2"	6.23	3.73	0.18
						Ice	6.61	4.20	0.23
RADIO 4415 B66A_CCIV3	A	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	1.64	0.68	0.05
						1/2"	1.80	0.79	0.06
						Ice	1.97	0.91	0.07
RADIO 4415 B66A_CCIV3	B	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	1.64	0.68	0.05
						1/2"	1.80	0.79	0.06
						Ice	1.97	0.91	0.07
RADIO 4415 B66A_CCIV3	C	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	1.64	0.68	0.05
						1/2"	1.80	0.79	0.06
						Ice	1.97	0.91	0.07
RADIO 4449 B71 B85A_T-MOBILE	A	From Face	4.00	0.0000	178.00	2" Ice			
						No Ice	1.97	1.59	0.07
						1/2"	2.15	1.75	0.09
						Ice	2.33	1.92	0.12
						1" Ice	2.72	2.28	0.17

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment t °	Placement ft	C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K	
RADIO 4449 B71 B85A_T-MOBILE	B	From Face	4.00 0.00 2.00	0.0000	178.00	2" Ice			
						No Ice	1.97	1.59	0.07
						1/2"	2.15	1.75	0.09
						Ice	2.33	1.92	0.12
						1" Ice	2.72	2.28	0.17
RADIO 4449 B71 B85A_T-MOBILE	C	From Face	4.00 0.00 2.00	0.0000	178.00	2" Ice			
						No Ice	1.97	1.59	0.07
						1/2"	2.15	1.75	0.09
						Ice	2.33	1.92	0.12
						1" Ice	2.72	2.28	0.17
RADIO 4424 B25_TMO	A	From Face	4.00 0.00 2.00	0.0000	178.00	2" Ice			
						No Ice	2.05	1.61	0.09
						1/2"	2.23	1.77	0.11
						Ice	2.42	1.94	0.13
						1" Ice	2.81	2.30	0.19
RADIO 4424 B25_TMO	B	From Face	4.00 0.00 2.00	0.0000	178.00	2" Ice			
						No Ice	2.05	1.61	0.09
						1/2"	2.23	1.77	0.11
						Ice	2.42	1.94	0.13
						1" Ice	2.81	2.30	0.19
RADIO 4424 B25_TMO	C	From Face	4.00 0.00 2.00	0.0000	178.00	2" Ice			
						No Ice	2.05	1.61	0.09
						1/2"	2.23	1.77	0.11
						Ice	2.42	1.94	0.13
						1" Ice	2.81	2.30	0.19
*** ***									
Platform Mount [16' LP 403-1]	C	None		0.0000	168.00	No Ice	20.20	20.20	1.60
						1/2"	24.86	24.86	2.03
						Ice	29.59	29.59	2.53
						1" Ice	39.22	39.22	3.77
						2" Ice			
(2) LPA-80080/6CF w/ Mount Pipe	A	From Face	4.00 0.00 2.00	0.0000	168.00	No Ice	4.56	10.26	0.05
						1/2"	5.11	11.43	0.11
						Ice	5.61	12.31	0.19
						1" Ice	6.65	14.13	0.36
						2" Ice			
(2) LPA-80080/6CF w/ Mount Pipe	B	From Face	4.00 0.00 2.00	0.0000	168.00	No Ice	4.56	10.26	0.05
						1/2"	5.11	11.43	0.11
						Ice	5.61	12.31	0.19
						1" Ice	6.65	14.13	0.36
						2" Ice			
(2) LPA-80080/6CF w/ Mount Pipe	B	From Face	4.00 0.00 2.00	0.0000	168.00	No Ice	4.56	10.26	0.05
						1/2"	5.11	11.43	0.11
						Ice	5.61	12.31	0.19
						1" Ice	6.65	14.13	0.36
						2" Ice			
BXA-70063-6CF-2 w/ Mount Pipe	A	From Face	4.00 -2.67 2.00	0.0000	168.00	No Ice	7.81	5.40	0.04
						1/2"	8.36	6.55	0.10
						Ice	8.87	7.41	0.17
						1" Ice	9.93	9.18	0.33
						2" Ice			
BXA-70063-6CF-2 w/ Mount Pipe	B	From Face	4.00 -2.67 2.00	0.0000	168.00	No Ice	7.81	5.40	0.04
						1/2"	8.36	6.55	0.10
						Ice	8.87	7.41	0.17
						1" Ice	9.93	9.18	0.33
						2" Ice			
BXA-70063-6CF-2 w/ Mount Pipe	C	From Face	4.00 -2.67 2.00	0.0000	168.00	No Ice	7.81	5.40	0.04
						1/2"	8.36	6.55	0.10
						Ice	8.87	7.41	0.17
						1" Ice	9.93	9.18	0.33
						2" Ice			
BXA-171085-12BF-2 w/ Mount Pipe	A	From Face	4.00 2.67	0.0000	168.00	No Ice	4.97	5.23	0.04
						1/2"	5.52	6.39	0.09

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment t °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
			2.00			Ice 6.04	7.26	0.14
						1" Ice 7.09	9.05	0.27
						2" Ice		
BXA-171085-12BF-2 w/ Mount Pipe	B	From Face	4.00 2.67 2.00	0.0000	168.00	No Ice 4.97	5.23	0.04
						1/2" 5.52	6.39	0.09
						Ice 6.04	7.26	0.14
						1" Ice 7.09	9.05	0.27
						2" Ice		
BXA-171085-12BF-2 w/ Mount Pipe	C	From Face	4.00 2.67 2.00	0.0000	168.00	No Ice 4.97	5.23	0.04
						1/2" 5.52	6.39	0.09
						Ice 6.04	7.26	0.14
						1" Ice 7.09	9.05	0.27
						2" Ice		
(2) FD9R6004/2C-3L	A	From Face	4.00 0.00 2.00	0.0000	168.00	No Ice 0.31	0.08	0.00
						1/2" 0.39	0.12	0.01
						Ice 0.47	0.17	0.01
						1" Ice 0.65	0.29	0.02
						2" Ice		
(2) FD9R6004/2C-3L	B	From Face	4.00 0.00 2.00	0.0000	168.00	No Ice 0.31	0.08	0.00
						1/2" 0.39	0.12	0.01
						Ice 0.47	0.17	0.01
						1" Ice 0.65	0.29	0.02
						2" Ice		
(2) FD9R6004/2C-3L	C	From Face	4.00 0.00 2.00	0.0000	168.00	No Ice 0.31	0.08	0.00
						1/2" 0.39	0.12	0.01
						Ice 0.47	0.17	0.01
						1" Ice 0.65	0.29	0.02
						2" Ice		
*** Platform Mount [LP 303- 1_HR-1]	C	None		0.0000	156.00	No Ice 17.09	17.09	1.50
						1/2" 21.47	21.47	1.88
						Ice 25.72	25.72	2.35
						1" Ice 33.96	33.96	3.52
						2" Ice		
6'x2" Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	156.00	No Ice 1.43	1.43	0.02
						1/2" 1.92	1.92	0.03
						Ice 2.29	2.29	0.05
						1" Ice 3.06	3.06	0.09
						2" Ice		
6'x2" Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	156.00	No Ice 1.43	1.43	0.02
						1/2" 1.92	1.92	0.03
						Ice 2.29	2.29	0.05
						1" Ice 3.06	3.06	0.09
						2" Ice		
6'x2" Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	156.00	No Ice 1.43	1.43	0.02
						1/2" 1.92	1.92	0.03
						Ice 2.29	2.29	0.05
						1" Ice 3.06	3.06	0.09
						2" Ice		
7770.00 w/ Mount Pipe	A	From Leg	4.00 -6.00 2.00	0.0000	156.00	No Ice 5.75	4.25	0.06
						1/2" 6.18	5.01	0.10
						Ice 6.61	5.71	0.16
						1" Ice 7.49	7.16	0.29
						2" Ice		
7770.00 w/ Mount Pipe	B	From Leg	4.00 6.00 2.00	0.0000	156.00	No Ice 5.75	4.25	0.06
						1/2" 6.18	5.01	0.10
						Ice 6.61	5.71	0.16
						1" Ice 7.49	7.16	0.29
						2" Ice		
7770.00 w/ Mount Pipe	C	From Leg	4.00 6.00 2.00	0.0000	156.00	No Ice 5.75	4.25	0.06
						1/2" 6.18	5.01	0.10
						Ice 6.61	5.71	0.16
						1" Ice 7.49	7.16	0.29
						2" Ice		
HPA65R-BU6A w/ Mount	A	From Leg	4.00	0.0000	156.00	No Ice 8.09	7.19	0.07

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment t °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K	
Pipe			0.00 2.00			1/2" Ice 1" Ice 2" Ice	8.64 9.16 10.22 11.05	0.14 0.21 0.39	
SBNHH-1D65A w/ Mount Pipe	B	From Leg	4.00 -6.00 2.00	0.0000	156.00	No Ice 1/2" Ice 1" Ice 2" Ice	3.04 3.34 3.65 4.31	2.45 2.75 3.05 3.68	0.05 0.10 0.16 0.31
SBNHH-1D65A w/ Mount Pipe	C	From Leg	4.00 -6.00 2.00	0.0000	156.00	No Ice 1/2" Ice 1" Ice 2" Ice	3.04 3.34 3.65 4.31	2.45 2.75 3.05 3.68	0.05 0.10 0.16 0.31
80010964 w/ Mount Pipe	A	From Leg	4.00 6.00 2.00	0.0000	156.00	No Ice 1/2" Ice 1" Ice 2" Ice	8.61 9.18 9.77 10.98	4.10 4.59 5.10 6.16	0.12 0.19 0.26 0.45
80010964 w/ Mount Pipe	B	From Leg	4.00 0.00 2.00	0.0000	156.00	No Ice 1/2" Ice 1" Ice 2" Ice	8.61 9.18 9.77 10.98	4.10 4.59 5.10 6.16	0.12 0.19 0.26 0.45
80010964 w/ Mount Pipe	C	From Leg	4.00 0.00 2.00	0.0000	156.00	No Ice 1/2" Ice 1" Ice 2" Ice	8.61 9.18 9.77 10.98	4.10 4.59 5.10 6.16	0.12 0.19 0.26 0.45
(2) LGP21401	A	From Leg	4.00 0.00 2.00	0.0000	156.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.10 1.24 1.38 1.69	0.35 0.44 0.54 0.77	0.01 0.02 0.03 0.05
(2) LGP21401	B	From Leg	4.00 0.00 2.00	0.0000	156.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.10 1.24 1.38 1.69	0.35 0.44 0.54 0.77	0.01 0.02 0.03 0.05
(2) LGP21401	C	From Leg	4.00 0.00 2.00	0.0000	156.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.10 1.24 1.38 1.69	0.35 0.44 0.54 0.77	0.01 0.02 0.03 0.05
RRUS 4449 B5/B12	A	From Leg	4.00 0.00 2.00	0.0000	156.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.97 2.14 2.33 2.72	1.41 1.56 1.73 2.07	0.07 0.09 0.11 0.16
RRUS 4449 B5/B12	B	From Leg	4.00 0.00 2.00	0.0000	156.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.97 2.14 2.33 2.72	1.41 1.56 1.73 2.07	0.07 0.09 0.11 0.16
RRUS 4449 B5/B12	C	From Leg	4.00 0.00 2.00	0.0000	156.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.97 2.14 2.33 2.72	1.41 1.56 1.73 2.07	0.07 0.09 0.11 0.16
RRUS 8843 B2/B66A	A	From Leg	4.00 0.00 2.00	0.0000	156.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.64 1.80 1.97 2.32	1.35 1.50 1.65 1.99	0.07 0.09 0.11 0.16
RRUS 8843 B2/B66A	B	From Leg	4.00	0.0000	156.00	No Ice	1.64	1.35	0.07

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment t °	Placement ft	C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K	
			0.00			1/2"	1.80	1.50	0.09
			2.00			Ice	1.97	1.65	0.11
						1" Ice	2.32	1.99	0.16
						2" Ice			
RRUS 8843 B2/B66A	C	From Leg	4.00	0.0000	156.00	No Ice	1.64	1.35	0.07
			0.00			1/2"	1.80	1.50	0.09
			2.00			Ice	1.97	1.65	0.11
						1" Ice	2.32	1.99	0.16
						2" Ice			
DC6-48-60-18-8C-EV	A	From Leg	1.00	0.0000	156.00	No Ice	2.74	2.74	0.03
			0.00			1/2"	2.96	2.96	0.05
			2.00			Ice	3.20	3.20	0.08
						1" Ice	3.68	3.68	0.15
						2" Ice			
DC6-48-60-18-8F	C	From Leg	1.00	0.0000	156.00	No Ice	0.92	0.92	0.02
			0.00			1/2"	1.46	1.46	0.04
			2.00			Ice	1.64	1.64	0.06
						1" Ice	2.04	2.04	0.11
						2" Ice			

4'x2" Horizontal Mount Pipe	C	From Face	0.00	0.0000	75.00	No Ice	0.87	0.01	0.01
			0.00			1/2"	1.11	0.04	0.02
			0.00			Ice	1.36	0.07	0.03
						1" Ice	1.90	0.13	0.06
						2" Ice			
KS24019-L112A	C	From Face	4.00	0.0000	75.00	No Ice	0.14	0.14	0.01
			0.00			1/2"	0.20	0.20	0.01
			1.00			Ice	0.26	0.26	0.01
						1" Ice	0.41	0.41	0.02
						2" Ice			

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice

Comb. No.	Description
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	178 - 129.87	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-29.58	-3.79	5.37
			Max. Mx	8	-13.45	-558.58	-7.35
			Max. My	2	-13.30	7.29	585.71
			Max. Vy	8	16.92	-558.58	-7.35
			Max. Vx	2	-17.68	7.29	585.71
			Max. Torque	11			8.67
L2	129.87 - 84.83	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-41.64	-4.29	5.88
			Max. Mx	8	-22.79	-1391.57	-16.98
			Max. My	2	-22.70	17.05	1452.34
			Max. Vy	8	21.01	-1391.57	-16.98
			Max. Vx	2	-21.77	17.05	1452.34
			Max. Torque	11			8.64
L3	84.83 - 41.2833	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-57.75	-4.64	5.82
			Max. Mx	8	-35.71	-2372.00	-26.40
			Max. My	2	-35.66	26.44	2465.88
			Max. Vy	8	25.09	-2372.00	-26.40
			Max. Vx	2	-25.87	26.44	2465.88
			Max. Torque	11			8.60
L4	41.2833 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-80.78	-4.85	5.67
			Max. Mx	8	-55.03	-3667.10	-36.66
			Max. My	2	-55.03	36.67	3797.66
			Max. Vy	8	28.95	-3667.10	-36.66
			Max. Vx	2	-29.70	36.67	3797.66
			Max. Torque	11			8.54

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	27	80.78	0.04	8.32
	Max. H _x	20	55.05	28.92	0.21
	Max. H _z	2	55.05	0.21	29.67
	Max. M _x	2	3797.66	0.21	29.67
	Max. M _z	8	3667.10	-28.92	-0.21
	Max. Torsion	11	8.54	-25.15	-15.01
	Min. Vert	11	41.29	-25.15	-15.01
	Min. H _x	8	55.05	-28.92	-0.21
	Min. H _z	14	55.05	-0.21	-29.67
	Min. M _x	14	-3795.50	-0.21	-29.67
	Min. M _z	20	-3665.14	28.92	0.21
	Min. Torsion	23	-8.44	25.15	15.01

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead Only	45.88	0.00	0.00	-0.84	-0.77	-0.00
1.2 Dead+1.0 Wind 0 deg - No Ice	55.05	-0.21	-29.67	-3797.66	36.67	3.44
0.9 Dead+1.0 Wind 0 deg - No Ice	41.29	-0.21	-29.67	-3741.59	36.29	3.45
1.2 Dead+1.0 Wind 30 deg - No Ice	55.05	14.28	-25.59	-3270.51	-1801.33	-0.89
0.9 Dead+1.0 Wind 30 deg - No Ice	41.29	14.28	-25.59	-3222.21	-1774.79	-0.89
1.2 Dead+1.0 Wind 60 deg - No Ice	55.05	24.94	-14.65	-1867.08	-3157.18	-5.03
0.9 Dead+1.0 Wind 60 deg - No Ice	41.29	24.94	-14.65	-1839.44	-3110.75	-5.04
1.2 Dead+1.0 Wind 90 deg - No Ice	55.05	28.92	0.21	36.66	-3667.10	-7.84
0.9 Dead+1.0 Wind 90 deg - No Ice	41.29	28.92	0.21	36.29	-3613.18	-7.85
1.2 Dead+1.0 Wind 120 deg - No Ice	55.05	25.15	15.01	1930.08	-3194.36	-8.52
0.9 Dead+1.0 Wind 120 deg - No Ice	41.29	25.15	15.01	1901.89	-3147.36	-8.54
1.2 Dead+1.0 Wind 150 deg - No Ice	55.05	14.64	25.80	3305.72	-1866.15	-6.88
0.9 Dead+1.0 Wind 150 deg - No Ice	41.29	14.64	25.80	3257.37	-1838.57	-6.90
1.2 Dead+1.0 Wind 180 deg - No Ice	55.05	0.21	29.67	3795.50	-38.51	-3.39
0.9 Dead+1.0 Wind 180 deg - No Ice	41.29	0.21	29.67	3740.02	-37.63	-3.40
1.2 Dead+1.0 Wind 210 deg - No Ice	55.05	-14.28	25.59	3268.38	1799.33	0.99
0.9 Dead+1.0 Wind 210 deg - No Ice	41.29	-14.28	25.59	3220.65	1773.34	0.99
1.2 Dead+1.0 Wind 240 deg - No Ice	55.05	-24.94	14.65	1865.08	3155.12	5.08
0.9 Dead+1.0 Wind 240 deg - No Ice	41.29	-24.94	14.65	1837.98	3109.25	5.09
1.2 Dead+1.0 Wind 270 deg - No Ice	55.05	-28.92	-0.21	-38.54	3665.14	7.79
0.9 Dead+1.0 Wind 270 deg - No Ice	41.29	-28.92	-0.21	-37.65	3611.75	7.81
1.2 Dead+1.0 Wind 300 deg - No Ice	55.05	-25.15	-15.01	-1931.97	3192.56	8.42
0.9 Dead+1.0 Wind 300 deg - No Ice	41.29	-25.15	-15.01	-1903.27	3146.04	8.44

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
- No Ice						
1.2 Dead+1.0 Wind 330 deg	55.05	-14.64	-25.80	-3307.75	1864.41	6.83
- No Ice						
0.9 Dead+1.0 Wind 330 deg	41.29	-14.64	-25.80	-3258.85	1837.30	6.85
- No Ice						
1.2 Dead+1.0 Ice+1.0 Temp	80.78	0.00	-0.00	-5.67	-4.85	0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	80.78	-0.04	-8.32	-1086.13	3.30	0.73
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	80.78	4.05	-7.18	-937.31	-523.94	-0.14
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	80.78	7.05	-4.12	-538.84	-912.12	-0.98
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	80.78	8.17	0.04	2.47	-1057.18	-1.55
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	80.78	7.09	4.20	541.55	-920.35	-1.71
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	80.78	4.12	7.22	933.97	-538.21	-1.40
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	80.78	0.04	8.32	1074.55	-13.19	-0.72
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	80.78	-4.05	7.18	925.73	514.03	0.15
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	80.78	-7.05	4.12	527.27	902.21	0.98
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	80.78	-8.17	-0.04	-14.03	1047.28	1.55
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	80.78	-7.09	-4.20	-553.12	910.46	1.70
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	80.78	-4.12	-7.22	-945.55	528.33	1.40
Dead+Wind 0 deg - Service	45.88	-0.05	-6.99	-887.88	7.96	0.82
Dead+Wind 30 deg - Service	45.88	3.36	-6.02	-764.66	-421.41	-0.22
Dead+Wind 60 deg - Service	45.88	5.87	-3.45	-436.79	-738.08	-1.21
Dead+Wind 90 deg - Service	45.88	6.81	0.05	7.88	-857.19	-1.88
Dead+Wind 120 deg - Service	45.88	5.92	3.53	450.20	-746.84	-2.04
Dead+Wind 150 deg - Service	45.88	3.45	6.07	771.64	-436.60	-1.65
Dead+Wind 180 deg - Service	45.88	0.05	6.99	886.08	-9.59	-0.82
Dead+Wind 210 deg - Service	45.88	-3.36	6.02	762.86	419.77	0.23
Dead+Wind 240 deg - Service	45.88	-5.87	3.45	435.00	736.44	1.21
Dead+Wind 270 deg - Service	45.88	-6.81	-0.05	-9.67	855.56	1.87
Dead+Wind 300 deg - Service	45.88	-5.92	-3.53	-451.99	745.22	2.03
Dead+Wind 330 deg - Service	45.88	-3.45	-6.07	-773.43	434.97	1.65

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-45.88	0.00	0.00	45.88	0.00	0.000%
2	-0.21	-55.05	-29.67	0.21	55.05	29.67	0.000%
3	-0.21	-41.29	-29.67	0.21	41.29	29.67	0.000%
4	14.28	-55.05	-25.59	-14.28	55.05	25.59	0.000%
5	14.28	-41.29	-25.59	-14.28	41.29	25.59	0.000%
6	24.94	-55.05	-14.65	-24.94	55.05	14.65	0.000%
7	24.94	-41.29	-14.65	-24.94	41.29	14.65	0.000%
8	28.92	-55.05	0.21	-28.92	55.05	-0.21	0.000%
9	28.92	-41.29	0.21	-28.92	41.29	-0.21	0.000%
10	25.15	-55.05	15.01	-25.15	55.05	-15.01	0.000%

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
11	25.15	-41.29	15.01	-25.15	41.29	-15.01	0.000%
12	14.64	-55.05	25.80	-14.64	55.05	-25.80	0.000%
13	14.64	-41.29	25.80	-14.64	41.29	-25.80	0.000%
14	0.21	-55.05	29.67	-0.21	55.05	-29.67	0.000%
15	0.21	-41.29	29.67	-0.21	41.29	-29.67	0.000%
16	-14.28	-55.05	25.59	14.28	55.05	-25.59	0.000%
17	-14.28	-41.29	25.59	14.28	41.29	-25.59	0.000%
18	-24.94	-55.05	14.65	24.94	55.05	-14.65	0.000%
19	-24.94	-41.29	14.65	24.94	41.29	-14.65	0.000%
20	-28.92	-55.05	-0.21	28.92	55.05	0.21	0.000%
21	-28.92	-41.29	-0.21	28.92	41.29	0.21	0.000%
22	-25.15	-55.05	-15.01	25.15	55.05	15.01	0.000%
23	-25.15	-41.29	-15.01	25.15	41.29	15.01	0.000%
24	-14.64	-55.05	-25.80	14.64	55.05	25.80	0.000%
25	-14.64	-41.29	-25.80	14.64	41.29	25.80	0.000%
26	0.00	-80.78	0.00	-0.00	80.78	0.00	0.000%
27	-0.04	-80.78	-8.32	0.04	80.78	8.32	0.000%
28	4.05	-80.78	-7.18	-4.05	80.78	7.18	0.000%
29	7.05	-80.78	-4.12	-7.05	80.78	4.12	0.000%
30	8.17	-80.78	0.04	-8.17	80.78	-0.04	0.000%
31	7.09	-80.78	4.20	-7.09	80.78	-4.20	0.000%
32	4.12	-80.78	7.22	-4.12	80.78	-7.22	0.000%
33	0.04	-80.78	8.32	-0.04	80.78	-8.32	0.000%
34	-4.05	-80.78	7.18	4.05	80.78	-7.18	0.000%
35	-7.05	-80.78	4.12	7.05	80.78	-4.12	0.000%
36	-8.17	-80.78	-0.04	8.17	80.78	0.04	0.000%
37	-7.09	-80.78	-4.20	7.09	80.78	4.20	0.000%
38	-4.12	-80.78	-7.22	4.12	80.78	7.22	0.000%
39	-0.05	-45.88	-6.99	0.05	45.88	6.99	0.000%
40	3.36	-45.88	-6.02	-3.36	45.88	6.02	0.000%
41	5.87	-45.88	-3.45	-5.87	45.88	3.45	0.000%
42	6.81	-45.88	0.05	-6.81	45.88	-0.05	0.000%
43	5.92	-45.88	3.53	-5.92	45.88	-3.53	0.000%
44	3.45	-45.88	6.07	-3.45	45.88	-6.07	0.000%
45	0.05	-45.88	6.99	-0.05	45.88	-6.99	0.000%
46	-3.36	-45.88	6.02	3.36	45.88	-6.02	0.000%
47	-5.87	-45.88	3.45	5.87	45.88	-3.45	0.000%
48	-6.81	-45.88	-0.05	6.81	45.88	0.05	0.000%
49	-5.92	-45.88	-3.53	5.92	45.88	3.53	0.000%
50	-3.45	-45.88	-6.07	3.45	45.88	6.07	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	5	0.00000001	0.00018034
3	Yes	5	0.00000001	0.00008731
4	Yes	6	0.00000001	0.00019316
5	Yes	6	0.00000001	0.00006154
6	Yes	6	0.00000001	0.00021029
7	Yes	6	0.00000001	0.00006825
8	Yes	5	0.00000001	0.00050704
9	Yes	5	0.00000001	0.00024580
10	Yes	6	0.00000001	0.00018447
11	Yes	6	0.00000001	0.00005767
12	Yes	6	0.00000001	0.00022901
13	Yes	6	0.00000001	0.00007368
14	Yes	5	0.00000001	0.00035041
15	Yes	5	0.00000001	0.00016502
16	Yes	6	0.00000001	0.00019864
17	Yes	6	0.00000001	0.00006368
18	Yes	6	0.00000001	0.00018097
19	Yes	6	0.00000001	0.00005748
20	Yes	5	0.00000001	0.00066915

21	Yes	5	0.0000001	0.00032068
22	Yes	6	0.0000001	0.00023055
23	Yes	6	0.0000001	0.00007469
24	Yes	6	0.0000001	0.00018601
25	Yes	6	0.0000001	0.00005789
26	Yes	4	0.0000001	0.00010023
27	Yes	5	0.0000001	0.00078867
28	Yes	6	0.0000001	0.00015803
29	Yes	6	0.0000001	0.00016076
30	Yes	5	0.0000001	0.00077901
31	Yes	6	0.0000001	0.00015362
32	Yes	6	0.0000001	0.00016452
33	Yes	5	0.0000001	0.00076940
34	Yes	6	0.0000001	0.00015015
35	Yes	6	0.0000001	0.00014609
36	Yes	5	0.0000001	0.00076702
37	Yes	6	0.0000001	0.00016534
38	Yes	6	0.0000001	0.00015606
39	Yes	4	0.0000001	0.00025014
40	Yes	4	0.0000001	0.00066242
41	Yes	4	0.0000001	0.00088327
42	Yes	4	0.0000001	0.00051923
43	Yes	4	0.0000001	0.00070213
44	Yes	5	0.0000001	0.00006755
45	Yes	4	0.0000001	0.00028103
46	Yes	4	0.0000001	0.00071125
47	Yes	4	0.0000001	0.00059362
48	Yes	4	0.0000001	0.00054881
49	Yes	5	0.0000001	0.00007051
50	Yes	4	0.0000001	0.00064988

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	178 - 129.87	32.718	39	1.8012	0.0256
L2	134.12 - 84.83	17.615	39	1.3603	0.0102
L3	90.1633 - 41.2833	7.459	39	0.8228	0.0040
L4	47.7 - 0	2.004	39	0.3880	0.0014

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
178.00	Site Pro 1 RMQP-496-HK + PRK-1245 + HRK12 12.5' Platform with Handrails and Kickers	39	32.718	1.8012	0.0258	31650
168.00	Platform Mount [16' LP 403-1]	39	29.045	1.7073	0.0218	15825
156.00	Platform Mount [LP 303-1_HR-1]	39	24.744	1.5916	0.0172	7192
75.00	4"x2" Horizontal Mount Pipe	39	5.047	0.6536	0.0029	5413

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	178 - 129.87	139.810	2	7.6973	0.1063
L2	134.12 - 84.83	75.374	2	5.8242	0.0419
L3	90.1633 - 41.2833	31.932	2	3.5243	0.0163
L4	47.7 - 0	8.577	2	1.6610	0.0058

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
178.00	Site Pro 1 RMQP-496-HK + PRK-1245 + HRK12 12.5' Platform with Handrails and Kickers	2	139.810	7.6973	0.1095	7653
168.00	Platform Mount [16' LP 403-1]	2	124.147	7.2992	0.0925	3825
156.00	Platform Mount [LP 303-1_HR-1]	2	105.804	6.8082	0.0731	1735
75.00	4'x2" Horizontal Mount Pipe	2	21.605	2.7992	0.0121	1268

Compression Checks

Pole Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	KI/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
L1	178 - 129.87 (1)	TP29.64x19.5x0.25	48.13	0.00	0.0	22.610 5	-13.30	1322.71	0.010
L2	129.87 - 84.83 (2)	TP38.5x28.2446x0.375	49.29	0.00	0.0	44.057 5	-22.70	2577.36	0.009
L3	84.83 - 41.2833 (3)	TP46.8x36.6403x0.4375	48.88	0.00	0.0	62.528 1	-35.66	3657.89	0.010
L4	41.2833 - 0 (4)	TP54.5x44.5913x0.5	47.70	0.00	0.0	85.698 0	-55.03	5013.33	0.011

Pole Bending Design Data

Section No.	Elevation ft	Size	M _{ux} kip-ft	φM _{nx} kip-ft	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	M _{uy} kip-ft	φM _{ny} kip-ft	Ratio $\frac{M_{uy}}{\phi M_{ny}}$
L1	178 - 129.87 (1)	TP29.64x19.5x0.25	585.75	936.75	0.625	0.00	936.75	0.000
L2	129.87 - 84.83 (2)	TP38.5x28.2446x0.375	1452.44	2463.13	0.590	0.00	2463.13	0.000
L3	84.83 - 41.2833 (3)	TP46.8x36.6403x0.4375	2466.03	4208.93	0.586	0.00	4208.93	0.000
L4	41.2833 - 0 (4)	TP54.5x44.5913x0.5	3797.83	6831.09	0.556	0.00	6831.09	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V_u K	ϕV_n K	Ratio $\frac{V_u}{\phi V_n}$	Actual T_u kip-ft	ϕT_n kip-ft	Ratio $\frac{T_u}{\phi T_n}$
L1	178 - 129.87 (1)	TP29.64x19.5x0.25	17.69	396.81	0.045	3.47	990.22	0.004
L2	129.87 - 84.83 (2)	TP38.5x28.2446x0.375	21.77	773.21	0.028	3.45	2506.45	0.001
L3	84.83 - 41.2833 (3)	TP46.8x36.6403x0.4375	25.87	1097.37	0.024	3.44	4327.36	0.001
L4	41.2833 - 0 (4)	TP54.5x44.5913x0.5	29.70	1504.00	0.020	3.44	7112.49	0.000

Pole Interaction Design Data

Section No.	Elevation ft	Ratio P_u ϕP_n	Ratio M_{ux} ϕM_{nx}	Ratio M_{uy} ϕM_{ny}	Ratio V_u ϕV_n	Ratio T_u ϕT_n	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	178 - 129.87 (1)	0.010	0.625	0.000	0.045	0.004	0.638	1.050	4.8.2
L2	129.87 - 84.83 (2)	0.009	0.590	0.000	0.028	0.001	0.599	1.050	4.8.2
L3	84.83 - 41.2833 (3)	0.010	0.586	0.000	0.024	0.001	0.596	1.050	4.8.2
L4	41.2833 - 0 (4)	0.011	0.556	0.000	0.020	0.000	0.567	1.050	4.8.2

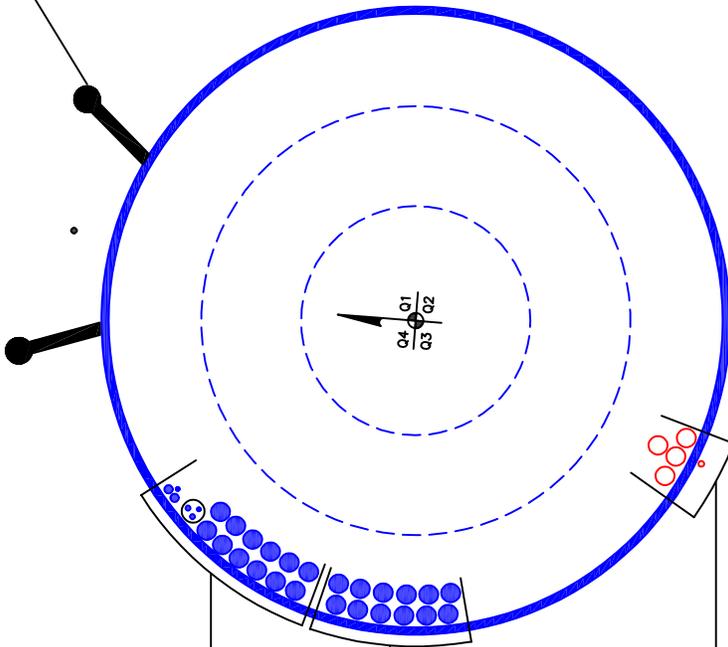
Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail
L1	178 - 129.87	Pole	TP29.64x19.5x0.25	1	-13.30	1388.85	60.7	Pass
L2	129.87 - 84.83	Pole	TP38.5x28.2446x0.375	2	-22.70	2706.23	57.1	Pass
L3	84.83 - 41.2833	Pole	TP46.8x36.6403x0.4375	3	-35.66	3840.78	56.8	Pass
L4	41.2833 - 0	Pole	TP54.5x44.5913x0.5	4	-55.03	5264.00	54.0	Pass
Summary								
Pole (L1)							60.7	Pass
RATING =							60.7	Pass

APPENDIX B
BASE LEVEL DRAWING



CLIMBING PEGS
W/ SAFETY CLIMB



(OTHER CONSIDERED EQUIPMENT - IN CONDUIT)
(1) 3/8" TO 156 FT LEVEL
(2) 7/16" TO 156 FT LEVEL
(OTHER CONSIDERED EQUIPMENT)
(1) 3/8" TO 156 FT LEVEL
(2) 3/4" TO 156 FT LEVEL
(12) 1-5/8" TO 156 FT LEVEL

(OTHER CONSIDERED EQUIPMENT)
(12) 1-5/8" TO 168 FT LEVEL

(PROPOSED EQUIPMENT CONFIGURATION)
(4) 1-5/8" TO 178 FT LEVEL
(1) 1/2" TO 75 FT LEVEL

APPENDIX C
ADDITIONAL CALCULATIONS

Monopole Base Plate Connection

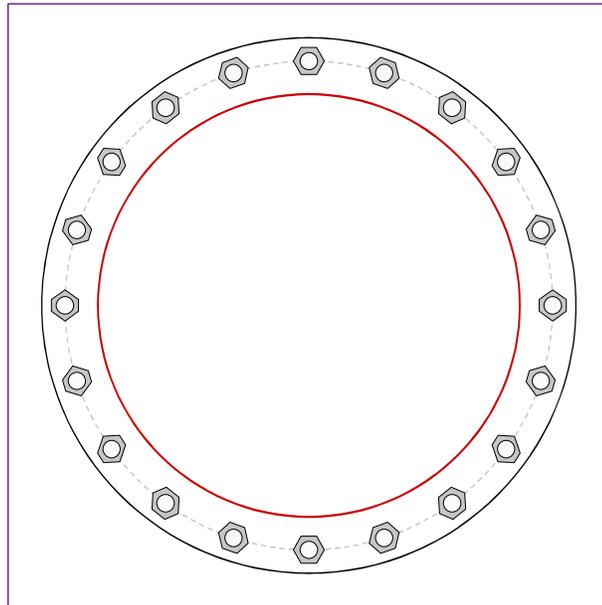


Site Info	
BU #	876369
Site Name	NTON / BUCKLEY BROA
Order #	538773 Rev. 1

Analysis Considerations	
TIA-222 Revision	H
Grout Considered:	No
I_{ar} (in)	1.0625

Applied Loads	
Moment (kip-ft)	3797.84
Axial Force (kips)	55.03
Shear Force (kips)	29.70

*TIA-222-H Section 15.5 Applied



Connection Properties	Analysis Results
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Anchor Rod Data
(20) 2-1/4" ϕ bolts (A615-75 N; $F_y=75$ ksi, $F_u=100$ ksi) on 63" BC
Base Plate Data
69" OD x 2.25" Plate (A871 Gr. 60; $F_y=60$ ksi, $F_u=75$ ksi)
Stiffener Data
N/A
Pole Data
54.5" x 0.5" 18-sided pole (A572-65; $F_y=65$ ksi, $F_u=80$ ksi)

Anchor Rod Summary		<i>(units of kips, kip-in)</i>
$Pu_c = 147.36$	$\phi Pn_c = 268.39$	Stress Rating
$Vu = 1.49$	$\phi Vn = 120.77$	52.3%
$Mu = n/a$	$\phi Mn = n/a$	Pass
Base Plate Summary		
Max Stress (ksi):	34.65	(Flexural)
Allowable Stress (ksi):	54	
Stress Rating:	61.1%	Pass

Pier and Pad Foundation



BU #: 876369
Site Name: HARWINTON / BU
App. Number: 538773 Rev. 1

TIA-222 Revision: H
Tower Type: Monopole

Top & Bot. Pad Rein. Different?:
Block Foundation?:
Rectangular Pad?:

Superstructure Analysis Reactions		
Compression, P_{comp} :	55.05	kips
Base Shear, V_{u_comp} :	29.67	kips
Moment, M_u :	3797.84	ft-kips
Tower Height, H :	178	ft
BP Dist. Above Fdn, bp_{dist} :	5.5	in

Foundation Analysis Checks				
	Capacity	Demand	Rating*	Check
<i>Lateral (Sliding) (kips)</i>	316.01	29.67	8.9%	Pass
<i>Bearing Pressure (ksf)</i>	6.00	1.67	27.8%	Pass
<i>Overturing (kip*ft)</i>	6058.82	4033.96	66.6%	Pass
<i>Pier Flexure (Comp.) (kip*ft)</i>	5906.85	3931.36	63.4%	Pass
<i>Pier Compression (kip)</i>	31187.52	89.24	0.3%	Pass
<i>Pad Flexure (kip*ft)</i>	3722.15	1652.38	42.3%	Pass
<i>Pad Shear - 1-way (kips)</i>	1004.09	224.31	21.3%	Pass
<i>Pad Shear - 2-way (Comp) (ksi)</i>	0.190	0.038	19.2%	Pass
<i>Flexural 2-way (Comp) (kip*ft)</i>	3015.04	2358.81	74.5%	Pass

Pier Properties		
Pier Shape:	Square	
Pier Diameter, $dpier$:	7	ft
Ext. Above Grade, E :	1	ft
Pier Rebar Size, Sc :	8	
Pier Rebar Quantity, mc :	46	
Pier Tie/Spiral Size, St :	4	
Pier Tie/Spiral Quantity, mt :	5	
Pier Reinforcement Type:	Tie	
Pier Clear Cover, cc_{pier} :	3	in

*Rating per TIA-222-H Section 15.5

Soil Rating*:	66.6%
Structural Rating*:	74.5%

Pad Properties		
Depth, D :	6.5	ft
Pad Width, W_1 :	28	ft
Pad Thickness, T :	3	ft
Pad Rebar Size (Top dir. 2), Sp_{top2} :	8	
Pad Rebar Quantity (Top dir. 2), mp_{top2} :	15	
Pad Rebar Size (Bottom dir. 2), Sp_2 :	8	
Pad Rebar Quantity (Bottom dir. 2), mp_2 :	34	
Pad Clear Cover, cc_{pad} :	3	in

Material Properties		
Rebar Grade, F_y :	60	ksi
Concrete Compressive Strength, F'_c :	4	ksi
Dry Concrete Density, δ_c :	150	pcf

Soil Properties		
Total Soil Unit Weight, γ :	125	pcf
Ultimate Gross Bearing, Q_{ult} :	8.000	ksf
Cohesion, C_u :	0.000	ksf
Friction Angle, ϕ :	38	degrees
SPT Blow Count, N_{blows} :		
Base Friction, μ :	0.6	
Neglected Depth, N :	4.00	ft
Foundation Bearing on Rock?	No	
Groundwater Depth, gw :	2	ft

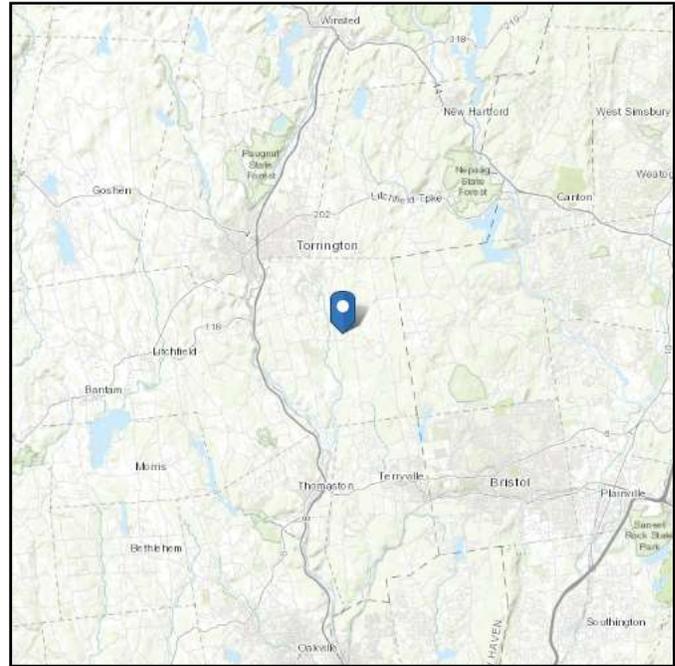
--Toggle between Gross and Net

ASCE 7 Hazards Report

Address:
No Address at This Location

Standard: ASCE/SEI 7-10
Risk Category: II
Soil Class: D - Stiff Soil

Elevation: 840.53 ft (NAVD 88)
Latitude: 41.757264
Longitude: -73.052556



Wind

Results:

Wind Speed:	118 Vmph
10-year MRI	76 Vmph
25-year MRI	85 Vmph
50-year MRI	90 Vmph
100-year MRI	97 Vmph

120 mph per municipality specific structural parameters

Data Source: ASCE/SEI 7-10 Fig. 26.5-1A and Figs. CC-1–CC-4, and Section 26.5.2, incorporating errata of March 12, 2014

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

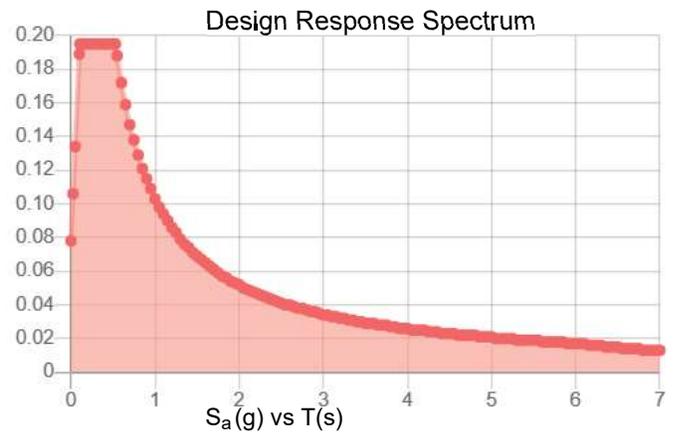
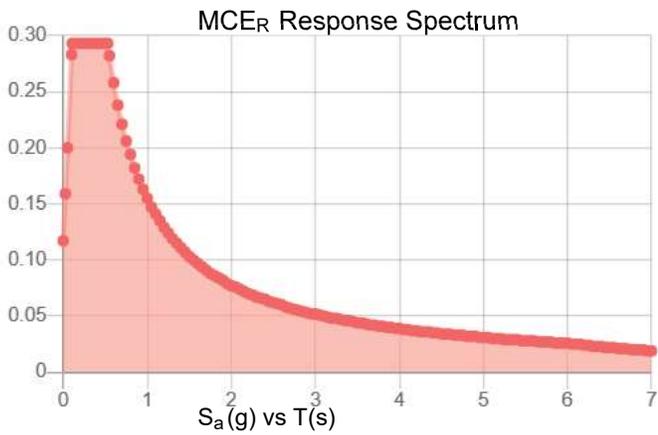
Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.

Site Soil Class: D - Stiff Soil

Results:

S_s :	0.183	S_{DS} :	0.195
S_1 :	0.065	S_{D1} :	0.103
F_a :	1.6	T_L :	6
F_v :	2.4	PGA :	0.093
S_{MS} :	0.293	PGA _M :	0.148
S_{M1} :	0.155	F _{PGA} :	1.6
		I_e :	1

Seismic Design Category B



Data Accessed:

Mon Mar 15 2021

Date Source:

USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-10 Ch. 21 are available from USGS.

Ice

Results:

Ice Thickness: 0.75 in.

Concurrent Temperature: 5 F

Gust Speed: 50 mph

Data Source: Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Mon Mar 15 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided “as is” and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

ASCE does not intend, nor should anyone interpret, the results provided by this Tool to replace the sound judgment of a competent professional, having knowledge and experience in the appropriate field(s) of practice, nor to substitute for the standard of care required of such professionals in interpreting and applying the contents of this Tool or the ASCE 7 standard.

In using this Tool, you expressly assume all risks associated with your use. Under no circumstances shall ASCE or its officers, directors, employees, members, affiliates, or agents be liable to you or any other person for any direct, indirect, special, incidental, or consequential damages arising from or related to your use of, or reliance on, the Tool or any information obtained therein. To the fullest extent permitted by law, you agree to release and hold harmless ASCE from any and all liability of any nature arising out of or resulting from any use of data provided by the ASCE 7 Hazard Tool.

Exhibit E

Mount Analysis

Date: **March 11, 2021**

Darcy Tarr
Crown Castle
6325 Ardrey Kell Road, Suite 600
Charlotte, NC 28277
(704) 405-6589



**GPD Engineering and Architecture
Professional Corporation**
520 South Main Street, Suite 2531
Akron, Ohio 44311
(216) 927-8663
CrownMA@gpdgroup.com

Subject: **Mount Replacement Analysis Report**

Carrier Designation: **Sprint PCS Loading Modification**
Carrier Site Number: CTNH558A
Carrier Site Name: CTNH558A

Crown Castle Designation: **Crown Castle BU Number:** 876369
Crown Castle Site Name: HARWINTON/BUCKLEY BROADCAST
Crown Castle JDE Job Number: 628849
Crown Castle Order Number: 538773 Rev. 1

Engineering Firm Designation: **GPD Report Designation:** 2021777.876369.02

Site Data: **64 Hungerford Lane, Harwinton, Litchfield County, CT 06791**
Latitude 41° 45' 26.15" Longitude -73° 3' 9.20"

Structure Information: **Tower Height & Type:** **178.0 ft Monopole Tower**
Mount Elevation: **178.0 ft**
Mount Type: **12.5 ft Platform Mount**

Dear Darcy Tarr,

GPD is pleased to submit this "**Mount Replacement Analysis Report**" to determine the structural integrity of Sprint PCS's antenna mounting system with the proposed appurtenance and equipment addition on the above mentioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

Platform Mount **Sufficient Capacity – 40.9%***
***See Section 4.1 of this report for the loading and structural modifications required in order for the mount to support the loading listed in Table 1.**

This analysis utilizes an ultimate 3-second gust wind speed of 120 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount analysis prepared by: Matt Steward

Respectfully Submitted by:



Christopher J. Scheks, P.E.
Connecticut #: 0030026

3/11/2021

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Supplemental Drawings

1) INTRODUCTION

This is a Proposed 12.5' Platform Mount designed by Site Pro 1 (Part #: RMQP-496-HK, dated 7/14/2014).

2) ANALYSIS CRITERIA

TIA-222 Revision:	TIA-222-H
Risk Category:	II
Ultimate Wind Speed:	120 mph
Exposure Category:	C
Topographic Factor at Base:	1
Topographic Factor at Mount:	1
Ice Thickness:	1.5 in
Wind Speed with Ice:	50 mph
Live Loading Wind Speed:	30 mph
Man Live Load at Mid/End-Points:	250 lb
Man Live Load at Mount Pipes:	500 lb

Table 1 - Proposed Equipment Configuration

Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount Details
178.0	180.0	3	Ericsson	AIR6449 B41_T-MOBILE	12.5 ft. Platform Mount
		3	RFS/Celwave	APX16DWV-16DWV-S-E-A20	
		3	RFS/Celwave	APXVAALL24_43-U-NA20_TMO	
		3	Ericsson	RADIO 4415 B66A_CCIV3	
		3	Ericsson	RADIO 4424 B25_TMO	
		3	Ericsson	RADIO 4449 B71 B85A_T-MOBILE	

3) ANALYSIS PROCEDURE

Table 2 - Documents Provided

Document	Remarks	Reference	Source
CCI Application	Crown Order Number 538773 Rev. 1	-	CCI
Mount Design	Site Pro 1 Part #: RMQP-496-HK, dated 7/14/2014	-	Site Pro 1
RF Data Sheet	Sprint Retain RFDS #: CTNH558A, dated 1/15/2021	-	CCI

3.1) Analysis Method

RISA-3D Edition (Version 17.0.2), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases.

A tool internally developed by GPD, using Microsoft Excel, was used to calculate wind loading on all appurtenances, dishes, and mount members for various load cases. Selected output from the analysis is included in Appendix B.

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 *Tower Mount Analysis* (Revision C).

3.2) Assumptions

- 1) The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- 2) The configuration of antennas, mounts, and other appurtenances are as specified in Table 1 and the referenced drawings.
- 3) All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- 4) This analysis assumes all information reference in Table 2 is current and correct.
- 5) Steel grades have been assumed as follows, unless noted otherwise:

Angle, Plate	ASTM A36 (GR 36)
HSS (Square)	ASTM 500 (GR B-46)
Pipe	ASTM A53 (GR 35)
Connection Bolts	ASTM A325

This analysis may be affected if any assumptions are not valid or have been made in error. GPD should be notified to determine the effect on the structural integrity of the antenna mounting system.

4) ANALYSIS RESULTS

Table 3 - Mount Component Stresses vs. Capacity (Platform Mount)

Notes	Component	Critical Member	Centerline (ft)	% Capacity	Pass / Fail
1,3	Standoff Arm	M46	178.0	16.9	Pass
	Cross Arm	M47		15.7	Pass
	Grating Angle	M49		39.7	Pass
	Toe Rail	M61		11.0	Pass
	Connection Plate (End)	M96		24.5	Pass
	Connection Plate (Mid)	M55		27.6	Pass
	Pipe Mount	C3		40.9	Pass
	Support Rail	M73		27.8	Pass
	Support Rail Corner	M90		38.9	Pass
	Platform Reinforcement Kicker	M92A		9.4	Pass
	Reinforcement Connection Plate	M91A		8.1	Pass
2,3	Mount to Tower Connection	-	9.5	Pass	
	Tieback to Tower Connection	-	6.5	Pass	

Structure Rating (max from all components) =	40.9%³
---	--------------------------

Notes:

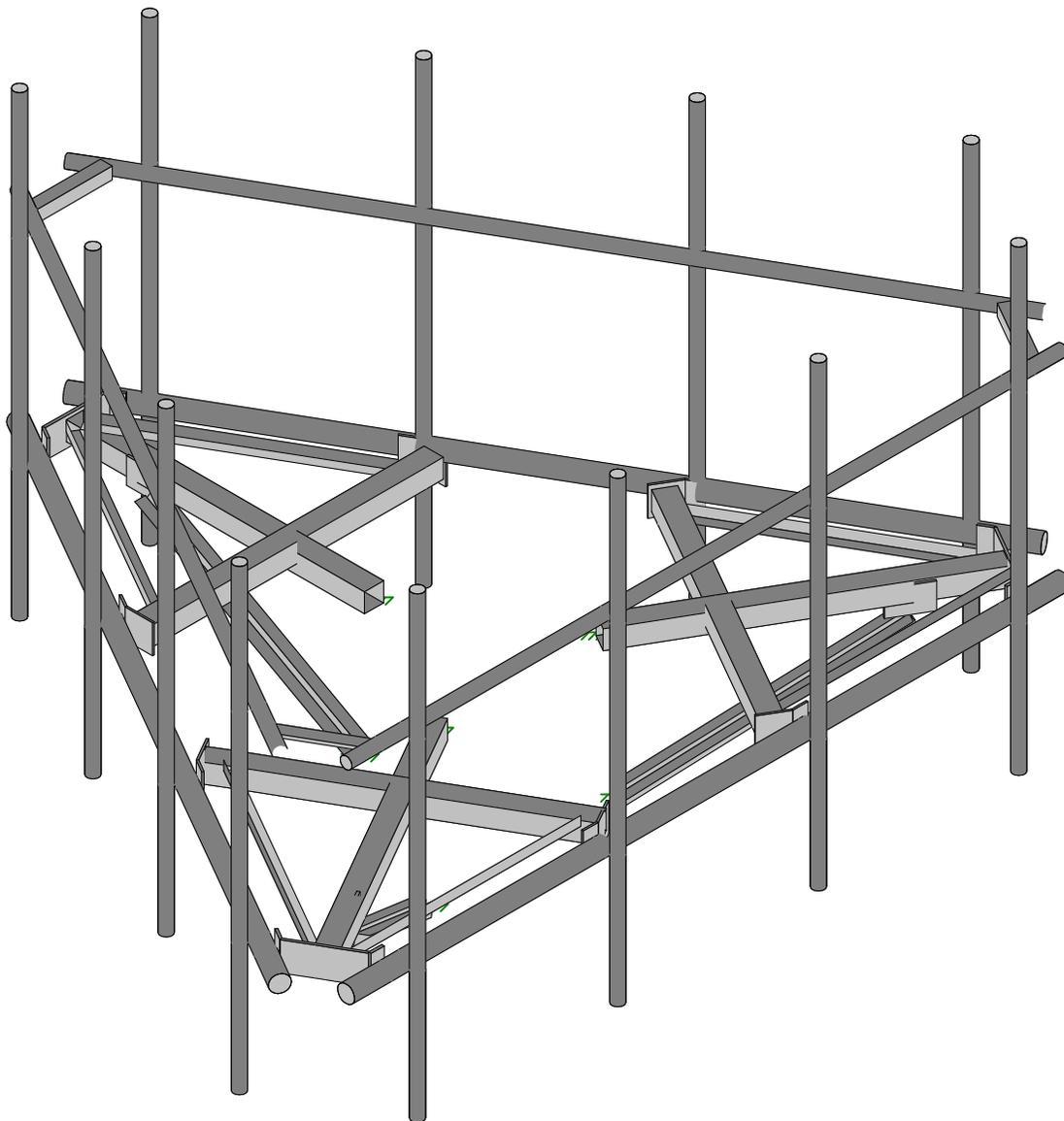
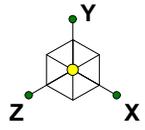
- 1) See additional documentation in "Appendix C - Software Analysis Output" for calculations supporting the % capacity consumed.
- 2) See additional documentation in "Appendix D - Additional Calculations" for calculations supporting the % capacity consumed.
- 3) Ratings per TIA-222-H section 15.5.

4.1) Recommendations

The mount has sufficient capacity to carry the proposed loading configuration. In order for the results of the analysis to be considered valid, the mount listed below shall be installed to support the proposed loading configuration.

Site Pro 1 RMQP-496-HK

APPENDIX A
WIRE FRAME AND RENDERED MODELS

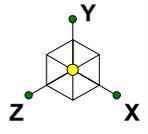


Envelope Only Solution

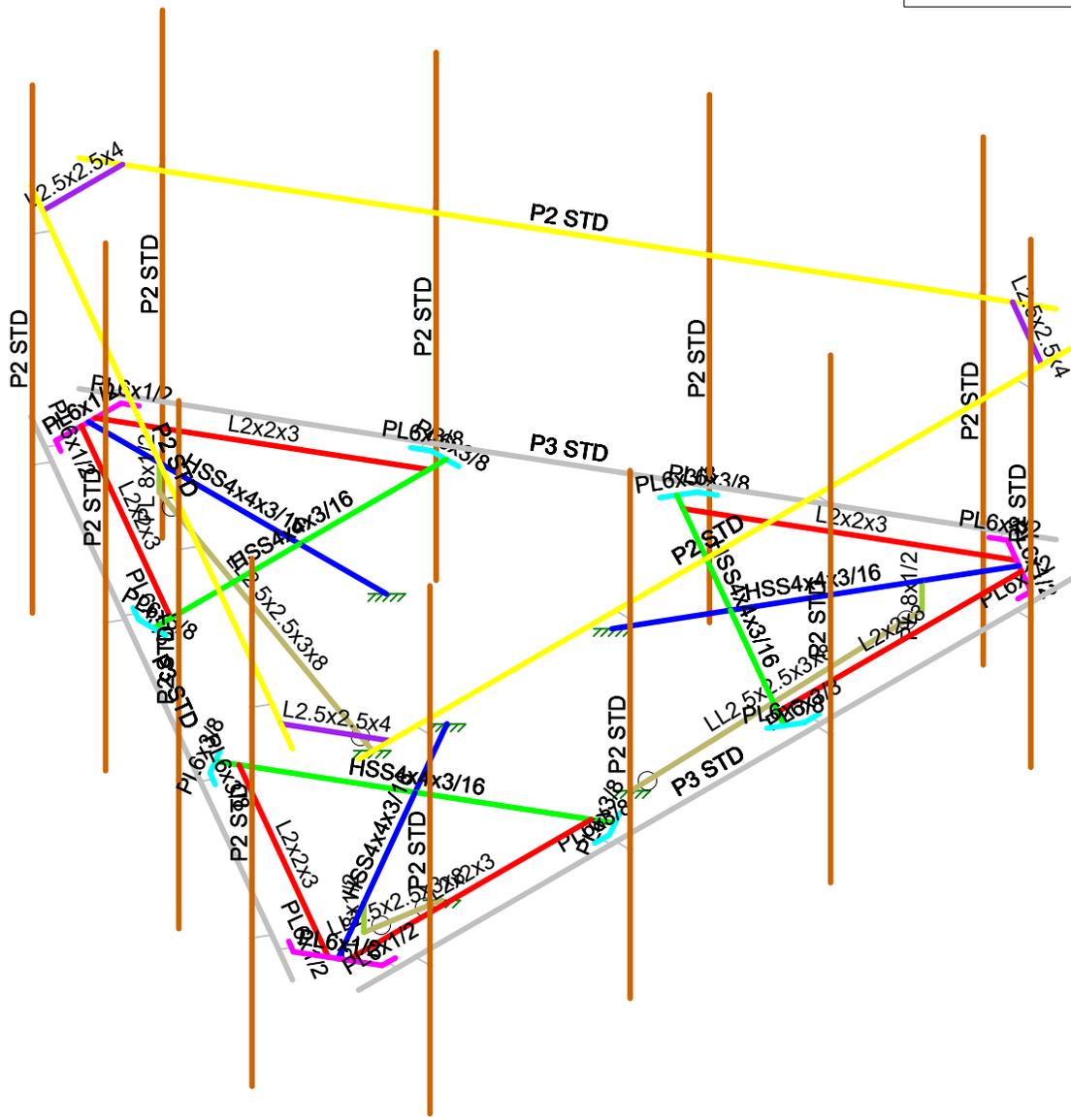
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Steward, Matthew
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SK - 1
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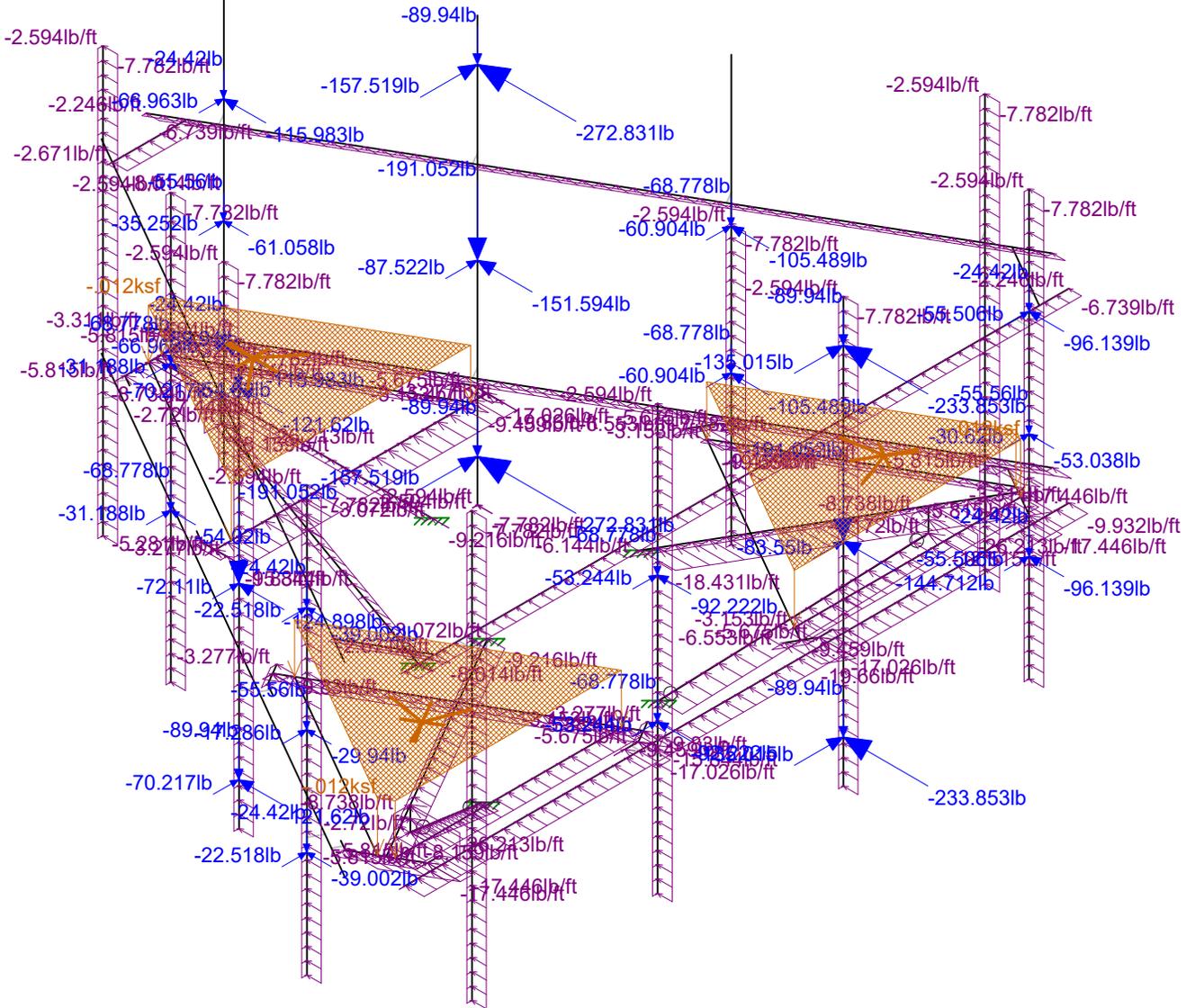
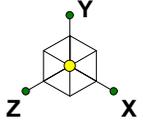


Section Sets	
█	Standoff Arm
█	Cross Arm
█	Grating Angle
█	Toe Rail
█	Connection Plate (End)
█	Connection Plate (Mid)
█	Pipe Mount
█	Support Rail
█	Support Rail Corner
█	Platform Reinforcement Kicker
█	Reinforcement Connection Plate
█	RIGID



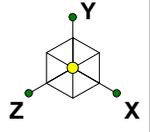
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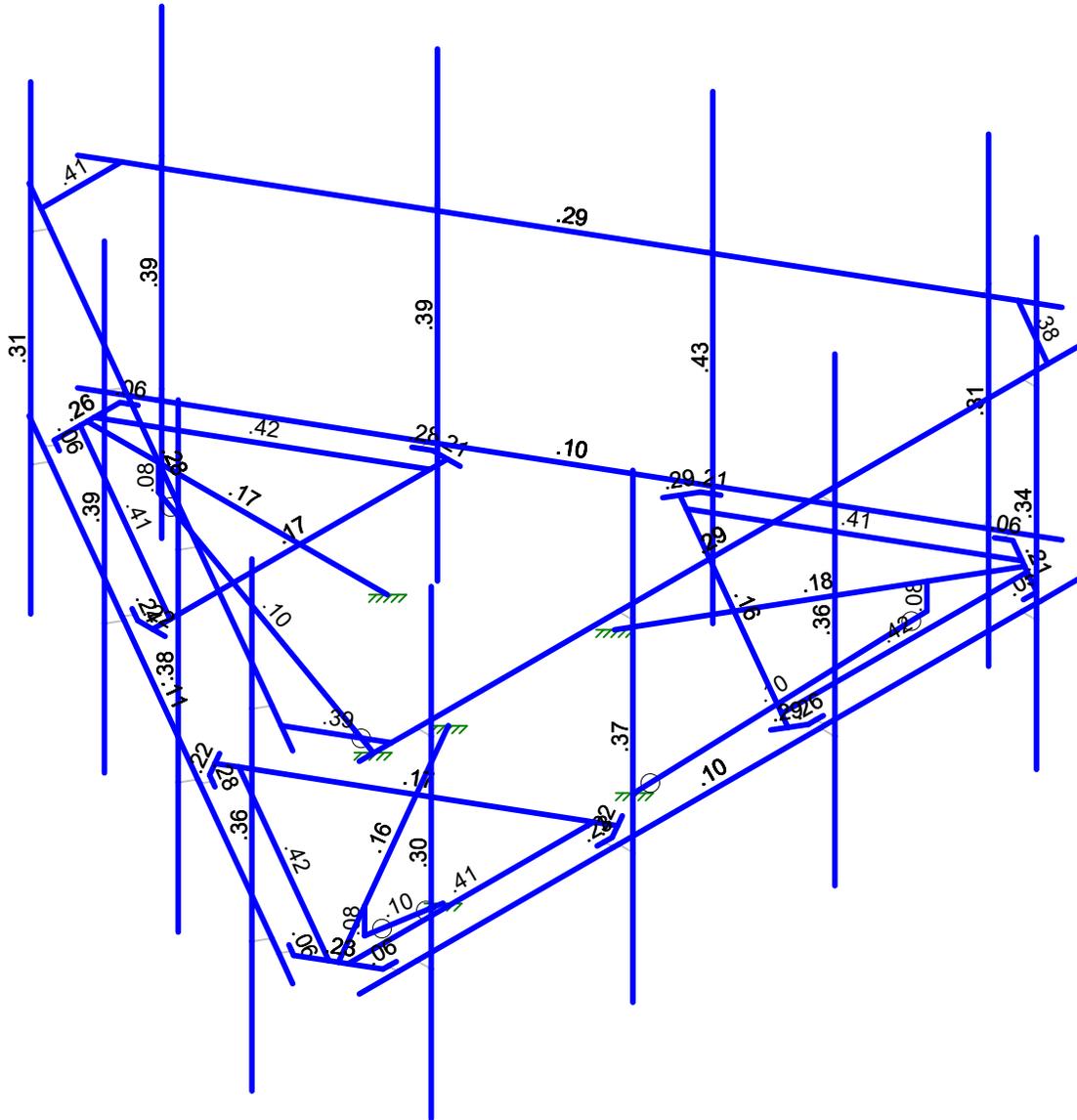


Loads: LC 16, 1.2 Dead + 1.0 Wind @ 210° - No Ice
Envelope Only Solution

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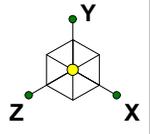


Code Check (Env)	
	No Calc
	> 1.0
	.90-1.0
	.75-.90
	.50-.75
	0.-.50



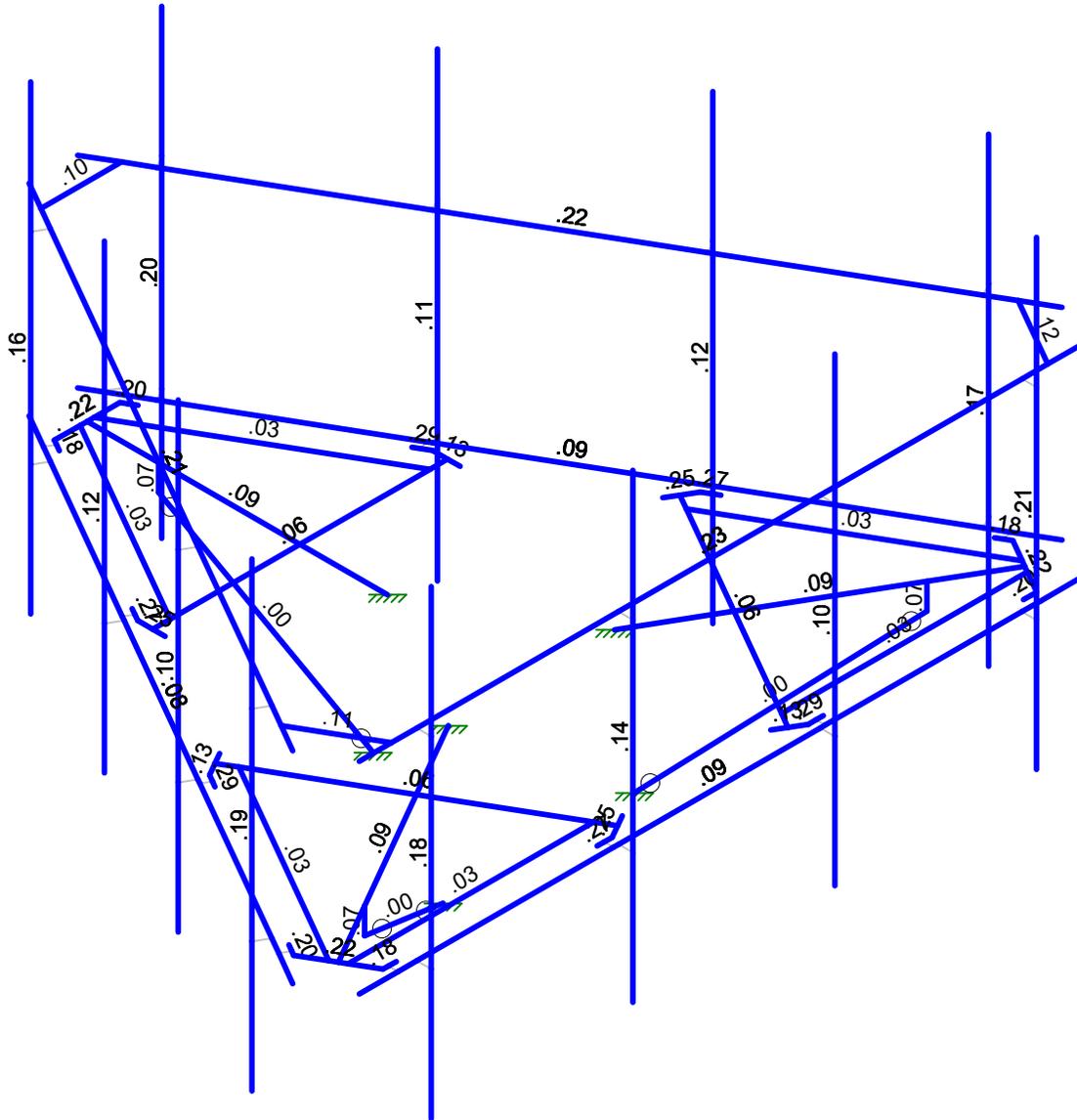
Member Code Checks Displayed (Enveloped)
Envelope Only Solution

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2021777.876369.02		Site Pro 1 - RMQP-496-HK.Loaded...



Shear Check
(Env)

- No Calc
- > 1.0
- .90-1.0
- .75-.90
- .50-.75
- 0.-.50



Member Shear Checks Displayed (Enveloped)
Envelope Only Solution

GPD	876369 - HARWINTON / BUCKLEY BROADCASTI	SK - 5
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APPENDIX B
SOFTWARE INPUT CALCULATIONS



Structure Information	
Structure Type:	Monopole
Structure Height:	178 ft
z (Mount Centerline) =	178 ft
Gh (Mount Gust Effect Factor) =	1.00
Risk Category:	II

Code Specifications	
TIA/EIA Code:	H
Ultimate Wind Speed (No Ice) =	120 mph (3-s gust)
Ultimate Wind Speed (With Ice) =	50 mph (3-s gust)
Ice Thickness	1.5 in
Exposure Category	C
Tower Base Elevation (AMSL)	841 ft

Topographic Inputs	
Topographic Feature:	N/A

Section Sets										No Ice		Ice Output	
Mount Components	Member Type	Length (in)	Side (Longest seeing wind) (in)	Other Side (in)	Calculated Dc, for ice weight (in)	Dc, for ice weight (in)	Area Type (Round or Flat)	K _s	User's Wind Multiplier	Normal Wind Force (lb/ft)*	Normal Ice Wind Force (lb/ft)*	Ice Weight (lb/ft)*	
Standoff Arm	Square/Rect.	62.500	4	4		5.66	Flat	0.90	1.00	27.31	6.12	16.12	
Cross Arm	Square/Rect.	61.000	4	4		5.66	Flat	0.90	1.00	27.10	6.09	16.12	
Grating Angle	Angle	52.000	2	2		2.83	Flat	0.90	1.00	16.18	4.20	9.99	
Toe Rail	Pipe	150.000	3.5	3.5		3.50	Round	0.90	1.00	16.99	5.53	11.44	
Connection Plate (End)	Square/Rect.	19.000	6	0.5		6.02	Flat	0.90	1.00	29.84	6.80	16.91	
Connection Plate (Mid)	Square/Rect.	8.500	6	0.375		6.01	Flat	0.90	1.00	29.13	6.80	16.89	
Pipe Mount	Pipe	96.000	2.375	2.375		2.38	Round	0.90	1.00	11.53	4.18	9.00	
Support Rail	Pipe	150.000	2.375	2.375		2.38	Round	0.90	1.00	11.53	4.99	9.00	
Support Rail Corner	Angle	15.000	2.5	2.5		3.54	Flat	0.90	1.00	13.71	3.85	11.52	
Platform Reinforcement Kicker	Other	50.000	2.5	5.5	5.5	5.50	Flat	0.90	1.00	18.54	4.60	15.78	
Reinforcement Connection Plate	Square/Rect.	4.000	8	0.5		8.02	Flat	0.90	1.00	38.83	8.49	21.24	

*All forces are unfactored.

Appurtenances							Shielding			No Ice		Ice Output	
Appurtenance Model	Loading Elevation (ft)	Height (in)	Front Width (in)	Side Depth (in)	Wt (lbs)	Type for Area	Front Shielding (%)	Side Shielding (%)	K _s and/or block shielding	Normal Wind Force (lbs)*	Wt (lbs) (no ice)*	Normal Wind Force (lbs) (w/ ice)*	Wt (lbs) (only ice)*
(3) AIR6449 B41_T-MOBILE	180	33.11	20.51	8.54	114.63	Flat	0%	0%	0.90	247.82	114.63	50.54	157.89
(3) APX16DWV-16DWV-S-E-A20	180	55.9	13.3	3.15	40.7	CFD	0%	0%	0.90	274.14	40.70	64.14	143.46
(3) APXVAALL24_43-U-NA20_TMO	180	95.9	24	8.5	149.9	CFD	0%	0%	0.90	642.42	149.90	132.67	439.05
(3) RADIO 4415 B66A_CCI3	180	14.9	13.2	5.4	46.3	Flat	0%	0%	0.90	71.77	46.30	16.62	55.31
(3) RADIO 4424 B25_TMO	180	17.1	14.4	11.3	86	Flat	0%	0%	0.90	89.86	86.00	20.21	88.35
(3) RADIO 4449 B71 B85A_T-MOBILE	180	17.91	13.2	10.63	73.21	Flat	0%	0%	0.90	86.27	73.21	19.53	83.88

*All forces are unfactored.

APPENDIX C
SOFTWARE ANALYSIS OUTPUT

Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (1E...	Density[k/ft...	Yield[ksi]	Ry	Fu[ksi]	Rt
1	A992	29000	11154	.3	.65	.49	50	1.1	65	1.1
2	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
3	A572 Gr.50	29000	11154	.3	.65	.49	50	1.1	65	1.1
4	A500 Gr.B RND	29000	11154	.3	.65	.527	42	1.4	58	1.3
5	A500 Gr.B Rect	29000	11154	.3	.65	.527	46	1.4	58	1.3
6	A53 Gr.B	29000	11154	.3	.65	.49	35	1.6	60	1.2
7	A1085	29000	11154	.3	.65	.49	50	1.4	65	1.3

Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design R...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	Standoff Arm	HSS4x4x3/16	None	None	A500 Gr.B Rect	Typical	2.859	6.944	6.944	10.39
2	Cross Arm	HSS4x4x3/16	None	None	A500 Gr.B Rect	Typical	2.859	6.944	6.944	10.39
3	Grating Angle	L2x2x3	None	None	A36 Gr.36	Typical	.722	.271	.271	.009
4	Toe Rail	P3 STD	None	None	A53 Gr.B	Typical	2.228	3.017	3.017	6.034
5	Connection Plate (End)	PL6x1/2	None	None	A36 Gr.36	Typical	3	.063	9	.237
6	Connection Plate (Mid)	PL6x3/8	None	None	A36 Gr.36	Typical	2.25	.026	6.75	.101
7	Pipe Mount	P2 STD	None	None	A53 Gr.B	Typical	1.075	.666	.666	1.331
8	Support Rail	P2 STD	None	None	A53 Gr.B	Typical	1.075	.666	.666	1.331
9	Support Rail Corner	L2.5x2.5x4	None	None	A36 Gr.36	Typical	1.19	.692	.692	.026
10	Platform Reinforcement ...	LL2.5x2.5x3x8	None	None	A36 Gr.36	Typical	1.805	2.703	1.093	.02
11	Reinforcement Connecti...	PL 8x1/2	None	None	A36 Gr.36	Typical	4	.083	21.333	.32

Member Primary Data

	Label	I Joint	J Joint	K Joint	Rotate(deg)	Section/Shape	Type	Design List	Material	Design Rules
1	M46	SA	N88			Standoff Arm	None	None	A500 Gr.B...	Typical
2	M47	N89	N90			Cross Arm	None	None	A500 Gr.B...	Typical
3	M48	N92	N93			Grating Angle	None	None	A36 Gr.36	Typical
4	M49	N94	N95		270	Grating Angle	None	None	A36 Gr.36	Typical
5	M50	N96	N97			Connection Pl...	None	None	A36 Gr.36	Typical
6	M51	N97	N98			Connection Pl...	None	None	A36 Gr.36	Typical
7	M52	N98	N99			Connection Pl...	None	None	A36 Gr.36	Typical
8	M53	N100	N101			Connection Pl...	None	None	A36 Gr.36	Typical
9	M54	N101	N102			Connection Pl...	None	None	A36 Gr.36	Typical
10	M55	N103	N104			Connection Pl...	None	None	A36 Gr.36	Typical
11	M56	N104	N105			Connection Pl...	None	None	A36 Gr.36	Typical
12	M57	N106	N107			RIGID	None	None	RIGID	Typical
13	M58	N108	N109			RIGID	None	None	RIGID	Typical
14	M59	N110	N111			RIGID	None	None	RIGID	Typical
15	M60	N112	N113			RIGID	None	None	RIGID	Typical
16	M16	N29	N30			Toe Rail	None	None	A53 Gr.B	Typical
17	M46A	SB	N93A			Standoff Arm	None	None	A500 Gr.B...	Typical
18	M47A	N94A	N95A			Cross Arm	None	None	A500 Gr.B...	Typical
19	M48A	N97A	N98A			Grating Angle	None	None	A36 Gr.36	Typical
20	M49A	N99A	N100A		270	Grating Angle	None	None	A36 Gr.36	Typical
21	M50A	N101A	N102A			Connection Pl...	None	None	A36 Gr.36	Typical
22	M51A	N102A	N103A			Connection Pl...	None	None	A36 Gr.36	Typical
23	M52A	N103A	N104A			Connection Pl...	None	None	A36 Gr.36	Typical
24	M53A	N105A	N106A			Connection Pl...	None	None	A36 Gr.36	Typical
25	M54A	N106A	N107A			Connection Pl...	None	None	A36 Gr.36	Typical
26	M55A	N108A	N109A			Connection Pl...	None	None	A36 Gr.36	Typical
27	M56A	N109A	N110A			Connection Pl...	None	None	A36 Gr.36	Typical
28	M57A	N111A	N112A			RIGID	None	None	RIGID	Typical
29	M58A	N113A	N114			RIGID	None	None	RIGID	Typical



Company : GPD
 Designer : Steward, Matthew
 Job Number : 2021777.876369.02
 Model Name : 876369 - HARWINTON / BUCKLEY BROADCASTI

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Member Primary Data (Continued)

	Label	I Joint	J Joint	K Joint	Rotate(deg)	Section/Shape	Type	Design List	Material	Design Rules
30	M59A	N115	N116			RIGID	None	None	RIGID	Typical
31	M60A	N117	N118			RIGID	None	None	RIGID	Typical
32	M61	N119	N120			Toe Rail	None	None	A53 Gr.B	Typical
33	M91	SC	N183			Standoff Arm	None	None	A500 Gr.B...	Typical
34	M92	N184	N185			Cross Arm	None	None	A500 Gr.B...	Typical
35	M93	N187	N188			Grating Angle	None	None	A36 Gr.36	Typical
36	M94	N189	N190		270	Grating Angle	None	None	A36 Gr.36	Typical
37	M95	N191	N192			Connection Pl...	None	None	A36 Gr.36	Typical
38	M96	N192	N193			Connection Pl...	None	None	A36 Gr.36	Typical
39	M97	N193	N194			Connection Pl...	None	None	A36 Gr.36	Typical
40	M98	N195	N196			Connection Pl...	None	None	A36 Gr.36	Typical
41	M99	N196	N197			Connection Pl...	None	None	A36 Gr.36	Typical
42	M100	N198	N199			Connection Pl...	None	None	A36 Gr.36	Typical
43	M101	N199	N200			Connection Pl...	None	None	A36 Gr.36	Typical
44	M102	N201	N202			RIGID	None	None	RIGID	Typical
45	M103	N203	N204			RIGID	None	None	RIGID	Typical
46	M104	N205	N206			RIGID	None	None	RIGID	Typical
47	M105	N207	N208			RIGID	None	None	RIGID	Typical
48	M106	N209	N210			Toe Rail	None	None	A53 Gr.B	Typical
49	M49B	N89A	N90A			RIGID	None	None	RIGID	Typical
50	M50B	N91A	N92A			RIGID	None	None	RIGID	Typical
51	M51B	N93B	N94B			RIGID	None	None	RIGID	Typical
52	M52B	N95B	N96B			RIGID	None	None	RIGID	Typical
53	A1	N97B	N101B			Pipe Mount	None	None	A53 Gr.B	Typical
54	A2	N98B	N102B			Pipe Mount	None	None	A53 Gr.B	Typical
55	A3	N99B	N103B			Pipe Mount	None	None	A53 Gr.B	Typical
56	A4	N100B	N104B			Pipe Mount	None	None	A53 Gr.B	Typical
57	M57B	N105B	N106B			RIGID	None	None	RIGID	Typical
58	M58B	N107B	N108B			RIGID	None	None	RIGID	Typical
59	M59B	N109B	N110B			RIGID	None	None	RIGID	Typical
60	M60B	N111B	N112B			RIGID	None	None	RIGID	Typical
61	B1	N113B	N117A			Pipe Mount	None	None	A53 Gr.B	Typical
62	B2	N114A	N118A			Pipe Mount	None	None	A53 Gr.B	Typical
63	B3	N115A	N119A			Pipe Mount	None	None	A53 Gr.B	Typical
64	B4	N116A	N120A			Pipe Mount	None	None	A53 Gr.B	Typical
65	M65	N121	N122			RIGID	None	None	RIGID	Typical
66	M66	N123	N124			RIGID	None	None	RIGID	Typical
67	M67	N125	N126			RIGID	None	None	RIGID	Typical
68	M68	N127	N128			RIGID	None	None	RIGID	Typical
69	C1	N129	N133			Pipe Mount	None	None	A53 Gr.B	Typical
70	C2	N130	N134			Pipe Mount	None	None	A53 Gr.B	Typical
71	C3	N131	N135			Pipe Mount	None	None	A53 Gr.B	Typical
72	C4	N132	N136			Pipe Mount	None	None	A53 Gr.B	Typical
73	M73	N137	N138			Support Rail	None	None	A53 Gr.B	Typical
74	M74	N139	N140			RIGID	None	None	RIGID	Typical
75	M75	N141	N142			RIGID	None	None	RIGID	Typical
76	M76	N143	N144			RIGID	None	None	RIGID	Typical
77	M77	N145	N146			RIGID	None	None	RIGID	Typical
78	M78	N147	N148		180	Support Rail C...	None	None	A36 Gr.36	Typical
79	M79	N149	N150			Support Rail	None	None	A53 Gr.B	Typical
80	M80	N151	N152			RIGID	None	None	RIGID	Typical
81	M81	N153	N154			RIGID	None	None	RIGID	Typical
82	M82	N155	N156			RIGID	None	None	RIGID	Typical
83	M83	N157	N158			RIGID	None	None	RIGID	Typical
84	M84	N159	N160		180	Support Rail C...	None	None	A36 Gr.36	Typical
85	M85	N161	N162			Support Rail	None	None	A53 Gr.B	Typical
86	M86	N163	N164			RIGID	None	None	RIGID	Typical



Member Primary Data (Continued)

	Label	I Joint	J Joint	K Joint	Rotate(deg)	Section/Shape	Type	Design List	Material	Design Rules
87	M87	N165	N166			RIGID	None	None	RIGID	Typical
88	M88	N167	N168			RIGID	None	None	RIGID	Typical
89	M89	N169	N170			RIGID	None	None	RIGID	Typical
90	M90	N171	N172		180	Support Rail C...	None	None	A36 Gr.36	Typical
91	M91A	N173	N174			Reinforcement...	None	None	A36 Gr.36	Typical
92	M92A	N174	RC			Platform Reinf...	None	None	A36 Gr.36	Typical
93	M93A	N177	N178		120	Reinforcement...	None	None	A36 Gr.36	Typical
94	M94A	N178	RA			Platform Reinf...	None	None	A36 Gr.36	Typical
95	M95A	N181	N182		240	Reinforcement...	None	None	A36 Gr.36	Typical
96	M96A	N182	RB			Platform Reinf...	None	None	A36 Gr.36	Typical

Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]
1	SA	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction
2	SB	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction
3	SC	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction
4	RC	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction
5	RA	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction
6	RB	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction

Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	
1	1.4 Dead	Yes	Y			1	1.4	0		0		0		0		0		0		0		0		0	
2	1.2 Dead + 1.0 Wind @ 0° - ...	Yes	Y			1	1.2	2	1	0		0		0		0		0		0		0		0	
3	0.9 Dead + 1.0 Wind @ 0° - ...	Yes	Y			1	.9	2	1	0		0		0		0		0		0		0		0	
4	1.2 Dead + 1.0 Wind @ 30° ...	Yes	Y			1	1.2	3	1	0		0		0		0		0		0		0		0	
5	0.9 Dead + 1.0 Wind @ 30° ...	Yes	Y			1	.9	3	1	0		0		0		0		0		0		0		0	
6	1.2 Dead + 1.0 Wind @ 60° ...	Yes	Y			1	1.2	4	1	0		0		0		0		0		0		0		0	
7	0.9 Dead + 1.0 Wind @ 60° ...	Yes	Y			1	.9	4	1	0		0		0		0		0		0		0		0	
8	1.2 Dead + 1.0 Wind @ 90° ...	Yes	Y			1	1.2	5	1	0		0		0		0		0		0		0		0	
9	0.9 Dead + 1.0 Wind @ 90° ...	Yes	Y			1	.9	5	1	0		0		0		0		0		0		0		0	
10	1.2 Dead + 1.0 Wind @ 120° ...	Yes	Y			1	1.2	6	1	0		0		0		0		0		0		0		0	
11	0.9 Dead + 1.0 Wind @ 120° ...	Yes	Y			1	.9	6	1	0		0		0		0		0		0		0		0	
12	1.2 Dead + 1.0 Wind @ 150° ...	Yes	Y			1	1.2	7	1	0		0		0		0		0		0		0		0	
13	0.9 Dead + 1.0 Wind @ 150° ...	Yes	Y			1	.9	7	1	0		0		0		0		0		0		0		0	
14	1.2 Dead + 1.0 Wind @ 180° ...	Yes	Y			1	1.2	8	1	0		0		0		0		0		0		0		0	
15	0.9 Dead + 1.0 Wind @ 180° ...	Yes	Y			1	.9	8	1	0		0		0		0		0		0		0		0	
16	1.2 Dead + 1.0 Wind @ 210° ...	Yes	Y			1	1.2	9	1	0		0		0		0		0		0		0		0	
17	0.9 Dead + 1.0 Wind @ 210° ...	Yes	Y			1	.9	9	1	0		0		0		0		0		0		0		0	
18	1.2 Dead + 1.0 Wind @ 240° ...	Yes	Y			1	1.2	10	1	0		0		0		0		0		0		0		0	
19	0.9 Dead + 1.0 Wind @ 240° ...	Yes	Y			1	.9	10	1	0		0		0		0		0		0		0		0	
20	1.2 Dead + 1.0 Wind @ 270° ...	Yes	Y			1	1.2	11	1	0		0		0		0		0		0		0		0	
21	0.9 Dead + 1.0 Wind @ 270° ...	Yes	Y			1	.9	11	1	0		0		0		0		0		0		0		0	
22	1.2 Dead + 1.0 Wind @ 300° ...	Yes	Y			1	1.2	12	1	0		0		0		0		0		0		0		0	
23	0.9 Dead + 1.0 Wind @ 300° ...	Yes	Y			1	.9	12	1	0		0		0		0		0		0		0		0	
24	1.2 Dead + 1.0 Wind @ 330° ...	Yes	Y			1	1.2	13	1	0		0		0		0		0		0		0		0	
25	0.9 Dead + 1.0 Wind @ 330° ...	Yes	Y			1	.9	13	1	0		0		0		0		0		0		0		0	
26	1.2 Dead + 1.0 Ice Wind @ ...	Yes	Y			1	1.2	15	1	14	1		1	0		0		0		0		0		0	
27	1.2 Dead + 1.0 Ice Wind @ ...	Yes	Y			1	1.2	16	1	14	1		1	0		0		0		0		0		0	
28	1.2 Dead + 1.0 Ice Wind @ ...	Yes	Y			1	1.2	17	1	14	1		1	0		0		0		0		0		0	
29	1.2 Dead + 1.0 Ice Wind @ ...	Yes	Y			1	1.2	18	1	14	1		1	0		0		0		0		0		0	
30	1.2 Dead + 1.0 Ice Wind @ ...	Yes	Y			1	1.2	19	1	14	1		1	0		0		0		0		0		0	
31	1.2 Dead + 1.0 Ice Wind @ ...	Yes	Y			1	1.2	20	1	14	1		1	0		0		0		0		0		0	
32	1.2 Dead + 1.0 Ice Wind @ ...	Yes	Y			1	1.2	21	1	14	1		1	0		0		0		0		0		0	



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Load Combinations (Continued)

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
33	1.2 Dead + 1.0 Ice Wind @ ...	Yes	Y		1	1.2	22	1	14	1		1	0	0	0	0								
34	1.2 Dead + 1.0 Ice Wind @ ...	Yes	Y		1	1.2	23	1	14	1		1	0	0	0	0								
35	1.2 Dead + 1.0 Ice Wind @ ...	Yes	Y		1	1.2	24	1	14	1		1	0	0	0	0								
36	1.2 Dead + 1.0 Ice Wind @ ...	Yes	Y		1	1.2	25	1	14	1		1	0	0	0	0								
37	1.2 Dead + 1.0 Ice Wind @ ...	Yes	Y		1	1.2	26	1	14	1		1	0	0	0	0								
38	1.2 Dead + 1.5 Live_M - A1 ...	Yes	Y		1	1.2	27	1.5	2	.063	0		0	0	0	0								
39	1.2 Dead + 1.5 Live_M - A1 ...	Yes	Y		1	1.2	27	1.5	3	.063	0		0	0	0	0								
40	1.2 Dead + 1.5 Live_M - A1 ...	Yes	Y		1	1.2	27	1.5	4	.063	0		0	0	0	0								
41	1.2 Dead + 1.5 Live_M - A1 ...	Yes	Y		1	1.2	27	1.5	5	.063	0		0	0	0	0								
42	1.2 Dead + 1.5 Live_M - A1 ...	Yes	Y		1	1.2	27	1.5	6	.063	0		0	0	0	0								
43	1.2 Dead + 1.5 Live_M - A1 ...	Yes	Y		1	1.2	27	1.5	7	.063	0		0	0	0	0								
44	1.2 Dead + 1.5 Live_M - A1 ...	Yes	Y		1	1.2	27	1.5	8	.063	0		0	0	0	0								
45	1.2 Dead + 1.5 Live_M - A1 ...	Yes	Y		1	1.2	27	1.5	9	.063	0		0	0	0	0								
46	1.2 Dead + 1.5 Live_M - A1 ...	Yes	Y		1	1.2	27	1.5	10	.063	0		0	0	0	0								
47	1.2 Dead + 1.5 Live_M - A1 ...	Yes	Y		1	1.2	27	1.5	11	.063	0		0	0	0	0								
48	1.2 Dead + 1.5 Live_M - A1 ...	Yes	Y		1	1.2	27	1.5	12	.063	0		0	0	0	0								
49	1.2 Dead + 1.5 Live_M - A1 ...	Yes	Y		1	1.2	27	1.5	13	.063	0		0	0	0	0								
50	1.2 Dead + 1.5 Live_M - A2 ...	Yes	Y		1	1.2	28	1.5	2	.063	0		0	0	0	0								
51	1.2 Dead + 1.5 Live_M - A2 ...	Yes	Y		1	1.2	28	1.5	3	.063	0		0	0	0	0								
52	1.2 Dead + 1.5 Live_M - A2 ...	Yes	Y		1	1.2	28	1.5	4	.063	0		0	0	0	0								
53	1.2 Dead + 1.5 Live_M - A2 ...	Yes	Y		1	1.2	28	1.5	5	.063	0		0	0	0	0								
54	1.2 Dead + 1.5 Live_M - A2 ...	Yes	Y		1	1.2	28	1.5	6	.063	0		0	0	0	0								
55	1.2 Dead + 1.5 Live_M - A2 ...	Yes	Y		1	1.2	28	1.5	7	.063	0		0	0	0	0								
56	1.2 Dead + 1.5 Live_M - A2 ...	Yes	Y		1	1.2	28	1.5	8	.063	0		0	0	0	0								
57	1.2 Dead + 1.5 Live_M - A2 ...	Yes	Y		1	1.2	28	1.5	9	.063	0		0	0	0	0								
58	1.2 Dead + 1.5 Live_M - A2 ...	Yes	Y		1	1.2	28	1.5	10	.063	0		0	0	0	0								
59	1.2 Dead + 1.5 Live_M - A2 ...	Yes	Y		1	1.2	28	1.5	11	.063	0		0	0	0	0								
60	1.2 Dead + 1.5 Live_M - A2 ...	Yes	Y		1	1.2	28	1.5	12	.063	0		0	0	0	0								
61	1.2 Dead + 1.5 Live_M - A2 ...	Yes	Y		1	1.2	28	1.5	13	.063	0		0	0	0	0								
62	1.2 Dead + 1.5 Live_M - A3 ...	Yes	Y		1	1.2	29	1.5	2	.063	0		0	0	0	0								
63	1.2 Dead + 1.5 Live_M - A3 ...	Yes	Y		1	1.2	29	1.5	3	.063	0		0	0	0	0								
64	1.2 Dead + 1.5 Live_M - A3 ...	Yes	Y		1	1.2	29	1.5	4	.063	0		0	0	0	0								
65	1.2 Dead + 1.5 Live_M - A3 ...	Yes	Y		1	1.2	29	1.5	5	.063	0		0	0	0	0								
66	1.2 Dead + 1.5 Live_M - A3 ...	Yes	Y		1	1.2	29	1.5	6	.063	0		0	0	0	0								
67	1.2 Dead + 1.5 Live_M - A3 ...	Yes	Y		1	1.2	29	1.5	7	.063	0		0	0	0	0								
68	1.2 Dead + 1.5 Live_M - A3 ...	Yes	Y		1	1.2	29	1.5	8	.063	0		0	0	0	0								
69	1.2 Dead + 1.5 Live_M - A3 ...	Yes	Y		1	1.2	29	1.5	9	.063	0		0	0	0	0								
70	1.2 Dead + 1.5 Live_M - A3 ...	Yes	Y		1	1.2	29	1.5	10	.063	0		0	0	0	0								
71	1.2 Dead + 1.5 Live_M - A3 ...	Yes	Y		1	1.2	29	1.5	11	.063	0		0	0	0	0								
72	1.2 Dead + 1.5 Live_M - A3 ...	Yes	Y		1	1.2	29	1.5	12	.063	0		0	0	0	0								
73	1.2 Dead + 1.5 Live_M - A3 ...	Yes	Y		1	1.2	29	1.5	13	.063	0		0	0	0	0								
74	1.2 Dead + 1.5 Live_M - A4 ...	Yes	Y		1	1.2	30	1.5	2	.063	0		0	0	0	0								
75	1.2 Dead + 1.5 Live_M - A4 ...	Yes	Y		1	1.2	30	1.5	3	.063	0		0	0	0	0								
76	1.2 Dead + 1.5 Live_M - A4 ...	Yes	Y		1	1.2	30	1.5	4	.063	0		0	0	0	0								
77	1.2 Dead + 1.5 Live_M - A4 ...	Yes	Y		1	1.2	30	1.5	5	.063	0		0	0	0	0								
78	1.2 Dead + 1.5 Live_M - A4 ...	Yes	Y		1	1.2	30	1.5	6	.063	0		0	0	0	0								
79	1.2 Dead + 1.5 Live_M - A4 ...	Yes	Y		1	1.2	30	1.5	7	.063	0		0	0	0	0								
80	1.2 Dead + 1.5 Live_M - A4 ...	Yes	Y		1	1.2	30	1.5	8	.063	0		0	0	0	0								
81	1.2 Dead + 1.5 Live_M - A4 ...	Yes	Y		1	1.2	30	1.5	9	.063	0		0	0	0	0								
82	1.2 Dead + 1.5 Live_M - A4 ...	Yes	Y		1	1.2	30	1.5	10	.063	0		0	0	0	0								
83	1.2 Dead + 1.5 Live_M - A4 ...	Yes	Y		1	1.2	30	1.5	11	.063	0		0	0	0	0								
84	1.2 Dead + 1.5 Live_M - A4 ...	Yes	Y		1	1.2	30	1.5	12	.063	0		0	0	0	0								
85	1.2 Dead + 1.5 Live_M - A4 ...	Yes	Y		1	1.2	30	1.5	13	.063	0		0	0	0	0								
86	1.2 Dead + 1.5 Live_M - B1 ...	Yes	Y		1	1.2	31	1.5	2	.063	0		0	0	0	0								
87	1.2 Dead + 1.5 Live_M - B1 ...	Yes	Y		1	1.2	31	1.5	3	.063	0		0	0	0	0								
88	1.2 Dead + 1.5 Live_M - B1 ...	Yes	Y		1	1.2	31	1.5	4	.063	0		0	0	0	0								
89	1.2 Dead + 1.5 Live_M - B1 ...	Yes	Y		1	1.2	31	1.5	5	.063	0		0	0	0	0								



Basic Load Cases (Continued)

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed Area(Me...)	Surface(P...
30	Live Load - A4	None					1		
31	Live Load - B1	None					1		
32	Live Load - B2	None					1		
33	Live Load - B3	None					1		
34	Live Load - B4	None					1		
35	Live Load - C1	None					1		
36	Live Load - C2	None					1		
37	Live Load - C3	None					1		
38	Live Load - C4	None					1		
39	Live Load - M46 (Start)	None					1		
40	Live Load - M46 (Mid...	None					1		
41	Live Load - M46 (End)	None					1		
42	Live Load - M47 (Start)	None					1		
43	Live Load - M47 (Mid...	None					1		
44	Live Load - M47 (End)	None					1		
45	Live Load - M48 (Start)	None					1		
46	Live Load - M48 (Mid...	None					1		
47	Live Load - M48 (End)	None					1		
48	Live Load - M49 (Start)	None					1		
49	Live Load - M49 (Mid...	None					1		
50	Live Load - M49 (End)	None					1		
51	Live Load - M16 (Start)	None					1		
52	Live Load - M16 (Mid...	None					1		
53	Live Load - M16 (End)	None					1		
54	Live Load - M46A (St...	None					1		
55	Live Load - M46A (Mi...	None					1		
56	Live Load - M46A (E...	None					1		
57	Live Load - M47A (St...	None					1		
58	Live Load - M47A (Mi...	None					1		
59	Live Load - M47A (E...	None					1		
60	Live Load - M48A (St...	None					1		
61	Live Load - M48A (Mi...	None					1		
62	Live Load - M48A (E...	None					1		
63	Live Load - M49A (St...	None					1		
64	Live Load - M49A (Mi...	None					1		
65	Live Load - M49A (E...	None					1		
66	Live Load - M61 (Start)	None					1		
67	Live Load - M61 (Mid...	None					1		
68	Live Load - M61 (End)	None					1		
69	Live Load - M91 (Start)	None					1		
70	Live Load - M91 (Mid...	None					1		
71	Live Load - M91 (End)	None					1		
72	Live Load - M92 (Start)	None					1		
73	Live Load - M92 (Mid...	None					1		
74	Live Load - M92 (End)	None					1		
75	Live Load - M93 (Start)	None					1		
76	Live Load - M93 (Mid...	None					1		
77	Live Load - M93 (End)	None					1		
78	Live Load - M94 (Start)	None					1		
79	Live Load - M94 (Mid...	None					1		
80	Live Load - M94 (End)	None					1		
81	Live Load - M106 (St...	None					1		
82	Live Load - M106 (Mi...	None					1		
83	Live Load - M106 (En...	None					1		
84	BLC 1 Transient Area...	None						51	
85	BLC 14 Transient Are...	None						51	



Envelope Joint Reactions

Joint	X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC		
1	SA	max	1832.211	15	1452.436	31	3842.959	22	1.412	33	1.613	4	1.187	27
2		min	-2870.055	2	410.602	25	-2042.856	11	.274	5	-1.599	17	.061	15
3	SB	max	1288.489	19	1452.451	34	2472.554	19	-.336	23	1.136	12	.967	73
4		min	-2332.766	6	412.202	7	-4265.801	6	-1.699	30	-1.123	25	-.049	103
5	SC	max	5165.185	14	1451.814	28	1043.478	21	.683	154	1.37	20	-.459	3
6		min	-3089.447	3	412.561	19	-1048.105	8	-.489	112	-1.357	9	-1.77	32
7	RC	max	421.589	3	2065.602	32	34.839	21	0	141	0	123	0	226
8		min	-3653.227	32	-230.361	3	-34.825	9	0	123	0	141	0	1
9	RA	max	1805.646	36	2041.931	36	187.009	11	0	175	0	175	0	49
10		min	-111.197	11	-115.629	11	-3127.378	36	0	49	0	49	0	175
11	RB	max	1824.25	28	2061.246	28	3157.12	28	0	83	0	83	0	83
12		min	-178.185	19	-189.748	19	-303.42	19	0	89	0	89	0	89
13	Totals:	max	5766.049	15	9941.518	31	5105.465	21						
14		min	-5766.051	2	2836.085	25	-5105.466	9						

Envelope AISC 15th(360-16): LRFD Steel Code Checks

Member	Shape	Code Check	Loc	LC	Shear	Loc	Dir	LC	phi	Pnc	phi	Pnt	phi	Mn	phi	Mn	Cb	Eqn
1	M46	HSS4x4x3	.177	0	2	.089	0	y	51	106237	118378	14.115	14.115	3	H1-1b			
2	M47	HSS4x4x3	.165	30.6	35	.059	30.6	y	27	106648	118378	14.115	14.115	1	H1-1b			
3	M48	L2x2x3	.411	25.5	1	.031	51.1	y	1	15920	23392.8	.558	1.169	1	H2-1			
4	M49	L2x2x3	.417	25.5	1	.030	0	z	1	15920	23392.8	.558	1.166	1	H2-1			
5	M50	PL6x1/2	.058	1.642	24	.180	2.765	y	1	95340	97200	1.012	12.15	2	H1-1b			
6	M51	PL6x1/2	.211	6.81	24	.222	6.81	y	1	60829	97200	1.012	12.15	1	H1-1b			
7	M52	PL6x1/2	.052	1.123	22	.205	0	y	49	95340	97200	1.012	12.15	3	H1-1b			
8	M53	PL6x3/8	.287	2.627	14	.250	2.627	y	29	62895	72900	.57	9.113	1	H1-1b			
9	M54	PL6x3/8	.209	1.595	8	.267	0	y	36	69770.62	72900	.57	9.113	1	H1-1b			
10	M55	PL6x3/8	.290	2.627	4	.130	2.627	y	33	62895	72900	.57	9.113	1	H1-1b			
11	M56	PL6x3/8	.264	1.595	12	.288	0	y	36	69770.62	72900	.57	9.113	1	H1-1b			
12	M16	P3 STD	.104	96.8	6	.093	96.8		4	29986.1	70196	6.124	6.124	3	H1-1b			
13	M46A	HSS4x4x3	.161	0	31	.088	0	y	1	106237	118378	14.115	14.115	2	H1-1b			
14	M47A	HSS4x4x3	.165	30.6	29	.059	30.6	y	30	106648	118378	14.115	14.115	1	H1-1b			
15	M48A	L2x2x3	.411	25.5	2	.031	51.1	y	2	15920	23392.8	.558	1.169	1	H2-1			
16	M49A	L2x2x3	.417	25.5	2	.030	0	z	2	15920	23392.8	.558	1.166	1	H2-1			
17	M50A	PL6x1/2	.060	1.642	6	.180	2.765	y	75	95340	97200	1.012	12.15	2	H1-1b			
18	M51A	PL6x1/2	.235	6.81	6	.222	6.81	y	2	60829	97200	1.012	12.15	1	H1-1b			
19	M52A	PL6x1/2	.058	1.123	6	.205	0	y	89	95340	97200	1.012	12.15	3	H1-1b			
20	M53A	PL6x3/8	.218	2.627	22	.251	2.627	y	33	62895	72900	.57	9.113	1	H1-1b			
21	M54A	PL6x3/8	.225	1.595	16	.267	0	y	28	69770.62	72900	.57	9.113	1	H1-1b			
22	M55A	PL6x3/8	.225	2.627	12	.128	2.627	y	36	62895	72900	.57	9.113	1	H1-1b			
23	M56A	PL6x3/8	.276	1.595	20	.289	0	y	28	69770.62	72900	.57	9.113	1	H1-1b			
24	M61	P3 STD	.115	96.8	14	.082	96.8		12	29986.1	70196	6.124	6.124	3	H1-1b			
25	M91	HSS4x4x3	.166	0	35	.088	0	y	1	106237	118378	14.115	14.115	2	H1-1b			
26	M92	HSS4x4x3	.165	30.6	33	.059	30.6	y	34	106648	118378	14.115	14.115	1	H1-1b			
27	M93	L2x2x3	.411	25.5	2	.031	51.1	y	2	15920	23392.8	.558	1.169	1	H2-1			
28	M94	L2x2x3	.417	25.5	2	.030	0	z	2	15920	23392.8	.558	1.166	1	H2-1			
29	M95	PL6x1/2	.061	1.642	14	.180	2.765	y	1	95340	97200	1.012	12.15	2	H1-1b			
30	M96	PL6x1/2	.257	6.81	14	.222	6.81	y	2	60829	97200	1.012	12.15	1	H1-1b			
31	M97	PL6x1/2	.064	1.123	3	.205	0	y	1	95340	97200	1.012	12.15	2	H1-1b			
32	M98	PL6x3/8	.224	2.627	6	.251	2.627	y	37	62895	72900	.57	9.113	1	H1-1b			
33	M99	PL6x3/8	.235	1.595	2	.268	0	y	32	69770.62	72900	.57	9.113	1	H1-1b			
34	M100	PL6x3/8	.209	2.627	20	.129	2.627	y	28	62895	72900	.57	9.113	1	H1-1b			
35	M101	PL6x3/8	.281	1.595	2	.289	0	y	32	69770.62	72900	.57	9.113	2	H1-1b			
36	M106	P3 STD	.103	54.6	1	.088	53.1		16	29986.1	70196	6.124	6.124	2	H1-1b			
37	A1	P2 STD	.337	69	22	.211	69		2	15808	33847	1.997	1.997	3	H1-1b			



Company : GPD
 Designer : Steward, Matthew
 Job Number : 2021777.876369.02
 Model Name : 876369 - HARWINTON / BUCKLEY BROADCASTI

Mar 10, 2021
 10:11 AM
 Checked By: _____

Envelope AISC 15th(360-16): LRFD Steel Code Checks (Continued)

Member	Shape	Code Check	Loc	LC	Shear	Loc	Dir	LC	phi*	Pnc	phi*	Pnt	phi*	Mn	phi*	Mn	Cb	Eqn
38	A2	P2 STD	.362	69	18	.103	69	24	15808	...	33847	...	1.997	1.997	2	...	H1-1b	
39	A3	P2 STD	.375	69	8	.140	69	4	15808	...	33847	...	1.997	1.997	3	...	H1-1b	
40	A4	P2 STD	.303	69	6	.180	27	2	15808	...	33847	...	1.997	1.997	3	...	H1-1b	
41	B1	P2 STD	.356	69	6	.191	27	8	15808	...	33847	...	1.997	1.997	3	...	H1-1b	
42	B2	P2 STD	.381	69	4	.104	69	6	15808	...	33847	...	1.997	1.997	3	...	H1-1b	
43	B3	P2 STD	.394	69	16	.122	69	10	15808	...	33847	...	1.997	1.997	4	...	H1-1b	
44	B4	P2 STD	.306	69	14	.161	69	20	15808	...	33847	...	1.997	1.997	4	...	H1-1b	
45	C1	P2 STD	.388	69	14	.197	27	16	15808	...	33847	...	1.997	1.997	4	...	H1-1b	
46	C2	P2 STD	.392	69	12	.113	48	14	15808	...	33847	...	1.997	1.997	2	...	H1-1b	
47	C3	P2 STD	.429	69	2	.118	69	18	15808	...	33847	...	1.997	1.997	4	...	H1-1b	
48	C4	P2 STD	.310	69	24	.171	69	4	15808	...	33847	...	1.997	1.997	4	...	H1-1b	
49	M73	P2 STD	.292	53.1	2	.229	12.5	2	6684.464	...	33847	...	1.997	1.997	3	...	H1-1b	
50	M78	L2.5x2.5x4	.379	0	24	.120	0	y	16	36167	...	38556	1.114	2.537	1	...	H2-1	
51	M79	P2 STD	.282	53.1	8	.213	12.5	8	6684.464	...	33847	...	1.997	1.997	3	...	H1-1b	
52	M84	L2.5x2.5x4	.393	0	8	.106	0	y	24	36167	...	38556	1.114	2.537	1	...	H2-1	
53	M85	P2 STD	.292	53.1	16	.220	12.5	16	6684.464	...	33847	...	1.997	1.997	3	...	H1-1b	
54	M90	L2.5x2.5x4	.408	0	16	.102	0	y	6	36167	...	38556	1.114	2.537	1	...	H2-1	
55	M91A	PL 8x1/2	.085	0	32	.071	0	y	32	120475	...	129600	1.35	21.6	1	...	H1-1b	
56	M92A	LL2.5x2.5x...	.099	50.9	32	.005	0	y	33	42543	...	58482	4.246	2.614	1	...	H1-1b*	
57	M93A	PL 8x1/2	.084	0	35	.070	0	y	36	120475	...	129600	1.35	21.6	1	...	H1-1b	
58	M94A	LL2.5x2.5x...	.098	50.9	36	.005	50.9	y	37	42543	...	58482	4.246	2.614	1	...	H1-1b*	
59	M95A	PL 8x1/2	.084	0	28	.071	0	y	28	120475	...	129600	1.35	21.6	1	...	H1-1b	
60	M96A	LL2.5x2.5x...	.098	50.9	28	.005	50.9	y	29	42543	...	58482	4.246	2.614	1	...	H1-1b*	

APPENDIX D
ADDITIONAL CALCULATIONS



TIA-222-H CONNECTION CHECK
Mount to Tower Connection - Typ. All Sectors
2021777.876369.02

Bolt Information	
Bolt Diameter (d)	0.625 in
Net Tensile Area (A _n)	0.226 in ²
# of Bolts Total (n)	4
Bolt Distance Up-Down	6 in
Bolt Distance Left-Right	6 in
Bolt Grade	A325N
Bolt Tensile Strength (F _{ub})	120 ksi

Flange Information	
Height (h)	8 in
Width (w)	8 in
Thickness (t)	0.75 in
Steel Grade	A36
Plate Yield Strength (F _y)	36 ksi
Support Arm Height	4 in
Support Arm Width	4 in

RISA 3D Reactions (Up-Down)	
Moment (M)	0.24 k-ft
Axial (T)	-2.59 kips
Shear (V)	4.51 kips

RISA 3D Reactions (Left-Right)	
Moment (M)	1.61 k-ft
Axial (T)	0.37 kips
Shear (V)	1.79 kips

Bolt Capacity (Up-Down)	
Nominal Tensile Strength (R _{nt})	27.120 kips
Nominal Shear Strength (R _{nv})	18.41 kips
Bolt Tensile Force (T _{ub})	-0.41 kips
Bolt Shear Force (V _{ub})	1.127 kips
T _{ub} /φR _{nt}	-0.01925
V _{ub} /φR _{nv}	0.07771
(V _{ub} /φR _{nv}) ² +(T _{ub} /φR _{nt}) ²	0.00673
Bolt Capacity =	7.8% OK

Bolt Capacity (Left-Right)	
Nominal Tensile Strength (R _{nt})	27.120 kips
Nominal Shear Strength (R _{nv})	18.41 kips
Bolt Tensile Force (T _{ub})	1.70 kips
Bolt Shear Force (V _{ub})	0.448 kips
T _{ub} /φR _{nt}	0.07982
V _{ub} /φR _{nv}	0.03092
(V _{ub} /φR _{nv}) ² +(T _{ub} /φR _{nt}) ²	0.00769
Bolt Capacity =	8.0% OK

*Rating per TIA-222-H, Section 15.5

*Rating per TIA-222-H, Section 15.5

Plate Capacity (Up-Down)	
Bolt Circle (D _{BC})	8.485 in
Effective Width (B _{eff})	7.48 in
Flexural Moment (M _u)	1.55 k-in
Flexural Strength (φM _n)	34.10 k-in
Plate Capacity=	4.3% OK

Plate Capacity (Left-Right)	
Bolt Circle (D _{BC})	8.485 in
Effective Width (B _{eff})	7.48 in
Flexural Moment (M _u)	3.41 k-in
Flexural Strength (φM _n)	34.10 k-in
Plate Capacity=	9.5% OK

*Rating per TIA-222-H, Section 15.5

*Rating per TIA-222-H, Section 15.5



TIA-222-H CONNECTION CHECK
Kicker to Tower Connection - Typ. All Sectors
2021777.876369.02

Bolt Information	
Bolt Diameter (d)	0.625 in
Net Tensile Area (A _n)	0.226 in ²
# of Bolts Total (n)	4
Bolt Grade	A325N
Bolt Tensile Strength (F _{ub})	120 ksi

RISA 3D Reactions	
Moment (M)	0.00 k-ft
Axial (T)	1.83 kips
Shear (V)	3.78 kips

Bolt Capacity	
Nominal Tensile Strength (R _{nt})	27.120 kips
Nominal Shear Strength (R _{nv})	18.41 kips
Bolt Tensile Force (T _{ub})	0.46 kips
Bolt Shear Force (V _{ub})	0.945 kips
$T_{ub}/\phi R_{nt}$	0.02139
$V_{ub}/\phi R_{nv}$	0.06516
$(V_{ub}/\phi R_{nv})^2 + (T_{ub}/\phi R_{nt})^2$	0.00494
Bolt Capacity =	6.5% OK

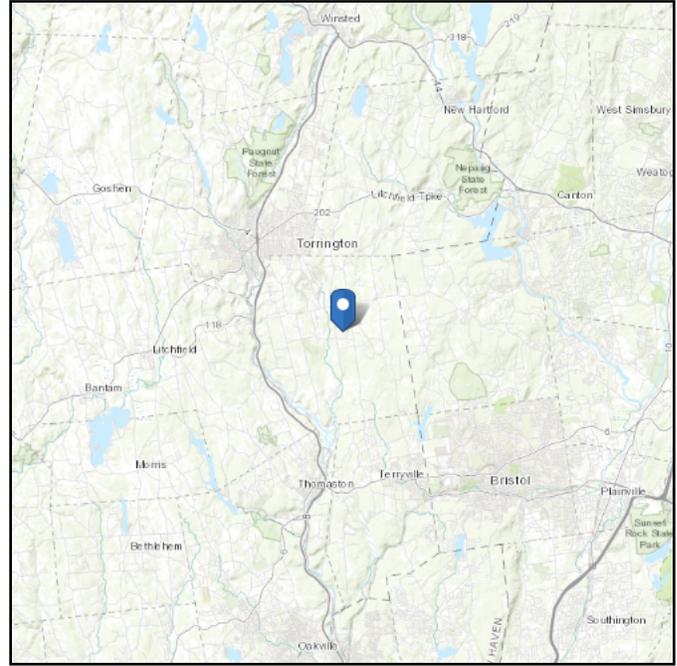
*Rating per TIA-222-H, Section 15.5

ASCE 7 Hazards Report

Address:
No Address at This Location

Standard: ASCE/SEI 7-10
Risk Category: II
Soil Class: D - Stiff Soil

Elevation: 840.53 ft (NAVD 88)
Latitude: 41.757264
Longitude: -73.052556



Wind

Results:

Wind Speed:	118 Vmph	120 mph per jurisdiction requirement
10-year MRI	76 Vmph	
25-year MRI	85 Vmph	
50-year MRI	90 Vmph	
100-year MRI	97 Vmph	

Data Source: ASCE/SEI 7-10 Fig. 26.5-1A and Figs. CC-1–CC-4, and Section 26.5.2, incorporating errata of March 12, 2014

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

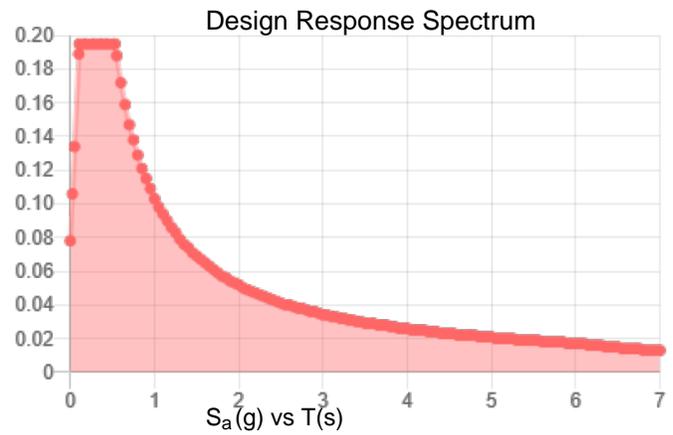
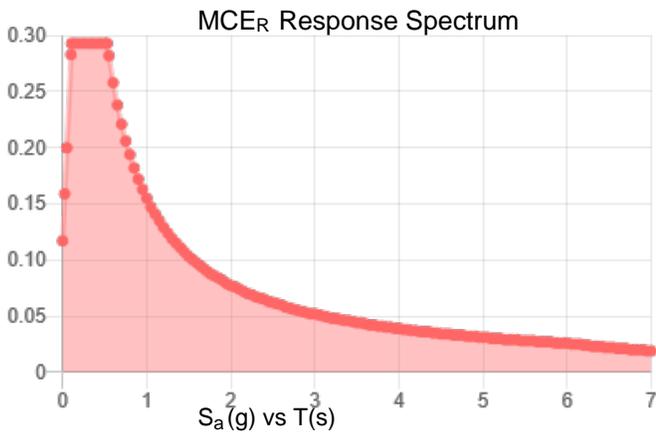
Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.

Site Soil Class: D - Stiff Soil

Results:

S_s :	0.183	S_{DS} :	0.195
S_1 :	0.065	S_{D1} :	0.103
F_a :	1.6	T_L :	6
F_v :	2.4	PGA :	0.093
S_{MS} :	0.293	PGA _M :	0.148
S_{M1} :	0.155	F _{PGA} :	1.6
		I_e :	1

Seismic Design Category B



Data Accessed:

Wed Mar 10 2021

Date Source:

USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-10 Ch. 21 are available from USGS.

Ice

Results:

Ice Thickness: 0.75 in.

Concurrent Temperature: 5 F

Gust Speed: 50 mph

Data Source: Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Wed Mar 10 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

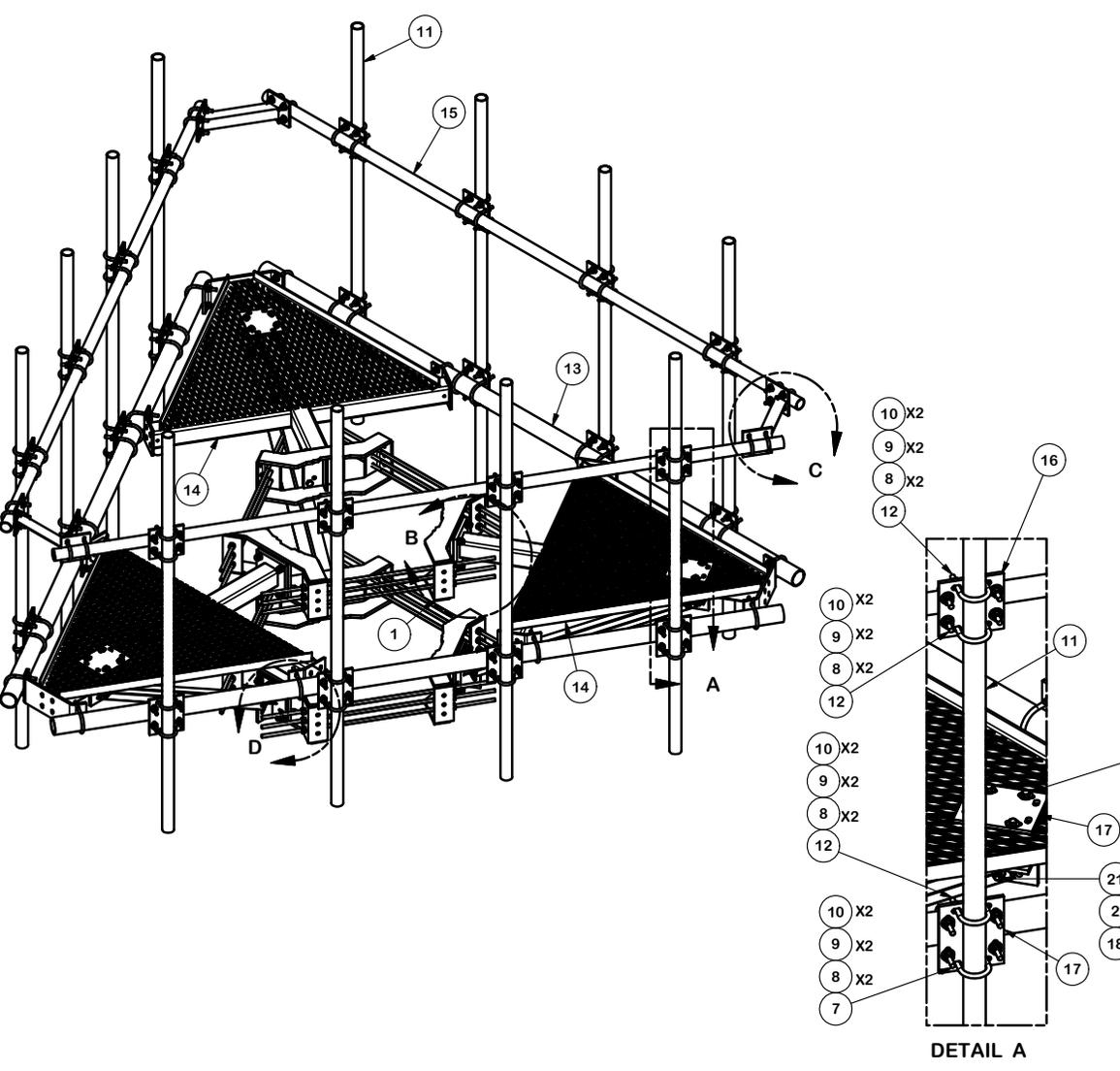
Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

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APPENDIX E
SUPPLEMENTAL DRAWINGS



PARTS LIST						
ITEM	QTY	PART NO.	PART DESCRIPTION	LENGTH	UNIT WT.	NET WT.
1	6	X-LWRM	RING MOUNT WELDMNT		68.16	408.95
2	66	G58LW	5/8" HDG LOCKWASHER		0.03	1.72
3	60	A58NUT	5/8" HDG A325 HEX NUT		0.13	7.78
4	18	G58R-24	5/8" x 24" THREADED ROD (HDG.)		0.55	9.88
4	18	G58R-48	5/8" x 48" THREADED ROD (HDG.)		0.55	9.88
5	24	A58234	5/8" x 2-3/4" HDG A325 HEX BOLT	2 3/4 in	0.36	8.53
6	24	A58FW	5/8" HDG A325 FLATWASHER		0.03	0.82
7	36	X-UB1306	1/2" X 3-5/8" X 6" X 3" U-BOLT (HDG.)		0.73	26.34
8	264	G12FW	1/2" HDG USS FLATWASHER		0.03	8.99
9	252	G12LW	1/2" HDG LOCKWASHER		0.01	3.50
10	252	G12NUT	1/2" HDG HEAVY 2H HEX NUT		0.07	18.03
11	12	P296	2-3/8" X 96" SCH. 40 GALVANIZED PIPE	96 in	30.76	369.08
12	84	X-UB1212	1/2" X 2-1/2" X 4-1/2" X 2" U-BOLT (HDG.)		0.73	61.46
13	3	P3150	3-1/2" X 150" SCH 40 GALVANIZED PIPE	150 in	94.80	284.40
14	3	X-SV196	LOW PROFILE PLATFORM CORNER		212.10	636.31
15	3	P2150	2-3/8" OD X 150" SCH 40 GALVANIZED PIPE	150 in	48.06	144.17
16	12	SCX2	CROSSOVER PLATE	7 in	4.80	57.56
17	15	SCX4	CROSSOVER PLATE	8 1/2 in	6.02	90.32
18	6	G58NUT	5/8" HDG HEAVY 2H HEX NUT		0.13	0.78
19	6	X-253993	PLATFORM REINFORCEMENT KIT ANGLE	52 25/32 in	14.33	85.99
20	6	X-253992	T-BRACKET FOR REINFORCEMENT KIT		13.55	81.27
21	6	G5802	5/8" x 2" HDG HEX BOLT GR5		0.27	1.62
22	12	G12065	1/2" x 6-1/2" HDG HEX BOLT GR5 FULL THREAD	6 1/2 in	0.41	4.91
23	3	X-AHCP	ANGLE HANDRAIL CORNER PLATE		12.92	38.76
TOTAL WT. #						2448.72

TOLERANCE NOTES
 TOLERANCES ON DIMENSIONS, UNLESS OTHERWISE NOTED ARE:
 SAWED, SHEARED AND GAS CUT EDGES ($\pm 0.030"$)
 DRILLED AND GAS CUT HOLES ($\pm 0.030"$) - NO CONING OF HOLES
 LASER CUT EDGES AND HOLES ($\pm 0.010"$) - NO CONING OF HOLES
 BENDS ARE $\pm 1/2$ DEGREE
 ALL OTHER MACHINING ($\pm 0.030"$)
 ALL OTHER ASSEMBLY ($\pm 0.060"$)

PROPRIETARY NOTE:
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DESCRIPTION
**12' 6" LOW PROFILE PLATFORM
 WITH TWELVE 2-3/8" ANTENNA MOUNTING
 PIPES, AND HANDRAIL**

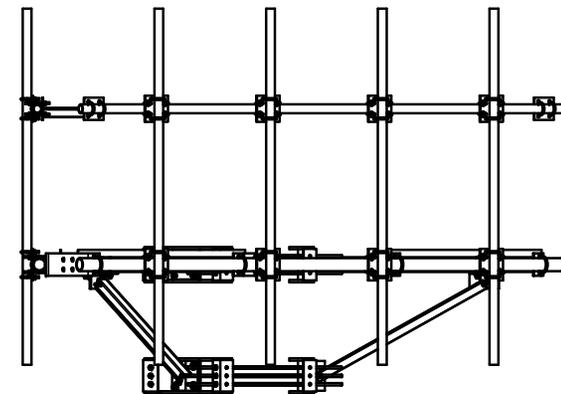
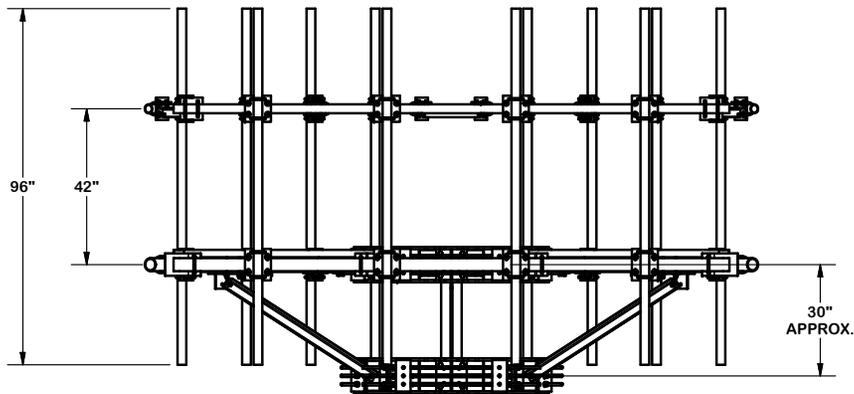
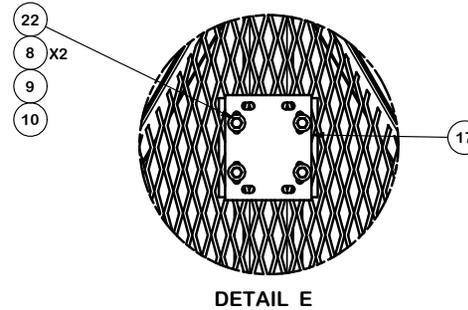
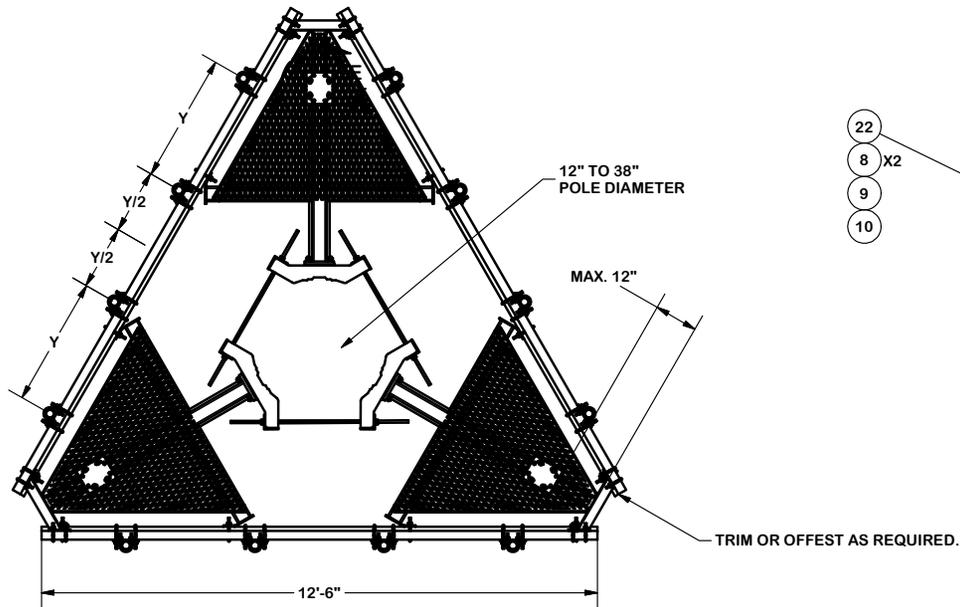
CPD NO. 4488	DRAWN BY CEK 7/14/2014	ENG. APPROVAL
CLASS 81	SUB 02	DRAWING USAGE CUSTOMER
CHECKED BY BMC 7/14/2014		

SITE PRO 1
 Engineering Support Team:
 1-888-753-7446

Locations:
 New York, NY
 Atlanta, GA
 Los Angeles, CA
 Plymouth, IN
 Salem, OR
 Dallas, TX

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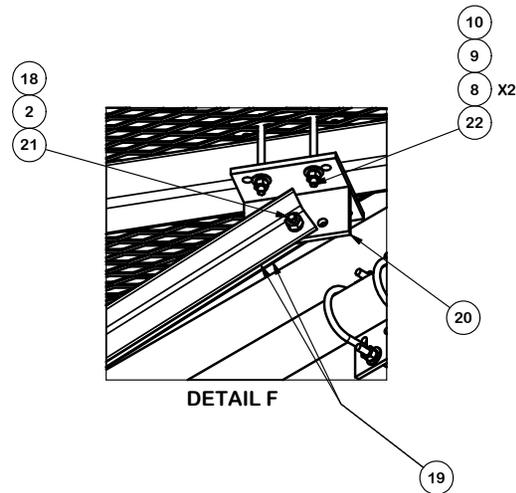
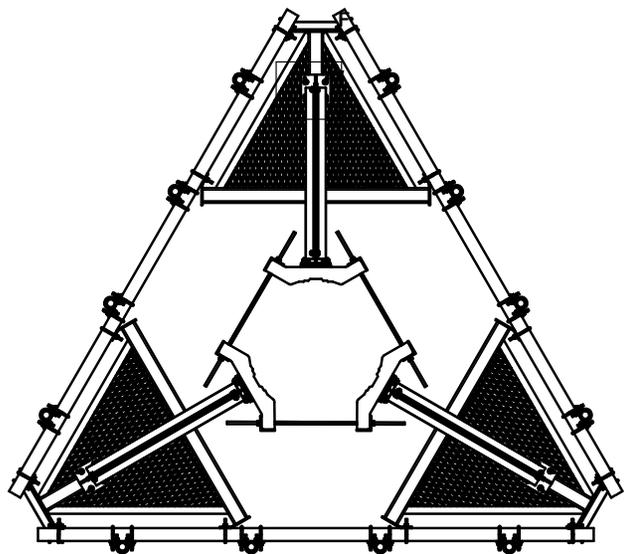
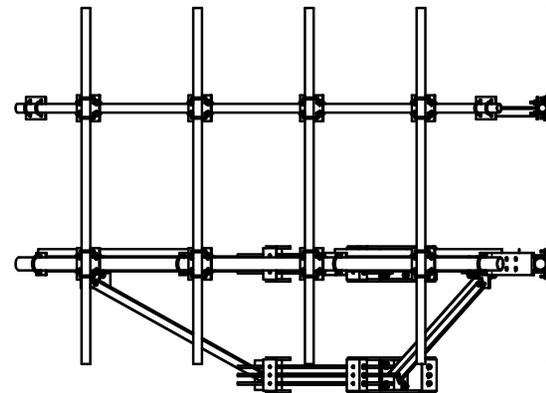
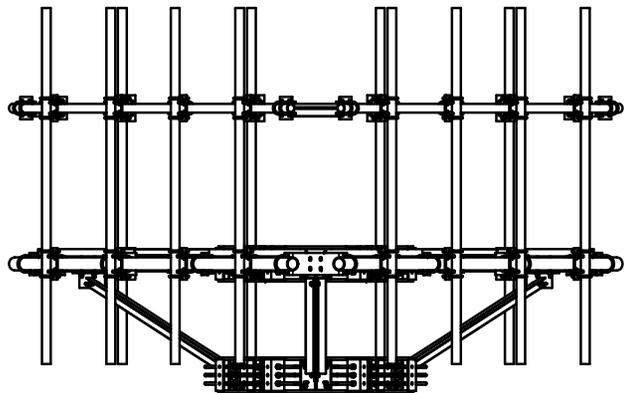
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Exhibit F

Power Density/RF Emissions Report

RADIO FREQUENCY EMISSIONS ANALYSIS REPORT
EVALUATION OF HUMAN EXPOSURE POTENTIAL
TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CTNH558A

64 Hungerford Lane
Harwinton, Connecticut 06791

April 21, 2021

EBI Project Number: 6221001884

Site Compliance Summary	
Compliance Status:	COMPLIANT
Site total MPE% of FCC general population allowable limit:	11.12%

April 21, 2021

T-Mobile

Attn: Jason Overbey, RF Manager
35 Griffin Road South
Bloomfield, Connecticut 06002

Emissions Analysis for Site: CTNH558A

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **64 Hungerford Lane** in **Harwinton, Connecticut** for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits; therefore, it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limits for the 600 MHz and 700 MHz frequency bands are approximately $400 \mu\text{W}/\text{cm}^2$ and $467 \mu\text{W}/\text{cm}^2$, respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 11 GHz frequency bands is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at 64 Hungerford Lane in Harwinton, Connecticut using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was focused at the base of the tower. For this report, the sample point is the top of a 6-foot person standing at the base of the tower. For power density calculations, the broadcast footprint of the AIR6449 antenna has been considered. Due to the beamforming nature of this antenna, the actual beam locations vary depending on demand and are narrow in nature. Using the broadcast footprint accounts for the potential location of beams at any given time.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 LTE channels (600 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) 1 NR channel (600 MHz Band) was considered for each sector of the proposed installation. This Channel has a transmit power of 80 Watts.
- 3) 2 LTE channels (700 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 4) 4 GSM channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 5) 2 LTE channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.

- 6) 2 LTE channels (AWS Band – 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 7) 1 LTE channel (BRS Band - 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 120 Watts.
- 8) 1 NR channel (BRS Band - 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 120 Watts.
- 9) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 10) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 11) The antennas used in this modeling are the RFS APX16DWV-16DWV-S-E-A20 for the 2100 MHz channel(s), the RFS APXVAALL24_43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz channel(s), the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz channel(s) in Sector A, the RFS APX16DWV-16DWV-S-E-A20 for the 2100 MHz channel(s), the RFS APXVAALL24_43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz channel(s), the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz channel(s) in Sector B, the RFS APX16DWV-16DWV-S-E-A20 for the 2100 MHz channel(s), the RFS APXVAALL24_43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz channel(s), the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz channel(s) in Sector C. This is based on feedback from the carrier with regard to anticipated antenna selection. All Antenna gain values and associated transmit power levels are shown in the Site Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

- 12) The antenna mounting height centerline of the proposed antennas is 180 feet above ground level (AGL).
- 13) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 14) All calculations were done with respect to uncontrolled / general population threshold limits.

T-Mobile Site Inventory and Power Data

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	RFS APX16DWV-16DWV-S-E-A20	Make / Model:	RFS APX16DWV-16DWV-S-E-A20	Make / Model:	RFS APX16DWV-16DWV-S-E-A20
Frequency Bands:	2100 MHz	Frequency Bands:	2100 MHz	Frequency Bands:	2100 MHz
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	180 feet	Height (AGL):	180 feet	Height (AGL):	180 feet
Channel Count:	2	Channel Count:	2	Channel Count:	2
Total TX Power (W):	120 Watts	Total TX Power (W):	120 Watts	Total TX Power (W):	120 Watts
ERP (W):	4,668.54	ERP (W):	4,668.54	ERP (W):	4,668.54
Antenna AI MPE %:	0.55%	Antenna BI MPE %:	0.55%	Antenna CI MPE %:	0.55%
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	RFS APXVAALL24_43-U-NA20	Make / Model:	RFS APXVAALL24_43-U-NA20	Make / Model:	RFS APXVAALL24_43-U-NA20
Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz	Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz	Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz
Gain:	12.95 dBd / 12.95 dBd / 13.65 dBd / 15.45 dBd / 15.45 dBd	Gain:	12.95 dBd / 12.95 dBd / 13.65 dBd / 15.45 dBd / 15.45 dBd	Gain:	12.95 dBd / 12.95 dBd / 13.65 dBd / 15.45 dBd / 15.45 dBd
Height (AGL):	180 feet	Height (AGL):	180 feet	Height (AGL):	180 feet
Channel Count:	11	Channel Count:	11	Channel Count:	11
Total TX Power (W):	440 Watts	Total TX Power (W):	440 Watts	Total TX Power (W):	440 Watts
ERP (W):	12,569.87	ERP (W):	12,569.87	ERP (W):	12,569.87
Antenna A2 MPE %:	2.17%	Antenna B2 MPE %:	2.17%	Antenna C2 MPE %:	2.17%
Antenna #:	3	Antenna #:	3	Antenna #:	3
Make / Model:	Ericsson AIR 6449	Make / Model:	Ericsson AIR 6449	Make / Model:	Ericsson AIR 6449
Frequency Bands:	2500 MHz / 2500 MHz	Frequency Bands:	2500 MHz / 2500 MHz	Frequency Bands:	2500 MHz / 2500 MHz
Gain:	17.3 dBd / 17.3 dBd	Gain:	17.3 dBd / 17.3 dBd	Gain:	17.3 dBd / 17.3 dBd
Height (AGL):	180 feet	Height (AGL):	180 feet	Height (AGL):	180 feet
Channel Count:	2	Channel Count:	2	Channel Count:	2
Total TX Power (W):	240 Watts	Total TX Power (W):	240 Watts	Total TX Power (W):	240 Watts
ERP (W):	12,888.76	ERP (W):	12,888.76	ERP (W):	12,888.76
Antenna A3 MPE %:	1.53%	Antenna B3 MPE %:	1.53%	Antenna C3 MPE %:	1.53%

Site Composite MPE %	
Carrier	MPE %
T-Mobile (Max at Sector A):	4.26%
Verizon	1.43%
Sprint	1.75%
AT&T	3.68%
Site Total MPE % :	11.12%

T-Mobile MPE % Per Sector	
T-Mobile Sector A Total:	4.26%
T-Mobile Sector B Total:	4.26%
T-Mobile Sector C Total:	4.26%
Site Total MPE % :	11.12%

T-Mobile Maximum MPE Power Values (Sector A)							
T-Mobile Frequency Band / Technology (Sector A)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ($\mu\text{W}/\text{cm}^2$)	Frequency (MHz)	Allowable MPE ($\mu\text{W}/\text{cm}^2$)	Calculated % MPE
T-Mobile 2100 MHz LTE	2	2334.27	180.0	5.54	2100 MHz LTE	1000	0.55%
T-Mobile 600 MHz LTE	2	591.73	180.0	1.41	600 MHz LTE	400	0.35%
T-Mobile 600 MHz NR	1	1577.94	180.0	1.87	600 MHz NR	400	0.47%
T-Mobile 700 MHz LTE	2	695.22	180.0	1.65	700 MHz LTE	467	0.35%
T-Mobile 1900 MHz GSM	4	1052.26	180.0	5.00	1900 MHz GSM	1000	0.50%
T-Mobile 1900 MHz LTE	2	2104.51	180.0	5.00	1900 MHz LTE	1000	0.50%
T-Mobile 2500 MHz LTE	1	6444.38	180.0	7.65	2500 MHz LTE	1000	0.77%
T-Mobile 2500 MHz NR	1	6444.38	180.0	7.65	2500 MHz NR	1000	0.77%
						Total:	4.26%

• NOTE: Totals may vary by approximately 0.01% due to summation of remainders in calculations.

Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector A:	4.26%
Sector B:	4.26%
Sector C:	4.26%
T-Mobile Maximum MPE % (Sector A):	4.26%
Site Total:	11.12%
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **11.12%** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.