

September 27, 2017

Melanie A. Bachman, Esq.  
Executive Director/Staff Attorney  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

Re: **Notice of Exempt Modification – Facility Modification  
219 New Park Road, Hartford, Connecticut**

Dear Ms. Bachman:

Cellco Partnership d/b/a Verizon Wireless (“Cellco”) currently maintains twelve (12) antennas at the 105-foot level of the existing 108-foot tower at 219 New Park Road in Hartford, Connecticut (the “Property”). The Property is owned by CL&P. The tower is owned by Crown Castle (“Crown”). Cellco’s use of this tower was approved by the Council in 2009 (Petition No. 889). Cellco now intends to modify its facility by replacing six (6) of its existing antennas with three (3) model SBNHH-1D65C, 700/2100 MHz antennas and three (3) model SBNHH-1D65C, 1900 MHz antennas, all at the same level on the tower. Cellco also intends to replace three (3) remote radio heads (“RRHs”), install six (6) new RRHs and install one (1) HYBRIFLEX™ fiber optic antenna cable, attached to the outside the monopole tower. Included in Attachment 1 are specifications for Cellco’s replacement antennas, RRHs and HYBRIFLEX™ cable.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Luke Bronin, Mayor for the City of Hartford; Jamie Bratt, Hartford Director of Planning and Economic Development; CL&P, the Property owner; and Crown, the tower owner.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing tower. Cellco’s replacement antennas and RRHs will be installed on Cellco’s existing platform at the 105-foot level on the existing tower.

17139473-v1

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2. The proposed modifications will not involve any change to ground-mounted equipment and, therefore, will not require the extension of the site boundary.

3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.

4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) safety standard. Far Field Approximation tables for each of Cellco's operating frequencies at the modified facility are included in Attachment 2. These tables indicate that Cellco's modified facility will operate well within the FCC standards for RF emissions.

5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.

6. The tower and its foundation, with certain modifications, can support Cellco's proposed modifications. (See Structural Analysis Report included in Attachment 3).

A copy of the parcel map and property owner information is included in Attachment 4. A Certificate of Mailing verifying that this filing was sent to municipal officials and the Property owner is included in Attachment 5.

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,



Kenneth C. Baldwin

Enclosures

Copy to:

Luke Bronin, Hartford Mayor

Jamie Bratt, Hartford Planning and Economic Development Director

CL&P

Crown

Tim Parks

# **ATTACHMENT 1**



## SBNHH-1D65B

**6-port sector antenna, 2x 698–896 and 4x 1695–2360 MHz, 65° HPBW, 2x RET. Both high bands share the same electrical tilt.**

- Interleaved dipole technology providing for attractive, low wind load mechanical package

### Electrical Specifications

Frequency Band, MHz	698–806	806–896	1695–1880	1850–1990	1920–2200	2300–2360
Gain, dBi	14.9	14.7	17.7	18.2	18.6	18.6
Beamwidth, Horizontal, degrees	68	66	69	66	63	58
Beamwidth, Vertical, degrees	12.1	10.7	5.6	5.2	5.0	4.5
Beam Tilt, degrees	0–14	0–14	0–7	0–7	0–7	0–7
USLS (First Lobe), dB	14	13	15	15	15	13
Front-to-Back Ratio at 180°, dB	27	29	28	28	28	27
Isolation, dB	25	25	25	25	25	25
Isolation, Intersystem, dB	30	30	30	30	30	30
VSWR   Return Loss, dB	1.5   14.0	1.5   14.0	1.5   14.0	1.5   14.0	1.5   14.0	1.5   14.0
PIM, 3rd Order, 2 x 20 W, dBc	-153	-153	-153	-153	-153	-153
Input Power per Port, maximum, watts	350	350	350	350	350	300
Polarization	±45°	±45°	±45°	±45°	±45°	±45°
Impedance	50 ohm	50 ohm	50 ohm	50 ohm	50 ohm	50 ohm

### Electrical Specifications, BASTA\*

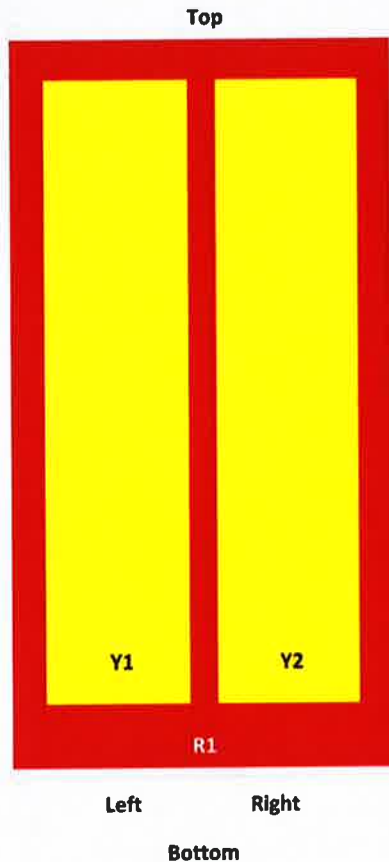
Frequency Band, MHz	698–806	806–896	1695–1880	1850–1990	1920–2200	2300–2360
Gain by all Beam Tilts, average, dBi	14.5	14.3	17.4	17.9	18.2	18.3
Gain by all Beam Tilts Tolerance, dB	±0.5	±0.8	±0.4	±0.3	±0.5	±0.3
Gain by Beam Tilt, average, dBi	0°   14.6	0°   14.5	0°   17.4	0°   17.8	0°   18.1	0°   18.2
	7°   14.6	7°   14.4	3°   17.5	3°   17.9	3°   18.3	3°   18.4
	14°   14.2	14°   13.6	7°   17.4	7°   17.9	7°   18.2	7°   18.4
Beamwidth, Horizontal Tolerance, degrees	±2.2	±3.4	±2	±4.6	±5.7	±4.3
Beamwidth, Vertical Tolerance, degrees	±0.8	±1	±0.3	±0.2	±0.3	±0.2
USLS, beampeak to 20° above beampeak, dB	16	14	16	16	16	15
Front-to-Back Total Power at 180° ± 30°, dB	25	26	27	26	26	26
CPR at Boresight, dB	22	23	21	20	20	22
CPR at Sector, dB	13	11	16	12	11	4

\* CommScope® supports NGMN recommendations on Base Station Antenna Standards (BASTA). To learn more about the benefits of BASTA, [download the whitepaper Time to Raise the Bar on BSAs.](#)

### Array Layout

SBNHH-1D65B

**SBNHH 65**



Array	Freq (MHz)	Ports	RET (MRET)	AISG RET UID
R1	698-896	1-2	1	ANXXXXXXXXXXXXXXXXX 1
Y1	1695-2360	3-4	2	ANXXXXXXXXXXXXXXXXX 2
Y2	1695-2360	5-6		

View from the front of the antenna  
 (Sizes of colored boxes are not true depictions of array sizes)

## General Specifications

Operating Frequency Band	1695 – 2360 MHz   698 – 896 MHz
Antenna Type	Sector
Band	Multiband
Performance Note	Outdoor usage

## Mechanical Specifications

RF Connector Quantity, total	6
RF Connector Quantity, low band	2
RF Connector Quantity, high band	4
RF Connector Interface	7-16 DIN Female

SBNHH-1D65B

Color	Light gray
Grounding Type	RF connector inner conductor and body grounded to reflector and mounting bracket
Radiator Material	Aluminum   Low loss circuit board
Radome Material	Fiberglass, UV resistant
Reflector Material	Aluminum
RF Connector Location	Bottom
Wind Loading, frontal	618.0 N @ 150 km/h 138.9 lbf @ 150 km/h
Wind Loading, lateral	197.0 N @ 150 km/h 44.3 lbf @ 150 km/h
Wind Loading, rear	728.0 N @ 150 km/h 163.7 lbf @ 150 km/h
Wind Speed, maximum	241 km/h   150 mph

## Dimensions

Length	1851.0 mm   72.9 in
Width	301.0 mm   11.9 in
Depth	180.0 mm   7.1 in
Net Weight, without mounting kit	18.4 kg   40.6 lb

## Remote Electrical Tilt (RET) Information

Input Voltage	10–30 Vdc
Internal RET	High band (1)   Low band (1)
Power Consumption, idle state, maximum	2.0 W
Power Consumption, normal conditions, maximum	13.0 W
Protocol	3GPP/AISG 2.0 (Multi-RET)
RET Interface	8-pin DIN Female   8-pin DIN Male
RET Interface, quantity	1 female   1 male

## Packed Dimensions

Length	2025.0 mm   79.7 in
Width	390.0 mm   15.4 in
Depth	296.0 mm   11.7 in
Shipping Weight	31.0 kg   68.3 lb

## Regulatory Compliance/Certifications

Agency	Classification
RoHS 2011/65/EU	Compliant by Exemption
China RoHS SJ/T 11364-2006	Above Maximum Concentration Value (MCV)
ISO 9001:2008	Designed, manufactured and/or distributed under this quality management system



SBNHH-1D65B

## Included Products

BSAMNT-1 — Wide Profile Antenna Downtilt Mounting Kit for 2.4 - 4.5 in (60 - 115 mm) OD round members. Kit contains one scissor top bracket set and one bottom bracket set.

## \* Footnotes

Performance Note      Severe environmental conditions may degrade optimum performance

# ALCATEL-LUCENT B13 RRH4X30-4R

Alcatel-Lucent B13 Remote Radio Head 4x30-4R is the newest addition of Remote Radio Head to the extended product line of Alcatel-Lucent's distributed Base Station solutions, aimed at facilitating smooth RF site acquisition and related civil engineering.

**Supporting 2Tx/4Tx MIMO and 4-way Rx diversity**, Alcatel-Lucent B13 RRH4x30-4R allows operators to have a compact radio solution to deploy LTE in the 700U band (700 MHz, 3GPP band 13), providing them with the means to achieve high capacity, high quality and high coverage with minimum site requirements.

The Alcatel-Lucent B13 RRH4x30-4R product has four transmit RF paths, offering the possibility to **select, via software only, 2Tx or 4Tx MIMO configurations** with either 2x60 W or 4x30 W RF output power. It supports also 4-way Rx diversity and up to 10MHz instantaneous bandwidth.

The Alcatel-Lucent B13 RRH4x30-4R is a near zero-footprint solution and operates noise free, simplifying negotiations with site property owners and minimizing environmental impacts.

Its compactness and slim design makes the Alcatel-Lucent B13 RRH4x30-4R easy to install close to the antenna: operators can therefore locate this Remote Radio Head where RF design conditions are deemed ideal, minimizing trade-offs between available sites and RF optimum sites, together with reducing the RF feeder needs and installation costs.

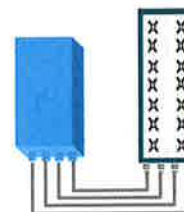


## FEATURES

- Supporting LTE in 700 MHz band (700U, 3GPP band 13)
- LTE 2Tx or 4Tx MIMO (SW switchable)
- Output power: Up to 2x60W or 4x30W
- 10MHz LTE carrier with 4Rx Diversity
- Convection-cooled (fan-less)
- Supports AISG 2.0 ALD devices (RET, TMA) through RS485 or RF ports

## BENEFITS

- Compact to reduce additional footprint when adding LTE in 700U band
- MIMO scheme operation selection (2Tx or 4Tx) by software only
- Improves downlink spectral efficiency through MIMO4
- Increases LTE coverage thanks to 4Rx diversity capability and best in class Rx sensitivity
- Flexible mounting options: Pole or Wall



4x30W with 4T4R  
or  
2x60W with 2T4R  
Can be switched between  
modes via SW w/o site  
visit



## TECHNICAL SPECIFICATIONS

Features & performance	
<b>Number of TX/RX paths</b>	4 duplexed (either 4T4R or 2T4R by SW)
<b>Frequency band</b>	U700 (C) (3GPP bands 13): DL: 746 - 756 MHz / UL: 777 - 787 MHz
<b>Instantaneous bandwidth - #carriers</b>	10MHz – 1 LTE carrier (In 10MHz occupied bandwidth)
<b>LTE carrier bandwidth</b>	10 MHz
<b>RF output power</b>	2x60W or 4x30W (by SW)
<b>Noise figure – RX Diversity scheme</b>	2 dB typ. (<2.5 dB max) – 2 or 4 way Rx diversity
<b>Sizes (HxWxD) in mm (in.)</b>	550 x 305 x 230 (21.6" x 12.0" x 9") (with solar shield)
<b>Volume in L</b>	38 (with solar shield)
<b>Weight in kg (lb) (w/o mounting HW)</b>	26 (57.2) (with solar shield)
<b>DC voltage range</b>	-40.5 to -57V at full performance, -38 to -57V with relaxation on power consumption
<b>DC power consumption</b>	550W typical @100% RF load ( in 2Tx or 4TX mode)
<b>Environmental conditions</b>	-40°C (-40°F) / +55°C (+131°F) IP65
<b>Wind load (@150km/h or 93mph)</b>	Frontal: <200N / Lateral : <150N
<b>Antenna ports</b>	4 ports 7/16 DIN female (50 ohms) VSWR < 1.5
<b>CPRI ports</b>	2 CPRI ports (HW ready for Rate7, 9.8 Gbps) SFP single mode dual fiber
<b>AISG interfaces</b>	1 AISG2.0 output (RS485) Integrated Smart Bias Tees (x2)
<b>Misc. Interfaces</b>	4 external alarms (1 connector) – 4 RF Tx & 4 RF Rx monitor ports - 1 DC connector (2 pins)
<b>Installation conditions</b>	Pole and wall mounting
<b>Regulatory compliance</b>	3GPP 36.141 / 3GPP 36.113 / GR-1089-CORE / GR-3108-CORE / UL 60950-1 / FCC Part 27

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# ALCATEL-LUCENT B25 RRH4X30

Alcatel-Lucent Band 25 Remote Radio Head 4x30W is the new addition of Remote Radio Head to the extended product line of Alcatel-Lucent's distributed Base Station solutions, aimed at facilitating smooth RF site acquisition and related civil engineering.

**Supporting 2Tx/4Tx MIMO and 4-way Rx diversity**, Alcatel-Lucent B25 RRH4x30 allows operators to have a compact radio solution to deploy LTE in the PCS band (1.9 GHz, 3GPP band 25), providing them with the means to achieve high capacity, high quality and high coverage with minimum site requirements.

The Alcatel-Lucent B25 RRH4x30 product has four transmit RF paths, offering the possibility to **select, via software only, 2Tx or 4Tx MIMO configurations** with either 2x60 W or 4x30 W RF output power. It supports also 4-way Rx diversity, LTE carriers from 3 MHz up to 20 MHz and up to 65 MHz instantaneous bandwidth.

The Alcatel-Lucent B25 RRH4x30 is a near zero-footprint solution and operates noise free, simplifying negotiations with site property owners and minimizing environmental impacts.

Its compactness and slim design makes the Alcatel-Lucent B25 RRH4x30 easy to install close to the antenna: operators can therefore locate this Remote Radio Head where RF design conditions are deemed ideal, minimizing trade-offs between available sites and RF optimum sites, together with reducing the RF feeder needs and installation costs.

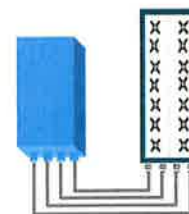


## FEATURES

- Supporting LTE in 1.9 GHz band (PCS, 3GPP band 2 & 25)
- LTE 2Tx or 4Tx MIMO (SW switchable)
- Output power: Up to 2x60W or 4x30W
- Ready for 3, 5, 10, 15 or 20MHz LTE carrier operation with 4Rx Diversity
- Ready to support up to 4 carriers anywhere in 65MHz instantaneous bandwidth
- Convection-cooled (fan-less)
- Supports AISG 2.0 devices (RET, TMA) through RS485 or RF ports

## BENEFITS

- Compact to reduce additional footprint when adding LTE in PCS band
- MIMO scheme operation selection (2Tx or 4Tx) by software only
- Full flexibility for multiple carriers operation over entire PCS spectrum
- Improves downlink spectral efficiency and cell edge throughput through MIMO4
- Increases LTE coverage thanks to 4-way Rx diversity capability and best in class Rx sensitivity
- Flexible mounting options (Pole or Wall)



4x30W with 4T4R  
or  
2x60W with 2T4R

Can be switched between modes via SW w/o site visit

## TECHNICAL SPECIFICATIONS

Features & performance	
<b>Number of TX/RX paths</b>	4 duplexed (either 4T4R or 2T4R by SW)
<b>Frequency band</b>	3GPP bands 2 & 25 (PCS-G) DL: 1930 - 1995 MHz UL: 1850 - 1915 MHz
<b>Instantaneous bandwidth - #carriers</b>	65MHz – Up to 4 LTE carriers (in 40MHz occupied bandwidth)
<b>LTE carrier bandwidth</b>	3, 5, 10, 15 or 20 MHz
<b>RF output power</b>	2x60W or 4x30W (by SW)
<b>Noise figure (3GPP band 2)</b>	2.0 dB typ. (<2.5 dB max)
<b>RX Diversity scheme</b>	2 or 4 way Rx diversity
<b>Sizes (HxWxD)(w/ solar shield) in mm (in.)</b>	538 x 304 x 182 (21.2" x 12.0" x 7.2")
<b>Volume (w/ solar shield) in L</b>	30
<b>Weight (w/ solar shield) in kg (lb)</b>	24 (53)
<b>DC voltage range</b>	-40.5 to -57V at full performance, -38 to -57V with relaxation on power consumption
<b>DC power consumption</b>	580W typical @100% RF load
<b>Environmental conditions</b>	-40°C (-40°F) / +55°C (+131°F) IP65
<b>Wind load (@150km/h or 93mph)</b>	Frontal: <200N / Lateral : <150N
<b>Antenna ports</b>	4 ports 7/16 DIN female (50 ohms) VSWR < 1.5 (> 14dB)
<b>CPRI ports</b>	2 CPRI ports (HW ready for Rate7 / 9.8 Gbps)
<b>AISG interfaces</b>	1 AISG2.0 output (RS485), +24V/2A DC power Integrated Smart Bias Tees (x2)
<b>Misc. Interfaces</b>	1 external alarms connector (4 alarms) 4 RF Tx & 4 RF Rx monitor ports 1 DC connector (2 pins)
<b>Installation conditions</b>	Pole and wall mounting
<b>Regulatory compliance</b>	3GPP 36.141 / 3GPP 36.113 / GR-1089-CORE / GR-3108-CORE / UL 60950-1 / FCC Part 27

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# ALCATEL-LUCENT B66A RRH4X45

The Alcatel-Lucent B66a Remote Radio Head 4x45 is the newest addition of Remote Radio Head to the extended product line of Alcatel-Lucent's distributed Base Station solutions, aimed at facilitating smooth RF site acquisition and related civil engineering. Its operational range covers beyond that of B4 (AWS) and B10 (AWS+).

**Supporting 2Tx/4Tx MIMO and 2-way/4-way Rx diversity**, the Alcatel-Lucent B66a RRH4x45 allows operators to have a compact radio solution to deploy LTE in the 2100 band (3GPP band 4, 10, and 66), providing them with the means to achieve high capacity, high quality, high reliability, large instantaneous bandwidth, and high coverage with minimum site requirements.

The Alcatel-Lucent B66a RRH4x45 product has four transmit RF paths, offering the possibility to **select, via software only, 2Tx or 4Tx MIMO configurations** with either 2x90W or 4x45W RF output power. It also supports 4-way Rx diversity at the 70 MHz instantaneous bandwidth.



The Alcatel-Lucent B66a RRH4x45 is a compact (near zero-footprint) solution and operates noise free, simplifying negotiations with site property owners and minimizing environmental impacts.

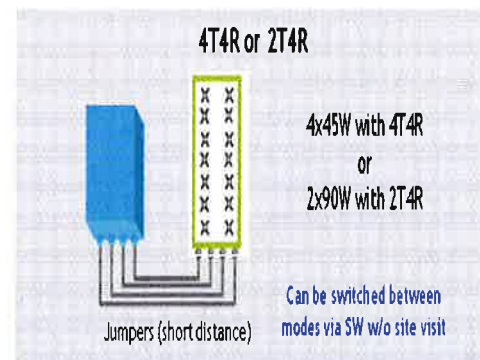
Its compactness and slim design makes the Alcatel-Lucent B66a RRH4x45 easy to install close to the antenna: operators can therefore locate this Remote Radio Head where RF design conditions are deemed ideal, minimizing trade-offs between available sites and RF optimum sites, together with reducing the RF feeder needs and installation costs.

## FEATURES

- Supporting LTE in 2110 - 2180 MHz band/DL, 1710-1780MHz/UL (3GPP band 4, 10, and 66a)
- LTE 2Tx or 4Tx MIMO (SW selectable)
- Configuration: 2T2R/2T4R/4T4R
- Output power: Up to 2x90W or 4x45W (SW configurable)
- 70MHz LTE carrier with 4Rx Diversity
- Convection-cooled (fan-less)
- Supports AISG 2.0 ALD devices (RET, TMA) through RS485 or RF ports

## BENEFITS

- Compact to reduce additional footprint when adding LTE in AWS 1-3 band
- Selection of MIMO configuration (2Tx or 4Tx) by software only
- Improves downlink spectral efficiency through 4Tx MIMO
- Increases LTE coverage thanks to 4Rx diversity capability and best in class Rx sensitivity
- Flexible mounting options: Pole or Wall



## TECHNICAL SPECIFICATIONS

Features & Performance	
<b>Number of TX/RX paths</b>	4 duplexed (either 4T4R or 2T4R selectable by SW)
<b>Frequency band</b>	AWS 1-3, B4/B66a DL: 2110-2180 MHz / UL: 1710-1780 MHz
<b>Instantaneous bandwidth - #carriers</b>	70 MHz – 4 LTE MIMO carriers (In 70 MHz occupied bandwidth)
<b>LTE carrier bandwidth</b>	5, 10, 15, 20 MHz
<b>RF output power</b>	2x90W or 4x45W (selectable by SW)
<b>Noise figure – RX Diversity scheme</b>	2 dB typical (<2.5 dB max) – 2 or 4 way Rx diversity
<b>Receiver Sensivity (FRC A1-3)</b>	-104.5 dBm maximum
<b>Sizes (HxWxD) in mm (in.)</b>	655x299x182 (25.8x11.8x7.2) (with solar shield) 640x290x160 (25.2x11.4x6.3) (without solar shield)
<b>Volume in Liters:</b>	35.5 (with solar shield) 29.7 (without solar shield)
<b>Weight in kg (lb) (w/o mounting HW)</b>	25.8kg (56.8lb) (with solar shield)
<b>DC voltage range</b>	Nominal: -48V, -40.5 to -57V at full performance, -38 to -57V with relaxation on power consumption
<b>DC power consumption</b>	750W typical @100% RF load (In 2Tx or 4Tx mode); Add 58W for 2A*29V for AISG
<b>Environmental conditions</b>	-40°C (-40°F) / +55°C (+131°F) UL50E Type 4 Enclosure
<b>Wind load (@150km/h or 93mph)</b>	250N (56lb) Frontal/150N (34lb) Lateral
<b>Antenna ports</b>	4 ports 4.3-10 female (50 ohms) VSWR < 1.5
<b>CPRI ports</b>	2 CPRI ports (HW ready for Rate 7, 9.8 Gbps) SFP: SMDF (HW supports also SMSF and MMDF)
<b>AISG interfaces</b>	1 AISG 2.0 output (RS485) Integrated Smart Bias Tees (x2)
<b>Misc. Interfaces</b>	4 external alarms (1 connector) 1 DC connector (2 pins)
<b>Installation conditions</b>	Pole and wall mounting
<b>Regulatory compliance</b>	3GPP 36.141 / 3GPP 36.113 / GR-487 / GR-1089-CORE / GR-3108-CORE / UL 60950-1 / FCC Part 27 / FCC Part 15 / GR-3178-CORE

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**HYBRIFLEX™ RRH Hybrid Feeder Cabling Solution, 1-5/8", Single-Mode Fiber**

**Product Description**

RFS' HYBRIFLEX Remote Radio Head (RRH) hybrid feeder cabling solution combines optical fiber and DC power for RRHs in a single lightweight aluminum corrugated cable, making it the world's most innovative solution for RRH deployments.

It was developed to reduce installation complexity and costs at Cellular sites. HYBRIFLEX allows mobile operators deploying an RRH architecture to standardize the RRH installation process and eliminate the need for and cost of cable grounding. HYBRIFLEX combines optical fiber (multi-mode or single-mode) and power in a single corrugated cable. It eliminates the need for junction boxes and can connect multiple RRHs with a single feeder. Standard RFS CELLFLEX® accessories can be used with HYBRIFLEX cable. Both pre-connectorized and on-site options are available.

**Features/Benefits**

- Aluminum corrugated armor with outstanding bending characteristics - minimizes installation time and enables mechanical protection and shielding
- Same accessories as 1 5/8" coaxial cable
- Outer conductor grounding - Eliminates typical grounding requirements and saves on installation costs
- Lightweight solution and compact design - Decreases tower loading
- Robust cabling - Eliminates need for expensive cable trays and ducts
- Installation of tight bundled fiber optic cable pairs directly to the RRH - Reduces CAPEX and wind load by eliminating need for interconnection
- Optical fiber and power cables housed in single corrugated cable - Saves CAPEX by standardizing RRH cable installation and reducing installation requirements
- Outdoor polyethylene jacket - Ensures long-lasting cable protection

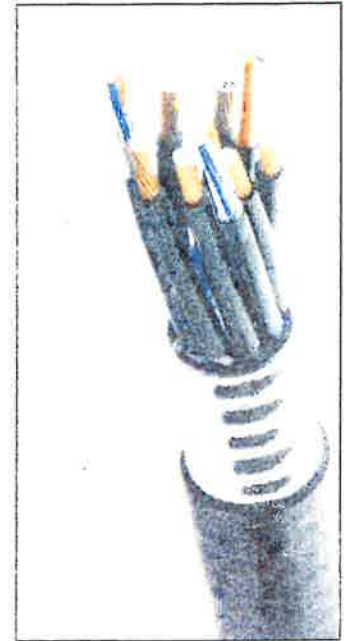


Figure 1: HYBRIFLEX Series

**Technical Specifications**

<b>Dimensions</b>			
Outer Conductor Armor	Corrugated Aluminum	[mm (in.)]	46.5 (1.83)
Jacket	Polyethylene, PE	[mm (in.)]	50.3 (1.98)
UV-Protection	Individual and External Jacket		Yes
<b>Weight and Bending</b>			
Weight, Approximate		[kg/m (lb/ft)]	1.9 (1.30)
Minimum Bending Radius, Single Bending		[mm (in.)]	200 (8)
Minimum Bending Radius, Repeated Bending		[mm (in.)]	500 (20)
Recommended/Maximum Clamp Spacing		[m (ft)]	1.0 / 1.2 (3.25 / 4.0)
<b>Electrical Properties</b>			
DC-Resistance Outer Conductor Armor		[Ω/km (Ω/1000ft)]	0.68 (0.205)
DC-Resistance Power Cable, 8.4mm <sup>2</sup> (8AWG)		[Ω/km (Ω/1000ft)]	2.1 (0.307)
<b>Other Properties</b>			
Version			Single-mode OM3
Quantity, Fiber Count			16 (8 pairs)
Core/Clad		[μm]	50/125
Primary Coating (Acrylate)		[μm]	245
Buffer Diameter, Nominal		[μm]	900
Secondary Protection, Jacket, Nominal		[mm (in.)]	2.0 (0.08)
Minimum Bending Radius		[mm (in.)]	104 (4.1)
Insertion Loss @ wavelength 850nm		dB/km	3.0
Insertion Loss @ wavelength 1310nm		dB/km	1.0
Standards (Meets or exceeds)			UL94-V0, UL1666 RoHS Compliant
<b>DC Power Cable Properties</b>			
Size (Power)		[mm (AWG)]	8.4 (8)
Quantity, Wire Count (Power)			16 (8 pairs)
Size (Alarm)		[mm (AWG)]	0.8 (18)
Quantity, Wire Count (Alarm)			4 (2 pairs)
Type			UV protected
Strands			19
Primary Jacket Diameter, Nominal		[mm (in.)]	6.8 (0.27)
Standards (Meets or exceeds)			NFPA 130, IEC 60332-1-2 UL Type XHHW-2, UL 44 UL-LS Limited Smoke, UL VW-1 IEEE-383 (1974), IEEE1202/FT4 RoHS Compliant
<b>Operating Temperature</b>			
Installation Temperature		[°C (°F)]	-40 to +65 (-40 to 149)
Operation Temperature		[°C (°F)]	-40 to +65 (-40 to 149)

\* This data is provisional and subject to change

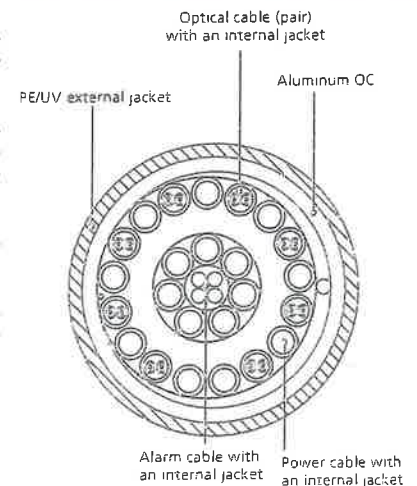


Figure 2: Construction Detail

All information contained in the present datasheet is subject to confirmation at time of ordering.

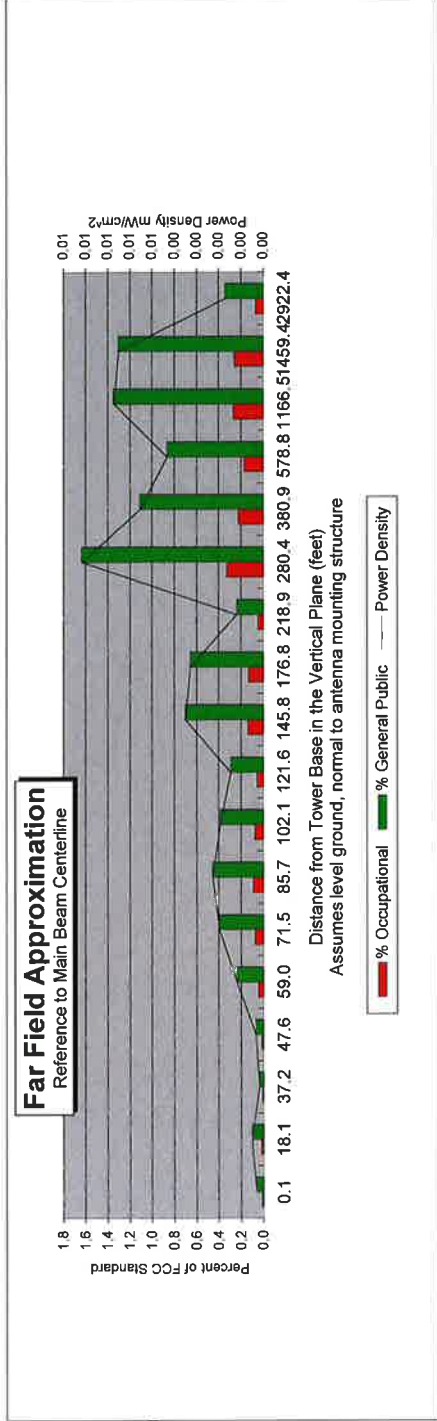
# **ATTACHMENT 2**

Far Field Approximation  
with downtilt variation

**Estimated Radiated Emission  
Single Emitter Far Field Model  
Dipole / Wire/ Yagi Antenna Types**



Location:	West Hartford 4, CT
Site #:	
Date:	09/26/17
Name:	Mark Brauer
File Name:	West Hartford 4, CT - FF Powe
Operating Freq. (MHz)	746.0
Antenna Height (ft)	105.0
Antenna Gain (dBi)	14.8
Antenna Size (in.)	72.0
Downtilt (degrees)	0.0
Feedline Loss (dB)	0.0
Power @ J4 (w)	2200.0
Number of Channels	1



Calc Angle	90.0	80.0	70.0	65.0	60.0	55.0	50.0	45.0	40.0	35.0	30.0	25.0	20.0	15.0	10.0	5.0	4.0	2.0
Solve for r. dx to antenna	102.0	103.6	108.6	112.6	117.8	124.6	133.2	144.3	158.8	177.9	204.1	241.5	298.4	394.3	587.7	1170.9	1463.0	2924.2
Distance from Antenna Structure Base in Horizontal plane	0.1	18.1	37.2	47.6	59.0	71.5	85.7	102.1	121.6	145.8	176.8	218.9	280.4	380.9	578.8	1166.5	1459.4	2922.4
Angle from Main Beam (reference to horizontal plane)	90	80	70	65	60	55	50	45	40	35	30	25	20	15	10	5	4	2
dB down from centerline (referenced to centerline)	36.76	34.35	38.52	35.34	29.54	26.8	25.59	25.63	25.99	21.21	20.29	23.24	13.03	12.3	9.92	2	0.2	0
Reflection Coefficient (1 to 4, 2.56 typical)	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56
Power Density (mW/cm²)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.00
Percent of Occupational Standard	0.0	0.0	0.0	0.0	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.0	0.3	0.2	0.2	0.3	0.3	0.1
Percent of General Population Standard	0.1	0.1	0.0	0.1	0.2	0.4	0.5	0.4	0.3	0.7	0.7	0.2	1.6	1.1	0.9	1.3	1.3	0.3

Antenna Type: SBNHH-1D65B  
Max%: 1.64%  
Instructions:

- 1) Fill in Site Location, Site number, Date, Name of Person Responsible for Date, and enter File Name to be saved as.
- 2) References to J4 refer to a point where the transmission line exits the equipment shelter and proceeds to the antenna(s). There is typically a connector located here where power measurements are made.
- 3) Enter Antenna Height (in feet to bottom of antenna), Antenna Gain (expressed as dBi, add 2.17 to dBi to obtain dBi), Antenna Size (vertical size in inches), Downtilt (in Degrees, enter zero if none), Feedline loss from J4 to Antenna, and J4 Power Density.
- 4) From manufacturer's plots, or data sheet, input Angle from mainbeam and dB below mainbeam centerline.
- 5) Enter Reflection coefficient (2.56 would be typical, 1 for free space)
- 6) Spreadsheet calculates actual power density, then relates as Occupational or General Population percentage of FCC Standard.
- 7) An odd distance may be entered in the rightmost column of the tower table.

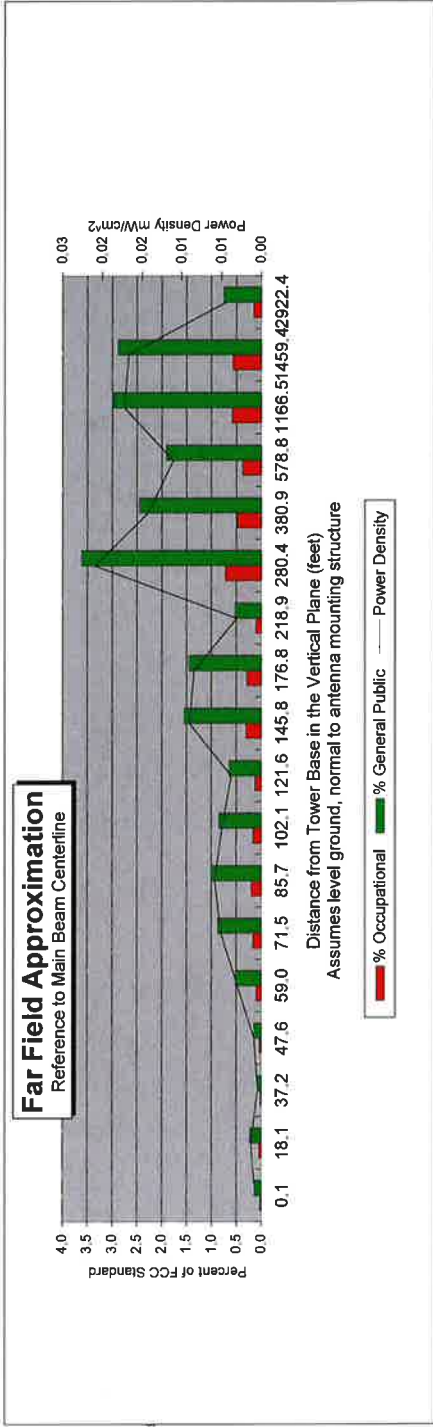


Far Field Approximation  
with downtilt variation

**Estimated Radiated Emission  
Single Emitter Far Field Model  
Dipole / Wire/ Yagi Antenna Types**



Location:	West Hartford 4, CT
Site #:	
Date:	09/26/17
Name:	Mark Brauer
File Name:	West Hartford 4, CT - FF Power
Operating Freq. (MHz)	869.0
Antenna Height (ft)	105.0
Antenna Gain (dBi)	16.7
Antenna Size (in.)	72.0
Downtilt (degrees)	0.0
Feedline Loss (dB)	0.0
Power @ J4 (w)	3690.0
Number of Channels	9



Calc. Angle	90.0	80.0	70.0	65.0	60.0	55.0	50.0	45.0	40.0	35.0	30.0	25.0	20.0	15.0	10.0	5.0	4.0	2.0
Solve for r. dx to antenna	102.0	103.6	108.6	112.6	117.8	124.6	133.2	144.3	158.8	177.9	204.1	241.5	298.4	394.3	587.7	1170.9	1463.0	2924.2
Distance from Antenna Structure Base in Horizontal plane	0.1	18.1	37.2	47.6	59.0	71.5	85.7	102.1	121.6	145.8	176.8	218.9	280.4	380.9	578.8	1166.5	1459.4	2922.4
Angle from Main Beam (reference to horizontal plane)	90	80	70	65	60	55	50	45	40	35	30	25	20	15	10	5	4	2
dB down from centerline (referenced to centerline)	36.76	34.35	38.52	35.34	29.54	26.8	25.59	25.63	25.99	21.21	20.29	23.24	13.03	12.3	9.92	2	0.2	0
Reflection Coefficient (1 to 4, 2.56 typical)	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56
Power Density (mW/cm²)	0.00	0.00	0.00	0.00	0.00	0.01	0.01	0.00	0.00	0.01	0.01	0.00	0.02	0.01	0.01	0.01	0.02	0.00
Percent of Occupational Standard	0.0	0.0	0.0	0.0	0.1	0.2	0.2	0.2	0.1	0.3	0.3	0.1	0.7	0.5	0.4	0.6	0.6	0.2
Percent of General Population Standard	0.1	0.2	0.1	0.1	0.5	0.9	1.0	0.8	0.6	1.5	1.5	0.5	3.6	2.4	1.9	3.0	2.9	0.8

Distance in feet below:

Antenna Type BXA-70063-6  
Max% 3.61%  
Instructions:

- 1) Fill in Site Location, Site number, Date, Name of Person Responsible for Date, and enter File Name to be saved as.
- 2) References to J4 refer to a point where the transmission line exits the equipment shelter and proceeds to the antenna(s). There is typically a connector located here where power measurements are made.
- 3) Enter Antenna Height (in feet to bottom of antenna), Antenna Gain (expressed as dBi, add 2.17 to dBd to obtain dBi), Antenna Size (vertical size in inches), Downtilt (in Degrees, enter zero if none), Feedline loss from J4 to Antenna, and J4 Power.
- 4) From manufacturer's plots, or data sheet, input Angle from mainbeam and dB below mainbeam centerline.
- 5) Enter Reflection coefficient (2.56 would be typical, 1 for free space)
- 6) Spreadsheet calculates actual power density, then relates as Occupational or General Population percentage of FCC Standard.
- 7) An odd distance may be entered in the rightmost column of the lower table.

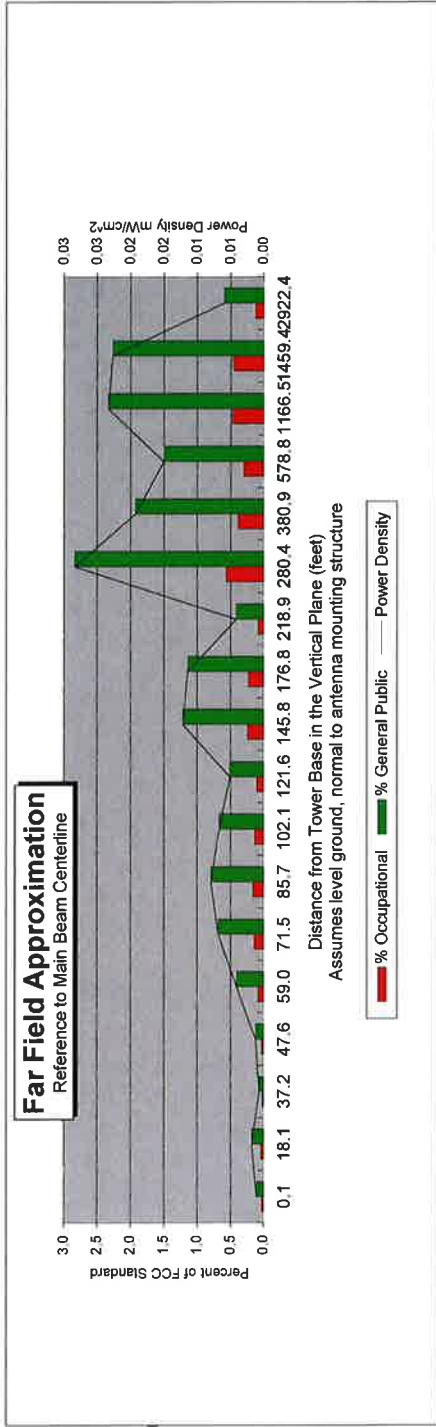
Far Field Approximation  
with downtilt variation

**Estimated Radiated Emission**  
**Single Emitter Far Field Model**  
**Dipole / Wire/ Yagi Antenna Types**



Location: West Hartford 4, CT  
 Site #:   
 Date: 09/26/17  
 Name: Mark Brauer  
 File Name: West Hartford 4, CT - FF Powe

Operating Freq. (MHz) **1970.0**  
 Antenna Height (ft): **105.0**  
 Antenna Gain (dBi): **18.3**  
 Antenna Size (in.): **72.0**  
 Downtilt (degrees): **0.0**  
 Feedline Loss (dB): **0.0**  
 Power @ J4 (w): **5000.0**  
 Number of Channels **1**



Calc Angle	90.0	80.0	70.0	65.0	60.0	55.0	50.0	45.0	40.0	35.0	30.0	25.0	20.0	15.0	10.0	5.0	4.0	2.0
Solve for r, dx to antenna	102.0	103.6	108.6	112.6	117.8	124.6	133.2	144.3	158.8	177.9	204.1	241.5	298.4	394.3	587.7	1170.9	1463.0	2924.2
Distance from Antenna Structure Base in Horizontal plane	0.1	18.1	37.2	47.6	59.0	71.5	85.7	102.1	121.6	145.8	176.8	218.9	280.4	380.9	578.8	1166.5	1459.4	2922.4
Angle from Main Beam (reference to horizontal plane)	90	80	70	65	60	55	50	45	40	35	30	25	20	15	10	5	4	2
dB down from centerline (referenced to centerline)	36.76	34.35	38.52	35.34	29.54	26.8	25.59	25.63	25.99	21.21	20.29	23.24	13.03	12.3	9.92	2	0.2	0
Reflection Coefficient (1 to 4, 2.56 typical)	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56
Power Density (mW/cm²)	0.00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.03	0.02	0.01	0.02	0.02	0.01
Percent of Occupational Standard	0.0	0.0	0.0	0.0	0.1	0.1	0.2	0.1	0.1	0.2	0.2	0.1	0.6	0.4	0.3	0.5	0.5	0.1
Percent of General Population Standard	0.1	0.2	0.1	0.1	0.4	0.7	0.8	0.7	0.5	1.2	1.1	0.4	2.8	1.9	1.5	2.3	2.3	0.6

Antenna Type SBNHH-1D65B  
 Max% 2.84%  
 Instructions:

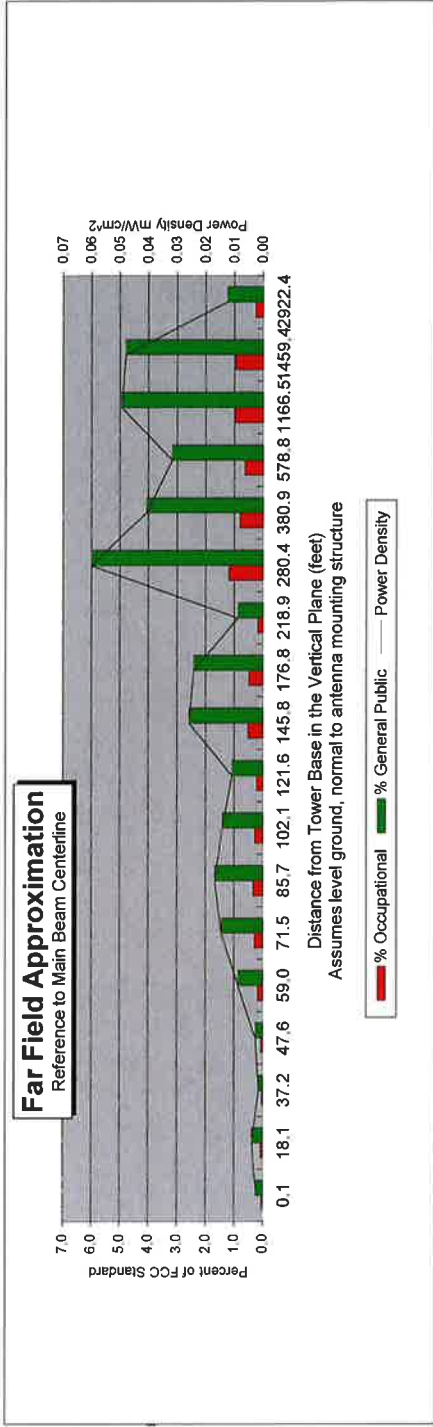
- 1) Fill in Site Location, Site number, Date, Name of Person Responsible for Date, and enter File Name to be saved as.
- 2) References to J4 refer to a point where the transmission line exits the equipment shelter and proceeds to the antenna(s). There is typically a connector located here where power measurements are made.
- 3) Enter Antenna Height (in feet to bottom of antenna), Antenna Gain (expressed as dBi, add 2.17 to dBi to obtain dBi), Antenna Size (vertical size in inches), Downtilt (in Degrees, enter zero if none), Feedline loss from J4 to Antenna, and J4 Pov
- 4) From manufacturer's plots, or data sheet, input Angle from mainbeam and dB below mainbeam centerline.
- 5) Enter Reflection coefficient (2.56 would be typical, 1 for free space)
- 6) Spreadsheet calculates actual power density, then relates as Occupational or General Population percentage of FCC Standard.
- 7) An odd distance may be entered in the rightmost column of the lower table.

Far Field Approximation with downtilt variation

**Estimated Radiated Emission  
Single Emitter Far Field Model  
Dipole / Wire/ Yagi Antenna Types**



Location:	West Hartford 4, CT
Site #:	
Date:	09/26/17
Name:	Mark Brauer
File Name:	West Hartford 4, CT - FF Power
Operating Freq. (MHz)	2110.0
Antenna Height (ft)	105.0
Antenna Gain (dBi)	18.2
Antenna Size (in.)	72.0
Downtilt (degrees)	0.0
Feedline Loss (dB)	0.0
Power @ J4 (w)	7400.0
Number of Channels	1



	90.0	80.0	70.0	65.0	60.0	55.0	50.0	45.0	40.0	35.0	30.0	25.0	20.0	15.0	10.0	5.0	4.0	2.0
Calc. Angle	102.0	103.6	108.6	112.6	117.8	124.6	133.2	144.3	158.8	177.9	204.1	241.5	298.4	394.3	587.7	1170.9	1463.0	2924.2
Solve for r. dx to antenna																		
Distance from Antenna Structure Base in Horizontal plane	0.1	18.1	37.2	47.6	59.0	71.5	85.7	102.1	121.6	145.8	176.8	218.9	280.4	380.9	578.8	1166.5	1459.4	2922.4
Angle from Main Beam (reference to horizontal plane)	90	80	70	65	60	55	50	45	40	35	30	25	20	15	10	5	4	2
dB down from centerline (referenced to centerline)	36.76	34.35	38.52	35.34	29.54	26.8	25.59	25.63	25.99	21.21	20.29	23.24	13.03	12.3	9.92	2	0.2	0
Reflection Coefficient (1 to 4, 2.56 typical)	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56	2.56
Power Density (mW/cm <sup>2</sup> )	0.00	0.00	0.00	0.00	0.01	0.01	0.02	0.01	0.01	0.03	0.02	0.01	0.06	0.04	0.03	0.05	0.05	0.01
Percent of Occupational Standard	0.0	0.1	0.0	0.0	0.2	0.3	0.3	0.3	0.2	0.5	0.5	0.2	1.2	0.8	0.6	1.0	1.0	0.3
Percent of General Population Standard	0.2	0.4	0.1	0.2	0.9	1.4	1.7	1.4	1.1	2.6	2.4	0.9	6.0	4.1	3.2	4.9	4.8	1.3

Antenna Type: SBNHH-1D65B  
Max%: 6.00%  
Instructions:

- 1) Fill in Site Location, Site number, Date, Name of Person Responsible for Date, and enter File Name to be saved as.
- 2) References to J4 refer to a point where the transmission line exits the equipment shelter and proceeds to the antenna(s). There is typically a connector located here where power measurements are made.
- 3) Enter Antenna Height (in feet to bottom of antenna), Antenna Gain (expressed as dBi, add 2.17 to dBd to obtain dBi), Antenna Size (vertical size in inches), Downtilt (in Degrees, enter zero if none), Feedline loss from J4 to Antenna, and J4 Power Density.
- 4) From manufacturer's plots, or data sheet, input Angle from mainbeam and dB below mainbeam centerline.
- 5) Enter Reflection coefficient (2.56 would be typical, 1 for free space)
- 6) Spreadsheet calculates actual power density, then relates as Occupational or General Population percentage of FCC Standard.
- 7) An odd distance may be entered in the rightmost column of the lower table.

# **ATTACHMENT 3**



May 16, 2017

Mr. Timothy Howell  
Crown Castle  
3530 Toringdon Way Suite 300  
Charlotte, NC 28277  
(980) 209 - 8242

B+T Group  
1717 S. Boulder, Suite 300  
Tulsa, OK 74119  
(918) 587-4630  
ModDwgs@btgrp.com

**Subject: Structural Modification Report**

**Carrier Designation:** **Verizon Wireless Co-Locate**  
**Carrier Site Number:** 118004  
**Carrier Site Name:** West Hartford 4

**Crown Castle Designation:** **Crown Castle BU Number:** 876363  
**Crown Castle Site Name:** Hartford - NU (SSUSA)  
**Crown Castle JDE Job Number:** 428941  
**Crown Castle Work Order Number:** 1385488  
**Crown Castle Application Number:** 382014 Rev. 1

**Engineering Firm Designation:** **B+T Group Project Number:** 85565.006.01

**Site Data:** **219 New Park Rd., Hartford, CT, Hartford County**  
**Latitude 41° 45' 2.79", Longitude -72° 42' 49.23"**  
**108 Ft - Monopole**

Dear Mr. Howell,

B+T Group is pleased to submit this "Structural Modification Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 1019805, in accordance with application 382014, revision 1.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC4.5: TSA specified load case with proposed modifications

**Sufficient Capacity**

Note: See Table 1 and Table 2 for the proposed and existing loading, respectively.

This analysis has been performed in accordance with the 2016 Connecticut State Building Code based upon an ultimate 3-second gust wind speed of 125 mph converted to a nominal 3-second gust wind speed of 97 mph per Section 1609.3 and Appendix N as required for use in the TIA-222-G Standard per Exception #5 of Section 1609.1.1. Exposure Category C and Risk Category II were used in this analysis.

All modifications and equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

We at B+T Group appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted by:  
B+T Engineering, Inc.  
PEC.0001564; Exp: 02/10/18

Robert M. Frazier, P.E.  
Project Engineer

Scott S. Vance, P.E.  
Engineer of Record



5/16/17

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Tower Modification Drawings

## 1) INTRODUCTION

This is a 108 ft. monopole designed by Summit in October of 2000. The monopole was originally designed for a wind speed of 90 mph per TIA/EIA-222-F. This monopole has been modified by Vertical Solutions in January of 2009, PJF in May of 2010 and B+T group in October of 2012 and those modifications were incorporated in this analysis.

## 2) ANALYSIS CRITERIA

The structural analysis was performed for this monopole in accordance with the requirements of TIA-222-G Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a 3-second gust wind speed of 97 mph with no ice, 50 mph with 1 inch ice thickness and 60 mph under service loads, exposure category C.

**Table 1 - Proposed Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
105.0	105.0	3	Alcatel Lucent	B13 RRH 4X30	1	1-5/8	--
		3	Alcatel Lucent	B25 RRH4X30			
		3	Alcatel Lucent	RRH4X45-AWS4 B66			
		6	Commscope	SBNHH-1D65B			
		1	RFS Celwave	DB-T1-6Z-8AB-0Z			

**Table 2 - Existing Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
105.0	105.0	3	Alcatel Lucent	RRH2X40-AWS	6	1-1/4	3
		3	Antel	BXA-171063-12BF			
		3	Antel	BXA-70063/6CF			
		3	Antel	BXA-171063-12BF	12	1-5/8 1-1/4	1
		3	Antel	BXA-70063/6CF			
		1	RFS Celwave	DB-T1-6Z-8AB-0Z			
		1	--	Sector Mount [SM 402-3]			
98.0	102.0	1	Dragonwave	A-ANT-18G-2-C	3	5/16 1/2 5/8 1-1/4	1
		1	Dragonwave	A-ANT-23G-1-C			
		2	Dragonwave	HORIZON COMPACT			
	99.0	3	Alcatel Lucent	TD-RRH8x20-25			
		3	Argus	LLPX310R			
		1	RFS Celwave	APXV9ERR18-C-A20			
		2	RFS Celwave	APXVSPP18-C-A20			
		3	RFS Celwave	APXVTM14-C-120			
		3	RFS Celwave	IBC1900BB-1			
		3	RFS Celwave	IBC1900HG-2A			
		3	Samsung	WIMAX DAP HEAD			
98.0	1	--	Platform Mount [LP 1201-1]				

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
96.0	96.0	3	Alcatel Lucent	800MHz 2X50W RRH W/Filter	--	--	1
		3	Alcatel Lucent	PCS 1900MHz 4x45W-65MHz			
	1	--	Side Arm Mount [SO 102-3]				
	95.0	3	Alcatel Lucent	PCS 1900MHz 4x45W-65MHz			
88.0	88.0	12	Decibel	844G65VTZAS	--	--	2
		1	--	Platform Mount [LP 304-1]			
81.0	81.0	1	--	T-Arm Mount [TA 602-3]	6	7/8	1
	80.0	3	Andrew	HBX-6516DS-VTM	1	5/16	
74.0	76.0	1	Lucent	KS24019-L112A	1	1/2	1
	74.0	1	--	Side Arm Mount [SO 701-1]			

- Notes:  
 1) Existing Equipment  
 2) Abandoned Equipment To Be Removed.  
 3) Equipment To Be Removed

**Table 3 - Design Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
98.0	98.0	1	--	14' Low Profile Platform	--	--
		12	Dapa	48000 PCS Panel		
88.0	88.0	1	--	14' Low Profile Platform	--	--
		12	Dapa	48000 PCS Panel		
76.0	76.0	1	GPS	Antenna w/ Mount	--	--



### 3) ANALYSIS PROCEDURE

**Table 4 - Documents Provided**

Document	Remarks	Reference	Source
Online Application	Verizon Wireless Co-Locate Rev#1	382014	CCI Sites
Tower Manufacturer Drawing	Summit, Job No. 11049	1947570	CCI Sites
Foundation Drawings	Summit, Job No. 11049	1613616	CCI Sites
Geotech Report	FDH, Project No. 08-10012E G1	2337384	CCI Sites
Tower Modification Drawings	VSI, Date: 01/28/09	2445632	CCI Sites
	PJF, Project No. 67310-0013	2445631	CCI Sites
	B+T Group, Project No. 85565.001	3348853	CCI Sites
Modification Inspection Report	PJF, Project No. 67309-0057	2445632	CCI Sites
	PJF, Project No. 67310-0013	2445631	CCI Sites
	TEP, Project No. 128633	4424435	CCI Sites
Antenna Configuration	Previous SA by TEP, Project No. 25682.110529	6732835	CCI Sites

#### 3.1) Analysis Method

tnxTower (version 7.0.5.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

#### 3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) Mount areas and weights are assumed based on photographs provided.

This analysis may be affected if any assumptions are not valid or have been made in error. B+T Group should be notified to determine the effect on the structural integrity of the tower.

#### 4) ANALYSIS RESULTS

**Table 5 - Section Capacity (Summary) - LC4.5**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	108 - 98.5	Pole	TP8.625x8.625x0.313	1	-1.998	-	71.8	Pass <sup>1</sup>
L2	98.5 - 98	Pole	TP16.5x8.625x0.313	2	-2.008	-	22.8	Pass <sup>1</sup>
L3	98 - 82.33	Pole	TP19.008x16.5x0.188	3	-7.228	-	77.5	Pass <sup>1</sup>
L4	82.33 - 76.25	Pole	TP19.981x19.008x0.323	4	-8.953	-	83.2	Pass <sup>1</sup>
L5	76.25 - 74.5	Pole	TP20.261x19.981x0.462	5	-9.239	-	69.9	Pass <sup>1</sup>
L6	74.5 - 58.08	Pole	TP22.89x20.261x0.537	6	-12.282	-	92.1	Pass <sup>1</sup>
L7	58.08 - 57.75	Pole	TP22.942x22.89x0.698	7	-12.365	-	73.3	Pass <sup>1</sup>
L8	57.75 - 47	Pole	TP24.663x22.942x0.683	8	-14.026	-	78.1	Pass <sup>1</sup>
L9	47 - 40.5	Pole	TP25.328x23.768x0.723	9	-17.079	-	84.2	Pass <sup>1</sup>
L10	40.5 - 27.75	Pole	TP27.369x25.328x0.677	10	-20.428	-	97.4	Pass <sup>1</sup>
L11	27.75 - 19.5	Pole	TP28.689x27.369x0.689	11	-22.796	-	92.1	Pass <sup>1</sup>
L12	19.5 - 14	Pole	TP29.569x28.689x0.799	12	-24.493	-	90.5	Pass <sup>1</sup>
L13	14 - 13.08	Pole	TP29.717x29.569x0.787	13	-24.800	-	91.0	Pass <sup>1</sup>
L14	13.08 - 13	Pole	TP29.729x29.717x0.752	14	-24.837	-	94.1	Pass <sup>1</sup>
L15	13 - 0	Pole	TP31.81x29.729x0.762	15	-29.247	-	94.9	Pass <sup>1</sup>
							Summary	
						Pole (L10)	97.4	Pass <sup>1</sup>
						<b>RATING =</b>	<b>97.4</b>	<b>Pass<sup>1</sup></b>

**Table 6 - Tower Component Stresses vs. Capacity - LC4.5**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Flange Bolts	98.0	49.2	Pass
1	Flange Plate	98.0	71.6	Pass
1	Anchor Rods	Base	81.5	Pass
1	Base Plate	Base	58.1	Pass
1	Base Foundation (Soil Interaction)	Base	52.5	Pass
1	Base Foundation (Steel)	Base	92.1	Pass

<b>Structure Rating (max from all components) =</b>	<b>97.4%</b>
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Notes:

- 1) See additional documentation in "Appendix C - Additional Calculations" for calculations supporting the % capacity listed.

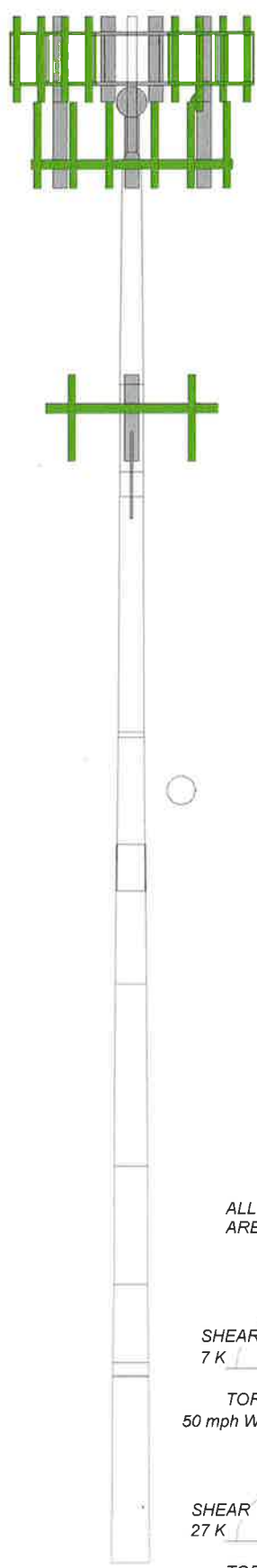
#### 4.1) Recommendations

- 1) All modifications proposed in this report shall be installed in accordance with the attached drawings (Appendix D) for the determined available structural capacity to be effective.

**APPENDIX A**

**tnxTOWER OUTPUT**

Section	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1
Length (ft)	13.000	009640	5.500	8.250	12.750	10.750	0.330	16.420	1.750	6.080	15.670	0.500	9.500		
Number of Sides	18	18	18	18	18	18	18	18	18	18	18	18	18	18	18
Thickness (in)	0.762	007387	0.799	0.689	0.677	0.663	0.698	0.537	0.462	0.323	0.188	0.313	0.313		
Socket Length (ft)						3.250									
Top Dia (in)	29.729	29.7569	28.689	27.369	25.328	22.942	22.890	20.261	19.981	19.008	16.500	8.625	8.625		
Bot Dia (in)	31.810	29.7297	29.569	28.689	27.369	24.863	22.942	22.890	20.261	19.981	16.500	8.625	8.625		
Grade	37.631628694	32.328355558	25.6421569	22.864621ksi	36.688641ksi	34.946983ksi	35.199151ksi	38.234343ksi	43.351692ksi	A607-65	A53-B-35				
Weight (K)	15.3	0002	1.2	1.6	2.3	1.7	0.1	2.0	0.2	0.4	0.6	0.0	0.3		



### DESIGNED APPURTENANCE LOADING

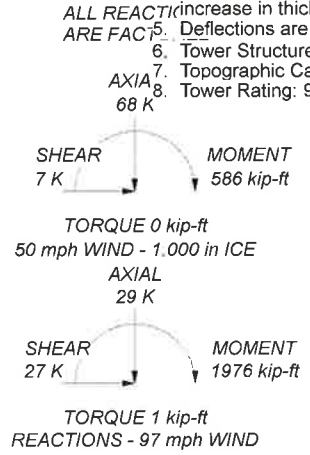
TYPE	ELEVATION	TYPE	ELEVATION
BXA-171063-12BF w/ Mount Pipe (E)	105	IBC1900HG-2A (E)	98
BXA-171063-12BF w/ Mount Pipe (E)	105	IBC1900HG-2A (E)	98
BXA-171063-12BF w/ Mount Pipe (E)	105	IBC1900HG-2A (E)	98
BXA-70063/6CF w/ Mount Pipe (E)	105	HORIZON COMPACT (E-Clearwire)	98
BXA-70063/6CF w/ Mount Pipe (E)	105	HORIZON COMPACT (E-Clearwire)	98
BXA-70063/6CF w/ Mount Pipe (E)	105	WIMAX DAP HEAD (E-Clearwire)	98
DB-T1-6Z-8AB-0Z (E)	105	WIMAX DAP HEAD (E-Clearwire)	98
(2) SBNHH-1D65B (P)	105	WIMAX DAP HEAD (E-Clearwire)	98
(2) SBNHH-1D65B (P)	105	TD-RRH8x20-25 (E)	98
(2) SBNHH-1D65B (P)	105	TD-RRH8x20-25 (E)	98
B13 RRH 4X30 (P)	105	TD-RRH8x20-25 (E)	98
B13 RRH 4X30 (P)	105	Platform Mount [LP 1201-1] (E)	98
B13 RRH 4X30 (P)	105	A-ANT-18G-2-C (E)	98
DB-T1-6Z-8AB-0Z (P)	105	A-ANT-23G-1-C (E)	98
RRH4X45-AWS4 B66 (P)	105	PCS 1900MHz 4x45W-65MHz (E)	96
RRH4X45-AWS4 B66 (P)	105	PCS 1900MHz 4x45W-65MHz (E)	96
RRH4X45-AWS4 B66 (P)	105	PCS 1900MHz 4x45W-65MHz (E)	96
B25 RRH4X30 (P)	105	PCS 1900MHz 4x45W-65MHz (E)	96
B25 RRH4X30 (P)	105	800MHz 2X50W RRH W/FILTER (E)	96
B25 RRH4X30 (P)	105	800MHz 2X50W RRH W/FILTER (E)	96
Secor Mount [SM 402-3] (E)	105	800MHz 2X50W RRH W/FILTER (E)	96
APXV9ERR18-C-A20 w/ Mount Pipe (E)	98	4' x 2" Pipe Mount (E)	96
APXV9ERR18-C-A20 w/ Mount Pipe (E)	98	4' x 2" Pipe Mount (E)	96
APXVSP18-C-A20 w/ Mount Pipe (E)	98	4' x 2" Pipe Mount (E)	96
APXVSP18-C-A20 w/ Mount Pipe (E)	98	Side Arm Mount [SO 102-3] (E)	96
LLPX310R w/ Mount Pipe (E-Clearwire)	98	PCS 1900MHz 4x45W-65MHz (E)	96
LLPX310R w/ Mount Pipe (E-Clearwire)	98	PCS 1900MHz 4x45W-65MHz (E)	96
LLPX310R w/ Mount Pipe (E-Clearwire)	98	HBX-6516DS-VTM w/ Mount Pipe (E)	81
LLPX310R w/ Mount Pipe (E-Clearwire)	98	6' x 2" Mount Pipe (E)	81
LLPX310R w/ Mount Pipe (E-Clearwire)	98	6' x 2" Mount Pipe (E)	81
LLPX310R w/ Mount Pipe (E-Clearwire)	98	6' x 2" Mount Pipe (E)	81
APXVTM14-C-120 w/ Mount Pipe (E)	98	T-Arm Mount [TA 602-3] (E)	81
APXVTM14-C-120 w/ Mount Pipe (E)	98	HBX-6516DS-VTM w/ Mount Pipe (E)	81
APXVTM14-C-120 w/ Mount Pipe (E)	98	HBX-6516DS-VTM w/ Mount Pipe (E)	81
IBC1900BB-1 (E)	98	KS24019-L112A (E)	74
IBC1900BB-1 (E)	98	Side Arm Mount [SO 701-1] (E)	74


### MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A53-B-35	35 ksi	63 ksi	36.593546ksi	37 ksi	52 ksi
A607-65	65 ksi	80 ksi	36.688841ksi	37 ksi	52 ksi
43.351692ksi	43 ksi	58 ksi	39.884621ksi	40 ksi	55 ksi
38.234343ksi	38 ksi	53 ksi	39.613462ksi	40 ksi	55 ksi
35.199151ksi	35 ksi	50 ksi	36.758558ksi	39 ksi	54 ksi
34.946983ksi	35 ksi	50 ksi	37.349432ksi	37 ksi	52 ksi
36.657402ksi	37 ksi	52 ksi	37.631626ksi	38 ksi	53 ksi

### TOWER DESIGN NOTES

1. Tower is located in Hartford County, Connecticut.
2. Tower designed for Exposure C to the TIA-222-G Standard.
3. Tower designed for a 97 mph basic wind in accordance with the TIA-222-G Standard.
4. Tower is also designed for a 50 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Structure Class II.
7. Topographic Category 1 with Crest Height of 0.000 ft
8. Tower Rating: 97.4%





**B+T Group**  
1717 S Boulder, Suite 300  
Tulsa, OK 74119  
Phone: (918) 587-4630  
FAX: (918) 587-4630

Job: **85565.006.01 - HARTFORD - NU (SSUSA), CT (BU# 8763)**

Project: \_\_\_\_\_

Client: Crown Castle    Drawn by: CJangonda    App'd: \_\_\_\_\_

Code: TIA-222-G    Date: 05/02/17    Scale: NTS

Path: \_\_\_\_\_    Dwg No: E-1

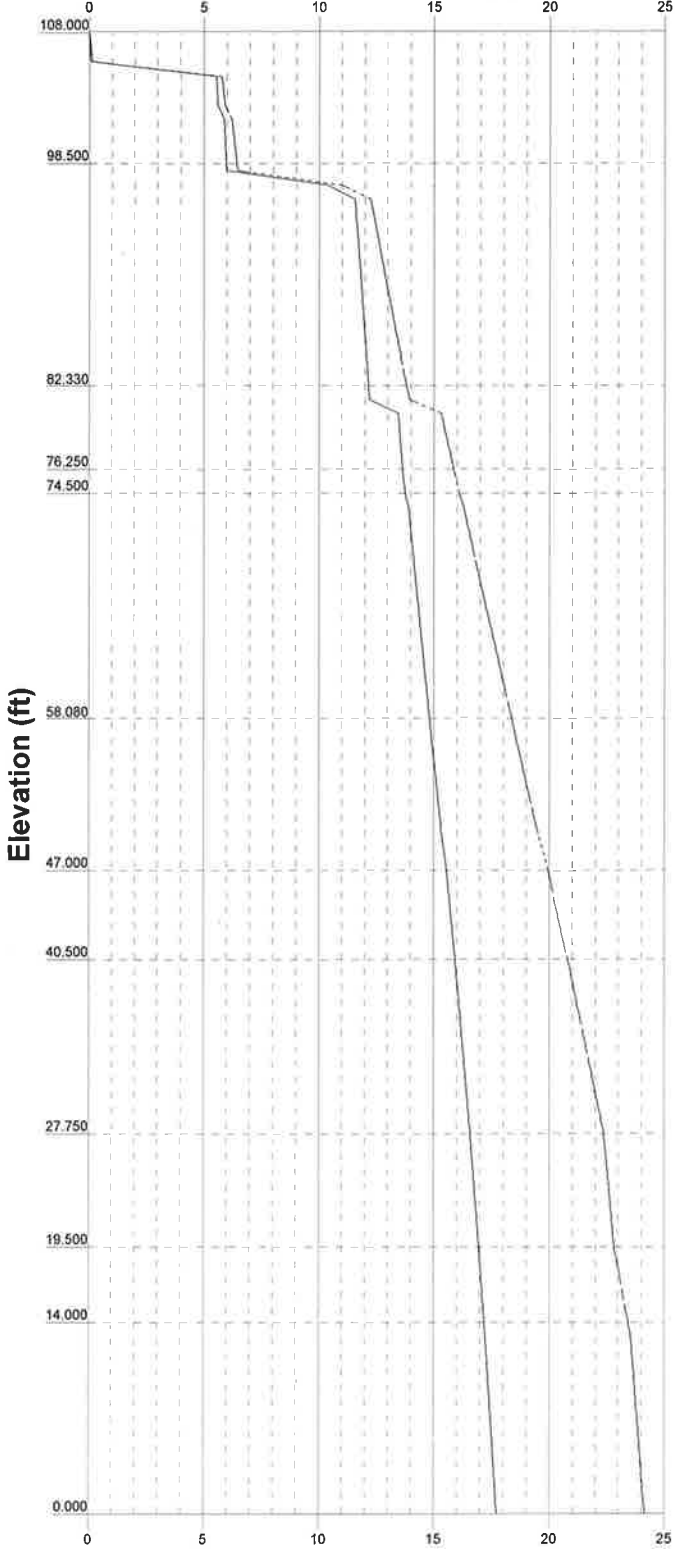
Vx

Vz

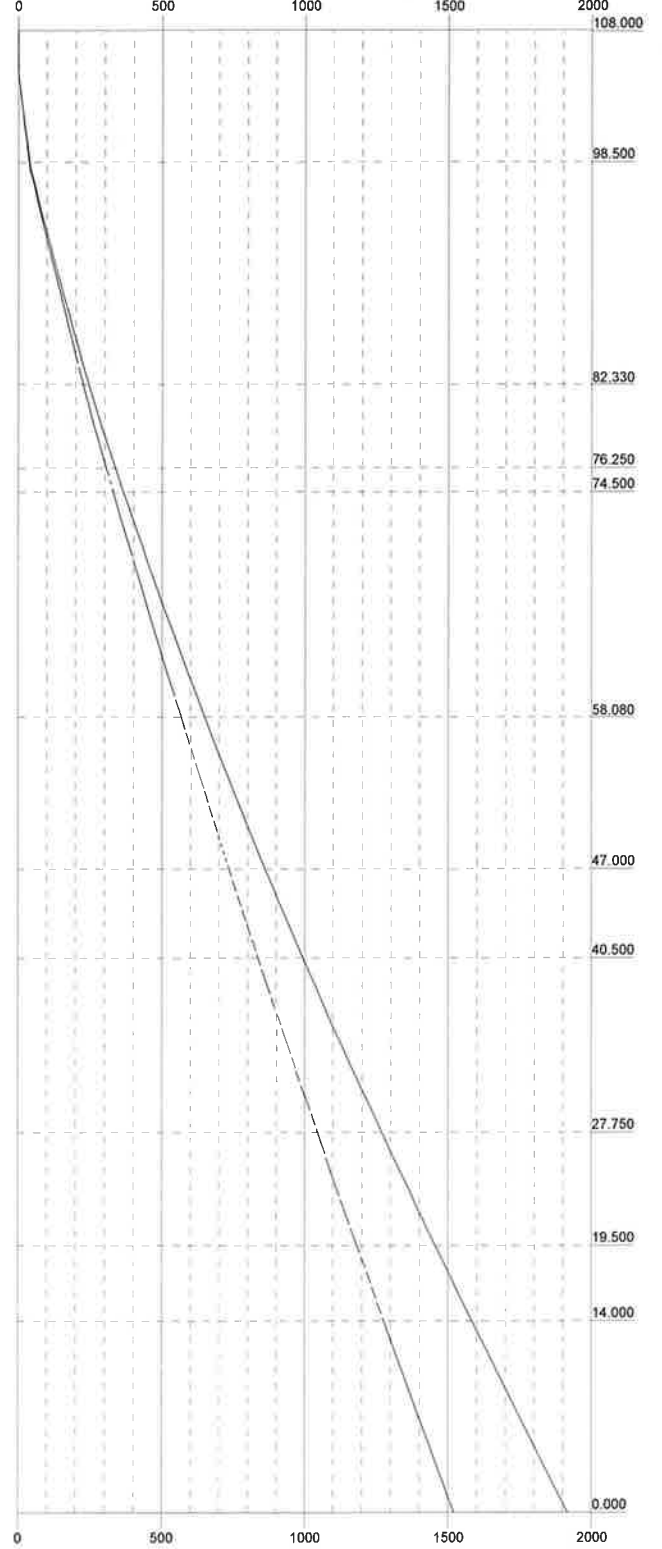
Mx


Mz

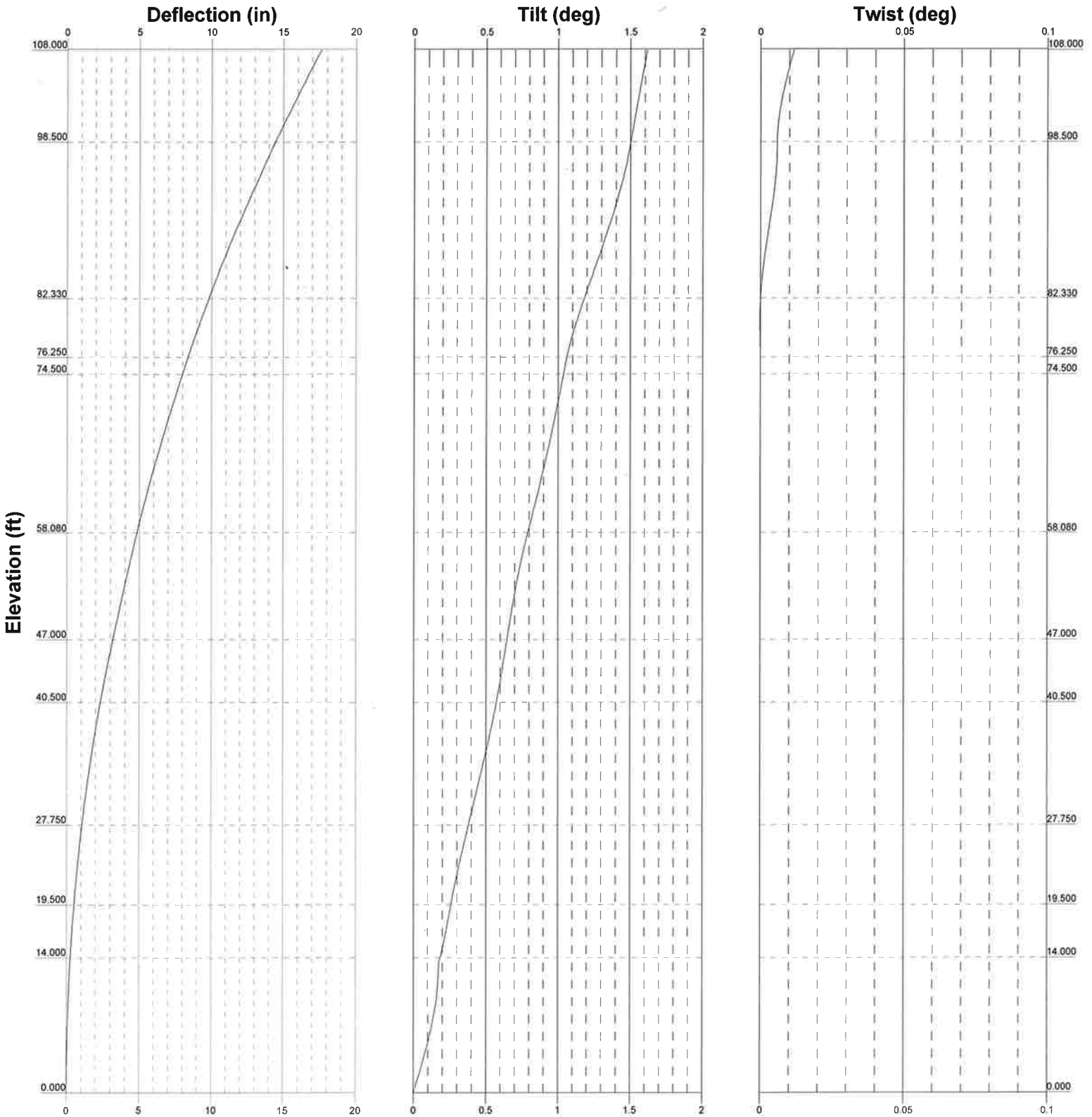
### Global Mast Shear (K)



### Global Mast Moment (kip-ft)



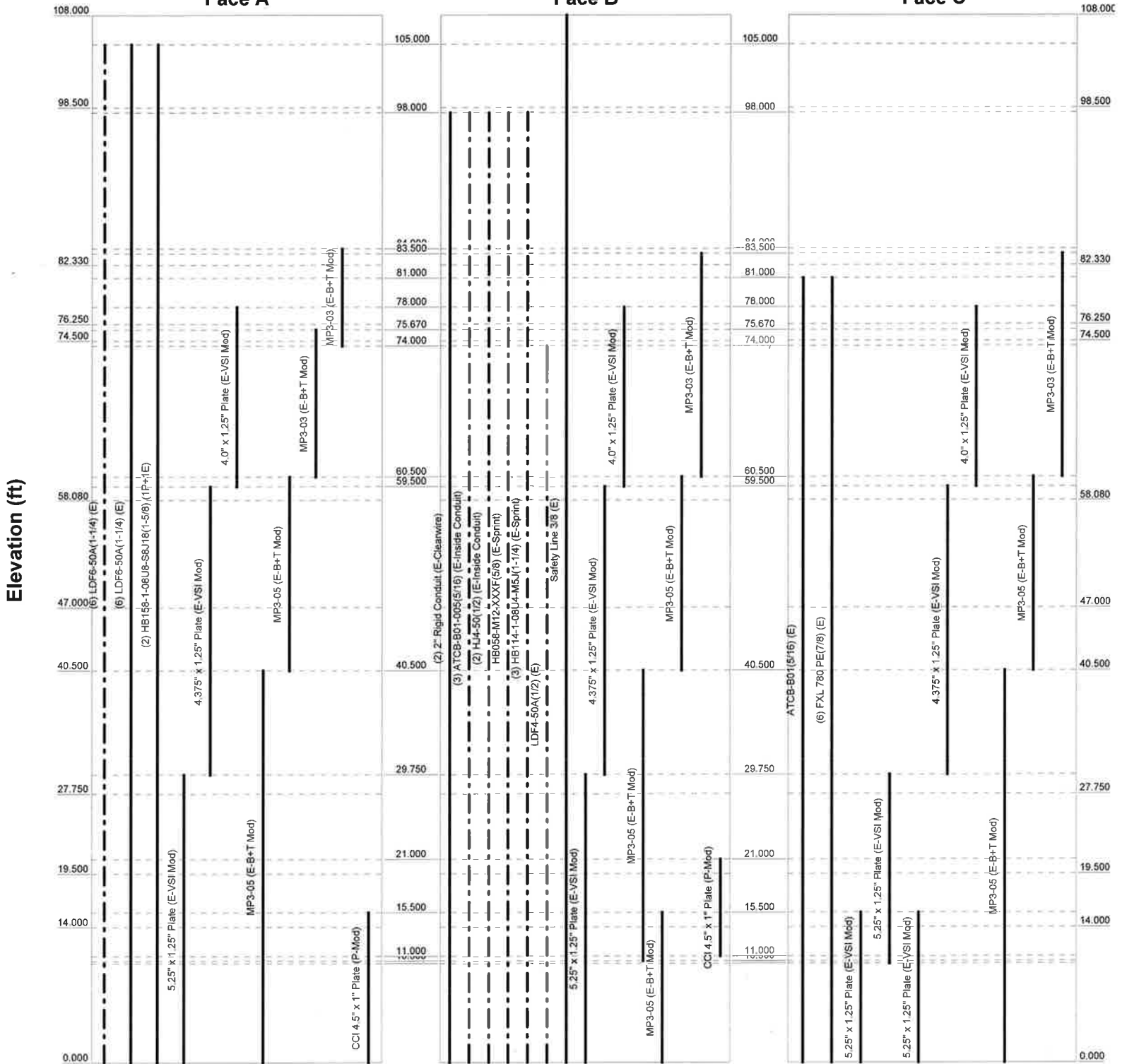
 <b>B+T Group</b> 1717 S Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job: 85565.006.01 - HARTFORD - NU (SSUSA), CT (BU# 8763)</b>		
	Project:		
	Client: Crown Castle	Drawn by: CJangonda	App'd:
	Code: TIA-222-G	Date: 05/02/17	Scale: NTS
	Path:	Dwg No: E-4	



Face A

Face B

Face C



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	Project:		
	Client: Crown Castle	Drawn by: CJangonda	App'd:
	Code: TIA-222-G	Date: 05/02/17	Scale: NTS
	Path:	Dwg No: E-7	

<b>tnxTower</b>  <b>B+T Group</b> 1717 S Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b> 85565.006.01 - HARTFORD - NU (SSUSA),CT (BU# 876363)	<b>Page</b> 1 of 19
	<b>Project</b>	<b>Date</b> 18:16:28 05/02/17
	<b>Client</b> Crown Castle	<b>Designed by</b> CJangonda

## Tower Input Data

There is a pole section.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Hartford County, Connecticut.

Basic wind speed of 97 mph.

Structure Class II.

Exposure Category C.

Topographic Category 1.

Crest Height 0.000 ft.

Nominal ice thickness of 1.000 in.

Ice thickness is considered to increase with height.

Ice density of 56.000 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50.000 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

<ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>Include Bolts In Member Capacity</li> <li>Leg Bolts Are At Top Of Section</li> <li>Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>SR Members Have Cut Ends</li> <li>SR Members Are Concentric</li> </ul>	<ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>√ Use Clear Spans For Wind Area</li> <li>Use Clear Spans For KL/r</li> <li>Retension Guys To Initial Tension</li> <li>√ Bypass Mast Stability Checks</li> <li>√ Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>Autocalc Torque Arm Areas</li> <li>Add IBC .6D+W Combination</li> <li>Sort Capacity Reports By Component</li> <li>Triangulate Diamond Inner Bracing</li> <li>Treat Feed Line Bundles As Cylinder</li> </ul>	<ul style="list-style-type: none"> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>SR Leg Bolts Resist Compression</li> <li>All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>√ Consider Feed Line Torque</li> <li>Include Angle Block Shear Check</li> <li>Use TIA-222-G Bracing Resist. Exemption</li> <li>Use TIA-222-G Tension Splice Exemption</li> <li style="text-align: center;">Poles</li> <li>√ Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> </ul>
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## Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft		in	in	in	in	
L1	108.000-98.500	9.500	0.000	Round	8.625	8.625	0.313		A53-B-35 (35 ksi)
L2	98.500-98.000	0.500	0.000	Round	8.625	16.500	0.313		A53-B-35 (35 ksi)
L3	98.000-82.330	15.670	0.000	18	16.500	19.008	0.188	0.750	A607-65



<b>tnxTower</b>  <b>B+T Group</b> 1717 S Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b> 85565.006.01 - HARTFORD - NU (SSUSA),CT (BU# 876363)	<b>Page</b> 2 of 19
	<b>Project</b>	<b>Date</b> 18:16:28 05/02/17
	<b>Client</b> Crown Castle	<b>Designed by</b> CJangonda

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade (ksi)
L4	82.330-76.250	6.080	0.000	18	19.008	19.981	0.323	1.292	43.351692ksi (43 ksi)
L5	76.250-74.500	1.750	0.000	18	19.981	20.261	0.462	1.847	38.234343ksi (38 ksi)
L6	74.500-58.080	16.420	0.000	18	20.261	22.890	0.537	2.148	35.199151ksi (35 ksi)
L7	58.080-57.750	0.330	0.000	18	22.890	22.942	0.698	2.791	34.946983ksi (35 ksi)
L8	57.750-47.000	10.750	3.250	18	22.942	24.663	0.683	2.733	36.657402ksi (37 ksi)
L9	47.000-40.500	9.750	0.000	18	23.768	25.328	0.723	2.891	36.593546ksi (37 ksi)
L10	40.500-27.750	12.750	0.000	18	25.328	27.369	0.677	2.707	36.688841ksi (37 ksi)
L11	27.750-19.500	8.250	0.000	18	27.369	28.689	0.689	2.755	39.884621ksi (40 ksi)
L12	19.500-14.000	5.500	0.000	18	28.689	29.569	0.799	3.196	39.613462ksi (40 ksi)
L13	14.000-13.080	0.920	0.000	18	29.569	29.717	0.787	3.150	38.758558ksi (39 ksi)
L14	13.080-13.000	0.080	0.000	18	29.717	29.729	0.752	3.009	37.349432ksi (37 ksi)
L15	13.000-0.000	13.000		18	29.729	31.810	0.762	3.047	37.631626ksi (38 ksi)

### Tapered Pole Properties

Section	Tip Dia. in	Area in <sup>2</sup>	I in <sup>4</sup>	r in	C in	I/C in <sup>3</sup>	J in <sup>4</sup>	I/Q in <sup>2</sup>	w in	w/t
L1	8.625	8.161	70.586	2.941	4.313	16.368	141.172	4.078	0.000	0
	8.625	8.161	70.586	2.941	4.313	16.368	141.172	4.078	0.000	0
L2	8.625	8.161	70.586	2.941	4.313	16.368	141.172	4.078	0.000	0
	16.500	15.892	520.728	5.724	8.250	63.119	1041.456	7.941	0.000	0
L3	16.755	9.708	326.368	5.791	8.382	38.937	653.165	4.855	2.574	13.728
	19.301	11.201	501.242	6.681	9.656	51.909	1003.144	5.601	3.015	16.082
L4	19.301	19.162	845.200	6.633	9.656	87.530	1691.513	9.583	2.777	8.594
	20.290	20.160	984.257	6.979	10.150	96.966	1969.810	10.082	2.948	9.124
L5	20.290	28.603	1376.878	6.929	10.150	135.646	2755.568	14.304	2.704	5.857
	20.574	29.014	1437.007	7.029	10.293	139.613	2875.904	14.510	2.753	5.964
L6	20.574	33.615	1652.283	7.002	10.293	160.528	3306.739	16.811	2.621	4.881
	23.243	38.094	2404.666	7.935	11.628	206.802	4812.496	19.051	3.084	5.743
L7	23.243	49.142	3057.563	7.878	11.628	262.951	6119.150	24.575	2.801	4.014
	23.296	49.259	3079.448	7.897	11.655	264.223	6162.947	24.634	2.810	4.028
L8	23.296	48.270	3021.563	7.902	11.655	259.257	6047.103	24.140	2.835	4.15
	25.043	52.001	3777.824	8.513	12.529	301.531	7560.618	26.006	3.138	4.593
L9	24.663	52.868	3547.208	8.181	12.074	293.788	7099.083	26.439	2.911	4.027
	25.719	56.448	4317.666	8.735	12.867	335.568	8641.013	28.229	3.186	4.407
L10	25.719	52.956	4065.728	8.751	12.867	315.987	8136.806	26.483	3.267	4.826
	27.791	57.340	5161.248	9.476	13.903	371.223	10329.287	28.675	3.626	5.357
L11	27.791	58.317	5244.531	9.471	13.903	377.214	10495.963	29.164	3.605	5.235
	29.132	61.203	6062.332	9.940	14.574	415.966	12132.641	30.607	3.837	5.572
L12	29.132	70.730	6950.927	9.901	14.574	476.937	13911.000	35.372	3.643	4.56
	30.026	72.962	7630.050	10.213	15.021	507.950	15270.140	36.488	3.798	4.753
L13	30.026	71.932	7528.419	10.218	15.021	501.185	15066.743	35.973	3.818	4.849
	30.175	72.300	7644.550	10.270	15.096	506.394	15299.159	36.157	3.844	4.882
L14	30.175	69.154	7329.687	10.282	15.096	485.537	14669.019	34.584	3.906	5.193
	30.188	69.185	7339.411	10.287	15.103	485.972	14688.480	34.599	3.908	5.196

<b>tnxTower</b>  <b>B+T Group</b> 1717 S Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b> 85565.006.01 - HARTFORD - NU (SSUSA),CT (BU# 876363)	<b>Page</b> 3 of 19
	<b>Project</b>	<b>Date</b> 18:16:28 05/02/17
	<b>Client</b> Crown Castle	<b>Designed by</b> CJangonda

Section	Tip Dia. in	Area in <sup>2</sup>	I in <sup>4</sup>	r in	C in	I/C in <sup>3</sup>	J in <sup>4</sup>	I/O in <sup>2</sup>	w in	w/t
L15	30.188	70.029	7424.139	10.284	15.103	491.582	14858.048	35.021	3.892	5.11
	32.301	75.059	9141.468	11.022	16.159	565.703	18294.964	37.537	4.258	5.591

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A <sub>f</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft <sup>2</sup>	in					in	in	in
L1				1	1	1			
108.000-98.500									
0									
L2				1	1	1			
98.500-98.000									
L3				1	1	1			
98.000-82.330									
L4				1	1	1.01961			
82.330-76.250									
L5				1	1	1.13109			
76.250-74.500									
L6				1	1	0.979328			
74.500-58.080									
L7				1	1	0.924481			
58.080-57.750									
L8				1	1	0.936878			
57.750-47.000									
L9				1	1	0.944304			
47.000-40.500									
L10				1	1	0.957892			
40.500-27.750									
L11				1	1	0.968253			
27.750-19.500									
L12				1	1	0.883534			
19.500-14.000									
L13				1	1	0.95556			
14.000-13.080									
L14				1	1	0.998738			
13.080-13.000									
L15				1	1	0.970114			
13.000-0.000									

**Feed Line/Linear Appurtenances - Entered As Round Or Flat**

Description	Sector	Component Type	Placement	Total Number	Number Per Row	Start/End Position	Width or Diameter	Perimeter	Weight
			ft				in	in	klf
LDF6-50A(1-1/4) (E)	A	Surface Ar (CaAa)	105.000 - 0.000	6	6	-0.500 -0.200	1.550		0.001
HB158-1-08U8-S8J18(1-5/8) (1P+1E) ***%**	A	Surface Ar (CaAa)	105.000 - 0.000	2	2	-0.300 -0.200	1.980		0.001
2" Rigid Conduit (E-Clearwire) ***%**	B	Surface Ar (CaAa)	98.000 - 0.000	2	2	-0.170 0.000	2.000		0.003
ATCB-B01(5/16) (E)	C	Surface Ar (CaAa)	81.000 - 0.000	1	1	-0.360 -0.350	0.315		0.000
FXL 780 PE(7/8) (E) ***%**	C	Surface Ar (CaAa)	81.000 - 0.000	6	3	-0.500 -0.350	1.090		0.000

# tnxTower

**B+T Group**  
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Description	Sector	Component Type	Placement ft	Total Number	Number Per Row	Start/End Position	Width or Diameter in	Perimeter in	Weight klf
Safety Line 3/8 (E) **@**	B	Surface Ar (CaAa)	108.000 - 0.000	1	1	0.000 0.000	0.375		0.000
5.25" x 1.25" Plate (E-VSI Mod)	A	Surface Af (CaAa)	29.750 - 0.000	1	1	0.200 0.200	5.250	13.000	0.000
5.25" x 1.25" Plate (E-VSI Mod)	B	Surface Af (CaAa)	29.750 - 0.000	1	1	0.000 0.000	5.250	13.000	0.000
5.25" x 1.25" Plate (E-VSI Mod)	C	Surface Af (CaAa)	15.500 - 0.000	1	1	-0.300 -0.300	5.250	13.000	0.000
5.25" x 1.25" Plate (E-VSI Mod)	C	Surface Af (CaAa)	29.750 - 10.250	1	1	0.000 0.000	5.250	13.000	0.000
5.25" x 1.25" Plate (E-VSI Mod)	C	Surface Af (CaAa)	15.500 - 0.000	1	1	0.300 0.300	5.250	13.000	0.000
4.375" x 1.25" Plate (E-VSI Mod)	A	Surface Af (CaAa)	59.500 - 29.750	1	1	0.200 0.200	4.375	11.250	0.000
4.375" x 1.25" Plate (E-VSI Mod)	B	Surface Af (CaAa)	59.500 - 29.750	1	1	0.000 0.000	4.375	11.250	0.000
4.375" x 1.25" Plate (E-VSI Mod)	C	Surface Af (CaAa)	59.500 - 29.750	1	1	0.000 0.000	4.375	11.250	0.000
4.0" x 1.25" Plate (E-VSI Mod)	A	Surface Af (CaAa)	78.000 - 59.500	1	1	0.200 0.200	4.000	10.500	0.000
4.0" x 1.25" Plate (E-VSI Mod)	B	Surface Af (CaAa)	78.000 - 59.500	1	1	0.000 0.000	4.000	10.500	0.000
4.0" x 1.25" Plate (E-VSI Mod)	C	Surface Af (CaAa)	78.000 - 59.500	1	1	0.000 0.000	4.000	10.500	0.000
MP3-05 (E-B+T Mod)	A	Surface Af (CaAa)	40.500 - 0.000	1	1	-0.500 -0.500	5.330	14.840	0.000
MP3-05 (E-B+T Mod)	B	Surface Af (CaAa)	40.500 - 10.500	1	1	-0.500 -0.500	5.330	14.840	0.000
MP3-05 (E-B+T Mod)	B	Surface Af (CaAa)	15.500 - 0.000	1	1	-0.200 -0.200	5.330	14.840	0.000
MP3-05 (E-B+T Mod)	C	Surface Af (CaAa)	40.500 - 0.000	1	1	-0.500 -0.500	5.330	14.840	0.000
MP3-05 (E-B+T Mod)	A	Surface Af (CaAa)	60.500 - 40.500	1	1	-0.500 -0.500	5.330	14.840	0.000
MP3-05 (E-B+T Mod)	B	Surface Af (CaAa)	60.500 - 40.500	1	1	-0.500 -0.500	5.330	14.840	0.000
MP3-05 (E-B+T Mod)	C	Surface Af (CaAa)	60.500 - 40.500	1	1	-0.500 -0.500	5.330	14.840	0.000
MP3-03 (E-B+T Mod)	A	Surface Af (CaAa)	75.670 - 60.500	1	1	-0.500 -0.500	4.060	11.260	0.000
MP3-03 (E-B+T Mod)	A	Surface Af (CaAa)	84.000 - 74.000	1	1	-0.300 -0.300	4.060	11.260	0.000
MP3-03 (E-B+T Mod)	B	Surface Af (CaAa)	83.500 - 60.500	1	1	-0.500 -0.500	4.060	11.260	0.000
MP3-03 (E-B+T Mod)	C	Surface Af (CaAa)	83.500 - 60.500	1	1	-0.500 -0.500	4.060	11.260	0.000
CCI 4.5" x 1" Plate (P-Mod)	A	Surface Af (CaAa)	15.500 - 0.000	1	1	0.000 0.000	4.500	11.000	0.000
CCI 4.5" x 1" Plate (P-Mod)	B	Surface Af (CaAa)	21.000 - 11.000	1	1	0.300 0.300	4.500	11.000	0.000

# tnxTower

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**Project**

**Date**  
 18:16:28 05/02/17

**Client**  
 Crown Castle

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 CJangonda

## Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	C <sub>A</sub> A <sub>1</sub>		Weight klf
						ft <sup>2</sup> /ft	klf	
LDF6-50A(1-1/4) (E)	A	No	Inside Pole	105.000 - 0.000	6	No Ice	0.000	0.001
						1/2" Ice	0.000	0.001
						1" Ice	0.000	0.001
ATCB-B01-005(5/16) (E-Inside Conduit)	B	No	Inside Pole	98.000 - 0.000	3	No Ice	0.000	0.000
						1/2" Ice	0.000	0.000
						1" Ice	0.000	0.000
HJ4-50(1/2) (E-Inside Conduit)	B	No	Inside Pole	98.000 - 0.000	2	No Ice	0.000	0.000
						1/2" Ice	0.000	0.000
						1" Ice	0.000	0.000
***%*** HB058-M12-XXXF(5/8) (E-Sprint)	B	No	Inside Pole	98.000 - 0.000	1	No Ice	0.000	0.000
						1/2" Ice	0.000	0.000
						1" Ice	0.000	0.000
HB114-1-08U4-M5J(1-1/4) (E-Sprint)	B	No	Inside Pole	98.000 - 0.000	3	No Ice	0.000	0.001
						1/2" Ice	0.000	0.001
						1" Ice	0.000	0.001
***%*** LDF4-50A(1/2) (E)	B	No	Inside Pole	74.000 - 0.000	1	No Ice	0.000	0.000
						1/2" Ice	0.000	0.000
						1" Ice	0.000	0.000
**@**								

## Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>1</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>1</sub> Out Face ft <sup>2</sup>	Weight K
L1	108.000-98.500	A	0.000	0.000	8.619	0.000	0.064
		B	0.000	0.000	0.356	0.000	0.002
		C	0.000	0.000	0.000	0.000	0.000
L2	98.500-98.000	A	0.000	0.000	0.663	0.000	0.005
		B	0.000	0.000	0.019	0.000	0.000
		C	0.000	0.000	0.000	0.000	0.000
L3	98.000-82.330	A	0.000	0.000	21.908	0.000	0.154
		B	0.000	0.000	7.647	0.000	0.157
		C	0.000	0.000	0.792	0.000	0.000
L4	82.330-76.250	A	0.000	0.000	13.343	0.000	0.060
		B	0.000	0.000	7.941	0.000	0.061
		C	0.000	0.000	6.984	0.000	0.007
L5	76.250-74.500	A	0.000	0.000	5.463	0.000	0.017
		B	0.000	0.000	3.116	0.000	0.018
		C	0.000	0.000	2.978	0.000	0.003
L6	74.500-58.080	A	0.000	0.000	44.770	0.000	0.161
		B	0.000	0.000	29.842	0.000	0.167
		C	0.000	0.000	28.545	0.000	0.026
L7	58.080-57.750	A	0.000	0.000	0.971	0.000	0.003
		B	0.000	0.000	0.678	0.000	0.003
		C	0.000	0.000	0.652	0.000	0.001
L8	57.750-47.000	A	0.000	0.000	31.643	0.000	0.105
		B	0.000	0.000	22.091	0.000	0.109
		C	0.000	0.000	21.242	0.000	0.017
L9	47.000-40.500	A	0.000	0.000	19.133	0.000	0.064
		B	0.000	0.000	13.358	0.000	0.066
		C	0.000	0.000	12.844	0.000	0.010
L10	40.500-27.750	A	0.000	0.000	37.821	0.000	0.125

<b>tnxTower</b>  <b>B+T Group</b> 1717 S Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b> 85565.006.01 - HARTFORD - NU (SSUSA),CT (BU# 876363)	<b>Page</b> 6 of 19
	<b>Project</b>	<b>Date</b> 18:16:28 05/02/17
	<b>Client</b> Crown Castle	<b>Designed by</b> CJangonda

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
L11	27,750-19,500	B	0.000	0.000	26.493	0.000	0.130
		C	0.000	0.000	25.486	0.000	0.020
		A	0.000	0.000	25.487	0.000	0.081
L12	19,500-14,000	B	0.000	0.000	19.282	0.000	0.084
		C	0.000	0.000	17.505	0.000	0.013
		A	0.000	0.000	18.116	0.000	0.054
L13	14,000-13,080	B	0.000	0.000	17.562	0.000	0.056
		C	0.000	0.000	14.295	0.000	0.009
		A	0.000	0.000	3.532	0.000	0.009
L14	13,080-13,000	B	0.000	0.000	3.532	0.000	0.009
		C	0.000	0.000	3.562	0.000	0.001
		A	0.000	0.000	0.307	0.000	0.001
L15	13,000-0,000	B	0.000	0.000	0.307	0.000	0.001
		C	0.000	0.000	0.310	0.000	0.000
		A	0.000	0.000	49.911	0.000	0.127
		B	0.000	0.000	32.332	0.000	0.132
		C	0.000	0.000	41.365	0.000	0.020
		A	0.000	0.000			

**Feed Line/Linear Appurtenances Section Areas - With Ice**

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
L1	108,000-98,500	A	2.242	0.000	0.000	18,059	0.000	0.325
		B		0.000	0.000	4.615	0.000	0.070
		C		0.000	0.000	0.000	0.000	0.000
L2	98,500-98,000	A	2.230	0.000	0.000	1,386	0.000	0.025
		B		0.000	0.000	0.242	0.000	0.004
		C		0.000	0.000	0.000	0.000	0.000
L3	98,000-82,330	A	2.211	0.000	0.000	44,883	0.000	0.801
		B		0.000	0.000	25,323	0.000	0.524
		C		0.000	0.000	1,309	0.000	0.020
L4	82,330-76,250	A	2.183	0.000	0.000	24,405	0.000	0.425
		B		0.000	0.000	17,941	0.000	0.322
		C		0.000	0.000	15,457	0.000	0.249
L5	76,250-74,500	A	2.172	0.000	0.000	9,658	0.000	0.161
		B		0.000	0.000	6,523	0.000	0.112
		C		0.000	0.000	6,352	0.000	0.100
L6	74,500-58,080	A	2.144	0.000	0.000	81,732	0.000	1.322
		B		0.000	0.000	61,407	0.000	1.042
		C		0.000	0.000	59,810	0.000	0.933
L7	58,080-57,750	A	2.116	0.000	0.000	1,709	0.000	0.027
		B		0.000	0.000	1,305	0.000	0.022
		C		0.000	0.000	1,273	0.000	0.019
L8	57,750-47,000	A	2.094	0.000	0.000	55,469	0.000	0.870
		B		0.000	0.000	42,303	0.000	0.696
		C		0.000	0.000	41,257	0.000	0.624
L9	47,000-40,500	A	2.057	0.000	0.000	33,539	0.000	0.526
		B		0.000	0.000	25,578	0.000	0.421
		C		0.000	0.000	24,946	0.000	0.377
L10	40,500-27,750	A	2.006	0.000	0.000	65,069	0.000	0.985
		B		0.000	0.000	49,510	0.000	0.787
		C		0.000	0.000	48,271	0.000	0.701
L11	27,750-19,500	A	1,934	0.000	0.000	42,582	0.000	0.620
		B		0.000	0.000	33,996	0.000	0.517
		C		0.000	0.000	31,742	0.000	0.441
L12	19,500-14,000	A	1,869	0.000	0.000	29,730	0.000	0.418
		B		0.000	0.000	28,508	0.000	0.415

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	<b>Project</b>	<b>Date</b> 18:16:28 05/02/17
	<b>Client</b> Crown Castle	<b>Designed by</b> CJangonda

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
L13	14.000-13.080	C	1.830	0.000	0.000	24.493	0.000	0.325
		A		0.000	0.000	5.678	0.000	0.077
		B		0.000	0.000	5.548	0.000	0.078
L14	13.080-13.000	C	1.823	0.000	0.000	5.682	0.000	0.072
		A		0.000	0.000	0.493	0.000	0.007
		B		0.000	0.000	0.482	0.000	0.007
L15	13.000-0.000	C	1.698	0.000	0.000	0.493	0.000	0.006
		A		0.000	0.000	78.422	0.000	0.998
		B		0.000	0.000	53.324	0.000	0.734
		C		0.000	0.000	66.036	0.000	0.789

### Feed Line Center of Pressure

Section	Elevation ft	CP <sub>X</sub> in	CP <sub>Z</sub> in	CP <sub>X</sub> Ice in	CP <sub>Z</sub> Ice in
L1	108.000-98.500	-0.711	0.088	-0.469	0.008
L2	98.500-98.000	-0.984	0.129	-0.717	0.038
L3	98.000-82.330	-0.745	-0.076	-0.469	-0.167
L4	82.330-76.250	-0.430	-0.183	-0.188	-0.146
L5	76.250-74.500	-0.286	-0.014	-0.096	-0.007
L6	74.500-58.080	-0.123	-0.067	0.013	-0.030
L7	58.080-57.750	-0.100	-0.079	0.021	-0.035
L8	57.750-47.000	-0.103	-0.082	0.022	-0.036
L9	47.000-40.500	-0.105	-0.084	0.023	-0.038
L10	40.500-27.750	-0.103	-0.093	0.024	-0.043
L11	27.750-19.500	-0.028	-0.082	0.068	-0.034
L12	19.500-14.000	0.127	0.019	0.165	0.030
L13	14.000-13.080	0.030	-0.081	0.073	-0.055
L14	13.080-13.000	0.030	-0.081	0.073	-0.056
L15	13.000-0.000	0.070	-0.225	0.122	-0.160

### Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
L1	2	LDF6-50A(1-1/4)	98.50 - 105.00	1.0000	1.0000
L1	3	HB158-1-08U8-S8J18(1-5/8)	98.50 - 105.00	1.0000	1.0000
L1	17	Safety Line 3/8	98.50 - 108.00	1.0000	1.0000
L2	2	LDF6-50A(1-1/4)	98.00 - 98.50	1.0000	1.0000
L2	3	HB158-1-08U8-S8J18(1-5/8)	98.00 - 98.50	1.0000	1.0000
L2	17	Safety Line 3/8	98.00 - 98.50	1.0000	1.0000
L3	2	LDF6-50A(1-1/4)	82.33 - 98.00	1.0000	1.0000
L3	3	HB158-1-08U8-S8J18(1-5/8)	82.33 - 98.00	1.0000	1.0000
L3	5	2" Rigid Conduit	82.33 - 98.00	1.0000	1.0000
L3	17	Safety Line 3/8	82.33 - 98.00	1.0000	1.0000
L3	43	MP3-03	82.33 - 84.00	1.0000	1.0000
L3	44	MP3-03	82.33 - 83.50	1.0000	1.0000
L3	45	MP3-03	82.33 - 83.50	1.0000	1.0000
L4	2	LDF6-50A(1-1/4)	76.25 - 82.33	1.0000	1.0000
L4	3	HB158-1-08U8-S8J18(1-5/8)	76.25 - 82.33	1.0000	1.0000
L4	5	2" Rigid Conduit	76.25 - 82.33	1.0000	1.0000
L4	12	ATCB-B01(5/16)	76.25 - 81.00	1.0000	1.0000
L4	13	FXL 780 PE(7/8)	76.25 - 81.00	1.0000	1.0000
L4	17	Safety Line 3/8	76.25 - 82.33	1.0000	1.0000

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	<b>Project</b>	<b>Date</b> 18:16:28 05/02/17
	<b>Client</b> Crown Castle	<b>Designed by</b> CJangonda

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
L4	29	4.0" x 1.25" Plate	76.25 - 78.00	1.0000	1.0000
L4	30	4.0" x 1.25" Plate	76.25 - 78.00	1.0000	1.0000
L4	31	4.0" x 1.25" Plate	76.25 - 78.00	1.0000	1.0000
L4	43	MP3-03	76.25 - 82.33	1.0000	1.0000
L4	44	MP3-03	76.25 - 82.33	1.0000	1.0000
L4	45	MP3-03	76.25 - 82.33	1.0000	1.0000
L5	2	LDF6-50A(1-1/4)	74.50 - 76.25	1.0000	1.0000
L5	3	HB158-1-08U8-S8J18(1-5/8)	74.50 - 76.25	1.0000	1.0000
L5	5	2" Rigid Conduit	74.50 - 76.25	1.0000	1.0000
L5	12	ATCB-B01(5/16)	74.50 - 76.25	1.0000	1.0000
L5	13	FXL 780 PE(7/8)	74.50 - 76.25	1.0000	1.0000
L5	17	Safety Line 3/8	74.50 - 76.25	1.0000	1.0000
L5	29	4.0" x 1.25" Plate	74.50 - 76.25	1.0000	1.0000
L5	30	4.0" x 1.25" Plate	74.50 - 76.25	1.0000	1.0000
L5	31	4.0" x 1.25" Plate	74.50 - 76.25	1.0000	1.0000
L5	42	MP3-03	74.50 - 75.67	1.0000	1.0000
L5	43	MP3-03	74.50 - 76.25	1.0000	1.0000
L5	44	MP3-03	74.50 - 76.25	1.0000	1.0000
L5	45	MP3-03	74.50 - 76.25	1.0000	1.0000
L6	2	LDF6-50A(1-1/4)	58.08 - 74.50	1.0000	1.0000
L6	3	HB158-1-08U8-S8J18(1-5/8)	58.08 - 74.50	1.0000	1.0000
L6	5	2" Rigid Conduit	58.08 - 74.50	1.0000	1.0000
L6	12	ATCB-B01(5/16)	58.08 - 74.50	1.0000	1.0000
L6	13	FXL 780 PE(7/8)	58.08 - 74.50	1.0000	1.0000
L6	17	Safety Line 3/8	58.08 - 74.50	1.0000	1.0000
L6	25	4.375" x 1.25" Plate	58.08 - 59.50	1.0000	1.0000
L6	26	4.375" x 1.25" Plate	58.08 - 59.50	1.0000	1.0000
L6	27	4.375" x 1.25" Plate	58.08 - 59.50	1.0000	1.0000
L6	29	4.0" x 1.25" Plate	59.50 - 74.50	1.0000	1.0000
L6	30	4.0" x 1.25" Plate	59.50 - 74.50	1.0000	1.0000
L6	31	4.0" x 1.25" Plate	59.50 - 74.50	1.0000	1.0000
L6	38	MP3-05	58.08 - 60.50	1.0000	1.0000
L6	39	MP3-05	58.08 - 60.50	1.0000	1.0000
L6	40	MP3-05	58.08 - 60.50	1.0000	1.0000
L6	42	MP3-03	60.50 - 74.50	1.0000	1.0000
L6	43	MP3-03	74.00 - 74.50	1.0000	1.0000
L6	44	MP3-03	60.50 - 74.50	1.0000	1.0000
L6	45	MP3-03	60.50 - 74.50	1.0000	1.0000
L7	2	LDF6-50A(1-1/4)	57.75 - 58.08	1.0000	1.0000
L7	3	HB158-1-08U8-S8J18(1-5/8)	57.75 - 58.08	1.0000	1.0000
L7	5	2" Rigid Conduit	57.75 - 58.08	1.0000	1.0000
L7	12	ATCB-B01(5/16)	57.75 - 58.08	1.0000	1.0000
L7	13	FXL 780 PE(7/8)	57.75 - 58.08	1.0000	1.0000
L7	17	Safety Line 3/8	57.75 - 58.08	1.0000	1.0000
L7	25	4.375" x 1.25" Plate	57.75 - 58.08	1.0000	1.0000
L7	26	4.375" x 1.25" Plate	57.75 - 58.08	1.0000	1.0000
L7	27	4.375" x 1.25" Plate	57.75 - 58.08	1.0000	1.0000
L7	38	MP3-05	57.75 - 58.08	1.0000	1.0000
L7	39	MP3-05	57.75 - 58.08	1.0000	1.0000
L7	40	MP3-05	57.75 - 58.08	1.0000	1.0000
L8	2	LDF6-50A(1-1/4)	47.00 - 57.75	1.0000	1.0000
L8	3	HB158-1-08U8-S8J18(1-5/8)	47.00 - 57.75	1.0000	1.0000
L8	5	2" Rigid Conduit	47.00 - 57.75	1.0000	1.0000
L8	12	ATCB-B01(5/16)	47.00 - 57.75	1.0000	1.0000
L8	13	FXL 780 PE(7/8)	47.00 - 57.75	1.0000	1.0000
L8	17	Safety Line 3/8	47.00 - 57.75	1.0000	1.0000
L8	25	4.375" x 1.25" Plate	47.00 - 57.75	1.0000	1.0000
L8	26	4.375" x 1.25" Plate	47.00 - 57.75	1.0000	1.0000
L8	27	4.375" x 1.25" Plate	47.00 - 57.75	1.0000	1.0000
L8	38	MP3-05	47.00 - 57.75	1.0000	1.0000
L8	39	MP3-05	47.00 - 57.75	1.0000	1.0000
L8	40	MP3-05	47.00 - 57.75	1.0000	1.0000

# tnxTower

**B+T Group**  
 1717 S Boulder, Suite 300  
 Tulsa, OK 74119  
 Phone: (918) 587-4630  
 FAX: (918) 587-4630

**Job**  
 85565.006.01 - HARTFORD - NU (SSUSA),CT (BU# 876363)

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**Project**

**Date**  
 18:16:28 05/02/17

**Client**  
 Crown Castle

**Designed by**  
 CJangonda

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
L10	2	LDF6-50A(1-1/4)	27.75 - 40.50	1.0000	1.0000
L10	3	HB158-1-08U8-S8J18(1-5/8)	27.75 - 40.50	1.0000	1.0000
L10	5	2" Rigid Conduit	27.75 - 40.50	1.0000	1.0000
L10	12	ATCB-B01(5/16)	27.75 - 40.50	1.0000	1.0000
L10	13	FXL 780 PE(7/8)	27.75 - 40.50	1.0000	1.0000
L10	17	Safety Line 3/8	27.75 - 40.50	1.0000	1.0000
L10	19	5.25" x 1.25" Plate	27.75 - 29.75	1.0000	1.0000
L10	20	5.25" x 1.25" Plate	27.75 - 29.75	1.0000	1.0000
L10	22	5.25" x 1.25" Plate	27.75 - 29.75	1.0000	1.0000
L10	25	4.375" x 1.25" Plate	29.75 - 40.50	1.0000	1.0000
L10	26	4.375" x 1.25" Plate	29.75 - 40.50	1.0000	1.0000
L10	27	4.375" x 1.25" Plate	29.75 - 40.50	1.0000	1.0000
L10	33	MP3-05	27.75 - 40.50	1.0000	1.0000
L10	34	MP3-05	27.75 - 40.50	1.0000	1.0000
L10	36	MP3-05	27.75 - 40.50	1.0000	1.0000
L11	2	LDF6-50A(1-1/4)	19.50 - 27.75	1.0000	1.0000
L11	3	HB158-1-08U8-S8J18(1-5/8)	19.50 - 27.75	1.0000	1.0000
L11	5	2" Rigid Conduit	19.50 - 27.75	1.0000	1.0000
L11	12	ATCB-B01(5/16)	19.50 - 27.75	1.0000	1.0000
L11	13	FXL 780 PE(7/8)	19.50 - 27.75	1.0000	1.0000
L11	17	Safety Line 3/8	19.50 - 27.75	1.0000	1.0000
L11	19	5.25" x 1.25" Plate	19.50 - 27.75	1.0000	1.0000
L11	20	5.25" x 1.25" Plate	19.50 - 27.75	1.0000	1.0000
L11	22	5.25" x 1.25" Plate	19.50 - 27.75	1.0000	1.0000
L11	33	MP3-05	19.50 - 27.75	1.0000	1.0000
L11	34	MP3-05	19.50 - 27.75	1.0000	1.0000
L11	36	MP3-05	19.50 - 27.75	1.0000	1.0000
L11	49	CCI 4.5" x 1" Plate	19.50 - 21.00	1.0000	1.0000
L12	2	LDF6-50A(1-1/4)	14.00 - 19.50	1.0000	1.0000
L12	3	HB158-1-08U8-S8J18(1-5/8)	14.00 - 19.50	1.0000	1.0000
L12	5	2" Rigid Conduit	14.00 - 19.50	1.0000	1.0000
L12	12	ATCB-B01(5/16)	14.00 - 19.50	1.0000	1.0000
L12	13	FXL 780 PE(7/8)	14.00 - 19.50	1.0000	1.0000
L12	17	Safety Line 3/8	14.00 - 19.50	1.0000	1.0000
L12	19	5.25" x 1.25" Plate	14.00 - 19.50	1.0000	1.0000
L12	20	5.25" x 1.25" Plate	14.00 - 19.50	1.0000	1.0000
L12	21	5.25" x 1.25" Plate	14.00 - 15.50	1.0000	1.0000
L12	22	5.25" x 1.25" Plate	14.00 - 19.50	1.0000	1.0000
L12	23	5.25" x 1.25" Plate	14.00 - 15.50	1.0000	1.0000
L12	33	MP3-05	14.00 - 19.50	1.0000	1.0000
L12	34	MP3-05	14.00 - 19.50	1.0000	1.0000
L12	35	MP3-05	14.00 - 15.50	1.0000	1.0000
L12	36	MP3-05	14.00 - 19.50	1.0000	1.0000
L12	47	CCI 4.5" x 1" Plate	14.00 - 15.50	1.0000	1.0000
L12	49	CCI 4.5" x 1" Plate	14.00 - 19.50	1.0000	1.0000
L13	2	LDF6-50A(1-1/4)	13.08 - 14.00	1.0000	1.0000
L13	3	HB158-1-08U8-S8J18(1-5/8)	13.08 - 14.00	1.0000	1.0000
L13	5	2" Rigid Conduit	13.08 - 14.00	1.0000	1.0000
L13	12	ATCB-B01(5/16)	13.08 - 14.00	1.0000	1.0000
L13	13	FXL 780 PE(7/8)	13.08 - 14.00	1.0000	1.0000
L13	17	Safety Line 3/8	13.08 - 14.00	1.0000	1.0000
L13	19	5.25" x 1.25" Plate	13.08 - 14.00	1.0000	1.0000
L13	20	5.25" x 1.25" Plate	13.08 - 14.00	1.0000	1.0000
L13	21	5.25" x 1.25" Plate	13.08 - 14.00	1.0000	1.0000
L13	22	5.25" x 1.25" Plate	13.08 - 14.00	1.0000	1.0000
L13	23	5.25" x 1.25" Plate	13.08 - 14.00	1.0000	1.0000
L13	33	MP3-05	13.08 - 14.00	1.0000	1.0000
L13	34	MP3-05	13.08 - 14.00	1.0000	1.0000
L13	35	MP3-05	13.08 - 14.00	1.0000	1.0000
L13	36	MP3-05	13.08 - 14.00	1.0000	1.0000
L13	47	CCI 4.5" x 1" Plate	13.08 - 14.00	1.0000	1.0000
L13	49	CCI 4.5" x 1" Plate	13.08 - 14.00	1.0000	1.0000



<b>tnxTower</b>  <b>B+T Group</b> 1717 S Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b> 85565.006.01 - HARTFORD - NU (SSUSA),CT (BU# 876363)	<b>Page</b> 10 of 19
	<b>Project</b>	<b>Date</b> 18:16:28 05/02/17
	<b>Client</b> Crown Castle	<b>Designed by</b> CJangonda

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	$K_a$ No Ice	$K_a$ Ice
L14	2	LDF6-50A(1-1/4)	13.00 - 13.08	1.0000	1.0000
L14	3	HB158-1-08U8-S8J18(1-5/8)	13.00 - 13.08	1.0000	1.0000
L14	5	2" Rigid Conduit	13.00 - 13.08	1.0000	1.0000
L14	12	ATCB-B01(5/16)	13.00 - 13.08	1.0000	1.0000
L14	13	FXL 780 PE(7/8)	13.00 - 13.08	1.0000	1.0000
L14	17	Safety Line 3/8	13.00 - 13.08	1.0000	1.0000
L14	19	5.25" x 1.25" Plate	13.00 - 13.08	1.0000	1.0000
L14	20	5.25" x 1.25" Plate	13.00 - 13.08	1.0000	1.0000
L14	21	5.25" x 1.25" Plate	13.00 - 13.08	1.0000	1.0000
L14	22	5.25" x 1.25" Plate	13.00 - 13.08	1.0000	1.0000
L14	23	5.25" x 1.25" Plate	13.00 - 13.08	1.0000	1.0000
L14	33	MP3-05	13.00 - 13.08	1.0000	1.0000
L14	34	MP3-05	13.00 - 13.08	1.0000	1.0000
L14	35	MP3-05	13.00 - 13.08	1.0000	1.0000
L14	36	MP3-05	13.00 - 13.08	1.0000	1.0000
L14	47	CCI 4.5" x 1" Plate	13.00 - 13.08	1.0000	1.0000
L14	49	CCI 4.5" x 1" Plate	13.00 - 13.08	1.0000	1.0000
L15	2	LDF6-50A(1-1/4)	0.00 - 13.00	1.0000	1.0000
L15	3	HB158-1-08U8-S8J18(1-5/8)	0.00 - 13.00	1.0000	1.0000
L15	5	2" Rigid Conduit	0.00 - 13.00	1.0000	1.0000
L15	12	ATCB-B01(5/16)	0.00 - 13.00	1.0000	1.0000
L15	13	FXL 780 PE(7/8)	0.00 - 13.00	1.0000	1.0000
L15	17	Safety Line 3/8	0.00 - 13.00	1.0000	1.0000
L15	19	5.25" x 1.25" Plate	0.00 - 13.00	1.0000	1.0000
L15	20	5.25" x 1.25" Plate	0.00 - 13.00	1.0000	1.0000
L15	21	5.25" x 1.25" Plate	0.00 - 13.00	1.0000	1.0000
L15	22	5.25" x 1.25" Plate	10.25 - 13.00	1.0000	1.0000
L15	23	5.25" x 1.25" Plate	0.00 - 13.00	1.0000	1.0000
L15	33	MP3-05	0.00 - 13.00	1.0000	1.0000
L15	34	MP3-05	10.50 - 13.00	1.0000	1.0000
L15	35	MP3-05	0.00 - 13.00	1.0000	1.0000
L15	36	MP3-05	0.00 - 13.00	1.0000	1.0000
L15	47	CCI 4.5" x 1" Plate	0.00 - 13.00	1.0000	1.0000
L15	49	CCI 4.5" x 1" Plate	11.00 - 13.00	1.0000	1.0000

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement	$C_{A1}$ Front	$C_{A1}$ Side	Weight	
			ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
BXA-171063-12BF w/Mount Pipe (E)	A	From Leg	4.000 0.000 0.000	0.000	105.000	No Ice 1/2" Ice 1" Ice	4.963 5.510 6.022	5.220 6.377 7.243	0.040 0.086 0.139
BXA-171063-12BF w/Mount Pipe (E)	B	From Leg	4.000 0.000 0.000	0.000	105.000	No Ice 1/2" Ice 1" Ice	4.963 5.510 6.022	5.220 6.377 7.243	0.040 0.086 0.139
BXA-171063-12BF w/Mount Pipe (E)	C	From Leg	4.000 0.000 0.000	0.000	105.000	No Ice 1/2" Ice 1" Ice	4.963 5.510 6.022	5.220 6.377 7.243	0.040 0.086 0.139
BXA-70063/6CF w/ Mount Pipe (E)	A	From Leg	4.000 0.000 0.000	0.000	105.000	No Ice 1/2" Ice 1" Ice	7.819 8.370 8.886	5.407 6.558 7.422	0.042 0.101 0.168
BXA-70063/6CF w/ Mount Pipe (E)	B	From Leg	4.000 0.000 0.000	0.000	105.000	No Ice 1/2" Ice 1" Ice	7.819 8.370 8.886	5.407 6.558 7.422	0.042 0.101 0.168

# tnxTower

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**Project**

**Date**  
 18:16:28 05/02/17

**Client**  
 Crown Castle

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 CJangonda

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight
			Horz	Vert			Front	Side	
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
BXA-70063/6CF w/ Mount Pipe (E)	C	From Leg	4.000	0.000	0.000	105.000	No Ice 7.819	5.407	0.042
			0.000				1/2" Ice 8.370	6.558	0.101
			0.000				1" Ice 8.886	7.422	0.168
DB-T1-6Z-8AB-0Z (E)	A	From Leg	4.000	0.000	0.000	105.000	No Ice 4.800	2.000	0.044
			0.000				1/2" Ice 5.070	2.193	0.080
			0.000				1" Ice 5.348	2.393	0.120
(2) SBNHH-1D65B (P)	A	From Leg	4.000	0.000	0.000	105.000	No Ice 8.160	5.396	0.041
			0.000				1/2" Ice 8.619	5.853	0.091
			0.000				1" Ice 9.085	6.317	0.148
(2) SBNHH-1D65B (P)	B	From Leg	4.000	0.000	0.000	105.000	No Ice 8.160	5.396	0.041
			0.000				1/2" Ice 8.619	5.853	0.091
			0.000				1" Ice 9.085	6.317	0.148
(2) SBNHH-1D65B (P)	C	From Leg	4.000	0.000	0.000	105.000	No Ice 8.160	5.396	0.041
			0.000				1/2" Ice 8.619	5.853	0.091
			0.000				1" Ice 9.085	6.317	0.148
B13 RRH 4X30 (P)	A	From Leg	4.000	0.000	0.000	105.000	No Ice 2.055	1.320	0.056
			0.000				1/2" Ice 2.241	1.475	0.073
			0.000				1" Ice 2.433	1.638	0.093
B13 RRH 4X30 (P)	B	From Leg	4.000	0.000	0.000	105.000	No Ice 2.055	1.320	0.056
			0.000				1/2" Ice 2.241	1.475	0.073
			0.000				1" Ice 2.433	1.638	0.093
B13 RRH 4X30 (P)	C	From Leg	4.000	0.000	0.000	105.000	No Ice 2.055	1.320	0.056
			0.000				1/2" Ice 2.241	1.475	0.073
			0.000				1" Ice 2.433	1.638	0.093
DB-T1-6Z-8AB-0Z (P)	C	From Leg	4.000	0.000	0.000	105.000	No Ice 4.800	2.000	0.044
			0.000				1/2" Ice 5.070	2.193	0.080
			0.000				1" Ice 5.348	2.393	0.120
RRH4X45-AWS4 B66 (P)	A	From Leg	4.000	0.000	0.000	105.000	No Ice 2.660	1.586	0.064
			0.000				1/2" Ice 2.878	1.769	0.084
			0.000				1" Ice 3.104	1.959	0.108
RRH4X45-AWS4 B66 (P)	B	From Leg	4.000	0.000	0.000	105.000	No Ice 2.660	1.586	0.064
			0.000				1/2" Ice 2.878	1.769	0.084
			0.000				1" Ice 3.104	1.959	0.108
RRH4X45-AWS4 B66 (P)	C	From Leg	4.000	0.000	0.000	105.000	No Ice 2.660	1.586	0.064
			0.000				1/2" Ice 2.878	1.769	0.084
			0.000				1" Ice 3.104	1.959	0.108
B25 RRH4X30 (P)	A	From Leg	4.000	0.000	0.000	105.000	No Ice 2.120	1.293	0.053
			0.000				1/2" Ice 2.308	1.448	0.070
			0.000				1" Ice 2.504	1.611	0.090
B25 RRH4X30 (P)	B	From Leg	4.000	0.000	0.000	105.000	No Ice 2.120	1.293	0.053
			0.000				1/2" Ice 2.308	1.448	0.070
			0.000				1" Ice 2.504	1.611	0.090
B25 RRH4X30 (P)	C	From Leg	4.000	0.000	0.000	105.000	No Ice 2.120	1.293	0.053
			0.000				1/2" Ice 2.308	1.448	0.070
			0.000				1" Ice 2.504	1.611	0.090
Sector Mount [SM 402-3] (E)	C	None			0.000	105.000	No Ice 18.910	18.910	0.851
							1/2" Ice 26.780	26.780	1.233
							1" Ice 34.650	34.650	1.616
***%**									
APXV9ERR18-C-A20 w/ Mount Pipe (E)	A	From Leg	4.000	0.000	0.000	98.000	No Ice 8.262	7.471	0.088
			0.000				1/2" Ice 8.822	8.656	0.158
			1.000				1" Ice 9.346	9.556	0.237
APXVSPP18-C-A20 w/ Mount Pipe (E)	B	From Leg	4.000	0.000	0.000	98.000	No Ice 8.262	6.946	0.083
			0.000				1/2" Ice 8.822	8.127	0.151
			1.000				1" Ice 9.346	9.021	0.227
APXVSPP18-C-A20 w/ Mount Pipe	C	From Leg	4.000	0.000	0.000	98.000	No Ice 8.262	6.946	0.083
			0.000				1/2" Ice 8.822	8.127	0.151

<b>tnxTower</b>  <b>B+T Group</b> 1717 S Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b> 85565.006.01 - HARTFORD - NU (SSUSA),CT (BU# 876363)	<b>Page</b> 12 of 19
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	<b>Client</b> Crown Castle	<b>Designed by</b> CJangonda

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>A</sub>		Weight
			Horz	Vert			Front	Side	
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
(E)			1.000			1" Ice	9.346	9.021	0.227
LLPX310R w/ Mount Pipe (E-Clearwire)	A	From Leg	4.000		0.000	No Ice	4.538	2.985	0.045
			0.000			1/2" Ice	4.892	3.528	0.083
			1.000			1" Ice	5.254	4.087	0.126
LLPX310R w/ Mount Pipe (E-Clearwire)	B	From Leg	4.000		0.000	No Ice	4.538	2.985	0.045
			0.000			1/2" Ice	4.892	3.528	0.083
			1.000			1" Ice	5.254	4.087	0.126
LLPX310R w/ Mount Pipe (E-Clearwire)	C	From Leg	4.000		0.000	No Ice	4.538	2.985	0.045
			0.000			1/2" Ice	4.892	3.528	0.083
			1.000			1" Ice	5.254	4.087	0.126
APXVTM14-C-120 w/ Mount Pipe (E)	A	From Leg	4.000		0.000	No Ice	6.580	4.959	0.077
			0.000			1/2" Ice	7.031	5.754	0.132
			1.000			1" Ice	7.473	6.472	0.193
APXVTM14-C-120 w/ Mount Pipe (E)	B	From Leg	4.000		0.000	No Ice	6.580	4.959	0.077
			0.000			1/2" Ice	7.031	5.754	0.132
			1.000			1" Ice	7.473	6.472	0.193
APXVTM14-C-120 w/ Mount Pipe (E)	C	From Leg	4.000		0.000	No Ice	6.580	4.959	0.077
			0.000			1/2" Ice	7.031	5.754	0.132
			1.000			1" Ice	7.473	6.472	0.193
IBC1900BB-1 (E)	A	From Leg	4.000		0.000	No Ice	0.966	0.463	0.022
			0.000			1/2" Ice	1.091	0.558	0.030
			1.000			1" Ice	1.223	0.660	0.039
IBC1900BB-1 (E)	B	From Leg	4.000		0.000	No Ice	0.966	0.463	0.022
			0.000			1/2" Ice	1.091	0.558	0.030
			1.000			1" Ice	1.223	0.660	0.039
IBC1900BB-1 (E)	C	From Leg	4.000		0.000	No Ice	0.966	0.463	0.022
			0.000			1/2" Ice	1.091	0.558	0.030
			1.000			1" Ice	1.223	0.660	0.039
IBC1900HG-2A (E)	A	From Leg	4.000		0.000	No Ice	0.966	0.463	0.022
			0.000			1/2" Ice	1.091	0.558	0.030
			1.000			1" Ice	1.223	0.660	0.039
IBC1900HG-2A (E)	B	From Leg	4.000		0.000	No Ice	0.966	0.463	0.022
			0.000			1/2" Ice	1.091	0.558	0.030
			1.000			1" Ice	1.223	0.660	0.039
IBC1900HG-2A (E)	C	From Leg	4.000		0.000	No Ice	0.966	0.463	0.022
			0.000			1/2" Ice	1.091	0.558	0.030
			1.000			1" Ice	1.223	0.660	0.039
HORIZON COMPACT (E-Clearwire)	A	From Leg	4.000		0.000	No Ice	0.721	0.368	0.012
			0.000			1/2" Ice	0.828	0.450	0.018
			4.000			1" Ice	0.942	0.539	0.026
HORIZON COMPACT (E-Clearwire)	B	From Leg	4.000		0.000	No Ice	0.721	0.368	0.012
			0.000			1/2" Ice	0.828	0.450	0.018
			4.000			1" Ice	0.942	0.539	0.026
WIMAX DAP HEAD (E-Clearwire)	A	From Leg	4.000		0.000	No Ice	1.547	0.684	0.033
			0.000			1/2" Ice	1.704	0.800	0.045
			1.000			1" Ice	1.868	0.923	0.058
WIMAX DAP HEAD (E-Clearwire)	B	From Leg	4.000		0.000	No Ice	1.547	0.684	0.033
			0.000			1/2" Ice	1.704	0.800	0.045
			1.000			1" Ice	1.868	0.923	0.058
WIMAX DAP HEAD (E-Clearwire)	C	From Leg	4.000		0.000	No Ice	1.547	0.684	0.033
			0.000			1/2" Ice	1.704	0.800	0.045
			1.000			1" Ice	1.868	0.923	0.058
TD-RRH8x20-25 (E)	A	From Leg	4.000		0.000	No Ice	3.704	1.294	0.066
			0.000			1/2" Ice	3.946	1.465	0.090
			1.000			1" Ice	4.196	1.642	0.117
TD-RRH8x20-25 (E)	B	From Leg	4.000		0.000	No Ice	3.704	1.294	0.066
			0.000			1/2" Ice	3.946	1.465	0.090

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	<b>Client</b> Crown Castle	<b>Designed by</b> CJangonda

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz	Vert					
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
TD-RRH8x20-25 (E)	C	From Leg	1.000		0.000	98.000	1" Ice	1.642	0.117
			4.000				No Ice	1.294	0.066
			0.000				1/2" Ice	1.465	0.090
			1.000				1" Ice	1.642	0.117
Platform Mount [LP 1201-1] (E)	C	None			0.000	98.000	No Ice	23.100	2.100
							1/2" Ice	26.800	2.500
							1" Ice	30.500	2.900
**%**									
PCS 1900MHz 4x45W-65MHz (E)	A	From Leg	2.000		0.000	96.000	No Ice	2.238	0.060
			0.000				1/2" Ice	2.441	0.083
			0.000				1" Ice	2.651	0.110
			0.000						
PCS 1900MHz 4x45W-65MHz (E)	B	From Leg	2.000		0.000	96.000	No Ice	2.238	0.060
			0.000				1/2" Ice	2.441	0.083
			0.000				1" Ice	2.651	0.110
			0.000						
PCS 1900MHz 4x45W-65MHz (E)	C	From Leg	2.000		0.000	96.000	No Ice	2.238	0.060
			0.000				1/2" Ice	2.441	0.083
			0.000				1" Ice	2.651	0.110
			0.000						
PCS 1900MHz 4x45W-65MHz (E)	A	From Leg	2.000		0.000	96.000	No Ice	2.238	0.060
			0.000				1/2" Ice	2.441	0.083
			-1.000				1" Ice	2.651	0.110
			0.000						
PCS 1900MHz 4x45W-65MHz (E)	B	From Leg	2.000		0.000	96.000	No Ice	2.238	0.060
			0.000				1/2" Ice	2.441	0.083
			-1.000				1" Ice	2.651	0.110
			0.000						
PCS 1900MHz 4x45W-65MHz (E)	C	From Leg	2.000		0.000	96.000	No Ice	2.238	0.060
			0.000				1/2" Ice	2.441	0.083
			-1.000				1" Ice	2.651	0.110
			0.000						
800MHz 2X50W RRH W/FILTER (E)	A	From Leg	2.000		0.000	96.000	No Ice	1.932	0.064
			0.000				1/2" Ice	2.109	0.086
			0.000				1" Ice	2.293	0.111
			0.000						
800MHz 2X50W RRH W/FILTER (E)	B	From Leg	2.000		0.000	96.000	No Ice	1.932	0.064
			0.000				1/2" Ice	2.109	0.086
			0.000				1" Ice	2.293	0.111
			0.000						
800MHz 2X50W RRH W/FILTER (E)	C	From Leg	2.000		0.000	96.000	No Ice	1.932	0.064
			0.000				1/2" Ice	2.109	0.086
			0.000				1" Ice	2.293	0.111
			0.000						
4' x 2" Pipe Mount (E)	A	From Leg	1.000		0.000	96.000	No Ice	0.785	0.029
			0.000				1/2" Ice	1.028	0.035
			0.000				1" Ice	1.281	0.044
			0.000						
4' x 2" Pipe Mount (E)	B	From Leg	1.000		0.000	96.000	No Ice	0.785	0.029
			0.000				1/2" Ice	1.028	0.035
			0.000				1" Ice	1.281	0.044
			0.000						
4' x 2" Pipe Mount (E)	C	From Leg	1.000		0.000	96.000	No Ice	0.785	0.029
			0.000				1/2" Ice	1.028	0.035
			0.000				1" Ice	1.281	0.044
			0.000						
Side Arm Mount [SO 102-3] (E)	C	None			0.000	96.000	No Ice	3.000	0.081
							1/2" Ice	3.480	0.111
							1" Ice	3.960	0.141
**%**									
**%**									
HBX-6516DS-VTM w/ Mount Pipe (E)	A	From Leg	4.000		0.000	81.000	No Ice	3.241	0.029
			0.000				1/2" Ice	3.914	0.062
			-1.000				1" Ice	4.564	0.101
HBX-6516DS-VTM w/ Mount Pipe (E)	B	From Leg	4.000		0.000	81.000	No Ice	3.241	0.029
			0.000				1/2" Ice	3.914	0.062
			-1.000				1" Ice	4.564	0.101
HBX-6516DS-VTM w/ Mount Pipe (E)	C	From Leg	4.000		0.000	81.000	No Ice	3.241	0.029
			0.000				1/2" Ice	3.914	0.062

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz	Lateral					
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
(E)			-1.000				4.389	4.564	0.101
6' x 2" Mount Pipe	A	From Leg	4.000		0.000	81.000	No Ice 1.425	1.425	0.022
(E)			0.000				1/2" Ice 1.925	1.925	0.033
			-1.000				1" Ice 2.294	2.294	0.048
6' x 2" Mount Pipe	B	From Leg	4.000		0.000	81.000	No Ice 1.425	1.425	0.022
(E)			0.000				1/2" Ice 1.925	1.925	0.033
			-1.000				1" Ice 2.294	2.294	0.048
6' x 2" Mount Pipe	C	From Leg	4.000		0.000	81.000	No Ice 1.425	1.425	0.022
(E)			0.000				1/2" Ice 1.925	1.925	0.033
			-1.000				1" Ice 2.294	2.294	0.048
T-Arm Mount [TA 602-3]	C	None			0.000	81.000	No Ice 11.590	11.590	0.774
(E)							1/2" Ice 15.440	15.440	0.990
							1" Ice 19.290	19.290	1.206
***%**									
KS24019-L112A	A	From Leg	3.000		0.000	74.000	No Ice 0.141	0.141	0.005
(E)			0.000				1/2" Ice 0.198	0.198	0.007
			2.000				1" Ice 0.262	0.262	0.009
Side Arm Mount [SO 701-1]	A	From Leg	1.500		0.000	74.000	No Ice 0.850	1.670	0.065
(E)			0.000				1/2" Ice 1.140	2.340	0.079
			0.000				1" Ice 1.430	3.010	0.093
***%**									

### Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets:		Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weight
				Horz	Lateral						
			ft	ft	°	°	ft	ft	ft <sup>2</sup>	K	
A-ANT-18G-2-C	A	Paraboloid	From	4.000		52.000		98.000	2.175	No Ice 3.720	0.027
(E)		w/Shroud (HP)	Leg	0.000						1/2" Ice 4.010	0.048
				4.000						1" Ice 4.300	0.068
A-ANT-23G-1-C	B	Paraboloid	From	4.000		81.000		98.000	1.275	No Ice 1.280	0.007
(E)		w/Shroud (HP)	Leg	0.000						1/2" Ice 1.450	0.010
				4.000						1" Ice 1.620	0.012
***%**											

### Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 30 deg - No Ice
5	0.9 Dead+1.6 Wind 30 deg - No Ice
6	1.2 Dead+1.6 Wind 60 deg - No Ice
7	0.9 Dead+1.6 Wind 60 deg - No Ice
8	1.2 Dead+1.6 Wind 90 deg - No Ice
9	0.9 Dead+1.6 Wind 90 deg - No Ice

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Comb. No.	Description
10	1.2 Dead+1.6 Wind 120 deg - No Ice
11	0.9 Dead+1.6 Wind 120 deg - No Ice
12	1.2 Dead+1.6 Wind 150 deg - No Ice
13	0.9 Dead+1.6 Wind 150 deg - No Ice
14	1.2 Dead+1.6 Wind 180 deg - No Ice
15	0.9 Dead+1.6 Wind 180 deg - No Ice
16	1.2 Dead+1.6 Wind 210 deg - No Ice
17	0.9 Dead+1.6 Wind 210 deg - No Ice
18	1.2 Dead+1.6 Wind 240 deg - No Ice
19	0.9 Dead+1.6 Wind 240 deg - No Ice
20	1.2 Dead+1.6 Wind 270 deg - No Ice
21	0.9 Dead+1.6 Wind 270 deg - No Ice
22	1.2 Dead+1.6 Wind 300 deg - No Ice
23	0.9 Dead+1.6 Wind 300 deg - No Ice
24	1.2 Dead+1.6 Wind 330 deg - No Ice
25	0.9 Dead+1.6 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	108 - 98.5	17.607	50	1.615	0.010
L2	98.5 - 98	14.449	50	1.500	0.006
L3	98 - 82.33	14.292	50	1.498	0.006
L4	82.33 - 76.25	9.810	50	1.179	0.002
L5	76.25 - 74.5	8.382	50	1.062	0.002
L6	74.5 - 58.08	7.997	50	1.036	0.002
L7	58.08 - 57.75	4.845	50	0.789	0.001
L8	57.75 - 47	4.791	50	0.784	0.001
L9	50.25 - 40.5	3.638	50	0.683	0.001
L10	40.5 - 27.75	2.342	50	0.570	0.001
L11	27.75 - 19.5	1.074	50	0.379	0.000
L12	19.5 - 14	0.526	50	0.256	0.000

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L13	14 - 13.08	0.271	50	0.185	0.000
L14	13.08 - 13	0.237	50	0.174	0.000
L15	13 - 0	0.234	50	0.172	0.000

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
105.000	BXA-171063-12BF w/Mount Pipe	50	16.586	1.569	0.008	4114
102.000	A-ANT-18G-2-C	50	15.581	1.529	0.007	3536
98.000	APXV9ERR18-C-A20 w/ Mount Pipe	50	14.292	1.498	0.006	3319
96.000	PCS 1900MHz 4x45W-65MHz	50	13.671	1.480	0.005	3704
81.000	HBX-6516DS-VTM w/ Mount Pipe	50	9.483	1.149	0.002	2383
74.000	KS24019-L112A	50	7.889	1.029	0.002	3868

### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	108 - 98.5	82.319	24	7.541	0.046
L2	98.5 - 98	67.622	24	7.024	0.027
L3	98 - 82.33	66.891	24	7.012	0.026
L4	82.33 - 76.25	45.965	24	5.527	0.012
L5	76.25 - 74.5	39.280	24	4.982	0.009
L6	74.5 - 58.08	37.480	24	4.860	0.009
L7	58.08 - 57.75	22.719	24	3.700	0.005
L8	57.75 - 47	22.465	24	3.680	0.005
L9	50.25 - 40.5	17.059	24	3.207	0.004
L10	40.5 - 27.75	10.983	24	2.676	0.003
L11	27.75 - 19.5	5.039	24	1.776	0.002
L12	19.5 - 14	2.466	24	1.202	0.001
L13	14 - 13.08	1.273	24	0.870	0.001
L14	13.08 - 13	1.111	24	0.814	0.001
L15	13 - 0	1.097	24	0.809	0.001

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
105.000	BXA-171063-12BF w/Mount Pipe	24	77.572	7.335	0.038	928
102.000	A-ANT-18G-2-C	24	72.895	7.157	0.031	797
98.000	APXV9ERR18-C-A20 w/ Mount Pipe	24	66.891	7.012	0.026	743
96.000	PCS 1900MHz 4x45W-65MHz	24	63.993	6.933	0.025	823
81.000	HBX-6516DS-VTM w/ Mount Pipe	24	44.433	5.390	0.011	518
74.000	KS24019-L112A	24	36.974	4.826	0.009	837

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## Compression Checks

## Pole Design Data

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub>	φP <sub>n</sub>	Ratio
							K	K	$\frac{P_u}{\phi P_n}$
L1	108 - 98.5 (1)	TP8.625x8.625x0.313	9.500	0.000	0.0	8.161	-1.998	257.065	0.008
L2	98.5 - 98 (2)	TP16.5x8.625x0.313	0.500	0.000	0.0	8.161	-2.008	257.065	0.008
L3	98 - 82.33 (3)	TP19.008x16.5x0.188	15.670	0.000	0.0	11.201	-7.228	831.494	0.009
L4	82.33 - 76.25 (4)	TP19.981x19.008x0.323	6.080	0.000	0.0	20.160	-8.953	998.934	0.009
L5	76.25 - 74.5 (5)	TP20.261x19.981x0.462	1.750	0.000	0.0	29.014	-9.239	1267.950	0.007
L6	74.5 - 58.08 (6)	TP22.89x20.261x0.537	16.420	0.000	0.0	38.094	-12.282	1532.640	0.008
L7	58.08 - 57.75 (7)	TP22.942x22.89x0.698	0.330	0.000	0.0	49.259	-12.365	1967.610	0.006
L8	57.75 - 47 (8)	TP24.663x22.942x0.683	10.750	0.000	0.0	50.873	-14.026	2131.560	0.007
L9	47 - 40.5 (9)	TP25.328x23.768x0.723	9.750	0.000	0.0	56.448	-17.079	2361.020	0.007
L10	40.5 - 27.75 (10)	TP27.369x25.328x0.677	12.750	0.000	0.0	57.340	-20.428	2404.560	0.008
L11	27.75 - 19.5 (11)	TP28.689x27.369x0.689	8.250	0.000	0.0	61.203	-22.796	2790.120	0.008
L12	19.5 - 14 (12)	TP29.569x28.689x0.799	5.500	0.000	0.0	72.962	-24.493	3303.600	0.007
L13	14 - 13.08 (13)	TP29.717x29.569x0.787	0.920	0.000	0.0	72.300	-24.800	3202.980	0.008
L14	13.08 - 13 (14)	TP29.729x29.717x0.752	0.080	0.000	0.0	69.185	-24.837	2953.530	0.008
L15	13 - 0 (15)	TP31.81x29.729x0.762	13.000	0.000	0.0	75.059	-29.247	3228.500	0.009

## Pole Bending Design Data

Section No.	Elevation ft	Size	M <sub>ux</sub>	φM <sub>ux</sub>	Ratio	M <sub>uy</sub>	φM <sub>uy</sub>	Ratio
			kip-ft	kip-ft	$\frac{M_{ux}}{\phi M_{ux}}$	kip-ft	kip-ft	$\frac{M_{uy}}{\phi M_{uy}}$
L1	108 - 98.5 (1)	TP8.625x8.625x0.313	40.250	56.708	0.710	0.000	56.708	0.000
L2	98.5 - 98 (2)	TP16.5x8.625x0.313	40.250	56.708	0.710	0.000	56.708	0.000
L3	98 - 82.33 (3)	TP19.008x16.5x0.188	246.313	321.129	0.767	0.000	321.129	0.000
L4	82.33 - 76.25 (4)	TP19.981x19.008x0.323	338.078	400.398	0.844	0.000	400.398	0.000
L5	76.25 - 74.5 (5)	TP20.261x19.981x0.462	366.051	508.446	0.720	0.000	508.446	0.000
L6	74.5 - 58.08 (6)	TP22.89x20.261x0.537	649.468	693.347	0.937	0.000	693.347	0.000
L7	58.08 - 57.75 (7)	TP22.942x22.89x0.698	655.534	879.517	0.745	0.000	879.517	0.000
L8	57.75 - 47 (8)	TP24.663x22.942x0.683	799.727	1007.033	0.794	0.000	1007.033	0.000
L9	47 - 40.5 (9)	TP25.328x23.768x0.723	1000.875	1169.633	0.856	0.000	1169.633	0.000
L10	40.5 - 27.75 (10)	TP27.369x25.328x0.677	1284.742	1297.283	0.990	0.000	1297.283	0.000
L11	27.75 - 19.5 (11)	TP28.689x27.369x0.689	1480.233	1580.258	0.937	0.000	1580.258	0.000
L12	19.5 - 14 (12)	TP29.569x28.689x0.799	1615.383	1916.592	0.843	0.000	1916.592	0.000
L13	14 - 13.08 (13)	TP29.717x29.569x0.787	1638.350	1869.483	0.876	0.000	1869.483	0.000
L14	13.08 - 13 (14)	TP29.729x29.717x0.752	1640.358	1728.858	0.949	0.000	1728.858	0.000
L15	13 - 0 (15)	TP31.81x29.729x0.762	1975.983	2027.717	0.974	0.000	2027.717	0.000



**Pole Shear Design Data**

Section No.	Elevation ft	Size	Actual	$\phi V_n$	Ratio	Actual	$\phi T_n$	Ratio
			$V_u$ K	K	$V_n$ $\phi V_n$	$T_u$ kip-ft	$T_n$ kip-ft	$T_u$ $\phi T_n$
L1	108 - 98.5 (1)	TP8.625x8.625x0.313	6.431	128.532	0.050	0.375	85.931	0.004
L2	98.5 - 98 (2)	TP16.5x8.625x0.313	6.478	250.300	0.026	0.384	85.931	0.004
L3	98 - 82.33 (3)	TP19.008x16.5x0.188	13.827	415.747	0.033	0.526	643.044	0.001
L4	82.33 - 76.25 (4)	TP19.981x19.008x0.323	15.866	499.467	0.032	0.574	801.777	0.001
L5	76.25 - 74.5 (5)	TP20.261x19.981x0.462	16.116	633.977	0.025	0.583	1018.133	0.001
L6	74.5 - 58.08 (6)	TP22.89x20.261x0.537	18.364	766.319	0.024	0.609	1388.392	0.000
L7	58.08 - 57.75 (7)	TP22.942x22.89x0.698	18.403	983.803	0.019	0.610	1761.192	0.000
L8	57.75 - 47 (8)	TP24.663x22.942x0.683	19.837	1065.780	0.019	0.154	2016.525	0.000
L9	47 - 40.5 (9)	TP25.328x23.768x0.723	21.382	1180.510	0.018	0.144	2342.125	0.000
L10	40.5 - 27.75 (10)	TP27.369x25.328x0.677	23.174	1202.280	0.019	0.132	2597.733	0.000
L11	27.75 - 19.5 (11)	TP28.689x27.369x0.689	24.244	1395.060	0.017	0.134	3164.383	0.000
L12	19.5 - 14 (12)	TP29.569x28.689x0.799	24.924	1651.800	0.015	0.143	3837.867	0.000
L13	14 - 13.08 (13)	TP29.717x29.569x0.787	25.034	1601.490	0.016	0.144	3743.533	0.000
L14	13.08 - 13 (14)	TP29.729x29.717x0.752	25.033	1476.760	0.017	0.144	3461.950	0.000
L15	13 - 0 (15)	TP31.81x29.729x0.762	26.616	1614.250	0.016	0.183	4060.383	0.000

**Pole Interaction Design Data**

Section No.	Elevation ft	Ratio	Ratio	Ratio	Ratio	Ratio	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
		$P_n$ $\phi P_n$	$M_{ux}$ $\phi M_{ux}$	$M_{uy}$ $\phi M_{uy}$	$V_u$ $\phi V_u$	$T_n$ $\phi T_n$			
L1	108 - 98.5 (1)	0.008	0.710	0.000	0.050	0.004	0.721	1.000	4.8.2 ✓
L2	98.5 - 98 (2)	0.008	0.710	0.000	0.026	0.004	0.719	1.000	4.8.2 ✓
L3	98 - 82.33 (3)	0.009	0.767	0.000	0.033	0.001	0.777	1.000	4.8.2 ✓
L4	82.33 - 76.25 (4)	0.009	0.844	0.000	0.032	0.001	0.854	1.000	4.8.2 ✓
L5	76.25 - 74.5 (5)	0.007	0.720	0.000	0.025	0.001	0.728	1.000	4.8.2 ✓
L6	74.5 - 58.08 (6)	0.008	0.937	0.000	0.024	0.000	0.945	1.000	4.8.2 ✓
L7	58.08 - 57.75 (7)	0.006	0.745	0.000	0.019	0.000	0.752	1.000	4.8.2 ✓
L8	57.75 - 47 (8)	0.007	0.794	0.000	0.019	0.000	0.801	1.000	4.8.2 ✓
L9	47 - 40.5 (9)	0.007	0.856	0.000	0.018	0.000	0.863	1.000	4.8.2 ✓
L10	40.5 - 27.75 (10)	0.008	0.990	0.000	0.019	0.000	0.999	1.000	4.8.2 ✓
L11	27.75 - 19.5 (11)	0.008	0.937	0.000	0.017	0.000	0.945	1.000	4.8.2 ✓
L12	19.5 - 14 (12)	0.007	0.843	0.000	0.015	0.000	0.850	1.000	4.8.2 ✓

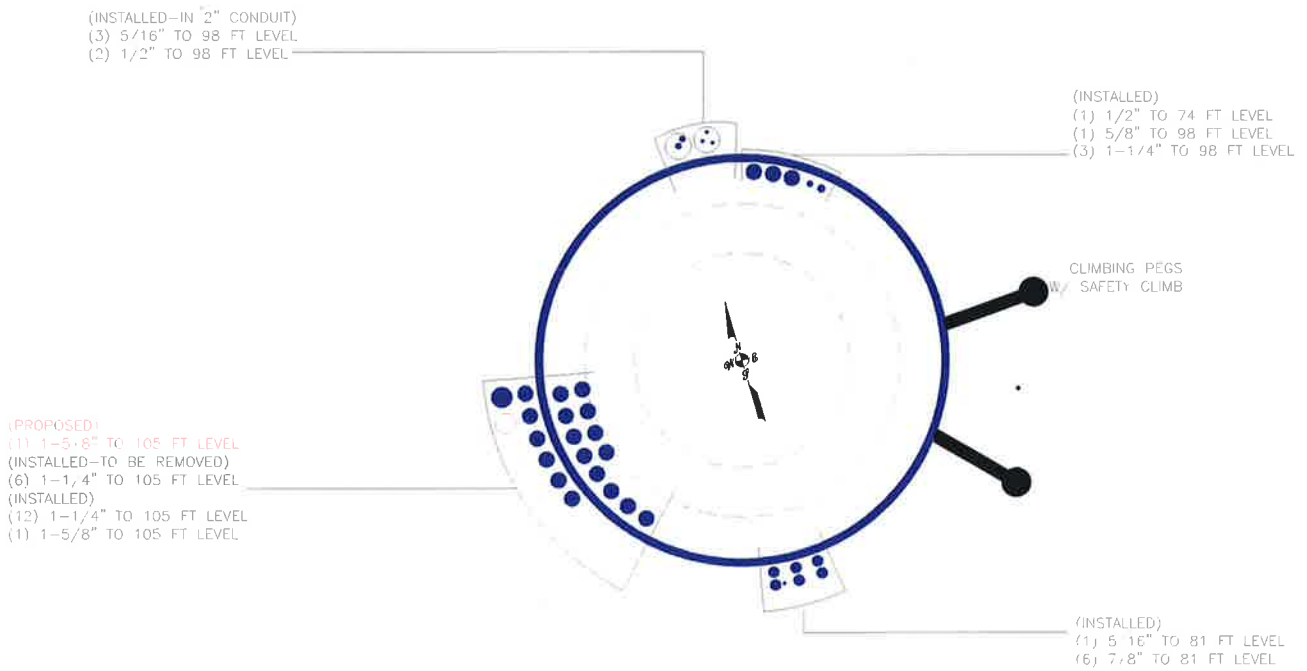
<b>tnxTower</b>  <b>B+T Group</b> 1717 S Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b> 85565.006.01 - HARTFORD - NU (SSUSA),CT (BU# 876363)	<b>Page</b> 19 of 19
	<b>Project</b>	<b>Date</b> 18:16:28 05/02/17
	<b>Client</b> Crown Castle	<b>Designed by</b> CJangonda

Section No.	Elevation ft	Ratio $P_u$	Ratio $M_{ux}$	Ratio $M_{uy}$	Ratio $V_u$	Ratio $T_u$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
		$\phi P_n$	$\phi M_{ux}$	$\phi M_{uy}$	$\phi V_n$	$\phi T_n$			
L13	14 - 13.08 (13)	0.008	0.876	0.000	0.016	0.000	0.884	1.000	4.8.2 ✓
L14	13.08 - 13 (14)	0.008	0.949	0.000	0.017	0.000	0.958	1.000	4.8.2 ✓
L15	13 - 0 (15)	0.009	0.974	0.000	0.016	0.000	0.984	1.000	4.8.2 ✓

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail	
L1	108 - 98.5	Pole	TP8.625x8.625x0.313	1	-1.998	257.065	71.8	Pass	
L2	98.5 - 98	Pole	TP16.5x8.625x0.313	2	-2.008	257.065	22.8	Pass	
L3	98 - 82.33	Pole	TP19.008x16.5x0.188	3	-7.228	831.494	77.5	Pass	
L4	82.33 - 76.25	Pole	TP19.981x19.008x0.323	4	-8.953	998.934	83.2	Pass	
L5	76.25 - 74.5	Pole	TP20.261x19.981x0.462	5	-9.239	1267.950	69.9	Pass	
L6	74.5 - 58.08	Pole	TP22.89x20.261x0.537	6	-12.282	1532.640	92.1	Pass	
L7	58.08 - 57.75	Pole	TP22.942x22.89x0.698	7	-12.365	1967.610	73.3	Pass	
L8	57.75 - 47	Pole	TP24.663x22.942x0.683	8	-14.026	2131.560	78.1	Pass	
L9	47 - 40.5	Pole	TP25.328x23.768x0.723	9	-17.079	2361.020	84.2	Pass	
L10	40.5 - 27.75	Pole	TP27.369x25.328x0.677	10	-20.428	2404.560	97.4	Pass	
L11	27.75 - 19.5	Pole	TP28.689x27.369x0.689	11	-22.796	2790.120	92.1	Pass	
L12	19.5 - 14	Pole	TP29.569x28.689x0.799	12	-24.493	3303.600	90.5	Pass	
L13	14 - 13.08	Pole	TP29.717x29.569x0.787	13	-24.800	3202.980	91.0	Pass	
L14	13.08 - 13	Pole	TP29.729x29.717x0.752	14	-24.837	2953.530	94.1	Pass	
L15	13 - 0	Pole	TP31.81x29.729x0.762	15	-29.247	3228.500	94.9	Pass	
						Summary			
						Pole (L10)		97.4	Pass
						<b>RATING =</b>		<b>97.4</b>	<b>Pass</b>

**APPENDIX B**  
**BASE LEVEL DRAWING**



BUSINESS UNIT:876565

**APPENDIX C**  
**ADDITIONAL CALCULATIONS**

Date	Time	Activity	Stress Level		Time Use	
			Low	High	Effective	Ineffective
10/10/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/11/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/12/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/13/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/14/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/15/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/16/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/17/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/18/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/19/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/20/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/21/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/22/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/23/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/24/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/25/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/26/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/27/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/28/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/29/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/30/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0
10/31/2020	8:00-9:00	Commute	0	0	0	0
	9:00-10:00	Work	0	0	0	0



# Reinforcement Capacity

Dimensions and Properties										Compression				Axial		LRFD						
Model	Weight (lb/ft)	Area (in <sup>2</sup> )	Moment of Inertia (in <sup>4</sup> )	Moment of Inertia (in <sup>4</sup> )	Centroid from Mating Edge (in)	Centroid from Bolt Hole Center (in)	Web Thickness (in)	Width (in)	Flange Width (in)	Flange Thickness (in)	Hole Diameter (in)	Yield Stress (ksi)	Ultimate Stress (ksi)	Slender. Ratio Coefficient	Unbraced Length (in)	Slender. Ratio Coefficient	Unbraced Length (in)	Allowable Axial (kip)	Allowable Axial w/ Increase (kip)	Governing Axial	Design Axial Strength (kip)	Governing Axial
MF303	9.9	2.92	0.66	6.57	0.59	0	0.30	4.06	1.57	0.64	1.21875	65	80	0.80	18	1.00	18	96.3	128.6	Rupture	144.7	Rupture
MF305	19.2	5.65	2.15	20.79	0.79	0	0.5	5.33	2.09	0.91	1.21875	65	80	0.80	18	1.00	18	198.5	269.3	Rupture	291.8	Rupture
VS-1.25M4	17.0	5.00	0.65	6.67	0.625	0	1.25	4	0	0	1.21875	65	80	0.80	21	1.00	21	135.9	181.3	Rupture	202.8	Rupture
VS-1.25M4.375	18.6	5.47	0.71	8.72	0.625	0	1.25	4.375	0	0	1.21875	65	80	0.80	21	1.00	21	154.7	206.3	Rupture	272.4	Rupture
VS-1.25M5.25	22.3	6.56	0.85	15.07	0.625	0	1.25	5.25	0	0	1.21875	65	80	0.80	18	1.00	18	198.6	266.6	Rupture	297.7	Rupture
CCJ-104.5	15.3	4.50	0.38	7.59	0.5	0	1	4.5	0	0	1.21875	65	80	0.80	20	1.00	20	128.8	173.7	Rupture	199.1	Rupture



# Stiffened or Unstiffened, Exterior Flange Plate - Any Bolt Material TIA Rev G

## Site Data

BU#: 876363  
 Site Name: HARTFORD - NU (SSUS)  
 App #: 382014 Rev. 1

Reactions		
Mu	40.25	ft-kips
Axial, Pu:	2.008	kips
Shear, Vu:	6.478	kips
Elevation:	98	feet

Bolt Threads:
X-Excluded
$\phi V_n = \phi(0.55 \cdot A_b \cdot F_u)$
$\phi = 0.75, \phi \cdot V_n$ (kips):
21.87

Pole Manufacturer: Other

If No stiffeners, Criteria: TIA G <-Only Applicable to Unstiffened Cases

## Bolt Data

Qty:	9	
Diameter (in.):	0.75	Bolt Fu: 120
Bolt Material:	A325	Bolt Fy: 92
N/A:	75	<-- Disregard
N/A:	55	<-- Disregard
Circle (in.):	19.5	

## Flange Bolt Results

Bolt Tension Capacity, $\phi \cdot T_n, B1$ :	30.06 kips
Adjusted $\phi \cdot T_n$ (due to $V_u = V_u / Q_t$ ), B:	30.04 kips
Max Bolt directly applied Tu:	10.79 Kips
Min. PL "tc" for B cap. w/o Pry:	1.858 in
Min PL "treq" for actual T w/ Pry:	0.846 in
Min PL "t1" for actual T w/o Pry:	1.113 in
T allowable with Prying:	15.06 kips $\alpha > 1$ case
Prying Force, q:	4.01 kips
Total Bolt Tension = Tu + q:	14.80 kips
Prying Bolt Stress Ratio = (Tu + q) / (B):	49.2% Pass

Non-Rigid
$\phi \cdot T_n$
$\phi T_n [(1 - (V_u / \phi V_n))^2]^{0.5}$

## Plate Data

Diam:	24	in
Thick, t:	1	in
Grade (Fy):	50	ksi
Strength, Fu:	65	ksi
Single-Rod B-eff:	3.01	in

## Exterior Flange Plate Results

Flexural Check	
Compression Side Plate Stress:	26.5 ksi
Allowable Plate Stress:	45.0 ksi
Compression Plate Stress Ratio:	58.8% Pass
Prying Occurs, PL Check:	
Tension Side Stress Ratio, $(treq/t)^2$ :	71.6% Pass

Non-Rigid
TIA G
$\phi \cdot F_y$
Comp. Y.L. Length:
17.49

## Stiffener Data (Welding at Both Sides)

Config:	0	*
Weld Type:		
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		<-- Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

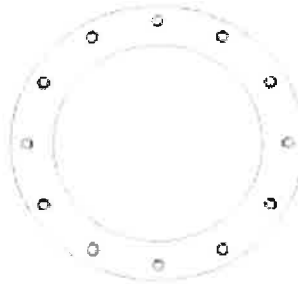
n/a

## Stiffener Results

Horizontal Weld :	n/a
Vertical Weld:	n/a
Plate Flex+Shear, $f_b / F_b + (f_v / F_v)^2$ :	n/a
Plate Tension+Shear, $f_t / F_t + (f_v / F_v)^2$ :	n/a
Plate Comp. (AISC Bracket):	n/a

## Pole Results

Pole Punching Shear Check: n/a



## Pole Data

Diam:	8.625	in
Thick:	0.3125	in
Grade:	35	ksi
# of Sides:	0	"0" IF Round
Fu	60	ksi
Reinf. Fillet Weld	0	"0" if None

\* 0 = none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

\*\* Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

# Stiffened or Unstiffened, Exterior Flange Plate - Any Bolt Material TIA Rev G

## Site Data

BU#: 876363  
 Site Name: HARTFORD - NU (SSUS)  
 App #: 382014 Rev. 1

Reactions		
Mu	40.25	ft-kips
Axial, Pu:	2.008	kips
Shear, Vu:	6.478	kips
Elevation:	98	feet

Bolt Threads:	
X-Excluded	
$\phi V_n = \phi(0.55 \cdot A_b \cdot F_u)$	
$\phi = 0.75, \phi \cdot V_n$ (kips):	
21.87	

Pole Manufacturer: Other

If No stiffeners, Criteria: TIA G <-Only Applicable to Unstiffened Cases

Bolt Data		
Qty:	9	
Diameter (in.):	0.75	Bolt Fu: 120
Bolt Material:	A325	Bolt Fy: 92
N/A:	75	<-- Disregard
N/A:	55	<-- Disregard
Circle (in.):	19.5	

Flange Bolt Results		Rigid	
Bolt Tension Capacity, $\phi \cdot T_n, B1$ :	30.06 kips	$\phi \cdot T_n$	
Adjusted $\phi \cdot T_n$ (due to $V_u = V_u / Q_t$ ), B:	30.04 kips	$\phi T_n [(1 - (V_u / \phi V_n)^2)^{0.5}]$	
Max Bolt directly applied Tu:	10.79 Kips		
Min. PL "tc" for B cap. w/o Pry:	0.633 in		
Min PL "treq" for actual T w/ Pry:	0.278 in		
Min PL "t1" for actual T w/o Pry:	0.379 in		
T allowable w/o Prying:	30.06 kips	$\alpha' < 0$ case	
Prying Force, q:	0.00 kips		
Total Bolt Tension = Tu + q:	10.79 kips		
Non-Prying Bolt Stress Ratio, Tu/B:	35.9% Pass		

Plate Data		
Diam:	24	in
Thick, t:	1	in
Grade (Fy):	50	ksi
Strength, Fu:	65	ksi
Single-Rod B-eff:	5.82	in

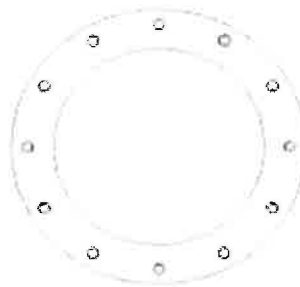
Exterior Flange Plate Results		Flexural Check		Rigid	
Compression Side Plate Stress:	7.4 ksi			TIA G	
Allowable Plate Stress:	45.0 ksi			$\phi \cdot F_y$	
Compression Plate Stress Ratio:	16.5% Pass			Comp. Y.L. Length:	
				10.39	
No Prying					
Tension Side Stress Ratio, (treq/t)^2:	7.7% Pass				

Stiffener Data (Welding at Both Sides)		
Config:	0	*
Weld Type:		
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		<-- Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

n/a  
**Stiffener Results**  
 Horizontal Weld : n/a  
 Vertical Weld: n/a  
 Plate Flex+Shear,  $f_b / F_b + (f_v / F_v)^2$ : n/a  
 Plate Tension+Shear,  $f_t / F_t + (f_v / F_v)^2$ : n/a  
 Plate Comp. (AISC Bracket): n/a

**Pole Results**  
 Pole Punching Shear Check: n/a

Pole Data		
Diam:	16.5	in
Thick:	0.1875	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None



\* 0 = none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

\*\* Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

# Anchor Rod Information for TIA/EIA-222-F and TIA-222-G-2

Site Information	
ID:	876363
Name:	HARTFORD - NU -SSUSA-
App. #:	382014; Rev.1



Base Reactions	
Moment:	1976 ft-kip
Axial:	29 kip
Shear:	27 kip
Base Plate Type:	Square

Design Information	
TIA Code:	G
ASIF:	1.000
Failure:	100%
eta Factor:	0.50

Original Anchor Rod Data	
Quantity:	8
Diameter:	2.25 in
Material:	A615 GR 75
Bolt Circle:	38.0 in
Bolt Spacing:	6 in
Bolt Group Area:	31.81 in <sup>2</sup>
Bolt Group MOIx:	5741 in <sup>4</sup>

Reactions Seen by Original AR Group

Moment:	1162.4 kip-ft
Axial:	29.2 kip
Shear:	26.6 kip

Original AR Capacity Check

Combined Load:	159.0 kip
Allowable load:	259.8 kip
AR Capacity:	61.2% <b>Pass</b>

First Added Anchor Rod Data	
Quantity:	3
Diameter:	2.25 in
Material:	A615 GR 75
Bolt Circle:	43.8 in
Bolt Group Area:	11.93 in <sup>2</sup>
Bolt Group MOIx:	2023 in <sup>4</sup>

Reactions Seen by First Added AR Group

Moment:	409.7 kip-ft
Axial:	0.0 kip
Shear:	0.0 kip

First Added AR Capacity Check

Combined Load:	211.7 kip
Allowable load:	259.8 kip
AR Capacity:	81.5% <b>Pass</b>

Second Added Anchor Rod Data	
Quantity:	3
Diameter:	2.25 in
Material:	A193 B7
Bolt Circle:	43.5 in
Bolt Group Area:	11.93 in <sup>2</sup>
Bolt Group MOIx:	1995 in <sup>4</sup>

Reactions Seen by Second Added AR Group

Moment:	403.9 kip-ft
Axial:	0.0 kip
Shear:	0.0 kip

Second Added AR Capacity Check

Combined Load:	210.1 kip
Allowable load:	324.8 kip
AR Capacity:	64.7% <b>Pass</b>

Third Added Anchor Rod Data	
Quantity:	
Diameter:	in
Material:	
Bolt Circle:	in
Bolt Group Area:	0.00 in <sup>2</sup>
Bolt Group MOIx:	0 in <sup>4</sup>

Reactions Seen by Second Added AR Group

Moment:	0.0 kip-ft
Axial:	0.0 kip
Shear:	0.0 kip

Second Added AR Capacity Check

Combined Load:	0.0 kip
Allowable load:	0.0 kip
AR Capacity:	0.0%

# Square, Stiffened / Unstiffened Base Plate, Any Rod Material - Rev. F /G

- Assumptions: 1) Rod groups at corners. Total # rods divisible by 4. Maximum total # of rods = 48 (12 per Corner).  
 2) Rod Spacing = Straight Center-to-Center distance between any (2) adjacent rods (same corner)  
 3) Clear space between bottom of leveling nut and top of concrete **not exceeding** (1)\*(Rod Diameter)

## Site Data

BU#: 876363  
 Site Name: HARTFORD - NU -SSUSA  
 App #: 382014; Rev. 1

## Anchor Rod Data

Eta Factor, $\eta$	0.5	TIA G (Fig. 4-4)
Qty:	8	
Diam:	2.25	in
Rod Material:	A615-J	
Yield, Fy:	75	ksi
Strength, Fu:	100	ksi
Bolt Circle:	38	in
Anchor Spacing:	6	in

## Plate Data

W=Side:	36	in
Thick:	2.5	in
Grade:	55	ksi
Clip Distance:	4	in

## Stiffener Data (Welding at both sides)

Configuration:	Unstiffened	
Weld Type:		**
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		<-- Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

## Pole Data

Diam:	31.81	in
Thick:	0.25	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round

## Base Reactions

TIA Revision:	G	
Factored Moment, Mu:	941	ft-kips
Factored Axial, Pu:	29	kips
Factored Shear, Vu:	27	kips

## Anchor Rod Results

TIA G --> Max Rod (Cu+ Vu/ $\eta$ ): 159.0 Kips  
 Axial Design Strength,  $\Phi * F_u * A_{net}$ : 260.0 Kips  
 Anchor Rod Stress Ratio: 61.1% **Pass**

## Base Plate Results

Base Plate Stress: 28.8 ksi  
 PL Design Bending Strength,  $\Phi * F_y$ : 49.5 ksi  
 Base Plate Stress Ratio: 58.1% **Pass**

## Flexural Check

## PL Ref. Data

Yield Line (in):	19.10
Max PL Length:	19.10

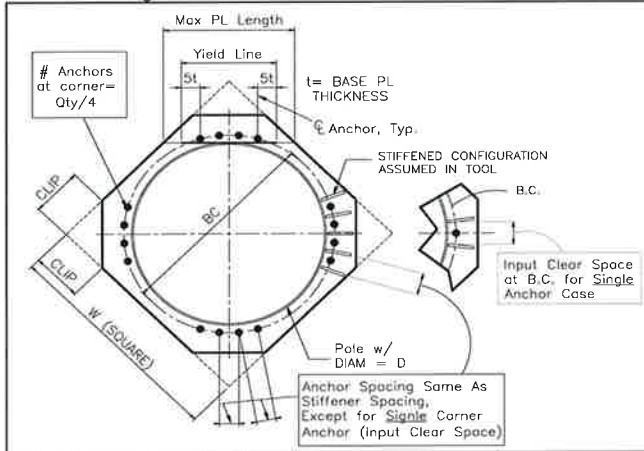
## N/A - Unstiffened

## Stiffener Results

Horizontal Weld : N/A  
 Vertical Weld: N/A  
 Plate Flex+Shear,  $f_b/F_b + (f_v/F_v)^2$ : N/A  
 Plate Tension+Shear,  $f_t/F_t + (f_v/F_v)^2$ : N/A  
 Plate Comp. (AISC Bracket): N/A

## Pole Results

Pole Punching Shear Check: N/A



\*\* Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

Proj. Number 85565.006.01  
 Proj. Name HARTFORD - NU (SSUSA), C  
 Code Rev. G

**Proposed Anchor Rods**

Diameter	2.25	in
Grade	A615-75	
Quantity	3	
Bolt Circle	43.81	in
AR Capacity	260	kips

**Existing Mfg Anchor Rods**

Diameter	2.25	in
Quantity	8	
Bolt Circle	38	in

**Tower Properties**

F <sub>ypole</sub> =	65	ksi
F <sub>Upole</sub> =	80	ksi
F <sub>ybase</sub> =	55	ksi
F <sub>Ubase</sub> =	70	ksi

**Foundation Properties**

Type	Pier	
Pier Diameter	5	ft
f <sub>c</sub>	3000	psi
Clear Cover	4	inch
Pad Width	7	ft
Vert.Rebar Size	11	
Vert. Quantity	12	
Tie Size	5	
f <sub>y rebar</sub>	60	ksi

<b>Summary Output</b>		
<b>- Anchor Rod Checks</b>		
Min. Embedment Length:	6.5	ft
Min. Pull Test Value:	152	kips
<b>- Anchor Rod Bracket Checks</b>		
Tube Stress:	98.0%	
Bracket Plate Check:	OK	
Max. Weld Stress:	62.8%	

**Anchor Rod Bracket Properties**

**Gusset Properties**

Thickness	1.25	inch
Pole to Tube CL	6	inch
Height	32	inch
Width at Tube	4	inch
F <sub>yplate</sub> =	65	ksi
F <sub>Uplate</sub> =	80	ksi
Gap =	0	inch
Notch =	0.75	inch

**Pipe /Tube Properties**

Size	HSS4x4x1/2	
L <sub>pipe</sub> =	10.5	inch
F <sub>y pipe</sub> =	50	ksi
D <sub>pipe</sub> =	4	inch
t <sub>pipe</sub> =	0.5	inch
A <sub>pipe</sub> =	6.02	inch <sup>2</sup>
I <sub>pipe</sub> =	11.9	inch <sup>4</sup>
r <sub>pipe</sub> =	1.41	inch

**Weld Properties**

F <sub>EXX</sub> =	70	ksi	Weld Material Grade
<b>- Bracket to Tube Weld</b>			
D <sub>v pipe</sub> =	7		Vertical fillet weld size in sixteenths
l <sub>weld pipe</sub> =	10.5	inch	Length of Vertical Weld to Pipe
<b>- Bracket to Pole Weld</b>			
Weld Type	Double Fillet		
Fillet Size	5		Vertical fillet weld size in sixteenths
H =	32	inch	Height of vertical weld from base plate
<b>- Base Plate Welds</b>			
D <sub>Hbp</sub> =	0.5625	inch	Gusset Bevel Size

**Additional Variables**

C <sub>1</sub> =	1.00	Electrode Strength Coefficient
K <sub>r1</sub> =	0	Transverse Reinforcement Index :
ψ <sub>1</sub> =	1	Rebar Location Factor :
ψ <sub>e</sub> =	1	Rebar Coatig Factor :
ψ <sub>s</sub> =	1	Rebar Size Factor :
λ =	1	Concrete Weight Factor :

Proj. Number 85565.006.01  
Proj. Name HARTFORD - NU (SSUSA), C  
Code Rev. G

**Proposed Anchor Rods**

Diameter	2.25	in
Grade	A193 Gr B7	
Quantity	3	
Bolt Circle	43.51	in
AR Capacity	325	kips

**Existing Mfg Anchor Rods**

Diameter	2.25	in
Quantity	8	
Bolt Circle	38	in

**Tower Properties**

F <sub>y pole</sub> =	65	ksi
F <sub>u pole</sub> =	80	ksi
F <sub>y base</sub> =	55	ksi
F <sub>u base</sub> =	70	ksi

**Foundation Properties**

Type	Pier	
Pier Diameter	5	ft
f <sub>c</sub>	3000	psi
Clear Cover	4	inch
Pad Width	7	ft
Vert. Rebar Size	11	
Vert. Quantity	12	
Tie Size	5	
f <sub>y rebar</sub>	60	ksi

<b>Summary Output</b>		
<b>- Anchor Rod Checks</b>		
Min. Embedment Length:	6.5	ft
Min. Pull Test Value:	190	kips
<b>- Anchor Rod Bracket Checks</b>		
Tube Stress:	91.7%	
Bracket Plate Check:	OK	
Max. Weld Stress:	82.4%	

**Anchor Rod Bracket Properties**



**Gusset Properties**

Thickness	1.25	inch
Pole to Tube CL	5.875	inch
Height	32	inch
Width at Tube	3.375	inch
F <sub>y plate</sub> =	65	ksi
F <sub>u plate</sub> =	80	ksi
Gap =	0	inch
Notch =	0.75	inch

**Pipe /Tube Properties**

Size	HSS5x5x1/2	
L <sub>pipe</sub> =	10.5	inch
F <sub>y pipe</sub> =	50	ksi
D <sub>pipe</sub> =	5	inch
t <sub>pipe</sub> =	0.5	inch
A <sub>pipe</sub> =	7.88	inch <sup>2</sup>
I <sub>pipe</sub> =	26	inch <sup>4</sup>
r <sub>pipe</sub> =	1.82	inch

**Weld Properties**

F <sub>EXX</sub> =	70	ksi	Weld Material Grade
<b>- Bracket to Tube Weld</b>			
D <sub>v pipe</sub> =	7		Vertical fillet weld size in sixteenths
l <sub>weld pipe</sub> =	10.5	inch	Length of Vertical Weld to Pipe
<b>- Bracket to Pole Weld</b>			
Weld Type	Double Fillet		
Fillet Size	5		Vertical fillet weld size in sixteenths
H =	32	inch	Height of vertical weld from base plate
<b>- Base Plate Welds</b>			
D <sub>t bp</sub> =	0.5625	inch	Gusset Bevel Size

**Additional Variables**

C <sub>1</sub> =	1.00	Electrode Strength Coefficient
K <sub>rt</sub> =	0	Transverse Reinforcement Index :
ψ <sub>1</sub> =	1	Rebar Location Factor :
ψ <sub>e</sub> =	1	Rebar Coatig Factor :
ψ <sub>s</sub> =	1	Rebar Size Factor :
λ =	1	Concrete Weight Factor :

PROJECT	<b>876363 - HARTFORD - NU (SSUSA), CT</b>		
SUBJECT	<b>Foundation Analysis</b>		
DATE	<b>05/16/17</b>	PAGE	1 OF 1



## Monopole Pad & Pier Foundation Analysis

Rev. Type: **G**

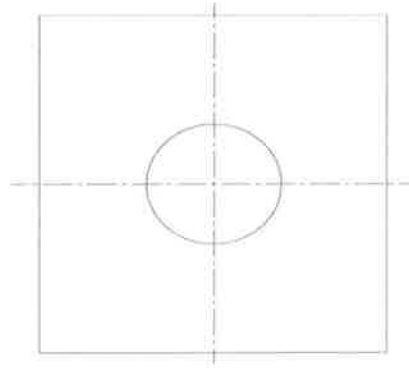
Design Loads:

	Input factored loads	
Shear:	<u>27.0</u>	kips
Moment:	<u>1,976.0</u>	ft-kips
Tower Height:	<u>108.0</u>	ft
Tower Weight:	<u>29.0</u>	kips

Pad & Pier Dimensions / Properties:

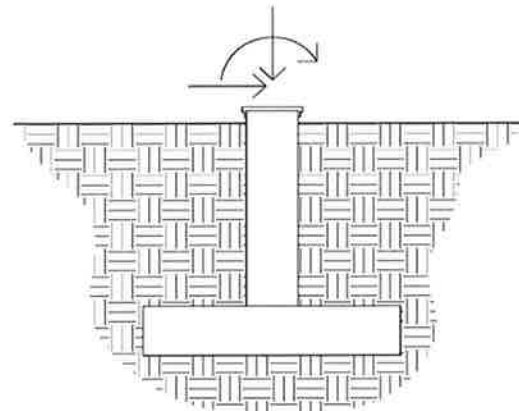
Pole Diameter at Base:	<u>31.81</u>	in
Bearing Depth:	<u>7.0</u>	ft
Pad Width:	<u>21.5</u>	ft
Neglected Depth:	<u>3.3</u>	ft
Thickness:	<u>3.0</u>	ft
Pier Diameter:	<u>5.0</u>	ft
Pier Height Above Grade:	<u>0.5</u>	ft
BP Dist. Above Pier:	<u>0.0</u>	in
Clear Cover:	<u>4.0</u>	in
Pier Rebar Size:	<u>11</u>	
Pier Rebar Quantity:	<u>12</u>	
Pad Rebar Size:	<u>8</u>	
Pad Rebar Quantity:	<u>22</u>	
Pier Tie Size:	<u>5</u>	
Tie Quantity:	<u>12</u>	
Rebar Yield Strength:	<u>60000</u>	psi
Concrete Strength:	<u>3000</u>	psi
Concrete Unit Weight:	<u>0.15</u>	kcf

21.5 FT



21.5 FT

Elevation Overview



Soil Data:

	Allowable Values	
Soil Unit Weight:	<u>0.111</u>	kcf
Ult. Bearing Capacity:	<u>6.900</u>	ksf
Angle of Friction:	<u>32.000</u>	deg
Cohesion:	<u>0.000</u>	ksf
Passive Pressure:	<u>0.000</u>	ksf
Base Friction:	<u>0.350</u>	

**\*\* Notes:**

### Summary of Results

Req'd Pier Diam.	OK
Overturning	52.5%
Shear Capacity	20.7%
Bearing	41.1%
Pad Shear - 1-way	37.4%
Pad Shear - 2-way	3.4%
Pad Moment Capacity	32.3%
Pier Moment Capacity	92.1%

**APPENDIX D**  
**TOWER MODIFICATION DRAWINGS**



# TOWER MODIFICATION DRAWINGS PREPARED FOR: CROWN CASTLE

## PROJECT CONTACTS:

### 1. CROWN PROJECT MANAGER

DAN VADNEY  
(518) 373-3510  
DAN.VADNEY@CROWNCASTLE.COM

### 2. CROWN CONSTRUCTION MANAGER

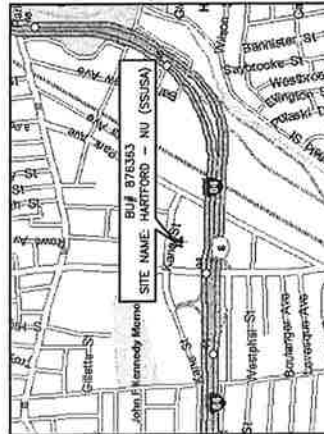
JAMES ANDREWS  
(860) 716-3032  
JAMES.ANDREWS.VENDOR@CROWNCASTLE.COM

### 3. B+T GROUP RFI CONTACT

ROBERT M. FRAZIER, P.E.  
(918) 587-4630  
RFRAZIER@BTGRP.COM  
MODDWGS@BTGRP.COM  
1717 S BOULDER AVENUE, SUITE 300  
TULSA, OK 74119

SITE NAME: HARTFORD - NU (SSUSA)  
BU NUMBER: 876363

SITE ADDRESS:  
219 NEW PARK RD.  
HARTFORD, CT 06106-2949  
HARTFORD COUNTY, USA



MAP

## DIRECTIONS

84 WEST TO EXIT 44. TURN RIGHT OFF RAMP AND CONTINUE STRAIGHT THROUGH LIGHT. TURN RIGHT ON NEW PARK AVENUE AND GO TO ADDRESS.

## TOWER INFORMATION

TOWER MANUFACTURER / JOB #: SUMMITT MANUFACTURING, LLC / 11049  
TOWER HEIGHT / TYPE: 108' MONOPOLE  
TOWER LOCATION: LAT. 41° 45' 2.79"  
LONG. -72° 42' 49.23"  
DATUM: (NAD 1983) ELEV. 117 FT AMSL  
STRUCTURAL DESIGN DRAWING REPORT: B+T GROUP / WO. # 1385488  
STRUCTURAL ANALYSIS REPORT: TEP / WO. # 1371815  
APPLICATION ID / REVISION #: 382014 / 1  
CCSITES DOCUMENT ID: 6732835

## CODE COMPLIANCE

THIS REINFORCEMENT DESIGN HAS BEEN PERFORMED IN ACCORDANCE WITH THE 2016 CONNECTICUT STATE BUILDING CODE BASED UPON AN ULTIMATE 3-SECOND GUST WIND SPEED OF 125 MPH CONVERTED TO A NOMINAL 3-SECOND GUST WIND SPEED OF 97 MPH PER SECTION 1609.3 AND APPENDIX N AS REQUIRED FOR USE IN THE TIA-222-G STANDARD PER EXCEPTION #5 OF SECTION 1609.1.1. EXPOSURE CATEGORY C AND RISK CATEGORY II WERE USED IN THIS REINFORCEMENT DESIGN.

## DRAWINGS INCLUDED

SHEET NUMBER	DESCRIPTION
S1	TITLE SHEET
S2	MODIFICATION INSPECTION NOTES AND CHECKLIST
S3	GENERAL NOTES
S4	NG2 BOLT NOTES AND DETAILS
S5	AJAX ONESIDE™ BOLT SPECIFICATIONS AND FORGBOLT NOTES AND DETAILS
S6	TIGHTENING PROCEDURE
S7	TOWER ELEV., SCHEDULE AND TX LINE DIST. DIAGRAM
S8	TOWER SECTION (0'-21')
S9	EXISTING FLAT PLATE MODIFICATION DETAILS
D1	TRANSITION STIFFENER DETAILS
D2	ANCHOR ROD BRACKET DETAILS
D3	ANCHOR ROD BRACKET DETAILS

ATTENTION ALL CONTRACTORS, ANYTIME YOU ACCESS A CROWN SITE FOR ANY REASON YOU ARE TO CALL THE CROWN NOC UPON ARRIVAL AND DEPARTURE, DAILY AT 800-788-7011.



**CROWN  
CASTLE**

REV	DATE	DESCRIPTION
0	02/16/17	ISSUED FOR CONSTRUCTION

PROJECT NO.:	85565.008.01
PROJECT ENG.:	ROBERT M. FRAZIER
DRAWN BY:	LUJ / GLS
CHECKED BY:	SSC

B+T ENGINEERING, INC.  
PEC.0001564  
Expires 02/15/18



IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS A PROFESSIONAL ENGINEER, TO SIGN THIS DOCUMENT.

HARTFORD - NU (SSUSA)  
876363

219 NEW PARK RD.  
HARTFORD, CT

EXISTING 108' MONOPOLE

SHEET TITLE

TITLE SHEET

SHEET NUMBER

**S1**

REVISION

**0**



# CROWN CASTLE

REV	DATE	DESCRIPTION
0	05/19/17	ISSUED FOR CONSTRUCTION

PROJECT NO: 86565.006.01  
 PROJECT ENG: ROBERT M. FRAZIER  
 DRAWN BY: UJU/GLS  
 CHECKED BY: SSC

84T ENGINEERING, INC.  
 PEC-0001564  
 Expires 02/10/18



IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE A LICENSED PROFESSIONAL ENGINEER, TO SEAL THIS DOCUMENT.

HARTFORD - NU (SSUA)  
 876363

210 NEW PARK RD.  
 HARTFORD, CT  
 EXISTING 10F MONOPOLE

SHEET TITLE  
 MODIFICATION INSPECTION NOTES AND CHECKLIST

SHEET NUMBER  
 S2

REVISION  
 0

**MI INSPECTOR** IS REQUIRED TO CONTACT THE GC AS SOON AS RECEIVING A PO FOR THE MI TO, AT A MINIMUM:

- REVIEW THE REQUIREMENTS OF THE MI CHECKLIST
- WORK WITH THE GC TO DEVELOP A SCHEDULE TO CONDUCT ON-SITE INSPECTIONS, INCLUDING FOUNDATION INSPECTIONS

THE MI INSPECTOR IS RESPONSIBLE FOR COLLECTING ALL GENERAL CONTRACTOR (GC) INSPECTIONS, AND TEST REPORTS, REVIEWING THE DOCUMENTS FOR ADHERENCE TO THE CONTRACT DOCUMENTS, CONDUCTING THE IN-FIELD INSPECTIONS, AND SUBMITTING THE MI REPORT TO CROWN.

**GENERAL CONTRACTOR**

SHALL BE REQUIRED TO CONTACT THE MI INSPECTOR AS SOON AS RECEIVING A PO FOR THE MODIFICATION INSTALLATION OR TURNKEY PROJECT TO, AT A MINIMUM:

- REVIEW THE REQUIREMENTS OF THE MI CHECKLIST
- WORK WITH THE MI INSPECTOR TO DEVELOP A SCHEDULE TO CONDUCT ON-SITE MI INSPECTIONS, INCLUDING FOUNDATION INSPECTIONS
- BETTER UNDERSTAND ALL INSPECTION AND TESTING REQUIREMENTS

THE GC SHALL PERFORM AND RECORD THE TEST AND INSPECTION RESULTS IN ACCORDANCE WITH THE REQUIREMENTS OF THE MI CHECKLIST AND ENG-SOW-10007.

**RECOMMENDATIONS**

THE GENERAL CONTRACTOR'S RECOMMENDATIONS AND SUGGESTIONS ARE OFFERED TO ENHANCE THE EFFICIENCY AND EFFECTIVENESS OF DELIVERING A MI REPORT:

- IT IS SUGGESTED THAT THE GC PROVIDE A MINIMUM OF 5 BUSINESS DAYS NOTICE, PREFERABLY 10, TO THE MI INSPECTOR AS TO WHEN THE SITE WILL BE READY FOR THE MI INSPECTION.
- THE GC AND MI INSPECTOR COORDINATE CLOSELY THROUGHOUT THE ENTIRE PROJECT.
- WHEN POSSIBLE, IT IS PREFERRED TO HAVE THE GC AND MI INSPECTOR ON-SITE SIMULTANEOUSLY FOR ANY GUY WIRE TENSIONING OR RE-TENSIONING OPERATIONS.
- IT MAY BE BENEFICIAL TO INSTALL ALL TOWER MODIFICATIONS PRIOR TO CONDUCTING THE FOUNDATION INSPECTIONS TO ALLOW FOUNDATION AND MI INSPECTION(S) TO COMMENCE WITH WHEN POSSIBLE.
- IT IS PREFERRED TO HAVE THE GC AND MI INSPECTOR ON-SITE DURING THE FOUNDATION INSPECTIONS TO CORRECT ANY DEFICIENCIES DURING THE INITIAL MI. THEREFORE, THE GC MAY CHOOSE TO COORDINATE THE MI CAREFULLY TO ENSURE ALL CONSTRUCTION FACILITIES ARE AT THEIR DISPOSAL WHEN THE MI INSPECTOR IS ON SITE.

**CANCELLATION OR DELAYS IN SCHEDULED MI**

IF THE GC AND MI INSPECTOR AGREE TO A DATE ON WHICH THE MI WILL BE CONDUCTED, AND EITHER PARTY CANCELS OR DELAYS, CROWN SHALL NOT BE RESPONSIBLE FOR ANY COSTS, FEES, LOSS OF DEPOSITS AND/OR OTHER PENALTIES RELATED TO THE CANCELLATION OR DELAY INCURRED BY EITHER PARTY FOR ANY TIME (E.G. TRAVEL AND LODGING, COSTS OF KEEPING EQUIPMENT ON-SITE, ETC.). IF CROWN CONTRACTS DIRECTLY FOR A THIRD PARTY MI, EXCEPTIONS MAY BE MADE IN THE EVENT THAT THE DELAY/CANCELLATION IS CAUSED BY WEATHER OR OTHER CONDITIONS THAT MAY COMPROMISE THE SAFETY OF THE PARTIES INVOLVED.

**CORRECTION OF FAILING MI'S**

IF THE MODIFICATION INSTALLATION WOULD FAIL THE MI (FAILED MP), THE GC SHALL WORK WITH CROWN TO COORDINATE A REMEDIATION PLAN IN ONE OF TWO WAYS:

- CORRECT FAILING ISSUES TO COMPLY WITH THE SPECIFICATIONS CONTAINED IN THE ORIGINAL MI REPORT.
- OR, WITH CROWN'S APPROVAL, THE GC MAY WORK WITH THE EOR TO RE-ANALYZE THE MODIFICATION/REINFORCEMENT USING THE AS-BUILT CONDITION

**MI VERIFICATION INSPECTIONS**

CROWN RESERVES THE RIGHT TO CONDUCT A MI VERIFICATION INSPECTION TO VERIFY THE ACCURACY AND COMPLETENESS OF PREVIOUSLY COMPLETED MI INSPECTIONS(S) ON TOWER MODIFICATION PROJECTS.

ALL VERIFICATION INSPECTIONS SHALL BE HELD TO THE SAME SPECIFICATIONS AND REQUIREMENTS IN THE CONTRACT DOCUMENTS AND IN ACCORDANCE WITH ENG-SOW-10007.

VERIFICATION INSPECTION MAY BE CONDUCTED BY AN INDEPENDENT A/EV/AESV FIRM AFTER A MODIFICATION PROJECT IS COMPLETED, AS MARKED BY THE DATE OF AN ACCEPTED 'PASSING MI' ON TESTS AS NOTED IN THE MI REPORT FOR THE ORIGINAL PROJECT.

**REQUIRED PHOTOS**

BETWEEN THE GC AND THE MI INSPECTOR THE FOLLOWING PHOTOGRAPHS, AT A MINIMUM, ARE TO BE TAKEN AND INCLUDED IN THE MI REPORT:

- PRE-CONSTRUCTION GENERAL SITE CONDITION
- PHOTOGRAPHS DURING THE REINFORCEMENT MODIFICATION CONSTRUCTION/ERECTION AND INSPECTION
- RAW MATERIALS
- PHOTOS OF ALL CRITICAL DETAILS
- FOUNDATION MODIFICATIONS
- BOLT INSTALLATION AND TORQUE
- FINAL INSTALLED CONDITION
- SURFACE COATING REPAIR
- POST CONSTRUCTION PHOTOGRAPHS
- FINAL INFIELD CONDITION

PHOTOS OF ELEVATED MODIFICATIONS TAKEN FROM THE GROUND SHALL BE CONSIDERED INADEQUATE.

THIS IS NOT A COMPLETE LIST OF REQUIRED PHOTOS, PLEASE REFER TO ENG-SOW-10007.

## MI CHECKLIST

REQUIRED	REPORT ITEM	BRIEF DESCRIPTION
	<b>PRE-CONSTRUCTION</b>	
X	MI CHECKLIST DRAWING	THIS CHECKLIST SHALL BE INCLUDED IN THE MI REPORT. ONCE THE PRE-MODIFICATION MAPPING IS COMPLETE AND PRIOR TO FABRICATION, THE CONTRACTOR SHALL PROVIDE DETAILED ASSEMBLY DRAWINGS AND/OR SHOP DRAWINGS AS NECESSARY FOR NON-STANDARD PARTS. THESE ARE TO INCLUDE, BUT ARE NOT LIMITED TO, A VISUAL LAYOUT OF NEW REINFORCEMENT, EXISTING REINFORCEMENT, AND ANY OTHER ITEMS THAT MAY AFFECT SUCCESSFUL INSTALLATION OF MODIFICATIONS ON THE TOWER. THESE DRAWINGS SHALL BE SUBMITTED TO THE EOR FOR APPROVAL. APPROVED ASSEMBLY/SHOP DRAWINGS SHALL BE SUBMITTED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
X	EOR APPROVAL	A LETTER FROM THE FABRICATOR, STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH INDUSTRY STANDARDS AND THE CONTRACT DOCUMENTS SHALL BE PROVIDED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
X	FABRICATION INSPECTION	A VISUAL OBSERVATION BY A CWI OF A PORTION OF WELDING ON THE PROPOSED STRUCTURAL MEMBERS IS REQUIRED AND A WRITTEN REPORT SHALL BE PROVIDED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
X	FABRICATOR CERTIFIED WELD INSPECTION	WILL CERTIFICATION SHALL BE PROVIDED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
X	MATERIAL TEST REPORT (MTR)	CRITICAL SHOP WELDS THAT REQUIRE TESTING (PER ENG-STD-10069) ARE NOTED ON THESE CONTRACT DRAWINGS. A CERTIFIED WELD INSPECTOR SHALL PERFORM NON-DESTRUCTIVE EXAMINATION AND A REPORT SHALL BE PROVIDED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
N/A	FABRICATOR NDE INSPECTION	A NDE (PER ENG-SOW-10033) OF THE POLE TO BASE PLATE CONNECTION IS REQUIRED AND A WRITTEN REPORT SHALL BE PROVIDED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
N/A	NDE REPORT OF MONOPOLE BASE PLATE	THE MATERIAL SHIPPING LIST SHALL BE PROVIDED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
X	PACKING SLIPS	<b>CONSTRUCTION (PERFORMED BY CONTRACTOR)</b> A LETTER FROM THE GENERAL CONTRACTOR STATING THAT THE WORKMANSHIP WAS PERFORMED IN ACCORDANCE WITH INDUSTRY STANDARDS AND THESE CONTRACT DRAWINGS.
X	CONSTRUCTION INSPECTIONS	A VISUAL OBSERVATION OF THE EXCAVATION AND REBAR SHALL BE PERFORMED BEFORE PLACING THE CONCRETE. A WRITTEN REPORT SHALL BE PROVIDED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
N/A	FOUNDATION INSPECTIONS	THE CONCRETE MIX DESIGN SLUMP TEST AND COMPRESSIVE STRENGTH TESTS SHALL BE PROVIDED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
N/A	CONCRETE COMP. STRENGTH AND SLUMP TESTS	POST INSTALLED ANCHOR ROD VERIFICATION SHALL BE PERFORMED IN ACCORDANCE WITH CROWN REQUIREMENTS AND A REPORT SHALL BE PROVIDED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
N/A	POST INSTALLED ANCHOR ROD VERIFICATION	THE GENERAL CONTRACTOR SHALL PROVIDE DOCUMENTATION TO THE MI INSPECTOR THAT CERTIFIES THAT THE GROUT WAS INSTALLED IN ACCORDANCE WITH CROWN ENG-PROC-10012 FOR INCLUSION IN THE MI REPORT.
N/A	BASE PLATE GROUT VERIFICATION	A CERTIFIED WELD INSPECTOR SHALL INSPECT AND TEST AS NECESSARY ALL FIELD WELDS. CWI SHALL FOLLOW ALL THE PROCEDURES SPECIFIED IN CROWN STANDARD DOCUMENTS ENG-SOW-10066, ENG-STD-10089 AND SKV-SID-10159. A REPORT SHALL BE PROVIDED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
X	CONTRACTOR'S CERTIFIED WELD INSPECTION	FOUNDATION SUB-GRADES SHALL BE INSPECTED AND APPROVED BY A GEOTECHNICAL ENGINEER AND A REPORT SHALL BE PROVIDED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
N/A	EARTHWORK: LIFT AND DENSITY	THE GENERAL CONTRACTOR SHALL PROVIDE DOCUMENTATION TO THE MI INSPECTOR VERIFYING THAT ANY ON-SITE COLD GALVANIZING WAS APPLIED IN ACCORDANCE WITH ENG-BUL-10149.
X	ON SITE COLD GALVANIZING VERIFICATION	THE GENERAL CONTRACTOR SHALL PROVIDE A REPORT TO THE MI INSPECTOR INDICATING THE TEMPERATURE AND TENSION IN EVERY GUY CABLE AS PART OF PLUMB AND TENSION PROCEDURE FOR INCLUSION IN THE MI REPORT.
N/A	GUY WIRE TENSION REPORT	THE GENERAL CONTRACTOR SHALL SUBMIT A COPY OF THE CONTRACT DRAWINGS EITHER EXISTING "INSTALLED AS DESIGNED" OR NOTING ANY CHANGES THAT WERE REQUIRED AND APPROVED BY THE ENGINEER OF RECORD.
X	GC AS-BUILT DOCUMENTS	<b>POST-CONSTRUCTION</b> THE MI INSPECTOR SHALL OBSERVE AND REPORT ANY DISCREPANCIES BETWEEN THE CONTRACTORS REDLINE DRAWING AND THE ACTUAL COMPLETED INSTALLATION.
X	MI INSPECTOR REDLINE OR RECORD DRAWING(S)	POST-INSTALLED ANCHOR RODS SHALL BE TESTED IN ACCORDANCE WITH ENG-PROC-10119 AND A REPORT SHALL BE PROVIDED TO THE MI INSPECTOR FOR INCLUSION IN THE MI REPORT.
N/A	POST INSTALLED ANCHOR ROD PULL-OUT TESTING	PHOTOGRAPHS SHALL BE SUBMITTED TO THE MI WHICH DOCUMENT ALL PHASES OF THE CONSTRUCTION. THE PHOTOS SHALL BE ORGANIZED IN A MANNER THAT EASILY IDENTIFIES THE EXACT LOCATION OF THE PHOTO.
X	PHOTOGRAPHS	ADDITIONAL TESTING AND INSPECTIONS:

NOTE: X DENOTES A DOCUMENT NEEDED FOR THE MI REPORT AND N/A DENOTES A DOCUMENT THAT IS NOT REQUIRED FOR THE MI REPORT

**MODIFICATION INSPECTION NOTES:**

**GENERAL**

THE MODIFICATION INSPECTION (MI) IS A VISUAL INSPECTION OF TOWER MODIFICATIONS AND A REVIEW OF CONSTRUCTION INSPECTIONS AND OTHER REPORTS TO ENSURE THE INSTALLATION WAS CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS, NAMELY THE MODIFICATION DRAWINGS, AS DESIGNED BY THE ENGINEER OF RECORD (EOR). THE MI IS TO CONFIRM INSTALLATION CONFIGURATION AND WORKMANSHIP ONLY AND IS NOT A DESIGN OF THE MODIFICATION DESIGN ITSELF. NOR DOES THE MI INSPECTOR TAKE OWNERSHIP OF THE MODIFICATION DESIGN. OWNERSHIP OF THE STRUCTURAL MODIFICATION DESIGN EFFECTIVENESS AND INTEGRITY RESIDES WITH THE EOR AT ALL TIMES.

ALL MI'S SHALL BE CONDUCTED BY A CROWN ENGINEERING VENDOR (AEV) OR ENGINEERING SERVICE VENDOR (AESV) THAT IS APPROVED TO PERFORM ELEVATED WORK FOR CROWN. SEE ENG-BUL-10173 LIST OF APPROVED MI VENDORS.

TO ENSURE THAT THE REQUIREMENTS OF THE MI ARE MET, IT IS VITAL THAT THE GENERAL CONTRACTOR (GC) AND THE MI INSPECTOR BEGIN COMMUNICATING AND COORDINATING AS SOON AS A PO IS RECEIVED. IT IS EXPECTED THAT EACH PARTY WILL BE PROACTIVE IN REACHING OUT TO THE OTHER PARTY. IF CONTACT INFORMATION IS NOT KNOWN, CONTACT YOUR CROWN POINT OF CONTACT (POC).

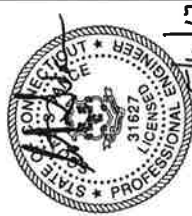
REFER TO ENG-SOW-10007 : MODIFICATION INSPECTION SOW FOR FURTHER DETAILS AND REQUIREMENTS.

# CROWN CASTLE

REV	DATE	DESCRIPTION
0	02/14/17	ISSUED FOR CONSTRUCTION

PROJECT NO: 86665.006.01  
 PROJECT ENG: ROBERT M. FRAZIER  
 DRAWN BY: ULIJ/GLS  
 CHECKED BY: SSC

B+T ENGINEERING, INC.  
 P.E.C. 00015684  
 Expires 02/10/18



IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO SIGN THIS DOCUMENT.

HARTFORD - NU (SSUSA)  
 876363  
 219 NEW PARK RD.  
 HARTFORD, CT  
 EXISTING 108' MONOPOLE

SHEET TITLE  
 GENERAL NOTES

SHEET NUMBER: **S3**  
 REVISION: **0**

## GENERAL NOTES

- 1.1 ALL WORK SHALL COMPLY WITH THE TIA-222-G STANDARD AS WELL AS ANY OTHER GOVERNING BUILDING CODES.
- 1.2 ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE TIA-222-G STANDARD AS WELL AS ANY OTHER GOVERNING BUILDING CODES.
- 1.3 A MINIMUM OF TWO COATS OF ZINGA COLD GALVANIZING COMPOUND (OR APPROVED EQUIVALENT) SHALL BE APPLIED TO ANY FIELD CUTS OR FIELD DRILLED HOLES.
- 1.4 THE USE OF A GAS TORCH OR WELDER WILL NOT BE PERMITTED ON THE TOWER WITHOUT THE CONSENT OF THE OWNER.
- 1.5 STATE THE TOWER IS LOCATED. THE ANALYSIS SHALL USE A MINIMUM WIND SPEED OF 45 mph (3-sec) PER TIA-1019.
- 1.6 AND RESQUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR FOR THE EXECUTION OF THE WORK CONTAINED HEREIN AND SHALL MEET ANSI/ASSE A10.48 (LATEST EDITION); FEDERAL, STATE AND LOCAL REGULATIONS; AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL ADHERE TO ANSI/ASSE A10.48 (LATEST EDITION) AND CROWN STANDARD CED-STD-10253 INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION TO CERTIFY THE SUPPORTING STRUCTURE(S) IN ACCORDANCE WITH THE ANSI/TIA-322 (LATEST EDITION).
- 1.7 ALL THE PARTS STARTING WITH "CCL-" DESIGNATION - REFER TO "CROWN CASTLE APPROVED REINFORCEMENT COMPONENTS DRAWING" FOR DETAILS.
- 1.8 ALL STEEL SHALL BE HOT-DIP GALVANIZED WITH CORRESPONDING 20MM DIAMETER SLEEVE WITH SPECIFIED STEEL GRADE.
- 1.9 ALL STEEL SHALL BE HOT-DIP GALVANIZED AFTER FABRICATOR IN ACCORDANCE WITH ASTM A123. ALTERNATIVELY, ALL NEW STIFFENER PLATE STEEL REINFORCING MAY BE COLD GALVANIZED AS FOLLOWS: APPLY A MINIMUM OF TWO COATS OF ZRC-BRAND ZINC-RICH COLD GALVANIZING COMPOUND. FILM THICKNESS: 1-800-831-3275 FOR PRODUCT INFORMATION.
- 1.10 ALL SHIMS SHALL BE ASTM A36 STEEL, UNLESS OTHERWISE SPECIFIED.
- 1.11 ALL SHIMS SHALL BE ASSUMED 60X60 OR GREATER, PER STANDARD SPICE DETAIL.
- 1.12 SHOP WELDS ARE ASSUMED EROXO OR GREATER, PER STANDARD SPICE DETAIL.
- 1.13 IF SCOPE OF MODIFICATION REQUIRES REMOVAL OF TOWER ID TAG, IT MUST BE REPLACED.
- 1.14 THE CLIMBING FACILITIES, SAFETY CLIMB AND ALL PARTS THEREOF SHALL NOT BE IMPEDED, MODIFIED OR ALTERED WITHOUT THE EXPRESS APPROVAL OF THE ENGINEER OF RECORD OR TOWER OWNER.
- 1.15 OFFICE ASSEMBLY CLIMBING FACILITIES SHALL BE PROVIDED AT EACH CHANGE IN ALIGNMENT. IF NEW REINFORCEMENT REQUIRES STEP-BOLT BRACKETS, INSTALL PRIOR TO GALVANIZATION OF STEEL.
- 1.16 ANY WORK PERFORMED WITHOUT A PREFABRICATION MAPPING IS DONE AT THE RISK OF THE GC AND/OR FABRICATOR.

## FABRICATION

- 2.1 ALL WORK SHALL BE DONE IN ACCORDANCE WITH A.I.S.C. \*SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.\*
- 2.2 STRUCTURAL STEEL SHALL MEET THE FOLLOWING SPECIFICATIONS:

A. STEEL SHAPES AND PLATES, U.N.O.	YIELD	ASTM SPECS
B. STEEL PIPE (HSS TUBING)	65ksi	A572
	50ksi	A500 GR. C

- 2.3 ALL NEW MATERIAL INCLUDING STRUCTURAL STEEL AND FASTENERS SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 AND A153.
- 2.4 WELDING SHALL BE DONE IN ACCORDANCE WITH STRUCTURAL WELDING CODE (LATEST EDITION). ELECTRODES SHALL BE E60 SERIES.
- 2.5 CONTRACTOR SHALL PROVIDE SHOP FABRICATION DRAWINGS TO B+T GROUP 5 DAYS PRIOR TO FABRICATION.

## FIELD NDE MINIMUM REQUIREMENTS

- 3.1 ALL NDE SHALL BE IN ACCORDANCE WITH AWS D1.1.
- 3.2 FOR NEW BASE STIFFENERS, COMPLETE PENETRATION AND ANCHOR ROD BRACKETS, COMPLETE JOINT PENETRATION WELDS SHALL BE 100% INSPECTED BY UT. ALL PARTIAL JOINT PENETRATION AND FILLET WELDS SHALL BE 100% INSPECTED BY MT.
- 3.3 FOR NEW FLAT PLATE REINFORCEMENT AT THE BASE OF THE TOWER, COMPLETE JOINT PENETRATION WELDS SHALL BE 100% INSPECTED BY UT. ALL PARTIAL JOINT PENETRATION AND FILLET WELDS SHALL BE 100% INSPECTED BY UT. TO A HEIGHT OF 10'-0".
- 3.4 PLEASE SEE ENG-SOW-10033; TOWER BASE PLATE NDE AND ENG-BUL-10055; NDE REQUIREMENTS FOR MONOPOLE TO PREVENT CONNECTION FAILURE. NOTIFY THE E.O.R. AND CROWN ENGINEERING IMMEDIATELY IF ANY CRACKS ARE SUSPECTED OR HAVE BEEN IDENTIFIED. THE NDE SHALL INCLUDE ALL EXISTING MODIFICATIONS THAT HAVE BEEN WELDED TO THE BASE PLATE.
- 3.5 ALL TESTING LIMITATIONS SHALL BE NOTED IN THE NDE REPORT.

## KEY NOTES

1. TOWER MODIFICATION I.D.



1717 S. BOULDER AVE.  
SUITE 300  
PULASKI, OHIO 44130  
PH: 440.328.4600  
WWW.B+TGRP.COM

# CROWN CASTLE

REV.	DATE	DESCRIPTION
0	02/14/17	ISSUED FOR CONSTRUCTION

PROJECT NO.: 85665-006-01  
PROJECT ENG.: ROBERT M. FRAZIER  
DRAWN BY: UJJ/GLS  
CHECKED BY: SSC

B+T ENGINEERING, INC.  
PCC.0001564  
Expires 02/10/18



IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THE PERSON IS LICENSED, TO REPRODUCE OR COLLECT REPRODUCTION RIGHTS IN ANY MANNER.

HARTFORD - NU (SSUSA)  
876383

210 NEW PARK RD.  
HARTFORD, CT  
EXISTING 100' MONOPOLE

SHEET TITLE  
NG2 BOLT NOTES  
AND DETAILS

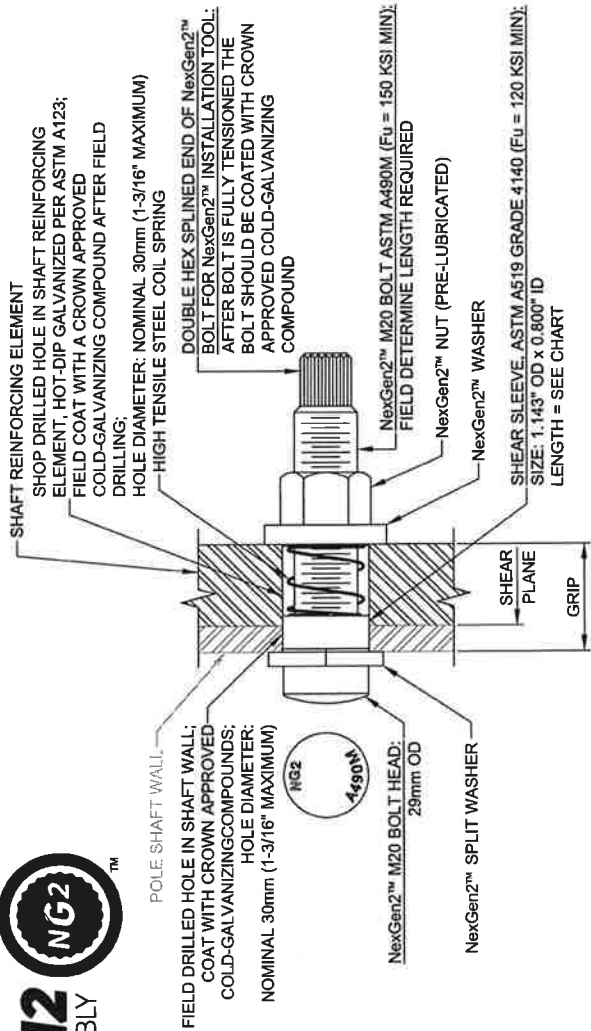
SHEET NUMBER: **S4**  
REVISION: **0**

EXTERIOR OF POLE SHAFT

INTERIOR OF POLE SHAFT



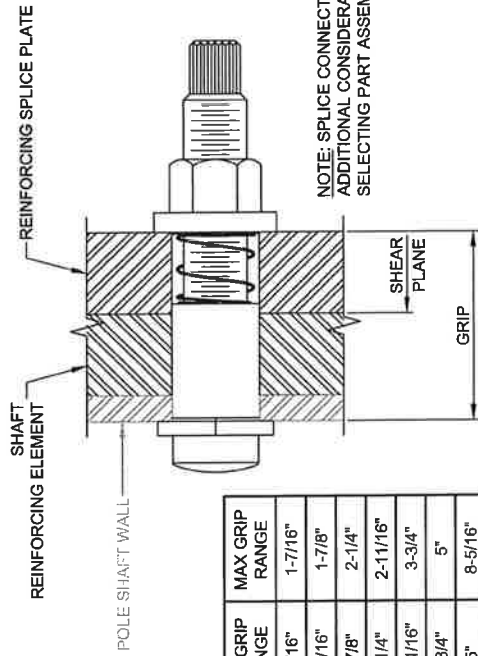
**NEXGEN2**  
BLIND BOLT ASSEMBLY  
- PATENT PENDING -



**NOTES:**

- ALL SHOP AND FIELD DRILLED HOLES SHALL BE NOMINAL 30mm DIAMETER; THE MAXIMUM HOLE DIAMETER PERMITTED IS 1 3/16".
- NexGen2™ COMPLETE ASSEMBLY SHALL BE MAGN 565 COATED PER ASTM F2865 AS APPROPRIATE.
- INSTALL PER MANUFACTURER'S INSTRUCTIONS.

MANUFACTURER:  
AUFFASTENERS  
959 LAKE ROAD  
MEDINA, OHIO, USA 44256  
PHONE: 440-232-5060  
WEBSITES: WWW.AUFFASTENERS.COM WWW.AFTOWER.COM



NOTE: SPLICE CONNECTIONS REQUIRE ADDITIONAL CONSIDERATION WHEN SELECTING PART ASSEMBLIES

PART NUMBER	BOLT LENGTH	SLEEVE LENGTH	MIN GRIP RANGE	MAX GRIP RANGE
2NG2036	M20x95	1-1/16"	1-7/16"	1-7/16"
2NG2048	M20x95	1-3/16"	1-7/16"	1-7/8"
2NG2057	M20x95	1-5/8"	1-7/8"	2-1/4"
2NG2068	M20x135	2"	2-1/4"	2-11/16"
2NG2096	M20x135	2-7/16"	2-11/16"	3-3/4"
2NG2127	M20x175	3"	3-3/4"	5"
2NG2212	M20x250	4"	5"	8-5/16"

# CROWN CASTLE

REV	DATE	DESCRIPTION
0	05/14/17	ISSUED FOR CONSTRUCTION

PROJECT NO: 85665 006 01  
 PROJECT ENG: ROBERT M. FRAZIER  
 DRAWN BY: UJJ / G.S.  
 CHECKED BY: SSC

B+T ENGINEERING, INC.  
 P.E.C. 0001584  
 Expires 02/10/18



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HARTFORD - NU (SSUSA)  
 876983

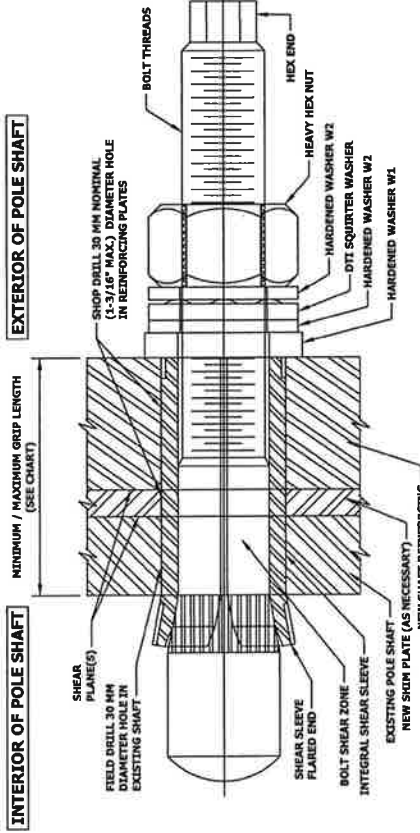
219 NEW PARK RD  
 HARTFORD, CT  
 EXISTING 108' MONOPOLE

SHEET TITLE  
 FORGEBOLT NOTES  
 AND DETAILS

SHEET NUMBER  
**55**

REVISION  
**0**

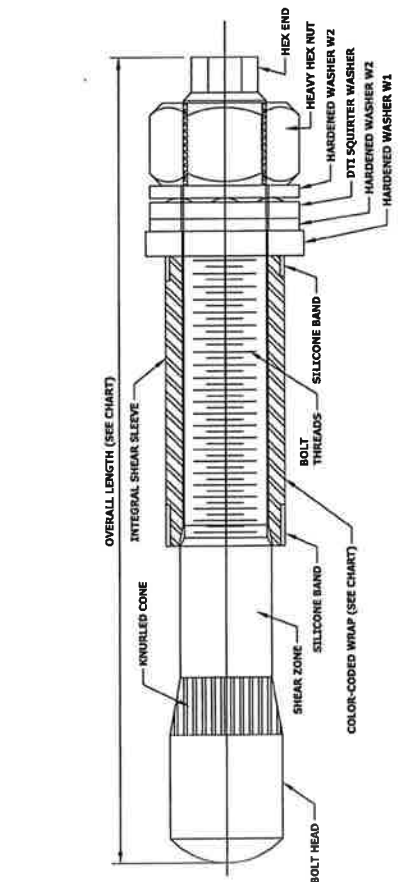
FORGEBOLT™ NOTE SHEET: A325/PC8.8 LANDSCAPE VERSION DATE 01/29/2015; Rev. 1.0 04/23/2015  
**NOTES: 1. ALL STRUCTURAL BOLTS SHALL BE INSTALLED AND TIGHTENED TO THE PRETENSIONED CONDITION ACCORDING TO THE REQUIREMENTS OF THE AISC 'SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS', DEC. 31, 2009.**  
**2. ALL STRUCTURAL BOLTS SHALL BE INSPECTED ACCORDING TO THE REQUIREMENTS OF THE AISC 'SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS', DEC. 31, 2009.**



## PRE-INSTALLED FORGEBOLT™ ASSEMBLY DETAIL 1

### BOLT HOLE NOTES:

- ALL SHOP-DRILLED HOLES SHALL BE NOMINAL 30 MM DIAMETER. THE MAXIMUM SHOP-DRILLED HOLE DIAMETER PERMITTED IS 1-3/16".
- ALL FIELD-DRILLED HOLES SHALL BE NOMINAL 30 MM DIAMETER. THE MAXIMUM FIELD-DRILLED HOLE DIAMETER PERMITTED IS 30 MM.



## INSTALLED FORGEBOLT™ ASSEMBLY DETAIL 2

DISTRIBUTOR CONTACT:  
**PRECISION TOWER PRODUCTS**  
 PHONE: 888-926-4857  
 EMAIL: info@precisiontowerproducts.com  
 WEB: www.precisiontowerproducts.com  
**CONTAINS PROPRIETARY INFORMATION PATENT PENDING**  
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### FORGEBOLT™ Installation

Follow all Manufacturer/Distributor Recommendations for Installation, Tightening, and Inspection.

- FIELD DRILL HOLES TO 30 MM DIAMETER.
- SELECT CORRECT BOLT SIZE FOR INSTALLATION GRIP (REFER TO PLANS).
- INSERT BOLT ASSEMBLY THROUGH HOLES IN SHAFT REINFORCING PLATES AND SEAT THE HARDENED WASHER W1 FLUSH AGAINST OUTSIDE OF PLATE.
- HAND TIGHTEN NUT TO FINGER TIGHT.
- TIGHTEN NUT TO PRETENSIONED CONDITION AND UNTIL DTI SHOWS PROPER INDICATION.
- PROPERLY DOCUMENT AND INSPECT BOLT TIGHTENING PER PLAN REQUIREMENTS.

### FORGEBOLT™ AISC Group A Material: ASTM A325 and PC8.8 (Tensile Stress, Fu = 120 ksi minimum)

GROUP	FORGEBOLT™ Size (mm)	Overall Length (inches)	Estimated Weight Each (lbs)	Grip Range (inch)	Comment	Color Code
A	135	5.31	1.3	3/8" to 1"	--	RED
	160	6.30	1.6	3/4" to 1-1/2"	--	GREEN
	195	7.68	1.9	1-1/4" to 2-1/4"	--	BLUE
	260	10.24	2.6	2" to 3-1/2"	Splice Bolt	YELLOW
	365	14.37	3.6	3-1/2" to 5-1/2"	Flange Jump Bolt	ORANGE
	440	17.32	4.3	5-1/2" to 8-1/2"	Flange Jump Bolt	BLACK

DTI Note: Each Group A (A325/PC8.8) FORGEBOLT™ assembly shall have a 'Squitter' DTI that is compatible with a M20-PC8.8 bolt.



# CROWN CASTLE

REV	DATE	DESCRIPTION
0	02/16/17	ISSUED FOR CONSTRUCTION

PROJECT NO: 85555.008.01  
 PROJECT ENG: ROBERT M. FRAZIER  
 DRAWN BY: LUJ/JLS  
 CHECKED BY: SSC

84-T ENGINEERING, INC.  
 P.E.C. 0001584  
 Expires 02/10/18

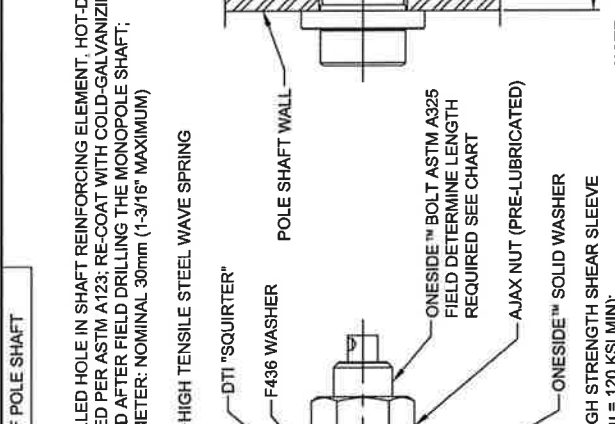
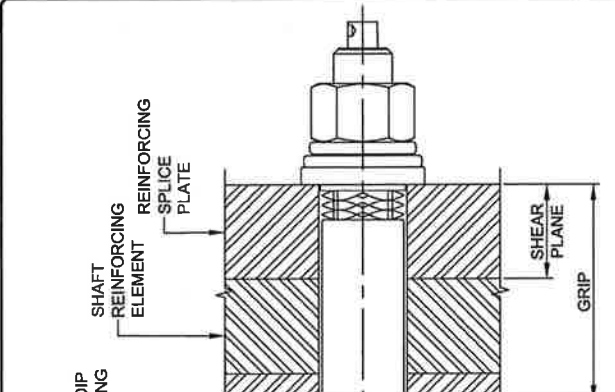


IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ASSESS THE QUALITY OF THIS DOCUMENT.

HARTFORD - NJ (SSUSA)  
 876383  
 218 NEW PARK RD.  
 HARTFORD, CT  
 EXISTING 100' MONOPOLE

SHEET TITLE  
 AJAX ONESIDE™ BOLT  
 SPECIFICATIONS AND  
 TIGHTENING PROCEDURE

SHEET NUMBER  
**S6**  
 REVISION  
**0**



NOTE:  
 SPLICE CONNECTIONS REQUIRE  
 ADDITIONAL CONSIDERATION WHEN  
 SELECTING PART ASSEMBLIES

SIZE: 1.143" OD x 0.800" ID  
 LENGTH = SEE CHART

## AJAX ONESIDE BOLT DETAIL

CODE	SIZE	COLOR	SLEEVE LENGTH	GRIP	GRIP IMP
OSBA20.65-6	M20 x 65	ORANGE	6.0 (0.236")	12.5 / 20.0	0.500" / 0.787"
OSBA20.95-14	M20 x 95	BLACK	14.0 (0.551")	20.0 / 32.0	0.787" / 1.259"
OSBA20.95-22	M20 x 95	GREEN	22.0 (0.866")	30.0 / 50.0	1.181" / 1.968"
OSBA20.95-30	M20 x 95	YELLOW	30.0 (1.181")	40.5 / 50.0	1.595" / 1.968"
OSBA20.135-39	M20 x 135	BLUE	39.0 (1.535")	49.0 / 77.0	1.929" / 3.031"
OSBA20.135-48	M20 x 135	BROWN	48.0 (1.889")	60.5 / 77.0	2.375" / 3.031"
OSBA20.135-57	M20 x 135	PURPLE	57.0 (2.244")	67.0 / 90.0	2.637" / 3.543"
OSBA20.165-76	M20 x 165	RED	76.0 (3.000")	87.0 / 120.0	3.425" / 4.724"
OSBA20.250	M20 x 250	SILVER	MTO	121.0 / 211.0	4.724" / 8.310"

### INTERIOR OF POLE SHAFT

### EXTERIOR OF POLE SHAFT



MANUFACTURER INSTALLATION VIDEO



[https://www.youtube.com/watch?v=zGBS0eLzZew&feature=em-share\\_video\\_user](https://www.youtube.com/watch?v=zGBS0eLzZew&feature=em-share_video_user)

**MANUFACTURER**  
 AJAX FASTENERS  
 SALES + TECH: [ONESIDE@AJAXFAST.COM.AU](mailto:ONESIDE@AJAXFAST.COM.AU)

**DISTRIBUTOR**  
 IRA SVENSGAARD AND ASSOCIATES  
 PETER SVENDSGAARD - [PETERS@IRASVENS.COM](mailto:PETERS@IRASVENS.COM)  
 JOHN KILLAM - [JOHN@IRASVENS.COM](mailto:JOHN@IRASVENS.COM)  
 PHONE (630) 647-8225  
 FAX (630) 647-9229

- BOLT ASSEMBLY AND INSTALLATION:**
- BOLT MUST BE PURCHASED PRE-ASSEMBLED.
  - FOLLOW BOLT AND DTI MANUFACTURERS INSTRUCTIONS FOR INSTALLATION.
- INSPECTION:**
- A MINIMUM OF 4 OUT OF 5 SQUIRTER® DTI PROTRUSIONS SHALL BE ENGAGED IN ANY AJAX/DTI BOLT ASSEMBLY IN THE REINFORCING MEMBERS. A FEELER GAGE MAY BE USED TO VERIFY PROTRUSION COMPRESSION.
  - INSPECTIONS SHALL BE IN ACCORDANCE WITH THE MANUFACTURERS REQUIREMENTS AND CROWN DOCUMENT ENG-SOW-10007: MODIFICATION INSPECTION SOW.

# CROWN CASTLE

ISSUED FOR:

REV	DATE	DESCRIPTION
0	02/16/17	ISSUED FOR CONSTRUCTION

PROJECT NO: 8555.006.01

PROJECT ENG: ROBERT M. FRAZIER

DRAWN BY: UJJ / GLS

CHECKED BY: SSC

B+T ENGINEERING, INC.  
 PEC.0001564

Expires 02/10/18



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HARTFORD - NU (SSUSA)

876363

219 NEW PARK RD.  
 HARTFORD, CT

EXISTING 10" MONOPOLE

SHEET TITLE

TOWER ELEV. SCHEDULE  
 AND TX LINE DIST. DIAGRAM

SHEET NUMBER

S7

REVISOR

0

### CCI-FLAT PLATE-BILL OF MATERIALS (6SKS)

BOTTOM ELEVATION	TOP ELEVATION	FLAT PLATE DESIGNATION	FLAT PLATE LENGTH	FLAT PLATE QUANTITY	FLAT #	BOLTS PER PLATE	TOTAL BOLT QTY	TERMINATION BOLTS (BOTTOM)	TERMINATION BOLTS (TOP)	MAXIMUM INTERMEDIATE BOLT SPACING	TOTAL STEEL WEIGHT
0'-6"	15'-6"	CCI-SFP-04510015	15'-0"	1	9	19	19	6	6	20"	230 LBS.
11'-0"	21'-0"	CCI-SFP-04510010	10'-0"	1	1	16	16	6	6	20"	153 LBS.
							35				383 LBS.

ALL BOLTS SHALL BE PRE-APPROVED BLIND M20 BOLTS WITH HIGH STRENGTH SHEAR SLEEVES (ASTM A519 WITH MIN. Fu=120 KSI). CONTACT SUPPLIER FOR MATERIAL (PLATE AND BOLTS) AND INSTALLATION PROCEDURES.

### NOTES:

- CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER FITTING OF REINFORCEMENT ON MONOPOLES. SHIMS FOR MONOPOLE REINFORCEMENT MEMBER SHALL BE REQUIRED WHERE GAPS BETWEEN THE POLE SHAFT AND REINFORCING MEMBER EXIST AT FASTENER LOCATIONS. FOR INTERMEDIATE CONNECTIONS, THE MINIMUM SHIM LENGTH AND WIDTH SHALL BE THE WIDTH OF THE REINFORCING MEMBER. FOR TERMINATION CONNECTIONS, A CONTINUOUS SHIM PLATE (PREFERRED) OR EQUIVALENT INDIVIDUAL SHIM PLATES THE WIDTH OF THE REINFORCING MEMBER MAY BE USED. SHIM THICKNESS SHALL BE NO LESS THAN 1/16". STACKING OF SHIMS IS PERMITTED. FINGER SHIMS AND HORSESHOE SHIMS ARE PERMITTED. STACKED SHIMS SHALL BE NO GREATER THAN 1/4" WITHOUT E.O.R. APPROVAL.
- BOTTOM PLATE FITTING SHALL BEGIN AT THE PROPOSED ELEVATION OF THE BOTTOM OF THE FLAT PLATE. MULTIPLE SHIMS SHALL BE USED TO BRIDGE THE GAPS BETWEEN THE BOTTOM OF THE FLAT PLATE AND THE BOTTOM OF THE POLE SHAFT. FOR MULTIPLE PLATE SPICES TOGETHER, THE TOP OF THE FLAT PLATE IS TO BE PLACED SUCH THAT THERE IS NO MORE THAN 3" DIFFERENCE BETWEEN THE ACTUAL OVERALL LENGTH OF THE SPAN AND THE PROPOSED OVERALL LENGTH OF THE SPAN, FROM THE BOTTOM OF THE BOTTOM PLATE TO THE TOP OF THE TOP PLATE.

### EXISTING MEMBER SCHEDULE

SECTION	NUMBER OF SIDES	THICKNESS	ASTM STEEL GRADE	Fy (ksi)	TOP DIAMETER	LAP SPICE
1	12	0.2500"	A607	65	31.810"	39"
2	12	0.1875"	A607	65	24.863"	16.500"
3	ROUND	0.3125"	A53-E	35	8.625"	8.625"

EXISTING BASE PLATE GRADE = 55 ksi (ASTM GRADE = A572)

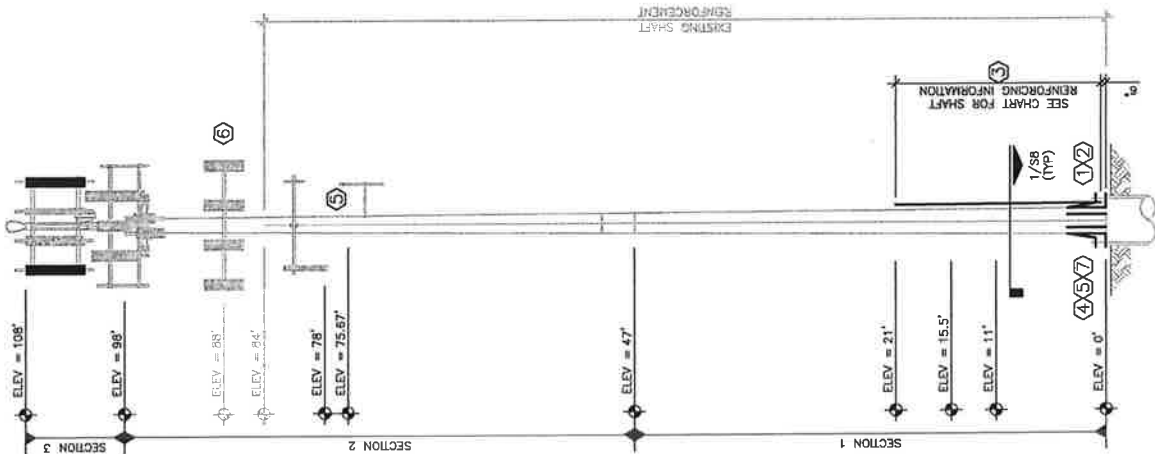
### EXISTING TOWER HAS BEEN PREVIOUSLY MODIFIED

REFERENCE DRAWINGS BY:	DATE
PAUL J. FORD & COMPANY	01/29/09
B+T GROUP	05/21/10
	10/15/12

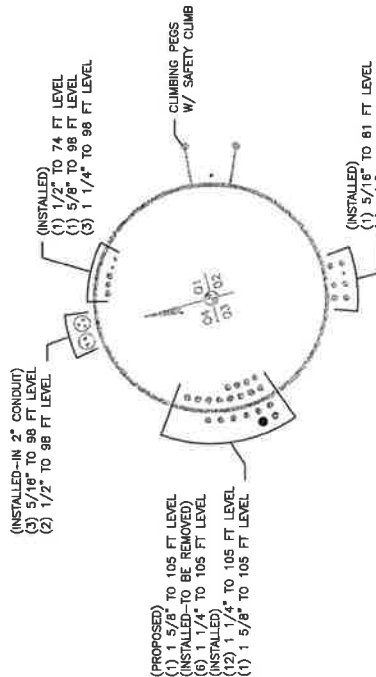
### TOWER MODIFICATIONS:

- REMOVE EXISTING TRANSITION STIFFENER AT 0' RE: SHEET SB.
- REMOVE EXISTING ANCHOR ROD BRACKET AND INSTALL NEW ANCHOR ROD BRACKET AT 0' RE: SHEET SB.
- INSTALL NEW REINFORCING ELEMENTS FROM 0' TO 21' RE: SHEET SB.
- INSTALL NEW TRANSITION STIFFENERS WITH FOOT PAD AT 0' RE: SHEET SB.
- INSTALL ADDITIONAL TERMINATION BOLTS TO EXISTING FLAT PLATE REINFORCEMENT AT 0', 75.67' AND 78' RE: SHEET SB.
- REMOVE EXISTING EQUIPMENT AND MOUNT FROM 88' LEVEL RE: SHEET SB.
- INSTALL SIGNAGE NOTING OBSTRUCTION IN CLIMBING PATH.

- PRIOR TO FABRICATION AND INSTALLATION, CONTRACTOR SHALL VISIT THE SITE TO CHECK CRITICAL DIMENSIONS. GIVEN LENGTH AND QUANTITIES PROVIDED ARE FOR QUOTING PURPOSES ONLY AND SHALL NOT BE USED FOR FABRICATION. ANY WORK PERFORMED WITHOUT A PREFABRICATION MAPPING IS DONE AT THE RISK OF THE GENERAL CONTRACTOR AND/OR FABRICATOR. NEW FABRICATIONS AND/OR REVISIONS MUST BE DISTRIBUTED AS SHOWN IN THE TX LINE DIST. DIAGRAM RE: DETAIL 2/57.
- MODIFICATIONS SHALL BE COMPLETED PRIOR TO ADDING THE PROPOSED APPURTENANCES.



1 TOWER ELEVATION  
 SCALE: N.T.S.



2 TX LINE DISTRIBUTION DIAGRAM  
 SCALE: N.T.S.



B+T GRP  
1777 S. BOULDER AVE.  
SUITE 300  
TULSA, OK 74119  
PH: (918) 587-4600  
www.btg.com

# CROWN CASTLE

REV	DATE	DESCRIPTION
0	02/16/17	ISSUED FOR CONSTRUCTION

PROJECT NO: 85565.08.01  
PROJECT ENG: ROBERT M. FRAZIER  
DRAWN BY: ULJ / SLS  
CHECKED BY: SSC

B+T ENGINEERING, INC.  
REG. 0001564  
Expires 02/10/18

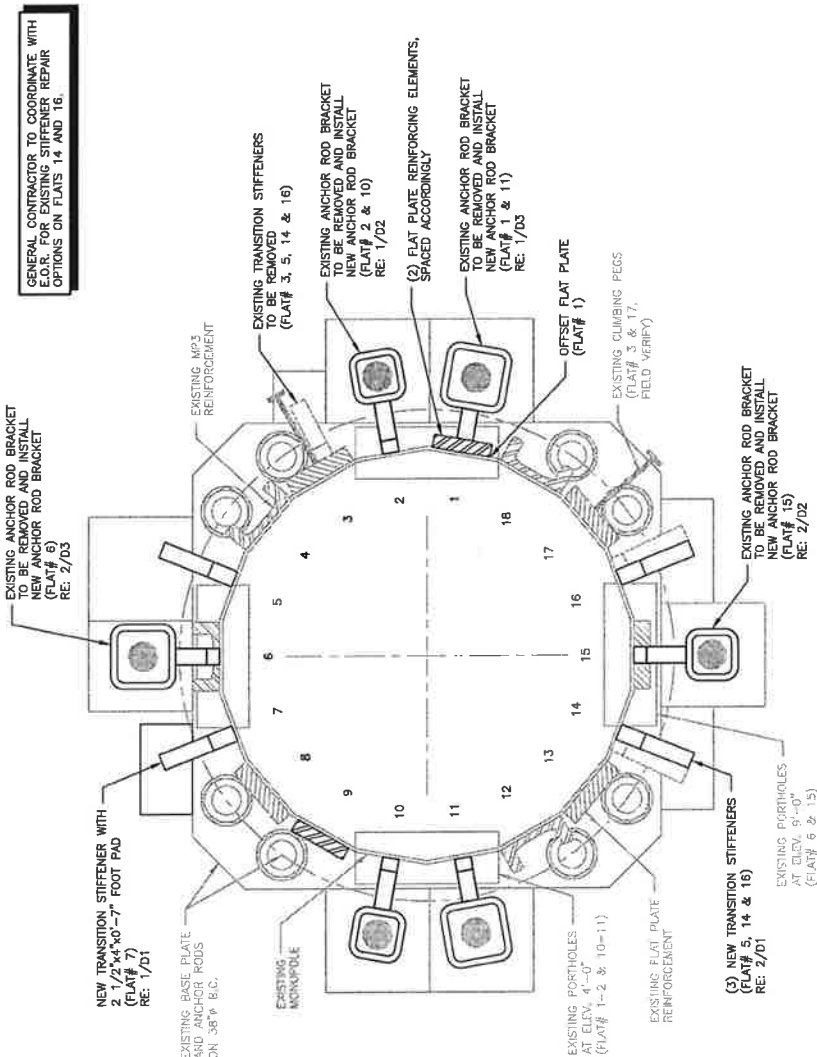


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HARTFORD - NU (SSUSA)  
876363  
210 NEW PARK RD.  
HARTFORD, CT  
EXISTING 108' MONOPOLE

SHEET TITLE  
TOWER SECTION  
(0-21)

SHEET NUMBER  
S8  
REVISION  
0



GENERAL CONTRACTOR TO COORDINATE WITH C.O.R. FOR EXISTING STIFFENER REPAIR OPTIONS ON FLATS 14 AND 16.

1 TOWER SECTION (0-21)  
SCALE: N.T.S.



# CROWN CASTLE

REV	DATE	DESCRIPTION
0	02/10/17	ISSUED FOR CONSTRUCTION

PROJECT NO: 85595.006.01  
 PROJECT ENG: ROBERT M. FRAZIER  
 DRAWN BY: LUJU/GLS  
 CHECKED BY: SSC

B+T ENGINEERING, INC.  
 PEC.0001564  
 Expires 02/10/18



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HARTFORD - NJ (SSUSA)  
 876383  
 218 NEW PARK RD.  
 HARTFORD, CT  
 EXISTING 108 MONOPOLE

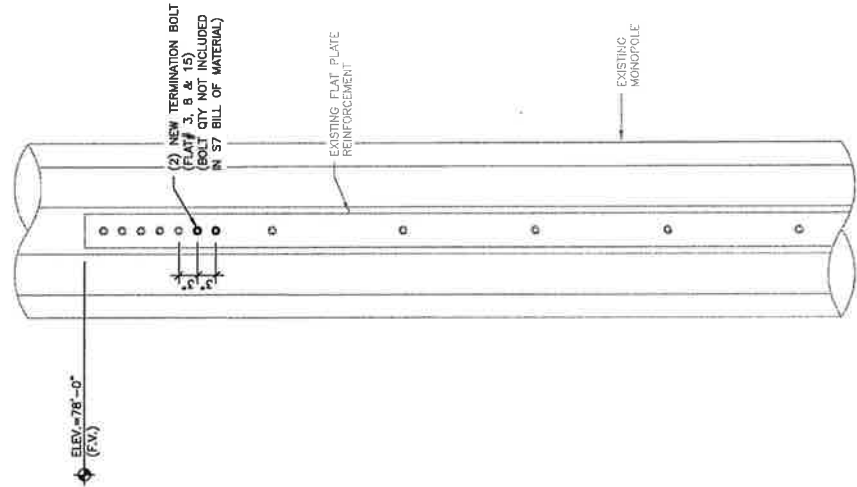
SHEET TITLE  
 EXISTING FLAT PLATE  
 MODIFICATION DETAILS

SHEET NUMBER  
**S9**

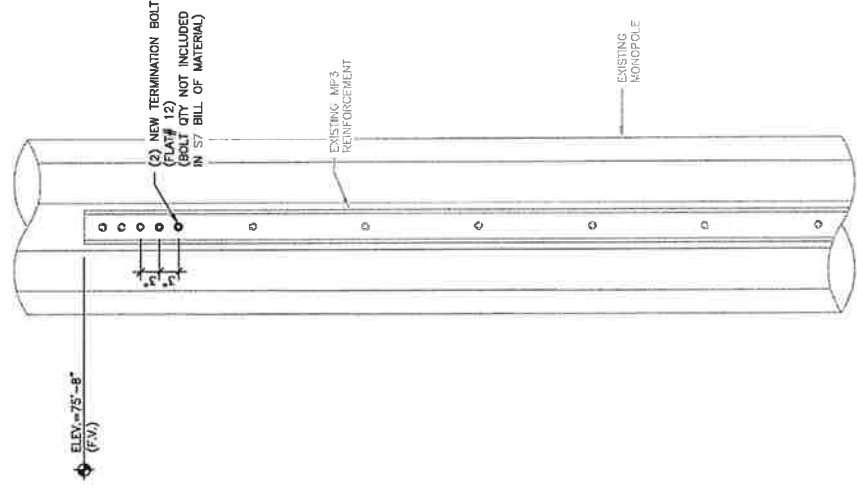
REVISION  
**0**

NOTE: SOME EXISTING REINFORCEMENT PLATES ARE NOT SHOWN FOR CLARITY.

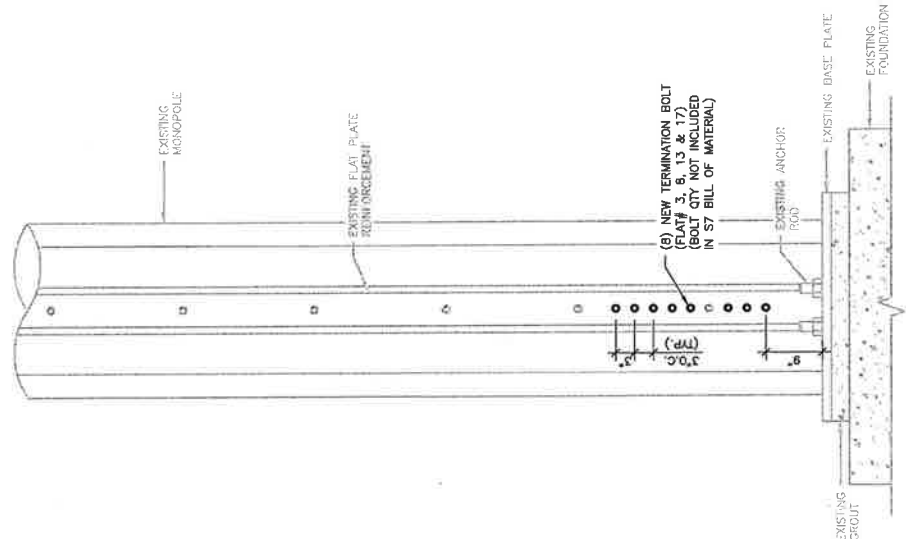
CONTRACTOR NOTE:  
 FIELD VERIFY EXISTING BOLT LOCATIONS.  
 CONTACT B+T GROUP FOR ANY DISCREPANCIES.



3 EXISTING FLAT PLATE MODIFICATION DETAIL (78)  
 SCALE: N.T.S.



2 EXISTING FLAT PLATE MODIFICATION DETAIL (75.67)  
 SCALE: N.T.S.



1 EXISTING FLAT PLATE MODIFICATION DETAIL (0)  
 SCALE: N.T.S.



# CROWN CASTLE

ISSUED FOR:	
REV	DATE DESCRIPTION
0	05/16/17 ISSUED FOR CONSTRUCTION

PROJECT NO: 8585 086 01  
PROJECT ENG: ROBERT M. FRAZIER  
DRAWN BY: ULJ/GLS  
CHECKED BY: SSC

B+T ENGINEERING, INC.  
REG. 0001584  
Expires 02/10/18

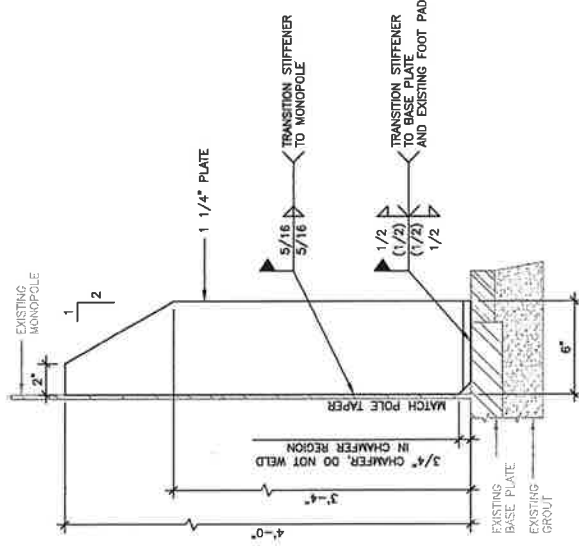


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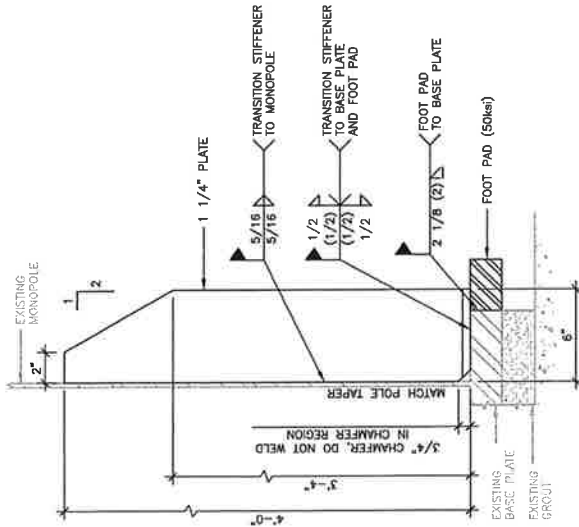
HARTFORD - NJ (SSUSA)  
876363  
218 NEW PARK RD.  
HARTFORD, CT  
EXISTING 10" MONOPOLE

SHEET TITLE  
TRANSITION STIFFENER  
DETAILS

SHEET NUMBER: **D1**  
REVISION: **0**



② TRANSITION STIFFENER DETAIL  
SCALE: N.T.S.



① TRANSITION STIFFENER DETAIL  
SCALE: N.T.S.



# CROWN CASTLE

REV	DATE	DESCRIPTION
0	05/14/17	ISSUED FOR CONSTRUCTION

PROJECT NO: 85565.06101  
 PROJECT ENG: ROBERT M. FRAZIER  
 DRAWN BY: LUJ/GLS  
 CHECKED BY: SSC

B+T ENGINEERING, INC.  
 P.E.C. 0001584  
 Expires 02/10/18



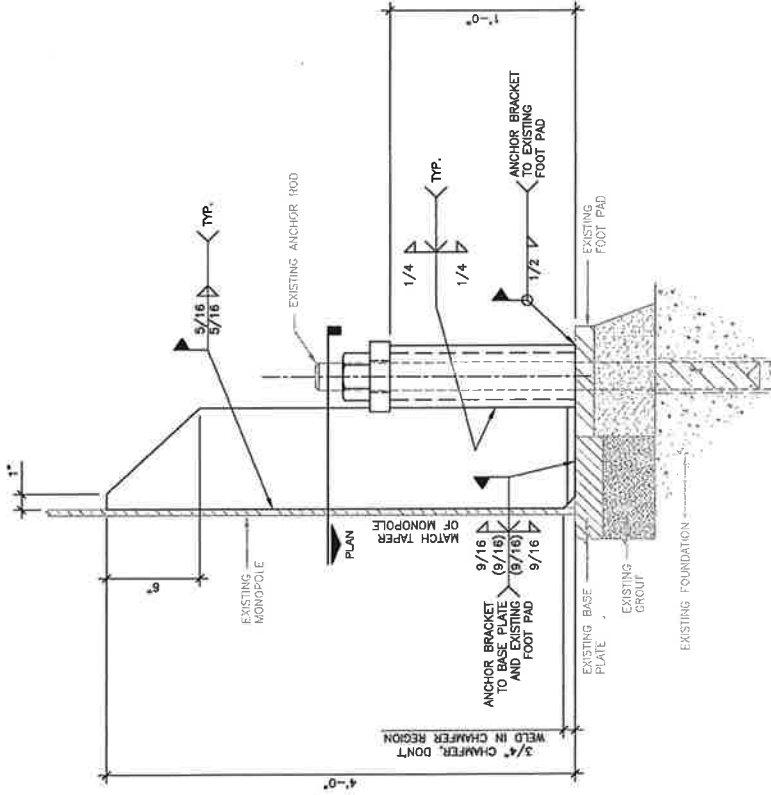
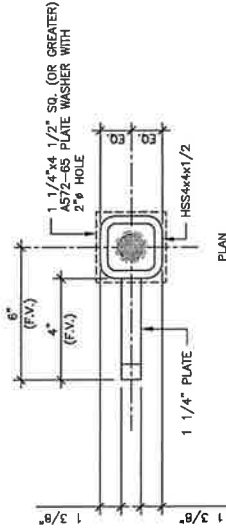
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HARTFORD - NU (SSUSA)  
 876363

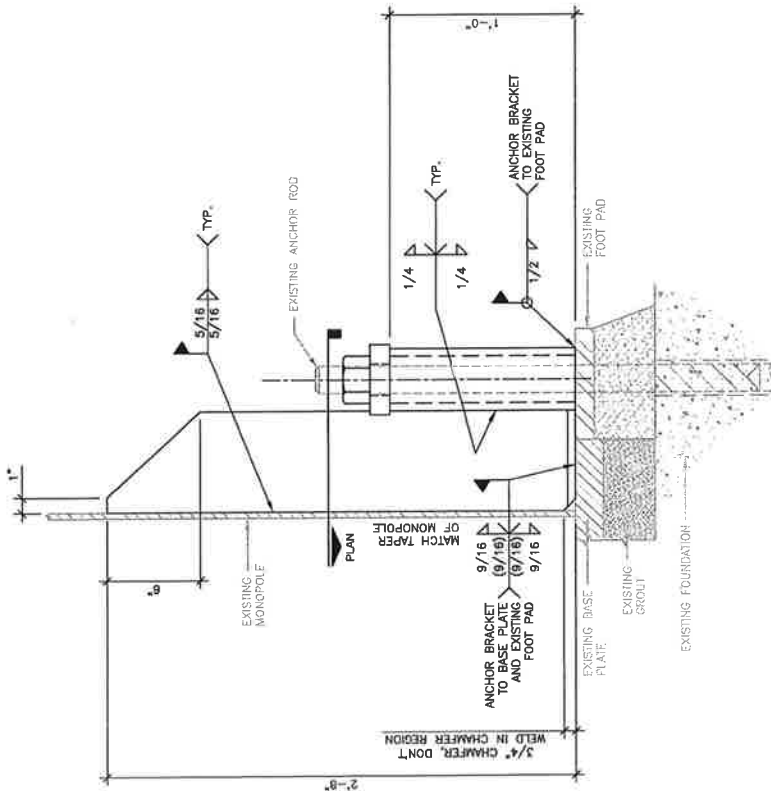
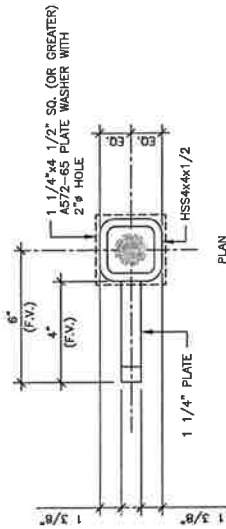
219 NEW PARK RD.  
 HARTFORD, CT  
 EXISTING 108' MONOPOLE

SHEET TITLE  
 ANCHOR ROD BRACKET  
 DETAILS

SHEET NUMBER: **D2**  
 REVISION: **0**



2 ANCHOR ROD BRACKET DETAIL  
 SCALE: N.T.S.



1 ANCHOR ROD BRACKET DETAIL  
 SCALE: N.T.S.



**B+T GRP**  
 1777 S. BOULDER AVE.  
 SUITE 300  
 PUEBLO, CO 81001  
 PH: (303) 791-6850  
 WWW.BTGRP.COM

# CROWN CASTLE

ISSUED FOR:  
 REV. DATE DESCRIPTION  
 0 05/14/17 ISSUED FOR CONSTRUCTION

PROJECT NO.: 85665.006.01  
 PROJECT ENG.: ROBERT M. FRAZIER  
 DRAWN BY: LUJ/GLS  
 CHECKED BY: SSC

B+T ENGINEERING, INC.  
 PEC.0001584  
 Expires 02/10/18



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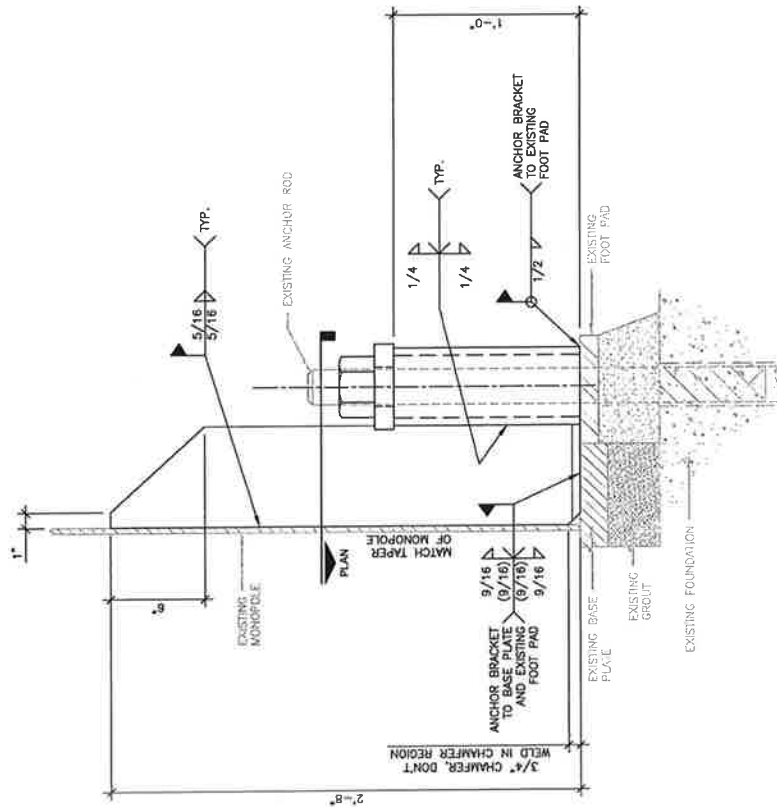
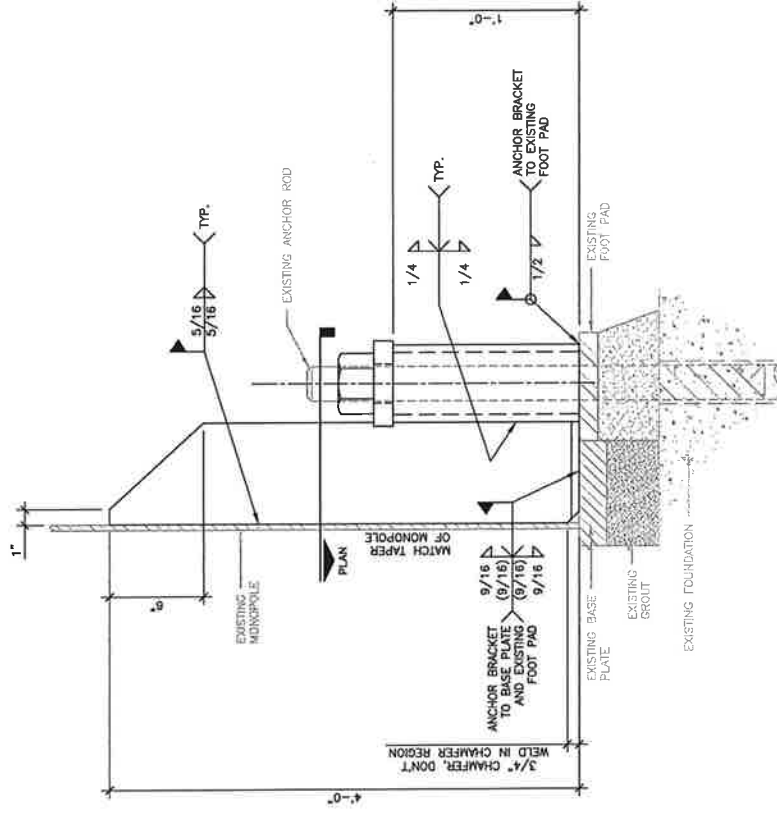
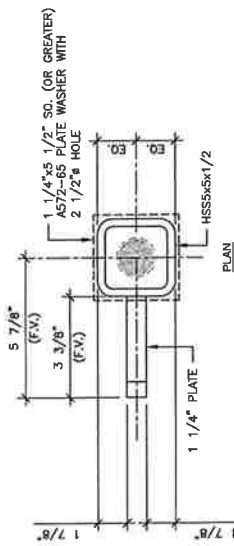
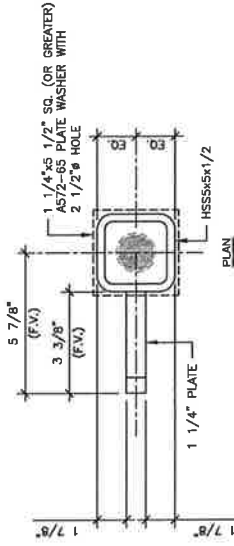
HARTFORD - NU (SSUSA)  
 876363

218 NEW PARK RD.  
 HARTFORD, CT  
 EXISTING 108' MONOPOLE

SHEET TITLE  
 ANCHOR ROD BRACKET  
 DETAILS

SHEET NUMBER  
**D3**

REVISION  
**0**



② ANCHOR ROD BRACKET DETAIL  
 SCALE: N.T.S.

① ANCHOR ROD BRACKET DETAIL  
 SCALE: N.T.S.

# **ATTACHMENT 4**



### Unofficial Property Record Card - City of Hartford, CT

#### General Property Data

Parcel Identification 138-472-001	Property Location 0219 NEW PARK AV HARTFORD
Property Owner CONN LIGHT & POWER CO	Property Use OTHER UTILITY
Mailing Address PO BOX 270	Most Recent Sale Date 7/2/1982
City HARTFORD	Legal Reference 01977 0129
Mailing State CT Zip 06141-0270	Grantor
ParcelZoning MS-3	Sale Price 0
	Land Area 7.140 acres

#### Current Property Assessment

Fiscal Year 2016	Total Value 1,117,900
Land Value 1,094,870	Building Value 12,460

#### Building Description

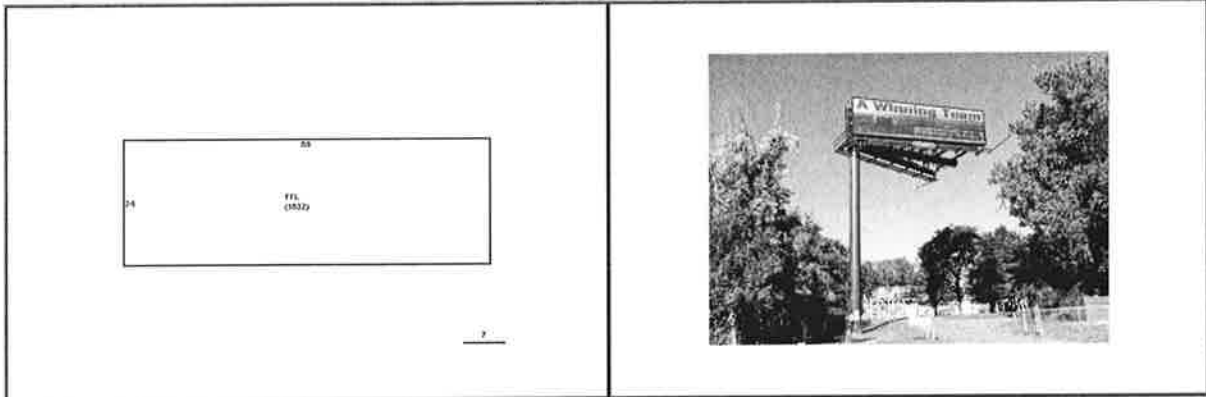
Building Style WAREHSE	Foundation Type Concrete	Flooring Type COMBINATION
# of Living Units 0	Frame Type Steel Light	Basement Floor N/A
Year Built 1978	Roof Structure GABLE/HIP	Heating Type Electric
Building Grade Economy	Roof Cover Metal	Heating Fuel Electric
Building Condition Average	Siding Metal	Air Conditioning 0%
Finished Area (SF) 1632	Interior Walls DRYWALL	# of Bsmt Garages 0
Number Rooms 0	Number Beds 0	# of Full Baths 0
# of 3/4 Baths 0	# of 1/2 Baths 0	# of Other Fixtures 0

#### Legal Description

#### Narrative Description of Property

This property contains 7.140 acres of land mainly classified as OTHER UTILITY with a(n) WAREHSE style building, built about 1978 , having Metal exterior and Metal roof cover, with 0 unit(s), 0 room(s), 0 bedroom(s), 0 bath(s), 0 half bath(s).

#### Property Images



Disclaimer: This information is believed to be correct but is subject to change and is not warranted.

# **ATTACHMENT 5**





# Certificate of Mailing — Firm

Name and Address of Sender

Kenneth C. Baldwin, Esq.  
Robinson & Cole LLP  
280 Trumbull Street  
Hartford, CT 06103

TOTAL NO.  
of Pieces Listed by Sender

3

TOTAL NO.  
of Pieces Received at Post Office™

3

Affix Stamp Here

Postmark with Date of Receipt.

neopost<sup>SM</sup>  
09/27/2017  
**US POSTAGE \$002.38**  
ZIP 06103  
041L122033E

Postmaster, per (name of receiving employee)

*[Handwritten Signature]*

ANN STREET STATION  
06103  
SEP 27 2017

USPS® Tracking Number  
Firm-specific Identifier

Address  
(Name, Street, City, State, and ZIP Code™)

Postage

Fee

Special Handling

Parcel Airlift

1.

Luke Bronin, Mayor  
City of Hartford  
550 Main Street  
Hartford, CT 06103

2.

Jamie Bratt, AICP, Director of Planning and  
Economic Development  
City of Hartford  
250 Constitution Plaza  
Hartford, CT 06103

3.

CL&P  
P.O. Box 270  
Hartford, CT 06141-0270

4.

5.

6.