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ORIGINAL

Via Federal Express

December 12, 2007

RECEIVED
DEC 13 2007

CONNECTICUT
SITING COUNCIL

S. Derek Phelps
Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

RE: Sprint Nextel Corporation - Exempt Modification

Dear Mr. Phelps:

Enclosed for filing is Sprint Nextel Corporation's Notice of Exempt Modification for the addition of WiMAX antennas to an existing tower at 2755 State Street in Hamden, Connecticut. I have also enclosed a check in the amount of \$500.00 to cover the filing fee. If you have any questions, please feel free to contact me.

Very truly yours,

BROWN RUDNICK BERLACK ISRAELS LLP

By: Thomas J. Regan /cm
Thomas J. Regan

cc: Town of Hamden via 1st Class Mail

40246856 v1 - MERECM - 025064/0015

ORIGINAL

CONNECTICUT SITING COUNCIL

RECEIVED
DEC 13 2007

EM-SPRINT-NEXTEL-062-071213

In re:

Sprint Nextel Corporation's Notice to Make an
Exempt Modification to an Existing Facility at
2755 State Street, Hamden, Connecticut.

: EXEMPT MODIFICATION NO. _____

: December 12, 2007

NOTICE OF EXEMPT MODIFICATION

Pursuant to Conn. Agencies Regs. §§ 16-50j-73 and 16-50j-72(b), Sprint Nextel Corporation ("Sprint") hereby gives notice to the Connecticut Siting Council ("Council") and the Town of Hamden of Sprint's intent to make an exempt modification to an existing lattice tower (the "Tower") located at 2755 State Street in Hamden, Connecticut. Specifically, Sprint plans to add three WiMAX antennas and one microwave dish to its current antenna array. Under the Council's regulations (Conn. Agencies Regs. § 16-50j-72(b)), Sprint's plans do not constitute a modification subject to the Council's review because Sprint will not change the height of the Tower, will not extend the boundaries of the compound, will not increase the noise levels at the site, and will not increase the total radio frequency electromagnetic radiation power density at the site to levels above applicable standards.

Sprint is currently undertaking an upgrade to its wireless communications system in Connecticut. As part of the upgrade, Sprint is implementing WiMAX technology to enable enhanced wireless data communications. In order to accomplish the upgrade at this site, Sprint plans to add three WiMAX antennas and one microwave dish to the existing antenna configuration and install additional WiMAX-related electronic equipment at the base of the Tower.

The Tower is a 120-foot lattice tower located at 2755 State Street in Hamden, Connecticut (Latitude 41° 21' 19.68" N, Longitude 72° 53' 25.07" W). The Tower is owned by Crown Castle International. AT&T is also located on the Tower. Currently, Sprint has six CDMA network antennas located on the Tower (two per sector) with an antenna centerline at 120 feet. The CDMA equipment is located on a steel platform on an 8-foot by 11-foot concrete pad at the base of the Tower within the existing compound. A site plan with the Tower specifications is attached.

Sprint plans to add one KMW-AM-X-WM-17-65-00T (WiMAX) antenna to the empty center pipe mast on each of the three sectors. The antenna centerline will remain the same (120 feet) as the existing antennas. The WiMAX antennas will require six coaxial cables, 1-5/8" in diameter as well as three KMW KMDAPS2050000 Tower Mounted Amplifiers to be installed. Sprint will also install one Andrew VHLP2-23-2WH microwave dish on the same platform with one 1/2" coaxial cable. To confirm the Tower can support these changes, Sprint commissioned GPD Associates to perform a structural analysis of the Tower (attached). According to the structural analysis, dated November 30, 2007, "the tower and its foundation are sufficient for the proposed, existing, and reserved loadings."

Sprint will expand the steel frame and concrete pad by two feet to accommodate the WiMAX radio and power cabinets. This will not require the size of the compound to be increased. Sprint will also install a new power protection cabinet ("PPC") and remove the old PPC. In addition, Sprint plans to mount a global positioning system (GPS) antenna to the PPC. Therefore, excluding brief, minor, construction-related noise during the addition of the antennas and the installation of the equipment cabinets, Sprint's changes to the Tower will not increase noise levels at the site.

The addition of the WiMAX antennas and microwave dish to Sprint's existing antenna array will not adversely impact the health and safety of the surrounding community or the people working on the Tower. The total radio frequency exposure measured around the Tower will be well below the National Council on Radiation Protection and Measurements' ("NCRP") standard adopted by the Federal Communications Commission ("FCC"). The worst-case power density analysis measured at the base of the Tower indicates that the WiMAX antennas and microwave dish will emit 4.208% and 0.02%, respectively, of the NCRP's standard for maximum permissible exposure. A cumulative power density analysis indicates that together, all of the antennas on the Tower will emit only 46.1437% of the NCRP's standard for maximum permissible exposure. Therefore, the power density levels will be well below the FCC mandated radio frequency exposure limits in all locations around the Tower, even with extremely conservative assumptions. The power density analysis is attached.

In conclusion, Sprint's proposed plan to add three WiMAX antennas, one microwave dish and the associated WiMAX equipment to the site does not constitute a modification subject to the Council's jurisdiction because Sprint will not increase the height of the Tower, will not extend the boundaries of the site, will not increase the noise levels at the site, and the total radio frequency electromagnetic radiation power density will stay within all applicable standards. *See* Conn. Agencies Regs. § 16-50j-72.

Sprint Nextel Corporation

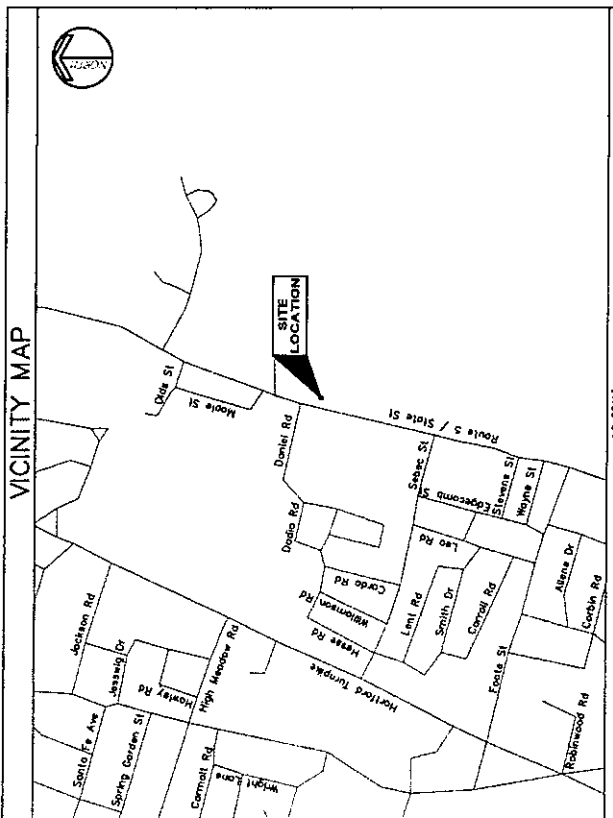
By: Thomas J. Regan /cm
Thomas J. Regan

Brown Rudnick Berlack Israels LLP
185 Asylum Street, CityPlace I
Hartford, CT 06103-3402
Email - tregan@brownrudnick.com
Phone - 860.509.6522
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**AMONTOWESE AMODIO SELF STORE, 2755 ST
CT01YC344 / CT03XC011
2755 STATE STREET
HAMDEN, CT 06517**

NOT FOR CONSTRUCTION



DRIVING DIRECTIONS

FROM 100 CORPORATE PLACE, ROCKY HILL, CT.:

HEAD SOUTH ON CORPORATE PL TOWARD WEST ST. TURN
LEFT AT WEST ST. SLIGHT RIGHT TO MERGE ONTO I-91 S.
TOWARD NEW HAVEN. TAKE EXIT 10 TO MERGE ONTO CT-4
N. TOWARD MT CARMEL/HANDEN. TAKE THE EXIT TOWARD
DEVINE ST. TURN RIGHT AT DEVINE ST./US-5.

PROJECT INDEX

SITE NUMBER:	CT01YC344 / CT03XC011
SITE NAME:	MONTWHESE AMODIO SELF STORE, 2755 ST
SITE ADDRESS:	2755 STATE STREET HAMDEN, CT 06517
APPLICANT:	SPRINT NEXTEL CORP. 1 INTERNATIONAL BLYD., SUITE 600 MAHWAH, NJ 07495
APPLICANT REPRESENTATIVE:	TRANSCEND WIRELESS, LLC 479 ROUTE 17 NORTH, 2ND FLOOR, NJ 07430
CONTACT:	JASON DEIBERT (347) 284-8617
PROPERTY OWNER:	CROWN CASTLE INTERNATIONAL 46 BROADWAY ALBANY, NY 12204
JURISDICTION:	CONNECTICUT SITING COUNCIL
TAX MAP / LOT:	2332/008
ZONING DISTRICT:	CDD1
CORINATES:	
LATITUDE:	41° 21' 19.68"N (41.35546944 N)
LONGITUDE:	72° 53' 25.07"W (72.89029967 W)

SHEET INDEX

SHEET NO:	SHEET TITLE	REVISION HISTORY	
		NO:	DATE
T01	TITLE SHEET	0	12 / 05 / 07
SC01	COMPOUND PLAN	0	12 / 05 / 07
SC02	TOWER ELEVATION	0	12 / 05 / 07

APPLICANT:

SPRINT NEXTEL
CORP.

1 INTERNATIONAL BLVD, SUITE 800
MAHWAH, NJ 07495

TRANSCEND WIRELESS, LLC
479 ROUTE 17 NORTH,
2ND FLOOR
MAHWAH, NJ 07430

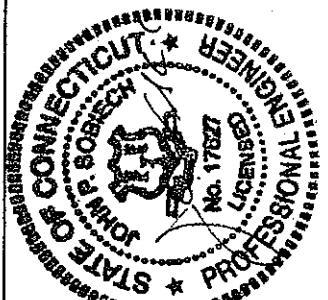
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CLOUGH HARBOUR & ASSOCIATES LLP
 161 Winthrop Circle, PO Box 5269 • Albany, NY 12205-0269
 Main (518) 453-4500 • www.cloughharbour.com

CMA PROJECT NO.
17161 - 3002 - 1601

NO.	SUBMITTAL		
0	12/05/07	ISSUED FOR SITING COUNCIL	
	BY: PAL	CHK: RJT	APPRO: JPS



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SITE ID:
CT01YC344--CT03XC011

SITE NAME:
MONTOWESE AMODIO
SELF STORE, 2755 ST

SITE ADDRESS:
2755 STATE STREET
HAMDEN, CT 06517
NEW HAVEN COUNTY

SHEET TITLE
TITLE SHEET

SHEET NUMBER

Top



1 INTERNATIONAL BLVD, SUITE 800
MAHWAH, NJ 07495

TRANSCEND WIRELESS, LLC
479 ROUTE 17 NORTH,
2ND FLOOR
MAHWAH, NJ 07430

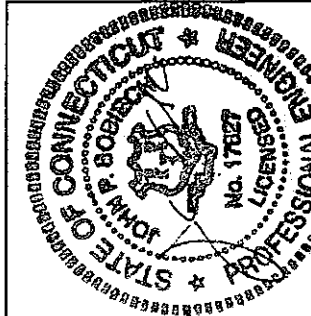
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CLOUGH HARBOUR & ASSOCIATES LLP
10 Vivian Drive, PO Box 5269 - Albany, NY 12205-0269
Main: 518-453-5000 • Fax: 518-453-5001 • www.cha-engineers.com

CH&A PROJECT NO:
17181 - 3002 - 1801

SUBMITTAL			
NO	DATE	ISSUED FOR	BY
0	12/05/07	FOR SITING COUNCIL	BY: PAL
			CHK: RJT
			APP'D: JPS

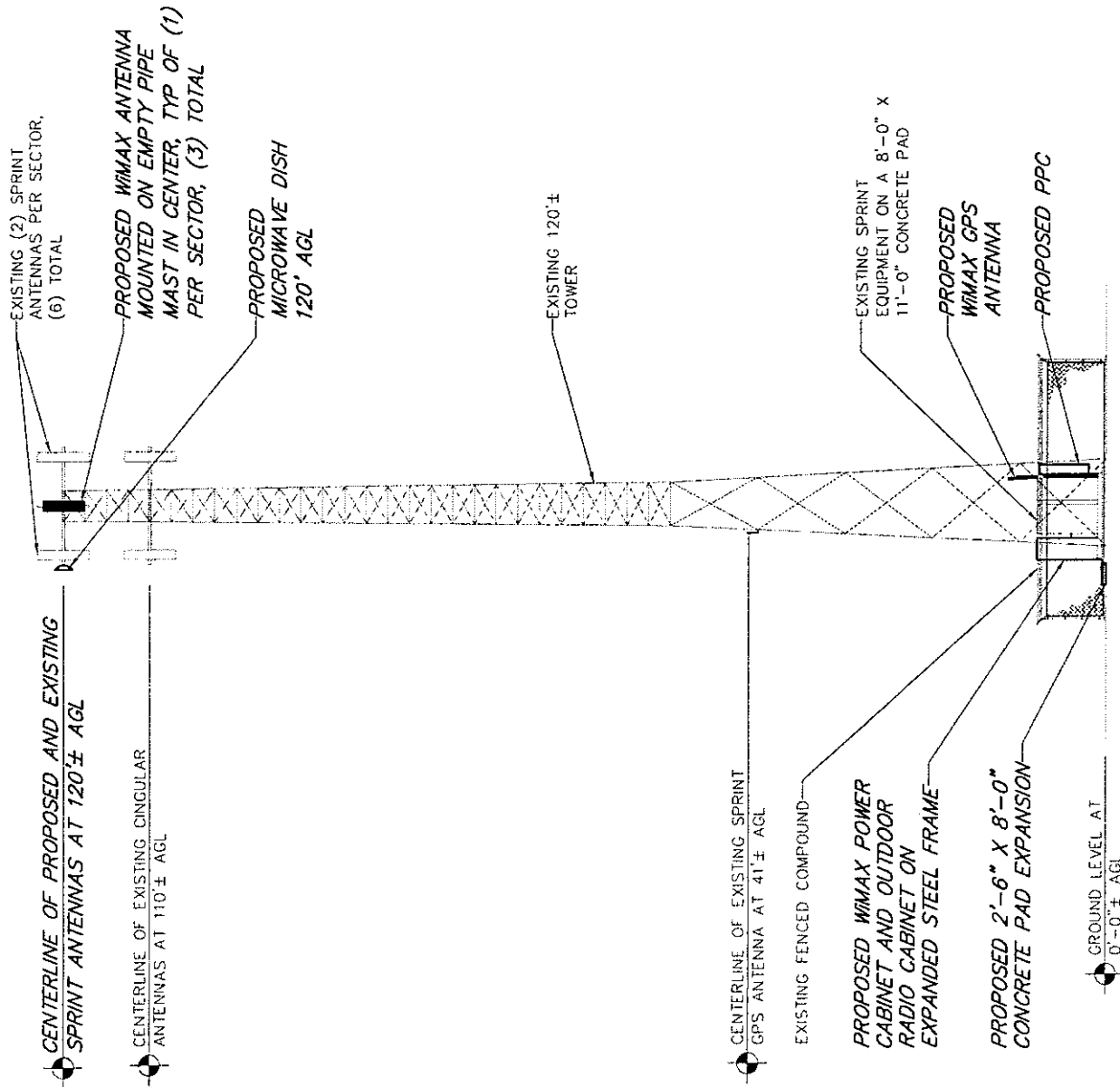


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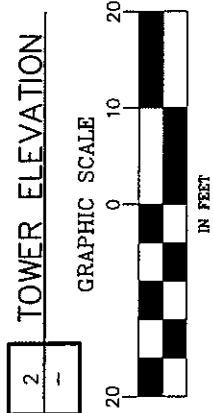
SITE ID: CT01YC344-CT03XC011
SITE NAME:
MONTWESE AMODIO
SELF STORE, 2755 ST
SITE ADDRESS:
2755 STATE STREET
HAMDEN, CT 06517
NEW HAVEN COUNTY

SHEET TITLE
TOWER ELEVATION

SHEET NUMBER
SC02



ANTENNA AZIMUTHS:
SECTOR (1) = 20°
SECTOR (2) = 190°
SECTOR (3) = 280°
MICROWAVE = 126.7°



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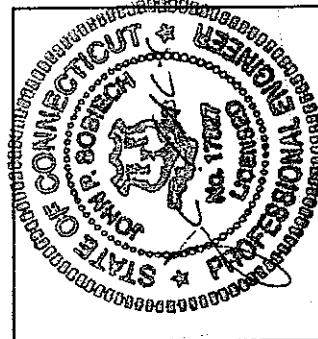
APPLICANT:

**SPRINT NEXTEL
CORP.**
1 INTERNATIONAL BLVD., SUITE 800
MAHWAH, NJ 07430

TRANSCEND WIRELESS, LLC
479 ROUTE 17 NORTH,
2ND FLOOR
MAHWAH, NJ 07430

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CH&A
CLOUGH HARBOUR & ASSOCIATES LLP
18 Wenden Circle, PO Box 589 - Albany, NY 12201-0589
Phone: 518 433-4300 • www.chaengineers.com
C-H-A PROJECT NO.
17181 - 3002 - 1801

SUBMITTAL			
NO.	12/05/07	ISSUED FOR SITING COUNCIL	
0	BY: PAL	CHK: RLT	APRD: JPS

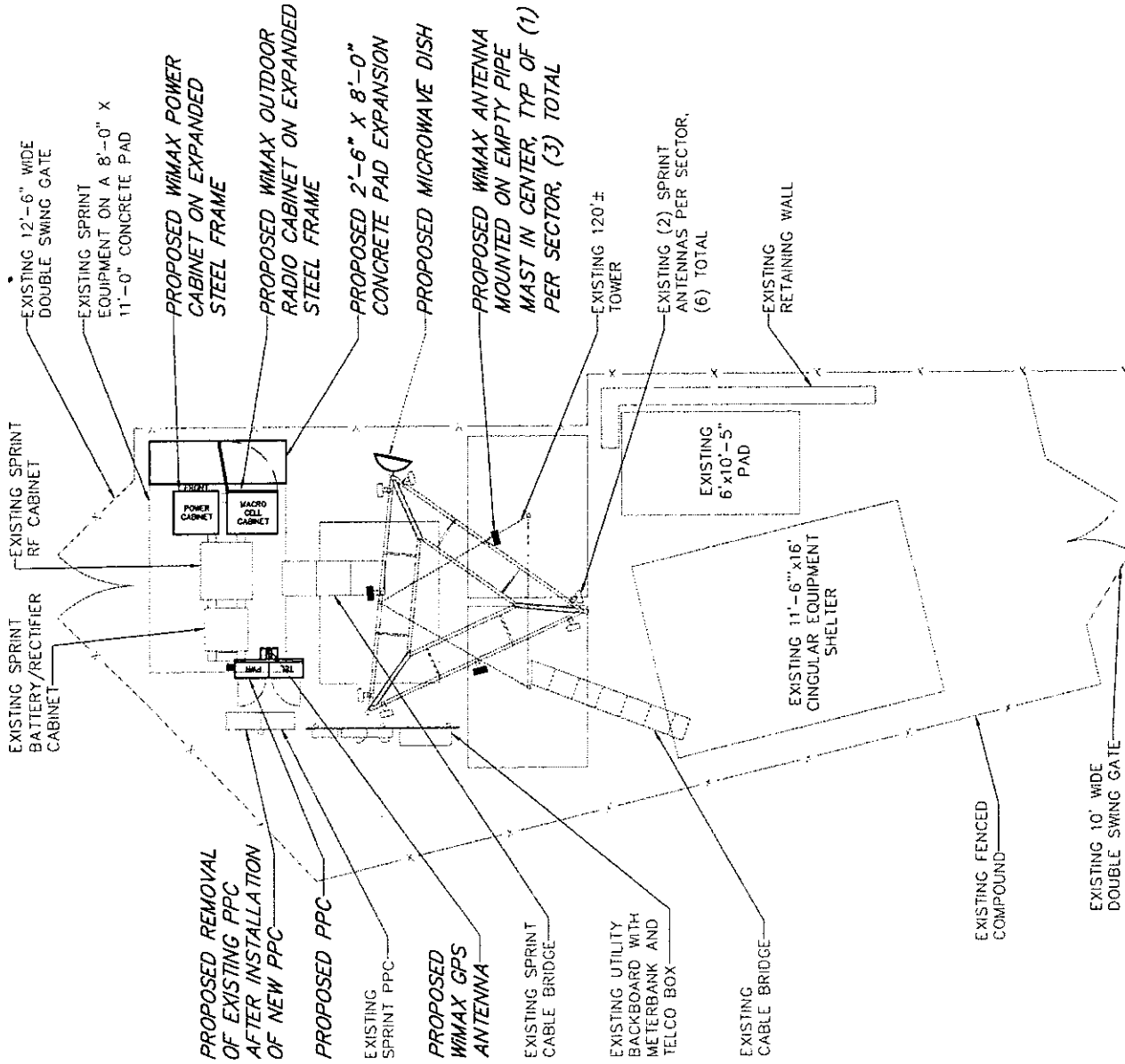


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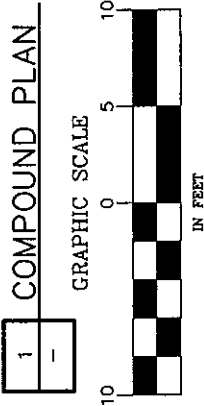
SITE ID: CT01YC344-CT03XC011
SITE NAME: MONTOWESE AMODIO
SELF STORE, 2755 ST
SITE ADDRESS: 2755 STATE STREET
HAMDEN, CT 06517
NEW HAVEN COUNTY

SHEET TITLE
COMPOUND PLAN

SHEET NUMBER
SC01



ANTENNA AZIMUTHS:
SECTOR (1) = 20°
SECTOR (2) = 190°
SECTOR (3) = 280°
MICROWAVE = 126.7°



NOT FOR CONSTRUCTION

Date: November 30, 2007



Eva Morales
Crown Castle International
46 Broadway
Albany, NY 12204
(518) 433-6250

GPD Associates
520 South Main St., Suite 2531
Akron, Ohio 44311
(614) 210-0751
mimiller@gpdgroup.com

Subject: Structural Analysis Report

Carrier Designation Sprint PCS Co-locate
Sprint PCS Job Name: Montowese Amodio Self Store, 27
Sprint PCS Job Number: CT03XC011

Crown Castle Designation Crown Castle BU Number: 876312
Crown Castle Site Name: Montowese Amodio Self Store
Crown Castle JDE Job Number: 95988

GPD Associate Designation GPD Associates Project Number: 2007287.88

Site Data 2755 State Street, Hamden, Connecticut 06473
Latitude 41° 21' 19.67", Longitude 72° 53' 25.13"
120' PiROD Self Support Tower

Dear Ms. Morales,

GPD is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the aforementioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 265840, in accordance with application 53295, revision 3. The purpose of the analysis is to determine the suitability of the tower with the existing and reserved loading configurations and the addition of the following proposed loading configuration:

Elev. 120' (3) KMW AM-X-WM-17-65-00T Antennas on an existing 13' LP Platform w/ (6) 1-5/8" coax
(1) Andrew VHLP2-23-2WH Dish on the same platform w/ (1) 1/2" coax
(3) KMW KMDAPS2050000 Tower Mounted Amplifiers mounted behind the antennas

This analysis has been performed in accordance with the TIA/EIA-222-F standard and the 2005 CBC based upon a wind speed condition of 85 mph. Based on our analysis we have determined the tower and its foundation are sufficient for the proposed, existing, and reserved loadings as referenced in Tables 1 and 2.

We at GPD appreciate the opportunity of providing our continuing professional services to you and Crown Castle International. If you have any questions please do not hesitate to call.

Respectfully submitted,

David B. Granger, P.E.
Connecticut #: 17557

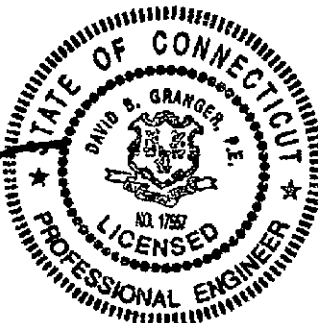


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APPENDIX A

 RISA Tower Output File

APPENDIX B

 Tower Elevation Drawing

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 Base Level Drawing

APPENDIX D

 Foundation Analysis

EXECUTIVE SUMMARY

The purpose of this analysis was to verify that the modified structure is capable of carrying the proposed loading configuration as specified by Sprint PCS to Crown Castle International. This report was commissioned by Ms. Eva Morales of Crown Castle International.

The tower is structurally satisfactory for the proposed loading configuration for a basic wind speed of 85 mph with ½" radial ice (25% reduction) in accordance with TIA/EIA-222-F and the 2005 CBC. The tower rating/capacity is 104.3%, which is within customary engineering tolerance and is therefore satisfactory.

The foundation reactions, with the proposed loads, were found to be less than the capacity of the existing foundations. Therefore, the foundation is adequate. The foundation rating/capacity is 100.8%, which is within customary engineering tolerance and is therefore satisfactory.

ANALYSIS CRITERIA

The current requirements of TIA/EIA-222-F and the 2005 CBC are for a basic wind speed of 85 mph with ½" of radial ice. A 25% reduction in wind load is allowed when wind and ice are applied simultaneously. TIA/EIA-222-F requires towers within New Haven County, Connecticut be analyzed with an 85 mph wind speed.

Table 1 – Proposed Antenna and Cable Information

Center Line Elevation (feet)	Number Of Antenna	Antenna Manufacturer	Antenna Model	Mount Type	Number Of Feed Lines	Feed Line Size (inches)
120*	3	KMW	AM-X-WM-17-65-00T		6	1-5/8
	1	Andrew	VHLP2-23-2WH		1	1/2
	3	KMW	KMDAPS2050000 TMA's			

* Both the MLA and Proposed loading scenarios were considered. The MLA loading was found to control the analysis.

Table 2 – Existing and Reserved Antenna and Cable Information

Center Line Elevation (feet)	Number Of Antenna	Antenna Manufacturer	Antenna Model	Mount Type	Number Of Feed Lines	Feed Line Size (inches)
120	4	Decibel	DB950F40T2E-M	16'-6" LP Platform	4	1-1/4
	2	Decibel	DB950G65E-M		2	1-1/4
110	6	Powerwave	7770.00	(3) 12' T-frames	12	1-5/8
	3 (Reserved)	Powerwave	7770.00			
	6	Powerwave	LGP21403 TMA's			
	6 (Reserved)	Powerwave	LGP21403 TMA's			
61	1 (Reserved)	Kathrein	738 449	2' Standoff	1	1/2
	1 (Reserved)	Trimble	Bullet III		1	1/2
41	1	Trimble	Bullet III	Leg	1	1/2

TOWER DESCRIPTION

The 120' tower is supported on three legs and has seven major sections. It has a triangular cross section made of bolted connections from 0' – 50' and welded connections from 50' – 120', with an "X" frame configuration. The tower is fabricated with PiROD truss legs and angle diagonals from 0' – 50' and solid round legs and diagonals from 50' – 120'.

The tower was originally designed for Sprint PCS by PiROD, Inc. of Plymouth, Indiana for a 90 mph wind speed with ½" radial ice in accordance with EIA/TIA-222-F.

ANALYSIS PROCEDURE

Table 4 – Documents Provided

Document	Remarks	Reference	Source
Original Tower Drawings	PiROD File #: A-113604 Rev F, dated 11/4/97	Doc ID # 1611638	Crown DMZ
Foundation Drawings	PiROD File #: A-113604 Rev F, dated 11/4/97	Doc ID # 1611716	Crown DMZ

Analysis Methods

RISA Tower (Version 5.0.2.0), a commercially available software program, was used to create a three-dimensional model of the tower and calculate member stresses for various dead, live, wind, and ice load cases. All loads were computed in accordance with the ANSI/EIA/TIA-222-F and all local building code requirements. Selected output from the analysis is included in Appendix A.

Assumptions

1. Tower and structures were built in accordance with the manufacturer's specifications.
2. The tower and structures have been maintained in accordance with manufacturer's specifications.
3. The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 & 2, and the referenced drawings.
4. Dishes are oriented per Crown CAD elevation drawings

If any of these assumptions are not valid or have been made in error, this analysis may be affected, and GPD Associates should be allowed to review any new information to determine its effect on the structural integrity of the tower.

ANALYSIS RESULTS

Table 5 – Tower Summary

Notes	Member	Strength (KSI)	Capacity	Elevation (feet)	Results
	Legs	50	104.3%	0 – 20	Pass
	Leg Bolts	44	81.6%	70	Pass
2	Diagonals	36	65.0%	0 – 20	Pass
	Diag. Bolts	21	50.1%	0 – 20	Pass
	Anchor Bolts	150	53.3%	0	Pass
1	Foundation	Compression	69.8%		Pass
1		Uplift	100.8%		Pass
Structure Rating: 104.3%					

- 1) See additional documentation in Appendix D for calculations supporting the % capacity used.
- 2) Yield Strength varies see Appendix B.

Recommended Modifications

The tower and its foundations are sufficient for the proposed loads and do not require modifications.

DISCLAIMER OF WARRANTIES

The engineering services rendered by GPD ASSOCIATES in connection with this Structural Analysis are limited to a computer analysis of the tower structure, size and capacity of its members. GPD ASSOCIATES does not analyze the fabrication, including welding, except as included in this report.

The purpose of this report is to assess the feasibility of adding appurtenances usually accompanied by transmission lines. Any mentions of structural modifications are reasonable estimates and should not be used as a precise construction document. Precise modification drawings are obtainable from GPD Associates, but are beyond the scope of this report.

GPD ASSOCIATES makes no warranties, expressed or implied, in connection with this report and disclaims any liability arising from material, fabrication, and erection of this tower. GPD ASSOCIATES will not be responsible whatsoever for, or on account of, consequential or incidental damages sustained by any person, firm, or organization as a result of any data or conclusions contained in this report. The maximum liability of GPD ASSOCIATES pursuant to this report will be limited to the total fee received for preparation of this report.

APPENDIX A

RISA Tower Output File

RISATower GPD Associates 520 South Main St. Suite 2531 Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101	Job	Monotowese Amodio Self Store - BU# 876312	Page	1 of 3
	Project	2007287.88	Date	16:18:04 11/29/07
	Client	Crown Castle	Designed by	mimiller

Tower Input Data

The main tower is a 3x free standing tower with an overall height of 120.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 3.50 ft at the top and 10.00 ft at the base.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in New Haven County, Connecticut.

Basic wind speed of 85 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 74 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight klf
LDF7-50A (1-5/8 FOAM)	C	No	Ar (Leg)	120.00 - 8.00	0.0000	0.09	9	6	1.0000	1.9800		0.00
LDF4P-50A (1/2 FOAM)	C	No	Ar (Leg)	40.00 - 8.00	0.0000	0.09	1	1	1.0000	1.0000		0.00
LDF7-50A (1-5/8 FOAM)	B	No	Ar (Leg)	110.00 - 8.00	0.0000	0.09	12	6	1.0000	1.9800		0.00
LDF4RN-50A (1/2 FOAM)	B	No	Ar (Leg)	60.00 - 8.00	0.0000	0.09	2	1	1.0000	0.6300		0.00

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K
PiROD 13' Low Profile Platform (Monopole)	C	None		0.0000	120.00	No Ice 15.70 1/2" Ice 20.10	No Ice 15.70 1/2" Ice 20.10	1.30 1.76
(3) FV65-14-00NA2	B	From Face	3.06 -2.57 2.00	-40.0000	120.00	No Ice 8.40 1/2" Ice 8.95	No Ice 5.28 1/2" Ice 5.74	0.03 0.08
(3) FV65-14-00NA2	C	From Face	3.94 0.69 2.00	10.0000	120.00	No Ice 8.40 1/2" Ice 8.95	No Ice 5.28 1/2" Ice 5.74	0.03 0.08
(3) FV65-14-00NA2	A	From Face	3.76 -1.37	-20.0000	120.00	No Ice 8.40 1/2" Ice 8.95	No Ice 5.28 1/2" Ice 5.74	0.03 0.08

RISA Tower GPD Associates 520 South Main St. Suite 2531 Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101	Job	Monotowese Amodio Self Store - BU# 876312	Page	2 of 3
	Project	2007287.88	Date	16:18:04 11/29/07
	Client	Crown Castle	Designed by	mimiller

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
PiROD 12' Lightweight T-Frame	A	From Leg	2.00 2.00 0.00 0.00	0.0000	110.00	No Ice 1/2" Ice	10.20 16.20	10.20 16.20	0.25 0.35
PiROD 12' Lightweight T-Frame	B	From Leg	2.00 0.00 0.00	0.0000	110.00	No Ice 1/2" Ice	10.20 16.20	10.20 16.20	0.25 0.35
PiROD 12' Lightweight T-Frame	C	From Leg	2.00 0.00 0.00	0.0000	110.00	No Ice 1/2" Ice	10.20 16.20	10.20 16.20	0.25 0.35
(3) 7770.00	A	From Leg	0.00 4.00 0.00 0.00	0.0000	110.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
(3) 7770.00	B	From Leg	0.00 4.00 0.00 0.00	0.0000	110.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
(3) 7770.00	C	From Leg	0.00 4.00 0.00 0.00	0.0000	110.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
(4) LGP2140X	A	From Leg	0.00 4.00 0.00 0.00	0.0000	110.00	No Ice 1/2" Ice	0.00 0.00	0.37 0.48	0.02 0.02
(4) LGP2140X	B	From Leg	0.00 4.00 0.00 0.00	0.0000	110.00	No Ice 1/2" Ice	0.00 0.00	0.37 0.48	0.02 0.02
(4) LGP2140X	C	From Leg	0.00 4.00 0.00 0.00	0.0000	110.00	No Ice 1/2" Ice	0.00 0.00	0.37 0.48	0.02 0.02
BULLET III	A	From Leg	0.00 0.00 1.00	0.0000	60.00	No Ice 1/2" Ice	0.10 0.18	0.10 0.18	0.00 0.00
738-454	A	From Leg	0.00 0.00 1.00	0.0000	60.00	No Ice 1/2" Ice	0.02 0.05	0.02 0.05	0.00 0.00
BULLET III	A	From Leg	0.00 0.00 1.00	0.0000	41.00	No Ice 1/2" Ice	0.10 0.18	0.10 0.18	0.00 0.00

Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T1	120	Leg	A325N	0.6250	4	2.88	12.89	0.223 ✓	1.333	Bolt DS
T2	110	Leg	A325N	0.6250	5	10.25	12.89	0.795 ✓	1.333	Bolt DS
T4	70	Leg	A325N	1.0000	6	23.75	34.52	0.688 ✓	1.333	Bolt Tension
T5	50	Leg	A325N	1.0000	6	23.34	34.56	0.675 ✓	1.333	Bolt Tension
		Diagonal	A325N	1.0000	1	5.23	8.16	0.641 ✓	1.333	Member Bearing
T6	40	Leg	A325N	1.0000	6	25.84	34.56	0.748 ✓	1.333	Bolt Tension

RISATower GPD Associates 520 South Main St. Suite 2531 Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101	Job	Monotowese Amodio Self Store - BU# 876312	Page	3 of 3
	Project	2007287.88	Date	16:18:04 11/29/07
	Client	Crown Castle	Designed by	mimiller

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T7	20	Diagonal	A325N	1.0000	1	3.60	8.16	0.441 ✓	1.333	Member Bearing
		Leg	A354-BD	1.0000	6	27.59	38.88	0.710 ✓	1.333	Bolt Tension
		Diagonal	A325N	1.0000	1	5.45	8.16	0.668 ✓	1.333	Member Bearing

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail	
T1	120 - 110	Leg	1 1/2	3	10.29	35.15	29.3	Pass	
		Diagonal	5/8	14	-1.94	4.48	43.4	Pass	
		Horizontal	5/8	30	-0.15	1.86	8.3	Pass	
		Top Girt	3/4	5	-0.55	3.85	14.2	Pass	
		Bottom Girt	3/4	7	-0.70	3.85	18.1	Pass	
T2	110 - 90	Leg	1 3/4	39	46.01	58.59	78.5	Pass	
		Diagonal	3/4	50	-3.96	7.69	51.6	Pass	
		Horizontal	3/4	59	-0.73	3.16	23.1	Pass	
		Top Girt	7/8	41	-1.25	7.14	17.4	Pass	
		Bottom Girt	7/8	44	-2.15	5.51	39.1	Pass	
T3	90 - 70	Leg	2	101	-100.88	97.95	103.0	Pass	
		Diagonal	7/8	111	-4.91	11.74	41.8	Pass	
		Horizontal	7/8	158	-1.62	5.31	30.5	Pass	
		Top Girt	1	105	-1.80	9.34	19.2	Pass	
		Bottom Girt	1	107	-2.79	7.43	37.5	Pass	
T4	70 - 50	Leg	2 1/2	165	-156.16	164.50	94.9	Pass	
		Diagonal	7/8	224	-6.01	11.59	51.8	Pass	
		Horizontal	7/8	222	-1.75	4.26	41.1	Pass	
		Top Girt	1	169	-1.89	7.46	25.3	Pass	
		Bottom Girt	1	171	-1.73	6.07	28.6	Pass	
T5	50 - 40	Leg	Pirol 105245	229	-153.41	184.67	83.1	Pass	
		Diagonal	L2 1/2x2 1/2x3/16	236	-5.32	12.14	43.8	Pass	
T6	40 - 20	Leg	Pirol 105217	238	-174.97	184.67	94.7	Pass	
		Diagonal	L2 1/2x2 1/2x3/16	248	-4.05	10.46	38.7	Pass	
T7	20 - 0	Leg	Pirol 105217	253	-192.55	184.67	104.3	Pass	
		Diagonal	L2 1/2x2 1/2x3/16	257	-4.93	7.58	65.0	Pass	
							Summary		
							Leg (T7)	104.3	Pass
							Diagonal (T7)	65.0	Pass
							Horizontal (T4)	41.1	Pass
							Top Girt (T4)	25.3	Pass
							Bottom Girt (T2)	39.1	Pass
							Bolt Checks	59.7	Pass
							RATING=	104.3	Pass

APPENDIX B

Tower Elevation Drawing

Section	T7	T6	T5	T4	T3	T2	T1
Legs	Piled 105217		Piled 105245	SR 2 1/2	SR 2	SR 1 3/4	SR 1 1/2
Leg Grade				A572-50			
Diagonals	L2 1/2x2 1/2x3/16				SR 7/8	SR 3/4	SR 5/8
Diagonal Grade	A36						
Top Girts	N.A.				SR 1	SR 7/8	SR 3/4
Bottom Girts	N.A.				SR 1	SR 7/8	SR 3/4
Horizontals	N.A.				SR 7/8	SR 3/4	SR 5/8
Face Width (ft)	8	6	5	4.5	4		3.5
# Panels @ (ft)	5 @ 10				31 @ 2.33288		
Weight (K)	2.3	2.2	1.2	1.7	1.2	0.9	0.3

120.0 ft

110.0 ft

90.0 ft

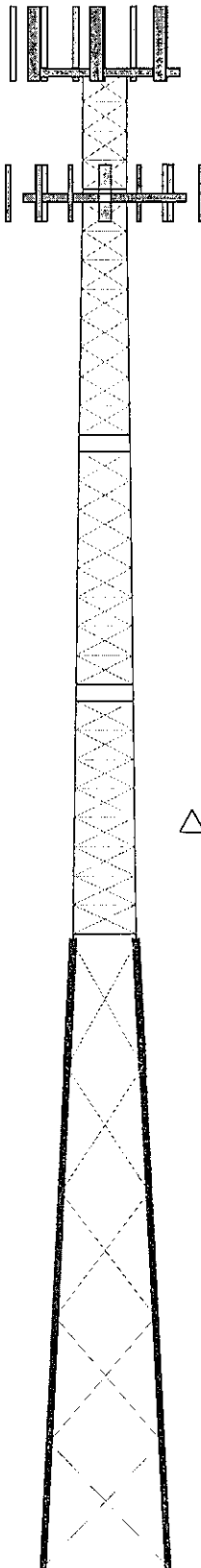
70.0 ft

50.0 ft

40.0 ft

20.0 ft

0.0 ft



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
PIROD 13' Low Profile Platform	120	(3) 7770.00	110
(3) FV65-14-00NA2	120	(3) 7770.00	110
(3) FV65-14-00NA2	120	(4) LGP2140X	110
(3) FV65-14-00NA2	120	(4) LGP2140X	110
PIROD 12' Lightweight T-Frame	110	(4) LGP2140X	110
PIROD 12' Lightweight T-Frame	110	BULLET III	60
PIROD 12' Lightweight T-Frame	110	738-454	60
(3) 7770.00	110	BULLET III	41

MATERIAL STRENGTH

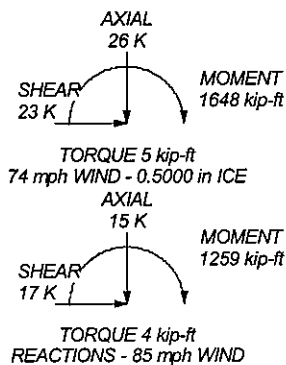
GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

TOWER DESIGN NOTES

1. Tower is located in New Haven County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 74 mph basic wind with 0.50 in ice.
4. Deflections are based upon a 60 mph wind.
5. TOWER RATING: 104.3%

MAX. CORNER REACTIONS AT BASE:

DOWN: 199 K
 UPLIFT: -172 K
 SHEAR: 16 K

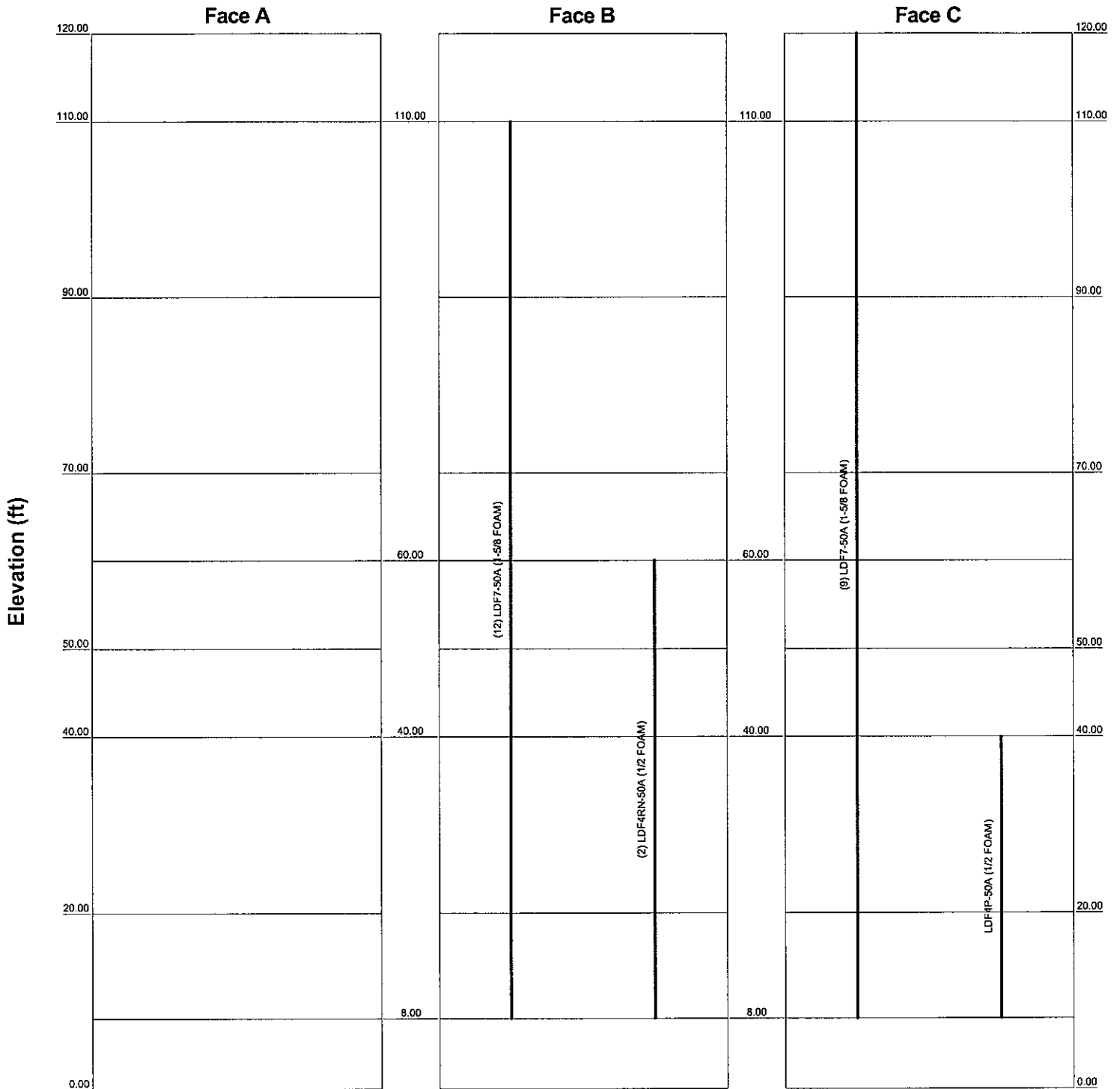


GPD Associates
 520 South Main St. Suite 2531
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 Phone: (330) 572-2100
 FAX: (330) 572-2101

Job: **Monotowese Amodio Self Store - BU# 876312**
 Project: **2007287.88**
 Client: **Crown Castle** Drawn by: **mimiller** App'd:
 Code: **TIA/EIA-222-F** Date: **11/30/07** Scale: **NTS**
 Path: **G:\Telecom\200728788\RISA Model\876312.dwg** Dwg No. **E-1**

Feedline Distribution Chart 0' - 120'

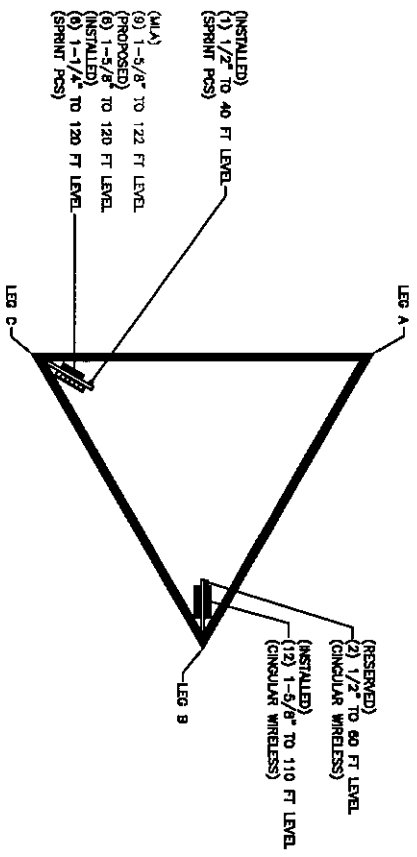
Round Flat App In Face App Out Face Truss Leg



GPD Associates		Job: Monotowese Amodio Self Store - BU# 876312	
520 South Main St. Suite 2531		Project: 2007287.88	
Akron, OH 44311		Client: Crown Castle	Drawn by: mimiller
Phone: (330) 572-2100		Code: TIA/EIA-222-F	Date: 11/30/07
FAX: (330) 572-2101		Path: G:\Telecom\2007287.88\TIA Model\876312.eri	Scale: NTS
			Dwg No. E-7

APPENDIX C

Base Level Drawing



BUSINESS UNIT: 876312 TOWER ID: C_BASELEVEL

- SOLID BLUE CIRCLE DENOTES EXISTING FEEDLINE
- OPEN RED CIRCLE DENOTES PROPOSED FEEDLINE
- OPEN BLUE CIRCLE DENOTES RESERVED FEEDLINE
- X BLUE "X" DENOTES LOCATION NOT GIVEN

NOTE: ASSUME FEEDLINE ATTACHMENT HEIGHT TO TOWER STEEL AT 8- FEET ABOVE FINISHED GRADE UNLESS OTHERWISE SPECIFIED

SCALE: 1
N.T.S.

APPENDIX D

Foundation Analysis



GPD GROUP

Engineers . Architects . Planners

Job 2057287.88

Calculated By MLM Date 11/29/07

Sheet No. 1 Of 1

Checked By _____ Date _____

UPLIFT

Ultimate Tension = 40k/pile

$$\frac{40k}{2} = 20k \quad 20k \cdot 6 \text{ piles} = 120k$$

$$W_c = 9.5' \times 7' \times 4' \times 1.5 \text{ kcf}$$

$$W_c = 39.9k$$

$$W_s = 0.47 \times 43' \times 60$$

$$W_s = 10.8k$$

$$\uparrow \text{Allow} = 10.8k + 39.9k + 20k$$

$$\uparrow \text{Allow} = 170.7k$$

COMPRESSION

Ultimate Compression = 95k

$$\frac{95}{2} = 47.5k/pile$$

$$47.5k/pile \times 6 \text{ piles} = 285k$$

$$\downarrow \text{Allow} = 285k$$

[illegible]