# 1280 Route 46 West, Suite 9, Parsippany NJ, 07054 

Melanie Bachman
Executive Director
CT Siting Council
10 Franklin Square
New Britain, CT 06051
Re: Notice of Exempt Modification Application
395 Round Hill Road, Greenwich, CT 06831
Latitude: N41.095117
Longitude: W73.6642
Dear Ms. Bachman:
Sprint currently maintains 3 existing panel antennas at the $100^{\prime}$ ' centerline level of the existing $115^{\prime}$ flagpole. Sprint proposes to swap 3 panel antennas at the $100^{\prime}$ centerline on the tower. Sprint further proposes to add 3 remote radio heads at the base of the pole. Sprint is performing a new high-performance upgrade for cellular mobile communications. It is designed to increase the capacity and speed of mobile telephone networks.

Please accept this letter as notification to the Council, pursuant to R.C.S.A. Section 16-50j-73, for construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2). In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter is being sent to First Selectman Peter Tesei of the Town of Greenwich as well as Katie DeLuca, Director of Planning for the Town of Greenwich and Round Hill Community Church, owner of the property

Attached is a summary of the planned modifications, including power density calculations reflecting the change in Sprint's operations at the site. Also included is documentation of the structural sufficiency of the tower with proposed modifications to accommodate the revised antenna configuration as well as the latest CSC decision, tax sheet and tax map.

## Existing Facility

CSC Summary Statement - CT43XC856-395 Round Hill Rd, Greenwich CT 06831

The Communications Tower facility is located at 395 Round Hill Rd, Greenwich CT 06831 and is owned by the Round Hill Community Church, the Site coordinates are: N41.095117 W73.6642.

The existing facility consists of a 115 ' Flagpole. Sprint currently operates wireless communications equipment on a platform on a concrete slab at the facility and has 3 antennas mounted at centerline of $100^{\prime}$.

The planned modifications to the facility fall within the activities explicitly provided for in R.C.S.A. $16-50 \mathrm{j}$ 72(b)(2)

1. The height of the overall structure will be unaffected.
2. The proposed changes will not require an extension of the property boundaries.
3. The proposed additions will not increase the noise level at the existing facility by six decibels or more, or to levels that exceed state and/or local criteria
4. The changes will not increase the calculated "worst case" power density for the combined operations at the site to a level at or above the Federal Communications Commission safety standard.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, Sprint respectfully submits that the proposed changes at the referenced site constitute exempt modifications under R.C.S.A Section §16-50j-72(b)(2).

Respectfully submitted,

ryan@mackenzierealtyconsulting.com

Additional Recipients:
First Selectman Peter Tesei for the Town of Greenwich- Via FedEx
Katie DeLuca, Director of Planning for the Town of Greenwich - Via FedEx
Round Hill Community Church, owner of the property


Sprint


Cherundolo Consulting


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## FIBER PLUMBING

 DIAGRAM
## C-5




(1) TYPICAL POWER \& GROUNDING ONE-LINE DIAGRAM

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6. RUN TLLCO CoNOUT OR CABEE BETENT TEEPHONE UTUTI




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16. Bono Antenn Mounte grakis, Mrerfer cable ground
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## Cherundolo <br> Consulting



GROUNDING DETAILS

E-1


## Structural Analysis Report

Prepared for:
KGI
805 Las Cimas Parkway
Building Three, Suite 370
Austin, TX 78746

ATTN: Mr. Sean Rock


Prepared By:
Jung Hyun Hong


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## Introduction

The purpose of this report is to summarize results of a structural analysis performed on the 114 ft monopole to reflect the change in loading by Sprint.

## Supporting Documents

| Tower Drawings | EEI Drawing \#GS56652-2, dated September 28, 2007 |
| :--- | :--- |
| Foundation Drawing | EEI Drawing \#14679S-115.0, dated February 12, 2007 |
| Geotechnical Report | Clarence Welti Associate, dated Feruary 6, 2007 |

## Analysis

The tower was analyzed using American Tower Corporation's tower analysis software. This program considers an elastic three-dimensional model and second-order effects per ANSI/TIA-222.

| Basic Wind Speed: | 93 mph (3-Second Gust) Vasd/120 mph (3-Second Gust) Vult |
| :--- | :--- |
| Basic Wind Speed w/ Ice: | 50 mph (3-Second Gust) w/3/4" radial ice concurrent |
| Code: | ANSI/TIA-222-G/2015 IBC/2018 Connecticut State Building Code |
| Structure Class: | II |
| Exposure Category: | D (Hurricane Zone) |
| Topographic Category: | 1 |
| Crest Height: | 0 ft |
| Spectral Response: | SS $=0.26, \mathrm{~S}_{1}=0.07$ |
| Site Class: | D-StiffSoil |

## Conclusion

Based on the analysis results, the structure meets the requirements per the applicable codes listed above. The tower and foundation can support the equipment as described in this report.

The pier reinforcement is less than the minimum allowance. Due to this, the pier should be frequently monitored for cracking/spalling.

If you have any questions or require additional information, please contact Semaan Engineering Solutions at 402-289-1888.

## Existing and Reserved Equipment

This loading is included in the analysis.

| Centerline <br> Elevation(ft) |  | Oty. | Antenna | Mount Type | Coax (in) | Carrier |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mount | Equip. |  |  |  |  |  |
| 110.0 | 110.0 | 3 | DBXNH-6565A-A2M | Flush Mount Inside Canister | (12) $15 / 8^{\prime \prime}$ | T-Mobile |
|  |  | 3 | TMAT1921XB6811A |  |  |  |
|  |  | 3 | 78211066 |  |  |  |
| 100.0 | - | - | - | Flush Mount Inside Canister | (6) $15 / 8{ }^{\prime \prime}$ | Sprint |
| 90.0 | - | - | - | Empty Flush Mount Inside Canister | - | - |

## Equipment to be Removed

This loading is not included in the analysis.

| Centerline <br> Elevation(ft) |  | Qty. | Antenna | Mount Type | Coax (in) | Carrier |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mount | Equip. |  |  |  |  |  |
| 100.0 | 100.0 | 3 | RR65-18-00DPL2 | - | - | Sprint |

## Proposed Equipment

This loading is included in the analysis.

| Centerline <br> Elevation(ft) |  | Qty. | Antenna | Mount Type | Coax (in) | Carrier |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mount | Equip. |  |  |  |  |  |
| 100.0 | 100.0 | 3 | APXVSPP18-C-A20 | Existing Flush Mount Inside Canister | (2) $11 / 4^{\prime \prime}$ | Sprint |
|  |  | 6 | KIT-FD9R6004/1C-DL |  |  |  |
|  |  | 9 | IBC1900HG-SA |  |  |  |

Install proposed coax inside the pole shaft.

## Structure Usages

| Structural Component | Controlling Usage | Pass/Fail |
| :---: | :---: | :---: |
| Shaft | $31 \%$ | Pass |
| Base Plate | $31 \%$ | Pass |
| Anchor Bolts | $51 \%$ | Pass |
| Flange Bolts | $9 \%$ | Pass |

## Foundations

| Reaction Component | Analysis Reactions | \% of Usage |
| :---: | :---: | :---: |
| Moment (Kips-Ft) | 504.0 | $46 \%$ |
| Axial (Kips) | 27.0 | $18 \%$ |
| Shear (Kips) | 8.9 | $19 \%$ |
| Reinf. Conc. Foundation Capacity | N/A | $22 \%$ |

The structure base reactions resulting from this analysis were found to be acceptable through analysis based on geotechnical and foundation information, therefore no modification or reinforcement of the foundation will be required.

## Deflection and Sway*

| Antenna <br> Elevation (ft) | Antenna | Carrier | Deflection <br> $(\mathrm{ft})$ | Sway (Rotation) <br> $\left({ }^{\circ}\right)$ |
| :---: | :---: | :---: | :---: | :---: |
| 100.0 | APXVSPP18-C-A20 |  | 0.350 | 0.752 |
|  | KIT-FD9R6004/1C-DL | Sprint |  |  |
|  | IBC1900HG-SA |  |  |  |

[^0]
## Standard Conditions

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessary limited, to:
-- Information supplied by the client regarding the structure itself, antenna, mounts and feed line loading on the structure and its components, or other relevant information.
-- Information from drawings in the possession of Semaan Engineering Solutions, or generated by field inspections or measurements of the structure.

It is the responsibility of the client to ensure that the information provided to Semaan Engineering Solutions Holdings and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and that their capacity has not significantly changed from the "as new" condition.

Unless explicitly agreed by both the client and Semaan Engineering Solutions, all services will be performed in accordance with the current revision of ANSI/TIA -222. The design basic wind speed will be determined based on the minimum basic wind speed as prescribed in ANSI/TIA-222. Although every effort is taken to ensure that the loading considered is adequate to meet the requirements of all applicable regulatory entities, we can provide no assurance to meet any other local and state codes or requirements. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement.

All services are performed, results obtained, and recommendations made in accordance with generally accepted engineering principles and practices. Semaan Engineering Solutions Holdings is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.


| Sections Properties |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Shaft Section | Length <br> (ft) | Diame <br> Accro Top | eter (in) oss Flats Bottom | Thick <br> (in) | Joint <br> Type | Overlap Length (in) | Taper (in/ft) | Steel Grade (ksi) |
| 1 | 52.658 | 33.39 | 41.00 | 0.250 |  | 0.000 | 0.144341 | 65 |
| 2 | 36.158 | 29.25 | 34.46 | 0.188 | Slip Joint | 57.781 | 0.144341 | 65 |
| 3 | 10.000 | 5.750 | 5.750 | 2.875 | Butt Joint | 0.000 | 0.000000 | - 50 |
| 4 | 10.000 | 4.500 | 4.500 | 2.250 | Butt Joint | 0.000 | 0.000000 | - 50 |
| 5 | 10.000 | 4.500 | 4.500 | 0.674 | Butt Joint | 0.000 | 0.000000 | 35 |


|  | Discrete Appurtenance |  |  |
| :--- | :--- | :--- | :--- |
| Attach <br> Eev (ft) | Force <br> Eev (ft) | Qty | Description |
| 110.000 | 110.000 | 3 | 782 11066 |
| 110.000 | 110.000 | 3 | TMAT1921XB6811A |
| 110.000 | 110.000 | 3 | DBXNH-6565A-A2M |
| 110.000 | 110.000 | 1 | Flush Mount |
| 100.000 | 100.000 | 9 | IBC1900HG-SA |
| 100.000 | 100.000 | 6 | KT-FD9R6004/1C-DL |
| 100.000 | 100.000 | 3 | APXVSPP18-C-A20 |
| 100.000 | 100.000 | 1 | Flush Mount |
| 90.000 | 90.000 | 1 | Flush Mount |
| 81.500 | 83.000 | 1 | GPS |
| 81.500 | 81.500 | 1 | 3 ft Standoff |


| Linear Appurtenance |  |  |  |
| :--- | :--- | :--- | :--- |
| Elev (ft) |  |  | Exposed |
| From | To | Description | To Wind |
| 104.0 | 114.0 | Concealment | Yes |
| 94.000 | 104.0 | Concealment | Yes |
| 84.000 | 94.000 | Concealment | Yes |
| 0.000 | 100.0 | .32" | No |
| 0.000 | 100.0 | $11 / 4^{\prime \prime}$ Coax | No |
| 0.000 | 100.0 | $15 / 8^{\prime \prime}$ Coax | No |
| 0.000 | 110.0 | $.32^{\prime \prime}$ | No |
| 0.000 | 110.0 | $15 / 8^{\prime \prime}$ Coax | No |
| 0.000 | 81.500 | $1 / 2^{\prime \prime}$ Coax | No |


| Load Cases |  |
| :---: | :---: |
| 1.2D + 1.6W | 93 mph w ith No Ice |
| 0.9D + 1.6W | 93 mph with No Ice (Reduced DL) |
| $1.2 \mathrm{D}+1.0 \mathrm{Di}+1.0 \mathrm{Wi}$ | 50 mph w ith 0.75 in Radial lce |
| $(1.2+0.2 S d s) * D L+E$ | Seismic Equivalent Lateral Forces Method |
| $(1.2+0.2 \mathrm{Sds}) * \mathrm{DL}+\mathrm{E}$ | Seismic Equivalent Modal Analysis Method |
| (0.9-0.2Sds) * DL + E | Seismic (Reduced DL) Equivalent Lateral |
| (0.9-0.2Sds) * DL + E | Seismic (Reduced DL) Equivalent Modal |
| 1.0D + 1.0W | Serviceability 60 mph |




| Site Number: | $27741 \_B$ | Code: ANSI/TIA-222-G | © 2007-2019 by ATCIP LLC. All rights reserved. |
| :--- | :--- | ---: | ---: | ---: |
| Site Name: | Round Hill CT, Greenwich, CT | Engineering Number: REV01 | 9/16/2019 9:52:08 AM |
| Customer: | KG |  |  |

## Analysis Parameters

| Location: | Greenwich County, CT |  |  |
| :--- | :--- | :--- | ---: |
| Code: | ANSI/TIA-222-G | Helght (ft): | 114 |
| Shape: | 18 Sides. Sect 3: Round Solid. Sect 4: Round Solid. Sect 5: Round | 41.00 |  |
| Pole Type: | Custom | Top Diameter (in): | 4.50 |
| Pole Manfacturer: | 田 | Taper (in/ft): | 0.144 |

## Ice \& Wind Parameters

| Structure Class: | II | Design Wind Speed Without Ice: | $\mathbf{9 3 ~ m p h}$ |
| :--- | :--- | :--- | :--- |
| Exposure Catagory: | D | Design Wind Speed With Ice: | $\mathbf{5 0 ~ m p h}$ |
| Topographic Catagory: | 1 | Operational Wind Speed: | 60 mph |
| Crest Height: | 0.0 ft | Design Ice Thickness: | 0.75 in |

## Seismic Parameters

Analys is Method: Equivalent Modal Analysis \& Equivalent Lateral Force Methods
Site Class: D-Stlff Soil
Period Based on Rayleigh Method (sec): 1.53

| $\mathrm{T}_{\mathrm{L}}(\mathrm{sec})$ : | 6 | p: | 1.3 | $C_{5}$ : | 0.050 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{S}_{\mathbf{s}}$ : | 0.259 | $S_{1}$ : | 0.071 | $C_{8}$ Max: | 0.050 |
| $F_{a}$ : | 1.593 | $F_{\mathbf{v}}$ : | 2.400 | $C_{\text {s }}$ Min: | 0.030 |
| $S_{\text {ds }}$ : | 0.275 | $S_{\text {d1 }}$ : | 0.114 |  |  |

## Load Cases

$1.2 D+1.6 W$
$0.9 D+1.6 W$
$1.2 D+1.0 D i+1.0 W I$
$(1.2+0.2 S d s)$ * $D L+E E . F M$
$(1.2+0.2 S d s)$ DL. + EEMAM
$(0.9-0.2 S d s)$ * DL + EELFM
$(0.9-0.2 S d s)$ *DL + EEMAM
$1.0 D+1.0 W$

93 mph with No Ice
93 mph with No Ice (Reduced DL)
50 mph with 0.75 in Radial Ice
Seismic Equivalent Lateral Forces Method
Seismic Equivalent Modal Analysis Method
Seismic (Reduced DL) Equivalent Lateral Forces Method
Seismic (Reduced DL) Equivalent Modal Analys is Method
Serviceability 60 mph


## Discrete Appurtenance Properties

| Attach <br> Elev <br> (ft) | Description | Qty | Weight (ib) | $\begin{aligned} & \text { No ice } \\ & \text { EPAa } \\ & \text { (sf) } \end{aligned}$ | Orientation Factor | Weight (ib) | $\begin{gathered} \text { _ Ice } \\ \text { EPAa } \\ \text { (sf) } \\ \hline \end{gathered}$ | Orientation Factor | Distance From Face <br> (ft) | Vert Ecc <br> (ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 110.00 | 78211066 | 3 | 1.76 | 0.000 | 1.00 | 5.33 | 0.000 | 1.00 | 0.000 | 0.000 |
| 110.00 | DBXNH-6565A-A2M | 3 | 34.20 | 0.000 | 1.00 | 153.17 | 0.000 | 1.00 | 0.000 | 0.000 |
| 110.00 | Fush Mount | 1 | 120.00 | 0.000 | 1.00 | 282.20 | 0.000 | 1.00 | 0.000 | 0.000 |
| 110.00 | TMAT1921XB6811A | 3 | 17.60 | 0.000 | 1.00 | 35.49 | 0.000 | 1.00 | 0.000 | 0.000 |
| 100.00 | APXVSPP18-C-A20 | 3 | 57.00 | 0.000 | 1.00 | 171.46 | 0.000 | 1.00 | 0.000 | 0.000 |
| 100.00 | Fush Mount | 1 | 120.00 | 0.000 | 1.00 | 280.64 | 0.000 | 1.00 | 0.000 | 0.000 |
| 100.00 | IBC1900HG-SA | 9 | 22.00 | 0.000 | 1.00 | 66.18 | 0.000 | 1.00 | 0.000 | 0.000 |
| 100.00 | KIT-FD9R6004/1C-DL | 6 | 6.40 | 0.000 | 1.00 | 17.57 | 0.000 | 1.00 | 0.000 | 0.000 |
| 90.00 | Fush Mount | 1 | 120.00 | 0.000 | 1.00 | 278.93 | 0.000 | 1.00 | 0.000 | 0.000 |
| 81.50 | 3 ft Standoff | 1 | 40.00 | 2.630 | 1.00 | 115.55 | 8.247 | 1.00 | 0.000 | 0.000 |
| 81.50 | GPS | 1 | 10.00 | 0.070 | 1.00 | 13.74 | 0.200 | 1.00 | 0.000 | 1.500 |
|  | Totals | 32 | 978.08 | 2,768.38 |  |  |  | Number of Loadings : |  | 11 |

## Linear Appurtenance Properties

| Elev From (ft) | Elev To (ft) | Qty | Description | Coax Diameter (in) | Coax Weight ( 1 b /ft) | Flat | Protected Width (in) | Exposed <br> To Wind | Carrier |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 104.00 | 114.00 | 1 | Concealment (0.5) | 30.00 | 16.95 | N | 12.75 | Y |  |
| 0.00 | 110.00 | 1 | .32" | 0.32 | 0.06 | $N$ | 0.00 | N | T-Mobile |
| 0.00 | 110.00 | 12 | 15/8" Coax | 1.98 | 1.04 | N | 0.00 | N | T-Mobile |
| 94.00 | 104.00 | 1 | Concealment (0.5) | 30.00 | 16.95 | N | 12.75 | Y |  |
| 0.00 | 100.00 | 1 | .32" | 0.32 | 0.06 | N | 0.00 | N | Sprint |
| 0.00 | 100.00 | 2 | 1 1/4" Coax | 1.55 | 0.66 | N | 0.00 | N | Sprint |
| 0.00 | 100.00 | 6 | 15/8" Coax | 1.98 | 1.04 | N | 0.00 | N | Sprint |
| 84.00 | 94.00 | 1 | Concealment (0.5) | 30.00 | 16.95 | N | 12.12 | Y |  |
| 0.00 | 81.50 | 1 | 1/2" Coax | 0.65 | 0.16 | $N$ | 0.00 | N | Sprint |

Site Number: 27741_B
Site Name: Round Hill CT, Greenwich, CT
Customer: KG

Segment Properties (Max Len : 5.ft)

| Seg T Elev (ft) | Description | Thick <br> (in) | Flat Dia <br> (in) | Area $\left(\mathrm{in}^{2}\right)$ | $\underset{\left(\text { in }^{4}\right)}{ }$ | Wht Ratio | D/t Ratio | $\underset{(\mathbf{k s i})}{\text { Fy }}$ | $\underset{\left(\mathrm{in}^{3}\right)}{\mathbf{S}}$ | $\underset{\left(i^{3}\right)}{\mathbf{Z}}$ | Weight <br> (lb) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0.00 |  | 0.2500 | 41.000 | 32.334 | 6,783.7 | 27.51 | 164.00 | 69.0 | 325.9 | 0.0 | 0.0 |
| 5.00 |  | 0.2500 | 40.278 | 31.761 | 6,429.6 | 27.00 | 161.11 | 69.6 | 314.4 | 0.0 | 545.3 |
| 10.00 |  | 0.2500 | 39.557 | 31.189 | 6.088 .1 | 26.49 | 158.23 | 70.2 | 303.1 | 0.0 | 535.5 |
| 15.00 |  | 0.2500 | 38.835 | 30.616 | 5,758.9 | 25.98 | 155.34 | 70.8 | 292.1 | 0.0 | 525.8 |
| 20.00 |  | 0.2500 | 38.113 | 30.043 | 5,441.7 | 25.47 | 152.45 | 71.4 | 281.2 | 0.0 | 516.0 |
| 25.00 |  | 0.2500 | 37.391 | 29.471 | 5,136.4 | 24.96 | 149.57 | 72.0 | 270.6 | 0.0 | 506.3 |
| 30.00 |  | 0.2500 | 36.670 | 28.898 | 4,842.8 | 24.45 | 146.68 | 72.6 | 260.1 | 0.0 | 496.5 |
| 35.00 |  | 0.2500 | 35.948 | 28.325 | 4,560.6 | 23.94 | 143.79 | 73.2 | 249.9 | 0.0 | 486.8 |
| 40.00 |  | 0.2500 | 35.226 | 27.753 | 4,289.5 | 23.43 | 140.91 | 73.8 | 239.8 | 0.0 | 477.1 |
| 45.00 |  | 0.2500 | 34.505 | 27.180 | 4,029.4 | 22.93 | 138.02 | 74.4 | 230.0 | 0.0 | 467.3 |
| 47.84 | Bot-Section 2 | 0.2500 | 34.094 | 26.855 | 3,886.4 | 22.64 | 136.38 | 74.8 | 224.5 | 0.0 | 261.3 |
| 50.00 |  | 0.2500 | 33.783 | 26.607 | 3,780.1 | 22.42 | 135.13 | 75.0 | 220.4 | 0.0 | 345.3 |
| 52.66 | Top - Section 1 | 0.1875 | 33.774 | 19.988 | 2,848.7 | 30.35 | 180.13 | 65.7 | 166.1 | 0.0 | 421.0 |
| 55.00 |  | 0.1875 | 33.436 | 19.786 | 2,763.6 | 30.03 | 178.33 | 66.1 | 162.8 | 0.0 | 158.5 |
| 60.00 |  | 0.1875 | 32.715 | 19.357 | 2,587.5 | 29.35 | 174.48 | 66.9 | 155.8 | 0.0 | 333.0 |
| 65.00 |  | 0.1875 | 31.993 | 18.927 | 2,419.1 | 28.68 | 170.63 | 67.7 | 148.9 | 0.0 | 325.7 |
| 70.00 |  | 0.1875 | 31.271 | 18.498 | 2,258.1 | 28.00 | 166.78 | 68.5 | 142.2 | 0.0 | 318.4 |
| 75.00 |  | 0.1875 | 30.549 | 18.069 | 2,104.4 | 27.32 | 162.93 | 69.3 | 135.7 | 0.0 | 311.1 |
| 80.00 |  | 0.1875 | 29.828 | 17.639 | 1,957.9 | 26.64 | 159.08 | 70.1 | 129.3 | 0.0 | 303.8 |
| 81.50 |  | 0.1875 | 29.611 | 17.510 | 1,915.3 | 26.44 | 157.93 | 70.3 | 127.4 | 0.0 | 89.7 |
| 84.00 | Top - Section 2 | 0.1875 | 29.250 | 17.295 | 1,845.7 | 26.10 | 156.00 | 70.7 | 124.3 | 0.0 | 148.0 |
| 84.00 | Bot-Section 3 | 2.8750 | 5.750 | 25.967 | 53.7 | 0.00 | 2.00 | 50.0 | 18.7 | 31.7 |  |
| 85.00 |  | 2.8750 | 5.750 | 25.967 | 53.7 | 0.00 | 2.00 | 50.0 | 18.7 | 31.7 | 88.4 |
| 90.00 |  | 2.8750 | 5.750 | 25.967 | 53.7 | 0.00 | 2.00 | 50.0 | 18.7 | 31.7 | 441.8 |
| 94.00 | Top - Section 3 | 2.8750 | 5.750 | 25.967 | 53.7 | 0.00 | 2.00 | 50.0 | 18.7 | 31.7 | 353.4 |
| 94.00 | Bot - Section 4 | 2.2500 | 4.500 | 15.904 | 20.1 | 0.00 | 2.00 | 50.0 | 8.9 | 15.2 |  |
| 95.00 |  | 2.2500 | 4.500 | 15.904 | 20.1 | 0.00 | 2.00 | 50.0 | 8.9 | 15.2 | 54.1 |
| 100.0 |  | 2.2500 | 4.500 | 15.904 | 20.1 | 0.00 | 2.00 | 50.0 | 8.9 | 15.2 | 270.6 |
| 104.0 | Top - Section 4 | 2.2500 | 4.500 | 15.904 | 20.1 | 0.00 | 2.00 | 50.0 | 8.9 | 15.2 | 216.5 |
| 104.0 | Bot - Section 5 | 0.6740 | 4.500 | 8.101 | 14.8 | 0.00 | 6.68 | 35.0 | 6.6 | 10.0 |  |
| 105.0 |  | 0.6740 | 4.500 | 8.101 | 14.8 | 0.00 | 6.68 | 35.0 | 6.6 | 10.0 | 27.6 |
| 110.0 |  | 0.6740 | 4.500 | 8.101 | 14.8 | 0.00 | 6.68 | 35.0 | 6.6 | 10.0 | 137.8 |
| 114.0 |  | 0.6740 | 4.500 | 8.101 | 14.8 | 0.00 | 6.68 | 35.0 | 6.6 | 10.0 | 110.3 |
| $9,272.8$ |  |  |  |  |  |  |  |  |  |  |  |

Site Number: 27741_B
Site Name: Round Hill CT, Greenwich, CT
Engineering Number: REV01
9/16/2019 9:52:09 AM
Customer: KG

Load Case: 1.2D+1.6W
93 mph with No Ice
25 Iterations
Gust Response Factor : 1.10
Wind Importance Factor : 1.00
Dead Load Factor : 1.20
Wind Load Factor : 1.60

## Applied Segment Forces Summary

|  |  | Shaft Forces |  | Discrete Forces |  |  |  | Linear Forces |  | Sum of Forces |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Seg Eev (ft) | Description | Wind FX <br> (lb) | Dead <br> Load <br> (lb) | Wind FX <br> (b) | Torsion MY (lb-ft) | $\begin{aligned} & \text { Moment } \\ & \text { MZ } \\ & \text { (lb-ft) } \end{aligned}$ | Dead <br> Load <br> (lb) | Wind FX <br> (b) | Dead <br> Load <br> (lb) | Wind FX <br> (Ib) | Dead Load (b) | $\begin{gathered} \text { Torsion } \\ \text { MY } \\ \text { (lb-ft) } \\ \hline \end{gathered}$ | Moment MZ <br> ( l ) |
| 0.00 |  | 213.1 | 0.0 |  |  |  |  | 0.0 | 0.0 | 213.1 | 0.0 | 0.0 | 0.0 |
| 5.00 |  | 422.4 | 654.3 |  |  |  |  | 0.0 | 121.9 | 422.4 | 776.2 | 0.0 | 0.0 |
| 10.00 |  | 414.8 | 642.6 |  |  |  |  | 0.0 | 121.9 | 414.8 | 764.5 | 0.0 | 0.0 |
| 15.00 |  | 414.8 | 630.9 |  |  |  |  | 0.0 | 121.9 | 414.8 | 752.8 | 0.0 | 0.0 |
| 20.00 |  | 423.3 | 619.2 |  |  |  |  | 0.0 | 121.9 | 423.3 | 741.2 | 0.0 | 0.0 |
| 25.00 |  | 431.2 | 607.5 |  |  |  |  | 0.0 | 121.9 | 431.2 | 729.5 | 0.0 | 0.0 |
| 30.00 |  | 436.1 | 595.8 |  |  |  |  | 0.0 | 121.9 | 436.1 | 717.8 | 0.0 | 0.0 |
| 35.00 |  | 438.9 | 584.2 |  |  |  |  | 0.0 | 121.9 | 438.9 | 706.1 | 0.0 | 0.0 |
| 40.00 |  | 439.9 | 572.5 |  |  |  |  | 0.0 | 121.9 | 439.9 | 694.4 | 0.0 | 0.0 |
| 45.00 |  | 345.0 | 560.8 |  |  |  |  | 0.0 | 121.9 | 345.0 | 682.7 | 0.0 | 0.0 |
| 47.84 | Bot - Section 2 | 220.6 | 313.6 |  |  |  |  | 0.0 | 69.3 | 220.6 | 382.9 | 0.0 | 0.0 |
| $50.00$ |  | $213.4$ | $414.4$ |  |  |  |  | 0.0 | 52.6 | 213.4 | 467.0 | 0.0 | 0.0 |
| 52.66 | Top-Section 1 | 221.1 | $505.2$ |  |  |  |  | 0.0 | 64.8 | 221.1 | 570.0 | 0.0 | 0.0 |
| 55.00 |  | 323.2 | 190.2 |  |  |  |  | 0.0 | 57.1 | 323.2 | 247.3 | 0.0 | 0.0 |
| 60.00 |  | 437.9 | 399.6 |  |  |  |  | 0.0 | 121.9 | 437.9 | 521.5 | 0.0 | 0.0 |
| 65.00 |  | 434.1 | 390.8 |  |  |  |  | 0.0 | 121.9 | 434.1 | 512.7 | 0.0 | 0.0 |
| 70.00 |  | 429.8 | $382.1$ |  |  |  |  | 0.0 | 121.9 | 429.8 | 504.0 | 0.0 | 0.0 |
| 75.00 |  | 424.8 | $373.3$ |  |  |  |  | 0.0 | 121.9 | 424.8 | 495.2 | 0.0 | 0.0 |
| 80.00 |  | 273.9 | $364.5$ |  |  |  |  | 0.0 | 121.9 | 273.9 | 486.4 | 0.0 | 0.0 |
| 81.50 | Appertunance(s) | 166.9 | 107.6 | 138.5 | 0.0 | 5.4 | 60.0 | 0.0 | 36.6 | 305.4 | 204.2 | 0.0 | 0.0 |
| 84.00 | Top - Section 2 | 118.9 | 177.7 |  |  |  |  | 0.0 | 60.5 | 118.9 | 238.1 | 0.0 | 0.0 |
| 85.00 |  | 89.5 | 106.0 |  |  |  |  | 31.3 | 44.5 | 120.8 | 150.6 | 0.0 | 0.0 |
| 90.00 | Appertunance(s) | 134.9 | 530.2 | 0.0 | 0.0 | 0.0 | 144.0 | 157.4 | 222.7 | 292.2 | 896.8 | 0.0 | 0.0 |
| 94.00 | Top - Section 3 | 72.1 | 424.1 |  |  |  |  | 127.0 | 178.1 | 199.1 | 602.3 | 0.0 | 0.0 |
| 95.00 |  | 71.4 | 64.9 |  |  |  |  | 33.5 | 44.5 | 104.9 | 109.5 | 0.0 | 0.0 |
| 100.00 | Appertunance(s) | 107.5 | 324.7 | 0.0 | 0.0 | 0.0 | 632.9 | 168.6 | 222.7 | 276.2 | 1,180.3 | 0.0 | 0.0 |
| 104.00 | Top - Section 4 | 60.0 | $259.8$ |  |  |  |  | 136.0 | 141.6 | 196.0 | 401.3 | 0.0 | 0.0 |
| $105.00$ |  | 72.6 109.3 | 33.1 165.4 |  |  |  |  | 34.1 171.5 | 35.4 | 106.7 | 68.5 | 0.0 | 0.0 |
| $\begin{aligned} & 110.00 \\ & 114.00 \end{aligned}$ | Appertunance(s) | 109.3 48.8 | $165.4$ | 0.0 | 0.0 | 0.0 | 336.8 | $171.5$ $1389$ | $176.9$ | $280.8$ | $679.2$ | 0.0 | 0.0 |
| 114.00 |  | 48.8 | 132.3 |  |  |  |  | $138.2$ |  | 186.9 | 213.7 | 0.0 | 0.0 |
|  |  |  |  |  |  |  |  | Totals: |  | 9,146.23 | 15,496.6 | 0.00 | 0.00 |

Site Number: 27741_B
Site Name: Round Hill CT, Greenwich, CT Engineering Number: REV01 9/16/2019 9:52:09 AM
Customer: KG

| Load Case: $1.2 \mathrm{D}+1.6 \mathrm{~W}$ | 93 mph with No lce | 25 Iterations |
| :--- | :---: | :---: |
| Gust Response Factor : 1.10 |  | Wind Importance Factor : 1.00 |
| Dead Load Factor: 1.20 |  |  |
| Wind Load Factor : 1.60 |  |  |

## Calculated Forces

| Seg Elev <br> (ft) | $\begin{gathered} \mathrm{Pu} \\ \mathrm{FY}(-) \\ \text { (kips) } \end{gathered}$ | Vu FX (-) (kips) | $\begin{gathered} \text { Tu } \\ \text { MY } \\ \text { (ft-kips) } \end{gathered}$ | $\begin{gathered} \text { Mu } \\ \text { MZ } \\ \text { (ft-kips) } \end{gathered}$ | Mu MX (ft-kips) | Resultant Moment (ft-kips) | phi <br> (kips) | phi <br> (kips) |  | phi Mn (ft-kips) | Total Deflect (in) | Rotation (deg) | Ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0.00 | -15.49 | -8.95 | 0.00 | -503.96 | 0.00 | 503.96 | 2,009.33 | 1,004.66 | 3,370.2 | 1,687.62 | 0.00 | 0.00 | 0.306 |
| 5.00 | -14.70 | -8.55 | 0.00 | -459.22 | 0.00 | 459.22 | 1,990.85 | 995.43 | 3,279.75 | 1,642.31 | 0.05 | -0.10 | 0.287 |
| 10.00 | -13.92 | -8.16 | 0.00 | -416.48 | 0.00 | 416.48 | 1,971.76 | 985.88 | 3,189.37 | 1,597.06 | 0.21 | -0.20 | 0.268 |
| 15.00 | -13.16 | -7.76 | 0.00 | -375.70 | 0.00 | 375.70 | 1,952.06 | 976.03 | 3,099.16 | 1,551.88 | 0.47 | -0.29 | 0.249 |
| 20.00 | -12.41 | -7.35 | 0.00 | -336.91 | 0.00 | 336.91 | 1,931.73 | 965.87 | 3,009.15 | 1,506.81 | 0.82 | -0.38 | 0.230 |
| 25.00 | -11.67 | -6.93 | 0.00 | -300.16 | 0.00 | 300.16 | 1,910.79 | 955.40 | 2,919.43 | 1,461.88 | 1.26 | -0.46 | 0.211 |
| 30.00 | -10.95 | -6.50 | 0.00 | -265.52 | 0.00 | 265.52 | 1,889.23 | 944.62 | 2,830.03 | 1,417.12 | 1.79 | -0.54 | 0.193 |
| 35.00 | -10.24 | -6.07 | 0.00 | -233.01 | 0.00 | 233.01 | 1,867.06 | 933.53 | 2,741.01 | 1,372.54 | 2.40 | -0.61 | 0.175 |
| 40.00 | -9.54 | -5.63 | 0.00 | -202.68 | 0.00 | 202.68 | 1,844.27 | 922.13 | 2,652.43 | 1,328.19 | 3.08 | -0.68 | 0.158 |
| 45.00 | -8.86 | -5.28 | 0.00 | -174.52 | 0.00 | 174.52 | 1.820 .86 | 910.43 | 2,564.34 | 1,284.08 | 3.82 | -0.74 | 0.141 |
| 47.84 | -8.48 | -5.06 | 0.00 | -159.50 | 0.00 | 159.50 | 1,807.27 | 903.64 | 2,514.51 | $1,259.12$ | 4.28 | -0.78 | 0.131 |
| 50.00 | -8.01 | -4.85 | 0.00 | -148.58 | 0.00 | 148.58 | 1,796.83 | 898.42 | 2,476.80 | 1,240.24 | 4.63 | -0.80 | 0.124 |
| 52.66 | -7.44 | -4.62 | 0.00 | -135.69 | 0.00 | 135.69 | 1,181.92 | 590.96 | 1,634.85 | 818.64 | 5.09 | -0.83 | 0.172 |
| 55.00 | -7.20 | -4.30 | 0.00 | -124.87 | 0.00 | 124.87 | 1,176.68 | 588.34 | 1,611.12 | 806.76 | 5.50 | -0.85 | 0.161 |
| 60.00 | -6.68 | -3.86 | 0.00 | -103.37 | 0.00 | 103.37 | 1,165.04 | 582.52 | 1,560.37 | 781.35 | 6.43 | -0.91 | 0.138 |
| 65.00 | -6.17 | -3.42 | 0.00 | -84.07 | 0.00 | 84.07 | 1,152.79 | 576.40 | 1,509.51 | 755.88 | 7.41 | -0.96 | 0.117 |
| 70.00 | -5.67 | -2.99 | 0.00 | -66.95 | 0.00 | 66.95 | 1.139.92 | 569.96 | 1,458.59 | 730.38 | 8.45 | -1.01 | 0.097 |
| 75.00 | -5.18 | -2.56 | 0.00 | -52.01 | 0.00 | 52.01 | 1,126.44 | 563.22 | 1,407.67 | 704.88 | 9.52 | -1.05 | 0.078 |
| 80.00 | -4.70 | -2.28 | 0.00 | -39.22 | 0.00 | 39.22 | 1,112.33 | 556.17 | 1,356.80 | 679.41 | 10.64 | -1.08 | 0.062 |
| 81.50 | -4.50 | -1.97 | 0.00 | -35.80 | 0.00 | 35.80 | 1,107.98 | 553.99 | 1,341.56 | 671.78 | 10.98 | -1.08 | 0.057 |
| 84.00 | -4.27 | -1.85 | 0.00 | -30.88 | 0.00 | 30.88 | 1,100.61 | 550.30 | 1,316.18 | 659.07 | 11.55 | -1.10 | 0.051 |
| 84.00 | -4.27 | -1.85 | 0.00 | -30.88 | 0.00 | 30.88 | 1,168.53 | 584.26 | 139.98 | 118.82 | 11.55 | -1.10 | 0.264 |
| 85.00 | -4.11 | -1.75 | 0.00 | -29.03 | 0.00 | 29.03 | 1,168.53 | 584.26 | 139.98 | 118.82 | 11.78 | -1.10 | 0.248 |
| 90.00 | -3.21 | -1.46 | 0.00 | -20.29 | 0.00 | 20.29 | 1,168.53 | 584.26 | 139.98 | 118.82 | 13.30 | -1.76 | 0.174 |
| 94.00 | -2.61 | -1.25 | 0.00 | -14.45 | 0.00 | 14.45 | 1,168.53 | 584.26 | 139.98 | 118.82 | 14.93 | -2.12 | 0.124 |
| 94.00 | -2.61 | -1.25 | 0.00 | -14.45 | 0.00 | 14.45 | 715.69 | 357.85 | 67.10 | 56.95 | 14.93 | -2.12 | 0.257 |
| 95.00 | -2.49 | -1.16 | 0.00 | -13.20 | 0.00 | 13.20 | 715.69 | 357.85 | 67.10 | 56.95 | 15.38 | -2.20 | 0.235 |
| 100.00 | -1.32 | -0.84 | 0.00 | -7.40 | 0.00 | 7.40 | 715.69 | 357.85 | 67.10 | 56.95 | 18.10 | -2.93 | 0.132 |
| 104.00 | -0.93 | -0.63 | 0.00 | -4.03 | 0.00 | 4.03 | 715.69 | 357.85 | 67.10 | 56.95 | 20.70 | -3.25 | 0.072 |
| 104.00 | -0.93 | -0.63 | 0.00 | -4.03 | 0.00 | 4.03 | 255.19 | 127.60 | 34.57 | 26.17 | 20.70 | -3.25 | 0.158 |
| 105.00 | -0.86 | -0.52 | 0.00 | -3.40 | 0.00 | 3.40 | 255.19 | 127.60 | 34.57 | 26.17 | 21.38 | -3.30 | 0.133 |
| 110.00 | -0.20 | -0.20 | 0.00 | -0.80 | 0.00 | 0.80 | 256.19 | 127.60 | 34.57 | 26.17 | 24.97 | -3.50 | 0.031 |
| 114.00 | 0.00 | -0.19 | 0.00 | 0.00 | 0.00 | 0.00 | 255.19 | 127.60 | 34.57 | 26.17 | 27.91 | -3.53 | 0.000 |



Applied Segment Forces Summary

|  |  | Shaft Forces |  | Discrete Forces |  |  |  | Linear Forces |  | Sum of Forces |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Seg Dev <br> (ft) | Description | Wind FX <br> (Ib) | Dead Load <br> (b) | Wind FX <br> (Ib) | Torsion MY (lb-ft) | $\begin{aligned} & \text { Moment } \\ & \text { MZ } \\ & \text { (lb-ft) } \end{aligned}$ | Dead <br> Load <br> (lb) | Wind FX <br> (lb) | Dead <br> Load <br> (lb) | Wind FX <br> (lb) | Dead <br> Load <br> (lb) | Torsion MY (lb-ft) | Moment MZ <br> (b) |
| 0.00 |  | 213.1 | 0.0 |  |  |  |  | 0.0 | 0.0 | 213.1 | 0.0 | 0.0 | 0.0 |
| 5.00 |  | 422.4 | 490.7 |  |  |  |  | 0.0 | 91.4 | 422.4 | 582.2 | 0.0 | 0.0 |
| 10.00 |  | 414.8 | 482.0 |  |  |  |  | 0.0 | 91.4 | 414.8 | 573.4 | 0.0 | 0.0 |
| 15.00 |  | 414.8 | 473.2 |  |  |  |  | 0.0 | 91.4 | 414.8 | 564.6 | 0.0 | 0.0 |
| 20.00 |  | 423.3 | 464.4 |  |  |  |  | 0.0 | 91.4 | 423.3 | 555.9 | 0.0 | 0.0 |
| 25.00 |  | 431.2 | 455.7 |  |  |  |  | 0.0 | 91.4 | 431.2 | 547.1 | 0.0 | 0.0 |
| 30.00 |  | 436.1 | 446.9 |  |  |  |  | 0.0 | 91.4 | 436.1 | 538.3 | 0.0 | 0.0 |
| 35.00 |  | 438.9 | 438.1 |  |  |  |  | 0.0 | 91.4 | 438.9 | 529.6 | 0.0 | 0.0 |
| 40.00 |  | 439.9 | 429.3 |  |  |  |  | 0.0 | 91.4 | 439.9 | 520.8 | 0.0 | 0.0 |
| 45.00 |  | 345.0 | 420.6 |  |  |  |  | 0.0 | 91.4 | 345.0 | 512.0 | 0.0 | 0.0 |
| 47.84 | Bot - Section 2 | 220.6 | 235.2 |  |  |  |  | 0.0 | 52.0 | 220.6 | 287.2 | 0.0 | 0.0 |
| 50.00 |  | 213.4 | 310.8 |  |  |  |  | 0.0 | 39.5 | 213.4 | 350.3 | 0.0 | 0.0 |
| 52.66 | Top - Section 1 | 221.1 | 378.9 |  |  |  |  | 0.0 | 48.6 | 221.1 | 427.5 | 0.0 | 0.0 |
| 55.00 |  | 323.2 | 142.7 |  |  |  |  | 0.0 | 42.8 | 323.2 | 185.5 | 0.0 | 0.0 |
| 60.00 |  | 437.9 | 299.7 |  |  |  |  | 0.0 | 91.4 | 437.9 | 391.1 | 0.0 | 0.0 |
| 65.00 |  | 434.1 | 293.1 |  |  |  |  | 0.0 | 91.4 | 434.1 | 384.6 | 0.0 | 0.0 |
| 70.00 |  | 429.8 | 286.5 |  |  |  |  | 0.0 | 91.4 | 429.8 | 378.0 | 0.0 | 0.0 |
| 75.00 |  | 424.8 | 280.0 |  |  |  |  | 0.0 | 91.4 | 424.8 | 371.4 | 0.0 | 0.0 |
| 80.00 |  | 273.9 | 273.4 |  |  |  |  | 0.0 | 91.4 | 273.9 | 364.8 | 0.0 | 0.0 |
| 81.50 | Appertunance(s) | 166.9 | 80.7 | 138.5 | 0.0 | 5.4 | 45.0 | 0.0 | 27.4 | 305.4 | 153.2 | 0.0 | 0.0 |
| 84.00 | Top - Section 2 | 118.9 | 133.2 |  |  |  |  | 0.0 | 45.4 | 118.9 | 178.6 | 0.0 | 0.0 |
| 85.00 |  | 89.5 | 79.5 |  |  |  |  | 31.3 | 33.4 | 120.8 | 112.9 | 0.0 | 0.0 |
| 90.00 | Appertunance(s) | 134.9 | 397.6 | 0.0 | 0.0 | 0.0 | 108.0 | 157.4 | 167.0 | 292.2 | 672.6 | 0.0 | 0.0 |
| 94.00 | Top - Section 3 | 72.1 | 318.1 |  |  |  |  | 127.0 | 133.6 | 199.1 | 451.7 | 0.0 | 0.0 |
| 95.00 |  | 71.4 | 48.7 |  |  |  |  | 33.5 | 33.4 | 104.9 | 82.1 | 0.0 | 0.0 |
| 100.00 | Appertunance(s) | 107.5 | 243.5 | 0.0 | 0.0 | 0.0 | 474.7 | 168.6 | 167.0 | 276.2 | 885.2 | 0.0 | 0.0 |
| 104.00 | Top - Section 4 | 60.0 | 194.8 |  |  |  |  | 136.0 | 106.2 | 196.0 | 301.0 | 0.0 | 0.0 |
| 105.00 |  | 72.6 | 24.8 |  |  |  |  | 34.1 | 26.5 | 106.7 | 51.4 | 0.0 | 0.0 |
| 110.00 | Appertunance(s) | 109.3 | 124.1 | 0.0 | 0.0 | 0.0 | 252.6 | $171.5$ | $132.7$ | $280.8$ | $509.4$ | 0.0 | 0.0 |
| 114.00 |  | 48.8 | 99.2 |  |  |  |  | $138.2$ | $61.0$ | $186.9$ | $160.3$ | 0.0 | 0.0 |
|  |  |  |  |  |  |  |  | Totals: |  | $9,146.23$ | $11,622.4$ | 0.00 | 0.00 |

Page: 6

| Site Number: 27741_B <br> Site Name: Round Hill CT, Greenwich, CT <br> Customer: KG |  | Code: ANSITTIA-222-G | © 2007-2019 by ATC IP LLC. All |
| :---: | :---: | :---: | :---: |
|  |  | Engineering Number: REV01 | 9/16/2019 9:52:10 AM |
|  |  |  |  |
| Load Case: $0.9 \mathrm{D}+1.6 \mathrm{~W}$ <br> Gust Response Factor: 1.10 <br> Dead Load Factor : 0.90 <br> Wind Load Factor : 1.60 |  | 93 mph with No Ice (Reduced DL) | 25 Iterations |
|  |  |  | Wind Importance Factor : 1.00 |
|  |  |  |  |

## Calculated Forces

| Seg Elev <br> (ft) | $\begin{gathered} \mathrm{Pu} \\ \mathrm{FY}(-) \\ \text { (kips) } \end{gathered}$ | Vu FX (-) (kips) | $\begin{gathered} \text { Tu } \\ \text { MY } \\ \text { (ft-kips) } \end{gathered}$ | $\begin{gathered} \begin{array}{c} \text { Mu } \\ \text { MZ } \\ \text { (ft-kips) } \end{array} \end{gathered}$ |  | Resultant Moment (ft-kips) | phi <br> (kips) | phi <br> (kips) |  | phi Mn (ft-kips) | Total Deflect (in) | Rotation (deg) | Ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0.00 | -11.61 | -8.94 | 0.00 | -501.34 | 0.00 | 501.34 | 2,009.33 | 1,004.66 | 3,370.24 | 1,687.62 | 0.00 | 0.00 | 0.303 |
| 5.00 | -11.02 | -8.54 | 0.00 | -456.62 | 0.00 | 456.62 | 1,990.85 | 995.43 | 3,279.75 | 1,642.31 | 0.05 | -0.10 | 0.284 |
| 10.00 | -10.43 | -8.14 | 0.00 | -413.92 | 0.00 | 413.92 | 1,971.76 | 985.88 | 3,189.37 | 1,597.06 | 0.21 | -0.20 | 0.265 |
| 15.00 | -9.86 | -7.74 | 0.00 | -373.22 | 0.00 | 373.22 | 1,952.06 | 976.03 | 3,099.16 | 1,551.88 | 0.47 | -0.29 | 0.246 |
| 20.00 | -9.29 | -7.33 | 0.00 | -334.53 | 0.00 | 334.53 | 1,931.73 | 965.87 | 3,009.15 | 1,506.81 | 0.82 | -0.38 | 0.227 |
| 25.00 | -8.74 | -6.90 | 0.00 | -297.90 | 0.00 | 297.90 | 1,910.79 | 955.40 | 2,919.43 | 1,461.88 | 1.26 | -0.46 | 0.208 |
| 30.00 | -8.20 | -6.47 | 0.00 | -263.39 | 0.00 | 263.39 | 1,889.23 | 944.62 | 2,830.03 | 1,417.12 | 1.78 | -0.54 | 0.190 |
| 35.00 | -7.66 | -6.04 | 0.00 | -231.02 | 0.00 | 231.02 | 1,867.06 | 933.53 | 2,741.01 | 1,372.54 | 2.38 | -0.61 | 0.172 |
| 40.00 | -7.14 | -5.60 | 0.00 | -200.83 | 0.00 | 200.83 | 1,844.27 | 922.13 | 2,652.43 | 1,328.19 | 3.06 | -0.68 | 0.155 |
| 45.00 | -6.63 | -5.25 | 0.00 | -172.83 | 0.00 | 172.83 | 1.820 .86 | 910.43 | 2,564.34 | 1,284.08 | 3.80 | -0.74 | 0.138 |
| 47.84 | -6.34 | -5.03 | 0.00 | -157.90 | 0.00 | 157.90 | 1,807.27 | 903.64 | 2,514.51 | 1,259.12 | 4.25 | -0.77 | 0.129 |
| 50.00 | -5.99 | -4.82 | 0.00 | -147.04 | 0.00 | 147.04 | 1,796.83 | 898.42 | 2,476.80 | 1,240.24 | 4.60 | -0.80 | 0.122 |
| 52.66 | -5.57 | -4.59 | 0.00 | -134.23 | 0.00 | 134.23 | 1,181.92 | 590.96 | 1,634.85 | 818.64 | 5.05 | -0.82 | 0.169 |
| 55.00 | -5.38 | -4.27 | 0.00 | -123.48 | 0.00 | 123.48 | 1,176.68 | 588.34 | 1,611.12 | 806.76 | 5.47 | -0.85 | 0.158 |
| 60.00 | -5.00 | -3.83 | 0.00 | -102.12 | 0.00 | 102.12 | 1,165.04 | 582.52 | 1,560.37 | 781.35 | 6.38 | -0.91 | 0.135 |
| 65.00 | -4.62 | -3.40 | 0.00 | -82.96 | 0.00 | 82.96 | 1,152.79 | 576.40 | 1,509.51 | 755.88 | 7.36 | -0.96 | 0.114 |
| 70.00 | -4.24 | -2.96 | 0.00 | -65.98 | 0.00 | 65.98 | 1,139.92 | 569.96 | 1.458.59 | 730.38 | 8.39 | -1.00 | 0.094 |
| 75.00 | -3.88 | -2.53 | 0.00 | -51.17 | 0.00 | 51.17 | 1,126.44 | 563.22 | 1,407.67 | 704.88 | 9.45 | -1.04 | 0.076 |
| 80.00 | -3.52 | -2.25 | 0.00 | -38.50 | 0.00 | 38.50 | 1,112.33 | 556.17 | 1,356.80 | 679.41 | 10.56 | -1.07 | 0.060 |
| 81.50 | -3.37 | -1.95 | 0.00 | -35.12 | 0.00 | 35.12 | 1,107.98 | 553.99 | 1,341.56 | 671.78 | 10.89 | -1.08 | 0.055 |
| 84.00 | -3.19 | -1.82 | 0.00 | -30.25 | 0.00 | 30.25 | 1,100.61 | 550.30 | 1,316.18 | 659.07 | 11.46 | -1.09 | 0.049 |
| 84.00 | -3.19 | -1.82 | 0.00 | -30.25 | 0.00 | 30.25 | 1,168.53 | 584.26 | 139.98 | 118.82 | 11.46 | -1.09 | 0.257 |
| 85.00 | -3.07 | -1.72 | 0.00 | -28.43 | 0.00 | 28.43 | 1,168.53 | 584.26 | 139.98 | 118.82 | 11.69 | -1.09 | 0.242 |
| 90.00 | -2.39 | -1.43 | 0.00 | -19.83 | 0.00 | 19.83 | 1,168.53 | 584.26 | 139.98 | 118.82 | 13.19 | -1.73 | 0.169 |
| 94.00 | -1.95 | -1.22 | 0.00 | -14.10 | 0.00 | 14.10 | 1,168.53 | 584.26 | 139.98 | 118.82 | 14.80 | -2.09 | 0.120 |
| 94.00 | -1.95 | -1.22 | 0.00 | -14.10 | 0.00 | 14.10 | 715.69 | 357.85 | 67.10 | 56.95 | 14.80 | -2.09 | 0.250 |
| 95.00 | -1.86 | -1.13 | 0.00 | -12.88 | 0.00 | 12.88 | 715.69 | 357.85 | 67.10 | 56.95 | 15.24 | -2.16 | 0.229 |
| 100.00 | -0.98 | -0.82 | 0.00 | -7.22 | 0.00 | 7.22 | 715.69 | 357.85 | 67.10 | 56.95 | 17.91 | -2.87 | 0.128 |
| 104.00 | -0.69 | -0.61 | 0.00 | -3.93 | 0.00 | 3.93 | 715.69 | 357.85 | 67.10 | 56.95 | 20.46 | -3.19 | 0.070 |
| 104.00 | -0.69 | -0.61 | 0.00 | -3.93 | 0.00 | 3.93 | 255.19 | 127.60 | 34.57 | 26.17 | 20.46 | -3.19 | 0.153 |
| 105.00 | -0.64 | -0.51 | 0.00 | -3.32 | 0.00 | 3.32 | 255.19 | 127.60 | 34.57 | 26.17 | 21.14 | -3.24 | 0.129 |
| 110.00 | -0.15 | -0.20 | 0.00 | -0.78 | 0.00 | 0.78 | 255.19 | 127.60 | 34.57 | 26.17 | 24.65 | -3.44 | 0.031 |
| 114.00 | 0.00 | -0.19 | 0.00 | 0.00 | 0.00 | 0.00 | 255.19 | 127.60 | 34.57 | 26.17 | 27.55 | -3.47 | 0.000 |


| Site Number: $27741 \_B$ $\quad$ Code: ANSIITIA-222-G <br> Site Name: Round Hill CT, Greenwich, CT Engineering Number: REV01 <br> Customer: KG:  | © 2007-2019 by ATC IP LLC. All rights reserved. 9/16/2019 9:52:10 AM |
| :---: | :---: |
| Load Case: $1.2 \mathrm{D}+1.0 \mathrm{Di}+1.0 \mathrm{Wi}$ | 24 Iterations |
| Gust Response Factor: $1.10 \quad$ Ice Dead Load Factor : 1.00 Dead Load Factor: 1.20 Wind Load Factor : 1.00 | Wind Importance Factor: 1.00 Ice Importance Factor : 1.00 |

## Applied Segment Forces Summary

|  |  | Shaft Forces |  | Discrete Forces |  |  |  | Linear Forces |  | Sum of Forces |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Seg ■ev (ft) | Description | Wind FX <br> (Ib) | Dead Load (b) | Wind FX <br> (Ib) | Torsion MY (lb-ft) | $\begin{aligned} & \text { Moment } \\ & \text { MZ } \\ & \text { (lb-ft) } \end{aligned}$ | Dead Load <br> (lb) | Wind FX <br> (b) | Dead Load <br> (lb) | Wind FX <br> (Ib) | Dead Load (lb) | $\begin{gathered} \text { Torsion } \\ \text { MY } \\ \text { (lb-ft) } \end{gathered}$ | Moment MZ <br> (lb) |
| 0.00 |  | 75.2 | 0.0 |  |  |  |  | 0.0 | 0.0 | 75.2 | 0.0 | 0.0 | 0.0 |
| 5.00 |  | 149.5 | 965.7 |  |  |  |  | 0.0 | 121.9 | 149.5 | 1,087.6 | 0.0 | 0.0 |
| 10.00 |  | 147.6 | 977.9 |  |  |  |  | 0.0 | 121.9 | 147.6 | 1,099.8 | 0.0 | 0.0 |
| 15.00 |  | 148.1 | 976.4 |  |  |  |  | 0.0 | 121.9 | 148.1 | 1,098.3 | 0.0 | 0.0 |
| 20.00 |  | 151.7 | 969.7 |  |  |  |  | 0.0 | 121.9 | 151.7 | 1,091.6 | 0.0 | 0.0 |
| 25.00 |  | 154.9 | 960.3 |  |  |  |  | 0.0 | 121.9 | 154.9 | 1,082.2 | 0.0 | 0.0 |
| 30.00 |  | 157.1 | 949.0 |  |  |  |  | 0.0 | 121.9 | 157.1 | 1,070.9 | 0.0 | 0.0 |
| 35.00 |  | 158.5 | 936.5 |  |  |  |  | 0.0 | 121.9 | 158.5 | 1,058.4 | 0.0 | 0.0 |
| 40.00 |  | 159.3 | 923.1 |  |  |  |  | 0.0 | 121.9 | 159.3 | 1,045.0 | 0.0 | 0.0 |
| 45.00 |  | 125.2 | 908.9 |  |  |  |  | 0.0 | 121.9 | 125.2 | 1,030.8 | 0.0 | 0.0 |
| 47.84 | Bot - Section 2 | 80.2 | 511.0 |  |  |  |  | 0.0 | 69.3 | 80.2 | 580.3 | 0.0 | 0.0 |
| 50.00 |  | 77.6 | 565.3 |  |  |  |  | 0.0 | 52.6 | 77.6 | 617.9 | 0.0 | 0.0 |
| 52.66 | Top-Section 1 | 80.5 | 690.0 |  |  |  |  | 0.0 | 64.8 | 80.5 | 754.8 | 0.0 | 0.0 |
| 55.00 |  | 117.9 | 352.3 |  |  |  |  | 0.0 | 57.1 | 117.9 | 409.5 | 0.0 | 0.0 |
| 60.00 |  | 160.1 | 740.8 |  |  |  |  | 0.0 | 121.9 | 160.1 | 862.7 | 0.0 | 0.0 |
| 65.00 |  | 159.1 | 727.7 |  |  |  |  | 0.0 | 121.9 | 159.1 | 849.6 | 0.0 | 0.0 |
| 70.00 |  | 158.0 | 714.3 |  |  |  |  | 0.0 | 121.9 | 158.0 | 836.3 | 0.0 | 0.0 |
| 75.00 |  | 156.6 | 700.7 |  |  |  |  | 0.0 | 121.9 | 156.6 | 822.6 | 0.0 | 0.0 |
| 80.00 |  | 101.2 | 686.8 |  |  |  |  | 0.0 | 121.9 | 101.2 | 808.7 | 0.0 | 0.0 |
| 81.50 | Appertunance(s) | 61.8 | 204.1 | 78.3 | 0.0 | 2.8 | 126.3 | 0.0 | 36.6 | 140.1 | 366.9 | 0.0 | 0.0 |
| 84.00 | Top - Section 2 | 42.8 | 336.9 |  |  |  |  | 0.0 | 60.5 | 42.8 | 397.4 | 0.0 | 0.0 |
| 85.00 |  | 25.5 | 120.9 |  |  |  |  | 14.4 | 161.5 | 39.9 | 282.4 | 0.0 | 0.0 |
| 90.00 | Appertunance(s) | 38.4 | 605.0 | 0.0 | 0.0 | 0.0 | 422.9 | 72.4 | 808.1 | 110.8 | 1,836.0 | 0.0 | 0.0 |
| 94.00 | Top - Section 3 | 20.9 | 484.4 |  |  |  |  | 58.5 | 647.4 | 79.4 | 1,131.8 | 0.0 | 0.0 |
| 95.00 |  | 22.5 | 77.5 |  |  |  |  | 15.3 | 162.0 | 37.8 | 239.5 | 0.0 | 0.0 |
| 100.00 | Appertunance(s) | 33.9 | 387.8 | 0.0 | 0.0 | 0.0 | 1,563.7 | 76.9 | 810.5 | 110.8 | 2,762.0 | 0.0 | 0.0 |
| 104.00 | Top - Section 4 | 19.0 | 310.5 |  |  |  |  | 62.1 | 612.7 | 81.0 | 923.2 | 0.0 | 0.0 |
| 105.00 |  | 23.0 | 45.8 |  |  |  |  | 15.6 | 153.3 | 38.5 | 199.1 | 0.0 | 0.0 |
| $110.00$ | Appertunance(s) | $34.6$ | $229.3$ | 0.0 | 0.0 | 0.0 | 921.9 | 78.4 | $767.1$ | $113.0$ | 1,918.2 | 0.0 | 0.0 |
| 114.00 |  | 15.5 | 183.7 |  |  |  |  | 63.2 | 554.2 | 78.7 | $737.9$ | 0.0 | 0.0 |
|  |  |  |  |  |  |  |  | Totals: |  | 3,390.81 | 27,001.4 | 0.00 | 0.00 |

Page: 8

Site Number: 27741_B
Site Name: Round Hill CT, Greenwich, CT
Engineering Number: REV01
9/16/2019 9:52:10 AM
Customer: KGI

| Load Case: $1.2 \mathrm{D}+1.0 \mathrm{Di}+1.0 \mathrm{Wi}$ | 50 mph with 0.75 in Radial Ice | 24 Iterations |
| :--- | ---: | ---: |
| Gust Response Factor: 1.10 | Ice Dead Load Factor : 1.00 | Wind importance Factor: 1.00 |
| Dead Load Factor: 1.20 | Ice Importance Factor : 1.00 |  |
| Wind Load Factor: 1.00 |  |  |

## Calculated Forces

| Seg Elev (ft) | $\begin{gathered} \mathrm{Pu} \\ \mathrm{FY}(-) \\ \text { (kips) } \\ \hline \end{gathered}$ | Vu FX (-) (kips) | $\begin{gathered} \text { Tu } \\ \text { MY } \\ \text { (ft-kips) } \end{gathered}$ | $\begin{gathered} M \mathbf{M u} \\ \text { MZ } \\ \text { (ft-kips) } \end{gathered}$ | $\begin{gathered} \text { Mu } \\ \text { MX } \\ \text { (ft-kips) } \end{gathered}$ | Resultant Moment (ft-kips) | phi Pn (kips) | phi Vn (kips) | $\begin{gathered} \text { phi } \\ \text { In } \\ \text { (ft-kips) } \end{gathered}$ | $\begin{gathered} \text { phi } \\ \text { Mn } \\ \text { (ft-kips) } \end{gathered}$ | Total Deflect (in) | Rotation (deg) | Ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0.00 | -27.00 | -3.32 | 0.00 | -196.12 | 0.00 | 196.12 | 2,009.33 | 1,004.66 | 3,370.24 | 1,687.62 | 0.00 | 0.00 | 0.130 |
| 5.00 | -25.91 | -3.19 | 0.00 | -179.50 | 0.00 | 179.50 | 1,990.85 | 995.43 | 3,279.75 | 1,642.31 | 0.02 | -0.04 | 0.122 |
| 10.00 | -24.81 | -3.06 | 0.00 | -163.54 | 0.00 | 163.54 | 1,971.76 | 985.88 | 3.189.37 | 1,597.06 | 0.08 | -0.08 | 0.115 |
| 15.00 | -23.71 | -2.92 | 0.00 | -148.24 | 0.00 | 148.24 | 1,952.06 | 976.03 | 3,099.16 | 1,551.88 | 0.18 | -0.11 | 0.108 |
| 20.00 | -22.62 | -2.78 | 0.00 | -133.61 | 0.00 | 133.61 | 1,931.73 | 965.87 | 3,009.15 | 1,506.81 | 0.32 | -0.15 | 0.100 |
| 25.00 | -21.53 | -2.64 | 0.00 | -119.70 | 0.00 | 119.70 | 1,910.79 | 955.40 | 2,919.43 | 1,461.88 | 0.50 | -0.18 | 0.093 |
| 30.00 | -20.46 | -2.49 | 0.00 | -106.51 | 0.00 | 106.51 | 1,889.23 | 944.62 | 2,830.03 | 1,417.12 | 0.70 | -0.21 | 0.086 |
| 35.00 | -19.40 | -2.33 | 0.00 | -94.07 | 0.00 | 94.07 | 1,867.06 | 933.53 | 2,741.01 | 1,372.54 | 0.94 | -0.24 | 0.079 |
| 40.00 | -18.36 | -2.18 | 0.00 | -82.39 | 0.00 | 82.39 | 1,844.27 | 922.13 | 2,652.43 | 1,328.19 | 1.21 | -0.27 | 0.072 |
| 45.00 | -17.33 | -2.05 | 0.00 | -71.50 | 0.00 | 71.50 | 1,820.86 | 910.43 | 2,564.34 | 1.284.08 | 1.51 | -0.30 | 0.065 |
| 47.84 | -16.75 | -1.98 | 0.00 | -65.66 | 0.00 | 65.66 | 1,807.27 | 903.64 | 2,514.51 | ,259.12 | 1.69 | -0.31 | 0.061 |
| 50.00 | -16.13 | -1.90 | 0.00 | -61.40 | 0.00 | 61.40 | 1,796.83 | 898.42 | 2,476.80 | 1,240.24 | 1.83 | -0.32 | 0.058 |
| 52.66 | -15.37 | -1.82 | 0.00 | -56.35 | 0.00 | 56.35 | 1,181.92 | 590.96 | 1,634.85 | 818.64 | 2.01 | -0.33 | 0.082 |
| 55.00 | -14.96 | -1.70 | 0.00 | -52.10 | 0.00 | 52.10 | 1,176.68 | 588.34 | 1,611.12 | 806.76 | 2.18 | -0.34 | 0.077 |
| 60.00 | -14.10 | -1.54 | 0.00 | -43.60 | 0.00 | 43.60 | 1,165.04 | 582.52 | 1,560.37 | 781.35 | 2.55 | -0.37 | 0.068 |
| 65.00 | -13.25 | -1.38 | 0.00 | -35.90 | 0.00 | 35.90 | 1,152.79 | 576.40 | 1,509.51 | 755.88 | 2.94 | -0.39 | 0.059 |
| 70.00 | -12.42 | -1.22 | 0.00 | -29.00 | 0.00 | 29.00 | 1,139.92 | 569.96 | 1,458.59 | 730.38 | 3.36 | -0.41 | 0.051 |
| 75.00 | -11.60 | -1.06 | 0.00 | -22.90 | 0.00 | 22.90 | 1,126.44 | 563.22 | 1,407.67 | 704.88 | 3.79 | -0.42 | 0.043 |
| 80.00 | -10.79 | -0.95 | 0.00 | -17.60 | 0.00 | 17.60 | 1,112.33 | 556.17 | 1,356.80 | 679.41 | 4.24 | -0.44 | 0.036 |
| 81.50 | -10.42 | -0.81 | 0.00 | -16.17 | 0.00 | 16.17 | 1,107.98 | 553.99 | 1,341.56 | 671.78 | 4.38 | -0.44 | 0.033 |
| 84.00 | -10.02 | -0.77 | 0.00 | -14.13 | 0.00 | 14.13 | 1,100.61 | 550.30 | 1,316.18 | 659.07 | 4.61 | -0.45 | 0.031 |
| 84.00 | -10.02 | -0.77 | 0.00 | -14.13 | 0.00 | 14.13 | 1,168.53 | 584.26 | 139.98 | 118.82 | 4.61 | -0.45 | 0.128 |
| 85.00 | -9.74 | -0.75 | 0.00 | -13.37 | 0.00 | 13.37 | 1.168.53 | 584.26 | 139.98 | 118.82 | 4.71 | -0.45 | 0.121 |
| 90.00 | -7.90 | -0.66 | 0.00 | -9.60 | 0.00 | 9.60 | 1,168.53 | 584.26 | 139.98 | 118.82 | 5.34 | -0.75 | 0.088 |
| 94.00 | -6.77 | -0.57 | 0.00 | -6.98 | 0.00 | 6.98 | 1,168.53 | 584.26 | 139.98 | 118.82 | 6.05 | -0.93 | 0.065 |
| 94.00 | -6.77 | -0.57 | 0.00 | -6.98 | 0.00 | 6.98 | 715.69 | 357.85 | 67.10 | 56.95 | 6.05 | -0.93 | 0.132 |
| 95.00 | -6.53 | -0.55 | 0.00 | -6.41 | 0.00 | 6.41 | 715.69 | 357.85 | 67.10 | 56.95 | 6.25 | -0.96 | 0.122 |
| 100.00 | -3.77 | -0.40 | 0.00 | -3.64 | 0.00 | 3.64 | 715.69 | 357.85 | 67.10 | 56.95 | 7.46 | -1.32 | 0.069 |
| 104.00 | -2.85 | -0.30 | 0.00 | -2.02 | 0.00 | 2.02 | 715.69 | 357.85 | 67.10 | 56.95 | 8.64 | -1.48 | 0.039 |
| 104.00 | -2.85 | -0.30 | 0.00 | -2.02 | 0.00 | 2.02 | 255.19 | 127.60 | 34.57 | 26.17 | 8.64 | -1.48 | 0.088 |
| 105.00 | -2.65 | -0.26 | 0.00 | -1.72 | 0.00 | 1.72 | 255.19 | 127.60 | 34.57 | 26.17 | 8.95 | -1.50 | 0.076 |
| 110.00 | -0.74 | -0.10 | 0.00 | -0.40 | 0.00 | 0.40 | 255.19 | 127.60 | 34.57 | 26.17 | 10.59 | -1.61 | 0.018 |
| 114.00 | 0.00 | -0.08 | 0.00 | 0.00 | 0.00 | 0.00 | 255.19 | 127.60 | 34.57 | 26.17 | 11.95 | -1.62 | 0.000 |

Site Number: 27741_B
Code: ANSI/TIA-222-G
© 2007-2019 by ATC IP LLC. All rights reserved.
Site Name: Round Hill CT, Greenwich, CT
Engineering Number: REV01
9/16/2019 9:52:10 AM
Customer: KG

| Load Case: $1.0 \mathrm{D}+1.0 \mathrm{~W}$ | Serviceability 60 mph | 23 Iterations |
| :--- | :--- | :--- |
| Gust Response Factor: 1.10 | Wind Importance Factor: 1.00 |  |
| Dead Load Factor: 1.00 |  |  |
| Wind Load Factor: 1.00 |  |  |

Applied Segment Forces Summary

|  |  | Shaft Forces |  | Discrete Forces |  |  |  | Linear Forces |  | Sum of Forces |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Seg日ev (ft) | Description | Wind FX <br> (lb) | Dead Load (lb) | Wind FX <br> (b) | $\begin{aligned} & \text { Torsion } \\ & \text { MY } \\ & \text { (lb-ft) } \end{aligned}$ | $\begin{aligned} & \text { Moment } \\ & \text { MZ } \\ & \text { (lb-ft) } \end{aligned}$ | Dead Load <br> (lb) | Wind FX <br> (lb) | Dead Load (Ib) | Wind FX <br> (Ib) | Dead Load (lb) | $\begin{gathered} \text { Torsion } \\ \text { MY } \\ \text { (lb-ft) } \end{gathered}$ | Moment MZ <br> (Ib) |
| 0.00 |  | 55.4 | 0.0 |  |  |  |  | 0.0 | 0.0 | 55.4 | 0.0 | 0.0 | 0.0 |
| 5.00 |  | 109.9 | 545.3 |  |  |  |  | 0.0 | 101.6 | 109.9 | 646.9 | 0.0 | 0.0 |
| 10.00 |  | 107.9 | 535.5 |  |  |  |  | 0.0 | 101.6 | 107.9 | 637.1 | 0.0 | 0.0 |
| 15.00 |  | 107.9 | 525.8 |  |  |  |  | 0.0 | 101.6 | 107.9 | 627.4 | 0.0 | 0.0 |
| 20.00 |  | 110.1 | 516.0 |  |  |  |  | 0.0 | 101.6 | 110.1 | 617.6 | 0.0 | 0.0 |
| 25.00 |  | 112.2 | 506.3 |  |  |  |  | 0.0 | 101.6 | 112.2 | 607.9 | 0.0 | 0.0 |
| 30.00 |  | 113.5 | 496.5 |  |  |  |  | 0.0 | 101.6 | 113.5 | 598.1 | 0.0 | 0.0 |
| 35.00 |  | 114.2 | 486.8 |  |  |  |  | 0.0 | 101.6 | 114.2 | 588.4 | 0.0 | 0.0 |
| 40.00 |  | 114.4 | 477.1 |  |  |  |  | 0.0 | 101.6 | 114.4 | 578.7 | 0.0 | 0.0 |
| 45.00 |  | 89.7 | 467.3 |  |  |  |  | 0.0 | 101.6 | 89.7 | 568.9 | 0.0 | 0.0 |
| 47.84 | Bot - Section 2 | 57.4 | 261.3 |  |  |  |  | 0.0 | 57.8 | 57.4 | 319.1 | 0.0 | 0.0 |
| 50.00 |  | 55.5 | 345.3 |  |  |  |  | 0.0 | 43.8 | 55.5 | 389.2 | 0.0 | 0.0 |
| 52.66 | Top-Section 1 | 57.5 | 421.0 |  |  |  |  | 0.0 | 54.0 | 57.5 | 475.0 | 0.0 | 0.0 |
| 55.00 |  | 84.1 | 158.5 |  |  |  |  | 0.0 | 47.6 | 84.1 | 206.1 | 0.0 | 0.0 |
| 60.00 |  | 113.9 | 333.0 |  |  |  |  | 0.0 | 101.6 | 113.9 | 434.6 | 0.0 | 0.0 |
| 65.00 |  | 112.9 | 325.7 |  |  |  |  | 0.0 | 101.6 | 112.9 | 427.3 | 0.0 | 0.0 |
| 70.00 |  | 111.8 | 318.4 |  |  |  |  | 0.0 | 101.6 | 111.8 | 420.0 | 0.0 | 0.0 |
| 75.00 |  | 110.5 | 311.1 |  |  |  |  | 0.0 | 101.6 | 110.5 | 412.7 | 0.0 | 0.0 |
| 80.00 |  | 71.3 | 303.8 |  |  |  |  | 0.0 | 101.6 | 71.3 | 405.4 | 0.0 | 0.0 |
| 81.50 | Appertunance(s) | 43.4 | 89.7 | 36.0 | 0.0 | 1.4 | 50.0 | 0.0 | 30.5 | 79.4 | 170.2 | 0.0 | 0.0 |
| 84.00 | Top - Section 2 | 30.9 | 148.0 |  |  |  |  | 0.0 | 50.4 | 30.9 | 198.4 | 0.0 | 0.0 |
| 85.00 |  | 23.3 | 88.4 |  |  |  |  | 8.1 | 37.1 | 31.4 | 125.5 | 0.0 | 0.0 |
| 90.00 | Appertunance(s) | 35.1 | 441.8 | 0.0 | 0.0 | 0.0 | 120.0 | 40.9 | 185.5 | 76.0 | 747.4 | 0.0 | 0.0 |
| 94.00 | Top - Section 3 | 18.8 | 353.4 |  |  |  |  | 33.0 | 148.4 | 51.8 | 501.9 | 0.0 | 0.0 |
| 95.00 |  | 18.6 | 54.1 |  |  |  |  | 8.7 | 37.1 | 27.3 | 91.2 | 0.0 | 0.0 |
| 100.00 | Appertunance(s) | 28.0 | 270.6 | 0.0 | 0.0 | 0.0 | 527.4 | 43.9 | 185.5 | 71.8 | 983.5 | 0.0 | 0.0 |
| 104.00 | Top - Section 4 | 16.6 | 216.5 |  |  |  |  | 35.4 | 118.0 | 51.0 | 334.4 | 0.0 | 0.0 |
| 105.00 |  | 18.9 | 27.6 |  |  |  |  | 8.9 | 29.5 | 27.8 | 57.1 | 0.0 | 0.0 |
| 110.00 | Appertunance(s) | 28.4 | 137.8 | 0.0 | 0.0 | 0.0 | 280.7 | 44.6 | 147.4 | 73.1 | 566.0 | 0.0 | 0.0 |
| 114.00 |  | 12.7 | 110.3 |  |  |  |  | 35.9 | 67.8 | 48.6 | 178.1 | 0.0 | 0.0 |
|  |  |  |  |  |  |  |  |  | ls: | 2,379.35 | 12,913.8 | 0.00 | 0.00 |

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## Calculated Forces

| Seg Elev <br> (ft) | $\begin{gathered} \mathrm{Pu} \\ \mathrm{FY}(-) \\ \text { (kips) } \end{gathered}$ | $\begin{aligned} & \text { Vu } \\ & \text { FX }(-) \\ & \text { (kips) } \end{aligned}$ | $\begin{gathered} \text { Tu } \\ \text { MY } \\ \text { (ft-kips) } \end{gathered}$ | $\begin{gathered} \text { Mu } \\ \text { MZZ } \\ \text { (ft-kips) } \end{gathered}$ |  | Resultant Moment (ft-kips) | phi Pn <br> (kips) | $\begin{gathered} \text { phi } \\ \mathbf{V n} \\ \text { (kips) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { phi } \\ \mathrm{Tn} \\ \text { (ft-kips) } \end{gathered}$ |  | Total Deflect (in) | Rotation (deg) | Ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0.00 | -12.91 | -2.33 | 0.00 | -130.62 | 0.00 | 130.62 | 2,009.33 | ,004.66 | 3,370.24 | 1,687.62 | 0.00 | 0.00 | 0.084 |
| 5.00 | -12.27 | -2.22 | 0.00 | -118.99 | 0.00 | 118.99 | 1,990.85 | 995.43 | 3,279.75 | 1,642.31 | 0.01 | -0.03 | 0.079 |
| 10.00 | -11.63 | -2.12 | 0.00 | -107.88 | 0.00 | 107.88 | 1,971.76 | 985.88 | 3,189.37 | 1,597.06 | 0.06 | -0.05 | 0.073 |
| 15.00 | -11.00 | -2.01 | 0.00 | -97.28 | 0.00 | 97.28 | 1,952.06 | 976.03 | 3,099.16 | 1,551.88 | 0.12 | -0.08 | 0.068 |
| 20.00 | -10.38 | -1.91 | 0.00 | -87.21 | 0.00 | 87.21 | 1,931.73 | 965.87 | 3,009.15 | 1,506.81 | 0.21 | -0.10 | 0.063 |
| 25.00 | -9.77 | -1.80 | 0.00 | -77.67 | 0.00 | 77.67 | 1,910.79 | 955.40 | 2,919.43 | 1,461.88 | 0.33 | -0.12 | 0.058 |
| 30.00 | -9.17 | -1.69 | 0.00 | -68.69 | 0.00 | 68.69 | 1,889.23 | 944.62 | 2,830.03 | 1,417.12 | 0.46 | -0.14 | 0.053 |
| 35.00 | -8.59 | -1.57 | 0.00 | -60.26 | 0.00 | 60.26 | 1,867.06 | 933.53 | 2,741.01 | 1,372.54 | 0.62 | -0.16 | 0.049 |
| 40.00 | -8.01 | -1.46 | 0.00 | -52.39 | 0.00 | 52.39 | 1,844.27 | 922.13 | 2,652.43 | 1,328.19 | 0.80 | -0.18 | 0.044 |
| 45.00 | -7.44 | -1.37 | 0.00 | -45.10 | 0.00 | 45.10 | 1,820.86 | 910.43 | 2,564.34 | 1,284.08 | 0.99 | -0.19 | 0.039 |
| 47.84 | -7.12 | -1.31 | 0.00 | -41.20 | 0.00 | 41.20 | 1,807.27 | 903.64 | 2,514.51 | 1,259.12 | 1.11 | -0.20 | 0.037 |
| 50.00 | -6.73 | -1.26 | 0.00 | -38.37 | 0.00 | 38.37 | 1,796.83 | 898.42 | 2,476.80 | 1,240.24 | 1.20 | -0.21 | 0.035 |
| 52.66 | -6.26 | -1.20 | 0.00 | -35.04 | 0.00 | 35.04 | 1,181.92 | 590.96 | 1,634.85 | 818.64 | 1.32 | -0.21 | 0.048 |
| 55.00 | -6.05 | -1.11 | 0.00 | -32.23 | 0.00 | 32.23 | 1,176.68 | 588.34 | 1,611.12 | 806.76 | 1.42 | -0.22 | 0.045 |
| 60.00 | -5.61 | -1.00 | 0.00 | -26.67 | 0.00 | 26.67 | 1,165.04 | 582.52 | 1,560.37 | 781.35 | 1.66 | -0.24 | 0.039 |
| 65.00 | -5.19 | -0.89 | 0.00 | -21.67 | 0.00 | 21.67 | 1,152.79 | 576.40 | 1,509.51 | 755.88 | 1.92 | -0.25 | 0.033 |
| 70.00 | -4.77 | -0.77 | 0.00 | -17.24 | 0.00 | 17.24 | 1.139.92 | 569.96 | 1,458.59 | 730.38 | 2.19 | -0.26 | 0.028 |
| 75.00 | -4.36 | -0.66 | 0.00 | -13.38 | 0.00 | 13.38 | 1,126.44 | 563.22 | 1,407.67 | 704.88 | 2.47 | -0.27 | 0.023 |
| 80.00 | -3.95 | -0.59 | 0.00 | -10.08 | 0.00 | 10.08 | 1,112.33 | 556.17 | 1,356.80 | 679.41 | 2.75 | -0.28 | 0.018 |
| 81.50 | -3.78 | -0.51 | 0.00 | -9.19 | 0.00 | 9.19 | 1,107.98 | 553.99 | 1,341.56 | 671.78 | 2.84 | -0.28 | 0.017 |
| 84.00 | -3.58 | -0.48 | 0.00 | -7.92 | 0.00 | 7.92 | 1,100.61 | 550.30 | 1,316.18 | 659.07 | 2.99 | -0.28 | 0.015 |
| 84.00 | -3.58 | -0.48 | 0.00 | -7.92 | 0.00 | 7.92 | 1,168.53 | 584.26 | 139.98 | 118.82 | 2.99 | -0.28 | 0.070 |
| 85.00 | -3.46 | -0.45 | 0.00 | -7.45 | 0.00 | 7.45 | 1.168.53 | 584.26 | 139.98 | 118.82 | 3.05 | -0.28 | 0.066 |
| 90.00 | -2.71 | -0.37 | 0.00 | -5.20 | 0.00 | 5.20 | 1,168.53 | 584.26 | 139.98 | 118.82 | 3.44 | -0.45 | 0.046 |
| 94.00 | -2.21 | -0.32 | 0.00 | -3.70 | 0.00 | 3.70 | 1,168.53 | 584.26 | 139.98 | 118.82 | 3.86 | -0.55 | 0.033 |
| 94.00 | -2.21 | -0.32 | 0.00 | -3.70 | 0.00 | 3.70 | 715.69 | 357.85 | 67.10 | 56.95 | 3.86 | -0.55 | 0.068 |
| 95.00 | -2.12 | -0.30 | 0.00 | -3.38 | 0.00 | 3.38 | 715.69 | 357.85 | 67.10 | 56.95 | 3.98 | -0.57 | 0.062 |
| 100.00 | -1.13 | -0.22 | 0.00 | -1.90 | 0.00 | 1.90 | 715.69 | 357.85 | 67.10 | 56.95 | 4.68 | -0.75 | 0.035 |
| 104.00 | -0.80 | -0.16 | 0.00 | -1.03 | 0.00 | 1.03 | 715.69 | 357.85 | 67.10 | 56.95 | 5.34 | -0.83 | 0.019 |
| 104.00 | -0.80 | -0.16 | 0.00 | -1.03 | 0.00 | 1.03 | 255.19 | 127.60 | 34.57 | 26.17 | 5.34 | -0.83 | 0.043 |
| 105.00 | -0.74 | -0.13 | 0.00 | -0.87 | 0.00 | 0.87 | 255.19 | 127.60 | 34.57 | 26.17 | 5.52 | -0.85 | 0.036 |
| 110.00 | -0.18 | -0.05 | 0.00 | -0.21 | 0.00 | 0.21 | 255.19 | 127.60 | 34.57 | 26.17 | 6.44 | -0.90 | 0.009 |
| 114.00 | 0.00 | -0.05 | 0.00 | 0.00 | 0.00 | 0.00 | 255.19 | 127.60 | 34.57 | 26.17 | 7.20 | -0.91 | 0.000 |

Site Number: 27741_B
Site Name: Round Hill CT, Greenwich, CT
Customer:

## Equivalent Lateral Forces Method Analysis

## (Based on ASCE7-10 Chapters 11, 12, 15)

Spectral Response Acceleration for Short Period $\left(S_{s}\right): \quad 0.26$

Spectral Response Acceleration at 1.0 Second Period $\left(S_{1}\right): \quad 0.07$
Long-Period Transition Period ( $T_{\mathrm{L}}$ ): 6
Importance Factor ( $I_{\mathrm{E}}$ ): 1.00
Site Coefficient $F_{a}: \quad 1.59$
Site Coeffiecient $F_{\mathbf{v}}$ : 2.40
$\begin{array}{ll}\text { Response Modification Coefficient (R): } & 1.50\end{array}$
Design Spectral Response Acceleration at Short Period ( $\mathbf{S}_{\mathrm{ds}}$ ): $\mathbf{0 . 2 8}$
Design Spectral Response Acceleration at 1.0 Second Period ( $\mathbf{S}_{\mathrm{d} 1}$ ): 0.11
Seismic Response Coefficient $\left(C_{s}\right): \quad 0.05$
Upper Limit $\mathrm{C}_{8} \quad 0.05$
Lower Limit $\mathrm{C}_{\mathrm{s}} \quad 0.03$
$\begin{array}{ll}\text { Period based on Rayleigh Method (sec): } & 1.53\end{array}$
Redundancy Factor (p): 1.30
Seismic Force Distribution Exponent ( $k$ ): 1.51
Total Unfactored Dead Load: 12.91 k
Seismic Base Shear (日): 0.83 k

| Site Number: | $27741 \_B$ | Code: ANSITTIA-222-G | © 2007-2019 by ATC IP LLC. All nights reserved. |
| :--- | :--- | :--- | ---: | :--- |
| Site Name: | Round Hill CT, Greenwich, CT | Engineering Number: REV01 | 9/16/2019 9:52:11 AM |
| Customer: | KG |  |  |

## Equivalent Modal Forces Analysis

(Based on ASCE7-10 Chapters 11, 12 \& 15 and ANSI/TIA-G, section 2.7)

| Spectral Response Acceleration for Short Period $\left(S_{s}\right):$ | 0.26 |
| :--- | :--- |
| Spectral Response Acceleration at 1.0 Second Period $\left(S_{1}\right):$ | 0.07 |
| Importance Factor $\left(I_{E}\right):$ | 1.00 |
| Site Coefficient $F_{\mathrm{a}}:$ | 1.59 |
| Site Coefficient $F_{v}$ | 2.40 |
| Response Modification Coefficient (R): | 1.50 |
| Design Spectral Response Acceleration at Short Period $\left(S_{d s}\right):$ | 0.28 |
| Desing Spectral Response Acceleration at 1.0 Second Period $\left(S_{d 1}\right):$ | 0.11 |
| Period Based on Rayleigh Method (sec): | 1.53 |
| Redundancy Factor $(p):$ | 1.30 |

Load Case $(1.2+0.2 S d s)^{*}$ DL + EELFM Seismic Equivalent Lateral Forces Method

| Segment | Height Above Base <br> (ft) | Weight <br> (b) | a | b | C | Saz | Horizontal Force (Ib) | Vertical Force (b) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 29 | 112.00 | 178 | 1.824 | 1.651 | 1.020 | 0.470 | 72 | 150 |
| 28 | 107.50 | 285 | 1.681 | 1.050 | 0.785 | 0.348 | 86 | 241 |
| 27 | 104.50 | 57 | 1.588 | 0.742 | 0.654 | 0.277 | 14 | 48 |
| 26 | 102.00 | 334 | 1.513 | 0.534 | 0.558 | 0.223 | 65 | 283 |
| 25 | 97.50 | 456 | 1.382 | 0.252 | 0.414 | 0.139 | 55 | 385 |
| 24 | 94.50 | 91 | 1.299 | 0.119 | 0.335 | 0.092 | 7 | 77 |
| 23 | 92.00 | 502 | 1.231 | 0.036 | 0.278 | 0.058 | 25 | 424 |
| 22 | 87.50 | 627 | 1.113 | -0.062 | 0.195 | 0.010 | 6 | 530 |
| 21 | 84.50 | 125 | 1.038 | -0.098 | 0.151 | -0.013 | -1 | 106 |
| 20 | 82.75 | 198 | 0.996 | -0.111 | 0.129 | -0.023 | -4 | 168 |
| 19 | 80.75 | 120 | 0.948 | -0.119 | 0.107 | -0.032 | -3 | 102 |
| 18 | 77.50 | 405 | 0.873 | -0.121 | 0.077 | -0.040 | -14 | 343 |
| 17 | 72.50 | 413 | 0.764 | -0.104 | 0.044 | -0.039 | -14 | 349 |
| 16 | 67.50 | 420 | 0.663 | -0.075 | 0.023 | -0.023 | -9 | 355 |
| 15 | 62.50 | 427 | 0.568 | -0.041 | 0.011 | 0.000 | 0 | 361 |
| 14 | 57.50 | 435 | 0.481 | -0.009 | 0.006 | 0.025 | 10 | 367 |
| 13 | 53.83 | 206 | 0.421 | 0.011 | 0.006 | 0.041 | 7 | 174 |
| 12 | 51.33 | 475 | 0.383 | 0.023 | 0.007 | 0.050 | 21 | 401 |
| 11 | 48.92 | 389 | 0.348 | 0.033 | 0.009 | 0.056 | 19 | 329 |
| 10 | 46.42 | 319 | 0.313 | 0.042 | 0.011 | 0.062 | 17 | 270 |
| 9 | 42.50 | 569 | 0.263 | 0.053 | 0.016 | 0.067 | 33 | 481 |
| 8 | 37.50 | 579 | 0.205 | 0.062 | 0.023 | 0.069 | 34 | 489 |
| 7 | 32.50 | 588 | 0.154 | 0.068 | 0.030 | 0.068 | 34 | 497 |
| 6 | 27.50 | 598 | 0.110 | 0.071 | 0.036 | 0.065 | 34 | 505 |
| 5 | 22.50 | 608 | 0.074 | 0.072 | 0.040 | 0.063 | 33 | 514 |
| 4 | 17.50 | 618 | 0.045 | 0.071 | 0.042 | 0.060 | 32 | 522 |
| 3 | 12.50 | 627 | 0.023 | 0.065 | 0.039 | 0.055 | 30 | 530 |
| 2 | 7.50 | 637 | 0.008 | 0.052 | 0.030 | 0.045 | 25 | 538 |
| 1 | 2.50 | 647 | 0.001 | 0.023 | 0.013 | 0.022 | 12 | 547 |
| Flush Mount | 110.00 | 120 | 1.760 | 1.362 | 0.909 | 0.414 | 43 | 101 |
| DBXNH-6565A-A2M | 110.00 | 103 | 1.760 | 1.362 | 0.909 | 0.414 | 37 | 87 |
| TMAT1921XB6811A | 110.00 | 53 | 1.760 | 1.362 | 0.909 | 0.414 | 19 | 45 |
| 78211066 | 110.00 | 5 | 1.760 | 1.362 | 0.909 | 0.414 | 2 | 4 |
| Flush Mount | 100.00 | 120 | 1.454 | 0.395 | 0.490 | 0.184 | 19 | 101 |

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Site Number: 27741_B
Code: ANSITTIA-222-G © 2007-2019 by ATC IP LLC. All ights reserved.
Site Name: Round Hill CT, Greenwich, CT
Engineering Number: REV01
9/16/2019 9:52:11 AM
Customer: KG

| APXVSPP18-C-A20 | 100.00 | 171 | 1.454 | 0.395 | 0.490 | 0.184 | 27 | 144 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| KIT-FD9R6004/1C-DL | 100.00 | 38 | 1.454 | 0.395 | 0.490 | 0.184 | 6 | 32 |
| IBC1900HG-SA | 100.00 | 198 | 1.454 | 0.395 | 0.490 | 0.184 | 32 | 167 |
| Flush Mount | 90.00 | 120 | 1.178 | -0.015 | 0.239 | 0.035 | 4 | 101 |
| 3 ft Standoff | 81.50 | 40 | 0.966 | -0.117 | 0.115 | -0.029 | -1 | 34 |
| GPS | 81.50 | 10 | 0.966 | -0.117 | 0.115 | -0.029 | 0 | 8 |
|  |  | 12,914 | 36.276 | 11.065 | 11.157 | 4.560 | 813 | 10,912 |

Load Case $1.2+0.2 S d s) *$ DL + EEMAM Seismic Equivalent Modal Analysis Method


| 29 | 112.00 | 178 | 1.824 | 1.651 | 1.020 | 0.470 | 72 | 150 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 28 | 107.50 | 285 | 1.681 | 1.050 | 0.785 | 0.348 | 86 | 241 |
| 27 | 104.50 | 57 | 1.588 | 0.742 | 0.654 | 0.277 | 14 | 48 |
| 26 | 102.00 | 334 | 1.513 | 0.534 | 0.558 | 0.223 | 65 | 283 |
| 25 | 97.50 | 456 | 1.382 | 0.252 | 0.414 | 0.139 | 55 | 385 |
| 24 | 94.50 | 91 | 1.299 | 0.119 | 0.335 | 0.092 | 7 | 77 |
| 23 | 92.00 | 502 | 1.231 | 0.036 | 0.278 | 0.058 | 25 | 424 |
| 22 | 87.50 | 627 | 1.113 | -0.062 | 0.195 | 0.010 | 6 | 530 |
| 21 | 84.50 | 125 | 1.038 | -0.098 | 0.151 | -0.013 | -1 | 106 |
| 20 | 82.75 | 198 | 0.996 | -0.111 | 0.129 | -0.023 | -4 | 168 |
| 19 | 80.75 | 120 | 0.948 | -0.119 | 0.107 | -0.032 | -3 | 102 |
| 18 | 77.50 | 405 | 0.873 | -0.121 | 0.077 | -0.040 | -14 | 343 |
| 17 | 72.50 | 413 | 0.764 | -0.104 | 0.044 | -0.039 | -14 | 349 |
| 16 | 67.50 | 420 | 0.663 | -0.075 | 0.023 | -0.023 | -9 | 355 |
| 15 | 62.50 | 427 | 0.568 | -0.041 | 0.011 | 0.000 | 0 | 361 |
| 14 | 57.50 | 435 | 0.481 | -0.009 | 0.006 | 0.025 | 10 | 367 |
| 13 | 53.83 | 206 | 0.421 | 0.011 | 0.006 | 0.041 | 7 | 174 |
| 12 | 51.33 | 475 | 0.383 | 0.023 | 0.007 | 0.050 | 21 | 401 |
| 11 | 48.92 | 389 | 0.348 | 0.033 | 0.009 | 0.056 | 19 | 329 |
| 10 | 46.42 | 319 | 0.313 | 0.042 | 0.011 | 0.062 | 17 | 270 |
| 9 | 42.50 | 569 | 0.263 | 0.053 | 0.016 | 0.067 | 33 | 481 |
| 8 | 37.50 | 579 | 0.205 | 0.062 | 0.023 | 0.069 | 34 | 489 |
| 7 | 32.50 | 588 | 0.154 | 0.068 | 0.030 | 0.068 | 34 | 497 |
| 6 | 27.50 | 598 | 0.110 | 0.071 | 0.036 | 0.065 | 34 | 505 |
| 5 | 22.50 | 608 | 0.074 | 0.072 | 0.040 | 0.063 | 33 | 514 |
| 4 | 17.50 | 618 | 0.045 | 0.071 | 0.042 | 0.060 | 32 | 522 |
| 3 | 12.50 | 627 | 0.023 | 0.065 | 0.039 | 0.055 | 30 | 530 |
| 2 | 7.50 | 637 | 0.008 | 0.052 | 0.030 | 0.045 | 25 | 538 |
| 1 | 2.50 | 647 | 0.001 | 0.023 | 0.013 | 0.022 | 12 | 547 |
| Flush Mount | 110.00 | 120 | 1.760 | 1.362 | 0.909 | 0.414 | 43 | 101 |
| DBXNH-6565A-A2M | 110.00 | 103 | 1.760 | 1.362 | 0.909 | 0.414 | 37 | 87 |
| TMAT1921XB6811A | 110.00 | 53 | 1.760 | 1.362 | 0.909 | 0.414 | 19 | 45 |
| 78211066 | 110.00 | 5 | 1.760 | 1.362 | 0.909 | 0.414 | 2 | 4 |
| Flush Mount | 100.00 | 120 | 1.454 | 0.395 | 0.490 | 0.184 | 19 | 101 |
| APXVSPP18-C-A20 | 100.00 | 171 | 1.454 | 0.395 | 0.490 | 0.184 | 27 | 144 |
| KT-FD9R6004/1C-DL | 100.00 | 38 | 1.454 | 0.395 | 0.490 | 0.184 | 6 | 32 |
| IBC1900HG-SA | 100.00 | 198 | 1.454 | 0.395 | 0.490 | 0.184 | 32 | 167 |
| Flush Mount | 90.00 | 120 | 1.178 | -0.015 | 0.239 | 0.035 | 4 | 101 |
| 3 ft Standoff | 81.50 | 40 | 0.966 | -0.117 | 0.115 | -0.029 | -1 | 34 |
| GPS | 81.50 | 10 | 0.966 | -0.117 | 0.115 | -0.029 | 0 | 8 |
| 12,914 |  |  | 36.276 | 11.065 | 11.157 | 4.560 | 813 | 10,912 |


| Site Number: 27741_B <br> Site Name: Round Hill CT, Greenwich, CT <br> Customer: KG |  |  | Code: ANSIITIA-222-G <br> Engineering Number: REV01 |  |  |  | © 2007-2019 by ATC IP LLC. All rights reserved. 9/16/2019 9:52:11 AM |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Load Case (0.9-0.2Sds) * DL + E ELFM |  |  | Seismic (Reduced DL.) Equivalent Lateral Forces Method |  |  |  |  |  |
| Segment | Height Above Base <br> (ft) | Weight <br> (b) | a | b | c | Saz | Horizontal Force (b) | Vertical Force (lb) |
| 29 | 112.00 | 178 | 1.824 | 1.651 | 1.020 | 0.470 | 72 | 150 |
| 28 | 107.50 | 285 | 1.681 | 1.050 | 0.785 | 0.348 | 86 | 241 |
| 27 | 104.50 | 57 | 1.588 | 0.742 | 0.654 | 0.277 | 14 | 48 |
| 26 | 102.00 | 334 | 1.513 | 0.534 | 0.558 | 0.223 | 65 | 283 |
| 25 | 97.50 | 456 | 1.382 | 0.252 | 0.414 | 0.139 | 55 | 385 |
| 24 | 94.50 | 91 | 1.299 | 0.119 | 0.335 | 0.092 | 7 | 77 |
| 23 | 92.00 | 502 | 1.231 | 0.036 | 0.278 | 0.058 | 25 | 424 |
| 22 | 87.50 | 627 | 1.113 | -0.062 | 0.195 | 0.010 | 6 | 530 |
| 21 | 84.50 | 125 | 1.038 | -0.098 | 0.151 | -0.013 | -1 | 106 |
| 20 | 82.75 | 198 | 0.996 | -0.111 | 0.129 | -0.023 | -4 | 168 |
| 19 | 80.75 | 120 | 0.948 | -0.119 | 0.107 | -0.032 | -3 | 102 |
| 18 | 77.50 | 405 | 0.873 | -0.121 | 0.077 | -0.040 | -14 | 343 |
| 17 | 72.50 | 413 | 0.764 | -0.104 | 0.044 | -0.039 | -14 | 349 |
| 16 | 67.50 | 420 | 0.663 | -0.075 | 0.023 | -0.023 | -9 | 355 |
| 15 | 62.50 | 427 | 0.568 | -0.041 | 0.011 | 0.000 | 0 | 361 |
| $14$ | 57.50 | 435 | 0.481 | -0.009 | 0.006 | 0.025 | 10 | 367 |
| $13$ | 53.83 | 206 | 0.421 | 0.011 | 0.006 | 0.041 | 7 | 174 |
| 12 | 51.33 | 475 | 0.383 | 0.023 | 0.007 | 0.050 | 21 | 401 |
| 11 | 48.92 | 389 | 0.348 | 0.033 | 0.009 | 0.056 | 19 | 329 |
| 10 | 46.42 | 319 | 0.313 | 0.042 | 0.011 | 0.062 | 17 | 270 |
| 9 | 42.50 | 569 | 0.263 | 0.053 | 0.016 | 0.067 | 33 | 481 |
| 8 | 37.50 | 579 | 0.205 | 0.062 | 0.023 | 0.069 | 34 | 489 |
| 7 | 32.50 | 588 | 0.154 | 0.068 | 0.030 | 0.068 | 34 | 497 |
| 6 | 27.50 | 598 | 0.110 | 0.071 | 0.036 | 0.065 | 34 | 505 |
| 5 | 22.50 | 608 | 0.074 | 0.072 | 0.040 | 0.063 | 33 | 514 |
| 4 | 17.50 | 618 | 0.045 | 0.071 | 0.042 | 0.060 | 32 | 522 |
| 3 | 12.50 | 627 | 0.023 | 0.065 | 0.039 | 0.055 | 30 | 530 |
| $2$ | 7.50 | 637 | 0.008 | 0.052 | 0.030 | 0.045 | 25 | 538 |
| $1$ | 2.50 | 647 | 0.001 | 0.023 | 0.013 | 0.022 | 12 | 547 |
| Flush Mount | 110.00 | 120 | 1.760 | 1.362 | 0.909 | 0.414 | 43 | 101 |
| DBXNH-6565A-A2M | 110.00 | 103 | 1.760 | 1.362 | 0.909 | 0.414 | 37 | 87 |
| TMAT1921XB6811A | 110.00 | 53 | 1.760 | 1.362 | 0.909 | 0.414 | 19 | 45 |
| 78211066 | 110.00 | 5 | 1.760 | 1.362 | 0.909 | 0.414 | 2 | 4 |
| Flush Mount | 100.00 | 120 | 1.454 | 0.395 | 0.490 | 0.184 | 19 | 101 |
| APXVSPP18-C-A20 | 100.00 | 171 | 1.454 | 0.395 | 0.490 | 0.184 | 27 | 144 |
| KIT-FD9R6004/1C-DL. | 100.00 | 38 | 1.454 | 0.395 | 0.490 | 0.184 | 6 | 32 |
| IBC1900HG-SA | 100.00 | 198 | 1.454 | 0.395 | 0.490 | 0.184 | 32 | 167 |
| Flush Mount | 90.00 | 120 | 1.178 | -0.015 | 0.239 | 0.035 | 4 | 101 |
| 3 ft Standoff | 81.50 | 40 | 0.966 | -0.117 | 0.115 | -0.029 | -1 | 34 |
| GPS | 81.50 | 10 | 0.966 | -0.117 | 0.115 | -0.029 | 0 | 8 |
|  |  | 12,914 | 36.276 | 11.065 | 11.157 | 4.560 | 813 | 10,912 |

Load Case (0.9-0.2Sds) * DL + E EMAM Seismic (Reduced DL) Equivalent Modal Analysis Method

| Segment | Height Above Base <br> (ft) | Weight <br> (ib) | a | b | c | Saz | Horizontal Force (Ib) | Vertical Force (Ib) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 29 | 112.00 | 178 | 1.824 | 1.651 | 1.020 | 0.470 | 72 | 150 |
| 28 | 107.50 | 285 | 1.681 | 1.050 | 0.785 | 0.348 | 86 | 241 |
| 27 | 104.50 | 57 | 1.588 | 0.742 | 0.654 | 0.277 | 14 | 48 |
| 26 | 102.00 | 334 | 1.513 | 0.534 | 0.558 | 0.223 | 65 | 283 |
| 25 | 97.50 | 456 | 1.382 | 0.252 | 0.414 | 0.139 | 55 | 385 |


| Site Number: | 27741_B | Code: ANSITTIA-222-G | © 2007-2019 by ATC IP LLC. All inghts reserved. |
| :--- | :--- | ---: | ---: | ---: |
| Site Name: | Round Hill CT, Greenwich, CT | Engineering Number: REV01 |  |
| Customer: | KG |  |  |


| 24 | 94.50 | 91 | 1.299 | 0.119 | 0.335 | 0.092 | 7 | 77 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 23 | 92.00 | 502 | 1.231 | 0.036 | 0.278 | 0.058 | 25 | 424 |
| 22 | 87.50 | 627 | 1.113 | -0.062 | 0.195 | 0.010 | 6 | 530 |
| 21 | 84.50 | 125 | 1.038 | -0.098 | 0.151 | -0.013 | -1 | 106 |
| 20 | 82.75 | 198 | 0.996 | -0.111 | 0.129 | -0.023 | -4 | 168 |
| 19 | 80.75 | 120 | 0.948 | -0.119 | 0.107 | -0.032 | -3 | 102 |
| 18 | 77.50 | 405 | 0.873 | -0.121 | 0.077 | -0.040 | -14 | 343 |
| 17 | 72.50 | 413 | 0.764 | -0.104 | 0.044 | -0.039 | -14 | 349 |
| 16 | 67.50 | 420 | 0.663 | -0.075 | 0.023 | -0.023 | -9 | 355 |
| 15 | 62.50 | 427 | 0.568 | -0.041 | 0.011 | 0.000 | 0 | 361 |
| 14 | 57.50 | 435 | 0.481 | -0.009 | 0.006 | 0.025 | 10 | 367 |
| 13 | 53.83 | 206 | 0.421 | 0.011 | 0.006 | 0.041 | 7 | 174 |
| 12 | 51.33 | 475 | 0.383 | 0.023 | 0.007 | 0.050 | 21 | 401 |
| 11 | 48.92 | 389 | 0.348 | 0.033 | 0.009 | 0.056 | 19 | 329 |
| 10 | 46.42 | 319 | 0.313 | 0.042 | 0.011 | 0.062 | 17 | 270 |
| 9 | 42.50 | 569 | 0.263 | 0.053 | 0.016 | 0.067 | 33 | 481 |
| 8 | 37.50 | 579 | 0.205 | 0.062 | 0.023 | 0.069 | 34 | 489 |
| 7 | 32.50 | 588 | 0.154 | 0.068 | 0.030 | 0.068 | 34 | 497 |
| 6 | 27.50 | 598 | 0.110 | 0.071 | 0.036 | 0.065 | 34 | 505 |
| 5 | 22.50 | 608 | 0.074 | 0.072 | 0.040 | 0.063 | 33 | 514 |
| 4 | 17.50 | 618 | 0.045 | 0.071 | 0.042 | 0.060 | 32 | 522 |
| 3 | 12.50 | 627 | 0.023 | 0.065 | 0.039 | 0.055 | 30 | 530 |
| 2 | 7.50 | 637 | 0.008 | 0.052 | 0.030 | 0.045 | 25 | 538 |
| $1$ | 2.50 | 647 | 0.001 | 0.023 | 0.013 | 0.022 | 12 | 547 |
| Flush Mount | 110.00 | 120 | 1.760 | 1.362 | 0.909 | 0.414 | 43 | 101 |
| DBXNH-6565A-A2M | 110.00 | 103 | 1.760 | 1.362 | 0.909 | 0.414 | 37 | 87 |
| TMAT1921XB6811A | 110.00 | 53 | 1.760 | 1.362 | 0.909 | 0.414 | 19 | 45 |
| 78211066 | 110.00 | 5 | 1.760 | 1.362 | 0.909 | 0.414 | 2 | 4 |
| Flush Mount | 100.00 | 120 | 1.454 | 0.395 | 0.490 | 0.184 | 19 | 101 |
| APXVSPP18-C-A20 | 100.00 | 171 | 1.454 | 0.395 | 0.490 | 0.184 | 27 | 144 |
| KIT-FD9R6004/1C-DL | 100.00 | 38 | 1.454 | 0.395 | 0.490 | 0.184 | 6 | 32 |
| IBC1900HG-SA | 100.00 | 198 | 1.454 | 0.395 | 0.490 | 0.184 | 32 | 167 |
| Flush Mount | 90.00 | 120 | 1.178 | -0.015 | 0.239 | 0.035 | 4 | 101 |
| 3 ft Standoff | 81.50 | 40 | 0.966 | -0.117 | 0.115 | -0.029 | -1 | 34 |
| GPS | 81.50 | 10 | 0.966 | -0.117 | 0.115 | -0.029 | 0 | 8 |
|  |  | 12,914 | 36.276 | 11.065 | 11.157 | 4.560 | 813 | 10,912 |

Site Number: 27741_B
Site Name: Round Hill CT, Greenwich, CT
Customer: KG

## Analysis Summary

| Load Case | Reactions |  |  |  |  |  | Max Usage |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Shear FX (kips) | Shear FZ (kips) | Axial FY (kips) | $\begin{aligned} & \text { Moment } \\ & \text { MX } \\ & \text { (ft-kips) } \end{aligned}$ | Moment MY (ft-kips) | Moment MZ (ft-kips) |  | Interaction Ratio |
| 1.2D + 1.6W | 8.95 | 0.00 | 15.49 | 0.00 | 0.00 | 503.96 | 0.00 | 0.31 |
| $0.9 \mathrm{D}+1.6 \mathrm{~W}$ | 8.94 | 0.00 | 11.61 | 0.00 | 0.00 | 501.34 | 0.00 | 0.30 |
| $1.2 \mathrm{D}+1.0 \mathrm{Di}+1.0 \mathrm{Wi}$ | 3.32 | 0.00 | 27.00 | 0.00 | 0.00 | 196.12 | 94.00 | 0.13 |
| $(1.2+0.2 \mathrm{Sds})^{*}$ DL + E ELFM | 0.83 | 0.00 | 15.40 | 0.00 | 0.00 | 67.58 | 84.00 | 0.06 |
| (1.2 + 0.2Sds) * DL + E EMAM | 0.80 | 0.00 | 15.40 | 0.00 | 0.00 | 61.07 | 94.00 | 0.09 |
| (0.9-0.2Sds) * DL + E ELFM | 0.83 | 0.00 | 10.37 | 0.00 | 0.00 | 67.07 | 84.00 | 0.06 |
| (0.9-0.2Sds) * DL + E EMAM | 0.80 | 0.00 | 10.37 | 0.00 | 0.00 | 60.55 | 94.00 | 0.09 |
| 1.0D + 1.0W | 2.33 | 0.00 | 12.91 | 0.00 | 0.00 | 130.62 | 0.00 | 0.08 |

## Base Plate and Bolt Analysis

## Reinforcement:

Moment:
Shear/Leg:
Compression/Leg:
TIA-222 Code Revision (F/G):
Anchor Bolt Arrangement:
Monopole Shaft Diameter (Across Flats):
Lower Monopole Thickness:
\# of Sides of Pole:
Monopole Shaft Yield Strength:
Baseplate Diameter / Length:
Base Plate Thickness:
Base Plate Yield Strength:
Baseplate Detail Type:
Include Plate Thickness Beyond Bolt Circle:
Stress Increase:
Fillet Weld Size:
Weld Type (CJP or F/F):
Weld Strength:

G
Corners
41.0 in
0.250 in
18
65 ksi
47.50
2.00 in
60 ksi

D
Y
1.00
0.375 in

CJP
70 ksi

Anchor Bolts

| Anchor Bolt Yield Strength: | 75 ksi |
| :--- | ---: |
| Anchor Bolt Ultimate Strength: | 100 ksi |
| Anchor Bolt Diameter: | 2.25 in |
| Anchor Bolt Circle: | 49.00 in |
| \# of Anchor Bolts: | 4 |
| Minimum Anchor Bolt Separation: | 6.00 in |
| Additional Anchor Bolts Installed: | N |


|  | Baseplate Flexural Capacity |  |  |  |  | Baseplate Shear Capacity |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Failure <br> Mode: | Effective <br> Width $(\mathrm{in})$ | Moment <br> $(\mathrm{k}$ - in$)$ | S/Z <br> $\left(\mathrm{in}^{5}\right)$ | Capacity <br> $(\mathrm{k}$-in) | Usage | Shear <br> $(\mathrm{k})$ | Area <br> $\left(\mathrm{in}^{〔}\right)$ | Capacity <br> $(\mathrm{k})$ | Usage |
| AA | 29.84 | 468.5 | 29.8 | 1611.4 | 0.29 | 127.2 | 59.7 | 1933.7 | 0.07 |
| AB | 29.84 | 461.4 | 29.8 | 1611.4 | 0.29 | 127.2 | 59.7 | 1933.7 | 0.07 |
| BA | 30.83 | 508.7 | 30.8 | 1665.0 | 0.31 | 127.2 | 61.7 | 1998.0 | 0.06 |
| BB | 30.83 | 508.7 | 30.8 | 1665.0 | 0.31 | 127.2 | 61.7 | 1998.0 | 0.06 |

## Anchor Bolt Capacity

Area of Bolt:
Inertia of Bolt:
Total Bolt Inertia:
Maximum Bolt Tension:
Maximum Bolt Compression:
Bolt Shear:
Tensile Bolt Capacity:
Compressive Bolt Capacity:
Shear Bolt Capacity:
Interaction Equation:
$3.25 \mathrm{in}^{2}$
0.84 in $^{4}$
3902.2 in $^{4}$
119.4 k
127.2 k
2.2 k
259.8 k
259.8 k
140.3 k
0.51 Result:

OK

Base Weld Capacity

Force / Weld:
Weld Capacity:
Interaction Equation:
SES Base Plate Design Moment:
Design Stress:
SES Base Plate Allowable Stress / Moment Capacity:
Usage:

Moment Factor:

$$
3.6 \text { k/in }
$$

$23.8 \mathrm{k} / \mathrm{in}$
0.15 Result:

OK
508.7 k-in
15.8 ksi
1738.9 ksi / k-in
0.29

Length Factor:
1.00
0.96

| Site Number: | SE60XC302 |
| :--- | :---: |
| Site Name: | Splinter |
| Job Number: | REV01 |
| Engineer: | AHB |
| Date: | $\mathbf{9 / 1 6 / 2 0 1 9}$ |

Flange @ 84'
Reinforcement:
Moment:
Shear/Leg:
Compression/Leg:
N
$30.9 \mathrm{k}-\mathrm{ft}$
1.9 k
4.3 k

TIA-222 Code Revision (F/G):
Anchor Bolt Arrangement:
Monopole Shaft Diameter:
Lower Monopole Thickness:
Monople Shaft Yield Strength:
Baseplate Diameter / Length:
Base Plate Thickness:
Base Plate Yield Strength:
Fillet Weld Size:
Weld Type (CJP or F/F):
Weld Strength:
Baseplate Detail Type:
Include Plate Thickness Beyond Bolt Circle:
Stress Increase:
Additional Anchor Bolts Installed:


Anchor Bolts
Anchor Bolt Yield Strength:
Anchor Bolt Ultimate Strength:
Anchor Bolt Diameter:
Anchor Bolt Circle:
\# of Anchor Bolts:
Minimum Anchor Bolt Separation:
Additional Anchor Bolts Installed:

92 ksi
120 ksi
1.00 in
26.00 in

12
6.00 in

N

## Anchor Bolt Capacity

Area of Bolt:
Inertia of Bolt:
Total Bolt Inertia:
Maximum Bolt Tension:
Maximum Bolt Compression:
$0.61 \mathrm{in}^{2}$
$0.03 \mathrm{in}^{4}$
614.6 in $^{4}$
4.4 k

Bolt Shear: $\quad 0.2 \mathrm{k}$
Tensile Bolt Capacity: 58.2 k
Compressive Bolt Capacity: 58.2 k
Shear Bolt Capacity:
Interaction Equation:
26.2 k
0.09 Result:

OK

| Site Number: | SE60XC302 |
| :--- | :---: |
| Site Name: | Splinter |
| Job Number: | REV01 |
| Engineer: | AHB |
| Date: | $\mathbf{9 / 1 6 / 2 0 1 9}$ |

Flange @ 94'

Reinforcement:

Moment:
Shear/Leg:
Compression/Leg:
TIA-222 Code Revision (F/G):
Anchor Bolt Arrangement:
Monopole Shaft Diameter:
Lower Monopole Thickness:
Monople Shaft Yield Strength:
Baseplate Diameter / Length:
Base Plate Thickness:
Base Plate Yield Strength:
Fillet Weld Size:
Weld Type (CJP or F/F):
Weld Strength:
Baseplate Detail Type:
Include Plate Thickness Beyond Bolt Circle:
Stress Increase:
Additional Anchor Bolts Installed:

```
N
14.5 k - ft
1.3 k
2.6 k
```

|  |
| :---: |
| Round |
|  |  |
|  |  |
|  |  |
|  |
|  |
|  |
|  |
|  |
|  |
|  |
| N |

Anchor Bolt Capacity

| Area of Bolt: | $0.61 \mathrm{in}^{2}$ |
| :--- | ---: |
| Inertia of Bolt: | $0.03 \mathrm{in}^{4}$ |
| Total Bolt Inertia: | $614.6 \mathrm{in}^{4}$ |
| Maximum Bolt Tension: | 2.0 k |
| Maximum Bolt Compression: | 2.4 k |
| Bolt Shear: | 0.1 k |
| Tensile Bolt Capacity: | 58.2 k |
| Compressive Bolt Capacity: | 58.2 k |
| Shear Bolt Capacity: | 26.2 k |
| Interaction Equation: | 0.04 Result: |
|  | OK |

OK

Anchor Bolts
Anchor Bolt Yield Strength: Anchor Bolt Ultimate Strength:
Anchor Bolt Diameter:
Anchor Bolt Circle:
\# of Anchor Bolts:
Minimum Anchor Bolt Separation:
Additional Anchor Bolts Installed:

92 ksi 120 ksi 1.00 in 26.00 in

12
6.00 in

Site Number:
Site Name:
Job Number: Engineer:

SE60XC302

Date:

Splinter REV01 AHB 9/16/2019

## Flange @ 104'

Reinforcement:
Moment:
Shear/Leg:
Compression/Leg:


```
4.0 k -ft
0.6 k
0.9 k
```

TIA-222 Code Revision (F/G):
Anchor Bolt Arrangement:
Monopole Shaft Diameter:
Lower Monopole Thickness:
Monople Shaft Yield Strength:
Baseplate Diameter / Length:

## Base Plate Thickness:

Base Plate Yield Strength:
Fillet Weld Size:
Weld Type (CJP or F/F):
Weld Strength:
Baseplate Detail Type:
Include Plate Thickness Beyond Bolt Circle: Stress Increase:
Additional Anchor Bolts Installed:


Anchor Bolts
Anchor Bolt Yield Strength:
92 ksi
Anchor Bolt Ultimate Strength:
120 ksi
Anchor Bolt Diameter:
1.00 in

Anchor Bolt Circle:
\# of Anchor Bolts:
Minimum Anchor Bolt Separation:
Additional Anchor Bolts Installed:

## Anchor Bolt Capacity

Area of Bolt:
$0.61 \mathrm{in}^{2}$
Inertia of Bolt:
Total Bolt Inertia:
Maximum Bolt Tension:
$0.03 \mathrm{in}^{4}$ 614.6 in $^{4}$ 0.5 k

Maximum Bolt Compression:
0.7 k

Bolt Shear:
0.1 k

Tensile Bolt Capacity:
58.2 k

Compressive Bolt Capacity:
58.2 k
26.2 k
0.01 Result:

OK

Site Name:
Site Number:
Engineering Number:
Engineer:
Date:
Tower Type:

## Design Loads (Factored) - Analysis per TIA-222-G Standards

Design / Analysis / Mapping:
Compression/Leg:
Uplift/Leg:
Total Shear:
Moment:
Tower + Appurtenance Weight:
Depth to Base of Foundation $(1+t-h)$ :
Diameter of Pier (d):
Height of Pier above Ground (h):
Width of Pad (W):
Length of Pad (L):
Thickness of Pad ( t ):
Tower Leg Center to Center:
Number of Tower Legs:
Tower Center from Mat Center:
Depth Below Ground Surface to Water Table:
Unit Weight of Concrete:
Unit Weight of Soil Above Water Table:
Unit Weight of Water:
Unit Weight of Soil Below Water Table:
Friction Angle of Uplift:
Ultimate Coefficient of Shear Friction:
Ultimate Compressive Bearing Pressure:
Ultimate Passive Pressure on Pad Face:
$\phi_{\text {Soil and Concrete Weight: }}$ :
$\phi_{\text {soil }}$ :

Round Hill CT
27741_B
REVO1
JHH
09/16/19
MP

Program Last Updated: 5/13/2014


| Concrete Strength ( $f_{\text {c }}$ ): | 4000 psi |
| :---: | :---: |
| Pad Tension Steel Depth: | 32.00 in |
| $\phi_{\text {Shear }}$ : | 0.75 |
| $\phi_{\text {Flexure / Tension: }}$ | 0.90 |
| $\phi_{\text {compression: }}$ | 0.65 |
| $\beta$ : | 0.85 |
| Bottom Pad Rebar Size \#: | 8 |
| \# of Bottom Pad Rebar: | 16 |
| Pad Bottom Steel Area: | $12.64 \mathrm{in}^{2}$ |
| Pad Steel $\mathrm{F}_{\mathrm{y}}$ : | 60000 psi |
| Top Pad Rebar Size \#: | 8 |
| \# of Top Pad Rebar: | 16 |
| Pad Top Steel Area: | $12.64 \mathrm{in}^{2}$ |
| Pier Rebar Size \#: | 8 |
| Pier Steel Area (Single Bar): | $0.79 \mathrm{in}^{2}$ |
| \# of Pier Rebar: | 22 |
| Pier Steel $\mathrm{F}_{\mathrm{Y}}$ : | 60000 psi |
| Pier Cage Diameter: | 64.0 in |
| Rebar Strain Limit: | 0.008 |
| Steel Elastic Modulus: | 29000 ksi |
| Tie Rebar Size \#: | 4 |
| Tie Steel Area (Single Bar): | $0.20 \mathrm{in}^{2}$ |
| Tie Spacing: | 6 in |
| Tie Steel $\mathrm{F}_{\mathrm{y}}$ : | 60000 psi |

## Overturning Moment Usage

Design OTM:
557.7 k-ft

OTM Resistance:
1201.7 k-ft

Design OTM / OTM Resistance:
0.46 Result: OK

## Soil Bearing Pressure Usage

Net Bearing Pressure: 1601 psf
Factored Nominal Bearing Pressure:
Net Bearing Pressure/Factored Nominal Bearing Pressure:
Load Direction Controling Design Bearing Pressure:
9000 psf
0.18 Result: OK

Diagonal to Pad Edge

## Sliding Factor of Safety

Total Factored Sliding Resistance:
Sliding Design / Sliding Resistance:
46.7 k
0.19 Result: OK

Factored One Way Shear $\left(V_{u}\right)$ :
One Way Shear Capacity $\left(\phi V_{c}\right)$ :
$V_{u} / \phi V_{c}$ :
Load Direction Controling Shear Capacity:
Lower Steel Pad Factored Moment $\left(\mathrm{M}_{\mathrm{u}}\right)$ :
Lower Steel Pad Moment Capacity ( $\phi \mathrm{M}_{\mathrm{n}}$ ):
$M_{u} / \phi M_{n}$ :
Load Direction Controling Flexural Capacity:
Upper Steel Pad Factored Moment ( $\mathrm{M}_{\mathrm{u}}$ ):
Upper Steel Pad Moment Capacity $\left(\phi \mathrm{M}_{\mathrm{n}}\right)$ :
$M_{u} / \phi M_{n}$ :
Lower Pad Flexural Reinforcement Ratio:
Upper Pad Flexural Reinforcement Ratio:
Lower Pad Reinforcement Spacing:
Upper Pad Reinforcement Spacing:
Factored Punching Shear $\left(V_{u}\right)$ :
Nominal Punching Shear Capacity $\left(\phi_{c} V_{n}\right)$ :
$V_{u} / \phi V_{c}$ :
Factored Moment in $\operatorname{Pier}\left(\mathrm{M}_{\mathrm{u}}\right)$ :
Pier Moment Capacity $\left(\phi \mathrm{M}_{\mathrm{n}}\right)$ :
$M_{u} / \phi M_{n}$ :
Factored Shear in Pier $\left(\mathrm{V}_{\mathrm{u}}\right)$ :
Pier Shear Capacity $\left(\phi V_{n}\right)$ :
$V_{u} / \phi V_{c}$ :
Pier Shear Reinforcement Ratio:
Factored Tension in Pier $\left(T_{u}\right)$ :
Pier Tension Capacity $\left(\phi T_{n}\right)$ :
$T_{u} / \phi T_{n}$ :
Factored Compression in $\operatorname{Pier}\left(\mathrm{P}_{\mathrm{u}}\right)$ :
Pier Compression Capacity $\left(\phi P_{n}\right)$ :
$\mathrm{P}_{\mathrm{u}} / \phi \mathrm{P}_{\mathrm{n}}$ :
Pier Compression Reinforcement Ratio:
$M_{u} / \phi_{B} M_{n}+T_{u} / \phi_{T} T_{n}:$
28.7 k
359.9 k - ACI11.3.1.1
0.08 Result: OK

Diagonal to Pad Edge
162.9 k-ft
1736.8 k-ft - ACl10.3
0.09 Result: OK

Diagonal to Pad Edge
100.2 k-ft
$1790.2 \mathrm{k}-\mathrm{ft}$
0.06 Result: OK
0.0022 OK - Minimum Reinforcement Ratio Met - ACI10.5.1
0.0022 OK - Minimum Reinforcement Ratio Met - ACI10.5.1

11 in - Pad Reinforcing Spacing OK - ACI7.12.2.2 \& 10.5.4
11 in - Pad Reinforcing Spacing OK - ACI7.12.2.2 \& 10.5.4
0.0 k
1983.7 k - ACI11.12.2.1
0.00 Result: OK
530.8 k-ft
2451.3 k-ft
0.22 Result: OK
9.0 k
386.3 k
0.02 Result: OK
0.0005 No Ties Necessary for Shear - ACI11.5.6.1
0.0 k
938.5 k
0.00 Result: OK
0.0 k
7167.7 k - ACII0.3.6.2
0.00 Result: OK
0.004 NG - Increase Pier Steel - ACl10.9.1 \& 10.8.4
0.22 Result: OK

Nominal and Design Moment Capacity and Factored Design Loads


## ASCE 7 Hazards Report

| Standard: | ASCE/SEI 7-10 | Elevation: 378.96 ft (NAVD 88) |
| :--- | :--- | :--- |
| Risk Category: | II | Latitude: 41.095117 |
| Soil Class: | D-Stiff Soil | Longitude: $-\mathbf{7 3 . 6 6 4 2 1 9}$ |



AMERICAN SOCIETY OF CIVIL ENGINEERS

## Seismic

## Site Soil Class:

D - Stiff Soil

## Results:

| $\mathrm{S}_{\mathrm{S}}:$ | 0.259 |
| :--- | :--- |
| $\mathrm{~S}_{1}:$ | 0.071 |
| $\mathrm{~F}_{\mathrm{a}}:$ | 1.593 |
| $\mathrm{~F}_{\mathrm{V}}:$ | 2.4 |
| $\mathrm{~S}_{\mathrm{MS}}:$ | 0.412 |
| $\mathrm{~S}_{\mathrm{M} 1}:$ | 0.169 |

## Seismic Design Category

B

| $\mathrm{S}_{\mathrm{DS}}:$ | 0.275 |
| :--- | :--- |
| $\mathrm{~S}_{\mathrm{D1}}:$ | 0.113 |
| $\mathrm{~T}_{\mathrm{L}}:$ | 6 |
| $\mathrm{PGA}:$ | 0.152 |
| $\mathrm{PGA}:$ | 0.228 |
| $\mathrm{~F}_{\mathrm{PGA}}:$ | 1.495 |
| $\mathrm{I}_{\mathrm{B}}:$ | 1 |




Data Accessed:
Date Source:

Wed Sep 112019
USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-10 Ch. 21 are available from USGS.

## Results:

| Ice Thickness: | 0.75 in. |
| :--- | :--- |
| Concurrent Temperature: | 15 F |

Gust Speed: $\quad 50 \mathrm{mph}$
Data Source:
Date Accessed:
Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8
Wed Sep 112019
Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3 -second gust speeds, for a 50 -year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

ASCE does not intend, nor should anyone interpret, the results provided by this Tool to replace the sound judgment of a competent professional, having knowledge and experience in the appropriate field(s) of practice, nor to substitute for the standard of care required of such professionals in interpreting and applying the contents of this Tool or the ASCE 7 standard.

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| (APPENDIX N) MUNICIPALITY - SPECIFIC STRUCTURAL DESIGN PARAMETERS |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | MCE Spectral Accelerations (\%g) |  | Wind Design Parameters |  |  |  |  |  |  |  |  |
| $\frac{2}{\overline{1}}$ |  |  |  | Ultimate Design Wind Speeds, $V_{\text {ult }}$ (mph) |  |  | Nominal Design Wind Speeds, $V_{\text {asd }}$ (mph) |  |  | Wind-Borne Debris Regions ${ }^{1}$ |  |  |
| N |  | Ss | $\mathrm{S}_{1}$ | Risk Cat.I | Risk Cat.II | Risk Cat III-IV | Risk Cat. I | Risk Cat. II | Risk Cat. <br> III-IV |  |  |  |
| Enfield | 35 | 0.176 | 0.065 | 110 | 125 | 130 | 85 | 97 | 101 |  |  | Yes |
| Essex | 30 | 0.168 | 0.059 | 120 | 135 | 145 | 93 | 105 | 112 |  | Type A | Yes |
| Fairfield | 30 | 0.215 | 0.065 | 115 | 125 | 135 | 89 | 97 | 105 |  | Type B | Yes |
| Farmington | 35 | 0.183 | 0.064 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Franklin | 30 | 0.171 | 0.061 | 120 | 130 | 140 | 93 | 101 | 108 |  | Type A | Yes |
| Glastonbury | 30 | 0.180 | 0.063 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Goshen | 40 | 0.181 | 0.065 | 105 | 115 | 125 | 81 | 89 | 97 |  |  |  |
| Granby | 35 | 0.176 | 0.065 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |
| Greenwich | 30 | 0.259 | 0.070 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |
| Griswold | 30 | 0.168 | 0.060 | 125 | 135 | 145 | 97 | 105 | 112 |  | Type A | Yes |
| Groton | 30 | 0.160 | 0.058 | 125 | 135 | 145 | 97 | 105 | 112 | Type B | Type A | Yes |
| Guilford | 30 | 0.176 | 0.061 | 120 | 130 | 140 | 93 | 101 | 108 |  | Type B | Yes |
| Haddam | 30 | 0.175 | 0.061 | 120 | 130 | 140 | 93 | 101 | 108 |  |  | Yes |
| Hamden | 30 | 0.185 | 0.063 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Hampton | 35 | 0.172 | 0.062 | 120 | 130 | 140 | 93 | 101 | 108 |  |  | Yes |
| Hartford | 30 | 0.181 | 0.064 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Hartland | 40 | 0.175 | 0.065 | 110 | 120 | 125 | 85 | 93 | 97 |  |  | Yes |
| Harwinton | 35 | 0.183 | 0.065 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |
| Hebron | 30 | 0.177 | 0.063 | 120 | 130 | 140 | 93 | 101 | 108 |  |  | Yes |
| Kent | 40 | 0.188 | 0.065 | 105 | 115 | 120 | 81 | 89 | 93 |  |  |  |
| Killingly | 40 | 0.171 | 0.062 | 120 | 130 | 140 | 93 | 101 | 108 |  |  | Yes |
| Killingworth | 30 | 0.173 | 0.061 | 120 | 130 | 140 | 93 | 101 | 108 |  |  | Yes |
| Lebanon | 30 | 0.173 | 0.062 | 120 | 130 | 140 | 93 | 101 | 108 |  |  | Yes |
| Ledyard | 30 | 0.163 | 0.059 | 125 | 135 | 145 | 97 | 105 | 112 |  | Type A | Yes |
| Lisbon | 30 | 0.169 | 0.061 | 125 | 135 | 145 | 97 | 105 | 112 |  | Type A | Yes |
| Litchfield | 40 | 0.184 | 0.065 | 110 | 120 | 125 | 85 | 93 | 97 |  | Type ${ }^{\text {a }}$ | Yes |
| Lyme | 30 | 0.164 | 0.059 | 125 | 135 | 145 | 97 | 105 | 112 |  | Type A | Yes |
| Madison | 30 | 0.173 | 0.060 | 120 | 130 | 140 | 93 | 101 | 108 |  | Type B | Yes |
| Manchester | 30 | 0.178 | 0.064 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Mansfield | 35 | 0.173 | 0.062 | 120 | 130 | 140 | 93 | 101 | 108 |  |  | Yes |
| Marlborough | 30 | 0.177 | 0.062 | 120 | 130 | 140 | 93 | 101 | 108 |  |  | Yes |
| Meriden | 30 | 0.183 | 0.063 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Middlebury | 35 | 0.191 | 0.064 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |
| Middlefield | 30 | 0.181 | 0.063 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Middletown | 30 | 0.180 | 0.063 | 115 | 130 | 135 | 89 | 101 | 105 |  |  | Yes |
| Milford | 30 | 0.194 | 0.063 | 115 | 125 | 135 | 89 | 97 | 105 |  | Type B | Yes |
| Monroe | 30 | 0.205 | 0.065 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |
| Montville | 30 | 0.165 | 0.059 | 125 | 135 | 145 | 97 | 105 | 112 |  | Type A | Yes |
| Morris | 35 | 0.187 | 0.065 | 110 | 120 | 125 | 85 | 93 | 97 |  |  | Yes |
| Naugatuck | 30 | 0.190 | 0.064 | 110 | 125 | 135 | 85 | 97 | 105 |  |  | Yes |
| New Britain | 30 | 0.183 | 0.064 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| New Canaan | 30 | 0.240 | 0.068 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |
| New Fairfield | 35 | 0.212 | 0.067 | 105 | 115 | 125 | 81 | 89 | 97 |  |  |  |
| New Hartford | 40 | 0.180 | 0.065 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |

# RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS 

Sprint Existing Facility
Site ID: CT43XC856
Round Hill Community Ranch
395 Round Hill Road
Greenwich, Connecticut 0683I
October 7, 2019
EBI Project Number: 6219005060

| Site Compliance Summary |  |
| :---: | :---: |
| Compliance Status: | COMPLIANT |
| Site total MPE\% of <br> FCC general <br> population <br> allowable limit: | $\mathbf{1 4 . 9 4 \%}$ |

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October 7, 2019
Sprint
Attn: RF Engineering Manager
I International Boulevard, Suite 800
Mahwah, New Jersey 07495

Emissions Analysis for Site: CT43XC856-Round Hill Community Ranch

EBI Consulting was directed to analyze the proposed Sprint facility located at $\mathbf{3 9 5}$ Round Hill Road in Greenwich, Connecticut for the purpose of determining whether the emissions from the Proposed Sprint Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (\% MPE) as listed in the FCC OET Bulletin 65 Edition 97-Oland ANSI/IEEE Std C95.I. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu \mathrm{W} / \mathrm{cm}^{2}$ ). The number of $\mu \mathrm{W} / \mathrm{cm}^{2}$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits; therefore, it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR I.I307(b)(I) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu \mathrm{W} / \mathrm{cm}^{2}$ ). The general population exposure limits for the 600 MHz and 700 MHz frequency bands are approximately $400 \mu \mathrm{~W} / \mathrm{cm}^{2}$ and $467 \mu \mathrm{~W} / \mathrm{cm}^{2}$, respectively. The general population exposure limit for the $1900 \mathrm{MHz}(\mathrm{PCS}), 2100 \mathrm{MHz}(\mathrm{AWS})$ and II GHz frequency bands is $1000 \mu \mathrm{~W} / \mathrm{cm}^{2}$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.
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Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

## CALCULATIONS

Calculations were done for the proposed Sprint Wireless antenna facility located at 395 Round Hill Road in Greenwich, Connecticut using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since Sprint is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was focused at the base of the tower. For this report, the sample point is the top of a 6 -foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

1) 2 CDMA channels ( 800 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 50 Watts per Channel.
2) 4 PCS channels ( 1900 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
3) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-0I recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
4) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused
environmental | engineering | due diligence
parabolic microwave dishes, was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
5) The antennas used in this modeling are the RFS APXVSPPI8-C-A20 for the $800 \mathrm{MHz} / 1900$ MHz channel(s) in Sector A, the RFS APXVSPPI8-C-A20 for the $800 \mathrm{MHz} / 1900 \mathrm{MHz}$ channel(s) in Sector B, the RFS APXVSPPI8-C-A20 for the $800 \mathrm{MHz} / 1900 \mathrm{MHz}$ channel(s) in Sector C. This is based on feedback from the carrier with regard to anticipated antenna selection. All Antenna gain values and associated transmit power levels are shown in the Site Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
6) The antenna mounting height centerline of the proposed antennas is 100 feet above ground level (AGL).
7) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
8) All calculations were done with respect to uncontrolled / general population threshold limits.
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## Sprint Site Inventory and Power Data

| Sector: | A | Sector: | B | Sector: | C |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Antenna \#: | I | Antenna \#: | I | Antenna \#: | I |
| Make / Model: | RFS APXVSPPI8-C-A20 | Make / Model: | RFS APXVSPPI8-C-A20 | Make / Model: | RFS APXVSPPI8-C-A20 |
| Frequency Bands: | $800 \mathrm{MHz} / 1900 \mathrm{MHz}$ | Frequency Bands: | $800 \mathrm{MHz} / 1900 \mathrm{MHz}$ | Frequency Bands: | $800 \mathrm{MHz} / 1900 \mathrm{MHz}$ |
| Gain: | 13.4 dBd / 15.9 dBd | Gain: | 13.4 dBd / 15.9 dBd | Gain: | 13.4 dBd / 15.9 dBd |
| Height (AGL): | 100 feet | Height (AGL): | 100 feet | Height (AGL): | 100 feet |
| Channel Count: | 6 | Channel Count: | 6 | Channel Count: | 6 |
| Total TX Power (W): | 260 Watts | Total TX Power (W): | 260 Watts | Total TX Power (W): | 260 Watts |
| ERP (W): | 8,412.48 | ERP (W): | 8,412.48 | ERP (W): | 8,412.48 |
| Antenna AI MPE \%: | $3.71 \%$ | Antenna BI MPE \%: | $3.71 \%$ | Antenna CI MPE \%: | $3.71 \%$ |

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| Site Composite MPE \% |  |
| :---: | :---: |
| Carrier | MPE \% |
| Sprint (Max at Sector A): | $3.71 \%$ |
| AT\&T | $4.89 \%$ |
| T-Mobile | $2.58 \%$ |
| Verizon | $3.76 \%$ |
| Site Total MPE \%: | $14.94 \%$ |


| Sprint MPE \% Per Sector |  |
| ---: | :---: |
| Sprint Sector A Total: | $3.71 \%$ |
| Sprint Sector B Total: | $3.71 \%$ |
| Sprint Sector C Total: | $3.71 \%$ |
|  |  |
| Site Total MPE \% : | $14.94 \%$ |

## Sprint Maximum MPE Power Values (Sector A)

| Sprint Frequency Band / <br> Technology <br> (Sector A) | $\#$ <br> Channels | Watts ERP <br> (Per <br> Channel) | Height <br> (feet) | Total Power <br> Density <br> $\left(\boldsymbol{\mu W} / \mathbf{c m}^{2}\right)$ | Frequency <br> (MHz) | Allowable MPE <br> $\left(\boldsymbol{\mu W} / \mathbf{c m}^{2}\right)$ | Calculated \% MPE |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Sprint 800 MHz CDMA | 2 | 1093.88 | 100.0 | 7.87 | 800 MHz CDMA | 533 |  |
| Sprint 1900 MHz PCS | 4 | 1556.18 | 100.0 | 22.38 | 1900 MHz PCS | 1000 | $1.48 \%$ |
| $2.24 \%$ | Total: | $3.71 \%$ |  |  |  |  |  |

- NOTE: Totals may vary by approximately $0.01 \%$ due to summation of remainders in calculations.
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## Summary

All calculations performed for this analysis yielded results that were within the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the Sprint facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

| Sprint Sector | Power Density Value (\%) |
| :---: | :---: |
| Sector A: | $3.71 \%$ |
| Sector B: | $3.71 \%$ |
| Sector C: | $3.71 \%$ |
| Sprint Maximum MPE <br> \% (Sector A): | $3.71 \%$ |
| Site Total: |  |
|  |  |
| Site Compliance Status: | COMPLIANT |

The anticipated composite MPE value for this site assuming all carriers present is $\mathbf{I 4 . 9 4 \%}$ of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a $5 \%$ contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable $100 \%$ threshold standard per the federal government.

DOCKET NO. 309 - Cellco Partnership d/b/a Verizon Wireless application for a Certificate of Environmental Compatibility and Public Need for the construction, maintenance, and operation of a wireless telecommunications facility located at the Round Hill Community Church, 395 Round Hill Road, Greenwich, \} Connecticut.

Connecticut
Siting
Council
February 6, 2007

## Decision and Order

Pursuant to the foregoing Findings of Fact and Opinion, the Connecticut Siting Council (Council) finds that the effects associated with the construction, operation, and maintenance of a telecommunications facility, including effects on the natural environment; ecological integrity and balance; public health and safety; scenic, historic, and recreational values; forests and parks; air and water purity; and fish and wildlife are not disproportionate, either alone or cumulatively with other effects, when compared to need, are not in conflict with the policies of the State concerning such effects, and are not sufficient reason to deny the application, and therefore directs that a Certificate of Environmental Compatibility and Public Need, as provided by General Statutes $\S 16-50 \mathrm{k}$, be issued to Cellco Partnership d/b/a Verizon Wireless, hereinafter referred to as the Certificate Holder, for a telecommunications facility at 395 Round Hill Road Greenwich, Connecticut.

The facility shall be constructed, operated, and maintained substantially as specified in the Council's record in this matter, and subject to the following conditions:

1. The facility shall be constructed as a two-monopole facility with internally mounted antennas, no taller than necessary to provide the proposed telecommunications services, sufficient to accommodate the antennas of Cellco Partnership $\mathrm{d} / \mathrm{b} / \mathrm{a}$ Verizon Wireless and other entities, both public and private, but such towers shall not exceed a height of 115 feet above ground level.
2. The Certificate Holder shall prepare a Development and Management (D\&M) Plan for this site in compliance with Sections 16-50j-75 through 16-50j-77 of the Regulations of Connecticut State Agencies. The D\&M Plan shall be served on the Town of Greenwich for comment, and all parties and intervenors as listed in the service list, and submitted to and approved by the Council prior to the commencement of facility construction and shall include:
a) a final site plan(s) of site development to include specifications for the towers, tower foundations, antennas, equipment compound, radio equipment, access road, utility line, and landscaping; and
b) construction plans for site clearing, water drainage, and erosion and sedimentation control consistent with the 2002 Connecticut Guidelines for Soil Erosion and Sediment Control, as amended.
3. The Certificate Holder shall, prior to the commencement of operation, provide the Council worst-case modeling of electromagnetic radio frequency power density of all proposed entities' antennas at the closest point of uncontrolled access to the base of the facility, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin No. 65, August 1997. The Certificate Holder shall ensure a recalculated report of electromagnetic radio frequency power density is submitted to the Council if and when circumstances in operation cause a change in power density above the levels calculated and provided pursuant to this Decision and Order.
4. Upon the establishment of any new State or federal radio frequency standards applicable to frequencies of this facility, the facility granted herein shall be brought into compliance with such standards.
5. The Certificate Holder shall permit public or private entities to share space on the proposed facility for fair consideration, or shall provide any requesting entity with specific legal, technical, environmental, or economic reasons precluding such tower sharing.
6. The Certificate Holder shall provide reasonable space within one of the towers for no compensation for any Town of Greenwich public safety services (police, fire and medical services), provided such use can be accommodated and is compatible with the structural integrity of the tower.
7. If the facility authorized herein is not fully constructed and providing wireless services within eighteen months from the date of the mailing of the Council's Findings of Fact, Opinion, and Decision and Order (collectively called "Final Decision"), this Decision and Order shall be void, and the Certificate Holder shall dismantle both towers and remove all associated equipment or reapply for any continued or new use to the Council before any such use is made. The time between the filing and resolution of any appeals of the Council's Final Decision shall not be counted in calculating this deadline.
8. If the one or both towers of the facility ceases to provide wireless services for a period of one year, this Decision and Order shall be void, and the Certificate Holder shall dismantle the unused portion of the facility and remove all associated equipment or reapply for any continued or new use to the Council before any such use is made.
9. The Certificate Holder shall remove any nonfunctioning antenna, and associated antenna mounting equipment, within 60 days of the date the antenna ceased to function.
10. Any request for extension of the time periods referred to in Conditions $7 \& 8$ shall be filed with the Council not later than sixty days prior to the expiration date of this Certificate and shall be served on all parties and intervenors, as listed in the service list, and the Town of Greenwich. Any proposed modifications to this Decision and Order shall likewise be so served.
11. In accordance with Section 16-50j-77 of the Regulations of Connecticut State Agencies, the Certificate Holder shall provide the Council with written notice two weeks prior to the commencement of site construction activities. In addition, the Certificate Holder shall provide the Council with written notice of the completion of site construction and the commencement of site operation.

Pursuant to General Statutes § $16-50$ p, the Council hereby directs that a copy of the Findings of Fact, Opinion, and Decision and Order be served on each person listed below, and notice of issuance shall be published in The Greenwich Time.

By this Decision and Order, the Council disposes of the legal rights, duties, and privileges of each party named or admitted to the proceeding in accordance with Section 16-50j-17 of the Regulations of Connecticut State Agencies.

Docket No. 309
Decision and Order
Page 3
The parties and intervenors to this proceeding are:
Applicant
Cellco Partnership d/b/a
Verizon Wireless

## Representatives

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[^0]:    *Deflection and Sway was evaluated considering a design wind speed of 60 mph (3-Second Gust) per ANSI/TIA-222-G

