



**Crown Castle**  
3530 Toringdon Way Suite 300  
Charlotte NC 28277

Tel (704) 405-6600

March 23, 2015

Melanie A. Bachman  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

**RE: T-Mobile-Exempt Modification - Crown Site BU: 841290**  
**T-Mobile Site ID: CT11069A**  
**Located at: 363 Riversville Road, Greenwich, CT 06831**

Dear Ms. Bachman:

This letter and exhibits are submitted on behalf of T-Mobile. T-Mobile is making modifications to certain existing sites in its Connecticut system in order to implement their 700MHz technology. Please accept this letter and exhibits as notification, pursuant to § 16-50j-73 of the Regulations of Connecticut State Agencies (“R.C.S.A.”), of construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In compliance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Mr. Peter Tesei, First Selectman for the Town of Greenwich and Greenwich Council Boy Scouts of America, Property Owner.

T-Mobile plans to modify the existing wireless communications facility owned by Crown Castle and located at **363 Riversville Road, Greenwich, CT 06831**. Attached are a compound plan and elevation depicting the planned changes (Exhibit-1), and documentation of the structural sufficiency of the structure to accommodate the revised antenna configuration (Exhibit-2). Also included is a power density table report reflecting the modification to T-Mobile’s operations at the site (Exhibit-3).

The changes to the facility do not constitute a modification as defined in Connecticut General Statutes (“C.G.S.”) § 16-50i(d) because the general physical characteristics of the facility will not be significantly changed. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in the R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing tower. T-Mobile’s replacement antennas will be located at the same elevation on the existing tower.
2. There will be no proposed modifications to the ground and no extension of boundaries.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more.

4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative General Power Density table report for T-Mobile's modified facility is included as Exhibit-3.
5. A Structural Modification Report confirming that the tower and foundation can support T-Mobile's proposed modifications is included as Exhibit-2.

For the foregoing reasons, T-Mobile respectfully submits the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

Jerry Feathers  
Real Estate Specialist

Enclosure

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes

Tab 2: Exhibit-2: Structural Modification Report

Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: Mr. Peter Tesei, First Selectman  
Town of Greenwich  
101 Field Point Road  
Greenwich, CT 06830

cc: Greenwich Council Boy Scouts of America  
63 Mason Street  
Greenwich, CT 06830

















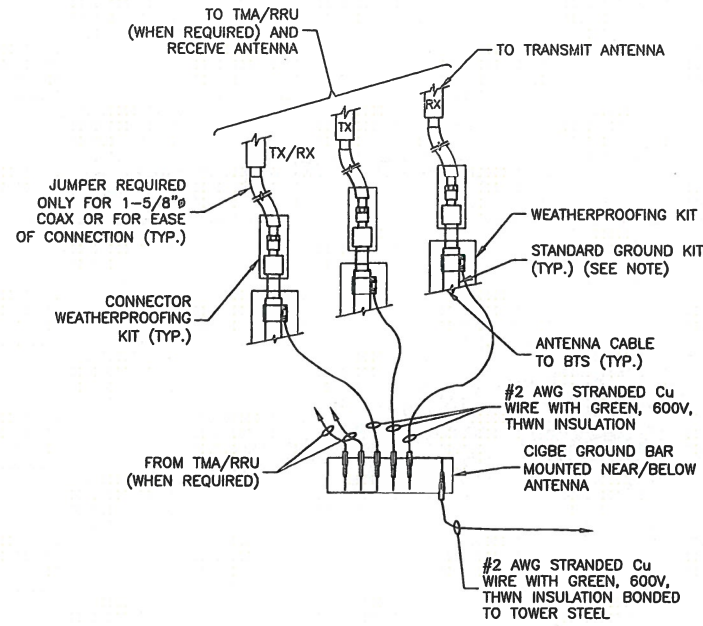






**GROUNDING NOTES:**

1. THE CONTRACTOR SHALL REVIEW AND INSPECT THE EXISTING FACILITY GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRICT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ), THE SITE-SPECIFIC (UL, LPI, OR NFPA) LIGHTNING PROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND TIA GROUNDING STANDARDS. THE CONTRACTOR SHALL REPORT ANY VIOLATIONS OR ADVERSE FINDINGS TO THE ENGINEER FOR RESOLUTION.
2. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GE'S) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS. ALL AVAILABLE GROUNDING ELECTRODES SHALL BE CONNECTED TOGETHER IN ACCORDANCE WITH THE NEC.
3. THE CONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND B1) FOR GROUND ELECTRODE SYSTEMS. USE OF OTHER METHODS MUST BE PRE-APPROVED BY THE ENGINEER IN WRITING.
4. THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS ON TOWER SITES AND 10 OHMS OR LESS ON ROOFTOP SITES. WHEN ADDING ELECTRODES, CONTRACTOR SHALL MAINTAIN A MINIMUM DISTANCE BETWEEN THE ADDED ELECTRODE AND ANY OTHER EXISTING ELECTRODE EQUAL TO THE BURIED LENGTH OF THE ROD. IDEALLY, CONTRACTOR SHALL STRIVE TO KEEP THE SEPARATION DISTANCE EQUAL TO TWICE THE BURIED LENGTH OF THE RODS.
5. THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT.
6. METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH 8 AWG COPPER WIRE AND UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
7. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO TRANSMISSION EQUIPMENT.
8. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED. BACK-TO-BACK CONNECTIONS ON OPPOSITE SIDES OF THE GROUND BUS ARE PERMITTED.
9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
10. USE OF 90° BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45° BENDS CAN BE ADEQUATELY SUPPORTED. IN ALL CASES, BENDS SHALL BE MADE WITH A MINIMUM BEND RADIUS OF 8 INCHES.
11. EACH INTERIOR TRANSMISSION CABINET FRAME/PLINTH SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH 8 AWG STRANDED GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRE UNLESS NOTED OTHERWISE IN THE DETAILS. EACH OUTDOOR CABINET FRAME/PLINTH SHALL BE DIRECTLY CONNECTED TO THE BURIED GROUND RING WITH 2 AWG SOLID TIN-PLATED COPPER WIRE UNLESS NOTED OTHERWISE IN THE DETAILS.
12. ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING, SHALL BE 2 AWG SOLID TIN-PLATED COPPER UNLESS OTHERWISE INDICATED.
13. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE. CONNECTIONS TO ABOVE GRADE UNITS SHALL BE MADE WITH EXOTHERMIC WELDS WHERE PRACTICAL OR WITH 2 HOLE MECHANICAL TYPE BRASS CONNECTORS WITH STAINLESS STEEL HARDWARE, INCLUDING SET SCREWS. HIGH PRESSURE CRIMP CONNECTORS MAY ONLY BE USED WITH WRITTEN PERMISSION FROM T-MOBILE MARKET REPRESENTATIVE.
14. EXOTHERMIC WELDS SHALL BE PERMITTED ON TOWERS ONLY WITH THE EXPRESS APPROVAL OF THE TOWER MANUFACTURER OR THE CONTRACTORS STRUCTURAL ENGINEER.
15. ALL WIRE TO WIRE GROUND CONNECTIONS TO THE INTERIOR GROUND RING SHALL BE FORMED USING HIGH PRESS CRIMPS OR SPLIT BOLT CONNECTORS WHERE INDICATED IN THE DETAILS.
16. ON ROOFTOP SITES WHERE EXOTHERMIC WELDS ARE A FIRE HAZARD COPPER COMPRESSION CAP CONNECTORS MAY BE USED FOR WIRE TO WIRE CONNECTIONS. 2 HOLE MECHANICAL TYPE BRASS CONNECTORS WITH STAINLESS STEEL HARDWARE, INCLUDING SET SCREWS SHALL BE USED FOR CONNECTION TO ALL ROOFTOP TRANSMISSION EQUIPMENT AND STRUCTURAL STEEL.
17. COAX BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR USING TWO-HOLE MECHANICAL TYPE BRASS CONNECTORS AND STAINLESS STEEL HARDWARE.
18. APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
19. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.
20. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
21. BOND ALL METALLIC OBJECTS WITHIN 8 FT OF THE BURIED GROUND RING WITH 2 AWG SOLID TIN-PLATED COPPER GROUND CONDUCTOR. DURING EXCAVATION FOR NEW GROUND CONDUCTORS, IF EXISTING GROUND CONDUCTORS ARE ENCOUNTERED, BOND EXISTING GROUND CONDUCTORS TO NEW CONDUCTORS.
22. GROUND CONDUCTORS USED IN THE FACILITY GROUND AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC PLASTIC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (E.G., NON-METALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT WITH LISTED BONDING FITTINGS.

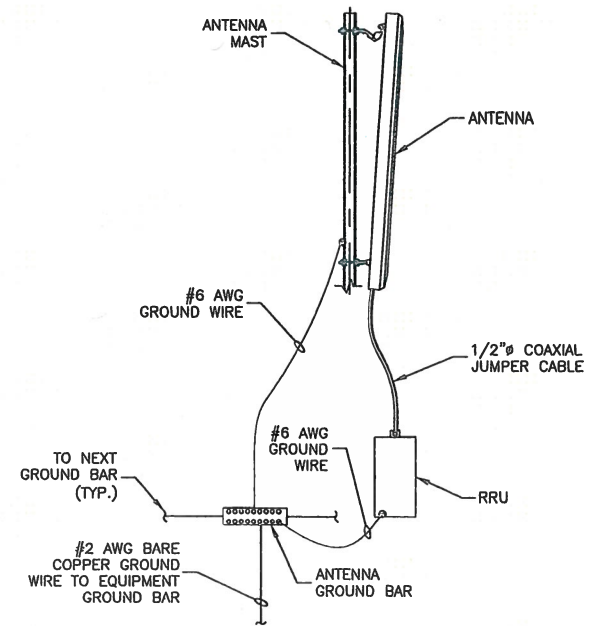


- NOTE:**
1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO CIGBE.

**CONNECTION OF GROUND WIRES TO GROUNDING BAR (CIGBE)**

SCALE: N.T.S.

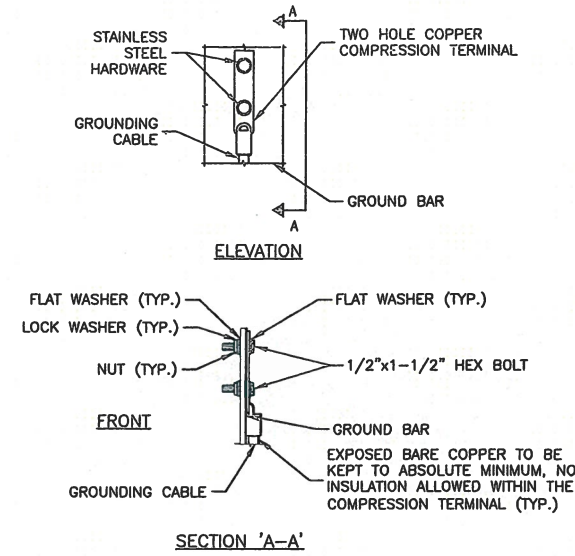
1



**TYPICAL ANTENNA GROUNDING DETAIL**

SCALE: N.T.S.

3

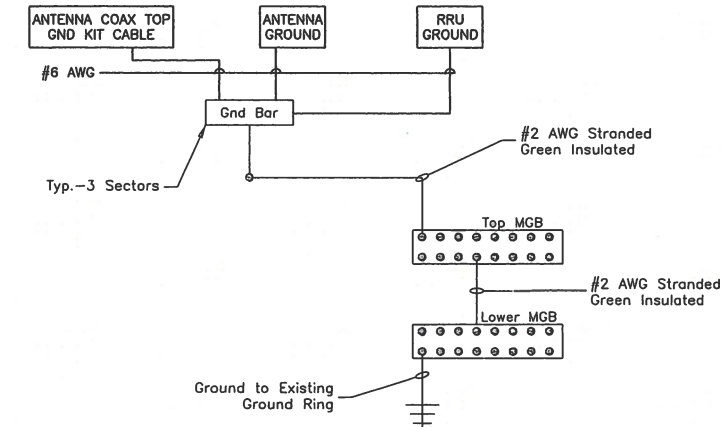


- NOTES:**
1. DOUBLING UP OR STACKING OF CONNECTIONS IS NOT PERMITTED.
  2. OXIDE INHIBITING COMPOUND TO BE USED AT ALL LOCATIONS.

**TYPICAL GROUND BAR MECHANICAL CONNECTION DETAIL**

SCALE: N.T.S.

2



- NOTES:**
1. BOND ANTENNA GROUNDING KIT CABLE TO TOP CIGBE
  2. BOND ANTENNA GROUNDING KIT CABLE TO BOTTOM CIGBE.
  3. SCHEMATIC GROUNDING DIAGRAM IS TYPICAL FOR EACH SECTOR.
  4. VERIFY EXISTING GROUND SYSTEM IS INSTALLED PER T-MOBILE STANDARDS.

**SCHEMATIC GROUNDING DIAGRAM**

SCALE: N.T.S.

4



T-MOBILE NORTHEAST LLC  
4 SYLVAN WAY  
PARSIPPANY, NJ 07054



CROWN CASTLE  
500 WEST CUMMINGS PARK, SUITE 3600  
WOBURN, MA 01801

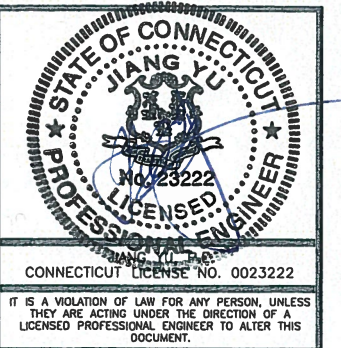
**CT11069A GREENWICH NORTH**

**CONSTRUCTION DRAWINGS**

0	03/20/15 ISSUED AS FINAL
A	03/17/15 ISSUED FOR REVIEW



**Dewberry Engineers Inc.**  
600 PARSIPPANY ROAD  
SUITE 301  
PARSIPPANY, NJ 07054  
PHONE: 973.739.9400  
FAX: 973.739.9710



DRAWN BY:	JC
REVIEWED BY:	BSH
CHECKED BY:	GHN
PROJECT NUMBER:	50066258
JOB NUMBER:	50072421
SITE ADDRESS:	

363 RIVERSVILLE ROAD  
GREENWICH, CT 06831  
FAIRFIELD COUNTY

SHEET TITLE

GROUNDING NOTES & DETAILS

SHEET NUMBER





Date: **February 27, 2015**

Sean Dempsey  
Crown Castle  
3530 Toringdon Way Suite 300  
Charlotte, NC 28277  
(704) 405-6565

GPD Group  
520 South Main St, Suite 2531  
Akron, OH 44311  
(614) 859-1607  
[dpalkovic@gpdgroup.com](mailto:dpalkovic@gpdgroup.com)

**Subject:** **Structural Analysis Report**

**Carrier Designation:** **T-Mobile Co-Locate**  
**Carrier Site Number:** CT11069A  
**Carrier Site Name:** Greenwich/Boy Scouts\_2

**Crown Castle Designation:** **Crown Castle BU Number:** 841290  
**Crown Castle Site Name:** GREENWICH NORTH  
**Crown Castle JDE Job Number:** 324170  
**Crown Castle Work Order Number:** 1014237  
**Crown Castle Application Number:** 284770 Rev. 0

**Engineering Firm Designation:** **GPD Group Project Number:** 2015777.841290.02

**Site Data:** **363 Riversville Road, Greenwich, CT 06831, Fairfield County**  
**Latitude 41° 3' 58.6", Longitude -73° 40' 17.4"**  
**160 Foot – EEI Monopole Tower**

Dear Sean Dempsey,

GPD Group is pleased to submit this “**Structural Analysis Report**” to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural ‘Statement of Work’ and the terms of Crown Castle Purchase Order Number 759639, in accordance with application 284770, revision 0.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment **Sufficient Capacity**  
Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

The analysis has been performed in accordance with the TIA/EIA-222-F standard and the 2005 Connecticut (CT) State Building Code based upon a wind speed of 85 mph fastest mile.

We at GPD Group appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Structural analysis prepared by: Kevin Liccar, EI

Respectfully submitted by:

John N. Kabak, P.E.  
Connecticut #: PEN.0028336





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## 1) INTRODUCTION

The existing 160' monopole consists of five major sections that are connected with slip joints and a bolted flange connection at the 152' elevation. It has an 18-sided cross section and is evenly tapered from 59" (flat-flat) at the base to 29" (flat-flat) at the top. The structure is galvanized and does not have aviation lighting.

The tower was designed for the URS Corporation by Engineered Endeavors, Incorporated (EEI) of Mentor, Ohio in April of 2003. The tower was also designed for a basic wind speed of 85 mph with ½" radial ice (with a 25% reduction in wind speed when wind and ice loads were considered simultaneously) in accordance with the TIA/EIA-222-F standard.

## 2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures and the 2005 Connecticut (CT) State Building using a fastest mile wind speed of 85 mph with no ice, 38 mph with 0.75 inch ice thickness (in accordance with ASCE 7 ice conditions) and 50 mph under service loads.

**Table 1 - Proposed Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
160.0	163.0	3	Commscope	LNx-6515DS-VTM			
		3	Ericsson	RRUS 11 B12			



**Table 2 - Existing and Reserved Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
160.0	163.0	2	Communication Components Inc.	DTMA-1819-DD-12	13	1-5/8	1
		1	RFS Celwave	ATMAA1412D-1A20			
		3	Ericsson	ERICSSON AIR 21 B2A B4P			
		3	Ericsson	ERICSSON AIR 21 B4A B2P			
		3	RFS Celwave	ATMAA1412D-1A20			
	1		Platform Mount [LP 1201-1]				
153.0	153.0	3	Ericsson	TME-RRUS-11	2	3/8	
		1	Raycap	TME-DC6-48-60-18-8F			
		1		Side Arm Mount [SO 102-3]			
149.0	151.0	6	Powerwave Tech.	7770.00	12	1-5/8	
		6	Powerwave Tech.	LGP21401			
	149.0	6	Powerwave Tech.	LGP21401			
		3	Powerwave Tech.	P65-16-XLH-RR			
		1		Platform Mount [LP 1201-1]			
140.0	142.0	6	Amphenol	WWX063X19G00	2	1-5/8	2
		1	Antel	BXA-70063/8CFx2			
		1	RFS Celwave	DB-T1-6Z-8AB-0Z			
		2	Decibel	DB844H80E-XY			
		2	Powerwave Tech.	P65-16-XL-M			
		4	RFS Celwave	APL868013-42T0			
	140.0	1		Platform Mount [LP 1201-1]	18	1-5/8	
		3	Alcatel Lucent	TME-RRH2x60 AWS			
120.0	122.0	3	Alcatel Lucent	TD-RRH8x20-25	3 2	1-1/4 1/2	2
		3	RFS Celwave	APXVTM14-C-120			
		1		Handrail Kit [NA 510-1]			
		3	RFS Celwave	APXVSPP18-C-A20			
	120.0	1		Platform Mount [LP 1201-1]			
119.0	119.0	3	Alcatel Lucent	TME-1900MHz RRH			
		3	Alcatel Lucent	TME-800MHZ RRH			
		1		Side Arm Mount [SO 102-3]			
72.0	73.0	2	GPS	GPS_A	2	1/2	
	72.0	1		Side Arm Mount [SO 701-1]			

- Notes:  
 1) Existing equipment to be removed; not considered in this analysis  
 2) Reserved Equipment



**Table 3 - Design Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
160	160	3		Panel Antenna		
150	150	1		Low Profile Platform		
		12	Allgon	ALP 11011		
140	140	1		Low Profile Platform		
		12	Allgon	ALP 11011		
130	130	1		Low Profile Platform		
		12	Allgon	ALP 11011		

### 3) ANALYSIS PROCEDURE

**Table 4 - Documents Provided**

Document	Remarks	Reference	Source
Tower Drawings / Specifications	EEl Project #: 5590, Dated 04/10/2003	Doc ID #: 5121537	CCIsites
Foundation Drawings / Specifications	EEl Project #: 5590, Dated 04/10/2003	Doc ID #: 5121536	CCIsites
Foundation Exploration Report	WEI Project #: 2009-895, Dated 09/04/2009	Doc ID #: 4468638	CCIsites
Geotechnical Report	WEI Project #: 2009-895, Dated 09/04/2009	Doc ID #: 5121535	CCIsites

#### 3.1) Analysis Method

tnxTower (version 6.1.4.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

#### 3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by TIA/EIA-222-F.

This analysis may be affected if any assumptions are not valid or have been made in error. GPD Group should be notified to determine the effect on the structural integrity of the tower.



#### 4) ANALYSIS RESULTS

**Table 5 - Section Capacity (Summary)**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	160 - 152	Pole	TP30.62x29x0.1875	1	-3.56	908.48	10.9	Pass
L2	152 - 111.29	Pole	TP38.86x30.62x0.25	2	-16.35	1547.47	59.3	Pass
L3	111.29 - 77.42	Pole	TP45.09x37.263x0.3125	3	-23.23	2245.56	82.1	Pass
L4	77.42 - 36.46	Pole	TP52.62x43.2359x0.4375	4	-35.42	3665.31	76.9	Pass
L5	36.46 - 0	Pole	TP59x50.3353x0.5	5	-52.21	4826.45	77.6	Pass
							Summary	
						Pole (L3)	82.1	Pass
						Rating =	82.1	Pass

**Table 6 - Tower Component Stresses vs. Capacity – LC7**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	66.9	Pass
1	Base Plate	0	84.2	Pass
1	Base Foundation	0	25.0	Pass
1	Base Foundation Soil Interaction	0	53.8	Pass
1	Flange Bolts	152	14.2	Pass
1	Flange Plate	152	13.0	Pass

<b>Structure Rating (max from all components) =</b>	<b>84.2%</b>
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Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

#### 4.1) Recommendations

The existing tower and its foundation are sufficient for the proposed loading and do not require modifications.



## 5) DISCLAIMER OF WARRANTIES

GPD GROUP has not performed a site visit to the tower to verify the member sizes or antenna/coax loading. If the existing conditions are not as represented on the tower elevation contained in this report, we should be contacted immediately to evaluate the significance of the discrepancy. This is not a condition assessment of the tower or foundation. This report does not replace a full tower inspection. The tower and foundations are assumed to have been properly fabricated, erected, maintained, in good condition, twist free, and plumb.

The engineering services rendered by GPD GROUP in connection with this Structural Analysis are limited to a computer analysis of the tower structure and theoretical capacity of its main structural members. No allowance was made for any damaged, bent, missing, loose, or rusted members (above and below ground). No allowance was made for loose bolts or cracked welds.

This analysis is limited to the designated maximum wind and seismic conditions per the governing tower standards and code. Wind forces resulting in tower vibrations near the structure's resonant frequencies were not considered in this analysis and are outside the scope of this analysis. Lateral loading from any dynamic response was not evaluated under a time-domain based fatigue analysis.

GPD GROUP does not analyze the fabrication of the structure (including welding). It is not possible to have all the very detailed information needed to perform a thorough analysis of every structural sub-component and connection of an existing tower. GPD GROUP provides a limited scope of service in that we cannot verify the adequacy of every weld, plate connection detail, etc. The purpose of this report is to assess the capability of adding appurtenances usually accompanied by transmission lines to the structure.

It is the owner's responsibility to determine the amount of ice accumulation in excess of the code specified amount, if any, that should be considered in the structural analysis.

The attached sketches are a schematic representation of the analyzed tower. If any material is fabricated from these sketches, the contractor shall be responsible for field verifying the existing conditions, proper fit, and clearance in the field. Any mentions of structural modifications are reasonable estimates and should not be used as a precise construction document. Precise modification drawings are obtainable from GPD GROUP, but are beyond the scope of this report.

Miscellaneous items such as antenna mounts, etc., have not been designed or detailed as a part of our work. We recommend that material of adequate size and strength be purchased from a reputable tower manufacturer.

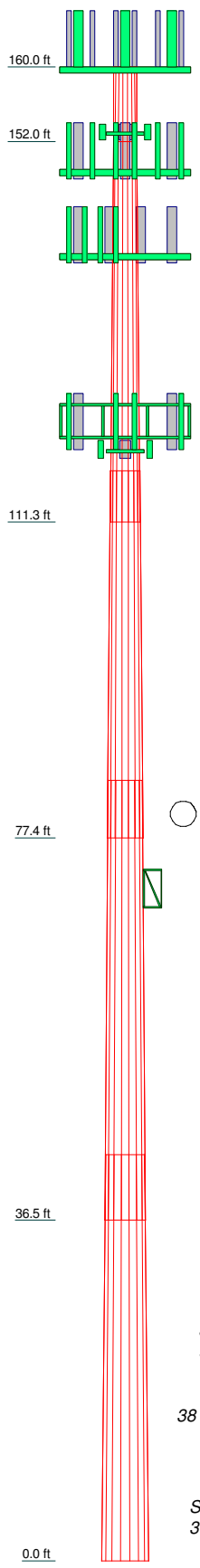
Towers are designed to carry gravity, wind, and ice loads. All members, legs, diagonals, struts, and redundant members provide structural stability to the tower with little redundancy. Absence or removal of a member can trigger catastrophic failure unless a substitute is provided before any removal. Legs carry axial loads and derive their strength from shorter unbraced lengths by the presence of redundant members and their connection to the diagonals with bolts or welds. If the bolts or welds are removed without providing any substitute to the frame, the leg is subjected to a higher unbraced length that immediately reduces its load carrying capacity. If a diagonal is also removed in addition to the connection, the unbraced length of the leg is greatly increased, jeopardizing its load carrying capacity. Failure of one leg can result in a tower collapse because there is no redundancy. Redundant members and diagonals are critical to the stability of the tower.

GPD GROUP makes no warranties, expressed and/or implied, in connection with this report and disclaims any liability arising from material, fabrication, and erection of this tower. GPD GROUP will not be responsible whatsoever for, or on account of, consequential or incidental damages sustained by any person, firm, or organization as a result of any data or conclusions contained in this report. The maximum liability of GPD GROUP pursuant to this report will be limited to the total fee received for preparation of this report.

**APPENDIX A**  
**BASE LEVEL DRAWING**



Section	1	2	3	4	5
Length (ft)	8.00	40.71	39.29	47.13	43.54
Number of Sides	18	18	18	18	18
Thickness (in)	0.1875	0.2500	0.3125	0.4375	0.5000
Socket Length (ft)		5.42	6.17	7.08	50.3353
Top Dia (in)	29.0000	30.6200	37.2630	43.2359	50.0000
Bot Dia (in)	30.6200	38.8600	45.0900	52.6200	59.0000
Grade			A572-65		
Weight (K)	0.5	3.8	5.4	10.6	12.7



**DESIGNED APPURTENANCE LOADING**

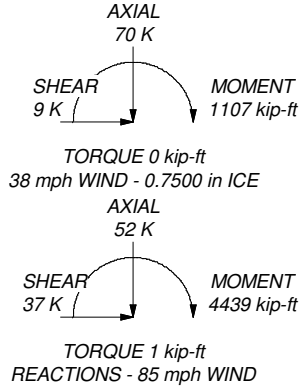
TYPE	ELEVATION	TYPE	ELEVATION
Platform Mount [LP 1201-1]	160	Platform Mount [LP 1201-1]	140
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	160	P65-16-XL-M w/ Mount Pipe	140
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	160	P65-16-XL-M w/ Mount Pipe	140
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	160	(2) APL868013-42T0 w/ Mount Pipe	140
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	160	(2) APL868013-42T0 w/ Mount Pipe	140
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	160	TME-RRH2x60-PCS	140
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	160	TME-RRH2x60-PCS	140
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	160	TME-RRH2x60-PCS	140
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	160	TME-RRH2x60 AWS	140
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	160	TME-RRH2x60 AWS	140
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	160	TME-RRH2x60 AWS	140
ATMAA1412D-1A20	160	(2) WWX063X19G00 w/ Mount Pipe	140
ATMAA1412D-1A20	160	(2) WWX063X19G00 w/ Mount Pipe	140
ATMAA1412D-1A20	160	(2) WWX063X19G00 w/ Mount Pipe	140
Pipe Mount 6x2.375"	160	DB-T1-6Z-8AB-0Z	140
Pipe Mount 6x2.375"	160	BXA-70063/8CFx2 w/ Mount Pipe	140
Pipe Mount 6x2.375"	160	(2) DB844H80E-XY w/ Mount Pipe	140
Pipe Mount 6x2.375"	160	Handrail Kit [NA 510-1]	122
LNX-6515DS-VTM w/ Mount Pipe	160	Platform Mount [LP 1201-1]	120
LNX-6515DS-VTM w/ Mount Pipe	160	APXVSP18-C-A20 w/ Mount Pipe	120
LNX-6515DS-VTM w/ Mount Pipe	160	APXVSP18-C-A20 w/ Mount Pipe	120
RRUS 11 B12	160	APXVSP18-C-A20 w/ Mount Pipe	120
RRUS 11 B12	160	(2) 6' x 2" Mount Pipe	120
RRUS 11 B12	160	(2) 6' x 2" Mount Pipe	120
Side Arm Mount [SO 102-3]	153	(2) 6' x 2" Mount Pipe	120
TME-RRUS-11	153	APXVTM14-C-120 w/ Mount Pipe	120
TME-RRUS-11	153	APXVTM14-C-120 w/ Mount Pipe	120
TME-RRUS-11	153	APXVTM14-C-120 w/ Mount Pipe	120
TME-DC6-48-60-18-8F	153	TD-RRH8x20-25	120
Platform Mount [LP 1201-1]	149	TD-RRH8x20-25	120
(2) 7770.00 w/ Mount Pipe	149	TD-RRH8x20-25	120
(2) 7770.00 w/ Mount Pipe	149	Side Arm Mount [SO 102-3]	119
P65-16-XLH-RR w/ mount pipe	149	TME-1900MHz RRH w/ 4' x 2" Mount Pipe	119
P65-16-XLH-RR w/ mount pipe	149	TME-1900MHz RRH w/ 4' x 2" Mount Pipe	119
P65-16-XLH-RR w/ mount pipe	149	TME-1900MHz RRH w/ 4' x 2" Mount Pipe	119
(2) LGP21401	149	TME-800MHZ RRH	119
(2) LGP21401	149	TME-800MHZ RRH	119
(2) LGP21401	149	TME-800MHZ RRH	119
(2) LGP21401	149	TME-800MHZ RRH	119
(2) LGP21401	149	Side Arm Mount [SO 701-1]	72
(2) LGP21401	149	(2) GPS_A	72

**MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

**TOWER DESIGN NOTES**

1. Tower is located in Fairfield County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 38 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 82.1%

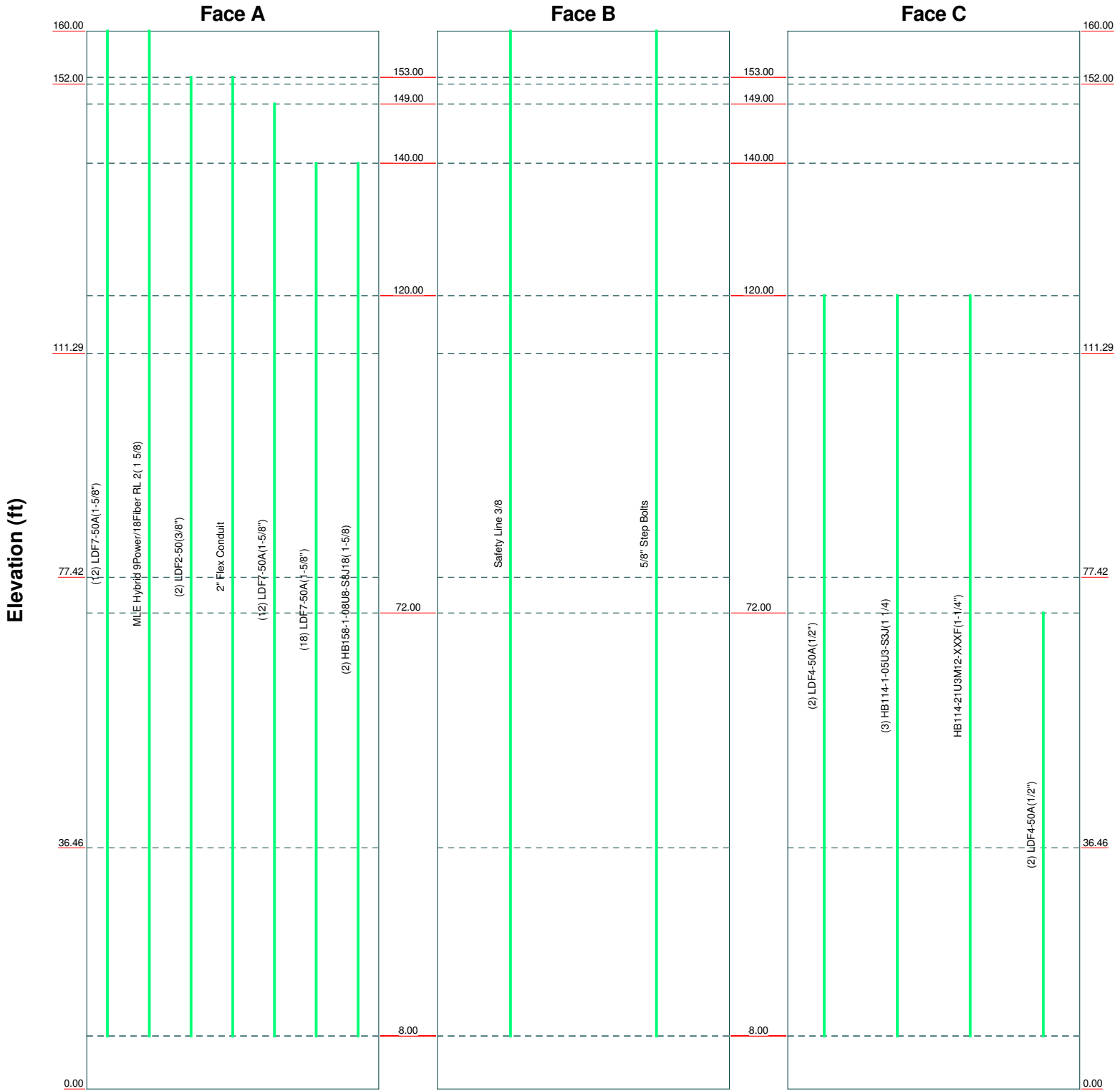


**GPD Group**  
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 Akron, OH 44311  
 Phone: (330) 572-2100  
 FAX: (330) 572-2101

Job: **GREENWICH NORTH (BU #: 841290)**  
 Project: **2015777.841290.02**  
 Client: Crown Castle USA, Inc. Drawn by: kliccar App'd:  
 Code: TIA/EIA-222-F Date: 02/27/15 Scale: NTS  
 Path: T:\Crown841290\02\Inx\841290.erl Dwg No. E-1

# Feed Line Distribution Chart 0' - 160'

— Round   
 — Flat   
 — App In Face   
 — App Out Face   
 — Truss Leg



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<b>Job: GREENWICH NORTH (BU #: 841290)</b>			
Project: 2015777.841290.02			
Client: Crown Castle USA, Inc.	Drawn by: kliccar	App'd:	
Code: TIA/EIA-222-F	Date: 02/27/15	Scale: NTS	
Path: T:\Crown\841290\02\tx\841290.erl			Dwg No. E-7



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	<b>Client</b>	Crown Castle USA, Inc.	<b>Designed by</b>	kliccar

## Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in Fairfield County, Connecticut.

Basic wind speed of 85 mph.

Nominal ice thickness of 0.7500 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 38 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

<ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>√ Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>Include Bolts In Member Capacity</li> <li>Leg Bolts Are At Top Of Section</li> <li>Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>Add IBC .6D+W Combination</li> </ul>	<ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>Use Clear Spans For Wind Area</li> <li>Use Clear Spans For KL/r</li> <li>Retension Guys To Initial Tension</li> <li>√ Bypass Mast Stability Checks</li> <li>√ Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>Autocalc Torque Arm Areas</li> <li>SR Members Have Cut Ends</li> <li>Sort Capacity Reports By Component</li> <li>Triangulate Diamond Inner Bracing</li> <li>Use TIA-222-G Tension Splice Capacity</li> <li>Exemption</li> </ul>	<ul style="list-style-type: none"> <li>Treat Feedline Bundles As Cylinder</li> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>SR Leg Bolts Resist Compression</li> <li>All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>Consider Feedline Torque</li> <li>Include Angle Block Shear Check</li> <li style="text-align: center; background-color: #e0e0e0;">Poles</li> <li>√ Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> </ul>
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## Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	160.00-152.00	8.00	0.00	18	29.0000	30.6200	0.1875	0.7500	A572-65 (65 ksi)
L2	152.00-111.29	40.71	5.42	18	30.6200	38.8600	0.2500	1.0000	A572-65 (65 ksi)
L3	111.29-77.42	39.29	6.17	18	37.2630	45.0900	0.3125	1.2500	A572-65 (65 ksi)
L4	77.42-36.46	47.13	7.08	18	43.2359	52.6200	0.4375	1.7500	A572-65 (65 ksi)
L5	36.46-0.00	43.54		18	50.3353	59.0000	0.5000	2.0000	A572-65 (65 ksi)

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	<b>Client</b> Crown Castle USA, Inc.	<b>Designed by</b> kliccar

### Tapered Pole Properties

Section	Tip Dia. in	Area in <sup>2</sup>	I in <sup>4</sup>	r in	C in	I/C in <sup>3</sup>	J in <sup>4</sup>	I/Q in <sup>2</sup>	w in	w/t
L1	29.4474	17.1470	1798.4090	10.2284	14.7320	122.0750	3599.1844	8.5751	4.7740	25.461
	31.0924	18.1111	2119.1346	10.8035	15.5550	136.2353	4241.0576	9.0573	5.0591	26.982
L2	31.0924	24.0986	2808.1400	10.7814	15.5550	180.5302	5619.9750	12.0516	4.9491	19.796
	39.4595	30.6370	5770.1059	13.7066	19.7409	292.2922	11547.8043	15.3214	6.3994	25.597
L3	38.9342	36.6502	6321.9884	13.1174	18.9296	333.9740	12652.2955	18.3286	6.0083	19.226
	45.7856	44.4137	11250.5543	15.8960	22.9057	491.1679	22515.9125	22.2111	7.3858	23.635
L4	45.1503	59.4309	13753.2027	15.1934	21.9638	626.1754	27524.5022	29.7211	6.8395	15.633
	53.4317	72.4619	24928.5533	18.5248	26.7310	932.5723	49889.9082	36.2378	8.4911	19.408
L5	52.5425	79.0886	24815.6294	17.6915	25.5703	970.4854	49663.9118	39.5518	7.9790	15.958
	59.9102	92.8395	40140.4258	20.7675	29.9720	1339.2642	80333.6694	46.4286	9.5040	19.008

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A <sub>f</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in
ft	ft <sup>2</sup>	in						
L1 160.00-152.00				1	1	1		
L2 152.00-111.29				1	1	1		
L3 111.29-77.42				1	1	1		
L4 77.42-36.46				1	1	1		
L5 36.46-0.00				1	1	1		

### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement	Total Number	C <sub>A</sub> A <sub>A</sub>	Weight
				ft		ft <sup>2</sup> /ft	plf
LDF7-50A(1-5/8")	A	No	Inside Pole	160.00 - 8.00	12	No Ice	0.82
						1/2" Ice	0.82
						1" Ice	0.82
						2" Ice	0.82
						4" Ice	0.82
MLE Hybrid 9Power/18Fiber RL 2(15/8)	A	No	Inside Pole	160.00 - 8.00	1	No Ice	1.07
						1/2" Ice	1.07
						1" Ice	1.07
						2" Ice	1.07
						4" Ice	1.07
LDF2-50(3/8")	A	No	Inside Pole	153.00 - 8.00	2	No Ice	0.08
						1/2" Ice	0.08
						1" Ice	0.08
						2" Ice	0.08
						4" Ice	0.08
2" Flex Conduit	A	No	Inside Pole	153.00 - 8.00	1	No Ice	0.32
						1/2" Ice	0.32
						1" Ice	0.32
						2" Ice	0.32
						4" Ice	0.32
LDF7-50A(1-5/8")	A	No	Inside Pole	149.00 - 8.00	12	No Ice	0.82
						1/2" Ice	0.82
						1" Ice	0.82
						2" Ice	0.82
						4" Ice	0.82
LDF7-50A(1-5/8")	A	No	Inside Pole	140.00 - 8.00	18	No Ice	0.82



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Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	C <sub>AA</sub>	Weight plf
HB158-1-08U8-S8J18(1-5/8)	A	No	Inside Pole	140.00 - 8.00	2	1/2" Ice	0.82
						1" Ice	0.82
						2" Ice	0.82
						4" Ice	0.82
						No Ice	1.30
						1/2" Ice	1.30
						1" Ice	1.30
LDF4-50A(1/2")	C	No	Inside Pole	120.00 - 8.00	2	2" Ice	1.30
						4" Ice	1.30
						No Ice	0.15
						1/2" Ice	0.15
						1" Ice	0.15
						2" Ice	0.15
						4" Ice	0.15
HB114-1-05U3-S3J(1-1/4)	C	No	Inside Pole	120.00 - 8.00	3	No Ice	0.90
						1/2" Ice	0.90
						1" Ice	0.90
						2" Ice	0.90
						4" Ice	0.90
						No Ice	1.22
						1/2" Ice	1.22
HB114-21U3M12-XXX F(1-1/4")	C	No	Inside Pole	120.00 - 8.00	1	1" Ice	1.22
						2" Ice	1.22
						4" Ice	1.22
						No Ice	0.15
						1/2" Ice	0.15
						1" Ice	0.15
						2" Ice	0.15
LDF4-50A(1/2")	C	No	Inside Pole	72.00 - 8.00	2	4" Ice	0.15
						No Ice	0.15
						1/2" Ice	0.15
						1" Ice	0.15
						2" Ice	0.15
						4" Ice	0.15
						No Ice	0.22
Safety Line 3/8	B	No	CaAa (Out Of Face)	160.00 - 8.00	1	1/2" Ice	0.75
						1" Ice	1.28
						2" Ice	2.34
						4" Ice	4.46
						No Ice	1.00
						1/2" Ice	1.56
						1" Ice	2.73
5/8" Step Bolts	B	No	CaAa (Out Of Face)	160.00 - 8.00	1	2" Ice	6.91
						4" Ice	22.58
						No Ice	0.04
						1/2" Ice	0.14
						1" Ice	0.24
						2" Ice	0.44
						4" Ice	0.84

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
L1	160.00-152.00	A	0.000	0.000	0.000	0.000	0.09
		B	0.000	0.000	0.000	0.633	0.01
		C	0.000	0.000	0.000	0.000	0.00
L2	152.00-111.29	A	0.000	0.000	0.000	0.000	1.33
		B	0.000	0.000	0.000	3.223	0.05
		C	0.000	0.000	0.000	0.000	0.04
L3	111.29-77.42	A	0.000	0.000	0.000	0.000	1.31
		B	0.000	0.000	0.000	2.681	0.04
		C	0.000	0.000	0.000	0.000	0.14
L4	77.42-36.46	A	0.000	0.000	0.000	0.000	1.58
		B	0.000	0.000	0.000	3.243	0.05
		C	0.000	0.000	0.000	0.000	0.18
L5	36.46-0.00	A	0.000	0.000	0.000	0.000	1.10
		B	0.000	0.000	0.000	2.253	0.03

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Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
		C	0.000	0.000	0.000	0.000	0.13

### Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
L1	160.00-152.00	A	0.904	0.000	0.000	0.000	0.000	0.09
		B		0.000	0.000	0.000	3.525	0.03
		C		0.000	0.000	0.000	0.000	0.00
L2	152.00-111.29	A	0.885	0.000	0.000	0.000	0.000	1.33
		B		0.000	0.000	0.000	17.634	0.15
		C		0.000	0.000	0.000	0.000	0.04
L3	111.29-77.42	A	0.851	0.000	0.000	0.000	0.000	1.31
		B		0.000	0.000	0.000	14.671	0.12
		C		0.000	0.000	0.000	0.000	0.14
L4	77.42-36.46	A	0.801	0.000	0.000	0.000	0.000	1.58
		B		0.000	0.000	0.000	17.178	0.14
		C		0.000	0.000	0.000	0.000	0.18
L5	36.46-0.00	A	0.750	0.000	0.000	0.000	0.000	1.10
		B		0.000	0.000	0.000	11.368	0.09
		C		0.000	0.000	0.000	0.000	0.13

### Feed Line Center of Pressure

Section	Elevation ft	CP <sub>X</sub> in	CP <sub>Z</sub> in	CP <sub>X</sub> Ice in	CP <sub>Z</sub> Ice in
L1	160.00-152.00	0.0997	0.0575	0.4624	0.2669
L2	152.00-111.29	0.1001	0.0578	0.4687	0.2706
L3	111.29-77.42	0.1005	0.0581	0.4822	0.2784
L4	77.42-36.46	0.1009	0.0582	0.4784	0.2762
L5	36.46-0.00	0.0781	0.0451	0.3641	0.2102

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>AA</sub> Front ft <sup>2</sup>	C <sub>AA</sub> Side ft <sup>2</sup>	Weight K	
Platform Mount [LP 1201-1]	C	None		0.0000	160.00	No Ice	23.10	23.10	2.10
						1/2" Ice	26.80	26.80	2.50
						1" Ice	30.50	30.50	2.90
						2" Ice	37.90	37.90	3.70
						4" Ice	52.70	52.70	5.30
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	A	From Centroid-Face	4.00 0.00 3.00	0.0000	160.00	No Ice	6.90	5.72	0.11
						1/2" Ice	7.46	6.63	0.17
						1" Ice	8.00	7.42	0.24
						2" Ice	9.10	9.07	0.39
						4" Ice	11.44	12.58	0.82
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	B	From Centroid-Face	4.00 0.00 3.00	0.0000	160.00	No Ice	6.90	5.72	0.11
						1/2" Ice	7.46	6.63	0.17
						1" Ice	8.00	7.42	0.24



# tnxTower

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>AA</sub> Front ft <sup>2</sup>	C <sub>AA</sub> Side ft <sup>2</sup>	Weight K	
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	C	From Centroid-Fa ce	4.00 0.00 3.00	0.0000	160.00	2" Ice	9.10	9.07	0.39
						4" Ice	11.44	12.58	0.82
						No Ice	6.90	5.72	0.11
						1/2" Ice	7.46	6.63	0.17
						1" Ice	8.00	7.42	0.24
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	A	From Centroid-Fa ce	4.00 0.00 3.00	0.0000	160.00	2" Ice	9.10	9.07	0.39
						4" Ice	11.44	12.58	0.82
						No Ice	6.90	5.72	0.11
						1/2" Ice	7.46	6.63	0.17
						1" Ice	8.00	7.42	0.24
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	B	From Centroid-Fa ce	4.00 0.00 3.00	0.0000	160.00	2" Ice	9.10	9.07	0.39
						4" Ice	11.44	12.58	0.82
						No Ice	6.90	5.72	0.11
						1/2" Ice	7.46	6.63	0.17
						1" Ice	8.00	7.42	0.24
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	C	From Centroid-Fa ce	4.00 0.00 3.00	0.0000	160.00	2" Ice	9.10	9.07	0.39
						4" Ice	11.44	12.58	0.82
						No Ice	6.90	5.72	0.11
						1/2" Ice	7.46	6.63	0.17
						1" Ice	8.00	7.42	0.24
ATMAA1412D-1A20	A	From Centroid-Fa ce	4.00 0.00 3.00	0.0000	160.00	2" Ice	9.10	9.07	0.39
						4" Ice	11.44	12.58	0.82
						No Ice	1.17	0.47	0.01
						1/2" Ice	1.31	0.57	0.02
						1" Ice	1.47	0.69	0.03
ATMAA1412D-1A20	B	From Centroid-Fa ce	4.00 0.00 3.00	0.0000	160.00	2" Ice	1.81	0.95	0.06
						4" Ice	2.58	1.57	0.14
						No Ice	1.17	0.47	0.01
						1/2" Ice	1.31	0.57	0.02
						1" Ice	1.47	0.69	0.03
ATMAA1412D-1A20	C	From Centroid-Fa ce	4.00 0.00 3.00	0.0000	160.00	2" Ice	1.81	0.95	0.06
						4" Ice	2.58	1.57	0.14
						No Ice	1.17	0.47	0.01
						1/2" Ice	1.31	0.57	0.02
						1" Ice	1.47	0.69	0.03
Pipe Mount 6'x2.375"	A	From Centroid-Fa ce	4.00 0.00 2.00	0.0000	160.00	2" Ice	1.81	0.95	0.06
						4" Ice	2.58	1.57	0.14
						No Ice	1.43	1.43	0.03
						1/2" Ice	1.92	1.92	0.04
						1" Ice	2.29	2.29	0.05
Pipe Mount 6'x2.375"	B	From Centroid-Fa ce	4.00 0.00 2.00	0.0000	160.00	2" Ice	3.06	3.06	0.09
						4" Ice	4.70	4.70	0.23
						No Ice	1.43	1.43	0.03
						1/2" Ice	1.92	1.92	0.04
						1" Ice	2.29	2.29	0.05
Pipe Mount 6'x2.375"	C	From Centroid-Fa ce	4.00 0.00 2.00	0.0000	160.00	2" Ice	3.06	3.06	0.09
						4" Ice	4.70	4.70	0.23
						No Ice	1.43	1.43	0.03
						1/2" Ice	1.92	1.92	0.04
						1" Ice	2.29	2.29	0.05
LNX-6515DS-VTM w/ Mount Pipe	A	From Centroid-Fa ce	4.00 0.00 3.00	0.0000	160.00	2" Ice	3.06	3.06	0.09
						4" Ice	4.70	4.70	0.23
						No Ice	11.64	9.79	0.08
						1/2" Ice	12.34	11.30	0.17
						1" Ice	13.04	12.80	0.27
						2" Ice	14.48	15.12	0.50
						4" Ice	17.71	19.94	1.14

<b>Job</b>	GREENWICH NORTH (BU #: 841290)	<b>Page</b>	6 of 13
<b>Project</b>	2015777.841290.02	<b>Date</b>	16:01:41 02/27/15
<b>Client</b>	Crown Castle USA, Inc.	<b>Designed by</b>	kliccar

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight
			Horz	Lateral			Front	Side	
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
LNX-6515DS-VTM w/ Mount Pipe	B	From Centroid-Face	4.00	0.0000	160.00	No Ice	11.64	9.79	0.08
			0.00	0.0000		1/2" Ice	12.34	11.30	0.17
			3.00	0.0000		1" Ice	13.04	12.80	0.27
				0.0000		2" Ice	14.48	15.12	0.50
				0.0000		4" Ice	17.71	19.94	1.14
LNX-6515DS-VTM w/ Mount Pipe	C	From Centroid-Face	4.00	0.0000	160.00	No Ice	11.64	9.79	0.08
			0.00	0.0000		1/2" Ice	12.34	11.30	0.17
			3.00	0.0000		1" Ice	13.04	12.80	0.27
				0.0000		2" Ice	14.48	15.12	0.50
				0.0000		4" Ice	17.71	19.94	1.14
RRUS 11 B12	A	From Centroid-Face	4.00	0.0000	160.00	No Ice	3.31	1.36	0.05
			0.00	0.0000		1/2" Ice	3.55	1.54	0.07
			3.00	0.0000		1" Ice	3.80	1.73	0.10
				0.0000		2" Ice	4.33	2.13	0.15
				0.0000		4" Ice	5.50	3.04	0.31
RRUS 11 B12	B	From Centroid-Face	4.00	0.0000	160.00	No Ice	3.31	1.36	0.05
			0.00	0.0000		1/2" Ice	3.55	1.54	0.07
			3.00	0.0000		1" Ice	3.80	1.73	0.10
				0.0000		2" Ice	4.33	2.13	0.15
				0.0000		4" Ice	5.50	3.04	0.31
RRUS 11 B12	C	From Centroid-Face	4.00	0.0000	160.00	No Ice	3.31	1.36	0.05
			0.00	0.0000		1/2" Ice	3.55	1.54	0.07
			3.00	0.0000		1" Ice	3.80	1.73	0.10
				0.0000		2" Ice	4.33	2.13	0.15
				0.0000		4" Ice	5.50	3.04	0.31
Side Arm Mount [SO 102-3]	C	None		0.0000	153.00	No Ice	3.00	3.00	0.08
				0.0000		1/2" Ice	3.48	3.48	0.11
				0.0000		1" Ice	3.96	3.96	0.14
				0.0000		2" Ice	4.92	4.92	0.20
				0.0000		4" Ice	6.84	6.84	0.32
TME-RRUS-11	A	From Leg	1.50	0.0000	153.00	No Ice	3.25	1.37	0.05
			0.00	0.0000		1/2" Ice	3.49	1.55	0.07
			0.00	0.0000		1" Ice	3.74	1.74	0.09
				0.0000		2" Ice	4.27	2.14	0.15
				0.0000		4" Ice	5.43	3.04	0.31
TME-RRUS-11	B	From Leg	1.50	0.0000	153.00	No Ice	3.25	1.37	0.05
			0.00	0.0000		1/2" Ice	3.49	1.55	0.07
			0.00	0.0000		1" Ice	3.74	1.74	0.09
				0.0000		2" Ice	4.27	2.14	0.15
				0.0000		4" Ice	5.43	3.04	0.31
TME-RRUS-11	C	From Leg	1.50	0.0000	153.00	No Ice	3.25	1.37	0.05
			0.00	0.0000		1/2" Ice	3.49	1.55	0.07
			0.00	0.0000		1" Ice	3.74	1.74	0.09
				0.0000		2" Ice	4.27	2.14	0.15
				0.0000		4" Ice	5.43	3.04	0.31
TME-DC6-48-60-18-8F	A	From Leg	1.50	0.0000	153.00	No Ice	1.47	1.47	0.02
			0.00	0.0000		1/2" Ice	1.67	1.67	0.04
			0.00	0.0000		1" Ice	1.88	1.88	0.06
				0.0000		2" Ice	2.33	2.33	0.11
				0.0000		4" Ice	3.38	3.38	0.24
Platform Mount [LP 1201-1]	C	None		0.0000	149.00	No Ice	23.10	23.10	2.10
				0.0000		1/2" Ice	26.80	26.80	2.50
				0.0000		1" Ice	30.50	30.50	2.90
				0.0000		2" Ice	37.90	37.90	3.70
				0.0000		4" Ice	52.70	52.70	5.30
(2) 7770.00 w/ Mount Pipe	A	From Centroid-Le	4.00	0.0000	149.00	No Ice	6.22	4.35	0.06
			0.00	0.0000		1/2" Ice	6.77	5.20	0.11



<p><b>tnxTower</b></p> <p><b>GPD Group</b> 520 South Main St, Suite 2531 Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101</p>	<b>Job</b>	GREENWICH NORTH (BU #: 841290)	<b>Page</b>	7 of 13
	<b>Project</b>	2015777.841290.02	<b>Date</b>	16:01:41 02/27/15
	<b>Client</b>	Crown Castle USA, Inc.	<b>Designed by</b>	kliccar

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>AA</sub> Front ft <sup>2</sup>	C <sub>AA</sub> Side ft <sup>2</sup>	Weight K
		g	2.00			1" Ice 7.30	5.92	0.16
						2" Ice 8.38	7.41	0.29
						4" Ice 10.69	10.76	0.68
(2) 7770.00 w/ Mount Pipe	B	From Centroid-Le	4.00 0.00	0.0000	149.00	No Ice 6.22	4.35	0.06
		g	2.00			1/2" Ice 6.77	5.20	0.11
						1" Ice 7.30	5.92	0.16
						2" Ice 8.38	7.41	0.29
						4" Ice 10.69	10.76	0.68
(2) 7770.00 w/ Mount Pipe	C	From Centroid-Le	4.00 0.00	0.0000	149.00	No Ice 6.22	4.35	0.06
		g	2.00			1/2" Ice 6.77	5.20	0.11
						1" Ice 7.30	5.92	0.16
						2" Ice 8.38	7.41	0.29
						4" Ice 10.69	10.76	0.68
P65-16-XLH-RR w/ mount pipe	A	From Centroid-Le	4.00 0.00	0.0000	149.00	No Ice 8.64	6.36	0.08
		g	0.00			1/2" Ice 9.29	7.54	0.14
						1" Ice 9.91	8.43	0.22
						2" Ice 11.18	10.24	0.39
						4" Ice 13.83	14.10	0.89
P65-16-XLH-RR w/ mount pipe	B	From Centroid-Le	4.00 0.00	0.0000	149.00	No Ice 8.64	6.36	0.08
		g	0.00			1/2" Ice 9.29	7.54	0.14
						1" Ice 9.91	8.43	0.22
						2" Ice 11.18	10.24	0.39
						4" Ice 13.83	14.10	0.89
P65-16-XLH-RR w/ mount pipe	C	From Centroid-Le	4.00 0.00	0.0000	149.00	No Ice 8.64	6.36	0.08
		g	0.00			1/2" Ice 9.29	7.54	0.14
						1" Ice 9.91	8.43	0.22
						2" Ice 11.18	10.24	0.39
						4" Ice 13.83	14.10	0.89
(2) LGP21401	A	From Centroid-Le	4.00 0.00	0.0000	149.00	No Ice 1.29	0.36	0.01
		g	2.00			1/2" Ice 1.45	0.48	0.02
						1" Ice 1.61	0.60	0.03
						2" Ice 1.97	0.87	0.05
						4" Ice 2.79	1.52	0.14
(2) LGP21401	B	From Centroid-Le	4.00 0.00	0.0000	149.00	No Ice 1.29	0.36	0.01
		g	2.00			1/2" Ice 1.45	0.48	0.02
						1" Ice 1.61	0.60	0.03
						2" Ice 1.97	0.87	0.05
						4" Ice 2.79	1.52	0.14
(2) LGP21401	C	From Centroid-Le	4.00 0.00	0.0000	149.00	No Ice 1.29	0.36	0.01
		g	2.00			1/2" Ice 1.45	0.48	0.02
						1" Ice 1.61	0.60	0.03
						2" Ice 1.97	0.87	0.05
						4" Ice 2.79	1.52	0.14
(2) LGP21401	A	From Centroid-Le	4.00 0.00	0.0000	149.00	No Ice 1.29	0.36	0.01
		g	0.00			1/2" Ice 1.45	0.48	0.02
						1" Ice 1.61	0.60	0.03
						2" Ice 1.97	0.87	0.05
						4" Ice 2.79	1.52	0.14
(2) LGP21401	B	From Centroid-Le	4.00 0.00	0.0000	149.00	No Ice 1.29	0.36	0.01
		g	0.00			1/2" Ice 1.45	0.48	0.02
						1" Ice 1.61	0.60	0.03
						2" Ice 1.97	0.87	0.05
						4" Ice 2.79	1.52	0.14
(2) LGP21401	C	From Centroid-Le	4.00 0.00	0.0000	149.00	No Ice 1.29	0.36	0.01
		g	0.00			1/2" Ice 1.45	0.48	0.02
						1" Ice 1.61	0.60	0.03
						2" Ice 1.97	0.87	0.05

<b>tnxTower</b>  <b>GPD Group</b> 520 South Main St, Suite 2531 Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101	<b>Job</b>	GREENWICH NORTH (BU #: 841290)	<b>Page</b>	8 of 13
	<b>Project</b>	2015777.841290.02	<b>Date</b>	16:01:41 02/27/15
	<b>Client</b>	Crown Castle USA, Inc.	<b>Designed by</b>	kliccar

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz	Lateral						Vert
Platform Mount [LP 1201-1]	C	None			0.0000	140.00	4" Ice	2.79	1.52	0.14
							No Ice	23.10	23.10	2.10
							1/2" Ice	26.80	26.80	2.50
							1" Ice	30.50	30.50	2.90
							2" Ice	37.90	37.90	3.70
P65-16-XL-M w/ Mount Pipe	A	From Centroid-Le g	4.00	0.00	0.0000	140.00	4" Ice	52.70	52.70	5.30
							No Ice	8.64	6.36	0.07
							1/2" Ice	9.29	7.54	0.14
							1" Ice	9.91	8.43	0.21
							2" Ice	11.18	10.24	0.38
P65-16-XL-M w/ Mount Pipe	C	From Centroid-Le g	4.00	0.00	0.0000	140.00	4" Ice	13.83	14.10	0.88
							No Ice	8.64	6.36	0.07
							1/2" Ice	9.29	7.54	0.14
							1" Ice	9.91	8.43	0.21
							2" Ice	11.18	10.24	0.38
(2) APL868013-42T0 w/ Mount Pipe	A	From Centroid-Le g	4.00	0.00	0.0000	140.00	4" Ice	13.83	14.10	0.88
							No Ice	3.10	4.92	0.02
							1/2" Ice	3.48	5.60	0.06
							1" Ice	3.88	6.28	0.11
							2" Ice	4.76	7.71	0.22
(2) APL868013-42T0 w/ Mount Pipe	B	From Centroid-Le g	4.00	0.00	0.0000	140.00	4" Ice	6.66	10.83	0.54
							No Ice	3.10	4.92	0.02
							1/2" Ice	3.48	5.60	0.06
							1" Ice	3.88	6.28	0.11
							2" Ice	4.76	7.71	0.22
TME-RRH2X60-PCS	A	From Leg	4.00	0.00	0.0000	140.00	4" Ice	6.66	10.83	0.54
							No Ice	2.57	2.01	0.06
							1/2" Ice	2.79	2.22	0.08
							1" Ice	3.02	2.43	0.10
							2" Ice	3.52	2.89	0.16
TME-RRH2X60-PCS	B	From Leg	4.00	0.00	0.0000	140.00	4" Ice	4.61	3.92	0.31
							No Ice	2.57	2.01	0.06
							1/2" Ice	2.79	2.22	0.08
							1" Ice	3.02	2.43	0.10
							2" Ice	3.52	2.89	0.16
TME-RRH2X60-PCS	C	From Leg	4.00	0.00	0.0000	140.00	4" Ice	4.61	3.92	0.31
							No Ice	2.57	2.01	0.06
							1/2" Ice	2.79	2.22	0.08
							1" Ice	3.02	2.43	0.10
							2" Ice	3.52	2.89	0.16
TME-RRH2x60 AWS	A	From Leg	4.00	0.00	0.0000	140.00	4" Ice	4.61	3.92	0.31
							No Ice	2.57	2.01	0.06
							1/2" Ice	2.79	2.22	0.08
							1" Ice	3.02	2.43	0.10
							2" Ice	3.52	2.89	0.16
TME-RRH2x60 AWS	B	From Leg	4.00	0.00	0.0000	140.00	4" Ice	4.61	3.92	0.31
							No Ice	2.57	2.01	0.06
							1/2" Ice	2.79	2.22	0.08
							1" Ice	3.02	2.43	0.10
							2" Ice	3.52	2.89	0.16
TME-RRH2x60 AWS	C	From Leg	4.00	0.00	0.0000	140.00	4" Ice	4.61	3.92	0.31
							No Ice	2.57	2.01	0.06
							1/2" Ice	2.79	2.22	0.08
							1" Ice	3.02	2.43	0.10
							2" Ice	3.52	2.89	0.16
(2) WWX063X19G00 w/	A	From Leg	4.00	0.00	0.0000	140.00	4" Ice	4.61	3.92	0.31
							No Ice	9.06	7.28	0.06

<p><b>tnxTower</b></p> <p><b>GPD Group</b> 520 South Main St, Suite 2531 Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101</p>	<b>Job</b>	GREENWICH NORTH (BU #: 841290)	<b>Page</b>	9 of 13
	<b>Project</b>	2015777.841290.02	<b>Date</b>	16:01:41 02/27/15
	<b>Client</b>	Crown Castle USA, Inc.	<b>Designed by</b>	kliccar

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz	Lateral						Vert
Mount Pipe			0.00			1/2" Ice	9.73	8.50	0.13	
			2.00			1" Ice	10.38	9.47	0.21	
						2" Ice	11.68	11.35	0.40	
						4" Ice	14.42	15.44	0.92	
(2) WWX063X19G00 w/ Mount Pipe	B	From Leg	4.00		0.0000	140.00	No Ice	9.06	7.28	0.06
			0.00				1/2" Ice	9.73	8.50	0.13
			2.00				1" Ice	10.38	9.47	0.21
							2" Ice	11.68	11.35	0.40
							4" Ice	14.42	15.44	0.92
(2) WWX063X19G00 w/ Mount Pipe	C	From Leg	4.00		0.0000	140.00	No Ice	9.06	7.28	0.06
			0.00				1/2" Ice	9.73	8.50	0.13
			2.00				1" Ice	10.38	9.47	0.21
							2" Ice	11.68	11.35	0.40
							4" Ice	14.42	15.44	0.92
DB-T1-6Z-8AB-0Z	A	From Leg	4.00		0.0000	140.00	No Ice	5.60	2.33	0.04
			0.00				1/2" Ice	5.92	2.56	0.08
			2.00				1" Ice	6.24	2.79	0.12
							2" Ice	6.91	3.28	0.21
							4" Ice	8.37	4.37	0.45
BXA-70063/8CFx2 w/ Mount Pipe	B	From Leg	4.00		0.0000	140.00	No Ice	10.69	7.97	0.05
			0.00				1/2" Ice	11.31	9.37	0.13
			2.00				1" Ice	11.93	10.62	0.22
							2" Ice	13.19	12.80	0.42
							4" Ice	16.17	17.35	1.00
(2) DB844H80E-XY w/ Mount Pipe	C	From Leg	4.00		0.0000	140.00	No Ice	3.30	4.92	0.03
			0.00				1/2" Ice	3.69	5.60	0.07
			2.00				1" Ice	4.12	6.28	0.12
							2" Ice	5.01	7.71	0.23
							4" Ice	6.92	10.83	0.56
Handrail Kit [NA 510-1]	C	None			0.0000	122.00	No Ice	6.00	6.00	0.23
							1/2" Ice	8.50	8.50	0.34
							1" Ice	11.00	11.00	0.45
							2" Ice	16.00	16.00	0.68
							4" Ice	26.00	26.00	1.14
Platform Mount [LP 1201-1]	C	None			0.0000	120.00	No Ice	23.10	23.10	2.10
							1/2" Ice	26.80	26.80	2.50
							1" Ice	30.50	30.50	2.90
							2" Ice	37.90	37.90	3.70
							4" Ice	52.70	52.70	5.30
APXVSPP18-C-A20 w/ Mount Pipe	A	From Centroid-Le g	4.00		0.0000	120.00	No Ice	8.26	6.71	0.08
			0.00				1/2" Ice	8.81	7.66	0.14
			2.00				1" Ice	9.36	8.49	0.22
							2" Ice	10.50	10.20	0.39
							4" Ice	12.88	13.98	0.87
APXVSPP18-C-A20 w/ Mount Pipe	B	From Centroid-Le g	4.00		0.0000	120.00	No Ice	8.26	6.71	0.08
			0.00				1/2" Ice	8.81	7.66	0.14
			2.00				1" Ice	9.36	8.49	0.22
							2" Ice	10.50	10.20	0.39
							4" Ice	12.88	13.98	0.87
APXVSPP18-C-A20 w/ Mount Pipe	C	From Centroid-Le g	4.00		0.0000	120.00	No Ice	8.26	6.71	0.08
			0.00				1/2" Ice	8.81	7.66	0.14
			2.00				1" Ice	9.36	8.49	0.22
							2" Ice	10.50	10.20	0.39
							4" Ice	12.88	13.98	0.87
(2) 6' x 2" Mount Pipe	A	From Centroid-Le g	4.00		0.0000	120.00	No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.92	1.92	0.03
			2.00				1" Ice	2.29	2.29	0.05



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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz	Vert						ft
			Lateral		°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
			ft	ft						
						2" Ice	3.06	3.06	0.09	
						4" Ice	4.70	4.70	0.23	
(2) 6' x 2" Mount Pipe	B	From Centroid-Le g	4.00 0.00 2.00		0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.43 1.92 2.29 3.06 4.70	1.43 1.92 2.29 3.06 4.70	0.02 0.03 0.05 0.09 0.23
(2) 6' x 2" Mount Pipe	C	From Centroid-Le g	4.00 0.00 2.00		0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.43 1.92 2.29 3.06 4.70	1.43 1.92 2.29 3.06 4.70	0.02 0.03 0.05 0.09 0.23
APXVTM14-C-120 w/ Mount Pipe	A	From Centroid-Le g	4.00 0.00 2.00		0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	7.13 7.66 8.18 9.26 11.53	4.96 5.75 6.47 8.01 11.41	0.08 0.13 0.19 0.34 0.75
APXVTM14-C-120 w/ Mount Pipe	B	From Centroid-Le g	4.00 0.00 2.00		0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	7.13 7.66 8.18 9.26 11.53	4.96 5.75 6.47 8.01 11.41	0.08 0.13 0.19 0.34 0.75
APXVTM14-C-120 w/ Mount Pipe	C	From Centroid-Le g	4.00 0.00 2.00		0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	7.13 7.66 8.18 9.26 11.53	4.96 5.75 6.47 8.01 11.41	0.08 0.13 0.19 0.34 0.75
TD-RRH8x20-25	A	From Centroid-Le g	4.00 0.00 2.00		0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.72 5.01 5.32 5.95 7.31	1.70 1.92 2.15 2.62 3.68	0.07 0.10 0.13 0.20 0.40
TD-RRH8x20-25	B	From Centroid-Le g	4.00 0.00 2.00		0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.72 5.01 5.32 5.95 7.31	1.70 1.92 2.15 2.62 3.68	0.07 0.10 0.13 0.20 0.40
TD-RRH8x20-25	C	From Centroid-Le g	4.00 0.00 2.00		0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.72 5.01 5.32 5.95 7.31	1.70 1.92 2.15 2.62 3.68	0.07 0.10 0.13 0.20 0.40
Side Arm Mount [SO 102-3]	C	None			0.0000	119.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	3.00 3.48 3.96 4.92 6.84	3.00 3.48 3.96 4.92 6.84	0.08 0.11 0.14 0.20 0.32
TME-1900MHz RRH w/ 4' x 2" Mount Pipe	A	From Leg	1.50 0.00 0.00		0.0000	119.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	3.36 3.72 4.10 4.90 6.73	4.67 5.18 5.70 6.81 9.38	0.06 0.10 0.15 0.26 0.57
TME-1900MHz RRH w/ 4' x 2" Mount Pipe	B	From Leg	1.50 0.00 0.00		0.0000	119.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	3.36 3.72 4.10 4.90 6.73	4.67 5.18 5.70 6.81 9.38	0.06 0.10 0.15 0.26 0.57

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz	Lateral					
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
TME-1900MHz RRH w/ 4' x 2" Mount Pipe	C	From Leg	1.50	0.0000	119.00	No Ice	3.36	4.67	0.06
			0.00			1/2" Ice	3.72	5.18	0.10
			0.00			1" Ice	4.10	5.70	0.15
						2" Ice	4.90	6.81	0.26
						4" Ice	6.73	9.38	0.57
TME-800MHz RRH	A	From Leg	1.50	0.0000	119.00	No Ice	2.49	2.07	0.05
			0.00			1/2" Ice	2.71	2.27	0.07
			0.00			1" Ice	2.93	2.48	0.10
						2" Ice	3.41	2.93	0.16
						4" Ice	4.46	3.93	0.32
TME-800MHz RRH	B	From Leg	1.50	0.0000	119.00	No Ice	2.49	2.07	0.05
			0.00			1/2" Ice	2.71	2.27	0.07
			0.00			1" Ice	2.93	2.48	0.10
						2" Ice	3.41	2.93	0.16
						4" Ice	4.46	3.93	0.32
TME-800MHz RRH	C	From Leg	1.50	0.0000	119.00	No Ice	2.49	2.07	0.05
			0.00			1/2" Ice	2.71	2.27	0.07
			0.00			1" Ice	2.93	2.48	0.10
						2" Ice	3.41	2.93	0.16
						4" Ice	4.46	3.93	0.32
Side Arm Mount [SO 701-1]	B	From Leg	1.50	0.0000	72.00	No Ice	0.85	1.67	0.07
			0.00			1/2" Ice	1.14	2.34	0.08
			0.00			1" Ice	1.43	3.01	0.09
						2" Ice	2.01	4.35	0.12
						4" Ice	3.17	7.03	0.18
(2) GPS_A	B	From Leg	3.00	0.0000	72.00	No Ice	0.30	0.30	0.00
			0.00			1/2" Ice	0.37	0.37	0.00
			1.00			1" Ice	0.46	0.46	0.01
						2" Ice	0.65	0.65	0.02
						4" Ice	1.15	1.15	0.08

### Maximum Tower Deflections - Service Wind

Section No.	Elevation	Horz. Deflection	Gov. Load Comb.	Tilt	Twist
		in		°	°
L1	160 - 152	29.789	27	1.6162	0.0004
L2	152 - 111.29	27.091	27	1.6022	0.0004
L3	116.71 - 77.42	15.990	27	1.3348	0.0003
L4	83.59 - 36.46	8.033	33	0.9107	0.0002
L5	43.54 - 0	2.187	33	0.4515	0.0001

### Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
ft			in	°	°	ft
160.00	Platform Mount [LP 1201-1]	27	29.789	1.6162	0.0004	34755
153.00	Side Arm Mount [SO 102-3]	27	27.427	1.6047	0.0004	25087
149.00	Platform Mount [LP 1201-1]	27	26.086	1.5928	0.0004	16711
140.00	Platform Mount [LP 1201-1]	27	23.117	1.5476	0.0004	9873
122.00	Handrail Kit [NA 510-1]	27	17.515	1.3931	0.0003	5432

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Elevation	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
120.00	Platform Mount [LP 1201-1]	27	16.930	1.3717	0.0003	5178
119.00	Side Arm Mount [SO 102-3]	27	16.642	1.3607	0.0003	5069
72.00	Side Arm Mount [SO 701-1]	33	5.903	0.7684	0.0002	4591

### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	160 - 152	85.903	2	4.6616	0.0013
L2	152 - 111.29	78.128	2	4.6213	0.0013
L3	116.71 - 77.42	46.133	2	3.8512	0.0009
L4	83.59 - 36.46	23.183	8	2.6284	0.0006
L5	43.54 - 0	6.315	8	1.3034	0.0003

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
160.00	Platform Mount [LP 1201-1]	2	85.903	4.6616	0.0013	12207
153.00	Side Arm Mount [SO 102-3]	2	79.096	4.6285	0.0013	8811
149.00	Platform Mount [LP 1201-1]	2	75.233	4.5941	0.0013	5872
140.00	Platform Mount [LP 1201-1]	2	66.677	4.4639	0.0012	3464
122.00	Handrail Kit [NA 510-1]	2	50.529	4.0192	0.0010	1902
120.00	Platform Mount [LP 1201-1]	2	48.845	3.9574	0.0009	1812
119.00	Side Arm Mount [SO 102-3]	2	48.013	3.9257	0.0009	1774
72.00	Side Arm Mount [SO 701-1]	8	17.038	2.2178	0.0005	1596

### Compression Checks

### Pole Design Data

Section No.	Elevation ft	Size	L ft	L <sub>a</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P/P <sub>a</sub>
L1	160 - 152 (1)	TP30.62x29x0.1875	8.00	0.00	0.0	37.630	18.1111	-3.56	681.53	0.005
L2	152 - 111.29 (2)	TP38.86x30.62x0.25	40.71	0.00	0.0	39.000	29.7665	-16.35	1160.89	0.014
L3	111.29 - 77.42 (3)	TP45.09x37.263x0.3125	39.29	0.00	0.0	39.000	43.1945	-23.23	1684.59	0.014
L4	77.42 - 36.46 (4)	TP52.62x43.2359x0.4375	47.13	0.00	0.0	39.000	70.5044	-35.42	2749.67	0.013
L5	36.46 - 0 (5)	TP59x50.3353x0.5	43.54	0.00	0.0	39.000	92.8395	-52.21	3620.74	0.014

### Pole Bending Design Data

Section No.	Elevation ft	Size	Actual M <sub>x</sub> kip-ft	Actual f <sub>bx</sub> ksi	Allow. F <sub>bx</sub> ksi	Ratio f <sub>bx</sub> /F <sub>bx</sub>	Actual M <sub>y</sub> kip-ft	Actual f <sub>by</sub> ksi	Allow. F <sub>by</sub> ksi	Ratio f <sub>by</sub> /F <sub>by</sub>
L1	160 - 152 (1)	TP30.62x29x0.1875	59.70	5.258	37.630	0.140	0.00	0.000	37.630	0.000
L2	152 - 111.29 (2)	TP38.86x30.62x0.25	695.34	30.247	39.000	0.776	0.00	0.000	39.000	0.000
L3	111.29 - 77.42	TP45.09x37.263x0.3125	1630.38	42.121	39.000	1.080	0.00	0.000	39.000	0.000



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Section No.	Elevation ft	Size	Actual $M_x$ kip-ft	Actual $f_{bx}$ ksi	Allow. $F_{bx}$ ksi	Ratio $\frac{f_{bx}}{F_{bx}}$	Actual $M_y$ kip-ft	Actual $f_{by}$ ksi	Allow. $F_{by}$ ksi	Ratio $\frac{f_{by}}{F_{by}}$
L4	77.42 - 36.46 (3)	TP52.62x43.2359x0.4375	2902.17	39.456	39.000	1.012	0.00	0.000	39.000	0.000
L5	36.46 - 0 (5) (4)	TP59x50.3353x0.5	4439.01	39.774	39.000	1.020	0.00	0.000	39.000	0.000

### Pole Shear Design Data

Section No.	Elevation ft	Size	Actual $V$ K	Actual $f_v$ ksi	Allow. $F_v$ ksi	Ratio $\frac{f_v}{F_v}$	Actual $T$ kip-ft	Actual $f_{vt}$ ksi	Allow. $F_{vt}$ ksi	Ratio $\frac{f_{vt}}{F_{vt}}$
L1	160 - 152 (1)	TP30.62x29x0.1875	6.79	0.375	26.000	0.029	0.01	0.000	26.000	0.000
L2	152 - 111.29 (2)	TP38.86x30.62x0.25	26.61	0.894	26.000	0.069	0.19	0.004	26.000	0.000
L3	111.29 - 77.42 (3)	TP45.09x37.263x0.3125	29.79	0.690	26.000	0.053	0.22	0.003	26.000	0.000
L4	77.42 - 36.46 (4)	TP52.62x43.2359x0.4375	33.55	0.476	26.000	0.037	0.55	0.004	26.000	0.000
L5	36.46 - 0 (5)	TP59x50.3353x0.5	36.93	0.398	26.000	0.031	0.57	0.003	26.000	0.000

### Pole Interaction Design Data

Section No.	Elevation ft	Ratio $P$ $P_a$	Ratio $f_{bx}$ $F_{bx}$	Ratio $f_{by}$ $F_{by}$	Ratio $f_v$ $F_v$	Ratio $f_{vt}$ $F_{vt}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	160 - 152 (1)	0.005	0.140	0.000	0.029	0.000	0.145	1.333	H1-3+VT ✓
L2	152 - 111.29 (2)	0.014	0.776	0.000	0.069	0.000	0.791	1.333	H1-3+VT ✓
L3	111.29 - 77.42 (3)	0.014	1.080	0.000	0.053	0.000	1.095	1.333	H1-3+VT ✓
L4	77.42 - 36.46 (4)	0.013	1.012	0.000	0.037	0.000	1.025	1.333	H1-3+VT ✓
L5	36.46 - 0 (5)	0.014	1.020	0.000	0.031	0.000	1.035	1.333	H1-3+VT ✓

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	$P$ K	$SF * P_{allow}$ K	% Capacity	Pass Fail
L1	160 - 152	Pole	TP30.62x29x0.1875	1	-3.56	908.48	10.9	Pass
L2	152 - 111.29	Pole	TP38.86x30.62x0.25	2	-16.35	1547.47	59.3	Pass
L3	111.29 - 77.42	Pole	TP45.09x37.263x0.3125	3	-23.23	2245.56	82.1	Pass
L4	77.42 - 36.46	Pole	TP52.62x43.2359x0.4375	4	-35.42	3665.31	76.9	Pass
L5	36.46 - 0	Pole	TP59x50.3353x0.5	5	-52.21	4826.45	77.6	Pass

Summary ELC: Load Case 7

Pole (L3) 82.1 Pass  
Rating = 82.1 Pass

**APPENDIX B**  
**BASE LEVEL DRAWING**

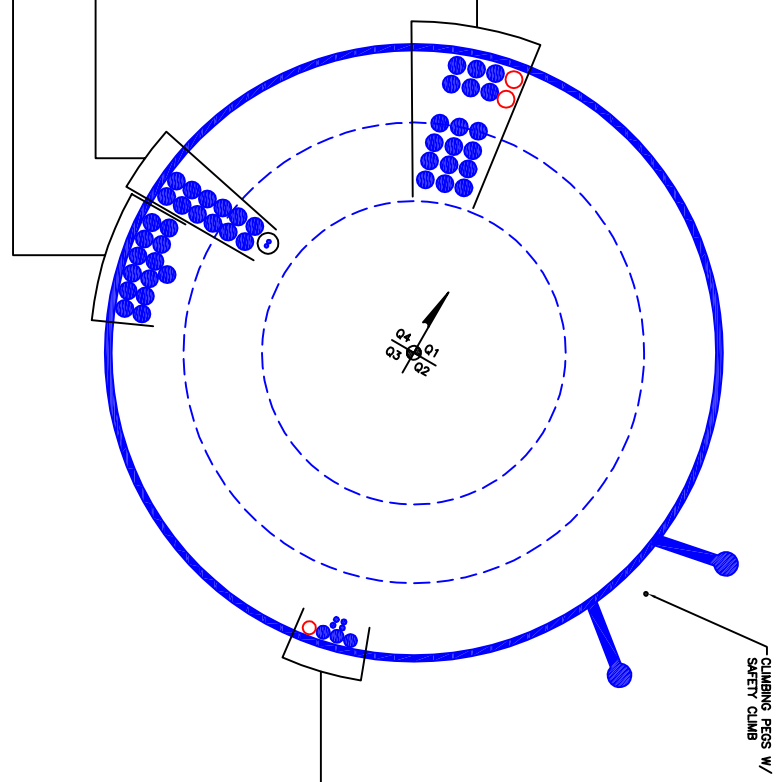


CROWN REGION ADDRESS  
USA

(RESERVED)  
(2) 1-5/8" TO 140 FT LEVEL  
(INSTALLED)  
(18) 1-5/8" TO 140 FT LEVEL

(RESERVED)  
(1) 1-1/4" TO 120 FT LEVEL  
(INSTALLED)  
(2) 1/2" TO 72 FT LEVEL  
(2) 1/2" TO 120 FT LEVEL  
(3) 1-1/4" TO 120 FT LEVEL

(INSTALLED-IN 2" CONDUIT)  
(2) 3/8" TO 153 FT LEVEL  
(INSTALLED)  
(12) 1-5/8" TO 149 FT LEVEL  
(INSTALLED)  
(13) 1-5/8" TO 160 FT LEVEL



BUSINESS UNIT: 841290 TOWER ID: C-BASELEVEL

BASE LEVEL DRAWING

\\na\vol\apps\mike\cable\841290\Tower\A1-0.dwg

- 20/03/14 NEW BUILD PER WORK ORDER # 730620
- 09/08/14 UPDATED PER WORK ORDER # 779659
- 18/08/14 UPDATED PER WORK ORDER # 830768
- 20/1/2015 UPDATED PER WORK ORDER # 992019
- 24/2/2015 UPDATED PER WORK ORDER 1014218

DW  
SLW  
ALM  
SAC  
BNH

DRAWN BY: RJC  
CHECKED BY: RJC  
DRAWING DATE: 2008/14

SITE NUMBER:

SITE NAME:

GREENWICH NORTH

BUSINESS UNIT NUMBER

841290

SITE ADDRESS

363 RIVERSVILLE ROAD  
GREENWICH, CT 06831  
FAIRFIELD COUNTY  
USA

SHEET TITLE

BASE LEVEL

SHEET NUMBER

A1-0

1



**APPENDIX C**  
**ADDITIONAL CALCULATIONS**

# Stiffened or Unstiffened, Exterior Flange Plate - Any Bolt Material TIA Rev F

## Site Data

BU#: 841290  
 Site Name: GREENWICH NORTH  
 App #: 284770 Rev. 0

## Reactions

Moment:	59.7	ft-kips
Axial:	3.56	kips
Shear:	6.79	kips
Elevation:	152	feet

Pole Manufacturer: Other

## Bolt Data

Qty:	12	Bolt Fu:	120
Diameter (in.):	1	Bolt Fy:	92
Bolt Material:	A325	Bolt Fty:	44.00
N/A:		<-- Disregard	
N/A:		<-- Disregard	
Circle (in.):	35		

If No stiffeners, Criteria: AISC ASD <-Only Applicable to Unstiffened Cases

## Flange Bolt Results

Bolt Tension Capacity, B:	46.07 kips
Max Bolt directly applied T:	6.53 Kips
Min. PL "tc" for B cap. w/o Pry:	0.986 in
Min PL "treq" for actual T w/ Pry:	0.271 in
Min PL "t1" for actual T w/o Pry:	0.371 in
T allowable w/o Prying:	46.07 kips
Prying Force, Q:	0.00 kips
Total Bolt Tension=T+Q:	6.53 kips
Non-Prying Bolt Stress Ratio, T/B:	14.2% <b>Pass</b>

Rigid
Service, ASD
Fty*ASIF

## Plate Data

Diam:	38	in
Thick, t:	1	in
Grade (Fy):	60	ksi
Strength, Fu:	75	ksi
Single-Rod B-eff:	8.10	in

## Exterior Flange Plate Results

Flexural Check	
Compression Side Plate Stress:	7.8 ksi
Allowable Plate Stress:	60.0 ksi
Compression Plate Stress Ratio:	13.0% <b>Pass</b>
No Prying	
Tension Side Stress Ratio, (treq/t)^2:	7.4% <b>Pass</b>

Rigid
Service ASD
0.75*Fy*ASIF
Comp. Y.L. Length:
16.95

## Stiffener Data (Welding at Both Sides)

Config:	0	*
Weld Type:		
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		<-- Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

n/a

## Stiffener Results

Horizontal Weld :	n/a
Vertical Weld:	n/a
Plate Flex+Shear, fb/Fb+(fv/Fv)^2:	n/a
Plate Tension+Shear, ft/Ft+(fv/Fv)^2:	n/a
Plate Comp. (AISC Bracket):	n/a

## Pole Results

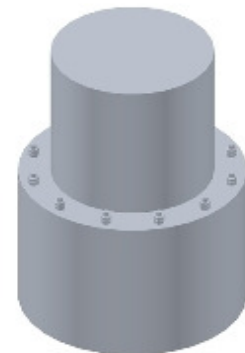
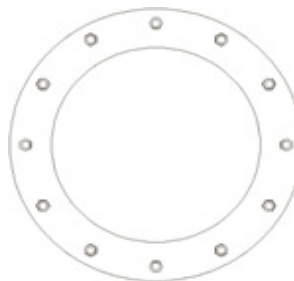
Pole Punching Shear Check: n/a

## Pole Data

Diam:	30.62	in
Thick:	0.1875	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None

## Stress Increase Factor

ASIF:	1.333
-------	-------



\* 0 = none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

\*\* Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

# Stiffened or Unstiffened, UngROUTed, Circular Base Plate - Any Rod Material

## TIA Rev F

### Site Data

BU#: 841290
Site Name: GREENWICH NORTH
App #: 284770 Rev. 0
Pole Manufacturer: <b>Other</b>

Reactions	
Moment:	4439.01 ft-kips
Axial:	52.23 kips
Shear:	36.9 kips

### Anchor Rod Data

Qty:	24	
Diam:	2.25	in
Rod Material:	A615-J	
Strength (Fu):	100	ksi
Yield (Fy):	75	ksi
Bolt Circle:	67	in

If No stiffeners, Criteria: **AISC ASD** <-Only Applicable to Unstiffened Cases

### Anchor Rod Results

Maximum Rod Tension: 130.3 Kips  
 Allowable Tension: 195.0 Kips  
 Anchor Rod Stress Ratio: 66.9% **Pass**

<b>Rigid</b>
Service ASD
Fty*ASIF

### Plate Data

Diam:	73	in
Thick:	2.25	in
Grade:	60	ksi
Single-Rod B-eff:	7.80	in

### Base Plate Results

Base Plate Stress: 50.5 ksi  
 Allowable Plate Stress: 60.0 ksi  
 Base Plate Stress Ratio: 84.2% **Pass**

### Flexural Check

<b>Rigid</b>
Service ASD
0.75*Fy*ASIF
Y.L. Length: 31.75

### Stiffener Data (Welding at both sides)

Config:	0	*
Weld Type:		
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		<-- Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

n/a

### Stiffener Results

Horizontal Weld : n/a  
 Vertical Weld: n/a  
 Plate Flex+Shear, fb/Fb+(fv/Fv)^2: n/a  
 Plate Tension+Shear, ft/Ft+(fv/Fv)^2: n/a  
 Plate Comp. (AISC Bracket): n/a

### Pole Results

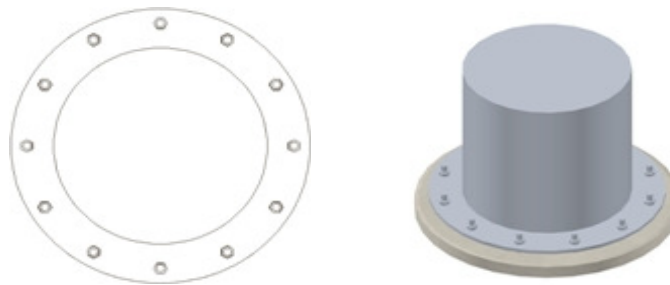
Pole Punching Shear Check: n/a

### Pole Data

Diam:	59	in
Thick:	0.5	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None

### Stress Increase Factor

ASIF:	1.333
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\* 0 = none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

\*\* Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes





**Mat Foundation Analysis**  
**GREENWICH NORTH (BU #: 841290)**  
**2015777.841290.02**

General Info	
Code	TIA/EIA-222-F (LRFD)
Bearing On	Soil
Foundation Type	Mono Pad
Pier Type	Square
Reinforcing Known	Yes
Max Capacity	1.1

Tower Reactions	
Moment, M	4439.01 k-ft
Axial, P	52.23 k
Shear, V	36.9 k

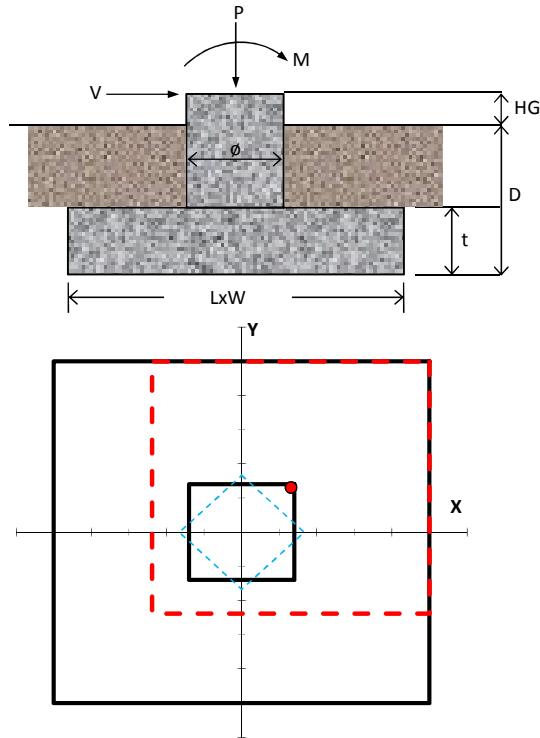
Pad & Pier Geometry		
Pier Width, $\phi$	7	ft
Pad Length, L	25	ft
Pad Width, W	25	ft
Pad Thickness, t	4.5	ft
Depth, D	9.5	ft
Height Above Grade, HG	0.5	ft

Pad & Pier Reinforcing		
Rebar Fy	60	ksi
Concrete Fc'	3	ksi
Clear Cover	3.5	in
Reinforced Top & Bottom?	Yes	
Pad Reinforcing Size	# 11	
Pad Quantity Per Layer	19	
Pier Rebar Size	# 11	
Pier Quantity of Rebar	23	

Soil Properties	
Soil Type	Granular
Soil Unit Weight	120 pcf
Angle of Friction, $\phi$	34 °
Bearing Type	Net
Ultimate Bearing	30 ksf
Water Table Depth	99 ft
Frost Depth	5 ft

Bearing Summary			Load Case
Qxmax	2.80	ksf	1.2D+1.6W
Qymax	2.80	ksf	1.2D+1.6W
Qmax @ 45°	3.03	ksf	1.2D+1.6W
Q <sub>(all) Gross</sub>	23.36	ksf	
<b>Controlling Capacity</b>	<b>13.0%</b>	<b>Pass</b>	

Overturning Summary (Required FS=1.0)			Load Case
FS(ot)x	1.86	≥1.0	0.9D+1.6W
FS(ot)y	1.86	≥1.0	0.9D+1.6W
<b>Controlling Capacity</b>	<b>53.8%</b>	<b>Pass</b>	





**Base Foundation Reinforcement Check**  
**GREENWICH NORTH (BU #: 841290)**  
**2015777.841290.02**

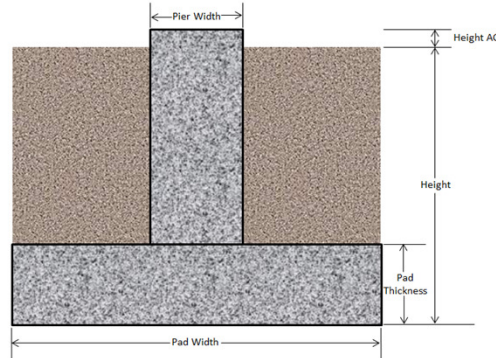
**Code**  
**TIA/EIA-222-F**

Tower Reactions	
Moment	4439.01 k-ft
Axial	52.23 k
Shear	36.9 k

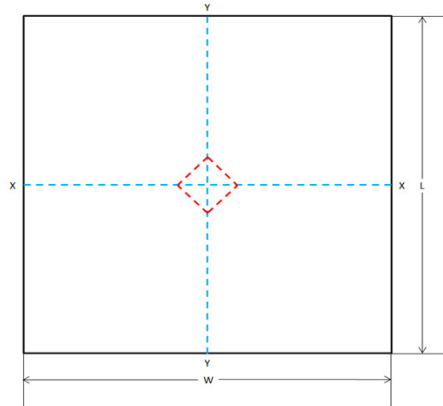
Overall Capacities		
Reinforcement Capacity	25.0%	<b>OK</b>
As Min Met?	Yes	
<b>Controlling Capacity</b>	<b>25.0%</b>	<b>OK</b>

<--- Reinforcement unknown; minimums assumed

Pad & Pier Geometry	
Height	9.5 ft
Height above Grade	0.5 ft
Pad Length, L	25 ft
Pad Width, W	25 ft
Pad Thickness	4.5 ft
Pier Shape	Square
Square Pier Width	7 ft



Pad & Pier Reinforcing	
Reinforcing Known	No
$f'_c$	3 ksi
Clear Cover	3.5 in
Rebar $F_y$	60 ksi
Reinforced Top & Bottom?	Yes
Pad Rebar Size	# 11
Pad Rebar Quantity	19
Pier Rebar Size	# 11
Pier Rebar Quantity	23



Unit Weights	
Concrete Unit Weight	150 pcf
Soil Unit Weight	120 pcf

Orthogonal Bearing	
$Q_{max}$	3.84 ksf
$Q_{min}$	0.00 ksf
Bearing Length	18.23 ft

Pad Moment Capacity		
$M_u =$	62.88 k-ft	
$\phi M_n =$	251.94 k-ft	
Moment Capacity	25.0%	<b>OK</b>
One-Way (Wide-Beam) Shear		
$V_u =$	206.35 kips	
$\phi V_n =$	1192.57 kips	
Shear Capacity	17.3%	<b>OK</b>
Two-Way (Punching) Shear		
$V_u =$	689.86 kips	
$\phi V_n =$	4210.09 kips	
Shear Capacity	16.4%	<b>OK</b>
Pier Compression		
$P_u =$	67.90 kips	
$\phi P_n =$	10428.14 kips	
Compression Capacity	0.7%	<b>OK</b>

RADIO FREQUENCY EMISSIONS ANALYSIS REPORT  
EVALUATION OF HUMAN EXPOSURE POTENTIAL  
TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CT11069A

Greenwich / Boy Scouts 2  
363 Riversville Road  
Greenwich, CT 06831

**March 10, 2015**

**EBI Project Number: 6215001457**

Site Compliance Summary	
Compliance Status:	<b>COMPLIANT</b>
Site total MPE% of FCC general public allowable limit:	<b>41.26 %</b>

March 10, 2015

T-Mobile USA  
Attn: Jason Overbey, RF Manager  
35 Griffin Road South  
Bloomfield, CT 06002

Emissions Analysis for Site: **CT11069A – Greenwich / Boy Scouts 2**

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **363 Riversville Road, Greenwich, CT**, for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The number of  $\mu\text{W}/\text{cm}^2$  calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The general population exposure limit for the 700 MHz Band is  $467 \mu\text{W}/\text{cm}^2$ , and the general population exposure limit for the PCS and AWS bands is  $1000 \mu\text{W}/\text{cm}^2$ . Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

## **CALCULATIONS**

Calculations were done for the proposed T-Mobile Wireless antenna facility located at **363 Riversville Road, Greenwich, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 GSM channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel
- 2) 2 UMTS channels (AWS Band – 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 3) 2 LTE channels (AWS Band – 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 4) 1 LTE channel (700 MHz Band) was considered for each sector of the proposed installation. This channel has a transmit power of 30 Watts.
- 5) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.

- 6) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antennas used in this modeling are the **Ericsson AIR21 (B4A/B2P & B2A/B4P)** for 1900 MHz (PCS) and 2100 MHz (AWS) channels and the **Commscope LNX-6515DS-VTM** for 700 MHz channels. This is based on feedback from the carrier with regards to anticipated antenna selection. The **Ericsson AIR21 (B4A/B2P & B2A/B4P)** have a maximum gain of **15.9 dBd** at its main lobe. The **Commscope LNX-6515DS-VTM** has a maximum gain of **14.6 dBd** at its main lobe. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antenna mounting height centerline of the proposed antennas is **163 feet** above ground level (AGL).
- 9) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general public threshold limits.

**T-Mobile Site Inventory and Power Data**

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	Ericsson AIR21 B4A/B2P	Make / Model:	Ericsson AIR21 B4A/B2P	Make / Model:	Ericsson AIR21 B4A/B2P
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	163	Height (AGL):	163	Height (AGL):	163
Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)
Channel Count	2	Channel Count	2	# PCS Channels:	2
Total TX Power:	120	Total TX Power:	120	# AWS Channels:	120
ERP (W):	4,668.54	ERP (W):	4,668.54	ERP (W):	4,668.54
Antenna A1 MPE%	0.68	Antenna B1 MPE%	0.68	Antenna C1 MPE%	0.68
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	Ericsson AIR21 B2A/B4P	Make / Model:	Ericsson AIR21 B2A/B4P	Make / Model:	Ericsson AIR21 B2A/B4P
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	163	Height (AGL):	163	Height (AGL):	163
Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)
Channel Count	4	Channel Count	4	Channel Count	4
Total TX Power:	120	Total TX Power:	120	Total TX Power:	120
ERP (W):	4,668.54	ERP (W):	4,668.54	ERP (W):	4,668.54
Antenna A2 MPE%	0.68	Antenna B2 MPE%	0.68	Antenna C2 MPE%	0.68
Antenna #:	3	Antenna #:	3	Antenna #:	3
Make / Model:	Commscope LNX-6515DS-VTM	Make / Model:	Commscope LNX-6515DS-VTM	Make / Model:	Commscope LNX-6515DS-VTM
Gain:	14.6 dBd	Gain:	14.6 dBd	Gain:	14.6 dBd
Height (AGL):	163	Height (AGL):	163	Height (AGL):	163
Frequency Bands	700 MHz	Frequency Bands	700 MHz	Frequency Bands	700 MHz
Channel Count	1	Channel Count	1	Channel Count	1
Total TX Power:	30	Total TX Power:	30	Total TX Power:	30
ERP (W):	865.21	ERP (W):	865.21	ERP (W):	865.21
Antenna A3 MPE%	0.27	Antenna B3 MPE%	0.27	Antenna C3 MPE%	0.27

Site Composite MPE%	
Carrier	MPE%
T-Mobile	4.90
AT&T	12.71 %
Verizon Wireless	11.92 %
Sprint	11.73 %
<b>Site Total MPE %:</b>	<b>41.26 %</b>

T-Mobile Sector 1 Total:	1.63 %
T-Mobile Sector 2 Total:	1.63 %
T-Mobile Sector 3 Total:	1.63 %
<b>Site Total:</b>	<b>41.26 %</b>

## Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general public exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general public exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector 1:	1.63 %
Sector 2:	1.63 %
Sector 3 :	1.63 %
T-Mobile Total:	4.90 %
Site Total:	41.26 %
Site Compliance Status:	<b>COMPLIANT</b>

The anticipated composite MPE value for this site assuming all carriers present is **41.26%** of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.



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