March 23, 2015

Melanie A. Bachman Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

RE: T-Mobile-Exempt Modification - Crown Site BU: 841290

T-Mobile Site ID: CT11069A

Located at: 363 Riversville Road, Greenwich, CT 06831

Dear Ms. Bachman:

This letter and exhibits are submitted on behalf of T-Mobile. T-Mobile is making modifications to certain existing sites in its Connecticut system in order to implement their 700MHz technology. Please accept this letter and exhibits as notification, pursuant to § 16-50j-73 of the Regulations of Connecticut State Agencies ("R.C.S.A."), of construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In compliance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Mr. Peter Tesei, First Selectman for the Town of Greenwich and Greenwich Council Boy Scouts of America, Property Owner.

T-Mobile plans to modify the existing wireless communications facility owned by Crown Castle and located at **363 Riversville Road**, **Greenwich**, **CT 06831**. Attached are a compound plan and elevation depicting the planned changes (Exhibit-1), and documentation of the structural sufficiency of the structure to accommodate the revised antenna configuration (Exhibit-2). Also included is a power density table report reflecting the modification to T-Mobile's operations at the site (Exhibit-3).

The changes to the facility do not constitute a modification as defined in Connecticut General Statutes ("C.G.S.") § 16-50i(d) because the general physical characteristics of the facility will not be significantly changed. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in the R.C.S.A. § 16-50j-72(b)(2).

- 1. The proposed modifications will not result in an increase in the height of the existing tower. T-Mobile's replacement antennas will be located at the same elevation on the existing tower.
- 2. There will be no proposed modifications to the ground and no extension of boundaries.
- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more.

- 4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative General Power Density table report for T-Mobile's modified facility is included as Exhibit-3.
- 5. A Structural Modification Report confirming that the tower and foundation can support T-Mobile's proposed modifications is included as Exhibit-2.

For the foregoing reasons, T-Mobile respectfully submits the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

Jerry Feathers Real Estate Specialist

Enclosure

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes

Tab 2: Exhibit-2: Structural Modification Report

Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: Mr. Peter Tesei, First Selectman Town of Greenwich 101 Field Point Road

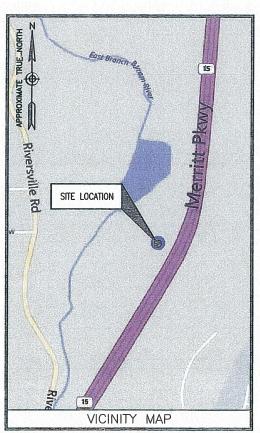
Greenwich, CT 06830

cc: Greenwich Council Boy Scouts of America

63 Mason Street Greenwich, CT 06830

T-MOBILE NORTHEAST LLC

T-MOBILE SITE #: CT11069A CROWN CASTLE BU #: 841290 SITE NAME: GREENWICH NORTH 363 RIVERSVILLE ROAD GREENWICH, CT 06831 FAIRFIELD COUNTY



FROM PARSIPPANY, NJ:

HEAD NORTHWEST ON SYLVAN WAY. TURN RIGHT ONTO US—202 N. TAKE THE RAMP ONTO 1-80 E. MERGE ONTO 1-80 E. TAKE EXIT 43 FOR 1-287 TOWARD MORRISTOWN/MAHWAH. KEEP LEFT A THE FORK AND MERGE ONTO 1-287 N. TAKE THE 1-87 S/1-287/NEW YORK THRUWAY EXIT TOWARD TAPPAN ZEE BR/NEW YORK CITY, MERGE ONTO 1-287 E/1-87 S. KEEP LEFT AT THE FORK TO CONTINUE ON 1-287 E, FOLLOW SIGNS FOR WHITE PLAINS/RYE. TAKE EXIT 8E FOR WESTCHESTER AVE. CONTINUE ONTO WESTCHESTER AVE. CONTINUE ONTO WESTCHESTER AVE. TAKE SIGHT LEFT ONTO ANDERSON HILL RD. TURN LEFT ONTO NY-120A N/KING ST. TURN RIGHT ONTO SHERWOOD AVE. TURN LEFT ONTO RIVERSVILLE RD. SITE WILL BE ON THE RIGHT.

ENGINEER

DEWBERRY ENGINEERS INC.
600 PARSIPPANY ROAD
SUITE 301
PARSIPPANY, NJ 07054

CONTACT: BRYAN HUFF PHONE #: (973) 576-0147

CONSTRUCTION
CROWN CASTLE
500 WEST CUMMINGS PARK, SUITE 3600
WOBURN, MA 01801

CONTACT: WARREN KELLEHER PHONE #: (781) 970-0055

CONSULTANT TEAM

SITE NAME: GREENWICH NORTH

SITE NUMBER: CT11069A

TOWER OWNER:

CROWN CASTLE 500 WEST CUMMINGS PARK, SUITE 3600 WOBURN, MA 01801

> APPLICANT/DEVELOPER: T-MOBILE NORTHEAST LLC 4 SYLVAN WAY PARSIPPANY, NJ 07054

> > **COORDINATES:**

LATITUDE: 41'-03'-58.6" N (NAD83) LONGITUDE: 73'-40'-17.4" W (NAD83) (PER CROWN CASTLE)

CONFIGURATION

702Cu

PROJECT SUMMARY

SITE ADDRESS: 363 RIVERSVILLE ROAD GREENWICH, CT 06831 FAIRFIELD COUNTY

PROJECT DIRECTORY

INSTALL (3) NEW ANTENNAS.

INSTALL (3) NEW RRU'S.

SCOPE OF WORK

THIS DOCUMENT WAS DEVELOPED TO REFLECT A SPECIFIC SITE AND ITS SITE CONDITIONS AND IS NOT TO BE USED FOR ANOTHER SITE OR WHEN OTHER CONDITIONS PERTAIN, REUSE OF THIS DOCUMENT IS AT THE SOLE RISK OF THE USER.

A.D.A. COMPLIANCE: FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION

SHT. NO.	DESCRIPTION
T-1	TITLE SHEET
G-1	GENERAL NOTES .
C-1	COMPOUND PLAN & EQUIPMENT PLANS
C-2	ANTENNA LAYOUTS & ELEVATIONS
C-3	CONSTRUCTION DETAILS
E-1	GROUNDING NOTES & DETAILS
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	SHEET INDEX

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T-MOBILE NORTHEAST LLC 4 SYLVAN WAY PARSIPPANY, NJ 07054



CROWN CASTLE 500 WEST CUMMINGS PARK, SUITE 3600 WOBURN, MA 01801

CT11069A GREENWICH NORTH

	CONST	RUCTION DRAWINGS
	110,424,611	
-	150551 1976 2	
	1000	
1	03/20/15	ISSUED AS FINAL
1	03/17/15	ISSUED AS FINAL ISSUED FOR REVIEW



Dewberry Engineers Inc.

600 PARSIPPANY ROAD SUITE 301 PARSIPPANY, NJ 07054 PHONE: 973.739.9400 FAX: 973.739.9710



DRAWN BY:	, JC
REVIEWED BY:	BSH
CHECKED BY:	GHN

PROJECT NUMBER: 50066258

JOB NUMBER: 50072421

SITE ADDRESS:

363 RIVERSVILLE ROAD GREENWICH, CT 06831 FAIRFIELD COUNTY

SHEET TITLE

TITLE SHEET

SHEET NUMBER

T-1

GENERAL NOTES:

- FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY: PROJECT MANAGEMENT CROWN CASTLE CONTRACTOR GENERAL CONTRACTOR (CONSTRUCTION) OWNER T—MOBILE OEM - ORIGINAL EQUIPMENT MANUFACTURER
- 2. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPUSHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF PROJECT
- ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL AMS, ORDINANCES, RULES, REQUIATIONS, AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE
- ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 5. DRAWINGS PROVIDED HERE ARE NOT TO SCALE UNLESS OTHERWISE NOTED AND ARE INTENDED TO SHOW
- UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
- THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- IF THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY PROJECT MANAGEMENT.
- CONTRACTOR SHALL DETERMINE ACTUAL ROUTING OF CONDUIT, POWER AND T1 CABLES, GROUNDING CABLES AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING, CONTRACTOR SHALL UTILIZE EXISTING TRAYS AND/OR SHALL ADD NEW TRAYS AS NECESSARY. CONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH PROJECT MANAGEMENT.
- 10. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF
- 11. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 12. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION
- 13. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT DESCRIBED HEREIN. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES AND FOR COORDINATING ALL PORTIONS OF THE WORK UNDER THE CONTRACT.
- 14. CONTRACTOR SHALL NOTIFY DEWBERRY 48 HOURS IN ADVANCE OF POURING CONCRETE, OR BACKFILLING TRENCHES, SEALING ROOF AND WALL PENETRATIONS & POST DOWNS, FINISHING NEW WALLS OR FINAL ELECTRICAL CONNECTIONS FOR ENGINEER REVIEW.
- 15. CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK.
 ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS MUST BE VERIFIED. CONTRACTOR
 SHALL NOTIFY PROJECT MANAGEMENT OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING
- 16. THE EXISTING CELL SITE IS IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION WORK BY CONTRACTOR SHALL NOT DISRUPT THE EXISTING NORMAL OPERATION. ANY WORK ON EXISTING EQUIPMENT MUST BE COORDINATED WITH CONTRACTOR. ALSO, WORK SHOULD BE SCHEDULED FOR AN APPROPRIATE MAINTENANCE WINDOW USUALLY IN LOW TRAFFIC PERIODS AFTER MIDNIGHT.
- 17. SINCE THE CELL SITE IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE ADVISED TO BE WORN TO ALERT OF ANY DANGEROUS EXPOSURE LEVELS.

SITE WORK GENERAL NOTES:

- 1. THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES PRIOR TO THE START OF CONSTRUCTION.
- ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE AND THE WORLD AND THE WO SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO:
- B) CONFINED SPACE C) FLECTRICAL SAFETY
- TRENCHING & EXCAVATION.
- 3. ALL SITE WORK SHALL BE AS INDICATED ON THE DRAWINGS AND PROJECT SPECIFICATIONS.
- IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES, TOP SOIL AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
- ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF CONTRACTOR, OWNER AND/OR LOCAL UTILITIES.
- 6. CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION.
- 7. THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE T-MOBILE SPECIFICATION FOR SITE
- THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE TRANSMISSION EQUIPMENT AND TOWER AREAS.
- NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT
- THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION, SEE SOIL COMPACTION NOTES.
- 11. THE AREAS OF THE OWNER'S PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION
- 12. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL JURISDICTION'S GUIDELINES FOR EROSION AND SEDIMENT CONTROL.

ELECTRICAL INSTALLATION NOTES:

- ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE LOCAL CODES.
- CONTRACTOR SHALL MODIFY EXISTING CABLE TRAY SYSTEM AS REQUIRED TO SUPPORT RF AND TRANSPORT CABLING TO THE NEW BTS EQUIPMENT. CONTRACTOR SHALL SUBMIT MODIFICATIONS TO PROJECT MANAGEMENT
- 3. CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT
- WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE
- ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC
- 6. CABLES SHALL NOT BE ROUTED THROUGH LADDER-STYLE CABLE TRAY RUNGS.
- EACH END OF EVERY POWER, POWER PHASE CONDUCTOR (I.E., HOTS), GROUNDING, AND T1 CONDUCTOR AND CABLE SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2 INCH PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC & OSHA, AND MATCH EXISTING INSTALLATION REQUIREMENTS.
- ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH ENGRAVED LAMACOID PLASTIC LABELS. ALL EQUIPMENT SHALL BE LABELED WITH THEIR VOLTAGE RATING, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING, AND BRANCH CIRCUIT ID NUMBERS (I.E., PANELBOARD AND CIRCUIT ID'S).
- PANELBOARDS (ID NUMBERS) AND INTERNAL CIRCUIT BREAKERS (CIRCUIT ID NUMBERS) SHALL BE CLEARLY LABELED WITH ENGRAVED LAMACOID PLASTIC LABELS.
- 10. ALL TIE WRAPS SHALL BE CUT FLUSH WITH APPROVED CUTTING TOOL TO REMOVE SHARP EDGES.
- 11. POWER, CONTROL, AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE CONDUCTOR (SIZE 14 AWG OR LARGER), 600V, OIL RESISTANT THHN OR THWN-Z, CLASS B STRANDED COPPER CABLE RATED FOR 90 'C (WET AND DRY) OPERATION; LISTED OR LABELED FOR THE LOCATION AND RACEWAY SYSTEM USED, UNLESS OTHERWISE SPECIFIED.
- 12. POWER PHASE CONDUCTORS (I.E., HOTS) SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2 INCH PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUIAL) PHASE CONDUCTOR COLOR CODES SHALL CONFORM WITH THE NEC & OSHA AND MATCH EXISTING INSTALLATION REQUIREMENTS.
- 13. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE CONDUCTOR (SIZE 6 AWG OR LARGER), 600V, OIL RESISTANT THHN OR THWN-2 GREEN INSULATION, CLASS B STRANDED COPPER CABLE RATED FOR 90°C (WET AND DRY) OPERATION; LISTED OR LABELED FOR THE LOCATION AND RACEWAY SYSTEM USED, UNLESS OTHERWISE SPECIFIED.
- 14. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED OUTDOORS, OR BELOW GRADE, SHALL BE SINGLE CONDUCTOR #2 AWG SOLID TINNED COPPER CABLE, UNLESS OTHERWISE SPECIFIED.
- 15. POWER AND CONTROL WIRING, NOT IN TUBING OR CONDUIT, SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (SIZE 14 AWG OR LARGER), 600V, OIL RESISTANT THHN OR THWN-2, CLASS B STRANDED COPPER CABLE RATED FOR 90°C (WET AND DRY) OPERATION; WITH OUTER JACKET; LISTED OR LABELED FOR THE LOCATION USED, UNLESS OTHERWISE SPECIFIED.
- 16. ALL POWER AND POWER GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRENUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRENUTS SHALL BE RATED FOR OPERATION AT NO LESS THAN 75°C (90°C IF AVAILABLE).
- 17. RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE, AND NEC.
- 18. NEW RACEWAY OR CABLE TRAY WILL MATCH THE EXISTING INSTALLATION WHERE POSSIBLE.
- 19. ELECTRICAL METALLIC TUBING (EMT) OR RIGID NONMETALLIC CONDUIT (L.E., RIGID PVC SCHEDULE 40. OR RIGID PVC SCHEDULE 80 FOR LOCATIONS SUBJECT TO PHYSICAL DAMAGE) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.
- 20. ELECTRICAL METALLIC TUBING (EMT), ELECTRICAL NONMETALLIC TUBING (ENT), OR RIGID NONMETALLIC CONDUIT (RIGID PVC, SCHEDULE 40) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.
- 21. GALVANIZED STEEL INTERMEDIATE METALLIC CONDUIT (IMC) SHALL BE USED FOR OUTDOOR LOCATIONS ABOVE
- 22. RIGID NONMETALLIC CONDUIT (I.E., RIGID PVC SCHEDULE 40 OR RIGID PVC SCHEDULE 80) SHALL BE USED UNDERGROUND; DIRECT BURIED, IN AREAS OF OCCASIONAL LIGHT VEHICLE TRAFFIC OR ENCASED IN REINFORCED CONCRETE IN AREAS OF HEAVY VEHICLE TRAFFIC.
- 23. LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.
- 24. CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION—TYPE AND APPROVED FOR THE LOCATION USED. SETSCREW FITTINGS ARE NOT ACCEPTABLE.
- 25. CABINETS, BOXES, AND WIREWAYS SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH
- 26. CABINETS, BOXES, AND WIREWAYS TO MATCH THE EXISTING INSTALLATION WHERE POSSIBLE.
- 27. WIREWAYS SHALL BE EPOXY-COATED (GRAY) AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARD; SHALL BE PANDUIT TYPE E (OR EQUAL); AND RATED NEMA 1 (OR BETTER) INDOORS, OR NEMA 3R (OR BETTER) OUTDOORS.
- 28. EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES, AND PULL BOXES SHALL BE GALVANIZED OR EPOXY-COATED SHEET STEEL, SHALL MEET OR EXCEED UL 50, AND RATED NEMA 1 (OR BETTER) INDOORS, OR NEMA 3R (OR BETTER) OUTDOORS.
- 29. METAL RECEPTACLE, SWITCH, AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY-COATED, OR NON-CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1; AND RATED NEMA 1 (OR BETTER) INDOORS, OR WEATHER
- 30. NONMETALLIC RECEPTACLE, SWITCH, AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2; AND RATED NEMA 1 (OR BETTER) INDOORS, OR WEATHER PROTECTED (WP OR BETTER) OUTDOORS.
- 31. THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM PROJECT MANAGEMENT BEFORE
- 32. THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD AGAINST LIFE AND PROPERTY.

CONCRETE AND REINFORCING STEEL NOTES:

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST—IN—PLACE CONCRETE.
- ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS, UNLESS NOTED OTHERWISE, A HIGHER STRENGTH (4000 PSI) MAY BE USED, ALL CONCRETING WORK SHALL BE DONE IN ACCORDANCE WITH ACI 318 CODE REQUIREMENTS.
- REINFORCING STEEL SHALL CONFORM TO ASTM A 615, GRADE 60, DEFORMED UNLESS NOTED OTHERWISE. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185 WELDED STEEL WIRE FABRIC UNLESS NOTED OTHERWISE (UNO). SPLICES SHALL BE CLASS "B" AND ALL HOOKS SHALL BE STANDARD, UNO.
- 4. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN

CONCRETE CAST AGAINST EARTH.......3 IN. CONCRETE EXPOSED TO EARTH OR WEATHER: CONCRETE NOT EXPOSED TO FARTH OR WEATHER OR NOT CAST AGAINST THE GROUND:

- 5. A CHAMFER 3/4" SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNO. IN ACCORDANCE WITH
- INSTALLATION OF CONCRETE EXPANSION/WEDGE ANCHOR, SHALL BE PER MANUFACTURER'S WRITTEN RECOMMENDED PROCEDURE. THE ANCHOR BOLT, DOWEL OR ROD SHALL CONFORM TO MANUFACTURER'S RECOMMENDATION FOR EMBEDMENT DEPTH OR AS SHOWN ON THE DRAWINGS. NO REBAR SHALL BE CUT WITHOUT PRIOR CONTRACTOR APPROVAL WHEN DRILLING HOLES IN CONCRETE. SPECIAL INSPECTIONS, REQUIRED BY GOVERNING CODES, SHALL BE PERFORMED IN ORDER TO MAINTAIN MANUFACTURER'S MAXIMUM ALLOWABLE LOADS. ALL EXPANSION/WEDGE ANCHORS SHALL BE STAINLESS STEEL OR HOT DIPPED GALVANIZED. EXPANSION
- CONCRETE CYLINDER TEST IS NOT REQUIRED FOR SLAB ON GRADE WHEN CONCRETE IS LESS THAN 50 CUBIC YARDS (IBC 1905.6.2.3) IN THAT EVENT THE FOLLOWING RECORDS SHALL BE PROVIDED BY THE CONCRETE SUPPLIER:
 - (A) RESULTS OF CONCRETE CYLINDER TESTS PERFORMED AT THE
 - SUPPLIER'S PLANT.
 - SUPPLIER'S PLANT,

 (B) CERTIFICATION OF MINIMUM COMPRESSIVE STRENGTH FOR
 THE CONCRETE GRADE SUPPLIED.

 FOR GREATER THAN 50 CUBIC YARDS THE GC SHALL PERFORM THE CONCRETE CYLINDER TEST.
- 8. AS AN ALTERNATIVE TO ITEM 7, TEST CYLINDERS SHALL BE TAKEN INITIALLY AND THEREAFTER FOR EVERY 50 YARDS OF CONCRETE FROM EACH DIFFERENT BATCH PLANT.
- EQUIPMENT SHALL NOT BE PLACED ON NEW PADS FOR SEVEN DAYS AFTER PAD IS POURED, UNLESS IT IS VERIFIED BY CYLINDER TESTS THAT COMPRESSIVE STRENGTH HAS BEEN ATTAINED.

STRUCTURAL STEEL NOTES:

- ALL STEEL WORK SHALL BE PAINTED OR GALVANIZED IN ACCORDANCE WITH THE DRAWINGS UNLESS NOTED OTHERWISE. STRUCTURAL STEEL SHALL BE ASTM-A-36 UNLESS OTHERWISE NOTED ON THE SITE SPECIFIC DRAWINGS. STEEL DESIGN, INSTALLATION AND BOLTING SHALL BE PERFORMED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "MANUAL OF STEEL CONSTRUCTION"
- ALL WELDING SHALL BE PERFORMED USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AISC. WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC "MANUAL OF STEEL CONSTRUCTION". PAINTED SURFACES SHALL BE TOUCHED UP.
- BOLTED CONNECTIONS SHALL BE ASTM A325 BEARING TYPE $(3/4^n0)$ Connections and shall have minimum of two bolts unless noted otherwise.
- NON-STRUCTURAL CONNECTIONS FOR STEEL GRATING MAY USE 5/8" DIA. ASTM A 307 BOLTS UNLESS NOTED
- INSTALLATION OF CONCRETE EXPANSION/WEDGE ANCHOR, SHALL BE PER MANUFACTURER'S WRITTEN RECOMMENDED PROCEDURE. THE ANCHOR BOLT, DOWEL OR ROD SHALL CONFORM TO MANUFACTURER'S RECOMMENDATION FOR EMBEDMENT DEPTH OR AS SHOWN ON THE DRAWINGS. NO REBAR SHALL BE CUT WITHOUT PRIOR CONTRACTOR APPROVAL WHEN DRILLING HOLES IN CONCRETE. SPECIAL INSPECTIONS, REQUIRED BY GOVERNING CODES, SHALL BE PERFORMED IN ORDER TO MAINTAIN MANUFACTURER'S MAXIMUM ALLOWABLE ANCHORS SHALL BE STAINLESS STEEL OR HOT DIPPED GALVANIZED. LOADS, ALL EXPANSION/WEDGE BOLTS SHALL BE PROVIDED BY RAMSET/REDHEAD OR APPROVED EQUAL.
- CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ENGINEER REVIEW & APPROVAL ON PROJECTS REQUIRING STRUCTURAL STEEL.
- 7. ALL STRUCTURAL STEEL WORK SHALL BE DONE IN ACCORDANCE WITH AISC SPECIFICATIONS.

CONSTRUCTION NOTES:

- FIELD VERIFICATION CONTRACTOR SHALL FIELD VERIFY SCOPE OF WORK, T-MOBILE ANTENNA PLATFORM LOCATION AND ANTENNAS TO BE REPLACED
- CONTRACTOR SHALL COORDINATE RF WORK AND PROCEDURES WITH PROJECT MANAGEMENT.
- 3. CABLE LADDER RACK: CONTRACTOR SHALL FURNISH AND INSTALL CABLE LADDER RACK, CABLE TRAY, AND CONDUIT AS REQUIRED TO SUPPORT CABLES TO THE NEW BTS LOCATION
- GROUNDING OF ALL EQUIPMENT AND ANTENNAS IS NOT CONSIDERED PART OF THE SCOPE OF THIS PROJECT AND IS THE RESPONSIBILITY OF THE OWNER AND CONTRACTOR AT THE TIME OF CONSTRUCTION, ALL EQUIPMENT AND ANTENNAS TO BE INSTALLED AND GROUNDED IN ACCORDANCE WITH GOVERNING BUILDING CODE, MANUFACTURER RECOMMENDATIONS AND OWNER SPECIFICATIONS.



T-MOBILE NORTHEAST LLC 4 SYLVAN WAY PARSIPPANY, NJ 07054



CROWN CASTLE 500 WEST CUMMINGS PARK, SUITE 3600 WOBURN, MA 01801

CT11069A **GREENWICH NORTH**

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Dewberry Engineers Inc. 600 PARSIPPANY ROAD

PARSIPPANY, NJ 07054 PHONE: 973 739 9400 FAX: 973.739.9710 1111111111



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DRAWN BY:	JC
REVIEWED BY:	BSH

CHECKED BY: GHN

PROJECT NUMBER 50066258 JOB NUMBER: 50072421

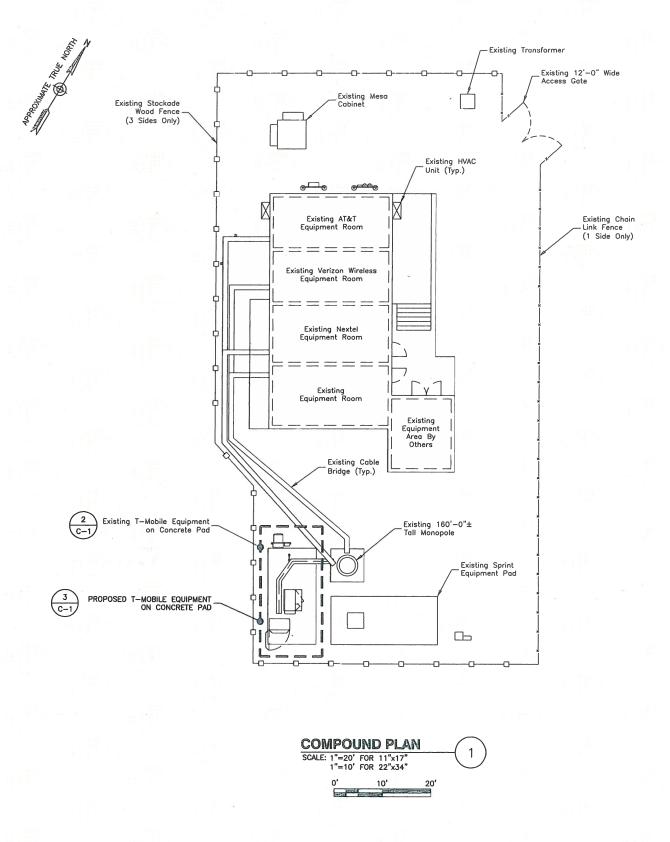
SITE ADDRESS:

363 RIVERSVILLE ROAD GREENWICH, CT 06831 FAIRFIELD COUNTY

SHEET TITLE

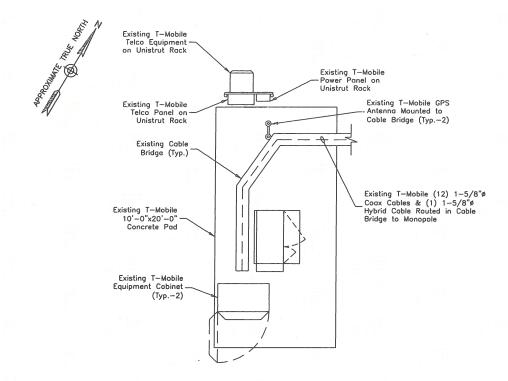
GENERAL NOTES

SHEET NUMBER

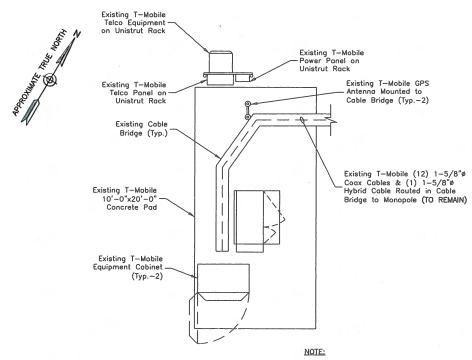


NOTES:

- 1. NORTH ARROW SHOWN AS APPROXIMATE.
- 2. NOT ALL INFORMATION IS SHOWN FOR CLARITY.
- ALL PROPOSED EQUIPMENT, INCLUDING ANTENNAS, RRU'S, COAX, ETC., SHALL BE MOUNTED IN ACCORDANCE WITH THE TOWER STRUCTURAL ANALYSIS BY GPD GROUP DATED FEBRUARY 27, 2015.







1. NO EQUIPMENT IS PROPOSED AT GRADE.





T-MOBILE NORTHEAST LLC 4 SYLVAN WAY PARSIPPANY, NJ 07054



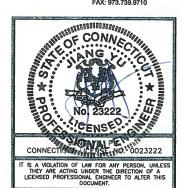
CROWN CASTLE 500 WEST CUMMINGS PARK, SUITE 3600 WOBURN, MA 01801

CT110**6**9A GREENWICH NORTH

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Dewberry Engineers Inc.
600 PARSIPPANY ROAD
SUITE 301
PARSIPPANY, NJ 07054
PHONE: 973,739,9400



DRAWN BY:	JC
REVIEWED BY:	BSH
CHECKED BY:	GHN

PROJECT NUMBER: 50066258

JOB NUMBER: 50072421

SITE ADDRESS:

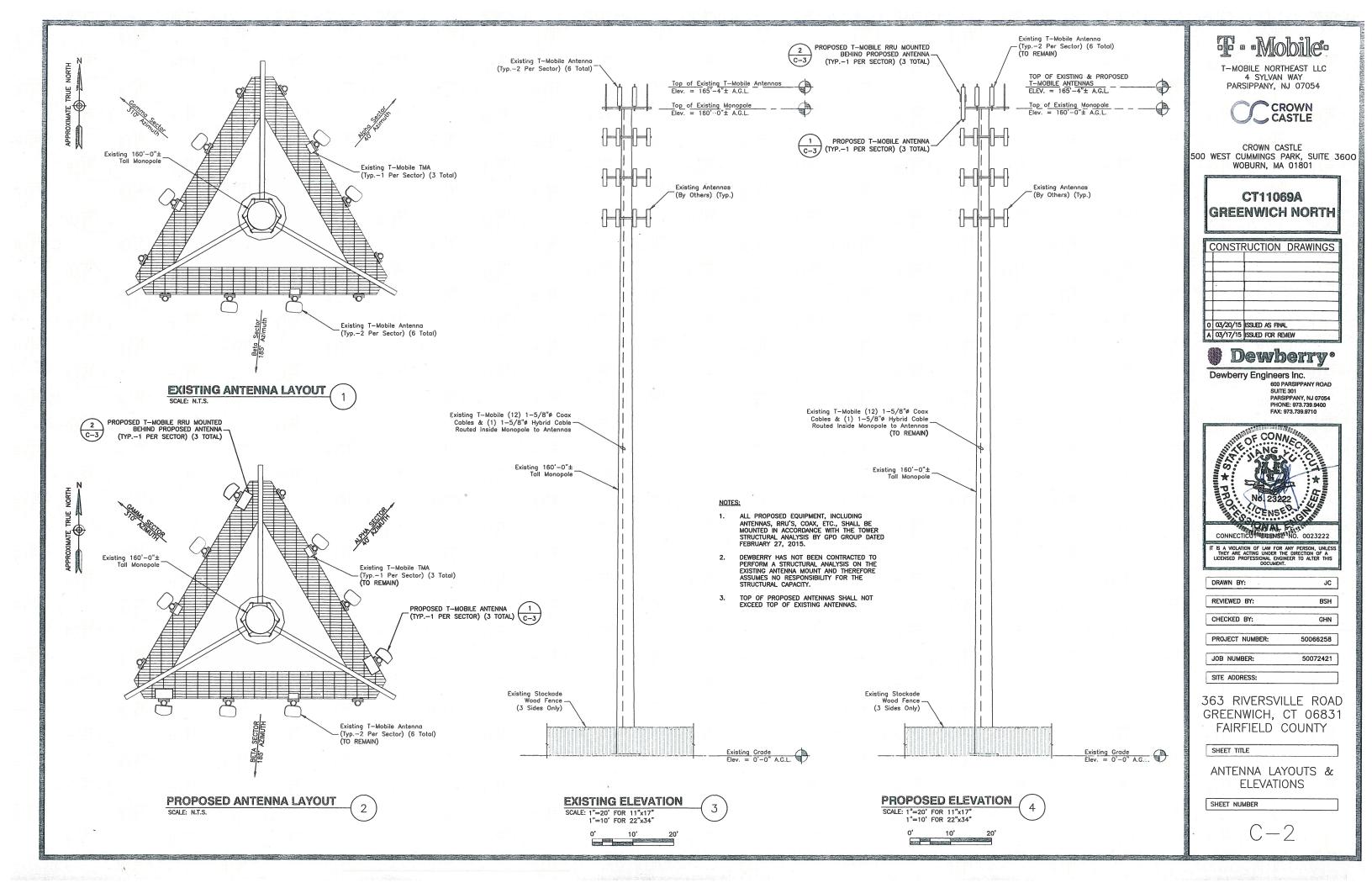
363 RIVERSVILLE ROAD GREENWICH, CT 06831 FAIRFIELD COUNTY

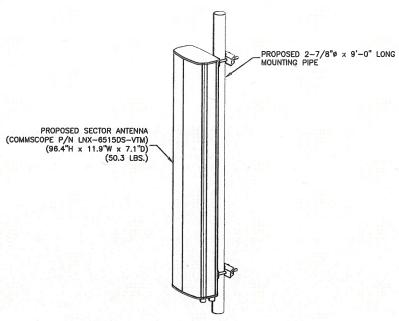
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COMPOUND PLAN & EQUIPMENT PLANS

SHEET NUMBER

C-1

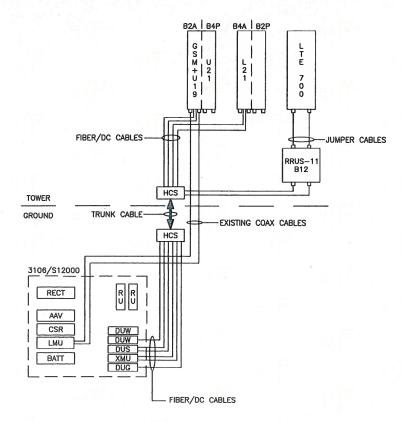




NOTES:

- 1. MOUNT ANTENNAS PER MANUFACTURER'S RECOMMENDATIONS.
- 2. GROUND ANTENNAS AND MOUNTS PER MANUFACTURER'S RECOMMENDATIONS AND T-MOBILE STANDARDS.
- 3. CONFIRM REQUIRED ANTENNAS WITH THE LATEST RFDS.

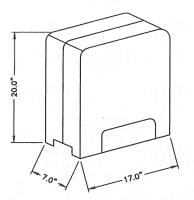
ISOMETRIC ANTENNA DETAIL SCALE: N.T.S.



SITE CONFIGURATION 702Cu

CONTRACTOR SOLLAR

3



ERICSSON RRUS-11 B12

SPECIFICATIONS:
HEIGHT: 20.0"
WIDTH: 17.0"
DEPTH: 7.0"
WEIGHT: 50.7 LBS

RRU NOTES:

- 1. MOUNT EQUIPMENT PER MANUFACTURER'S RECOMMENDATIONS.
- GROUND EQUIPMENT AND MOUNTS PER MANUFACTURER'S RECOMMENDATIONS AND T-MOBILE STANDARDS.
- 3. CONFIRM REQUIRED EQUIPMENT WITH THE LATEST RFDS.

RRUS-11 - REMOTE RADIO UNIT
SCALE: N.T.S.

	4 1	DESIGN CONF	IGURATIO	NC				
	ANTENNAS		COAX		COAX	EXISTING		
	EXISTING	PROPOSED	EXISTING	PROPOSED	LENGTH	HCS		
	ERICSSON AIR21 B2A B4P	EXISTING TO REMAIN						
ALPHA	ERICSSON AIR21 B4A B2P	EXISTING TO REMAIN	(4) 1-5/8"ø	_	213'-0"			
	- H.J	COMMSCOPE LNX-6515DS-VTM	1111			ı		
	ERICSSON AIR21 B2A B4P	EXISTING TO REMAIN						
BETA	ERICSSON AIR21 B4A B2P	EXISTING TO REMAIN	(4) 1-5/8"ø	(4) 1-5/8"ø	(4) 1-5/8"ø -	_	213'-0"	(1) 1-5/8' @ 213'-0'
	-	COMMSCOPE LNX-6515DS-VTM						
	ERICSSON AIR21 B2A B4P	EXISTING TO REMAIN	240 g K					
GAMMA	ERICSSON AIR21 B4A B2P	EXISTING TO REMAIN	(4) 1-5/8"ø	_	213'-0"			
	_ 1/1	COMMSCOPE LNX-6515DS-VTM						

T Mobile

T-MOBILE NORTHEAST LLC 4 SYLVAN WAY PARSIPPANY, NJ 07054



CROWN CASTLE 500 WEST CUMMINGS PARK, SUITE 3600 WOBURN, MA 01801

CT11069A GREENWICH NORTH

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Dewberry Engineers Inc.

600 PARSIPPANY ROAD SUITE 301 PARSIPPANY, NJ 07054 PHONE: 973.739.9400 FAX: 973.739.9710

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CONNECTICUT LICENSE NO. 0023222

If IS A VIOLATION OF LAW FOR ANY PERSON, UNILESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER TO ALTER THIS OCCUMENT.

DRAWN BY:	JC
REVIEWED BY:	BSH
CHECKED BY:	GHN
I AND A REPORT OF	

PROJECT NUMBER: 50066258

JOB NUMBER: 50072421

SITE ADDRESS:

363 RIVERSVILLE ROAD GREENWICH, CT 06831 FAIRFIELD COUNTY

SHEET TITLE

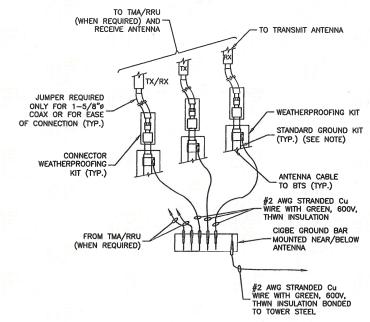
CONSTRUCTION DETAILS

SHEET NUMBER

C - 3

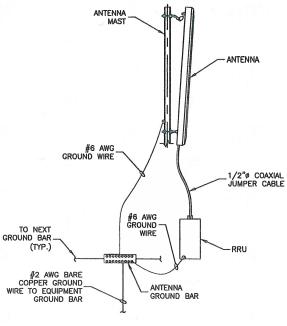
GROUNDING NOTES:

- THE CONTRACTOR SHALL REVIEW AND INSPECT THE EXISTING FACILITY GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRICT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ). THE SITE—SPECIFIC (UL, LPI, OR NFPA) LIGHTING PROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND TIA GROUNDING STANDARDS. THE CONTRACTOR SHALL REPORT ANY VIOLATIONS OR ADVERSE FINDINGS TO THE ENGINEER FOR RESOLUTION.
- ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GES'S) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS, ALL AWALABLE GROUNDING ELECTRODES SHALL BE CONNECTED TOGETHER IN ACCORDANCE WITH THE NEC.
- THE CONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS. USE OF OTHER METHODS MUST BE PRE-APPROVED BY THE
- THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS ON TOWER SITES AND 10 OHMS OR LESS ON ROOFTOP SITES. WHEN ADDING ELECTRODES, CONTRACTOR SHALL MAINTAIN A MINIMUM DISTANCE BETWEEN THE ADDED ELECTRODE AND ANY OTHER EXISTING ELECTRODE EQUAL TO THE BURIED LENGTH OF THE ROD. IDEALLY, CONTRACTOR SHALL STRIVE TO KEEP THE SEPARATION DISTANCE EQUAL TO TWICE THE BURIED LENGTH OF THE RODS.
- THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT.
- METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH 6 AWG COPPER WIRE AND UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO TRANSMISSION EQUIPMENT.
- CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED. BACK-TO-BACK CONNECTIONS ON OPPOSITE SIDES OF THE GROUND BUS ARE PERMITTED.
- ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
- 10. USE OF 90' BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45' BENDS CAN BE ADEQUATELY SUPPORTED. IN ALL CASES, BENDS SHALL BE MADE WITH A MINIMUM BEND RADIUS OF 8
- EACH INTERIOR TRANSMISSION CABINET FRAME/PLINTH SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH 6 AWG STRANDED, GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRE UNLESS NOTED OTHERWISE IN THE DETAILS, EACH OUTDOOR CABINET FRAME/PLINTH STALL BE DIRECTLY CONNECTED TO THE BURIED GROUND RING WITH 2 AWG SOLID TIN-PLATED COPPER WIRE UNLESS NOTED OTHERWISE IN T
- ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING, SHALL BE 2 AWG SOLID TIN-PLATED COPPER UNLESS OTHERWISE INDICATED.
- EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE. CONNECTIONS TO ABOVE GRADE UNITS SHALL BE MADE WITH EXOTHERMIC WELDS WHERE PRACTICAL OR WITH 2 HOLE MECHANICAL TYPE BRASS CONNECTORS WITH STAILESS STEEL HARDWARE, INCLUDING SET SCREWS. HIGH PRESSURE CRIMP CONNECTORS MAY ONLY BE USED WITH WRITTEN PERMISSION FROM T-MOBILE MARKET
- EXOTHERMIC WELDS SHALL BE PERMITTED ON TOWERS ONLY WITH THE EXPRESS APPROVAL OF THE TOWER MANUFACTURER OR THE CONTRACTORS STRUCTURAL ENGINEER.
- 15. ALL WIRE TO WIRE GROUND CONNECTIONS TO THE INTERIOR GROUND RING SHALL BE FORMED USING HIGH PRESS CRIMPS OR SPLIT BOLT CONNECTORS WHERE INDICATED IN THE DETAILS.
- ON ROOFTOP SITES WHERE EXOTHERMIC WELDS ARE A FIRE HAZARD COPPER COMPRESSION CAP CONNECTORS MAY BE USED FOR WIRE TO WIRE CONNECTORS. 2 HOLE MECHANICAL TYPE BRASS CONNECTORS WITH STAINLESS STEEL HARDWARE, INCLUDING SET SCREWS SHALL BE USED FOR CONNECTION TO ALL ROOFTOP TRANSMISSION EQUIPMENT AND
- 17. COAX BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR USING TWO-HOLE MECHANICAL TYPE BRASS CONNECTORS AND STAINLESS STEEL
- APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- 19. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL
- 20. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
- 21. BOND ALL METALLIC OBJECTS WITHIN 6 FT OF THE BURIED GROUND RING WITH 2 AWG SOLID TIM-PLATED COPPER GROUND CONDUCTOR. DURING EXCAVATION FOR NEW GROUND CONDUCTORS, IF EXISTING GROUND CONDUCTORS ARE ENCOUNTERED, BOND EXISTING GROUND CONDUCTORS ARE ENCOUNTERED, BOND EXISTING GROUND CONDUCTORS.
- 22. GROUND CONDUCTORS USED IN THE FACILITY GROUND AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC PLASTIC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (E.G., NON-METALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT WITH LISTED BONDING

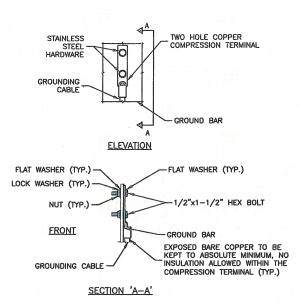


1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND

CONNECTION OF GROUND WIRES TO GROUNDING BAR (CIGBE)



TYPICAL ANTENNA **GROUNDING DETAIL** SCALE: N.T.S



NOTES:

- 1. DOUBLING UP OR STACKING OF CONNECTIONS IS NOT PERMITTED.
- 2. OXIDE INHIBITING COMPOUND TO BE USED AT ALL LOCATIONS

TYPICAL GROUND BAR **MECHANICAL CONNECTION DETAIL**

ANTENNA COAX TOP ANTENNA GROUND RRU GROUND #6 AWG Gnd Bar #2 AWG Stranded Typ.-3 Sectors Top MGB 00000000 #2 AWG Stranded Lower MGB 90000000 Ground to Existing

NOTES:

- 1. BOND ANTENNA GROUNDING KIT CABLE TO TOP CIGBE
- 2. BOND ANTENNA GROUNDING KIT CABLE TO BOTTOM CIGBE
- 3. SCHEMATIC GROUNDING DIAGRAM IS TYPICAL FOR FACH SECTOR
- VERIFY EXISTING GROUND SYSTEM IS INSTALLED PER T-MOBILE

SCHEMATIC GROUNDING DIAGRAM SCALE: N.T.S.



T-MOBILE NORTHEAST LLC 4 SYLVAN WAY PARSIPPANY, NJ 07054



CROWN CASTLE 500 WEST CUMMINGS PARK, SUITE 3600 WOBURN, MA 01801

CT11069A **GREENWICH NORTH**

	CONSTR	RUCTION DRAWINGS
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0	03/20/15	ISSUED AS FINAL
Α	03/17/15	ISSUED FOR REVIEW



Dewberry Engineers Inc.

600 PARSIPPANY ROAD SUITE 301 PARSIPPANY, NJ 07054 PHONE: 973.739.9400 FAX: 973.739.9710



REVIEWED BY:	BSH
CHECKED BY:	GHN
PROJECT NUMBER:	50066258
JOB NUMBER:	50072421

JC

363 RIVERSVILLE ROAD GREENWICH, CT 06831 FAIRFIELD COUNTY

SHEET TITLE

SITE ADDRESS:

DRAWN BY:

GROUNDING NOTES & DETAILS

SHEET NUMBER



Date: February 27, 2015

Sean Dempsey Crown Castle 3530 Toringdon Way Suite 300 Charlotte, NC 28277 (704) 405-6565 GPD Group 520 South Main St, Suite 2531 Akron, OH 44311 (614) 859-1607

dpalkovic@gpdgroup.com

Subject: Structural Analysis Report

Carrier Designation: T-Mobile Co-Locate

Carrier Site Number: CT11069A

Carrier Site Name: Greenwich/Boy Scouts_2

Crown Castle Designation: Crown Castle BU Number: 841290

Crown Castle Site Name: GREENWICH NORTH

Crown Castle JDE Job Number: 324170
Crown Castle Work Order Number: 1014237
Crown Castle Application Number: 284770 Rev. 0

Engineering Firm Designation: GPD Group Project Number: 2015777.841290.02

Site Data: 363 Riversville Road, Greenwich, CT 06831, Fairfield County

Latitude 41° 3′ 58.6″, Longitude -73° 40′ 17.4″

160 Foot - EEI Monopole Tower

Dear Sean Dempsey,

GPD Group is pleased to submit this "**Structural Analysis Report**" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 759639, in accordance with application 284770, revision 0.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment

Sufficient Capacity

Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

The analysis has been performed in accordance with the TIA/EIA-222-F standard and the 2005 Connecticut (CT) State Building Code based upon a wind speed of 85 mph fastest mile.

We at *GPD Group* appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Structural analysis prepared by: Kevin Liccar, El

Respectfully submitted by:

John N. Kabak, P.E. Connecticut #: PEN.0028336



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1) INTRODUCTION

The existing 160' monopole consists of five major sections that are connected with slip joints and a bolted flange connection at the 152' elevation. It has an 18-sided cross section and is evenly tapered from 59" (flat-flat) at the base to 29" (flat-flat) at the top. The structure is galvanized and does not have aviation lighting.

The tower was designed for the URS Corporation by Engineered Endeavors, Incorporated (EEI) of Mentor, Ohio in April of 2003. The tower was also designed for a basic wind speed of 85 mph with ½" radial ice (with a 25% reduction in wind speed when wind and ice loads were considered simultaneously) in accordance with the TIA/EIA-222-F standard.

2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures and the 2005 Connecticut (CT) State Building using a fastest mile wind speed of 85 mph with no ice, 38 mph with 0.75 inch ice thickness (in accordance with ASCE 7 ice conditions) and 50 mph under service loads.

Table 1 - Proposed Antenna and Cable Information

Mounting Level (ft)	Fla a4! a	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
160.0	163.0	3	Commscope	LNX-6515DS-VTM			
100.0	103.0	3	Ericsson	RRUS 11 B12			

Table 2 - Existing and Reserved Antenna and Cable Information

Mounting Level (ft)	Center	Number of Antennas	Antenna and Cable Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
		2	Communication Components Inc.	DTMA-1819-DD-12			1
	163.0	1	RFS Celwave	ATMAA1412D-1A20			
160.0	163.0	3	Ericsson	ERICSSON AIR 21 B2A B4P			
		3	Ericsson	ERICSSON AIR 21 B4A B2P	13	1-5/8	
		3	RFS Celwave	ATMAA1412D-1A20	13	1-5/0	
	160.0	1		Platform Mount [LP 1201-1]			
		3	Ericsson	TME-RRUS-11			
153.0	153.0	1	Raycap	TME-DC6-48-60-18-8F	2	3/8	
		1		Side Arm Mount [SO 102-3]			
	151.0	6	Powerwave Tech.	7770.00			
	131.0	6	Powerwave Tech.	LGP21401			
149.0		6	Powerwave Tech.	LGP21401	12	1-5/8	
	149.0	3	Powerwave Tech.	P65-16-XLH-RR			
		1		Platform Mount [LP 1201-1]			
		6	Amphenol	WWX063X19G00		1-5/8	
	142.0	1	Antel	BXA-70063/8CFx2	2		2
		1	RFS Celwave	DB-T1-6Z-8AB-0Z			
		2	Decibel	DB844H80E-XY			ľ
140.0		2	Powerwave Tech.	P65-16-XL-M			
		4	RFS Celwave	APL868013-42T0	18	1-5/8	
	140.0	1		Platform Mount [LP 1201-1]			
		3	Alcatel Lucent	TME-RRH2x60 AWS			2
		3	Alcatel Lucent	TME-RRH2X60-PCS			2
		3	Alcatel Lucent	TD-RRH8x20-25	4	4.4/4	
	400.0	3	RFS Celwave	APXVTM14-C-120	1	1-1/4	2
120.0	122.0	1		Handrail Kit [NA 510-1]			
		3	RFS Celwave	APXVSPP18-C-A20	3 2	1-1/4 1/2	,
	120.0	1		Platform Mount [LP 1201-1]		1/2	
		3	Alcatel Lucent	TME-1900MHz RRH			
119.0	119.0	3	Alcatel Lucent	TME-800MHZ RRH			
		1		Side Arm Mount [SO 102-3]			
70.0	73.0	2	GPS	GPS_A		4 /0	
72.0	72.0	1		Side Arm Mount [SO 701-1]	2	1/2	

Notes:

Existing equipment to be removed; not considered in this analysis

1) 2) Reserved Equipment

Table 3 - Design Antenna and Cable Information

Mounting Level (ft)	Flevation	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
160	160	3		Panel Antenna		
150	150	1		Low Profile Platform		
130	130	12	Allgon	ALP 11011		
140	140	1		Low Profile Platform		
140	140	12	Allgon	ALP 11011		
130	130	1		Low Profile Platform		
130	130	12	Allgon	ALP 11011		

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

Document	Remarks	Reference	Source
Tower Drawings / Specifications	EEI Project #: 5590, Dated 04/10/2003	Doc ID #: 5121537	CCIsites
Foundation Drawings / Specifications	EEI Project #: 5590, Dated 04/10/2003	Doc ID #: 5121536	CCIsites
Foundation Exploration Report	WEI Project #: 2009-895, Dated 09/04/2009	Doc ID #: 4468638	CCIsites
Geotechnical Report	WEI Project #: 2009-895, Dated 09/04/2009	Doc ID #: 5121535	CCIsites

3.1) Analysis Method

tnxTower (version 6.1.4.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by TIA/EIA-222-F.

This analysis may be affected if any assumptions are not valid or have been made in error. GPD Group should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	160 - 152	Pole	TP30.62x29x0.1875	1	-3.56	908.48	10.9	Pass
L2	152 - 111.29	Pole	TP38.86x30.62x0.25	2	-16.35	1547.47	59.3	Pass
L3	111.29 - 77.42	Pole	TP45.09x37.263x0.3125	3	-23.23	2245.56	82.1	Pass
L4	77.42 - 36.46	Pole	TP52.62x43.2359x0.4375	4	-35.42	3665.31	76.9	Pass
L5	36.46 - 0	Pole	TP59x50.3353x0.5	5	-52.21	4826.45	77.6	Pass
							Summary	
						Pole (L3)	82.1	Pass
						Rating =	82.1	Pass

Table 6 - Tower Component Stresses vs. Capacity - LC7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	66.9	Pass
1	Base Plate	0	84.2	Pass
1	Base Foundation	0	25.0	Pass
1	Base Foundation Soil Interaction	0	53.8	Pass
1	Flange Bolts	152	14.2	Pass
1	Flange Plate	152	13.0	Pass

Structure Rating (max from all components) = 84.2%
--

Notes:

4.1) Recommendations

The existing tower and its foundation are sufficient for the proposed loading and do not require modifications.

¹⁾ See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

5) DISCLAIMER OF WARRANTIES

GPD GROUP has not performed a site visit to the tower to verify the member sizes or antenna/coax loading. If the existing conditions are not as represented on the tower elevation contained in this report, we should be contacted immediately to evaluate the significance of the discrepancy. This is not a condition assessment of the tower or foundation. This report does not replace a full tower inspection. The tower and foundations are assumed to have been properly fabricated, erected, maintained, in good condition, twist free, and plumb.

The engineering services rendered by GPD GROUP in connection with this Structural Analysis are limited to a computer analysis of the tower structure and theoretical capacity of its main structural members. No allowance was made for any damaged, bent, missing, loose, or rusted members (above and below ground). No allowance was made for loose bolts or cracked welds.

This analysis is limited to the designated maximum wind and seismic conditions per the governing tower standards and code. Wind forces resulting in tower vibrations near the structure's resonant frequencies were not considered in this analysis and are outside the scope of this analysis. Lateral loading from any dynamic response was not evaluated under a time-domain based fatigue analysis.

GPD GROUP does not analyze the fabrication of the structure (including welding). It is not possible to have all the very detailed information needed to perform a thorough analysis of every structural sub-component and connection of an existing tower. GPD GROUP provides a limited scope of service in that we cannot verify the adequacy of every weld, plate connection detail, etc. The purpose of this report is to assess the capability of adding appurtenances usually accompanied by transmission lines to the structure.

It is the owner's responsibility to determine the amount of ice accumulation in excess of the code specified amount, if any, that should be considered in the structural analysis.

The attached sketches are a schematic representation of the analyzed tower. If any material is fabricated from these sketches, the contractor shall be responsible for field verifying the existing conditions, proper fit, and clearance in the field. Any mentions of structural modifications are reasonable estimates and should not be used as a precise construction document. Precise modification drawings are obtainable from GPD GROUP, but are beyond the scope of this report.

Miscellaneous items such as antenna mounts, etc., have not been designed or detailed as a part of our work. We recommend that material of adequate size and strength be purchased from a reputable tower manufacturer.

Towers are designed to carry gravity, wind, and ice loads. All members, legs, diagonals, struts, and redundant members provide structural stability to the tower with little redundancy. Absence or removal of a member can trigger catastrophic failure unless a substitute is provided before any removal. Legs carry axial loads and derive their strength from shorter unbraced lengths by the presence of redundant members and their connection to the diagonals with bolts or welds. If the bolts or welds are removed without providing any substitute to the frame, the leg is subjected to a higher unbraced length that immediately reduces its load carrying capacity. If a diagonal is also removed in addition to the connection, the unbraced length of the leg is greatly increased, jeopardizing its load carrying capacity. Failure of one leg can result in a tower collapse because there is no redundancy. Redundant members and diagonals are critical to the stability of the tower.

GPD GROUP makes no warranties, expressed and/or implied, in connection with this report and disclaims any liability arising from material, fabrication, and erection of this tower. GPD GROUP will not be responsible whatsoever for, or on account of, consequential or incidental damages sustained by any person, firm, or organization as a result of any data or conclusions contained in this report. The maximum liability of GPD GROUP pursuant to this report will be limited to the total fee received for preparation of this report.

APPENDIX A BASE LEVEL DRAWING

30.6200 29.0000 0.1875 8.00 8 0.5 152.0 ft 0.2500 5.45 40.71 8 Ŋ 111.3 ft 39.29 6.17 5.4 A572-65 77.4 ft 52.6200 8 36.5 ft 43.54 59.0000 12.7 8 0.0 ft 33.0 Number of Sides Thickness (in) Top Dia (in) Bot Dia (in) Length (ft) Weight (K)

DESIGNED APPURTENANCE LOADING

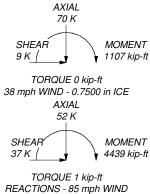
TYPE	ELEVATION	TYPE	ELEVATION
Platform Mount [LP 1201-1]	160	Platform Mount [LP 1201-1]	140
ERICSSON AIR 21 B2A B4P w/ Mount	160	P65-16-XL-M w/ Mount Pipe	140
Pipe		P65-16-XL-M w/ Mount Pipe	140
ERICSSON AIR 21 B2A B4P w/ Mount	160	(2) APL868013-42T0 w/ Mount Pipe	140
Pipe		(2) APL868013-42T0 w/ Mount Pipe	140
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	160	TME-RRH2X60-PCS	140
ERICSSON AIR 21 B4A B2P w/ Mount	160	TME-RRH2X60-PCS	140
Pipe	160	TME-RRH2X60-PCS	140
ERICSSON AIR 21 B4A B2P w/ Mount	160	TME-RRH2x60 AWS	140
Pipe		TME-RRH2x60 AWS	140
ERICSSON AIR 21 B4A B2P w/ Mount	160	TME-RRH2x60 AWS	140
Pipe		(2) WWX063X19G00 w/ Mount Pipe	140
ATMAA1412D-1A20	160	(2) WWX063X19G00 w/ Mount Pipe	140
ATMAA1412D-1A20	160	(2) WWX063X19G00 w/ Mount Pipe	140
ATMAA1412D-1A20	160	DB-T1-6Z-8AB-0Z	140
Pipe Mount 6'x2.375"	160	BXA-70063/8CFx2 w/ Mount Pipe	140
Pipe Mount 6'x2.375"	160	(2) DB844H80E-XY w/ Mount Pipe	140
Pipe Mount 6'x2.375"	160	Handrail Kit [NA 510-1]	122
LNX-6515DS-VTM w/ Mount Pipe	160	Platform Mount [LP 1201-1]	120
LNX-6515DS-VTM w/ Mount Pipe	160	APXVSPP18-C-A20 w/ Mount Pipe	120
LNX-6515DS-VTM w/ Mount Pipe	160	APXVSPP18-C-A20 w/ Mount Pipe	120
RRUS 11 B12	160	APXVSPP18-C-A20 w/ Mount Pipe	120
RRUS 11 B12	160	(2) 6' x 2" Mount Pipe	120
RRUS 11 B12	160	(2) 6' x 2" Mount Pipe	120
Side Arm Mount [SO 102-3]	153	(2) 6' x 2" Mount Pipe	120
TME-RRUS-11	153	APXVTM14-C-120 w/ Mount Pipe	120
TME-RRUS-11	153	APXVTM14-C-120 w/ Mount Pipe	120
TME-RRUS-11	153	APXVTM14-C-120 w/ Mount Pipe	120
TME-DC6-48-60-18-8F	153	TD-RRH8x20-25	120
Platform Mount [LP 1201-1]	149	TD-RRH8x20-25	120
(2) 7770.00 w/ Mount Pipe	149	TD-RRH8x20-25	120
(2) 7770.00 w/ Mount Pipe	149	Side Arm Mount [SO 102-3]	119
(2) 7770.00 w/ Mount Pipe	149	TME-1900MHz RRH w/ 4' x 2" Mount	119
P65-16-XLH-RR w/ mount pipe	149	Pipe	
P65-16-XLH-RR w/ mount pipe	149	TME-1900MHz RRH w/ 4' x 2" Mount	119
P65-16-XLH-RR w/ mount pipe	149	Pipe	
(2) LGP21401	149	TME-1900MHz RRH w/ 4' x 2" Mount	119
(2) LGP21401	149	Pipe	
(2) LGP21401	149	TME-800MHZ RRH	119
(2) LGP21401	149	TME-800MHZ RRH	119
(2) LGP21401	149	TME-800MHZ RRH	119
(2) LGP21401	149	Side Arm Mount [SO 701-1]	72
	1	(2) GPS_A	72

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
Δ572-65	65 kei	80 kei			

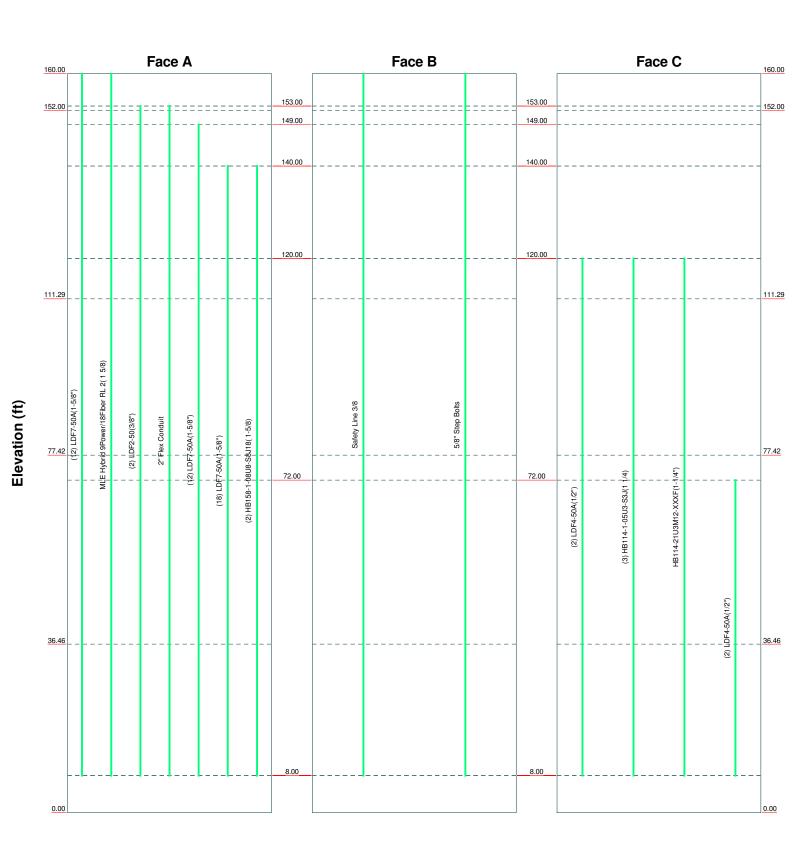
TOWER DESIGN NOTES

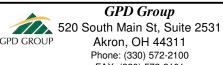
- 1. Tower is located in Fairfield County, Connecticut.
- Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
 Tower is also designed for a 38 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
- 4. Deflections are based upon a 50 mph wind.5. TOWER RATING: 82.1%



ob: GREENWICH NORT	H (BU #: 84	1290)
Project: 2015777.841290.02		
Client: Crown Castle USA, Inc.		
Code: TIA/EIA-222-F	Date: 02/27/15	Scale: NTS
Path: T:\Crown\841290\02\tnx\841290.eri		Dwg No. E-1

| U - 16U | | App Out Face | Truss Leg





FAX: (330) 572-2101

GREENWICH NORTH (BU #: 841290)								
roject: 2015777.841290.02								
lient: Crown Castle USA, Inc.	Drawn by: kliccar	App'd:						
ode: TIA/EIA-222-F	Date: 02/27/15	Scale: NT						
ath: T:\Crown\841290\02\tny\841290 eri	,	Dwg No. F-						

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Client	0 0 11 1104 1	Designed by
	Crown Castle USA, Inc.	kliccar

Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in Fairfield County, Connecticut.

Basic wind speed of 85 mph.

Nominal ice thickness of 0.7500 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 38 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

- √ Use Code Stress Ratios
- √ Use Code Safety Factors Guys
 - Escalate Ice
 Always Use Max Kz
 Use Special Wind Profile
 Include Bolts In Member Capacity
 Leg Bolts Are At Top Of Section
 Secondary Horizontal Braces Leg
 Use Diamond Inner Bracing (4 Sided)
 Add IBC .6D+W Combination

Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
 Use Clear Spans For Wind Area
 Use Clear Spans For KL/r
 Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- V See Azimuti Dish Coefficients

 √ Project Wind Area of Appurt.

 Autocalc Torque Arm Areas

 SR Members Have Cut Ends

 Sort Capacity Reports By Component

 Triangulate Diamond Inner Bracing

 Use TIA-222-G Tension Splice Capacity

 Exemption

Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation

√ Consider Feedline Torque
Include Angle Block Shear Check
Poles

 ✓ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets

Tapered Pole Section Geometry

Section	Elevation	Section	Splice	Number	Тор	Bottom	Wall	Bend	Pole Grade
		Length	Length	of	Diameter	Diameter	Thickness	Radius	
	ft	ft	ft	Sides	in	in	in	in	
L1	160.00-152.00	8.00	0.00	18	29.0000	30.6200	0.1875	0.7500	A572-65 (65 ksi)
L2	152.00-111.29	40.71	5.42	18	30.6200	38.8600	0.2500	1.0000	A572-65
L3	111.29-77.42	39.29	6.17	18	37.2630	45.0900	0.3125	1.2500	(65 ksi) A572-65
									(65 ksi)
L4	77.42-36.46	47.13	7.08	18	43.2359	52.6200	0.4375	1.7500	A572-65 (65 ksi)
L5	36.46-0.00	43.54		18	50.3353	59.0000	0.5000	2.0000	À572-65
									(65 ksi)

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Client	Crown Castle USA, Inc.	Designed by kliccar

Tapered Pole Properties

Section	Tip Dia.	Area	I	r	С	I/C	J	It/Q	w	w/t
	in	in^2	in^4	in	in	in^3	in^4	in^2	in	
L1	29.4474	17.1470	1798.4090	10.2284	14.7320	122.0750	3599.1844	8.5751	4.7740	25.461
	31.0924	18.1111	2119.1346	10.8035	15.5550	136.2353	4241.0576	9.0573	5.0591	26.982
L2	31.0924	24.0986	2808.1400	10.7814	15.5550	180.5302	5619.9750	12.0516	4.9491	19.796
	39.4595	30.6370	5770.1059	13.7066	19.7409	292.2922	11547.8043	15.3214	6.3994	25.597
L3	38.9342	36.6502	6321.9884	13.1174	18.9296	333.9740	12652.2955	18.3286	6.0083	19.226
	45.7856	44.4137	11250.5543	15.8960	22.9057	491.1679	22515.9125	22.2111	7.3858	23.635
L4	45.1503	59.4309	13753.2027	15.1934	21.9638	626.1754	27524.5022	29.7211	6.8395	15.633
	53.4317	72.4619	24928.5533	18.5248	26.7310	932.5723	49889.9082	36.2378	8.4911	19.408
L5	52.5425	79.0886	24815.6294	17.6915	25.5703	970.4854	49663.9118	39.5518	7.9790	15.958
	59.9102	92.8395	40140.4258	20.7675	29.9720	1339.2642	80333.6694	46.4286	9.5040	19.008

Tower	Gusset	Gusset	Gusset Grade Adjust. Factor	Adjust.	Weight Mult.		Double Angle
Elevation	Area	Thickness	A_f	Factor		Stitch Bolt	Stitch Bolt
	(per face)			A_r		Spacing	Spacing
						Diagonals	Horizontals
ft	ft^2	in				in	in
L1			1	1	1		
160.00-152.00							
L2			1	1	1		
152.00-111.29							
L3			1	1	1		
111.29-77.42							
L4 77.42-36.46			1	1	1		
L5 36.46-0.00			1	1	1		

Feed Line/Linear Appurtenances - Entered As Area

Description	Face	Allow	Component	Placement	Total		$C_A A_A$	Weight
	or	Shield	Type		Number			
	Leg			ft			ft²/ft	plf
LDF7-50A(1-5/8")	A	No	Inside Pole	160.00 - 8.00	12	No Ice	0.00	0.82
						1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82
						2" Ice	0.00	0.82
						4" Ice	0.00	0.82
MLE Hybrid	A	No	Inside Pole	160.00 - 8.00	1	No Ice	0.00	1.07
9Power/18Fiber RL 2(1						1/2" Ice	0.00	1.07
5/8)						1" Ice	0.00	1.07
						2" Ice	0.00	1.07
						4" Ice	0.00	1.07
LDF2-50(3/8")	Α	No	Inside Pole	153.00 - 8.00	2	No Ice	0.00	0.08
						1/2" Ice	0.00	0.08
						1" Ice	0.00	0.08
						2" Ice	0.00	0.08
						4" Ice	0.00	0.08
2" Flex Conduit	Α	No	Inside Pole	153.00 - 8.00	1	No Ice	0.00	0.32
						1/2" Ice	0.00	0.32
						1" Ice	0.00	0.32
						2" Ice	0.00	0.32
						4" Ice	0.00	0.32
LDF7-50A(1-5/8")	A	No	Inside Pole	149.00 - 8.00	12	No Ice	0.00	0.82
						1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82
						2" Ice	0.00	0.82
						4" Ice	0.00	0.82
LDF7-50A(1-5/8")	A	No	Inside Pole	140.00 - 8.00	18	No Ice	0.00	0.82

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Description	Face or	Allow Shield	Component Type	Placement	Total Number		$C_A A_A$	Weight
	Leg	Snieia	Туре	ft	Number		ft²/ft	plf
	Les			Ji		1/2" Ice	0.00	$\frac{p_{ij}}{0.82}$
						1" Ice	0.00	0.82
						2" Ice	0.00	0.82
						4" Ice	0.00	0.82
HB158-1-08U8-S8J18(Α	No	Inside Pole	140.00 - 8.00	2	No Ice	0.00	1.30
1-5/8)	11	110	mside i oic	140.00 0.00	2	1/2" Ice	0.00	1.30
1 3/0)						1" Ice	0.00	1.30
						2" Ice	0.00	1.30
						4" Ice	0.00	1.30
LDF4-50A(1/2")	C	No	Inside Pole	120.00 - 8.00	2	No Ice	0.00	0.15
EDI + 3011(1/2)	C	110	mside i oic	120.00 0.00	2	1/2" Ice	0.00	0.15
						1" Ice	0.00	0.15
						2" Ice	0.00	0.15
						4" Ice	0.00	0.15
HB114-1-05U3-S3J(1	C	No	Inside Pole	120.00 - 8.00	3	No Ice	0.00	0.90
1/4)	C	110	more role	120.00 0.00	5	1/2" Ice	0.00	0.90
171)						1" Ice	0.00	0.90
						2" Ice	0.00	0.90
						4" Ice	0.00	0.90
HB114-21U3M12-XXX	С	No	Inside Pole	120.00 - 8.00	1	No Ice	0.00	1.22
F(1-1/4")						1/2" Ice	0.00	1.22
, ,						1" Ice	0.00	1.22
						2" Ice	0.00	1.22
						4" Ice	0.00	1.22
LDF4-50A(1/2")	С	No	Inside Pole	72.00 - 8.00	2	No Ice	0.00	0.15
						1/2" Ice	0.00	0.15
						1" Ice	0.00	0.15
						2" Ice	0.00	0.15
						4" Ice	0.00	0.15
Safety Line 3/8	В	No	CaAa (Out Of	160.00 - 8.00	1	No Ice	0.04	0.22
•			Face)			1/2" Ice	0.14	0.75
			,			1" Ice	0.24	1.28
						2" Ice	0.44	2.34
						4" Ice	0.84	4.46
5/8" Step Bolts	В	No	CaAa (Out Of	160.00 - 8.00	1	No Ice	0.04	1.00
1			Face)			1/2" Ice	0.14	1.56
			,			1" Ice	0.24	2.73
						2" Ice	0.44	6.91
						4" Ice	0.84	22.58

Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	A_R	A_F	C_AA_A	$C_A A_A$	Weight
Section	Elevation				In Face	Out Face	
	ft		ft^2	ft^2	ft^2	ft^2	K
L1	160.00-152.00	A	0.000	0.000	0.000	0.000	0.09
		В	0.000	0.000	0.000	0.633	0.01
		C	0.000	0.000	0.000	0.000	0.00
L2	152.00-111.29	A	0.000	0.000	0.000	0.000	1.33
		В	0.000	0.000	0.000	3.223	0.05
		C	0.000	0.000	0.000	0.000	0.04
L3	111.29-77.42	A	0.000	0.000	0.000	0.000	1.31
		В	0.000	0.000	0.000	2.681	0.04
		C	0.000	0.000	0.000	0.000	0.14
L4	77.42-36.46	A	0.000	0.000	0.000	0.000	1.58
		В	0.000	0.000	0.000	3.243	0.05
		C	0.000	0.000	0.000	0.000	0.18
L5	36.46-0.00	A	0.000	0.000	0.000	0.000	1.10
		В	0.000	0.000	0.000	2.253	0.03

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Tower	Tower	Face	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation				In Face	Out Face	
	ft		ft^2	ft^2	ft^2	ft^2	K
		С	0.000	0.000	0.000	0.000	0.13

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation	or	Thickness			In Face	Out Face	
	ft	Leg	in	ft^2	ft^2	ft^2	ft^2	K
L1	160.00-152.00	A	0.904	0.000	0.000	0.000	0.000	0.09
		В		0.000	0.000	0.000	3.525	0.03
		C		0.000	0.000	0.000	0.000	0.00
L2	152.00-111.29	A	0.885	0.000	0.000	0.000	0.000	1.33
		В		0.000	0.000	0.000	17.634	0.15
		C		0.000	0.000	0.000	0.000	0.04
L3	111.29-77.42	A	0.851	0.000	0.000	0.000	0.000	1.31
		В		0.000	0.000	0.000	14.671	0.12
		C		0.000	0.000	0.000	0.000	0.14
L4	77.42-36.46	A	0.801	0.000	0.000	0.000	0.000	1.58
		В		0.000	0.000	0.000	17.178	0.14
		C		0.000	0.000	0.000	0.000	0.18
L5	36.46-0.00	A	0.750	0.000	0.000	0.000	0.000	1.10
		В		0.000	0.000	0.000	11.368	0.09
		C		0.000	0.000	0.000	0.000	0.13

Feed Line Center of Pressure

Section	Elevation	CP_X	CP_Z	CP_X	CP_Z
				Ice	Ice
	ft	in	in	in	in
L1	160.00-152.00	0.0997	0.0575	0.4624	0.2669
L2	152.00-111.29	0.1001	0.0578	0.4687	0.2706
L3	111.29-77.42	0.1005	0.0581	0.4822	0.2784
L4	77.42-36.46	0.1009	0.0582	0.4784	0.2762
L5	36.46-0.00	0.0781	0.0451	0.3641	0.2102

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C_AA_A Front	C_AA_A Side	Weight
			ft ft ft	0	ft		ft ²	ft ²	K
Platform Mount [LP 1201-1]	С	None	,	0.0000	160.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	23.10 26.80 30.50 37.90 52.70	23.10 26.80 30.50 37.90 52.70	2.10 2.50 2.90 3.70 5.30
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	A	From Centroid-Fa ce	4.00 0.00 3.00	0.0000	160.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	6.90 7.46 8.00 9.10 11.44	5.72 6.63 7.42 9.07 12.58	0.11 0.17 0.24 0.39 0.82
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	В	From Centroid-Fa ce	4.00 0.00 3.00	0.0000	160.00	No Ice 1/2" Ice 1" Ice	6.90 7.46 8.00	5.72 6.63 7.42	0.11 0.17 0.24

Job		Page
	GREENWICH NORTH (BU #: 841290)	5 of 13
Project		Date
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Client	Crown Castle USA, Inc.	Designed by kliccar

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		$C_A A_A$ Front	C_AA_A Side	Weigh
			Vert ft ft ft	o	ft		ft²	ft ²	K
			J.			2" Ice	9.10	9.07	0.39
						4" Ice	11.44	12.58	0.82
ERICSSON AIR 21 B2A	C	From	4.00	0.0000	160.00	No Ice	6.90	5.72	0.11
B4P w/ Mount Pipe		Centroid-Fa	0.00			1/2" Ice	7.46	6.63	0.17
		ce	3.00			1" Ice	8.00	7.42	0.24
						2" Ice	9.10	9.07	0.39
						4" Ice	11.44	12.58	0.82
ERICSSON AIR 21 B4A	Α	From	4.00	0.0000	160.00	No Ice	6.90	5.72	0.11
B2P w/ Mount Pipe		Centroid-Fa	0.00			1/2" Ice	7.46	6.63	0.17
		ce	3.00			1" Ice	8.00	7.42	0.24
						2" Ice	9.10	9.07	0.39
	_	_				4" Ice	11.44	12.58	0.82
ERICSSON AIR 21 B4A	В	From	4.00	0.0000	160.00	No Ice	6.90	5.72	0.11
B2P w/ Mount Pipe		Centroid-Fa	0.00			1/2" Ice	7.46	6.63	0.17
		ce	3.00			1" Ice	8.00	7.42	0.24
						2" Ice	9.10	9.07	0.39
CDICCCON AID 21 D4A	0	Б	4.00	0.0000	160.00	4" Ice	11.44	12.58	0.82
ERICSSON AIR 21 B4A	C	From Centroid-Fa	4.00	0.0000	160.00	No Ice	6.90	5.72	0.11
B2P w/ Mount Pipe			0.00			1/2" Ice	7.46	6.63	0.17
		ce	3.00			1" Ice	8.00	7.42 9.07	0.24
						2" Ice 4" Ice	9.10 11.44	12.58	0.39 0.82
ATMAA1412D-1A20	A	From	4.00	0.0000	160.00	No Ice	11.44	0.47	0.82
ATMAA1412D-1A20	А	Centroid-Fa	0.00	0.0000	100.00	1/2" Ice	1.17	0.47	0.01
		ce centroid-ra	3.00			1" Ice	1.47	0.57	0.02
		cc	3.00			2" Ice	1.81	0.95	0.06
						4" Ice	2.58	1.57	0.14
ATMAA1412D-1A20	В	From	4.00	0.0000	160.00	No Ice	1.17	0.47	0.01
	_	Centroid-Fa	0.00			1/2" Ice	1.31	0.57	0.02
		ce	3.00			1" Ice	1.47	0.69	0.03
						2" Ice	1.81	0.95	0.06
						4" Ice	2.58	1.57	0.14
ATMAA1412D-1A20	C	From	4.00	0.0000	160.00	No Ice	1.17	0.47	0.01
		Centroid-Fa	0.00			1/2" Ice	1.31	0.57	0.02
		ce	3.00			1" Ice	1.47	0.69	0.03
						2" Ice	1.81	0.95	0.06
						4" Ice	2.58	1.57	0.14
Pipe Mount 6'x2.375"	A	From	4.00	0.0000	160.00	No Ice	1.43	1.43	0.03
		Centroid-Fa	0.00			1/2" Ice	1.92	1.92	0.04
		ce	2.00			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
						4" Ice	4.70	4.70	0.23
Pipe Mount 6'x2.375"	В	From	4.00	0.0000	160.00	No Ice	1.43	1.43	0.03
		Centroid-Fa	0.00			1/2" Ice	1.92	1.92	0.04
		ce	2.00			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
	_	_				4" Ice	4.70	4.70	0.23
Pipe Mount 6'x2.375"	C	From	4.00	0.0000	160.00	No Ice	1.43	1.43	0.03
		Centroid-Fa	0.00			1/2" Ice	1.92	1.92	0.04
		ce	2.00			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
INV 6515DC V/TM /	A	Eur	4.00	0.0000	160.00	4" Ice	4.70	4.70	0.23
LNX-6515DS-VTM w/	A	From	4.00	0.0000	160.00	No Ice	11.64	9.79	0.08
Mount Pipe		Centroid-Fa	0.00			1/2" Ice 1" Ice	12.34 13.04	11.30	0.17
		ce	3.00					12.80	0.27
						2" Ice	14.48	15.12	0.50

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Project		Date
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Client	Crown Castle USA, Inc.	Designed by kliccar

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C_AA_A Front	C _A A _A Side	Weight
	Leg		Vert ft ft ft	٥	ft		ft ²	ft²	K
LNX-6515DS-VTM w/	В	From	4.00	0.0000	160.00	No Ice	11.64	9.79	0.08
Mount Pipe		Centroid-Fa	0.00			1/2" Ice	12.34	11.30	0.17
•		ce	3.00			1" Ice	13.04	12.80	0.27
						2" Ice	14.48	15.12	0.50
						4" Ice	17.71	19.94	1.14
LNX-6515DS-VTM w/	C	From	4.00	0.0000	160.00	No Ice	11.64	9.79	0.08
Mount Pipe		Centroid-Fa	0.00 3.00			1/2" Ice	12.34 13.04	11.30	0.17 0.27
		ce	3.00			1" Ice 2" Ice	13.04	12.80 15.12	0.27
						4" Ice	17.71	19.12	1.14
RRUS 11 B12	Α	From	4.00	0.0000	160.00	No Ice	3.31	1.36	0.05
14105 11 512		Centroid-Fa	0.00	0.0000	100.00	1/2" Ice	3.55	1.54	0.07
		ce	3.00			1" Ice	3.80	1.73	0.10
						2" Ice	4.33	2.13	0.15
						4" Ice	5.50	3.04	0.31
RRUS 11 B12	В	From	4.00	0.0000	160.00	No Ice	3.31	1.36	0.05
		Centroid-Fa	0.00			1/2" Ice	3.55	1.54	0.07
		ce	3.00			1" Ice	3.80	1.73	0.10
						2" Ice	4.33	2.13	0.15
RRUS 11 B12	С	From	4.00	0.0000	160.00	4" Ice No Ice	5.50 3.31	3.04	0.31 0.05
KKUS 11 B12	C	Centroid-Fa	0.00	0.0000	100.00	1/2" Ice	3.55	1.36 1.54	0.03
		ce centroid-ra	3.00			1" Ice	3.80	1.73	0.10
		cc	5.00			2" Ice	4.33	2.13	0.15
						4" Ice	5.50	3.04	0.31
Side Arm Mount [SO 102-3]	C	None		0.0000	153.00	No Ice	3.00	3.00	0.08
						1/2" Ice	3.48	3.48	0.11
						1" Ice	3.96	3.96	0.14
						2" Ice	4.92	4.92	0.20
						4" Ice	6.84	6.84	0.32
TME-RRUS-11	A	From Leg	1.50	0.0000	153.00	No Ice	3.25	1.37	0.05
			0.00			1/2" Ice	3.49	1.55	0.07
			0.00			1" Ice	3.74	1.74	0.09
						2" Ice 4" Ice	4.27 5.43	2.14 3.04	0.15 0.31
TME-RRUS-11	В	From Leg	1.50	0.0000	153.00	No Ice	3.43	1.37	0.05
TME-KKUS-TI	ь	Prom Leg	0.00	0.0000	155.00	1/2" Ice	3.49	1.55	0.03
			0.00			1" Ice	3.74	1.74	0.09
						2" Ice	4.27	2.14	0.15
						4" Ice	5.43	3.04	0.31
TME-RRUS-11	C	From Leg	1.50	0.0000	153.00	No Ice	3.25	1.37	0.05
			0.00			1/2" Ice	3.49	1.55	0.07
			0.00			1" Ice	3.74	1.74	0.09
						2" Ice	4.27	2.14	0.15
THE DOC 40 CO 10 OF		Б. Т	1.50	0.0000	152.00	4" Ice	5.43	3.04	0.31
TME-DC6-48-60-18-8F	A	From Leg	1.50	0.0000	153.00	No Ice 1/2" Ice	1.47	1.47	0.02
			0.00			1" Ice	1.67 1.88	1.67 1.88	0.04 0.06
			0.00			2" Ice	2.33	2.33	0.11
						4" Ice	3.38	3.38	0.11
Platform Mount [LP 1201-1]	C	None		0.0000	149.00	No Ice	23.10	23.10	2.10
	-					1/2" Ice	26.80	26.80	2.50
						1" Ice	30.50	30.50	2.90
						2" Ice	37.90	37.90	3.70
						4" Ice	52.70	52.70	5.30
(2) 7770.00 w/ Mount Pipe	A	From	4.00	0.0000	149.00	No Ice	6.22	4.35	0.06
		Centroid-Le	0.00			1/2" Ice	6.77	5.20	0.11

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	GREENWICH NORTH (BU #: 841290)	7 of 13
Project		Date
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Client	Crown Castle USA, Inc.	Designed by kliccar

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		$C_A A_A$ Front	C_AA_A Side	Weigh
			Vert ft ft	0	ft		ft²	ft²	K
		g	2.00			1" Ice	7.30	5.92	0.16
		5	2.00			2" Ice	8.38	7.41	0.10
						4" Ice	10.69	10.76	0.68
2) 7770.00 w/ Mount Pipe	В	From	4.00	0.0000	149.00	No Ice	6.22	4.35	0.06
		Centroid-Le	0.00			1/2" Ice	6.77	5.20	0.11
		g	2.00			1" Ice	7.30	5.92	0.16
						2" Ice 4" Ice	8.38 10.69	7.41 10.76	0.29 0.68
2) 7770.00 w/ Mount Pipe	С	From	4.00	0.0000	149.00	No Ice	6.22	4.35	0.06
2) 1110.00 W/ Wloulit Tipe	C	Centroid-Le	0.00	0.0000	142.00	1/2" Ice	6.77	5.20	0.11
		g g	2.00			1" Ice	7.30	5.92	0.16
		8				2" Ice	8.38	7.41	0.29
						4" Ice	10.69	10.76	0.68
P65-16-XLH-RR w/ mount	A	From	4.00	0.0000	149.00	No Ice	8.64	6.36	0.08
pipe		Centroid-Le	0.00			1/2" Ice	9.29	7.54	0.14
		g	0.00			1" Ice	9.91	8.43	0.22
						2" Ice	11.18	10.24	0.39
P65-16-XLH-RR w/ mount	В	From	4.00	0.0000	149.00	4" Ice No Ice	13.83 8.64	14.10 6.36	0.89 0.08
pipe	Ь	Centroid-Le	0.00	0.0000	149.00	1/2" Ice	9.29	7.54	0.08
pipe		g	0.00			1" Ice	9.29	8.43	0.14
		5	0.00			2" Ice	11.18	10.24	0.39
						4" Ice	13.83	14.10	0.89
P65-16-XLH-RR w/ mount	C	From	4.00	0.0000	149.00	No Ice	8.64	6.36	0.08
pipe		Centroid-Le	0.00			1/2" Ice	9.29	7.54	0.14
		g	0.00			1" Ice	9.91	8.43	0.22
						2" Ice	11.18	10.24	0.39
(2) I CD21401		Г	4.00	0.0000	1.40.00	4" Ice	13.83	14.10	0.89
(2) LGP21401	A	From Centroid-Le	4.00	0.0000	149.00	No Ice 1/2" Ice	1.29 1.45	0.36 0.48	0.01 0.02
			0.00 2.00			1" Ice	1.43	0.48	0.02
		g	2.00			2" Ice	1.97	0.87	0.05
						4" Ice	2.79	1.52	0.14
(2) LGP21401	В	From	4.00	0.0000	149.00	No Ice	1.29	0.36	0.01
• •		Centroid-Le	0.00			1/2" Ice	1.45	0.48	0.02
		g	2.00			1" Ice	1.61	0.60	0.03
						2" Ice	1.97	0.87	0.05
	_	_				4" Ice	2.79	1.52	0.14
(2) LGP21401	C	From	4.00	0.0000	149.00	No Ice	1.29	0.36	0.01
		Centroid-Le	0.00			1/2" Ice	1.45	0.48 0.60	0.02 0.03
		g	2.00			1" Ice 2" Ice	1.61 1.97	0.87	0.03
						4" Ice	2.79	1.52	0.03
(2) LGP21401	Α	From	4.00	0.0000	149.00	No Ice	1.29	0.36	0.01
(=) = == = = = = = = = = = = = = = = = =		Centroid-Le	0.00			1/2" Ice	1.45	0.48	0.02
		g	0.00			1" Ice	1.61	0.60	0.03
						2" Ice	1.97	0.87	0.05
						4" Ice	2.79	1.52	0.14
(2) LGP21401	В	From	4.00	0.0000	149.00	No Ice	1.29	0.36	0.01
		Centroid-Le	0.00			1/2" Ice	1.45	0.48	0.02
		g	0.00			1" Ice	1.61	0.60	0.03
						2" Ice 4" Ice	1.97 2.79	0.87 1.52	0.05 0.14
(2) LGP21401	С	From	4.00	0.0000	149.00	No Ice	1.29	0.36	0.14
(2) 20121701	C	Centroid-Le	0.00	0.0000	112.00	1/2" Ice	1.45	0.48	0.01
		g g	0.00			1" Ice	1.61	0.60	0.03
		J				2" Ice	1.97	0.87	0.05

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	GREENWICH NORTH (BU #: 841290)	8 of 13
Project		Date
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Client	Crown Castle USA, Inc.	Designed by kliccar

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		$C_A A_A$ Front	C _A A _A Side	Weight
	- 0		Vert ft ft ft	0	ft		ft ²	ft²	K
						4" Ice	2.79	1.52	0.14
Platform Mount [LP 1201-1]	C	None		0.0000	140.00	No Ice	23.10	23.10	2.10
						1/2" Ice 1" Ice	26.80 30.50	26.80 30.50	2.50 2.90
						2" Ice	37.90	37.90	3.70
						4" Ice	52.70	52.70	5.30
P65-16-XL-M w/ Mount Pipe	Α	From	4.00	0.0000	140.00	No Ice	8.64	6.36	0.07
1		Centroid-Le	0.00			1/2" Ice	9.29	7.54	0.14
		g	2.00			1" Ice	9.91	8.43	0.21
						2" Ice	11.18	10.24	0.38
						4" Ice	13.83	14.10	0.88
P65-16-XL-M w/ Mount Pipe	C	From	4.00	0.0000	140.00	No Ice	8.64	6.36	0.07
		Centroid-Le	0.00			1/2" Ice	9.29	7.54	0.14
		g	2.00			1" Ice 2" Ice	9.91 11.18	8.43	0.21 0.38
						4" Ice	13.83	10.24 14.10	0.38
(2) APL868013-42T0 w/	A	From	4.00	0.0000	140.00	No Ice	3.10	4.92	0.02
Mount Pipe		Centroid-Le	0.00	0.0000	110.00	1/2" Ice	3.48	5.60	0.06
		g	2.00			1" Ice	3.88	6.28	0.11
		C				2" Ice	4.76	7.71	0.22
						4" Ice	6.66	10.83	0.54
(2) APL868013-42T0 w/	В	From	4.00	0.0000	140.00	No Ice	3.10	4.92	0.02
Mount Pipe		Centroid-Le	0.00			1/2" Ice	3.48	5.60	0.06
		g	2.00			1" Ice	3.88	6.28	0.11
						2" Ice	4.76	7.71	0.22
TME BRIDY 60 BCS	A	Enom Loo	4.00	0.0000	140.00	4" Ice No Ice	6.66 2.57	10.83 2.01	0.54 0.06
TME-RRH2X60-PCS	А	From Leg	0.00	0.0000	140.00	1/2" Ice	2.79	2.01	0.08
			0.00			1" Ice	3.02	2.43	0.10
			0.00			2" Ice	3.52	2.89	0.16
						4" Ice	4.61	3.92	0.31
TME-RRH2X60-PCS	В	From Leg	4.00	0.0000	140.00	No Ice	2.57	2.01	0.06
			0.00			1/2" Ice	2.79	2.22	0.08
			0.00			1" Ice	3.02	2.43	0.10
						2" Ice	3.52	2.89	0.16
THE PRIMAYOR DOG	-	Б. Т	4.00	0.0000	1.40.00	4" Ice	4.61	3.92	0.31
TME-RRH2X60-PCS	C	From Leg	4.00	0.0000	140.00	No Ice	2.57	2.01	0.06
			0.00			1/2" Ice 1" Ice	2.79 3.02	2.22 2.43	0.08 0.10
			0.00			2" Ice	3.52	2.43	0.16
						4" Ice	4.61	3.92	0.31
TME-RRH2x60 AWS	A	From Leg	4.00	0.0000	140.00	No Ice	2.57	2.01	0.06
			0.00			1/2" Ice	2.79	2.22	0.08
			0.00			1" Ice	3.02	2.43	0.10
						2" Ice	3.52	2.89	0.16
						4" Ice	4.61	3.92	0.31
TME-RRH2x60 AWS	В	From Leg	4.00	0.0000	140.00	No Ice	2.57	2.01	0.06
			0.00			1/2" Ice	2.79	2.22	0.08
			0.00			1" Ice	3.02	2.43	0.10
						2" Ice 4" Ice	3.52 4.61	2.89 3.92	0.16 0.31
TME-RRH2x60 AWS	C	From Leg	4.00	0.0000	140.00	No Ice	2.57	2.01	0.31
THE KKIIZAUU AWS		110m Leg	0.00	0.0000	1-10.00	1/2" Ice	2.79	2.22	0.08
			0.00			1" Ice	3.02	2.43	0.10
			00			2" Ice	3.52	2.89	0.16
						4" Ice	4.61	3.92	0.31
(2) WWX063X19G00 w/	Α	From Leg	4.00	0.0000	140.00	No Ice	9.06	7.28	0.06

GPD Group 520 South Main St, Suite 2531

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Project		Date
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Client	Crown Castle USA, Inc.	Designed by kliccar

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C_AA_A Front	C_AA_A Side	Weight
	Lig		Vert ft ft	0	ft		ft ²	ft²	K
Mount Pipe			ft 0.00			1/2" Ice	9.73	8.50	0.13
Would Tipe			2.00			1" Ice	10.38	9.47	0.13
			2.00			2" Ice	11.68	11.35	0.40
						4" Ice	14.42	15.44	0.92
(2) WWX063X19G00 w/	В	From Leg	4.00	0.0000	140.00	No Ice	9.06	7.28	0.06
Mount Pipe			0.00			1/2" Ice	9.73	8.50	0.13
			2.00			1" Ice	10.38	9.47	0.21
						2" Ice	11.68	11.35	0.40
(2) ************************************			4.00	0.0000	4.40.00	4" Ice	14.42	15.44	0.92
(2) WWX063X19G00 w/	C	From Leg	4.00	0.0000	140.00	No Ice	9.06	7.28	0.06
Mount Pipe			0.00			1/2" Ice	9.73	8.50	0.13
			2.00			1" Ice	10.38	9.47	0.21
						2" Ice	11.68	11.35	0.40
DD T1 67 9AD 07	Α.	Enom Loo	4.00	0.0000	140.00	4" Ice	14.42 5.60	15.44	0.92 0.04
DB-T1-6Z-8AB-0Z	Α	From Leg	4.00	0.0000	140.00	No Ice 1/2" Ice	5.92	2.33	0.04
			0.00 2.00			1" Ice	6.24	2.56 2.79	0.08
			2.00			2" Ice	6.91	3.28	0.12
						4" Ice	8.37	4.37	0.45
BXA-70063/8CFx2 w/ Mount	В	From Leg	4.00	0.0000	140.00	No Ice	10.69	7.97	0.05
Pipe		Trom Leg	0.00	0.0000	1 10.00	1/2" Ice	11.31	9.37	0.13
1.pc			2.00			1" Ice	11.93	10.62	0.22
						2" Ice	13.19	12.80	0.42
						4" Ice	16.17	17.35	1.00
(2) DB844H80E-XY w/	C	From Leg	4.00	0.0000	140.00	No Ice	3.30	4.92	0.03
Mount Pipe			0.00			1/2" Ice	3.69	5.60	0.07
			2.00			1" Ice	4.12	6.28	0.12
						2" Ice	5.01	7.71	0.23
						4" Ice	6.92	10.83	0.56
Handrail Kit [NA 510-1]	C	None		0.0000	122.00	No Ice	6.00	6.00	0.23
						1/2" Ice	8.50	8.50	0.34
						1" Ice	11.00	11.00	0.45
						2" Ice 4" Ice	16.00	16.00	0.68
Platform Mount [LP 1201-1]	С	None		0.0000	120.00	No Ice	26.00 23.10	26.00 23.10	1.14 2.10
Trationii Woult [Er 1201-1]	C	None		0.0000	120.00	1/2" Ice	26.80	26.80	2.50
						1" Ice	30.50	30.50	2.90
						2" Ice	37.90	37.90	3.70
						4" Ice	52.70	52.70	5.30
APXVSPP18-C-A20 w/	Α	From	4.00	0.0000	120.00	No Ice	8.26	6.71	0.08
Mount Pipe		Centroid-Le	0.00			1/2" Ice	8.81	7.66	0.14
•		g	2.00			1" Ice	9.36	8.49	0.22
						2" Ice	10.50	10.20	0.39
						4" Ice	12.88	13.98	0.87
APXVSPP18-C-A20 w/	В	From	4.00	0.0000	120.00	No Ice	8.26	6.71	0.08
Mount Pipe		Centroid-Le	0.00			1/2" Ice	8.81	7.66	0.14
		g	2.00			1" Ice	9.36	8.49	0.22
						2" Ice	10.50	10.20	0.39
ADVIVODD10 C 120 '	C	F	4.00	0.0000	100.00	4" Ice	12.88	13.98	0.87
APXVSPP18-C-A20 w/	C	From	4.00	0.0000	120.00	No Ice	8.26	6.71	0.08
Mount Pipe		Centroid-Le	0.00			1/2" Ice 1" Ice	8.81	7.66	0.14
		g	2.00			2" Ice	9.36 10.50	8.49 10.20	0.22 0.39
						4" Ice	12.88	13.98	0.39
(2) 6' x 2" Mount Pipe	Α	From	4.00	0.0000	120.00	No Ice	1.43	1.43	0.02
(2) O A 2 Product Tipe	<i>1</i> 1	Centroid-Le		0.0000	120.00			1.43	0.02
		Centroid-Le	0.00			1/2" Ice	1.92	[.97	()()3

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Client	Crown Castle USA, Inc.	Designed by kliccar

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C _A A _A Front	C_AA_A Side	Weight
			Vert ft ft ft	٥	ft		ft ²	ft ²	K
			Ji			2" Ice	3.06	3.06	0.09
						4" Ice	4.70	4.70	0.23
(2) 6' x 2" Mount Pipe	В	From	4.00	0.0000	120.00	No Ice	1.43	1.43	0.02
		Centroid-Le	0.00			1/2" Ice	1.92	1.92	0.03
		g	2.00			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
(2) (! 2!! M+ Di	C	F	4.00	0.0000	120.00	4" Ice	4.70	4.70	0.23
(2) 6' x 2" Mount Pipe	C	From	4.00	0.0000	120.00	No Ice	1.43	1.43	0.02
		Centroid-Le	0.00			1/2" Ice 1" Ice	1.92 2.29	1.92 2.29	0.03 0.05
		g	2.00			2" Ice	3.06	3.06	0.03
						4" Ice	4.70	4.70	0.09
APXVTM14-C-120 w/	A	From	4.00	0.0000	120.00	No Ice	7.13	4.70	0.23
Mount Pipe	A	Centroid-Le	0.00	0.0000	120.00	1/2" Ice	7.13	5.75	0.08
Would Tipe			2.00			1" Ice	8.18	6.47	0.19
		g	2.00			2" Ice	9.26	8.01	0.19
						4" Ice	11.53	11.41	0.75
APXVTM14-C-120 w/	В	From	4.00	0.0000	120.00	No Ice	7.13	4.96	0.08
Mount Pipe	_	Centroid-Le	0.00	0.0000	120.00	1/2" Ice	7.66	5.75	0.13
mount Tipe		g	2.00			1" Ice	8.18	6.47	0.19
		8				2" Ice	9.26	8.01	0.34
						4" Ice	11.53	11.41	0.75
APXVTM14-C-120 w/	C	From	4.00	0.0000	120.00	No Ice	7.13	4.96	0.08
Mount Pipe		Centroid-Le	0.00			1/2" Ice	7.66	5.75	0.13
-		g	2.00			1" Ice	8.18	6.47	0.19
						2" Ice	9.26	8.01	0.34
						4" Ice	11.53	11.41	0.75
TD-RRH8x20-25	Α	From	4.00	0.0000	120.00	No Ice	4.72	1.70	0.07
		Centroid-Le	0.00			1/2" Ice	5.01	1.92	0.10
		g	2.00			1" Ice	5.32	2.15	0.13
						2" Ice	5.95	2.62	0.20
		_				4" Ice	7.31	3.68	0.40
TD-RRH8x20-25	В	From	4.00	0.0000	120.00	No Ice	4.72	1.70	0.07
		Centroid-Le	0.00			1/2" Ice	5.01	1.92	0.10
		g	2.00			1" Ice	5.32	2.15	0.13
						2" Ice	5.95	2.62	0.20
TD-RRH8x20-25	С	From	4.00	0.0000	120.00	4" Ice No Ice	7.31 4.72	3.68 1.70	0.40 0.07
1D-KKH6X20-23	C	Centroid-Le	0.00	0.0000	120.00	1/2" Ice	5.01	1.70	0.07
		g	2.00			1" Ice	5.32	2.15	0.10
		5	2.00			2" Ice	5.95	2.62	0.20
						4" Ice	7.31	3.68	0.40
Side Arm Mount [SO 102-3]	C	None		0.0000	119.00	No Ice	3.00	3.00	0.08
	_					1/2" Ice	3.48	3.48	0.11
						1" Ice	3.96	3.96	0.14
						2" Ice	4.92	4.92	0.20
						4" Ice	6.84	6.84	0.32
TME-1900MHz RRH w/ 4' x	Α	From Leg	1.50	0.0000	119.00	No Ice	3.36	4.67	0.06
2" Mount Pipe		-	0.00			1/2" Ice	3.72	5.18	0.10
			0.00			1" Ice	4.10	5.70	0.15
						2" Ice	4.90	6.81	0.26
		_				4" Ice	6.73	9.38	0.57
TME-1900MHz RRH w/ 4' x	В	From Leg	1.50	0.0000	119.00	No Ice	3.36	4.67	0.06
2" Mount Pipe			0.00			1/2" Ice	3.72	5.18	0.10
			0.00			1" Ice	4.10	5.70	0.15
						2" Ice	4.90	6.81	0.26
						4" Ice	6.73	9.38	0.57

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FAX: (330) 572-2101

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Client	Crown Castle USA, Inc.	Designed by kliccar

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C_AA_A Front	C_AA_A Side	Weight
			Vert ft ft ft	0	ft		ft ²	ft ²	K
ГМЕ-1900MHz RRH w/ 4' х	C	From Leg	1.50	0.0000	119.00	No Ice	3.36	4.67	0.06
2" Mount Pipe			0.00			1/2" Ice	3.72	5.18	0.10
			0.00			1" Ice	4.10	5.70	0.15
						2" Ice	4.90	6.81	0.26
						4" Ice	6.73	9.38	0.57
TME-800MHZ RRH	Α	From Leg	1.50	0.0000	119.00	No Ice	2.49	2.07	0.05
		Trom Log	0.00	0.0000	117.00	1/2" Ice	2.71	2.27	0.07
			0.00			1" Ice	2.93	2.48	0.10
						2" Ice	3.41	2.93	0.16
						4" Ice	4.46	3.93	0.32
TME-800MHZ RRH	В	From Leg	1.50	0.0000	119.00	No Ice	2.49	2.07	0.05
		Trom Log	0.00	0.0000	117.00	1/2" Ice	2.71	2.27	0.07
			0.00			1" Ice	2.93	2.48	0.10
			0.00			2" Ice	3.41	2.93	0.16
						4" Ice	4.46	3.93	0.32
TME-800MHZ RRH	C	From Leg	1.50	0.0000	119.00	No Ice	2.49	2.07	0.05
THE GOOMINE RRAIT	C	Trom Leg	0.00	0.0000	117.00	1/2" Ice	2.71	2.27	0.07
			0.00			1" Ice	2.93	2.48	0.10
			0.00			2" Ice	3.41	2.93	0.16
						4" Ice	4.46	3.93	0.32
Side Arm Mount [SO 701-1]	В	From Leg	1.50	0.0000	72.00	No Ice	0.85	1.67	0.07
Side Film Would [SO 701 1]		Trom Leg	0.00	0.0000	72.00	1/2" Ice	1.14	2.34	0.08
			0.00			1" Ice	1.43	3.01	0.09
			0.00			2" Ice	2.01	4.35	0.12
						4" Ice	3.17	7.03	0.12
(2) GPS_A	В	From Leg	3.00	0.0000	72.00	No Ice	0.30	0.30	0.00
(2) 32 5 2 1	2	Trom Evg	0.00	0.0000	. 2.00	1/2" Ice	0.37	0.37	0.00
			1.00			1" Ice	0.46	0.46	0.00
			1.00			2" Ice	0.65	0.65	0.02
						4" Ice	1.15	1.15	0.02

Maximum Tower Deflections - Service Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	٥	٥
L1	160 - 152	29.789	27	1.6162	0.0004
L2	152 - 111.29	27.091	27	1.6022	0.0004
L3	116.71 - 77.42	15.990	27	1.3348	0.0003
L4	83.59 - 36.46	8.033	33	0.9107	0.0002
L5	43.54 - 0	2.187	33	0.4515	0.0001

Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	0	ft
160.00	Platform Mount [LP 1201-1]	27	29.789	1.6162	0.0004	34755
153.00	Side Arm Mount [SO 102-3]	27	27.427	1.6047	0.0004	25087
149.00	Platform Mount [LP 1201-1]	27	26.086	1.5928	0.0004	16711
140.00	Platform Mount [LP 1201-1]	27	23.117	1.5476	0.0004	9873
122.00	Handrail Kit [NA 510-1]	27	17.515	1.3931	0.0003	5432

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Client	Crown Castle USA, Inc.	Designed by kliccar

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	0	ft
120.00	Platform Mount [LP 1201-1]	27	16.930	1.3717	0.0003	5178
119.00	Side Arm Mount [SO 102-3]	27	16.642	1.3607	0.0003	5069
72.00	Side Arm Mount [SO 701-1]	33	5.903	0.7684	0.0002	4591

Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	٥	0
L1	160 - 152	85.903	2	4.6616	0.0013
L2	152 - 111.29	78.128	2	4.6213	0.0013
L3	116.71 - 77.42	46.133	2	3.8512	0.0009
L4	83.59 - 36.46	23.183	8	2.6284	0.0006
L5	43.54 - 0	6.315	8	1.3034	0.0003

Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	٥	٥	ft
160.00	Platform Mount [LP 1201-1]	2	85.903	4.6616	0.0013	12207
153.00	Side Arm Mount [SO 102-3]	2	79.096	4.6285	0.0013	8811
149.00	Platform Mount [LP 1201-1]	2	75.233	4.5941	0.0013	5872
140.00	Platform Mount [LP 1201-1]	2	66.677	4.4639	0.0012	3464
122.00	Handrail Kit [NA 510-1]	2	50.529	4.0192	0.0010	1902
120.00	Platform Mount [LP 1201-1]	2	48.845	3.9574	0.0009	1812
119.00	Side Arm Mount [SO 102-3]	2	48.013	3.9257	0.0009	1774
72.00	Side Arm Mount [SO 701-1]	8	17.038	2.2178	0.0005	1596

Compression Checks

Pole Design Data

Section	Elevation	Size	L	L_u	Kl/r	F_a	A	Actual	Allow.	Ratio
No.								P	P_a	P
	ft		ft	ft		ksi	in^2	K	K	P_a
L1	160 - 152 (1)	TP30.62x29x0.1875	8.00	0.00	0.0	37.630	18.1111	-3.56	681.53	0.005
L2	152 - 111.29 (2)	TP38.86x30.62x0.25	40.71	0.00	0.0	39.000	29.7665	-16.35	1160.89	0.014
L3	111.29 - 77.42	TP45.09x37.263x0.3125	39.29	0.00	0.0	39.000	43.1945	-23.23	1684.59	0.014
	(3)									
L4	77.42 - 36.46 (4)	TP52.62x43.2359x0.4375	47.13	0.00	0.0	39.000	70.5044	-35.42	2749.67	0.013
L5	36.46 - 0 (5)	TP59x50.3353x0.5	43.54	0.00	0.0	39.000	92.8395	-52.21	3620.74	0.014

Pole Bending Design Data

Section	Elevation	Size	Actual	Actual	Allow.	Ratio	Actual	Actual	Allow.	Ratio
No.			$M_{\scriptscriptstyle X}$	f_{bx}	F_{bx}	f_{bx}	M_y	f_{by}	F_{by}	f_{by}
	ft		kip-ft	ksi	ksi	F_{bx}	kip-ft	ksi	ksi	F_{by}
L1	160 - 152 (1)	TP30.62x29x0.1875	59.70	5.258	37.630	0.140	0.00	0.000	37.630	0.000
L2	152 - 111.29 (2)	TP38.86x30.62x0.25	695.34	30.247	39.000	0.776	0.00	0.000	39.000	0.000
L3	111.29 - 77.42	TP45.09x37.263x0.3125	1630.38	42.121	39.000	1.080	0.00	0.000	39.000	0.000

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	Crown Castle USA, Inc.	kliccar

Section No.	Elevation ft	Size	Actual M _x kip-ft	Actual f _{bx} ksi	Allow. F _{bx} ksi	Ratio $\frac{f_{bx}}{F_{bx}}$	Actual M _y kip-ft	Actual f _{by} ksi	Allow. F _{by} ksi	$\frac{Ratio}{f_{by}}$
L4	(3) 77.42 - 36.46 (4)	TP52.62x43.2359x0.4375	2902.17	39.456	39.000	1.012	0.00	0.000	39.000	0.000
L5	36.46 - 0 (5)	TP59x50.3353x0.5	4439.01	39.774	39.000	1.020	0.00	0.000	39.000	0.000

			Pole S	Shear	Desig	ın Da	ıta			
Section No.	Elevation ft	Size	Actual V K	Actual f _v ksi	Allow. F _v ksi	Ratio $\frac{f_v}{F_v}$	Actual T kip-ft	Actual f _{vt} ksi	Allow. F _{vt} ksi	$\frac{\textit{Ratio}}{f_{vt}}$
L1	160 - 152 (1)	TP30.62x29x0.1875	6.79	0.375	26.000	0.029	0.01	0.000	26.000	0.000
L2	152 - 111.29 (2)	TP38.86x30.62x0.25	26.61	0.894	26.000	0.069	0.19	0.004	26.000	0.000
L3	111.29 - 77.42	TP45.09x37.263x0.3125	29.79	0.690	26.000	0.053	0.22	0.003	26.000	0.000
L4	77.42 - 36.46	TP52.62x43.2359x0.4375	33.55	0.476	26.000	0.037	0.55	0.004	26.000	0.000
L5	36.46 - 0 (5)	TP59x50.3353x0.5	36.93	0.398	26.000	0.031	0.57	0.003	26.000	0.000

			F	Pole Int	teraction	on Des	ign Da	ta	
Section No.	Elevation ft	Ratio P P _a	Ratio $\frac{f_{bx}}{F_{bx}}$	Ratio $\frac{f_{by}}{F_{by}}$	Ratio f_{v} F_{v}	$\frac{\textit{Ratio}}{\textit{f}_{vt}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	160 - 152 (1)	0.005	0.140	0.000	0.029	0.000	0.145	1.333	H1-3+VT 🗸
L2	152 - 111.29 (2)	0.014	0.776	0.000	0.069	0.000	0.791	1.333	H1-3+VT 🗸
L3	111.29 - 77.42	0.014	1.080	0.000	0.053	0.000	1.095	1.333	H1-3+VT 🗸
L4	77.42 - 36.46 (4)	0.013	1.012	0.000	0.037	0.000	1.025	1.333	H1-3+VT 🗸
L5	36.46 - 0 (5)	0.014	1.020	0.000	0.031	0.000	1.035	1.333	H1-3+VT 🗸

			Section Cap	acity T	able			
Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
L1	160 - 152	Pole	TP30.62x29x0.1875	1	-3.56	908.48	10.9	Pass
L2	152 - 111.29	Pole	TP38.86x30.62x0.25	2	-16.35	1547.47	59.3	Pass
L3	111.29 - 77.42	Pole	TP45.09x37.263x0.3125	3	-23.23	2245.56	82.1	Pass
L4	77.42 - 36.46	Pole	TP52.62x43.2359x0.4375	4	-35.42	3665.31	76.9	Pass
L5	36.46 - 0	Pole	TP59x50.3353x0.5	5	-52.21	4826.45	77.6	Pass
						Summary	ELC:	Load Case 7
						Pole (L3) Rating =	82.1 82.1	Pass Pass

APPENDIX B BASE LEVEL DRAWING

WAR REVENUES (TWICK BLOCKS FOR TEMPLATES) THE BLOCK STD-VER S. 1. Juny 1. BASE LEVEL DRAWING 1:-0:-1

BUSINESS UNIT: 841290 TOWER ID: C_BASELEVEL

SITE ADDRESS

363 RIVERSVILLE ROAD
GREENWICH, CT 06831
FAIRFIELD COUNTY
USA

BASE LEVEL

841290

GREENWICH NORTH
BUSINESS UNIT NUMBER

SITE NAME

A1-0

SITE NUMBER: SITE NAME:

DRAWN BY: RJC CHECKED BY: DRAWING DATE: 20/03/14

29/03/14 NEW BUILD PER WORK ORDER # 73002

09/08/14 UPDATED PER WORK ORDER # 370020

19/08/14 UPDATED PER WORK ORDER # 300708

20/1/2015 UPDATED PER WORK ORDER 982019

24/2/2015 UPDATED PER WORK ORDER 1014218

DW SLW ALM SAC BMH

(INSTALLED-IN 2" CONDUIT)
(2) 3/8" TO 153 FT LEVEL
(INSTALLED)
(12) 1-5/8" TO 149 FT LEVEL-(INSTALLED) (13) 1-5/8" TO 160 FT LEVEL-(RESERVED)
(2) 1-5/8" TO 140 FT LEVEL
(INSTALLED)
(18) 1-5/8" TO 140 FT LEVEL-(RESERVED)
(1) 1-1/4" TO 120 FI LEVEL
(INSTALLED)
(2) 1/2" TO 72 FI LEVEL
(2) 1/2" TO 120 FI LEVEL
(3) 1-1/4" TO 120 FI LEVEL

NSA

CROWN REGION ADDRESS

-CLIMBING PEGS W/ SAFETY CLIMB

APPENDIX C ADDITIONAL CALCULATIONS

Stiffened or Unstiffened, Exterior Flange Plate - Any Bolt Material TIA Rev F

Site Data

BU#: 841290

Pole Manufacturer:

Site Name: GREENWICH NORTH

App #: 284770 Rev. 0

Reactions		
Moment:	59.7	ft-kips
Axial:	3.56	kips
Shear:	6.79	kips
Elevation:	152	feet

		-
Bolt Data		
Qty: 12		
Diameter (in):	Bolt Fur	120

Other

Diameter (in.) Bolt Material: Bolt Fy: 92 N/A: <-- Disregard Bolt Fty: N/A <-- Disregard 44.00 35 Circle (in.)

Reactions		
Moment:	59.7	ft-kips
Axial:	3.56	kips
Shear:	6.79	kips
Elevation:	152	feet

If No stiffeners, Criteria: Flange Bolt Results

Bolt Tension Capacity, B: 46.07 kips Max Bolt <u>directly</u> applied T: 6.53 Kips

Min. PL "tc" for B cap. w/o Pry: 0.986 in Min PL "treq" for actual T w/ Pry: 0.271 in Min PL "t1" for actual **T w/o Pry**: 0.371 in

> T allowable w/o Prying: 46.07 kips α'<0 case

AISC ASD <-Only Applicable to Unstiffened Cases

Prying Force, Q: 0.00 kips Total Bolt Tension=T+Q: 6.53 kips Non-Prying Bolt Stress Ratio, T/B: 14.2% Pass

PI	ate Data	
Diam:	38	in
Thick, t:	1	in
Grade (Fy):	60	ksi
Strength, Fu:	75	ksi
Sinale-Rod B-eff:	8.10	lin

Config:

Weld Type: Groove Depth:

Exterior Flange Plate Results Flexural Check Compression Side Plate Stress: 7.8 ksi

Allowable Plate Stress: 60.0 ksi Compression Plate Stress Ratio: 13.0% Pass

No Prying

Tension Side Stress Ratio, (treq/t)^2: 7.4% Pass

Rigid
Service ASD
0.75*Fy*ASIF
Comp. Y.L. Length:
16.95

Rigid

Service, ASD

Fty*ASIF

Stiffener Data (Welding at Both Sides)

in **

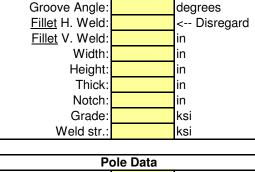
n/a

Stiffener Results

Horizontal Weld: n/a Vertical Weld: n/a Plate Flex+Shear, fb/Fb+(fv/Fv)^2: n/a Plate Tension+Shear, ft/Ft+(fv/Fv)^2: n/a Plate Comp. (AISC Bracket): n/a

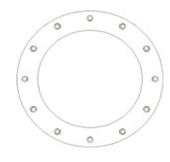
Pole Results

Pole Punching Shear Check: n/a



Pole Data		
Diam:	30.62	in
Thick:	0.1875	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None

Stress Increase Factor		
ASIF:	1.333	





^{* 0 =} none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

^{**} Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

Stiffened or Unstiffened, Ungrouted, Circular Base Plate - Any Rod Material

TIA Rev F

Site Data

BU#: 841290

Site Name: GREENWICH NORTH

App #: 284770 Rev. 0

Pole Manufacturer: Other

Anchor Rod Data		
Qty:	24	
Diam:	2.25	in
Rod Material:	A615-J	
Strength (Fu):	100	ksi
Yield (Fy):	75	ksi
Bolt Circle:	67	in

Plate Data		
Diam:	73	in
Thick:	2.25	in
Grade:	60	ksi
Single-Rod B-eff:	7.80	in

Stiffener Data (Welding at both sides)		
Config:	0	*
Weld Type:		
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		< Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

Pole Data		
Diam:	59	in
Thick:	0.5	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None

Stress Increase Factor		
ASIF:	1.333	

Reactions		
Moment:	4439.01	ft-kips
Axial:	52.23	kips
Shear:	36.9	kips

If No stiffeners, Criteria:	AISC ASD	<-Only Applcable to Unstiffened Cases
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Anchor Rod Results

Maximum Rod Tension: 130.3 Kips Allowable Tension: 195.0 Kips Anchor Rod Stress Ratio: 66.9% Pass

Base Plate Results	Flexural Check
Base Plate Stress:	50.5 ksi
Allowable Plate Stress:	60.0 ksi
Base Plate Stress Ratio:	84.2% Pass

Rigid
Service ASD
0.75*Fy*ASIF
Y.L. Length:
31.75

Rigid

Service, ASD

Fty*ASIF

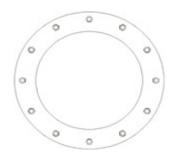
<u>n/a</u>

Stiffener Results

Horizontal Weld: n/a Vertical Weld: n/a Plate Flex+Shear, fb/Fb+(fv/Fv)^2: n/a Plate Tension+Shear, ft/Ft+(fv/Fv)^2: n/a Plate Comp. (AISC Bracket): n/a

Pole Results

Pole Punching Shear Check: n/a





Analysis Date: 2/27/2015

^{* 0 =} none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

^{**} Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes



Mat Foundation Analysis GREENWICH NORTH (BU #: 841290) 2015777.841290.02

General Info		
Code	TIA/EIA-222-F (LRFD)	
Bearing On	Soil	
Foundation Type	Mono Pad	
Pier Type	Square	
Reinforcing Known	Yes	
Max Capacity	1.1	

Tower Reactions			
Moment, M	4439.01	k-ft	
Axial, P	52.23	k	
Shear, V	36.9	k	

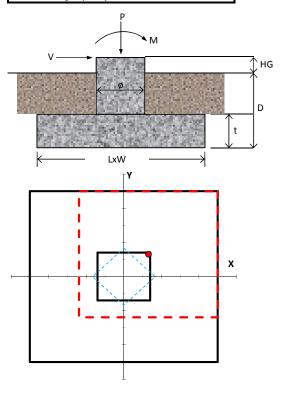
Pad & Pier Geometry			
Pier Width, ø	7	ft	
Pad Length, L	25	ft	
Pad Width, W	25	ft	
Pad Thickness, t	4.5	ft	
Depth, D	9.5	ft	
Height Above Grade, HG	0.5	ft	

Pad & Pier Reinforcing			
Rebar Fy	60	ksi	
Concrete Fc'	3	ksi	
Clear Cover	3.5	in	
Reinforced Top & Bottom?	Yes		
Pad Reinforcing Size	# 11		
Pad Quantity Per Layer	19		
Pier Rebar Size	# 11		
Pier Quantity of Rebar	23		

Soil Properties				
Soil Type	Granular			
Soil Unit Weight	120	pcf		
Angle of Friction, ø	34	0		
Bearing Type	Net			
Ultimate Bearing	30	ksf		
Water Table Depth	99	ft		
Frost Depth	5	ft		

Bearing S	ummary		Load Case
Qxmax	2.80	ksf	1.2D+1.6W
Qymax	2.80	ksf	1.2D+1.6W
Qmax @ 45°	3.03	ksf	1.2D+1.6W
Q _{(all) Gross}	23.36	ksf	
Controlling Capacity	13.0%	Pass	

Overturning Summary (Required FS=1.0)		Load Case	
FS(ot)x	1.86	≥1.0	0.9D+1.6W
FS(ot)y	1.86	≥1.0	0.9D+1.6W
Controlling Capacity	53.8%	Pass	



GPD Mat Foundation Analysis - V1.02



Base Foundation Reinforcement Check GREENWICH NORTH (BU #: 841290) 2015777.841290.02

Code TIA/EIA-222-F

Tower Reactions				
Moment	4439.01	k-ft		
Axial	52.23	k		
Shear	36.9	k		

Pad & Pier Geometry				
Height	9.5	ft		
Height above Grade	0.5	ft		
Pad Length, L	25	ft		
Pad Width, W	25	ft		
Pad Thickness	4.5	ft		
Pier Shape	Square			
Square Pier Width	7	ft		

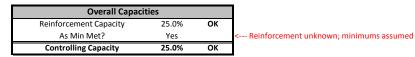
Pad & Pier Reinforcing				
Reinforcing Known	No			
f _c '	3	ksi		
Clear Cover	3.5	in		
Rebar Fy	60	ksi		
Reinforced Top & Bottom?	Yes			
Pad Rebar Size	# 11			
Pad Rebar Quantity	19			
Pier Rebar Size	# 11			
Pier Rebar Quantity	23			

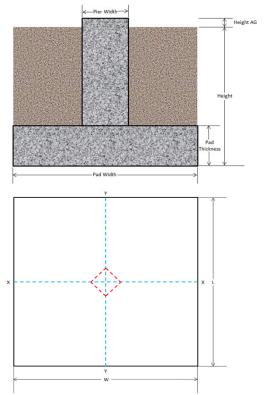
Unit Weights				
Concrete Unit Weight	150	pcf		
Soil Unit Weight	120	pcf		

Orthogonal Bearing			
Q _{max}	3.84	ksf	
Q_{min}	0.00	ksf	
Bearing Length 18.23 ft			

Pad Moment Capacity			
M _u =	62.88	k-ft	
φM _n =	251.94	k-ft	
Moment Capacity	25.0%	ОК	
One-Way (Wide-Be	am) Shear		
V _u =	206.35	kips	
φV _n =	1192.57	kips	
Shear Capacity	17.3%	ОК	
Two-Way (Punching) Shear			
V _u =	689.86	kips	
φV _n =	4210.09	kips	
Shear Capacity	16.4%	ОК	
Pier Compression			
P _u =	67.90	kips	
$\Phi P_n =$	10428.14	kips	
Compression Capacity	0.7%	ОК	

Base Foundation Reinforcement - V1.09







RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CT11069A

Greenwich / Boy Scouts 2 363 Riversville Road Greenwich, CT 06831

March 10, 2015

EBI Project Number: 6215001457

Site Compliance Summary		
Compliance Status:	COMPLIANT	
Site total MPE% of		
FCC general public	41.26 %	
allowable limit:		



March 10, 2015

T-Mobile USA Attn: Jason Overbey, RF Manager 35 Griffin Road South Bloomfield, CT 06002

Emissions Analysis for Site: CT11069A – Greenwich / Boy Scouts 2

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **363 Riversville Road**, **Greenwich**, **CT**, for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter (μ W/cm²). The number of μ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter (μ W/cm²). The general population exposure limit for the 700 MHz Band is 467 μ W/cm², and the general population exposure limit for the PCS and AWS bands is 1000 μ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at **363 Riversville Road, Greenwich, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 GSM channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel
- 2) 2 UMTS channels (AWS Band 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 3) 2 LTE channels (AWS Band 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 4) 1 LTE channel (700 MHz Band) was considered for each sector of the proposed installation. This channel has a transmit power of 30 Watts.
- 5) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.



- 6) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antennas used in this modeling are the Ericsson AIR21 (B4A/B2P & B2A/B4P) for 1900 MHz (PCS) and 2100 MHz (AWS) channels and the Commscope LNX-6515DS-VTM for 700 MHz channels. This is based on feedback from the carrier with regards to anticipated antenna selection. The Ericsson AIR21 (B4A/B2P & B2A/B4P) have a maximum gain of 15.9 dBd at its main lobe. The Commscope LNX-6515DS-VTM has a maximum gain of 14.6 dBd at its main lobe. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antenna mounting height centerline of the proposed antennas is **163 feet** above ground level (AGL).
- 9) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general public threshold limits.



T-Mobile Site Inventory and Power Data

Sector:	A	Sector:	В	Sector:	С
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	Ericsson AIR21 B4A/B2P	Make / Model:	Ericsson AIR21 B4A/B2P	Make / Model:	Ericsson AIR21 B4A/B2P
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	163	Height (AGL):	163	Height (AGL):	163
Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)
Channel Count	2	Channel Count	2	# PCS Channels:	2
Total TX Power:	120	Total TX Power:	120	# AWS Channels:	120
ERP (W):	4,668.54	ERP (W):	4,668.54	ERP (W):	4,668.54
Antenna A1 MPE%	0.68	Antenna B1 MPE%	0.68	Antenna C1 MPE%	0.68
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	Ericsson AIR21 B2A/B4P	Make / Model:	Ericsson AIR21 B2A/B4P	Make / Model:	Ericsson AIR21 B2A/B4P
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	163	Height (AGL):	163	Height (AGL):	163
Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)
Channel Count	4	Channel Count	4	Channel Count	4
Total TX Power:	120	Total TX Power:	120	Total TX Power:	120
ERP (W):	4,668.54	ERP (W):	4,668.54	ERP (W):	4,668.54
Antenna A2 MPE%	0.68	Antenna B2 MPE%	0.68	Antenna C2 MPE%	0.68
Antenna #:	3	Antenna #:	3	Antenna #:	3
Make / Model:	Commscope LNX- 6515DS-VTM	Make / Model:	Commscope LNX- 6515DS-VTM	Make / Model:	Commscope LNX- 6515DS-VTM
Gain:	14.6 dBd	Gain:	14.6 dBd	Gain:	14.6 dBd
Height (AGL):	163	Height (AGL):	163	Height (AGL):	163
Frequency Bands	700 MHz	Frequency Bands	700 MHz	Frequency Bands	700 MHz
Channel Count	1	Channel Count	1	Channel Count	1
Total TX Power:	30	Total TX Power:	30	Total TX Power:	30
ERP (W):	865.21	ERP (W):	865.21	ERP (W):	865.21
Antenna A3 MPE%	0.27	Antenna B3 MPE%	0.27	Antenna C3 MPE%	0.27

Site Composite MPE%		
Carrier	MPE%	
T-Mobile	4.90	
AT&T	12.71 %	
Verizon Wireless	11.92 %	
Sprint	11.73 %	
Site Total MPE %:	41.26 %	

T-Mobile Sector 1 Total:	1.63 %	
T-Mobile Sector 2 Total:	1.63 %	
T-Mobile Sector 3 Total:	1.63 %	
Site Total:	41.26 %	



Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general public exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general public exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector 1:	1.63 %
Sector 2:	1.63 %
Sector 3:	1.63 %
T-Mobile Total:	4.90 %
Site Total:	41.26 %
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **41.26%** of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

Scott Heffernan

RF Engineering Director

EBI Consulting

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