

February 16, 2016

Melanie A. Bachman  
Acting Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

**Re: Notice of Exempt Modification – Facility Modification  
5 Perryridge Road, Greenwich, Connecticut**

Dear Ms. Bachman:

Cellco Partnership d/b/a Verizon Wireless (“Cellco”) currently maintains twelve (12) antennas at the 124-foot level of the existing 164-foot tower at 5 Perryridge Road in Greenwich (the “Property”). The tower and the Property are owned by Greenwich Hospital. Cellco’s shared use of this tower was approved by the Council in 2002. The original tower site was approved by the Council in Docket No. 73. Cellco now intends to replace six (6) of its existing antennas with three (3) model SBNHH-1D65B, 700/1900 MHz antennas and three (3) model SBNHH-1D65B, 2100 MHz antennas, all at the same 124-foot level on the tower. Cellco also intends to replace six (6) remote radio heads (“RRHs”), install three (3) new RRHs and install one (1) HYBRIFLEX™ fiber optic antenna cable. Included in Attachment 1 are specifications for Cellco’s replacement antennas, RRHs and HYBRIFLEX™ cable.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Peter Tesei, First Selectman of the Town of Greenwich and Greenwich Hospital, the owner of the Property and the tower.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

Melanie A. Bachman  
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1. The proposed modifications will not result in an increase in the height of the existing tower. Cellco's replacement antennas and RRHs will be installed on its existing antenna platform at the 124-foot level.
2. The proposed modifications will not involve any change to ground-mounted equipment and, therefore, will not require the extension of the site boundary.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) safety standard. A cumulative General Power Density table for Cellco's modified facility is included in Attachment 2.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The tower and its foundation can support Cellco's proposed modifications. (*See Structural Analysis Report included in Attachment 3*).

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,



Kenneth C. Baldwin

Enclosures

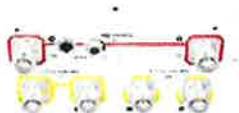
Copy to:

Peter Tesei, Greenwich First Selectman  
Greenwich Hospital  
Tim Parks

# **ATTACHMENT 1**

## SBNHH-1D65B

**Andrew® Tri-band Antenna, 698–896 and 2 x 1710–2360 MHz, 65° horizontal beamwidth, internal RET. Both high bands share the same electrical tilt.**



- Interleaved dipole technology providing for attractive, low wind load mechanical package

### Electrical Specifications

Frequency Band, MHz	698–806	806–896	1710–1880	1850–1990	1920–2180	2300–2360
Gain, dBi	14.9	14.7	17.7	18.2	18.6	18.6
Beamwidth, Horizontal, degrees	68	66	69	66	63	58
Beamwidth, Vertical, degrees	12.1	10.7	5.6	5.2	5.0	4.5
Beam Tilt, degrees	0–14	0–14	0–7	0–7	0–7	0–7
USLS, dB	14	13	15	15	15	13
Front-to-Back Ratio at 180°, dB	27	29	28	28	28	27
CPR at Boresight, dB	20	23	20	20	17	21
CPR at Sector, dB	14	10	12	10	9	1
Isolation, dB	25	25	25	25	25	25
Isolation, Intersystem, dB	30	30	30	30	30	30
VSWR   Return Loss, dB	1.5   14.0	1.5   14.0	1.5   14.0	1.5   14.0	1.5   14.0	1.5   14.0
PIM, 3rd Order, 2 x 20 W, dBc	-153	-153	-153	-153	-153	-153
Input Power per Port, maximum, watts	350	350	350	350	350	300
Polarization	±45°	±45°	±45°	±45°	±45°	±45°
Impedance	50 ohm	50 ohm	50 ohm	50 ohm	50 ohm	50 ohm

### Electrical Specifications, BASTA\*

Frequency Band, MHz	698–806	806–896	1710–1880	1850–1990	1920–2180	2300–2360
Gain by all Beam Tilts, average, dBi	14.5	14.3	17.4	17.9	18.2	18.3
Gain by all Beam Tilts Tolerance, dB	±0.5	±0.8	±0.4	±0.3	±0.5	±0.3
Gain by Beam Tilt, average, dBi	0°   14.6	0°   14.5	0°   17.4	0°   17.8	0°   18.1	0°   18.2
	7°   14.6	7°   14.4	3°   17.5	3°   17.9	3°   18.3	3°   18.4
	14°   14.2	14°   13.6	7°   17.4	7°   17.9	7°   18.2	7°   18.4
Beamwidth, Horizontal Tolerance, degrees	±2.2	±3.4	±2	±4.6	±5.7	±4.3
Beamwidth, Vertical Tolerance, degrees	±0.8	±1	±0.3	±0.2	±0.3	±0.2
USLS, dB	16	14	16	16	16	15
Front-to-Back Total Power at 180° ± 30°, dB	25	26	27	26	26	26
CPR at Boresight, dB	22	23	21	20	20	22
CPR at Sector, dB	13	11	16	12	11	4

\* CommScope® supports NGMN recommendations on Base Station Antenna Standards (BASTA). To learn more about the benefits of BASTA, [download the whitepaper Time to Raise the Bar on BSAs.](#)

### General Specifications

Antenna Brand	Andrew®
Antenna Type	DualPol® multiband with internal RET
Band	Multiband
Brand	DualPol®   Teletilt®
Operating Frequency Band	1710 – 2360 MHz   698 – 896 MHz

### Mechanical Specifications

# Product Specifications

COMMScope®

SBNHH-1D65B



Color	Light gray
Lightning Protection	dc Ground
Radiator Material	Aluminum   Low loss circuit board
Radome Material	Fiberglass, UV resistant
Reflector Material	Aluminum
RF Connector Interface	7-16 DIN Female
RF Connector Location	Bottom
RF Connector Quantity, total	6
Wind Loading, maximum	617.7 N @ 150 km/h 138.9 lbf @ 150 km/h
Wind Speed, maximum	241.4 km/h   150.0 mph

## Dimensions

Depth	181.0 mm   7.1 in
Length	1828.0 mm   72.0 in
Width	301.0 mm   11.9 in
Net Weight	18.4 kg   40.6 lb

## Remote Electrical Tilt (RET) Information

Input Voltage	10–30 Vdc
Power Consumption, idle state, maximum	2.0 W
Power Consumption, normal conditions, maximum	13.0 W
Protocol	3GPP/AISG 2.0 (Multi-RET)
RET Interface	8-pin DIN Female   8-pin DIN Male
RET Interface, quantity	1 female   1 male
RET System	Teletilt®

## Regulatory Compliance/Certifications

### Agency

RoHS 2011/65/EU  
China RoHS SJ/T 11364-2006  
ISO 9001:2008

### Classification

Compliant by Exemption  
Above Maximum Concentration Value (MCV)  
Designed, manufactured and/or distributed under this quality management system



## Included Products

BSAMNT-1 — Wide Profile Antenna Downtilt Mounting Kit for 2.4 - 4.5 in (60 - 115 mm) OD round members. Kit contains one scissor top bracket set and one bottom bracket set.

# ALCATEL-LUCENT B13 RRH4X30-4R

Alcatel-Lucent B13 Remote Radio Head 4x30-4R is the newest addition of Remote Radio Head to the extended product line of Alcatel-Lucent's distributed Base Station solutions, aimed at facilitating smooth RF site acquisition and related civil engineering.

**Supporting 2Tx/4Tx MIMO and 4-way Rx diversity**, Alcatel-Lucent B13 RRH4x30-4R allows operators to have a compact radio solution to deploy LTE in the 700U band (700 MHz, 3GPP band 13), providing them with the means to achieve high capacity, high quality and high coverage with minimum site requirements.

The Alcatel-Lucent B13 RRH4x30-4R product has four transmit RF paths, offering the possibility to **select, via software only, 2Tx or 4Tx MIMO configurations** with either 2x60 W or 4x30 W RF output power. It supports also 4-way Rx diversity and up to 10MHz instantaneous bandwidth.

The Alcatel-Lucent B13 RRH4x30-4R is a near zero-footprint solution and operates noise free, simplifying negotiations with site property owners and minimizing environmental impacts.

Its compactness and slim design makes the Alcatel-Lucent B13 RRH4x30-4R easy to install close to the antenna: operators can therefore locate this Remote Radio Head where RF design conditions are deemed ideal, minimizing trade-offs between available sites and RF optimum sites, together with reducing the RF feeder needs and installation costs.

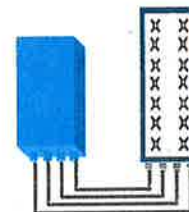


## FEATURES

- Supporting LTE in 700 MHz band (700U, 3GPP band 13)
- LTE 2Tx or 4Tx MIMO (SW switchable)
- Output power: Up to 2x60W or 4x30W
- 10MHz LTE carrier with 4Rx Diversity
- Convection-cooled (fan-less)
- Supports AISG 2.0 ALD devices (RET, TMA) through RS485 or RF ports

## BENEFITS

- Compact to reduce additional footprint when adding LTE in 700U band
- MIMO scheme operation selection (2Tx or 4Tx) by software only
- Improves downlink spectral efficiency through MIMO4
- Increases LTE coverage thanks to 4Rx diversity capability and best in class Rx sensitivity
- Flexible mounting options: Pole or Wall



4x30W with 4T4R  
or  
2x60W with 2T4R

Can be switched between modes via SW w/o site visit

# TECHNICAL SPECIFICATIONS

Features & performance	
Number of TX/RX paths	4 duplexed (either 4T4R or 2T4R by SW)
Frequency band	U700 (C) (3GPP bands 13): DL: 746 - 756 MHz / UL: 777 - 787 MHz
Instantaneous bandwidth - #carriers	10MHz – 1 LTE carrier (in 10MHz occupied bandwidth)
LTE carrier bandwidth	10 MHz
RF output power	2x60W or 4x30W (by SW)
Noise figure – RX Diversity scheme	2 dB typ. (<2.5 dB max) – 2 or 4 way Rx diversity
Sizes (HxWxD) in mm (in.)	550 x 305 x 230 (21.6" x 12.0" x 9") (with solar shield)
Volume in L	38 (with solar shield)
Weight in kg (lb) (w/o mounting HW)	26 (57.2) (with solar shield)
DC voltage range	-40.5 to -57V at full performance, -38 to -57V with relaxation on power consumption
DC power consumption	550W typical @100% RF load ( In 2Tx or 4TX mode)
Environmental conditions	-40°C (-40°F) / +55°C (+131°F)
Wind load (@150km/h or 93mph)	IP65 Frontal: <200N / Lateral : <150N
Antenna ports	4 ports 7/16 DIN female (50 ohms) VSWR < 1.5
CPRI ports	2 CPRI ports (HW ready for Rate7, 9.8 Gbps) SFP single mode dual fiber
AISG interfaces	1 AISG2.0 output (RS485) Integrated Smart Bias Tees (x2)
Misc. Interfaces	4 external alarms (1 connector) – 4 RF Tx & 4 RF Rx monitor ports - 1 DC connector (2 pins)
Installation conditions	Pole and wall mounting
Regulatory compliance	3GPP 36.141 / 3GPP 36.113 / GR-1089-CORE / GR-3108-CORE / UL 60950-1 / FCC Part 27

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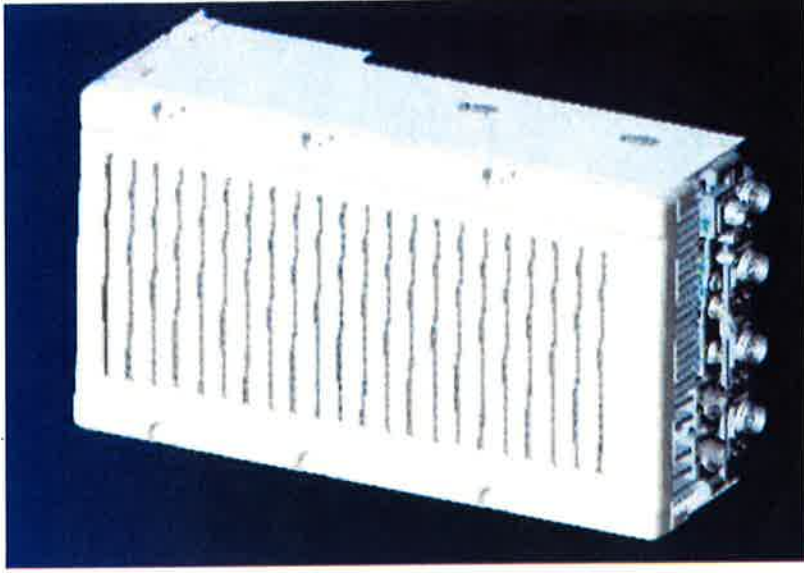
# NEW PCS RF MODULES FOR VZW

## RRH2X60 - HW CHARACTERISTICS

LR14.3

RRH2x60	
RF Output Power	2x60W (4x30W HW Ready)
Instantaneous Bandwidth	60MHz
Target Reliability (Annual Return Rate)	<2%
Receiver	4 Branch Rx
Features	AISG 2.0 for RET/TMA
Power	-48VDC Internal Smart Bias-T
CPRI Ports	2 CPRI Rate 5 Ports
External Alarms	4 External User Alarms
Monitor Ports	TX, RX
Environmental	GR487 Compliance
RF Connectors	7/16 DIN (downward facing)
Dimensions	22"(h) x 12"(w) x 9.4" (d)**
Weight	55lb**

\*\* - Includes solar shield but not mounting brackets (8 lbs.)





# VZW Network Equipment Reporting Form (NERF)

Vendor	Alcatel-Lucent		Model	B66a RRH 4Tx/4Rx 4x45W or 2x 90W (SW selectable)		Function	RRH for distributed architecture with a CPRI interface between digital and RF processing components. The RRH has 4 Tx ports and 4 Rx ports. Can be SW configured for 2 Tx with 90W rf per port or 4 Tx with 45W rf per port. The RRH has passive cooling only.		
*1) Equipment Configuration	*2) Heat Release @ 50°F Intake Temp [W]		*3) Airflow Rate @ 100% Activity Rate [cfm]		*4) Dimensions [in]		Non-Thermal Data		
	100% Activity	50% Activity	Nominal (70°F)	Max (95°F)	External (WxDxH)	Clear (F/R/S)	Installed Weight [lb]	*5) Sound @ Nominal [L <sub>wad</sub> ]	*6) Name Plate [W]
Minimum			N/A Convection cooled	N/A Convection cooled	w/o Solar Shield W = 11.4in D = 6.7in H = 25.2in (W=290mm) (D=170mm) (H=640mm)	Front: 12" Rear: 7.5" Right: 12" Left: 12" Top: 12" Bottom: 24"			
Typical			N/A Convection cooled	N/A Convection cooled	with Solar Shield W = 12in D = 7.6in H = 25.8in (W=304mm) (D=193mm) (H=655mm)		62lb 72 lb(w mounting brackets)	N/A Convection cooled	
Full	825W (add 60W for AISG)	TBD	N/A Convection cooled	N/A Convection cooled	N/A			N/A Convection cooled	
*7) Equipment EC-Class	N/A Convection cooled	*10) Fan Speed	N/A Convection cooled	*13) Fan Hot-Swap	N/A Convection cooled	*16) Environ. Tests	N/A Convection cooled	*18) Temp. Rise [°F]	N/A Convection cooled
*8) Non-Optimal EC-Class	N/A Convection cooled	*11) Fan Logic	N/A Convection cooled	*14) Shut-Down	N/A Convection cooled	*17) Allow. Max [°F]	N/A Convection cooled	*19) Rec. Max [°F]	N/A Convection cooled
*9) Exhaust Openings	N/A Convection cooled	*12) Fan Alarm	N/A Convection cooled	*15) Temp. Access	N/A Convection cooled	*17) Allow. Min [°F]	N/A Convection cooled	*19) Rec. Min [°F]	N/A Convection cooled
Power Reporting									
Power Input	-48V	No. Power Supplies	N/A (Customer provided power plant)		Number of Inputs per Power Supply	1			
*24) Maximum Demand (total system in Watts)	825W (add 60W for AISG)	Maximum Input (each power supply in Watts)	N/A (Customer provided power plant)		Maximum Output (each power supply in Watts)	58W (to AISG port, 29V/2A)			
Power Supply Connection Type	DC entry via Conduit Box	Power Supply Make & Model	N/A (Customer provided power plant)						
Input Protection	no input fuse	Input Protection Make & Model	N/A (Customer provided power plant)						
Redundancy Scheme	N/A								
Nominal Voltage	-48VDC	Maximum Voltage	-57V		Minimum Voltage	-38V			
*25) Max Current at Nominal Voltage	17.2A (add 1.2A if AISG port loaded 2A*29V)	*25) Max Current at Maximum Voltage	14.5A (add 1A if AISG port loaded 2A*29V)		*25) Max Current at Minimum Voltage	21.7A (add 1.5A if AISG port loaded 2A*29V)			

Return completed forms to Engineering and Operations Support (EOS)

[Richard.damiano@verizonwireless.com](mailto:Richard.damiano@verizonwireless.com)



HYBRIFLEX™ RRH Hybrid Feeder Cabling Solution, 1-5/8", Single-Mode Fiber

Product Description

RFS' HYBRIFLEX Remote Radio Head (RRH) hybrid feeder cabling solution combines optical fiber and DC power for RRHs in a single lightweight aluminum corrugated cable, making it the world's most innovative solution for RRH deployments.

It was developed to reduce installation complexity and costs at Cellular sites. HYBRIFLEX allows mobile operators deploying an RRH architecture to standardize the RRH installation process and eliminate the need for and cost of cable grounding. HYBRIFLEX combines optical fiber (multi-mode or single-mode) and power in a single corrugated cable. It eliminates the need for junction boxes and can connect multiple RRHs with a single feeder. Standard RFS CELLFLEX® accessories can be used with HYBRIFLEX cable. Both pre-connectorized and on-site options are available.

Features/Benefits

- Aluminum corrugated armor with outstanding bending characteristics - minimizes installation time and enables mechanical protection and shielding
- Same accessories as 1 5/8" coaxial cable
- Outer conductor grounding - Eliminates typical grounding requirements and saves on installation costs
- Lightweight solution and compact design - Decreases tower loading
- Robust cabling - Eliminates need for expensive cable trays and ducts
- Installation of tight bundled fiber optic cable pairs directly to the RRH - Reduces CAPEX and wind load by eliminating need for interconnection
- Optical fiber and power cables housed in single corrugated cable - Saves CAPEX by standardizing RRH cable installation and reducing installation requirements
- Outdoor polyethylene jacket - Ensures long-lasting cable protection

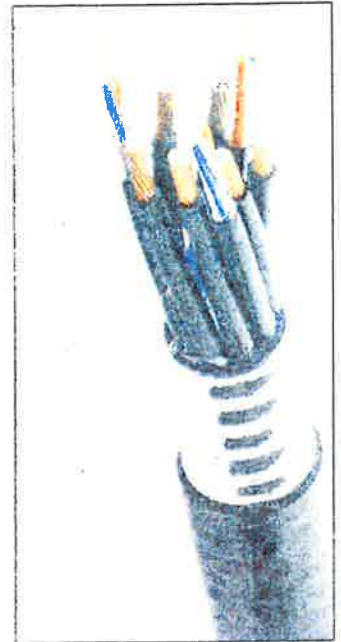


Figure 1: HYBRIFLEX Series

Technical Specifications

Outer Conductor Armor:	Corrugated Aluminum	(mm (in))	46.5 (1.83)
Jacket:	Polyethylene, PE	(mm (in))	50.3 (1.98)
UV-Protection:	Individual and External Jacket		Yes
<b>Weight and Bending Characteristics</b>			
Weight, Approximate		(kg/m (lb/ft))	1.9 (1.30)
Minimum Bending Radius, Single Bending		(mm (in))	200 (8)
Minimum Bending Radius, Repeated Bending		(mm (in))	500 (20)
Recommended/Maximum Clamp Spacing		(m (ft))	1.0 / 1.2 (3.25 / 4.0)
<b>Electrical Properties</b>			
DC-Resistance Outer Conductor Armor		(Ω/km (Ω/1000ft))	068 (0.205)
DC-Resistance Power Cable, 8.4mm <sup>2</sup> (8AWG)		(Ω/km (Ω/1000ft))	2.1 (0.307)
<b>Optical Properties</b>			
Version			Single-mode OM3
Quantity, Fiber Count			16 (8 pairs)
Core/Clad		(μm)	50/125
Primary Coating (Acrylate)		(μm)	245
Buffer Diameter, Nominal		(μm)	900
Secondary Protection, Jacket, Nominal		(mm (in))	2.0 (0.08)
Minimum Bending Radius		(mm (in))	104 (4.1)
Insertion Loss @ wavelength 850nm		dB/km	3.0
Insertion Loss @ wavelength 1310nm		dB/km	1.0
Standards (Meets or exceeds)			UL34-V0, UL1666 RoHS Compliant
<b>Power Cable Properties</b>			
Size (Power)		(mm (AWG))	8.4 (8)
Quantity, Wire Count (Power)			16 (8 pairs)
Size (Alarm)		(mm (AWG))	0.8 (18)
Quantity, Wire Count (Alarm)			4 (2 pairs)
Type			UV protected
Strands			19
Primary Jacket Diameter, Nominal		(mm (in))	6.8 (0.27)
Standards (Meets or exceeds)			NFPA 130, ICEA S-95-658 UL Type XHHW-2, UL 44 UL-LS Limited Smoke, UL VW-1 IEEE-383 (1974), IEEE1202/FT4 RoHS Compliant
<b>Operating Range</b>			
Installation Temperature		(°C (°F))	-40 to +65 (-40 to 149)
Operation Temperature		(°C (°F))	-40 to +65 (-40 to 149)

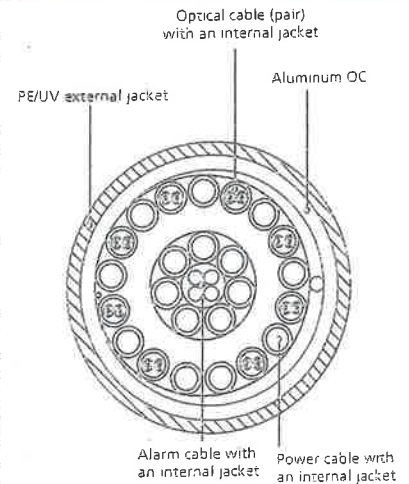


Figure 2: Construction Detail

All information contained in the present datasheet is subject to confirmation at time of ordering.

This data is provisional and subject to change

RFS The Clear Choice®

HB158-1-08U8-S&J18

Rev: P1

Print Date: 27.5.2012

Radio Frequency Systems

# **ATTACHMENT 2**

Site Name: Greenwich Relo Tower Height: 164ft		General	Power	Density				
CARRIER	# OF CHAN.	WATTS ERP	HEIGHT	CALC. POWER DENS	FREQ.	MAX. PERMISS. EXP.	FRACTION MPE	Total
*AT&T GSM	6	427	134	0.0562	1900	1.0000	0.56%	
*AT&T GSM	15	296	134	0.0975	880	0.5867	1.66%	
*AT&T UMTS	1	500	134	0.0110	880	0.5867	0.19%	
*AT&T UMTS	2	500	134	0.0219	1900	1.0000	0.22%	
*AT&T LTE	1	500	134	0.0110	740	0.4933	0.22%	
*MW to Bruce	1	4878	160	0.0740	17960	1.0000	0.74%	
*MW to PD	1	122	160	0.0018	18762	1.0000	0.02%	
*MW to Putnam	1	4878	160	0.0740	17500	1.0000	0.74%	
*Trunked System	1	148	164	0.0021	886.7875	0.5912	0.04%	
*Trunked System	1	148	164	0.0021	867.0625	0.5780	0.04%	
*Trunked System	1	148	164	0.0021	868.15	0.5788	0.04%	
*Trunked System	1	148	164	0.0021	868.4	0.5789	0.04%	
*Trunked System	1	148	164	0.0021	868.7	0.5791	0.04%	
*Trunked System	1	148	164	0.0021	868.7	0.5791	0.04%	
*Mutual Aid	1	218	155	0.0035	866.0125	0.5773	0.06%	
*Mutual Aid	1	218	155	0.0035	866.5125	0.5777	0.06%	
*CMED	1	150	151	0.0026	463	0.3087	0.08%	
*Fire Paging	1	100	125	0.0025	164.175	0.2000	0.13%	
*SP Hotline	1	100	110	0.0033	154.175	0.2000	0.17%	
*Sprint CDMA/LTE	3	69	155	0.0034	1900	1.0000	0.03%	
*Sprint CDMA/LTE	1	39	155	0.0006	850	0.5667	0.01%	
*Sprint CDMA/LTE	2	69	155	0.0022	2500	1.0000	0.02%	
*Clearwire	2	153	154	0.0050	2496	1.0000	0.05%	
*Clearwire	1	211	154	0.0035	11 GHz	1.0000	0.03%	
*T-Mobile PCS (GSM/UMTS)	6	1706	144	0.1933	1900/2100	1.0000	1.93%	
*T-Mobile AWS (UMTS)	1	865	144	0.0163	700	0.4667	0.35%	
*T-Mobile AWS (LTE)	12	100	113	0.0377	851	0.5673	0.66%	
*Nextel	3	562	154	0.0277	2657	1.0000	0.28%	
*Sprint/Nextel WiMAX	3	562	154	0.0256	2657	1.0000	2.56%	
*Eversource	1	250	116.5		937	0.6247	0.11%	
*Eversource	1	250	116.5		154	0.1027	11.55%	
*Eversource	1	250	112		37	0.0247	8.78%	
<b>Verizon PCS</b>	<b>1</b>	<b>2413</b>	<b>124</b>	<b>0.0564</b>	<b>1970</b>	<b>1.0000</b>	<b>5.64%</b>	
<b>Verizon Cellular</b>	<b>9</b>	<b>319</b>	<b>124</b>	<b>0.0671</b>	<b>869</b>	<b>0.5793</b>	<b>11.59%</b>	
<b>Verizon AWS</b>	<b>1</b>	<b>2302</b>	<b>124</b>	<b>0.0538</b>	<b>2145</b>	<b>1.0000</b>	<b>5.38%</b>	
<b>Verizon 700</b>	<b>1</b>	<b>540</b>	<b>124</b>	<b>0.0126</b>	<b>746</b>	<b>0.4973</b>	<b>2.54%</b>	<b>56.60%</b>
* Source: Siting Council								

# **ATTACHMENT 3**

**Structural Analysis Report**

*164-ft Existing EEL Monopole*

*Proposed Verizon Wireless  
Antenna Upgrade*

*Verizon Site Ref: Greenwich*

*5 Perryridge Road  
Greenwich, CT*

*CEN TEK Project No. 15001.145*

*Date: January 20, 2016*



**Prepared for:**

**Verizon Wireless  
99 East River Road, 9<sup>th</sup> Floor  
East Hartford, CT 06108**

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- TOWER CAPACITY
- FOUNDATION AND ANCHORS
- CONCLUSION

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## Introduction

The purpose of this report is to summarize the results of the non-linear, P- $\Delta$  structural analysis of the antenna upgrade proposed by Verizon Wireless on the existing monopole (tower) owned and operated by Greenwich Hospital located in Greenwich, Connecticut.

The host tower is a 164-ft tall, five-section, eighteen sided, tapered monopole, originally designed and manufactured by Engineered Endeavors Incorporated (EEI); project no. 11030 dated August 21, 2002. The tower geometry, structure member sizes and foundation system information were obtained from the original manufacturers design documents.

Antenna and appurtenance information were obtained a previous structural analysis report prepared by Centek; job no; 14263.00, dated March 17, 2015, a previous structural analysis report prepared by AECOM; job no; 02162500, dated May 4, 2015 and a Verizon RF data sheet.

The tower is made up of five (5) tapered vertical sections consisting of A572-65 pole sections. The bottom four (4) vertical tower sections are slip joint connected while the top section is flange connected. The diameter of the pole (flat-flat) is 47.0-in at the top and 76.0-in at the base.

Verizon proposes the removal of six (6) panel antennas and six (6) remote radio heads and the installation of six (6) panel antennas, nine (9) remote radio heads and one (1) main distribution box mounted to the existing low profile platform. Refer to the Antenna and Appurtenance Summary below for a detailed description of the proposed antenna and appurtenance configuration.

## Antenna and Appurtenance Summary

The existing, proposed and future loads considered in this analysis consist of the following:

- TOWN (EXISTING):  
Antennas: One (1) 12-ft Omni-directional whip antenna, two (2) 10-ft Omni-directional whip antennas, two (2) 8-ft Omni-directional whip antennas, one Kathrein Scala 2' square panel and one (1) camera mounted on a PiROD 13-ft low profile platform with an elevation of 164-ft above grade level.  
Coax Cables: Six (6) 1/2"  $\varnothing$ , one (1) 5/8"  $\varnothing$ , three (3) 7/8"  $\varnothing$  and two (2) 1-1/4"  $\varnothing$  coax cables running on the inside of the existing tower.
- TOWN (EXISTING):  
Antennas: Two (2) 4 FT Dishes and one (1) 2 Ft Dish mounted on three 4'x4" pipe mounts with a RAD center elevation of 160-ft above grade level.  
Coax Cables: Three (3) 1-1/4"  $\varnothing$  coax cables running on the inside of the existing tower.
- CLEARWIRE (EXISTING):  
Antennas: Three (3) Argus LLPX310R panel antennas, three (3) Samsung FDD-R6-RRH, two (2) Dragonwave Horizon ODU's and two (2) Dragonwave A-ANT-23-G-2-C dishes mounted on the Sprint 13-ft low profile platform with a RAD center elevation of 154-ft above the existing tower base plate.  
Coax Cables: Two (2) 2"  $\varnothing$  conduits and two (2) 5/8"  $\varnothing$  coax cables running on the inside of the existing tower.



- **SPRINT (EXISTING):**  
Antennas: Two (2) RFS APXVSP18-C-A20 panel antennas, one (1) Powerwave P40-16-XLPP-RR-A panel antennas, three (3) RFS APXVTM14 panel antennas and one (1) GPS antenna mounted to a low profile platform with a RAD center elevation of 154-ft above the existing tower base plate. Three (3) ALU 1900 MHz RRH's, three (3) ALU 800 MHz RRH's and three (3) ALU TD-RRH-820 remote radio heads mounted on a universal tr-bracket below the existing low profile platform.  
Coax Cables: Six (6) 1-5/8" Ø Hybriflex cables and one (1) 1/2" Ø coax cable running on the inside of the existing tower.
- **T-MOBILE (EXISTING):**  
Antennas: Three (3) RFS APX16PV-16PVL panel antennas, three (3) Andrew LNX-6515DS panel antennas, nine (9) TMA's and three (3) Bias Tee's mounted on a PiROD 13-ft low profile platform with a RAD center elevation of 144-ft above grade level.  
Coax Cables: Twelve (12) 1-5/8" Ø coax cables inside the monopole and six (6) 1-5/8" Ø cables on the exterior of the existing tower.
- **AT&T (EXISTING):**  
Antennas: Six (6) Ericsson RRUS-11 and one (1) Raycap DC6-48-60-18-8F surge arrester mounted to one (1) universal ring mount with a RAD center elevation of 138-ft above grade level.  
Coax Cables: One (1) fiber cable and two (2) dc control cables running on the inside of the existing tower.
- **AT&T (EXISTING):**  
Antennas: Six (6) Powerwave 7770.00 panel antennas, three (3) Powerwave P65-16-XLH-RR panel antennas, six (6) LGP21401 TMA's and six (6) LGP21901 diplexers mounted on a PiROD 13-ft low profile platform with a RAD center elevation of 134-ft above grade level.  
Coax Cables: Twelve (12) 1-5/8" Ø coax cables running on the inside of the existing tower
- **EVERSOURCE ENERGY (EXISTING):**  
Antennas: Two (2) Decibel DB586-Y omni-directional whips (one upright and one inverted), one (1) Telewave ANT150F2 omni-directional whip, one (1) Comprod 531-70HD dipole and one (1) tower top amplifier mounted on a PiROD 13-ft low profile platform with an elevation of 114-ft above grade level.  
Coax Cables: Two (2) 1-5/8" Ø, two (2) 7/8" Ø and one (1) 1/2" Ø coax cables running on the inside of the existing tower on the inside of the existing tower.
- **UNKNOWN (EXISTING):**  
Antennas: Three GPS antennas mounted on three (3) standoffs with a RAD center elevation of 50-ft above grade level.  
Coax Cables: Three (3) 7/8" Ø coax cables running on the exterior of the existing tower.

- VERIZON (EXISTING TO REMAIN):  
Antennas: Six (6) Decibel DB844H65E-XY panel antennas, three (3) RYMSA MG D3-800T0 panel antennas, six (6) RFS FD9R6004/2C-3L Diplexers and one (1) Raycap RC2DC-3315-PF-48 main distribution box mounted on a 13-ft low profile platform with a RAD center elevation of 124-ft above grade level.  
Coax Cables: Six (6) 1-5/8"  $\varnothing$  coax cables and one (1) 1-5/8"  $\varnothing$  fiber cable running inside the monopole.
- VERIZON (EXISTING TO REMOVE):  
Antennas: Three (3) Andrew LNX-6514DS-T4M panel antennas, three (3) RYMSA MG D3-800T0 panel antennas, three (3) Alcatel-Lucent RRH2x40-AWS Remote Radio Heads and three (3) Alcatel-Lucent RRH2x40-07-U Remote Radio Heads mounted on a PIROD 13-ft low profile platform with a RAD center elevation of 124-ft above grade level.
- VERIZON (PROPOSED):  
Antennas: Six (6) Andrew SBNHH-1D65B panel antennas, three (3) Alcatel-Lucent RRH2x60-700 remote radio heads, three (3) Alcatel-Lucent RRH2x60-PCS remote radio heads, three (3) Alcatel-Lucent RRH4x45/2x90-AWS remote radio heads and one (1) Raycap RC2DC-3315-PF-48 main distribution box mounted on a 13-ft low profile platform with a RAD center elevation of 124-ft above grade level.  
Coax Cables: One (1) 1-5/8"  $\varnothing$  fiber cable running inside the monopole.

### Primary Assumptions Used in the Analysis

- The tower structure's theoretical capacity not including any assessment of the condition of the tower.
- The tower carries the horizontal and vertical loads due to the weight of antennas, ice load and wind.
- Tower is properly installed and maintained.
- Tower is in plumb condition.
- Tower loading for antennas and mounts as listed in this report.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds are fabricated with ER-70S-6 electrodes.
- All members are assumed to be as specified in the original tower design documents or reinforcement drawings.
- All members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
- All member protective coatings are in good condition.
- All tower members were properly designed, detailed, fabricated, installed and have been properly maintained since erection.
- Any deviation from the analyzed antenna loading will require a new analysis for verification of structural adequacy.
- All coax cables to be installed as indicated in this report.

## A n a l y s i s

The existing tower was analyzed using a comprehensive computer program entitled tnxTower. The program analyzes the tower, considering the worst case loading condition. The tower is considered as loaded by concentric forces along the tower shaft, and the model assumes that the shaft members are subjected to bending, axial, and shear forces.

The existing tower was analyzed for the controlling basic wind speed (fastest mile) with no ice and a 75% reduction of wind force with ½ inch accumulative ice to determine stresses in members as per guidelines of TIA/EIA-222-F-96 entitled “Structural Standards for Steel Antenna Towers and Antenna Supporting Structures”, the American Institute of Steel Construction (AISC) and the Manual of Steel Construction; Allowable Stress Design (ASD).

The controlling wind speed is determined by evaluating the local available wind speed data as provided in Appendix K of the CSBC<sup>1</sup> and the wind speed data available in the TIA/EIA-222-F-96 Standard. The higher of the two wind speeds is utilized in preparation on the tower analysis.

## T o w e r L o a d i n g

Tower loading was determined by the basic wind speed as applied to projected surface areas with modification factors per TIA/EIA-222-F, gravity loads of the tower structure and its components, and the application of ½” radial ice on the tower structure and its components.

Basic Wind Speed:	Fairfield; v = 85 mph (fastest mile) Greenwich; v = 100 mph (3 second gust) equivalent to v = 80 mph (fastest mile) <i>TIA/EIA wind speed controls.</i>	<i>[Section 16 of TIA/EIA-222-F-96]</i> <i>[Appendix K of the 2005 CT Building Code Supplement]</i>
Load Cases:	<u>Load Case 1</u> ; 85 mph wind speed w/ no ice plus gravity load – used in calculation of tower stresses and rotation.  <u>Load Case 2</u> ; 74 mph wind speed w/ ½” radial ice plus gravity load – used in calculation of tower stresses. The 74 mph wind speed velocity represents 75% of the wind pressure generated by the 85 mph wind speed.  <u>Load Case 3</u> ; Seismic – not checked	<i>[Section 2.3.16 of TIA/EIA-222-F-96]</i> <i>[Section 2.3.16 of TIA/EIA-222-F-96]</i> <i>[Section 1614.5 of State Bldg. Code 2005] does not control in the design of this structure type</i>

<sup>1</sup> The 2005 Connecticut State Building Code as amended by the 2009 CT State Supplement. (CSBC)

## Tower Capacity

Tower stresses were calculated utilizing the structural analysis software tnxTower. Allowable stresses were determined based on Table 5 of the TIA/EIA code with a 1/3 increase per Section 3.1.1.1 of the same code.

- Calculated stresses were found to be within allowable limits. In Load Case 1, per tnxTower "Section Capacity Table", this tower was found to be at **48.3%** of its total capacity.

Tower Section	Elevation	Stress Ratio (percentage of capacity)	Result
Pole Shaft (L5)	1.50'-39.88'	48.3%	<b>PASS</b>

- The tower deflection (tilt) was found to be within allowable limits.

Deflection (degrees)	Proposed	Allowable <sup>(1)</sup>	Result
Tilt	1.42	1.9	<b>PASS</b>

(1) Allowable tilt taken from original EEI design documents job no. 11030 dated 8/21/02.

## Foundation and Anchors

The existing foundation consists of a 9.0  $\varnothing$  x 28.0-ft long reinforced concrete caisson. The sub-grade conditions used in the analysis of the existing foundation were obtained from the aforementioned EEI design report; project no. 11030 dated August 21, 2002. The base of the tower is connected to the foundation by means of (30) 2.25"  $\varnothing$ , ASTM A615-75 anchor bolts embedded approximately 7-ft into the concrete foundation structure.

- The tower base reactions developed from the governing Load Case 1 were used in the verification of the foundation and its anchors:

Location	Vector	Proposed Reactions
Base	Shear	45 kips
	Compression	82 kips
	Moment	5036 kip-ft

- The foundation was found to be within allowable limits.

Foundation	Design Limit	Proposed Loading	Result
Reinforced Concrete Caisson	Moment Capacity	61.9%	<b>PASS</b>
	Lateral Deflection	0.57 in. <sup>(1)</sup>	<b>PASS</b>

(1) Lateral deflection typically limited to 1.0 in. for monopole tower structures.

- The flange bolts and plate were found to be within allowable limits.

Tower Component	Design Limit	Stress Ratio (percentage of capacity)	Result
Flange Bolts	Tension	55.8%	PASS
Flange Plate	Bending	43.1%	PASS

- The anchor bolts and base plate were found to be within allowable limits.

Tower Component	Design Limit	Stress Ratio (percentage of capacity)	Result
Anchor Bolts	Combined Axial and Bending	49.5%	PASS
Base Plate	Bending	38.8%	PASS

### Conclusion

This analysis shows that the subject tower **is adequate** to support the proposed modified antenna configuration.

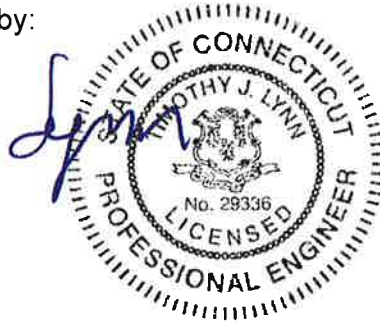
The analysis is based, in part, on the information provided to this office by Verizon Wireless. If the existing conditions are different than the information in this report, Centek Engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

Respectfully Submitted by:



Timothy J. Lynn, PE  
 Structural Engineer



Standard Conditions for Furnishing of  
Professional Engineering Services on  
Existing Structures

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of Centek Engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provided to Centek Engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the “as new” condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222
- All services performed, results obtained, and recommendations made are in accordance with generally accepted engineering principles and practices. Centek Engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

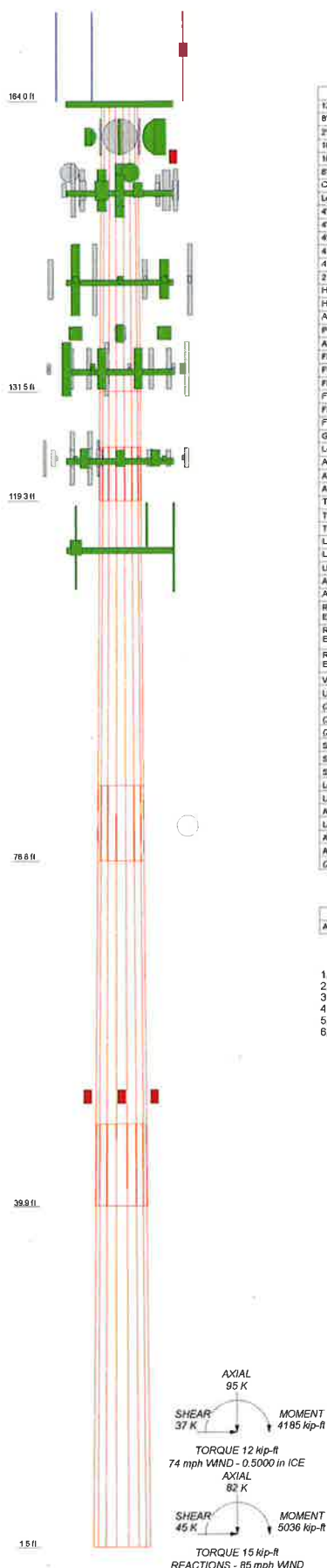
## GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM

tnxTower, is an integrated structural analysis and design software package for Designed specifically for the telecommunications industry, tnxTower, formerly ERITower, automates much of the tower analysis and design required by the TIA/EIA 222 Standard.

### tnxTower Features:

- tnxTower can analyze and design 3- and 4-sided guyed towers, 3- and 4-sided self-supporting towers and either round or tapered ground mounted poles with or without guys.
- The program analyzes towers using the TIA-222-G (2005) standard or any of the previous TIA/EIA standards back to RS-222 (1959). Steel design is checked using the AISC ASD 9th Edition or the AISC LRFD specifications.
- Linear and non-linear (P-delta) analyses can be used in determining displacements and forces in the structure. Wind pressures and forces are automatically calculated.
- Extensive graphics plots include material take-off, shear-moment, leg compression, displacement, twist, feed line, guy anchor and stress plots.
- tnxTower contains unique features such as True Cable behavior, hog rod take-up, foundation stiffness and much more.

Section	1	2	3	4	5
Length (ft)	32.50	12.21	48.50	47.33	47.63
Number of Sides	18	18	18	18	18
Thickness (in)	0.3750	0.3750	0.4375	0.5625	0.5625
Socket Length (ft)	6.00	6.00	8.42	9.25	9.25
Top Dia (in)	47.0000	53.4200	54.0685	60.4813	66.7412
Bot Dia (in)	53.4200	56.1500	62.9700	69.8900	76.0000
Grade	8.5	2.7	12.8	18.5	20.5
Weight (K)					20.5
					A572-65



DESIGNED APPURTENANCE LOADING

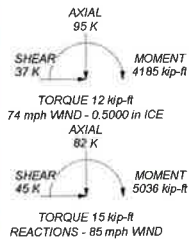
TYPE	ELEVATION	TYPE	ELEVATION
12" x 3" Dia Ones (Tower Existing)	164	(2) RRU5-11 (ATT Existing)	138
8" x 3" Dia Ones (Tower Existing)	164	(2) RRU5-11 (ATT Existing)	138
2x2 Panel (Tower Existing)	164	OCB-48-60-18-8F Surge Arrestor (ATT Existing)	138
10" x 3" Dia Ones (Tower Existing)	164	Valmont Uni-Tr Bracket (ATT Existing)	134
10" x 3" Dia Ones (Tower Existing)	164	P65-16-XLH-RR (ATT Existing)	134
8" x 3" Dia Ones (Tower Existing)	164	7770-00 (ATT Existing)	134
Camera (Tower Existing)	164	7770-00 (ATT Existing)	134
Low Profile Platform (Tower Existing)	160	P65-16-XLH-RR (ATT Existing)	134
4x4" Pipe Mount (Tower Existing)	160	(2) LQP21401 TMA (ATT Existing)	134
4x4" Pipe Mount (Tower Existing)	160	(2) LQP21401 TMA (ATT Existing)	134
4x4" Pipe Mount (Tower Existing)	160	(2) LQP21401 TMA (ATT Existing)	134
4 FT DISH (Tower Existing)	160	(2) LQP21901 Diplexer (ATT Existing)	134
4 FT DISH (Tower Existing)	160	(2) LQP21901 Diplexer (ATT Existing)	134
2 FT DISH (Tower Existing)	160	(2) LQP21901 Diplexer (ATT Existing)	134
Horizon ODU (Cleanwire Existing)	154	Low Profile Platform (ATT Existing)	134
Horizon ODU (Cleanwire Existing)	154	7770-00 (ATT Existing)	134
APXVSP18-C-A20 (Sprint Existing)	154	7770-00 (ATT Existing)	134
P40-16-XLPP-RR-A (Sprint Existing)	154	P65-16-XLH-RR (ATT Existing)	134
APXVSP18-C-A20 (Sprint Existing)	154	7770-00 (ATT Existing)	134
FD-RRH 4x45 1900 (Sprint Existing)	154	7770-00 (ATT Existing)	134
FD-RRH 4x45 1900 (Sprint Existing)	154	OB844655E-XY (Verizon Existing)	124
FD-RRH 4x45 1900 (Sprint Existing)	154	MG D3-8007X (Verizon Existing)	124
FD-RRH 2x50 800 (Sprint Existing)	154	SBN9H-1D65B (Verizon Proposed)	124
FD-RRH 2x50 800 (Sprint Existing)	154	SBN9H-1D65B (Verizon Proposed)	124
FD-RRH 2x50 800 (Sprint Existing)	154	OB844655E-XY (Verizon Existing)	124
GPS (Sprint Existing)	154	MG D3-8007X (Verizon Existing)	124
Low Profile Platform (Sprint Existing)	154	SBN9H-1D65B (Verizon Proposed)	124
APXVTM14 (Sprint Existing)	154	SBN9H-1D65B (Verizon Proposed)	124
APXVTM14 (Sprint Existing)	154	OB844655E-XY (Verizon Existing)	124
APXVTM14 (Sprint Existing)	154	OB844655E-XY (Verizon Existing)	124
TD-RRH20-25 (Sprint Existing)	154	(2) F09R6004-2C-3L Diplexer (Verizon Existing)	124
TD-RRH20-25 (Sprint Existing)	154	(2) F09R6004-2C-3L Diplexer (Verizon Existing)	124
TD-RRH20-25 (Sprint Existing)	154	(2) F09R6004-2C-3L Diplexer (Verizon Existing)	124
LLPX310R (Cleanwire Existing)	154	RRH4452-00-AWS (Verizon Proposed)	124
LLPX310R (Cleanwire Existing)	154	RRH4452-00-AWS (Verizon Proposed)	124
LLPX310R (Cleanwire Existing)	154	RRH4452-00-AWS (Verizon Proposed)	124
A-Ant 23G-2-C (Cleanwire Existing)	154	RRH4452-00-AWS (Verizon Proposed)	124
A-Ant 23G-2-C (Cleanwire Existing)	154	RRH4452-00-AWS (Verizon Proposed)	124
Remote Radio Head FD R6 RRRH (Cleanwire Existing)	151.5	RRH4452-00-AWS (Verizon Proposed)	124
Remote Radio Head FD R6 RRRH (Cleanwire Existing)	151.5	RRH4452-00-AWS (Verizon Proposed)	124
Remote Radio Head FD R6 RRRH (Cleanwire Existing)	151.5	RRH4452-00-AWS (Verizon Proposed)	124
Valmont Uni-Tr Bracket (Sprint Existing)	151.5	RRH4452-00-AWS (Verizon Proposed)	124
UXI-60110G (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
(3) TMA 10"x8"x3" (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
(3) TMA 10"x8"x3" (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
(3) TMA 10"x8"x3" (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
Smart Bias T (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
Smart Bias T (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
Smart Bias T (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
Low Profile Platform (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
UXI-65110G (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
APX18PW-18PVA-E (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
UXI-65110G (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
APX18PW-18PVA-E (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
APX18PW-18PVA-E (T-Mobile Existing)	144	RRH4452-00-AWS (Verizon Proposed)	124
(2) RRU5-11 (ATT Existing)	138	RRH4452-00-AWS (Verizon Proposed)	124

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
2. Tower is also designed for a 74 mph basic wind with 0.50 in ice.
3. Deflections are based upon a 50 mph wind.
4. Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
5. Welds are fabricated with ER-70S-6 electrodes.
6. TOWER RATING: 48.3%





<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15001.145 - Greenwich	<b>Page</b> 1 of 23
	<b>Project</b> 164' EEI Monopole - 5 Perryridge Rd., Greenwich, CT	<b>Date</b> 13:18:35 01/20/16
	<b>Client</b> Verizon Wireless	<b>Designed by</b> TJL

## Tower Input Data

There is a pole section.  
 This tower is designed using the TIA/EIA-222-F standard.  
 The following design criteria apply:  
 Basic wind speed of 85 mph.  
 Nominal ice thickness of 0.5000 in.  
 Ice density of 56 pcf.  
 A wind speed of 74 mph is used in combination with ice.  
 Temperature drop of 50 °F.  
 Deflections calculated using a wind speed of 50 mph.  
 Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards..  
 Welds are fabricated with ER-70S-6 electrodes..  
 A non-linear (P-delta) analysis was used.  
 Pressures are calculated at each section.  
 Stress ratio used in pole design is 1.333.  
 Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

- |  |   |   |
|--|---|---|
| <ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>Include Bolts In Member Capacity</li> <li>Leg Bolts Are At Top Of Section</li> <li>Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>Add IBC .6D+W Combination</li> </ul> | <ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>Use Clear Spans For Wind Area</li> <li>Use Clear Spans For KL/r</li> <li>Retension Guys To Initial Tension</li> <li>Bypass Mast Stability Checks</li> <li>Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>Autocalc Torque Arm Areas</li> <li>SR Members Have Cut Ends</li> <li>√ Sort Capacity Reports By Component</li> <li>Triangulate Diamond Inner Bracing</li> <li>Use TIA-222-G Tension Splice Capacity Exemption</li> </ul> | <ul style="list-style-type: none"> <li>Treat Feedline Bundles As Cylinder</li> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>SR Leg Bolts Resist Compression</li> <li>All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>√ Consider Feedline Torque</li> <li>Include Angle Block Shear Check</li> <li style="text-align: center;">Poles</li> <li>Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> </ul> |
|--|---|---|

## Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	164.00-131.50	32.50	0.00	18	47.0000	53.4200	0.3125	1.2500	A572-65 (65 ksi)
L2	131.50-119.29	12.21	6.00	18	53.4200	56.1500	0.3750	1.5000	A572-65 (65 ksi)
L3	119.29-78.79	46.50	8.42	18	54.0585	62.9700	0.4375	1.7500	A572-65 (65 ksi)
L4	78.79-39.88	47.33	9.25	18	60.4813	69.6600	0.5625	2.2500	A572-65 (65 ksi)
L5	39.88-1.50	47.63		18	66.7412	76.0000	0.5625	2.2500	A572-65

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	<b>Client</b> Verizon Wireless	<b>Designed by</b> TJJ

Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft		in	in	in	in	(65 ksi)

### Tapered Pole Properties

Section	Tip Dia.	Area	I	r	C	I/C	J	I/Q	w	w/t
	in	in <sup>2</sup>	in <sup>4</sup>	in	in	in <sup>3</sup>	in <sup>4</sup>	in <sup>2</sup>	in	
L1	47.7251	46.3082	12752.5270	16.5741	23.8760	534.1149	25521.8341	23.1585	7.7220	24.71
	54.2441	52.6760	18769.9004	18.8532	27.1374	691.6627	37564.4987	26.3430	8.8519	28.326
L2	54.2441	63.1368	22444.4518	18.8310	27.1374	827.0684	44918.4365	31.5744	8.7419	23.312
	57.0162	66.3862	26091.2194	19.8001	28.5242	914.7047	52216.7704	33.1994	9.2224	24.593
L3	56.0600	74.4594	27047.4669	19.0354	27.4617	984.9157	54130.5236	37.2368	8.7443	19.987
	63.9414	86.8342	42898.2727	22.1990	31.9888	1341.0421	85852.9920	43.4253	10.3127	23.572
L4	63.0724	106.9776	48524.0652	21.2712	30.7245	1579.3269	97111.9796	53.4990	9.6547	17.164
	70.7346	123.3649	74413.8720	24.5296	35.3873	2102.8424	148925.659	61.6942	11.2702	20.036
							7			
L5	69.5966	118.1537	65376.3617	23.4934	33.9045	1928.2498	130838.747	59.0881	10.7564	19.123
	77.1724	134.6842	96834.1984	26.7803	38.6080	2508.1382	193795.813	67.3549	12.3860	22.02
							7			

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A <sub>f</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft <sup>2</sup>	in					in	in
L1				1	1	1		
164.00-131.50				1	1	1		
L2				1	1	1		
131.50-119.29				1	1	1		
L3				1	1	1		
119.29-78.79				1	1	1		
L4				1	1	1		
78.79-39.88				1	1	1		
L5				1	1	1		
39.88-1.50				1	1	1		

### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement	Total Number	C <sub>AA</sub>	Weight
				ft		ft <sup>2</sup> /ft	plf
1/2 (Town Existing)	A	No	Inside Pole	164.00 - 4.50	6	No Ice 1/2" Ice	0.00 0.25
5/8 (Town Existing)	A	No	Inside Pole	164.00 - 4.50	1	No Ice 1/2" Ice	0.00 0.40
7/8 (Town Existing)	A	No	Inside Pole	164.00 - 4.50	3	No Ice 1/2" Ice	0.00 0.54
1 1/4 (Town Existing)	A	No	Inside Pole	164.00 - 4.50	5	No Ice 1/2" Ice	0.00 0.66
1/2 (Sprint Existing)	B	No	Inside Pole	154.00 - 7.50	1	No Ice 1/2" Ice	0.00 0.25
2" Rigid Conduit (Clearwire Existing)	B	No	Inside Pole	154.00 - 7.50	2	No Ice 1/2" Ice	0.00 2.80
LDF4.5-50 (5/8 FOAM)	B	No	Inside Pole	154.00 - 7.50	2	No Ice	0.00 0.15

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Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number		$C_{AA}$	Weight
							$ft^2/ft$	$plf$
(Clearwire Existing)						1/2" Ice	0.00	0.15
1 5/8	B	No	Inside Pole	144.00 - 4.50	12	No Ice	0.00	1.04
(T-Mobile Existing)						1/2" Ice	0.00	1.04
1 5/8	A	No	Inside Pole	134.00 - 11.50	12	No Ice	0.00	1.04
(AT&T Existing)						1/2" Ice	0.00	1.04
1 5/8	C	No	Inside Pole	124.00 - 7.50	6	No Ice	0.00	1.04
(Verizon Existing)						1/2" Ice	0.00	1.04
RG6-Fiber	A	No	Inside Pole	134.00 - 11.50	1	No Ice	0.00	0.00
(AT&T Existing)						1/2" Ice	0.00	0.00
#8 AWG Copper Wire	A	No	Inside Pole	134.00 - 11.50	2	No Ice	0.00	0.00
(AT&T Existing)						1/2" Ice	0.00	0.00
7/8	B	No	CaAa (Out Of Face)	51.50 - 4.50	3	No Ice	0.11	0.54
						1/2" Ice	0.21	1.52
HYBRIFLEX 1-5/8"	B	No	Inside Pole	154.00 - 7.50	6	No Ice	0.00	1.90
(Sprint Existing)						1/2" Ice	0.00	1.90
HYBRIFLEX 1-5/8"	C	No	Inside Pole	124.00 - 7.50	1	No Ice	0.00	1.90
(Verizon Existing)						1/2" Ice	0.00	1.90
7/8	C	No	Inside Pole	114.00 - 1.50	2	No Ice	0.00	0.54
(Eversource Existing)						1/2" Ice	0.00	0.54
1 5/8	C	No	Inside Pole	114.00 - 1.50	2	No Ice	0.00	1.04
(Eversource Existing)						1/2" Ice	0.00	1.04
1/2	C	No	Inside Pole	114.00 - 1.50	1	No Ice	0.00	0.25
(Eversource Existing)						1/2" Ice	0.00	0.25
1 5/8	B	No	CaAa (Out Of Face)	144.00 - 7.50	1	No Ice	0.20	1.04
(T-Mobile - Existing)						1/2" Ice	0.30	2.55
1 5/8	B	No	CaAa (Out Of Face)	144.00 - 7.50	5	No Ice	0.00	1.04
(T-Mobile - Existing)						1/2" Ice	0.00	2.55
HYBRIFLEX 1-5/8"	C	No	Inside Pole	124.00 - 7.50	1	No Ice	0.00	1.90
(Verizon Existing)						1/2" Ice	0.00	1.90

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	$A_R$ $ft^2$	$A_F$ $ft^2$	$C_{AA}$ In Face $ft^2$	$C_{AA}$ Out Face $ft^2$	Weight K
L1	164.00-131.50	A	0.000	0.000	0.000	0.000	0.25
		B	0.000	0.000	0.000	2.475	0.63
		C	0.000	0.000	0.000	0.000	0.00
L2	131.50-119.29	A	0.000	0.000	0.000	0.000	0.24
		B	0.000	0.000	0.000	2.418	0.44
		C	0.000	0.000	0.000	0.000	0.05
L3	119.29-78.79	A	0.000	0.000	0.000	0.000	0.78
		B	0.000	0.000	0.000	8.019	1.47
		C	0.000	0.000	0.000	0.000	0.53
L4	78.79-39.88	A	0.000	0.000	0.000	0.000	0.75
		B	0.000	0.000	0.000	11.574	1.43
		C	0.000	0.000	0.000	0.000	0.52
L5	39.88-1.50	A	0.000	0.000	0.000	0.000	0.60
		B	0.000	0.000	0.000	18.193	1.27
		C	0.000	0.000	0.000	0.000	0.46

### Feed Line/Linear Appurtenances Section Areas - With Ice

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Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
L1	164.00-131.50	A	0.500	0.000	0.000	0.000	0.000	0.25
		B		0.000	0.000	0.000	3.725	0.74
		C		0.000	0.000	0.000	0.000	0.00
L2	131.50-119.29	A	0.500	0.000	0.000	0.000	0.000	0.24
		B		0.000	0.000	0.000	3.639	0.55
		C		0.000	0.000	0.000	0.000	0.05
L3	119.29-78.79	A	0.500	0.000	0.000	0.000	0.000	0.78
		B		0.000	0.000	0.000	12.069	1.84
		C		0.000	0.000	0.000	0.000	0.53
L4	78.79-39.88	A	0.500	0.000	0.000	0.000	0.000	0.75
		B		0.000	0.000	0.000	18.950	1.82
		C		0.000	0.000	0.000	0.000	0.52
L5	39.88-1.50	A	0.500	0.000	0.000	0.000	0.000	0.60
		B		0.000	0.000	0.000	32.044	1.67
		C		0.000	0.000	0.000	0.000	0.46

### Feed Line Center of Pressure

Section	Elevation ft	CP <sub>X</sub> in	CP <sub>Z</sub> in	CP <sub>X</sub> Ice in	CP <sub>Z</sub> Ice in
L1	164.00-131.50	0.1010	0.0583	0.1478	0.0853
L2	131.50-119.29	0.2465	0.1423	0.3573	0.2063
L3	119.29-78.79	0.2473	0.1428	0.3593	0.2074
L4	78.79-39.88	0.3715	0.2145	0.5821	0.3361
L5	39.88-1.50	0.5677	0.3278	0.9362	0.5405

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight K	
4'x4" Pipe Mount (Town Existing)	A	From Face	0.50	0.0000	160.00	No Ice	1.32	1.32	0.04
			0.00			1/2" Ice	1.58	1.58	0.06
			0.00						
4'x4" Pipe Mount (Town Existing)	B	From Face	0.50	0.0000	160.00	No Ice	1.32	1.32	0.04
			0.00			1/2" Ice	1.58	1.58	0.06
			0.00						
4'x4" Pipe Mount (Town Existing)	C	From Face	0.50	0.0000	160.00	No Ice	1.32	1.32	0.04
			0.00			1/2" Ice	1.58	1.58	0.06
			0.00						
12' x 3" Dia Omni (Town Existing)	A	From Face	4.00	0.0000	164.00	No Ice	3.60	3.60	0.04
			-4.00			1/2" Ice	4.83	4.83	0.06
			5.00						
8' x 3" Dia Omni (Town Existing)	A	From Face	4.00	0.0000	164.00	No Ice	2.40	2.40	0.03
			4.00			1/2" Ice	3.19	3.19	0.04
			5.00						
2'x2' Panel	B	From Face	4.00	0.0000	164.00	No Ice	5.60	0.72	0.02

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight
			Horz	Vert			Front	Side	
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
(Town Existing)			4.00			1/2" Ice	5.92	0.88	0.05
			5.00						
10' x 3" Dia Omni (Town Existing)	B	From Face	4.00		0.0000	164.00	No Ice	3.00	0.03
			4.00			1/2" Ice	4.03	4.03	0.05
			5.00						
10' x 3" Dia Omni (Town Existing)	A	From Face	4.00		0.0000	164.00	No Ice	3.00	0.03
			-4.00			1/2" Ice	4.03	4.03	0.05
			5.00						
8' x 3" Dia Omni (Town Existing)	A	From Face	4.00		0.0000	164.00	No Ice	2.40	0.03
			4.00			1/2" Ice	3.19	3.19	0.04
			5.00						
Camera (Town Existing)	C	From Face	4.00		0.0000	164.00	No Ice	3.00	0.10
			-4.00			1/2" Ice	4.00	4.00	0.15
			2.00						
Low Profile Platform (Town Existing)	C	None			0.0000	164.00	No Ice	15.70	1.30
						1/2" Ice	20.10	20.10	1.76
LLPX310R (Clearwire Existing)	A	From Face	3.00		0.0000	154.00	No Ice	4.83	0.03
			0.00			1/2" Ice	5.18	2.21	0.05
			0.00						
LLPX310R (Clearwire Existing)	B	From Face	3.00		0.0000	154.00	No Ice	4.83	0.03
			0.00			1/2" Ice	5.18	2.21	0.05
			0.00						
LLPX310R (Clearwire Existing)	C	From Face	3.00		0.0000	154.00	No Ice	4.83	0.03
			0.00			1/2" Ice	5.18	2.21	0.05
			0.00						
Remote Radio Head FD R6 RRH (Clearwire Existing)	A	From Face	3.00		0.0000	151.50	No Ice	1.80	0.03
			0.00			1/2" Ice	1.99	0.92	0.04
			0.00						
Remote Radio Head FD R6 RRH (Clearwire Existing)	B	From Face	3.00		0.0000	151.50	No Ice	1.80	0.03
			0.00			1/2" Ice	1.99	0.92	0.04
			0.00						
Remote Radio Head FD R6 RRH (Clearwire Existing)	C	From Face	3.00		0.0000	151.50	No Ice	1.80	0.03
			0.00			1/2" Ice	1.99	0.92	0.04
			0.00						
Horizon ODU (Clearwire Existing)	A	None			0.0000	154.00	No Ice	0.79	0.00
						1/2" Ice	0.91	0.25	0.00
Horizon ODU (Clearwire Existing)	C	None			0.0000	154.00	No Ice	0.79	0.00
						1/2" Ice	0.91	0.25	0.00
APXVSP18-C-A20 (Sprint Existing)	A	From Face	4.00		0.0000	154.00	No Ice	8.26	0.06
			0.00			1/2" Ice	8.81	5.74	0.11
			0.00						
P40-16-XLPP-RR-A (Sprint Existing)	B	From Face	4.00		0.0000	154.00	No Ice	10.50	0.05
			0.00			1/2" Ice	10.98	3.87	0.11
			0.00						
APXVSP18-C-A20 (Sprint Existing)	C	From Face	4.00		0.0000	154.00	No Ice	8.26	0.06
			0.00			1/2" Ice	8.81	5.74	0.11
			0.00						
FD-RRH 4x45 1900 (Sprint Existing)	A	From Face	4.00		0.0000	154.00	No Ice	2.71	0.06
			2.00			1/2" Ice	2.94	3.02	0.08
			0.00						
FD-RRH 4x45 1900 (Sprint Existing)	B	From Face	4.00		0.0000	154.00	No Ice	2.71	0.06
			2.00			1/2" Ice	2.94	3.02	0.08
			0.00						
FD-RRH 4x45 1900 (Sprint Existing)	C	From Face	4.00		0.0000	154.00	No Ice	2.71	0.06
			2.00			1/2" Ice	2.94	3.02	0.08
			0.00						
FD-RRH 2x50 800	A	From Face	4.00		0.0000	154.00	No Ice	2.40	0.06

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz	Vert					
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
(Sprint Existing)			-2.00			1/2" Ice	2.61	2.46	0.09
			0.00						
FD-RRH 2x50 800	B	From Face	4.00		0.0000	154.00	No Ice	2.40	2.25
(Sprint Existing)			-2.00				1/2" Ice	2.61	2.46
			0.00						
FD-RRH 2x50 800	C	From Face	4.00		0.0000	154.00	No Ice	2.40	2.25
(Sprint Existing)			-2.00				1/2" Ice	2.61	2.46
			0.00						
Valmont Uni-Tri Bracket	A	None			0.0000	151.50	No Ice	1.75	1.75
(Sprint Existing)							1/2" Ice	1.94	1.94
GPS	C	From Face	4.00		0.0000	154.00	No Ice	1.00	1.00
(Sprint Existing)			-6.00				1/2" Ice	1.50	1.50
			3.00						
Low Profile Platform	C	None			0.0000	154.00	No Ice	15.70	15.70
(Sprint Existing)							1/2" Ice	20.10	20.10
APXVTM14	A	From Face	4.00		0.0000	154.00	No Ice	6.90	3.61
(Sprint Existing)			2.00				1/2" Ice	7.35	3.97
			0.00						
APXVTM14	B	From Face	4.00		0.0000	154.00	No Ice	6.90	3.61
(Sprint Existing)			2.00				1/2" Ice	7.35	3.97
			0.00						
APXVTM14	C	From Face	4.00		0.0000	154.00	No Ice	6.90	3.61
(Sprint Existing)			2.00				1/2" Ice	7.35	3.97
			0.00						
TD-RRH8x20-25	A	From Face	4.00		0.0000	154.00	No Ice	4.72	1.70
(Sprint Existing)			2.00				1/2" Ice	5.01	1.92
			0.00						
TD-RRH8x20-25	B	From Face	4.00		0.0000	154.00	No Ice	4.72	1.70
(Sprint Existing)			2.00				1/2" Ice	5.01	1.92
			0.00						
TD-RRH8x20-25	C	From Face	4.00		0.0000	154.00	No Ice	4.72	1.70
(Sprint Existing)			2.00				1/2" Ice	5.01	1.92
			0.00						
APX16PV-16PVL-E	A	From Face	4.00		0.0000	144.00	No Ice	6.65	1.98
(T-Mobile Existing)			-5.00				1/2" Ice	7.08	2.30
			0.00						
LNX-6515DS	A	From Face	4.00		0.0000	144.00	No Ice	11.45	7.70
(T-Mobile Existing)			5.00				1/2" Ice	12.06	8.29
			0.00						
APX16PV-16PVL-E	B	From Face	4.00		0.0000	144.00	No Ice	6.65	1.98
(T-Mobile Existing)			-5.00				1/2" Ice	7.08	2.30
			0.00						
LNX-6515DS	B	From Face	4.00		0.0000	144.00	No Ice	11.45	7.70
(T-Mobile Existing)			5.00				1/2" Ice	12.06	8.29
			0.00						
APX16PV-16PVL-E	C	From Face	4.00		0.0000	144.00	No Ice	6.65	1.98
(T-Mobile Existing)			-5.00				1/2" Ice	7.08	2.30
			0.00						
LNX-6515DS	C	From Face	4.00		0.0000	144.00	No Ice	11.45	7.70
(T-Mobile Existing)			5.00				1/2" Ice	12.06	8.29
			0.00						
(3) TMA 10"x8"x3"	A	From Face	4.00		0.0000	144.00	No Ice	0.78	0.29
(T-Mobile Existing)			0.00				1/2" Ice	0.90	0.38
			0.00						
(3) TMA 10"x8"x3"	B	From Face	4.00		0.0000	144.00	No Ice	0.78	0.29
(T-Mobile Existing)			0.00				1/2" Ice	0.90	0.38
			0.00						

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	15001.145 - Greenwich	<b>Page</b>	7 of 23
	<b>Project</b>	164' EEI Monopole - 5 Perryridge Rd., Greenwich, CT	<b>Date</b>	13:18:35 01/20/16
	<b>Client</b>	Verizon Wireless	<b>Designed by</b>	TJL

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight	
			Horz	Vert			Front	Side		
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
(3) TMA 10"x8"x3" (T-Mobile Existing)	C	From Face	4.00 0.00 0.00		0.0000	144.00	No Ice 1/2" Ice	0.78 0.90	0.29 0.38	0.02 0.02
Smart Bias T (T-Mobile Existing)	A	From Face	4.00 0.00 0.00		0.0000	144.00	No Ice 1/2" Ice	0.16 0.21	0.08 0.12	0.00 0.00
Smart Bias T (T-Mobile Existing)	B	From Face	4.00 0.00 0.00		0.0000	144.00	No Ice 1/2" Ice	0.16 0.21	0.08 0.12	0.00 0.00
Smart Bias T (T-Mobile Existing)	C	From Face	4.00 0.00 0.00		0.0000	144.00	No Ice 1/2" Ice	0.16 0.21	0.08 0.12	0.00 0.00
Low Profile Platform (T-Mobile Existing)	C	None			0.0000	144.00	No Ice 1/2" Ice	15.70 20.10	15.70 20.10	1.30 1.76
(2) RRUS-11 (AT&T Existing)	A	From Face	0.50 0.00 0.00		0.0000	138.00	No Ice 1/2" Ice	2.99 3.23	1.25 1.41	0.05 0.07
(2) RRUS-11 (AT&T Existing)	B	From Face	0.50 0.00 0.00		0.0000	138.00	No Ice 1/2" Ice	2.99 3.23	1.25 1.41	0.05 0.07
(2) RRUS-11 (AT&T Existing)	C	From Face	0.50 0.00 0.00		0.0000	138.00	No Ice 1/2" Ice	2.99 3.23	1.25 1.41	0.05 0.07
DC6-48-60-18-8F Surge Arrestor (AT&T Existing)	C	From Face	0.50 0.00 0.00		0.0000	138.00	No Ice 1/2" Ice	2.23 2.45	2.23 2.45	0.02 0.04
Valmont Uni-Tri Bracket (AT&T Existing)	C	None			0.0000	138.00	No Ice 1/2" Ice	1.75 1.94	1.75 1.94	0.29 0.31
7770.00 (AT&T Existing)	A	From Face	3.00 -2.00 0.00		0.0000	134.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
7770.00 (AT&T Existing)	A	From Face	3.00 2.00 0.00		0.0000	134.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
P65-16-XLH-RR (AT&T Existing)	A	From Face	3.00 6.00 0.00		0.0000	134.00	No Ice 1/2" Ice	8.40 8.95	4.70 5.15	0.06 0.11
7770.00 (AT&T Existing)	B	From Face	3.00 -2.00 0.00		0.0000	134.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
7770.00 (AT&T Existing)	B	From Face	3.00 2.00 0.00		0.0000	134.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
P65-16-XLH-RR (AT&T Existing)	B	From Face	3.00 6.00 0.00		0.0000	134.00	No Ice 1/2" Ice	8.40 8.95	4.70 5.15	0.06 0.11
7770.00 (AT&T Existing)	C	From Face	3.00 2.00 0.00		0.0000	134.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
7770.00 (AT&T Existing)	C	From Face	3.00 -2.00 0.00		0.0000	134.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
P65-16-XLH-RR (AT&T Existing)	C	From Face	3.00 6.00 0.00		0.0000	134.00	No Ice 1/2" Ice	8.40 8.95	4.70 5.15	0.06 0.11
(2) LGP21401 TMA (AT&T Existing)	A	From Face	3.00 -2.00		0.0000	134.00	No Ice 1/2" Ice	0.95 1.09	0.37 0.48	0.02 0.02

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	<b>Project</b>	164' EEI Monopole - 5 Perryridge Rd., Greenwich, CT	<b>Date</b>	13:18:35 01/20/16
	<b>Client</b>	Verizon Wireless	<b>Designed by</b>	TJL

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight	
			Horz	Vert			Front	Side		
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
(2) LGP21401 TMA (AT&T Existing)	B	From Face	0.00		0.0000	134.00	No Ice	0.95	0.37	0.02
			3.00				1/2" Ice	1.09	0.48	0.02
(2) LGP21401 TMA (AT&T Existing)	C	From Face	0.00		0.0000	134.00	No Ice	0.95	0.37	0.02
			3.00				1/2" Ice	1.09	0.48	0.02
(2) LGP21901 Diplexer (AT&T Existing)	A	From Face	0.00		0.0000	134.00	No Ice	0.23	0.12	0.01
			3.00				1/2" Ice	0.30	0.17	0.01
(2) LGP21901 Diplexer (AT&T Existing)	B	From Face	0.00		0.0000	134.00	No Ice	0.23	0.12	0.01
			3.00				1/2" Ice	0.30	0.17	0.01
(2) LGP21901 Diplexer (AT&T Existing)	C	From Face	0.00		0.0000	134.00	No Ice	0.23	0.12	0.01
			3.00				1/2" Ice	0.30	0.17	0.01
Low Profile Platform (AT&T Existing)	C	None	0.00		0.0000	134.00	No Ice	15.70	15.70	1.30
			4.00				1/2" Ice	20.10	20.10	1.76
DB844H65E-XY (Verizon Existing)	A	From Face	0.00		0.0000	124.00	No Ice	2.87	4.20	0.01
			4.00				1/2" Ice	3.18	4.57	0.04
MG D3-800TX (Verizon Existing)	A	From Face	0.00		0.0000	124.00	No Ice	3.45	2.22	0.00
			4.00				1/2" Ice	3.80	2.55	0.02
SBNHH-1D65B (Verizon Proposed)	A	From Face	0.00		0.0000	124.00	No Ice	8.33	5.34	0.04
			4.00				1/2" Ice	8.88	5.79	0.09
SBNHH-1D65B (Verizon Proposed)	A	From Face	0.00		0.0000	124.00	No Ice	8.33	5.34	0.04
			4.00				1/2" Ice	8.88	5.79	0.09
DB844H65E-XY (Verizon Existing)	A	From Face	0.00		0.0000	124.00	No Ice	2.87	4.20	0.01
			4.00				1/2" Ice	3.18	4.57	0.04
DB844H65E-XY (Verizon Existing)	A	From Face	0.00		0.0000	124.00	No Ice	2.87	4.20	0.01
			4.00				1/2" Ice	3.18	4.57	0.04
MG D3-800TX (Verizon Existing)	A	From Face	0.00		0.0000	124.00	No Ice	3.45	2.22	0.00
			4.00				1/2" Ice	3.80	2.55	0.02
SBNHH-1D65B (Verizon Proposed)	A	From Face	0.00		0.0000	124.00	No Ice	8.33	5.34	0.04
			4.00				1/2" Ice	8.88	5.79	0.09
SBNHH-1D65B (Verizon Proposed)	A	From Face	0.00		0.0000	124.00	No Ice	8.33	5.34	0.04
			4.00				1/2" Ice	8.88	5.79	0.09
DB844H65E-XY (Verizon Existing)	A	From Face	0.00		0.0000	124.00	No Ice	2.87	4.20	0.01
			4.00				1/2" Ice	3.18	4.57	0.04
DB844H65E-XY (Verizon Existing)	A	From Face	0.00		0.0000	124.00	No Ice	2.87	4.20	0.01
			4.00				1/2" Ice	3.18	4.57	0.04
MG D3-800TX (Verizon Existing)	A	From Face	0.00		0.0000	124.00	No Ice	3.45	2.22	0.00
			4.00				1/2" Ice	3.80	2.55	0.02
SBNHH-1D65B (Verizon Proposed)	A	From Face	0.00		0.0000	124.00	No Ice	8.33	5.34	0.04
			4.00				1/2" Ice	8.88	5.79	0.09



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	<b>Project</b>	164' EEI Monopole - 5 Perryridge Rd., Greenwich, CT	<b>Date</b>	13:18:35 01/20/16
	<b>Client</b>	Verizon Wireless	<b>Designed by</b>	TJL

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight	
			Horz	Vert			Front	Side		
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
SBNHH-1D65B (Verizon Proposed)	A	From Face	4.00 4.00 0.00		0.0000	124.00	No Ice 1/2" Ice	8.33 8.88	5.34 5.79	0.04 0.09
DB844H65E-XY (Verizon Existing)	A	From Face	4.00 6.00 0.00		0.0000	124.00	No Ice 1/2" Ice	2.87 3.18	4.20 4.57	0.01 0.04
(2) FD9R6004/2C-3L Diplexer (Verizon Existing)	B	From Face	3.00 0.00 0.00		0.0000	124.00	No Ice 1/2" Ice	0.37 0.45	0.08 0.14	0.00 0.01
(2) FD9R6004/2C-3L Diplexer (Verizon Existing)	C	From Face	3.00 0.00 0.00		0.0000	124.00	No Ice 1/2" Ice	0.37 0.45	0.08 0.14	0.00 0.01
(2) FD9R6004/2C-3L Diplexer (Verizon Existing)	A	From Face	3.00 0.00 0.00		0.0000	124.00	No Ice 1/2" Ice	0.37 0.45	0.08 0.14	0.00 0.01
RRH4x45/2x90-AWS (Verizon Proposed)	A	From Face	4.00 4.00 0.00		0.0000	124.00	No Ice 1/2" Ice	3.01 3.26	1.91 2.13	0.08 0.10
RRH4x45/2x90-AWS (Verizon Proposed)	B	From Face	4.00 4.00 0.00		0.0000	124.00	No Ice 1/2" Ice	3.01 3.26	1.91 2.13	0.08 0.10
RRH4x45/2x90-AWS (Verizon Proposed)	C	From Face	4.00 4.00 0.00		0.0000	124.00	No Ice 1/2" Ice	3.01 3.26	1.91 2.13	0.08 0.10
RRH4x30-B13 (Verizon Proposed)	A	From Face	4.00 0.00 0.00		0.0000	124.00	No Ice 1/2" Ice	2.52 2.74	1.89 2.09	0.06 0.08
RRH4x30-B13 (Verizon Proposed)	B	From Face	4.00 0.00 0.00		0.0000	124.00	No Ice 1/2" Ice	2.52 2.74	1.89 2.09	0.06 0.08
RRH4x30-B13 (Verizon Proposed)	C	From Face	4.00 0.00 0.00		0.0000	124.00	No Ice 1/2" Ice	2.52 2.74	1.89 2.09	0.06 0.08
RRH2x60-PCS (Verizon Proposed)	A	From Face	4.00 -4.00 0.00		0.0000	124.00	No Ice 1/2" Ice	2.51 2.73	1.55 1.74	0.06 0.07
RRH2x60-PCS (Verizon Proposed)	B	From Face	4.00 -4.00 0.00		0.0000	124.00	No Ice 1/2" Ice	2.51 2.73	1.55 1.74	0.06 0.07
RRH2x60-PCS (Verizon Proposed)	C	From Face	4.00 -4.00 0.00		0.0000	124.00	No Ice 1/2" Ice	2.51 2.73	1.55 1.74	0.06 0.07
RC2DC-3315-PF-48 (Verizon Existing)	A	From Face	1.00 1.00 0.00		0.0000	124.00	No Ice 1/2" Ice	3.52 3.77	2.29 2.51	0.03 0.05
RC2DC-3315-PF-48 (Verizon Proposed)	B	From Face	1.00 1.00 0.00		0.0000	124.00	No Ice 1/2" Ice	3.52 3.77	2.29 2.51	0.03 0.05
Low Profile Platform (Verizon Existing)	C	None			0.0000	124.00	No Ice 1/2" Ice	15.70 20.10	15.70 20.10	1.30 1.76
Low Profile Platform	C	None			0.0000	114.00	No Ice 1/2" Ice	15.70 20.10	15.70 20.10	1.30 1.76
GPS	A	From Face	1.50 0.00 0.00		0.0000	51.50	No Ice 1/2" Ice	1.00 1.50	1.00 1.50	0.01 0.01
GPS	B	From Face	1.50 0.00		0.0000	51.50	No Ice 1/2" Ice	1.00 1.50	1.00 1.50	0.01 0.01

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	<b>Client</b> Verizon Wireless	<b>Designed by</b> T.J.L.

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz Lateral	Vert						ft
GPS	C	From Face	0.00		0.0000	51.50	No Ice	1.00	1.00	0.01
			1.50				1/2" Ice	1.50	1.50	0.01
			0.00							
531-70HD (Eversource Existing)	C	From Face	3.00		0.0000	114.00	No Ice	6.00	6.00	0.04
			-6.00				1/2" Ice	6.90	6.90	0.05
			0.00							
DB586-Y (Eversource Existing)	C	From Face	3.00		0.0000	114.00	No Ice	1.01	1.01	0.01
			5.00				1/2" Ice	1.28	1.28	0.02
			2.50							
DB586-Y (Eversource Existing)	C	From Face	3.00		0.0000	114.00	No Ice	1.01	1.01	0.01
			5.00				1/2" Ice	1.28	1.28	0.02
			-2.50							
ANT150F2 (Eversource Existing)	C	From Face	3.00		0.0000	114.00	No Ice	1.29	1.29	0.02
			-3.00				1/2" Ice	1.60	1.60	0.03
			2.50							
Tower Top Amplifier (Eversource Existing)	C	From Face	3.00		0.0000	114.00	No Ice	3.11	1.17	0.04
			5.00				1/2" Ice	3.35	1.34	0.06
			0.00							

### Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets:		Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weight	
				Horz Lateral	Vert							ft
4 FT DISH (Town Existing)	A	Paraboloid w/Shroud (HP)	From Leg	1.00		Worst		160.00	4.00	No Ice	12.56	0.17
				0.00						1/2" Ice	13.09	0.24
				0.00								
4 FT DISH (Town Existing)	B	Paraboloid w/Shroud (HP)	From Leg	1.00		Worst		160.00	4.00	No Ice	12.56	0.17
				0.00						1/2" Ice	13.09	0.24
				0.00								
2 FT DISH (Town Existing)	C	Paraboloid w/Shroud (HP)	From Leg	1.00		Worst		160.00	2.00	No Ice	3.14	0.03
				0.00						1/2" Ice	3.41	0.04
				0.00								
A-Ant-23G-2-C (Clearwire Existing)	A	Paraboloid w/Radome	From Face	3.10		Worst		154.00	2.17	No Ice	3.72	0.03
				-2.52						1/2" Ice	4.01	0.05
				2.00								
A-Ant-23G-2-C (Clearwire Existing)	C	Paraboloid w/Radome	From Face	3.80		Worst		154.00	2.17	No Ice	3.72	0.03
				-1.24						1/2" Ice	4.01	0.05
				2.00								

### Tower Pressures - No Ice

$$G_H = 1.690$$

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	<b>Client</b>	Verizon Wireless		<b>Designed by</b>	TJL

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	A <sub>G</sub>	F <sub>a</sub>	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face
ft	ft		psf	ft <sup>2</sup>	c	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
L1 164.00-131.50	147.53	1.534	28	135.985	A	0.000	135.985	135.985	100.00	0.000	0.000
					B	0.000	135.985	100.00	0.000	2.475	
					C	0.000	135.985	100.00	0.000	0.000	
L2 131.50-119.29	125.34	1.464	27	55.744	A	0.000	55.744	55.744	100.00	0.000	0.000
					B	0.000	55.744	100.00	0.000	2.418	
					C	0.000	55.744	100.00	0.000	0.000	
L3 119.29-78.79	98.89	1.368	25	199.426	A	0.000	199.426	199.426	100.00	0.000	0.000
					B	0.000	199.426	100.00	0.000	8.019	
					C	0.000	199.426	100.00	0.000	0.000	
L4 78.79-39.88	59.42	1.183	22	213.639	A	0.000	213.639	213.639	100.00	0.000	0.000
					B	0.000	213.639	100.00	0.000	11.574	
					C	0.000	213.639	100.00	0.000	0.000	
L5 39.88-1.50	20.36	1	18	231.142	A	0.000	231.142	231.142	100.00	0.000	0.000
					B	0.000	231.142	100.00	0.000	18.193	
					C	0.000	231.142	100.00	0.000	0.000	

### Tower Pressure - With Ice

$G_H = 1.690$

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	t <sub>z</sub>	A <sub>G</sub>	F <sub>a</sub>	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face
ft	ft		psf	in	ft <sup>2</sup>	c	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
L1 164.00-131.50	147.53	1.534	21	0.5000	138.694	A	0.000	138.694	138.694	100.00	0.000	0.000
						B	0.000	138.694	100.00	0.000	3.725	
						C	0.000	138.694	100.00	0.000	0.000	
L2 131.50-119.29	125.34	1.464	20	0.5000	56.761	A	0.000	56.761	56.761	100.00	0.000	0.000
						B	0.000	56.761	100.00	0.000	3.639	
						C	0.000	56.761	100.00	0.000	0.000	
L3 119.29-78.79	98.89	1.368	19	0.5000	202.801	A	0.000	202.801	202.801	100.00	0.000	0.000
						B	0.000	202.801	100.00	0.000	12.069	
						C	0.000	202.801	100.00	0.000	0.000	
L4 78.79-39.88	59.42	1.183	16	0.5000	216.881	A	0.000	216.881	216.881	100.00	0.000	0.000
						B	0.000	216.881	100.00	0.000	18.950	
						C	0.000	216.881	100.00	0.000	0.000	
L5 39.88-1.50	20.36	1	14	0.5000	234.341	A	0.000	234.341	234.341	100.00	0.000	0.000
						B	0.000	234.341	100.00	0.000	32.044	
						C	0.000	234.341	100.00	0.000	0.000	

### Tower Pressure - Service

$G_H = 1.690$

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	A <sub>G</sub>	F <sub>a</sub>	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face
ft	ft		psf	ft <sup>2</sup>	c	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
L1 164.00-131.50	147.53	1.534	10	135.985	A	0.000	135.985	135.985	100.00	0.000	0.000
					B	0.000	135.985	100.00	0.000	2.475	
					C	0.000	135.985	100.00	0.000	0.000	
L2	125.34	1.464	9	55.744	A	0.000	55.744	55.744	100.00	0.000	0.000

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Section Elevation ft	z ft	K <sub>Z</sub>	q <sub>z</sub> psf	A <sub>G</sub> ft <sup>2</sup>	F <sub>a c e</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	A <sub>R</sub> ft <sup>2</sup>	A <sub>leg</sub> ft <sup>2</sup>	Leg %	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>
131.50-119.29					B	0.000	55.744		100.00	0.000	2.418
					C	0.000	55.744		100.00	0.000	0.000
L3 119.29-78.79	98.89	1.368	9	199.426	A	0.000	199.426	199.426	100.00	0.000	0.000
					B	0.000	199.426		100.00	0.000	8.019
					C	0.000	199.426		100.00	0.000	0.000
L4 78.79-39.88	59.42	1.183	8	213.639	A	0.000	213.639	213.639	100.00	0.000	0.000
					B	0.000	213.639		100.00	0.000	11.574
					C	0.000	213.639		100.00	0.000	0.000
L5 39.88-1.50	20.36	1	6	231.142	A	0.000	231.142	231.142	100.00	0.000	0.000
					B	0.000	231.142		100.00	0.000	18.193
					C	0.000	231.142		100.00	0.000	0.000

### Tower Forces - No Ice - Wind Normal To Face

Section Elevation ft	Add Weight K	Self Weight K	F <sub>a c e</sub> e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
L1 164.00-131.50	0.88	5.47	A	1	0.65	1	1	1	135.985	4.35	133.99	C
			B	1	0.65	1	1	1	135.985			
			C	1	0.65	1	1	1	135.985			
L2 131.50-119.29	0.73	2.69	A	1	0.65	1	1	1	55.744	1.77	144.88	C
			B	1	0.65	1	1	1	55.744			
			C	1	0.65	1	1	1	55.744			
L3 119.29-78.79	2.78	12.76	A	1	0.65	1	1	1	199.426	5.88	145.08	C
			B	1	0.65	1	1	1	199.426			
			C	1	0.65	1	1	1	199.426			
L4 78.79-39.88	2.70	18.55	A	1	0.65	1	1	1	213.639	5.54	142.26	C
			B	1	0.65	1	1	1	213.639			
			C	1	0.65	1	1	1	213.639			
L5 39.88-1.50	2.32	20.49	A	1	0.65	1	1	1	231.142	5.26	137.18	C
			B	1	0.65	1	1	1	231.142			
			C	1	0.65	1	1	1	231.142			
Sum Weight:	9.41	59.96						OTM	1847.17 kip-ft	22.80		

### Tower Forces - No Ice - Wind 60 To Face

Section Elevation ft	Add Weight K	Self Weight K	F <sub>a c e</sub> e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
L1 164.00-131.50	0.88	5.47	A	1	0.65	1	1	1	135.985	4.35	133.99	C
			B	1	0.65	1	1	1	135.985			
			C	1	0.65	1	1	1	135.985			
L2 131.50-119.29	0.73	2.69	A	1	0.65	1	1	1	55.744	1.77	144.88	C
			B	1	0.65	1	1	1	55.744			
			C	1	0.65	1	1	1	55.744			
L3 119.29-78.79	2.78	12.76	A	1	0.65	1	1	1	199.426	5.88	145.08	C
			B	1	0.65	1	1	1	199.426			
			C	1	0.65	1	1	1	199.426			
L4 78.79-39.88	2.70	18.55	A	1	0.65	1	1	1	213.639	5.54	142.26	C
			B	1	0.65	1	1	1	213.639			

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Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
L5 39.88-1.50	2.32	20.49	C	1	0.65	1	1	1	213.639	5.26	137.18	C
			A	1	0.65	1	1	1	231.142			
			B	1	0.65	1	1	1	231.142			
			C	1	0.65	1	1	1	231.142			
Sum Weight:	9.41	59.96						OTM	1847.17 kip-ft	22.80		

### Tower Forces - No Ice - Wind 90 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
L1 164.00-131.50	0.88	5.47	A	1	0.65	1	1	1	135.985	4.35	133.99	C
			B	1	0.65	1	1	1	135.985			
			C	1	0.65	1	1	1	135.985			
L2 131.50-119.29	0.73	2.69	A	1	0.65	1	1	1	55.744	1.77	144.88	C
			B	1	0.65	1	1	1	55.744			
			C	1	0.65	1	1	1	55.744			
L3 119.29-78.79	2.78	12.76	A	1	0.65	1	1	1	199.426	5.88	145.08	C
			B	1	0.65	1	1	1	199.426			
			C	1	0.65	1	1	1	199.426			
L4 78.79-39.88	2.70	18.55	A	1	0.65	1	1	1	213.639	5.54	142.26	C
			B	1	0.65	1	1	1	213.639			
			C	1	0.65	1	1	1	213.639			
L5 39.88-1.50	2.32	20.49	A	1	0.65	1	1	1	231.142	5.26	137.18	C
			B	1	0.65	1	1	1	231.142			
			C	1	0.65	1	1	1	231.142			
Sum Weight:	9.41	59.96						OTM	1847.17 kip-ft			

### Tower Forces - With Ice - Wind Normal To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
L1 164.00-131.50	0.99	6.49	A	1	0.65	1	1	1	138.694	3.37	103.82	C
			B	1	0.65	1	1	1	138.694			
			C	1	0.65	1	1	1	138.694			
L2 131.50-119.29	0.84	3.11	A	1	0.65	1	1	1	56.761	1.39	113.95	C
			B	1	0.65	1	1	1	56.761			
			C	1	0.65	1	1	1	56.761			
L3 119.29-78.79	3.14	14.25	A	1	0.65	1	1	1	202.801	4.61	113.75	C
			B	1	0.65	1	1	1	202.801			
			C	1	0.65	1	1	1	202.801			
L4 78.79-39.88	3.09	20.14	A	1	0.65	1	1	1	216.881	4.41	113.42	C
			B	1	0.65	1	1	1	216.881			
			C	1	0.65	1	1	1	216.881			
L5 39.88-1.50	2.72	22.21	A	1	0.65	1	1	1	234.341	4.32	112.62	C
			B	1	0.65	1	1	1	234.341			

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Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
Sum Weight:	10.78	66.20	C	1	0.65	1	1	1 OTM	234.341 1450.85 kip-ft	18.11		

**Tower Forces - With Ice - Wind 60 To Face**

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
L1 164.00-131.50	0.99	6.49	A	1	0.65	1	1	1	138.694	3.37	103.82	C
			B	1	0.65	1	1	1	138.694			
			C	1	0.65	1	1	1	138.694			
L2 131.50-119.29	0.84	3.11	A	1	0.65	1	1	1	56.761	1.39	113.95	C
			B	1	0.65	1	1	1	56.761			
			C	1	0.65	1	1	1	56.761			
L3 119.29-78.79	3.14	14.25	A	1	0.65	1	1	1	202.801	4.61	113.75	C
			B	1	0.65	1	1	1	202.801			
			C	1	0.65	1	1	1	202.801			
L4 78.79-39.88	3.09	20.14	A	1	0.65	1	1	1	216.881	4.41	113.42	C
			B	1	0.65	1	1	1	216.881			
			C	1	0.65	1	1	1	216.881			
L5 39.88-1.50	2.72	22.21	A	1	0.65	1	1	1	234.341	4.32	112.62	C
			B	1	0.65	1	1	1	234.341			
			C	1	0.65	1	1	1	234.341			
Sum Weight:	10.78	66.20						OTM	1450.85 kip-ft	18.11		

**Tower Forces - With Ice - Wind 90 To Face**

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
L1 164.00-131.50	0.99	6.49	A	1	0.65	1	1	1	138.694	3.37	103.82	C
			B	1	0.65	1	1	1	138.694			
			C	1	0.65	1	1	1	138.694			
L2 131.50-119.29	0.84	3.11	A	1	0.65	1	1	1	56.761	1.39	113.95	C
			B	1	0.65	1	1	1	56.761			
			C	1	0.65	1	1	1	56.761			
L3 119.29-78.79	3.14	14.25	A	1	0.65	1	1	1	202.801	4.61	113.75	C
			B	1	0.65	1	1	1	202.801			
			C	1	0.65	1	1	1	202.801			
L4 78.79-39.88	3.09	20.14	A	1	0.65	1	1	1	216.881	4.41	113.42	C
			B	1	0.65	1	1	1	216.881			
			C	1	0.65	1	1	1	216.881			
L5 39.88-1.50	2.72	22.21	A	1	0.65	1	1	1	234.341	4.32	112.62	C
			B	1	0.65	1	1	1	234.341			
			C	1	0.65	1	1	1	234.341			
Sum Weight:	10.78	66.20						OTM	1450.85 kip-ft	18.11		

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### Tower Forces - Service - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
L1 164.00-131.50	0.88	5.47	A	1	0.65	1	1	1	135.985	1.51	46.36	C
			B	1	0.65	1	1	135.985				
			C	1	0.65	1	1	135.985				
L2 131.50-119.29	0.73	2.69	A	1	0.65	1	1	1	55.744	0.61	50.13	C
			B	1	0.65	1	1	55.744				
			C	1	0.65	1	1	55.744				
L3 119.29-78.79	2.78	12.76	A	1	0.65	1	1	1	199.426	2.03	50.20	C
			B	1	0.65	1	1	199.426				
			C	1	0.65	1	1	199.426				
L4 78.79-39.88	2.70	18.55	A	1	0.65	1	1	1	213.639	1.92	49.22	C
			B	1	0.65	1	1	213.639				
			C	1	0.65	1	1	213.639				
L5 39.88-1.50	2.32	20.49	A	1	0.65	1	1	1	231.142	1.82	47.47	C
			B	1	0.65	1	1	231.142				
			C	1	0.65	1	1	231.142				
Sum Weight:	9.41	59.96						OTM	639.16 kip-ft	7.89		

### Tower Forces - Service - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
L1 164.00-131.50	0.88	5.47	A	1	0.65	1	1	1	135.985	1.51	46.36	C
			B	1	0.65	1	1	135.985				
			C	1	0.65	1	1	135.985				
L2 131.50-119.29	0.73	2.69	A	1	0.65	1	1	1	55.744	0.61	50.13	C
			B	1	0.65	1	1	55.744				
			C	1	0.65	1	1	55.744				
L3 119.29-78.79	2.78	12.76	A	1	0.65	1	1	1	199.426	2.03	50.20	C
			B	1	0.65	1	1	199.426				
			C	1	0.65	1	1	199.426				
L4 78.79-39.88	2.70	18.55	A	1	0.65	1	1	1	213.639	1.92	49.22	C
			B	1	0.65	1	1	213.639				
			C	1	0.65	1	1	213.639				
L5 39.88-1.50	2.32	20.49	A	1	0.65	1	1	1	231.142	1.82	47.47	C
			B	1	0.65	1	1	231.142				
			C	1	0.65	1	1	231.142				
Sum Weight:	9.41	59.96						OTM	639.16 kip-ft	7.89		

### Tower Forces - Service - Wind 90 To Face

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Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
L1 164.00-131.50	0.88	5.47	A	1	0.65	1	1	1	135.985	1.51	46.36	C
			B	1	0.65	1	1	1	135.985			
			C	1	0.65	1	1	1	135.985			
L2 131.50-119.29	0.73	2.69	A	1	0.65	1	1	1	55.744	0.61	50.13	C
			B	1	0.65	1	1	1	55.744			
			C	1	0.65	1	1	1	55.744			
L3 119.29-78.79	2.78	12.76	A	1	0.65	1	1	1	199.426	2.03	50.20	C
			B	1	0.65	1	1	1	199.426			
			C	1	0.65	1	1	1	199.426			
L4 78.79-39.88	2.70	18.55	A	1	0.65	1	1	1	213.639	1.92	49.22	C
			B	1	0.65	1	1	1	213.639			
			C	1	0.65	1	1	1	213.639			
L5 39.88-1.50	2.32	20.49	A	1	0.65	1	1	1	231.142	1.82	47.47	C
			B	1	0.65	1	1	1	231.142			
			C	1	0.65	1	1	1	231.142			
Sum Weight:	9.41	59.96						OTM	639.16 kip-ft	7.89		

### Force Totals

Load Case	Vertical Forces	Sum of Forces X	Sum of Forces Z	Sum of Overturning Moments, M <sub>x</sub>	Sum of Overturning Moments, M <sub>z</sub>	Sum of Torques
	K	K	K	kip-ft	kip-ft	kip-ft
Leg Weight	59.96					
Bracing Weight	0.00					
Total Member Self-Weight	59.96			0.75	-1.10	
Total Weight	82.22			0.75	-1.10	
Wind 0 deg - No Ice		-0.08	-44.61	-4890.26	1.39	-13.20
Wind 30 deg - No Ice		22.49	-38.59	-4233.75	-2479.65	-15.30
Wind 60 deg - No Ice		39.03	-22.24	-2442.60	-4296.56	-13.30
Wind 90 deg - No Ice		45.11	0.08	3.24	-4962.51	-7.73
Wind 120 deg - No Ice		39.10	22.37	2448.41	-4299.05	-0.10
Wind 150 deg - No Ice		22.62	38.67	4237.74	-2483.96	7.57
Wind 180 deg - No Ice		0.08	44.61	4891.77	-3.59	13.20
Wind 210 deg - No Ice		-22.49	38.59	4235.25	2477.45	15.30
Wind 240 deg - No Ice		-39.03	22.24	2444.11	4294.36	13.30
Wind 270 deg - No Ice		-45.11	-0.08	-1.73	4960.31	7.73
Wind 300 deg - No Ice		-39.10	-22.37	-2446.91	4296.85	0.10
Wind 330 deg - No Ice		-22.62	-38.67	-4236.23	2481.76	-7.57
Member Ice	6.24					
Total Weight Ice	95.25			1.03	-1.73	
Wind 0 deg - Ice		-0.06	-36.77	-4053.44	0.05	-10.80
Wind 30 deg - Ice		18.52	-31.82	-3509.36	-2054.32	-12.31
Wind 60 deg - Ice		32.15	-18.33	-2024.67	-3558.69	-10.53
Wind 90 deg - Ice		37.15	0.06	2.80	-4109.98	-5.92
Wind 120 deg - Ice		32.21	18.44	2029.80	-3560.47	0.28
Wind 150 deg - Ice		18.63	31.88	3513.19	-2057.39	6.40
Wind 180 deg - Ice		0.06	36.77	4055.50	-3.50	10.80
Wind 210 deg - Ice		-18.52	31.82	3511.42	2050.86	12.31
Wind 240 deg - Ice		-32.15	18.33	2026.73	3555.24	10.53
Wind 270 deg - Ice		-37.15	-0.06	-0.74	4106.53	5.92
Wind 300 deg - Ice		-32.21	-18.44	-2027.74	3557.01	-0.28
Wind 330 deg - Ice		-18.63	-31.88	-3511.13	2053.93	-6.40
Total Weight	82.22			0.75	-1.10	
Wind 0 deg - Service		-0.03	-15.44	-1692.87	1.89	-4.57



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Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, $M_x$ kip-ft	Sum of Overturning Moments, $M_z$ kip-ft	Sum of Torques kip-ft
Wind 30 deg - Service		7.78	-13.35	-1465.70	-856.60	-5.29
Wind 60 deg - Service		13.50	-7.69	-845.93	-1485.29	-4.60
Wind 90 deg - Service		15.61	0.03	0.38	-1715.72	-2.68
Wind 120 deg - Service		13.53	7.74	846.46	-1486.15	-0.03
Wind 150 deg - Service		7.83	13.38	1465.61	-858.09	2.62
Wind 180 deg - Service		0.03	15.44	1691.91	0.17	4.57
Wind 210 deg - Service		-7.78	13.35	1464.75	858.66	5.29
Wind 240 deg - Service		-13.50	7.69	844.97	1487.35	4.60
Wind 270 deg - Service		-15.61	-0.03	-1.34	1717.78	2.68
Wind 300 deg - Service		-13.53	-7.74	-847.42	1488.21	0.03
Wind 330 deg - Service		-7.83	-13.38	-1466.56	860.15	-2.62

## Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 60 deg - No Ice
5	Dead+Wind 90 deg - No Ice
6	Dead+Wind 120 deg - No Ice
7	Dead+Wind 150 deg - No Ice
8	Dead+Wind 180 deg - No Ice
9	Dead+Wind 210 deg - No Ice
10	Dead+Wind 240 deg - No Ice
11	Dead+Wind 270 deg - No Ice
12	Dead+Wind 300 deg - No Ice
13	Dead+Wind 330 deg - No Ice
14	Dead+Ice+Temp
15	Dead+Wind 0 deg+Ice+Temp
16	Dead+Wind 30 deg+Ice+Temp
17	Dead+Wind 60 deg+Ice+Temp
18	Dead+Wind 90 deg+Ice+Temp
19	Dead+Wind 120 deg+Ice+Temp
20	Dead+Wind 150 deg+Ice+Temp
21	Dead+Wind 180 deg+Ice+Temp
22	Dead+Wind 210 deg+Ice+Temp
23	Dead+Wind 240 deg+Ice+Temp
24	Dead+Wind 270 deg+Ice+Temp
25	Dead+Wind 300 deg+Ice+Temp
26	Dead+Wind 330 deg+Ice+Temp
27	Dead+Wind 0 deg - Service
28	Dead+Wind 30 deg - Service
29	Dead+Wind 60 deg - Service
30	Dead+Wind 90 deg - Service
31	Dead+Wind 120 deg - Service
32	Dead+Wind 150 deg - Service
33	Dead+Wind 180 deg - Service
34	Dead+Wind 210 deg - Service
35	Dead+Wind 240 deg - Service
36	Dead+Wind 270 deg - Service
37	Dead+Wind 300 deg - Service
38	Dead+Wind 330 deg - Service

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### Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	164 - 131.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-20.30	-0.95	-0.83
			Max. Mx	5	-15.01	-359.61	5.49
			Max. My	8	-15.02	5.46	-352.75
			Max. Vy	11	-20.38	358.69	-6.24
			Max. Vx	2	-20.16	-6.26	351.86
			Max. Torque	11			1.30
L2	131.5 - 119.29	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-22.28	-1.13	-0.94
			Max. Mx	5	-16.72	-489.09	6.64
			Max. My	8	-16.73	6.59	-480.79
			Max. Vy	11	-21.30	488.01	-7.45
			Max. Vx	2	-21.07	-7.49	479.82
			Max. Torque	9			-10.47
L3	119.29 - 78.79	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-43.61	1.79	1.00
			Max. Mx	11	-34.51	1640.25	-4.37
			Max. My	2	-34.53	-4.16	1611.92
			Max. Vy	5	33.57	-1639.63	4.39
			Max. Vx	8	33.06	4.60	-1611.80
			Max. Torque	9			-17.73
L4	78.79 - 39.88	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-65.36	0.34	0.16
			Max. Mx	5	-54.52	-3025.28	1.06
			Max. My	8	-54.53	1.05	-2977.82
			Max. Vy	5	39.13	-3025.28	1.06
			Max. Vx	8	38.63	1.05	-2977.82
			Max. Torque	9			-15.78
L5	39.88 - 1.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-95.25	-1.73	-1.03
			Max. Mx	5	-82.21	-5035.53	-3.21
			Max. My	8	-82.21	-3.56	-4963.69
			Max. Vy	5	45.13	-5035.53	-3.21
			Max. Vx	8	44.63	-3.56	-4963.69
			Max. Torque	9			-15.58

### Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	18	95.25	-37.15	-0.06
	Max. H <sub>x</sub>	11	82.22	45.11	0.08
	Max. H <sub>z</sub>	2	82.22	0.08	44.61
	Max. M <sub>x</sub>	2	4962.17	0.08	44.61
	Max. M <sub>z</sub>	5	5035.53	-45.11	-0.08
	Max. Torsion	3	15.27	-22.49	38.59
	Min. Vert	30	82.22	-15.61	-0.03
	Min. H <sub>x</sub>	5	82.22	-45.11	-0.08
	Min. H <sub>z</sub>	8	82.22	-0.08	-44.61
	Min. M <sub>x</sub>	8	-4963.69	-0.08	-44.61
	Min. M <sub>z</sub>	11	-5033.32	45.11	0.08
	Min. Torsion	9	-15.29	22.49	-38.59

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Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
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### Tower Mast Reaction Summary

Load Combination	Vertical K	Shear <sub>y</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Dead Only	82.22	0.00	0.00	0.75	-1.10	0.00
Dead+Wind 0 deg - No Ice	82.22	-0.08	-44.61	-4962.17	1.33	-13.18
Dead+Wind 30 deg - No Ice	82.22	22.49	-38.59	-4296.03	-2516.22	-15.27
Dead+Wind 60 deg - No Ice	82.22	39.03	-22.24	-2478.57	-4359.83	-13.27
Dead+Wind 90 deg - No Ice	82.22	45.11	0.08	3.21	-5035.53	-7.72
Dead+Wind 120 deg - No Ice	82.22	39.10	22.37	2484.34	-4362.27	-0.08
Dead+Wind 150 deg - No Ice	82.22	22.62	38.67	4300.00	-2520.45	7.58
Dead+Wind 180 deg - No Ice	82.22	0.08	44.61	4963.69	-3.57	13.21
Dead+Wind 210 deg - No Ice	82.22	-22.49	38.59	4297.56	2513.98	15.29
Dead+Wind 240 deg - No Ice	82.22	-39.03	22.24	2480.11	4357.60	13.27
Dead+Wind 270 deg - No Ice	82.22	-45.11	-0.08	-1.68	5033.32	7.69
Dead+Wind 300 deg - No Ice	82.22	-39.10	-22.37	-2482.82	4360.06	0.06
Dead+Wind 330 deg - No Ice	82.22	-22.62	-38.67	-4298.49	2518.23	-7.57
Dead+Ice+Temp	95.25	0.00	0.00	1.03	-1.73	0.00
Dead+Wind 0 deg+Ice+Temp	95.25	-0.06	-36.77	-4127.46	0.00	-10.79
Dead+Wind 30 deg+Ice+Temp	95.25	18.52	-31.82	-3573.47	-2091.89	-12.29
Dead+Wind 60 deg+Ice+Temp	95.25	32.15	-18.33	-2061.71	-3623.72	-10.50
Dead+Wind 90 deg+Ice+Temp	95.25	37.15	0.06	2.76	-4185.04	-5.89
Dead+Wind 120 deg+Ice+Temp	95.25	32.21	18.44	2066.77	-3625.45	0.30
Dead+Wind 150 deg+Ice+Temp	95.25	18.63	31.88	3577.26	-2094.89	6.42
Dead+Wind 180 deg+Ice+Temp	95.25	0.06	36.77	4129.51	-3.46	10.81
Dead+Wind 210 deg+Ice+Temp	95.25	-18.52	31.82	3575.54	2088.43	12.30
Dead+Wind 240 deg+Ice+Temp	95.25	-32.15	18.33	2063.77	3620.27	10.49
Dead+Wind 270 deg+Ice+Temp	95.25	-37.15	-0.06	-0.70	4181.60	5.87
Dead+Wind 300 deg+Ice+Temp	95.25	-32.21	-18.44	-2064.71	3622.01	-0.31
Dead+Wind 330 deg+Ice+Temp	95.25	-18.63	-31.88	-3575.21	2091.44	-6.41
Dead+Wind 0 deg - Service	82.22	-0.03	-15.44	-1716.67	-0.26	-4.57
Dead+Wind 30 deg - Service	82.22	7.78	-13.35	-1486.15	-871.46	-5.29
Dead+Wind 60 deg - Service	82.22	13.50	-7.69	-857.22	-1509.45	-4.60
Dead+Wind 90 deg - Service	82.22	15.61	0.03	1.61	-1743.28	-2.67
Dead+Wind 120 deg - Service	82.22	13.53	7.74	860.21	-1510.30	-0.03
Dead+Wind 150 deg - Service	82.22	7.83	13.38	1488.52	-872.93	2.62
Dead+Wind 180 deg - Service	82.22	0.03	15.44	1718.19	-1.96	4.57
Dead+Wind 210 deg - Service	82.22	-7.78	13.35	1487.68	869.25	5.29
Dead+Wind 240 deg - Service	82.22	-13.50	7.69	858.74	1507.24	4.59
Dead+Wind 270 deg - Service	82.22	-15.61	-0.03	-0.09	1741.07	2.67
Dead+Wind 300 deg - Service	82.22	-13.53	-7.74	-858.69	1508.08	0.02
Dead+Wind 330 deg - Service	82.22	-7.83	-13.38	-1487.00	870.71	-2.62

### Solution Summary

Load Comb.	Sum of Applied Forces				Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K		
1	0.00	-82.22	0.00	0.00	82.22	0.00	0.000%	
2	-0.08	-82.22	-44.61	0.08	82.22	44.61	0.000%	
3	22.49	-82.22	-38.59	-22.49	82.22	38.59	0.000%	
4	39.03	-82.22	-22.24	-39.03	82.22	22.24	0.000%	

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
5	45.11	-82.22	0.08	-45.11	82.22	-0.08	0.000%
6	39.10	-82.22	22.37	-39.10	82.22	-22.37	0.000%
7	22.62	-82.22	38.67	-22.62	82.22	-38.67	0.000%
8	0.08	-82.22	44.61	-0.08	82.22	-44.61	0.000%
9	-22.49	-82.22	38.59	22.49	82.22	-38.59	0.000%
10	-39.03	-82.22	22.24	39.03	82.22	-22.24	0.000%
11	-45.11	-82.22	-0.08	45.11	82.22	0.08	0.000%
12	-39.10	-82.22	-22.37	39.10	82.22	22.37	0.000%
13	-22.62	-82.22	-38.67	22.62	82.22	38.67	0.000%
14	0.00	-95.25	0.00	0.00	95.25	0.00	0.000%
15	-0.06	-95.25	-36.77	0.06	95.25	36.77	0.000%
16	18.52	-95.25	-31.82	-18.52	95.25	31.82	0.000%
17	32.15	-95.25	-18.33	-32.15	95.25	18.33	0.000%
18	37.15	-95.25	0.06	-37.15	95.25	-0.06	0.000%
19	32.21	-95.25	18.44	-32.21	95.25	-18.44	0.000%
20	18.63	-95.25	31.88	-18.63	95.25	-31.88	0.000%
21	0.06	-95.25	36.77	-0.06	95.25	-36.77	0.000%
22	-18.52	-95.25	31.82	18.52	95.25	-31.82	0.000%
23	-32.15	-95.25	18.33	32.15	95.25	-18.33	0.000%
24	-37.15	-95.25	-0.06	37.15	95.25	0.06	0.000%
25	-32.21	-95.25	-18.44	32.21	95.25	18.44	0.000%
26	-18.63	-95.25	-31.88	18.63	95.25	31.88	0.000%
27	-0.03	-82.22	-15.44	0.03	82.22	15.44	0.000%
28	7.78	-82.22	-13.35	-7.78	82.22	13.35	0.000%
29	13.50	-82.22	-7.69	-13.50	82.22	7.69	0.000%
30	15.61	-82.22	0.03	-15.61	82.22	-0.03	0.000%
31	13.53	-82.22	7.74	-13.53	82.22	-7.74	0.000%
32	7.83	-82.22	13.38	-7.83	82.22	-13.38	0.000%
33	0.03	-82.22	15.44	-0.03	82.22	-15.44	0.000%
34	-7.78	-82.22	13.35	7.78	82.22	-13.35	0.000%
35	-13.50	-82.22	7.69	13.50	82.22	-7.69	0.000%
36	-15.61	-82.22	-0.03	15.61	82.22	0.03	0.000%
37	-13.53	-82.22	-7.74	13.53	82.22	7.74	0.000%
38	-7.83	-82.22	-13.38	7.83	82.22	13.38	0.000%

### Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	4	0.00000001	0.00042444
3	Yes	4	0.00000001	0.00054726
4	Yes	4	0.00000001	0.00085711
5	Yes	4	0.00000001	0.00025502
6	Yes	4	0.00000001	0.00054586
7	Yes	4	0.00000001	0.00048215
8	Yes	4	0.00000001	0.00042643
9	Yes	4	0.00000001	0.00090069
10	Yes	4	0.00000001	0.00051243
11	Yes	4	0.00000001	0.00025306
12	Yes	4	0.00000001	0.00054920
13	Yes	4	0.00000001	0.00069550
14	Yes	4	0.00000001	0.00000001
15	Yes	5	0.00000001	0.00002617
16	Yes	5	0.00000001	0.00003251
17	Yes	5	0.00000001	0.00003462

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18	Yes	5	0.0000001	0.00002540
19	Yes	5	0.0000001	0.00003223
20	Yes	5	0.0000001	0.00003185
21	Yes	5	0.0000001	0.00002618
22	Yes	5	0.0000001	0.00003504
23	Yes	5	0.0000001	0.00003233
24	Yes	5	0.0000001	0.00002541
25	Yes	5	0.0000001	0.00003221
26	Yes	5	0.0000001	0.00003323
27	Yes	4	0.0000001	0.00006287
28	Yes	4	0.0000001	0.00006847
29	Yes	4	0.0000001	0.00008420
30	Yes	4	0.0000001	0.00003925
31	Yes	4	0.0000001	0.00003722
32	Yes	4	0.0000001	0.00004147
33	Yes	4	0.0000001	0.00006299
34	Yes	4	0.0000001	0.00009169
35	Yes	4	0.0000001	0.00006037
36	Yes	4	0.0000001	0.00003914
37	Yes	4	0.0000001	0.00003756
38	Yes	4	0.0000001	0.00005953

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	164 - 131.5	10.608	30	0.4910	0.0043
L2	131.5 - 119.29	7.316	30	0.4659	0.0043
L3	125.29 - 78.79	6.717	30	0.4548	0.0044
L4	87.21 - 39.88	3.448	30	0.3457	0.0022
L5	49.13 - 1.5	1.163	30	0.2109	0.0010

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
164.00	12' x 3" Dia Omni	30	10.608	0.4910	0.0043	226201
160.00	4 FT DISH	30	10.196	0.4895	0.0043	226201
156.00	A-Ant-23G-2-C	30	9.784	0.4878	0.0043	141376
154.00	LLPX310R	30	9.579	0.4869	0.0043	113101
151.50	Remote Radio Head FD R6 RRH	30	9.322	0.4856	0.0043	90480
144.00	APX16PV-16PVL-E	30	8.559	0.4805	0.0043	56550
138.00	(2) RRUS-11	30	7.957	0.4746	0.0043	43500
134.00	7770.00	30	7.561	0.4696	0.0043	37676
124.00	DB844H65E-XY	30	6.595	0.4522	0.0044	28038
114.00	Low Profile Platform	30	5.670	0.4284	0.0040	24670
51.50	GPS	30	1.265	0.2201	0.0011	11199

### Maximum Tower Deflections - Design Wind

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	164 - 131.5	30.647	5	1.4184	0.0123
L2	131.5 - 119.29	21.136	5	1.3462	0.0125
L3	125.29 - 78.79	19.406	5	1.3142	0.0126
L4	87.21 - 39.88	9.960	5	0.9986	0.0064
L5	49.13 - 1.5	3.358	5	0.6092	0.0030

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
164.00	12' x 3" Dia Omni	5	30.647	1.4184	0.0123	78534
160.00	4 FT DISH	5	29.456	1.4140	0.0123	78534
156.00	A-Ant-23G-2-C	5	28.266	1.4092	0.0123	49084
154.00	LLPX310R	5	27.672	1.4066	0.0123	39267
151.50	Remote Radio Head FD R6 RRH	5	26.932	1.4028	0.0124	31413
144.00	APX16PV-16PVL-E	5	24.728	1.3882	0.0124	19633
138.00	(2) RRUS-11	5	22.989	1.3713	0.0125	15102
134.00	7770.00	5	21.844	1.3568	0.0125	13079
124.00	DB844H65E-XY	5	19.052	1.3066	0.0126	9718
114.00	Low Profile Platform	5	16.381	1.2377	0.0116	8551
51.50	GPS	5	3.654	0.6359	0.0031	3877

### Compression Checks

### Pole Design Data

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P/P <sub>a</sub>
L1	164 - 131.5 (1)	TP53.42x47x0.3125	32.50	162.50	103.4	13.959	52.6760	-15.00	735.30	0.020
L2	131.5 - 119.29 (2)	TP56.15x53.42x0.375	12.21	162.50	100.9	14.665	64.7894	-16.72	950.11	0.018
L3	119.29 - 78.79 (3)	TP62.97x54.0585x0.4375	46.50	162.50	90.2	18.264	84.5934	-34.51	1545.03	0.022
L4	78.79 - 39.88 (4)	TP69.66x60.4813x0.5625	47.33	162.50	81.6	21.155	120.1620	-54.52	2542.07	0.021
L5	39.88 - 1.5 (5)	TP76x66.7412x0.5625	47.63	162.50	72.8	23.922	134.6840	-82.21	3221.92	0.026

### Pole Bending Design Data

Section No.	Elevation ft	Size	Actual M <sub>x</sub> kip-ft	Actual f <sub>bx</sub> ksi	Allow. F <sub>bx</sub> ksi	Ratio f <sub>bx</sub> /F <sub>bx</sub>	Actual M <sub>y</sub> kip-ft	Actual f <sub>by</sub> ksi	Allow. F <sub>by</sub> ksi	Ratio f <sub>by</sub> /F <sub>by</sub>
L1	164 - 131.5 (1)	TP53.42x47x0.3125	362.69	-6.293	36.775	0.171	0.00	0.000	36.775	0.000
L2	131.5 - 119.29 (2)	TP56.15x53.42x0.375	492.81	-6.789	39.000	0.174	0.00	0.000	39.000	0.000

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15001.145 - Greenwich	<b>Page</b> 23 of 23
	<b>Project</b> 164' EEI Monopole - 5 Perryridge Rd., Greenwich, CT	<b>Date</b> 13:18:35 01/20/16
	<b>Client</b> Verizon Wireless	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	Actual $M_x$ kip-ft	Actual $f_{bx}$ ksi	Allow. $F_{bx}$ ksi	Ratio $\frac{f_{bx}}{F_{bx}}$	Actual $M_y$ kip-ft	Actual $f_{by}$ ksi	Allow. $F_{by}$ ksi	Ratio $\frac{f_{by}}{F_{by}}$
L3	119.29 - 78.79 (3)	TP62.97x54.0585x0.4375	1640.25	-15.468	39.000	0.397	0.00	0.000	39.000	0.000
L4	78.79 - 39.88 (4)	TP69.66x60.4813x0.5625	3025.28	-18.200	39.000	0.467	0.00	0.000	39.000	0.000
L5	39.88 - 1.5 (5)	TP76x66.7412x0.5625	5035.53	-24.092	39.000	0.618	0.00	0.000	39.000	0.000

### Pole Interaction Design Data

Section No.	Elevation ft	Size	Ratio $P$ $P_n$	Ratio $f_{bx}$ $F_{bx}$	Ratio $f_{by}$ $F_{by}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	164 - 131.5 (1)	TP53.42x47x0.3125	0.020	0.171	0.000	0.192	1.333	H1-3 ✓
L2	131.5 - 119.29 (2)	TP56.15x53.42x0.375	0.018	0.174	0.000	0.192	1.333	H1-3 ✓
L3	119.29 - 78.79 (3)	TP62.97x54.0585x0.4375	0.022	0.397	0.000	0.419	1.333	H1-3 ✓
L4	78.79 - 39.88 (4)	TP69.66x60.4813x0.5625	0.021	0.467	0.000	0.488	1.333	H1-3 ✓
L5	39.88 - 1.5 (5)	TP76x66.7412x0.5625	0.026	0.618	0.000	0.643	1.333	H1-3 ✓

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF* $P_{allow}$ K	% Capacity	Pass Fail
L1	164 - 131.5	Pole	TP53.42x47x0.3125	1	-15.00	980.15	14.4	Pass
L2	131.5 - 119.29	Pole	TP56.15x53.42x0.375	2	-16.72	1266.50	14.4	Pass
L3	119.29 - 78.79	Pole	TP62.97x54.0585x0.4375	3	-34.51	2059.52	31.4	Pass
L4	78.79 - 39.88	Pole	TP69.66x60.4813x0.5625	4	-54.52	3388.58	36.6	Pass
L5	39.88 - 1.5	Pole	TP76x66.7412x0.5625	5	-82.21	4294.82	48.3	Pass
Summary								
Pole (L5)							48.3	Pass
<b>RATING =</b>							<b>48.3</b>	<b>Pass</b>

**Flange Bolt and Plate Analysis:****Input Data:**Tower Reactions:

Overturing Moment =	OM := 360ft-kips	(Input From tnxTower)
Shear Force =	Shear := 20.5-kips	(Input From tnxTower)
Axial Force =	Axial := 20.5-kips	(Input From tnxTower)

Flange Bolt Data:

Use ASTM A325

Number of Flange Bolts =	N := 12	(User Input)
Diameter of Bolt Circle =	$D_{bc}$ := 58.00-in	(User Input)
Bolt Ultimate Strength =	$F_u$ := 120-ksi	(User Input)
Bolt Yield Strength =	$F_y$ := 92-ksi	(User Input)
Bolt Modulus =	E := 29000-ksi	(User Input)
Diameter of Flange Bolts =	D := 1.00-in	(User Input)

Flange Plate Data:

Use ASTM A36

Plate Yield Strength =	$F_{y_{bp}}$ := 36.00-ksi	(User Input)
Flange Plate Thickness =	$t_{bp}$ := 1.00-in	(User Input)
Flange Plate Diameter =	$D_{bp}$ := 61.00-in	(User Input)
Outer Pole Diameter =	$D_{pole}$ := 53.42-in	(User Input)



**Geometric Layout Data:**

Distance from Bolts to Centroid of Pole:

Radius of Bolt Circle =:

$$R_{bc} := \frac{D_{bc}}{2} = 29\text{-in}$$

Distance to Bolts =

$$i := 1..N$$

$$d_i := \begin{cases} \theta \leftarrow 2\pi \cdot \left(\frac{i}{N}\right) \\ d \leftarrow R_{bc} \cdot \sin(\theta) \end{cases}$$

$$d_1 = 14.50\text{-in}$$

$$d_7 = -14.50\text{-in}$$

$$d_2 = 25.11\text{-in}$$

$$d_8 = -25.11\text{-in}$$

$$d_3 = 29.00\text{-in}$$

$$d_9 = -29.00\text{-in}$$

$$d_4 = 25.11\text{-in}$$

$$d_{10} = -25.11\text{-in}$$

$$d_5 = 14.50\text{-in}$$

$$d_{11} = -14.50\text{-in}$$

$$d_6 = 0.00\text{-in}$$

etc.

Critical Distances For Bending in Plate:

Outer Pole Radius =

$$R_{pole} := \frac{D_{pole}}{2} = 26.7\text{-in}$$

Moment Arms of Bolts about Neutral Axis =

$$MA_i := \text{if}(d_i \geq R_{pole}, d_i - R_{pole}, 0\text{in})$$

$$MA_1 = 0.00\text{-in}$$

$$MA_7 = 0.00\text{-in}$$

$$MA_2 = 0.00\text{-in}$$

$$MA_8 = 0.00\text{-in}$$

$$MA_3 = 2.29\text{-in}$$

$$MA_9 = 0.00\text{-in}$$

$$MA_4 = 0.00\text{-in}$$

$$MA_{10} = 0.00\text{-in}$$

$$MA_5 = 0.00\text{-in}$$

$$MA_{11} = 0.00\text{-in}$$

$$MA_6 = 0.00\text{-in}$$

etc

Effective Width of Plate for Bending =

$$B_{eff} := .8 \cdot 2 \cdot \sqrt{\left(\frac{D_{bp}}{2}\right)^2 - \left(\frac{D_{pole}}{2}\right)^2} = 23.6\text{-in}$$

**Flange Bolt Analysis:**

Calculated Flange Bolt Properties:

Polar Moment of Inertia =

$$I_p := \sum_i (d_i)^2 = 5.046 \times 10^3 \cdot \text{in}^2$$

Gross Area of Bolt =

$$A_g := \frac{\pi}{4} \cdot D^2 = 0.785 \cdot \text{in}^2$$

Check Flange Bolt Tension Force:

Maximum Tensile Force =

$$T_{\text{Max}} := \text{OM} \cdot \frac{R_{bc}}{I_p} - \frac{\text{Axial}}{N} = 23.1 \cdot \text{kips}$$

Allowable Tensile Force =

$$T_{\text{ALL.Gross}} := 1.333 \cdot (0.33 \cdot A_g \cdot F_u) = 41.5 \cdot \text{kips}$$

(1.333 increase  
 allowed per TIA/EIA)

Bolt Tension % of Capacity =

$$\frac{T_{\text{Max}}}{T_{\text{ALL.Gross}}} = 55.8\%$$

Condition1 =

$$\text{Condition1} := \text{if} \left( \frac{T_{\text{Max}}}{T_{\text{ALL.Gross}}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

Condition1 = "OK"

**Flange Plate Analysis:**

Force from Bolts =

$$C_i := \frac{OM \cdot d_i}{I_p} + \frac{Axial}{N}$$

$C_1 = 14.1 \cdot \text{kips}$

$C_7 = -10.7 \cdot \text{kips}$

$C_2 = 23.2 \cdot \text{kips}$

$C_8 = -19.8 \cdot \text{kips}$

$C_3 = 26.5 \cdot \text{kips}$

$C_9 = -23.1 \cdot \text{kips}$

$C_4 = 23.2 \cdot \text{kips}$

$C_{10} = -19.8 \cdot \text{kips}$

$C_5 = 14.1 \cdot \text{kips}$

$C_{11} = -10.7 \cdot \text{kips}$

$C_6 = 1.7 \cdot \text{kips}$

etc.

Maximum Bending Stress in Plate =

$$f_{bp} := \sum_i \frac{6 \cdot C_i \cdot MA_i}{(B_{eff} t_{bp})^2} = 15.5 \cdot \text{ksi}$$

Allowable Bending Stress in Plate =

$F_{bp} := 1.33 \cdot 0.75 \cdot F_{y_{bp}} = 35.9 \cdot \text{ksi}$

Plate Bending Stress % of Capacity =

$\frac{f_{bp}}{F_{bp}} = 43.1\%$

Condition3 =

Condition2 := if  $\left( \frac{f_{bp}}{F_{bp}} < 1.00, "Ok", "Overstressed" \right)$

Condition2 = "Ok"

**Anchor Bolt and Base Plate Analysis:****Input Data:**Tower Reactions:

Overturing Moment =	OM := 5036-ft-kips	(Input From tnxTower)
Shear Force =	Shear := 45-kips	(Input From tnxTower)
Axial Force =	Axial := 82-kips	(Input From tnxTower)

Anchor Bolt Data:

Use ASTM A615 Grade 75		
Number of Anchor Bolts =	N := 30	(User Input)
Diameter of Bolt Circle =	$D_{bc}$ := 86.00-in	(User Input)
Bolt "Column" Distance =	l := 3.0-in	(User Input)
Bolt Ultimate Strenght =	$F_u$ := 100-ksi	(User Input)
Bolt Yeild Strenght =	$F_y$ := 75-ksi	(User Input)
Bolt Modulus =	E := 29000-ksi	(User Input)
Diameter of Anchor Bolts =	D := 2.25-in	(User Input)
Threads per Inch =	n := 4.5	(User Input)

Base Plate Data:

Use ASTM A572 GR 60		
Plate Yield Strength =	$F_{y_{bp}}$ := 60-ksi	(User Input)
Base Plate Thickness =	$t_{bp}$ := 3.0-in	(User Input)
Base Plate Diameter =	$D_{bp}$ := 92.00-in	(User Input)
Outer Pole Diameter =	$D_{pole}$ := 76.00-in	(User Input)

**Geometric Layout Data:**

Distance from Bolts to Centroid of Pole:

Radius of Bolt Circle =:  $R_{bc} := \frac{D_{bc}}{2} = 43\text{-in}$

Distance to Bolts =  $i := 1.. N$

$$d_i := \begin{cases} \theta \leftarrow 2 \cdot \pi \cdot \left( \frac{i}{N} \right) \\ d \leftarrow R_{bc} \cdot \sin(\theta) \end{cases}$$

$d_1 = 8.94\text{-in}$	$d_7 = 42.76\text{-in}$
$d_2 = 17.49\text{-in}$	$d_8 = 42.76\text{-in}$
$d_3 = 25.27\text{-in}$	$d_9 = 40.90\text{-in}$
$d_4 = 31.96\text{-in}$	$d_{10} = 37.24\text{-in}$
$d_5 = 37.24\text{-in}$	$d_{11} = 31.96\text{-in}$
$d_6 = 40.90\text{-in}$	etc.

Critical Distances For Bending in Plate:

Outer Pole Radius =  $R_{pole} := \frac{D_{pole}}{2} = 38\text{-in}$

Moment Arms of Bolts about Neutral Axis =  $MA_i := \text{if}(d_i \geq R_{pole}, d_i - R_{pole}, 0\text{in})$

$MA_1 = 0.00\text{-in}$	$MA_7 = 4.76\text{-in}$
$MA_2 = 0.00\text{-in}$	$MA_8 = 4.76\text{-in}$
$MA_3 = 0.00\text{-in}$	$MA_9 = 2.90\text{-in}$
$MA_4 = 0.00\text{-in}$	$MA_{10} = 0.00\text{-in}$
$MA_5 = 0.00\text{-in}$	$MA_{11} = 0.00\text{-in}$
$MA_6 = 2.90\text{-in}$	etc

Effective Width of Baseplate for Bending =  $B_{eff} := .8 \cdot 2 \cdot \sqrt{\left( \frac{D_{bp}}{2} \right)^2 - \left( \frac{D_{pole}}{2} \right)^2} = 41.5\text{-in}$

**Anchor Bolt Analysis:**

Calculated Anchor Bolt Properties:

Polar Moment of Inertia =

$$I_p := \sum_i (d_i)^2 = 2.773 \times 10^4 \cdot \text{in}^2$$

Gross Area of Bolt =

$$A_g := \frac{\pi}{4} \cdot D^2 = 3.976 \cdot \text{in}^2$$

Net Area of Bolt =

$$A_n := \frac{\pi}{4} \cdot \left( D - \frac{0.9743 \cdot \text{in}}{n} \right)^2 = 3.248 \cdot \text{in}^2$$

Net Diameter =

$$D_n := \frac{2 \cdot \sqrt{A_n}}{\sqrt{\pi}} = 2.033 \cdot \text{in}$$

Radius of Gyration of Bolt =

$$r := \frac{D_n}{4} = 0.508 \cdot \text{in}$$

Section Modulus of Bolt =

$$S_x := \frac{\pi \cdot D_n^3}{32} = 0.826 \cdot \text{in}^3$$

Check Anchor Bolt Tension Force:

Maximum Tensile Force =

$$T_{\text{Max}} := \text{OM} \cdot \frac{R_{bc}}{I_p} - \frac{\text{Axial}}{N} = 91 \cdot \text{kips}$$

Allowable Tensile Force =

$$T_{\text{ALL.Gross}} := 1.333 \cdot (0.33 \cdot A_g \cdot F_u) = 174.9 \cdot \text{kips} \quad (1.333 \text{ increase allowed per TIA/EIA})$$

$$T_{\text{ALL.Net}} := 1.333 \cdot (0.60 \cdot A_n \cdot F_y) = 194.812 \cdot \text{kips} \quad (1.333 \text{ increase allowed per TIA/EIA})$$

Bolt Tension % of Capacity =

$$\frac{T_{\text{Max}}}{T_{\text{ALL.Net}}} = 46.7\% \quad \text{Bolts are "upset bolts". Use net area per AISC}$$

Condition1 =

$$\text{Condition1} := \text{if} \left( \frac{T_{\text{Max}}}{T_{\text{ALL.Net}}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

**Condition1 = "OK"**

Check Anchor Bolt Bending Stress:

Maximum Bending Moment =

$$M_x := \left( \frac{\text{Shear}}{N} \right) \cdot l = 0.375 \cdot \text{ft} \cdot \text{kips}$$

Maximum Bending Stress =

$$f_{bx} := \frac{M_x}{S_x} = 5.5 \cdot \text{ksi}$$

Allowable Bending Stress =

$$F_{bx} := 1.333 \cdot 0.6 \cdot F_y = 60 \cdot \text{ksi} \quad (1.333 \text{ increase allowed per TIA/EIA})$$

Check Combined Stress Requirement:

Per ASCE Manual 72: "If the clearance between the base plate and concrete does not exceed two times the bolt diameter a bending stress analysis of the bolts is NOT normally required."

$$l := \begin{cases} l & \text{if } l > 2 \cdot D_n \\ 0 & \text{otherwise} \end{cases} = 0 \text{ in}$$

$$f_{bx} := \begin{cases} f_{bx} & \text{if } l > 2 \cdot D_n \\ 0 & \text{otherwise} \end{cases} = 0 \text{ ksi}$$

Check Anchor Bolt Compression/Combined Stress:

Maximum Compressive Force =

$$C_{Max} := OM \cdot \frac{R_{bc}}{I_p} + \frac{Axial}{N} = 96.4 \text{ kips}$$

Maximum Compressive Stress =

$$f_a := \frac{C_{Max}}{A_n} = 29.7 \text{ ksi}$$

$$K := 0.65$$

$$C_c := \sqrt{\frac{2 \cdot \pi^2 \cdot E}{F_y}} = 87.364$$

$$F_a := \begin{cases} \frac{\left[ 1 - \frac{\left( \frac{K \cdot l}{r} \right)^2}{2 \cdot C_c^2} \right] \cdot F_y}{\frac{5}{3} + \frac{3 \cdot \left( \frac{K \cdot l}{r} \right)}{8 \cdot C_c} - \frac{\left( \frac{K \cdot l}{r} \right)^3}{8 \cdot C_c^3}} & \text{if } \frac{K \cdot l}{r} \leq C_c \\ \frac{12 \cdot \pi^2 \cdot E}{23 \cdot \left( \frac{K \cdot l}{r} \right)^2} & \text{if } \frac{K \cdot l}{r} > C_c \end{cases} = 45 \text{ ksi}$$

Allowable Compressive Stress =

$$F_a := 1.333 \cdot F_a = 60 \text{ ksi} \quad (1.333 \text{ increase allowed per TIA/EIA})$$

Combined Stress % of Capacity =

$$\left( \frac{f_a}{F_a} + \frac{f_{bx}}{F_{bx}} \right) = 49.5 \%$$

Condition 2 =

$$\text{Condition2} := \text{if } \left( \frac{f_a}{F_a} + \frac{f_{bx}}{F_{bx}} \right) \leq 1.00, \text{"OK"}, \text{"Overstressed"}$$

Condition2 = "OK"

**Base Plate Analysis:**

Force from Bolts =  $C_i := \frac{OM \cdot d_i}{I_p} + \frac{Axial}{N}$

$C_1 = 22.2 \cdot \text{kips}$

$C_7 = 95.9 \cdot \text{kips}$

$C_2 = 40.8 \cdot \text{kips}$

$C_8 = 95.9 \cdot \text{kips}$

$C_3 = 57.8 \cdot \text{kips}$

$C_9 = 91.8 \cdot \text{kips}$

$C_4 = 72.4 \cdot \text{kips}$

$C_{10} = 83.9 \cdot \text{kips}$

$C_5 = 83.9 \cdot \text{kips}$

$C_{11} = 72.4 \cdot \text{kips}$

$C_6 = 91.8 \cdot \text{kips}$

etc.

Maximum Bending Stress in Plate =  $f_{bp} := \sum_i \frac{6 \cdot C_i \cdot M A_i}{(B_{eff} t_{bp})^2} = 23.2 \cdot \text{ksi}$

Allowable Bending Stress in Plate =  $F_{bp} := 1.33 \cdot 0.75 \cdot F_y = 59.9 \cdot \text{ksi}$

Plate Bending Stress % of Capacity =  $\frac{f_{bp}}{F_{bp}} = 38.8 \cdot \%$

Condition3 =  $\text{Condition3} := \text{if} \left( \frac{f_{bp}}{F_{bp}} < 1.00, \text{"Ok"}, \text{"Overstressed"} \right)$

Condition3 = "Ok"



**Caisson Foundation:**

Input Data:

Shear Force =	S := 45k	<i>USER INPUT-FROM tnx Tower</i>
Overtuming Moment =	M := 5036ft-k	<i>USER INPUT-FROM tnx Tower</i>
Applied Axial Load =	A1 := 82k	<i>USER INPUT-FROM tnx Tower</i>
Bending Moment =	Mu := 5274ft-k	<i>USER INPUT-FROM LPILE</i>
Moment Capacity =	Mn := 12300ft-k	<i>USER INPUT-FROM LPILE</i>
Foundation Diameter =	d := 9.0ft	<i>USER INPUT</i>
Overall Length of Caisson =	Lc := 28.0ft	<i>USER INPUT</i>
Depth From Top of Caisson to Grade =	Lpag := 1.0ft	<i>USER INPUT</i>
Number of Rebar =	n := 33	<i>USER INPUT</i>
Area of Rebar =	Ar := 1.560in <sup>2</sup>	<i>USER INPUT</i>
Rebar Yield Strength =	fy := 60ksi	<i>USER INPUT</i>
Concrete Comp Strength =	fc := 3ksi	<i>USER INPUT</i>

Check Moment Capacity:

Factor of Safety =	$FS := \frac{0.9 \cdot Mn}{Mu} = 2.1$
Factor of Safety Required =	FS <sub>reqd</sub> := 1.3
	FOSCheck := if(FS ≥ FS <sub>reqd</sub> , "OK", "NO GOOD")
	<b>FOSCheck = "OK"</b>

LPILE Plus for windows, Version 5.0 (5.0.47)

Analysis of Individual Piles and Drilled Shafts  
Subjected to Lateral Loading Using the p-y Method

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This program is licensed to:

TJL  
Centek Engineering

Files Used for Analysis

Path to file locations: J:\Jobs\1500100.WI\145 - Greenwich CT\Backup Documentation\Foundation\  
Name of input data file: Greenwich Hospital Caisson Analysis.lpd  
Name of output file: Greenwich Hospital Caisson Analysis.lpo  
Name of plot output file: Greenwich Hospital Caisson Analysis.lpp  
Name of runtime file: Greenwich Hospital Caisson Analysis.lpr

Time and Date of Analysis

Date: January 21, 2016 Time: 10:16:57

Problem Title

15001.145 - Greenwich

Program Options

Units Used in Computations - US Customary Units: Inches, Pounds

Basic Program Options:

Analysis Type 3:

- Computation of Nonlinear Bending Stiffness and Ultimate Bending Moment Capacity with Pile Response Computed Using Nonlinear EI

Computation Options:

- Only internally-generated p-y curves used in analysis
- Analysis does not use p-y multipliers (individual pile or shaft action only)
- Analysis assumes no shear resistance at pile tip
- Analysis for fixed-length pile or shaft only
- Analysis includes computation of foundation stiffness matrix elements
- Output pile response for full length of pile
- Analysis assumes no soil movements acting on pile
- No additional p-y curves to be computed at user-specified depths

Solution Control Parameters:

- Number of pile increments = 100
- Maximum number of iterations allowed = 100
- Deflection tolerance for convergence = 1.0000E-04 in
- Maximum allowable deflection = 1.0000E+02 in

Printing Options:

- Values of pile-head deflection, bending moment, shear force, and soil reaction are printed for full length of pile.
- Printing Increment (spacing of output points) = 8

Pile Structural Properties and Geometry

Pile Length = 336.00 in  
Depth of ground surface below top of pile = 12.00 in  
slope angle of ground surface = 0.00 deg.  
Structural properties of pile defined using 2 points

Point No.	Point Depth in	Pile Diameter in	Moment of Inertia in**4	Pile Area Sq.in	Modulus of Elasticity lbs/Sq.in
1	0.0000	108.00000	6678285.	9160.9000	3600000.
2	336.0000	108.00000	6678285.	9160.9000	3600000.

Please note that because this analysis makes computations of ultimate moment capacity and pile response using nonlinear bending stiffness that the above values of moment of inertia and modulus of are not used for any computations other than total stress due to combined axial loading and bending.

-----  
Soil and Rock Layering Information  
-----

The soil profile is modelled using 4 layers

Layer 1 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 12.000 in  
 Distance from top of pile to bottom of layer = 48.000 in  
 p-y subgrade modulus k for top of soil layer = 20.000 lbs/in\*\*3  
 p-y subgrade modulus k for bottom of layer = 20.000 lbs/in\*\*3

Layer 2 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 48.000 in  
 Distance from top of pile to bottom of layer = 72.000 in  
 p-y subgrade modulus k for top of soil layer = 90.000 lbs/in\*\*3  
 p-y subgrade modulus k for bottom of layer = 90.000 lbs/in\*\*3

Layer 3 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 72.000 in  
 Distance from top of pile to bottom of layer = 132.000 in  
 p-y subgrade modulus k for top of soil layer = 150.000 lbs/in\*\*3  
 p-y subgrade modulus k for bottom of layer = 150.000 lbs/in\*\*3

Layer 4 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 132.000 in  
 Distance from top of pile to bottom of layer = 360.000 in  
 p-y subgrade modulus k for top of soil layer = 250.000 lbs/in\*\*3  
 p-y subgrade modulus k for bottom of layer = 250.000 lbs/in\*\*3

(Depth of lowest layer extends 24.00 in below pile tip)

-----  
Effective Unit weight of Soil vs. Depth  
-----

Effective unit weight of soil with depth defined using 8 points

Point No.	Depth X in	Eff. Unit weight lbs/in**3
1	12.00	0.05800
2	48.00	0.05800
3	48.00	0.06900
4	72.00	0.06900
5	72.00	0.06900
6	132.00	0.06900
7	132.00	0.07500
8	360.00	0.07500

-----  
shear strength of soils  
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Shear strength parameters with depth defined using 8 points

Point No.	Depth X in	Cohesion c lbs/in**2	Angle of Friction Deg.	E50 or k_rm	RQD %
1	12.000	0.00000	20.00	-----	-----
2	48.000	0.00000	20.00	-----	-----
3	48.000	0.00000	30.00	-----	-----
4	72.000	0.00000	30.00	-----	-----
5	72.000	0.00000	35.00	-----	-----
6	132.000	0.00000	35.00	-----	-----
7	132.000	0.00000	42.00	-----	-----
8	360.000	0.00000	42.00	-----	-----

Notes:

- (1) cohesion = uniaxial compressive strength for rock materials.
- (2) Values of E50 are reported for clay strata.
- (3) Default values will be generated for E50 when input values are 0.
- (4) RQD and k<sub>rm</sub> are reported only for weak rock strata.

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Loading Type

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Static loading criteria was used for computation of p-y curves.

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Pile-head Loading and Pile-head Fixity Conditions

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Number of loads specified = 1

Load Case Number 1

Pile-head boundary conditions are Shear and Moment (BC Type 1)

Shear force at pile head = 45000.000 lbs  
 Bending moment at pile head = 60432000.000 in-lbs  
 Axial load at pile head = 82000.000 lbs

Non-zero moment at pile head for this load case indicates the pile-head may rotate under the applied pile-head loading, but is not a free-head (zero moment) condition.

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Computations of Nominal Moment Capacity and Nonlinear Bending Stiffness

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Number of sections = 1

Pile Section No. 1

The sectional shape is a circular drilled shaft (bored pile).

Outside Diameter = 108.0000 in

Material Properties:

Compressive Strength of Concrete = 3.000 kip/in\*\*2  
 Yield Stress of Reinforcement = 60. kip/in\*\*2  
 Modulus of Elasticity of Reinforcement = 29000. kip/in\*\*2  
 Number of Reinforcing Bars = 33  
 Area of Single Bar = 1.56000 in\*\*2  
 Number of Rows of Reinforcing Bars = 33  
 Area of Steel = 51.480 in\*\*2  
 Area of Shaft = 9160.884 in\*\*2  
 Percentage of Steel Reinforcement = 0.562 percent  
 Cover Thickness (edge to bar center) = 4.000 in

Unfactored Axial Squash Load Capacity = 26317.78 kip

Distribution and Area of Steel Reinforcement

Row Number	Area of Reinforcement in**2	Distance to Centroidal Axis in
1	1.560	49.943
2	1.560	49.491
3	1.560	48.591
4	1.560	47.250
5	1.560	45.482
6	1.560	43.301
7	1.560	40.729
8	1.560	37.787
9	1.560	34.504
10	1.560	30.908
11	1.560	27.032
12	1.560	22.911
13	1.560	18.583
14	1.560	14.087
15	1.560	9.463
16	1.560	4.753
17	1.560	0.000
18	1.560	-4.753

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19	1.560	-9.463
20	1.560	-14.087
21	1.560	-18.583
22	1.560	-22.911
23	1.560	-27.032
24	1.560	-30.908
25	1.560	-34.504
26	1.560	-37.787
27	1.560	-40.729
28	1.560	-43.301
29	1.560	-45.482
30	1.560	-47.250
31	1.560	-48.591
32	1.560	-49.491
33	1.560	-49.943

Axial Thrust Force = 82000.00 lbs

Bending Moment in-lbs	Bending Stiffness lb-in <sup>2</sup>	Bending Curvature rad/in	Maximum Strain in/in	Neutral Axis Position inches	Max. Concrete Stress psi	Max. Steel Stress psi
11414231.	2.282846E+13	5.000000E-07	0.00002982	59.63957995	91.65855472	805.95273
22713346.	2.271335E+13	0.00000100	0.00005695	56.94726223	173.46083	1533.82825
33889228.	2.259282E+13	0.00000150	0.00008404	56.02467781	253.76626	2260.60995
44950013.	2.247501E+13	0.00000200	0.00011120	55.60135764	332.91450	2989.59403
55888709.	2.235548E+13	0.00000250	0.00013831	55.32455367	410.51012	3716.92425
55888709.	1.862957E+13	0.00000300	0.00008475	28.25072104	251.86855	6585.26020
55888709.	1.596820E+13	0.00000350	0.00009698	27.70787948	286.94015	7737.90198
55888709.	1.397218E+13	0.00000400	0.00010922	27.30433148	321.78427	8890.12812
55888709.	1.241971E+13	0.00000450	0.00012150	26.99999839	356.48342	10041.10960
55888709.	1.117774E+13	0.00000500	0.00013401	26.80165869	391.56411	11185.54770
55888709.	1.016158E+13	0.00000550	0.00014623	26.58701652	425.53418	12338.33789
55888709.	9.314785E+12	0.00000600	0.00015846	26.41074711	459.27928	13490.67585
55888709.	8.598263E+12	0.00000650	0.00017072	26.26400882	492.79859	14642.55901
55888709.	7.984101E+12	0.00000700	0.00018298	26.14049309	526.09142	15793.98339
55888709.	7.451828E+12	0.00000750	0.00019527	26.03556508	559.15687	16944.94691
55888709.	6.986089E+12	0.00000800	0.00020757	25.94575828	591.99425	18095.44521
55888709.	6.575142E+12	0.00000850	0.00021988	25.86841410	624.60270	19245.47588
55888709.	6.209857E+12	0.00000900	0.00023221	25.80146939	656.98144	20395.03527
55888709.	5.883022E+12	0.00000950	0.00024456	25.74329549	689.12967	21544.11969
55888709.	5.588871E+12	0.00001000	0.00025693	25.69258565	721.04650	22692.72658
55888709.	5.322734E+12	0.00001050	0.00026931	25.64828736	752.73117	23840.85174
55888709.	5.080792E+12	0.00001100	0.00028170	25.60953480	784.18276	24988.49246
55888709.	4.859888E+12	0.00001150	0.00029412	25.57561988	815.40057	26135.64365
55888709.	4.657392E+12	0.00001200	0.00030655	25.54594392	846.38360	27282.30322
55888709.	4.471097E+12	0.00001250	0.00031900	25.52000803	877.13098	28428.46761
55888709.	4.299131E+12	0.00001300	0.00033147	25.49739379	907.64196	29574.13189
55888709.	4.139904E+12	0.00001350	0.00034395	25.47773749	937.91549	30719.29394
56875893.	4.062564E+12	0.00001400	0.00035645	25.46073657	967.95086	31863.94794
58786948.	4.054272E+12	0.00001450	0.00036897	25.44612068	997.74700	33008.09206
60695811.	4.046387E+12	0.00001500	0.00038150	25.43366128	1027.30302	34151.72197
62602478.	4.038870E+12	0.00001550	0.00039406	25.42316204	1056.61816	35294.83211
64506932.	4.031683E+12	0.00001600	0.00040663	25.41444594	1085.69138	36437.41935
66409145.	4.024797E+12	0.00001650	0.00041922	25.40735525	1114.52159	37579.48160
68309125.	4.018184E+12	0.00001700	0.00043183	25.40176123	1143.10810	38721.01162
70206839.	4.011819E+12	0.00001750	0.00044446	25.39753836	1171.44970	39862.00801
72102285.	4.005682E+12	0.00001800	0.00045710	25.39458364	1199.54557	41002.46489
73995441.	3.999754E+12	0.00001850	0.00046977	25.39280051	1227.39463	42142.37890
75886308.	3.994016E+12	0.00001900	0.00048245	25.39210850	1254.99607	43281.74341
77774844.	3.988454E+12	0.00001950	0.00049515	25.39242393	1282.34854	44420.55828
81544940.	3.977802E+12	0.00002050	0.00052061	25.39583248	1336.30340	46696.50925
85305581.	3.967701E+12	0.00002150	0.00054615	25.40254980	1389.25057	48970.19950
89056656.	3.958074E+12	0.00002250	0.00057177	25.41219610	1441.18170	51241.58899
92798045.	3.948853E+12	0.00002350	0.00059747	25.42445594	1492.08815	53510.63786
96529602.	3.939984E+12	0.00002450	0.00062326	25.43906218	1541.96080	55777.30855
1.002512E+08	3.931420E+12	0.00002550	0.00064912	25.45579916	1590.79072	58041.55639
1.039252E+08	3.921707E+12	0.00002650	0.00067499	25.47129697	1638.40962	60000.00000
1.068280E+08	3.884653E+12	0.00002750	0.00069920	25.42557603	1681.78208	60000.00000
1.092595E+08	3.833668E+12	0.00002850	0.00072239	25.34706026	1722.25728	60000.00000
1.113800E+08	3.775595E+12	0.00002950	0.00074489	25.25046533	1760.55709	60000.00000
1.135610E+08	3.723312E+12	0.00003050	0.00076860	25.20000011	1800.02423	60000.00000
1.150016E+08	3.650844E+12	0.00003150	0.00079085	25.10622472	1836.03974	60000.00000
1.165715E+08	3.586814E+12	0.00003250	0.00081179	24.97807735	1869.04745	60000.00000
1.179405E+08	3.520611E+12	0.00003350	0.00083216	24.84045750	1900.36246	60000.00000
1.192686E+08	3.457062E+12	0.00003450	0.00085245	24.70859259	1930.82137	60000.00000
1.204244E+08	3.392236E+12	0.00003550	0.00087221	24.56940204	1959.75559	60000.00000
1.215767E+08	3.330868E+12	0.00003650	0.00089202	24.43896621	1988.05836	60000.00000
1.225621E+08	3.268322E+12	0.00003750	0.00091128	24.30086035	2014.87474	60000.00000
1.235350E+08	3.208702E+12	0.00003850	0.00093055	24.17004794	2041.04198	60000.00000
1.244538E+08	3.150728E+12	0.00003950	0.00094965	24.04180080	2066.33346	60000.00000
1.252674E+08	3.093022E+12	0.00004050	0.00096839	23.91095620	2090.49701	60000.00000
1.260782E+08	3.038029E+12	0.00004150	0.00098717	23.78728598	2114.08833	60000.00000
1.268617E+08	2.984982E+12	0.00004250	0.01005588	23.66784507	2136.97232	60000.00000
1.275335E+08	2.931805E+12	0.00004350	0.01024216	23.54402035	2158.70955	60000.00000
1.282027E+08	2.880959E+12	0.00004450	0.01042248	23.42652673	2179.90011	60000.00000
1.288288E+08	2.831402E+12	0.00004550	0.01064470	23.39999861	2205.03779	60000.00000

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1.295860E+08	2.786796E+12	0.00004650	0.00108339	23.29865295	2225.33439	60000.00000
1.301197E+08	2.739362E+12	0.00004750	0.00110067	23.17198938	2243.48313	60000.00000
1.306512E+08	2.693840E+12	0.00004850	0.00111798	23.05120307	2261.14354	60000.00000
1.311807E+08	2.650114E+12	0.00004950	0.00113533	22.93593997	2278.31270	60000.00000
1.317079E+08	2.608078E+12	0.00005050	0.00115271	22.82587820	2294.98787	60000.00000
1.321707E+08	2.566421E+12	0.00005150	0.00116975	22.71357304	2310.81132	60000.00000
1.325956E+08	2.525631E+12	0.00005250	0.00118661	22.60209507	2325.95890	60000.00000
1.330187E+08	2.486330E+12	0.00005350	0.00120350	22.49534529	2340.63809	60000.00000
1.334397E+08	2.448435E+12	0.00005450	0.00122042	22.39306945	2354.84639	60000.00000
1.338588E+08	2.411870E+12	0.00005550	0.00123737	22.29503256	2368.58128	60000.00000
1.342759E+08	2.376564E+12	0.00005650	0.00125436	22.20101255	2381.84002	60000.00000
1.346177E+08	2.341177E+12	0.00005750	0.00127088	22.10225147	2394.23735	60000.00000
1.349504E+08	2.306844E+12	0.00005850	0.00128738	22.00652236	2406.14496	60000.00000
1.352813E+08	2.273635E+12	0.00005950	0.00130391	21.91450435	2417.60020	60000.00000
1.359376E+08	2.210367E+12	0.00006150	0.00133706	21.74087745	2439.14289	60000.00000
1.367504E+08	2.153550E+12	0.00006350	0.00137160	21.60000032	2459.59723	60000.00000
1.372406E+08	2.095277E+12	0.00006550	0.00141056	21.53523463	2480.28893	60000.00000
1.377220E+08	2.040326E+12	0.00006750	0.00144166	21.35798353	2494.71419	60000.00000
1.381974E+08	1.988452E+12	0.00006950	0.00147288	21.19253522	2507.51437	60000.00000
1.386668E+08	1.939396E+12	0.00007150	0.00150421	21.03792411	2518.67100	60000.00000
1.391068E+08	1.892610E+12	0.00007350	0.00153541	20.88996917	2528.09393	60000.00000
1.394605E+08	1.847159E+12	0.00007550	0.00156588	20.74014419	2535.66310	60000.00000
1.398086E+08	1.803982E+12	0.00007750	0.00159646	20.59945697	2541.65350	60000.00000
1.401511E+08	1.762906E+12	0.00007950	0.00162715	20.46723479	2546.04701	60000.00000
1.404878E+08	1.723776E+12	0.00008150	0.00165794	20.34287900	2548.82519	60000.00000
1.408186E+08	1.686451E+12	0.00008350	0.00168886	20.22584242	2549.96907	60000.00000
1.411115E+08	1.650427E+12	0.00008550	0.00171956	20.11183137	2545.92056	60000.00000
1.413481E+08	1.615407E+12	0.00008750	0.00174971	19.99673241	2540.94734	60000.00000
1.415822E+08	1.581924E+12	0.00008950	0.00177998	19.88801926	2543.61589	60000.00000
1.415822E+08	1.547347E+12	0.00009150	0.00181170	19.79999882	2547.09008	60000.00000
1.417490E+08	1.516032E+12	0.00009350	0.00185130	19.79999882	2549.59126	60000.00000
1.423932E+08	1.491028E+12	0.00009550	0.00188574	19.74594158	2548.50327	60000.00000
1.425996E+08	1.462560E+12	0.00009750	0.00191516	19.64267439	2544.28942	60000.00000
1.428046E+08	1.435222E+12	0.00009950	0.00194467	19.54444760	2540.06005	60000.00000
1.429989E+08	1.408856E+12	0.00010150	0.00197410	19.44922704	2539.88153	60000.00000
1.431431E+08	1.383026E+12	0.00010350	0.00200266	19.34940702	2543.39111	60000.00000
1.432862E+08	1.358163E+12	0.00010550	0.00203131	19.25416070	2546.16623	60000.00000
1.434280E+08	1.334214E+12	0.00010750	0.00206005	19.16324347	2548.19745	60000.00000
1.435686E+08	1.311129E+12	0.00010950	0.00208887	19.07643324	2549.47513	60000.00000
1.437079E+08	1.288860E+12	0.00011150	0.00211778	18.99351436	2549.98931	60000.00000
1.438438E+08	1.267346E+12	0.00011350	0.00214688	18.91523033	2547.05771	60000.00000
1.439784E+08	1.246567E+12	0.00011550	0.00217606	18.84038061	2543.51707	60000.00000
1.441123E+08	1.226488E+12	0.00011750	0.00220531	18.76859826	2539.96573	60000.00000
1.442455E+08	1.207075E+12	0.00011950	0.00223462	18.69973522	2536.40355	60000.00000
1.443779E+08	1.188295E+12	0.00012150	0.00226399	18.63364989	2535.80622	60000.00000
1.444985E+08	1.170028E+12	0.00012350	0.00229314	18.56794757	2539.42921	60000.00000
1.445901E+08	1.152112E+12	0.00012550	0.00232163	18.49907166	2542.40611	60000.00000
1.446811E+08	1.134754E+12	0.00012750	0.00235018	18.43281251	2544.90002	60000.00000
1.447714E+08	1.117926E+12	0.00012950	0.00237879	18.36905748	2546.90564	60000.00000
1.448611E+08	1.101606E+12	0.00013150	0.00240746	18.30769068	2548.41744	60000.00000
1.449502E+08	1.085769E+12	0.00013350	0.00243619	18.24861556	2549.42994	60000.00000
1.450386E+08	1.070395E+12	0.00013550	0.00246498	18.19173557	2549.93747	60000.00000
1.451252E+08	1.055456E+12	0.00013750	0.00249390	18.13746268	2548.54472	60000.00000
1.452105E+08	1.040935E+12	0.00013950	0.00252293	18.08554584	2545.66235	60000.00000
1.455127E+08	1.014026E+12	0.00014350	0.00258300	18.00000054	2539.54654	60000.00000
1.466286E+08	9.940921E+11	0.00014750	0.00265500	18.00000054	2531.33751	60000.00000
1.476109E+08	9.743296E+11	0.00015150	0.00272700	18.00000054	2536.11274	60000.00000
1.476109E+08	9.492664E+11	0.00015550	0.00279250	17.95818383	2542.73992	60000.00000
1.476109E+08	9.254604E+11	0.00015950	0.00284997	17.86811954	2546.34139	60000.00000
1.476109E+08	9.028191E+11	0.00016350	0.00290656	17.77710575	2548.65823	60000.00000
1.476109E+08	8.812593E+11	0.00016750	0.00296333	17.69154435	2549.84149	60000.00000
1.476109E+08	8.607051E+11	0.00017150	0.00302149	17.61803681	2547.72168	60000.00000
1.476109E+08	8.410879E+11	0.00017550	0.00308050	17.55268532	2543.05534	60000.00000
1.476109E+08	8.223450E+11	0.00017950	0.00313962	17.49090654	2538.36822	60000.00000
1.476109E+08	8.044192E+11	0.00018350	0.00319886	17.43248802	2533.65963	60000.00000
1.476109E+08	7.872583E+11	0.00018750	0.00325823	17.37722701	2528.92917	60000.00000
1.476109E+08	7.708143E+11	0.00019150	0.00331773	17.32494324	2524.17626	60000.00000
1.476109E+08	7.550431E+11	0.00019550	0.00337735	17.27546936	2519.40035	60000.00000
1.476109E+08	7.399044E+11	0.00019950	0.00343813	17.23374599	2526.25270	60000.00000
1.476109E+08	7.253608E+11	0.00020350	0.00349961	17.19710487	2532.43665	60000.00000
1.476109E+08	7.113780E+11	0.00020750	0.00356127	17.16274256	2537.72896	60000.00000
1.476109E+08	6.979240E+11	0.00021150	0.00362252	17.12773973	2542.00275	60000.00000
1.476109E+08	6.849695E+11	0.00021550	0.00368217	17.08663756	2545.14857	60000.00000
1.476109E+08	6.724872E+11	0.00021950	0.00374198	17.04774660	2547.52767	60000.00000
1.476109E+08	6.604516E+11	0.00022350	0.00380195	17.01097029	2549.12186	60000.00000

Unfactored (Nominal) Moment Capacity at Concrete Strain of 0.003 = 147610.93014 in-kip

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 Computed Values of Load Distribution and Deflection  
 for Lateral Loading for Load Case Number 1  
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Pile-head boundary conditions are Shear and Moment (Pile-head Condition Type 1)  
 Specified shear force at pile head = 45000.000 lbs

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Specified moment at pile head = 60432000.000 in-lbs  
 Specified axial load at pile head = 82000.000 lbs

Depth X in	Deflect. y in	Moment M lbs-in	Shear V lbs	Slope S Rad.	Total Stress lbs/in**2	Flx. Rig. EI lbs-in**2	Soil Res. p lbs/in	Es*h F/L
0.000	0.571582	6.04E+07	45000.	-0.003989	497.599	4.05E+12	0.000	0.000
26.880	0.469800	6.16E+07	43907.	-0.003583	507.403	4.04E+12	-139.813	999.936
53.760	0.379034	6.28E+07	35076.	-0.003169	516.364	4.04E+12	-866.442	7680.688
80.640	0.299487	6.33E+07	-332.256	-0.002749	520.711	4.04E+12	-1876.334	21051.
107.520	0.231249	6.25E+07	-63748.	-0.002329	514.244	4.04E+12	-2707.553	39340.
134.400	0.174159	5.98E+07	-1.39E+05	-0.001922	492.409	4.05E+12	-3670.431	70813.
161.280	0.127135	5.47E+07	-2.37E+05	-0.001652	451.483	2.24E+13	-3533.735	93392.
188.160	0.083582	4.71E+07	-3.24E+05	-0.001590	390.165	2.24E+13	-2884.861	1.16E+05
215.040	0.041542	3.75E+07	-3.87E+05	-0.001540	312.242	2.25E+13	-1712.998	1.39E+05
241.920	0.000701	2.67E+07	-4.12E+05	-0.001501	224.553	2.27E+13	-33.609	1.61E+05
268.800	-0.039290	1.58E+07	-3.85E+05	-0.001476	136.908	2.28E+13	2148.191	1.84E+05
295.680	-0.078774	6.57E+06	-2.92E+05	-0.001463	62.055	2.28E+13	4836.373	2.06E+05
322.560	-0.118042	8.35E+05	-1.20E+05	-0.001459	15.705	2.28E+13	8040.443	2.29E+05

Please note that because this analysis makes computations of ultimate moment capacity and pile response using nonlinear bending stiffness that the above values of total stress due to combined axial stress and bending may not be representative of actual conditions.

Output Verification:

Computed forces and moments are within specified convergence limits.

Output Summary for Load Case No. 1:

Pile-head deflection = 0.57158216 in  
 Computed slope at pile head = -0.00398861  
 Maximum bending moment = 63290392. lbs-in  
 Maximum shear force = -411984.10450 lbs  
 Depth of maximum bending moment = 80.64000000 in  
 Depth of maximum shear force = 241.92000 in  
 Number of iterations = 58  
 Number of zero deflection points = 1

Summary of Pile Response(s)

Definition of Symbols for Pile-Head Loading Conditions:

Type 1 = Shear and Moment, y = pile-head displacement in  
 Type 2 = Shear and Slope, M = Pile-head Moment lbs-in  
 Type 3 = Shear and Rot. Stiffness, V = Pile-head Shear Force lbs  
 Type 4 = Deflection and Moment, S = Pile-head Slope, radians  
 Type 5 = Deflection and Slope, R = Rot. Stiffness of Pile-head in-lbs/rad

Load Type	Pile-Head Condition 1	Pile-Head Condition 2	Axial Load lbs	Pile-Head Deflection in	Maximum Moment in-lbs	Maximum Shear lbs	
1	V=	45000.	M= 6.04E+07	82000.0000	0.5715822	6.3290E+07	-411984.

Computed Pile-head Stiffness Matrix Members  
 K22, K23, K32, K33 for Superstructure

Top y in	Shear React. lbs	Mom. React. in-lbs	K22 lbs/in	K32 in-lbs/in
0.00116275	4500.00007	899596.23772	3870140.	7.736808E+08
0.00350022	13546.34980	2708054.	3870140.	7.736808E+08
0.00554772	21470.45646	4292165.	3870140.	7.736808E+08
0.00700044	27092.69961	5416109.	3870140.	7.736808E+08
0.00812726	31453.65019	6287908.	3870140.	7.736808E+08
0.00904794	35016.80627	7000219.	3870140.	7.736808E+08
0.00982637	38029.41180	7602470.	3870140.	7.736808E+08
0.01050067	40639.04941	8124163.	3870140.	7.736808E+08
0.01109544	42940.91292	8584330.	3870140.	7.736808E+08
0.01162749	45000.00000	8995962.	3870140.	7.736808E+08
Top Rota. rad	Shear React. lbs	Mom. React. in-lbs	K23 lbs/rad	K33 in-lbs/rad

Greenwich Hospital Caisson Analysis.lpo

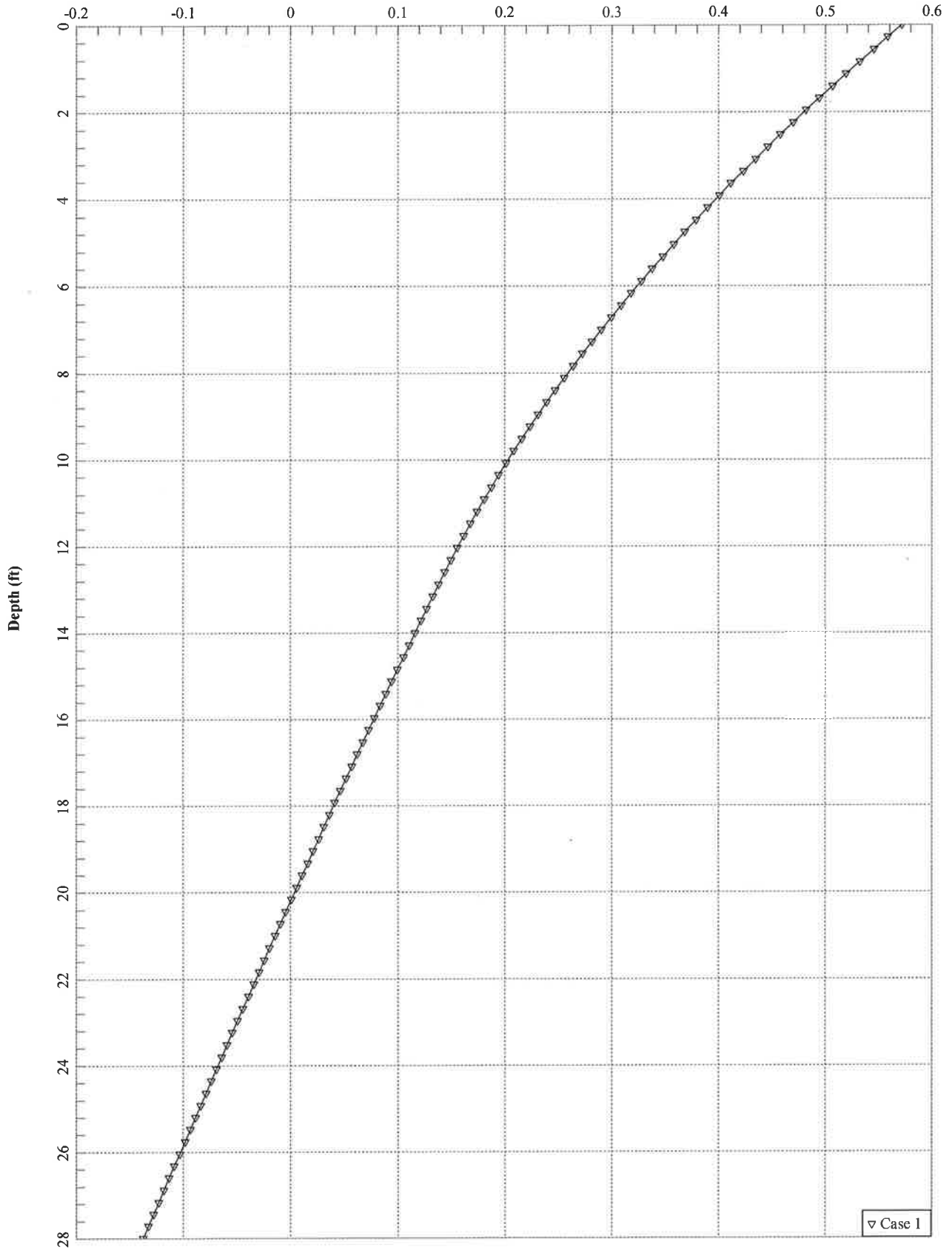
0.00003215	24877.33942	6043200.	7.736808E+08	1.879424E+11
0.00009691	74891.06556	18191845.	7.727510E+08	1.877095E+11
0.00015395	118714.53980	28833392.	7.711441E+08	1.872955E+11
0.00019459	149816.69475	36383689.	7.699161E+08	1.869777E+11
0.00022622	173946.54184	42240155.	7.689139E+08	1.867185E+11
0.00025213	193665.42128	47025236.	7.681203E+08	1.865126E+11
0.00027409	210339.85212	51070965.	7.674192E+08	1.863310E+11
0.00029314	224785.69507	54575534.	7.668133E+08	1.861740E+11
0.00031108	237532.29042	57666783.	7.635691E+08	1.853751E+11
0.00033240	248959.55632	60432000.	7.489795E+08	1.818060E+11

K22 = abs(Shear Reaction/Top y)  
 K23 = abs(Shear Reaction/Top Rotation)  
 K32 = abs(Moment Reaction/Top y)  
 K33 = abs(Moment Reaction/Top Rotation)

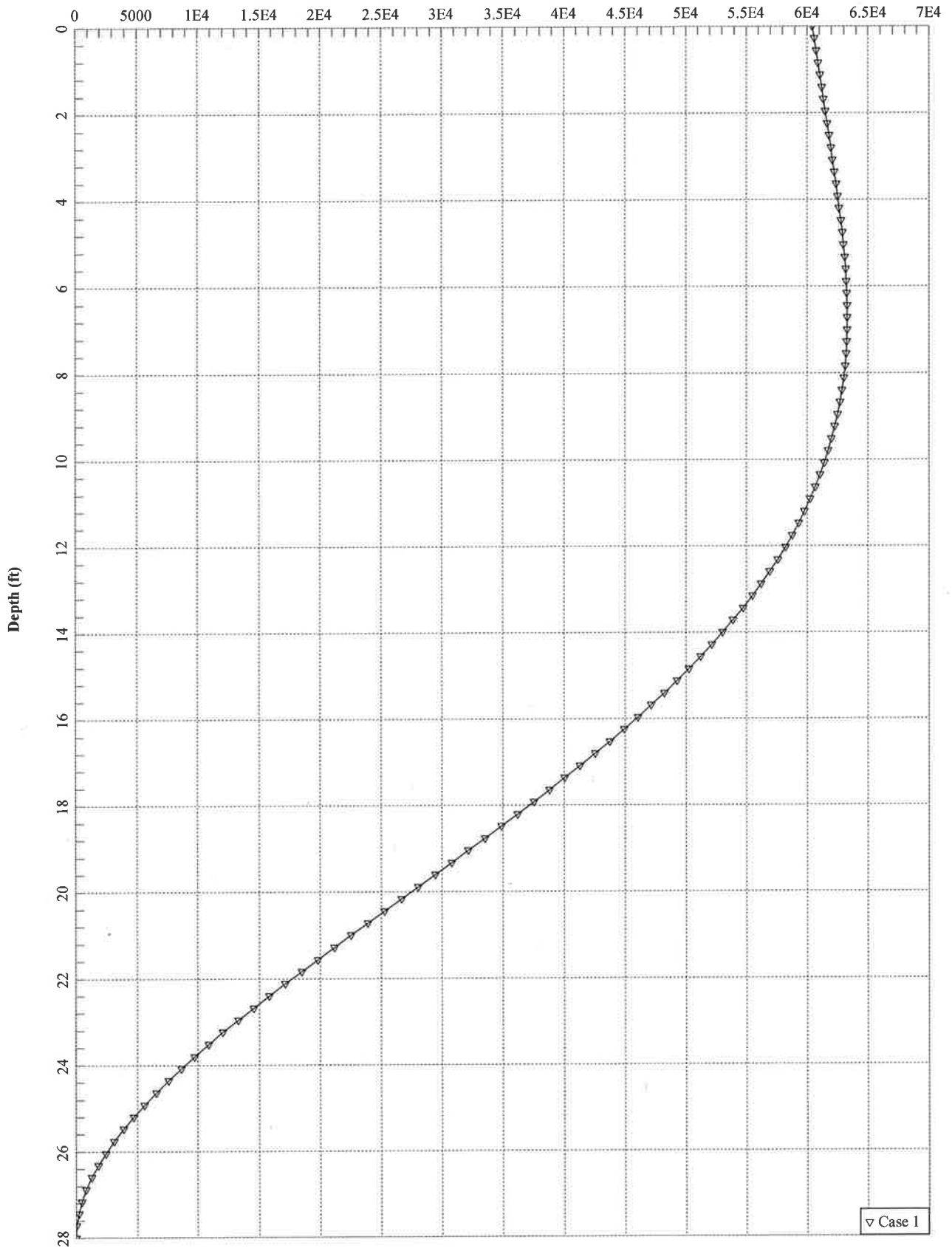
The analysis ended normally.



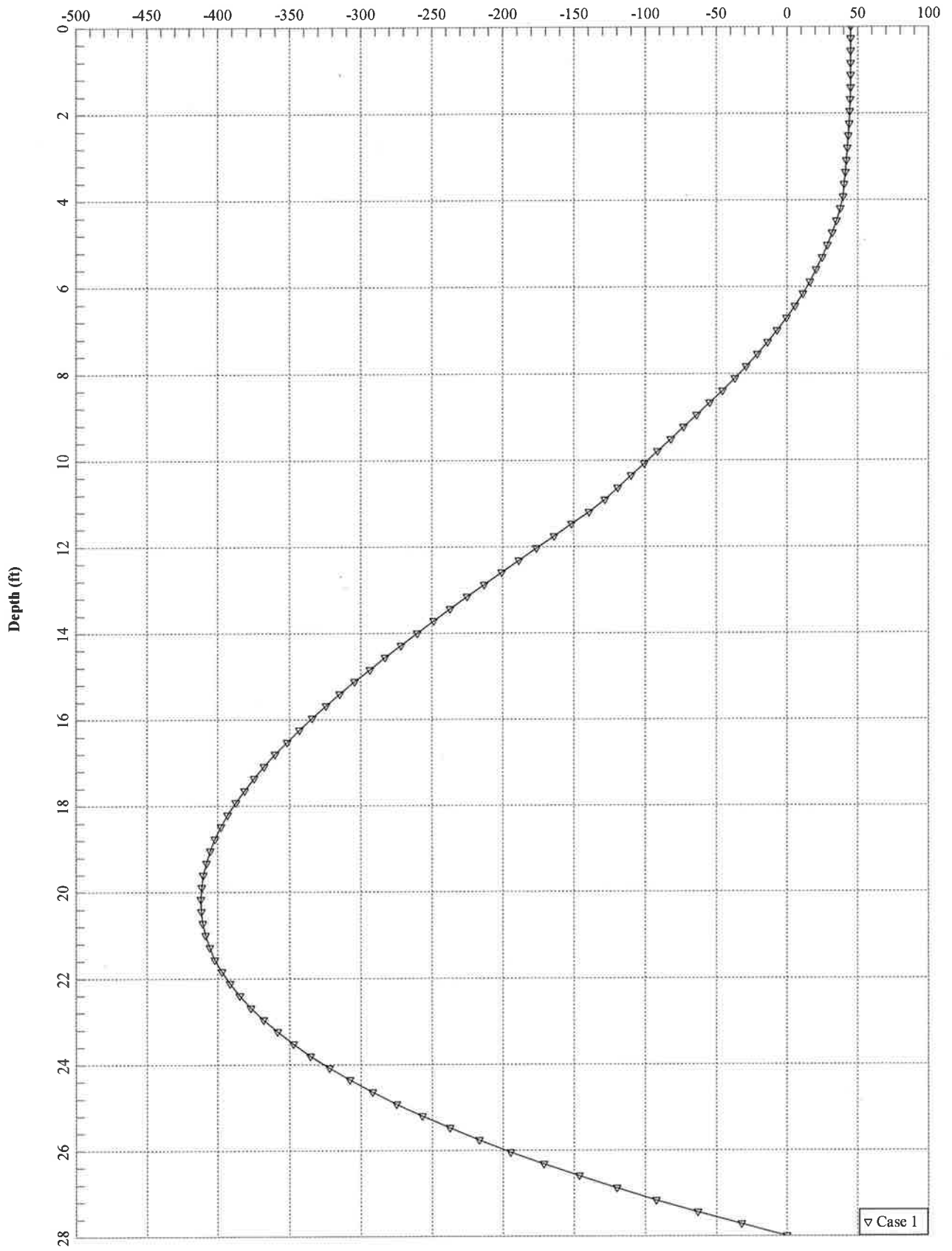
Lateral Deflection (in)



Bending Moment (in-kips)



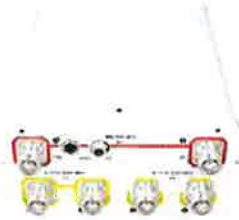
Shear Force (kips)



SITE NAME	GREENWICH CT		ECP - CELL #	5	89
LATITUDE	41-02-02.17 N		LONGITUDE	73-37-51 W	
ANTMO Notes: Swap 40W RRH's for 60W. Replace existing antennas w/ HEX-Port antennas. Lease and Zone for PCS LTE (60W RRH's).			SAVE BUTTON	0009	
			STRUCTURE TYPE	MONOPOLE	
<b>700 MHz - Existing Config</b>	<b>ALPHA</b>		<b>BETA</b>		<b>GAMMA</b>
EQUIPMENT TYPE	ALU 700 MHz RRH		ALU 700 MHz RRH		ALU 700 MHz RRH
ANTENNA TYPE	LNX-6514DS-T4M		LNX-6514DS-T4M		LNX-6514DS-T4M
QTY OF ANTENNAS PER FACE	1		1		1
ORIENTATION (DEG)	30		160		270
DOWN TILT (ELEC + MECH)	4 Elec + 2 Mech		4 Elec + 5 Mech		4 Elec + 4 Mech
RAD CTR (FT AGL)	124		124		124
TMA - QTY / MODEL					
DIPLEXER - QTY / MODEL					
RRH - QTY/MODEL	1	ALU RH_2X40-700U	1	ALU RH_2X40-700U	1 ALU RH_2X40-700U
SECTOR DISTRIBUTION BOX					
MAIN DISTRIBUTION BOX					
<b>700 MHz - LTE Future Config</b>	<b>ALPHA</b>		<b>BETA</b>		<b>GAMMA</b>
EQUIPMENT TYPE	ALU 700 MHz RRH		ALU 700 MHz RRH		ALU 700 MHz RRH
ANTENNA TYPE	SBNHH-1D65B		SBNHH-1D65B		SBNHH-1D65B
QTY OF ANTENNAS PER FACE	1		1		1
ORIENTATION (DEG)	30		160		270
DOWN TILT (ELEC + MECH)	4 Elec + 0 Mech		8 Elec + 0 Mech		4 Elec + 0 Mech
RAD CTR (FT AGL)	124		124		124
TMA - QTY / MODEL					
DIPLEXER - QTY / MODEL					
RRH - QTY/MODEL	1	ALU RH_2X60-700U	1	ALU RH_2X60-700U	1 ALU RH_2X60-700U
SECTOR DISTRIBUTION BOX					
MAIN DISTRIBUTION BOX					
<b>1900 PCS - Current Config</b>	<b>ALPHA</b>		<b>BETA</b>		<b>GAMMA</b>
EQUIPMENT TYPE	PCS Modcell 4.0		PCS Modcell 4.0		PCS Modcell 4.0
ANTENNA TYPE	MG D3-800T0		MG D3-800T0		MG D3-800T0
QTY OF ANTENNAS PER FACE	1		1		1
ORIENTATION (DEG)	30		150		270
DOWN TILT (ELEC + MECH)	0 Elec + 0 Mech		0 Elec + 0 Mech		0 Elec + 0 Mech
RAD CTR (FT AGL)	124		124		124
TMA - QTY / MODEL					
DIPLEXER - QTY / MODEL	2		2		2
RRH - QTY/MODEL					
SECTOR DISTRIBUTION BOX					
MAIN DISTRIBUTION BOX					
<b>1900 PCS - Future Config</b>	<b>ALPHA</b>		<b>BETA</b>		<b>GAMMA</b>
EQUIPMENT TYPE	ALU 1900 MHz RRH		ALU 1900 MHz RRH		ALU 1900 MHz RRH
ANTENNA TYPE	SBNHH-1D65B		SBNHH-1D65B		SBNHH-1D65B
QTY OF ANTENNAS PER FACE	0 (shared w/ LTE 700)		0 (shared w/ LTE 700)		0 (shared w/ LTE 700)
ORIENTATION (DEG)	30		160		270
DOWN TILT (ELEC + MECH)	2 Elec + 0 Mech		2 Elec + 0 Mech		2 Elec + 0 Mech
RAD CTR (FT AGL)	124		124		124
TMA - QTY / MODEL					
DIPLEXER - QTY / MODEL					
RRH - QTY/MODEL	1	ALU RH_2X60-PCS	1	ALU RH_2X60-PCS	1 ALU RH_2X60-PCS
SECTOR DISTRIBUTION BOX					
MAIN DISTRIBUTION BOX	1		DB-T1-6Z-8AB-0Z		
<b>2100 Mhz - Current Config</b>	<b>ALPHA</b>		<b>BETA</b>		<b>GAMMA</b>
EQUIPMENT TYPE	ALU 2100 MHz RRH		ALU 2100 MHz RRH		ALU 2100 MHz RRH
ANTENNA TYPE	MG D3-800T0		MG D3-800T0		MG D3-800T0
QTY OF ANTENNAS PER FACE	1		1		1
ORIENTATION (DEG)	30		150		270
DOWN TILT ( MECH/DEG )	0		0		0
RAD CTR (FT AGL)	124		124		124
TMA - QTY / MODEL					
DIPLEXER - QTY / MODEL					
RRH - QTY/MODEL	1	ALU RH_2X40-AWS	1	ALU RH_2X40-AWS	1 ALU RH_2X40-AWS
SECTOR DISTRIBUTION BOX					
MAIN DISTRIBUTION BOX	1		DB-T1-6Z-8AB-0Z		
<b>2100 Mhz - Current Config</b>	<b>ALPHA</b>		<b>BETA</b>		<b>GAMMA</b>
EQUIPMENT TYPE	ALU 2100 MHz RRH		ALU 2100 MHz RRH		ALU 2100 MHz RRH
ANTENNA TYPE	SBNHH-1D65B		SBNHH-1D65B		SBNHH-1D65B
QTY OF ANTENNAS PER FACE	1		1		1
ORIENTATION (DEG)	30		160		270
DOWN TILT ( MECH/DEG )	2 Elec + 0 Mech		2 Elec + 0 Mech		2 Elec + 0 Mech
RAD CTR (FT AGL)	124		124		124
TMA - QTY / MODEL					
DIPLEXER - QTY / MODEL					
RRH - QTY/MODEL	1	ALU RH_2X60-AWS	1	ALU RH_2X60-AWS	1 ALU RH_2X60-AWS
SECTOR DISTRIBUTION BOX					
MAIN DISTRIBUTION BOX	1		DB-T1-6Z-8AB-0Z		

850 Cellular - No Change		ALPHA				BETA				GAMMA					
EQUIPMENT TYPE		Cellular Modcell 4.0HD				Cellular Modcell 4.0HD				Cellular Modcell 4.0HD					
ANTENNA TYPE		DB844G65A-XY				DB844G65A-XY				DB844G65A-XY					
QTY OF ANTENNAS PER FACE		2				2				2					
ORIENTATION (DEG)		30				150				270					
DOWN TILT ( MECH/DEG )		0				0				0					
RAD CTR ( FT AGL)		124				124				124					
TMA - QTY / MODEL															
DIPLEXER - QTY / MODEL		2				2				2					
DIPLEXER KIT - QTY / MODEL															
<b>NUMBER OF CABLE'S NEEDED</b>						<b>ESTIMATED CABLE LENGTH</b>									
MAINLINE SIZE		1 5/8"		TOTAL # OF MAINLINES		6		MAINLINE (FT)							
JUMPER SIZE		1/2 "		TOTAL # OF TOP JUMPERS		12		TOP JUMPER (FT)		12					
<b>Equipment Cable Ordering</b>		<b>MAIN CABLE</b>						<b>TOP JUMPER #</b>		12 + 0					
FIBER LINE SIZE		1 5/8"		TOTAL # OF FIBER LINES		2		FIBER LINE MODEL #		HB158-1-08U8-S8J18					
JUMPER SIZE		5/8"		TOTAL # OF TOP JUMPERS		9		TOP JUMPER MODEL #		HB058-1-08U1-S1J18					
<b>Fiber Cable Ordering</b>		<b>FIBER CABLE</b>		1 + 1		1		<b>TOP JUMPER #</b>		3 + 3					
<b>TX / RX FREQUENCIES</b>						<b>TX POWER OUTPUT</b>									
<b>Cellular A-Band</b>				<b>PCS F-Band</b>				<b>700 Mhz C - B</b>				Cellular (Watts)		20	
TX - 869-880,890-891.5 MHz				TX - 1970-1975				TX - 746-757				PCS LTE (Watts)		60	
RX - 824-835,845-846.5 MHz				RX - 1890-1895				RX - 776-787				LTE 700 & AWS (Watts)		60	
<b>ALPHA</b>				<b>BETA</b>				<b>GAMMA</b>							
Ant	Freq	Func.	Color Code	Ant	Freq	Func.	Color Code	Ant	Freq	Func.	Color Code				
A1	800	Tx1/Rx0	RED	A7	800	Tx2/Rx0	BLUE	A13	800	Tx3/Rx0	GREEN				
A2	1900	Tx1/Rx0	RED/	A8	1900	Tx2/Rx0	BLUE/ WHITE	A14	1900	Tx3/Rx0	GREEN/WHITE				
A3	700	Tx1/Rx0	RED/	A9	700	Tx2/Rx0	BLUE/ ORANGE	A15	700	Tx3/Rx0	GREEN/ORANGE				
A4	700	Tx4/Rx1	RED/RED/ ORANGE	A10	700	Tx5/Rx1	BLUE/BLUE/ ORANGE	A16	700	Tx6/Rx1	GREEN/GREEN/ ORANGE				
A5	1900	Tx4/Rx1	RED/RED/ WHITE	A11	1900	Tx5/Rx1	BLUE/BLUE/ WHITE	A17	1900	Tx6/Rx1	GREEN/GREEN/ WHITE				
A6	800	Tx4/Rx1	RED/RED	A12	800	Tx5/Rx1	BLUE/BLUE	A18	800	Tx6/Rx1	GREEN/GREEN				
F1-A	1700	Tx/Rx	RED/	F1-B	1700	Tx/Rx	BLUE/BROWN	F1-C	1700	Tx/Rx	GREEN/BROWN				
F1-D	1700	Tx/Rx	RED/RED/	F1-E	1700	Tx/Rx	BLUE/BLUE/BR	F1-F	1700	Tx/Rx	GREEN/GREEN/BROWN				
<b>RF ENGINEER</b>				<b>RF MANAGER</b>				<b>INITIALS</b>				<b>DATE</b>			
Prepared By: Ryan Ulanday				Alex Restrepo				RU				12/15/2015			

## Site Configuration



## SBNHH-1D65B

**Andrew® Tri-band Antenna, 698–896 and 2 x 1710–2360 MHz, 65° horizontal beamwidth, internal RET. Both high bands share the same electrical tilt.**

- Interleaved dipole technology providing for attractive, low wind load mechanical package

### Electrical Specifications

Frequency Band, MHz	698–806	806–896	1710–1880	1850–1990	1920–2180	2300–2360
Gain, dBi	14.9	14.7	17.7	18.2	18.6	18.6
Beamwidth, Horizontal, degrees	68	66	69	66	63	58
Beamwidth, Vertical, degrees	12.1	10.7	5.6	5.2	5.0	4.5
Beam Tilt, degrees	0–14	0–14	0–7	0–7	0–7	0–7
USLS, dB	14	13	15	15	15	13
Front-to-Back Ratio at 180°, dB	27	29	28	28	28	27
CPR at Boresight, dB	20	23	20	20	17	21
CPR at Sector, dB	14	10	12	10	9	1
Isolation, dB	25	25	25	25	25	25
Isolation, Intersystem, dB	30	30	30	30	30	30
VSWR   Return Loss, dB	1.5   14.0	1.5   14.0	1.5   14.0	1.5   14.0	1.5   14.0	1.5   14.0
PIM, 3rd Order, 2 x 20 W, dBc	-153	-153	-153	-153	-153	-153
Input Power per Port, maximum, watts	350	350	350	350	350	300
Polarization	±45°	±45°	±45°	±45°	±45°	±45°
Impedance	50 ohm	50 ohm	50 ohm	50 ohm	50 ohm	50 ohm

### Electrical Specifications, BASTA\*

Frequency Band, MHz	698–806	806–896	1710–1880	1850–1990	1920–2180	2300–2360
Gain by all Beam Tilts, average, dBi	14.5	14.3	17.4	17.9	18.2	18.3
Gain by all Beam Tilts Tolerance, dB	±0.5	±0.8	±0.4	±0.3	±0.5	±0.3
	0°   14.6	0°   14.5	0°   17.4	0°   17.8	0°   18.1	0°   18.2
Gain by Beam Tilt, average, dBi	7°   14.6	7°   14.4	3°   17.5	3°   17.9	3°   18.3	3°   18.4
	14°   14.2	14°   13.6	7°   17.4	7°   17.9	7°   18.2	7°   18.4
Beamwidth, Horizontal Tolerance, degrees	±2.2	±3.4	±2	±4.6	±5.7	±4.3
Beamwidth, Vertical Tolerance, degrees	±0.8	±1	±0.3	±0.2	±0.3	±0.2
USLS, dB	16	14	16	16	16	15
Front-to-Back Total Power at 180° ± 30°, dB	25	26	27	26	26	26
CPR at Boresight, dB	22	23	21	20	20	22
CPR at Sector, dB	13	11	16	12	11	4

\* CommScope® supports NGMN recommendations on Base Station Antenna Standards (BASTA). To learn more about the benefits of BASTA, [download the whitepaper Time to Raise the Bar on BSAs.](#)

### General Specifications

Antenna Brand	Andrew®
Antenna Type	DualPol® multiband with internal RET
Band	Multiband
Brand	DualPol®   Teletilt®
Operating Frequency Band	1710 – 2360 MHz   698 – 896 MHz

### Mechanical Specifications

# Product Specifications

COMMSCOPE®

SBNHH-1D65B



Color	Light gray
Lightning Protection	dc Ground
Radiator Material	Aluminum   Low loss circuit board
Radome Material	Fiberglass, UV resistant
Reflector Material	Aluminum
RF Connector Interface	7-16 DIN Female
RF Connector Location	Bottom
RF Connector Quantity, total	6
Wind Loading, maximum	617.7 N @ 150 km/h 138.9 lbf @ 150 km/h
Wind Speed, maximum	241.4 km/h   150.0 mph

## Dimensions

Depth	181.0 mm   7.1 in
Length	1828.0 mm   72.0 in
Width	301.0 mm   11.9 in
Net Weight	18.4 kg   40.6 lb

## Remote Electrical Tilt (RET) Information

Input Voltage	10-30 Vdc
Power Consumption, idle state, maximum	2.0 W
Power Consumption, normal conditions, maximum	13.0 W
Protocol	3GPP/AISG 2.0 (Multi-RET)
RET Interface	8-pin DIN Female   8-pin DIN Male
RET Interface, quantity	1 female   1 male
RET System	Teletilt®

## Regulatory Compliance/Certifications

### Agency

RoHS 2011/65/EU  
China RoHS SJ/T 11364-2006  
ISO 9001:2008

### Classification

Compliant by Exemption  
Above Maximum Concentration Value (MCV)  
Designed, manufactured and/or distributed under this quality management system



## Included Products

BSAMNT-1 — Wide Profile Antenna Downtilt Mounting Kit for 2.4 - 4.5 in (60 - 115 mm) OD round members. Kit contains one scissor top bracket set and one bottom bracket set.

# ALCATEL-LUCENT B13 RRH4X30-4R

Alcatel-Lucent B13 Remote Radio Head 4x30-4R is the newest addition of Remote Radio Head to the extended product line of Alcatel-Lucent's distributed Base Station solutions, aimed at facilitating smooth RF site acquisition and related civil engineering.

**Supporting 2Tx/4Tx MIMO and 4-way Rx diversity**, Alcatel-Lucent B13 RRH4x30-4R allows operators to have a compact radio solution to deploy LTE in the 700U band (700 MHz, 3GPP band 13), providing them with the means to achieve high capacity, high quality and high coverage with minimum site requirements.

The Alcatel-Lucent B13 RRH4x30-4R product has four transmit RF paths, offering the possibility to **select, via software only, 2Tx or 4Tx MIMO configurations** with either 2x60 W or 4x30 W RF output power. It supports also 4-way Rx diversity and up to 10MHz instantaneous bandwidth.

The Alcatel-Lucent B13 RRH4x30-4R is a near zero-footprint solution and operates noise free, simplifying negotiations with site property owners and minimizing environmental impacts.

Its compactness and slim design makes the Alcatel-Lucent B13 RRH4x30-4R easy to install close to the antenna: operators can therefore locate this Remote Radio Head where RF design conditions are deemed ideal, minimizing trade-offs between available sites and RF optimum sites, together with reducing the RF feeder needs and installation costs.

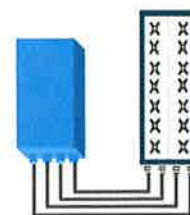


## FEATURES

- Supporting LTE in 700 MHz band (700U, 3GPP band 13)
- LTE 2Tx or 4Tx MIMO (SW switchable)
- Output power: Up to 2x60W or 4x30W
- 10MHz LTE carrier with 4Rx Diversity
- Convection-cooled (fan-less)
- Supports AISG 2.0 ALD devices (RET, TMA) through RS485 or RF ports

## BENEFITS

- Compact to reduce additional footprint when adding LTE in 700U band
- MIMO scheme operation selection (2Tx or 4Tx) by software only
- Improves downlink spectral efficiency through MIMO4
- Increases LTE coverage thanks to 4Rx diversity capability and best in class Rx sensitivity
- Flexible mounting options: Pole or Wall



4x30W with 4T4R  
or  
2x60W with 2T4R

Can be switched between modes via SW w/o site visit



## TECHNICAL SPECIFICATIONS

Features & performance	
<b>Number of TX/RX paths</b>	4 duplexed (either 4T4R or 2T4R by SW)
<b>Frequency band</b>	U700 (C) (3GPP bands 13): DL: 746 - 756 MHz / UL: 777 - 787 MHz
<b>Instantaneous bandwidth - #carriers</b>	10MHz – 1 LTE carrier (in 10MHz occupied bandwidth)
<b>LTE carrier bandwidth</b>	10 MHz
<b>RF output power</b>	2x60W or 4x30W (by SW)
<b>Noise figure – RX Diversity scheme</b>	2 dB typ. (<2.5 dB max) – 2 or 4 way Rx diversity
<b>Sizes (HxWxD) in mm (in.)</b>	550 x 305 x 230 (21.6" x 12.0" x 9") (with solar shield)
<b>Volume in L</b>	38 (with solar shield)
<b>Weight in kg (lb) (w/o mounting HW)</b>	26 (57.2) (with solar shield)
<b>DC voltage range</b>	-40.5 to -57V at full performance, -38 to -57V with relaxation on power consumption
<b>DC power consumption</b>	550W typical @100% RF load ( in 2Tx or 4TX mode)
<b>Environmental conditions</b>	-40°C (-40°F) / +55°C (+131°F) IP65
<b>Wind load (@150km/h or 93mph)</b>	Frontal: <200N / Lateral : <150N
<b>Antenna ports</b>	4 ports 7/16 DIN female (50 ohms) VSWR < 1.5
<b>CPRI ports</b>	2 CPRI ports (HW ready for Rate7, 9.8 Gbps) SFP single mode dual fiber
<b>AISG interfaces</b>	1 AISG2.0 output (RS485) Integrated Smart Bias Tees (x2)
<b>Misc. Interfaces</b>	4 external alarms (1 connector) – 4 RF Tx & 4 RF Rx monitor ports - 1 DC connector (2 pins)
<b>Installation conditions</b>	Pole and wall mounting
<b>Regulatory compliance</b>	3GPP 36.141 / 3GPP 36.113 / GR-1089-CORE / GR-3108-CORE / UL 60950-1 / FCC Part 27

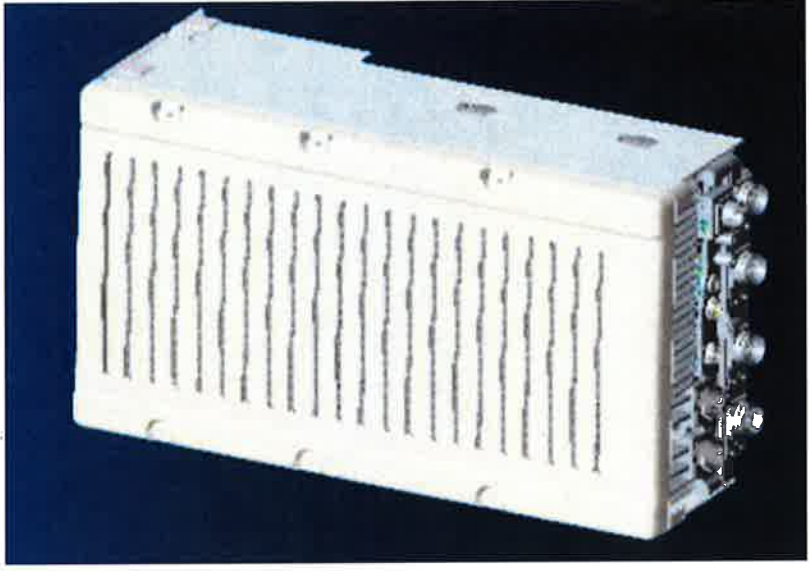
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# NEW PCS RF MODULES FOR VZW

## RRH2X60 - HW CHARACTERISTICS

LR14.3

RRH2x60	
RF Output Power	2x60W (4x30W HW Ready)
Instantaneous Bandwidth	60MHz
Target Reliability (Annual Return Rate)	<2%
Receiver	4 Branch Rx
Features	AISG 2.0 for RET/TMA
Power	-48VDC
CPRI Ports	Internal Smart Bias-T
External Alarms	2 CPRI Rate 5 Ports
Monitor Ports	4 External User Alarms
Environmental	TX, RX
RF Connectors	GR487 Compliance
Dimensions	7/16 DIN (downward facing)
Weight	22"(h) x 12"(w)x 9.4" (d)**
	55lb**



\*\* - Includes solar shield but not mounting brackets (8 lbs.)

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# VZW Network Equipment Reporting Form (NERF)

<b>Vendor</b>	Alcatel-Lucent		<b>Model</b>	B66a RRH 4Tx/4Rx 4x45W or 2x 90W (SW selectable)		<b>Function</b>	RRH for distributed architecture with a CPRI interface between digital and RF processing components. The RRH has 4 Tx ports and 4 Rx ports. Can be SW configured for 2 Tx with 90W rf per port or 4 Tx with 45W rf per port. The RRH has passive cooling only.		
<b>*1)Equipment Configuration</b>	<b>*2)Heat Release @50°F Intake Temp [W]</b>		<b>*3)Airflow Rate @ 100% Activity Rate [cfm]</b>		<b>*4)Dimensions [in]</b>		<b>Non-Thermal Data</b>		
	<b>100% Activity</b>	<b>50% Activity</b>	<b>Nominal (70°F)</b>	<b>Max (95°F)</b>	<b>External (WxDxH)</b>	<b>Clear (F/R/S)</b>	<b>Installed Weight [lb]</b>	<b>*5)Sound @ Nominal [LwAd]</b>	<b>*6)Name Plate [W]</b>
<b>Minimum</b>			N/A Convection cooled	N/A Convection cooled	w/o Solar Shield W = 11.4in D = 6.7in H = 25.2in (W=290mm) (D=170mm) (H=640mm)	Front: 12" Rear: 7.5" Right: 12" Left: 12" Top: 12" Bottom: 24"			
<b>Typical</b>			N/A Convection cooled	N/A Convection cooled	with Solar Shield W = 12in D = 7.6in H = 25.8in (W=304mm) (D=193mm) (H=655mm)		62lb 72 lb(w mounting brackets)	N/A Convection cooled	
<b>Full</b>	825W (add 60W for AISG)	TBD	N/A Convection cooled	N/A Convection cooled	N/A			N/A Convection cooled	
<b>*7)Equipment EC-Class</b>	N/A Convection cooled	<b>*10)Fan Speed</b>	N/A Convection cooled	<b>*13)Fan Hot-Swap</b>	N/A Convection cooled	<b>*16)Environ. Tests</b>	N/A Convection cooled	<b>*18)Temp. Rise [°F]</b>	N/A Convection cooled
<b>*8)Non-Optimal EC-Class</b>	N/A Convection cooled	<b>*11)Fan Logic</b>	N/A Convection cooled	<b>*14)Shut-Down</b>	N/A Convection cooled	<b>*17)Allow. Max [°F]</b>	N/A Convection cooled	<b>*19)Rec. Max [°F]</b>	N/A Convection cooled
<b>*9)Exhaust Openings</b>	N/A Convection cooled	<b>*12)Fan Alarm</b>	N/A Convection cooled	<b>*15)Temp. Access</b>	N/A Convection cooled	<b>*17)Allow. Min [°F]</b>	N/A Convection cooled	<b>*19)Rec. Min [°F]</b>	N/A Convection cooled
<b>Power Reporting</b>									
<b>Power Input</b>	-48V	<b>No. Power Supplies</b>	N/A (Customer provided power plant)		<b>Number of Inputs per Power Supply</b>	1			
<b>*24)Maximum Demand (total system in Watts)</b>	825W (add 60W for AISG)	<b>Maximum Input (each power supply in Watts)</b>	N/A (Customer provided power plant)		<b>Maximum Output (each power supply in Watts)</b>	58W (to AISG port, 29V/2A)			
<b>Power Supply Connection Type</b>	DC entry via Conduit Box	<b>Power Supply Make &amp; Model</b>	N/A (Customer provided power plant)						
<b>Input Protection</b>	no input fuse	<b>Input Protection Make &amp; Model</b>	N/A (Customer provided power plant)						
<b>Redundancy Scheme</b>	N/A								
<b>Nominal Voltage</b>	-48VDC	<b>Maximum Voltage</b>	-57V		<b>Minimum Voltage</b>	-38V			
<b>*25)Max Current at Nominal Voltage</b>	17.2A (add 1.2A if AISG port loaded 2A*29V)	<b>*25)Max Current at Maximum Voltage</b>	14.5A (add 1A if AISG port loaded 2A*29V)		<b>*25)Max Current at Minimum Voltage</b>	21.7A (add 1.5A if AISG port loaded 2A*29V)			

Return completed forms to Engineering and Operations Support (EOS)  
[Richard.damiano@verizonwireless.com](mailto:Richard.damiano@verizonwireless.com)



**Product Description**

The RFS Distribution Box design comes with the option for pluggable over voltage protection (OVP) for up to 6 remote radios and the connection for 6 pairs of optical fiber with LC optical fiber cable management. There is a hybrid cable input with a jumper configuration for power and optical fiber to the remote radio heads (RRHs). A custom wall, a 2-inch pole, and an H-Frame mounting bracket are included. Both the compact and standard design are available with lightning protection.



**Features/Benefits**

- Designed to accommodate varying diameters of HYBRIFLEX™ (combined power and fiber optic) cables – up to 2 inches
- Supports Single- and Multi-Mode Optical fiber
- NEMA 4x rated enclosure – allows flexibility for indoor or outdoor installation on a roof or tower top
- Weatherproof enclosure and ports – improves system reliability
- Modular design – makes replacement or addition of OVP easy without removal of other components within the box
- Strikesorb OVP technology – protects equipment from damaging surges up to 60 kA on an 8/20 waveform and up to 5 kA on a 10/350 waveform (certain models only)
- Low residual voltage and high impedance – ideally suited for RRH technology – won't shut down the RRH the way spark gap technology does (certain models only)



**Technical Specifications**

**Mechanical Specifications**

Model Number	DB-B1-6C-8AB-0Z	DB-T1-6Z-8AB-0Z
Enclosure Design	Standard, 6 OVP's	Standard without OVP
Dimensions - H x W x D, mm (in)	610 x 610 x 254 (24 x 24 x 10)	610 x 610 x 254 (24 x 24 x 10)
Weight, kg (lb)	20 (44)	20 (44)
Suppression Connection Method	Compression lug, #2-#14 AWG Copper, #2-#12 Aluminum	
Fiber Connection Method	LC-LC Single- or Multi-mode duplex	
Environmental Rating	NEMA 4x	
Operating Temperature, °C (°F)	-40 to +80 (-40 to +176)	
UV Protection	ISO 4892-2 Method A Xenon-Arc 2160 hrs	

**Electrical Specifications**

Nominal Operating Voltage	48 VDC	
Nominal Discharge Current (I <sub>n</sub> ) per UL 1449 3rd Ed	20 kA 8/20 μs	N/A
Maximum Discharge Current (I <sub>max</sub> ) per NEMA LS-1	60 kA 8/20 μs	N/A
Maximum Impulse (Lightning) Current (I <sub>imp</sub> ) per IEC 61643-1	5 kA 10/350 μs	N/A
Maximum Continuous Operating Voltage (U <sub>c</sub> )	75 VDC	N/A
Voltage Protection Rating per UL1449 3rd Ed	400 V	N/A
Protection Class as per IEC 61643-1	Class 1	N/A
Strikesorb OVP Compliance	ANSI/UL 1449-3rd Ed	N/A
	IEEE C62.41	N/A
	NEMA LS-1	N/A
	IEC 61643-1	N/A
	IEC 61643-12	N/A
	EN 61643-11	N/A

\* This data is provisional and subject to change.

All information contained in the present datasheet is subject to confirmation at time of ordering.