

October 13, 2015

Members of the Siting Council Connecticut Siting Council Ten Franklin Square New Britain, CT 06051

> RE: Notice of Exempt Modification 150 Butternut Hollow Rd, Greenwich, CT 06830 Longitude: -73.638854 Latitude: 41.096927 T-Mobile Site#: CT11070B_L700

Members of the Siting Council:

On behalf of T-Mobile, Northeast Site Solutions (NSS) is submitting an exempt modification application to the Connecticut Siting Council for modification of existing equipment at a tower facility located at 150 Butternut Hollow Rd, Greenwich, CT 06830.

The 150 Butternut Hollow Rd Greenwich, CT 0683 facility consists of a 180' Lattice Tower owned and operated by Connecticut State Police. In order to accommodate technological changes and enhance system performance in the State of Connecticut, T-Mobile plans to modify the equipment configurations at many of its existing cell sites. Please accept this letter and attachments as notification, pursuant to R.C.S.A. Section 16-50j-73, of construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2). In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter and attachments is being sent to the chief elected official of the municipality in which the affected cell site is located.

As part of T-Mobile's L700 Project, T-Mobile desires to upgrade their equipment to meet the new standards of 4G technology. The new equipment will allow customers to download files and browse the internet at a high rate of speed while also allowing their phones to be compatible with the latest 4G technology.

Attached is a summary of the planned modifications, including power density calculations reflecting the change in T-Mobile's operations at the site along with the required fee of \$650.

54 Main Street Unit 3 | Sturbridge Ma 01566 | f: 413-521-0558 | www.northeastsitesolutions.com



The changes to the facility do not constitute modifications as defined in Connecticut General Statutes significantly changed or altered. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in R.C.S.A. Section 16-50j-72(b)(2).

1. The overall height of the structure will be unaffected.

2. The proposed changes will not extend the site boundaries. There will be no effect on the site compound other than the new equipment cabinet.

3. The proposed changes will not increase the noise level at the existing facility by six decibels or more.

4. The changes in radio frequency power density will not increase the calculated "worst case" power density for the combined operations at the site to a level at or above the applicable standard for uncontrolled environments as calculated for a mixed frequency site.

For the foregoing reasons, Northeast Site Solutions (NSS) on behalf of T-Mobile, respectfully submits that the proposed changes at the referenced site constitute exempt modifications under R.C.S.A.Section 16-50j-72(b)(2).

Please feel free to call me at 860.209.4690 with any questions you may have concerning this matter.

Sincerely,

Denise Sabo Mobile: 860-209-4690 Fax: 413-521-0558 Office: 199 Brickyard Rd, Farmington, CT 06032 Email: denise@northeastsitesolutions.com

cc: The Honorable Peter J. Tesei, First Selectman, Town of Greenwich State of Connecticut, Department of Public Safety, Division of State Police

Exhibit A

T - Mobile **T-MOBILE NORTHEA** SITE #: CT11070B SITE NAME: CONNECTICUT STAT SITE ADDRESS: **150 BUTTERNUT HOLLOW RE** GREENWICH, CT 06830 WIRELESS BROADBAND FACIL CONSTRUCTION DRAWINGS (702CC CONFIGURATION)

VICINITY MAP



DO NOT SCALE DRAWINGS

CONTRACTOR SHALL VERIFY PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ARCHITECT IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

CALL BEFORE YOU DIG: WWW.CBYD.COM
CALL 800 922 4455, OR 811 CALL THREE WORKING DAYS PRIOR TO DIGGING
SAFETY PRECAUTIONS SHALL BE IMPLEMENTED BY CONTRACTOR(S) AT ALL TRENCHING IN ACCORDANCE WITH CURRENT OSHA STANDARDS.
COLOR CODE FOR UTILITY LOCATIONS
Electric – Red Sewer – Green
GAS/OIL – YELLOW SURVEY – PINK
TEL/CATV - ORANGE PROPOSED EXCAVATION - WHITE
WATER PLUE RECLAIMED WATER - PURPLE

GENERAL NOTES

- . THE CONTRACTOR SHALL GIVE ALL NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES. RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY. MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS, AND LOCAL AND STATE JURISDICTIONAL CODES BEARING ON THE PERFORMANCE OF THE WORK. THE WORK PERFORMED ON THE PROJECT AND THE MATERIALS INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES.
- . THE ARCHITECT/ENGINEER HAVE MADE EVERY EFFORT TO SET FORTH IN THE CONSTRUCTION AND CONSTRUCT DOCUMENTS THE COMPLETE SCOPE OF WORK. THE CONTRACTOR BIDDING THE JOB IS NEVERTHELESS CAUTIONED THAT MINOR OMISSIONS OR ERRORS IN THE DRAWINGS AND OR SPECIFICATIONS SHALL NOT EXCUSE SAID CONTRACTOR FROM COMPLETING THE PROJECT AND IMPROVEMENTS IN ACCORDANCE WITH THE INTENT OF THESE DOCUMENTS
- . THE CONTRACTOR OR BIDDER SHALL BEAR THE RESPONSIBILITY OF NOTIFYING (IN WRITING) THE T-MOBILE REPRESENTATIVE OF ANY CONFLICTS. ERRORS. OR OMISSIONS PRIOR TO THE SUBMISSION OF THE CONTRACTOR'S PROPOSAL OR PERFORMANCE OF WORK. IN THE EVENT OF DISCREPANCIES. THE CONTRACTOR SHALL PRICE THE MORE COSTLY OR EXPENSIVE WORK, UNLESS DIRECTED IN WRITING OTHERWISE
- . THE SCOPE OF WORK SHALL INCLUDE FURNISHING OF ALL MATERIALS, EQUIPMENT, LABOR AND ALL OTHER MATERIALS AND LABOR DEEMED NECESSARY TO COMPLETE THE WORK/PROJECT AS DESCRIBED HEREIN
- . THE CONTRACTOR SHALL VISIT THE JOB SITE PRIOR TO THE SUBMISSION OF BIDS OR PERFORMING WORK TO FAMILIARIZE HIMSELF WITH THE FIELD CONDITIONS AND TO VERIFY THAT THE PROJECT CAN BE CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS
- 5. THE CONTRACTOR SHALL OBTAIN AUTHORIZATION TO PROCEED WITH CONSTRUCTION PRIOR TO STARTING WORK ON ANY ITEM NOT CLEARLY DEFINED BY THE CONSTRUCTION DRAWINGS/CONTRACT DOCUMENTS
- 7. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS 18. REFER TO STRUCTURAL ANALYSIS DOCUMENT ENTITLED, ACCORDING TO THE MANUFACTURER'S/VENDOR'S SPECIFICATIONS UNLESS NOTED OTHERWISE OR WHERE LOCAL CODES OR ORDINANCES TAKE PRECEDENCE
- . THE CONTRACTOR SHALL PROVIDE A FULL SET OF CONSTRUCTION DOCUMENTS AT THE SITE LIPDATED WITH THE LATEST REVISIONS. AND ADDENDUM OR CLARIFICATIONS AVAILABLE FOR THE USE BY ALL PERSONNEL INVOLVED WITH THE PROJECT.

9. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT DESCRIBED HEREIN. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS. METHODS. TECHNIQUES, SEQUENCES, AND PROCEDURES AND FOR COORDINATING ALL PORTIONS OF THE WORK UNDER CONTRACT.

- 10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ANY PERMITS AND INSPECTIONS WHICH ARE REQUIRED FOR THE WORK BY THE ARCHITECT/ENGINEER, THE STATE, COUNTY, OR LOCAL GOVERNMENT AUTHORITY.
- 11. THE CONTRACTOR SHALL MAKE NECESSARY PROVISIONS TO PROTECT EXISTING IMPROVEMENTS, EASEMENTS, PAVING, CURBING ETC., DURING CONSTRUCTION. UPON COMPLETION OF WORK, THE CONTRACTOR SHALL REPAIR ANY DAMAGE THAT MAY HAVE OCCURRED DUE TO CONSTRUCTION ON OR ABOUT THE PROPERTY
- 12. THE CONTRACTOR SHALL KEEP THE GENERAL WORK AREA CLEAN AND HAZARD FREE DURING CONSTRUCTION AND DISPOSE OF ALL DIRT. DEBRIS, RUBBISH AND REMOVE FOUIPMENT NOT SPECIFIED AS REMAINING ON PROPERTY. PREMISES SHALL BE LEFT IN CLEAN CONDITION AND FREE FROM PAINT SPOTS, DUST, OR SMUDGES OF ANY NATURE
- 13. THE CONTRACTOR SHALL COMPLY WITH ALL OSHA REQUIREMENTS. AS WELL AS THE LATEST EDITIONS OF ANY PERTINENT STATE SAFETY REGULATIONS.
- 14. THE CONTRACTOR SHALL NOTIFY THE T-MOBILE REPRESENTATIVE WHERE A CONFLICT OCCURS ON ANY OF THE CONTRACT DOCUMENTS. THE CONTRACTOR IS NOT TO ORDER MATERIAL OR CONSTRUCT ANY PORTION OF THE WORK THAT IS IN CONFLICT UNTIL CONFLICT IS RESOLVED BY THE T-MOBILE REPRESENTATIVE. 15. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS. ELEVATIONS.
- PROPERTY LINES, ETC., ON THE JOB.
- 16. THE CONTRACTOR SHALL RETURN ALL DISTURBED AREAS TO THEIR ORIGINAL CONDITION AT THE COMPLETION OF WORK.
- 17. ATLANTIS GROUP, INC. HAS NOT CONDUCTED A STRUCTURAL ANALYSIS FOR THIS PROJECT AND DOES NOT ASSUME ANY LIABILITY FOR THE ADEQUACY OF THE STRUCTURE AND COMPONENTS.
- "DETAILED STRUCTURAL ANALYSIS AND EVALUATION OF AN EXISTING 180' SELF SUPPORTING LATTICE TOWER WITH STACK-N-BOLT SYSTEM AND FOUNDATION FOR PROPOSED ANTENNA ARRANGEMENT" PREPARED BY AECOM. "T-MOBILE SITE ID CT1070B", DATED MARCH 4, 2015.

SITE INFO	RMATION	
E NUMBER:	СТ11070В	AF
E NAME: E ADDRESS: ./LONG.: RISDICTION:	CONNECTICUT STATE POLICE #2 150 BUTTERNUT HOLLOW RD GREENWICH ,CT 06830 N 41.096927 / W -73.638854 FAIRFIELD COUNTY	PF
PERTY OWNER:		AF
ODE CON	IPLIANCE	

AST LLC ATE POLICE #2		T-MOBILE NORTHEAST, LLC 35 GRIFEN ROAD SOUTH BLOOMFIELD, CT 06002 OFFICE: (860) 692-7109 FAX:(860) 692-7109 FAX:(860) 692-7159 1340 Centre Street, Suite 212 Newton Center, MA 02459 Office: 617–965–0789 Fax: 617–965–0789 Fax: 617–9213–5056 DISCRIPTION REVISION 02/16/15 ISSUED FOR ROVEN 02/16/15 REVISION 02/16/15 REVISION 1 03/05/15 FINA. 60 2 2
SITE INFORMATION SITE INFORMATION SITE NUMBER: CT11070B SITE NAME: CONNECTICUT STATE POLICE #2 SITE ADDRESS: 150 BUTTERNUT HOLLOW RD GREENWICH, CT 06830 LAT./LONG: N 41.096927 / W -73.638854 JURISDICTION: FAIRFIELD COUNTY PROPERTY OWNER: STATE POLICE PAUL ZITO PUBLIC SAFETY DIRECTOR OF TELECOMMUNICATIONS CT DEPARTMENT OF EMERGENCY SERVICES AND PUBLIC PROTECTION DIVISION OF STATE POLICE 1111 COUNTRY CLUB ROAD MIDDLETOWN, CT 06457 860-685-8345 - FAX 860-305-5275 - CELL 860-685-8008 24/7 EMERGENCIES	PROJECT SUB—CONTRACTORS APPLICANT: T-MOBILE NORTHEAST, LLC. 35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002 (860) 692–7100 PROJECT MANAGER LISA LIN ALLEN NORTHEAST SITE SOLUTIONS 54 MAIN STREET STURBRIDGE, MA 01566 (508) 434–5237 ARCHITECT/ENGINEER: ATLANTIS GROUP INC. 1340 CENTRE STREET SUITE 212 NEWTON CENTER, MA 02459 (617) 965–0789	DEPT. DATE APP'D REVISIONS RF MAL ZONING OPS STE PROJECT NO: CT11070B DRAWN BY: FG CHECKED BY: SM OF OR OF CHECKED BY: SM
CODE COMPLIANCE <u>CONNECTICUT STATE BUILDING CODE</u> 2005 CONNECTICUT BUILDING CODE WITH 2013 AMENDMENT 2011 NATIONAL ELECTRICAL CODE CONSTRUCTION TYPE: 2B USE GROUP: N/A	SHEET INDEX SHEET DESCRIPTION T-1 TITLE SHEET N-1 GENERAL AND ELECTRICAL NOTES A-1 PLOT PLAN, SITE PLAN AND ELEVATION A-2 ANTENNA PLAN AND DETAILS E-1 GROUNDING DIAGRAM E-2 GROUNDING DETAILS	CT11070B SITE NAME CONNECTICUT STATE POLICE #2 SITE ADDRESS 150 BUTTERNUT HOLLOW RD GREENWICH ,CT 06830 SHEET TITLE TITLE SHEET SHEET NUMBER T-1

ELECTRICAL NOTES: WORK INCLUDED

- 1. INCLUDE ALL LABOR, MATERIALS, EQUIPMENT, PLANT SERVICES AND ADMINISTRATIVE TASKS REQUIRED TO COMPLETE AND MAKI OPERABLE THE ELECTRICAL WORK SHOWN ON THE DRAWINGS AND SPECIFIED HEREIN, INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
- A. PREPARE AND SUBMIT SHOP DRAWINGS, DIAGRAMS AND ILLUSTRATIONS
- B. PROCURE ALL NECESSARY PERMITS AND APPROVALS AND PAY ALL REQUIRED FEES AND CHARGES IN CONNECTION WITH THE WORK OF THIS CONTRACT.
- C. SUBMIT AS-BUILT DRAWINGS. OPERATING AND MAINTENANCE INSTRUCTIONS AND MANUALS.
- D. EXECUTE ALL CUTTING, DRILLING, ROUGH AND FINISH PATCHING OF EXISTING OR NEWLY INSTALLED CONSTRUCTION REQUIRED FOR THE WORK OF THIS CONTRACT FOR SLAB PENETRATIONS THROUGH POST TENSION SLABS, X-RAY EXACT AREA OF PENETRATION PRIOR TO PERFORMING WORK. COORDINATE ALL X-RAY WORK WITH BUILDING ENGINEER
- E. PROVIDE HANGERS, SUPPORTS, FOUNDATIONS, STRUCTURAL SUPPORTS, AND BASES FOR CONDUIT AND EQUIPMENT PROVIDED OR INSTALLED UNDER THE WORK OF HIS CONTRACT. PROVIDE COUNTER FLASHING, SLEEVES AND SEALS FOR FLOOR AND WALL PENETRATIONS.
- F. MAINTAIN ALL EXISTING ELECTRICAL SERVICES IN THE BUILDING AREAS NOT AFFECTED BY THE ALTERATION DURING THE PROGRESS OF THE WORK INCLUDING PROVIDING ALL TEMPORARY JUMPERS, CONDUITS, CAPS, PROTECTIVE DEVICES. CONNECTIONS AND EQUIPMENT REQUIRED. PROVIDE TEMPORARY LIGHT AND POWER FOR CONSTRUCTION
- 2. IT IS THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS TO CALL FOR AN INSTALLATION THAT IS COMPLETE IN EVERY RESPECT. IT IS NOT THE INTENT TO GIVE EVERY DETAIL ON THE DRAWINGS AND IN THE SPECIFICATIONS. IF AN ITEM OF WORK IS INDICATED IN THE DRAWINGS, IT IS CONSIDERED SUFFICIENT FOR INCLUSION IN THE CONTRACT FURNISH AND INSTALL ALL MATERIAL AND EQUIPMENT USUALLY FURNISHED OR NEEDED TO MAKE A COMPLETE INSTALLATION WHETHER OR NOT SPECIFICALLY MENTIONED IN THE CONTRACT DOCUMENTS.

GENERAL REQUIREMENTS

- 1. PROVIDE ALL WORK IN ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE (NEC) AND LOCAL AND STATE ELECTRICAL CODES
- 2. THE ELECTRICAL PLANS ARE DIAGRAMMATIC ONLY. REFER TO THE ARCHITECTURAL PLANS FOR THE EXACT DIMENSIONS OF THE BUILDING
- 3 LOAD CALCULATIONS ARE BASED ON EXISTING BUILDING INFORMATION/DRAWINGS PROVIDED TO ENGINEERING. CONTRACTOR IS TO VERIEY ALL EXISTING RATINGS AND LOADS PRIOR TO PURCHASING OF SPECIFIED EQUIPMENT FOR COMPLIANCE TO NEC CONTRACTOR TO NOTIFY ENGINEER OF ANY DISCREPANCIES AND REQUEST FURTHER DIRECTION BY FNGINFFR.
- EXISTING BUILDING EQUIPMENT IS NOTED ON THE DRAWINGS NEW OR RELOCATED EQUIPMENT IS SHOWN WITH SOLID LINES. FUTURE EQUIPMENT (NOT IN THIS CONTRACT) IS DEPICTED WITH SHADED LINES. REQUEST CLARIFICATION OF DRAWINGS OR OF SPECIFICATIONS PRIOR TO PRICING OR INSTALLATION. 5. GENERAL
- A. AFTER CAREFULLY STUDYING THE DRAWINGS AND SPECIFICATIONS. AND BEFORE SUBMITTING THE PROPOSAL. MAKE A MANDATORY SITE VISIT TO ASCERTAIN CONDITIONS OF THE SITE, AND THE NATURE AND EXACT QUANTITY OF WORK TO BE PERFORMED. NO EXTRA COMPENSATION WILL BE ALLOWED FOR FAILURE TO NOTIFY THE OWNER. IN WRITING. OF ANY DISCREPANCIES THAT MAY HAVE BEEN NOTED BETWEEN THE EXISTING CONDITIONS AND THE DRAWINGS AND SPECIFICATIONS
- B. VERIFY ALL MEASUREMENTS AT THE SITE AND BE RESPONSIBLE FOR CORRECTNESS OF SAME
- 6. QUALITY, WORKMANSHIP, MATERIALS AND SAFETY A. PROVIDE NEW MATERIALS AND EQUIPMENT OF A DOMESTIC MANUFACTURER BY THOSE REGULARLY ENGAGED IN THE PRODUCTION AND MANUFACTURE OF SPECIFIED MATERIALS AND EQUIPMENT. WHERE UL, OR OTHER AGENCY, HAS ESTABLISHED STANDARDS FOR MATERIALS PROVIDE MATERIALS WHICH ARE LISTED AND LABELED ACCORDINGLY. THE COMMERCIALLY STANDARD ITEMS OF EQUIPMENT AND THE SPECIFIC NAMES MENTIONED HEREIN ARE INTENDED FOR THE PROPER FUNCTIONING OF THE WORK
- B. WORK SHALL BE PERFORMED BY WORKMEN SKILLED IN THE TRADE REQUIRED FOR THE WORK, INSTALL MATERIALS AND EQUIPMENT TO PRESENT A NEAT APPEARANCE WHEN COMPLETED AND IN ACCORDANCE WITH THE APPROVED RECOMMENDATIONS OF THE MANUFACTURER AND IN ACCORDANCE WITH CONTRACT DOCUMENTS.
- C. PROVIDE LABOR, MATERIALS, APPARATUS AND APPLIANCES ESSENTIAL TO THE FUNCTIONING OF THE SYSTEMS DESCRIBED OR INDICATED HEREIN, OR WHICH MAY BE REASONABLY IMPLIED AS ESSENTIAL WHENEVER MENTIONED IN THE CONTRACT DOCUMENT OR NOT. D MAKE WRITTEN REQUESTS FOR SUPPLEMENTARY
- INSTRUCTIONS TO ARCHITECT/ENGINEER IN CASE OF DOUBT AS TO WORK INTENDED OR IN EVENT OF NEED FOR EXPLANATION THEREOF
- E. PERFORMANCE AND MATERIAL REQUIREMENTS SCHEDULED OR SPECIFIED ARE MINIMUM STANDARD ACCEPTABLE. THE TO JUDGE THE QUALITY OF EQUIPMENT THAT DEVIATES FROM THE CONTRACT DOCUMENT REMAINS SOLELY ARCHITECT/ENGINEER. CONTRACT DOCUMENT OR NOT.
- 1. GUARANTEE MATERIALS, PARTS AND LABOR FOR WORK FOR ONE YEAR FROM THE DATE OF ISSUANCE OF OCCUPANCY PERMIT. DURING THAT PERIOD. MAKE GOOD FAULTS OR IMPERFECTIONS THAT MAY ARISE DUE TO DEFECTS OR OMISSIONS IN MATERIALS OR WORKMANSHIP WITH NO ADDITIONAL COMPENSATION AND AS DIRECTED BY ARCHITECT

- CLEANING 1. REMOVE ALL CONSTRUCTION DEBRIS RESULTING FROM THE WORK
- 2. CLEAN EQUIPMENT AND SYSTEMS FOLLOWING THE COMPLETION OF THE PROJECT TO THE SATISFACTION OF THE ENGINEER.
- COORDINATION AND SUPERVISION
 - CAREFULLY LAY OUT ALL WORK IN ADVANCE TO AVOID UNNECESSARY CUTTING, CHANNELING, CHASING OR DRILLING OF FLOORS WALLS PARTITIONS CEILINGS OR OTHER SURFACES. WHERE SUCH WORK IS NECESSARY, HOWEVER, PATCH AND REPAIR THE WORK IN AN APPROVED MANNER BY SKILLED MECHANICS AT NO ADDITIONAL COST TO THE OWNER. RENDER FULL COOPERATION TO OTHER TRADES WHERE WORK WILL BE INSTALLED IN CLOSE PROXIMITY TO WORK OF OTHER TRADES. ASSIST IN WORKING OUT SPACE CONDITIONS IF WORK IS INSTALLED BEFORE COORDINATION WITH OTHER TRADES. OR CAUSES INTERFERENCE MAKE CHANGES NECESSARY TO CORRECT CONDITIONS WITHOUT EXTRA CHARGE

SUBMITTALS

- 1 AS-BUILT DRAWINGS A. UPON COMPLETION OF THE WORK, FURNISH TO THE OWNER "AS-BUILT" DRAWINGS.
- 2. SERVICE MANUALS: A. UPON COMPLETION OF THE WORK, FULLY INSTRUCT T-MOBILE AS TO THE OPERATION AND MAINTENANCE OF ALL MATERIAL, FOUIPMENT AND SYSTEMS.
- PROVIDE 3 COMPLETE BOUND SETS OF INSTRUCTIONS FOR OPERATING AND MAINTAINING ALL SYSTEMS AND EQUIPMENT.

CUTTING AND PATCHING

- PROVIDE ALL CUTTING, DRILLING, ROUGH AND FINISH PATCHING REQUIRED TO COMPLETE THE WORK.
- 2. OBTAIN OWNER APPROVAL PRIOR TO CUTTING THROUGH FLOORS OR WALLS FOR PIPING OR CONDUIT.
- TESTS, INSPECTION AND APPROVAL
- . BEFORE ENERGIZING ANY ELECTRICAL INSTALLATION, INSPECT EACH UNIT IN DETAIL. TIGHTEN ALL BOLTS AND CONNECTIONS (TORQUE-TIGHTEN WHERE REQUIRED) AND DETERMINE THAT ALL COMPONENTS ARE ALIGNED, AND THE EQUIPMENT IS IN SAFE, OPERATIONAL CONDITION
- 2. PROVIDE THE COMPLETE ELECTRICAL SYSTEM FREE OF GROUND FAULTS AND SHORT CIRCUITS SUCH THAT THE SYSTEM WILL OPERATE SATISFACTORILY UNDER FULL LOAD CONDITIONS, WITHOUT EXCESSIVE HEATING AT ANY POINT IN THE SYSTEM
- SPECIAL REQUIREMENTS
 - 1. DO NOT LEAVE ANY WORK INCOMPLETE NOR ANY HAZARDOUS SITUATIONS CREATED WHICH WILL AFFECT THE LIFE OR SAFETY OF THE PUBLIC AND/OR BUILDING OCCUPANTS. DO NOT INTERFERE WITH OR CUTOFF ANY OF THE EXISTING SERVICES VITHOUT THE OWNER'S WRITTEN PERMISSION.
 - 2 WHEN NECESSARY TO TEMPORARILY DISCONNECT ANY EXISTING BUILDING UTILITIES AND SERVICE SYSTEMS, INCLUDING FEEDER OR BRANCH CIRCUITING SUPPLYING EXISTING FACILITIES CONFER WITH THE OWNER AND ARRANGE THE PERIOD OF
 - INTERRUPTION FOR A TIME MUTUALLY AGREED LIPON SHUTDOWN NOTE: SCHEDULE AND NOTIFY OWNER 48 HOURS PRIOR TO SHUTDOWN. ALL SHUTDOWN WORK TO BE SCHEDULED AT A TIME CONVENIENT TO OWNER.
- GROUNDING
- 1. ROUTE ALL GROUNDING CONDUCTORS AS SHOWN ON CONDUIT/GROUNDING RISER.
- 2. ROUTE 500 KCMIL CU. THHN CONDUCTOR FROM THE MGB LOCATION TO BUILDING STEEL. VERIFY BUILDING STEEL IS EFFECTIVELY GROUNDED PER NEC TO THE MAIN SERVICE
- GROUNDING ELECTRODE CONDUCTOR (GEC) 3. MAKE ALL GROUND CONNECTIONS FROM MGB TO ELECTRICAL EQUIPMENT WITH 2 HOLE, CRIMP TYPE, BURNDY COMPRESSION TERMINATIONS, SIZED AS REQUIRED.
- 4. USE 1 HOLE, CRIMP TYPE, BURNDY COMPRESSIONS TERMINATIONS, SIZED AS REQUIRED, AT EQUIPMENT GROUND CONNECTIONS
- 5. HIRE AN INDEPENDENT LAB TO PERFORM THE SPECIFIED OHMS TESTING. PROVIDE 4 SETS OF THE CERTIFIED DOCUMENTS TO THE OWNER FOR VERIFICATION PRIOR TO THE PROJECT

COMPLETION RACEWAYS

- 1. ALL WIRING TO BE INSTALLED IN CONDUIT SYSTEMS IN ACCORDANCE WITH THE FOLLOWING:
- A. EXTERIOR FEEDERS AND CONTROL, WHERE UNDERGROUND, TO BE IN SCH 40 PVC.
- B. EXTERIOR, ABOVE GROUND POWER CONDUITS TO BE
- GALVANIZED RIGID STEEL (RGS). C. ALL TELECOMMUNICATION CONDUITS, INTERIOR/EXTERIOR, TO
- BF FMT D. INSTALL PULL ROPES IN ALL NEW EMPTY CONDUITS INSTALLED
- ON THIS PROJECT. E. ALL TELECOM CONDUITS AND PULL BOXES INSTALLED ON THIS PROJECT TO BE LABELED "T-MOBILE". OWNER WILL
- PROVIDE LABELS FOR CONTRACTOR TO INSTALL F. INTERIOR FEEDERS TO BE INSTALLED IN E.M.T. WITH STEEL
- COMPRESSION FITTINGS G. MINIMUM SIZE CONDUIT TO BE 3/4" TRADE SIZE
- UNLESS OTHERWISE INDICATED ON THE DRAWINGS H. FINAL CONNECTIONS TO MOTORS AND VIBRATING EQUIPMENT
- TO BE INSTALLED IN LIQUID-TIGHT FLEXIBLE METAL CONDUIT. I. CONDUIT TO BE RUN CONCEALED IN CEILINGS, FINISHED AREAS OR DRYWALL PARTITIONS, UNLESS OTHERWISE NOTED
- THE ROUTING OF CONDUITS INDICATED ON THE DRAWINGS IS DIAGRAMMATIC BEFORE INSTALLING ANY WORK EXAMINE THE WORKING LAYOUTS AND SHOP DRAWINGS OF THE OTHER TRADES TO DETERMINE THE EXACT LOCATIONS AND
- K. ALL EXTERIOR MOUNTING HARDWARE TO BE GALVANIZED STEEL. COORDINATE WITH BUILDING ENGINEER PRIOR TO ATTACHING TO BUILDING STRUCTURE.

- RACEWAYS CONT'D
 - L. PENETRATIONS OF WALLS, FLOORS AND ROOFS, FOR THE PASSAGE OF ELECTRICAL RACEWAYS, TO BE PROPERLY SEALED AFTER INSTALLATION OF RACEWAYS SO AS TO MAINTAIN THE STRUCTURAL OR WATERPROOF INTEGRITY OF THE WALL, FLOOR OR ROOF SYSTEM TO BE PENETRATED. SEAL ALL CONDUIT PENETRATIONS THROUGH FIRE OR SMOKE RATED WALLS, CEILINGS OR SMOKE TIGHT CORRIDOR PARTITIONS TO MAINTAIN PROPER RATING OF WALL OR CEILING.
 - M. PROVIDE ALL CONDUIT ENDS WITH INSULATED METALLIC GROUNDING BUSHINGS
 - N. CONDUIT TO BE SUPPORTED AT MAXIMUM DISTANCE OF 8'-0", OR AS REQUIRED BY NEC, IN HORIZONTAL AND VERTICAL DIRECTIONS.
 - O. PROVIDE STAILLESS STEEL BLANK COVER PLATES FOR ALL JUNCTION BOXES AND/OR OUTLET BOXES NOT USED IN EXPOSED AREAS. PROVIDE ALL OTHER UNUSED BOXES WITH STANDARD STEEL COVER PLATES.
 - P. WHERE APPLICABLE, PROVIDE ROOFTOP CONDUIT SUPPORT SYSTEM, CONFORMING TO ROOFTOP WARRANTY REQUIREMENTS, PER BUILDING

WIRES AND CABLES

- I. CONTRACTOR TO COORDINATE WITH EQUIPMENT SUPPLIER AND VENDOR FOR EXACT FOUIPMENT OVER-CURRENT PROTECTION VOLTAGE, WIRE SIZE AND PLUG CONFIGURATION, IF APPLICABLE, PRIOR TO BID. 2. ALL EQUIPMENT/DEVICES TO BE PROVIDED WITH INSULATED
- GROUND CONDUCTOR.
- 3. ALL WIRE AND CABLE TO BE 600VOLT, COPPER, WITH THWN/ THHN INSULATION, EXCEPT AS NOTED. 4. WIRE FOR POWER AND LIGHTING WILL NOT BE LESS THAN NO.
- 12AWG. ALL WIRE NO. 8 AND LARGER TO BE STRANDED. 5. CONTROL WIRING IS NOT TO BE LESS THAN NO. 14AWG
- FLEXIBLE IN SINGLE CONDUCTORS OR MULTI-CONDUCTOR CABLES. CONTROL WIRING WILL CONSIST OF MULTI-CONDUCTOR CABLES WHEREVER POSSIBLE. CABLES TO BE PROVIDED WITH AN OVERALL FLAME-RETARDANT, EXTRUDED JACKET AND RATED FOR PLENUM USE, ALL CONTROL WIRE TO BE 600VOLT RATED 6. WIRE PREVIOUSLY PULLED INTO CONDUIT IS CONSIDERED USED
- AND IS NOT TO BE RE-PULLED. 7. HOME RUNS AND BRANCH CIRCUIT WIRING FOR 20A, 120V CIRCUITS:

LENGTH (FT.)	HOME RUN WIRE SIZE
0 TO 50	NO. 12
51 TO 100	NO. 10
101 TO 150	NO. 8

- 8. VOLTAGE DROP IS NOT TO EXCEED 3%.
- 9. MAKE ALL CONNECTIONS WITH UL APPROVED, SOLDERLESS, PRESSURE TYPE INSULATED CONNECTORS: SCOTCHLOK OR AND APPROVED EQUAL.
- WIRING DEVICES
- 1. ALL RECEPTACLES INSTALLED IN THIS PROJECT TO BE GROUNDING TYPE. WITH GROUNDING PIN SLOT CONNECTED TO DEVICE GROUND SCREW FOR GROUND WIRE CONNECTION. DISCONNECT SWITCHES AND FUSES
- 1. DISCONNECT SWITCHES TO BE VOLTAGE-RATED TO SUIT THE CHARACTERISTICS OF THE SYSTEM FROM WHICH THEY ARE
- 2. PROVIDE HEAVY-DUTY, METAL-ENCLOSED, EXTERNALLY-OPERATED DISCONNECT SWITCHES, FUSED OR UNFUSED, OF SUCH TYPE AND SIZE AS REQUIRED TO PROPERLY PROTECT OR DISCONNECT THE LOAD FOR WHICH THEY ARE INTENDED.
- 3. PROVIDE NEMA 1 DISCONNECT SWITCHES FOR INTERIOR
- INSTALLATION, NEMA 3R FOR EXTERIOR INSTALLATION. 4 DISCONNECT SWITCHES TO BE MANUFACTURED BY A. GENERAL ELECTRIC COMPANY
- B. SQUARE-D PROVIDE RK-1 TYPE FUSES, UNLESS NOTED OTHERWISE. INSTALLATION
- 1. INSTALL DISCONNECT SWITCHES WHERE INDICATED ON
- DRAWINGS. 2. INSTALL FUSES IN FUSIBLE DISCONNECT SWITCHES. FUSES
- MUST MATCH IN TYPE AND RATING. 3. FUSES TO BE MOUNTED SO THAT THE LABELS SHOWING THEIR RATINGS CAN BE READ WITHOUT REQUIRING FUSE REMOVAL.
- 4. FURNISH AND DEPOSIT SPARE FUSES AT THE JOB SITE AS
- FOLLOWS A. THREE SPARES FOR EACH TYPE AND SIZE, IN EXCESS OF 60A, USED FOR INITIAL FUSING.
- B. TEN PERCENT SPARES FOR EACH TYPE AND SIZE, UP TO
- AND INCLUDING 60A, USED FOR INITIAL FUSING. IN NO CASE WILL LESS THAN THREE FUSES OF ONE PARTICULAR TYPE AND SIZE BE FURNISHED.

GENERAL NOTES:

- INTENT 1. THESE SPECIFICATIONS AND CONSTRUCTION DRAWINGS ACCOMPANYING THEM DESCRIBE THE WORK TO BE DONE AND
- THE MATERIALS TO BE FURNISHED FOR CONSTRUCTION. 2. THE DRAWINGS AND SPECIFICATIONS ARE INTENDED TO BE FULLY EXPLANATORY AND SUPPLEMENTARY. HOWEVER, SHOULD ANYTHING BE SHOWN, INDICATED, OR SPECIFIED ON ONE AND NOT THE OTHER, IT SHALL BE DONE THE SAME AS IF SHOWN
- INDICATED OR SPECIFIED IN BOTH 3. THE INTENTION OF THE DOCUMENTS IS TO INCLUDE ALL LABOR AND MATERIALS REASONABLY NECESSARY FOR THE PROPER
- EXECUTION AND COMPLETION OF THE WORK AS STIPULATED IN THE CONTRACT.
- 4. THE PURPOSE OF THE SPECIFICATIONS IS TO INTERPRET THE INTENT OF THE DRAWINGS AND TO DESIGNATE THE METHOD OF THE PROCEDURE, TYPE AND QUALITY OF MATERIALS REQUIRED TO COMPLETE THE WORK.
- MINOR DEVIATIONS FROM THE DESIGN LAYOUT ARE ANTICIPATED AND SHALL BE CONSIDERED AS PART OF THE WORK, NO CHANGES THAT ALTER THE CHARACTER OF THE WORK WILL BE MADE OR PERMITTED BY THE OWNER WITHOUT ISSUING A CHANGE ORDER.

CONFLICTS

CONTRACTS AND WARRANTIES

ADDITIONAL DETAILS.

STORAGE

CI FANUP

2. FXTERIOR

INTERIOR

SHOP DRAWINGS

APPROVAL

OWNER.

SHEFTS

PRODUCTS AND SUBSTITUTIONS

FORFIGN MATTER

ADJACENT SURFACES

FINISHED SURFACES

RELATED DOCUMENTS AND COORDINATION

1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATIONS OF ALL MEASUREMENTS AT THE SITE BEFORE ORDERING ANY MATERIALS OR DOING ANY WORK, NO EXTRA CHARGE OR COMPENSATION SHALL BE ALLOWED DUE TO DIFFERENCE BETWEEN ACTUAL DIMENSIONS AND DIMENSIONS INDICATED ON THE CONSTRUCTION DRAWINGS. ANY SUCH DISCREPANCY IN DIMENSION WHICH MAY BE FOUND SHALL BE SUBMITTED TO THE OWNER FOR CONSIDERATION BEFORE THE CONTRACTOR PROCEEDS WITH THE WORK IN THE AFFECTED AREAS. 2. THE BIDDER, IF AWARDED THE CONTRACT, WILL NOT BE

- ALLOWED ANY EXTRA COMPENSATION BY REASON OF ANY MATTER OR THING CONCERNING SUCH BIDDER MIGHT HAVE FULLY INFORMED THEMSELVES PRIOR TO THE BIDDING
- 3. NO PLEA OF IGNORANCE OF CONDITIONS THAT EXIST, OR OF DIFFICULTIES OR CONDITIONS THAT MAY BE ENCOUNTERED. OR OF ANY OTHER RELEVANT MATTER CONCERNING THE WORK TO BE PERFORMED IN THE EXECUTION OF THE WORK WILL BE ACCEPTED AS AN EXCUSE FOR ANY FAILURE OR OMISSION THE PART OF THE CONTRACTOR TO FULFILL EVERY DETAIL OF THE REQUIREMENTS OF THE CONTRACT DOCUMENTS GOVERNING THE WORK

1. CONTRACTOR IS RESPONSIBLE FOR APPLICATION AND PAYMENT

1. ALL MATERIALS MUST BE STORED IN A LEVEL AND DRY FASHION

AND IN A MANNER THAT DOES NOT NECESSARILY OBSTRUCT THE

FLOW OF OTHER WORK. ANY STORAGE METHOD MUST MEET ALL

1. THE CONTRACTORS SHALL, AT ALL TIMES, KEEP THE SITE FREE

COMPLETION OF THE WORK. THEY SHALL REMOVE ALL RUBBISH

FROM AND ABOUT THE BUILDING AREA, INCLUDING ALL THEIR

A. VISUALLY INSPECT EXTERIOR SURFACES AND REMOVE ALL TRACES OF SOIL, WASTE MATERIALS, SMUDGES AND OTHER

CLEANLINESS. HOSE DOWN THE EXTERIOR OF THE STRUCTURE.

A VISUALLY INSPECT INTERIOR SURFACE AND REMOVE ALL TRACES OF SOIL, WASTE MATERIALS, SMUDGES AND OTHER

C. REMOVE PAINT DROPPINGS, SPOTS, STAINS, AND DIRT FROM

1. GENERAL CARPENTRY, ELECTRICAL AND ANTENNA DRAWINGS ARE

1. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AS REQUIRED AND

CONTRACTOR MUST REFER TO ALL DRAWINGS. ALL COORDINATION

FOREIGN MATTER FROM WALLS FLOOR AND CEILING

B. REMOVE ALL TRACES OF SPLASHED MATERIALS FROM

CHANGE ORDER PROCEDURE: 1. REFER TO SECTION 17 OF SIGNED MCSA: SEE PROFESSIONAL SERVICE AGREEMENT FOR MCSA.

INTERRELATED IN PERFORMANCE OF THE WORK THE

LISTED IN THESE SPECIFICATIONS TO THE OWNER FOR

2. ALL SHOP DRAWINGS SHALL BE REVIEWED, CHECKED AND

CORRECTED BY CONTRACTOR PRIOR TO SUBMITTAL TO THE

1. SUBMIT 3 COPIES OF EACH REQUEST FOR SUBSTITUTION. IN

INCLUDE RELATED SPECIFICATION SECTION AND DRAWING

COMPLIANCE WITH THE REQUIREMENTS FOR SUBSTITUTIONS

SAMPLES TO THE OWNER FOR APPROVAL IN LIEU OF CUT

2. SUBMIT ALL NECESSARY PRODUCT DATA AND CUT SHEETS

WHICH PROPERLY INDICATE AND DESCRIBE THE ITEMS

NUMBERS AND COMPLETE DOCUMENTATION SHOWING

EACH REQUEST, IDENTIFY THE PRODUCT OR FABRICATION OR INSTALLATION METHOD TO BE REPLACED BY THE SUBSTITUTION.

PRODUCTS AND MATERIALS BEING INSTALLED. THE CONTRACTOR

SHALL IF DEFMED NECESSARY BY THE OWNER SUBMIT ACTUAL

ARCHITECTURAL SYMBOLS

STORAGE

38

DETAIL REFERENCE KEY

- DRAWING DETAIL NUMBER-

EXISTING N.I.C.

LSHEFT NUMBER OF DETAIL -

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- RFFFR TO

RE: 2/A-3

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B. REMOVE ALL TRACES OF SPLASHED MATERIALS FROM

ADJACENT SURFACES. C. IF NECESSARY, TO ACHIEVE A UNIFORM DEGREE OF

TOOLS. SCAFFOLDING AND SURPLUS MATERIALS AND SHALL

RECOMMENDATIONS OF THE ASSOCIATED MANUFACTURER.

FROM ACCUMULATION OF WASTE MATERIALS OR RUBBISH

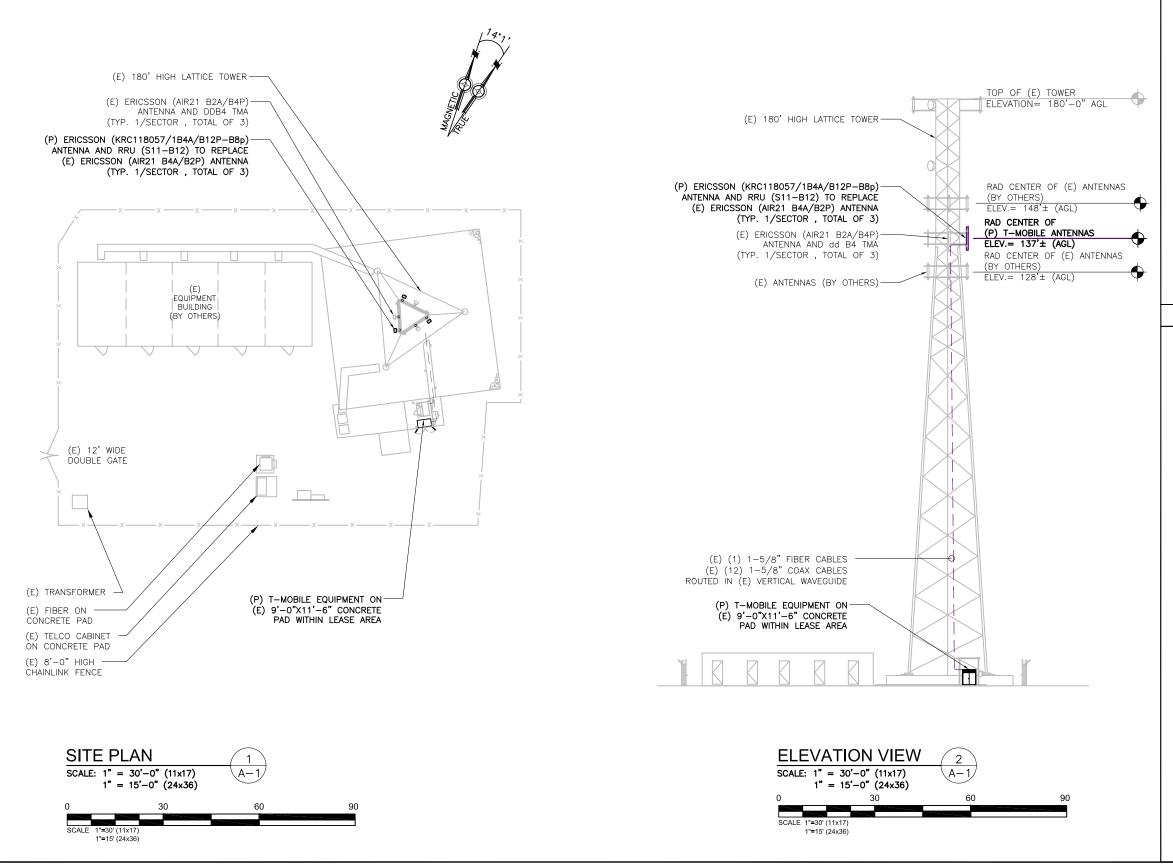
CAUSED BY THEIR EMPLOYEES AT WORK AND AT THE

LEAVE THEIR WORK CLEAN AND READY TO USE

2. SEE MASTER CONTRACTION SERVICES AGREEMENT FOR

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	MTL (N)	METAL NEW	SITE ADDRESS 150 BUTTERNUT
BOLS	NIC NTS OC	NOT IN CONTRACT NOT TO SCALE ON CENTER	HOLLOW RD GREENWICH ,CT 06830
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<u>GENERAL SITE NOTES</u>

1. SITE INFORMATION WAS OBTAINED FROM A FIELD INVESTIGATION PERFORMED BY ATLANTIS GROUP, INC. CONTRACTOR TO FIELD VERIFY DIMENSIONS AS NECESSARY BEFORE CONSTRUCTION.

2. THE PROPOSED DEVELOPMENT DOES NOT INCLUDE SIGNS OF ADVERTISING.

3. THE PROPOSED DEVELOPMENT IS UNMANNED AND THEREFORE DOES NOT REQUIRE A MEANS OF WATER SUPPLY OR SEWAGE DISPOSAL.

 ${\rm 4.}$ No landscaping work is proposed in conjunction with this development other than that which is shown.

5. THE PROPOSED DEVELOPMENT DOES NOT INCLUDE OUTDOOR STORAGE OR ANY SOLID WASTE RECEPTACLES.

6. UTILITIES SHOWN ON PLAN ARE TAKEN FROM OWNERS RECORDS AND FIELD LOCATION OF VISIBLE SURFACE FEATURES. THE EXISTENCE, EXTENT AND EXACT HORIZONTAL AND VERTICAL LOCATIONS OF UTILITIES HAS NOT BEEN VERIFIED. ANY CONTRACTOR PERFORMING WORK ON THIS SITE MUST CONTACT CALL BEFORE YOU DIG THREE WORKING DAYS PRIOR TO COMMENCING WORK.

7. ALL OBSOLETE OR UNUSED FACILITIES SHALL BE REMOVED WITHIN 12 MONTHS OF CESSATION OF OPERATIONS.

<u>SITE LEGEND</u>

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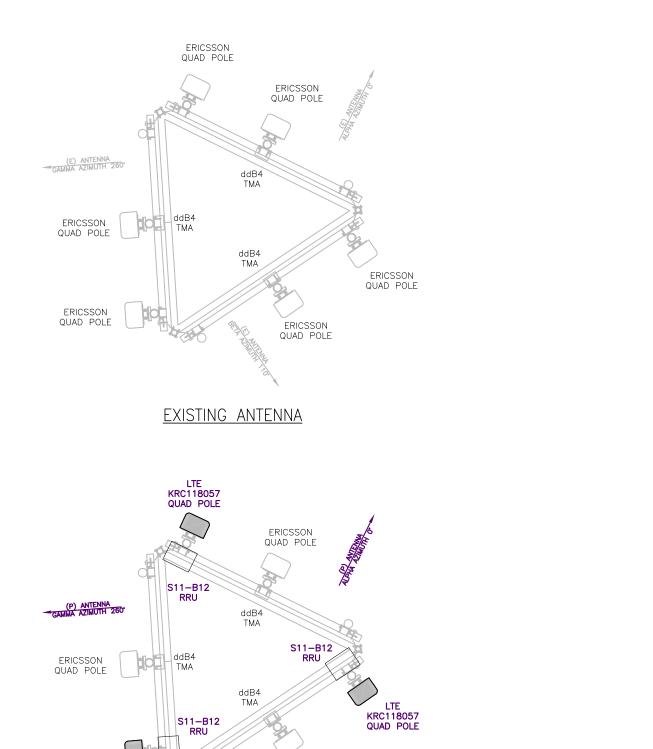
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PROP. UMTS/GSM ANTENNA

EX. GSM ANTENNA

EX. UMTS ANTENNA

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ERICSSON QUAD POLE

<u>1</u>

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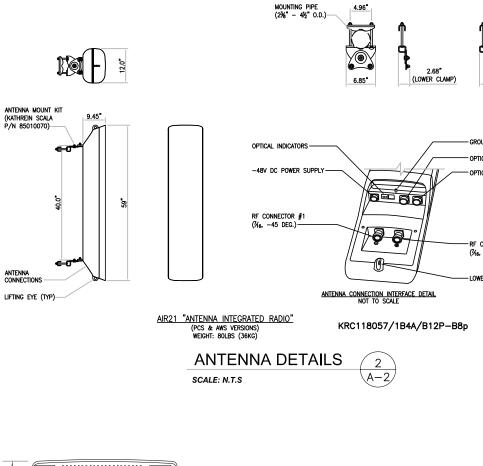
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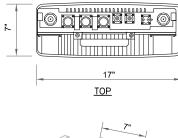
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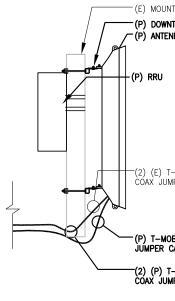
ANTENNA PLAN

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LTE KRC118057 QUAD POLE





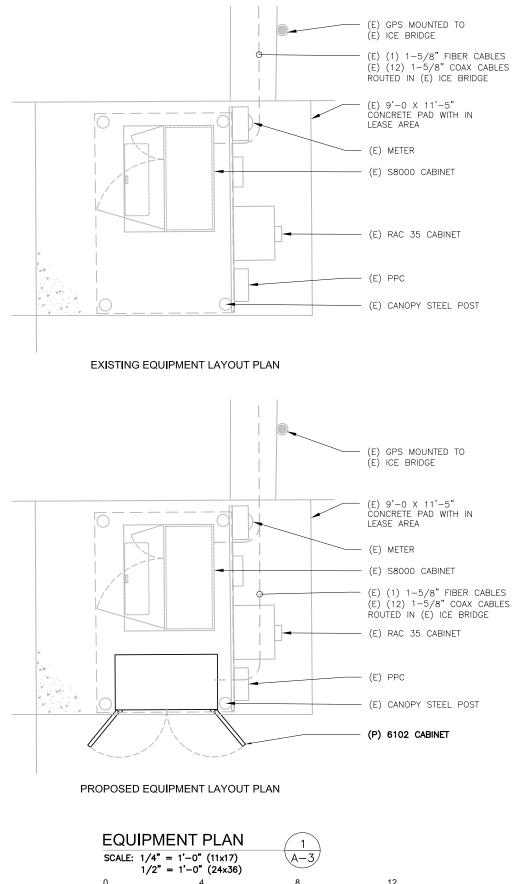


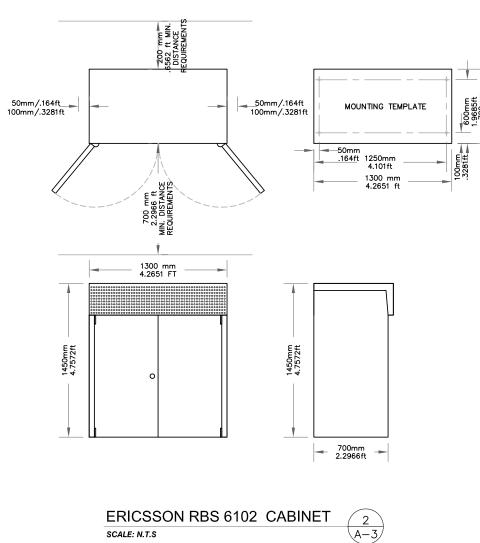
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REFER TO STRUCTURAL ANALYSIS DOCUMENT ENTITLED, "DETAILED STRUCTURAL ANALYSIS AND EVALUATION OF AN EXISTING 180' SELF SUPPORTING LATTICE TOWER WITH STACK-N-BOLT SYSTEM AND FOUNDATION FOR PROPOSED ANTENNA ARRANGEMENT" PREPARED BY AECOM, "T-MOBILE SITE ID CT1070B", DATED MARCH 4, 2015.	T-MOBILE NORTHEAST, LLC 35 GRIFIN ROAD SOUTH BLOOMFIELD, CT 06002 OFFICE: (680),092-7100
J.46" (UPPER CLAMP)	FAX:(860) 692-7159 TLANTIS G R O U P 1340 Centre Street, Suite 212 Newton Center, MA 02459 Office: 617–965–0789 Fax: 617–213–5056
SROUNDING POINT OPTICAL CABLE INPUT #1 OPTICAL CABLE INPUT #2	SUBMITTALS DATE DESCRIPTION REVISION 02/16/15 ISSUED FOR Review A 02/18/15 REVISION 0 02/18/15 REVISION 1 03/05/15 FINAL CD 2
RF CONNECTOR ∯2 %6, +45 DEG.) LOWER LIFTING EYE	DEPT. DATE APP'D REVISIONS RFE RF MAN.
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NTING PIPE NTILT KIT INNA	DF CONNECTION
	PROFESSIONAL SEAL THIS DOCUMENT IS THE CREATION, DESIGN, PROPERTY AND COPYRIGHTED WORK OF T-MOBILE. ANY DUPLICATION OR USE WITHOUT EXPRESS WRITTEN CONSENT IS STRICTLY PROHIBITED.
T-MOBILE UMTS MPERS	SITE NAME CT11070B SITE NAME CONNECTICUT STATE POLICE #2 SITE ADDRESS 150 DUTTEDNUT
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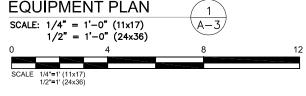
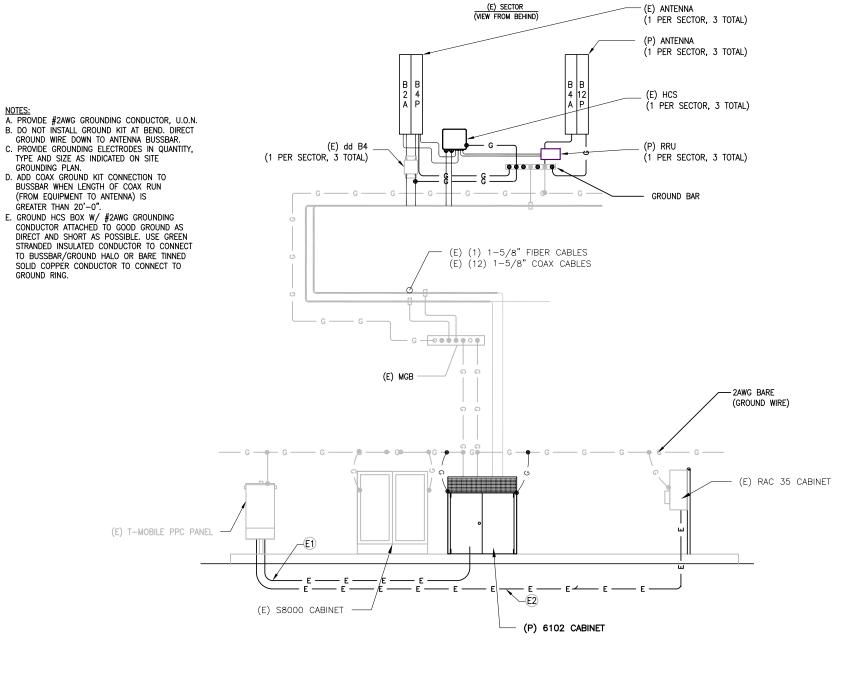


Image:	REFER TO STRUCTURAL ANALYSIS DOCUMENT ENTITLED, "DETAILED STRUCTURAL ANALYSIS AND EVALUATION OF AN EXISTING 180' SELF SUPPORTING LATTICE TOWER WITH STACK-N-BOLT SYSTEM AND FOUNDATION FOR PROPOSED ANTENNA ARRANGEMENT" PREPARED BY AECOM, "T-MOBILE SITE ID CT1070B", DATED MARCH 4, 2015.	T-MOBILE NORTHEAST, LLC 35 GRIFFIN ROAD SOUTH BLOWFFELD, CT 06002 OFFICE: (860) 692-7100 FAX:(860) 692-7100 FAX:(860) 692-7100 FAX:(860) 692-7100 FAX:(860) 692-7100 FAX:(860) 692-7159 SUBMITALS DATE DATE DATE DESCRIPTION REVISION 02/16/15 INAL CD 2
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GROUNDING DIAGRAM

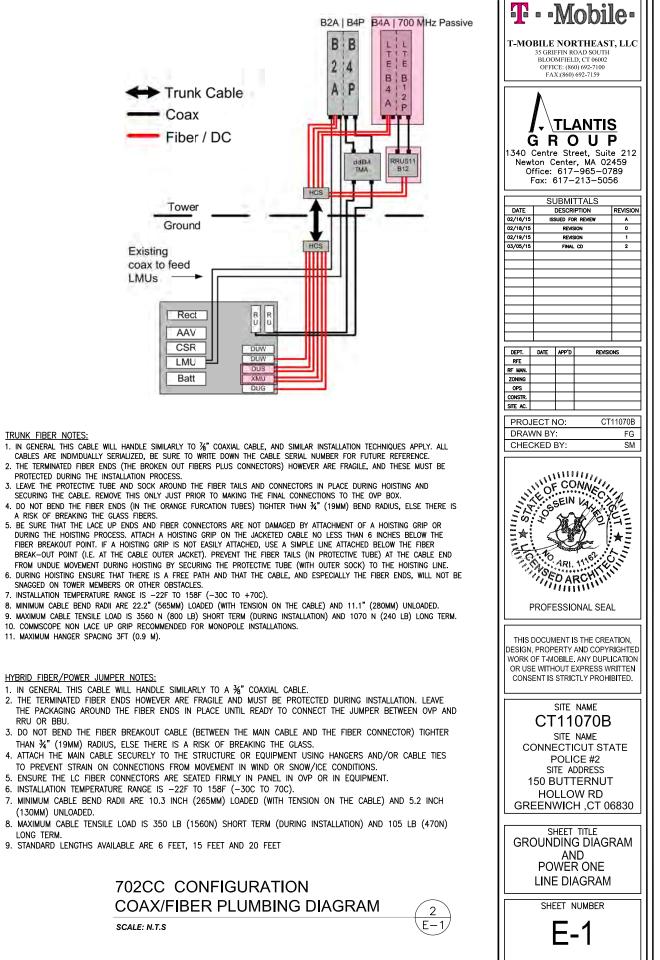
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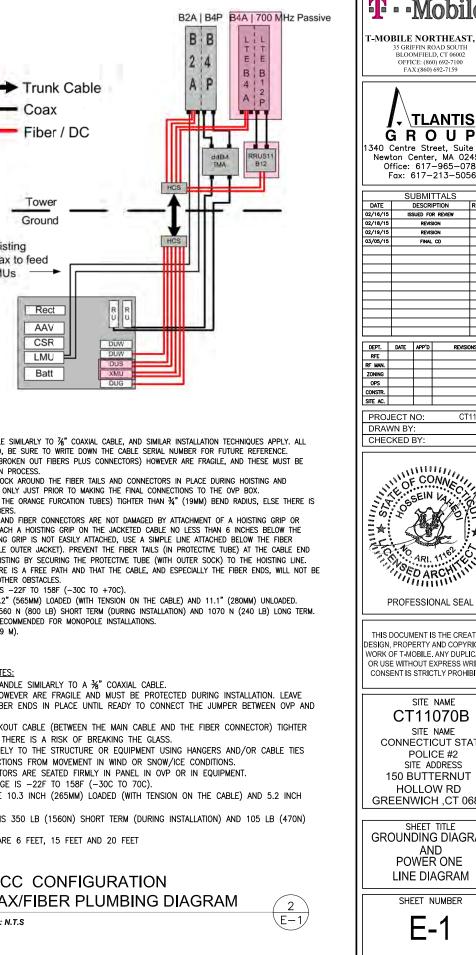
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E-1,

(E) ANTENNA

- B. DO NOT INSTALL GROUND KIT AT BEND. DIRECT
- TYPE AND SIZE AS INDICATED ON SITE
- BUSSBAR WHEN LENGTH OF COAX RUN (FROM EQUIPMENT TO ANTENNA) IS
- E. GROUND HCS BOX W/ #2AWG GROUNDING CONDUCTOR ATTACHED TO GOOD GROUND AS DIRECT AND SHORT AS POSSIBLE. USE GREEN STRANDED INSULATED CONDUCTOR TO CONNECT TO BUSSBAR/GROUND HALO OR BARE TINNED SOLID COPPER CONDUCTOR TO CONNECT TO GROUND RING.





FG

SM

TRUNK FIBER NOTES:

- PROTECTED DURING THE INSTALLATION PROCESS.
- A RISK OF BREAKING THE GLASS FIBERS.

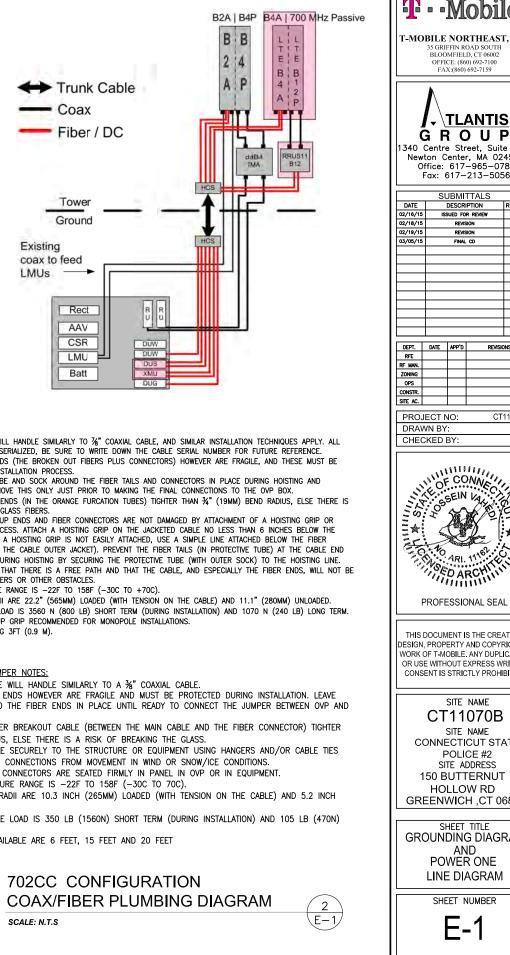
- SNAGGED ON TOWER MEMBERS OR OTHER OBSTACLES. 7. INSTALLATION TEMPERATURE RANGE IS -22F TO 158F (-30C TO +70C).

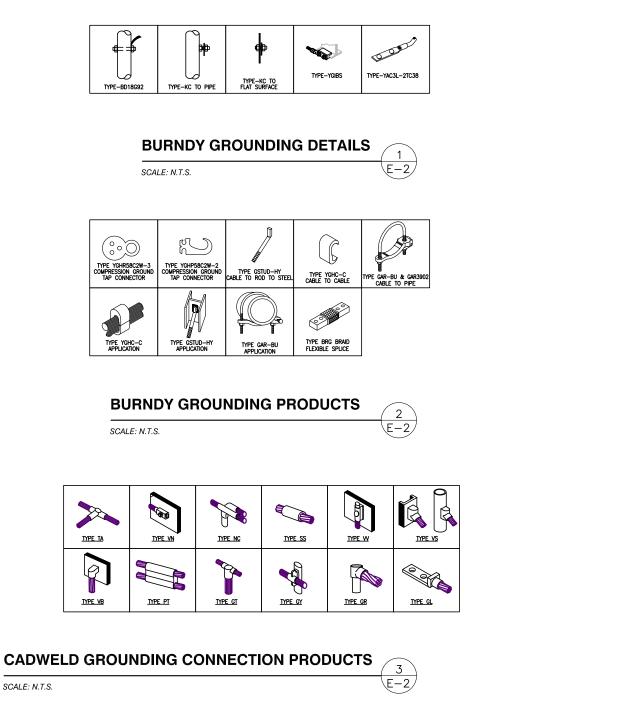
- 11. MAXIMUM HANGER SPACING 3FT (0.9 M).

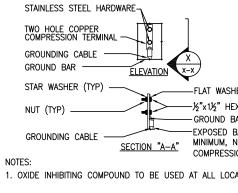
HYBRID FIBER/POWER JUMPER NOTES:

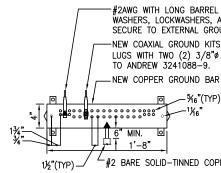
- RRU OR BBU

- 6. INSTALLATION TEMPERATURE RANGE IS -22F TO 158F (-30C TO 70C).
- (130MM) UNLOADED.
- LONG TERM
- 9. STANDARD LENGTHS AVAILABLE ARE 6 FEET, 15 FEET AND 20 FEET









NOTES:

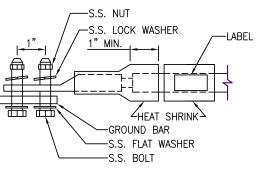
- 1. ALL HARDWARE STAINLESS STEEL COAT ALL SURFACES
- 2. FOR GROUND BOND TO STEEL ONLY: INSERT A TOOTH STEEL, COAT ALL SURFACES WITH KOPR-SHIELD.
- 3. ALL HOLES ARE COUNTERSUNK 1/6".

TYPICAL GROUND BAR CONN

SCALE: N.T.S. #6 FROM COAX CABLE GROUNDING KITS (TYP) -insu Mou ò)) (**o** -EXOTHERMICALLY WELD (2) #2 TO MAIN GROUND BAR--BARE #2 IN ¾" C TO GROUND RING -1'-4" -0 0 0 0 0 0 0 0 0 0 0 0 0 ò 1" (TYP) HOL MOL MAIN GROUND BAR HOLE CONFIGURATION

GRO

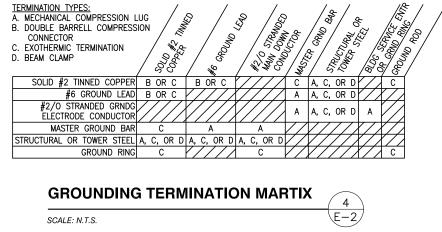
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	T - Mobile
	T-MOBILE NORTHEAST, LLC 35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002
	OFFICE: (860) 692-7100 FAX:(860) 692-7159
AT WASHER (TYP) x1½" HEX BOLT COUND BAR POSED BARE COPPER TO BE KEPT TO ABSOLUTE NIMUM, NO INSULATION ALLOWED WITHIN THE MPRESSION TERMINAL (TYP.)	TLANTIS G R O U P 1340 Centre Street, Suite 212 Newton Center, MA 02459
ALL LOCATIONS.	Office: 617-965-0789 Fax: 617-213-5056
G BARREL COMPRESSION LUGS, USE STAR ASHERS, AND STAINLESS STEEL HARDWARE TO RNAL GROUND BAR BY GENERAL CONTRACTOR.	SUBMITTALS DATE DESCRIPTION REVISION 02/16/15 ISSUED FOR REVIEW A 02/16/15 REVISION 0 02/16/15 REVISION 0
DUND KITS WITH LONG BARREL COMPRESSION 2) 3/8"Ø BOLTS AND LOCK WASHERS SIMILAR 088-9.	102/19/15 REVISION 1 103/05/15 FINAL CD 2
54 7(1770)	
- %ε"(TYP) -1¼ε"	
INED COPPER CONDUCTOR TO GROUND BUS.	DEPT. DATE APP'D REVISIONS RF
RFACES WITH KOPR-SHIELD BEFORE MATING. , TOOTH WASHER BETWEEN LUG AND D.	CONSTR. Image: Construct of the second
CONNECTIONS DETAIL 5 E-2	CHECKED BY: SM
INSULATORS ON BRACKETS MOUNTED TO MONOPOLE	SEIN LA SEIN LA CARL 1160 CONTRACTOR
MAIN GROUND BAR (HERGER CAT. NO TGBI142220G)	PROFESSIONAL SEAL
HOLE FOR HOLE FOR HOLM BRACKET	THIS DOCUMENT IS THE CREATION, DESIGN, PROPERTY AND COPYRIGHTED WORK OF T-MOBILE. ANY DUPLICATION OR USE WITHOUT EXPRESS WRITTEN CONSENT IS STRICTLY PROHIBITED.
GROUND BAR DETAIL SCALE: N.T.S.	SITE NAME CT11070B SITE NAME CONNECTICUT STATE POLICE #2 SITE ADDRESS 150 BUTTERNUT
LUG NOTES: 1. ALL HARDWARE IS 18–8 STAINLESS STEEL, INCLUDING LOCK WASHERS	HOLLOW RD GREENWICH ,CT 06830
 ALL HARDWARE SHALL BE S.S. ³/₈"ø OR LARGER. FOR GROUND BOND TO STEEL ONLY: INSERT A DRAGON TOOTH WASHER BETWEEN LUG AND STEEL. COAT ALL 	GROUNDING DETAILS
SURFACES WITH ANTI-OXIDIZATION COMPOUND PRIOR TO MATING.	SHEET NUMBER
ROUND BAR DETAIL	E-2
CALE: N.T.S. $E-Z$	

Exhibit B



Submitted to Northeast Site Solutions 199 Brickyard Road Farmington, CT 06032 Submitted by AECOM 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 September 17, 2015

DETAILED STRUCTURAL ANALYSIS AND MODIFICATION OF AN EXISTING 180' SELF SUPPORTING LATTICE TOWER WITH STACK-N-BOLT SYSTEM AND FOUNDATION FOR PROPOSED ANTENNA ARRANGEMENT

•••**T**••Mobile•

Site ID : Site Name: Site Address: CT11070B CT State Police_2 150 Butternut Hollow Road Greenwich, Connecticut CSP Tower # 74

NSS-036

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- 2. INTRODUCTION
- 3. ANALYSIS METHODOLOGY AND LOADING CONDITIONS
- 4. FINDINGS AND EVALUATION
- 5. CONCLUSIONS
- 6. ANALYSIS DATA
 - TOWER REINFORCEMENT DRAWINGS SK-1 THROUGH SK-3
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 - PLS-TOWER INPUT / OUTPUT SUMMARY
 - PLS-TOWER NODE LOCATIONS
 - PLS-TOWER MEMBER LOCATIONS
 - PLS-TOWER DETAILED OUTPUT
 - CONNECTION BETWEEN TOWERS EVALUATION
 - FOUNDATION EVALUATION

1. EXECUTIVE SUMMARY

This report summarizes the structural analysis and modification of the 180' dual lattice tower located off of Butternut Hollow Road in Greenwich, Connecticut. The analysis was conducted in accordance with the 2005 Connecticut State Building Code, the TIA/EIA-222-F standard and additional requirements of the Connecticut State Police for wind velocity of 90 mph concurrent with $\frac{1}{2}$ " ice design wind load. The antenna loading considered in the analysis consists of all existing and proposed antennas, transmission lines, and ancillary items as outlined in the Introduction section of this report.

The proposed T-Mobile installation is as follows:

Proposed Antenna and Mount	Carrier	Antenna Center Elevation
<u>Remove:</u> (3) AIR B4A/B2P Panel Antennas	T-Mobile (Existing)	@ 137'
<u>Install:</u> (3) AIR B4A/B12P Panel Antennas (3) Ericsson RRUS-11 RRH Units	T-Mobile (Proposed)	@ 137'

The results of an initial analysis indicated the existing tower structure did not have enough capacity for the proposed loading conditions stated above. The tower structure requires modifications shown on SK-1 through SK-3. Once the modifications indicated on sheets SK-1 through SK-3 are performed, the modified structure and foundation are considered structurally adequate with the wind classification specified above with the existing and proposed antenna loading.

The tower deflection (sway) is 0.72 degrees and the tower rotation (twist) is 0.02 degrees. These figures are below the Connecticut State Police Specification of 0.75 degrees for deflection (sway) and (rotation) twist.

1. **EXECUTIVE SUMMARY** (continued)

This analysis is based on:

- 1) The tower structure's theoretical capacity not including any assessment of the condition of the tower.
- 2) Member sizes and tower geometry of the outer tower taken from manufacturers drawings prepared by Rohn Industries, Inc., file number 28325, dated December 28, 1992.
- 3) Member sizes and tower geometry of the inner tower taken from design calculations and drawings prepared by Towertek Industries Inc., signed and sealed May 9, 2002.
- 4) Foundation modifications taken from drawings prepared by Walker Engineering Incorporated, Job number 0206-237R2, signed and sealed November 26, 2002.
- 5) Antenna inventory provided by the Connecticut State Police via email on February 1 2015.
- 6) Proposed antennas via T-Mobile Radio Frequency Data Sheet (RFDS) form, dated February 5, 2015.
- 7) Previous structural analysis performed by AECOM on behalf of T-Mobile project number NSS-035, signed and sealed on August 20, 2015.
- 8) Antenna inventory as specified in section 2 and 6 of this report.

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This report is only valid as per the assumptions and data utilized in this report for antenna inventory, mounts and associated cables. The user of this report shall field verify the antenna, cabling, and mount configuration used, as well as the physical condition of the tower members, connections, and foundation. Notify the engineer in writing immediately if any of the information in this report is found to be other than specified.

If you should have any questions, please call.

Sincerely,

AECOM, contracting as URS Corporation AES,

Richard A. Sambor, P.E. Senior Structural Engineer

RAS/mcd Cc: IA, CF/Book – AECOM

NSS-036

2. INTRODUCTION

The subject tower is located off of Butternut Hollow Road in Greenwich, Connecticut. The original outer structure is a self-supporting three-legged 180' steel tapered lattice tower manufactured by Rohn Industries with a Stack-N-Bolt system installed inside the original tower, designed by Towertek.

The existing structure supports numerous communication antennas. The inventory is summarized below:

Antenna Type	Carrier	Mount	Centerline Elevation / Leg	Cable
(3) 6' HP Dish	CSP 75 to 77 (reserved)	Dish Mount	180 / ABC	N/A
(1) PD-420	NEU – 55 (existing)	3' Stand-Off	180 / A	(1) 7/8"
(1) DB-583	TOG – 5 (existing)	Shared with Above	180 / A	(1) 1-5/8"
(1) Scala OGT9-806N (1) Sinclair SC479- HF1LDF (1 upright & 1 inverted)	CSP - 1 & 3 (existing)	3' Stand-Off	180 / B	(2) 1-5/8"
(2) Sinclair SC479- HF1LDF (inverted) (1) TMA	CSP 2, 4 & 74 (existing)	3' Stand-Off	180 / C	(2) 1-5/8" (1) 1/2"
(1) PD-420	NEU – 20 (existing)	Shared with Above (Omni @ 180)	178 / A	(1) 7/8"
6' HP Dish with Radome	TOG – 7 (existing)	Dish Mount	177 / B	(1) Elliptical Cable
(1) Sinclair SC479- HF1LDF	CSP – 67 (existing)	Leg Mount	176 / C	(1) 1-5/8"
6' Dish	CSP – 31 (existing)	Dish Mount	176 / A	(1) WEP65
DB-586-Y	TOG - 6 (existing)	Leg Mounted	174 / A	(1) 1-5/8"
(1) AP1185	Stamford 63 (existing)	3' Arm	165 / A	(1) 1-1/4"
(1) AP1185 (1) TMA	Stamford 64 & 65 (existing)	3' Arm	160 / A	(1) 1-1/4" (1) 3/8"
Gabriel GLF6-940	SPD - 9 (existing)	Dish Mount	160 / A	(1) EW90
(3) Sinclair SC-479- HF1LDF (1 upright, 2 inverted) (1) TMA	CSP 70 to 73 (existing)	3' Stand-Off	160	(3) 1-5/8" (1) 1/2"
 (6) Powerwave 7770 (12) TMAs (3) Powerwave P65-16-XLH-RR (6) Ericsson RRU (1) Raycap Surge Suppressor 	AT&T (existing)	Side Arm	150 / ABC	(12) 1-5/8" (1) Fiber Optic Cable (2) DC Cables

Antenna Type	Carrier	Mount	Centerline Elevation / Leg	Cable
(3) AIR B4A/B12P Panel Antennas (3) Ericsson RRUS-11 RRH Units	T-Mobile (Proposed)	See Below Mount	137 / ABC	See Below Cables
(3) AIR B2A/B4P Panel Antennas (3) (AWS) TMA's	T-Mobile (existing)	Face Mounted	137 / ABC	(12) 1-5/8" (1) Fiber Optic Cable
(1) DB-586-Y	NEU-19 (existing)	Leg Mounted	135 / B	(1) 7/8"
(1) Celwave PD1142	CSP – 21 (existing)	Shared with Above	135 / B	(1) 7/8"
(1) Kreco CO41AN	NEU – 18 (existing)	3' Stand-off	130 / A	(1) 7/8"
 (3) BXA-171063-8BF- EDIN-0 Panel Antennas (AWS) (3) SLCP 2x6014 Panels (700 MHz) (6) Andrew DB844H80- XY Panels (BXA-171063-8BF- EDIN-2 Panel Antennas (PCS) (6) Diplexers (1) Raycap DB-T1-6Z- 8AB-0Z Distribution Box (AWS) 	Verizon (existing)	(3) Boom Gates (existing)	130 / ABC	(12) 1 5/8" (1) 1-5/8" Fiber Optic Cable
(3) APXVSPP18-C Panel Antennas (6) RRH	Sprint (existing)	Boom Gate (existing)	117 / ABC	(3) Hybriflex Cables
(1) Celwave PD1142	NEU – 17 (existing)	3' Stand-off	115 / A	(1) 7/8"
(1) Celwave PD1142	NEU – 16 (existing)	Shared with Above	110 / A	(1) 1-5/8"
(1) Celwave PD1142	CSP – 66 (existing)	Leg Mounted	80 / A	(1) 7/8"
(1) 10' Dipole	DOT – 56 (existing)	3' Arm	80 / B	(1) 7/8"
(1) Celwave PD1142	DEP – 54 (existing)	Leg Mounted	80 / C	(1) 7/8"
(1) GPS	Sprint - 69 (existing)	Leg Mounted	62 / B	(1) 1/2"
(1) GPS (TMG-26N)	Verizon - 68 (existing)	Leg Mounted	60 / C	(1) 1/2"

This structural analysis and evaluation of the communications tower was performed by AECOM for T-Mobile. The purpose of this analysis was to investigate the structural integrity of the modified tower structure with its existing and proposed antenna loads. The analysis was also conducted to evaluate twist (rotation), sway (deflection), and stress on the tower.

3. ANALYSIS METHODOLOGY AND LOADING CONDITIONS

The structural analysis was done in accordance with the 2005 Connecticut State Building Code, TIA/EIA-222-F—Structural Standard for Steel Antenna Towers and Antenna Supporting Structures, and the American Institute of Steel Construction (AISC) Manual of Steel Construction—Allowable Stress Design (ASD).

The analysis was conducted using PLS-Tower. Two load conditions were evaluated as shown below which were compared to allowable stresses according to AISC and TIA/EIA.

Load Condition 1 = 90 mph (fastest mile) Wind Load + Tower Dead Load Load Condition 2 = 90 mph (fastest mile) Wind Load (with ice) + Ice Load + Tower Dead Load

The TIA/EIA standard permits one-third increase in allowable stresses for towers and monopoles less than 700 feet tall. For purposes of this analysis, in computing the load capacity the allowable stresses of the tower members were increased by one-third.

4. FINDINGS AND EVALUATION

The combined axial and bending stresses on the tower structure were evaluated to compare with the allowable stress in accordance with AISC. The results of an initial analysis indicated that the existing tower did not have enough capacity to support the proposed loading conditions. The tower structure requires modifications shown on SK-1 through SK-3. Once the modifications indicated on sheets SK-1 through SK-3 are performed, the modified structure is considered structurally adequate with the wind load classification specified with the existing and proposed antenna loading noted herein. See the below tables for tower capacity and tower deflection (sway) and rotation (twist) figures:

Component	Allowable	Actual
Twist	0.75%	0.02°
Sway	0.75° –	0.72°

Proposed Tower Component Stress vs Capacity Summary

Component	Component Size	Controlling Member	Stress (% Capacity)	Pass/Fail
Rohn Diagonal	L2.5x2.5x1/4"	Rohn-DC51	90.44	Pass
Rohn Leg Modified Pipe 5 STD w/ 1⁄4" bent plate		Rohn-LD3P	82.46	Pass
Rohn Horizontal	L1.75x1.75x3/16	Rohn-H12	9.20	Pass
Interior Tower Diagonal	L5x5x5/8	SNB-DI11	75.06	Pass
Interior Tower Leg	Pipe 8 SCH 80 (Extra Strong)	SNB-LH2P	91.08	Pass
Interior Tower Horizontal	Pipe4x0.494 (Pipe 4 XXS)	SNB-H8dP	6.70	Pass
Tower Connection	A325 Bolt	3/4" Bolt	78.6	Pass
Foundation 36.5' Square		Overturning Moment (F.S. = 2.0 min)	2.06 / 97.28	Pass

Notes:

- 1. "SNB" member designations under the "Controlling Member" section of the above table refer to the interior tower members in the PLS-Tower analysis program.
- 2. "F.S." refers to the Factor of Safety of the tower foundation to resist the tower from turning over by a multiplied value of 2.0, as required by the Connecticut State Building Code.

5. CONCLUSIONS

The results of an initial analysis indicated the existing tower structure did not have enough capacity for the proposed loading conditions stated above. The tower structure requires modifications shown on SK-1 through SK-3. Once the modifications indicated on sheets SK-1 through SK-3 are performed, the modified structure and foundation are considered structurally adequate with the wind classification specified above with the existing and proposed antenna loading.

The tower deflection (sway) is 0.72 degrees and the tower rotation (twist) is 0.02 degrees. These figures are below the Connecticut State Police Specification of 0.75 degrees for deflection (sway) and (rotation) twist.

Limitations/Assumptions:

This report is based on the following:

- A. Tower is properly installed and maintained.
- B. All members and their geometry are as specified in the original manufacturer drawings and are in good condition.
- C. All required members are in place.
- D. All bolts are in place and are properly tightened.
- E. Tower is in plumb condition.
- F. All member protective coatings are in good condition.
- G. All tower members were properly designed, detailed, fabricated, installed, and have been properly maintained since erection.
- H. Foundations are in good condition without defect and were properly constructed to support original design loads as specified in the original design documents.

AECOM is not responsible for any modifications completed prior to or hereafter in which AECOM is not or was not directly involved. Modifications include but are not limited to:

- A. Adding antennas
- B. Removing/replacing antennas
- C. Adding coaxial cables

AECOM hereby states that this document represents the entire report and that it assumes no liability for any factual changes that may occur after the date of this report. All representations, recommendations, and conclusions are based upon information contained and set forth herein. If you are aware of any information which conflicts with that which is contained herein, or you are aware of any defects arising from original design, material, fabrication, or erection deficiencies, you should disregard this report and immediately contact AECOM. AECOM disclaims all liability for any representation, recommendation, or conclusion not expressly stated herein.

Ongoing and Periodic Inspection and Maintenance:

After the Contractor has successfully completed the installation and the work has been accepted, the owner will be responsible for the ongoing and periodic inspection and maintenance of the tower.

The owner shall refer to TIA/EIA-222-F for recommendations for maintenance and inspection. The frequency of the inspection and maintenance intervals is to be determined by the owner based upon actual site and environmental conditions. It is recommended that a complete and thorough inspection of the entire tower structural system be performed at least yearly and more frequently as conditions warrant. According to TIA/EIA-222-F section 14.1, Note 1: It is recommended that the structure be inspected after severe wind and/or ice storms or other extreme loading conditions.

6. ANALYSIS DATA

TOWER REINFORCEMENT DRAWINGS SK-1 THROUGH SK-3

GENERAL CONSTRUCTION NOTES

- 1 ALL WORK SHALL COMPLY WITH THE CONNECTICUT STATE BUILDING AND LIFE SAFETY CODES, SUPPLEMENTS AND AMENDMENTS
- 2. CONTRACTOR IS TO REVIEW ALL DRAWINGS AND NOTES IN THE CONTRACT DOCUMENT SET CONTRACTOR SHALL COORDINATE ALL WORK SHOWN IN THE SET OF DRAWINGS. THE CONTRACTOR SHALL PROVIDE A COMPLETE SET OF DRAWINGS TO ALL SUB-CONTRACTORS AND ALL RELATED PARTIES. THE SUB-CONTRACTORS SHALL EXAMINE ALL THE DRAWINGS AND SPECIFICATIONS FOR THE INFORMATION THAT AFFECTS THEIR WORK.
- CONTRACTOR SHALL PROVIDE A COMPLETE BUILD-OUT WITH ALL FINISHES, STRUCTURAL, MECHANICAL, AND ELECTRICAL COMPONENTS AND PROVIDE ALL ITEMS AS SHOWN OR INDICATED ON DRAWINGS OR WRITTEN IN SPECIFICATIONS.
- 4. CONTRACTOR SHALL FURNISH ALL MATERIAL, LABOR AND EQUIPMENT TO COMPLETE THE WORK AND FURNISH A COMPLETED JOB ALL IN ACCORDANCE WITH LOCAL AND STATE GOVERNING AUTHORITIES AND OTHER AUTHORITIES HAVING LAWFUL JURISDICTION OVER THE WORK.
- CONTRACTOR SHALL SECURE AND PAY FOR ALL PERMITS AND ALL INSPECTIONS REQUIRED AND SHALL ALSO PAY FEES REQUIRED FOR THE GENERAL CONSTRUCTION AND ELECTRICAL SUB-CONTRACTORS SHALL PAY FOR THEIR PERMITS.
- 6. CONTRACTOR SHALL MAINTAIN A CURRENT SET OF DRAWINGS ON SITE AT ALL TIMES AND ENSURE THE DISTRIBUTION OF NEW DRAWINGS TO SUB-CONTRACTORS AND OTHER RELEVANT PARTIES AS SOON AS THEY ARE MADE AVAILABLE, ALL OLD DRAWINGS SHALL BE MARKED VOID AND REMOVED FROM THE CONTRACT AREA_CONTRACTOR SHALL FURNISH 'AS-BUILT' SET OF DRAWINGS TO OWNER UPON COMPLETION OF PROJECT.
- 7. INSTALLATION OF THIS WIRELESS COMMUNICATIONS EQUIPMENT SITE REQUIRES WORK IN THE IMMEDIATE VICINITY OF EXISTING OPERATING TELECOMMUNICATION SYSTEMS. THE CONTRACTOR SHALL PROVIDE AND COORDINATE THE METHODS OF PROTECTION WITH THE VARIOUS TELECOMMUNICATION CARRIERS AND THE TOWER OWNER. THERE SHALL BE NO INTERRUPTION OF OPERATION WITHOUT TIMELY COORDINATION WITH AND APPROVAL BY THE VARIOUS COMMUNICATIONS OPERATORS INCLUDING THE CONNECTICUT STATE POLICE.
- 8, THE REINFORCEMENT OF PORTIONS OF THIS TOWER STRUCTURE WILL AFFECT CRITICAL CONNECTICUT STATE POLICE ANTENNAS.
- NO MOVEMENT, ALTERATION, OR DISCONNECTION OF CONNECTICUT STATE POLICE ANTENNAS MAY OCCUR WITHOUT THE NOTIFICATION AND APPROVAL OF THE CONNECTICUT STATE POLICE, CONTACT THE NETWORK CONTROL CENTER AT 860-865-8008.
- 10, TOWER REINFORCING WORK AFFECTING CRITICAL CONNECTICUT STATE POLICE ANTENNAS MAY BE REQUIRED TO BE CONDUCTED AT TIMES AS DETERMINED BY THE REQUIREMENTS OF THE CONNECTICUT STATE POLICE.

- 11. ALL EQUIPMENT AND PRODUCTS PURCHASED ARE TO BE REVIEWED BY CONTRACTOR AND ALL APPLICABLE SUB-CONTRACTORS FOR ANY CONDITION PER MFR'S RECOMMENDATIONS, CONTRACTOR TO SUPPLY THESE ITEMS AT NO COST TO OWNER OR ARCHITECT.
- 12. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ON-SITE SAFETY FROM THE TIME THE JOB IS AWARDED UNTIL ALL WORK IS COMPLETE AND ACCEPTED BY THE OWNER.
- 13. CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO ARCHITECT FOR REVIEW, DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITTAL TO THE ARCHITECT FOR REVIEW
- 14. THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES, AND EXISTING CONDITIONS AT THE SITE, PRIOR TO FABRICATION AND/OR INSTALLATION OF ANY WORK IN THE CONTRACT AREA, SUBMIT ANY DISCREPANCIES FROM THE DRAWINGS TO THE ARCHITECT,
- 5. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURE AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY.
- 16. CONTRACTOR TO CONTACT "CALL BEFORE YOU DIG" AT 1-800-922-4455 TO VERIFY AND IDENTIFY THE EXACT LOCATIONS OF ALL UNDERGROUND UTILITIES AND OBSTRUCTIONS IDENTIFIED PRIOR TO COMMENCING WORK IN THE CONTRACT AREA.
- 17, CONTRACTOR SHALL COMPLY WITH OWNER ENVIRONMENTAL ENGINEER ON ALL METHODS AND PROVISIONS FOR ALL EXCAVATION ACTIVITIES INCLUDING SOIL DISPOSAL. ALL BACKFILL MATERIALS TO BE PROVIDED BY THE CONTRACTOR.
- 18. DIMENSIONS OF EXISTING TOWER ARE BASED ON MANUFACTURER'S DRAWINGS PREPARED BY ROHN INDUSTRIES, INC., DATED DECEMBER 1992, AND ARE NOT GUARANTEED. CONTRACTOR SHALL TAKE FIELD DIMENSIONS AS NECESSARY TO ASSURE PROPER FIT OF ALL FINISHED WORK AND SHALL ASSUME FULL RESPONSIBILITY FOR THEIR ACCURACY. WHEN SHOP DRAWINGS BASED ON FIELD MEASUREMENT ARE SUBMITTED FOR REVIEW, DIMENSIONS ARE PROVIDED FOR THE ENGINEER'S REFERENCE ONLY.
- 19. TOWER INVENTORY IS BASED ON INFORMATION OBTAINED BY CONNECTICUT STATE POLICE DATED FEBRUARY 2015_
- 20. CONTRACTOR TO VERIFY REQUIRED CLEARANCES INCLUDING BUT NOT LIMITED TO EXISTING BUILDINGS, EQUIPMENT PADS AND SHELTERS PRIOR TO COMMENCING WORK.
- 21. THE CONTRACTOR IS RESPONSIBLE FOR THE STABILITY OF THE STRUCTURE DURING CONSTRUCTION. NO MEMBER OF THE TOWER SHALL BE LEFT DISCONNECTED FOR THE NEXT WORKING DAY. THE CONTRACTOR SHALL BE AWARE OF WEATHER AND WIND CONDITIONS AND NOT PERFORM MEMBER REPLACEMENT IN A WIND.

STRUCTURAL NOTES

STRUCTURAL STEEL MATERIAL

STRUCTURAL STEEL BEAMS, CHANNELS, PLATES	A36
STRUCTURAL ANGLES:	
ANGLE SIZE 2-1/2"x2-1/2"x1/4" AND SMALLER	A36
ANGLE SIZE GREATER THAN 2-1/2"x2-1/2"x1/4" A 572-G	50
EXISTING TOWER LEG ROHN PIPE	50

STRUCTURAL STEEL SHALL CONFORM TO ALL THE REQUIREMENTS OF THE ASTM SPECIFICATION, AS REFERENCED IN THE CODE.

UNLESS OTHERWISE NOTED, ALL STEEL WILL BE GALVANIZED IN ACCORDANCE WITH ASTM 123 AFTER FABRICATION. TOUCH UP ALL DAMAGED GALVANIZED STEEL WITH APPROVED COLD ZINC, "GALVANOX", "DRY GALV", "ZINC-II", OR APPROVED EQUIVALENT, IN ACCORDANCE WITH MANUFACTURERS GUIDELINES. TOUCH-UP DAMAGED NON GALVANIZED STEEL WITH SAME PAINT APPLIED IN SHOP OR FIELD.

SHOP AND ERECTION DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL STEEL WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. SUBMIT 2 SETS OF PRINTS FOR THE ENGINEER REVIEW.

MILL BEARING ENDS OF COLUMNS, STIFFENERS, AND OTHER BEARING SURFACES TO TRANSFER LOAD OVER ENTIRE CROSS SECTION

THE OMISSION OF ANY MATERIAL THAT WAS SHOWN ON THE CONTRACT DRAWINGS SHALL NOT RELIEVE THE CONTRACTOR OF PROVIDING THE SAME

CONNECTIONS / FIELD ASSEMBLY:

BOLTED CONNECTIONS: UNLESS OTHERWISE NOTED, ALL JOINTS ARE SLIP CRITICAL TYPE, REQUIRING $5/8^{\prime\prime}$ DIA. A325-N BOLTS, A563 NUTS AND F436 WASHERS, ALL GALVANIZED. BEVELED WASHERS SHALL BE USED ON BEAM FLANGES HAVING A SLOPE GREATER THAN 1:20,

STRUCTURE IS DESIGNED TO BE LEVEL AND PLUMB, SELF-SUPPORTING AND STABLE AFTER WORK IS COMPLETED,

COMMENCEMENT OF WORK WITHOUT NOTIFYING THE ENGINEER OF ANY DISCREPANCIES WILL BE CONSIDERED ACCEPTANCE OF PRECEDING WORK.

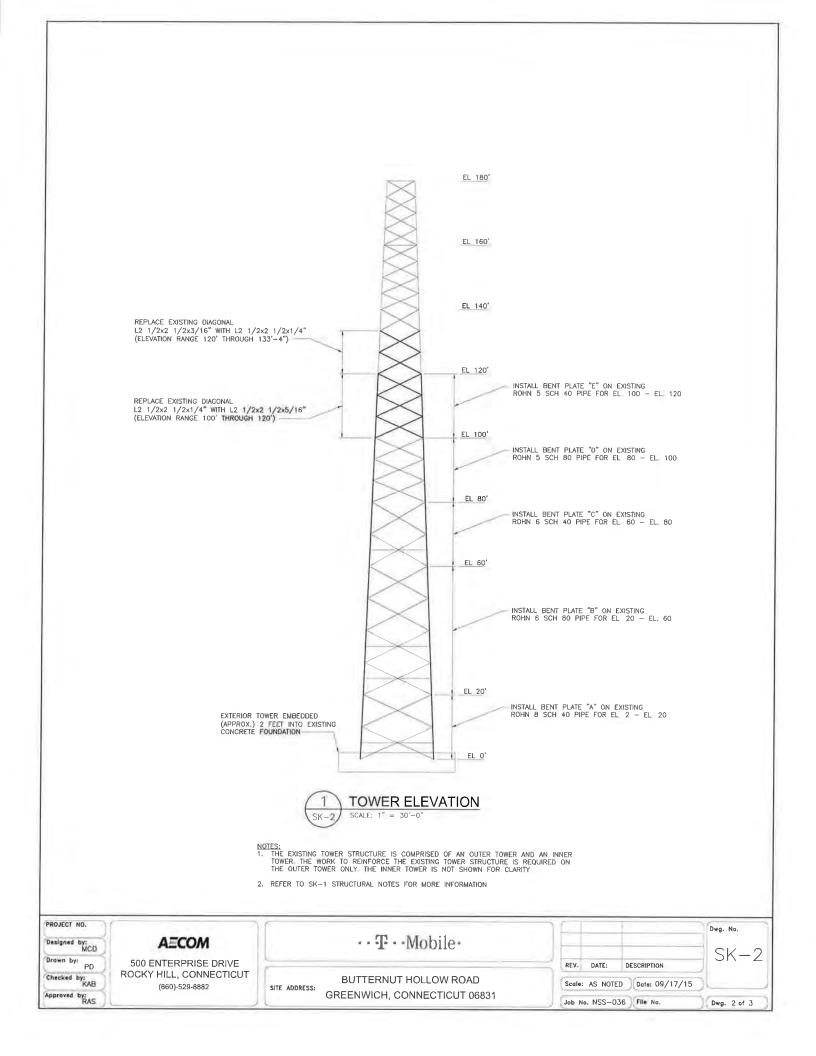
INSPECTIONS:

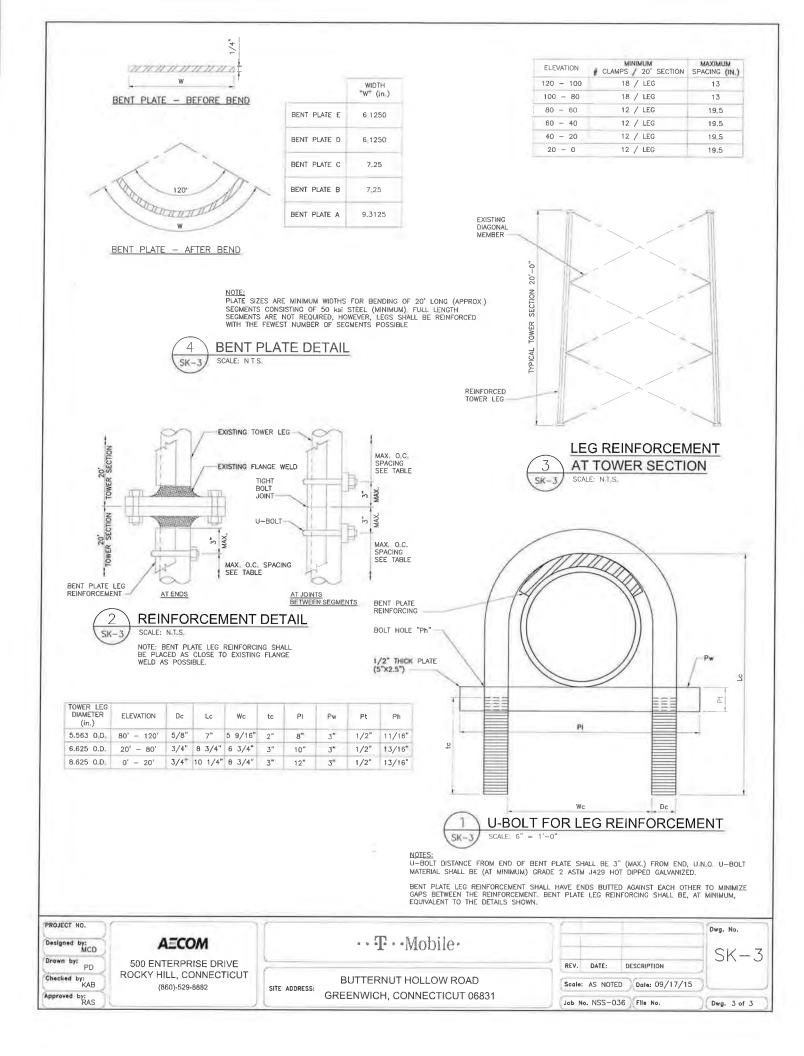
SPECIAL INSPECTIONS ARE REQUIRED PER THE CODE FOR STRUCTURAL STEEL WORK.

OWNER WILL SUPPLY THE SERVICES OF A SPECIAL INSPECTOR AND TESTING AGENTS AS REQUIRED. CONTRACTOR SHALL COORDINATE INSPECTIONS OF FABRICATOR'S AND ERECTOR'S WORK AND MATERIALS TO MEET THE REQUIREMENTS OF THE STATEMENT OF SPECIAL INSPECTIONS FOR THIS PROJECT.

COPIES OF TESTING AND INSPECTION REPORTS WILL BE PROVIDED TO THE OWNER, BUILDING OFFICIAL, ENGINEER OF RECORD AND CONTRACTOR.

Designed by: MCD	AECOM		$\cdots T \cdots Mobile \cdot$				Dwg. No.
Drawn by: PD	500 ENTERPRISE DRIVE	2		REV.	DATE:	DESCRIPTION	
Checked by: KAB	ROCKY HILL, CONNECTICUT (860)-529-8882	SITE ADDRESS:	BUTTERNUT HOLLOW ROAD	Scale:	AS NOTED	Dale: 09/17/15	<u></u>
Approved by: RAS	·		GREENWICH, CONNECTICUT 06831	Job N	. NSS-03	5 File No.	Dwg. 1 of 3





TOWER LEG REINFORCEMENT - CALCULATION DATA

AEC	COM			Page	of
Job	180' ROHN SSV Tower w/ SNB Reinf Greenwich	Project No.	NSS-035	Sheet	of
Description	Bent Plate Properties	Computed by	MCD	Date	09/14/15
		Checked by		Date	

Properties for 1/4" Bent Plate simulted as 120° cut pipe

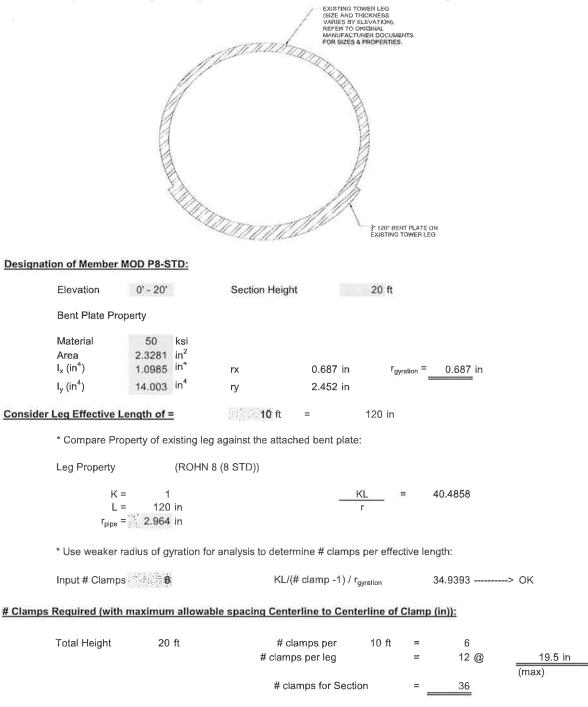
· /			0.2500
		ЭТН	
PLS Tower Modified Member Description	EXP-1 (L8 = P8 SCH	40)	
Elevation Range Arc Length of Plate Area of Plate	0' - 20' 9.3125 in 2.32813 in ²		
Moment of Inertia (I_x) (in ⁴)	1.0985 in ⁴	r _x	0.68691 in
Moment of Inertia (I_y) (in ⁴)	14.0027 in4	r _y	2.45246 in
Radius of Gyration (lowest value governs)		r gyration	= 0.68691 in
PLS Tower Modified Member Description	EXP-2 (L7 = P6 SCH	80)	
Elevation Range Arc Length of Plate Area of Plate	20' - 60' 7.25 in 1.8125 in ²		
Moment of Inertia (I _x) (in ⁴)	0.4926 in ⁴	r _x	0.52132 in
Moment of Inertia (I _y) (in ⁴)	6.2451 in ⁴	r _y	1.85623 in
Radius of Gyration (lowest value governs)		r _{gyration} :	= 0.52132 in
PLS Tower Modified Member Description	EXP-3 (L6 = P6 STD))	
Elevation Range Arc Length of Plate Area of Plate	60' - 80' 7.25 in 1.8125 in ²		
Moment of Inertia (I _x) (in ⁴)	0.4926 in ⁴	r _x	0.52132 in
Moment of Inertia (I_y) (in ⁴)	6.2451 in ⁴	r _y	1.85623 in
Radius of Gyration (lowest value governs)		r _{gyration} :	= <u>0.52132</u> in

AECOM

/ Tower w/ SNB Reinf -	Greenwich Project No		NSS-035	-			
			MCD	Date			
•	Checked by			Date	-		_
mber Description	EXP-4 (L5 = P5 SCH 8	30)					
	80' - 100'						
	6.125 in						
	1.53125 in ²						
	0.29 in ⁴	rx	0.4351	9 in			
	3.6557 in4	ry	1.5451	2 in			
t value governs)		r _{gyrati}	on = 0.4351	9 in =			
nber Description	EXP-5 (L4 = P5 SCH 4	10)					
	100' - 120'						
	6.125 in						
	1.53125 in ²						
	0.29 in ⁴	r _x	0.4351	9 in			
	0.20						
	Bent Plate Properties mber Description t value governs) mber Description	Checked by mber Description EXP-4 (L5 = P5 SCH & 80' - 100' 6.125 in 1.53125 in ² 0.29 in ⁴ 3.6557 in ⁴ 3.6557 in ⁴ t value governs) EXP-5 (L4 = P5 SCH 4 100' - 120' 6.125 in	Bent Plate Properties Computed by Checked by mber Description EXP-4 (L5 = P5 SCH 80) 80' - 100' 6.125 in 1.53125 in ² 0.29 in ⁴ r _x 3.6557 in ⁴ r _y t value governs) r _{gyrati} mber Description EXP-5 (L4 = P5 SCH 40) 100' - 120' 6.125 in 100' - 120' 6.125 in	Bent Plate Properties Computed by MCD Checked by Checked by MCD mber Description EXP-4 (L5 = P5 SCH 80) 80' - 100' 80' - 100' 6.125 in 1.53125 in ² 0.29 in ⁴ r _x 0.4351 3.6557 in ⁴ r _y t value governs) r _{gyration} = 0.4351 mber Description EXP-5 (L4 = P5 SCH 40) 100' - 120' 6.125 in	Bent Plate Properties Computed by Checked by MCD Date mber Description EXP-4 (L5 = P5 SCH 80) Date Date 80' - 100' 6.125 in 1.53125 in ² 0.29 in ⁴ r _x 0.43519 in 1.54512 in 1.53125 in ² 0.29 in ⁴ r _y 1.54512 in s.6557 in ⁴ r _y 1.54512 in t value governs) r _{gyration} = <u>0.43519</u> in mber Description EXP-5 (L4 = P5 SCH 40) 100' - 120' 6.125 in 100' - 120'	/ Tower w/ SNB Reinf Greenwich Project No. Bent Plate PropertiesNSS-035 Computed by MCDSheet Datember DescriptionEXP-4 (L5 = P5 SCH 80)80' - 100' 6.125 in 1.53125 in ² 0.29 in ⁴	/ Tower w/ SNB Reinf GreenwichProject No.NSS-035SheetofBent Plate PropertiesComputed by Checked byMCDDate09/14/15mber DescriptionEXP-4 (L5 = P5 SCH 80)80' - 100' 6.125 in6.125 in 1.53125 in ² 0.29 in ⁴ r_x0.43519 in 1.54512 int value governs) $r_{gyration} = 0.43519$ in 100' - 120' 6.125 in100' - 120' 6.125 in

AEC	COM			Page	of
Job	180' ROHN SSV Tower w/ SNB Reinf, - Greenwich	Project No.	NSS-035	Sheet	of
Description	PLS Tower MODification Properties	Computed by	MCD	Date	09/15/15
		Checked by		Date	

Quantity of Clamps required for MODified Tower legs



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o 180	' ROHN SSV Tower w/ SNB Reinf Greenwid	ch Project No.	NSS-035	Sheet	of
scription	PLS Tower MODification Properties	Computed by	MCD	Date	09/15/15
		Checked by		Date	
		MANUFA	3 ORIGINAL TURER DOCUMENTS \$ & PROPERTIES.		

Designation of Member MOD P6-SCH80:

	Elevation	20' - 60'	Section Height	40 ft				
	Bent Plate Pro	operty						
	Material Area I _x (in⁴)	50 ksi 1.8125 in ² 0.4926 in*			ation = 0.521 in			
	l _y (in ⁴)	6.2451 in ⁴	ry 1.	856 in				
Conside	r Leg Effective l	<u>_ength of =</u>	10 ft =	120 in				
	* Compare Property of existing leg against the attached bent plate:							
	Leg Property	(ROHN	6 EH (6 SCH 80))					
	K = L = r _{pipe} =	= 120 in		<u>KL</u> =	54.7945			
	* Use weaker i	radius of gyration fo	or analysis to determine	# clamps per effect	ctive length:			
	Input # Clamps	6	KL/(# clarr	np -1) / r _{gyration}	46.0366	-> OK		
# Clamps Required (with maximum allowable spacing Centerline to Centerline of Clamp (in)):								
	Total Height	40 ft	# clamps # clamps per		= 6 = 24@	19.5 in		
			# clamps	for Section	=72	(max)		

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AEC	COM			Page	of
Job	180' ROHN SSV Tower w/ SNB Reinf Greenw	ich Project No.	NSS-035	Sheet	of
Description	PLS Tower MODification Properties	Computed by	MCD	Date	09/15/15
		Checked by		Date	

Quantity of Clamps required for MODified Tower legs

			anda.	EXISTING TOWER LE (SIZE AND THICKNES REFER TO ORIGINAL MANUFACTURER DO FOR SIZES & PHOPEN	35 DN). DCUMENTS	ATE ON ER LEG		
Designation of Me	mber MOD P6-	STD:						
Elevatio	60' - 80		Section Height	20	ft			
Bent Pla	te Property							
Material Area I _x (in⁴)	50 1.8125 0.4926		rx	0.521 in	r _{gyratior}	n = 0.52	1 in	
l _y (in ⁴)	6.2451	in ⁴	ry	1.856 in			=	
Consider Leg Effe	ctive Length of	Ξ	10 ft	= 120	in			
* Compa	re Property of e	xisting leg ag	ainst the attach	ed bent plate:				
Leg Pro	perty	(ROHN 8 (8	STD))					
	K = 1 L = 120 r _{pipe} = 2.250	in		r KL	=	53.333	3	
* Use we	aker radius of g	yration for a	nalysis to detern	nine # clamps per	effectiv	ve length:		
Input # C	Clamps 6		KL/(#	clamp -1) / r _{gyration}		46.036	6>	> OK
# Clamps Required	(with maximu	m allowable	spacing Cente	rline to Centerlin	e of C	lamp (in)):		
Total He	ight 20	ft	# clar # clamps	nps per 10 per leg			6 2 @	19.5 in
			# clan	nps for Section		=3(6	(max)

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180' ROHN SSV Tower w/ SNB Reinf Greenv	vich Project No.	NSS-035	Sheet	of
scription PLS Tower MODification Properties	Computed by	MCD	Date	09/15/15
	Checked by		Date	
uantity of Clamps required)	ed Towe	r legs	

			er.		
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		E		ļ	
		A		Ľ.	1
				1/1	
		×40	Time	///	
					120° BENT PLATE ON EXISTING TOWER LEG
Designat	ion of Member	MOD P5-SCH80:			
	Elevation	80' - 100'	Section Heigh	nt	20 ft
	Bent Plate Pro	operty			
	Material	50 ksi			
	Area	1.5313 in ²			
	l _x (in⁴)	0.29 in [#]	rx	0.435 in	$r_{gyration} = 0.435$ in
	l _y (in ⁴)	3.6557 in ⁴	ry	1.545 in	
<u>Consider</u>	Leg Effective	Length of =	6.66667 ft	=	80 in

* Compare Property of existing leg against the attached bent plate:

Leg Property (ROHN 5 EH (5 SCH 80))

K =	1	KL	=	43.4783
L =	80 in	r		
r _{pipe} = :	1.840 in			

* Use weaker radius of gyration for analysis to determine # clamps per effective length:

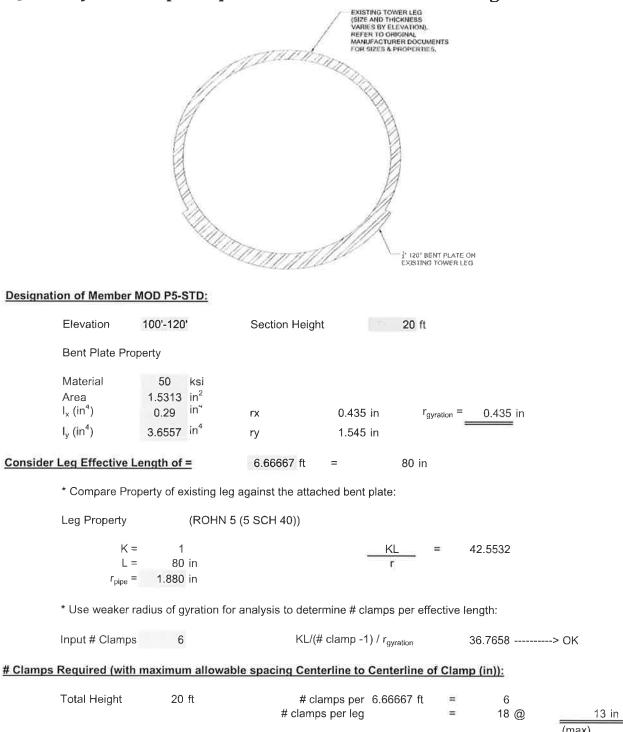
Input # Clamps	6	KL/(# clamp -1) / r _{gyration}	36.7658> OK
input # Clamps	Ų.	Tre (For Charling T) / I gyration	30.7030 0

Clamps Required (with maximum allowable spacing Centerline to Centerline of Clamp (in)):

Total Height	20 ft	# clamps per 6.66667 ft	=	6	
		# clamps per leg	=	18 @	13 in
					(max)
		# clamps for Section	=	54	

AEC	MO			Page	of
Job	180' ROHN SSV Tower w/ SNB Reinf Greenwi	ch Project No.	NSS-035	Sheet	of
Description	PLS Tower MODification Properties	Computed by	MCD	Date	09/15/15
		Checked by		Date	

Quantity of Clamps required for MODified Tower legs



clamps for Section

(max)

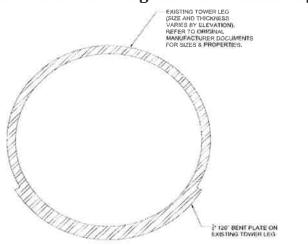
54

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				Page	of
Job	180' ROHN SSV Tower w/ SNB Reinf, - Greenwich	Project No.	NSS-035	Sheet	of
Description	PLS Tower MODification Properties	Computed by	MCD	Date	09/15/15
		Checked by		Date	

Properties for MODified Tower legs in PLS Tower Program



Determination of Member MOD P8-STD:

Existing Leg Property before MODification

Elevation	0' - 20'	
O.D.	8.75	in
thicnkess (t)	0.375	in
I.D.	8	in
Information from:	Above in	formation supplied from ROHN Communcation Tower Designs (10/22/1992)

Pipe Area	9.8666	in²
Pipe Weight	33.574	plf
Pipe Inertia	86.679	in ⁴
Radius of Gyration	2.964	in

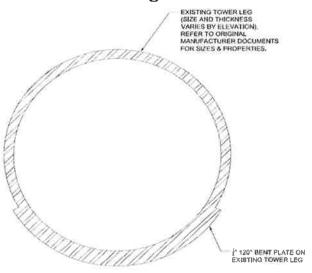
Consider the addition of a 120° bent plate attached to existing leg member

Area of Bent Plate weight of Plate	2.3281 in ² 7.9221 plf		12.1947 in ² 41.4958 plf
l _x (in ⁴)	1.0985 in4		
l _y (in⁴)	14.003 in4	Radius of Gyration of Section	
rx	0.6869 in	I _x (in ⁴)	114.133 in⁴
ry	2.4525 in	l _y (in⁴)	100,682 in⁴
		rx	3.05928 in
		ry	2.87336 in

Long Leg	8.75 in
Short Leg	7.75 in
Weight	41.496 plf
Gross Area	12.195 in ²
Radius of Gyration	2.8734 in

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Job	180' ROHN SSV Tower w/ SNB Reinf Greenwich	Project No.	NSS-035	Sheet	of
Description	PLS Tower MODification Properties	Computed by	MCD	Date	09/15/15
		Checked by		Date	

Properties for MODified Tower legs in PLS Tower Program



Determination of Member MOD P6-SCH 80:

Existing Leg Property before MODification

Elevation	20' - 60'	
O.D.	6.625	in
thicnkess (t)	0.432	in
I.D.	5.761	in
Information from	: Above ir	formation supplied from ROHN Communcation Tower Designs (10/22/1992)

Pipe Area	8.4049 in ²
Pipe Weight	28.6 plf
Pipe Inertia	40.491 in ⁴
Radius of Gyration	2.1949 in

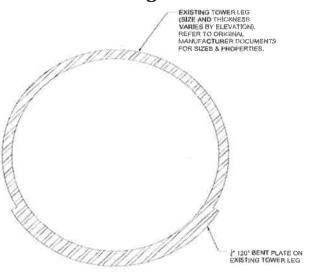
Consider the addition of a 120° bent plate attached to existing leg member

Area of Bent Plate weight of Plate I _x (in ⁴)	1.8125 6.1675 0.4926	plf	Total Area Total Weight	10.2174 in ² 34.7677 plf
l _y (in ⁴)	6.2451 0.5213	in⁴	Radius of Gyration of Section I _x (in ⁴)	114.133 in⁴
ry	1.8562	in	l _y (in⁴) rx	100.682 in ⁴ 3.34221 in
			ry	3.1391 in

Long Leg	6.625 in
Short Leg	5.511 in
Weight	34.768 plf
Gross Area	10.217 in ²
Radius of Gyration	3.1391 in

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Job	180' ROHN SSV Tower w/ SNB Reinf Greenwich	Project No.	NSS-035	Sheet	of
Description	PLS Tower MODification Properties	Computed by	MCD	Date	09/15/15
		Checked by		Date	

Properties for MODified Tower legs in PLS Tower Program



Determination of Member MOD P6-STD:

Existing Leg Property before MODification

Elevation	60' - 80'	
O.D.	6.625	în
thicnkess (t)	0.3325	in
I.D.	5.96	in
Information from:	Above in	formation supplied from ROHN Communcation Tower Designs (10/22/1992)

Pipe Area	6.573 in ²
Pipe Weight	22.367 plf
Pipe Inertia	32.624 in ⁴
Radius of Gyration	2.2278 in

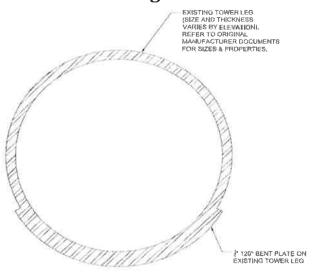
Consider the addition of a 120° bent plate attached to existing leg member

Area of Bent Plate weight of Plate I _x (in ⁴)	1.8125 6.1675 0.4926	plf	Total Area Total Weight	
l _y (in ⁴)	6.2451	in⁴	Radius of Gyration of Section	
rx	0.5213	in	I _x (in ⁴)	114.133 in⁴
ry	1.8562	in	l _y (in⁴)	100.682 in ⁴
			rx	3.68927 in
			ry	3.46506 in

Long Leg	6.625 in
Short Leg	5.71 in
Weight	28.534 plf
Gross Area	8.3855 in ²
Radius of Gyration	3.4651 in

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Job	180' ROHN SSV Tower w/ SNB Reinf Greenwich	Project No.	NSS-035	Sheet	of
Description	PLS Tower MODification Properties	Computed by	MCD	Date	09/15/15
		Checked by		Date	

Properties for MODified Tower legs in PLS Tower Program



Determination of Member MOD P5-SCH 80:

Existing Leg Property before MODification

Elevation	80' - 100	
O.D.	5.5	in
thicnkess (t)	0.375	in
I.D.	4.75	in
Information from:	Above in	formation supplied from ROHN Communcation Tower Designs (10/22/1992)

Pipe Area	6.0377 in ²
Pipe Weight	20.545 plf
Pipe Inertia	19.929 in ⁴
Radius of Gyration	1.8168 in

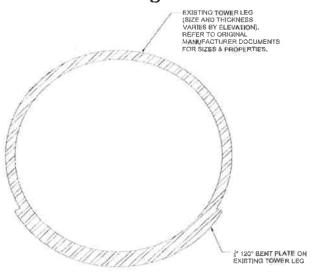
Consider the addition of a 120° bent plate attached to existing leg member

Area of Bent Plate weight of Plate I _x (in ⁴)	1.5313 5.2105 0.29	plf	Total Area Total Weight	7.569 in ² 25.7556 plf
I_y (in ⁴)	3.6557		Radius of Gyration of Section	
rx	0.4352	in	l _x (in ⁴)	114.133 in⁴
ry	1.5451	in	l _y (in ⁴)	100.682 in⁴
			rx	3.88316 in
			ry	3.64718 in

5.5 in
4.5 in
25.756 plf
7.569 in ²
3.6472 in

AEC	.OM			Page	of
Job	180' ROHN SSV Tower w/ SNB Reinf Greenwich	Project No.	NSS-035	Sheet	of
Description	PLS Tower MODification Properties	Computed by	MCD	Date	09/15/15
		Checked by		Date	

Properties for MODified Tower legs in PLS Tower Program



Determination of Member MOD P5-STD:

Existing Leg Property before MODification

Elevation	100' -120),
O.D.	5.5	in
thicnkess (t)	0.258	in
I.D.	4.984	in
2 012 029	202	52

Information from: Above information supplied from ROHN Communcation Tower Designs (10/22/1992)

Pipe Area	4.2488 in ²
Pipe Weight	14.458 plf
Pipe Inertia	14.629 in ⁴
Radius of Gyration	1.8556 in

Consider the addition of a 120° bent plate attached to existing leg member

Area of Bent Plate weight of Plate I _x (in ⁴)	1.5313 in ² 5.2105 plf 0.29 in ⁴	Total Area Total Weight	5.78005 in ² 19.6682 plf
l _y (in ⁴)	3.6557 in4	Radius of Gyration of Section	
rx	0.4352 in	ا _x (in ⁴)	114.133 in ⁴
ry	1.5451 in	l _y (in ⁴)	100.682 in ⁴
		rx	4.44364 in
		ry	4.17359 in

Properties to be used in PLS tower program

Long Leg	5.5 in
Short Leg	4.734 in
Weight	19.668 plf
Gross Area	5.7801 in ²
Radius of Gyration	4.1736 in

PLS-TOWER INPUT / OUTPUT SUMMARY

NSS-036 / Northeast Site Solutions Project Name : N Project Notes: E Project File : F

Butternut Hollow

P:\Projects\Telcom\Structurals_By_Location\Connecticut\GreenwichCSP#74\05-604#####-NSS-035R1\nss-035_t_mobile_tow 3:10:39 PM Thursday, September 17, 2015 Tower Version 10 62

Date run

URS Connecticut Licensed to

Successfully performed nonlinear analysis

Unusual number of fixed joints found: 6. Towers normally have from between 1 and 4 fixed joints. ?? Linear appurtenance "1-5(0" 0 180" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "1-5(0" 0 180" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "1-5(0" 0 175" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "1/2" 8 180" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "115" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "115" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "1115" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "1114" @ 175" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "1-14" @ 155" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "7.14" @ 115" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "7.14" @ 155" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "7.14" @ 155" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "7.14" @ 155" is included in the face zone (face so Linear appurtenance "3/0" [0 160" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1-5/8" @ 160" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1/2" @ 160" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1/2" @ 160" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1/2" @ 160" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1/2" @ 160" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1/2" @ 130" (relocated-NEU)" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "7/8" @ 130" (relocated-NEU)" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "7/8" @ 130" (relocated-NEU)" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1/8" @ 130" (relocated-NEU)" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1/8" @ T-MODILE" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1/8" @ T-MODILE" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1/8" @ T-MODILE" is included in the face zone (face sol Linear appurtenance "7/0" @ 80" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1/2" @ 60" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1/2" @ 20" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1/2" @ 20" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "7/8" @ 110" (relocated-NEU)" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? "Optic Fiber Cable @ VZW" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? "Hybriflex Cables @ Sprint" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? "7/8" @ 113" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear appurtenance "1-5/8" @ T-Mobile " is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? Linear appurtenance "1-5/8" @ VZW" is included in the face zone (face solidity ratio for Rev. F), but does not contribute to wind load; this is nonsensical ?? ç. The model has 28 warnings. Linear appurtenance Linear appurtenance Linear appurtenance

Member check option: TIA/EIA 222-F Connection rupture check: ASCE 10 Crossing diagonal check: ASCE 10

Crossing diagonal check: ASCE 10 [Alternate Unsupported RLOUT = 1] Loads from file: p:\projects\telcom\structurals_by_location\connecticut\greenwichcsp#74\05-604#####-nss-035r1\nss-035_eia

*** Analysis Results:

Maximum element usage is 91 08% for Angle "SNB-LH2P" in load case "W+I 0 deg"

Summary of Joint Support Reactions For All Load Cases;

. @ *	0	0	0	00	0	0	0	0	0	0
Found. Usage %	0	0	0	0	0 0	0 0	0	0 0	0 0	0 0
ing ent -k)	75	10 98	62	16	32	56	86	27	00 00	31 31
Bending Moment (ft-k)	'n	10	2	00	4	17	(**)	~	m	σ
Vert. Moment (ft-k)	01	0 08	00	01	04	04	10 0	07	00	00
Ve Mom (ft	0	0	0	0	Î	0	0	0	0	0
Long. Moment 1 (ft-k)	75	100	21	-4,44	-1.37	41	в В	13	-1.93	69
Mon Mon	S	10	ī	-4	ï	ი 1	ι")	1	ï	-4
an. -k)	13	<u></u> б	32	-7.73	10	83	02	42	48	04
Tran. Moment N (ft-k)	0-	1 59	N	6-				I	3 48	αρ Ι
Shear Force kips)	7 49	15 74	42	92		С M	59	33	51	66
-	5	15	σ	23	13	44	0	ι")	2	25
Vert. Force kips)	23	5	98	74	60	9 0	20	21	6 M	т С
	20	5	250	-210			150	179	150	-240
Tran. Force (kips)	84	27	50	080	86	30	50	16	2 50	47
_	Ч	(r)	œ	21	12 86	<i>с</i> С		ι η γ	2	22
Long. Force (kips)	26	29	06	-11 30	24	-20 54	-2 12	03	11	000
Lo Fo (ki	7 26	L L	4				2	-	-0-1	- 1 -
Joint Label	RohnJP	SNB-JP	hnJl	RohnJ2	SNB-J1	B-J2	RohnJP	SNB-JP	RohnJl	RohnJ2
:										
Case		deg	deg	deg	deg	deg	deg	deg	deg	deg
Load (06-	06-	061	- 90	- 90	- 90	- 60	-60	- 60	- 60
ř	I + N	I + M	I + N	I + M	I + N	I + M	I + M	I + N	I+M	I + N
		-	-	_		-	-			

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	00 0		0.04	1.04	1.1		2.16	1.11	- 04			1.14	1.10			0.04	10.00	1.0		1.1	0.74	110			1.1	
<u></u>	-9.19	00	6	0	0	ςΩ,	3	a	1	5	0	Η	3	1	8 0	<1	2	9 4	m	5	00	00	00	4	4	
œ	-15 92	0	0	S	6 6	Q	3 6	0	4	0	с т	σ	6.8	멍	ſ	5	2 3	00	۲	0	0	σ	σ	Ч	4,	
m	47 89	3 4	0	7 - 7	9	3.6	3.6	6	3 3	n	Д.	00	3	7 .4	5	9 0	Δ_i	Ω. 4	00	7 1	2	۲	7 1	2	5.2	
79.2	-299,54	04.4	69.2	22.0	22.0	55.1	55 . I	50 5	79.3	40.4	50.3	99.4	19.1	20.2	19.7	10.6	0 8	63 (3	02,5	0.2	6	0.1	0 0	6	5	
4	41.48	0 0	0 0	4.4	4	1.1	1.1	1.5	9.1	2.4	2.4	1.4	2.4	1.8	3.7	1 0	n,	с С	2 8	0.0	0	2	6.2	-	3.1	
2 2	-23.94	9.4	0 0	0 3	0 2	00 00	ന	2 1	1_0		0 0	6	2 2	7.2	2	1 3	4 0	ŝ	2		5 2	ŝ	ŝ	9	9	
SNB-	SNB-J	Щ	NS	RohnJ	Rohn	SNB-J	Ŋ	Rohn	SNB-	Rohn	Rohn	SNB-	SNB-	Rohn	SNB-	Rohn	щ	SNB-	SNB-	Rohr	SNB-	RohnJ	Rohn	SNB-J1	SNB-J2	
-60 de	ж. Ц	0 de	0 de	0 de	0 de	0 de	+1 0 de	60 de	60 de	60 de	1 60 de	1 60 de	1 60 de	1 90 de	I 90 de	90 de	0 de	90 de	90 de	Ice Onl	L + Ice O	+ Ice Onl				

Summary of Joint Support Reactions For All Load Cases in Direction of Leg:

	Joint	Joint	Метрег	ber Leg Dir. (kips)	Perpendicular To Leg (kips)	Horizontal To Leg - Res. (kips)	Horizontal To Leg - Long. (kips)	L Horizontal To Leg - Tran. (kips)	Long. Force (kips)	Tran. Force (kips)	Vert. Force (kips)
06 -	ЧŲ		Rohn-LI2P	19.933	8 258	8 264		9 -8 057	7.26	1	20.23
061		SNB-IaS	SNB-LI2P	19,110	16.485	16 497	-3.765	-16	15.29		19 73
- 90	RohnJl Ro	RohnIal Ro	Rohn-LI21	251.034	0 894	0 894		0		8 50	250.86
- 90		_	Rohn-LI22	-211.522	15.589	15 601	-1.0	-	11-		-210-74
06-		SNB-Ial :	SNB-LI21	302,936	2 . 649	2 650		0			302 63
- 90	с.	SNB-Ia2	SNB-LI22	-264,896	33 . 972	33 998		15	1		-263 36
- 60	μ		Rohn-LI2P	150,463	4.109	4 112	-1.497	(1) 		50	150-50
160	04		SNB-LI2P	179,216	6.800	6.804		0	1	3.16	179 31
- 60	-		Rohn-LI21	150,351	4.056	4 059	2.640			2.50	150 39
091	2		Rohn-LI22	-241,369	16 474	16.487	-14 241		-13	47	-240.53
091	-		SNB-LI21	179.128	6,795	66.799	3 636		- 2		179 23
00			SNB-LI22	-301,194	36.077	36.105	-31.268			48	-299 54
0	щ		Rohn-LI2P	304.706	1.416	1.418					304 41
	д		SNB-LI2P	369.802	6.424	6 - 429				-0.00	369 26
0	2		Rohn-LI21	-122 .602	12.906	12 916	10 223				-122.00
0	2		Rohn-LI22	-122.663	12 890	12.900			-10		-122 06
0	_		SNB-LI21	-156.302	27 522	27 543	21 857			14	-155.11
	N		SNB-LI22	-156.348	27.519	27 \$ 540	-21				-155 15
09	μ,	Ľ	Rohn-LI2P	150.466	4.130	4 133					150 50
09	ρ.		SNB-LI2P	179.214	6 . 801	6.806	3.164			-3.16	179 31
W+1 60 deg	_		Rohn-LI21	-241,313	16.475	16.488	14 213	80	-13,41	-22.44	-240.47
0.9	N		Rohn-LI22	150 294	4.037	4 - 040	-2 666		-0 0 0 -	-2.48	150 33
00	-		SNB-LI21	-301.145	36.078	36.106	31,270			-41.48	-299 49
9	SNB-J2		SNB-LI22	179.079	6 . 793	6 797		1 5.744	-2.22		179.18
000	RohnJP	_	Rohn-LI2P	19 933	8 * 258		1.839			-1.84	20 23
00	p.,		SNB-LI2P	19,110	16.486					-3.77	19-73
90	2	_	ohn-LI21	-211 466	15.590	15 602	13.873			-21.08	-210 69
6		~	Rohn-LI22	250 979	0 894		-0.078		4 06		250.80
00	Ę		SNB-LI21		33.974		30 332	15.362	Ĩ	- 30 С –	-263 31
M+1 90	N	SNB-Ia2	SNB-LI22	302.885	2 650	2 651	2 552	0	ŋ	-12.86	302 58
DL + Ice Only	RohnJP Ro	RohnIaS Ro	ohn-LI2P	19.940	7.976	7.983	0 000		1	00 0-	20 24
-	¢		COLL DIAD	000							

DL + Ice Only DL + Ice Only DL + Ice Only DL + Ice Only	RohnJI RohnIal Rohn-LI21 RohnJ2 RohnTa2 Rohn-LI22 SNB-J1 SNB-LI21 SNB-J2 SNB-Ia2 SNB-LI22	Rohn-LI21 Rohn-LI22 SNB-LI21 SNB-LI21 SNB-LI22	19 835 19 780 19 005 18 956	7 975 7 974 15 983 15 981	7 981 7 980 15 995 15 993	6 912 -6 911 13 853 -13 851	3 989 989 996 996
TTA Cochicae To							
LLA SECTIONS INFORMATION	TOLMALION						

20.13 20.08 19.62 19.57

-3 59 -3 59 -7 61 -7 61

Label	do.	BOTTON (Count P	Count	Width	Width	Area			Dead
	(ft)				(ft)	(ft)		Factor	Factor]	10
R	180 000	160 000	36	111	7.50	<u>.</u>	163 75		0	1
m	160.000	140.000		m 01	8.87		191 05		0	
U	140.000	120,000		72	10,23		218 31		0	
Ω	120 000	100,000		72	11.60		245 54		0	
ы	100 000	80,000	27	22	12.96	14_32	272 78	0.9000	0.9000	1.000
Ĺų	80 000	60 -000		19	14 32		300 04		0	
U	60,000	40.000		15	15 68		327	0.9000	0	17
Ξ	40 000	20.000	21	15	17.05			0.9000	0	17
н	20.000	00000	00 1-1	68	18.41		381.	0.9000	0	-

*** Overall summary for all load cases - Usage = Maximum Stress / Allowable Stress Printed capacities do not include EIA allowable stress increase for wind load cases. Printed capacities do not include the strength factor entered for each load cases. The Group Summary reports on the member and load case that resulted in maximum usage which may not necessarily be the same as that which produces maximum force.

Group Summary (Compression Fortion):

io. Of Bolts Comp.	
Curve No. Of No. Bolts Comp.	
L/R Length C Comp. Member (ft)	170,38 9,740 166,50 10,934 150,90 15,589 157,99 15,589 166,41 5,004 45,67 5,004 45,67 5,004 45,67 5,004 45,64 5,004 0,00 0,000 0,00 0,000 0,00 0,000 0,00 0,000 0,00 0,000 131,28 7,504 111,38 7,506 131,28 10,008 131,29 11,000 131,28 10,008 131,206 11,704 111,38 7,504 111,000 131,28 10,008 131,000 131,28 10,008 131,000 131,28 10,008 131,000 131,000 131,28 10,008 131,0000 131,0000 131,0000 131,0000000000
RLZ	
RLY	
RLX	
Comp. Conn. Bearing Capacity (kips)	8.156 8.156 8.156 8.156 8.156 8.000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.00000 0.00000 0.000000
<pre>L/R Comp. Conn. Comp. Conn. ity Shear Bearing Capacity Capacity ps) (kips) (kips)</pre>	2200 224 244 244 244 244 244 244 244 244
L/R C Capacity (kips)	111 112 112 112 113 113 113 113 113 113
comp. control Load Case	567 WHI 0 deg 9423 WHI 0 deg 0000 WH 0 deg 7560 WHI 0 deg 7560 WHI 0 deg 810 WHI 0 deg 810 WHI 0 deg 811 WHI 0 deg 812 WHI 0 deg 813 WHI 0 deg 813 WHI 0 deg 814 WHI 0 0 deg 824 WHI 90 deg 836 WHI 90 deg 836 WHI 90 deg 816 WHI 90 deg 816 WHI 90 deg 816 WHI 90 deg 8176 WHI 90 deg 816 WHI 90 deg 8176 WHI 90 deg 9176 WHI
Comp. Force (kips)	-2557 W+1 -4 423 W+1 -6 1423 W+1 -6 148 75 W+1 -7 808 W+1 -8 756 W+1 -13 819 W+1 -13 819 W+1 -13 819 W+1 -13 819 W+1 -2 150 W+1 -2 150 W+1 -2 150 W+1 -2 150 W+1 -122 850 W+1 -123 850 W+1 -123 850 W+1 -124 850 W+1 -127 W
Comp. Control Member	Rohn-DD461 Rohn-DD61 Rohn-DD31 Rohn-DD31 Rohn-DD31 Rohn-LD31 Rohn-LL4P Rohn-LL34P Rohn-LL34P Rohn-LL34P SNB-DD62 SNB-DD62 SNB-DD62 SNB-DD62 SNB-DD62 SNB-DD62 SNB-LL3P SNB-LL3P SNB-LL3P SNB-LL3P SNB-LL3P SNB-LL3P SNB-LL3P SNB-LL3P SNB-LL3P SNB-LL3P SNB-LL2P SNB-LL2P SNB-LL2P SNB-LL2P
Max Jse In Comp. &	660 37 767 57 767 57 767 57 767 75 767 75 767 75 717 72 717 72 718 72 71
Max Usage (72866 72866 72866 72975 7473 7473 7473 7474 747 747 747
ngle Steel Max N Size Strength Usage Use (ksi) %	
Angle Size	T 55X1 75X0 1875 2 5X2 55X0 1875 2 5X2 55X0 1875 3 5X3 55X0 255 3 5X3 55X0 255 8 1532 55X0 255 8 1596 5517 8 1265 5520 255 2 5520 1875 2 5520 255 3 7330 55 4 7420 525 3 7330 55 4 7420 525 5 5550 625 5 5550 625 5 5550 625 5 5550 625 5 5550 625 5 5550 625 5 7500 1875 8 126 551 126 5
Angle Type	S S S S S S S S S S S S S S S S S S S
Group Angle Desc. Type	Rohn Diagonal 1 Rohn Diagonal 3 Rohn Diagonal 3 Rohn Diagonal 5 Rohn Diagonal 6 Rohn Lagonal 6 Rohn Leg 2 Rohn Leg 3 Rohn Leg 4 Rohn Leg 3 Rohn Leg 3 Rohn Leg 3 Rohn Leg 4 Rohn Leg 3 SNB Diagonal 6 SNB Diagonal 6 SNB Leg 3 SNB Leg 3 S
Group Label	<u>к</u> к к к к к к к с с

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0 1 000	-	0	-	-	0	a	0 1 000	ri	r	e	-	-	000 0 0	0	0
000 1 000	-	312 0 500	-	÷	0	ó	000 1 000	÷	-	H	000 1 000	Q	0	0	0
0 0	0 0	16 3		0 0	10 8	13_5	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0
0.000	0 0 0 0	16 200	0 0 0 0	0 0 0 0	16.200	16 200	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0	000 0	0 0 0 0	0 0 0 0	0 0 0 0	0.00
79 736	109 782	6 759	9 011	6 485	6 854	7.864	312 544	238.476	196.191	189.152	117,722	0 0 0 0	0 0 0 0	0 0 0 0	000
0 deg	0 dea	6	60 dea	60 deg	90 deg	90 deg	0 deg	0	0	0 deg	0 deg				
	138 W+I	I + 2	291W+I		-8,265W+I	348W+I	1+W 040	810 W+I	332 W+I	261 W+I	128 W+I	000	000	000	000
		Ω I	0	0-	00	1	-285	-240	-183	-166	-129		0	0	0
SNB-H3dP	SNB-H8dP	Rohn-DB82	NB-WL-D1	SNB-WL-IlP	Rohn-DC51	Rohn-DD1	Rohn-LI1P	Rohn-LG2P	Rohn-LF1P	Rohn-LE3P	Rohn-LD3P				
1.20	2.14	57,03	2.42.5	3 62 5	90,44	89.15	68.40	4	70,09	65,93	82,46	00 " 0	00 00	00.00	0.00
1.41	6.70	57,03	2.42	3,62	36.0 90.44	89,15	68,40	75.74	70 09	65.93	82.46	000	00.00	0.00	00.00
36.0	36.0	36.0	36.0	36.0	36.0	36.0	50.0	50.0	50 0	50.0	50.*0	50 0	50,0	50.0	36.0
P3-425	P4-494	2X2X0 375	2X2X0 25	2 5X2 5X0 3125	2 5X2 5X0 25	2 5X2 5X0 3125	MOD-P8STD	MOD-P6SCH80	MOD-P6STD	MOD-P5SCH80	MOD-P5STD	2 5X2 5X0 375	5X0,3125	MOD-P4SCH80	2X2X0 25
		2		2 5X2	2 5X	2 5X2	Σ	MOD	M	MOD	M	2 5X2	2.5X2.	MOD	
Pipe	Pipe	SAE	SAE	SAE	SAE	SAE	EXP-1	EXP-2	EXE-3	EXP-4	EXP-5	SAE	SAE	EXP-6	SAE
ntal l	ntal 2	onal 8	cing l	cing 2	nal 3a	nal 4a	Leg 8a	Leg 7a	Leg 6a	Rohn Leg 5a EXP-4	Leg 4a	Diagonal 4b	dE leu	Rohn Leg 3a EXP-6	nal 2a
SNB Horizontal 1	SNB Horizontal	Rohn Diagonal	Wind Lacing 1	Wind Lacing 2	Rohn Diagonal 3a	Rohn Diagonal 4a	Rohn	Rohn	Rohn	Rohn	Rohn	Rohn Diago	Rohn Diagonal 3b	Rohn	Rohn Diagonal 2a
	SNB-H2	Rohn-D8	WLAC-1	WLAC-2	Rohn-D3a	Rohn-D4a	Rohn-L8a	Rohn-L7a	Rohn-L6a	Rohn-L5a	Rohn-L4a	Rohn-D4b	Rohn-D3b	Rohn-L3a	Rohn-D2a

Group Summary (Tension Portion):

Rohn-DAZ 2.345H-1 00 00 01.001	Steel Max Strength Usage (ksi)
3.430WH: 6.000 8.156 9.271 0.937 11.000 0.000 7.30WH: 6.000 6.000 8.156 9.271 0.934 11.000 0.000 7.30WH: 6.000 6.000 1.5.234 18.156 11.000 0.000 9.430WH: 6.000 0.000 0.000 0.000 0.000 0.000 0.000 1.355WH: 6.000 0.000 0.000 0.000 0.000 0.000 0.000 1.450WH: 6.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0	0.37 20.64
0.100000000000000000000000000000000000	20 14 - 27 03 77 36 - 77
7.363%H: -90 de 3.416%H: 0.000 de 3.400 de 3.4	00 0
9.4168HT 9.0 deg 37.004 16.200 12.187 15.234 116.100 10.000 10.4348HT -60 deg 124.200 0.000 0.000 0.000 0.000 0.000 31.9808HT -60 deg 124.200 0.000 0.000 0.000 0.000 0.000 31.9808HT 60 deg 124.200 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 <td< td=""><td>67 98 45 45</td></td<>	67 98 45 45
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	73.73 57.95
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	60.63 50.63
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	15.30 9.67
64.980WH1 60 69.000 0.000 <	34 80 22 96
0.000 0.000 <th< td=""><td>61.50 39.24</td></th<>	61.50 39.24
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0.000 0.000 <th< td=""><td>00 0</td></th<>	00 0
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	00 0
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2 1667WH 90 46 20 340 15 70 1 1000 10 1000 10 1000 10 1000 10 1000 10 1000 10 10000 1000 1000 100	9.20 5.04
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	12 21 11 93
6.129#HT 90 C 13.594 16.992 11.412 1 1000 8.937WHT 90 692 72.336 16.200 23.55 27.187 11.700 1 1<000	23.83 20
8 3737#1 -90 de 52.336 16.200 21.750 27.187 11.784 1 1000 12.723 232.625 24.000 32.625 40.781 17.700 1 1000 12.723 116.410 24.000 32.625 40.781 18.841 1 1.000 12.723 23.625 40.781 18.841 1 1.000 0.000 12.723 552 0.000 32.625 40.781 18.841 1 1.000 12.800 23.416 0.000 0.000 0.000 0.000 0.000 0.000 13.81 13.41 24 000 0.000 0.000 0.000 0.000 14.91 24 0.000 0.000 0.000 0.000 0.000 0.000 24.0347 14.400 231.729 0.000 0.000 0.000 0.000 0.000 24.037 14.41 0.000 0.000 0.000 0.000 0.000 0.000 24.047 0.000 0.000 0.000 0.000 0.000	43.15
8.691W+1 90 667 72	41.55
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	36.0 27.51 27.16
$ \begin{array}{c} 24 & 0.17W+1 & 90 & \text{deg} & 116 & 410 & 24,000 & 32,625 & 40,781 & 18,841 & 1 & 1,000 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 5,004 & 0.000 & 0.000 & 5,004 & 0.0000 & 5,004 & 0.0000 & 5,004 & 0.0000 & 5,004 & 0.0000 & 5,004 & 0.0000 & 5,004 & 0.0000 & 5,004 & 0.0000 & 0.000 & 0.0000 & 5,004 & 0.0000 & 0.000$	37 89
10.532WHT -60 deg 90.720 0.000 0.000 0.000 0.000 0.000 59.173WHT 60 deg 134.136 0.000 0.000 0.000 0.000 0.000 165.677WHT 60 deg 134.136 0.000 0.000 0.000 0.000 0.000 165.677WHT 66 deg 134.136 0.000 0.000 0.000 0.000 241.099WHT -60 deg 231.120 0.000 0.000 0.000 0.000 0.000 266.779WHT -66 deg 237.759 973 0.000 0.000 0.000 0.000 276.677WHT -60 deg 134.136 0.000 0.000 0.000 0.000 0.000 11.989WHT -60 deg 134.136 0.000 0.000 0.000 0.000 0.000 0.000 8.229 WHT 0 deg 23.973 16.200 16.312 19.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 </td <td>75 06</td>	75 06
58 592MHT 60 67 0 0000 0.000	12 32
B5 413WHI -60 de 123 552 D 000 0.000 0.000 0.000 0.000 245 473WHI -60 de 276 473 0 0.000 0.000 0.000 0.000 276 473WHI -60 de 276 473 0 0.000 0.000 0.000 0.000 276 473WHI -60 de 276 473 0 0.000 0.000 0.000 0.000 276 473WHI 0 deg 1759 973 0 0.000 0.000 0.000 0.000 1 662WHI 0 deg 17779 973 0 0.000 0.000 0.000 0.000 0.000 1 662WHI 0 deg 17779 973 0 0.000 0.000 0.000 0.000 0.000 1 662WHI 0 deg 1731 919 0 0.000 0.000 0.000 0.000 0.000 1 662WHI 0 deg 2331 123 16.200 16.312 19.655 11.000 0.000 0.000 1 662WHI 0 deg 23131 123 19.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 <td>46.47 3</td>	46.47 3
165 644WHT -60 de 231.120 0.000 0.000 0.000 0.000 0.000 0.000 2.000 0.00	72.46
241.099WHT -60 de 3776.480 0.000 0.000 0.000 0.000 0.000 276.079WHT -60 de 377759 0.000 0.000 0.000 0.000 0.000 16.62WHT 60 de 377759 0.000 0.000 0.000 0.000 0.000 11.98WHT 60 deg 17279.973 0.000 0.000 0.000 0.000 0.000 11.98WHT 60 deg 134.136 0.000 0.000 0.000 0.000 0.000 11.98WHT 60 deg 134.136 0.000 0.000 0.000 0.000 0.000 11.98WHT 60 deg 23.973 16.200 16.000 0.000 0.000 0.000 12.98WHT 0 deg 23.973 16.200 16.200 0.000 0.000 0.000 0.000 13.65 MHT 0 deg 23.973 16.200 10.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 <	36 0 77 77 53 75
275 79W+T -60 de 317 759 570 0.000	91.08
8 229 MH I 0 deg 17279 73 0.000 <td>85.80 59.67</td>	85.80 59.67
1. 662W+1 60 0 000 0 <	0.04 0.04
II. 988W+T -60 de 134.136 0.000	41
4.943 M+I 0 deg 23.973 16.312 19.055 11.238 1 1,000 0. 0.555 WHI 0 deg 21.304 0.000 0.000 0.000 0.006 0.000 </td <td>6,70 6.</td>	6,70 6.
0.355 M+I 0 cold 0.000<	57.03 22.88
0.442 W+r 0 deg 31 536 0.000 0.000 0.000 6.110 0.000 7.842W+r -90 de 22 144 16.200 10.875 13.594 13.177 1 1.000 0 7.842W+r -90 de 27 27 16.200 10.875 13.594 13.177 1 1.000 0 233 0.31W+r -60 de 366.683 0.000 0.000 0.000 0.000 0	2 42
7 842W+1 -90 62 2144 16 20 10,875 13,594 13,177 1 1000 0 29 031W+1 -90 de 27,082 16,200 13,594 13,577 1 1000 0 29 031W+1 -90 de 27,082 16,200 13,594 16,992 13,570 1 1,000 0 223 014W+1 -60 de 366.683 0.000 0,000 0,000 0,000 0 0000 0 0000 0	3 62 1 05
9.031WHI -90 de 27.082 10.000 13.594 13.1/7 1 1.000 0 233.014WHI -60 de 366.683 0.000 0.000 0.000 11.000 0 223.014WHI -60 de 366.683 0.000 0.000 0.000 10.008 0 0.000 220.078WHI -60 de 306.144 0.000 0.000 0.000 0.000 10.008 0 0.000 165.994WHI -60 de 221.187 0.000 0.000 0.000 0.000 0.008 0 0.000 165.772WHI -60 de 225.293 0.000 0.000 0.000 0.000 0.000 0.000	90 44 44 00
33.014W+I -90 de 21,002 13,594 15,992 13,570 1 1,000 0 22.017W+I -60 de 366.683 0.000 0,000 0,000 0,000 0,000 0,000 220.078W+I -60 de 306.144 0.000 0,000 0,000 0,000 0,000 165.664W+I -60 de 251.187 0.000 0,000 0,000 0,000 0,000 165.664W+I -60 de 251.187 0.000 0,000 0,000 0,000 0,000 165.772W+I -60 de 256.293 0,000 0,000 0,000 0,000 0,000	
233 014W+I -60 de 366.683 0.000 0.000 0.008 0 220 078W+I -60 de 366.144 0.000 0.000 0.008 0 0 165 964W+I -60 de 251.187 0.000 0.000 0.008 0 0 165 964W+I -60 de 251.187 0.000 0.000 0.008 0 0 135.772W+I -60 de 226.293 0.000 0.000 0.000 0.000 0 0	07 TO 10
220.078W+I -60 de 306,144 0.000 0.000 0.000 10.008 0 0. 165.964W+I -60 de 251,187 0.000 0.000 0.000 0.008 0 0. 135.772W+I -60 de 226,293 0.000 0.000 0.000 6.665 0 0.	68.40 47.66
165.964W+I -60 de 251.187 0.000 0.000 0.000 0.000 10.008 0 0. 155.772W+I -60 de 226.293 0.000 0.000 0.000 6.665 0 0.	50 0 75 74 53 92
135.772W+I -60 de 226.293 0.000 0.000 0.000 6.665 0 0	50 0 70 09 49 56 F
	65 93 45 00

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174.651 0.000 0.000 0.000 0.000
Rchn-LD32 105.699W+I -60 de 0.000 0.000 0.000 0.000
45.39 0.00 0.00 0.00
50.0 82.46 50.0 0.00 50.0 0.00 50.0 0.00 36.0 0.00 36.0 0.00
MOD-P5STD 2,5X2,5X0.375 2,5X2,5X0.3125 MOD-P4SCH80 22X2X0.25
Rohn Leg 4a EXP-5 Rohn Diagonal 4b SAE Rohn Diagonal 3b SAE Rohn Leg 3a EXP-6 Rohn Leg 3a EXP-6 Rohn Diagonal 2a SAE
Rohn-L4a Rohn-D4b Rohn-D3b Rohn-D3a Rohn-D3a Rohn-D2a

*** Maximum Stress Summary for Each Load Case

Summary of Maximum Usages by Load Case:

Element Type	Angle Angle Angle Angle Angle Angle
Element Label	Rohn-DC62 Rohn-DC52 SNB-LH2P Rohn-DB61 Rohn-DB61 Rohn-DC51 SNB-DI32
Maximun Usage %	90.30 91.08 91.08 90.62 30.56
Load Case	W+I -90 deg W+I -60 deg W+I 0 deg W+I 60 deg W+I 90 deg W+I 90 deg

s): n DLF:
structure (1b) Angles*Section Equipment:
0 0 U
Weight Weight Weight Total:
* *

65652.0 13037.5 78689.5

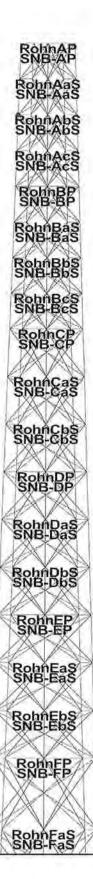
*** End of Report

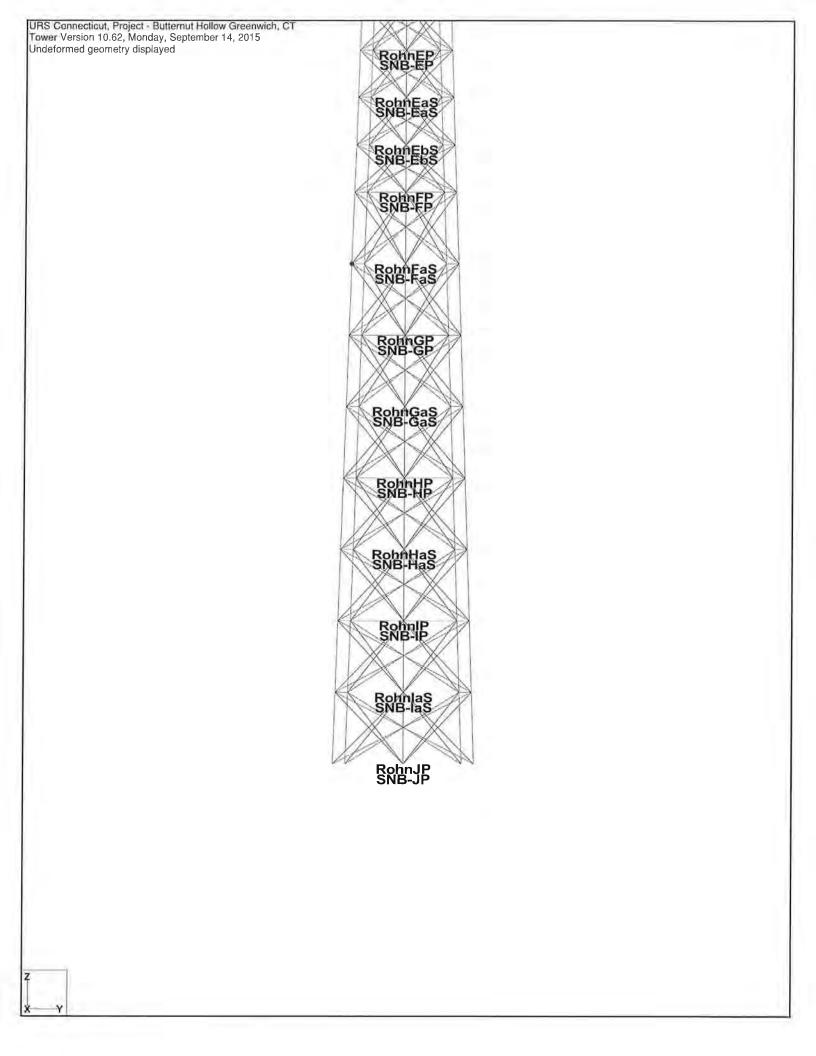
nss-035 t mobile
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PLS-TOWER NODE LOCATIONS

)) _____ URS Connecticut, Project - Butternut Hollow Greenwich, CT Tower Version 10.62, Monday, September 14, 2015 Undeformed geometry displayed

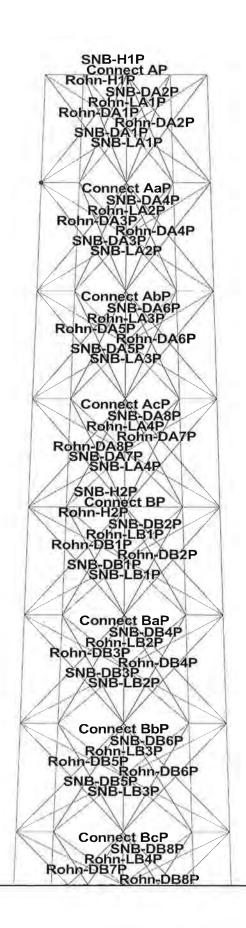
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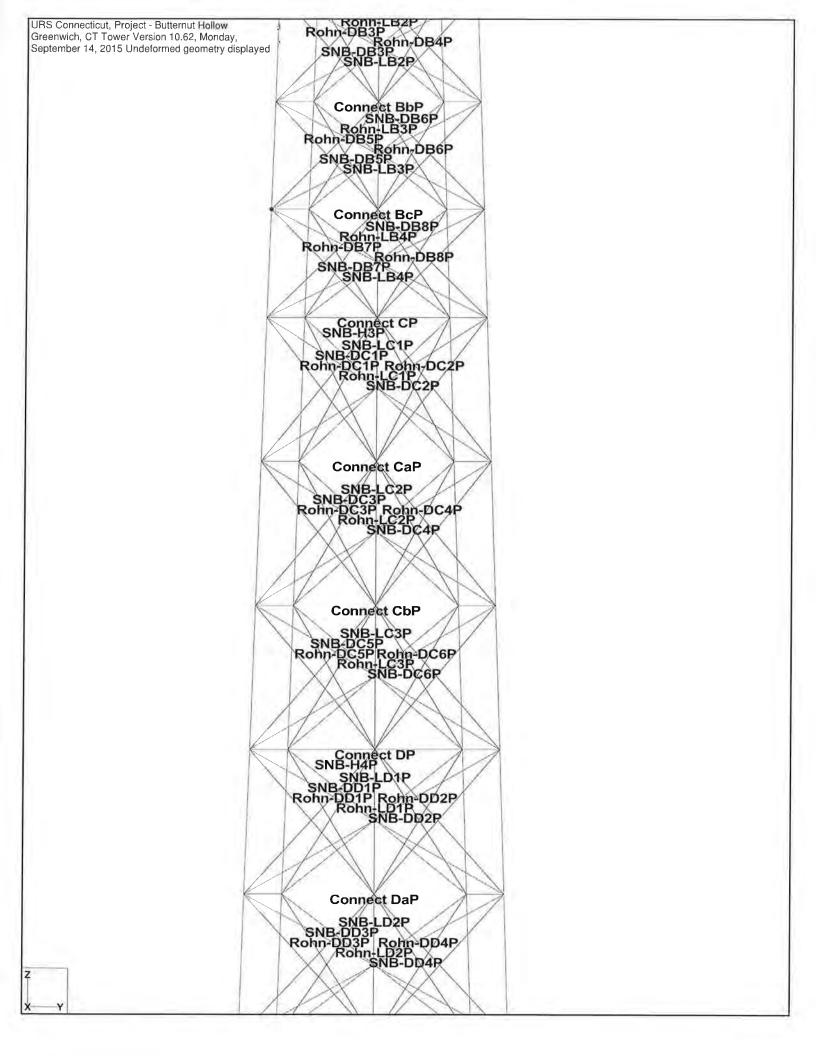


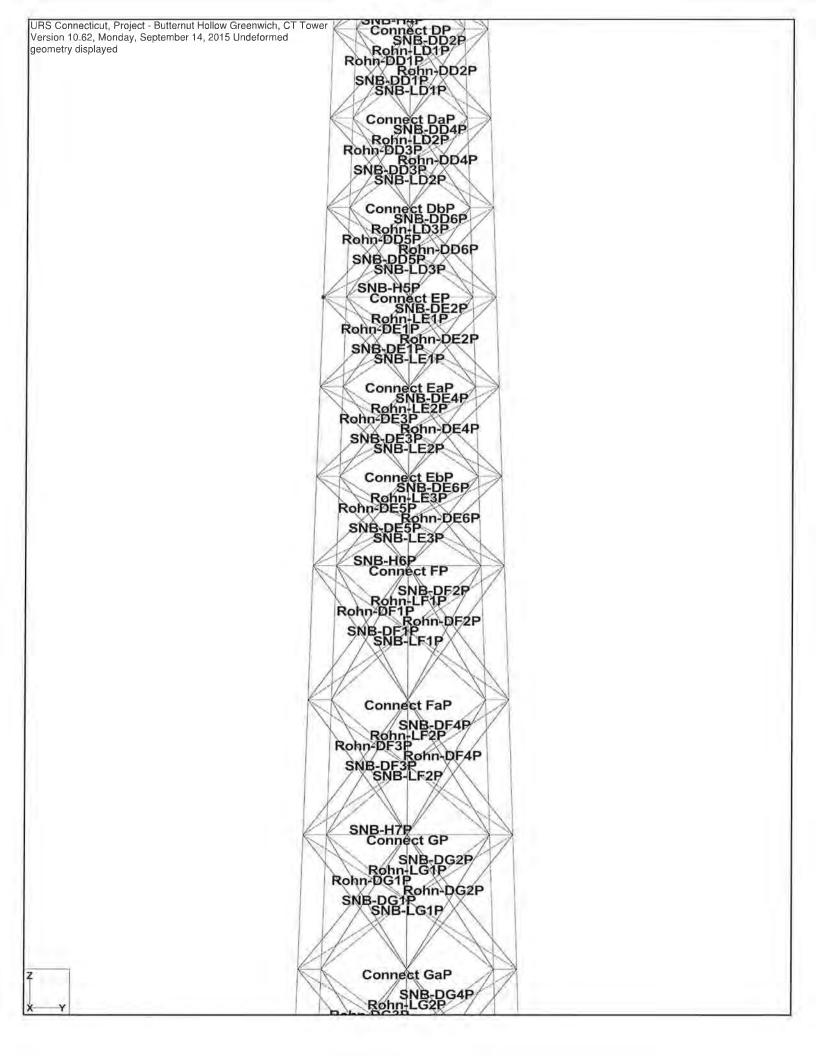


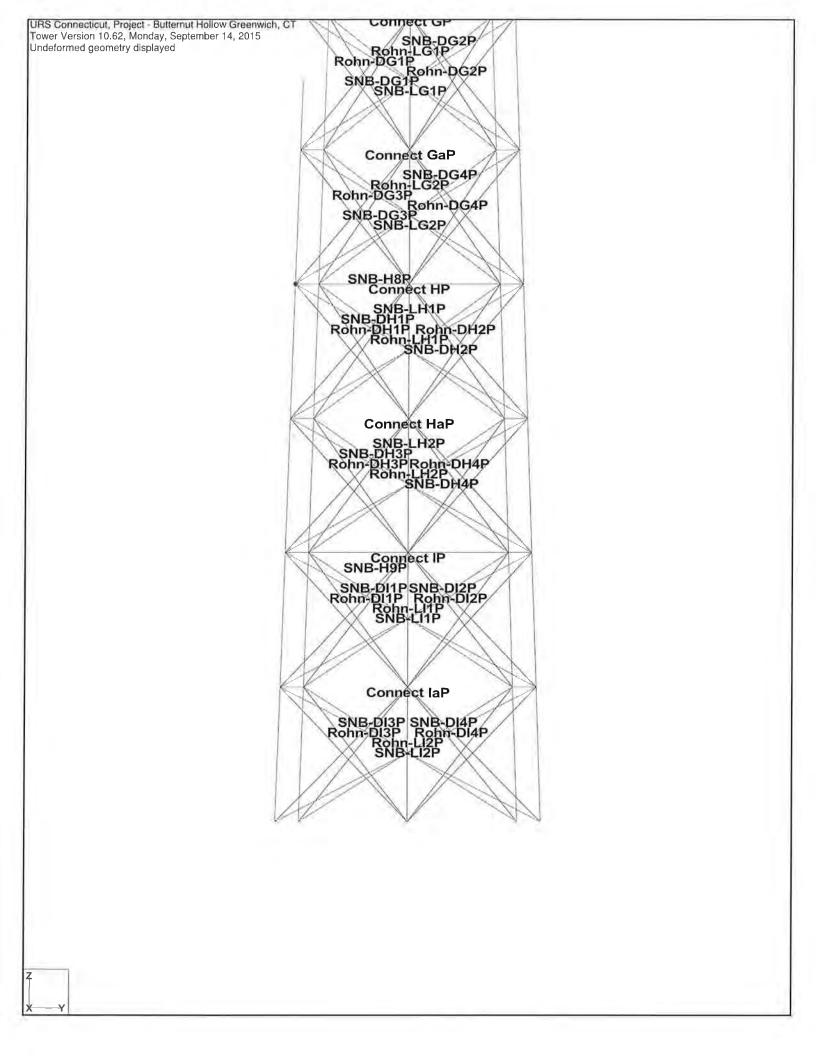
PLS-TOWER MEMBER LOCATIONS











PLS-TOWER DETAILED OUTPUT

* TOWER - Analysis and Design - Copyright Power Line Systems, Inc. 1986-2006 *

Project Name | NSS-036 / Northeast Site Solutions Project Notes Butternut Hollow

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URS Connecticut Licensed to

Successfully performed nonlinear analysis

Unsul number of fixed joints found: 6. Towers normally have from between 1 and 4 fixed joints. ?? Linear spurtemance '15'6' (0) 'is included in the face zone [face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear spurtemance '17'6' (10) 'is included in the face zone [face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear spurtemance '17'6' (10) 'is included in the face zone [face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear spurtemance '17'6' (10) 'is included in the face zone [face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear spurtemance '17'6' (10) 'is included in the face zone [face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear spurtemance '17'6' (10) 'is included in the face zone [face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear spurtemance '17'6' (10) 'is included in the face zone [face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear spurtemance '17'6' (10) 'is included in the face zone [face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear spurtemance '17'6' (10) 'is included in the face zone [face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear spurtemance '17'6' (10) 'is included in the face zone [face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear spurtemance '17'6' (10) 'is included in the face zone [face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear spurtemance '17'6' (10) 'is included in the face zone [face solidity ratio for Rev. F), but does not contribute to wind load: this is nonsensical ?? Linear spurtemance '17'6' (10) 'is included in the face zone [face solid

Nonlinear convergence parameters: Use Standard Parameters Member check option: TLA/EIA 222-F Connection rupture Check: ASCE 10 Crossing diagonal check: ASCE 10 [Alternate Unsupported RLOUT = 1]

Joints Geometry:

Joint Label	Symmetry X Coord. Y Coord. Code (ft) (ft)	X Coord. (ft)	Y Coord. (ft)	Z Coord. X Disp Y (ft) Rest	X Disp. Rest.	Y Disp. Z D. Rest. Re	Z Disp. Rest.	X Rot. Rest.	Y Rot. Rest.	Z Rot. Rest.
RohnAP 7	Tri-Symmetry	4 333	0	180	Free	Р К С	Free	Free	Free	
RohnBP	Tri-Symmetry	5 121	0	160		ы К С С С	Free Free	00 14 14	0 0 L L L	
RohnCP	Tri-Symmetry	5 909	a	140		F ree	Free	Р Г	с Ц	
RohnDP	Tri-Symmetry	6 695	a	120		Free	F C C C	5 L C C	н Ч	
RohnEP	Tri-Symmetry	7 481	D	100		е ЧСС	Free	н Ц	Free	
RohnFP	Tri-Symmetry	8 268	a	80	Free	Free	Free	5 ree	Free	
RohnGP	Tri-Symmetry	9 055	0	60	Free	Free	Free	Free	Free	
RohnHP	Tri-Symmetry	9 841	Ð	40	Free	е с Ч	Free	Frae	Free	
RohnIP	Tri-Symmetry	10-63	D	20		r ce	Free	Free	Free	

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ы Ш	Tri-Symmetry	5 481	0	0	F.C.C.	Free	Free	Free	Free	Free
SNB-FP	Tri-Symmetry	6 2 6 8	0	0	Ф Н	Free	Free	Free	л С	Free
а с С - 5	Trl-Symmetry	7 055	0 0	90	Free	Free T	Free	Free	н Ч	Free
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RohnB2	Tri-Gen 2	-2 561	4.435	160	U U H	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Free	Free	1 2	1 L L L L L L L L L L L L L L L L L L L
RohnCl	Tri-Gen 1	-2 954	-5,117	140	Free	Free	Free	Free	Free	H C C C C C C C C C C C C C C C C C C C
RohnC2	Tri-Gen 2	-2 954	5 .117	140	Э	Free	Free	Free	Free	Free
RohnDl	Tri-Gen 1	-3 348	-5-798	120	Ū H	F L C C	Free	Free	Free	Free
Konnuz	Trj-Gen Z	1. 1.400 1.400	0 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	120	Free	Free	Free	Free	Free	Free
Kohnel	Tri-Gen I	-3-741	-6-479	100	Ч.	9 9 1	Free	Free	Free	Free
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RohuIl	H	-5 315	-9 206	20	Free	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Free Free	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
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RohnJl	F	-5 *71	-9~89	0	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed
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SNB-F1	Tri-Gen 1	-3-134	-5 -428	80	Free	Free	Free	Free	Free	Free
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Ϋ́	Tri-Gen 1	-3 \$528	-6.11	60	Free	Free	Free	Free	Free	Free
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2010ND	-		FCT* D	-	FIXED	L X C J	Dexra	FIXED	Paxt.4	Fixed
Secondary	rv Joints:									

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Label	code	Origin	End I Joint	Fraction	End Fraction Elevation X Disp. Y Disp. foint Rest. Rest. (ft)	K Disp. Rest.	Y Disp. Rest.	Z Disp. Rest.	X Rot. Rest.	Y Rot. Rest.	Z Rot. Rest.
E O								(
T 00011100	דדד_קזווווהרדז	ACTILICS	NOTION	C 7 1 0	2	1001	1100	F.TGG	LTEE	F.YEE	F.ree
(ohnAbS T	Tri-Symmetry	RohnAP	RohnBP	0 2	0	Free	Free	Free	Free	Free	Free Free
	Tri-Symmetry	RohnAP	RohnBP	0.75	0	Free	Free	Free	Free	е Ч	Free
RohnBaS T	Tri-Symmetry		RohnCP	0.25	0	Free	Free	Free	Free	F ree	Free
RohnBbS T	Tri-Symmetry	RohnBP	RohnCP	0 2	0	Free	Free	Free	Free	Free	Free
RohnBcS T	Tri-Symmetry	RohnBP	RohnCP	0 75	0	Free	Free	Free	е Ч Гч	Free	Free
RohnCaS T	Tri-Symmetry	RohnCP	RohnDP	0 333	0	Free	F F F F F F F F F F F F F F F F F F F	Free	E Kee	Free	Free
RohnCbS T	Tri-Symmetry	RohnCP	RohnDF	0.667	0	Free	Free	Free	E N P P	Free	Free

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RohnDas RohnDbs RohnDbs RohnDas RohnCas RohnGas RohnGas RohnGas SNB-Aas SNB-Aas SNB-Aas SNB-Bas SNB-Bas	SNNB SNNB SNNB SNNB SNNB SNNB SNNB SNNB	NNB-WLB-112 SNB-WL-A12 SNB-WL-A25 SNB-WL-A32 SNB-WL-B15 SNB-WL-B15 SNB-WL-B15 SNB-WL-C15 SNB-WL-C15 SNB-WL-C15 SNB-WL-C15 SNB-WL-C15 SNB-WL-C23 SNB-WL-C23 SNB-WL-D25 SNB-WL-C25 SNB-WL-C25 SNB-WL-C25 SNB-WL-C25 SNB-WL-C25 SNB-WL-C25 SNB-WL-C25 SNB-WL-D25 SNB-WL-C25	WL-F1 -WL-F1 -WL-F1 -WL-F1 -WL-F2 -WL	NE - ML-13 R Churt 13 R Churd R R Churd R R Churd R R Churd R R Churd R R Churd R R Churd B R Ch

Bolt Bolt

Bolt Properties:

Hole Ultimate Default Default Shear Shear

Tri-Gen 1 H	CHADP ROL	1							
ri-Con 3	2	ODDEY	0.333	0	Free	Free	ee L	Free	Free
100 11		RohnEP	0.333	D	Free	Free	Free	Free	Р Т Ө Ө
ri-Gen 1	(Ľ.	COLLEP	0.667	0	Free	Free	14	Free	Free
ri-Gen Z	μ.	ohnEP	0.667	0	Free	Free	Free	Free	Free
-Gen	RohnEP Ro!	ChnEP	0.333	0	н Ч	Free	Free	Free	Free
2	щ		0.333	Ð	Free	F F F	Free	Free	Free
		RohnfP	0.667	0	Free	Free	Free	F Y C C	FI CE
64		RohnfP	0,667	0	F F C C	Free	54	Free	Free
a)		RohnGP	0 2	0	Free	Free	Free	Free	Free
01		RohnGP	0	D	ыхее	Free	Free	00 Li	Free
en A		RonnHP	0 2	0	ы Члее	Free	54	Free	Free
N)		RohnHP	5 O	0	ы Ч	Ш Ц	54	FICE	Бтее
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4.8	K I	aruuo	n i	0	F.Y.C.C.	Free	Free	Free	Free
14		RohuJP	5 C	D	л Ч	Ш Ц	Free	Free	Free
÷f		SNB-BP	0,25	0	Free	Free	Free	Free	Free
64		SNB-BP	0.25	0	Free	Free	Free	Free	Free
÷1		3-BP	0 5	0	Free	Free	Free	Free	Free
¢1		9 - B P	0*0	0	ы Ч	Free	Free	Free	Free
H	SNB-AP SNE	SNB-BP	0,75	D	Free	Free	Free	Free	Free
ry.		3-BP	0 75	a	Free	Free	Free	Free	Free
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2		SNB-CP	0 25	0	r) r r	Free	Free	Free	Free
-1	SNB-BP SNB	SNB-CP	0.5	a	Free	Free	Free	Free	Free
64		SNB-CP	0 .5	0	라 C C	Free	Free	Free	Free
H)		SNB-CP	0 ,75	0	Free	Free	Free	Free	Free
Tri-Gen 2 S		9-CP	0.75	0	Free F	Free	Free	Free	Free
4		SNB-DF	0.333	0	Free	Free	Free	Free	н С
Ν.		- DF	0.333	0	F K C C	Free	Free	Free	ы ЧССС
-11	SNB-CP SNE	SNB-DP	0.667	0	Free	Free	Free	Free	r r e e
N.		3-DP	0.667	0	F У С С С	Free	Free	Free	Free
		9 - E F	0.333	0	Free	Free	Free	Free	E LOG
N .	SNB-DP SNE	日 日 日 日	0.333	0	Free	Free	Free	Free	Free
÷0		3-55	0.667	0	Free	Free	Free	Free	Free
N -		SNB-EP	0.667	0	Free	В	Free	FX CC	Free
43		ц ц ц	0.333	0	Free	Free	Free	Free	Free
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ri-Gen		н-СЪ	0.5	0	Free	Free	ы Ц	Free	Free
Gen 2		9-0Ъ	0 I	0	Ľ	Free	Free	Free	Free
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-Gen 2		Ξ.	0 5	0	Free	Free	eer Free	Free	Free
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ri-Gen 2	SNB-IP SNE	B-JP	0.5	C	0	002		002	002

 $\begin{array}{c} \mathbf{c} \ \mathbf$

The model contains 60 primary and 123 secondary joints for a total of 183 joints $_{*}$

Steel Material Properties:

Member Bearing Hyp. 2 (ksi)	00
Member earing Hyp. 1 (ksi)	00
Member tupture Hyp. 2 (ksi)	00
r Member s Rupture F 2 Hyp. 1 (ksi)	00
Member Stres: Hyp. 2 (ksi)	00
A11.	
Member Stress Hyp. 1 (ksi)	00
.IIA	
Yield Ultimate Stress Stress All, Fy Fu (ksi) (ksi)	5 8 6 3
Yield Stress (ksi)	90 0.0
Steel Modulus erial of Label Elasticity (ksi)	2 9e+004 2 9e+004
Steel Material Label	A 36 A572-50

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 Label Diameter
 Diameter
 Shear
 End
 Bolt
 Capacity
 <thCapacity</th>
 Capacity
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Number Bolts Used By Type

Bolt Number Type Bolts

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	÷.	L.	m.	
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Angle Properties:

Type	Size		Leg		Weight	Area	Ra	Gyration	tio Gyration Gyration Rx Ry Ry	Gyration of Rz Angles	Angles	Width	Short Edge Dist.	Long Edge Dist.	Optimize Cost Factor	Section Modulus
		(in)	(ui)	(ini) (.	(in) (lbs/ft)	(in^2)	10.000	(ni)	(ii)	(ni)		(in)		(ii)		(in^3)
	5X5X0 625	'n	ц	0.625	20	5 86	6.2	1 52		0.978	1	ப	2 5	q	1 0000	
	4 X 4 X 0 = 62 5	4	4	0 625	15.7	4 61	4 8	1 2		0 779	-	4	0	0	0000	
	4 X 4 X 0 . 5	4	4	0 2	12.8	3 75	6.25	1 22		0.782	(H	4		0		
	4X4X0 25	4	4	0.25	6.6	1 94	13.5	1.25		0 795		4	2	0		
	3 5X3 5X0 25	с С	ς . Ω	0.25	8 2	1 69	11.5	1 09		0 694	÷	9 ° 2	1 - 75	0	1 0000	
	3X3X0 .5	m	m	0 . 5	9.4	2 75	4.38	0 898		0.584	(er		1.5	0	0000	
	3X3X0 "25	m	m	0.25	4 9	1 44	9.75	0 93	0 93	0 592	-	m	1.5	0	1 - 0 0 0 0	
	2 5X2 5X0 375	2 5	2.5	0 375	5	1 73	4 83	0 753		0 487			1 25		0000	1.50
	2 5X2 5X0 3125	2 5	2 5	0 3125	IJ		6			0.489	H	10	1 25	C	0000	
SAE	2 5X2 5X0 25	2 - 5	2 5	0.25	4 J		7.75			0 491	-		1.25	0	0000	2.54
	2 5X2 5X0 1875	2 • 5		0 1875	3 07		10.67			0 495			1.25	G	1 0000	
	2X2X0_375	2	0	0 375	4 7		m			0 389				0	1 0000	1.6
	2X2X0 3125	0		0.3125	3.92	1 15	3 8	0 601		0 39	Ŧ		1	0	0000 - 1	
	2X2X0 25	2	2	0.25	3.19	0 94	S	0 609		0 391	**	2	(H	0	1 0000	
	2X2X0.1875	2		0 1875	2.44	17-0	00	0 617		0 394	-	2	e	0	10000	
SAE 17	75X1 75X0 1875	1.75		0 1875	2-12	0 62	9	0.537		0 343	è	1.75	0 875	0	1 0000	
	0 1X0 1X1	T** 0	0,1	-1	0	800	2.17	9.99		99.99	H	0.1	0.05	0	1 0000	
	Pipe3EH	e س	2 9	0	10 3	2 8 3		1.14		1 = 14	-	0	0	0	0 0000	0
	Pipe4EH	4 5	3 826	0	15	4.14	-1	1 48		1.48	**	0	0	0	0~0000	
	Pipe5STD 1		5.047	0	14 6	4.03	-1	1.88		1.88	-	0	0	0	0 0000	
Pipe		563	4.813	0	20 8	5 * 72		1.85		1.85		0	G	0	0 0000	
Pipe		623	5,761	0	28_6	7 88	Ч	2 2		2 2	e	0	0	0	0 0 0 0 0	
Pipe		625	œ	0	35=78	9 8 6		2.96		2.96	-1	0	0	0	0 0000	.0
Pipe		4	3 364	0	12 5	3.44	-	1.31		1.31	H	0	0	0	0 0000	0
Pipe	÷		5.96	0	24 38	6.71	Г	2.22		2.22	H	0	0	0	0 0000	0
Pipe	P3-437		2.626	0	15	4 2	г	1.09		1.09		0	0	0	0 0 0 0 0	.0
Pipe		4 5	3.512	0	21-4	6.21		1.42		1.42	-	0	0	0	0.0000	
Pipe	w.,	6.625	ເດ ເກ	0	36 6	10.7	г	2.15		2.15		0	0	0	0 0000	.0
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Aggregate Angle Information:

Note: Estimate of surface area reported for painting purposes, not wind loading,

Angle Type	Angle Size	Materia Typ	l Total e Length (ft)	Total Surface Area (ft^2)	Total Weight (lbs)
SAE	75X1 75X0 187	en A	4.66	9	92
×,	2X2X0 187		191,3	27.5	99
SAE	2X0 37	E A	67.4	44.9	16
AL.	Z 5X2 5X0 1875		74.4	2.0	228-45
R.	5X2 5X0 2		155;8	29.8	38
SAE	X2 5X	A 3	722.		10
SAE	3X3X0 2	A572-5	273.2	73.2	339.

Sections: The adjustment factors below only apply to dead load and wind areas that are calculated for members in the model They do not apply to equipment or to manually input dead load and drag areas.

l Force a Solid f Face	ercN	NONe			None	None	None	O L C N	None
SAPS Round F Drag x Area S Factor	0.000	000	0000	0000	000	0 0 0 0	0 0 0 0	000	000
L SAFS Angle S a Drag x Area Dr c Factor L	0.000	0.000	000	0.000	000	0.000	0.000	0.000	0000
Transverse Longitudinal SA Drag x Area Drag x Area Dra Factor Factor For All For All	0.000	0.000	0.000	0.000	0.000	0.000	0.000	000	0.000
Transverse I rag x Årea Factor For All	0.000	0000	000	0.000	0.000	0.000	000 0	0.000	0.000
Ar Round Factor D For Face EIA Only	0 900	0 900	0.900	006 0	0 900	0.900	006 0	006 0	0 900
Af Flat Ar Round Factor Factor For Face For Face EIA Only EIA Only	006-0	0.900	0.900	0.900	0.900	0.900	0.900	0.900	0.900
Transverse Longitudinal Af Flat Ar Round Area Factor Area Factor Factor [(CD From (CD From For Face For Face (CD et om (CD et only EIA Only Code) EIA Only EIA Only	00010	0.000	0.000	0.000	0.000	0.000	0,000	0.000	0.000
Transverse] Area Factor (CD From Code)	0.000	0 0 0 0	0 0 0 0	0.000	0.000	0 0 0 0	0 0 0 0	0 0 0 0 0	0 000
Dead Transverse Longitudinal Load Dray X Area Dray X Area A djust. Factor Factor Factor For Face For Face	0.000	0.000	000 0	0.000	0.000	0.000	0.000	000.0	0.000
Transverse I Drag x Area Factor For Face	0 0 0 0	0 0 0 0	000 0	00000	0 0 0 0	0 0 0 0	0 0 0 0	00000	0.000 0
Joint Dead fining Load D ection Adjust. Bottom Factor	1.000	000	1.000	1 000	1.000	000	-000	1-000	1 000
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† Bolt Holes	1	-	**	-	-	-	-	-	-	e	17	-		r.
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Origin Joint	RohnAP	RohnAl	RohnA2	RohnAP	RohnAl	RohnA2	RohnAaS	RohnAal	RohnAa2	RohnAaS	RohnAal	RohnAa2	RohnAbS	RohnAbl
Member Group Section Symmetry Origin Label Label Label Code Joint	Tri-Symmetry	Tri-Gen l	Tri-Gen 2	Tri-Symmetry	Tri-Gen l	Tri-Gen 2	Tri-Symmetry	Tri-Gen 1	Tri-Gen 2	Tri-Symmetry	Tri-Gen 1	Tri-Gen 2	Tri-Symmetry	Tri-Gen l
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Member Label	Rohn-DA1P	Rohn-DAll	Rohn-DA12	Rohn-DA2P	Rohn-DA21	Rohn-DA22	Rohn-DA3P	Rohn-DA31	Rohn-DA32	Rohn-DA4P	Rohn-DA41	Rohn-DA42	Rohn-DA5P	Rohn-DA51

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SNB-H867 SNB-H867 SNB-H867 SNB-H967 SNB-H967 SNB-H967 SNB-H967 SNB-H967 SNB-M1-957 SNB-M1-A17 SNB-M1-A17 SNB-M1-A27 SNB-M1-A27 SNB-M1-A27 SNB-M1-A27 SNB-M1-A27 SNB-M1-A27 SNB-M1-A27 SNB-M1-A27 SNB-M1-A27 SNB-M1-A27 SNB-M1-A27 SNB-M1-C27 SNB-M	SNB-WL-H2P WLAC-Z SNB-WL-H2P WLAC-Z SNB-WL-I1P WLAC-2 SNB-WL-I1P WLAC-2 SNB-WL-12P WLAC-2 SNB-WL-13P WLAC-2 MHAC-2

The model contains 612 angle members $_{\odot}$

Section Joint Information:

Joint Elevation (ft)	180,000 175,000 175,000 175,000 175,000 176,000 170,000 176,000 166,000 166,000 166,000 166,000 166,000 176,000 176,000 175,000 175,000 175,000 175,000
Joint Label	RohnAr RohnAr RohnAr RohnAr RohnAr RohnAr RohnAr RohnAb RohnAb RohnAb RohnAb RohnAb RohnAb RohnAr RohnAr SonBr An SonBr An SonBr An SonBr An SonBr An SonBr An SonBr An SonBr An SonBr An SonBr SonBr SonBr SonBr SonBr An SonBr Son
Section Label	A A A A A A A A A A A A A A A A A A A

A SNB-Ab2 170,000 A SNB-Ab2 170,000 A SNB-Ac1 155,000 A SNB-Ac2 165,000 A SNB-Ac2 165,000 A SNB-Ac2 165,000 A SNB-Ac1 150,000 A SNB-ML-Als 180,000 A SNB-ML-Als 180,000 B SNB-ML-Als 180,000 B SNB-ML-Als 160,000 B SNB-Bl2 165,000 B SNB-C1 140,000 B SNB-Bl2 165,000 B SNB-Bl2 165,000 B SNB-Bl2 165,000 B SNB-C1 140,000 B SNB-NC-C1 140,000 B SNB-

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н	н	н	н	н	н	ы	н	н	н	н	

EIA Sections Information:

Label	z (ft)	-	Count	-	Width (ft)	34	-	Adjust Factor	Adjust Adjust Factor Factor	Ē
A	180 000	160.000	36	111	111 7 50	8 87	163.75		0006-0	17
(ff)	160,000		30	69		10 23	191 05	0 9000	0 9000	
U	140.000			72	10 23	11.60	218 31	0 9000	0006 0	
	120.000			72	11 60	12 96	245 54	0.9000	0 9000	
[1]	100 000			72	12 96	14.32	272.78	0	0 9000	-
[Li	80 000	60,000	21	51	14 32	15.68	300 04	0006-0	0006 0	000
U	60 . 000			51	15 68	17,05	327 29	0	0 0 0 0 0 0 0	
Ξ	40.000		21	51	17 05	18.41	354 57	0.9000	0006 0	000
H			18	99	18.41	19.78	381 92	0 9000	0 9000	1 000

Equipment Library:

Property Label	Number		Area	Area	EIA Antenna Type	Coef.		
		(sqt)	(ft^2)	(ft^2)			(ft)	(ft)
RFS PD1142 w/ ice	Omni	21 5	2 80	0.00	Circle	1.00	0.00	0.00
Andrew DB-583 w/ ice	inni	11 6	1L 0	0.00	Circle	00 1	00 0	000
Andrew DB844H80-XY	Panel	40 0	Э 58	0.00	Square	1.00	00.00	00 00
12` T-Arm	Mount	600 0	10.00	0.00	Square	1,00	00 0	00.00
3' Stand-Off	Mount	75 0	1 50	0.00	Square	1,00	00 0	0.00
Dipole	Dipole	71 0	6 00	0.00	Circle	1.00	0.00	0,00
6 Dish with Radome	Dish	220 0	29.00	0.00	Circle	1.00	00.00	0.00
	TMA	15 0	1 00	0	Square	1.00	00 00	0.00
	Grid Dish	120.0	20_00		Circle	1.00	00 0	0.00
GPS	GPS	20 0	1 00	0.00	Circle	1,00	00.00	0,00
Powerwave 7770	Panel	35 0	5 88		Square	1.00	0 0 0	0.00
Powerwave P65-16-XLH-RR	Panel	65.0	8.40		Square	1 00	0 0 0	0.0
Raycap Surge Suppressor	Other	20.0	1 27		Circle	1 00	00 0	0.00
Ericsson RRU	Other	80 0	5 45		Square	1.00	0.00	0.0
10x4 6x4 Panel	Panel	20 0	0 50		Square	1:00	00 0	0.00
DB586-Y	inni	10 0	0 40		Circle	1 00	00 0	0.00
8` Omni	Omni	160_0	$1 \le 60$		Circle	1_00	00 0	0.0
12` Omni	iпто	170 O	2 40		Circle	1 00	00.00	0.00
Boom Gate	Mount	470 0	24 40	0.00	Square	1 00	00 00	0.0
Scala OGT9-806 w/ ice	0mn i	22 = 7	1 98	0,00	circle	1 00	0.0 0	0.00
SC479-HEILDF w/	iппо	69 8	6 54	0.00	Circle	1 00	00.00	0.0
RFS APXVSPP18-C w/ ice	Panel	106.5	8 81	0.00	Square	1 00	0.00	0.00
ALU RRH	RRH	66.7	2 44	0.00	Square	1 00	00 00	00'0
6 Stand-Off	Mount	150 0	3 00	00.00	Square	1.00	00 00	0.0
Amphenol BXA-171063-8BF-EDIN-X	Panel	36 0	3 23	0.00	Square	1 - 00	00 0	0.0
Swedcom SLCP 2x6014	Panel	70 5	7 65	0.00	Square	1 00	00 0	0.0
Diplexer	Panel	10 0	0 = 20	0.00	Square	1.00	00 0	0.00
Raycap DB-T1-62-8AB-0Z Dist: Box	Panel	73_0	5.75	0010	Square	1 00	00 0	0.00
Ericsson AIR B2A/B4P	Panel	124 0	6 40	0,00	Square	1 00	0 00	0.00
Decibel PD-420 2 bay - Dipole	Dipole	24 . 7	2.48	00.00	Square	1.00	00 00	0.00
Ericsson B4A/B12P Panel	Panel	269.0	12 63	00 0	Sollare	00	000	00.00

RKHU-11 RRH Unit RRH 83,0 3,23 0,00

Equipment Connectivity:

			Angle (deg)
4-0	RohnAP	' Stand-O	
H-do.T.	KohnAl Ckadod	Stand-Of	20
55	RohnA2	6 Stand-	29
CSP-3	RohnA2	C479-HFILDF w/ ic	40.0
CSP-75	RohnAP PohnA1	6. Dish with Radome	0 0
SP-77	RohnA2	Dish with Radom	
CSP-1	RohnAl	la OGT9-806 w/	20
174	RohnAl		20.0
TOGEN	RohnAF	Andrew DB-583 w/	0
200	RohnA1	Decibel PD-420 2 bay - Uipole Decibel PD-420 2 bay - Dinole	-
SP-67	RohnAa1	Sinclair SC479-HFlLDF W/	0
SP-31	RohnAaS	6' Dish with Rad	0
91 1 (1)	RohnAaS	DB58	0-0
	RohnAbl	6 Dish with Rad	0
1 1 1 1	KOLACZ Dokaka2	SC4/9-HFTLUE W/	40.0
		COTO OCTATOO W/	
	RohnBP	LTIC CIF	2.0
	RohnBP	Stand-	
	RohnBl	3' Stand-	9
	RohnBP	10x4 6x4 Pa	9
-	RohnBP	Sinclair SC479-HFlLDF W/ ice	000
	RohnB2	inclair SC479-HF1LDF W/	2.0
	RohnB2		40.0
	RohnBP	3` Stand-Off	0.0
	RohnBP	0	9
-1 -	RohnBbS BotoBb3	12 T-Arm	000
	RohnBh2		2.0
	RohnBbS	L AVEWAW	
	RohnBbS	P65-16-XLH	20
	RohnBbS	Powerwave 7	0
	RohnBbS	Гo	0
	KonnBbS	ricsson	9
	XODUCIOX VOUCICOX	AMT GMT	9,9
	RohnBbS	2 Mill	9 C
	RohnBbS	TMA	9
-	RohnBbS	e Suppres	0.0
	RohnBbl	Powerwave 7	20.0
	Konnebl	HUX-9I-C9A	20 02
	KONNBD1 PotoBol	Verwave /	0.02
	Rohnehl Bohnehl	Ericsson KKU	
	RohnBbl	1	200
	RohnBb1	TMA	20-0
	RohnBbl	TMA	20.0
	RohnBbl		20.0
	RohnBb2	Powerwave 7	40.0
	KONDBOX DOTOBOX	L T 6 - X L H	40.04
	RohnBb2	Erinsonn Erinsonn	40.04
	RohnBb2	Ericsson RRU	0.0
	0		

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Square 1.00 0.00 0.00

240 240 240 240 240 240 240 240 240 240		
TMA TMA TMA TMA TMA TMA 3' Stand-Dff 8' Omni 12' T-Arm 12' T-Arm RFS APXVSPP18-C w/ ice ALU RRH ALU RRH	ALC REF ALC REF	КА КА С С С С С С С С С С С С С С С С С
RohnBb2 RohnBb2 RohnBb2 RohnBb2 RohnBb2 RohnD1 RohnD1 RohnD2 RohnD2 RohnD2 RohnD7 RohnD7	Rohmon Rohmon Rohmon Rohmon Rohmon Rohmon Rohmen Ro	A C C C C C C C C C C C C C C C C C C C
ATT-C6 ATT-C7 ATT-C7 ATT-C9 ATT-C9 ATT-C9 ATT-C9 C5P-21 C5P-21 Sprint-C Sprint-C Sprint-C Sprint-C Sprint-C	1111111111111111111111111111111111111	erizon-12.2 AM T-Mobile- T-Mobile- T-Mobile- T-Mobile- T-Mobile- T-Mobile- T-Mobile- T-Mobile-

Ericsson B4A/B12P Panel TMA	RRHU-II RRH Unit RRHU-II RRH Unit RRHU-II RRH Unit
RohnC2 RohnC2	RohnC2 RohnC2 RohnC2
T-Mobile-8 T-Mobile-9	T-Mobile-10 T-Mobile-11 T-Mobile-12

240 00 240 00 0 00 120 00 240 00

Linear Appurtenances:

Description From To Quantity Shape Width or Perimeter Unit In Include in Weight Face Wind Load

	(ft)	(ft)			(ui)	1) (uị)	(in) (lbs/ft)	Zone	
11 Cable Tray 2-150 (wind	64	150	2	Flat	24	0	0	Yes	Yes
ŋ	150	1 8 O		Flat	24	0	0	Yes	5 0 0 0
5/8" (2	180	3	Round	0	0	2 55	Yes	No
- 9 -	2	180	2	Round	0	0	1.52	Yes	No
/2" @ 1	2	180		Round	0	0	0.91	Yes	NO
al @	N	175	Ч	Round	0	0	2	Yes	NO
1-5/8" @ 1	2	175	2	Round	0	0	2 55	Yes	No
6	2	170		Round	0	0	2	Yes	No
-	2	170		Round	0	0	1.52	Yes	NO
1-1/4"	0	165	Ч	Round	0	0	1,91	Yes	NO
` (relocated-N	2	135	2	Round	0	0	1 52	Yes	NO
-	0	160	-1	Round	0	0	2	Yes	No
		160		Round	0	0	1.91	Yes	NO
-	N	160		Round	0	D	0.65	Yes	No
	0	160	C	Round	0	c	2 . 55	Yes	No
1/2" @ 160`	N	160	7	Round	0	0	16 0	Yes	NO
1-5/8"	2	150	12	Round	0	0	2 55	Yes	NO
	0	150	Г	Round	0	0	0 3	Yes	No
	2	150	2	Round	0	0	0 3	Yes	No
(reloc	2	130	-	Round	0	0	1.52	Yes	NO
	~	140	13	Round	0	D	2 55	Yes	No
	2	133,333	12	Round	0	0	2 . 55	Yes	No
Optic Fiber Cable @ VZW	2	133	1	Round	0	D	0 3	Yes	NO
Hybriflex Cables @ Sprint	2	120	e	Round	0	0	0 37	Yes	NO
7/8" @ 113`	2	113	Г	Round	0	0	1 + 52	Yes	No
	2	80	m	Round	0	0	1 .52	Yes	NO
യ -	2	60	2	Round	0	0	0.91	Yes	No
	2	20	Ч	Round	0	0	0.91	Yes	NO
7/8" @ 110` (relocated-NEU)	2	110	1	Round	0	o	1 +52	Yes	No

· · · Loads Data

Loads from file: p:\projects\telcom\structurals_by_location\connecticut\greenwichcsp#74\05-604####+-nss-035rl\nss-035.eia

Structure Height Summary (used for calculating wind/ice adjust with height): Structure height above ground 180,00 (ft) Elevation of structure bottom for wind height adjustment: 0,00 (ft) Structure height for structure gust response factor: 180,00 (ft) Structure gust response factor, Gn: 1,1209 Guy installation temperature: 60,00 (deg F) Tower Type: Triangular Latticed

EIA Rev. F Load Cases:

Joint Displ.	
Point Loads	
Ice Temperature ensity ./ft^3) (deg F)	000000
assic Wind Ice Ice Wind Dir. Thick. Density Speed (mph) (Deg) (in) (lbs/ft^3)	56 0000 56 0000 50 0000 56 0000 50 00000 50 0000 50 0000 50 0000 50 00000 50 00000000
Ice Thick. (in) (0 5000 5000 5000 0 5000 0 5000 5000 0 5000
Basic Wind Wind Dir. 7 Speed (mph) (Deg)	
	0000 0000 0000 0000 0000 0000 0000 0000 0000
Allowable Stress Increase Factor	
Strength 1 Factor	
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Equipment Load Case Information for "W+I -90 deg";

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NotF NotF Total Total

Equipment Load Case Information for "W+I -60 deg":

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Sinclair SC479-HFILDF w/ ice 6' Dish with Radome 6' Dish with Radome

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<pre>6° Dish with Radome Scala OGT9-806 w/ ice Decibel PD-420 2 bay - Dipole Decibel PD-420 2 bay - Dipole Sinclair SC479-HFILDF w/ ice Sinclair SC479-HFILDF w/ ice 004.6K4 Fanel 004.6K4 Panel 004.6K4 Panel 004.7770 006.6K4 Panel 004</pre>	TWA TWA TWA TWA TWA TWA TWA 3' Stand-Off 9 Onni 12' T-Arm 12' T-Ar
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	Face Face Face NotF NotF RR CF AE WF AAF CAF (ft^2) (ft^2)	0.75 1.81 75.6 5076 0.00 2.00 0.79 1.78 99.4 6337 0.00 2.00 0.80 1.78 118.3 7222 0.00 2.00 0.78 1.78 118.3 7222 0.00 2.00
120 00 33 59 0 50 24 300 1113 34 33 05 0 50 15 60 1113 34 33 05 0 50 15 60 1113 34 33 05 0 50 15 60 1113 34 33 05 0 50 1 60 60 113 34 34 62 0 50 1 00 60 <td>ce Face Face Face Face Face F AF AR RR*AR AG e DF DR 2)(ft^2)(ft^2)(ft^2)</td> <td>58 40-54 30.38 163.7 0.59 0.80 1.00 0 52 48 46 38 20 191.0 0.65 0.80 1.00 0 73 49.93 40.11 218.3 0.68 0.80 1.00 0 24 58.49 45.80 245.5 0.65 0.80 1.00 0</td>	ce Face Face Face Face Face F AF AR RR*AR AG e DF DR 2)(ft^2)(ft^2)(ft^2)	58 40-54 30.38 163.7 0.59 0.80 1.00 0 52 48 46 38 20 191.0 0.65 0.80 1.00 0 73 49.93 40.11 218.3 0.68 0.80 1.00 0 24 58.49 45.80 245.5 0.65 0.80 1.00 0
ALU RRH ALU RRH 3: Stand-Off II: ouni Ers polld2 w/ice Dipole RFS polld2 w/ice CFS CFS CFS CFS CFS CFS CFS CFS CFS CFS	Z of Ave. Elev. qzGh Ice Fa Bottom Above Gnd. Thick. (ft) (ft) (psf) (in) (ft ⁻	160-00 170-00 37.11 0.50 56. 140.00 150-00 35.80 0.50 76. 120-00 130-00 34.37 0.50 97. 120-00 110-00 32.77 0.50 100.
Sprint-9 Sprint-9 Sprint-9 NEC-17 Sprint-9 Sprint-9 Sprint-9 Sprint-9 Sprint-9 Sprint-9 Sprint-9 Sprint-9 Sprint-9 Sprint-9 Sprint-19 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 NEU-19-18 Verizon-05 Verizon-12/200 Verizo	Section Z of Label Top (ft)	A 180.00 B 160.00 C 140.00 C 140.00 D 120.00

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Equipment Label	Equipment El Property Set	evation qz Above Ground (ft) (ps	Thic (j	Total Wind Area (ft^2)	Wind Incidence Angle (deg)	222-G CA	222-G CS	222-G CM I	Antenna Axial oad FAM (lbs)	Antenna Side Load FSM (1bs)	Antenna Moment MM (ft-lbs)	Long. Load (lbs)	Trans. Load (lbs)	Vert. Load (lbs)
Top-A	6' Stand-Off	180 00 37		ł.	00 0							113 15		150
	6' Stand-Off	180 00 37	0		120 00							113.15		150
Top-Cl	6 Stand-Off		0	3,00	260,00							113 15	00 0	150 00
	6' Stand-Off	180 00 37	0		220.00							113 15		150_
	Sinclair SC479-HF1LDF w/ ice	180.00 37	0		240.00							246.67		69
	6 Dish with Radome	180,00 37	0	1	00 0							1093.80		220
	6 Dish with Radome	180.00 37	0	0	120.00							1093.80		220
	6 Dish with Radome	180 00 37	0 0		240.00							1093_80		220
	SCALA UGTY-BUB W/ ICE	75 00 001	50		00 02T							74 68		22
	ARE VERREAL WORKSHARE	10 00 001										21 15		۹. ۲.
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	Decibel PD-420 2 bav - Dipole	180.00 37	00		120 00							00.00		47
	Sinclair SC479-HFILDF w/ ice	175 00 37	0		00 00							204 60		7 0 4 W
	6' Dish with Radome	175.00 37	0	(1	00 0							1085 03		0000
	DB586-Y	175.00 37	0		00 0							14 97		10
	6' Dish with Radome	170_00 37	0	10	120,00							1076 08		220
	Sinclair SC479-HF1LDF w/ ice	165,00 36	0		240.00							240,61		69
	Scala OGT9-806 w/ ice	165,00 36	0		120.00							72_85		22
	10×4 6×4 Fanel	165,00 36	0		00 0							18.40		20
	GLF-940	160 00 36	0	14	00 0							729,38		120
	3 Stand-Off	160.00 36	0 0		00 0							54,70		75
	3 SCANO-UIL	95 DU 095	0 0		120.00							54 70		12
	Sinclair SC479-HEILTE W/ ice	160 00 36 160 00 36	00									18 23		50
	Sinclair SC479-HFILDF w/ ice	160 00 36	0		120-00							10 00 0		ם ת ט ס
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	TMA	160,00 36	0		240 00							36 47		200
	3` Stand-Off	160 00 36	0		0 0 0							54 70		12
	TMA	160 00 36	0		00 0							36.47		15
ATT-A	12` T-Arm	150-00 35	0	-	00 " 0							358 03		600
	12 T-Arm	150 00 35	0	-	120-00							358+03		600
ATT-C	LZ T-Arm	150-00 35	0 0		240 00							358,03		600.
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	Powerwave 7770	150 00 35	0		120.00							210 52		9 10 1 10
	Powerwave P65-16-XLH-RR	150 00 35	0		120.00							300 74		- 69
ATT-B3	Powerwave 7770	150 00 35	00		120 00							210,52		35
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124 269	124,00 269,00 83,00 83,00 83,00 83,00	Total Weight (Ibs)	4851 4851 6251 8612 10139 13715 15628 15628 15628 16609		Vert. Load (lbs)	150.00 150.00	150 00	69 80 220 00	220 00 22 70	15 00 11 60	24 70 24 70	220 00	220 00	22 70	120 00 75 00	75 00 20 00	69 80 69 80	69 80 15 00	15 00	000 000
1.40.5.9 - 31.		Total Wind [(1bs)	5835 7312 8416 8530 8738 8738 7872 7318 7613		Trans. Load (lbs)	00	000	20 FT	47.26 64.67	001	81 14 81 14	39 001 19 001	31 91 8 91	63 09	31 66 47 37	47.37	06,55 06,55	06.55 31.58	10 06 10 06	9 n T n T
6441	224,66 443,40 35,10 113,39 113,39 113,39 113,39	NotF WA (1bs)	000000000		Long. T Load (lbs)	0 00 0 00	0 0 0 0 0 0	400	00 70 70 70 70 70 70 70 70 70 70 70 70 7	9 8 9 6	20 00 C	1 01 0 1 01 0	0.4	1000	0 0 0 0 0 0	35	250	290 190	179.01 3	
(14)	V& 444	NotF AAR*CAR (ft^2)			Antenna I Moment MM ft-lbs) (U) U)	1, 1, 1	L U U	10		4 4 C	10	10 F	4	800					
		NotF CAR A	11 20 12 20 12 20 11 20 11 20 20 20 20		_															
		NotF AAR (ft^2)			Antenna Side Load FSM (lbs)															
		NotF CAF			nna ial FAM bs)															
		NotF AAF (ft^2)			U N U N															
		Face WF (lbs)	5835 7312 8416 8530 8530 8738 8738 7318 7318 7318 7613		-G 222 CS															
		Face AE (ft^2)	87 0 1114 7 137 8 137 8 146 0 157 1 166 1 166 7 166 7 174 5		222-															
		ace Face RR CF	75 1.81 79 1.78 80 1.78 77 1.78 74 1.78 73 1.83 73 1.83 71 1.86 71 1.88		222-G CA															
2000		ace F DR			Wind dence Angle (deg)	00 00	60 00 60 00		80 00	00 00		00 00	60,00 80,00	60.00 00.00	000000	60 00 00 00	00.00 60.00		300.000	>
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ហណល កាតាតា		ace Face AR RR*AR ^2)(ft^2)	54 464 936 438 438 438 438 438 438 438 438 438 438		n qzGh e d (psf)	(1)(1)(., ., .	0.010	0,0)	0.010	., (., (.)	() ()	(1) (1)	19 (9)	(1) (1)	0,0)	01.03.0	., (.) (0 35 80 35 80 35 80	1
400440	140 00 140 00 140 00 140 00 140 00	Fa (ft∧	558 40 552 48 773 49 80 79 58 79 71 53 72 70 72 70 72 70 72 70 70 70 70 70 70		Levation Above Ground (ft)	180 0	180 0	180 0	180 0 180 0	180 0 180 0	180 0 175 0	175 0	170 0	165 0 165 0	160 0 160 0	160 0 160 0	160.0	1 0 0 0 1	150 00	
/B4P Lane/ TMA	anel TMA Unit Unit Unit	29": E	50 50 50 50 50 50 50 100 50 1111 50 1111 50 50 50 50 50 50 50 50 50 50 50 50 50		ment El perty Set	-0ff -0ff		n e e e e e e e e e e e e e e e e e e e	dome ice	TMP	р р л л л л л л л л л л л л л л л л л л	dome 86-Y	dome ice	ice anel	-940 -0ff	-Off anel	ее ССС 	TMA TMA	TMA TMA Arm	110
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sson n B4/	B42 B42 RHU- RHU-	on for "V v. qzGh d. (psf) t)	00 37 11 00 37 11 00 34 37 00 32 77 00 28 90 00 28 90 00 28 16 00 23 23			io o i	0 1 1 0 1 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1	Dish Dish	0GT!	Andrew DB- D-120 2 52	20 2 ba	Dish v	Dish v 479-HF1	a OGT9- 10x'	'n	3. 10x4	SC479-HFILDF SC479-HFILDF	4 / 9-HE)	
E L L L L L L L L L L L L L L L L L L L	Ericescon R	Information F Ave. Elev. Above Gnd. (ft)	170,00 150,00 150,00 110,00 70,00 50,00 30,00 10,00	Information for					6' I Scala	And	Decibel PD-420 2 Day - I Decibel PD-420 2 Day - I Sinclair SC479-HflLDF ;	, 9	lair SC	Scala			Lair SC Lair SC	במוד מכ		
		l Case Informa Z of Ave. I Bottom Above (ft)	460 2000 2000 2000 2000 2000 2000 2000 2	a			5 0 0 0 0				Decibe		Sinclair				Sinclair	ATHC.		
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T-Mobil T-Mobil T-Mobil T-Mobil	T-MODILE-8 T-MODILE-8 T-MODILE-10 T-MODILE-11 T-MODILE-11 T-MODILE-12	ion	м 180 160 120 120 120 120 120 100 120 100 120 12	ment Lu	Equipment Label	ĔĔĬ	D O L	CSI	CSI CSI	CSI TT	NEG	CSI	Ϋ́Ĕ	C: STAN	1.0	STAD	CSI	CSI	STAM- ATT ATT	
	ΗΗΗ	EIA Sect Section Label		Equipment																

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12' T-Arm Powerwave 7770 Powerwave 7770 Powerwave 7770 Ericsson RRU Ericsson RRU Ericsson RRU FWA RAYCap Surge Suppressor TWA TWA TWA TWA Powerwave 7770 Ericsson RRU Ericsson RRU FWA Powerwave 7770 Powerwave 7770 Pow	DB5 DB5 B12 D12 D12 D12 D12 D12 D12 D12 D12 D12 D
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0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	in	0.50	0.50	0.50	0.50
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	Amphenol BXA-1/1003-8BF-EDIN-X Amphenol BXA-171063-8BF-EDIN-X	Swedcom SLCP 2x6014			Amphenol	Amphenol BXA-171063-8BF-	Swedcom	Andrew DB844H80-XY			Amphenol BXA-171063-8BF-EDIN-X	ALU RRH	ALU RRH	ALU RRH	Raycap DB-T1-62-8AB-02 Dist Box	Ericsson AIR B2A/B4P	Ericsson B4A/B12P Panel	TMA	Ericsson AIR B2A/B4P	Ericsson B4A/B12P Panel	TMA	Ericsson AIR B2A/B4P	Ericsson B4A/B12P Panel	TMA	RRHU-11 RRH Unit	RRHU-11 RRH Unit	RRHU-11 RRH Unit
Verizon-2 850 Verizon-3 850	Verizon-5_PCS	Verizon-6_700	Verizon-7_850	Verizon-8 850	Verizon-9_LTE	Verizon-10 PCS	Verizon-11_700	Verizon-12_850	Verizon-13_850	Verizon-14 LTE	Verizon-15_PCS	Verizon-16 AWS	Verizon-17_AWS	cizon-18 AWS	n	T-Mobile-1	T-Mobile-2	T-Mobile-3	1	-Mobile-	T-Mobile-6	T-Mobile-7	T-Mobile-8	T-Mobile-9	T-Mobile-10	T-Mobile-11	T-Mobile-12

61.97 107.33 40.00 55.91 96.89 36.00 55.91 97.83 40.00 55.91 97.83 40.00 61.97 107.33 40.00 61.97 107.33 40.00 55.91 96.84 36.00 55.91 96.84 36.00 55.91 96.84 36.00 55.91 96.84 36.00 61.97 107.33 40.00 61.97 107.33 40.00 61.97 107.33 40.00 61.97 107.33 40.00 55.91 96.84 36.00 92.23 73.15 66.70 42.23 73.15 66.70 92.23 73.15 66.70 112.33 194.07 269.00 117.55 30.40 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.07 269.00 117.55 3194.00 269.00 117.55 3194.00 269.00 117.55 3194.00 269.00 117.55 3194.00 269.00 117.55 3194.57 124.00 256.70 98.20 83.00

EIA Section Load Case Information for "W+I 60 deg"

Total Weight (lbs)	4851 4851 8612 130139 13715 15628 16409 17608
Total Wind (lbs)	5076 6337 7222 7259 7259 7259 72532 7080 6300 6305 6305
NotF 7 WA (lbs)	000000000
NotF AAR*CAR (ft^2)	000000000000000000000000000000000000000
NotF CAR	1.20 1.20 1.20 1.20 1.20
NotF AAR (ft^2)	
NotF CAF	00000000000000000000000000000000000000
NotF AAF (ft^2)	000000000000000000000000000000000000000
Face WF (lbs)	5076 6337 6337 7222 7259 7359 73532 7080 6300 6379
Face AE (ft^2) (75.6 99.4 1118.3 126.0 136.2 1411 141.8 1146.2 150.8
E ace CF	11111111111111111111111111111111111111
Face Face Face Face Face Face AG e DF DR RR CF ft^2)	1.00 0.75 1.00 0.75 1.00 0.78 1.00 0.78 1.00 0.77 1.00 0.73 1.00 0.73 1.00 0.73 1.00 0.71
Face DR	
Face DF	
០ ០ ប ដ	0000000000 10000000000 100000000000000
Face AG Ft^2)	163 7 59 6 191 0 65 6 6 245 3 0 65 6 2215 3 0 65 6 3272 8 0 63 6 320 0 58 6 3 3234 3 0 58 6 3254 3 0 58 6 381 9 54 0 54 381 9 54 0 54
Face RR*AR ft^2)(56,58 40,54 30.38 1 76,52 48,46 38,20 1 100,24 58,49 45,81 2 109,01 63,28 48 95 2 1109,01 63,28 48 95 2 1109,22 64,43 47,75 3 111,79 71 66 52,26 3 111,39 72 90 27 55,98 3
Face AR ft^2) (40.54 49.54 49.49 58.49 58.49 53.28 64.43 71.66 64.43 71.66 71.66
Face AF (ft^2) (56,58 76,52 97,73 100,24 109,32 111,79 111,79 1117,39 1118,53
Ice Thick. (in)	000000000
qzGh (psf)	2220021 23222 23222 23222 23222 2332
Elev. e Gnd. (ft)	170,00 37,11 150,00 35,80 110,00 32,77 90,00 32,77 90,00 32,77 70,00 28,80 70,00 28,80 70,00 28,13 30,00 23,23
Section Z of Z of Ave. Elev. qzGh Label Top Bottom Above Gnd. 7 (ft) (ft) (ft) (ft) (psf)	00000000000000000000000000000000000000
Z of Top E (ft)	A 180 00 160 00 E 160 00 160 00 D 120 00 120 00 D 120 00 120 00 E 100 00 80 00 E 100 00 80 00 G 60 00 G 60 00 G 20 00 F 20 00 00 00
Section Label	КШООВРΩЖН

Equipment Load Case Information for "W+I 90 deg":

equipment Label	Equipment Elevation qzGh Froperty Above Set Ground (ft) (psf)	levation Above Ground (ft)	ition qzGh Above cound (ft) (psf)	Ice Thick. (in)	Total Wind I Area (ft^2)	Total Wind Wind Incidence Area Angle (ft^2) (deq)	222-G CA	222-G CS	222-G CM	222-G Antenna Antenna CM Axial Side Load FAM Load FSM (1bs) (1bs)	Antenna Side Load FSM (1bs)	Antenna Long. Moment Load MM (ff-lbs) (lbs)	Trans. Load (1he)	Vert. Load
P-A	6 Stand-Off	180.00	37.272	0.50	3.00	270 00						00.0	113 15 150 00	150 00
Top-B	6 Stand-Off	160.00	37.72	0.50	3 00	30 00								
-C1	6 Stand-Off	180.00		0.50	3 00	170-00							111111111111111111111111111111111111111	DO DEL
	6' Stand-Off	180 00	37.72	0.50	3.00	130-00							113 15 150 00	
	Sinclair SC479-HF1LDF w/ ice	180.00		0.50	6 54	150 00								
	6' Dish with Radome	180.00		0.50	29 00	270-00								
CSP-76 6'	' Dish with Radome	180.00	37.72	0.50	29 00	30 00								
	Dish with Radome	180-00	37.72	0.50	29 00	150_00								
CSP-1 Scal	Scala OGT9-806 w/ ice	180.00		0.50	1 98	30 00								· ·
CSP-74	TMA	180.00	37.72	0.50	1-00	30 00							00-00	
TOG-5 And	Andrew DB-583 w/ ice	180_00		0.50	0 71	00 020								

1500000 27000000 27000000 27000000 27000000 27000000 27000000 27000000 270000000 270000000 270000000 270000000 2700000000 270000000000	
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 ~ ~ 0 ~ ~ ~ 0 ~ ~ ~ 0 ~ ~ ~ ~ ~ 0 ~
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	4 4 7 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
ふう/ なくしたいなながく// ひ 古田 コ C 0 コ C 1 ひ C 1 D C 1	S S 81-81-81-81-81-81-81-81-81-81-81-81-81-8
NNECC CCSSPT	

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100 100 100 100 100 100	പപരര പെപരര
	0.00

7 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	Total Weight (lbs)	4851 6251 6251 10139 13099 13715 15628 15628 16409
45.91 179.550 83.77 83.77 83.77 83.77 83.77 83.77 84.62 84.69 844.69 17.21 17.21 17.21 17.21 17.21 17.21 17.21 17.21 17.21 17.21 17.21 17.21 17.21 17.21 17.21 123.93 844.69 844.69 1123.93 844.69 844.69 1123.93 12	Total Wind 1 (lbs)	5266 5266 6581 7521 7652 7833 7366 6568 6558
	NotF WA (1bs)	00000000
	NotF AAR*CAR (ft^2)	
	NotF CAR A	11 20 20 20 20 20 20 20 20 20
	NotF AAR (ft^2)	
	NotF CAF	00000000000000000000000000000000000000
	NotF AAF (ft^2)	000000000000000000000000000000000000000
	Face WF (lbs)	5266 5281 7521 7652 7652 7668 7068 6558
	Face AE (ft^2)	78.5 103.2 1123.2 131.0 141.6 140.7 147.3 152.0
	Face CF	1111111 8672 8622 8622 8622 8622 8625 8625 8625 862
	e Face R RR	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
270 270 270 270 270 270 270 270 270 270	DE Fac	8885 1 00 8855 1 00 885 1 00 885 1 00 885 1 00 895 1 00 805 1 00 800 805 1 00 805 1 00 800 1 000 800 1 00 800 100 100 800 100 10000000000
Ч 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ace Fa	സരരരരസസസ ഉസമസയമര4 റ്റ്റ്റ്റ്റ്റ്റ്റ്റ്റ്റ്റ്റ്റ്റ്റ്റ്റ്
	Face Face Face Face Face Face Face Face	63 7 0 91 0 118 3 0 145 5 0 145 5 0 145 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
	2) AR	20 20 20 20 20 20 20 20 20 20 20 20 20 2
$\begin{array}{c} 0.00\\$	Face Fa AR RR*. (ft^2) (ft^	2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
	ace AF	6 58 4(6 58 4(73 43 4) 0 24 58 9 01 6 9 32 6 1 79 7 1 39 7 7 39 7
	0 deg к. т) (f	50 100 50 100 50 100 50 100 50 100 50 110 50 111
 Staa 11,42 12,42 12,42 12,42 12,42 14,12 14,12 14,14 1	"W+I sh Thi E) (00000000000000000000000000000000000000
RFS PD114 RFS PD114 RFS PD114 RFS PD114 Swedcom SLC Andrew DB8 XA-171063-8B XA-171063-8B XA-171063-8B Swedcom SLC Andrew DB8 XA-171063-8B Swedcom SLC Andrew DB8 Stores D8 Stores D8 Stores D8 Stores D8 Stores D8 Stores D8 Stores D8	on for "V 7. qzGh 1. (psf) :) (psf)	0 37 11 0 35 81 0 35 87 0 32 77 0 32 94 0 28 80 0 28 80 0 23 23
RFS PD1142 RFS PD1142 RFS PD1142 RFS PD1142 12 88 98 98 91 91 91 91 91 91 91 91 91 91 91 91 91	Case Information for Z of Ave. Elev. qz(ottom Above Gnd. (ft) (ps)	170.00 150.00 1130.00 110.00 70.00 50.00
ene ene ene ene ene ene ene ene ene ene	ase Inforn Z of Ave. ttom Above (ft)	
		1120 1120 1120 1200 1200 1200 1200 1200
CSP-66 CSP-66 DEP-55 DEP-55 GFS-66 GFS-66 GFS-66 GFS-66 GFS-66 GFS-69 GFS-66 GFS-69 GFS-69 GFS-69 GFS-69 GFS-69 GFS-69 GFS-69 GFS-66 GFS-66 GFS-66 GFS-66 GFS-66 GFS-69 GFS-66 GFS-66 GFS-66 GFS-69 GFS-66 GFS-66 GFS-69 GFS-66 GFS-69 GFS-66 GFS-69 GFS-66 GFS-66 GFS-69 GFS-66 GFS-69 GFS-66 GFS-69 GFS-69 GFS-66 GFS-69 GF	I O I	<pre>1 180 000 000 000 000 000 000 000 000 00</pre>
	EIA Sect. Section Label	ч ш О С Ы Г О Ш
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0 6837 17608 10,00 23,23 0,50 118,53 79,21 55,98 381,9 0,52 0,85 1,00 0.71 1,88 156,7 6837 0,00 2,00 0,00 1,20 0,00 I 20 00 0 00

Equipment Load Case Information for "DL + Ice Only":

Topology Control Contro Control Control <t< th=""><th>Labent</th><th>Equipment E Property Set</th><th>Elevation Above Ground (ft)</th><th>qzGh (psf)</th><th>Ice Thick. (in)</th><th>Total Wind Area (ft^2)</th><th>Wind Incidence Angle (deg)</th><th>222-G CA</th><th>222-G CS</th><th>222-6 CM 5</th><th>Antenna Axial Load FAM (lbs)</th><th>Antenna Side Load FSM (lbs)</th><th>Antenna] Moment MM (ft-lbs)</th><th>Long. T Load (lbs)</th><th>Trans. Load (lbs)</th><th>Vert. Load (lbs)</th></t<>	Labent	Equipment E Property Set	Elevation Above Ground (ft)	qzGh (psf)	Ice Thick. (in)	Total Wind Area (ft^2)	Wind Incidence Angle (deg)	222-G CA	222-G CS	222-6 CM 5	Antenna Axial Load FAM (lbs)	Antenna Side Load FSM (lbs)	Antenna] Moment MM (ft-lbs)	Long. T Load (lbs)	Trans. Load (lbs)	Vert. Load (lbs)
Statististististististististististististist	Top-A	Sr	180 00	0	81 F	3 00	00 0							00.00	00	50 00
Bitchild Control	Top-B	SC	180.00	0		3.00	120,00							0.00	000	150 00
Structure CC/17 CC/17 <thc 17<="" th=""> CC/17 CC/17 <</thc>	TO-do.T.	S S	180 00	0, 0		3 00	260.00							00 0	0.0	150.00
Thread (2) Control (2) <thcontrol (2)<="" th=""> <thcontrol (2)<="" th=""></thcontrol></thcontrol>	LOP-C2	0-4511.DF	TBO DO	20			220 00							00 0	00	150,00
C District Mich Mich Maddame District Mich Mich Mich Mich Mich Mich Mich Mich	CSP-75	ish with	180 00	20		00 60								00 00		69.80
C C Dist Alise Alise <td>CSP-76</td> <td>6' Dish with Radome</td> <td>180 00</td> <td>0</td> <td>112.4</td> <td>29.00</td> <td>120 00</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>00 022</td>	CSP-76	6' Dish with Radome	180 00	0	112.4	29.00	120 00									00 022
Statistic Science Note:	CSP-77	6' Dish with Radome	180 00	0		29,00	240 00							0000	00	220.00
Decisial	CSP-1	Scala OGT9-806 w/ ice	180 00	2		1,98	120 00							0 00	00	22.70
Decemblar JP-ACADA ACADA ACADA <td>CSP-/4</td> <td>TMA</td> <td>180.00</td> <td>0,0</td> <td></td> <td>1,00</td> <td>120_00</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>0.00</td> <td>00</td> <td>15 00</td>	CSP-/4	TMA	180.00	0,0		1,00	120_00							0.00	00	15 00
District	CLOCT NPTI	Andrew UB-583 W/ 100	180 00	20		1/ 0	00 00							00 0	00	II 60
Sinclinit Component Component <t< td=""><td>NEU-20</td><td>Decibel PD-420 2 Day - Dipole Decibel PD-420 2 bay - Dipole</td><td>TRO DO</td><td><u> </u></td><td></td><td>2 4 B</td><td>120 00</td><td></td><td></td><td></td><td></td><td></td><td></td><td>0000</td><td>000</td><td>24.70</td></t<>	NEU-20	Decibel PD-420 2 Day - Dipole Decibel PD-420 2 bay - Dipole	TRO DO	<u> </u>		2 4 B	120 00							0000	000	24.70
6' Data with Rights 775/00 0.00	CSP-67	Sinclair SC479-HFlLDF w/ ice	175 00	20	A 1	4 5										24 10
F District C District C District C District C District C District District <thdistrict< th=""> <thdistrint< th=""> <thdistrint< th=""></thdistrint<></thdistrint<></thdistrict<>	CSP-31	6 Dish with Radome	175	0		29 00	00 0									00 000
Statular 5: 7 bitsh Matches Tick Tick <t< td=""><td>TOG-6</td><td>DB586-Y</td><td>175</td><td>0</td><td></td><td>0.40</td><td>0 00</td><td></td><td></td><td></td><td></td><td></td><td></td><td>00 0</td><td>00</td><td>10 00</td></t<>	TOG-6	DB586-Y	175	0		0.40	0 00							00 0	00	10 00
Binclair CS(7)-HELLID: V. 1cs 155.00 0.00 0.59 6.40 0.00 0.00 0.00 0.00 0.00 0.00 0.00	TOG-7	6 Dish with Radome	170	0		29 00	120.00							00 0	00	220 00
Sett Control	CSP-4	Sinclair SC479-HFILDF w/ ice	165	0		6 54	240 00							00 0	00	69,80
Totalit Contract	STRML63	SCALA UGT9-8U6 W/ ICe	100 100 100	0.0	1.0	80	120 00							00 0	00	22,70
3. Stant-off 10000 000 059 159 12000 000 559 159 12000 000 559 551 12000 000 559 551 12000 000 559 551 12000 000 559 551 12000 000 059 551 12000 000 059 551 12000 000 000 000 000 000 000 000 000	SPD-9	TAURA FYON FYON				00 00								0 0 0	000	20.00
3' Stand-off 160.00 0.00 55 120.00 Sinclatir 3C(3)-FFLIDF M/100 100.00 0.00 55 0.00 Sinclatir 3C(3)-FFLIDF M/100 100.00 0.00 55 10.00 0.00 0.00 Sinclatir 3C(3)-FFLIDF M/100 0.00 55 15.00 0.00 </td <td>159-4</td> <td>Stand</td> <td>160 00</td> <td>0</td> <td></td> <td>1.50</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>00.0</td> <td></td> <td>75 00</td>	159-4	Stand	160 00	0		1.50								00.0		75 00
Sinclair SC77-FFILDF W lete 161.00 0.00 0.55 6.54 120.00 0.00 155 6.54 120.00 0.00 155 6.54 120.00 0.00 155 155 155 155 155 155 155 155 155 1	159-B	3 Stand-Off	160 00	0		1.50	120 00								80	
Sinclair SC(7)=HILDF w/ ice 1000 0.00 0.50 0.00 TYN TN 100,00 0.00 0.50 0.00 0.00 TYN TN 100,00 0.00 0.50 1.00 0.00 0.00 TYN TN 100,00 0.00 0.50 1.00 0.00	STAM-65	10×4 6×4 Panel	1.60 00	0		0.50	0 00							0000	000	20 00
Sinclair Sordy-HTLDF M/ ice 160,00 0.00 0.55 6.54 120,00 TMA 160,00 0.00 0.50 1.50 120,00 TMA 160,00 0.00 0.50 1.50 120,00 127 T-Arm 155,00 0.00 0.50 10.00 0.20 127 T-Arm 155,00 0.00 0.00 0.50 10.00 0.00 127 T-Arm 155,00 0.00 0.00 0.50 10.00 0.00 127 T-Arm 155,00 0.00 0.00 0.50 10.00 0.00 0.00 127 T-Arm 155,00 0.00 0.00 0.50 10.00 0.00 0.00 Powerwave P771 155,00 0.00 0.50 10.00 TMA 155,00 0.00 0.50 0.50 TMA 155,00 0.00 0.00 0.5	CSP-70	Sinclair SC479-HF1LDF w/ ice	160_00	0	- B.	6 54	00 * 0							0 00	8	69 80
HIGLIER Device Fig. 100 10 10 10 10 10 10 10 10 10 10 10 10	CSP-71	· · · ·	160 00	0		6 54	120.00							00 0	00	69.80
3' Stand-Off 100 0:00 0:00 0:00 0:00 0:00 0:00 0:0	000000		160 00	2.0		6.54	240 00							00 00	00	69 80
TMA 166 00 0.00 0.50 1.00 </td <td>160</td> <td>3` Stand-Off</td> <td>160 00</td> <td></td> <td>81.11</td> <td>1 - CO</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>000</td> <td>15 00</td>	160	3` Stand-Off	160 00		81.11	1 - CO									000	15 00
12 T-Attin 155,00 0.00 0.50 0.00 0.00 0.00 12 T-Attin 155,00 0.00 0.50 10,00 122,00 0.00 <td>STAM-64</td> <td>TMA</td> <td>160 00</td> <td>0</td> <td></td> <td>1 00</td> <td>00 0</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>15 00</td>	STAM-64	TMA	160 00	0		1 00	00 0									15 00
12. ТК	ATT-A	12 T-Arm	150 00	0		10.00	00 0							00 0		200 00
<pre>12' T-ATTT 12' T-ATTT 12' T-ATTTT 12' T-ATTTTT 12' T-ATTTTT 12' T-ATTTTTTT 12' T-ATTTTTTTT 12' T-ATTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTT</pre>	ATT-B	12° T-Arm	150-00	0		10.00	120.00							0 00	00	500 00
FOWBETWARKE FOWBETWARKE FOUNDETWARKE FOUNDETWARKE <td>ATT-C</td> <td>12' T-Arm</td> <td>150.00</td> <td>0,1</td> <td></td> <td>10 00</td> <td>240 00</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>00 * 0</td> <td>00</td> <td>500 00</td>	ATT-C	12' T-Arm	150.00	0,1		10 00	240 00							00 * 0	00	500 00
Powerwave TTT TTT <thttt< th=""> TTT <thttt< th=""> <thttt<< td=""><td>TV-IIV</td><td>POWERWAVE /</td><td>150 00</td><td>0</td><td></td><td>00 00 00 00 00 00</td><td>0.00</td><td></td><td></td><td></td><td></td><td></td><td></td><td>00.00</td><td>00</td><td>35,00</td></thttt<<></thttt<></thttt<>	TV-IIV	POWERWAVE /	150 00	0		00 00 00 00 00 00	0.00							00.00	00	35,00
Titosson RRU TWA TSU TVA TSU TVA TSU TVA TSU TVA TSU TVA <thttta< th=""> TSU TVA <thttta< th=""> <</thttta<></thttta<>	24-114 24-14	LIVENTERA STATEMO		20		0.4.0								00 00	00	65-00
Ericsson RRU 150.00 0.00 0.50 0.00<	ATT-A4	· Щ	150.00	0	4.01.4	2 G										00 00
TNA 150.00 0.00 0.00 0.00 0.00 TNA 150.00 0.00 0.00 0.00 0.00 0.00 TNA 150.00 0.00 0.00 0.00 0.00 0.00 0.00 TNA 150.00 0.00	ATT-A5	Ericsson RRU	150.00	0		5 45	00 0							0000		80,00
TMA 150.00 0.00 0.50 1.00 0.00 0.00 0.00 0.	ATT-A6	TMA	150 00	0		1 00	00*0							0 00	00	15 00
TRA 150.00 0.00 0.50 1.00 0.50 1.00 0.50 0.00 0.50 0.00 0.50 0.00 0.0	ATT-A7	TMA	150.00	0		1 00	00 * 0							00 * 0	00 *	15,00
Raycap Surge Suppressor 110000 0.000 0	gw-I.T.W	TMA	150 00	0		1.000	00 0							00 00	00	15,00
Proversive T701 150:00 0:00 0:00 0:00 0:00 Proversive T710 155:00 0:00 0:00 0:00 0:00 0:00 Proversive T710 155:00 0:00 0:00 0:00 0:00 0:00 0:00 0:00 Proversive T710 155:00 0:00 </td <td></td> <td>AMAL SAFARA SAFARA</td> <td></td> <td>20</td> <td></td> <td>0 0 T</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>00-00</td> <td>00</td> <td>15 00</td>		AMAL SAFARA SAFARA		20		0 0 T								00-00	00	15 00
Powerwave Folderwave Folderwa	ATT-FI	saytap autya arphtessor Powarwaya 7770		0.0		н и И И И И И И И И И И И И И И И И И И И								00 00	000	20 00
Fowerwave 7770 150.00 0.00 0.50 0.00	ATT-B2	OWEEW	150 00	0	81214	8 4D	120.00									10.00
Ericsson RRU 150.00 0.00 050 5.45 120.00 TMA 150.00 0.00 050 120.00 TMA 150.00 050 055 5.48 240.00 TMA 150.00 050 055 5.48 240.00 Ericsson RRU 150.00 050 055 5.45 240.00 TMA 150.00 050 055 5.45 240.00 TMA 150.00 050 050 050 050 050 050 050 050 05	ATT-B3	Powerwave 7770	150.00	0		5 88	120 00									
Ericsson RRU 150,00 0.00 0.50 0.00 0.00 TMA 150,00 0.00 0.50 120,00 0.00 0.00 TMA 150,00 0.00 0.50 120,00 0.00 0.00 0.00 TMA 150,00 0.00 0.50 1.00 120,00 0.00 <td< td=""><td>ATT-B4</td><td>Ericsson RRU</td><td>150.00</td><td>0</td><td>. e.</td><td>5.45</td><td>120.00</td><td></td><td></td><td></td><td></td><td></td><td></td><td>0 00</td><td>00</td><td>80 00</td></td<>	ATT-B4	Ericsson RRU	150.00	0	. e.	5.45	120.00							0 00	00	80 00
TMA 150.00 0.50 0.50 120.00 0.50 120.00 0.00 0.00 0.00 0.00 0.00 0.00 0	ATT-B5	ricsson R	150.00	0	1.1.4	5 45	120 00							0.00	00	80.00
TMA 150,00 0,00 120,00 0,00 0,00 0,00 0,00 0,0	ATT-B6	TMA	150-00	0		1.00	120 00							0.00	00	15,00
TMA 150,00 0,00 0,50 1,00 120,00 0,00 0,00 0,00 0,00 0,00 0	ATT-B/	TMA	150.00	0		1, 00	120,00							0 00	00	15.00
POWERWAVE 7770 150,00 0,00 0,50 1,20,400 0,00 0,00 0,00 0,00 0,00 0,00 0,	99-LIV	TMA	150.00	0,0	10.0	1 00	120,00							00 0	00.	15.00
-1 FOREFWAYE 1.7/0 199/00 9.50 3.48 249.00 C2 POWERWAYE 1.7/0 199/00 9.50 8.40 240.00 -3 POWERWAYE 77/0 199/00 0.00 0.50 5.48 240.00 -3 FEICSSON RRU 150.00 0.00 0.50 5.45 240.00 C4 EFICSSON RRU 150.00 0.00 0.50 5.45 240.00 C5 5.45 240.00 C6 0.00 0.00 0.00 0.00 0.50 5.45 240.00 C7 0.00 0.00 0.00 0.00 0.50 0.50 0.50 0.5	DA-TIS	TWA	00 001	20	1.4	1 - 0 0	120 00							0 0 0	00	15,00
C2 2.000 0.00 0.00 0.00 0.00 0.00 0.00 0	ATT-C1	POWERWAVE ///U Powerwave President	150 00	<u> </u>		0 C C C C C C C C C C C C C C C C C C C	240 00							00 0	8	35.00
-C4		CVELWEY FULL FULL FULL FULLEN		20		0 4 0 0 0 0 0	240,000							00.00	00	65 00
-C5 Ericsson RRU 150,00 0,00 0,50 5,45 240,00	ATT-C4	Ericsson RRU	150.00	0	e : - 4	0 4 0	240-00								00	
	ATT-C5	Ericsson RRU	150.00	0	1.4	5 45	240.00							0.00	00	B0 00

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00000	Face AG (ft^2)	163 7 191 0 218 3 245 5 272 8 300 0 3527 3 354 6 381 9
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140.00	Face AR t^2) (40.54 4845 49358493 58493 63.28 64.43 772.90 772.90
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Ericsson B4A/B12P RRHU-11 RRI RRHU-11 RRI RRHU-11 RRI RRHU-11 RRI ormation for "Di .	Z of Ave. Elev. qzGh ottom Above Gnd. (ft) (ft) (psf)	170.00 0.00 150.00 0.00 110.00 0.00 90.00 0.00 70.00 0.00 50.00 0.00 50.00 0.00 51.00 0.00 51.00 0.00
Eri Inform	Ave. Above	
ם א ב ב	of Z of Ave. Elev. Top Bottom Above Gnd. ft) (ft) (ft)	160.00 140.00 80.00 60.00 20.00 20.00
e-8 e-9 -11 -12 -12 Load	Z of Top B (ft)	180 00 160 00 160 00 140 00 120 00 120 00 120 00 100 00 120 00 100 00 100 00 80 00 100 00 20 00 40 00 20 00 40 00 20 00 20 00 20 00
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*** Analysis Results

Maximum element usage is 91.08% for Angle "SNB-LH2P" in load case "W+I 0 deg" Angle Forces For All Load Cases: Positive for tension - negative for compression

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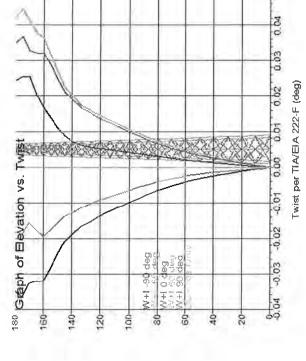
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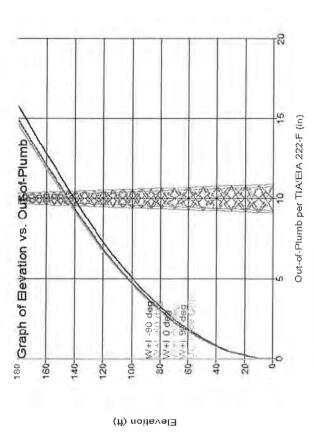
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Connect GaP Connect GaP Connect H1 Connect H	Connect lar Connect lar Connect la2 SNB-H1aP SNB-H1aP SNB-H1aP SNB-H1aP SNB-H1aP SNB-H1aP SNB-H1aP SNB-H1aP SNB-H1aP SNB-H1aP SNB-H2aP SNB-H2aP SNB-H2aP SNB-H2aP SNB-H2aP SNB-H3aP	SNDB- F13 F6 SNDB- F13 F6 SNDB- F13 F6 SNDB- F14 b P SNDB- F14 b P SNDB- F14 b P SNDB- F14 b P SNDB- F16 b P SNDB-	SNB- H 6 0 SNB- H 6 0 SNB- H 6 0 SNB- H 6 0 SNB- H 7 a SNB- H 7 a
Connect Connec		S NNB-41 S NNB-41 S NNB-41 S NNB-41 S NNB-41 S NNB-41 S NNB-41 S NNB-41 S NNB-42 S N	SNB-42 SN

7.941	7 943	7 943	-0 000	000 0-	-0 000	0 000	0 000	0 0 0 0	0.001	0 001	100 0	0 001	100 0	0.001	0 001	0 001	100 0	0 001	0 001	100 0	000 0	0 0 0 0	0 0 0 0	0 002	0_002	0 002	0 016	0 016	0 016
7 674	11 391	11 695	-0 111	0 146	-0 036	-0 131	0 149	-0 021	-0 179	0 192	-0 010	-0 264	0 272	-0.005	-0 261	0.270	-0 006	-0 277	0 282	-0,003	-0.341	0 342	-0.001	-0 323	0 326	0 001	-0 280	0 312	0.016
5 586	11 987	11 987	-0.111	0,052	0.054	-0.137	0.068	0.067	-0.195	0,099	0 099	-0.291	0.147	0.147	-0.287	0.145	0 145	-0.306	0.154	0,154	-0.378	0 189	0.189	-0.359	0.181	0.181	-0.313	0.180	0.180
3.225	10.507	9.918	-0.107	-0.108	0.204	-0.099	-0.101	0.201	-0.126	-0.125	0.252	-0.176	-0.176	0.355	-0.174	-0.174	0.352	-0.181	-0.182	0.366	+0.221	-0.221	0.442	-0.208	-0.208	0.420	-0.175	-0.175	0.397
6 093	6 0 9 9	5.592	0.052	-0.113	0.055	0.069	-0.137	0.070	0.098	-0.195	0.098	0.147	-0.291	0.147	0.145	-0.287	0.145	0.154	-0.306	0.155	0.189	-0.378	0.189	0,181	-0.359	0.181	0.180	-0.313	0,180
8 283	4 292	3.987	0.146	-0.112	-0.035	0.150	-0.130	-0.018	0,192	-0.180	110.0-	0.272	-0.264	-0.005	0.270	-0.261	-0.006	0.282	-0.277	-0.002	0.342	-0.341	-0,001	0,326	-0.323	0.001	0.312	-0.280	0.016
0000	000 0	000 0	-0 111	-0,,113	-0 036	-0 137	-0.137	-0,021	-0 195	-0 195	-0 011	-0.291	-0 291	-0.005	-0 287	-0-287	-0 006	-0 306	-0-306	-0 003	-0 378	-0 378	100 0-	-0 359	-0 359	0 0 0 0	-0 313	-0 313	000 0
8 283	11 987	11 987	0 146	0 146	0 204	0 150	0 149	0.201	0.192	0.192	0 252	0.272	0 272	0.355	0.270	0.270	0 352	0 282	0.282	0,366	0 342	0 342	0 442	0 326	0.326	0.420	0,312	0.312	0 397
4.63	6.70	6.70	0.54	054	0.75	0.73	0.73	52.0	1,25	1.25	0.93	2.42	2.42	1,31	1,37	1,37	0.84	1.87	1.87	0.87	2.92	2 . 92	1.05	3.43	0 T T	1,00	3.62	3.62	55 0
SNB-H9dP	SNB-H9eP	SNB-H9fP	SNB-WL-AlP	SNB-WL-A2P	SNB-WL-A3P	SNB-WL-BlP	SNB-WL-B2P	SNB-WI-B3P	SNB-WL-ClP	SNB-WL-C2P	SNB-WL-C3P	SNB-WL-DIP	SNB-WL-D2P	SNB-WL-D3P	SNB-WL-ElP	SNB-WL-E2P	SNB-WL-E3P	SNB-WL-F1P	SNB-WL-F2P	SNB-WL-F3P	SNB-WL-G1P	SNB-WL-G2P	SNB-WL-G3P	SNB-WL-H1P	SNB-WL-H2P	SNB-WL-H3P	SNB-WL-IlP	SNB-WL-I2P	SNB-WL-I3P
SNB-H2	SNB-H2	SNB-H2	WLAC-1	WLAC-2																									



Elevation (ft)



Twist and Out-of-Flumb for "W+I -90 deg":

о ћ Н	(ui											14													
Out of	10	0	0	0	0	0	-1	Ч	2	ന	cn	4	4	S	6	0	5	00	თ					12	
Sway		00																							
S	(de	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
st	eg)	00							0	0															
Twist	(de	0	Ŷ	01	0	0	01	0	0	0-	0	0-	0-	0-	0-	0-	0	01	0	0	0	0	9	0	0
Ton	ft)											94													
Elevation	-	0										99	$^{\circ}$	0		N	\sim	ŝ	4	∇	ŝ	ŝ	ō	Ó	5
Ele	3																								

175.00 -0.04 0.68 14.22 180.00 -0.03 0.68 14.93

Twist and Out-of-Plumb for "W+I -60 deg":

Out of Plumb	(ui)	0.00	0.03	0.20	0.41	0.76			2.26		3.50	4.05	4.65	5.28	5.98	6.72	3.48	8.32	5.17	9.80	10.50	11.16	11.86	12.54	13.25	13.95	14.65
Sway	(deg)	0.00	50.04		1	**	0.20	n;	14	3	LE 0	7	1	17	5	ŝ	10	ŝ	10	43	5	19	40	10		174	0.67
Twist	(deg)	0	0	-0.00		0	0	0		0		0			\odot	0		0	0	0	0	-0.02	0	0		-0.02	
Elevation	(ft)	- 74	0	1	30 00	40.00	50 00	60 00	70.00	0 0	9	93 34	0.00	06.6								155 00				175,00	

Twist and Out-of-Plumb for "W+I 0 deg":

Elevation Twist Sway Out of

SWAY OUT OF	-	0.0	0.0	0 2	2.0.4	7 0.8	2 1.2	7 1-8	1 2 4	3 2	е С	43 4 40	5 0	9 5 7	4 6 4	57 7.2	50 8.1	63 9.0	6 6 99	66 10 6	0 11 02
AS	(deg	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0				0
TWIST	(deg)		0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	C
FLEVACION	(ft)	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	6.6	93.34	0.00	06.6	13.3	20.0	6.6	5.50	40.0	45.0	0.05

12.80 13.54 14.30 15.04 15.80 0.71 0.71 0.72 0.72 0.72 0.02 0.02 0.03 0.03 00000 160. 165 170 175.

deg"; 60 I+M" Tol Twist and Out-of-Plumb

Sway (deg) (4eg) Elevation Twist (ff) (10,000 20,000 20,000 20,000 20,000 20,000 50,000 50,000 50,000 50,000 86,000 86,000 86,000 86,000 1113,54,113,54,113,113,54,113,56,113,56,113,56,113,56,113,56,113,56,113,56,113,56,113,56,113,56,113,56,113,56,113,56,113,56,113,56,113,56,113,56,1 ł

deg": 90 I+M.. for Twist and Out-of-Plumb

ä Out Sway Elevation Twist

Plumb (ni)	0.	0	0 21	4	r-		5	m	0	'n	e	5	ŋ	٢	8	Ó	4	m)	
(deg)	0	0	0,06	e	늰	2	2	2	m	3	4	4	4	S	ŝ	ŋ	6	6	
(deg)	0	0	0.00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
(ft)	0	0 0	20.00	0 0	0 0	0 0	0 " 0	0 0	0 0	6.0	m m	0.00	06.6	13 3	20 0	26.6	е е	40 0	

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 0.63
 10.00

 150
 00
 0.02
 0.65
 11
 71

 155
 00
 0.03
 0.67
 12.79
 78

 166
 00
 0.03
 0.67
 12.78
 78

 165
 00
 0.03
 0.67
 12.78
 78

 170
 00
 0.03
 0.67
 12.78
 78

 175
 00
 0.03
 0.67
 12.78
 78

 175
 00
 0.03
 0.67
 12.78
 78

 175
 00
 0.03
 0.68
 13.49
 17

 175
 00
 0.03
 0.68
 14.92
 14.92

Twist and Out-of-Plumb for "DL + Ice Only":

Elevation Twist Sway Out of

	(ur)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9	0	0	0	0	0	0	0	0	0	10.0	9	
י א ר נ	(deg)	00 0	0	0	0	0	0	0	0	0	00 0-	0	00 0-	$^{\circ}$	00 0-	00 0	0	-0.00	0	0	$\overline{\circ}$	00:0-	0	$^{\circ}$				
0 1 1 (1 1	(deg)	00 0	0	0	0	0	0	0	0	0	0	0		0	\bigcirc		\odot	\odot	0		\odot	0						
j j	(22)	0	0 0	Ō	0 0	0 0	0 0	0 0	0 0	0 0	6 6	е Э	0 00	06 - 6	13 3	20.00	26 .6	33 . 3.	40.0	45.0	50 0	55.0	60.09	65 .0	70.0	5.0	80 0	

*** Analysis Results for Load Case No. 1 "W+I -90 deg" - Number of iterations in SAPS 12

Equilibrium Joint Positions and Rotations for Load Case "W+I -90 deg":

1 1 1 1 0 1	ь ц	ons and Kot Y-Displ	ations io Z-Displ	r Load X-Rot	Case "W+I	5 1	: - DO		2-Dos
Label	9		-	deg	deg	de	-	\sim	5
RohnAP	000440	24	0833	0 671	100	0	4 33	24	66
RohnBP	0 00103	-1.0	0,0740	0.681	001	019	5	-1-0	59
RohnCP	0 00157	781	-0.064	0.614	.001	0.005	- 90	781	90-
Коппир	500000-	5/0°0-	10450°0-	0.1456	0 0	0 0025	0 693	-0.573	119 9
Rohnep	0 00301	254	0.0359	0	000	200	- C	000	ກ ອ ກອ
RohnGP	0 00343	0 144	0.0269	0.269	0000 0	002	0 C	0-144	n o n o
RohnHP	0 00377	0.0654	0.0180	0.184	002	004	2 M	0 0654	0
RohnIP	0.00376	0175	00907	0.093	019	002	9 0	0175	0
RohnJP				0 0 0 0	000	000	4	1	-
SNB-AP	000343	1.24	0.0802	0,679	0,003	000	33	1.24	01
SNB-BP	000247	1.00	0,0709	0.669	0.002	000*	12	1,00	59
SNB-CP	0.000794	-0.7793	-0,06174	0.628	.003	.002	90	-0 7793	139 9
SNB-DP	0.00131	0.57	0.0525	0 552	0.008	002	6.0	-0 57	19
SNB-EP	0.00180	0 396	0.0434	0.443	0_005	.002	47	0.396	6 6
SNB-FP	0 00223	0 254	0.0347	0.371	0.008	002	- 26	0 254	0
SNB-GP	0.00265	0 144	0.0260	0.274	0.010	003	05	0.144	6
SNB-HP	0.00299	0.0655	0.0174	0 184	0 007	003	8	0 0655	0
SNB-IP	0.00298	# 0174	00873	0.094	010	003	62	0174	6
SNB-JP	0			0 0 0 0	000	000	41		
RohnAl	000101	24	0.12	0.681	003	0.032	2 16	99	
RohnA2	0.00426	24	0394	0.678	0.02	0 034	2 16	50	18
RohnB1	-0.0004022	-1-004	124	0,667	0.03	028	0	15 439	
RohnB2	0 00437	0.0	0 023	0.670	0.02	0.031	2 55	42	16
RohnC1	000307	0 776	0 118	0.622	000	0.022	2 95	6.0	104
RohnC2	0 00337	0.780	,0104	0.622	0.09	0.023	2 95	4 33	14
RohnD1	000133	0.568	0.107	0,546	.003	0.017	3 34	36	
RohnD2	0 00346	0.573	00195	0.547	002	0 018	3 34	5.22	12
RohnEl	000548	0 392	0.0932	0.447	.003	0.012	5 - 7	87	S)
RohnE2	0.00302	398	00308	0.447	001	0.013	3 73	0 = 9	10
RohnF1	000590	0.251	0.0779	0,356	008	0.009	4 13	41	6
RohnF2	00303	25	.00615	0.358	002	0.010	4 13	6.90	0-0
RohnGl	00103	0-141	-0-060	0,265	000	0.007	4 52	98	5
RohnG2	00272	147	00664	0,266	0 0 0 0	0.007	52	7.69	0
RohnHl	00120	0.0620	0.0421	0,183	0 0 0 0 0	0.004	4 91	58	0
RohnH2	0027	0.0687	00613	0.178	100 0	0.004	4 91	45	0 0
RohnIl	100 0	.0143	0.0210	0,101	0.05	0% 0.02	5 31	0.7	9 9
RohnI2	0230	0.0208	00293	0,066	0.013	0, 002	5 31	18	Ň
RohuJl	0			0.000	000 -	000	5.7	8	0
RohnJ2		0	0	0 0 0 0	000	0007	5	9 . 0	0
SNB-Al	00170	1.24	0 104	0.675	-004	000	1.16	3 26	5 0 1
SNB-A2	17100-	1,24	0-056	0.682	003	001	1.16	775	თ
SNB-B1	00156	1 00	0.102	0.662	004	000	1 55	3.70	50
SNB-B2	.00158	L.00	0.0397	0,669	000	0.001	I 55	1 69	10
SNB-C1	.00143	777.0	0 0981	0 622	003	0 0 0 0 0	1 95	.16	- 16
SNB-C2	-00153	0.780	0.0251	0.629	- T O O -	0.001	1 95	.60	4
SNB-D1	00135	0.570	0 0909	0.541	000	0.001	2 34	63	5
SNB-D2	00147	0.573	0.0140	0.557	.002	0.002	2.34	94	12
SNB-E1	0,0013	0.394	0.0790	0.435	0.04	0.001	2 73	14	6 6
SNB-E2	00147	0.398	00772	0 445	001	0.001	2.73	34	99 99
SNB-F1	00133	0.252	0.0680	0.353	010	0.001	3.13	00	6.0
SNB-FZ	001488	-0.2569	-0 001259	0.367	100	100	13	5 17	ŝ
	1 5 T D D - 0	741 O	1500-0	507 0		100 0	201	201	59 95
	CTNN*N	1670 V	6/ TOO	7/7-0	000	100 0	200	5 96	
TULONO	277 T O O *	Tron A		6/T=0	C O O	TOOSO	5 0	0.0	39 96
20-DUC	SOTO	1910	07000			100 0		20	3 0
	100.		0-012C	5-04	000	TOOLO	ΤC - F	α T	n T
URS Connecti	cut - nss-03	5 t mobile							

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SNB-1-1 SNB-1-1 SNB-1-1 SNB-1-1 Robinals Suns-Pals	NN-WL-E3 NN-WL-F1 NN-WL-F1 NN-WL-F1 NN-WL-G1 NN-WL-G2 NN-WL-G2 NN-WL-H2 NN-WL-H1 NN-WL-H1 NN-WL-H1 NN-WL-H1 NN-WL-H1 NN-WL-H1 NN-WL-13 NN-

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Joint Support Reactions for Load Case "W+I -90 deg"

Z-M. Usage % Z Moment 1 (ft-k) Y-M. Usage Y Moment ((ft-k) X-M. Usage X Moment 1 (ft-k) Result. Usage 1 % Result. Force (kips) Uplift] Usage % Comp. Usage \$ Z Force (kips) Y Usage % X Y Usage Force U %(kips) X Force (kips) Joint Label

Max. Usage § Page 66/126

hnJP	7 . 26		1 84	0.10	20 23	0.0		21 58	0.0	-0-13	0.0	40 10	0.0		0 0	0.0
SNB-JP	15 29	0 0	3 77	0.0	19=73	0.0	0.0	25 24	0.0	1 59	0.0	10.9	0 0	0 08	000	
hnJl	4 .06		8 50	0.0	250 86			251 04	0.0	2 32	0 0	-1-5	0 0		0	0
hnJ2 -	-11 30		1.08	0.0	-210-74			212 10	0.0	-7-73	0	4-4-	UCU.		0	
[B-J]	5 24		2.86	0.0	302 63			302.95	0 0	4 10	0	4 1-	0		0 0	
IB-J2 -	-20 54		19.30	0.0	-263 36			267 07	0.0	07 0 0 -14 83 0 0 -9 4 0 0	0.0	01	0.0		0.0	0

Joint Displacements, Loads and Member Forces on Joints for Load Case "W+L -90 deg":

		1-1-4-	(kips)	Force (kips)	Force (kips)	Force (kips)	Disp. (ft)	Disp. (ft)	Disp. (ft)
RohnAP	000	380			380	L LO	-0-0004	246	-0-083
RohnBP	000	1.477			477	669	0.001	1 01	0
RohnCP	0	-1.181.	-0.9164	-0,0000	1.1811	0 9164	5	0 781	9
RondF	000	1:357		0	357	534	0.002	573	0.0
ЭÌ	000	0.573			573	860	00 0	ò	0
KONDEP		677 0			571. ×	159	0.003	0	0
NOTIFICE Dobodo	000	100			189	ηL	000		0
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Moments for Angles Modeled as Beams:

Angle Label	Torsion (ft-lbs)	Origin X Moment (ft-lbs)	Origin Y Moment (ft-lbs)	End X Moment (ft-lbs)	End Y Moment (ft-lbs)	X Shear (lbs)	Y Shear (lbs)
ohn-T.A1	8.0	17	0	2		- C	10
ohn-LA1	1 5	. m	1 10	n c	2	- L - G - L	2 4
ohn-LA1	1 8	5	5	0	4 2	101	. 0
ohn-LA2	4 8	4 6	0	38.4	Ι 5	0 5	0 4
ohn-LA2	1.5	n m	00	б 0	9	9	9
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ohn-LB4	3.0	567 3	12.5	50.4	13 9	63 5	5 3
ohn-LB4	4 3	60.0	08 1	13.5	35 3	6 9	0.0
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ohn-LC3	-5*0	138 6	75 1	91 2	З 2	7 8	5-1
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ohn-LD3	-4 3	336,2	3.4	59.7	37 5	18 5) ე
ohn-LD3	50	347.5	68 9	10.7	12.0	9.4	27.1
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onn-LEI	5 U U	459	5 2 6	08 0	126.5	20	39.6
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ohn-LE3	3.0	362.3	14 6	сл сл	73.8	- LO M	5
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ohn-LE3	-5-0	263.0	59.7	579-2	323 33	47.4	57.4
ohn-LFI	o i m	15	73.8	L-26	17.9	1.1	19.1
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9.1	7 0	1.1	-34.72	4 7	5.5	7.6	0	2.0	멍	10.1	5 2	5.2	1	E	22.9	2 9	9	1.6
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*** Analysis Results for Load Case No. 2 "W+I -60 deg" - Number of iterations in SAPS 12

Equilibrium Joint Positions and Rotations for Load Case "W+I -60 deg":

RohnAP RohnAP RohnAP RohnCP RohnCP RohnCP RohnCP RohnCP RohnCP RohnLP RohnLP RohnLP RohnLP RohnLP RohnLP RohnLP RohnLP RohnLP RohnLP RohnLP RohnLP RohnCP Ro	. 608		(121)		(deg)	(deg)			
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- IP	0.057	0.0147	0.0146	010	051	002	63	0.0147	0 0
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1H2 0	0343	0589	00927	150	087	0.001	4 88	8 46	0 0
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	1010	0.0182	-00449	051	030	000 0	5 30	9.18	\sim
Rohn.12	00						-0.71		
-A1 0	611	1.05	.0935	574	338	000	0 - 5 - 5	2 0 2	0
-A2 0	611	1.05	-0.052	580	337	000	555	962	0
-B1	0 49	0.854	0 0884	564	331	0.00-0	1.06	55	n
-B2 0	495	3.856	0.0354	568	328	0.000	1.06	1.84	16
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-E2 0	195	338	00297	378	218	000000	2 54	4 40	
-F1 0	125	0 213	0 0535	301	189	0.002	3 00	64	n on
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 <td deg": -60 I+M., Case Load for Support Reactions Rohnhacz Rohnhaz Sub-Baz Sub-Caz Sub-Baz Sub-Baz Sub-Baz Sub-Baz Sub-Baz Sub-Baz Sub-Caz Sub-Baz Sub-Caz Sub-Baz Sub-Caz Sub-Baz Sub-Baz

Joint

Max. Usage % Z-M. Usage Vr-M. Z Usage Moment U % (ft-k) Y Moment ((ft-k) X-M. Usage % X Moment 1 (ft-k) Result. Usage M % Result. F Force (kips) Uplift | Usage % Comp. 1 Usage * Z Force (kips) ک ک Usage F * : X Y Usage Force (%(kips) X Force (kips) Joint Label

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ohnJP -2	.12	0-0	1,50	0.0	150,50		0.0	150 52	0 0	-0-02	0.0	4 00	C	0 = 0.1	0.0	0.0
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SNB-J1 -2 22	22	0	2.47	0.0	179.23	0.0	0*0	179 26	0.0	6 89	0.0	-2 3	0.0	-0.07	0.0	0.0
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Joint Displacements, Loads and Member Forces on Joints for Load Case "W+I -60 deg";

Load Load Force F	Joint X	Externa	Externa	Exter	X Member	Y Member	Z Member	×	м	N
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Label	0 0	Load (kips)	Load (kips)	0 0	13 10	Force (kips)	Disp. (ft)	Disp. (ft)	Disp. (ft)
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Anna	687		• · · ·	0	190	516	608	058	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnBP	732	5	0	0	-268	669	492	0.857	0.102
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnCP	581	1 0	0	0	000	916	0 3799	(D	-0 0948
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnDF	667	1	÷.	0	156	534	277	485	084
$\begin{array}{cccccccccccccccccccccccccccccccccccc$		-275	4	0	ò	477	.860	191	336	072
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		349	6	÷.	0	606	159	121	215	059
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnGP	330	n i	-	0	572	397	067	122	~
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnHP	312	0	н.	0	540	525	0	055	031
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnIP	344	0	н.	0	- 596	9179	-004	014	2
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	2	182	с С	0	1,9324	180	9.518	000	000	000
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	4	010	4	0	0	122	134	609	.056	2
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	ωı	166	2	0	0	-288	324	493	855	80
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	10 1	229	m .	ő.	0	398	508	380	-661	082
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	ന	270	4	0	0	467	694	-278	.485	.074
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	SNB-EP	275	4		0	477	860	0 1920	m	2
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	SNB-FP	308	n i		0	533	.138	122	215	
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	AND-GE	0550	Ω.I		0	-572	397	0 68	0.122	-
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	SNB-HF	212	n i		0	540	525	029	055	Ξ.
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	VNG-LF	20	n (0	576	1 759	002	.014	014
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$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	TAULOA	000	υu			140 100	195	610	1 054	0.107
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$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnE2	.275	4		0	477	00	몃	0 3	2
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$\begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnF2	349	9	C.R.	0	606	159	126	218	0
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnG1	344	ŝ	-	-0 3442	596	417	071	119	9
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: 0 2758 -0 4776 -0 8607 -0 2758 0 4776 0 0 3080 -0 5335 -1 1382 -0 3080 0 5335 1	SNB-E1	275	0 - 4	1.1	-0.2758	477	. 860	195	0 33.	9
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Moments for Angles Modeled as Beams:

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*** Analysis Results for Load Case No. 3 "W+I O deg" - Number of iterations in SAPS 13

Equilibrium Joint Positions and Rotations for Load Case "W+I 0 deg":

	(£t)	(ft)	(ft)	(deg)	(deg)	(deg)	(ft)	(ft)	(ft)
RohnAP	31	00165	0.137	003	.711	021	5 647	0 00165	79
RohnBP	1.064	100.0	-0.1358	-0.0039	0.7043	-	6.185	0	159.9
RohnCP	578	= 0006/9	0 130	0000	661	600	6 733	0 000679	39
RohnEP	100 -	780000	TT D-		000	000	- 200		5 C
RohnFP	192	000406	0 0875	000	374	200	- 00 0 0 0 0 0 0 0 0 0	000040	- 0 n 0
RohnGP	0.150	.000317	0680	000 0	282	001	9 206	0.000317	10
RohnHP	65	0_00020	0 0476	0.000	194	.001	9.908	0.00020	0
RohnIP	.0153	000104	0 0238	0000	-107	000	10,65	0.000104	6.0
RohnJP GV AV	ē		, (000	000	000	11.42		
AR-DNC SNB-BD	- 4 G	202000			000	000	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	02000-0-	5
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SNB-EP	419	5 147e-00	0-0871	000	460	000	10 10	-5-147e-00	10
SNB-FP	.268	2_814e-00	0.0756	000	370	000*	6.537	-2.814e-00	5
SNB-GP	0.151	368e-00	0600	000	265	000	7.207	-1.368e-00	9.0
SNB-HP	673	4 566e-00	0.0428	000	184	000	7.908	-4.566e-00	9.9
SNB-IP	0161	4_43e-00	0 0217	000	098	000	8 644	-4_43e-00	5
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RohnC1	828	0 00130	0 0318	100	550	001	40	ר ד ד ר ו	2 9
RohnC2	827	00227	0 0318	0.007	648	0.021			1 1
RohnDl	608	0.0021	0 0227	001	580	000		10-10-1	10
RohnD2	. 607	00293	0 0226	0.002	- 579	019		5.79	
RohnE1	423	0 0 0 2 3 2	-0 015	000	473	008	0	-6.47	6.0
RohnE2	422	00297	0.0157	000	472	016	0	6.47	9.9
RohnFl	272	0.00295	0 0102	0.007	390	008	co	-7 -15	9.0
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Joint

Max. Usage % Z-M. Usage % Z Moment ((ft-k) Y-M. Usage % Y Moment 1 (ft-k) X-M. Usage % X Moment 1 (ft-k) Result. Usage 1 % Result.] Force (kips) Uplift I Usage % Comp. 1 Usage % Z Force (kips) Y Usage f Y Force (kips) X Usage * X Force (kips) Joint Label

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ohnJP -13.44	0 0 -0 05		0.0	0,0	304 71	0.0	0,03	0.0	1.9	0.0	00 0-	0-0	0.0
SNB-JP -20 96	0 0 -0 00	0.0 369.26	0-0	0.0	369 86	0.0	0.00	0.0	10	0	00 0-	0	0
ohnJl -10_30	0 0 -14 40		0.0	0.0	123 28	0.0	6.58	0.0	-4.0	0.0	00 0-	0-0	0 0
ohnJ2 -10 21	0 0 14 45		0.0	0 0	123,34	0.0	-6,61	0.0	-4.0	0.0	00 0-	0-0	0.0
VB-J1 -19 81	0 0 -27 14	0.0 -155,11	0.0	0.0	158.71	0.0	13.67	0.0	-6.9-	0.0	-0.07	0.0	0.0
VB-J2 -19 81	0 0 27 14	0.0 -155 15	0.0	0.0	158,75	0.0	-13.67	0.0	E-9-	0.0	0 07	0.0	0

Joint Displacements, Loads and Member Forces on Joints for Load Case "W+I 0 deg";

Joint X Label	External Y Load (kips)	: External Load (kips)	Z External Load (kips)	X Member Force (kips)	Y Member Force (kips)	Z Member Force (kips)	x Disp. (ft)	Y Disp. (ft)	z Disp. (ft)
1 KL	0	000	0	-1 3958	000	5	1	0	-0.1373
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364.6	7 49 1364 6	537 16 67 49 1364 6	2 1537 16 67 49 1364 6
537 1	2 42 -1537 1	364 69 -102 42 -1537 1	4 -1364 69 -102 42 -1537 1
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548 6	3 39 -1548 6	394 62 -23 39 -1548 6	7 -1394 62 -23 39 -1548 6

*** Analysis Results for Load Case No. 4 "W+I 60 deg" - Number of iterations in SAPS 12

Equilibrium Joint Positions and Rotations for Load Case "W+I 60 deg";

	(24)	(ft)	(ft)	(deg)	(deg)	(deg)	(ft)	(ft)	(ft)
R	608	1	-0,10	0 569	332	03	94	1 0	1
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ų (777	212	0 0535	0.314	166	0.002	6.3	215	n i n
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60 I+M., Case Load for Support Joint

Max. Usage * Z-M. Usage % Z Moment (ft-k) Y-M. Usage Y Moment ((ft-k) X-M. Usage | % X Moment ((ft-k) Result. Usage 1 % Result. Force (kips) Uplift] Usage % Comp. 1 Usage % Z Force (kips) r Usage * Y Force (kips) X Usage * X Force (kips) Joint Label

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	-2 11	0.0	-1.55 0.0	0.0	150 50	0.0	0.0	150.52 0.0	0.0	0 06	0.0		0 0	-0-01	0.0	0.0
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1- 11u	$-13_{\pm}11$		-22 44		-240 47	0.0	0.0	241.87	0.0	8 02	0.0		0 0	-0_{-01}	0	0
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	-23 94	0.0	-41_48	0.0	-299 49	0.0	0.0	303 30	0.0	15 92	0.0		0 0	00 0-	0 0	0.0
	-2.22		-2 47	0 0	179 18	0.0	0.0	179 21	0.0		0.0		0 0	0 07	0.0	0

Joint Displacements, Loads and Member Forces on Joints for Load Case "W+I 60 deg":

×
Z Member
Member
Member Y
External X
External Z
X External Y
Joint

Joint X	Exte	Exte	Externa	X Member	Y Member	Z Member	×	Т	23
тадет	kips)	kips)	Load (kips)	Force (kips)	Force (kips)	Force (kips)	Disp. (ft)	Disp. (ft)	Disp. (ft)
RohnAP		190	51	0 687	-1 1905	.516	609	059	-0-1080
uqo.		268	0 69	0 732		0	492	00	102
RohnCP		0069	-0.9164	00	-1,0069	0.9164	0.3799	0.6637	0.094
F-1 (664	860	275	9224 U-	n œ	191	4° (*	
RohnFP		606	1.159	0 349	0	159	121	00	059
RohnGP		572	397	330	0	397	067	122	0
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SNB-JP		316	978	848	2 8477	334	000	0000	0.0
RohnAl		345	.567	LLL	-	567	614	056	033
RohnA2		560	749	906	-	749	608	053	륗
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RohnD2		156	1.534	0.667		534	279	1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	0
RohnEl		477	0 * 8 60	275	-0-4776	860	196	338	0
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Moments for Angles Modeled as Beams:

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*** Analysis Results for Load Case No $_{\odot}$ 5 "W+I 90 deg" - Number of iterations in SAPS 12

Equilibrium Joint Positions and Rotations for Load Case "W+I 90 deg":

Label	(ft)	(ft)	(£t)	(deg)	(deg)	(deg)	(ft)	(ft)	(ft)
RohnAP	000457	24	083	-0.671	001	029	1 8	24	1 -
RohnBP	-0.001029	e i	- 1	0 T	0.0015	0.0193	- CU	1.01	159.9
Pohnor		TR/	0 0643	-0.614	100	002	600	781	5
RODDEP		795	04000	040 0-		200	ח ת פים	7/0	ο μ ο μ
Rohnep	0.00301	54	101	-0 373	100	200	- 9	24	10
RohnGP	-0.0034	144	0.0269	-0.269	000	002	. 05	144	0
RohnHP	00377	0 065	0.0180	-0 184	0.02	004	83	0.065	6.6
RohnIF	0.00376	0175	* 00907	-0.093	019	0.02	0.0	0175	0
RohnJP	100000	0		000 0	0000	000	Ч. Т		
SNB-AF	/ 25000 -0	24 C	0 0802	-0.679	0.004	000	(m) (m)	24	5
AND-DD AND-DD	577000 577000	0.0	60/0 0	10 001	200	000	210		50
SNB-DP	-0.001317	571	51550 UT	- 70 0- - U		200	200	1111	ח ר ח ר ד ר
SNB-EP	0.00180	396	0 0434	-0 442	0.005	002	04	396	10
SNB-FP	0.00223	254	0 0347	-0.370	0.008	002	26	254	0
SNB-GP	-0.0026	0.144	0.0260	-0.274	0.010	-003	.05	0 144	6 6
SNB-HP	0 00299	0655	0.0174	-0.184	0.007	- 003	833	0655	6 0
SNB-IP	0.00298	0174	00873	-0 034	010	0.03	. 62	0174	6
SNB-JP		¢	C F C	0000	000	000	9 41		22.
ROHn27	004200	4 C	1220	729 01	000	0.00	010	000	00 1
RohnB1	0 0043	100	0734	10 00-		100	7 C	200	2 1
RohnB2	000432	0	0 124	-0.669	0.005	029	201	5 43	0
RohnC1	0.00344	780	-0104	-0.622	008	023	2 95	33	14
RohnC2	015000	77.0	-0 118	-0.621	001	-022	95	5 89	139
RohnDl	0.00347	573	00197	-0.547	0,005	018	3 34	5.22	12
RohnDZ BohaF1	RSTODO 0	899 90 90	/01 0-	-0.546	E00	017	6 C C	36	
RohnE2	9000290	0000	000000	- 0 - 44 /	TOO O		5 15	0,03	D T O
RohnFl	10200°0	256	00613	-0 358	002	010	EL D	00.0	0 0
RohnF2	00056	251	0 0779	-0.356	0.008	000	4 13	14.6	00
RohnGl	.00272	.147	00663	-0-265	000	0.07	4.52	7.69	0.0
RohnG2	00103	0.141	0 0603	-0.265	0.000	001	4 52	7.98	0
RohnHl	00271	0687	.00612	-0.178	100.0	004	4.91	8.45	40 0
RohnHZ	02100 0	0.06208	-0-04213	-0.182	000	004	5	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ი ი
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RohnJ2	0	0	0	0000-0	000	000	10	000	
SNB-A1	:00169	. 24	0563	-0.683	002	001	1 16	776	29.
SNB-A2	00168	• 24	0.104	-0.674	003	000	1.16	3.26	1.4
SNB-B1	00158	00	0397	-0.668	000	0.01	1 55	69	16
ZU-UNU	Catoo	00'T	-0.10Z	-0.663	004	000	in i	3,70	1.1
CUTENS	C T C C	001	TC20				с л с п с	ng - 7	
SNB-D1	74100	- 14 - 14				000	D C C C	0 1	י ר י ר
SNB-D2	00135	570	000000	-0.541	000	001	10	144	10
SNB-El	.00147	398	00773	-0-445	100	001	2 73	4 34	0
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Max. Usage

Z-M. Usage

Z Moment (ft-k)

Y-M. Usage %

Y Moment (ft-k)

X-M. Usage *

X Moment ((ft-k)

Result. Usage M

Result. 1 Force (kips)

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Comp. U Usage %

Z Force (kips)

Y Usage %

Y Force ¹ (kips)

X Usage *

X Force (kips)

Joint Label

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2 5 24		0.0	302 58	0.0	0 0	302.90	0.0	-4.10	0.0	-1-4	0.0	0 04	0	6

Joint Displacements, Loads and Member Forces on Joints for Load Case "W+I 90 deg":

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$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		L	1.1	(cd+4)		(sdty)	(sdry)	(22)	(11)	(22)
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0.0000 1.1911 0.9464 0.0016 0.11811 0.0164 0.0016 0.1787 0.1281 0.0116 0.1787 0.0116 0.1787 0.0116 0.1787 0.0116 0.1787 0.0116 0.1787 0.0116 0.1787 0.0117	RohnBP	000		0 699	000	-1 4777	699	0.001	010	0 0741
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnCP	000		0.916	0 0 0 0	-1.1811	.916	0.001	781	c
0.0000 0.7235 -0.1807 -0.0000 -0.7246 -0.1000 0.025 (0.000) 0.00000 0.0000 0.0000	RohnDP	000	1.54	1 534	000	-1.3577	534	0 002	572	0
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$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Rohnff	000		1 15	000	-0.7246	.159	0,003	254	0
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnGP	000-		1 397	000	-0.6873	397	0,003	144	0
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnHP	000		1 525	000	-0.6489	.525	0,003	0.65	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	RohnIP	000		1 779	000	-0-7154	779	0.003	017	0.0
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0.0000 0.43457 0.3584 0.0002 1.7770 0.7770 0.7770 0.0000 0.47670 0.0000 0.6475 0.0002 1.7770 0.0013 0.5718 0.0013 0.5718 0.0012 1.7770 0.0013 0.5718 0.0014 0.0174 0.0174 0.0175 <td>SNB-AP</td> <td>000</td> <td></td> <td>134</td> <td>000</td> <td>-0.1463</td> <td>134</td> <td>000</td> <td>243</td> <td></td>	SNB-AP	000		134	000	-0.1463	134	000	243	
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$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	SNB-EP	000		0 860	000	-0.5735	860	0.001	396	0
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	SNB-FP	000		1 138	000	-0.6409	.138	0.002	254	0
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	SNB-GP	000		1 397	000	-0.6873	397	0.02	144	0
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Moments for Angles Modeled as Beams:

Angle Label	Torsion (ft-lbs)	Origin X Moment (ft-lbs)	Origin Y Moment (ft-lbs)	End X Moment (ft-lbs)	End Y Moment (ft-lbs)	X Shear (1bs)	Y Shear (1bs)
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*** Analysis Results for Load Case No., 6 "DL + Ice Only" - Number of iterations in SAPS 8

Equilibrium Joint Positions and Rotations for Load Case "DL + Ice Only":

Rehalf -0.000197 -0.000199 -	Joint Label	A-UISPI (ft)	r-ulspl (ft)	2-DISPL	(deg)	(deg)	(deg)	A-FUS (ft)	I-FOS (ft)	Z-PO: (ft)
	Апд	76000 0	0 000195	0 0778	000	0.0	0 0	4 332	0 000195	179
0.0002362 -0.0002361 0.0001	ONNEP	0.00146	0 000149	0 0698	000	000	9	5 12	0.000149	159.
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Joint Displacements, Loads and Member Forces on Joints for Load Case "DL + Ice ${\tt Only"}_{i}$

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	Label	Load (kips)	Load (kips)	Load (kips)	Force (kips)	Force (kips)	Force (kips)	Disp. (ft)	Disp. (ft)	Disp. (ft)
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Moments for Angles Modeled as Beams

Angle ' Label ()		Origin X Moment (ft-lbs)	Origin Y Moment (ft-lbs)		0.0	X Shear (1bs)	Y Shear (lbs)
ohn-LA1	0	0	0	0.5	-3.01	1 1	i)
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ohn-LB3	0	0 0	3 2	0	6	0	5 4
ohn-LB3	0	1.2	9 9	2,1	7 2	6	5
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ohn-LB4	0 0	1 o	n (0.0	0	0	ι Ω
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ohn-LC2	0 0	ц. О	4 7	0	2	0	0.0
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-242 44 326 46	26.4	26.4	26 4	371.6	71.6	71.6	71 6	71.6	71,6	85.6	85.6	85 6	85.6	85.6	85 6
-3 47		팋	12	11	5	1~	5	4 7	4 7	2	0.2	0 2	0.2	0 2	0 2
-598 93 1062 14	62 l	931 8	62 I 31 6	301.8	20 9	301,8	220.92	301,88	220.8	385,9	494 6	385 9	494 .6	385 9	494 6
-3.47 1.15	15	5	45	4 7	5	5	0	1	6. 8	2	0 2	0.2	0.2	0.2	0 2
-716 60 931 69	931 7	62 1	931 8	20 8	301 8	220 8	301 8	220 9	301 8	494 6	385 9	494 6	385 9	494.7	385 9
0.03	00	0	00	0	0	0	0	0 0	0	0	0	0	0	0	0
SNB-H6fP SNB-H7aP SNB-H7aP	B-H7C	NB-H7d	SNB-H7£P SNB-H7£P	B-H8a	SNB-H8bP	U	σ	-H8€	-H8£	гđ	SNB-H9bP	SNB-H9cP	U	SNB-H9eP	SNB-H9fP

*** Overall summary for all load cases - Usage = Maximum Stress / Allowable Stress Printed capacities do not include EIA allowable stress increase for wind load cases. Printed capacities do not include the strength factor enterted for each load cases The Group Summary reports on the member and load case that resulted in maximum usage which may not necessarily be the same as that which produces maximum force.

Group Summary (Compression Portion):

No. Of Bolts Comp.		000
Curve No.	****	000
L/R Length Comp. Member (ft)	211111 1 1111 11111 11111 111111 1111111	
	1170 1500 1500 1500 1500 1500 1500 1511 1111 1111 1111 1111 1111 1111 1111 1111	000
RLZ		0000
RLY	000000000000000000000000000000000000000	
RLX		000.00
comp. conn. Bearing Capacity (kips)	9,155 9,156 9,156 9,156 9,156 9,156 9,156 0,000000	000 0
comp. conn. c Shear Capacity (kips)	16 200 16 200 16 200 16 200 16 200 0 000 0 0000 0 000 0 0000 0 0000 0 0000 0 0000 0 0000 0 0000 0 0000 0 0000 0 00000 0 00000 0 00000 0 00000 0 00000000	
LA/R Capacity (kips)		000000000000000000000000000000000000000
Control Load Case	557 W+1 0 deg 1480 W+1 0 deg 1480 W+1 0 deg 156 W+1 0 deg 756 W+1 0 deg 756 W+1 0 deg 810 W+1 0 deg 815 W+1 0 deg 814 W+1 0 deg 815 W+1 0 deg 817 W+1 0 deg 9150 Geg deg deg 9150 W+1 0 deg 9160 M+1 0 deg 91760 M+1 0	
Force (kips)		
Control Member	Rohn-Dadi Rohn-Dadi Rohn-Dadi Rohn-Dadi Rohn-D651 Rohn-D631 Rohn-D631 Rohn-LA4P Rohn-LA4P SNB-D621 SNB	
Use In Comp.	0 8 6 / 0 / 0 / 0 / 0 / 0 / 0 / 0 / 0 / 0 /	0000
Usage U	「 4 4 0 8 4 4 0 0 0 0 0 0 0 0 0 0 0 0 1 1 1 1 5 4 0 4 0 0 4 0 0 4 0 0 4 0 0 4 0 1 0 0 0 0	
Size Strength Us (ksi)		
Size	<pre>75X1_75X0_875 2.5X2_3X0_1875 2.5X2_3X0_1875 2.5X2_3X0_1875 2.5X2_3X0_1875 3.5X3_55EH Pipe5ET Pipe5ET Pipe5ET Pipe5ET Pipe6EHS Pipe6ES 2.5X2,5X0,3125 2.5X2,5X0,535 2.5X2,5X0,535 2.5X2,5X0,535 2.5X2,5X0,535 2.5X2,5X0,535 2.5X2,5X2,5X0,535 2.5X2,5X2,5X0,535 2.5X2,5X2,5X2,5X5 2.5X2,5X2,5X2,5X5 2.5X2,5X2,5X5 2.5X2,5X2,5X5 2.5X2,5X2,5X5 2.5X2,5X5 2.5X2,5X5 2.5X2,5X5 2.5X2,5X5 2.5X2,5X5 2.5X2,5X5 2.5X2,5X5 2.5X2,5X5 2.5X2,5X5 2.5X2,5X5 2.5X2,5X5 2.5X5</pre>	2.5X2.5X0.3125 MOD-P4SCH80 222200 25
e l	SAE SAE SAE SAE SAE SAE SAE SAE SAE SAE	N N N N N N N N N N N N N N N N N N N
Desc	<pre>Diagonal Diagonal Diagonal Diagonal Robn Leg Robn Leg Robn Leg Robn Leg Robn Leg Robn Leg Robn Leg Robn Leg Robn Leg Robn Leg Sub Leg Robn Leg Robn Leg Sub Leg Robn Leg Sub Leg</pre>	Rohn Diagonal 3b Rohn Leg 3a Rohn Diagonal 2a
Label	Rohn-D1 R Rohn-D1 R Rohn-D2 R Rohn-D2 R Rohn-D2 R Rohn-D3 R Rohn-D3 R Rohn-L1 Rohn-L1 Rohn-L2 Rohn-L2 Rohn-L2 SNB-D3 SNB-D3 SNB-D3 SNB-D3 SNB-D3 SNB-D1 SNB-D3 SNB-D1 SNB-D1 SNB-D1 SNB-D1 SNB-D1 SNB-L2 SNB-	

Group Summary (Tension Portion) :

o. Of Hole Holes Diameter (in)	0 6875 0 6875 0 6875 0 6875	0 0 6875 0 6875		000	0000	0 6875 0 6875	0 6875	0 8125 0 8125 0 8125	000	000	0000	0 0	0.6875 0.6875 0	00	00	00	00
No. Of Holes D	0000	0000	1.000 0.000 0.000	0000	0000	0000	000	0000	0000				0000	0000	0000.000	0.000	000.000
No. Of N Bolts Tens.		0		000	0000	> =1 ++1	e4 e4 s	(1. y-1. y-1	000	000	0000		11 FL D	00	00	00	00
Length N Tens. Member (ft)		0 000 5 589 8 315	21 860 5 004 5 004			a 10 m h	1 412	17 700 18 841	5 004 6 665 665	10 008 10 008	2 002 4 066 7 472	11 238 4 066 6 110	.3 177 .3 570 0 008	10 008 10 008	1.14		000
	8 684 1 9 527 1 0 195 1	0 000 5 234 1 5 234 1				2 703	1 187 1	40.781 140.40	0000	000		000	13 594 1 16 992 1 0 000 1	000	0.000	0000	0000
Ten														-			
Tens. Conn. Bearing Capacity (kips)	8 156 8 156 8 156	0 000 12 187 12 187				13 594 10 875	13 594 21 750	32 625 32 625 32 625	000000000000000000000000000000000000000			16 312 0 000 0 000	10-875 13-594 0-000	000 0	000 0	4.1.4	0000
Tens. Conn. Shear Capacity (kips)	16 200 16 200 16 200	0 000 16 200 16 200	16 200 0 000 0 000			16 200 16 200	16 200 16 200	24 000				16 200 0 000 0 000	16,200 16,200 0,000	000 0	0000 0	000	000 0
Net Te Section Capacity (kips)	10.681 12.639 16.815	0 000 30 910 37 004	43 098 84 900 103 200	000 000 000 000		20 340 16 707	27 082 52 336 72 726	89 222 116 410	90 720 134 136 123 552	231 120 276 480 347 759	17279 973 88 668 134 136	23 973 20 304 31 536	22 144 27 082 366 683	306 144 251 187	226 293 174 651	000000	000
Tension Control Load Case	+1 90 deg +1 60 deg +1 60 deg	96-	90 1 - 60		рар О 1+м	06-	+I 90 deg +I -90 deg +I 90 de		+I -60 de +I 60 deg +I -60 de	- 60 de	0 deg 60 deg 60 deg	0 deg 0 deg 0 deg	+1 -90 de +1 -90 de +1 -60 de	+I -60 de +I -60 de	+I -60 de +I -60 de		
Tension Force (kips)	2 245W+I 4 309W+I 5 630W+I	U UUU 7 385W+I 9 416W+I	9 852W+I 10 944W+I 31 596W+I			2 162W+I 3 029W+I	6 129W+T 8 937W+I 8 601W+T	12 123W+I 24 017W+I	10 532W+I 58 592W+I 89 173W+I	165 644W 241 099W 276 679W	B 329 W+I 1 662W+I 11 988W+I	4 943 W+I 0 355 W+I 0 442 W+I	7 842W+I 9 031W+I 233 014W+I	220.078W+I 165.964W+I	135 772W+ 105 699W+		
Tension T Control Member	Rohn-DA72 Rohn-DB5P Rohn-DC1P	Rohn-DE51 Rohn-DF42	Rohn-DI42 Rohn-LA42 Rohn-LB42	רב סיד – HIDA	Rohn-H11	SNB-DA82 SNB-DB51	SNB-DD62 SNB-DE11 SNB-DE12	SNB-DH42 SNB-DI42	SNB-LA42 SNB-LC31 SNB-LD32	SNB-LF22 1 SNB-LH22 2 SNB-LH22 2		Rohn-DB7P SNB-WL-D3P SNB-WL-G3P	Rohn-DC51 Rohn-DD11 Rohn-LI22 2	$\sim \sim$	Rohn-LE32 1 Rohn-LD32 1		
Max Use In Tens,	0		60 63 9 67 22 96		0000	11 93 20 89	33 81 41 38 27 16	37 89 75 06	8 71 32 76 54 13	53 75 65 40 59 67	0 04 0 1 41 6 70		54.09 49.83 47.66	53.92 49.56	45,00 45,39		00.00
Max Usage U \$	60 37 86 74 77 36	0, 00 67, 98 73, 73	15.30 34.80	1000		12.21 23.83	43 15 41 55 27 51	37,89 75,06	12.32 46.47 72.46	77,77 91,08 85,80	0,04 1,41 6,70	57,03 2,42 3,62	90.44 89.15 68.40	75.74	65,93 82,46		000
Steel rength (ksi)	36.0 36.0		20.05 0.05 0.05		50.0 26.0		36.0 36.0			36.0 36.0 36.0				50.0	50.0	50.0 50.0	36.0
	75X1 75X0 1875 2X2X0 1875 2 5X2.5X0 1875	2 3X2 3X9 23 3X3X0 25 3 5X3 5X0 25	4X4X0,25 Pipe3EH Pipe3 5EH	Pipe5STD Pipe5STD D: De5EH	Pipe6EH Pipe6EH Pipe8EHS	2X2X0 3125 2X2X0 25	2 5X2 5X0 3125 3X3X0 5 4X4X0 5	4X4X0 625 5X5X0 625	P3-437 P4-494 Pipe5EH	P6-562 Pipe8XS Pipe10XS	0 1X0 1X1 P3-425 P4-494	2X2X0 375 2X2X0 25 2 5X2 5X0 3125	2 5X2 5X0 25 2 5X2 5X0 3125 MOD-P8STD	MOD-P6SCH80 MOD-P6STD	MOD-P5SCH80 MOD-P5STD	2 5X2 5X0 375 2 5X2 5X0 3125 Mod-Pascuer	2X2X0 25
Angle Type	N N N N N N N N N N N N N N N N N N N	N N N N N N N N N N N N N N N N N N N	P P P P P P P P P P P P P P P P P P P	0 0 0 0 0 1 0, 0 0 1 -1 -1 -1 - 1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -	1001 2002 2005 2001 2001 2001 2001 2001	SAE	N N N N N N N N N N N N N N N N N N N	SAE	9 9 9 9 7 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	9 0 0 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	P P P P P P P P P	SAE SAE SAE			සි සි	SAE SAE VOAE	SAE
Group Angle Desc. Type	Rohn Diagonal 1 Rohn Diagonal 2 Rohn Diagonal 3	Diagonal	Konn Diagonal / Rohn leg 1 Rohn leg 2 Rohn leg 3	Rohn Leg 4 Rohn Leg 4 Rohn Leg 5			SNB Diagonal 3 SNB Diagonal 4 SNB Diagonal 5	$\Box \Box$	SNB Leg 1 SNB Leg 2 SNB Leg 3	SNB Leg 4 SNB Leg 5 SNB Leg 5	Connect Towers SNB Horizontal 1 SNB Horizontal 2			ы 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Rohn Leg Rohn Leg	Rohn Diagonal 4b Rohn Diagonal 3b Rohn Len 3a	b rb I CV r
Group Label	Rohn-D1 R Rohn-D2 R Rohn-D3 R		Rohn-LU Rohn-LL Rohn-L2 Rohn-L2	Rohn-L4 Rohn-L5 Rohn-L5			SNB-D4 SNB-D4 SNB-D5		SNB-L1 SNB-L2 SNB-L3	SNB-L4 SNB-L5 SNB-L6	N N N N			Rohn-L7a Rohn-L6a		Rohn-D4b Rol Rohn-D3b Rol Rohn-L3a	in t

*** Maximum Stress Summary for Each Load Case

Summary of Maximum Usages by Load Case:

Load Case Maximum Element Element

Type		
Label	Rohn-DC62 Rohn-DC52	
ф	100	
Usage	90-30 19-40	
	deg deg	
	- 90	
	I+M I+M	

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W+I 0 deg 91.08 SNB-LH2P Angle W+I 60 deg 80.62 Rohn-DB61 Angle W+I 90 deg 90.44 Rohn-DC51 Angle DI + Ice Only 30.56 SNB-DI32 Angle

*** Weight of structure (lbs): Weight of Angles*Section DLF Weight of Equipment: Total:

*** End of Report

CONNECTION BETWEEN TOWERS EVALUATION

o scription	180' ROHN SSV Tower	w/ SNB Reinf Greenwich Project No. Computed by	NSS-036 MCD	Page of Sheet 1 of 1 Date 09/16/15
		Checked by		Date
Bolt Diam	eter	$Dia_{bolt} := \frac{3}{4}in$		
Bolt Shea	r Capacity	Capacity _{bolt} := 5.3kip Per AISC 9th Edit	tion p. 4-5 for 3/4"	A325 slip critical b
Shear Pla	ne Area	Area _{plate} = $\frac{3}{8}$ in $\left(\frac{3}{4}$ in + 1 in \right)	Area _{plate} =	$= 0.6563 \cdot in^2$
Yield Stre	ngth of Plate	Fy _{plate} := 36ksi		
⊃late Cap	acity	Capacity _{plate} := Area _{plate} · Fy _{plate}	Capacitypl	ate = 23.625 kip
J-Bolt Siz	e	$Dia_{ubolt} := \frac{1}{2}in$		
J-Bolt Are	а	Area _{ubolt} := $2 \cdot \pi \left(\frac{\text{Dia}_{\text{ubolt}}}{2} \right)^2$	Area _{ubolt} =	$= 0.3927 \cdot in^2$
		Fy _{ubolt} := 58ksi		
		Capacity _{ubolt} := Area _{ubolt} · Fy _{ubolt}	Capacityub	olt = 22.7765 · kip
Connectio	n Capacity	Capacity _{connection} := min(Capacity _{bolt} , Cap	pacity _{plate} , Capacity	/ubolt)
		$Capacity_{connection} = 5.3 \cdot kip$		
/lax Conn	ection Spacing	Spacing := 5ft		
Connectio	n Capacity per Foot	$Capacity_{LF} := \frac{Capacity_{connection}}{Spacing}$	Capacity _{LF}	$r = 1.06 \cdot \frac{\text{kip}}{\text{ft}}$
/lax Conn	ection Force	F _{max} := 8.329kip		
Connectio	n Spacing in PLS-Tower	Spacing _{PLS.Tower} := 10ft		
Connection	n Force per Foot	Force _{LF} := $\frac{F_{max}}{Spacing_{PLS}.Tower}$	$Force_{LF} =$	0.8329 kip
ercent Ca	apacity	$Percent_{capacity} := \frac{Force_{LF}}{Capacity_{LF}}$	Percentcapa	acity = 78.6.%

FOUNDATION EVALUATION

OM			Page	of
180' ROHN SSV Tower w/ SNB Reinf Greenwich Project No.			Sheet	of
Overturning Moment Calculation	Computed by	MCD	Date	09/17/15
	Checked by		Date	
	30' ROHN SSV Tower w/ SNB Reinf Green	30' ROHN SSV Tower w/ SNB Reinf Greenwich Project No. Overturning Moment Calculation Computed by	30' ROHN SSV Tower w/ SNB Reinf Greenwich Project No. NSS-035 Overturning Moment Calculation Computed by	Page

From PLS-Tower Output Summary

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Summary of Joint Support Reactions For All Load Cases:

Load Case	Joint Label	Long. Force (kips)	Tran. Force (kips)	Vert. Force (kips)	Shear Force (kips)	Tran. Moment (ft-k)	Long. Moment (ft-k)	Vert. Moment (ft-k)	Bending Moment (ft-k)	
W+I -90 deg	RohnJP	7.26	1.84	20.23		-0.13			5.75	0
W+I -90 deg	SNB-JP	15.29	3.77			1.59			10.98	Ő
W+I -90 deg	RohnJ1	4.06	8.5					0	2.62	0
W+I -90 deg	RohnJ2	-11.3	21.08			-7.73			8.91	0
W+I -90 deg	SNB-J1	5.24	12.86	302.63		4.1	-1.37	-0.04	4.32	0
W+I -90 deg	SNB-J2	-20.54	39.3	-263.36		-14.83	-9.41	-0.04	17.56	0
W+I -60 deg	RohnJP	-2.12	1.5	150.5		-0.02		0.01	3.98	0
W+I -60 deg	SNB-JP	-1.03	3.16	179.31	3.33	1.42	7.13	0.07	7.27	0
W+I -60 deg	RohnJ1	-0.11	2.5	150.39	2.51	3.48	-1.93	0	3.98	0
W+I -60 deg	RohnJ2	-13.06	22.47	-240.53	25.99	-8.04	-4.69	0	9.31	0
W+I -60 deg	SNB-J1	-2.22	2.47	179.23	3.32	6.89	-2.33	-0.07	7.27	0
W+I -60 deg	SNB-J2	-23.94	41.48	-299.54	47.89	-15.92	-9.19	0	18.39	0
W+I 0 deg	RohnJP	-13.44	-0.05	304.41	13.44	0.03	1.85	0	1.85	0
W+I0 deg	SNB-JP	-20.96	0	369.26	20.96	0	2.63	0	2.63	0
W+I 0 deg	RohnJ1	-10.3	-14.4	-122	17.7	6.58	-4.03	0	7.71	0
W+I0 deg	RohnJ2	-10.21	14.45	-122.06	17.69	-6.61	-3.97	0	7.71	0
W+I 0 deg	SNB-J1	-19.81	-27.14	-155.11	33.6	13.67	-6.34	-0.07	15.07	0
W+I 0 deg	SNB-J2	-19.81	27.14	-155.15	33.6	-13.67	-6.34	0.07	15.07	0
W+I 60 deg	RohnJP	-2.11	-1.55	150.5	2.62	0.06	3.98	-0.01	3.98	0
W+I 60 deg	SNB-JP	-1.03	-3.16	179.31	3.33	-1.42	7.13	-0.07	7.27	0
W+I 60 deg	RohnJ1	-13.11	-22.44	-240.47	25.99	8.02	-4.72	-0.01	9.31	0
W+I 60 deg	RohnJ2	-0.07	-2.48	150.33	2.48	-3.5	-1.9	0	3.98	0
W+I 60 deg	SNB-J1	-23.94	-41.48	-299.49	47.89	15.92	-9.19	0	18.39	0
W+I 60 deg	SNB-J2	-2.22	-2.47	179.18	3.32	-6.89	-2.33	0.07	7.27	0
W+I 90 deg	RohnJP	7.26	-1.84	20.23	7.49	0.13	5.75	-0.01	5.75	0
W+I 90 deg	SNB-JP	15.29	-3.77	19.73	15.74	-1.59	10.87	-0.08	10.98	0
W+I 90 deg	RohnJ1	-11.3	-21.08	-210.69	23.92	7.73	-4.44	-0.01	8.91	0
W+I 90 deg	RohnJ2	4.06	-8.5	250.8	9.42	-2.32	-1.21	0	2.62	0
W+I 90 deg	SNB-J1	-20.54	-39.3	-263.31	44.35	14.83	-9.4	0.04	17.56	0
W+I 90 deg	SNB-J2	5.24	-12.86	302.58	13.89	-4.1	-1.37	0.04	4.32	0
DL + Ice Only	RohnJP	7.18	0	20.24	7.18	0	5.75	0	5.75	0
DL + Ice Only	SNB-JP	15.22	0	19.71	15.22	0	10.87	0	10.87	0
DL + Ice Only	RohnJ1	-3.59	-6.22	20.13	7.19	4.98	-2.88	0	5.75	0
DL + Ice Only	RohnJ2	-3.59	6.22	20.08	7.19	-4.98	-2.88	0	5.76	0
DL + Ice Only	SNB-J1	-7.61	-13.18	19.62	15.22	9.42	-5.44	0	10.88	0
DL + Ice Only	SNB-J2	-7.61	13.18	19.57	15.22	-9.42	-5.44	0	10.88	0



Page

Sheet

Date

Date

Job Description 180' ROHN SSV Tower w/ SNB Reinf, - Greenwich Project No. Overturning Moment Calculation Computed by

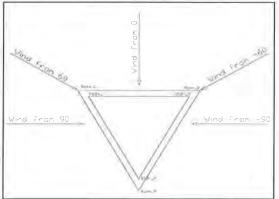
Project No. NSS-035 Computed by MCD Checked by

of	
of	
09/17/15	

Load Case	Joint	Vert.	Leg		Total	Shear	Total
	Label	Tower	Moment	Moment	Moment		Shear
2000		Forces (k)	Arm (ft)	(ft-k)	(ft-k)	(k)	(k)
W+I -90 deg	RohnJP	-0.01	0.00	0.00		7.49	
W+I -90 deg	SNB-JP	0.02	0.00	0.00		15.74	
W+I -90 deg	RohnJ1	230.73	11.42	2634.94	10602.1	9.42	114.8
W+I -90 deg	RohnJ2	-230.82	-11.42	2635.96	10002.1	23.92	114.0
W+I -90 deg	SNB-J1	283.01	9.42	2665.95	0	13.89	
W+I -90 deg	SNB-J2	-282.93	-9.42	2665.20		44.35	
W+I -60 deg	RohnJP	130.26	6.60	859.72		2.59	
W+I -60 deg	SNB-JP	159.6	5.44	868.22		3.33	
W+I -60 deg	RohnJ1	130.26	6.60	859.72	10362.7	2.51	85.6
W+I -60 deg	RohnJ2	-260.61	-13.18	3434.84	10302.7	25.99	
W+I -60 deg	SNB-J1	159.61	5.44	868.28		3.32	
W+I -60 deg	SNB-J2	-319.11	-10.88	3471.92		47.89	
W+I0deg	RohnJP	284.17	13.18	3745.36		13.44	137.0
W+I0 deg	SNB-JP	349.55	10.88	3803.10		20.96	
W+I0 deg	RohnJ1	-142.13	-6.60	938.06	11325.7	17.70	
W+I0 deg	RohnJ2	-142.14	-6.60	938.12	11525.7	17.69	
W+I0 deg	SNB-J1	-174.73	-5.44	950.53		33.60	
W+I0 deg	SNB-J2	-174.72	-5.44	950.48		33.60	
W+I 60 deg	RohnJP	130.26	6.60	859.72		2.62	
W+I 60 deg	SNB-JP	159.6	5.44	868.22		3.33	85.6
W+I 60 deg	RohnJ1	-260.6	-13.18	3434.71	10362.5	25.99	
W+I 60 deg	RohnJ2	130.25	6.60	859.65	10302.5	2.48	65.6
N+I 60 deg	SNB-J1	-319.11	-10.88	3471.92		47.89	
N+I 60 deg	SNB-J2	159.61	5.44	868.28		3.32	
N+I 90 deg	RohnJP	-0.01	0.00	0.00		7.49	
N+I 90 deg	SNB-JP	0.02	0.00	0.00		15.74	
N+I 90 deg	RohnJ1	-230.82	-11.42	2635.96	10601.9	23.92	114.8
N+I 90 deg	RohnJ2	230.72	11.42	2634.82	10001.9	9.42	114.8
N+I 90 deg	SNB-J1	-282.93	-9.42	2665.20		44.35	
N+I 90 deg	SNB-J2	283.01	9.42	2665.95		13.89	

Forces taken from PLS-Tower output with DL + Ice Only load case subtracted from Vertical Tower Forces

Dimensions taken from CAD drawing



AECO	M			Page	of
Job	180' ROHN SSV Tower w/ SNB Reinf Greenwich	Project No.	NSS-036	Sheet	1 of 4
Description	Foundation Analysis	Computed by	MCD	Date	09/17/15
		Checked by		Date	

PIER AND MAT FOUNDATION ANALYSIS - 3 PIERS

TOWER FORCES:

FOOTING DIMENSIONS:

Moment Caused by Tower	$M_t := 11325.7 \cdot kip \cdot ft$	Width of Footing
Shear at Base of Tower	S _t := 137 kip	Overall Depth of Footing
Height of Tower	$H_t := 180 \cdot ft$	Thickness of Footing
Width of Tower at Base	$W_t := 22.833 \cdot ft$	
Weight of Tower	$WT_t := 119.1 \cdot kip$	

MATERIAL PROPERTIES:

Compressive Strength of Concrete	fc:= 3000 psi	Unit Weight of Soil	$\gamma_s := 130 \cdot pcf$
Yield Strength of Steel Reinforcement	fy:= 60000 psi	Unit Weight of Concrete	$\gamma_c := 150 \cdot pcf$
Internal Friction Angle of Soil	$\phi_{s} := 36 \cdot \text{deg}$	Depth to Neglect	$n:=0{\cdot}ft$
Allowable Bearing Capacity	$q_s := 4000 \cdot psf$	Cohesion of Clay Type Soil Note: Use 0 for Sandy Soil	$c = 0 \cdot ksf$
Coefficient of Lateral Soil Pressure	$K_{p} := \frac{1 + \sin(\phi_{s})}{1 - \sin(\phi_{s})}$	$K_{p} = 3.8518$	

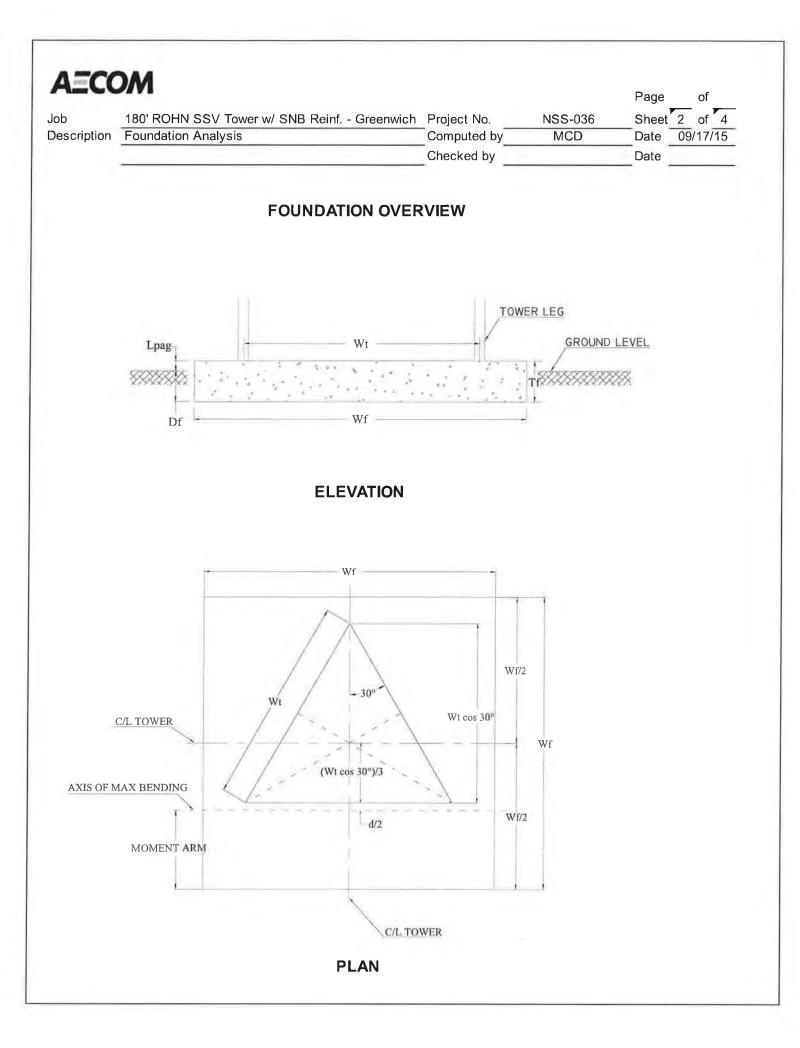
What is Position of Center of Tower with respect to Center of Pad?

1=Offset 2=Not Offset

 $Pos_{tower} := 2$

 $W_f := 36.5 \cdot ft$ $D_f := 3.5ft$

 $T_{f} := 6.0 \text{ ft}$



b escription	180' ROHN SS Foundation Ana	V Tower w/ SNB Reinf Greenwich	Project No.	NSS-036 MCD	Page of Sheet 3 of 4 Date 09/17/15
escription			Checked by	MCD	Date Date
ABILITY C	OF FOOTING				
Factor	of Safety Req'd:	$FS_{req} := 2.0$			
Passive	e Pressure:	$\mathbf{P}_{pn} := \mathbf{K}_{p} \cdot \boldsymbol{\gamma}_{s} \cdot \mathbf{n} + \mathbf{c} \cdot 2 \cdot \sqrt{\mathbf{K}_{p}}$			$P_{pn} = 0 \cdot ksf$
		$\mathbf{P}_{pt} := \mathbf{K}_{p} \cdot \boldsymbol{\gamma}_{s} \cdot \left(\mathbf{D}_{f} - \mathbf{T}_{f} \right) + \mathbf{c} \cdot 2 \cdot \sqrt{\mathbf{K}_{p}}$			$P_{pt} = -1.2518 \cdot ksf$
		$P_{top} := if[n < (D_f - T_f), P_{pt}, P_{pn}]$			$P_{top} = 0 \cdot ksf$
		$P_{bot} := K_p \cdot \gamma_s \cdot D_f + c \cdot 2 \cdot \sqrt{K_p}$			$P_{bot} = 1.7526 \cdot ksf$
		$P_{ave} := \frac{P_{top} + P_{bot}}{2}$			$P_{ave} = 0.8763 \cdot ksf$
Shear:		$T_{pp} := if \left[n < (D_f - T_f), T_f, (D_f - n) \right]$]		$T_{pp} = 3.5 \text{ ft}$
		$A_{pp} := W_{f} T_{pp}$			$A_{pp} = 127.75 \cdot ft^2$
Ultimate	e Shear:	$S_u := P_{ave} A_{pp}$			$S_u = 111.9465 \cdot kip$
Weight Concret		$WT_{c} := \left(W_{f}^{2} \cdot T_{f}\right) \gamma_{c}$			$WT_{c} = 1199.025 \cdot kt$
Weight above F		$WT_{s1} := 0$			$WT_{s1} = 0 \cdot kip$
Weight Wedge	of Soil at back face:	$WT_{s2} := \left\lfloor \frac{\left(D_{f} - n\right)^{2} \cdot tan\left(\Phi_{s}\right)}{2} \cdot W_{f}^{\dagger} \right\rfloor \cdot \gamma_{s}$			WT _{s2} = 21.1156·ki
	e to center of eg from Edge ng:	$X_{t1} := \frac{W_f}{2} - \frac{W_t \cdot \cos(30 \cdot \deg)}{2}$	$x_{t2} := \frac{w_f}{2}$	$\frac{W_{t} \cos(30 \cdot de)}{3}$	g)
		$X_t := if(Pos_{tower} = 1, X_{t1}, X_{t2})$			$X_{t} = 11.6587 \cdot ft$
Addition Footing:	al Offset of	$X_{off1} := \frac{W_f}{2} - \left(\frac{W_f \cos(30 \text{ deg})}{3} + 2\right)$	X_{t} $X_{off2} := 0$		
		$X_{off} := if(Pos_{tower} = 1, X_{off1}, X_{off2})$)		$X_{off} = 0.$ ft
Resistin	g Moment:	$M_{r} := \left(WT_{c} + WT_{s1}\right) \cdot \frac{W_{f}}{2} + WT_{t} \left(\frac{W_{f}}{2}\right)$	$\left(\frac{V_{f}}{2} - X_{off}\right) + S_{u'} \frac{T_{p}}{3}$	$\frac{pp}{p} + WT_{s2} \cdot \left(W_{s2}\right)$	$f + \frac{T_{pp} \cdot tan(\phi_s)}{3} \right)$ $M_r = 24975.003 \cdot kip$
Overturn	ing Moment:	$M_{ot} := M_t + S_t(T_f) + WT_t X_{off}$			$M_{ot} = 12147.7 \cdot kip$
Factor o	f Safety:	$FS := \frac{M_r}{M_{ot}}$			FS = 2.06
		M _{ot} SafetyCheck := if(FS > FS _{req} , "Okay"	"No Good"		SafetyCheck = "Oka

lob	-	SSV Tower w/ SNB Reinf Greenwich		NSS-036	Page ofSheet 4 of _4	
escription	Foundation A	Analysis	Computed by Checked by	MCD	DateDate	
EARING P	RESSURE CH	IECK:				
Pressure	e Applied:	$LOAD_{tot} := WT_c + WT_{s1} + WT_t$		LOA	AD _{tot} = 1318.125 kip	
		$A_{mat} := W_f^2$		A _{ma}	$t_{at} = 1332.25 \cdot ft^2$	
		$\mathbf{x} := \frac{\mathbf{W_f}^3}{6}$		S =	8104.5208· ft ³	
		$P_{max} \coloneqq \frac{LOAD_{tot}}{A_{mat}} + \frac{M_{ot}}{S}$		Pma	_x = 2.4883 · ksf	
		$P_{\min} := \frac{LOAD_{tot}}{A_{mat}} - \frac{M_{ot}}{S}$		P _{mi}	$_{\rm n}$ = -0.5095·ksf	
		MaxPressure := $if(P_{max} < q_s, "Okay")$, "No Good")	Max	Pressure = "Okay"	
		MinPressure := if $\left[\left(P_{\min} \ge 0 \right) \cdot \left(P_{\min} < 0 \right) \right]$	(q _s), "Okay", "No G	ood"] Min	Pressure = "No Good"	
Distance to	Resultant of F	Pressure Distribution:				
		$X_{p} := \frac{\frac{P_{max}}{P_{max} - P_{min}}}{\frac{W_{f}}{W_{f}}} \cdot \frac{1}{3}$		X _p =	= 10.0989· ft	
Distance to	Kern:	$X_k := \frac{W_f}{3}$		X _k =	= 12.1667· ft	
Since Resu	Itant Force is	Not in Kern, Area to which Pressure is	Applied Must be F	Reduced.		
Eccentricity	/:	$e = \frac{M_{ot}}{LOAD_{tot}}$		e = 9	9.2159	
Adjusted So	oil Pressure:	$q_a := \frac{2 \cdot \text{LOAD}_{\text{tot}}}{3 \cdot W_{f'} \left(\frac{W_f}{2} - e\right)}$			q _a = 2.6649·ksf	
Revised M	laximum:	$q_{max} := if(X_p < X_k, q_a, P_{max})$		q _{ma}	_x = 2.6649 kip	
		PressureCheck := $if(q_{max} < q_s, "Okay")$	", "No Good")	Pressu	reCheck = "Okay"	

About AECOM

AECOM (NYSE: ACM) is a global provider of professional technical and management support services to a broad range of markets, including transportation, facilities, environmental, energy, water and government. With approximately 45,000 employees around the world, AECOM is a leader in all of the key markets that it serves. AECOM provides a blend of global reach, local knowledge, innovation, and collaborative technical excellence in delivering solutions that enhance and sustain the world's built, natural, and social environments. A Fortune 500 company, AECOM serves clients in more than 100 countries and has annual revenue in excess of \$6 billion.

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Exhibit C



RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CT11070B

Connecticut State Police #2 150 Butternut Hollow Road Greenwich, CT 06830

March 18, 2015

EBI Project Number: 6215001497

Site Compliance Summary				
Compliance Status:	COMPLIANT			
Site total MPE% of				
FCC general public allowable limit:	70.20 %			



March 18, 2015

T-Mobile USA Attn: Jason Overbey, RF Manager 35 Griffin Road South Bloomfield, CT 06002

Emissions Analysis for Site: CT11070B - Connecticut State Police #2

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **150 Butternut Hollow Road, Greenwich, CT**, for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter (μ W/cm2). The number of μ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

<u>General population/uncontrolled exposure</u> limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter (μ W/cm²). The general population exposure limit for the 700 MHz Band is 467 μ W/cm², and the general population exposure limit for the PCS and AWS bands is 1000 μ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



<u>Occupational/controlled exposure</u> limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over their exposure and can exercise control over the potential for exposure and can exercise through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at **150 Butternut Hollow Road, Greenwich, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 GSM channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel
- 2) 2 UMTS channels (AWS Band 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 3) 2 LTE channels (AWS Band 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 4) 1 LTE channel (700 MHz Band) was considered for each sector of the proposed installation. This channel has a transmit power of 30 Watts.
- 5) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.



- 6) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antennas used in this modeling are the Ericsson AIR21 B2A/B4P for 1900 MHz (PCS) and 2100 MHz (AWS) channels and the Ericsson AIR21 B4A/B12P for 2100 MHz (AWS) and 700 MHz channels. This is based on feedback from the carrier with regards to anticipated antenna selection. The Ericsson AIR21 B2A/B4P has a maximum gain of 15.9 dBd at its main lobe for both 1900 MHz and 2100 MHz. The Ericsson AIR21 B4A/B12P has a maximum gain of 15.9 dBd at its main lobe for 2100 MHz. The Ericsson AIR21 B4A/B12P has a maximum gain of 15.9 dBd at its main lobe for 2100 MHz and a maximum gain of 13.6 dBd at its main lobe for 700 MHz. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antenna mounting height centerline of the proposed antennas is **137 feet** above ground level (AGL).
- 9) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general public threshold limits.



T-Mobile Site Inventory and Power Data

a		~	~	~ · ·	~
Sector:	A	Sector:	В	Sector:	С
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	Ericsson AIR21	Make / Model:	Ericsson AIR21	Make / Model:	Ericsson AIR21
Ivrake / Ivrouer.	B2A/B4P	Make / Model.	B2A/B4P	wiake / wiodei.	B2A/B4P
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	137	Height (AGL):	137	Height (AGL):	137
Eraguanay Danda	1900 MHz(PCS) /	Fraguanay Danda	1900 MHz(PCS) /	Fraguanay Danda	1900 MHz(PCS) /
Frequency Bands	2100 MHz (AWS)	Frequency Bands	2100 MHz (AWS)	Frequency Bands	2100 MHz (AWS)
Channel Count	4	Channel Count	4	# PCS Channels:	4
Total TX Power:	120	Total TX Power:	120	# AWS Channels:	120
ERP (W):	4,668.54	ERP (W):	4,668.54	ERP (W):	4,668.54
Antenna A1 MPE%	0.98	Antenna B1 MPE%	0.98	Antenna C1 MPE%	0.98
Antenna #:	2	Antenna #:	2	Antenna #:	2
	Ericsson AIR21		Ericsson AIR21		Ericsson AIR21
Make / Model:	B4A/B12P	Make / Model:	B4A/B12P	Make / Model:	B4A/B12P
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	137	Height (AGL):	137	Height (AGL):	137
En an Dan da	2100 MHz(AWS) /	Ensame Dan da	2100 MHz(AWS) /	En an Dan da	2100 MHz(AWS) /
Frequency Bands	700 MHz	Frequency Bands	700 MHz	Frequency Bands	700 MHz
Channel Count	3	Channel Count	3	Channel Count	3
Total TX Power:	150	Total TX Power:	150	Total TX Power:	150
ERP (W):	5,355.80	ERP (W):	5,355.80	ERP (W):	5,355.80
Antenna A2 MPE%	1.29	Antenna B2 MPE%	1.29	Antenna C2 MPE%	1.29

Site Composite MPE%				
Carrier	MPE%			
T-Mobile	6.79			
Nextel	4.60 %			
State Police	3.91 %			
Greenwich	2.42 %			
DOT	0.55 %			
NU	9.38 %			
Sprint	5.45 %			
AT&T	13.06 %			
Verizon Wireless	24.04 %			
Site Total MPE %:	70.20 %			

T-Mobile Sector 1 Total:	2.26 %	
T-Mobile Sector 2 Total:	2.26 %	
T-Mobile Sector 3 Total:	2.26 %	
Site Total:	70.20 %	



Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general public exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general public exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector 1:	2.26 %
Sector 2:	2.26 %
Sector 3 :	2.26 %
T-Mobile Total:	6.79 %
Site Total:	70.20 %
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **70.20%** of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

Scott Heffernan RF Engineering Director

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