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KENNETH C. BALDWIN

280 Trumbull Street Hartford, CT 06103-3597 Main (860) 275-8200 Fax (860) 275-8299 kbaldwin@rc.com Direct (860) 275-8345

Also admitted in Massachusetts

July 25, 2013

Melanie A. Bachman Acting Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

Re: Notice of Exempt Modification – Facility Modification 150 Lost Acres Road, Granby, Connecticut

Dear Ms. Bachman:

Cellco Partnership d/b/a Verizon Wireless ("Cellco") currently maintains twelve (12) wireless telecommunications antennas at the 160-foot level on an existing 170-foot tower at the above-referenced address. The tower is owned by SBA. Cellco's use of the tower was approved by the Council in 2008. Cellco now intends to replace two (2) of its existing antennas with two (2) model BXA-70063-6CF LTE antennas, at the same 160-foot level. Attached behind <u>Tab 1</u> are the specifications for Cellco's replacement antennas.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to William F. Smith, Jr., Town Manager of the Town of Granby. A copy of this letter is also being sent to John Lombardi, the owners of the property on which the tower is located.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing tower. Cellco's replacement antennas will be located at the 160-foot level on the existing 170-foot tower.



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Melanie A. Bachman July 25, 2013 Page 2

- 2. The proposed modifications will not involve any change to ground-mounted equipment and, therefore, will not require the extension of the site boundaries.
- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more.
- 4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to levels at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative General Power Density table for Cellco's modified facility is included behind <u>Tab 2</u>.

Also attached is a Structural Analysis confirming that the tower and foundation can support Cellco's proposed modifications. (See <u>Tab 3</u>). Please note, under the recommendations section on page 3 of the Structural Analysis, Cellco does not propose any modification to the coax cable location or configuration. The cables that exist today are installed as shown in "Figure 1".

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

Kenneth C. Baldwin

Enclosures Copy to:

William F. Smith, Jr., Granby Town Manger John Lombardi Sandy M. Carter



TAB 1



BXA-70063-6CF-EDIN-X

X-Pol | FET Panel | 63° | 14.5 dBd

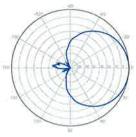
Electrical Characteristics 696-900 MHz 806-900 MHz Frequency bands 696-806 MHz Polarization ±45° Horizontal beamwidth 65° 63° Vertical beamwidth 13° 11° 14,0 dBd (16,1 dBi) 14.5 dBd (16.6 dBi) Gain Electrical downtilt (X) 0, 2, 3, 4, 5, 6, 8, 10 Impedance 50Ω VSWR ≤1.35:1 Upper sidelobe suppression (0°) -18.3 dB -18.2 dB Front-to-back ratio (+/-30°) -33.4 dB -36.3 dB Null fill 5% (-26.02 dB) < -25 dB Isolation between ports Input power with EDIN connectors 500 W Input power with NE connectors 300 W Lightning protection Direct Ground 2 Ports / EDIN or NE / Female / Center (Back) Connector(s) Mechanical Characteristics Dimensions Length x Width x Depth 1804 x 285 x 132 mm 71.0 x 11.2 x 5.2 in Depth with z-brackets 172 mm 6.8 in Weight without mounting brackets 7,9 kg 17 lbs Survival wind speed > 201 km/hr > 125 mph Side: 0.24 m² Wind area Front: 0.51 m² Front: 5.5 ft2 Side: 2.6 ft2 Front: 759 N Side: 391 N Front: 169 lbf Side: 89 lbf Wind load @ 161 km/hr (100 mph) **Mounting Options** Part Number Fits Pipe Diameter 3-Point Mounting & Downtilt Bracket Kit 36210008 40-115 mm 1.57-4.5 in 6.9 kg For concealment configurations, order BXA-70063-6CF-EDIN-X-FP Concealment Configurations

Replace 'X' with desired electrical downtilt.

Antenna is also available with NE connector(s). Replace "EDIN" with "NE" in the model number when ordering.



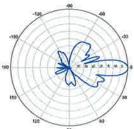
BXA-70063-6CF-EDIN-X



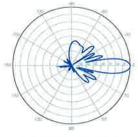
Horizontal | 750 MHz



BXA-70063-6CF-EDIN-0

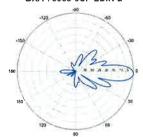


0° | Vertical | 750 MHz

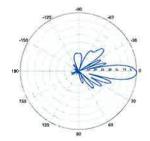


0° | Vertical | 850 MHz

BXA-70063-6CF-EDIN-2



2° | Vertical | 750 MHz



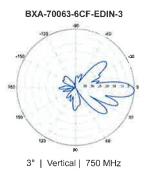
2° | Vertical | 850 MHz

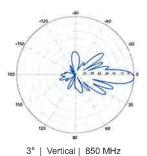
Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.

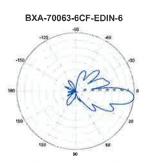


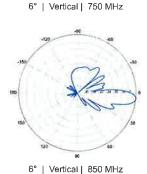
BXA-70063-6CF-EDIN-X

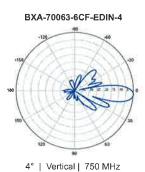
X-Pol | FET Panel | 63° | 14.5 dBd

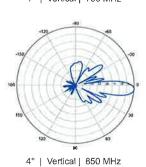


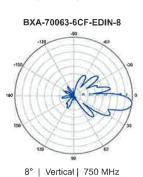


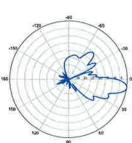






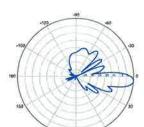




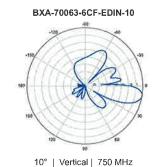


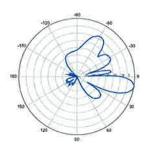












10° | Vertical | 850 MHz

TAB 2

TAB 3



FDH Engineering, Inc., 6521 Meridien Drive Raleigh, NC 27616, Ph. 919.755.1012

Structural Analysis for SBA Network Services, Inc.

170' Self-Support Tower

SBA Site Name: North Granby SBA Site ID: CT10017-A-02 Verizon Site Name: North Granby, CT

FDH Project Number 1340051400

Analysis Results

7 mary or 1 to ano					
Tower Components	87.4%	Sufficient			
Foundation	84.7%	Sufficient			

Prepared By:

Christopher B. Stryffeler, El Project Engineer Reviewed By: Christopher M. Murphy

> Christopher M Murphy, PE President CT PE License No. 25842

FDH Engineering, Inc. 6521 Meridien Drive Raleigh, NC 27616 (919) 755-1012 info@fdh-inc.com



Revision Date: 07/05/11

June 21, 2013

Prepared pursuant to TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures and the 2005 Connecticut State Building Code

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EXECUTIVE SUMMARY

At the request of SBA Network Services, Inc., FDH Engineering, Inc. performed a structural analysis of the existing self-supported tower located in North Granby, CT to determine whether the tower is structurally adequate to support both the existing and proposed loads pursuant to the *Structural Standards for Steel Antenna Towers and Antenna Supporting Structures, TIA/EIA-222-F* and the 2005 Connecticut State Building Code (CSBC). Information pertaining to the existing/proposed antenna loading, current tower geometry, the member sizes, and foundation dimensions was obtained from:

Rohn (Eng. File No. 37696MP) original design drawings dated August 3, 1998
FDH, Inc. (Job No. 07-0320T) TIA Inspection Report dated April 11, 2007
FDH Engineering, Inc. (09-07094E S2) Extension Drawings for 160' Self Support Tower dated October 23, 2009
FDH, Inc. (Job No. 09-10153T T1) TIA Inspection Report dated December 29, 2009
FDH Engineering, Inc. (09-07094E S2) Post-Construction Inspection Report dated January 6, 2010
SBA Network Services, Inc.

The basic design wind speed per the TIA/EIA-222-F standards and the 2005 CSBC is 80 mph without ice and 38 mph with 1" radial ice. Ice is considered to increase in thickness with height.

Conclusions

With the existing and proposed antennas from Verizon in place at 160 ft, the tower meets the requirements of the *TIA/EIA-222-F* standards and the *2005 CSBC* provided the **Recommendation** listed below is satisfied. Furthermore, provided the foundations were designed and constructed to support the original design reactions (See Rohn Eng. File No. 37696MP), the foundations should have the necessary capacity to support the existing and proposed loading. For a more detailed description of the analysis of the tower, see the **Results** section of this report.

Our structural analysis has been performed assuming all information provided to FDH Engineering, Inc. is accurate (i.e., the steel data, tower layout, existing antenna loading, and proposed antenna loading) and that the tower has been properly erected and maintained per the original design drawings.

Recommendation

To ensure the requirements of the *TIA/EIA-222-F* standards and the *2005 CSBC* are met with the existing and proposed loading in place, we have the following recommendation:

1. Coax lines must be installed as shown in Figure 1.

Document No. ENG-RPT-502S

APPURTENANCE LISTING

The proposed and existing antennas with their corresponding cables/coax lines are shown in Table 1. If the actual layout determined in the field deviates from the layout, FDH Engineering, Inc. should be contacted to perform a revised analysis.

Table 1 - Appurtenance Loading

Existing Loading:

Antenna Elevation (ft)	Description	Coax and Lines ¹	Carrier	Mount Elevation (ft)	Mount Type
170	(6) Powerwave 7770.00 (3) KMW AM-X-CD-17-65-001-RET (6) Powerwave LGP21401 TMAs (6) Ericsson RRUS11 RRUs (1) Andrew ABT-DF-DMADBH Surge Arrestor (1) Raycap DC6-48-60-18-8F Surge Arrestor	(12) 1-5/8" (1) 7/16" Fiber ² (2) DC Cables ²	AT&T	170	(3) T-Frames
160.5	(6) ADC Cleargain TMAs	<u> </u>			
160	(2) Antel BXA-70063/4CF (3) Antel BXA-171085-12BF (4) Antel LPA-80063/4CF (1) Antel BXA-70063/6CF (2) Antel LPA-80080/6CF (6) RFS FD9R6004/2C-3L Diplexers	(12) 1-5/8"	Verizon	159.1	(3) 14.5' Andrew T-Frames (QT-SF14-4-72)

Proposed Loading:

Antenna Elevation (ft)	Description	Coax and Lines	Carrier	Mount Elevation (ft)	Mount Type
160	(1) Antel BXA-70063/4CF (3) Antel BXA-171085-12BF (4) Antel LPA-80063/4CF (2) Antel BXA-70063/6CF (2) Antel LPA-80080/6CF (6) RFS FD9R6004/2C-3L Diplexers	(12) 1-5/8"	Verizon	159.1	(3) 14.5' Andrew T-Frames (QT-SF14-4-72)

See Figure 1 for coax location.
 AT&T has installed (1) 7/16" fiber and (2) DC cables inside (1) 3" flex conduit.

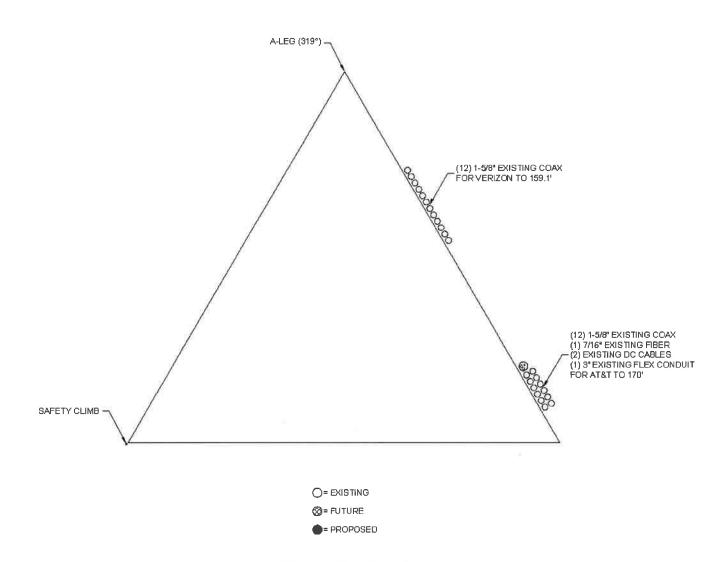


Figure 1 – Coax Layout

RESULTS

The following yield strength of steel for individual members was used for analysis:

Table 2 - Material Strength

Member Type	Yield Strength
Legs	50 ksi
Bracing	50 ksi & 36 ksi

Table 3 displays the summary of the ratio (as a percentage) of force in the member to their capacities. Values greater than 100% indicate locations where the maximum force in the member exceeds its capacity. *Note: Capacities up to 105% are considered acceptable.* **Table 4** displays the maximum foundation reactions.

If the assumptions outlined in this report differ from actual field conditions, FDH Engineering, Inc. should be contacted to perform a revised analysis. Furthermore, as no information pertaining to the allowable twist and sway requirements for the existing or proposed appurtenances was provided, deflection and rotation were not taken into consideration when performing this analysis.

See the **Appendix** for detailed modeling information.

Table 3 - Summary of Working Percentage of Structural Components

Section No.	Elevation ft	Component Type	Size	% Capacit y ⁺	Pass Fail
T1	170 - 160	Leg	ROHN 2.5 STD	10.6	Pass
		Diagonal	L1 3/4x1 3/4x3/16	18.0 29.4 (b)	Pass
		Top Girt	L2x2x3/16	12.2	Pass
T2	160 - 140	Leg	ROHN 2.5 STD	66.2	Pass
		Diagonal	L1 3/4x1 3/4x3/16	55.9 81.9 (b)	Pass
		Top Girt	L2x2x3/16	5.9	Pass
T3	140 - 120	Leg	ROHN 3 STD	87.4	Pass
		Diagonal	L2x2x3/16	54.9 60.4 (b)	Pass
		Top Girt	L2x2x3/16	2.9	Pass
T4	120 - 100	Leg	ROHN 3.5 EH	76.9	Pass
		Diagonal	L2 1/2x2 1/2x3/16	54.1 65.1 (b)	Pass
T5	100 - 80	Leg	ROHN 4 EH	77.1	Pass
		Diagonal	L2 1/2x2 1/2x3/16	78.2	Pass
T6	80 - 60	Leg	ROHN 5 EH	62.9	Pass
		Diagonal	L3x3x1/4	48.1 64.8 (b)	Pass
T7	60 - 40	Leg	ROHN 6 EHS	70.6	Pass
		Diagonal	L3 1/2x3 1/2x1/4	52.5 54.2 (b)	Pass
T8	40 - 20	Leg	ROHN 6 EH	64.9	Pass
		Diagonal	L3 1/2x3 1/2x1/4	67.0	Pass
T9	20 - 0	Leg	ROHN 6 EH	73.0	Pass
		Diagonal	L3 1/2x3 1/2x1/4	86.0	Pass

^{*} Capacities include 1/3 allowable stress increase for wind per the TIA/EIA-222-F standards.

Table 4 - Maximum Base Reactions

Load Type	Direction	Current Analysis (TIA/EIA-222-F)	Original Design (TIA/EIA-222-F)
Individual Foundation	Horizontal	21 k	29 k
	Uplift	175 k	214 k
	Compression	198 k	240 k
Overturning Moment		3,432 k-ft	4,053 k

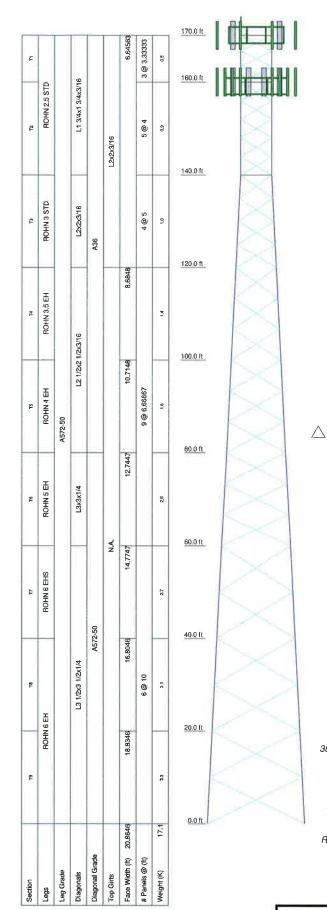
GENERAL COMMENTS

This engineering analysis is based upon the theoretical capacity of the structure. It is not a condition assessment of the tower and its foundation. It is the responsibility of SBA Network Services, Inc. to verify that the tower modeled and analyzed is the correct structure (with accurate antenna loading information) modeled. If there are substantial modifications to be made or the assumptions made in this analysis are not accurate, FDH Engineering, Inc. should be notified immediately to perform a revised analysis.

LIMITATIONS

All opinions and conclusions are considered accurate to a reasonable degree of engineering certainty based upon the evidence available at the time of this report. All opinions and conclusions are subject to revision based upon receipt of new or additional/updated information. All services are provided exercising a level of care and diligence equivalent to the standard and care of our profession. No other warranty or guarantee, expressed or implied, is offered. Our services are confidential in nature and we will not release this report to any other party without the client's consent. The use of this engineering work is limited to the express purpose for which it was commissioned and it may not be reused, copied, or distributed for any other purpose without the written consent of FDH Engineering, Inc.

APPENDIX



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
Lightning Rod	170	(2) RRUS-11	170
(2) 7770 00 w/ mount pipe	170	(2) RRUS-11	170
(2) 7770 00 w/ mount pipe	170	ABT-DFDM-ADBH	170
(2) 7770 00 w/ mount pipe	170	DC6-48-60-18-8F Surge Arrestor	170
AM-X-CD-17-65-001-RET w/ Mount	170	(3) T-Frames MNT	170
Pipe		BXA-70063/4CF W/ Mount Pipe	159.1
AM-X-CD-17-65-001-RET w/ Mount	170	(3) BXA-171085-12BF w/Mount Pipe	159.1
Pipe		(4) LPA-80063/4CF w/ Mount Pipe	159.1
AM-X-CD-17-65-001-RET w/ Mount Pipe	170	(2) BXA-70063/6CF W/Mount Pipe	159,1
(2) LGP21401 TMA	170	(2) LPA-80080/6CF w/ mount pipe	159.1
(2) LGP21401 TMA	170	(2) FD9R6004/2C-3CL Diplexer	159.1
'		(2) FD9R6004/2C-3CL Diplexer	159.1
(2) LGP21401 TMA	170	(2) FD9R6004/2C-3CL Diplexer	159.1
(2) RRUS-11	170	(3) 14.5' Andrew T-Frames MNT	159.1

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu	
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi	

TOWER DESIGN NOTES

- 1. Tower is located in Hartford County, Connecticut.
- Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
- Tower is also designed for a 38 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
- 4. Deflections are based upon a 50 mph wind.5. TOWER RATING: 87.4%

MAX. CORNER REACTIONS AT BASE: DOWN: 198 K SHEAR: 21 K UPLIFT: -175 K SHEAR: 19 K AXIAL 65 K SHEARY **MOMENT** 13 K / 1355 kip-ft TORQUE 9 kip-ft 38 mph WIND - 1.0000 in ICE AXIAL 26 K SHEAR MOMENT 3432 kip-ft TORQUE 35 kip-ft REACTIONS - 80 mph WIND

FDH Tower Analysis

FDH Engineering, Inc.

6521 Meridien Drive Raleigh, NC 27616 Phone: (919) 755 1012 FAX: (919) 755 1031

North Granby, CT1001	7-A-02	
Project: 1340051400	9	
Client: SBA Network Services, Inc.	Drawn by: Christopher B. Stryffeler, El	App d:
		Scale: NTS
Path:		Dwg No E-1