

KENNETH C. BALDWIN

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Also admitted in Massachusetts
and New York

June 3, 2021

Via Electronic Mail

Melanie A. Bachman, Esq.
Executive Director/Staff Attorney
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Re: **Notice of Exempt Modification – Facility Modification
2577 Main Street, Glastonbury, Connecticut**

Dear Attorney Bachman:

Cellco Partnership d/b/a Verizon Wireless (“Cellco”) currently maintains an existing wireless telecommunications facility at the above-referenced property address (the “Property”). The facility consists of antennas and remote radio heads attached to a tower and related equipment on the ground, near the base of the tower. The tower was approved by the Town of Glastonbury (“Town”) in 1999. Cellco’s shared use of the tower was approved by the Council in February of 2014 (TS-VER-054-140117). A copy of the Town’s tower approval and Council’s Tower Share approval are included in [Attachment 1](#).

Cellco now intends to modify its facility by replacing nine (9) existing antennas with three (3) Samsung MT6407-77A antennas and six (6) NHH-65B-R2B antennas and replacing nine (9) remote radio heads (“RRHs”) with six (6) newer model RRHs on Cellco’s existing antenna mounting structure. A set of project plans showing Cellco’s proposed facility modifications and new antennas and RRHs specifications are included in [Attachment 2](#).

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Glastonbury’s Town Manager and Land Use Officer.

Melanie A. Bachman, Esq.
June 3, 2021
Page 2

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing tower. Cellco's replacement antennas and RRHs will be installed on Cellco's existing antenna mounting structure.

2. The proposed modifications will not involve any change to ground-mounted equipment and, therefore, will not require the extension of the site boundary.

3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.

4. The installation of Cellco's new antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) safety standard. A cumulative General Power Density table for the modified facility is included in Attachment 3. The modified facility will be capable of providing Cellco's 5G wireless service.

5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.

6. According to the attached Structural Analysis ("SA") and Mount Analysis ("MA"), the existing tower, tower foundation, tower base plate and antenna mounting support structure, with certain modifications, can support Cellco's proposed modifications. Copies of the SA and MA are included in Attachment 4. Also included in Attachment 4 is a separate letter prepared by the consulting engineer responsible for the preparation of the MA verifying that the antenna model described in the MA, as a nL-Sub6 Antenna or L-Sub6 Antenna, is the Samsung 64T64R model antenna and RRH that will be installed on the tower.

A copy of the parcel map and Property owner information is included in Attachment 5. A Certificate of Mailing verifying that this filing was sent to municipal officials is included in Attachment 6.

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Melanie A. Bachman, Esq.
June 3, 2021
Page 3

Sincerely,

A handwritten signature in black ink, appearing to read "Kenneth C. Baldwin". The signature is fluid and cursive, with a long horizontal stroke at the end.

Kenneth C. Baldwin

Enclosures

Copy to:

Richard J. Johnson, Glastonbury Town Manager
Rebecca Augur, Director of Planning and Land Use Services
Saint Isidore and Maria Parish Corporation
Aleksey Tyurin

ATTACHMENT 1



STATE OF CONNECTICUT
CONNECTICUT SITING COUNCIL

Ten Franklin Square
New Britain, Connecticut 06051
Phone: (860) 827-2935
Fax: (860) 827-2950

*Glastonbury -
St Paul's R.C.C.*

September 3, 1999

Ronald C. Clark
Manager Real Estate Operations
Nextel Communications
100 Corporate Park
Rocky Hill, CT 06067

RE: TS-NEXTEL-054-990805 - Nextel Communications request for an order to approve tower sharing at a telecommunications tower to be replaced at the St. Paul's Roman Catholic Church on 2577 Main Street in Glastonbury, Connecticut.

Dear Mr. Clark:

At a public meeting held August 31, 1999, the Connecticut Siting Council (Council) ruled that the shared use of this existing tower site is technically, legally, environmentally, and economically feasible and meets public safety concerns, and therefore, in compliance with General Statutes § 16-50aa, the Council has ordered the shared use of this facility to avoid the unnecessary proliferation of tower structures.

This facility has been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequency now used on this tower. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Any deviation from this format may result in the Council implementing enforcement proceedings pursuant to General Statutes § 16-50u including, without limitation, imposition of expenses resulting from such failure and of civil penalties in an amount not less than one thousand dollars per day for each day of construction or operation in material violation.

This decision applies only to this request for tower sharing and is not applicable to any other request or construction.

The proposed shared use is to be implemented as specified in your letter dated August 4, 1999 and received by the Council on August 5, 1999. Please notify the Council when all work is complete.

Very truly yours,

Mortimer A. Gelston

Mortimer A. Gelston
Chairman

MAG/RKE/sll

cc: Richard J. Johnson, Town Manager, Town of Glastonbury



Town of Glastonbury

2155 MAIN STREET • P.O. BOX 6523 • GLASTONBURY, CONNECTICUT 06033-6523

July 18, 2000

Attorney Anthony B. Gioffre III
Cuddy & Feder & Worby LLP
90 Maple Avenue
White Plains, New York 10601-5196

RE: Approved Wetlands Regulated Activity at 2577 Main Street, Glastonbury, Connecticut

Dear Mr. Gioffre:

Pursuant to Section 4.(C).(2) of Public Act 96-157 you are hereby approved to conduct your activities of constructing a wireless telecommunications facility (including a 130' tower) within the wetlands' conservation buffer area at 2577 Main Street, as represented by the plans, reports and correspondence submitted to our office. This approval is contingent upon your responsibility:

1. to arrange and hold a preconstruction meeting at the site, with the site contractor, a Nextel representative and myself in attendance, in order to discuss the environmental safeguards to be taken during construction;
2. to stabilize the disturbed land areas with vegetation (e.g. grasses, legumes, herbaceous plants and shrubs) that provides conservation benefits regarding the adjacent wetlands;
3. to publish this approval (one time) in a newspaper that circulates in Glastonbury within 10 days of the date of this approval letter to you; and
4. to await a 15-day appeal period (15 days from the date of newspaper publication) before beginning the work within the buffer.

Failure to comply with these four requirements will automatically nullify this approval and conducting your activity would constitute a wetlands violation.

Attached for your consideration and potential use is a draft public notice advertisement that needs to be published once by you in a newspaper which circulates in Glastonbury.

Please call me at 652-7514 to advise me when the notice is to appear and in what newspaper, or if you have any questions.

Sincerely,

Tom Moeko, Environmental Planner

Enclosure

wctaglap.wpd

**PUBLIC NOTICE
TOWN OF GLASTONBURY, CT**

On July 18, 2000, the duly authorized agent of the Glastonbury Inland Wetlands and Watercourses Agency approved a wireless telecommunications facility, including a 130-foot replacement tower, within the wetlands' conservation buffer area at 2577 Main Street, west of St. Paul's Church.

Anthony B. Gioffre III, Legal Counsel



STATE OF CONNECTICUT

CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: siting.council@ct.gov

www.ct.gov/csc

February 11, 2014

Kenneth C. Baldwin, Esq.
Robinson & Cole LLP
280 Trumbull Street
Hartford, CT 06103

RE: **TS-VER-054-140117** - Cellco Partnership d/b/a Verizon Wireless request for an order to approve tower sharing at an existing telecommunications facility located at 2577 Main Street, Glastonbury, Connecticut.

Dear Attorney Baldwin:

At a public meeting held February 6, 2014, the Connecticut Siting Council (Council) ruled that the shared use of this existing tower site is technically, legally, environmentally, and economically feasible and meets public safety concerns, and therefore, in compliance with General Statutes § 16-50aa, the Council has ordered the shared use of this facility to avoid the unnecessary proliferation of tower structures with the following conditions:

- The coax shall be installed in accordance with the Structural Analysis Report prepared by FDH Engineering, Inc. dated September 24, 2013;
- Prior to antenna installation, the tower modifications listed in FDH Engineering Inc. (Project No. 1338401400) drawings dated June 17, 2013 and (Project No. 13SB5C1400) drawings dated September 10, 2013 prepared by FDH Engineering Inc. dated September 24, 2013, and stamped by Bradley Newman, shall be implemented;
- Within 45 days following completion of the antenna installation, Cellco shall provide documentation certified by a professional engineer that its installation complied with the requirements of the structural analysis;
- Furthermore, Cellco shall submit to the Council a Radio Frequency Exposure Report with field measurements taken in the vicinity of this facility within 60 days after the installation has been completed;
- Any deviation from the proposed installation as specified in the original tower share request and supporting materials with the Council shall render this decision invalid;
- Any material changes to the proposed installation as specified in the original tower share request and supporting materials filed with the Council shall require an explicit request for modification to the Council pursuant to Connecticut General Statutes § 16-50aa, including all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65;
- Not less than 45 days after completion of the proposed installation, the Council shall be notified in writing that the installation has been completed;
- The validity of this action shall expire one year from the date of this letter; and



- The applicant may file a request for an extension of time beyond the one year deadline provided that such request is submitted to the Council not less than 60 days prior to the expiration.

This decision is under the exclusive jurisdiction of the Council. This facility has been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower. Any deviation from this format may result in the Council implementing enforcement proceedings pursuant to General Statutes § 16-50u including, without limitation, imposition of expenses resulting from such failure and of civil penalties in an amount not less than one thousand dollars per day for each day of construction or operation in material violation.

This decision applies only to this request for tower sharing and is not applicable to any other request or construction. Please be advised that the validity of this action shall expire one year from the date of this letter.

The proposed shared use is to be implemented as specified in your letter dated January 17, 2014, including the placement of all necessary equipment and shelters within the tower compound.

Thank you for your attention and cooperation.

Very truly yours,

Handwritten signature of Robert Stein in cursive, with the initials "RMB" written in the upper right corner of the signature.

Robert Stein
Chairman

RS/MP/jb

- c: The Honorable Stewart Beckett III, Chairman, Town of Glastonbury
Richard J. Johnson, Town Manager, Town of Glastonbury
Kenith Leslie, Community Development Director, Town of Glastonbury
Sean Gormley, SBA

ATTACHMENT 2

verizon

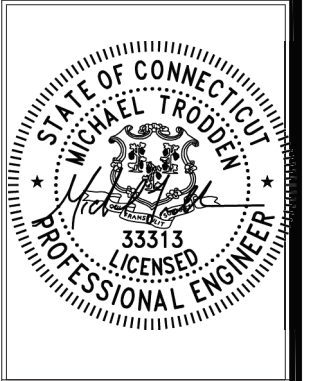
WIRELESS COMMUNICATIONS FACILITY

GLASTONBURY WEST CT 2577 MAIN STREET GLASTONBURY CT, 06033

Cellco Partnership d/b/a
verizon
20 ALEXANDER DRIVE
WALLINGFORD, CT 06492

ALL-POINTS
TECHNOLOGY CORPORATION
567 VAUXHALL STREET EXTENSION - SUITE 311
WATERFORD, CT 06385 PHONE: (860) 663-1697
WWW.ALLPOINTS TECH.COM FAX: (860) 663-0935

CONSTRUCTION DOCUMENTS		
NO	DATE	REVISION
0	02/19/21	FOR REVIEW: JRM
1	05/20/21	FOR FILING: JRM
2		
3		
4		
5		
6		



DESIGN PROFESSIONALS OF RECORD
PROF: MICHAEL S. TRODDEN P.E.
 COMP: ALL-POINTS TECHNOLOGY CORPORATION, P.C.
 ADD: 567 VAUXHALL STREET EXT. SUITE 311 WATERFORD, CT 06385
OWNER: SAINT ISIDORE AND MARIA ADDRESS: PARISH CORP 2577 MAIN STREET GLASTONBURY, CT 06033

GLASTONBURY WEST CT
 SITE 2577 MAIN STREET
 ADDRESS: GLASTONBURY CT, 06033
 APT FILING NUMBER: CT141_11930
 DRAWN BY: DRA
 DATE: 02/19/21 CHECKED BY: JRM
 VZ PROJECT CODE: 20202199190
 VZ LOCATION CODE: 468258
 VZ FUZE ID: 16232043

SHEET TITLE:
TITLE SHEET
SHEET NUMBER:
T-1

SITE DIRECTIONS

**START: 20 ALEXANDER DRIVE
WALLINGFORD, CONNECTICUT 06492**

**END: 2577 MAIN STREET
GLASTONBURY CT, 06033**

1. HEAD SOUTH TOWARD ALEXANDER DRIVE 279 FT
2. SLIGHT RIGHT TOWARDS ALEXANDER DRIVE 289 FT
3. TURN RIGHT TOWARD ALEXANDER DRIVE 167 FT
4. TURN RIGHT ONTO ALEXANDER DRIVE 0.3 MI
5. TURN RIGHT ONTO BARNES INDUSTRIAL ROAD S. 0.1 MI
6. TURN RIGHT ONTO CT-68 E 1.6 MI
7. CONTINUE STRAIGHT TO STAY ON CT-68 E 0.2 MI
8. TURN RIGHT TO MERGE ONTO CT-15 N TOWARD HARTFORD 1.6 MI
9. SHARP LEFT TO MERGE ONTO I-91 N TOWARD HARTFORD 0.3 MI
10. MERGE ONTO I-91 N 17.2 MI
11. TAKE EXIT ONTO CT-3 N TOWARD GLASTONBURY 1.8 MI
12. TAKE THE EXIT TOWARD GLASTONBURY /MAIN STREET 0.3 MI
13. USE THE LEFT 2 LANES TO TURN LEFT ONTO GLASTONBURY BLVD 0.3 MI
14. TURN RIGHT ONTO MAIN STREET (DESTINATION WILL BE ON THE RIGHT) 0.6 MI

SITE INFORMATION

VZ SITE NAME: GLASTONBURY WEST CT
 VZ PROJ FUZE I.D.: 16232043
 VZ LOCATION CODE: 20202199190
 VZ PROJECT CODE: 468258

LOCATION: 2577 MAIN STREET
GLASTONBURY CT, 06033

PROJECT SCOPE: REFER TO NOTE SHEET C-1 FOR SCOPE OF WORK.

MAP/BLOCK/LOT: D5/4140/W00038A/TWR

ZONING DISTRICT: TOWN CENTER

LATITUDE: 41° 42' 51.8004" N (41.714389° N)

LONGITUDE: 72° 36' 46.9008" W (72.613028° W)

SITE COORDINATES AND GROUND ELEVATION OBTAINED FROM GOOGLE EARTH.

GROUND ELEVATION: 58± AMSL

PROPERTY OWNER: SAINT ISIDORE AND MARIA PARISH CORP
2577 MAIN STREET
GLASTONBURY, CT 06033

APPLICANT: CELCO PARTNERSHIP
d/b/a VERIZON WIRELESS
20 ALEXANDER DRIVE
WALLINGFORD, CT 06492

LEGAL/REGULATORY COUNSEL: ROBINSON & COLE, LLP
KENNETH C. BALDWIN, ESQ.
280 TRUMBULL STREET
HARTFORD, CT 06103

ENGINEER CONTACT: ALL-POINTS TECHNOLOGY CORP., P.C.
567 VAUXHALL STREET EXTENSION - SUITE 311
WATERFORD, CT 06385
(860) 663-1697

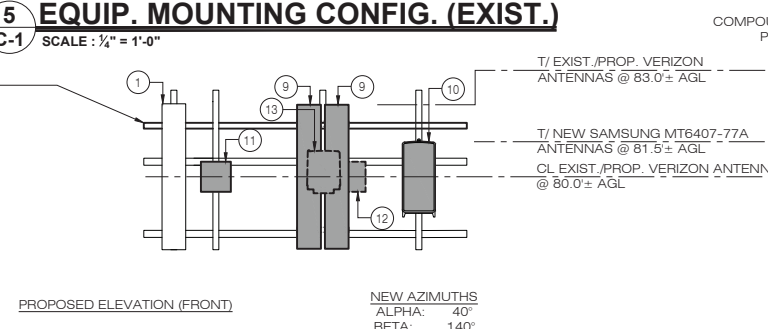
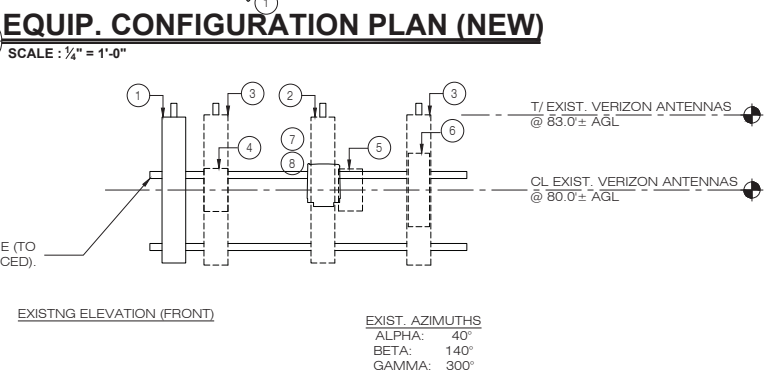
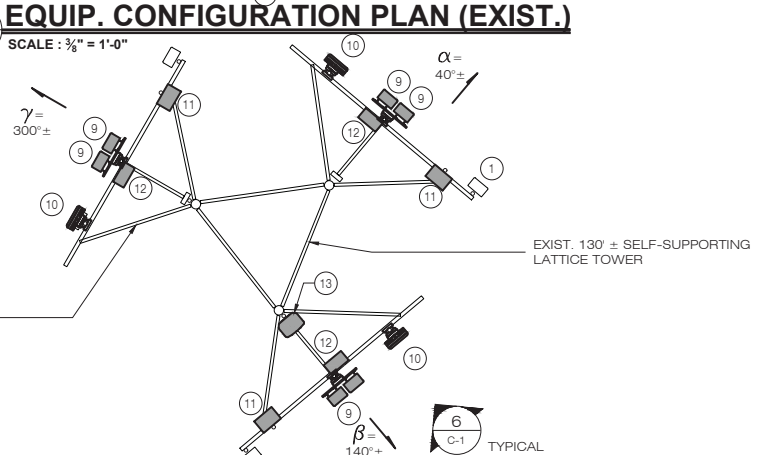
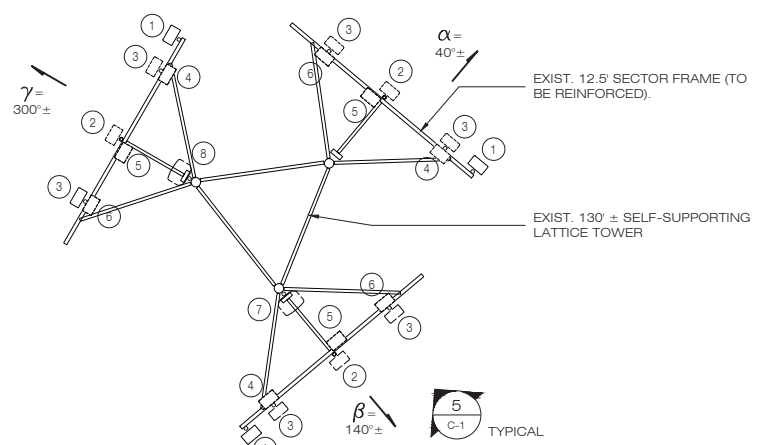
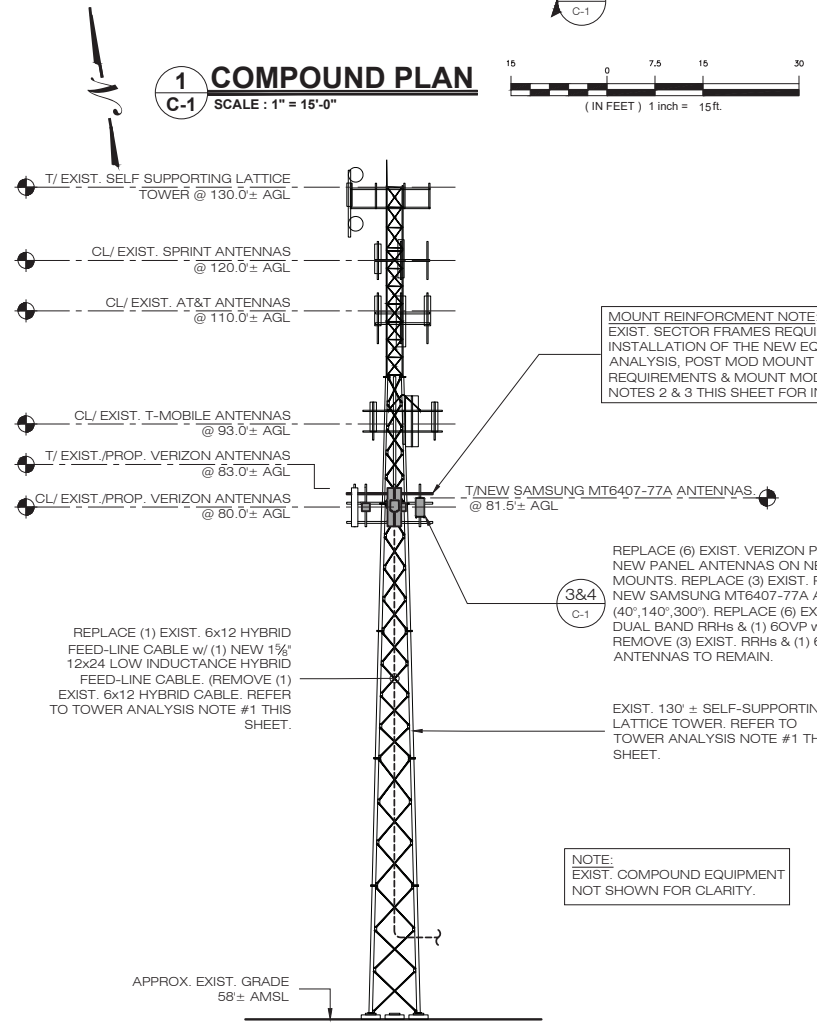
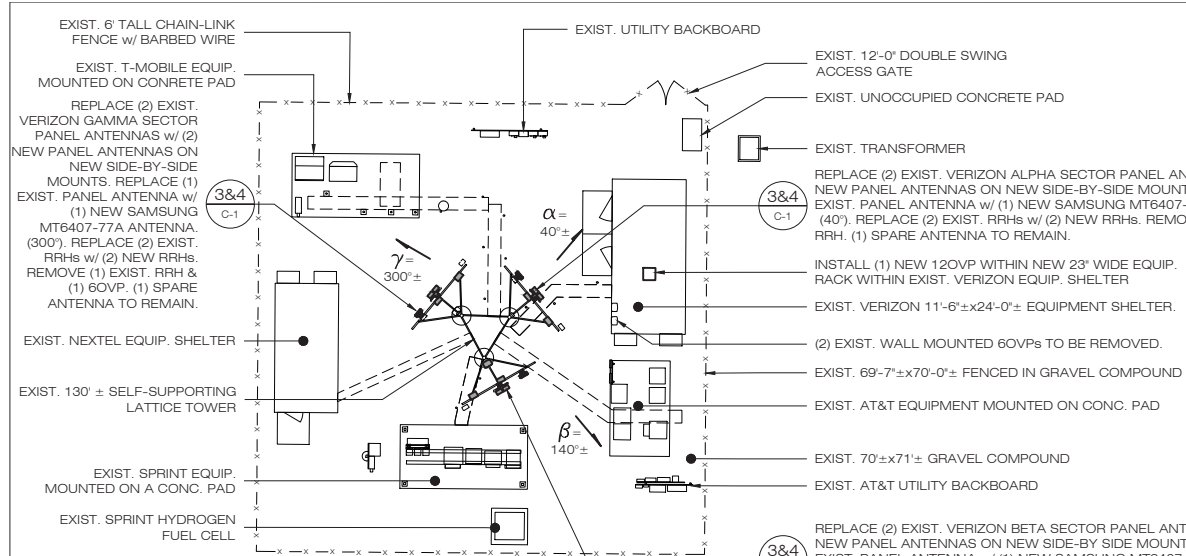
VERIZON SMART TOOL PROJECT # 10032203



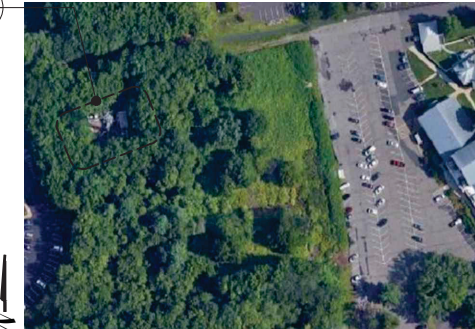
LOCATION MAP
SCALE: 1" = 1000'-0"

DRAWING INDEX

- T-1 TITLE SHEET
- C-1 COMPOUND PLAN, TOWER ELEVATION, EQUIPMENT CONFIGURATION PLANS & ELEVATIONS.
- B-1 RF BILL OF MATERIALS, MECHANICAL SPECIFICATIONS & EQUIPMENT DETAILS.
- N-1 NOTES & SPECIFICATIONS



- NOTES:**
- REFER TO TOWER STRUCTURAL ANALYSIS REPORT PREPARED BY TOWER ENGINEERING SOLUTIONS, DATED 05/17/21 AVAILABLE UNDER SEPARATE COVER.
 - REFER TO MOUNT ANALYSIS REPORT PREPARED BY MASER CONSULTING, P.A., PROJECT #20777629A MARKED REV.0, DATED 02/04/21 AVAILABLE UNDER SEPARATE COVER.
 - REFER TO POST MOD MOUNT ANALYSIS REPORT, PMI REQUIREMENTS & MOUNT MODIFICATION DESIGN DRAWINGS PREPARED BY MASER CONSULTING, P.A., PROJECT #20777629A DATED 03/10/21 & 03/09/21. AVAILABLE UNDER SEPARATE COVER.
 - BASE MAPPING FROM FIELD MEASUREMENTS TAKEN BY ALL-POINTS TECH. CORP., P.C. ON 01/28/21.
 - PROJECT SCOPE INCLUDES THE FOLLOWING:
 - REPLACEMENT OF SIX (6) EXIST. PANEL ANTENNAS w/ SIX (6) NEW PANEL ANTENNAS ON SIDE-BY-SIDE MOUNTS.
 - REPLACEMENT OF THREE (3) EXIST. PANEL ANTENNAS w/ (3) NEW SAMSUNG MT6407-77A ANTENNAS.
 - REPLACEMENT OF SIX (6) EXIST. RRHs w/ SIX (6) NEW DUAL BAND RRHs.
 - REPLACEMENT OF (1) EXIST. 60VP w/ (1) NEW 120VP.
 - REPLACEMENT OF (2) EXIST. 6x12 HYBRID FEED-LINE CABLES w/ (1) NEW 1 1/2" 12x24 LOW INDUCTANCE HYBRID FEED-LINE CABLE.
 - REPLACEMENT OF (2) EXIST. WALL MOUNTED 60VPs WITHIN EQUIP. SHELTER w/ (1) NEW RACK MOUNTED 120VP.
 - INSTALLATION OF (1) NEW 23' WIDE EQUIP. RACK WITHIN EQUIP. SHELTER.
 - REMOVAL OF (1) EXIST. 6x12 HYBRID CABLE.
 - REMOVAL OF THREE (3) EXIST. RRHs.
 - REMOVAL OF ALL UN-USED COAXIAL CABLE FEED-LINES.
 - ALL EXPOSED STEEL AND HARDWARE TO BE HOT DIP GALV. (HDG). PAINT TO MATCH EXIST. (WHERE APPLICABLE)
 - CAP & WEATHERPROOF ALL UN-USED CABLE ENTRY PORTS (WHERE APPLICABLE).
 - MOUNT & GROUND ALL NEW EQUIPMENT IN ACCORDANCE WITH NEC (NFPA-70), NESC AND MANUFACTURERS SPECIFICATION.
 - SECURE ALL NEW ANTENNA CABLES PER MANUFACTURER RECOMMENDATIONS.
 - BOND NEW ANTENNA MOUNTING PIPES TO ANTENNA SECTOR GROUND BAR w/ # 2 AWG, BOW, (WHERE APPLICABLE).
 - CONTRACTOR SHALL INSTALL NEW SIDE-BY-SIDE & DUAL-MOUNT BRACKETS PER ANTENNA MOUNT MANUFACTURER RECOMMENDATIONS, INCLUDING VERIFICATION OF MINIMUM PIPE MAST DIAMETER REQUIRED TO INSTALL NEW MOUNT BRACKETS. UNLESS NOTED OTHERWISE, CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD SHOULD EXIST. PIPE MASTS REQUIRE REPLACEMENT TO SUPPORT THE NEW MOUNT BRACKETS.
 - ANTENNA CONFIGURATIONS SHOWN HEREIN ARE FRONT ELEVATIONS.
 - ANTENNA SPACING DIMENSIONS ARE TO THE CENTER OF THE EXIST. ANTENNA AND PROP. ANTENNA FACE.
 - REFER TO THE FINAL RFDS PROVIDED BY VERIZON FOR THE LATEST INFORMATION REGARDING EQUIPMENT MODELS, REQUIRED CABLING & DOWN-TILT INFORMATION.
 - APPLY 3M FILM OVER ALL EXPOSED MMWAVE ANTENNAS COLOR TO MATCH EXIST. STRUCTURE (WHERE APPLICABLE) COORDINATE WITH VERIZON CONSTRUCTION MANAGER AND LL.
 - PAINT ALL NEW NON SAMSUNG MT6407-77A ANTENNAS & APPURTENANCES TO MATCH EXIST. STRUCTURE (WHERE APPLICABLE) COORDINATE W/ VERIZON CONSTRUCTION MANAGER & BUILDING OWNER.



GENERAL ABBREVIATION LIST:

• ABP	ABOVE BASE PLATE
• AGL	ABOVE GROUND LEVEL
• AMSL	ABOVE MEAN SEA LEVEL
• AWS	ADVANCED WIRELESS SERVICE
• HDG	HOT DIP GALVANIZED
• OVP	OVER VOLTAGE PROTECTION
• RRH	REMOTE RADIO HEAD
• V.I.F.	VERIFY IN FIELD
• W.P.	WORK POINT
• A.F.R.	ABOVE FINISH ROOF

SCOPE OF WORK (ALL) SECTORS

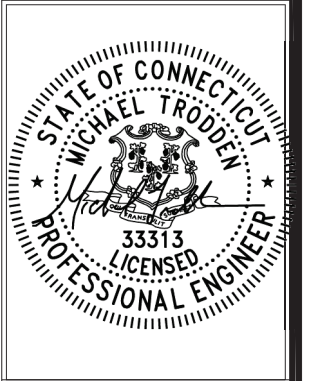
1 EXIST. ANTENNA (TO REMAIN AS SPARE) MODEL: ANDREW LNX-6514DS-A1M	4 EXIST. RRH (TO BE REPLACED) MODEL: NOKIA B13 RRH 4x30-700	7 EXIST. 6 OVP (TO BE REPLACED @ BETA) MODEL: RAYCAP RRFDC-3315-PF-48 (V.I.F.)	10 NEW ANTENNA MODEL: SAMSUNG MT6407-77A
2 EXIST. ANTENNA (TO BE REPLACED) MODEL: ANDREW LNX-6514DS-A1M	5 EXIST. RRH (TO BE REMOVED) MODEL: NOKIA B25 4x30W PCS RRH	8 EXIST. 6 OVP (TO BE REMOVED @ GAMMA) MODEL: RAYCAP RRFDC-3315-PF-48 (V.I.F.)	11 NEW DUAL BAND RRH MODEL: SAMSUNG B13/B5 RRH-BR04C (RFV01U-D2A)
3 EXIST. ANTENNA (TO BE REPLACED) MODEL: ANDREW HBXX-6517DS-A2M	6 EXIST. RRH (TO BE REPLACED) MODEL: NOKIA B4 2x60W AWS RRH	9 EXIST. RRH (TO BE REPLACED) MODEL: COMMScope NHH-65B-R2B MOUNTED ON NEW COMMScope SIDE-BY-SIDE MOUNT (P/N BASMNT-SBS-1-2)	12 NEW DUAL BAND RRH MODEL: SAMSUNG B66/B2A RRH-BR049 (RFV01U-D1A)
			13 NEW 120VP (@ BETA MOUNTED TO TOWER LEG) (TO MATCH EXIST.) MODEL: RAYCAP RVZDC-6627-PF-48

Cellco Partnership d/b/a
verizon
20 ALEXANDER DRIVE
WALLINGFORD, CT 06492

ALL-POINTS
TECHNOLOGY CORPORATION
567 VAUXHALL STREET EXTENSION - SUITE 311
WATERFORD, CT 06385 PHONE: (860)-953-1687
WWW.ALLPOINTS TECH.COM FAX: (860)-953-0935

CONSTRUCTION DOCUMENTS

NO	DATE	REVISION
0	02/19/21	FOR REVIEW: JRM
1	05/20/21	FOR FILING: JRM
2		
3		
4		
5		
6		



DESIGN PROFESSIONALS OF RECORD
PROF: MICHAEL S. TRODDEN P.E.
COMP: ALL-POINTS TECHNOLOGY CORPORATION, P.C.
ADD: 567 VAUXHALL STREET EXT. SUITE 311 WATERFORD, CT 06385
OWNER: SAINT ISIDORE AND MARIA ADDRESS: PARISH CORP 2577 MAIN STREET GLASTONBURY, CT 06033

GLASTONBURY WEST CT
SITE 2577 MAIN STREET
ADDRESS: GLASTONBURY CT, 06033
APT FILING NUMBER: CT141_11930
DRAWN BY: DRA
DATE: 02/19/21 CHECKED BY: JRM
VZ PROJECT CODE: 20202199190
VZ LOCATION CODE: 468258
VZ FUZE ID: 16232043

COMPOUND PLAN, TOWER ELEVATION, EQUIP. CONFIGURATION PLANS & ELEVATIONS

SHEET NUMBER: C-1

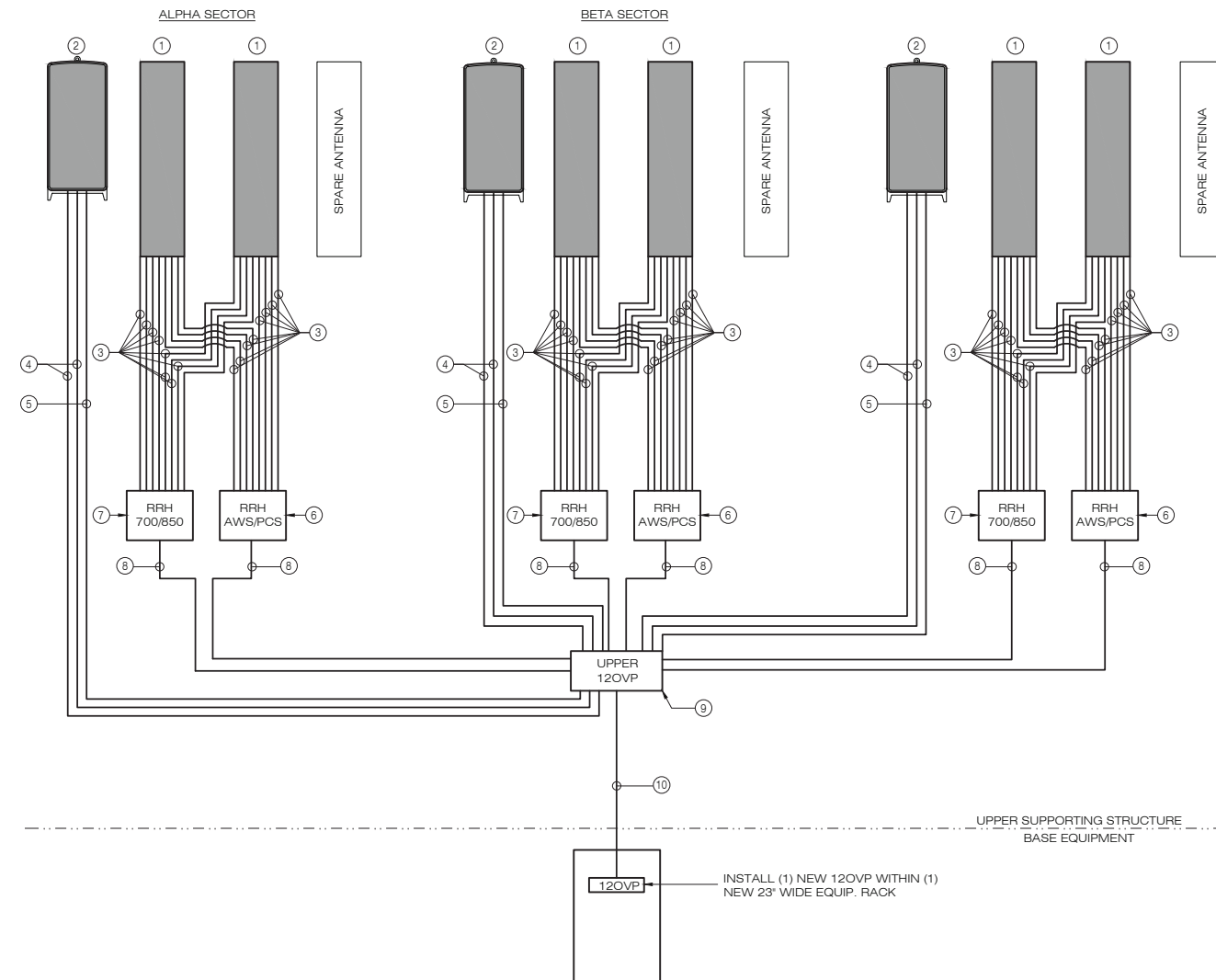
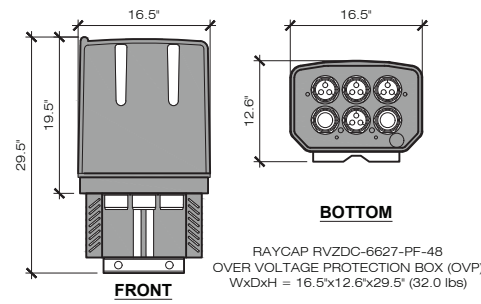
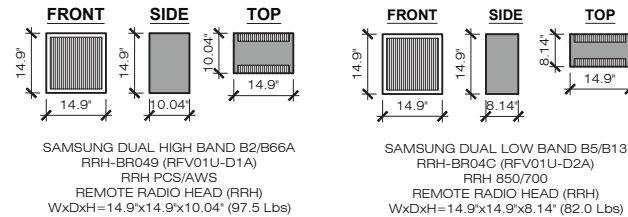
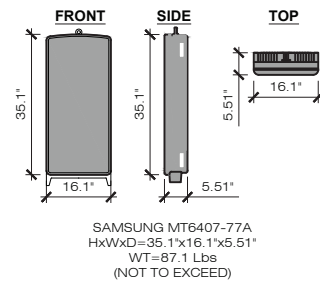
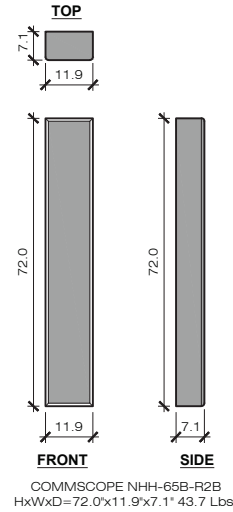
EQUIPMENT DATA								
EQUIPMENT SPECIFICATIONS								
SECTOR	ANTENNA MAKE/MODEL	QTY	AZIMUTH	EQUIPMENT STATUS	HEIGHT (IN)	WIDTH (IN)	DEPTH (IN)	WEIGHT (LBS)
ALPHA	SAMSUNG MT6407-77A	1	40°	NEW	35.1 ⁽⁵⁾	16.1 ⁽⁵⁾	5.51 ⁽⁵⁾	87.1 ⁽²⁾⁽⁵⁾
	700/850/1900/2100, COMMSCOPE NHH-65B-R2B	1	40°	NEW	72.0	11.9	7.1	43.7 ⁽²⁾
	700/850/1900/2100, COMMSCOPE NHH-65B-R2B	1	40°	NEW	72.0	11.9	7.1	43.7 ⁽²⁾
	SPARE: ANDREW LNX-6514DS-A1M	1	40°	ETR	80.6	11.9	7.1	32.2 ⁽²⁾
BETA	SAMSUNG MT6407-77A	1	140°	NEW	35.1 ⁽⁵⁾	16.1 ⁽⁵⁾	5.51 ⁽⁵⁾	87.1 ⁽²⁾⁽⁵⁾
	700/850/1900/2100, COMMSCOPE NHH-65B-R2B	1	140°	NEW	72.0	11.9	7.1	43.7 ⁽²⁾
	700/850/1900/2100, COMMSCOPE NHH-65B-R2B	1	140°	NEW	72.0	11.9	7.1	43.7 ⁽²⁾
	SPARE: ANDREW LNX-6514DS-A1M	1	140°	ETR	80.6	11.9	7.1	32.2 ⁽²⁾
GAMMA	SAMSUNG MT6407-77A	1	300°	NEW	35.1 ⁽⁵⁾	16.1 ⁽⁵⁾	5.51 ⁽⁵⁾	87.1 ⁽²⁾⁽⁵⁾
	700/850/1900/2100, COMMSCOPE NHH-65B-R2B	1	300°	NEW	72.0	11.9	7.1	43.7 ⁽²⁾
	700/850/1900/2100, COMMSCOPE NHH-65B-R2B	1	300°	NEW	72.0	11.9	7.1	43.7 ⁽²⁾
	SPARE: ANDREW LNX-6514DS-A1M	1	300°	ETR	80.6	11.9	7.1	32.2 ⁽²⁾
APPURTENANCE MAKE/MODEL								
	SAMSUNG B2/B66A RRH-BR049 (RFV01U-D1A)	3	-	NEW	14.9	14.9	10.04	97.5
	SAMSUNG B5/B13 RRH-BR04C (RFV01U-D2A)	3	-	NEW	14.9	14.9	8.14	82.0
	RAYCAP RVZDC-6627-PF-48	1	-	NEW	29.5	16.5	12.6	32.0

- (1) ETR DENOTES EXIST. TO REMAIN
- (2) WEIGHT WITHOUT MOUNTING BRACKET.
- (3) ANTENNA DATA BASED ON RFDS REV0 DATED 12/16/20
- (4) EQUIPMENT CONFIGURATION INDICATED ABOVE VIEWED FROM BEHIND.
- (5) NOT TO EXCEED

BILL OF MATERIALS				
		QUANTITY	LENGTH	COMMENTS
①	700/850/1900/2100	6		(COMMSCOPE NHH-65B-R2B) MOUNTED W/ NEW COMMSCOPE SIDE-BY-SIDE MOUNT (BASMNT-SBS-1-2)
②	SAMSUNG MT6407-77A	3		MOUNTED ON EXIST. PIPE MAST
③	1/2' JUMPER CABLE	48	15 FT	ROUTE FROM RRH TO ANTENNAS
④	ANTENNA LINK CABLES	6	15 M	ROUTE FROM UPPER OVP TO ANTENNAS
⑤	ANTENNA POWER CABLES	3	15 M	PROPRIETARY POWER CABLE FROM UPPER OVP TO ANTENNAS
⑥	AWS/PCS RRH	3		SAMSUNG B2/B66 RRH-BR049 (RFV01U-D1A) MOUNTED TO EXIST. PIPE MAST
⑦	700/850 RRH	3		SAMSUNG B5/B13 RRH-BR04C (RFV01U-D2A) MOUNTED TO EXIST. PIPE MAST
⑧	RRH CABLES	6	15M	PROPRIETARY POWER & FIBER CABLES
⑨	UPPER 12OVP	1		(RVZDC-6627-PF-48)
⑩	HYBRID CABLE	1	120' ±	12x24 LOW INDUCTANCE HYBRID CABLE (1½"Ø)

NOTES:

1. INFORMATION SHOWN HEREON IS FOR USE BY VERIZON EQUIPMENT OPERATIONS.
2. INFORMATION IS BASED ON RFDS REV0 DATED 12/16/20.
3. * DENOTES EQUIPMENT DESIGNATED "FOR LEASING ONLY" (WHERE APPLICABLE)
4. INSTALL ALARM BOARDS AT ALL OVPs WHERE REQUIRED. COORDINATE w/ VERIZON EQUIPMENT ENGINEERING.
5. INSTALL UP-CONVERTER(S) LOCATED AT BASE OVPs WHERE REQUIRED. COORDINATE w/ VERIZON EQUIPMENT ENGINEERING AS NECESSARY.
6. COORDINATE ANTENNA CABLING REQUIREMENTS WITH VERIZON ENGINEERING.
7. CONTRACTOR SHALL INSTALL NEW SIDE-BY-SIDE & DUAL-MOUNT BRACKETS PER ANTENNA MOUNT MANUFACTURER RECOMMENDATIONS, INCLUDING VERIFICATION OF MINIMUM PIPE MAST DIAMETER REQUIRED TO INSTALL NEW MOUNT BRACKETS. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD SHOULD EXIST. PIPE MAST REQUIRE REPLACEMENT TO SUPPORT THE NEW MOUNT BRACKETS.



Cellco Partnership d/b/a

verizon

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CONSTRUCTION DOCUMENTS

NO	DATE	REVISION
0	02/19/21	FOR REVIEW: JRM
1	05/20/21	FOR FILING: JRM
2		
3		
4		
5		
6		



DESIGN PROFESSIONALS OF RECORD

PROF: MICHAEL S. TRODDEN P.E.
COMP: ALL-POINTS TECHNOLOGY CORPORATION, P.C.
ADD: 567 VAUXHALL STREET EXT. SUITE 311
WATERFORD, CT 06385

OWNER: SAINT ISIDORE AND MARIA
ADDRESS: PARISH CORP
2577 MAIN STREET
GLASTONBURY, CT 06033

GLASTONBURY WEST CT

SITE ADDRESS: 2577 MAIN STREET
GLASTONBURY CT, 06033

APT FILING NUMBER: CT141_11930

DRAWN BY: DRA

DATE: 02/19/21 CHECKED BY: JRM

VZ PROJECT CODE: 20202199190

VZ LOCATION CODE: 468258

VZ FUZE ID: 16232043

SHEET TITLE:

BILL OF MATERIALS

SHEET NUMBER:

B-1

DESIGN BASIS:
GOVERNING CODES/DESIGN STANDARDS:
2015 INTERNATIONAL BUILDING CODE (IBC) AS AMENDED BY THE 2018 CONNECTICUT STATE BUILDING CODE
ASCE 7-10
TIA-222-G (TOWER)

01 GENERAL:
ABBREVIATIONS USED IN THESE SPECIFICATIONS INCLUDE THE FOLLOWING:

- ACI AMERICAN CONCRETE INSTITUTE
- ANSI AMERICAN NATIONAL STANDARDS INSTITUTE
- AWIS AMERICAN WELDING SOCIETY
- ASIS AMERICAN INSTITUTE OF STEEL CONSTRUCTION
- ASCE AMERICAN SOCIETY OF CIVIL ENGINEERS
- ASTM AMERICAN STANDARDS AND TESTING METHODS
- CRSI CONCRETE REINFORCING STEEL INSTITUTE
- ICC-ES INTERNATIONAL CODE COUNCIL EVALUATION SERVICE
- TH TELECOMMUNICATIONS INDUSTRY ASSOCIATION
- UL UNDERWRITERS LABORATORIES
- NES NATIONAL ELECTRICAL CODE
- NFPA NATIONAL FIRE PROTECTION ASSOCIATION
- OSHA OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION

EVERY INDIVIDUAL TRADE, DISCIPLINE, AND CONTRACTOR SHALL INCLUDE THESE GENERAL SPECIFICATIONS.
THE ENGINEER IS NOT RESPONSIBLE FOR NOR A GUARANTOR OF THE INSTALLING CONTRACTORS WORK ADEQUACY OF ANY SITE COMPONENT, SUPERVISION OF WORK, AND SAFETY IN, ON, OR ABOUT THE WORK SITE.

ANY REFERENCE HEREIN TO AN OR EQUAL ITEM, THAT EQUAL ITEM SHALL BE PRE-APPROVED BY THE CONSTRUCTION MANAGER BEFORE INSTALLATION.
ALL TRADES SHALL COORDINATE THEIR WORK WITH ALL OTHER TRADES AND OTHER WORK AND CONDITIONS AS APPROPRIATE OR REQUIRED TO AVOID CONFLICTS. RESOLVE AND COORDINATE ALL CONFLICTS WITH ALL AFFECTED WORK AND SITE OPERATIONS. COORDINATION WITH THE SITE SHALL BE WITH THE OWNER, OR OWNERS SPECIFIED REPRESENTATIVE, FOR EVERYTHING RELATED TO THE INSTALLATION OF THIS PROJECT.

ALL WORK SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE EDITIONS OF ALL APPLICABLE CODES AND SHALL BE ACCEPTABLE TO ALL AUTHORITIES HAVING JURISDICTION (A.H.) WHERE A CONFLICT EXISTS BETWEEN CODES, PLANS, SPECIFICATIONS, AND/OR SHALL THE MORE STRINGENT AUTHORITY SHALL APPLY. WHERE CONFLICT EXISTS BETWEEN PLANS AND SPECIFICATIONS, PLANS SHALL APPLY. WHERE CONFLICT EXISTS BETWEEN PLAN SHEETS, CONSTRUCTION MANAGER SHALL BE CONSULTED PRIOR TO COMMENCING ANY WORK.

CONTRACTOR SHALL PROVIDE ALL LABOR, MATERIALS, INSURANCE, EQUIPMENT, INSTALLATION, CONSTRUCTION TOOLS, TRANSPORTATION, ETC. FOR A COMPLETE AND NEWELY OPERATIVE AND USABLE SYSTEM THROUGHOUT AND AS INDICATED ON THE DRAWINGS AND AS SPECIFIED HEREIN AND/OR OTHERWISE REQUIRED.

CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS, INSTALLATIONS, AND EQUIPMENT IN THE FIELD PRIOR TO BID, FABRICATION, AND INSTALLATION OF ANY WORK.
CONTRACTORS SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO FABRICATION AND ERECTION OF ANY MATERIAL. THE ENGINEER SHALL BE NOTIFIED FOR INSPECTIONS PRIOR TO CLOSING PENETRATIONS AND OF ANY CONDITIONS WHICH PRECLUDE COMPLETION OF THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

CONTRACTOR SHALL VISIT THE SITE TO MANAGE AND GAIN APPROVAL FOR ALL TENANT DISTRIBUTIONS, POWER OUTAGES, WORK SCHEDULES, DEFINITION OF WORK AREA AND WORK STORAGE, NEWER BUILDING/SITE ACCESS, NOISE AND CLEANLINESS REQUIREMENTS WITH THE BUILDING/SITE MANAGEMENT PRIOR TO ALL WORK. ANY DISTRIBUTIONS SHALL BE KEPT TO A MINIMUM AND SHALL BE IMPLEMENTED ONLY UPON WRITTEN APPROVAL OF THE OWNER.

THE CONTRACTOR SHALL SAFEGUARD AGAINST CREATING ANY HAZARD AFFECTING TENANT EGRESS OR COMPROMISING SITE SECURITY MEASURES.
PRIOR TO ALL BELOW-GRADE WORK AND ANY SURFACE WORK IN A NEW AREA FOR STRUCTURES OR VEHICLES, CONTRACTOR SHALL ENGAGE A MARKOUT SERVICE TO IDENTIFY ANY UNDERGROUND STRUCTURES, CONDUITS, AND PRELINES IN THE AREA. ALL EXISTING SEWER, WATER, GAS, ELECTRIC, FIBER OPTIC, AND OTHER UNDERGROUND UTILITIES IDENTIFIED OR ENCOUNTERED SHALL BE PROTECTED AT ALL TIMES. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN DIGGING OR EXCAVATING IN ANY MANNER ABOVE OR NEAR SUCH UTILITIES. CONTRACTOR IS RESPONSIBLE FOR REPAIRS, REPLACEMENT, AND ALL DAMAGES DUE TO DAMAGE OF UTILITIES BY HIS OPERATIONS.

ALL EXISTING AND NEW EQUIPMENT AND MATERIAL LOCATIONS, ROUTING, ORIENTATION, MOUNTING, SPECIFICATIONS AND GENERAL INSTALLED CHARACTERISTICS SHALL BE CONSIDERED DIAGRAMMATIC ON THE PLANS. EXACT CONDITIONS SHALL BE DETERMINED IN THE FIELD PRIOR TO ANY INSTALLATION. ANY DIFFERENCES THAT MAY CAUSE SCHEDULE, COST OR QUALITY SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER OR ENGINEER PRIOR TO ANY WORK.

ALL REFERENCES HEREIN TO VERIFICATION OF ANY CONDITION OF SITE, FIELD PLANS, OR SPECIFICATIONS PRIOR TO ANY WORK SHALL BE THE FULL RESPONSIBILITY OF THE CONTRACTOR. ANY AND ALL ADDITIONS, MODIFICATIONS, CHANGES, REPAIR, OR DEMOLITION AS A RESULT OF FAILURE TO BRING ANY EXISTING CONDITION NEWLY TO THE ATTENTION OF THE OWNER OR ENGINEER SHALL BE THE FULL RESPONSIBILITY OF THE CONTRACTOR WITHOUT DELAY, COST, OR CHANGES IN QUALITY.

ALL NOTES THIS SHEET SHALL APPLY UNLESS SPECIFICALLY NOTED OTHERWISE ON THE INCLUDED DRAWINGS OR IN SEPARATE PLOTTED SPECIFICATIONS AS APPLICABLE. ALL SPECIFICATIONS SHALL BE CONSIDERED REQUIRED UNLESS APPROVED EQUAL BY THE OWNER, CONSTRUCTION MANAGER, OR ENGINEER AS APPLICABLE.
THE WORDS "PROVIDE" OR "INSTALL" SHALL MEAN FURNISH AND INSTALL.

CONTRACTOR SHALL PROVIDE ALL CUTTING AND PATCHING AS REQUIRED FOR THE INSTALLATION OF HIS WORK. ANY PATCHING SHALL MATCH EXISTING SURROUNDING AREA IN ALL RESPECTS. ALL REMOVED MATERIAL SHALL BE REMOVED FROM THE PREMISES DAILY IN AN APPROVED SAFE MANNER.

ALL SURPLUS MATERIAL SHALL BE REMOVED FROM THE SITE PROMPTLY WHEN DEEMED TO BE SURPLUS.
EVERY CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF HIS WORK AND NEWLY INSTALLED OR EXISTING WORK, INCLUDING PROTECTION OF THE SITE, ALL STRUCTURES, AND ALL OCCUPANTS. FURNISH, INSTALL, MAINTAIN, AND REMOVE AS APPROPRIATE, ALL APPROPRIATE BARRIERS, SAFETY GUARDS, SIGNAGE, AND SECURITY AS REQUIRED.

EVERY CONTRACTOR SHALL BE RESPONSIBLE FOR THEIR RESPECTIVE FEES, PERMITS, INSPECTIONS, TESTING, CERTIFICATES, AND ALL MANAGEMENT OF SAME REQUIRED FOR COMPLETION OF AND LEGAL OCCUPANCY OF THE FINISHED PROJECT.

ALL CONTRACTORS SHALL PROVIDE ALL NECESSARY TOOLS, FIXTURES, SERVICES, MATERIALS, JOB AIDS, AND PERSONNEL REQUIRED FOR THE EXECUTION OF THEIR WORK.
EACH CONTRACTOR SHALL GUARANTEE ALL MATERIALS AND WORKMANSHIP BY THEM TO BE FREE OF DEFECTS AND MAINTAINED FOR A PERIOD OF ONE YEAR AFTER ACCEPTANCE OF THE INSTALLATION BY THE OWNER AND ENGINEER.

ALL WORK SHALL BE PERFORMED BY LICENSED CONTRACTORS IN THE TRADE HAVING JURISDICTION.
ANY DEVIATION, MODIFICATION, ADDITION, OR CHANGE IN DESIGN SHALL NOT BE MADE WITHOUT WRITTEN APPROVAL OF THE OWNER OR ENGINEER.

ALL CONTRACTORS SHALL SUBMIT SHOP DRAWINGS OF ALL EQUIPMENT AND MATERIALS TO THE ENGINEER FOR APPROVAL PRIOR TO FABRICATION AND INSTALLATION, AND SHALL NOT PROCEED UNTIL ENGINEER APPROVAL IN WRITING IS RETURNED. EACH CONTRACTOR SHALL MAINTAIN ON JOB SITE A COMPLETE SET OF SHOP DRAWINGS WITH ANY DEVIATIONS FROM THE ORIGINAL DESIGN SHALL BE NOTED.

ALL MATERIALS AND EQUIPMENT SHALL BE NEW, WITHOUT BLEMISH OR DEFECT, AND SUITABLE AND LISTED FOR THE INSTALLATION AND SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS OR SPECIFICATIONS. ALL ITEMS OF EQUIPMENT OR MATERIAL THAT ARE OF ONE GENERIC TYPE SHALL BE ONE MANUFACTURER THROUGHOUT.

ALL MATERIALS, EQUIPMENT, TOOLS, AND ITEMS UNDER THE CONTRACTORS RESPONSIBILITY ON THE JOBSITE SHALL BE ADEQUATELY SECURED, MAINTAINED, AND PROTECTED, SO AS NOT TO BECOME DAMAGED OR CREATE ANY HAZARD TO PERSONNEL OR NEWERTY.

THE CONTRACTORS HOURS OF WORK SHALL BE IN ACCORDANCE WITH LOCAL CODES AND ORDINANCES AND BE APPROVED BY THE OWNER. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR ALL OF HIS CREW AND INSURE THAT EVERY CREW MEMBER FOLLOWS SAFE WORK PRACTICES. SAFETY TRAINING SHALL INCLUDE, BUT NOT BE LIMITED TO, FALL PROTECTION, CONFINED SPACE ENTRY, ELECTRICAL SAFETY, AND TRENCH/EXCAVATION SAFETY WHERE SUCH WORK IS EXECUTED OR ENCOUNTERED.

ALL TEMPORARY WORK REQUIRED OR SPECIFIED AS A PART OF THIS WORK, SHALL MEET ALL OF THE SAME REQUIREMENTS AS PERMANENT INSTALLATIONS. SHALL MEET ALL APPLICABLE CODE REQUIREMENTS, AND SHALL BE COMPLETELY REMOVED AFTER ITS PURPOSES HAVE BEEN SERVED.

ANY EXISTING UTILITY, SERVICE, STRUCTURE, EQUIPMENT, OR FIXTURE OBSTRUCTING THE WORK SHALL BE REMOVED AND/OR RELOCATED AS DIRECTED BY THE CONSTRUCTION MANAGER.
IF ASBESTOS IS ENCOUNTERED DURING WORK EXECUTION, CONTRACTOR SHALL IMMEDIATELY NOTIFY THE CONSTRUCTION

MANAGER AND CEASE ALL ACTIVITIES IN AFFECTED AREAS UNTIL NOTIFIED BY THE CONSTRUCTION TO RESUME OPERATIONS.
EXIST. ELECTRICAL AND MECHANICAL FIXTURES, PIPING, WIRING AND EQUIPMENT OBSTRUCTING THE WORK SHALL BE REMOVED AND/OR RELOCATED AS DIRECTED BY THE CONSTRUCTION MANAGER. TEMPORARY SERVICE INTERRUPTIONS MUST BE COORDINATED WITH OWNER.

05 STEEL:
THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS HEREIN.
MATERIALS:
WIDE FLANGE ASTM A992, GR 50
TUBING ASTM A500, GR B
PIPE ASTM A53, GR B
BOLTS ASTM A325
GRATING TYPE GW-2 (1-1/4X1/4" BARS)
EXISTING METALS ASTM A36

PROVIDE CERTIFICATION THAT WELDERS TO BE USED IN WORK ARE LICENSED AND HAVE SATISFACTORILY PASSED AWS QUALIFICATION TEST UNDER THE PROVISIONS OF APPENDIX D, PARTS II AND III OF THE AWS CODE FOR WELDING IN BUILDING CONSTRUCTION.

ALL BUILDING CONNECTION POINTS TO BE CENTERED ON EXISTING STRUCTURAL BEARING POINTS AND THE LOCATIONS ARE TO BE VERIFIED IN FIELD PRIOR TO THE FABRICATION OF STEEL.
DESIGN AND CONSTRUCTION OF STRUCTURAL STEEL SHALL CONFORM TO THE LATEST EDITION OF AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.

NON-STRUCTURAL CONNECTIONS FOR STEEL GRATING MAY USE 3/8" DIAMETER GALVANIZED ASTA A 307 BOLTS UNLESS OTHERWISE NOTED. ALL STEEL MATERIAL SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 ZINC HOT-DIPPED GALVANIZED COATING ON IRON AND STEEL PRODUCTS WITH A COATING WEIGHT OF 2 OZ/SQ FT.

ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE EXPOSED TO WEATHER SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 ZINC COATING (HOT-DIP) ON IRON AND STEEL HARDWARE.
DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED BY TOUCHING UP ALL DAMAGED GALVANIZED STEEL WITH COLD ZINC GALVANIZING DRY GALV, OR ZINC IT, IN ACCORDANCE WITH MANUFACTURERS GUIDELINES. TOUCH UP DAMAGED NON-GALVANIZED STEEL WITH SAME PAINT APPLIED IN SHOP OR FIELD.

THE ENGINEER SHALL BE NOTIFIED OF ANY INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NONCONFORMING MATERIALS OR CONDITIONS TO REMOVAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER REVIEW. FIELD CUTTING OF STRUCTURAL STEEL IS NOT PERMITTED EXCEPT WITH THE PRIOR APPROVAL OF THE ENGINEER.

CONTRACTOR TO REMOVE AND RE-INSTALL ALL FIRE PROOFING AS REQUIRED DURING CONSTRUCTION.
THE STEEL STRUCTURE SHALL BE DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER COMPLETION. IT IS THE CONTRACTORS SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION.

ALL STEEL ELEMENTS SHALL BE INSTALLED PLUMB AND LEVEL. TOWER MANUFACTURERS REPAIR DESIGNS SHALL PREVAIL FOR TOWER CONNECTIONS WHICH ARE DESIGNED BY THE FABRICATOR AND CONSTRUCTED IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC MANUAL OF STEEL CONSTRUCTION. CONNECTIONS SHALL BE PROVIDED TO CONFORM TO THE REQUIREMENTS OF TYPE 2 CONSTRUCTION.

STRUCTURAL CONNECTION BOLTS SHALL CONFORM TO ASTM A325. ALL BOLTS SHALL BE MINIMUM 3/8" DIAMETER AND EACH CONNECTION SHALL HAVE MINIMUM TWO BOLTS. LOCK WASHERS ARE NOT PERMITTED FOR A325 STEEL ASSEMBLIES. IF TENSION CONTROL BOLTS ARE USED, CONNECTIONS SHALL BE DESIGNED FOR SLIP CRITICAL BOLT ALLOWABLE LOAD VALUES.

DESIGN CONNECTIONS AT BEAM ENDS FOR 10 KIPS (MN).
ALL U-BOLTED CONNECTIONS SHALL BE COMPLETED WITH DOUBLE NUTS OR A LOCK WASHER.
CONTRACTOR SHALL COMPLY WITH AWS CODE FOR PROCEDURES, APPEARANCE AND QUALITY OF WELDS, AND WELDING PROCESSES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS STANDARD QUALIFICATION PROCEDURES. ALL WELDING SHALL BE PERFORMED USING E70XX ELECTRODES AND SHALL CONFORM TO AISC AND D1.1. WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE LARGER OF 1/4" FILLET OR MINIMUM SIZE PER TABLE J2 IN THE AISC MANUAL OF STEEL CONSTRUCTION. AT THE COMPLETION OF WELDING, ALL DAMAGE TO GALVANIZED COATING SHALL BE REPAIRED. SEE NOTE REGARDING DAMAGED GALVANIZED SURFACES.

ALL ARC AND GAS WELDING SHALL BE DONE BY A LICENSED AND CERTIFIED WELDER IN ACCORDANCE WITH AWS.
SEAL ALL PENETRATIONS AND SEAMS BETWEEN MASONRY AND STEEL WITH DOW CORNING 790 SILICONE BUILDING SEALANT OR EQUAL.

26 ELECTRICAL:
THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS HEREIN.
ALL ELECTRICAL CONDUCTORS:
• INSULATION SHALL BE MINIMUM 600V TYPE THHN, THWN, 2 OR XHHW.
• BRANCH CIRCUIT CONDUCTORS SHALL BE SOFT DRAWN 98% MINIMUM CONDUCTIVITY NEWLY REFINED COPPER.
• FEEDER CIRCUIT CONDUCTORS SHALL BE EITHER COPPER OR ALUMINUM OF THE APPROPRIATE SIZE FOR THE APPLICATION, OR AS SPECIFICALLY NOTED.
• PERMANENTLY LABEL OR TAG ALL CONDUCTORS WITH THEIR CIRCUIT DESIGNATION AT ALL TERMINATION ENDS, SPLICES, AND VISIBLE PASS-THROUGH IN ALL ENCLOSURES.

ALL CONDUIT, RACEWAY, WIREWAYS, DUCTS, ETC. SHALL BE LISTED AND SUITABLE FOR THE APPLICATION. ONLY THE FOLLOWING CONDUITS AS APPROVED AND LISTED FOR THE APPLICATION SHALL BE ACCEPTABLE:
• ELECTRICAL METALLIC TUBING (EMT),
• COMPRESSION COUPLINGS AND CONNECTORS ONLY MADE UP WRENCH TIGHT.
• FLEXIBLE METAL CONDUIT (FMC) AND LIQUIDTIGHT FLEXIBLE METAL CONDUIT (LFMC).
• FINAL CONNECTIONS TO VIBRATING OR ADJUSTABLE EQUIPMENT INCLUDING, BUT NOT LIMITED TO, LIGHT FIXTURES, HVAC UNITS, TRANSFORMERS, MOTORS, ETC. OR WHERE EQUIPMENT IS PLACED UPON SLAB ON-GRADE.
• RIGID GALVANIZED STEEL (RGS).
• ALL FITTINGS, CONNECTORS, AND COUPLINGS SHALL BE THREADED MADE UP WRENCH TIGHT.
• RIGID POLYVINYL CHLORIDE (PVC) SCHEDULE 40 OR SCHEDULE 80.
• MAY BE USED FOR SERVICES, EXTERIOR, BELOW GRADE, AND WET LOCATIONS.
• SHALL NOT BE USED IN CONCRETE SLABS NOR EXPOSED WITHIN A BUILDING OR STRUCTURE.
• METAL-CLAD CABLE (MC)
• CONCEALED INSTALLATIONS ONLY.

• WITHIN A DUCT WITH SMOOTH OR CORRUGATED METAL JACKET AND NO OUTER COVERING OVER THE METAL JACKET.
IN FINISHED SPACES, ALL CONDUITS SHALL BE CONCEALED EXCEPT TO MAKE A FINAL CONNECTION TO EQUIPMENT NOT MOUNTED IN OR AGAINST FINISH MATERIAL.
ALL FEEDER AND BRANCH CIRCUITS SHALL HAVE A SEPARATE NEWLY SIZED AND MARKED GROUNDING CONDUCTOR. PER APPLICABLE CODES, THAT BONDS ALL ENCLOSURES, BOXES, ETC. CONDUIT SHALL NOT BE USED AS A GROUNDING OR BONDING CONDUCTOR.

IF EXISTING ELECTRIC SERVICES TO REMAIN, CONTRACTOR SHALL VERIFY THAT IT MEETS PROJECT REQUIREMENTS WITHOUT MODIFICATION. IF IT IS TO BE ADDED OR REPLACED AS A PART OF THIS WORK, CONTRACTOR SHALL OBTAIN PERMITS, COORDINATE WITH A GAIN APPROVAL FROM THE ELECTRICAL UTILITY. ALL ELECTRICAL EQUIPMENT SHALL BE AS SPECIFIED AND AS APPROVED BY THE LOCAL UTILITY WHERE APPLICABLE.

ALL EQUIPMENT, ENCLOSURES, ETC. SHALL BE SUITABLE FOR THE INSTALLED ENVIRONMENT, MINIMUM NEMA 3R FOR ALL EXTERIOR INSTALLATIONS.
WIRING DEVICES SHALL BE SPECIFICATION GRADE AND WIRING DEVICE COVER PLATES SHALL BE PLASTIC WITH ENGRAVING AS SPECIFIED. COLOR SHALL BE Ivory. ALL DEVICES AND COVER PLATES SHALL BE OF THE SAME MANUFACTURER.

ALL FIRE-RATED PENETRATIONS SHALL BE SEALED USING A SUITABLE AND LISTED FIRE SEALING DEVICE OR GROUT THAT WILL MAINTAIN THE FIRE RATING OF THE STRUCTURE PENETRATED.
PROVIDE PERMANENTLY AFFIXED ENGRAVED NAMEPLATES FOR ALL CODE REQUIRED LABELING AND ON ALL PANELS, METERING DISCONNECTS, AND ELECTRICAL EQUIPMENT THAT IDENTIFIES EQUIPMENT SERVED, ELECTRICAL SOURCE WITH CIRCUIT IDENTIFICATION, AND VOLTAGES WITHIN.
ELECTRICAL CONTRACTOR IS RESPONSIBLE FOR ALL FINAL TERMINATIONS TO ALL EQUIPMENT.

ALL ELECTRICAL APPURTENANCES THAT ARE DISCONNECTED SHALL BE COMPLETELY FINISHED, FILLED, PAINTED, ETC. ALL PANEL SCHEDULES, EQUIPMENT LABELING, AND CODE-REQUIRED LABELING SHALL BE VERIFIED AND NEWLY COMPLETED TO MATCH THE INSTALLATION.
26 GROUNDING:
THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS HEREIN.

CONTRACTOR SHALL FURNISH AND INSTALL ALL TRANSMISSION CABLES, JUMPERS, CONNECTORS, GROUNDING STRAPS, ANTENNAS, MOUNT AND HARDWARE. ALL MATERIALS SHALL BE INSPECTED BY THE CONTRACTOR FOR DAMAGE UPON DELIVERY. JUMPERS SHALL BE SUPPLIED AT ANTENNAS AND EQUIPMENT INSIDE SHELTER. COORDINATE LENGTH OF JUMPER CABLES WITH OWNER. COORDINATE AND VERIFY ALL OF THE MATERIALS TO BE PROVIDED WITH OWNER PRIOR TO SUBMITTING BID AND ORDERING MATERIALS.

ANTENNAS: BOTH SIDES OF EQUIPMENT SHELTER WALL, AND JUMPER CABLES AT THE EQUIPMENT.
CONTRACTOR SHALL FURNISH AND INSTALL ALL CONNECTORS, ASSOCIATED CABLE MOUNTING AND GROUNDING HARDWARE, WALL MOUNTS, STANDOFFS, AND ALL ASSOCIATED HARDWARE TO INSTALL

REGULATIONS.
ALL GROUNDING ELECTRODES PRESENT AT EACH SERVICE LOCATION SHALL BE BONDED TOGETHER TO FORM THE GROUNDING ELECTRODE SYSTEM.
ALL EQUIPMENT ENCLOSURES, DEVICES, AND CONDUITS SHALL BE ACCOMPLISH THIS BOND WITH GROUNDING CONDUCTORS MINIMUM SIZED TO THE LARGEST GROUNDING CONDUCTOR PRESENT IN THE ENCLOSURE CONNECTED TO A GROUNDING TYPE BUSING EQUALLY SIZED OR MAXIMUM GROUND WIRE ACCOMMODATION AVAILABLE IN STANDARD MANUFACTURE FOR THE RESULTING SCHEDULED LESS EQUIPMENT GROUNDING AND LOAD SIDE BONDING CONDUCTORS SHALL BE SIZED PER THE CIRCUITS OVER-CURRENT PROTECTIVE DEVICE (OCPD) SIZE. WHERE THE UNGROUNDED CONDUCTORS ARE INCREASED IN SIZE ABOVE THE STANDARD FOR THE CIRCUITS OCPD, INCREASE THE GROUNDING CONDUCTOR NEWTOICATELY TO THE CROSS-SECTIONAL AREA OF THE UNGROUNDED CONDUCTORS.

SERVICE MAIN BONDING JUMPERS AND GROUNDING ELECTRODE CONDUCTORS SHALL BE SIZED AND INSTALLED PER THE MINIMUM OF ALL APPLICABLE CODES AND REGULATIONS.
26 LIGHTNING PROTECTION:
THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS AND THE GROUNDING SPECIFICATIONS HEREIN.

THE LIGHTNING PROTECTION GROUNDING SYSTEM (LPGS) SHALL CONSIST OF BONDING ALL EQUIPMENT AND CONDUCTIVE STRUCTURES TO LOCALIZED SINGLE-POINT GROUNDING CONNECTIONS (TYPICALLY GROUND BARS) WHICH ARE BONDED TOGETHER AND TO AN IN-GROUND SYSTEM. IF THE LPGS IS ON A BUILDING, IT SHALL BE EFFECTIVELY BONDED TO THE ELECTRICAL SERVICE MAIN AND TO AN IN-GROUND IN-GROUND RING WITH EVERY RING BONDED TOGETHER, AND ALL CONDUCTIVE STRUCTURES IN CLOSE PROXIMITY (FENCES, ICE BRIDGES, RELATED EQUIPMENT, ETC.) ALSO BONDED TO PROVIDE A COMMON ELECTRICAL EQUIPMENT/UTL SYSTEM FOR ALL CONDUCTIVE ELEMENTS AND STRUCTURES.

CONDUCTORS:
• MIN #2 AWG SOLID BARE TINNED COPPER (SBTC) FOR ALL IN-GROUND CONDUCTOR.
• MIN #2 AWG COPPER GREEN STRANDED FOR BONDING STRUCTURES, AND FOR INTER-SYSTEM BONDING OF INDIVIDUAL ELEMENTS SUCH AS GROUNDING RODS AND TOWER GROUND RINGS.
• MIN #6 AWG COPPER GREEN STRANDED OR ALL EQUIPMENT BONDING.

• INSTALL ALL IN-GROUND CONDUCTORS IN THE SAME HORIZONTAL PLANE OR IN A DOWNWARD DIRECTION AWAY FROM THE TOWER AND EQUIPMENT AREAS.
• AVOID LONG RUNS. MAKE DIRECT RUNS AS MUCH AS POSSIBLE.
• PLACE THROUGH NON-METALLIC SLEEVES WHEN PASSING THROUGH FLOORS, WALLS, CEILINGS, AND SIMILAR STRUCTURES.
• MAKE CONNECTIONS IN CONTACT WITH REINFORCING BARS USING EXOTHERMIC WELDING. MAKE ALL OTHER CONNECTIONS WITH EXOTHERMIC WELDING, IRREVERSIBLE COMPRESSION CONNECTORS, OR LISTED COMPRESSION TWO-HOLE LUGS.

• INSTALL ALL CONDUCTORS WITH A MINIMUM 18 INCH BEND RADIUS AND NO BEND LONGER THAN A 90 DEGREE ARC. ALL BENDS SHALL BE HORIZONTAL, OR DOWNWARD TOWARDS EARTH.
• ALL CONDUCTORS PASSING FROM ABOVE-GROUND TO IN-GROUND CONNECTIONS, WHEN EXPOSED, SHALL BE COVERED AND PROTECTED WITH A NON-METALLIC CONDUIT SEALED AT BOTH ENDS.
• IF 2 OR MORE IN-GROUND CONDUCTORS ARE IN THE SAME PATH (2 RINGS OVERLAPPING, BONDING FOLLOWING ANOTHER RING OR RADIAL, OR SIMILAR), COMBINE WITH A SHARED SINGLE EQUIPMENT AND TOWER GROUND RINGS SHALL BE:

• BONDED TO ANY CONDUCTIVE OBJECT OR STRUCTURE WITHIN 5 FEET OF GROUND RINGS AND WITHIN 20 FEET OF TOWER GROUND RINGS.
• INSTALLED MINIMUM 18 INCHES FROM FOUNDATIONS, FOOTINGS, AND SIMILAR.

INSTALL ALL IN-GROUND RINGS, RADIALS, BONDS CONNECTING THEM, AND ALL SIMILAR GROUNDING.
• MIN 30 INCHES BELOW GRADE, OR 6 INCHES BELOW THE FROST LINE, WHICHEVER IS GREATER DEPTH.
• MIN 2 FEET FROM FOUNDATIONS, FOOTINGS, OTHER GROUNDING SYSTEMS, AND SIMILAR STRUCTURES, EXCEPT WHEN MAKING A BOND TO ANY OF THESE STRUCTURES. DO NOT BOND TO FOUNDATION INTERNAL REINFORCEMENT.

ALL EQUIPMENT GROUNDED IN A COMMON AREA, COMPOUND, STRUCTURE, OR SIMILAR SHALL BE BONDED TO A SINGLE-POINT GROUND, PREFERABLY AN ISOLATED GROUND BAR. BOND THE GROUND BAR TO THE SYSTEM WITH MINIMUM 2 CONDUCTORS DIRECTED TO AN IN-GROUND RING. INSTALL 2 BONDING CONDUCTORS MINIMUM WITH EACH CONDUCTOR INSTALLED DIRECTIONALLY AWAY FROM EACH OTHER AND PARALLEL TO THE IN-GROUND RING, WITH NO TEE CONNECTIONS.

TOWER GROUNDING:
• EACH TOWER LEG SHALL BE BONDED TO ITS RING. SINGLE-LEGGED TOWERS, OR MONOPOLES, SHALL HAVE 2 BONDS ON OPPOSITE SIDES.
• BOND TO TOWER BASE, NOT TO VERTICAL TOWER STRUCTURE, AWAY FROM TOWER MOUNTING HARDWARE.
• EACH BOND SHALL HAVE A CORRESPONDING GROUND ROD ON THE RING.
• EACH BOND SHALL CONSIST OF 2 CONDUCTORS FROM THE TOWER TO ITS RING WITH EACH CONDUCTOR DIRECTED IN OPPOSITE DIRECTIONS WITH A PARALLEL CONNECTION TO THE RING ON OPPOSITE SIDES OF THE GROUND ROD.

EQUIPMENT AREA GROUNDING:
• COMMUNICATION AREAS ON EARTH SHALL HAVE A GROUND RING.
• BOND ALL EQUIPMENT TO A SINGLE-POINT GROUND (GROUND BAR).
• BOND THE EQUIPMENT SINGLE-POINT GROUND TO THE EQUIPMENT GROUND RING WITH MINIMUM 2 CONDUCTORS DIRECTED IN OPPOSITE DIRECTIONS WITH PARALLEL CONNECTIONS TO THE RING.
• IF EQUIPMENT IS ENCLOSED IN A SHELTER:
• IF THE SHELTER IS CONSIDERED TO BE EXPOSED TO A DIRECT LIGHTNING STRIKE, INSTALL A BUILDING LIGHTNING PROTECTION SYSTEM PER APPLICABLE VERSION OF NFPA 780.
• BOND ALL FIXED CONDUCTIVE BUILDING COMPONENTS TOGETHER AND TO THE BUILDING RING GROUND AT THE CORNERS. THIS IS TYPICALLY CALLED THE HALO GROUND. DO NOT BOND EQUIPMENT TO THE HALO GROUND.
• BOND ALL EQUIPMENT TOGETHER TO A SINGLE-POINT OR INTERIOR EQUIPMENT RING GROUND (EGR). BOND THE SINGLE-POINT OR EGR TO THE EXTERNAL EQUIPMENT RING GROUND.
• PLACE GROUND RODS AT THE EQUIPMENT GROUND RING CORNERS GROUND RODS.

• SEPARATION SPACE BETWEEN ANY 2 GROUND RODS SHALL BE NO CLOSER THAN THEIR DEPTH. THIS APPLIES TO ALL RODS IN THE COMPLETE SYSTEM.
• DRIVE VERTICALLY IN UNDISTURBED SOIL WITH THE TOP AT SAME DEPTH AS THE IN-GROUND CONDUCTOR. IF NOT POSSIBLE TO INSTALL VERTICALLY, PLACE AS CLOSE TO VERTICAL AS POSSIBLE AND IN A DIRECTION AWAY FROM THE NEAREST ABOVE-GROUND CONDUCTIVE ELEMENT (TOWER, EQUIPMENT, ETC.)
RADIALS (TYP. NEW DEDICATED COMMUNICATION STRIPS):
• WHERE FEASIBLE WITH ENOUGH SPACE AVAILABLE, INSTALL A MINIMUM OF A MAXIMUM 10 RING RADIALS.
• EACH RADIALS LENGTH SHALL BE MIN 20 FT, MAX 80 FT.
• EXTEND RADIALS PERPENDICULAR FROM RINGS IN AS STRAIGHT LINE AS POSSIBLE, AWAY FROM OTHER RING GROUNDS, RADIALS, BONDS, AND SIMILAR.
• A COMMON PRACTICE IS TO PLACE 4 RADIALS FROM THE TOWER RING TO THE 4 CORNERS OF THE AVAILABLE AREA.

AT A MINIMUM, BOND ALL COMPOUND CONDUCTIVE FENCE CORNER POSTS AND GATE POSTS TO THE LPGS. PREFERABLY, INSTALL A GROUND RING THAT FOLLOWS THE FENCE LINE, BONDING ALL POSTS TO THE RING.

27 ANTENNAS & CABLES:
THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS HEREIN.
THE CONTRACTOR SHALL FURNISH AND INSTALL ALL TRANSMISSION CABLES, JUMPERS, CONNECTORS, GROUNDING STRAPS, ANTENNAS, MOUNT AND HARDWARE. ALL MATERIALS SHALL BE INSPECTED BY THE CONTRACTOR FOR DAMAGE UPON DELIVERY. JUMPERS SHALL BE SUPPLIED AT ANTENNAS AND EQUIPMENT INSIDE SHELTER. COORDINATE LENGTH OF JUMPER CABLES WITH OWNER. COORDINATE AND VERIFY ALL OF THE MATERIALS TO BE PROVIDED WITH OWNER PRIOR TO SUBMITTING BID AND ORDERING MATERIALS.

ANTENNAS: BOTH SIDES OF EQUIPMENT SHELTER WALL, AND JUMPER CABLES AT THE EQUIPMENT.
CONTRACTOR SHALL FURNISH AND INSTALL ALL CONNECTORS, ASSOCIATED CABLE MOUNTING AND GROUNDING HARDWARE, WALL MOUNTS, STANDOFFS, AND ALL ASSOCIATED HARDWARE TO INSTALL

ALL CABLES AND ANTENNAS TO THE MANUFACTURERS AND OWNERS SPECIFICATIONS.
ANTENNA CABLES SHALL BE FOAM DIELECTRIC COAXIAL CABLES AS FOLLOWS:
• BASE STATION ANTENNAS:
• 7/8" DIAMETER FOR CABLE LENGTHS UP TO 100 FT.
• 1-5/8" DIAMETER FOR CABLE LENGTHS GREATER THAN 100 FT.
• GPS ANTENNAS:
• 7/8" DIAMETER FOR CABLE LENGTHS UP TO 200 FT.
• 1-5/8" DIAMETER FOR CABLE LENGTHS GREATER THAN 200 FT.

MINIMUM BENDING RADIUS FOR COAXIAL CABLES SHALL BE:
• 15 FT FOR 7/8" COAXIAL CABLES.
• 25 FT FOR 1-5/8" COAXIAL CABLES.
CABLE SHALL BE INSTALLED WITH A MINIMUM NUMBER OF BENDS WHERE POSSIBLE. CABLE SHALL NOT BE LEFT UNTERMINATED AND SHALL BE SEALED IMMEDIATELY AFTER BEING INSTALLED.
ALL EXTERIOR CABLE CONNECTIONS SHALL BE COVERED WITH A WATERPROOF SPLICING KIT.

CONTRACTOR SHALL VERIFY EXACT LENGTH AND DIRECTION OF TRAVEL IN FIELD PRIOR TO CONSTRUCTION.
CABLE SHALL BE FURNISHED AND INSTALLED WITHOUT SPLICES AND WITH CONNECTORS AT EACH END.
27 CABLE TRAY:
THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS HEREIN.
CABLE TRAY SHALL BE MADE OF EITHER CORROSION RESISTANT METAL OR WITH A CORROSION RESISTANT FINISH.
CABLE TRAY SHALL BE USE OF LADDER TRAY TYPE WITH FLAT COVER CLAMPED TO SIDE RAILS.
CABLE LADDER SHALL BE SIZED TO FIT ALL CABLES IN ACCORDANCE WITH NEC AND NEMA 11-15-84.
CABLE LADDER TRAYS SHALL BE NEMA CLASS 12A BY PV INDUSTRIES, INC. OR EQUAL.

CABLE LADDER TRAY SHALL BE SUPPORTED IN ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS.
ALL WORKMANSHIP SHALL CONFORM TO THESE REQUIREMENTS AND ALL LOCAL CODES AND STANDARDS TO ENSURE SAFE AND ADEQUATE GROUNDING SYSTEM.

CONSTRUCTION DOCUMENTS

NO	DATE	REVISION
0	02/19/21	FOR REVIEW: JRM
1	05/20/21	FOR FILING: JRM
2		
3		
4		
5		
6		

DESIGN PROFESSIONALS OF RECORD

PROF: MICHAEL S. TRODDEN P.E.
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GLASTONBURY WEST CT

SITE ADDRESS: 2577 MAIN STREET GLASTONBURY CT, 06033

APT FILING NUMBER: CT141_11930

DRAWN BY: DRA CHECKED BY: JRM

VZ PROJECT CODE: 20202199190

VZ LOCATION CODE: 468258

VZ FUZE ID: 16232043

SHEET TITLE:

NOTES & SPECIFICATIONS

SHEET NUMBER:

N-1

Cellco Partnership db/a



20 ALEXANDER DRIVE
WALLINGFORD, CT 06492

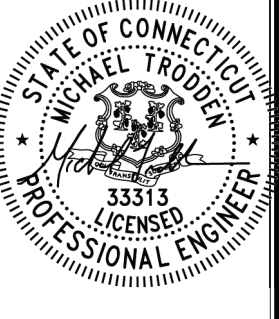


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CONSTRUCTION DOCUMENTS

NO	DATE	REVISION
0	02/19/21	FOR REVIEW: JRM
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3		
4		
5		
6		



STATE OF CONNECTICUT
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33313
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DESIGN PROFESSIONALS OF RECORD

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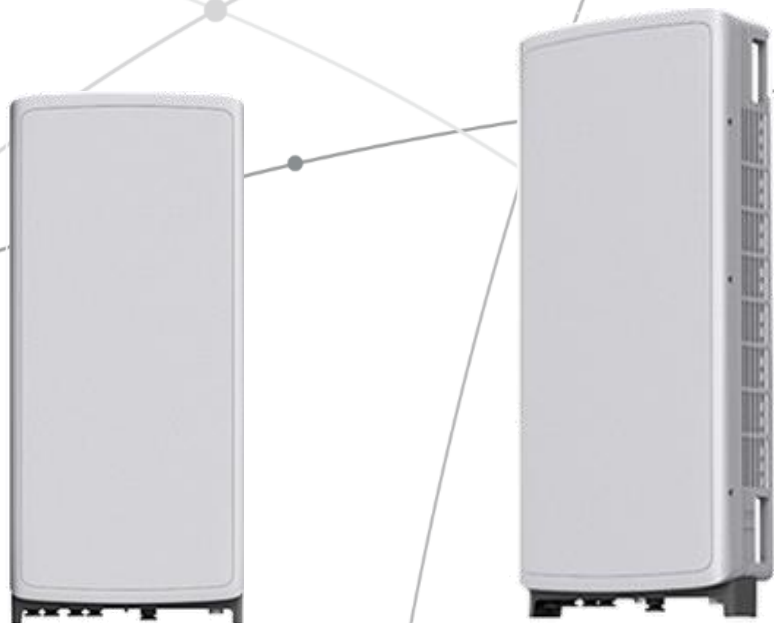
N-1

SAMSUNG C-Band 64T64R Massive MIMO Radio

for High Capacity and Wide Coverage

Samsung C-Band 64T64R Massive MIMO Radio enables mobile operators to increase coverage range, boost data speeds and ultimately offer enriched 5G experiences to users in the U.S..

Model Code : MT6407-77A



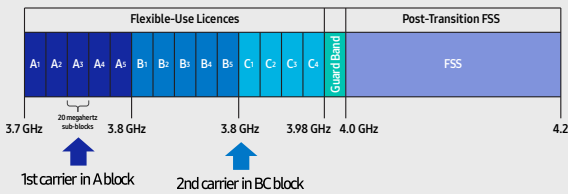
Points of Differentiation

Wide Bandwidth

With capability to support up to 2 CC carrier configuration, Samsung C-Band massive MIMO Radio supports 200 MHz bandwidth in the C-Band spectrum.

Samsung C-Band massive MIMO Radio covers the entire C-Band 280 MHz spectrum, so it can meet the operator's needs in current A block and future B/C blocks

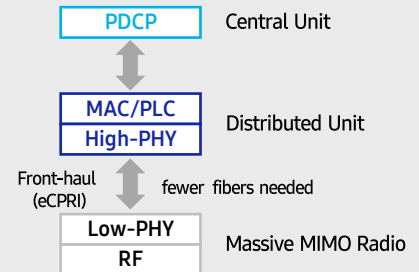
C-Band spectrum supported by Massive MIMO Radio



Future Proof Product

Samsung C-Band 64T64R Massive MIMO radio supports not only CPRI but also eCPRI as front-haul interface.

It enables operators can cut down on OPEX/CAPEX by reducing front-haul bandwidth through low layer split and using ethernet based higher efficient line.

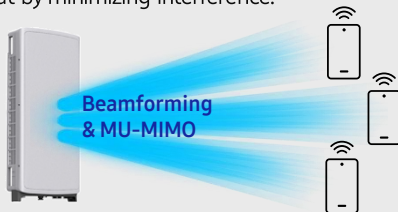


Enhanced Performance

C-Band massive MIMO Radio creates sharp beams and extends networks' coverage on the critical mid-band spectrum using a large number of antenna elements and high output power to boost data speeds.

This helps operators reduce their CAPEX as they now need less products to cover the same area than before.

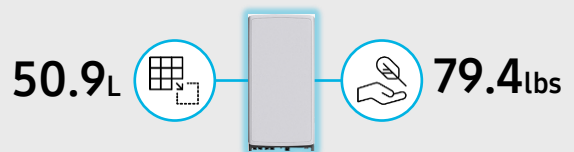
Furthermore, as C-Band massive MIMO Radio supports MU-MIMO (Multi-user MIMO), it enables to increase user throughput by minimizing interference.



Well Matched Design

Samsung C-Band Massive MIMO radio utilizes 64 antennas, supports up to 280MHz bandwidth, and delivers a 200W output power. despite the above advanced performance, the Radio has a compact size of 50.9L and 79.4lbs. This makes it easy to install the Radio.

It is designed to look solid and compact, with a low profile appearance so that, when installed, harmonizes well with the surrounding environment.



Technical Specifications

Item	Specification
Tech	NR
Band	n77
Frequency Band	3700 - 3980 MHz
EIRP	78.5dBm (53.0 dBm+25.5 dBi)
IBW/OBW	280 MHz / 200 MHz
Installation	Pole/Wall
Size/ Weight	16.06 x 35.06 x 5.51 inch (50.86L)/ 79.4 lbs



SAMSUNG



About Samsung Electronics Co., Ltd.

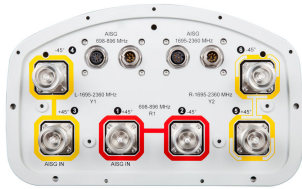
Samsung inspires the world and shapes the future with transformative ideas and technologies. The company is redefining the worlds of TVs, smartphones, wearable devices, tablets, digital appliances, network systems, and memory, system LSI, foundry and LED solutions.

129 Samsung-ro, Yeongtong-gu, Suwon-si Gyeonggi-do, Korea

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NHH-65B-R2B



6-port sector antenna, 2x 698–896 and 4x 1695–2360 MHz, 65° HPBW, 2x RET. Both high bands share the same electrical tilt.

- Interleaved dipole technology providing for attractive, low wind load mechanical package
- Internal SBT on low and high band allow remote RET control from the radio over the RF jumper cable
- Separate RS-485 RET input/output for low and high band
- One RET for low band and one RET for both high bands to ensure same tilt level for 4x Rx or 4x MIMO

General Specifications

Antenna Type	Sector
Band	Multiband
Color	Light gray
Effective Projective Area (EPA), frontal	0.26 m ² 2.799 ft ²
Effective Projective Area (EPA), lateral	0.22 m ² 2.368 ft ²
Grounding Type	RF connector body grounded to reflector and mounting bracket
Performance Note	Outdoor usage Wind loading figures are validated by wind tunnel measurements described in white paper WP-112534-EN
RF Connector Interface	7-16 DIN Female
RF Connector Location	Bottom
RF Connector Quantity, high band	4
RF Connector Quantity, low band	2
RF Connector Quantity, total	6

Remote Electrical Tilt (RET) Information, General

RET Interface	8-pin DIN Female 8-pin DIN Male
RET Interface, quantity	2 female 2 male

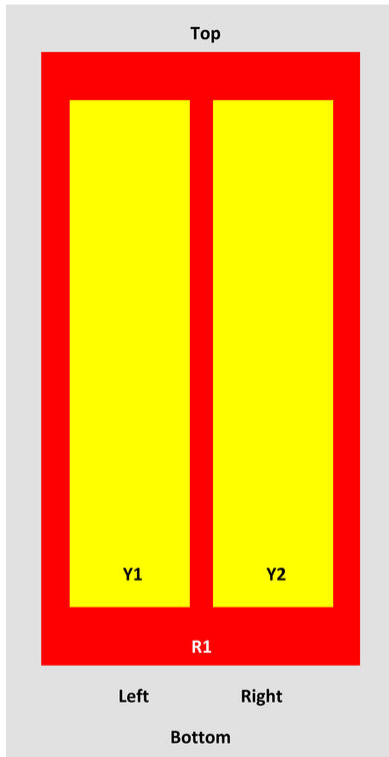
Dimensions

Width	301 mm 11.85 in
Length	1828 mm 71.969 in
Depth	180 mm 7.087 in

Array Layout

NHH-65B-R2B

NHH



Array	Freq (MHz)	Conns	RET (SRET)	AISG RET UID
R1	698-896	1-2	1	ANXXXXXXXXXXXXXXXXX1
Y1	1695-2360	3-4	2	ANXXXXXXXXXXXXXXXXX2
Y2	1695-2360	5-6		

View from the front of the antenna
(Sizes of colored boxes are not true depictions of array sizes)

Electrical Specifications

Impedance	50 ohm
Operating Frequency Band	1695 – 2360 MHz 698 – 896 MHz
Total Input Power, maximum	900 W @ 50 °C

Remote Electrical Tilt (RET) Information, Electrical

Protocol	3GPP/AISG 2.0 (Single RET)
Power Consumption, idle state, maximum	2 W
Power Consumption, normal conditions, maximum	13 W
Input Voltage	10–30 Vdc
Internal Bias Tee	Port 1 Port 3
Internal RET	High band (1) Low band (1)

NHH-65B-R2B

Electrical Specifications

Frequency Band, MHz	698–806	806–896	1695–1880	1850–1990	1920–2200	2300–2360
Gain, dBi	14.9	15	17.7	17.9	18.4	18.7
Beamwidth, Horizontal, degrees	65	60	71	69	64	57
Beamwidth, Vertical, degrees	12.4	11.2	5.7	5.2	4.9	4.6
Beam Tilt, degrees	0–14	0–14	0–7	0–7	0–7	0–7
USLS (First Lobe), dB	13	14	18	18	19	18
Front-to-Back Ratio at 180°, dB	30	29	31	30	29	31
Isolation, Cross Polarization, dB	25	25	25	25	25	25
Isolation, Inter-band, dB	30	30	30	30	30	30
VSWR Return loss, dB	1.5 14.0	1.5 14.0	1.5 14.0	1.5 14.0	1.5 14.0	1.5 14.0
PIM, 3rd Order, 2 x 20 W, dBc	-153	-153	-153	-153	-153	-153
Input Power per Port at 50° C, maximum, watts	300	300	300	300	300	300

Electrical Specifications, BASTA

Frequency Band, MHz	698–806	806–896	1695–1880	1850–1990	1920–2200	2300–2360
Gain by all Beam Tilts, average, dBi	14.5	14.5	17.3	17.7	18.1	18.5
Gain by all Beam Tilts Tolerance, dB	±0.6	±1.1	±0.4	±0.4	±0.5	±0.3
Gain by Beam Tilt, average, dBi	0° 14.4 7° 14.6 14° 14.3	0° 14.7 7° 14.7 14° 14.1	0° 17.2 4° 17.3 7° 17.3	0° 17.6 4° 17.7 7° 17.7	0° 18.0 4° 18.2 7° 18.1	0° 18.3 4° 18.5 7° 18.6
Beamwidth, Horizontal Tolerance, degrees	±2	±2.1	±3	±4.1	±6.5	±2.9
Beamwidth, Vertical Tolerance, degrees	±0.7	±0.7	±0.3	±0.2	±0.3	±0.2
USLS, beampeak to 20° above beampeak, dB	13	14	16	16	17	15
Front-to-Back Total Power at 180° ± 30°, dB	23	22	27	27	25	25
CPR at Boresight, dB	22	21	23	23	22	19
CPR at Sector, dB	10	7	16	13	11	4

Material Specifications

Radiator Material

Low loss circuit board

NHH-65B-R2B

Reflector Material Aluminum

Mechanical Specifications

Wind Loading at Velocity, frontal 278.0 N @ 150 km/h | 63.6 lbf @ 150 km/h
Wind Loading at Velocity, lateral 230.0 N @ 150 km/h | 51.7 lbf @ 150 km/h
Wind Loading at Velocity, maximum 120.7 lbf @ 150 km/h | 537.0 N @ 150 km/h
Wind Speed, maximum 241 km/h | 149.75 mph

Packaging and Weights

Width, packed 409 mm | 16.102 in
Depth, packed 299 mm | 11.772 in
Length, packed 1952 mm | 76.85 in
Net Weight, without mounting kit 19.8 kg | 43.651 lb
Weight, gross 32.3 kg | 71.209 lb

Regulatory Compliance/Certifications

Agency	Classification
CHINA-ROHS	Below maximum concentration value
ISO 9001:2015	Designed, manufactured and/or distributed under this quality management system
REACH-SVHC	Compliant as per SVHC revision on www.commscope.com/ProductCompliance
ROHS	Compliant



Included Products

BSAMNT-3 — Wide Profile Antenna Downtilt Mounting Kit for 2.4 - 4.5 in (60 - 115 mm) OD round members. Kit contains one scissor top bracket set and one bottom bracket set.

* Footnotes

Performance Note Severe environmental conditions may degrade optimum performance

SAMSUNG

Dual-Band Radio Unit AWS/PCS (B66/B2)

RFV01U-D1A

Samsung's RFV01U-D1A is a compact remote Radio Unit (RU) designed for deployments that require flexibility in installation and rapid onlining, without compromising on coverage, capacity or operational expenses.



The RFV01U-D1A RU targets dual-band support across Band 66 (AWS) and Band 2 (PCS), making it an ideal product for broad coverage footprints across multiple common mid-range frequencies.

The RU handles all Radio Frequency (RF) processing in a single, compact unit, and is designed to interface via CPRI with Samsung's CDU baseband offerings, in both distributed- and central-RAN configurations.

In addition to its minimal footprint and ease of installation, the RU is also designed to reduce cost of ownership through its integrated spectrum analyzer, which allows for remote RF monitoring, greatly reducing the need for on-site maintenance visits.

Features and Benefits

- Dual-band support for broad frequency coverage
- Minimal footprint reduces site costs
- Rapid, easy installation
- Flexibly deployable in any location
- Remote RF monitoring capability
- Convection cooled, silent operation
- Built-in Broadcast Auxiliary Services (BAS) filter ensures compliant AWS operation without impacting footprint

Key Technical Specifications

Duplex Type: FDD

Operating Frequencies:

B66: DL(2,110-2,180MHz)/UL(1,710-1,780MHz)

B2: DL(1,930-1,990MHz)/UL(1,850-1,910MHz)

Instantaneous Bandwidth:

70MHz(B66) + 60MHz(B2)

RF Chain: 4T4R/2T4R/2T2R

Output Power: Total 320W

DU-RU Interface: CPRI (10Gbps)

Dimensions: 380 x 380 x 255mm (36.8L)

Weight: 38.3kg

Input Power: -48V DC

Operating Temp.: -40 - 55°(w/o solar load)

Cooling: Natural convection

SAMSUNG

Dual-Band Radio Unit 700/850MHz (B13/B5) RFV01U-D2A

Samsung's RFV01U-D2A is a compact remote Radio Unit (RU) designed for deployments that require flexibility in installation and rapid onlining, without compromising on coverage, capacity or operational expenses.



The RFV01U-D2A RU targets dual-band support across Band 13 (700MHz) and Band 5 (850MHz), making it an ideal product for broad coverage footprints across multiple common low-end, long-range frequencies.

The RU handles all Radio Frequency (RF) processing in a single, compact unit, and is designed to interface via CPRI with Samsung's CDU baseband offerings, in both distributed- and central-RAN configurations.

In addition to its minimal footprint and ease of installation, the RU is also designed to reduce cost of ownership through its integrated spectrum analyzer, which allows for remote RF monitoring, greatly reducing the need for on-site maintenance visits.

Features and Benefits

- Dual-band support for broad frequency coverage
- Minimal footprint reduces site costs
- Rapid, easy installation
- Flexibly deployable in any location
- Remote RF monitoring capability
- Convection cooled, silent operation

Key Technical Specifications

Duplex Type: FDD
Operating Frequencies:
B13: DL(746-756MHz)/UL(777-787MHz)
B5: DL(869-894MHz)/UL(824-849MHz)
Instantaneous Bandwidth: 10MHz(B13) + 25MHz(B5)
RF Chain: 4T4R/2T4R/2T2R
Output Power: Total 320W
DU-RU Interface: CPRI (10Gbps)
Dimensions: 380 x 380 x 207mm (29.9L)
Weight: 31.9kg
Input Power: -48V DC
Operating Temp.: -40 - 55°(w/o solar load)
Cooling: Natural convection

ATTACHMENT 3

	General	Power	Density					
Site Name: Glastonbury W								
Tower Height: Verizon @ 80ft								
CARRIER	# OF CHAN.	WATTS ERP	HEIGHT	CALC. POWER DENS	FREQ.	MAX. PERMISS. EXP.	FRACTION MPE	Total
*T-Mobile	1	6444	93	2500	0.3062	1.0000	3.06%	
*T-Mobile	1	6444	93	2500	0.3062	1.0000	3.06%	
*T-Mobile	2	592	93	600	0.0563	0.4000	1.41%	
*T-Mobile	1	1578	93	600	0.0750	0.4000	1.87%	
*T-Mobile	2	649	93	700	0.0617	0.4667	1.32%	
*T-Mobile	2	2204	93	1900	0.2094	1.0000	2.09%	
*T-Mobile	2	1295	93	2100	0.1231	1.0000	1.23%	
*T-Mobile	4	1028	93	1900	0.1954	1.0000	1.95%	
*T-Mobile	2	2057	93	1900	0.1955	1.0000	1.95%	
*T-Mobile	2	2308	93	2100	0.2193	1.0000	2.19%	
*MetroPCS CDMA	3	727	100	2135	0.0888	1.0000	0.89%	
*MetroPCS LTE	1	1200	100	2130	0.0488	1.0000	0.49%	
*Clearwire	2	153	128	2496	0.0074	1.0000	0.07%	
*Clearwire	1	211	124	18 GHz	0.0054	1.0000	0.05%	
*Nextel	9	100	128	851	0.0217	0.5673	0.38%	
*AT&T-UMTS	2	400	110	850	0.0266	0.5667	0.47%	
*AT&T-UMTS	4	1194	110	1900	0.1588	1.0000	1.59%	
*AT&T-UMTS	4	1194	110	1900	0.1588	1.0000	1.59%	
*AT&T-PCS-UMTS	4	1236	110	2300	0.1644	1.0000	1.64%	
*AT&T-GSM	2	736	110	700	0.0489	0.4667	1.05%	
*AT&T-PCS-LTE	2	885	110	850	0.0589	0.5667	1.04%	
*AT&T-LTE	4	1181	110	2100	0.1571	1.0000	1.57%	
VZW 700	4	685	80	0.0154	751	0.5007	3.08%	
VZW Cellular	4	690	80	0.0155	874	0.5827	2.66%	
VZW PCS	4	1469	80	0.0330	1975	1.0000	3.30%	
VZW AWS	4	1652	80	0.0371	2120	1.0000	3.71%	
VZW CBAND	4	6531	80	0.1468	3730.005	1.0000	14.68%	
								58.42%
* Source: Siting Council								

ATTACHMENT 4



Tower Engineering Solutions

Phone (972) 483-0607, Fax (972) 975-9615
1320 Greenway Drive, Suite 600, Irving, Texas 75038

Structural Analysis Report

Existing 130 ft Nudd Corporation Self Supporting Tower

Customer Name: SBA Communications Corp

Customer Site Number: CT46126-A

Customer Site Name: Glastonbury-main St.

Carrier Name: Verizon (App#: 150210, v1)

Carrier Site ID / Name: 468258 / Glastonbury West CT

Site Location: 2577 Main Street

Glastonbury, Connecticut

Hartford County

Latitude: 41.714389

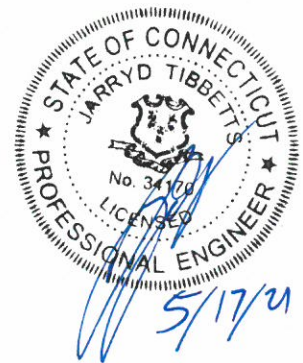
Longitude: -72.613028

Analysis Result:

Max Structural Usage: 99.8% [Pass]

Max Foundation Usage: 80% [Pass]

Additional Usage Caused by New Mount/Mount Modification: +2%



Report Prepared By: Ram Kodali



Tower Engineering Solutions

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Analysis Result:

Max Structural Usage: 99.8% [Pass]

Max Foundation Usage: 80% [Pass]

Additional Usage Caused by New Mount/Mount Modification: +2%

Report Prepared By: Ram Kodali

Introduction

The purpose of this report is to summarize the analysis results on the 130 ft Nudd Corporation Self Supporting Tower to support the proposed antennas and transmission lines in addition to those currently installed. Any modification listed under Sources of Information was assumed completed and was included in this analysis.

Sources of Information

Tower Drawings	Fred A Nudd Corp., Project # 6893B, dated 7/30/02
Foundation Drawing	Fred A Nudd Corp., Dwg # 99-6893-2, dated 9/16/99
Geotechnical Report	Tectonic Engineering Consultants P.C, Project # 1170.C057, dated 8/26/99
Modification Drawings	TowerCo, Job # 090403.05, dated 8/12/09; FDH, Project # 1338401400, dated 6/17/13; FDH, Project # 13SB5C1400, dated 9/10/13
Mount Analysis	Maser Consulting, Job # 20777629A, dated 3/9/21

Analysis Criteria

The rigorous analysis was performed in accordance with the requirements and stipulations of the TIA-222-G-2. In accordance with this standard, the structure was analyzed using **TESTowers**, a proprietary analysis software. The program considers the structure as an elastic 3-D model with second-order effects and temperature effects incorporated in the analysis. The analysis was performed using multiple wind directions.

Wind Speed Used in the Analysis:	Ultimate Design Wind Speed $V_{ult} = 124$ mph (3-Sec. Gust)/ Nominal Design Wind Speed $V_{asd} = 97$ mph (3-Sec. Gust)
Wind Speed with Ice:	50 mph (3-Sec. Gust) with 1" radial ice concurrent
Operational Wind Speed:	60 mph + 0" Radial ice
Standard/Codes:	TIA-222-G-2 / 2015 IBC / 2018 Connecticut State Building Code
Exposure Category:	C
Structure Class:	II
Topographic Category:	1
Crest Height:	0 ft
Seismic Parameters:	$S_S = 0.18, S_1 = 0.063$

This structural analysis is based upon the tower being classified as a Structure Class II; however, if a different classification is required subsequent to the date hereof, the tower classification will be changed to meet such requirement and a new structural analysis will be run.

Existing Antennas, Mounts and Transmission Lines

The table below summarizes the antennas, mounts and transmission lines that were considered in the analysis as existing on the tower.

Items	Elevation (ft)	Qty.	Antenna Descriptions	Mount Type & Qty.	Transmission Lines	Owner
1	120.0	3	APXVSPP18-C-A20 - Panel	(3) Sector Frame	(4) 1-1/4" Fiber	Sprint Nextel
2		3	APXVTM14-C-120 - Panel			
3		4	ACU-A20-N RET			
4		3	1900 4X45 65 MHz RRU			
5		3	800 MHz 2X50W RRU			
6		3	TD-RRH8x20-25			
7		3	800 MHz filter			
8	118.5	3	800 MHz filter	(3) T-Frames	(18) 1 1/4" (2) 1/2" Fiber (4) 3/4" DC (18) 3/8" in (1) 3" Conduit	AT&T
9	110.0	3	Kathrein 800 10121 - Panel			
10		2	CCI HPA-65R-BUU-H6 - Panel			
11		1	CCI HPA-65R-BUU-H8 - Panel			
12		2	CCI HPA65R-BU6A - Panel			
13		1	CCI HPA65R-BU8A - Panel			
14		2	Kathrein 800 10965 - Panel			
15		1	Kathrein 800 10966 - Panel			
16		6	Powerwave LGP21401			
17		6	Powerwave LGP21901			
18		12	Kathrein 860 10025			
19		3	Ericsson RRUS-32			
20		3	Ericsson 4449 B5/B12			
21		3	Ericsson RRUS 8843 B2 B66A			
22		2	Raycap DC6-48-60-18-8F			
23	3	Andrew ATSBT-TOP-MF				
24	93.0	3	APXVAARR24_43-U-NA20 - Panel	(3) T-Frames w/ (3)2" x- strong pipe & SitePro1 SFS-V Stabilizer kit	(10) 1 5/8" (1) 1 5/8" Hybrid (2) 1-1/4" Hybrid	T-Mobile
25		3	Ericsson AIR6449 B41 - Panel			
26		3	AIR 32 KRD9011461-B66A - Panel			
27		3	Ericsson KRY 112 144/2			
28		3	Commscope SDX1926Q-43			
29		3	Ericsson 4449 B71+B85			
30		3	Ericsson 4415 B25			
-	80.0	6	Andrew HBXX-6517DS-A2M - Panel	(3) Sector Frame	(2) 1 5/8" Hybrid	Verizon*
-		6	Andrew LNX-6514DS-A1M - Panel			
-		6	ALU RRH2x40-700			
-		3	ALU RRH2x40-AWS			
-		3	ALU RRH2x60-PCS			
-		2	RFS DB-T1-6Z-8AB-0Z			

*Verizon's existing loads are based on the current collocation form.

Proposed Carrier's Final Configuration of Antennas, Mounts and Transmission Lines

Information pertaining to the proposed carrier's final configuration of antennas and transmission lines was provided by SBA Communications Corp. The proposed antennas and lines are listed below.

Items	Elevation (ft)	Qty.	Antenna Descriptions	Mount Type & Qty.	Transmission Lines	Owner
31	80.0	6	Commscope NHH-65B-R2B - Panel	(3) Sector Frame w/ (3) BASMNT-SBS-1-2 & (3) VZSMART-SFK1 / VZSMART-SFK-3	(2) 1 5/8" Hybrid	Verizon
32		3	Samsung 64T64R - Panel			
33		3	Andrew LNX-6514DS-A1M - Panel			
34		3	B5/B13 RRH-BR04C (RFV01U-D2A)			
35		2	RFS DB-C1-12C-24AB-OZ			
36		3	B2/B66A RRH-BR049 (RFV01U-D1A)			

See the attached coax layout for the line placement considered in the analysis.

Analysis Results

The results of the structural analysis, performed for the wind and ice loading and antenna equipment as defined above, are summarized as the following:

Tower Component	Legs	Diagonals	Horizontals
Max. Usage:	99.8%	95.8%	34.1%
Pass/Fail	Pass	Pass	Pass

Foundations

	Compression (Kips)	Uplift (Kips)	Shear (Kips)
Analysis Reactions	408.2	379.7	21.3

The foundation has been investigated using the supplied documents and soils report and was found to be adequate. Therefore, no modification to the foundation will be required.

Operational Condition (Rigidity)

Operational characteristics of the tower are found to be within the limits prescribed by TIA-222 for the installed antennas. The maximum twist/sway at the elevation of the proposed equipment is 0.4398 degrees under the operational wind speed as specified in the Analysis Criteria.

Conclusions

Based on the analysis results, the existing structure and its foundation were found to be adequate to safely support the existing and proposed equipment and meet the minimum requirements per the TIA-222 Standard under the design basic wind speed as specified in the Analysis Criteria.

Standard Conditions

1. This analysis was performed based on the information supplied to **(TES) Tower Engineering Solutions, LLC**. Verification of the information provided was not included in the Scope of Work for **TES**. The accuracy of the analysis is dependent on the accuracy of the information provided.
2. The structural analysis was performance based upon the evidence available at the time of this report. All information provided by the client is considered to be accurate.
3. The analyses will be performed based on the codes as specified by the client or based on the best knowledge of the engineering staff of **TES**. In the absence of information to the contrary, all work will be performed in accordance with the latest relevant revision of ANSI/TIA-222. If wind speed and/or ice loads are different from the minimum values recommended by the ANSI/TIA-222 standard or other codes, **TES** should be notified in writing and the applicable minimum values provided by the client.
4. The configuration of the existing mounts, antennas, coax and other appurtenances were supplied by the customer for the current structural analysis. **TES** has not visited the tower site to verify the adequacy of the information provided. If there is any discrepancy found in the report regarding the existing conditions, **TES** should be notified immediately to evaluate the effect of the discrepancy on the analysis results.
5. The client will assume responsibility for rework associated with the differences in initially provided information, including tower and foundation information, existing and/or proposed equipment and transmission lines.
6. If a feasibility analysis was performed, final acceptance of changed conditions shall be based upon a rigorous structural analysis.

Structure: CT46126-A-SBA

Site Name: Glastonbury-main St.

Code: EIA/TIA-222-G

5/17/2021

Type: Self Support

Base Shape: Triangle

Basic WS: 97.00

Height: 130.00 (ft)

Base Width: 7.50

Basic Ice WS: 50.00

Base Elev: 0.00 (ft)

Top Width: 2.50

Operational WS: 60.00

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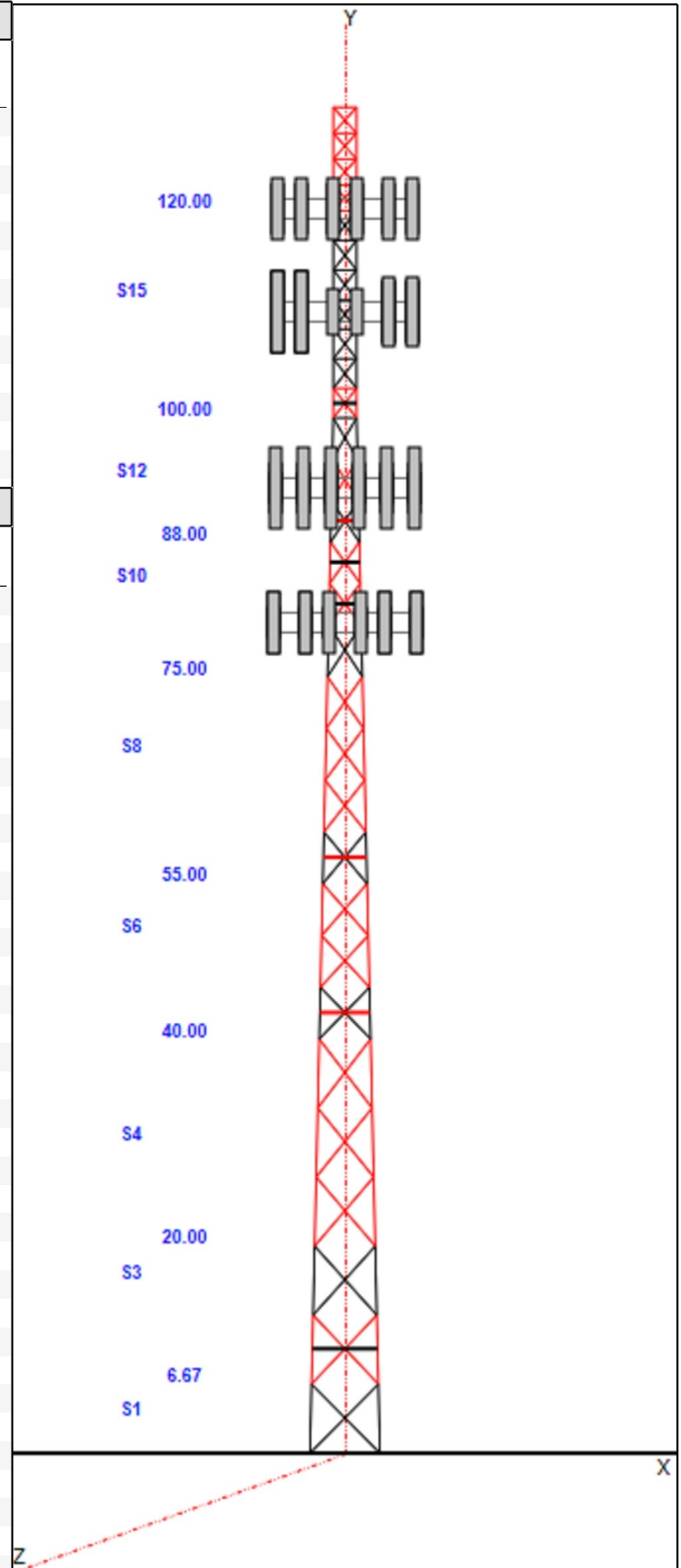


Section Properties

Sect	Leg Members	Diagonal Members	Horizontal Members
1	MOD 6PX+7.625x.301_1	DAE 2X2X0.1875	
2-3	PX 6" DIA PIPE	SAE 2X2X0.1875	
4	PX 6" DIA PIPE	DAE 1.75X1.75X.1875	
5	MOD 6PST+7.625x.301_	SAE 2X2X0.25	
6	MOD 6PST+7.625x.301_	DAE 1.5x1.5x.1875	
7-8	PST 6" DIA PIPE	DAE 1.5x1.5x.1875	
9	PST 6" DIA PIPE	SAE 2X2X0.25	
10	MOD 4"PST+5"PX1/2P	DAE 1.5x1.5x.1875	
11	PST 4" DIA PIPE	DAE 1.5x1.5x.1875	
12	PST 4" DIA PIPE	SAE 2X2X0.25	
13	PST 4" DIA PIPE	SAE 1.5X1.5X0.1875	SAE 1.25x1.25x0.1875
14-15	SOL 2" SOLID	SOL 3/4" SOLID	SAE 1.25x1.25x0.1875
16	SOL 1 1/2" SOLID	SOL 3/4" SOLID	SAE 1.25x1.25x0.1875

Discrete Appurtenances

Attach Elev (ft)	Force Elev (ft)	Qty	Description
130.00	130.00	1	Beacon
120.00	120.00	3	APXVSP18-C-A20
120.00	120.00	3	APXVTM14-C-120
120.00	120.00	4	ACU-A20-N RET
120.00	120.00	3	1900 4X45 65 MHz RRU
120.00	120.00	3	800 MHz 2X50W RRU
120.00	120.00	3	TD-RRH8x20-25
120.00	120.00	3	800 MHz filter
120.00	118.50	3	Sector Frame
118.50	118.50	3	800 MHz filter
110.00	108.50	3	T-Frames
110.00	110.00	3	800 10121
110.00	110.00	2	HPA-65R-BUU-H6
110.00	110.00	1	HPA-65R-BUU-H8
110.00	110.00	2	HPA65R-BU6A
110.00	110.00	1	HPA65R-BU8A
110.00	110.00	2	800 10965
110.00	110.00	1	800 10966
110.00	110.00	6	LGP21401
110.00	110.00	6	LGP21901
110.00	110.00	12	860 10025
110.00	110.00	3	RRUS-32
110.00	110.00	3	4449 B5/B12
110.00	110.00	3	8843 B2 B66A
110.00	110.00	2	DC6-48-60-18-8F
110.00	110.00	3	ATSBT-TOP-MF
93.00	93.00	3	SDX1926Q-43
93.00	93.00	3	4415 B25
93.00	91.50	3	T-Frames
93.00	93.00	3	AIR6449 B41
93.00	93.00	3	AIR 32 KRD9011461-B66A
93.00	93.00	3	APXVAARR24_43-U-NA20
93.00	93.00	3	KRY 112 144/2
93.00	93.00	3	4449 B71+B85
93.00	93.00	1	(3) SitePro1 SFS-V
93.00	93.00	1	(3) 2" X-Strong Pipe



Structure: CT46126-A-SBA

Site Name: Glastonbury-main St.	Code: EIA/TIA-222-G	5/17/2021
Type: Self Support	Base Shape: Triangle	Basic WS: 97.00
Height: 130.00 (ft)	Base Width: 7.50	Basic Ice WS: 50.00
Base Elev: 0.00 (ft)	Top Width: 2.50	Operational WS: 60.00



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80.00	80.00	3	T-Frames
80.00	80.00	6	NHH-65B-R2B
80.00	80.00	3	64T64R
80.00	80.00	3	LNx-6514DS-A1M
80.00	80.00	3	B5/B13 RRH-BR04C (RFV01U-D2A)
80.00	80.00	2	DB-C1-12C-24AB-0Z
80.00	80.00	1	(3) HR w/ V-Brace Kits
80.00	80.00	1	(3) Stabilizer Kit
80.00	80.00	3	BSAMNT-SBS-1-2
80.00	80.00	3	B2/B66A RRH-BR049 (RFV01U-D1A)

Linear Appurtenances

Elev From (ft)	Elev To (ft)	Qty	Description
0.00	130.00	1	Climbing Ladder
0.00	130.00	1	Safety Cable
0.00	130.00	1	W/G Ladder
0.00	130.00	2	W/G Ladder
0.00	120.00	4	1-1/4" Fiber
0.00	110.00	18	1 1/4" Coax
0.00	110.00	2	1/2" Fiber
0.00	110.00	1	3" Conduit
0.00	110.00	4	3/4" DC
0.00	93.00	10	1 5/8" Coax
0.00	93.00	1	1 5/8" Hybrid
0.00	93.00	2	1-1/4" Hybrid
0.00	80.00	2	1 5/8" Hybrid

Base Reactions

Leg	Overturning
Max Uplift: -379.74 (kips)	Moment: 2569.85 (ft-kips)
Max Down: 408.21 (kips)	Total Down: 37.66 (kips)
Max Shear: 21.32 (kips)	Total Shear: 31.60 (kips)

Structure: CT46126-A-SBA

Site Name: Glastonbury-main St.

Code: EIA/TIA-222-G

5/17/2021

Type: Self Support

Base Shape: Triangle

Basic WS: 97.00

Height: 130.00 (ft)

Base Width: 7.50

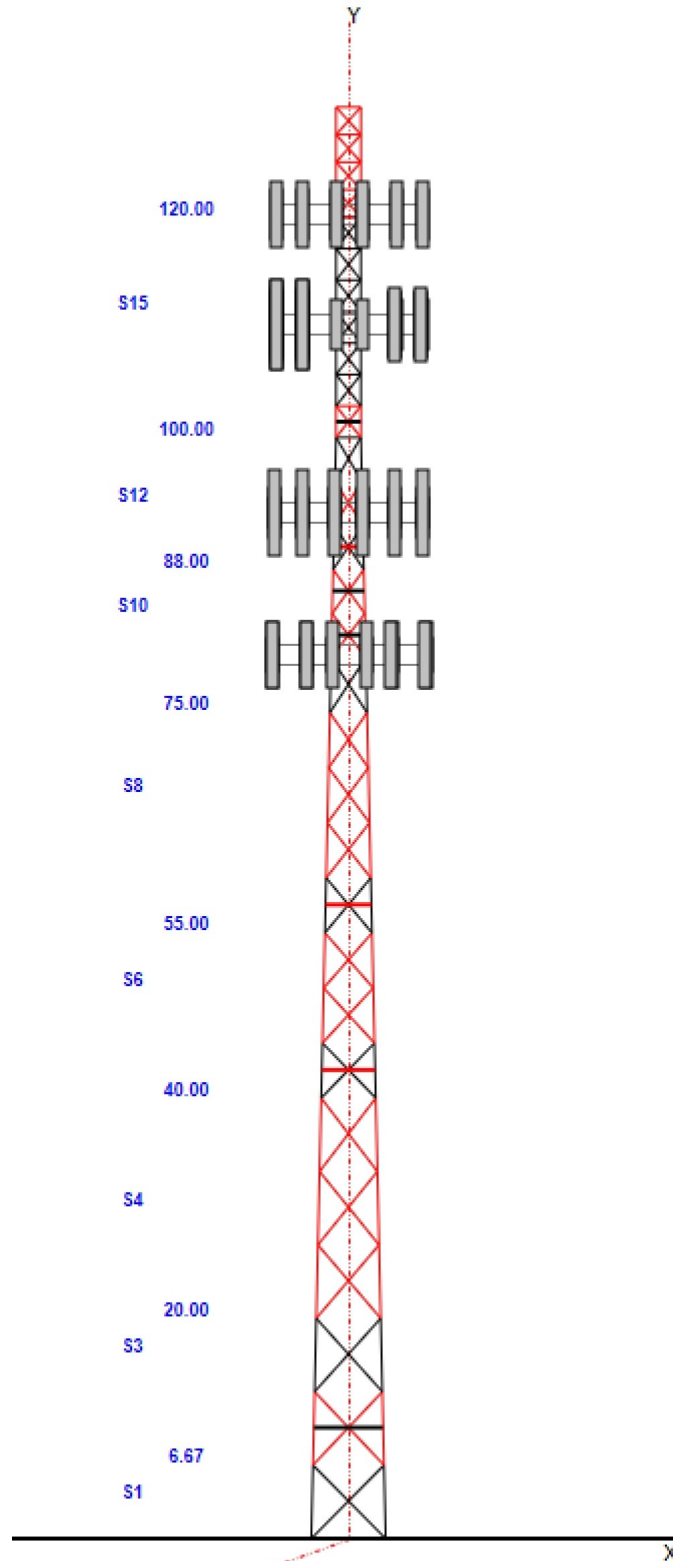
Basic Ice WS: 50.00

Base Elev: 0.00 (ft)

Top Width: 2.50

Operational WS: 60.00

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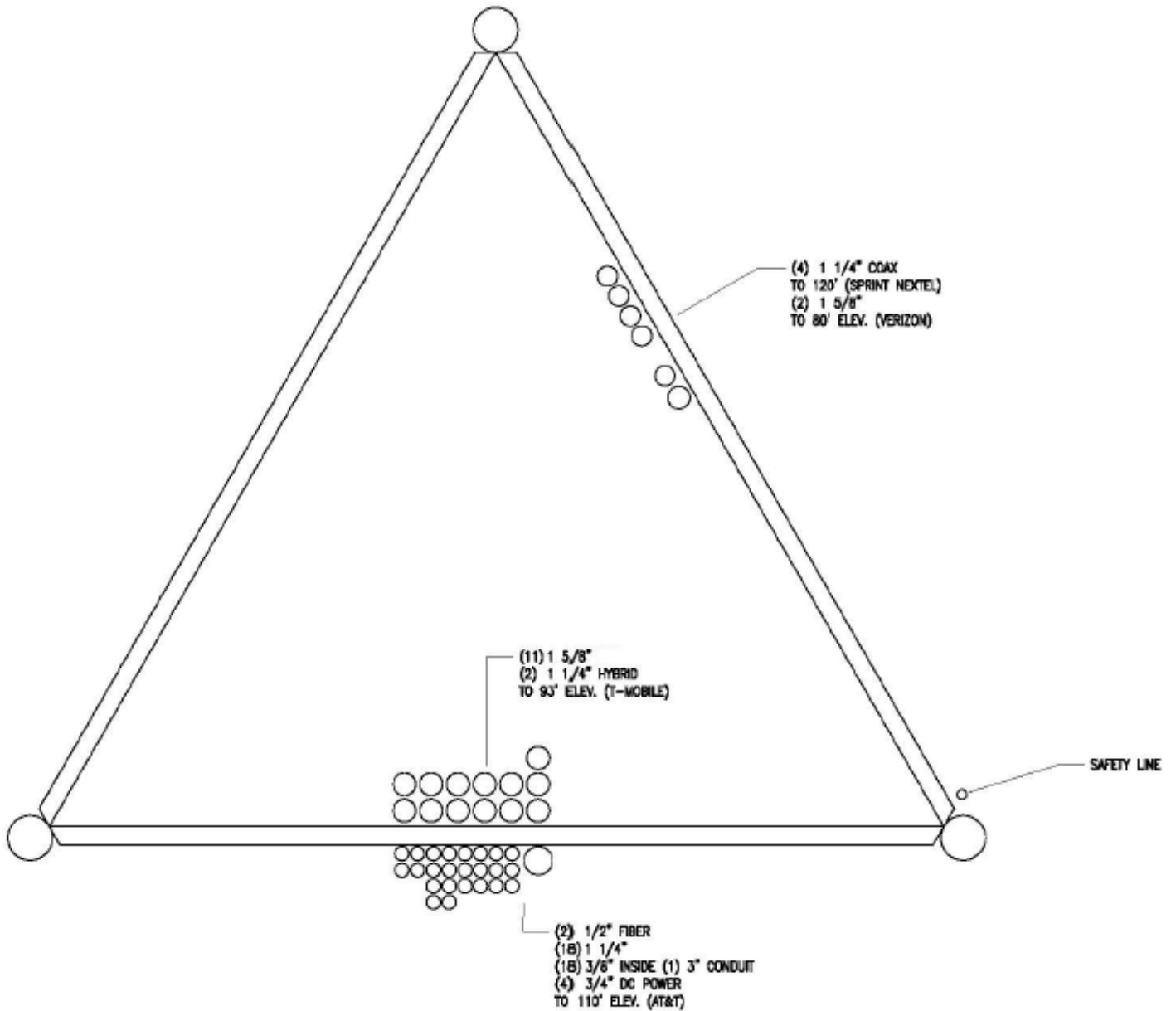
Structure: CT46126-A-SBA - Coax Line Placement

Type: Self Support
Site Name: Glastonbury-main St.
Height: 130.00 (ft)

5/17/2021



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Loading Summary

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



Page: 5

Discrete Appurtenances Properties

Attach Elev (ft)	Description	Qty	No Ice		Ice		Len (in)	Width (in)	Depth (in)	Ka	Orientation Factor	Vert Ecc (ft)
			Weight (lb)	CaAa (sf)	Weight (lb)	CaAa (sf)						
130.00	Beacon	1	36.00	2.720	210.38	3.963	28.000	17.500	17.500	1.00	1.00	0.000
120.00	APXVSP18-C-A20	3	57.00	8.020	330.31	9.728	72.000	11.800	7.000	0.80	0.83	0.000
120.00	APXVTM14-C-120	3	56.00	6.340	276.13	7.808	56.300	12.600	6.300	0.80	0.79	0.000
120.00	ACU-A20-N RET	4	1.00	0.140	6.56	0.524	4.000	2.000	3.500	0.80	0.67	0.000
120.00	1900 4X45 65 MHz RRU	3	60.00	2.710	164.52	4.346	25.100	11.100	10.700	0.80	0.67	0.000
120.00	800 MHz 2X50W RRU	3	64.00	2.400	163.92	3.849	19.000	13.000	12.200	0.80	0.67	0.000
120.00	TD-RRH8x20-25	3	70.00	4.050	221.90	5.126	26.100	18.600	6.700	0.80	0.67	0.000
120.00	800 MHz filter	3	68.30	3.460	185.36	5.161	19.200	18.500	10.400	0.80	0.67	0.000
120.00	Sector Frame	3	450.00	14.000	905.37	23.108	0.000	0.000	0.000	0.75	0.75	-1.500
118.50	800 MHz filter	3	68.30	3.460	185.36	5.161	19.200	18.500	10.400	0.80	0.67	0.000
110.00	T-Frames	3	450.00	14.000	905.37	23.108	0.000	0.000	0.000	0.75	0.75	-1.500
110.00	800 10121	3	46.30	5.150	234.21	6.529	54.500	10.300	5.900	0.80	0.79	0.000
110.00	HPA-65R-BUU-H6	2	51.00	9.660	387.73	11.459	72.000	14.800	9.000	0.80	0.85	0.000
110.00	HPA-65R-BUU-H8	1	68.00	12.980	463.13	15.108	92.400	14.800	7.400	0.80	0.79	0.000
110.00	HPA65R-BU6A	2	43.00	7.860	330.28	9.552	71.200	11.700	8.400	0.80	0.88	0.000
110.00	HPA65R-BU8A	1	54.00	11.230	417.52	13.396	96.000	11.700	7.600	0.80	0.86	0.000
110.00	800 10965	2	108.60	13.810	511.58	15.882	78.700	20.000	6.900	0.80	0.71	0.000
110.00	800 10966	1	125.70	17.360	608.25	19.729	96.000	20.000	6.900	0.80	0.72	0.000
110.00	LGP21401	6	14.10	1.290	46.46	2.372	14.400	9.200	2.600	0.80	0.67	0.000
110.00	LGP21901	6	5.50	0.230	15.45	0.706	4.000	6.000	3.000	0.80	0.67	0.000
110.00	860 10025	12	1.20	0.180	8.96	0.670	7.600	2.400	2.000	0.80	0.67	0.000
110.00	RRUS-32	3	77.00	3.870	233.15	4.362	29.900	13.300	9.500	0.80	0.75	0.000
110.00	4449 B5/B12	3	71.00	1.970	140.09	2.678	17.900	13.200	9.400	0.80	0.67	0.000
110.00	8843 B2 B66A	3	72.00	1.640	132.63	2.283	14.900	13.200	10.900	0.80	0.67	0.000
110.00	DC6-48-60-18-8F	2	31.80	0.920	111.82	1.487	24.000	11.000	11.000	0.90	0.90	0.000
110.00	ATSBT-TOP-MF	3	1.80	0.200	9.36	0.649	5.600	3.700	2.000	0.80	0.67	0.000
93.00	SDX1926Q-43	3	6.10	0.300	26.34	0.567	6.900	4.100	2.900	0.80	0.67	0.000
93.00	4415 B25	3	46.00	1.640	98.08	2.293	15.000	13.200	5.400	0.80	0.67	0.000
93.00	T-Frames	3	450.00	14.000	895.75	22.915	0.000	0.000	0.000	0.75	0.75	-1.500
93.00	AIR6449 B41	3	103.00	5.650	276.72	6.854	33.100	20.500	8.300	0.80	0.71	0.000
93.00	AIR 32 KR09011461-B66A	3	132.20	6.510	375.85	7.956	56.600	12.900	8.700	0.80	0.87	0.000
93.00	APXVAARR24_43-U-NA20	3	128.00	20.240	675.60	22.671	95.900	24.000	7.800	0.80	0.70	0.000
93.00	KRY 112 144/2	3	11.00	0.410	24.66	1.012	6.900	6.100	2.700	0.80	0.67	0.000
93.00	4449 B71+B85	3	74.00	1.650	166.53	2.351	15.000	13.200	9.300	0.80	0.67	0.000
93.00	(3) SitePro1 SFS-V	1	140.00	3.700	362.88	8.609	0.000	0.000	0.000	0.75	1.00	0.000
93.00	(3) 2" X-Strong Pipe	1	650.00	15.500	1684.78	36.063	0.000	0.000	0.000	0.75	1.00	0.000
80.00	T-Frames	3	500.00	17.500	1371.30	34.883	0.000	0.000	0.000	0.75	0.75	0.000
80.00	NHH-65B-R2B	6	43.70	8.080	307.42	9.720	72.000	11.900	7.100	0.80	0.83	0.000
80.00	64T64R	3	87.10	4.700	244.09	5.876	35.120	16.060	5.510	0.80	0.70	0.000
80.00	LNx-6514DS-A1M	3	38.80	8.090	302.52	9.720	72.000	11.900	7.100	0.80	0.80	0.000
80.00	B5/B13 RRH-BR04C (RFV01U-D2A)	3	70.30	1.870	162.49	2.603	15.000	15.000	8.100	0.80	0.67	0.000
80.00	DB-C1-12C-24AB-OZ	2	32.00	4.060	174.20	5.086	29.500	16.500	12.500	0.90	0.90	0.000
80.00	(3) HR w/ V-Brace Kits	1	650.00	15.500	1669.42	35.758	0.000	0.000	0.000	0.75	1.00	0.000
80.00	(3) Stabilizer Kit	1	180.00	6.100	462.30	14.072	0.000	0.000	0.000	0.75	1.00	0.000
80.00	BSAMNT-SBS-1-2	3	25.35	0.100	47.44	0.187	0.000	0.000	0.000	0.75	0.75	0.000
80.00	B2/B66A RRH-BR049	3	84.40	1.870	185.80	2.603	15.000	15.000	10.000	0.80	0.67	0.000
Totals:		136	12,938.55		38,082.20						Number of Appurtenances :	46

Loading Summary

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



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Linear Appurtenances Properties

Elev. From (ft)	Elev. To (ft)	Description	Qty	Width (in)	Weight (lb/ft)	Pct In Block	Spread On Faces	Bundling Arrangement	Cluster Dia (in)	Out of Zone	Spacing (in)	Orientation Factor	Ka Override
0.00	130.00	Climbing Ladder	1	1.50	6.90	100.00	2	Individual NR		N	1.00	1.00	
0.00	130.00	Safety Cable	1	0.38	0.27	100.00	2	Individual NR		N	1.00	1.00	
0.00	130.00	W/G Ladder	1	2.00	6.00	100.00	1	Individual NR		N	1.00	1.00	
0.00	130.00	W/G Ladder	2	2.00	6.00	100.00	2,3	Individual NR		N	1.00	1.00	
0.00	120.00	1-1/4" Fiber	4	1.25	0.95	100.00	2	Individual IR		N	1.00	1.00	
0.00	110.00	1 1/4" Coax	18	1.55	0.66	33.30	1	Block		N	0.50	1.00	
0.00	110.00	1/2" Fiber	2	0.50	0.16	50.00	1	Block		N	1.00	1.00	0
0.00	110.00	3" Conduit	1	3.00	1.78	100.00	1	Individual NR		N	1.00	1.00	
0.00	110.00	3/4" DC	4	0.75	0.40	50.00	1	Block		N	0.50	1.00	
0.00	93.00	1 5/8" Coax	10	1.98	1.04	100.00	1	Block		N	0.50	1.00	0
0.00	93.00	1 5/8" Hybrid	1	2.00	1.10	100.00	1	Individual NR		N	1.00	1.00	
0.00	93.00	1-1/4" Hybrid	2	1.25	0.95	50.00	1	Block		N	0.50	1.00	0
0.00	80.00	1 5/8" Hybrid	2	2.00	1.10	100.00	2	Individual IR		N	1.00	1.00	

Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



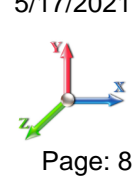
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Load Case: 1.2D + 1.6W Normal Wind	1.2D + 1.6W 97 mph Wind at Normal To Face
Wind Load Factor: 1.60	Wind Importance Factor: 1.00
Dead Load Factor: 1.20	
Ice Dead Load Factor: 0.00	Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Ice		Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)	
											Linear Area (sqft)	Linear Area (sqft)						
1	3.3	17.40	3.092	8.48	0.00	0.22	2.54	1.00	1.00	0.00	7.19	31.79	0.00	1,787.6	0.0	431.82	421.23	853.05
2	10.0	17.40	4.190	7.36	0.00	0.23	2.50	1.00	1.00	0.00	7.99	31.79	0.00	1,402.2	0.0	473.13	421.17	894.30
3	16.7	17.77	2.959	7.36	0.00	0.21	2.55	1.00	1.00	0.00	6.70	31.79	0.00	1,317.9	0.0	412.81	430.11	842.92
4	30.0	20.11	7.367	22.09	0.00	0.22	2.52	1.00	1.00	0.00	18.43	95.38	0.00	4,278.2	0.0	1268.12	1460.23	2,728.36
5	42.5	21.64	3.592	6.36	0.00	0.33	2.22	1.00	1.00	0.00	6.97	23.84	0.00	1,193.2	0.0	454.61	392.83	847.44
6	50.0	22.39	3.218	12.71	0.00	0.28	2.34	1.00	1.00	0.00	9.53	47.69	0.00	2,179.8	0.0	680.35	813.01	1,493.37
7	57.5	23.06	2.325	5.52	0.00	0.30	2.29	1.00	1.00	0.00	5.25	23.84	0.00	895.3	0.0	377.01	418.64	795.66
8	67.5	23.85	4.449	16.57	0.00	0.30	2.30	1.00	1.00	0.00	13.16	71.53	0.00	2,578.0	0.0	980.55	1299.05	2,279.61
9	77.5	24.56	1.875	5.52	0.00	0.35	2.16	1.00	1.00	0.00	4.96	23.84	0.00	833.1	0.0	357.80	445.80	803.60
10	84.0	24.98	3.471	6.50	0.00	0.34	2.20	1.00	1.00	0.00	7.26	35.48	0.00	1,457.2	0.0	543.66	660.25	1,203.91
11	90.0	25.34	1.652	3.00	0.00	0.34	2.18	1.00	1.00	0.00	3.44	17.74	0.00	572.0	0.0	258.59	334.95	593.55
12	94.0	25.58	1.503	3.00	0.00	0.35	2.16	1.00	1.00	0.00	3.30	11.98	0.00	488.5	0.0	247.94	323.28	571.22
13	98.0	25.80	1.320	3.00	0.00	0.36	2.14	1.00	1.00	0.00	3.13	10.06	0.00	435.4	0.0	235.02	321.17	556.19
14	101.4	25.99	0.660	1.39	0.00	0.27	2.38	1.00	1.00	0.00	1.50	7.18	0.00	325.5	0.0	126.35	231.06	357.41
15	111.4	26.51	1.458	8.44	0.00	0.22	2.54	1.00	1.00	0.00	6.40	28.69	0.00	1,677.0	0.0	586.82	975.91	1,562.73
16	125.0	27.16	1.233	4.20	0.00	0.21	2.57	1.00	1.00	0.00	3.69	6.56	0.00	715.4	0.0	350.61	285.33	635.94
													22,136.2	0.0			17,019.24	

Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



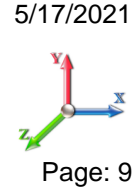
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Load Case: 1.2D + 1.6W 60° Wind	1.2D + 1.6W 97 mph Wind at 60° From Face
Wind Load Factor: 1.60	Wind Importance Factor: 1.00
Dead Load Factor: 1.20	
Ice Dead Load Factor: 0.00	Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Ice		Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)	
											Linear Area (sqft)	Linear Area (sqft)						
1	3.3	17.40	3.092	8.48	0.00	0.22	2.54	0.80	1.00	0.00	6.57	31.79	0.00	1,787.6	0.0	394.68	421.23	815.91
2	10.0	17.40	4.190	7.36	0.00	0.23	2.50	0.80	1.00	0.00	7.16	31.79	0.00	1,402.2	0.0	423.54	421.17	844.70
3	16.7	17.77	2.959	7.36	0.00	0.21	2.55	0.80	1.00	0.00	6.11	31.79	0.00	1,317.9	0.0	376.36	430.11	806.47
4	30.0	20.11	7.367	22.09	0.00	0.22	2.52	0.80	1.00	0.00	16.96	95.38	0.00	4,278.2	0.0	1166.76	1460.23	2,626.99
5	42.5	21.64	3.592	6.36	0.00	0.33	2.22	0.80	1.00	0.00	6.25	23.84	0.00	1,193.2	0.0	407.74	392.83	800.57
6	50.0	22.39	3.218	12.71	0.00	0.28	2.34	0.80	1.00	0.00	8.89	47.69	0.00	2,179.8	0.0	634.42	813.01	1,447.44
7	57.5	23.06	2.325	5.52	0.00	0.30	2.29	0.80	1.00	0.00	4.79	23.84	0.00	895.3	0.0	343.65	418.64	762.30
8	67.5	23.85	4.449	16.57	0.00	0.30	2.30	0.80	1.00	0.00	12.27	71.53	0.00	2,578.0	0.0	914.24	1299.05	2,213.29
9	77.5	24.56	1.875	5.52	0.00	0.35	2.16	0.80	1.00	0.00	4.58	23.84	0.00	833.1	0.0	330.73	445.80	776.53
10	84.0	24.98	3.471	6.50	0.00	0.34	2.20	0.80	1.00	0.00	6.57	35.48	0.00	1,457.2	0.0	491.70	660.25	1,151.95
11	90.0	25.34	1.652	3.00	0.00	0.34	2.18	0.80	1.00	0.00	3.10	17.74	0.00	572.0	0.0	233.72	334.95	568.68
12	94.0	25.58	1.503	3.00	0.00	0.35	2.16	0.80	1.00	0.00	3.00	11.98	0.00	488.5	0.0	225.35	323.28	548.63
13	98.0	25.80	1.320	3.00	0.00	0.36	2.14	0.80	1.00	0.00	2.86	10.06	0.00	435.4	0.0	215.17	321.17	536.34
14	101.4	25.99	0.660	1.39	0.00	0.27	2.38	0.80	1.00	0.00	1.37	7.18	0.00	325.5	0.0	115.24	231.06	346.31
15	111.4	26.51	1.458	8.44	0.00	0.22	2.54	0.80	1.00	0.00	6.11	28.69	0.00	1,677.0	0.0	560.09	975.91	1,536.00
16	125.0	27.16	1.233	4.20	0.00	0.21	2.57	0.80	1.00	0.00	3.44	6.56	0.00	715.4	0.0	327.18	285.33	612.51
													22,136.2	0.0			16,394.63	

Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



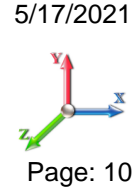
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Load Case: 1.2D + 1.6W 90° Wind	1.2D + 1.6W 97 mph Wind at 90° From Face
Wind Load Factor: 1.60	Wind Importance Factor: 1.00
Dead Load Factor: 1.20	
Ice Dead Load Factor: 0.00	Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	Wind qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Ice		Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
												Linear Area (sqft)	Linear Area (sqft)					
1	3.3	17.40	3.092	8.48	0.00	0.22	2.54	0.85	1.00	0.00	6.73	31.79	0.00	1,787.6	0.0	403.96	421.23	825.19
2	10.0	17.40	4.190	7.36	0.00	0.23	2.50	0.85	1.00	0.00	7.37	31.79	0.00	1,402.2	0.0	435.94	421.17	857.10
3	16.7	17.77	2.959	7.36	0.00	0.21	2.55	0.85	1.00	0.00	6.26	31.79	0.00	1,317.9	0.0	385.47	430.11	815.58
4	30.0	20.11	7.367	22.09	0.00	0.22	2.52	0.85	1.00	0.00	17.33	95.38	0.00	4,278.2	0.0	1192.10	1460.23	2,652.34
5	42.5	21.64	3.592	6.36	0.00	0.33	2.22	0.85	1.00	0.00	6.43	23.84	0.00	1,193.2	0.0	419.46	392.83	812.29
6	50.0	22.39	3.218	12.71	0.00	0.28	2.34	0.85	1.00	0.00	9.05	47.69	0.00	2,179.8	0.0	645.91	813.01	1,458.92
7	57.5	23.06	2.325	5.52	0.00	0.30	2.29	0.85	1.00	0.00	4.91	23.84	0.00	895.3	0.0	351.99	418.64	770.64
8	67.5	23.85	4.449	16.57	0.00	0.30	2.30	0.85	1.00	0.00	12.49	71.53	0.00	2,578.0	0.0	930.82	1299.05	2,229.87
9	77.5	24.56	1.875	5.52	0.00	0.35	2.16	0.85	1.00	0.00	4.67	23.84	0.00	833.1	0.0	337.50	445.80	783.30
10	84.0	24.98	3.471	6.50	0.00	0.34	2.20	0.85	1.00	0.00	6.74	35.48	0.00	1,457.2	0.0	504.69	660.25	1,164.94
11	90.0	25.34	1.652	3.00	0.00	0.34	2.18	0.85	1.00	0.00	3.19	17.74	0.00	572.0	0.0	239.94	334.95	574.90
12	94.0	25.58	1.503	3.00	0.00	0.35	2.16	0.85	1.00	0.00	3.07	11.98	0.00	488.5	0.0	230.99	323.28	554.28
13	98.0	25.80	1.320	3.00	0.00	0.36	2.14	0.85	1.00	0.00	2.93	10.06	0.00	435.4	0.0	220.13	321.17	541.30
14	101.4	25.99	0.660	1.39	0.00	0.27	2.38	0.85	1.00	0.00	1.40	7.18	0.00	325.5	0.0	118.02	231.06	349.08
15	111.4	26.51	1.458	8.44	0.00	0.22	2.54	0.85	1.00	0.00	6.18	28.69	0.00	1,677.0	0.0	566.77	975.91	1,542.69
16	125.0	27.16	1.233	4.20	0.00	0.21	2.57	0.85	1.00	0.00	3.50	6.56	0.00	715.4	0.0	333.04	285.33	618.37
														22,136.2	0.0	16,550.78		

Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



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Load Case: 0.9D + 1.6W Normal Wind	0.9D + 1.6W 97 mph Wind at Normal To Face
Wind Load Factor: 1.60	Wind Importance Factor: 1.00
Dead Load Factor: 0.90	
Ice Dead Load Factor: 0.00	Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Ice		Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)	
											Linear Area (sqft)	Linear Area (sqft)						
1	3.3	17.40	3.092	8.48	0.00	0.22	2.54	1.00	1.00	0.00	7.19	31.79	0.00	1,340.7	0.0	431.82	421.23	853.05
2	10.0	17.40	4.190	7.36	0.00	0.23	2.50	1.00	1.00	0.00	7.99	31.79	0.00	1,051.7	0.0	473.13	421.17	894.30
3	16.7	17.77	2.959	7.36	0.00	0.21	2.55	1.00	1.00	0.00	6.70	31.79	0.00	988.4	0.0	412.81	430.11	842.92
4	30.0	20.11	7.367	22.09	0.00	0.22	2.52	1.00	1.00	0.00	18.43	95.38	0.00	3,208.7	0.0	1268.12	1460.23	2,728.36
5	42.5	21.64	3.592	6.36	0.00	0.33	2.22	1.00	1.00	0.00	6.97	23.84	0.00	894.9	0.0	454.61	392.83	847.44
6	50.0	22.39	3.218	12.71	0.00	0.28	2.34	1.00	1.00	0.00	9.53	47.69	0.00	1,634.9	0.0	680.35	813.01	1,493.37
7	57.5	23.06	2.325	5.52	0.00	0.30	2.29	1.00	1.00	0.00	5.25	23.84	0.00	671.5	0.0	377.01	418.64	795.66
8	67.5	23.85	4.449	16.57	0.00	0.30	2.30	1.00	1.00	0.00	13.16	71.53	0.00	1,933.5	0.0	980.55	1299.05	2,279.61
9	77.5	24.56	1.875	5.52	0.00	0.35	2.16	1.00	1.00	0.00	4.96	23.84	0.00	624.8	0.0	357.80	445.80	803.60
10	84.0	24.98	3.471	6.50	0.00	0.34	2.20	1.00	1.00	0.00	7.26	35.48	0.00	1,092.9	0.0	543.66	660.25	1,203.91
11	90.0	25.34	1.652	3.00	0.00	0.34	2.18	1.00	1.00	0.00	3.44	17.74	0.00	429.0	0.0	258.59	334.95	593.55
12	94.0	25.58	1.503	3.00	0.00	0.35	2.16	1.00	1.00	0.00	3.30	11.98	0.00	366.4	0.0	247.94	323.28	571.22
13	98.0	25.80	1.320	3.00	0.00	0.36	2.14	1.00	1.00	0.00	3.13	10.06	0.00	326.6	0.0	235.02	321.17	556.19
14	101.4	25.99	0.660	1.39	0.00	0.27	2.38	1.00	1.00	0.00	1.50	7.18	0.00	244.1	0.0	126.35	231.06	357.41
15	111.4	26.51	1.458	8.44	0.00	0.22	2.54	1.00	1.00	0.00	6.40	28.69	0.00	1,257.7	0.0	586.82	975.91	1,562.73
16	125.0	27.16	1.233	4.20	0.00	0.21	2.57	1.00	1.00	0.00	3.69	6.56	0.00	536.5	0.0	350.61	285.33	635.94
													16,602.2	0.0	17,019.24			

Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II
		Page: 11



Load Case: 0.9D + 1.6W 60° Wind	0.9D + 1.6W 97 mph Wind at 60° From Face
Wind Load Factor: 1.60	Wind Importance Factor: 1.00
Dead Load Factor: 0.90	
Ice Dead Load Factor: 0.00	Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	Wind qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Ice		Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
												Linear Area (sqft)	Linear Area (sqft)					
1	3.3	17.40	3.092	8.48	0.00	0.22	2.54	0.80	1.00	0.00	6.57	31.79	0.00	1,340.7	0.0	394.68	421.23	815.91
2	10.0	17.40	4.190	7.36	0.00	0.23	2.50	0.80	1.00	0.00	7.16	31.79	0.00	1,051.7	0.0	423.54	421.17	844.70
3	16.7	17.77	2.959	7.36	0.00	0.21	2.55	0.80	1.00	0.00	6.11	31.79	0.00	988.4	0.0	376.36	430.11	806.47
4	30.0	20.11	7.367	22.09	0.00	0.22	2.52	0.80	1.00	0.00	16.96	95.38	0.00	3,208.7	0.0	1166.76	1460.23	2,626.99
5	42.5	21.64	3.592	6.36	0.00	0.33	2.22	0.80	1.00	0.00	6.25	23.84	0.00	894.9	0.0	407.74	392.83	800.57
6	50.0	22.39	3.218	12.71	0.00	0.28	2.34	0.80	1.00	0.00	8.89	47.69	0.00	1,634.9	0.0	634.42	813.01	1,447.44
7	57.5	23.06	2.325	5.52	0.00	0.30	2.29	0.80	1.00	0.00	4.79	23.84	0.00	671.5	0.0	343.65	418.64	762.30
8	67.5	23.85	4.449	16.57	0.00	0.30	2.30	0.80	1.00	0.00	12.27	71.53	0.00	1,933.5	0.0	914.24	1299.05	2,213.29
9	77.5	24.56	1.875	5.52	0.00	0.35	2.16	0.80	1.00	0.00	4.58	23.84	0.00	624.8	0.0	330.73	445.80	776.53
10	84.0	24.98	3.471	6.50	0.00	0.34	2.20	0.80	1.00	0.00	6.57	35.48	0.00	1,092.9	0.0	491.70	660.25	1,151.95
11	90.0	25.34	1.652	3.00	0.00	0.34	2.18	0.80	1.00	0.00	3.10	17.74	0.00	429.0	0.0	233.72	334.95	568.68
12	94.0	25.58	1.503	3.00	0.00	0.35	2.16	0.80	1.00	0.00	3.00	11.98	0.00	366.4	0.0	225.35	323.28	548.63
13	98.0	25.80	1.320	3.00	0.00	0.36	2.14	0.80	1.00	0.00	2.86	10.06	0.00	326.6	0.0	215.17	321.17	536.34
14	101.4	25.99	0.660	1.39	0.00	0.27	2.38	0.80	1.00	0.00	1.37	7.18	0.00	244.1	0.0	115.24	231.06	346.31
15	111.4	26.51	1.458	8.44	0.00	0.22	2.54	0.80	1.00	0.00	6.11	28.69	0.00	1,257.7	0.0	560.09	975.91	1,536.00
16	125.0	27.16	1.233	4.20	0.00	0.21	2.57	0.80	1.00	0.00	3.44	6.56	0.00	536.5	0.0	327.18	285.33	612.51
														16,602.2	0.0			16,394.63

Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II
		Page: 12

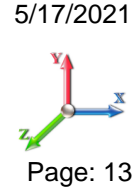


Load Case: 0.9D + 1.6W 90° Wind	0.9D + 1.6W 97 mph Wind at 90° From Face
Wind Load Factor: 1.60	Wind Importance Factor: 1.00
Dead Load Factor: 0.90	
Ice Dead Load Factor: 0.00	Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	Total Flat Area (psf) (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Ice		Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)	
											Linear Area (sqft)	Linear Area (sqft)						
1	3.3	17.40	3.092	8.48	0.00	0.22	2.54	0.85	1.00	0.00	6.73	31.79	0.00	1,340.7	0.0	403.96	421.23	825.19
2	10.0	17.40	4.190	7.36	0.00	0.23	2.50	0.85	1.00	0.00	7.37	31.79	0.00	1,051.7	0.0	435.94	421.17	857.10
3	16.7	17.77	2.959	7.36	0.00	0.21	2.55	0.85	1.00	0.00	6.26	31.79	0.00	988.4	0.0	385.47	430.11	815.58
4	30.0	20.11	7.367	22.09	0.00	0.22	2.52	0.85	1.00	0.00	17.33	95.38	0.00	3,208.7	0.0	1192.10	1460.23	2,652.34
5	42.5	21.64	3.592	6.36	0.00	0.33	2.22	0.85	1.00	0.00	6.43	23.84	0.00	894.9	0.0	419.46	392.83	812.29
6	50.0	22.39	3.218	12.71	0.00	0.28	2.34	0.85	1.00	0.00	9.05	47.69	0.00	1,634.9	0.0	645.91	813.01	1,458.92
7	57.5	23.06	2.325	5.52	0.00	0.30	2.29	0.85	1.00	0.00	4.91	23.84	0.00	671.5	0.0	351.99	418.64	770.64
8	67.5	23.85	4.449	16.57	0.00	0.30	2.30	0.85	1.00	0.00	12.49	71.53	0.00	1,933.5	0.0	930.82	1299.05	2,229.87
9	77.5	24.56	1.875	5.52	0.00	0.35	2.16	0.85	1.00	0.00	4.67	23.84	0.00	624.8	0.0	337.50	445.80	783.30
10	84.0	24.98	3.471	6.50	0.00	0.34	2.20	0.85	1.00	0.00	6.74	35.48	0.00	1,092.9	0.0	504.69	660.25	1,164.94
11	90.0	25.34	1.652	3.00	0.00	0.34	2.18	0.85	1.00	0.00	3.19	17.74	0.00	429.0	0.0	239.94	334.95	574.90
12	94.0	25.58	1.503	3.00	0.00	0.35	2.16	0.85	1.00	0.00	3.07	11.98	0.00	366.4	0.0	230.99	323.28	554.28
13	98.0	25.80	1.320	3.00	0.00	0.36	2.14	0.85	1.00	0.00	2.93	10.06	0.00	326.6	0.0	220.13	321.17	541.30
14	101.4	25.99	0.660	1.39	0.00	0.27	2.38	0.85	1.00	0.00	1.40	7.18	0.00	244.1	0.0	118.02	231.06	349.08
15	111.4	26.51	1.458	8.44	0.00	0.22	2.54	0.85	1.00	0.00	6.18	28.69	0.00	1,257.7	0.0	566.77	975.91	1,542.69
16	125.0	27.16	1.233	4.20	0.00	0.21	2.57	0.85	1.00	0.00	3.50	6.56	0.00	536.5	0.0	333.04	285.33	618.37
														16,602.2	0.0			16,550.78

Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



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Load Case: 1.2D + 1.0Di + 1.0Wi Normal Wind	1.2D + 1.0Di + 1.0Wi 50 mph Wind at Normal From Face
Wind Load Factor: 1.00	Wind Importance Factor: 1.00
Dead Load Factor: 1.20	
Ice Dead Load Factor: 1.00	Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	Total Flat Area (psf) (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Ice		Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)	
											Linear Area (sqft)	Linear Area (sqft)						
1	3.3	4.62	3.092	17.27	8.79	0.37	2.12	1.00	1.00	1.59	13.87	54.19	5.30	4,001.3	2213.8	115.80	148.09	263.89
2	10.0	4.62	4.190	17.03	9.66	0.41	2.05	1.00	1.00	1.77	15.07	56.23	5.92	3,741.2	2338.9	121.57	153.08	274.65
3	16.7	4.72	2.959	17.39	10.02	0.41	2.05	1.00	1.00	1.87	14.06	57.27	6.23	3,619.3	2301.5	115.90	160.27	276.17
4	30.0	5.34	7.367	53.08	30.99	0.44	1.99	1.00	1.00	1.98	42.08	175.58	19.81	12,438.	8160.3	380.69	528.47	909.16
5	42.5	5.75	3.592	14.80	8.44	0.58	1.82	1.00	1.00	2.05	14.40	44.48	5.13	3,395.3	2202.1	128.02	108.90	236.92
6	50.0	5.95	3.218	29.50	16.78	0.55	1.85	1.00	1.00	2.08	24.20	89.52	10.42	6,577.1	4397.3	226.01	244.45	470.46
7	57.5	6.13	2.325	13.85	8.33	0.58	1.81	1.00	1.00	2.11	12.49	45.00	5.29	3,213.0	2317.7	118.06	116.20	234.26
8	67.5	6.34	4.449	41.25	24.68	0.61	1.80	1.00	1.00	2.15	35.26	135.87	16.11	9,166.9	6588.9	342.10	346.21	688.31
9	77.5	6.53	1.875	13.64	8.12	0.68	1.78	1.00	1.00	2.18	12.78	45.54	5.45	2,874.2	2041.1	125.87	96.18	222.05
10	84.0	6.64	3.471	19.95	13.45	0.72	1.78	1.00	1.00	2.20	19.93	66.83	8.78	5,004.1	3547.0	199.91	124.87	324.79
11	90.0	6.73	1.652	9.64	6.63	0.75	1.79	1.00	1.00	2.21	9.85	33.51	4.42	2,323.2	1751.1	100.83	55.85	156.67
12	94.0	6.80	1.503	9.58	6.58	0.78	1.80	1.00	1.00	2.22	9.85	24.46	3.33	1,747.8	1259.3	102.58	47.10	149.68
13	98.0	6.86	1.320	10.45	7.45	0.88	1.90	1.00	1.00	2.23	11.24	21.46	2.97	1,620.3	1184.9	124.15	25.75	149.90
14	101.4	6.91	0.660	7.29	5.90	0.92	1.95	1.00	1.00	2.24	7.82	15.36	2.13	1,215.9	890.4	89.37	13.08	102.44
15	111.4	7.04	1.458	44.14	35.71	0.87	1.89	1.00	1.00	2.26	43.27	66.86	9.14	5,752.6	4075.7	489.45	91.56	581.01
16	125.0	7.22	1.233	27.35	23.15	0.95	2.01	1.00	1.00	2.28	28.87	17.99	3.81	2,608.9	1893.5	355.09	12.13	367.22
														69,299.7	47163.5			5,407.58

Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



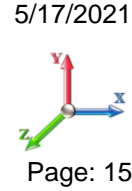
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Load Case: 1.2D + 1.0Di + 1.0Wi 60° Wind	1.2D + 1.0Di + 1.0Wi 50 mph Wind at 60° From Face
Wind Load Factor: 1.00	Wind Importance Factor: 1.00
Dead Load Factor: 1.20	
Ice Dead Load Factor: 1.00	Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	Total Flat Area (psf) (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Ice		Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)	
											Linear Area (sqft)	Linear Area (sqft)						
1	3.3	4.62	3.092	17.27	8.79	0.37	2.12	0.80	1.00	1.59	13.25	54.19	5.30	4,001.3	2213.8	110.64	148.09	258.73
2	10.0	4.62	4.190	17.03	9.66	0.41	2.05	0.80	1.00	1.77	14.23	56.23	5.92	3,741.2	2338.9	114.81	153.08	267.88
3	16.7	4.72	2.959	17.39	10.02	0.41	2.05	0.80	1.00	1.87	13.47	57.27	6.23	3,619.3	2301.5	111.02	160.27	271.29
4	30.0	5.34	7.367	53.08	30.99	0.44	1.99	0.80	1.00	1.98	40.61	175.58	19.81	12,438.	8160.3	367.36	528.47	895.83
5	42.5	5.75	3.592	14.80	8.44	0.58	1.82	0.80	1.00	2.05	13.68	44.48	5.13	3,395.3	2202.1	121.63	108.90	230.54
6	50.0	5.95	3.218	29.50	16.78	0.55	1.85	0.80	1.00	2.08	23.56	89.52	10.42	6,577.1	4397.3	220.00	244.45	464.45
7	57.5	6.13	2.325	13.85	8.33	0.58	1.81	0.80	1.00	2.11	12.03	45.00	5.29	3,213.0	2317.7	113.67	116.20	229.87
8	67.5	6.34	4.449	41.25	24.68	0.61	1.80	0.80	1.00	2.15	34.37	135.87	16.11	9,166.9	6588.9	333.46	346.21	679.67
9	77.5	6.53	1.875	13.64	8.12	0.68	1.78	0.80	1.00	2.18	12.40	45.54	5.45	2,874.2	2041.1	122.17	96.18	218.35
10	84.0	6.64	3.471	19.95	13.45	0.72	1.78	0.80	1.00	2.20	19.23	66.83	8.78	5,004.1	3547.0	192.95	124.87	317.82
11	90.0	6.73	1.652	9.64	6.63	0.75	1.79	0.80	1.00	2.21	9.52	33.51	4.42	2,323.2	1751.1	97.44	55.85	153.29
12	94.0	6.80	1.503	9.58	6.58	0.78	1.80	0.80	1.00	2.22	9.55	24.46	3.33	1,747.8	1259.3	99.45	47.10	146.55
13	98.0	6.86	1.320	10.45	7.45	0.88	1.90	0.80	1.00	2.23	10.97	21.46	2.97	1,620.3	1184.9	121.23	25.75	146.99
14	101.4	6.91	0.660	7.29	5.90	0.92	1.95	0.80	1.00	2.24	7.69	15.36	2.13	1,215.9	890.4	87.86	13.08	100.94
15	111.4	7.04	1.458	44.14	35.71	0.87	1.89	0.80	1.00	2.26	42.97	66.86	9.14	5,752.6	4075.7	486.15	91.56	577.71
16	125.0	7.22	1.233	27.35	23.15	0.95	2.01	0.80	1.00	2.28	28.63	17.99	3.81	2,608.9	1893.5	352.06	12.13	364.19
														69,299.7	47163.5			5,324.09

Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



Load Case: 1.2D + 1.0Di + 1.0Wi 90° Wind	1.2D + 1.0Di + 1.0Wi 50 mph Wind at 90° From Face
Wind Load Factor: 1.00	Wind Importance Factor: 1.00
Dead Load Factor: 1.20	
Ice Dead Load Factor: 1.00	Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total	Total	Ice	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear	Linear	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)	
			Flat Area (sqft)	Round Area (sqft)	Round Area (sqft)							Area (sqft)	Area (sqft)						
1	3.3	4.62	3.092	17.27	8.79	0.37	2.12	0.85	1.00	1.59	13.40	54.19	5.30	4,001.3	2213.8	111.93	148.09	260.02	
2	10.0	4.62	4.190	17.03	9.66	0.41	2.05	0.85	1.00	1.77	14.44	56.23	5.92	3,741.2	2338.9	116.50	153.08	269.57	
3	16.7	4.72	2.959	17.39	10.02	0.41	2.05	0.85	1.00	1.87	13.62	57.27	6.23	3,619.3	2301.5	112.24	160.27	272.51	
4	30.0	5.34	7.367	53.08	30.99	0.44	1.99	0.85	1.00	1.98	40.98	175.58	19.81	12,438.	8160.3	370.69	528.47	899.16	
5	42.5	5.75	3.592	14.80	8.44	0.58	1.82	0.85	1.00	2.05	13.86	44.48	5.13	3,395.3	2202.1	123.23	108.90	232.13	
6	50.0	5.95	3.218	29.50	16.78	0.55	1.85	0.85	1.00	2.08	23.72	89.52	10.42	6,577.1	4397.3	221.50	244.45	465.95	
7	57.5	6.13	2.325	13.85	8.33	0.58	1.81	0.85	1.00	2.11	12.15	45.00	5.29	3,213.0	2317.7	114.76	116.20	230.96	
8	67.5	6.34	4.449	41.25	24.68	0.61	1.80	0.85	1.00	2.15	34.59	135.87	16.11	9,166.9	6588.9	335.62	346.21	681.83	
9	77.5	6.53	1.875	13.64	8.12	0.68	1.78	0.85	1.00	2.18	12.50	45.54	5.45	2,874.2	2041.1	123.10	96.18	219.28	
10	84.0	6.64	3.471	19.95	13.45	0.72	1.78	0.85	1.00	2.20	19.41	66.83	8.78	5,004.1	3547.0	194.69	124.87	319.56	
11	90.0	6.73	1.652	9.64	6.63	0.75	1.79	0.85	1.00	2.21	9.60	33.51	4.42	2,323.2	1751.1	98.29	55.85	154.13	
12	94.0	6.80	1.503	9.58	6.58	0.78	1.80	0.85	1.00	2.22	9.62	24.46	3.33	1,747.8	1259.3	100.23	47.10	147.33	
13	98.0	6.86	1.320	10.45	7.45	0.88	1.90	0.85	1.00	2.23	11.04	21.46	2.97	1,620.3	1184.9	121.96	25.75	147.71	
14	101.4	6.91	0.660	7.29	5.90	0.92	1.95	0.85	1.00	2.24	7.72	15.36	2.13	1,215.9	890.4	88.24	13.08	101.31	
15	111.4	7.04	1.458	44.14	35.71	0.87	1.89	0.85	1.00	2.26	43.05	66.86	9.14	5,752.6	4075.7	486.97	91.56	578.54	
16	125.0	7.22	1.233	27.35	23.15	0.95	2.01	0.85	1.00	2.28	28.69	17.99	3.81	2,608.9	1893.5	352.82	12.13	364.95	
															69,299.7	47163.5			5,344.96

Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



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Load Case: 1.0D + 1.0W Normal Wind	1.0D + 1.0W 60 mph Wind at Normal To Face
Wind Load Factor: 1.00	Wind Importance Factor: 1.00
Dead Load Factor: 1.00	
Ice Dead Load Factor: 0.00	Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Ice		Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)	
											Area (sqft)	Linear Area (sqft)						
1	3.3	6.66	3.092	8.48	0.00	0.22	2.54	1.00	1.00	0.00	7.89	31.79	0.00	1,489.7	0.0	113.30	100.73	214.03
2	10.0	6.66	4.190	7.36	0.00	0.23	2.50	1.00	1.00	0.00	8.46	31.79	0.00	1,168.5	0.0	119.77	100.71	220.48
3	16.7	6.80	2.959	7.36	0.00	0.21	2.55	1.00	1.00	0.00	7.21	31.79	0.00	1,098.2	0.0	106.17	102.85	209.03
4	30.0	7.69	7.367	22.09	0.00	0.22	2.52	1.00	1.00	0.00	20.10	95.38	0.00	3,565.2	0.0	330.60	349.19	679.79
5	42.5	8.28	3.592	6.36	0.00	0.33	2.22	1.00	1.00	0.00	7.35	23.84	0.00	994.3	0.0	114.68	93.94	208.62
6	50.0	8.57	3.218	12.71	0.00	0.28	2.34	1.00	1.00	0.00	10.46	47.69	0.00	1,816.5	0.0	178.55	194.42	372.97
7	57.5	8.82	2.325	5.52	0.00	0.30	2.29	1.00	1.00	0.00	5.59	23.84	0.00	746.1	0.0	95.90	100.11	196.01
8	67.5	9.13	4.449	16.57	0.00	0.30	2.30	1.00	1.00	0.00	14.19	71.53	0.00	2,148.3	0.0	252.95	310.65	563.60
9	77.5	9.40	1.875	5.52	0.00	0.35	2.16	1.00	1.00	0.00	5.23	23.84	0.00	694.3	0.0	90.35	106.60	196.96
10	84.0	9.56	3.471	6.50	0.00	0.34	2.20	1.00	1.00	0.00	7.44	35.48	0.00	1,214.3	0.0	133.27	157.89	291.16
11	90.0	9.70	1.652	3.00	0.00	0.34	2.18	1.00	1.00	0.00	3.49	17.74	0.00	476.7	0.0	62.91	80.10	143.01
12	94.0	9.79	1.503	3.00	0.00	0.35	2.16	1.00	1.00	0.00	3.36	11.98	0.00	407.1	0.0	60.34	77.31	137.64
13	98.0	9.87	1.320	3.00	0.00	0.36	2.14	1.00	1.00	0.00	3.18	10.06	0.00	362.8	0.0	57.23	76.80	134.03
14	101.4	9.94	0.660	1.39	0.00	0.27	2.38	1.00	1.00	0.00	1.50	7.18	0.00	271.2	0.0	30.21	55.25	85.47
15	111.4	10.14	1.458	8.44	0.00	0.22	2.54	1.00	1.00	0.00	6.40	28.69	0.00	1,397.5	0.0	140.33	233.37	373.70
16	125.0	10.39	1.233	4.20	0.00	0.21	2.57	1.00	1.00	0.00	3.69	6.56	0.00	596.2	0.0	83.84	68.23	152.07
													18,446.9	0.0			4,178.57	

Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



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Load Case: 1.0D + 1.0W 60° Wind	1.0D + 1.0W 60 mph Wind at 60° From Face
Wind Load Factor: 1.00	Wind Importance Factor: 1.00
Dead Load Factor: 1.00	
Ice Dead Load Factor: 0.00	Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	Total Flat Area (psf) (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Ice		Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)	
											Linear Area (sqft)	Linear Area (sqft)						
1	3.3	6.66	3.092	8.48	0.00	0.22	2.54	0.80	1.00	0.00	7.27	31.79	0.00	1,489.7	0.0	104.42	100.73	205.15
2	10.0	6.66	4.190	7.36	0.00	0.23	2.50	0.80	1.00	0.00	7.62	31.79	0.00	1,168.5	0.0	107.91	100.71	208.62
3	16.7	6.80	2.959	7.36	0.00	0.21	2.55	0.80	1.00	0.00	6.62	31.79	0.00	1,098.2	0.0	97.46	102.85	200.31
4	30.0	7.69	7.367	22.09	0.00	0.22	2.52	0.80	1.00	0.00	18.62	95.38	0.00	3,565.2	0.0	306.36	349.19	655.55
5	42.5	8.28	3.592	6.36	0.00	0.33	2.22	0.80	1.00	0.00	6.63	23.84	0.00	994.3	0.0	103.47	93.94	197.41
6	50.0	8.57	3.218	12.71	0.00	0.28	2.34	0.80	1.00	0.00	9.82	47.69	0.00	1,816.5	0.0	167.57	194.42	361.99
7	57.5	8.82	2.325	5.52	0.00	0.30	2.29	0.80	1.00	0.00	5.12	23.84	0.00	746.1	0.0	87.92	100.11	188.04
8	67.5	9.13	4.449	16.57	0.00	0.30	2.30	0.80	1.00	0.00	13.30	71.53	0.00	2,148.3	0.0	237.09	310.65	547.74
9	77.5	9.40	1.875	5.52	0.00	0.35	2.16	0.80	1.00	0.00	4.86	23.84	0.00	694.3	0.0	83.88	106.60	190.48
10	84.0	9.56	3.471	6.50	0.00	0.34	2.20	0.80	1.00	0.00	6.75	35.48	0.00	1,214.3	0.0	120.84	157.89	278.73
11	90.0	9.70	1.652	3.00	0.00	0.34	2.18	0.80	1.00	0.00	3.16	17.74	0.00	476.7	0.0	56.96	80.10	137.06
12	94.0	9.79	1.503	3.00	0.00	0.35	2.16	0.80	1.00	0.00	3.06	11.98	0.00	407.1	0.0	54.93	77.31	132.24
13	98.0	9.87	1.320	3.00	0.00	0.36	2.14	0.80	1.00	0.00	2.92	10.06	0.00	362.8	0.0	52.48	76.80	129.28
14	101.4	9.94	0.660	1.39	0.00	0.27	2.38	0.80	1.00	0.00	1.37	7.18	0.00	271.2	0.0	27.56	55.25	82.81
15	111.4	10.14	1.458	8.44	0.00	0.22	2.54	0.80	1.00	0.00	6.11	28.69	0.00	1,397.5	0.0	133.94	233.37	367.31
16	125.0	10.39	1.233	4.20	0.00	0.21	2.57	0.80	1.00	0.00	3.44	6.56	0.00	596.2	0.0	78.24	68.23	146.47
														18,446.9	0.0			4,029.20

Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



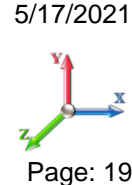
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Load Case: 1.0D + 1.0W 90° Wind	1.0D + 1.0W 60 mph Wind at 90° From Face
Wind Load Factor: 1.00	Wind Importance Factor: 1.00
Dead Load Factor: 1.00	
Ice Dead Load Factor: 0.00	Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	Total Flat Area (psf) (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Ice		Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)	
											Linear Area (sqft)	Linear Area (sqft)						
1	3.3	6.66	3.092	8.48	0.00	0.22	2.54	0.85	1.00	0.00	7.42	31.79	0.00	1,489.7	0.0	106.64	100.73	207.37
2	10.0	6.66	4.190	7.36	0.00	0.23	2.50	0.85	1.00	0.00	7.83	31.79	0.00	1,168.5	0.0	110.87	100.71	211.59
3	16.7	6.80	2.959	7.36	0.00	0.21	2.55	0.85	1.00	0.00	6.77	31.79	0.00	1,098.2	0.0	99.64	102.85	202.49
4	30.0	7.69	7.367	22.09	0.00	0.22	2.52	0.85	1.00	0.00	18.99	95.38	0.00	3,565.2	0.0	312.42	349.19	661.61
5	42.5	8.28	3.592	6.36	0.00	0.33	2.22	0.85	1.00	0.00	6.81	23.84	0.00	994.3	0.0	106.27	93.94	200.21
6	50.0	8.57	3.218	12.71	0.00	0.28	2.34	0.85	1.00	0.00	9.98	47.69	0.00	1,816.5	0.0	170.31	194.42	364.73
7	57.5	8.82	2.325	5.52	0.00	0.30	2.29	0.85	1.00	0.00	5.24	23.84	0.00	746.1	0.0	89.92	100.11	190.03
8	67.5	9.13	4.449	16.57	0.00	0.30	2.30	0.85	1.00	0.00	13.53	71.53	0.00	2,148.3	0.0	241.06	310.65	551.70
9	77.5	9.40	1.875	5.52	0.00	0.35	2.16	0.85	1.00	0.00	4.95	23.84	0.00	694.3	0.0	85.50	106.60	192.10
10	84.0	9.56	3.471	6.50	0.00	0.34	2.20	0.85	1.00	0.00	6.92	35.48	0.00	1,214.3	0.0	123.95	157.89	281.84
11	90.0	9.70	1.652	3.00	0.00	0.34	2.18	0.85	1.00	0.00	3.25	17.74	0.00	476.7	0.0	58.45	80.10	138.55
12	94.0	9.79	1.503	3.00	0.00	0.35	2.16	0.85	1.00	0.00	3.13	11.98	0.00	407.1	0.0	56.28	77.31	133.59
13	98.0	9.87	1.320	3.00	0.00	0.36	2.14	0.85	1.00	0.00	2.99	10.06	0.00	362.8	0.0	53.67	76.80	130.47
14	101.4	9.94	0.660	1.39	0.00	0.27	2.38	0.85	1.00	0.00	1.40	7.18	0.00	271.2	0.0	28.22	55.25	83.48
15	111.4	10.14	1.458	8.44	0.00	0.22	2.54	0.85	1.00	0.00	6.18	28.69	0.00	1,397.5	0.0	135.53	233.37	368.91
16	125.0	10.39	1.233	4.20	0.00	0.21	2.57	0.85	1.00	0.00	3.50	6.56	0.00	596.2	0.0	79.64	68.23	147.87
													18,446.9	0.0			4,066.54	

Force/Stress Compression Summary

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



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LEG MEMBERS

Sect	Top Elev	Member	Force (kips)		Load Case	Len (ft)	Bracing %			Fy (ksi)	Mem Cap (kips)	Leg Use %	Controls	
							X	Y	Z					
1	6.667	MOD - 6PX+7.625x.301_1/2P	-397.93	1.2D + 1.6W	Normal Wind	6.67	100	100	100	36.87	54.00	519.92	76.5	Member X
2	13.33	PX - 6" DIA PIPE	-383.67	1.2D + 1.6W	Normal Wind	6.67	50	50	50	18.27	54.00	397.62	96.5	Member X
3	20	PX - 6" DIA PIPE	-366.79	1.2D + 1.6W	Normal Wind	6.67	100	100	100	36.55	54.00	367.37	99.8	Member X
4	40	PX - 6" DIA PIPE	-348.52	1.2D + 1.6W	Normal Wind	6.67	100	100	100	36.54	54.00	367.38	94.9	Member X
5	45	MOD - 6PST+7.625x.301_5P	-293.15	1.2D + 1.6W	Normal Wind	5.00	50	50	50	13.90	54.00	435.04	67.4	Member X
6	55	MOD - 6PST+7.625x.301_5P	-275.77	1.2D + 1.6W	Normal Wind	5.00	100	100	100	27.80	54.00	415.57	66.4	Member X
7	60	PST - 6" DIA PIPE	-240.59	1.2D + 1.6W	Normal Wind	5.00	50	50	50	13.34	54.00	267.40	90.0	Member X
8	75	PST - 6" DIA PIPE	-222.62	1.2D + 1.6W	Normal Wind	5.00	100	100	100	26.68	54.00	256.37	86.8	Member X
9	80	PST - 6" DIA PIPE	-157.80	1.2D + 1.6W	Normal Wind	5.00	100	100	100	26.68	54.00	256.37	61.6	Member X
10	88	MOD - 4"PST+5"PX1/2P	-138.72	1.2D + 1.6W	Normal Wind	4.00	50	50	50	15.25	54.00	297.27	46.7	Member X
11	92	PST - 4" DIA PIPE	-105.41	1.2D + 1.6W	Normal Wind	4.00	50	50	50	15.90	54.00	151.02	69.8	Member X
12	96	PST - 4" DIA PIPE	-90.98	1.2D + 1.6W	Normal Wind	4.00	100	100	100	31.80	54.00	142.24	64.0	Member X
13	100	PST - 4" DIA PIPE	-76.56	1.2D + 1.6W	Normal Wind	4.00	100	100	100	31.80	54.00	142.24	53.8	Member X
14	102.8	SOL - 2" SOLID	-63.86	1.2D + 1.6W	Normal Wind	2.86	50	50	50	34.28	45.00	117.76	54.2	Member X
15	120	SOL - 2" SOLID	-51.02	1.2D + 1.6W	Normal Wind	2.86	100	100	100	68.57	45.00	93.37	54.6	Member X
16	130	SOL - 1 1/2" SOLID	-2.02	1.2D + 1.6W	Normal Wind	2.50	100	100	100	80.00	45.00	46.97	4.3	Member X

Splices

Sect	Top Elev	Load Case	Top Splice				Bottom Splice					
			Force (kips)	Cap (kips)	Use %	Bolt Type	Num Bolts	Load Case	Force (kips)	Cap (kips)	Use %	Bolt Type
1	6.667	1.2D + 1.6W Normal Wind	391.89	0.00	0.0			1.2D + 1.6W Normal Wind	408.34	0.00		
2	13.33	1.2D + 1.6W Normal Wind	375.88	0.00	0.0			1.2D + 1.6W Normal Wind	391.89	0.00		
3	20	1.2D + 1.6W Normal Wind	358.66	0.00	0.0			1.2D + 1.6W Normal Wind	375.88	0.00		
4	40	1.2D + 1.6W Normal Wind	300.69	0.00	0.0			1.2D + 1.6W Normal Wind	358.66	0.00	1 A325	8
5	45	1.2D + 1.6W Normal Wind	285.03	0.00	0.0			1.2D + 1.6W Normal Wind	300.69	0.00	1 A325	8
6	55	1.2D + 1.6W Normal Wind	250.89	0.00	0.0			1.2D + 1.6W Normal Wind	285.03	0.00		
7	60	1.2D + 1.6W Normal Wind	232.33	0.00	0.0			1.2D + 1.6W Normal Wind	250.89	0.00		
8	75	1.2D + 1.6W Normal Wind	170.12	0.00	0.0			1.2D + 1.6W Normal Wind	232.33	0.00	1 A325	8
9	80	1.2D + 1.6W Normal Wind	147.81	0.00	0.0			1.2D + 1.6W Normal Wind	170.12	0.00		
10	88	1.2D + 1.6W Normal Wind	115.17	0.00	0.0			1.2D + 1.6W Normal Wind	147.81	0.00	1 A325	8
11	92	1.2D + 1.6W Normal Wind	98.44	0.00	0.0			1.2D + 1.6W Normal Wind	115.17	0.00		
12	96	1.2D + 1.6W Normal Wind	84.16	0.00	0.0			1.2D + 1.6W Normal Wind	98.44	0.00		
13	100	1.2D + 1.6W Normal Wind	72.09	0.00	0.0			1.2D + 1.6W Normal Wind	84.16	0.00		
14	102.8	1.2D + 1.6W Normal Wind	58.75	0.00	0.0			1.2D + 1.6W Normal Wind	72.09	0.00	3/4 A325	6
15	120	1.2D + 1.0Di + 1.0Wi Normal Wi	4.71	0.00	0.0			1.2D + 1.6W Normal Wind	58.75	0.00		
16	130	1.2D + 1.0Di + 1.0Wi 90° Wind	0.28	0.00	0.0			1.2D + 1.0Di + 1.0Wi Normal Wi	4.71	0.00	3/4 A325	4

HORIZONTAL MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Len (ft)	Bracing %	Fy (ksi)	Mem		Shear Bear		Use %	Controls
								Cap (kips)	Num Bolts	Num Holes	Cap (kips)		
1	6.66							0.00	0	0			
2	13.3							0.00	0	0			
3	20							0.00	0	0			
4	40							0.00	0	0			
5	45							0.00	0	0			
6	55							0.00	0	0			
7	60							0.00	0	0			
8	75							0.00	0	0			
9	80							0.00	0	0			

Force/Stress Compression Summary

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



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HORIZONTAL MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Len (ft)	Bracing %			KL/R	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Bear		Use %	Controls
						X	Y	Z						Cap (kips)	Cap (kips)		
10	88									0.00	0	0					
11	92									0.00	0	0					
12	96									0.00	0	0					
13	100	SAE - 1.25x1.25x0.1875	-1.44	0.9D + 1.6W Normal Wind	2.50	100	100	100	122.95	36.00	6.34	1	1	7.95	7.50	23	Member Z
14	102	SAE - 1.25x1.25x0.1875	-1.64	0.9D + 1.6W Normal Wind	2.50	100	100	100	86.07	36.00	9.52	0	0			17	Member Z
15	120	SAE - 1.25x1.25x0.1875	-1.22	0.9D + 1.6W Normal Wind	2.50	100	100	100	86.07	36.00	9.52	0	0			13	Member Z
16	130	SAE - 1.25x1.25x0.1875	-0.16	0.9D + 1.6W 60° Wind	2.50	100	100	100	86.07	36.00	9.52	0	0			2	Member Z

DIAGONAL MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Len (ft)	Bracing %			KL/R	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Bear		Use %	Controls
						X	Y	Z						Cap (kips)	Cap (kips)		
1	6.66	DAE - 2X2X0.1875	-7.53	1.2D + 1.6W Normal Wind	9.91	50	50	25	96.96	36.00	28.25	2	2	15.90	37.1	47	Bolt Shear
2	13.3	SAE - 2X2X0.1875	-6.97	0.9D + 1.6W 90° Wind	9.67	50	50	50	147.21	36.00	7.40	1	1	9.72	7.50	94	Member Z
3	20	SAE - 2X2X0.1875	-7.19	0.9D + 1.6W 90° Wind	9.43	50	50	50	143.59	36.00	7.78	1	1	9.72	7.50	96	Bolt Bear
4	40	DAE - 1.75X1.75X.1875-7.88	-7.88	1.2D + 1.6W 90° Wind	8.75	50	50	25	97.77	36.00	24.29	2	2	15.90	37.1	50	Bolt Shear
5	45	SAE - 2X2X0.25	-6.57	1.2D + 1.6W 90° Wind	7.34	50	50	50	114.49	36.00	15.27	1	1	7.95	10.0	83	Bolt Shear
6	55	DAE - 1.5x1.5x.1875	-6.99	1.2D + 1.6W 90° Wind	6.98	50	50	25	92.77	36.00	21.83	2	2	15.90	37.1	44	Bolt Shear
7	60	DAE - 1.5x1.5x.1875	-7.46	1.2D + 1.6W 90° Wind	6.81	50	50	25	90.48	36.00	22.32	2	2	15.90	37.1	47	Bolt Shear
8	75	DAE - 1.5x1.5x.1875	-8.32	1.2D + 1.6W 90° Wind	6.33	50	50	25	84.03	36.00	23.68	2	2	15.90	37.1	52	Bolt Shear
9	80	SAE - 2X2X0.25	-8.03	1.2D + 1.6W 90° Wind	6.18	50	50	25	60.85	36.00	25.06	2	2	15.90	24.8	51	Bolt Shear
10	88	DAE - 1.5x1.5x.1875	-5.89	1.2D + 1.6W 90° Wind	5.12	50	50	25	80.44	36.00	24.43	2	2	15.90	37.1	37	Bolt Shear
11	92	DAE - 1.5x1.5x.1875	-6.34	1.2D + 1.6W 90° Wind	5.00	50	50	25	66.42	36.00	27.23	2	2	15.90	37.1	40	Bolt Shear
12	96	SAE - 2X2X0.25	-4.72	1.2D + 1.6W 90° Wind	4.88	50	50	50	86.20	36.00	20.60	1	1	7.95	10.0	59	Bolt Shear
13	100	SAE - 1.5X1.5X0.1875	-4.50	1.2D + 1.6W Normal Wind	4.77	50	50	50	103.28	36.00	9.79	1	1	7.95	7.50	60	Bolt Bear
14	102	SOL - 3/4" SOLID	-5.42	1.2D + 1.6W Normal Wind	3.80	50	50	50	109.34	36.00	7.63	0	0				T-Only
15	120	SOL - 3/4" SOLID	-5.05	1.2D + 1.6W Normal Wind	3.80	50	50	50	109.34	36.00	7.63	0	0				T-Only
16	130	SOL - 3/4" SOLID	-0.33	1.2D + 1.6W Normal Wind	3.54	50	50	50	101.82	36.00	8.29	0	0				T-Only

Force/Stress Tension Summary

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



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LEG MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Fy (ksi)	Mem Cap (kips)	Leg Use %	Controls
1	6.667	MOD - 6PX+7.625x.301_1/2P	371.49	0.9D + 1.6W 60° Wind	54	578.83	64.2	Member
2	13.333	PX - 6" DIA PIPE	358.30	0.9D + 1.6W 60° Wind	54	408.24	87.8	Member
3	20	PX - 6" DIA PIPE	342.60	0.9D + 1.6W 60° Wind	54	408.24	83.9	Member
4	40	PX - 6" DIA PIPE	325.63	0.9D + 1.6W 60° Wind	54	408.24	79.8	Member
5	45	MOD - 6PST+7.625x.301_5P	273.39	0.9D + 1.6W 60° Wind	54	441.73	61.9	Member
6	55	MOD - 6PST+7.625x.301_5P	257.17	0.9D + 1.6W 60° Wind	54	441.73	58.2	Member
7	60	PST - 6" DIA PIPE	224.46	0.9D + 1.6W 60° Wind	54	271.19	82.8	Member
8	75	PST - 6" DIA PIPE	207.32	0.9D + 1.6W 60° Wind	54	271.19	76.4	Member
9	80	PST - 6" DIA PIPE	145.27	0.9D + 1.6W 60° Wind	54	271.19	53.6	Member
10	88	MOD - 4"PST+5"PX1/2P	129.12	0.9D + 1.6W 60° Wind	54	302.78	42.6	Member
11	92	PST - 4" DIA PIPE	97.82	0.9D + 1.6W 60° Wind	54	154.06	63.5	Member
12	96	PST - 4" DIA PIPE	85.64	1.2D + 1.6W 60° Wind	54	154.06	55.6	Member
13	100	PST - 4" DIA PIPE	71.99	1.2D + 1.6W 60° Wind	54	154.06	46.7	Member
14	102.85	SOL - 2" SOLID	59.68	0.9D + 1.6W 60° Wind	45	127.23	46.9	Member
15	120	SOL - 2" SOLID	47.14	0.9D + 1.6W 60° Wind	45	127.23	37.0	Member
16	130	SOL - 1 1/2" SOLID	1.63	0.9D + 1.6W 60° Wind	45	71.57	2.3	Member

Splices

Sect	Top Elev	Top Splice					Bottom Splice						
		Load Case	Force (kips)	Cap (kips)	Use %	Bolt Type	Num Bolts	Load Case	Force (kips)	Cap (kips)	Use %	Bolt Type	Num Bolts
1	6.667	0.9D + 1.6W 60° Wind	364.71	0.00	0.0		0.9D + 1.6W 60° Wind	380.3	0.00				
2	13.333	0.9D + 1.6W 60° Wind	349.67	0.00	0.0		0.9D + 1.6W 60° Wind	364.7	0.00				
3	20	0.9D + 1.6W 60° Wind	333.70	0.00	0.0		0.9D + 1.6W 60° Wind	349.6	0.00				
4	40	0.9D + 1.6W 60° Wind	279.48	0.00	0.0		0.9D + 1.6W 60° Wind	333.7	424.08	78.7	1 A325	8	
5	45	0.9D + 1.6W 60° Wind	264.94	0.00	0.0		0.9D + 1.6W 60° Wind	279.4	424.08	65.9	1 A325	8	
6	55	0.9D + 1.6W 60° Wind	232.99	0.00	0.0		0.9D + 1.6W 60° Wind	264.9	0.00				
7	60	0.9D + 1.6W 60° Wind	215.56	0.00	0.0		0.9D + 1.6W 60° Wind	232.9	0.00				
8	75	0.9D + 1.6W 60° Wind	156.24	0.00	0.0		0.9D + 1.6W 60° Wind	215.5	424.08	50.8	1 A325	8	
9	80	0.9D + 1.6W 60° Wind	134.86	0.00	0.0		0.9D + 1.6W 60° Wind	156.2	0.00				
10	88	0.9D + 1.6W 60° Wind	106.48	0.00	0.0		0.9D + 1.6W 60° Wind	134.8	424.08	31.8	1 A325	8	
11	92	0.9D + 1.6W 60° Wind	90.17	0.00	0.0		0.9D + 1.6W 60° Wind	106.4	0.00				
12	96	1.2D + 1.6W 60° Wind	78.57	0.00	0.0		0.9D + 1.6W 60° Wind	90.17	0.00				
13	100	0.9D + 1.6W 60° Wind	67.11	0.00	0.0		1.2D + 1.6W 60° Wind	78.57	0.00				
14	102.85	0.9D + 1.6W 60° Wind	54.23	0.00	0.0		0.9D + 1.6W 60° Wind	67.11	180.60	37.2	3/4 A325	6	
15	120	0.9D + 1.6W 60° Wind	1.45	0.00	0.0		0.9D + 1.6W 60° Wind	54.23	0.00				
16	130		0.00	0.00	0.0		0.9D + 1.6W 60° Wind	1.45	120.40	1.2	3/4 A325	4	

HORIZONTAL MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	B.S. Cap (kips)	Use %	Controls
1	6.667	-			36	0.00	0	0					
2	13.333	-			36	0.00	0	0					
3	20	-			36	0.00	0	0					
4	40	-			36	0.00	0	0					
5	45	-			36	0.00	0	0					
6	55	-			36	0.00	0	0					
7	60	-			36	0.00	0	0					
8	75	-			36	0.00	0	0					
9	80	-			36	0.00	0	0					
10	88	-			36	0.00	0	0					

Force/Stress Tension Summary

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



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HORIZONTAL MEMBERS

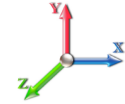
Sect	Top Elev	Member	Force (kips)	Load Case	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	B.S. Cap (kips)	Use %	Controls
11	92	-			36	0.00	0	0					
12	96	-			36	0.00	0	0					
13	100	SAE - 1.25x1.25x0.1875	1.77	1.2D + 1.6W 60° Wind	36	13.78	1	1	7.95	7.50	5.21	34.1	Blck Shear
14	102.85	SAE - 1.25x1.25x0.1875	2.05	1.2D + 1.6W 60° Wind	36	14.06	0	0				14.6	Member
15	120	SAE - 1.25x1.25x0.1875	1.60	1.2D + 1.6W 60° Wind	36	14.06	0	0				11.4	Member
16	130	SAE - 1.25x1.25x0.1875	0.20	1.2D + 1.6W 90° Wind	36	14.06	0	0				1.4	Member

DIAGONAL MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	B.S. Cap (kips)	Use %	Controls
1	6.667	DAE - 2X2X0.1875	6.44	0.9D + 1.6W 60° Wind	36	46.33	2	2	15.90	37.19	23.25	40.5	Bolt Shear
2	13.333	SAE - 2X2X0.1875	5.83	0.9D + 1.6W 90° Wind	36	23.00	1	1	9.72	7.50	7.25	80.5	Blck Shear
3	20	SAE - 2X2X0.1875	6.21	0.9D + 1.6W 90° Wind	36	23.00	1	1	9.72	7.50	7.25	85.8	Blck Shear
4	40	DAE - 1.75X1.75X.1875	6.79	0.9D + 1.6W 90° Wind	36	40.18	2	2	15.90	37.19	21.21	42.7	Bolt Shear
5	45	SAE - 2X2X0.25	6.06	1.2D + 1.6W 90° Wind	36	30.46	1	1	7.95	10.01	9.66	76.2	Bolt Shear
6	55	DAE - 1.5x1.5x.1875	6.50	1.2D + 1.6W 90° Wind	36	34.34	2	2	15.90	37.19	19.17	40.9	Bolt Shear
7	60	DAE - 1.5x1.5x.1875	6.59	1.2D + 1.6W 90° Wind	36	34.34	2	2	15.90	37.19	19.17	41.5	Bolt Shear
8	75	DAE - 1.5x1.5x.1875	7.73	1.2D + 1.6W 90° Wind	36	34.34	2	2	15.90	37.19	19.17	48.6	Bolt Shear
9	80	SAE - 2X2X0.25	7.49	1.2D + 1.6W 90° Wind	36	27.30	2	2	15.90	24.80	15.50	48.3	Blck Shear
10	88	DAE - 1.5x1.5x.1875	5.72	1.2D + 1.6W 90° Wind	36	34.34	2	2	15.90	37.19	19.17	36.0	Bolt Shear
11	92	DAE - 1.5x1.5x.1875	5.81	1.2D + 1.6W 90° Wind	36	34.34	2	2	15.90	37.19	19.17	36.5	Bolt Shear
12	96	SAE - 2X2X0.25	4.48	1.2D + 1.6W 90° Wind	36	30.46	1	1	7.95	10.01	9.66	56.4	Bolt Shear
13	100	SAE - 1.5X1.5X0.1875	4.12	1.2D + 1.6W 60° Wind	36	17.17	1	1	7.95	7.50	5.21	79.1	Blck Shear
14	102.85	SOL - 3/4" SOLID	5.09	1.2D + 1.6W 60° Wind	36	14.31	0	0				35.6	Member
15	120	SOL - 3/4" SOLID	4.82	1.2D + 1.6W 90° Wind	36	14.31	0	0				33.6	Member
16	130	SOL - 3/4" SOLID	0.42	1.2D + 1.6W 60° Wind	36	14.31	0	0				3.0	Member

Seismic Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



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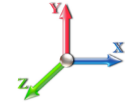
Load Case: 1.2D + 1.0E

Dead Load Factor	1.20	Sds	0.192	Ss	0.1800	Fa	1.6000	Ke	0.0000
Seismic Load Factor	1.00	Sd1	0.100	S1	0.0630	Fv	2.4000	Kg	0.0000
Seismic Importance Factor	1.00	SA	0.118	R	3.0000	Vs	1.4872	f1	1.1746

Sect #	Elev (ft)	Wz (lb)	Lateral			Fsz (lb)
			a	b	c	
1	3.33	1489.6	0.00	0.03	0.01	4.08
2	10.00	1168.5	0.01	0.06	0.03	7.07
3	16.67	1098.2	0.03	0.07	0.04	8.69
4	30.00	3565.1	0.10	0.07	0.04	38.59
5	42.50	994.32	0.20	0.06	0.02	13.97
6	50.00	1816.5	0.28	0.05	0.01	28.89
7	57.50	746.06	0.37	0.03	0.01	12.87
8	67.50	2148.3	0.51	-0.02	0.01	38.47
9	77.50	4268.3	0.67	-0.08	0.02	76.25
10	84.00	1214.3	0.79	-0.11	0.05	22.54
11	90.00	476.68	0.91	-0.12	0.09	9.92
12	94.00	4047.9	0.99	-0.11	0.13	95.90
13	98.00	362.84	1.07	-0.08	0.17	10.17
14	101.43	271.24	1.15	-0.04	0.22	8.98
15	111.43	7085.0	1.39	0.26	0.42	400.93
16	125.00	632.15	1.75	1.31	0.89	70.94

Seismic Section Forces

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



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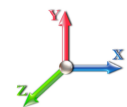
Load Case: 0.9D + 1.0E

Dead Load Factor	0.90	Sds	0.192	Ss	0.1800	Fa	1.6000	Ke	0.0000
Seismic Load Factor	1.00	Sd1	0.100	S1	0.0630	Fv	2.4000	Kg	0.0000
Seismic Importance Factor	1.00	SA	0.118	R	3.0000	Vs	1.4872	f1	1.1746

Sect #	Elev (ft)	Wz (lb)	Lateral			Fsz (lb)
			a	b	c	
1	3.33	1489.6	0.00	0.03	0.01	4.08
2	10.00	1168.5	0.01	0.06	0.03	7.07
3	16.67	1098.2	0.03	0.07	0.04	8.69
4	30.00	3565.1	0.10	0.07	0.04	38.59
5	42.50	994.32	0.20	0.06	0.02	13.97
6	50.00	1816.5	0.28	0.05	0.01	28.89
7	57.50	746.06	0.37	0.03	0.01	12.87
8	67.50	2148.3	0.51	-0.02	0.01	38.47
9	77.50	4268.3	0.67	-0.08	0.02	76.25
10	84.00	1214.3	0.79	-0.11	0.05	22.54
11	90.00	476.68	0.91	-0.12	0.09	9.92
12	94.00	4047.9	0.99	-0.11	0.13	95.90
13	98.00	362.84	1.07	-0.08	0.17	10.17
14	101.43	271.24	1.15	-0.04	0.22	8.98
15	111.43	7085.0	1.39	0.26	0.42	400.93
16	125.00	632.15	1.75	1.31	0.89	70.94

Support Forces Summary

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II


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Load Case	Node	FX (kips)	FY (kips)	FZ (kips)	(-) = Uplift (+) = Down
<hr style="border-top: 1px dashed black;"/>					
1.2D + 1.6W Normal Wind	1	0.00	408.21	-21.32	
	1a	8.62	-185.27	-5.14	
	1b	-8.62	-185.27	-5.14	
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1.2D + 1.6W 60° Wind	1	-0.17	207.61	-10.63	
	1a	-9.32	208.52	5.15	
	1b	-17.33	-378.47	-10.01	
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1.2D + 1.6W 90° Wind	1	-0.18	12.61	-0.37	
	1a	-15.90	352.86	9.04	
	1b	-15.08	-327.80	-8.67	
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0.9D + 1.6W Normal Wind	1	0.00	403.19	-21.17	
	1a	8.71	-187.47	-5.22	
	1b	-8.71	-187.47	-5.22	
<hr style="border-top: 1px dashed black;"/>					
0.9D + 1.6W 60° Wind	1	-0.19	203.54	-10.49	
	1a	-9.21	204.44	5.06	
	1b	-17.43	-379.74	-10.06	
<hr style="border-top: 1px dashed black;"/>					
0.9D + 1.6W 90° Wind	1	-0.20	9.46	-0.24	
	1a	-15.77	348.08	8.95	
	1b	-15.18	-329.30	-8.71	
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1.2D + 1.0Di + 1.0Wi Normal Wind	1	0.00	149.02	-6.04	
	1a	2.44	-20.85	-1.44	
	1b	-2.44	-20.85	-1.44	
<hr style="border-top: 1px dashed black;"/>					
1.2D + 1.0Di + 1.0Wi 60° Wind	1	-0.03	92.02	-3.05	
	1a	-2.66	92.29	1.50	
	1b	-4.96	-76.99	-2.87	
<hr style="border-top: 1px dashed black;"/>					
1.2D + 1.0Di + 1.0Wi 90° Wind	1	-0.03	35.78	-0.12	
	1a	-4.53	133.66	2.59	
	1b	-4.30	-62.12	-2.47	
<hr style="border-top: 1px dashed black;"/>					
1.2D + 1.0E	1	0.00	24.79	1.83	
	1a	2.33	6.44	-1.32	
	1b	-2.33	6.44	-1.32	
<hr style="border-top: 1px dashed black;"/>					
0.9D + 1.0E	1	0.00	21.60	1.96	
	1a	2.44	3.32	-1.39	
	1b	-2.44	3.32	-1.39	
<hr style="border-top: 1px dashed black;"/>					
1.0D + 1.0W Normal Wind	1	0.00	105.23	-5.46	
	1a	1.78	-36.92	-1.09	
	1b	-1.78	-36.92	-1.09	
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1.0D + 1.0W 60° Wind	1	-0.05	57.18	-2.90	
	1a	-2.54	57.39	1.41	
	1b	-3.91	-83.18	-2.26	
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1.0D + 1.0W 90° Wind	1	-0.05	10.46	-0.43	
	1a	-4.12	91.91	2.34	
	1b	-3.37	-70.99	-1.92	

Max Reactions

Leg

Overtuning

Max Uplift: -379.74 (kips)

Max Down: 408.21 (kips)

Max Shear: 21.32 (kips)

Moment: 2569.85 (ft-kips)

Total Down: 37.66 (kips)

Total Shear: 31.60 (kips)

Analysis Summary

Structure: CT46126-A-SBA	Code: EIA/TIA-222-G	5/17/2021
Site Name: Glastonbury-main St.	Exposure: C	
Height: 130.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II
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Max Reactions

	Leg	Overturning
Max Uplift:	-379.74 (kips)	Moment: 2569.85 (ft-kips)
Max Down:	408.21 (kips)	Total Down: 37.66 (kips)
Max Shear:	21.32 (kips)	Total Shear: 31.60 (kips)

Anchor Bolts

Bolt Size (in.): 1.50	Number Bolts: 8
Yield Strength (Ksi): 55.00	Tensile Strength (Ksi): 75.00
Detail Type: C	

Interaction Ratio: 0.62

Max Usages

Max Leg: 99.8% (1.2D + 1.6W Normal Wind - Sect 3)
 Max Diag: 95.8% (0.9D + 1.6W 90° Wind - Sect 3)
 Max Horiz: 34.1% (1.2D + 1.6W 60° Wind - Sect 13)

Max Deflection, Twist and Sway

Load Case	Elevation (ft)	Deflection (ft)	Twist (deg)	Sway (deg)
0.9D + 1.0E - Normal To Face	80.00	0.0400	0.0010	0.0650
	92.00	0.0546	0.0010	0.0785
	111.43	0.0858	0.0002	0.1001
	117.14	0.0959	0.0001	0.1010
	120.00	0.1009	0.0000	0.1010
	130.00	0.1185	0.0000	0.1011
0.9D + 1.6W 97 mph Wind at 60° From Face	80.00	1.1916	-0.0260	1.8243
	92.00	1.6009	-0.0240	2.1290
	111.43	2.4216	-0.0408	2.6413
	117.14	2.6825	-0.0423	2.6223
	120.00	2.8136	-0.0434	2.6773
	130.00	3.2713	-0.0410	2.6245
0.9D + 1.6W 97 mph Wind at 90° From Face	80.00	1.1945	-0.0279	1.8360
	92.00	1.6050	-0.0243	2.1404
	111.43	2.4302	0.0272	2.6862
	117.14	2.6924	0.0272	2.6269
	120.00	2.8244	0.0271	2.7349
	130.00	3.2847	0.0271	2.6402
0.9D + 1.6W 97 mph Wind at Normal To Face	80.00	1.1950	0.0402	1.8260
	92.00	1.6041	0.0438	2.1115
	111.43	2.4232	0.0267	2.5322
	117.14	2.6824	0.0258	2.6325
	120.00	2.8126	0.0250	2.5266
	130.00	3.2671	0.0253	2.6062

1.0D + 1.0W 60 mph Wind at 60° From Face	80.00	0.2854	0.0060	0.4360
	92.00	0.3833	0.0054	0.5094
	111.43	0.5796	-0.0019	0.6313
	117.14	0.6419	-0.0023	0.6266
	120.00	0.6733	-0.0025	0.6398
	130.00	0.7827	-0.0023	0.6271

1.0D + 1.0W 60 mph Wind at 90° From Face	80.00	0.2864	-0.0067	0.4398
	92.00	0.3847	-0.0059	0.5125
	111.43	0.5821	0.0018	0.6423
	117.14	0.6448	0.0016	0.6281
	120.00	0.6764	0.0015	0.6539
	130.00	0.7865	0.0015	0.6311

1.0D + 1.0W 60 mph Wind at Normal To Face	80.00	0.2877	0.0084	0.4391
	92.00	0.3859	0.0087	0.5065
	111.43	0.5820	0.0020	0.6071
	117.14	0.6441	-0.0016	0.6310
	120.00	0.6754	-0.0014	0.6057
	130.00	0.7843	-0.0014	0.6247

1.2D + 1.0Di + 1.0Wi 50 mph Wind at 60° From Face	80.00	0.3466	-0.0072	0.5335
	92.00	0.4667	-0.0065	0.6286
	111.43	0.7113	-0.0028	0.7911
	117.14	0.7897	-0.0034	0.7890
	120.00	0.8292	-0.0037	0.8048
	130.00	0.9672	-0.0036	0.7915

1.2D + 1.0Di + 1.0Wi 50 mph Wind at 90° From Face	80.00	0.3472	-0.0082	0.5375
	92.00	0.4676	-0.0074	0.6311
	111.43	0.7132	0.0027	0.8030
	117.14	0.7919	0.0025	0.7899
	120.00	0.8316	0.0024	0.8202
	130.00	0.9702	0.0024	0.7952


1.2D + 1.0Di + 1.0Wi 50 mph Wind at Normal From Face	80.00	0.3467	0.0102	0.5357
	92.00	0.4668	-0.0107	0.6214
	111.43	0.7097	-0.0031	0.7595
	117.14	0.7874	-0.0026	0.7900
	120.00	0.8266	-0.0023	0.7613
	130.00	0.9634	-0.0022	0.7845

1.2D + 1.0E - Normal To Face	80.00	0.0403	-0.0010	0.0656
	92.00	0.0549	-0.0010	0.0792
	111.43	0.0864	-0.0002	0.1008
	117.14	0.0965	-0.0001	0.1017
	120.00	0.1016	0.0000	0.1017
	130.00	0.1193	0.0000	0.1017

1.2D + 1.6W 97 mph Wind at 60° From Face	80.00	1.1992	-0.0262	1.8378
	92.00	1.6117	-0.0242	2.1459
	111.43	2.4392	-0.0413	2.6634
	117.14	2.7023	-0.0429	2.6448
	120.00	2.8345	-0.0440	2.6998
	130.00	3.2961	-0.0416	2.6469

1.2D + 1.6W 97 mph Wind at 90° From Face	80.00	1.2022	-0.0281	1.8498
	92.00	1.6159	-0.0245	2.1573
	111.43	2.4478	0.0276	2.7085
	117.14	2.7123	0.0277	2.6495
	120.00	2.8454	0.0276	2.7575
	130.00	3.3096	0.0276	2.6627

1.2D + 1.6W 97 mph Wind at Normal To Face	80.00	1.2027	0.0405	1.8397
	92.00	1.6149	-0.0442	2.1280
	111.43	2.4406	-0.0271	2.5541
	117.14	2.7020	-0.0263	2.6548
	120.00	2.8333	-0.0254	2.5488
	130.00	3.2917	-0.0258	2.6284

	Mat Foundation Design for Self Supporting Tower			Date
				5/17/2021
	Customer Name:	SBA Communications Corp	EIA/TIA Standard:	EIA-222-G
	Site Name:		Structure Height (Ft.):	130
	Site Number:	CT46126-A-SBA	Engineer Name:	Rama K.
Engr. Number:	107245	Engineer Login ID:		

Foundation Info Obtained from:

Analysis or Design?

Number of Tower Legs:

Base Reactions (Factored):

(1). Individual Leg:

Axial Load (Kips):	408.2	Uplift Force (Kips):	379.7
Shear Force (Kips):	21.3		

(2). Tower Base:

Total Vertical Load (Kips):	37.7	Total Shear Force (Kips):	31.6
Moment (Kips-ft):	2569.9		

Foundation Geometries:

Leg distance (Center-to-Center ft.):	7.5	Mods required -Yes/No ?:	No
Diameter of Pier (ft.):	Round 3.0	Pier Height A. G. (ft.):	0.50
Tower center to mat center (ft):		Depth of Base BG (ft.):	4.0
Length of Pad (ft.):	26	Width of Pad (ft.):	26
Thickness of Pad (ft):	3.00		

Material Properties and Rebar Info:

Concrete Strength (psi):	3000	Steel Elastic Modulus:	29000	ksi
Vertical bar yield (ksi)	60	Tie steel yield (ksi):	60	
Vertical Rebar Size #:	8	Tie / Stirrup Size #:	4	
Qty. of Vertical Rebars:	28	Tie Spacing (in):	12.0	
Pad Rebar Yield (Ksi):	60	Pad Steel Rebar Size (#):	8	
Concrete Cover (in.):	3	Unit Weight of Concrete:	150.0	pcf

Rebar at the bottom of the concrete pad:

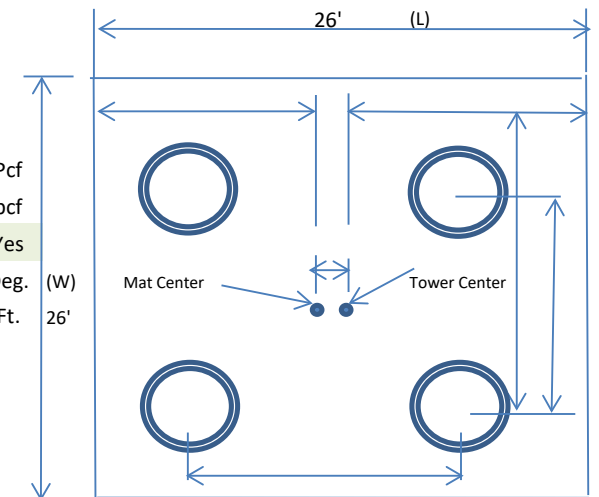
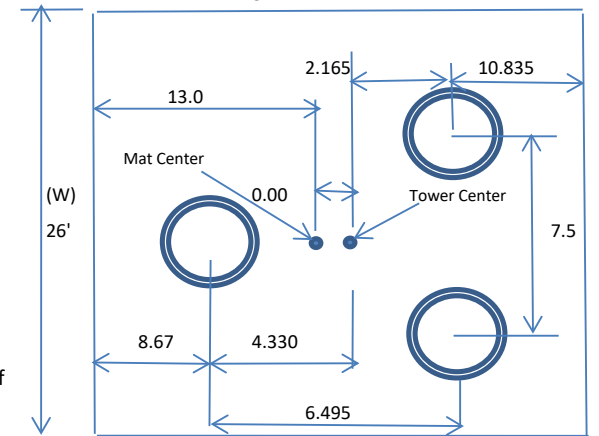
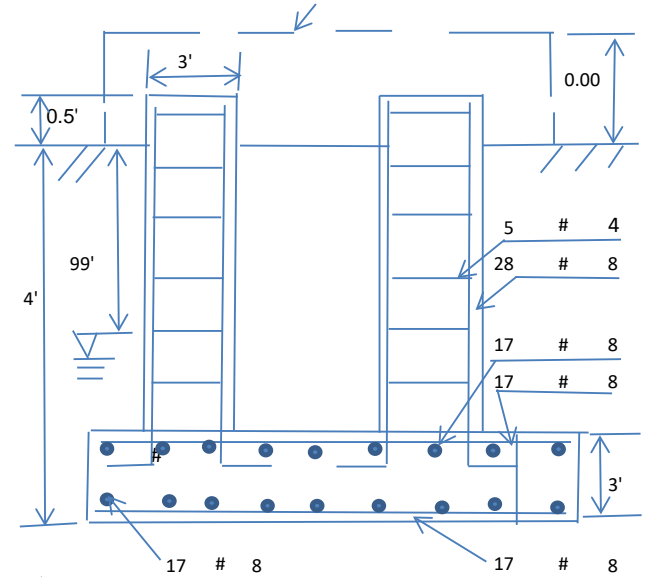
Qty. of Rebar in Pad (L):	17	Qty. of Rebar in Pad (W):	17
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Rebar at the top of the concrete pad:

Qty. of Rebar in Pad (L):	17	Qty. of Rebar in Pad (W):	17
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Soil Design Parameters:

Soil Unit Weight (pcf):	100.0	Soil Buoyant Weight:	50.0	Pcf
Water Table B.G.S. (ft):	99.0	Unit Weight of Water:	62.4	pcf
Ultimate Bearing Pressure (psf):	6000	Consider ties in concrete shear strength:	Yes	
Consider Soil Lateral Resistance ?	Yes	Enter soil C (psf) or Phi (deg.):	30.0	Deg. (W)
		Depth to ignor lateral resistance	1.0	Ft. 26'



Apply 1.35 for e/w per G/H: 1.35

Foundation Analysis and Design:	Uplift Strength Reduction Factor:	0.75	Compression Strength Reduction Factor:	0.75
Total Dry Soil Volume (cu. Ft.):	654.79	Total Dry Soil Weight (Kips):	65.48	
Total Buoyant Soil Volume (cu. Ft.):	0.00	Total Buoyant Soil Weight (Kips):	0.00	
Total Effective Soil Weight (Kips):	65.48	Weight from the Concrete Block at Top (K):	0.00	
Total Dry Concrete Volume (cu. Ft.):	2059.81	Total Dry Concrete Weight (Kips):	308.97	
Total Buoyant Concrete Volume (cu. Ft.):	0.00	Total Buoyant Concrete Weight (Kips):	0.00	
Total Effective Concrete Weight (Kips):	308.97	Total Vertical Load on Base (Kips):	412.11	

Check Soil Capacities:

Calculated Maxium Net Soil Pressure under the base (psf):	1764.50	<	Allowable Factored Soil Bearing (psf):	4500	0.39	OK!
Allowable Foundation Overturning Resistance (kips-ft.):	4870.7	>	Design Factored Momont (kips-ft):	2712	0.56	OK!
Factor of Safety Against Overturning (O. R. Moment/Design Moment):	1.80					OK!

Check the capacities of Reinforcing Concrete:

Strength reduction factor (Flexure and axial tension):	0.90	Strength reduction factor (Shear):	0.75
Strength reduction factor (Axial compression):	0.65	Wind Load Factor on Concrete Design:	1.00

(1) Concrete Pier:

Vertical Steel Rebar Area (sq. in./each):	0.79	Tie / Stirrup Area (sq. in./each):	0.20		
Calculated Moment Capacity (Mn,Kips-Ft):	942.8	>	Design Factored Moment (Mu, Kips-Ft)	32.0	0.03 OK!
Calculated Shear Capacity (Kips):	61.5	>	Design Factored Shear (Kips):	21.3	0.35 OK!
Calculated Tension Capacity (Tn, Kips):	1194.5	>	Design Factored Tension (Tu Kips):	379.7	0.32 OK!
Calculated Compression Capacity (Pn, Kips):	1320.4	>	Design Factored Axial Load (Pu Kips):	408.2	0.31 OK!
Moment & Tension Strength Combination:	0.03	OK!	Check Tie Spacing (Design/Req'd):	1.00	
Pier Reinforcement Ratio:	0.022		Reinforcement Ratio is satisfied per ACI		

(2).Concrete Pad:

One-Way Design Shear Capacity (L or W Direction, Kips):	833.1	>	One-Way Factored Shear (L/W-Dir Kips)	217.5	0.26	OK!
One-Way Design Shear Capacity (Diagonal Dir., Kips):	842.5	>	One-Way Factored Shear (Dia. Dir, Kips)	184.4	0.22	OK!
Lower Steel Pad Reinforcement Ratio (L or W-Direct.):	0.0013		Lower Steel Reinf. Ratio (Dia. Dir.):	0.0011		
Lower Steel Pad Moment Capacity (L or W-Dir. Kips-ft):	1933.5	>	Moment at Bottom (L-Direct. K-Ft):	1553.1	0.80	OK!
Lower Steel Pad Moment Capacity (Dia. Direction,K-ft):	1937.6	>	Moment at Bottom (Dia. Dir. K-Ft):	1286.0	0.66	OK!
Upper Steel Pad Reinforcement Ratio (L or W -Direction):	0.0013		Upper Steel Reinf. Ratio (Dia. Dir.):	0.0011		
Upper Steel Pad Moment Capacity (L or W-Dir., Kips-ft):	1933.5	>	Moment at the top (L-Dir Kips-Ft):	703.0	0.36	OK!
Upper Steel Pad Moment Capacity (Dia. Direction, K-ft):	1937.6	>	Moment at the top (Dia. Dir., K-Ft):	428.5	0.22	OK!
Punching Failure Capacity (Kips):	1149.2	>	Punch. Failure Factored Shear (K):	408.2	0.36	OK!



Maser Consulting Connecticut
2000 Midlantic Drive Suite 100
Mt. Laurel, NJ 08054
856.797.0412
greg.dulnik@colliersengineering.com

Post-Mod Antenna Mount Analysis Report and PMI Requirements

Mount Fix

SMART Tool Project #: 10044065
Maser Consulting Connecticut Project #: 20777629A

March 10, 2021

Site Information

Site ID: 468258-VZW / Glastonbury West CT
Site Name: Glastonbury West CT
Carrier Name: Verizon Wireless
Address: 2577 Main St.
Glastonbury, Connecticut 06033
Hartford County
Latitude: 41.7143289°
Longitude: -72.613028°

Structure Information

Tower Type: Self Support
Mount Type: 12.50-Ft T-Frame

FUZE ID # 16232043

Analysis Results

T-Frame: **65.9% Pass**

***Contractor PMI Requirements:

Included at the end of this MA report

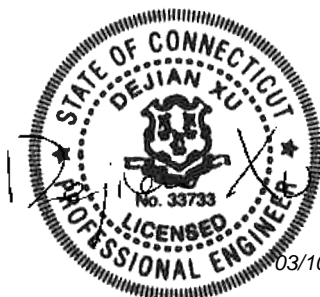
Available & Submitted via portal at <https://pmi.vzwsmart.com>

Contractor - Please Review Specific Site PMI Requirements Upon Award

Requirements also Noted on Mount Modification Drawings

Requirements may also be Noted on A & E drawings

Report Prepared By: Selene Chen



03/10/2021

Executive Summary:

The objective of this report is to summarize the analysis results of the antenna support mount including the proposed modifications at the subject facility for the final wireless telecommunications configuration, per the applicable codes and standards.

This analysis is inclusive of the mount structure only and does not address the structural capacity of the supporting structure. This mounting frame was not analyzed as an anchor attachment point for fall protection. All climbing activities are required to have a fall protection plan completed by a competent person.

Sources of Information:

Document Type	Remarks
<i>Radio Frequency Data Sheet (RFDS)</i>	<i>Verizon RFDS Site ID: 1702842, dated December 16, 2020</i>
<i>Mount Mapping Report</i>	<i>RKS Design & Engineering LLC., Site ID: SBA : CT46126, dated January 10, 2021</i>
<i>Mount Analysis Report</i>	<i>Maser Consulting Connecticut, Project #: 20777629A, dated February 4, 2021</i>
<i>Mount Modification Drawings</i>	<i>Maser Consulting Connecticut, Project #: 20777629A, dated March 9, 2021</i>

Analysis Criteria:

Codes and Standards:	ANSI/TIA-222-H
Wind Parameters:	Basic Wind Speed (Ultimate 3-sec. Gust), V_{ULT} : 118 mph
	Ice Wind Speed (3-sec. Gust): 50 mph
	Design Ice Thickness: 1.50 in
	Risk Category: II
	Exposure Category: C
	Topographic Category: 1
	Topographic Feature Considered: N/A
	Topographic Method: N/A
	Ground Elevation Factor, K_e : 0.999
Seismic Parameters:	S_s : 0.196
	S_1 : 0.055
Maintenance Parameters:	Wind Speed (3-sec. Gust): 30 mph
	Maintenance Live Load, L_v : 250 lbs.
	Maintenance Live Load, L_m : 500 lbs.
Analysis Software:	RISA-3D (V17)

- Any mount modifications listed under Sources of Information are assumed to have been installed per the design specifications.

Discrepancies between in-field conditions and the assumptions listed above may render this analysis invalid unless explicitly approved by Maser Consulting Connecticut.

Analysis Results:

Component	Utilization %	Pass/Fail
<i>Antenna Pipe</i>	39.6%	<i>Pass</i>
<i>Tie Back</i>	17.8%	<i>Pass</i>
<i>Face Vertical</i>	48.2%	<i>Pass</i>
<i>Face Horizontal</i>	61.7%	<i>Pass</i>
<i>Standoff Horizontal</i>	30.1%	<i>Pass</i>
<i>Standoff Pipe</i>	65.9%	<i>Pass</i>
<i>Mast Pipe</i>	39.6%	<i>Pass</i>
<i>Standoff Vertical</i>	62.3%	<i>Pass</i>
<i>Kicker</i>	18.8%	<i>Pass</i>
<i>Connection Check</i>	26.4%	<i>Pass</i>

Structure Rating – (Controlling Utilization of all Components)	65.9%
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Recommendation:

The existing mounts will be **SUFFICIENT** for the final loading after the proposed modifications are successfully completed.

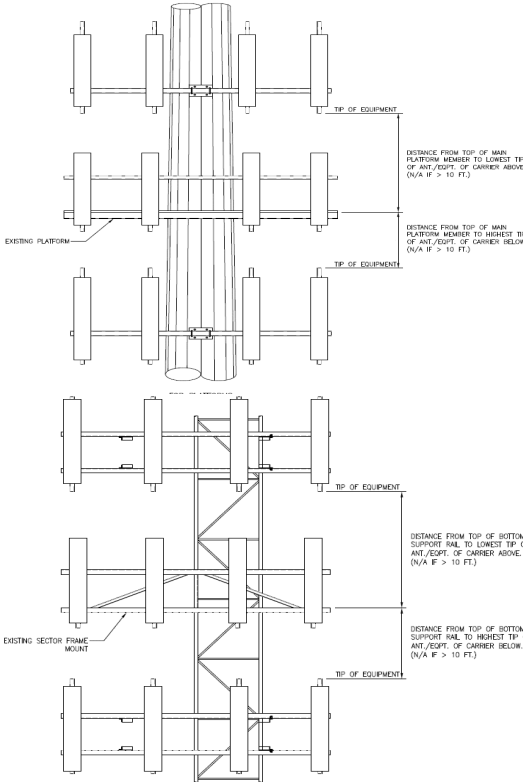
ANSI/ASSP rigging plan review services compliant with the requirements of ANSI/TIA 322 are available for a Construction Class IV site or other, if required. Separate review fees will apply.

Attachments:

- Mount Photos
- Mount Mapping Report (for reference only)
- Analysis Calculations
- 4. Contractor Required PMI Report Deliverables**
- Antenna Placement Diagrams
- TIA Adoption and Wind Speed Usage Letter



Mount Azimuth (Degree) for Each Sector			Tower Leg Azimuth (Degree) for Each Sector			Sector B										
Sector A:	50.00	Deg	Leg A:	60.00	Deg	Ant _{1a}	B4 RRH2x60-4R	10.63	5.75	36.50		81.0833	40.00	-9.00		218
Sector B:	150.00	Deg	Leg B:	180.00	Deg	Ant _{1b}	HBXX-6517DS-A2M	12.00	6.50	75.00		81.875	30.50	8.75	150.00	21, 218
Sector C:	350.00	Deg	Leg C:	300.00	Deg	Ant _{1c}										219
Sector D:		Deg	Leg D:		Deg	Ant _{2a}										219
Climbing Facility Information						Ant _{2b}	LNX-6514DS-A1M	12.00	7.00	72.75		81.5625	34.00	7.50	150.00	21, 219
Location:	300.00	Deg	On Leg C			Ant _{2c}										
Climbing Facility	Corrosion Type:	N/A				Ant _{3a}	B25 RRH 4x30	12.00	7.00	21.00		81.2917	37.00	-7.00		223
	Access:	Climbing path was unobstructed.				Ant _{3b}	HBXX-6517DS-A2M	12.00	6.50	75.00		81.8333	30.50	8.75	150.00	21, 223
	Condition:	Good condition.				Ant _{3c}										
						Ant _{4a}										
						Ant _{4b}	LNX-6514DS-A1M	12.00	7.00	72.75		81.5625	34.00	7.50	150.00	21, 223
						Ant _{4c}										
						Ant _{5a}										
						Ant _{5b}										
						Ant _{5c}										
						Ant on Standoff	RRFDC-3315-PF-48	15.00	10.00	18.50			57.00	6.00		485
						Ant on Standoff	B13 RRH4x30	12.00	7.50	21.00			24.75	-7.00		219
						Ant on Tower										
						Ant on Tower										
						Sector C										
						Ant _{1a}	B4 RRH2x60-4R	10.63	5.75	36.50		81	41.00	-9.00		225
						Ant _{1b}	HBXX-6517DS-A2M	12.00	6.50	75.00		81.8125	31.25	8.75	350.00	28, 225
						Ant _{1c}										
						Ant _{2a}										
						Ant _{2b}	LNX-6514DS-A1M	12.00	7.00	72.75		81.625	33.00	8.00	350.00	28, 229
						Ant _{2c}										
						Ant _{3a}	B25 RRH 4x30	12.00	7.00	21.00		81.2083	37.00	-7.00		233
						Ant _{3b}	HBXX-6517DS-A2M	12.00	6.50	75.00		81.7917	30.00	10.00	350.00	28, 234
						Ant _{3c}										
						Ant _{4a}										
						Ant _{4b}	LNX-6514DS-A1M	12.00	7.00	72.75		81.6667	33.00	8.00	350.00	28, 234
						Ant _{4c}										
						Ant _{5a}										
						Ant _{5b}										
						Ant _{5c}										
						Ant on Standoff	B13 RRH4x30	12.00	7.50	21.00			24.75	-7.00		229
						Ant on Standoff										
						Ant on Tower										
						Ant on Tower										
						Sector D										
						Ant _{1a}										
						Ant _{1b}										
						Ant _{1c}										
						Ant _{2a}										
						Ant _{2b}										
						Ant _{2c}										
						Ant _{3a}										
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						Ant _{4a}										
						Ant _{4b}										
						Ant _{4c}										
						Ant _{5a}										
						Ant _{5b}										
						Ant _{5c}										
						Ant on Standoff										
						Ant on Standoff										
						Ant on Tower										
						Ant on Tower										



Observed Safety and Structural Issues During the Mount Mapping		
Issue #	Description of Issue	Photo #

1	COAX : TOTAL (2) 1.50"Ø HYBRID	50
2		
3		
4		
5		
6		
7		
8		

Mapping Notes

1. Please report any visible structural or safety issues observed on the antenna mounts (Damaged members, loose connections, tilting mounts, safety climb issues, etc.)
2. If the thickness of the existing pipes or tubing can't be obtained from a general tool (such as Caliper), please use an ultrasonic measurement tool (thickness gauge) to measure the thickness.
3. Please create all required detail sketches of the mounts and insert them into the "Sketches" tab.
4. Please measure and enter the bolt sizes and types under the Members Box in the spreadsheet of the mount type.
5. Take and label the photos of the tower, mounts, connections, antennas and all measurements. Minimum 50 photos are required.
6. Please measure and report the size and length of all existing antenna mounting pipes.
7. Please measure and report the antenna information for all sectors.
8. Don't delete or rearrange any sheet or contents of any sheet from this mapping form.

Standard Conditions

1. Obvious safety and structural issues/deficiencies noticed at the time of the mount mapping are to be reported in this mapping. However, this mount mapping is not a condition assessment of the mount.



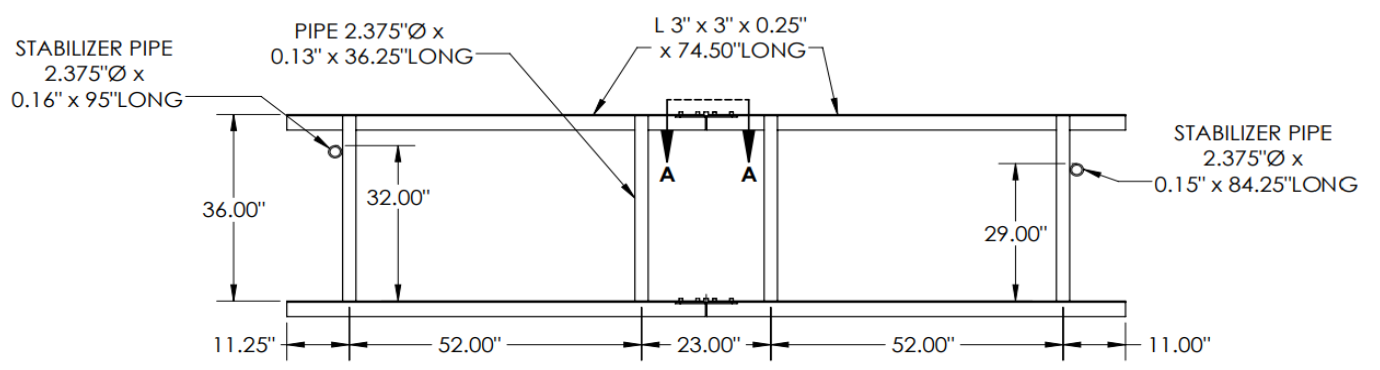
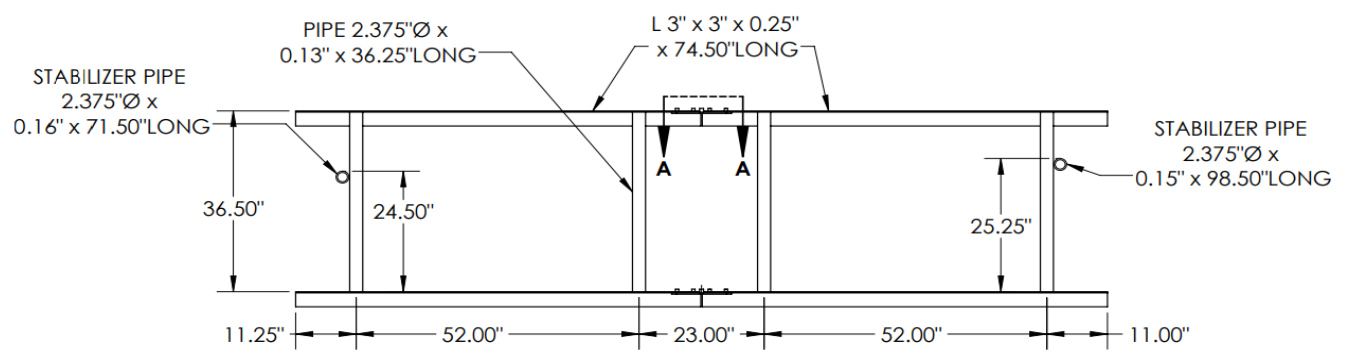
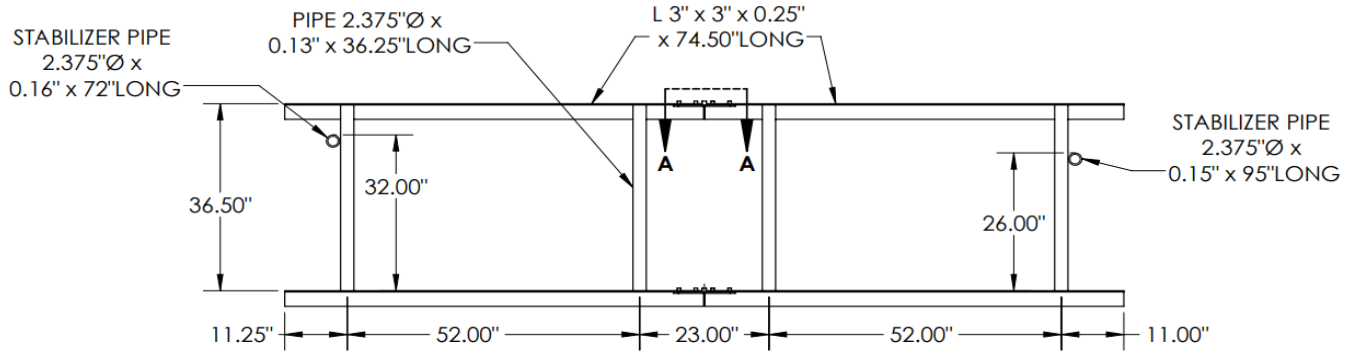
Antenna Mount Mapping Form (PATENT PENDING)

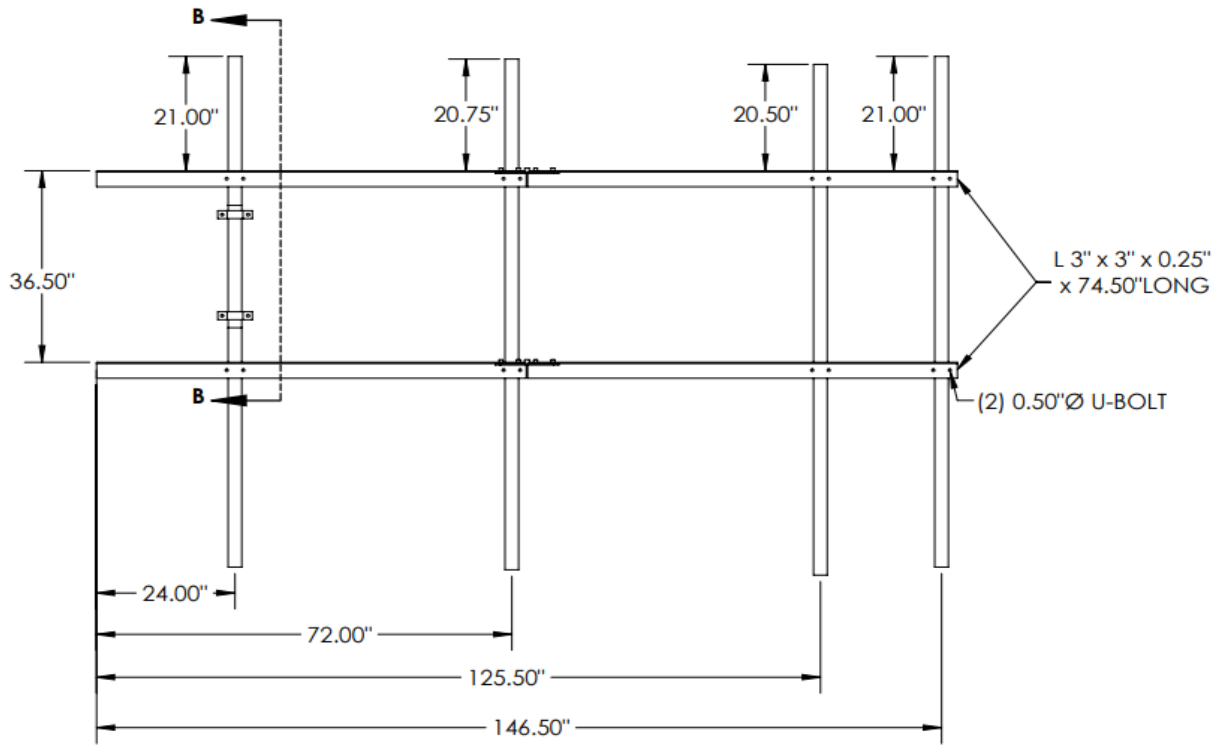
FCC #
1231397

Tower Owner:	SBA	Mapping Date:	1/10/2021
Site Name:	VZW : GLASTONBURY WEST CT.	Tower Type:	Self Support
Site Number or ID:	SBA : CT46126	Tower Height (Ft.):	UNKNOWN
Mapping Contractor:	RKS Design & Engineering LLC.	Mount Elevation (Ft.):	80.75

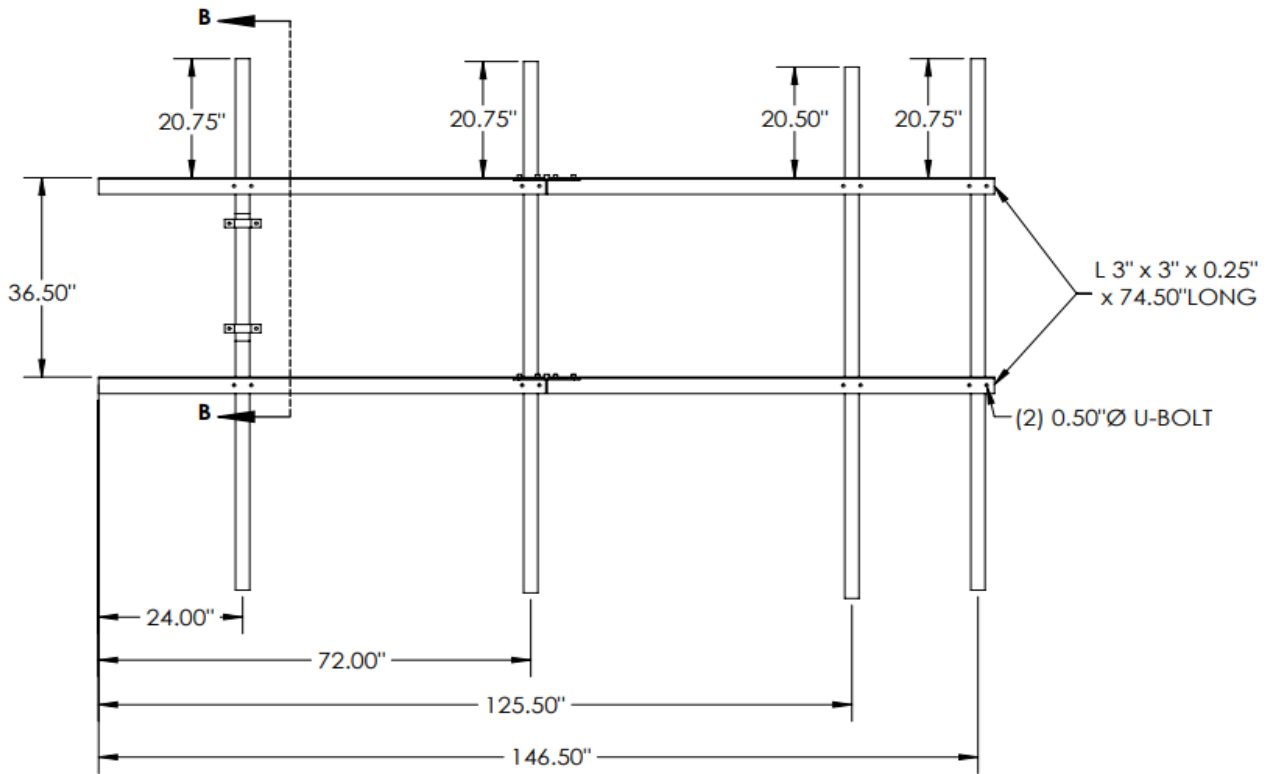
This antenna mapping form is the property of TES and under **PATENT PENDING**. The formation contained herein is considered confidential in nature and is to be used only for the specific customer it was intended for. Reproduction, transmission, publication, modification or disclosure by any method is prohibited except by express written permission of TES. All means and methods are the responsibility of the contractor and the work shall be compliant with ANSI/ASSE A 10.48, OSHA, FCC, FAA and other safety requirements that may apply. TES is not warranting the usability of the safety climb as it must be assessed prior to each use in compliance with OSHA requirements.

Please Insert Sketches of the Antenna Mount

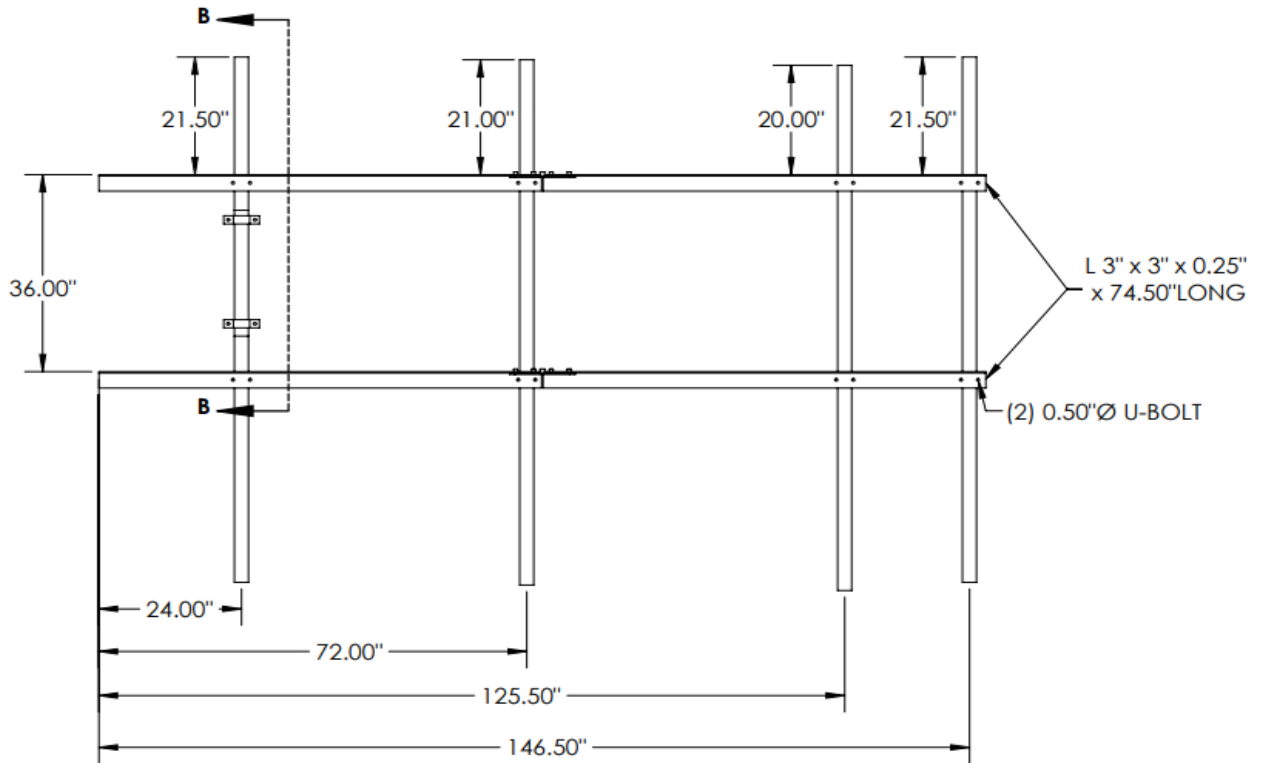




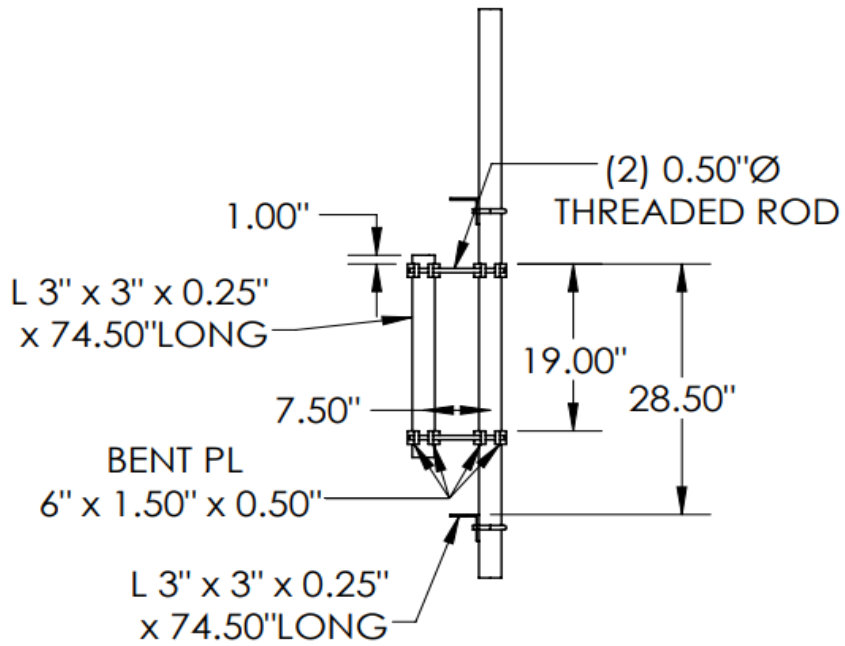
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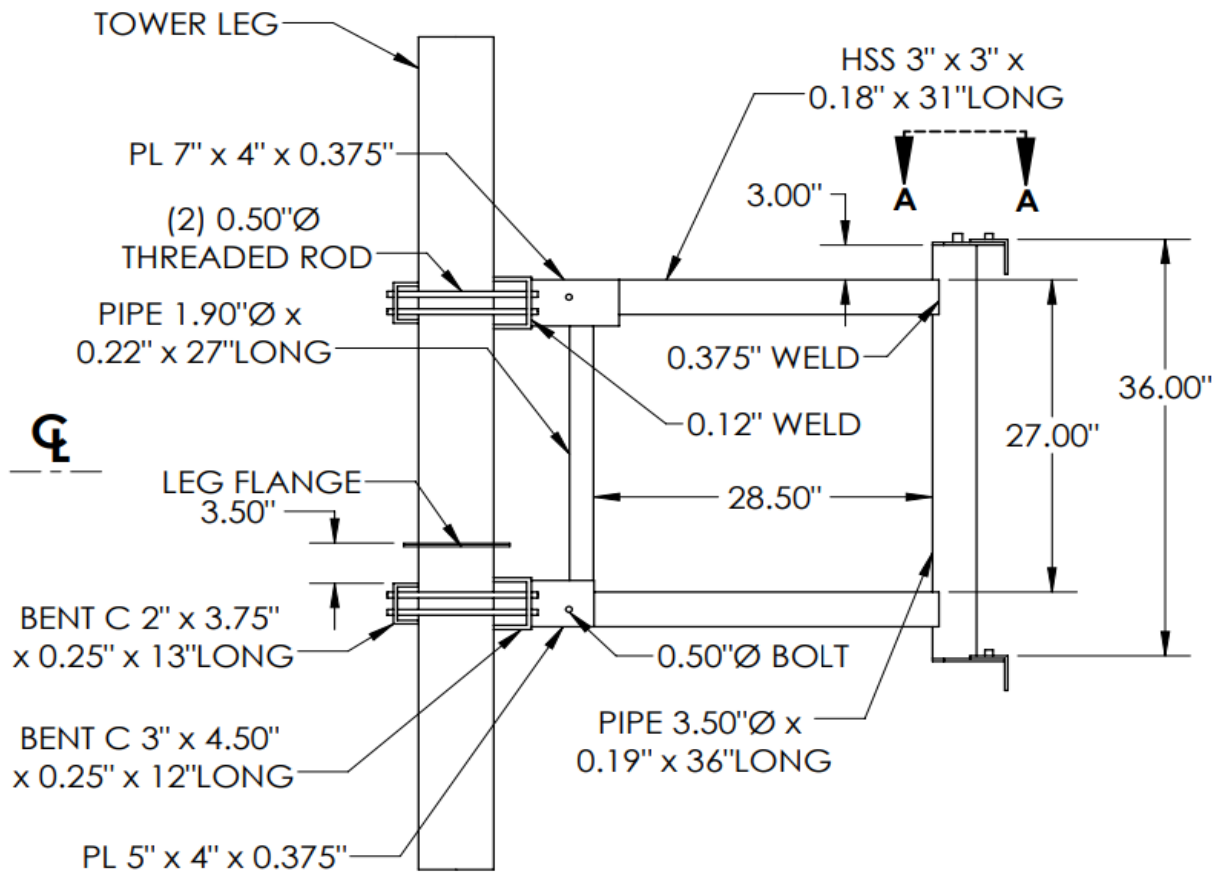
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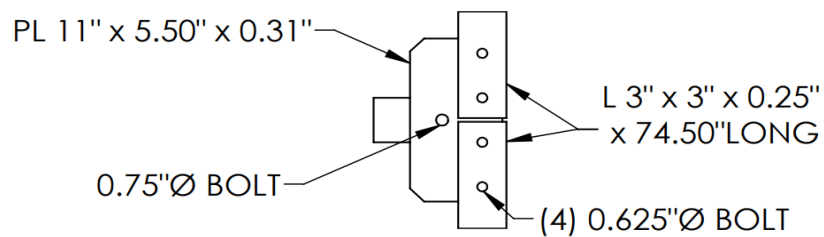
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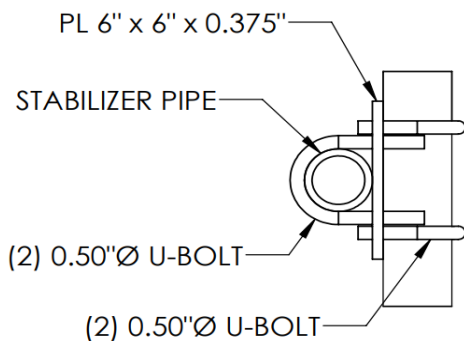
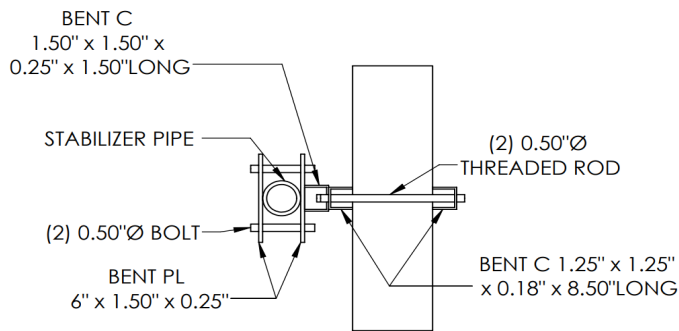
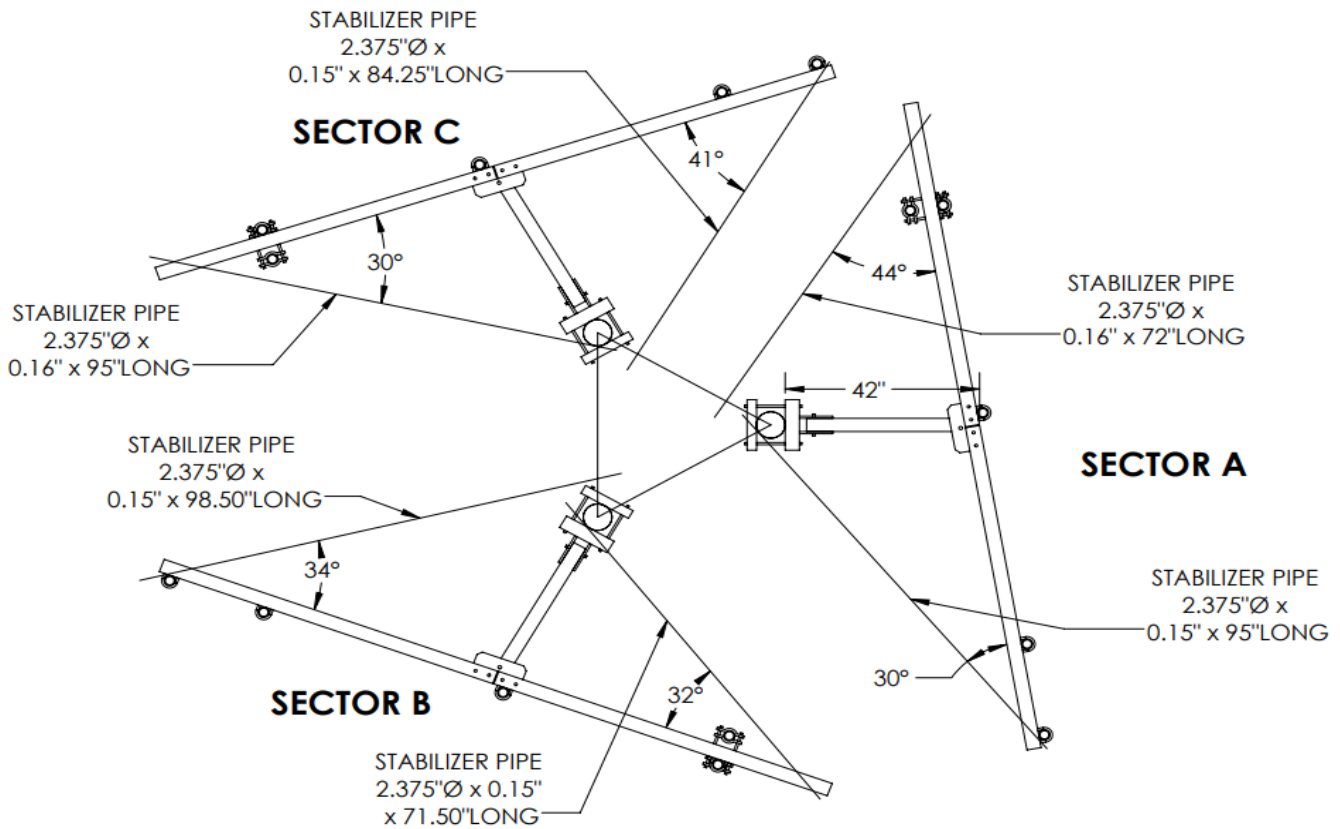
SECTION B-B

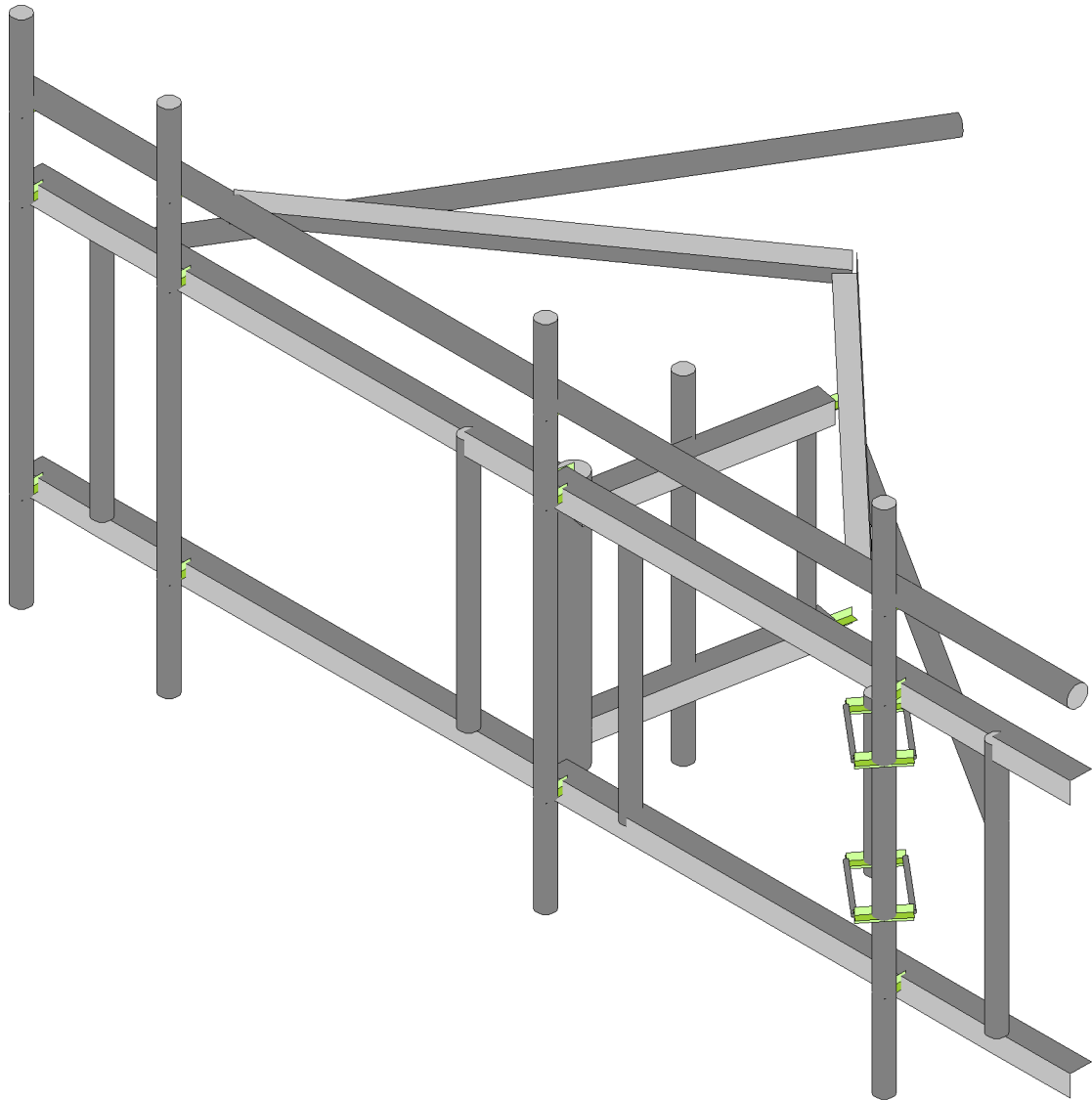
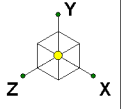


STAND-OFF ASSEMBLY : 1



SECTION A-A





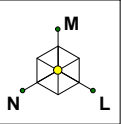
Maser Consulting

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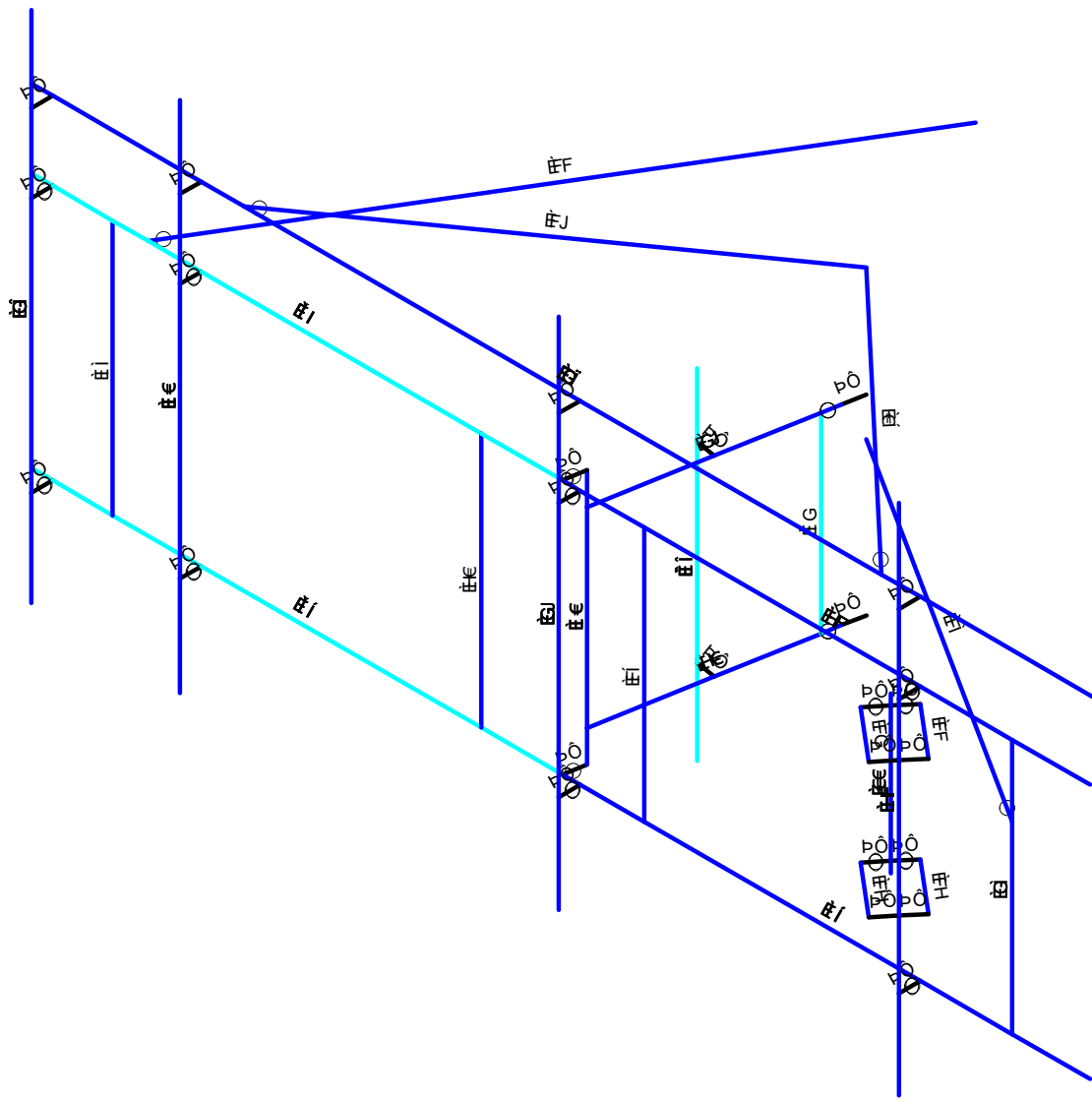
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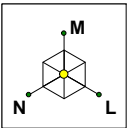


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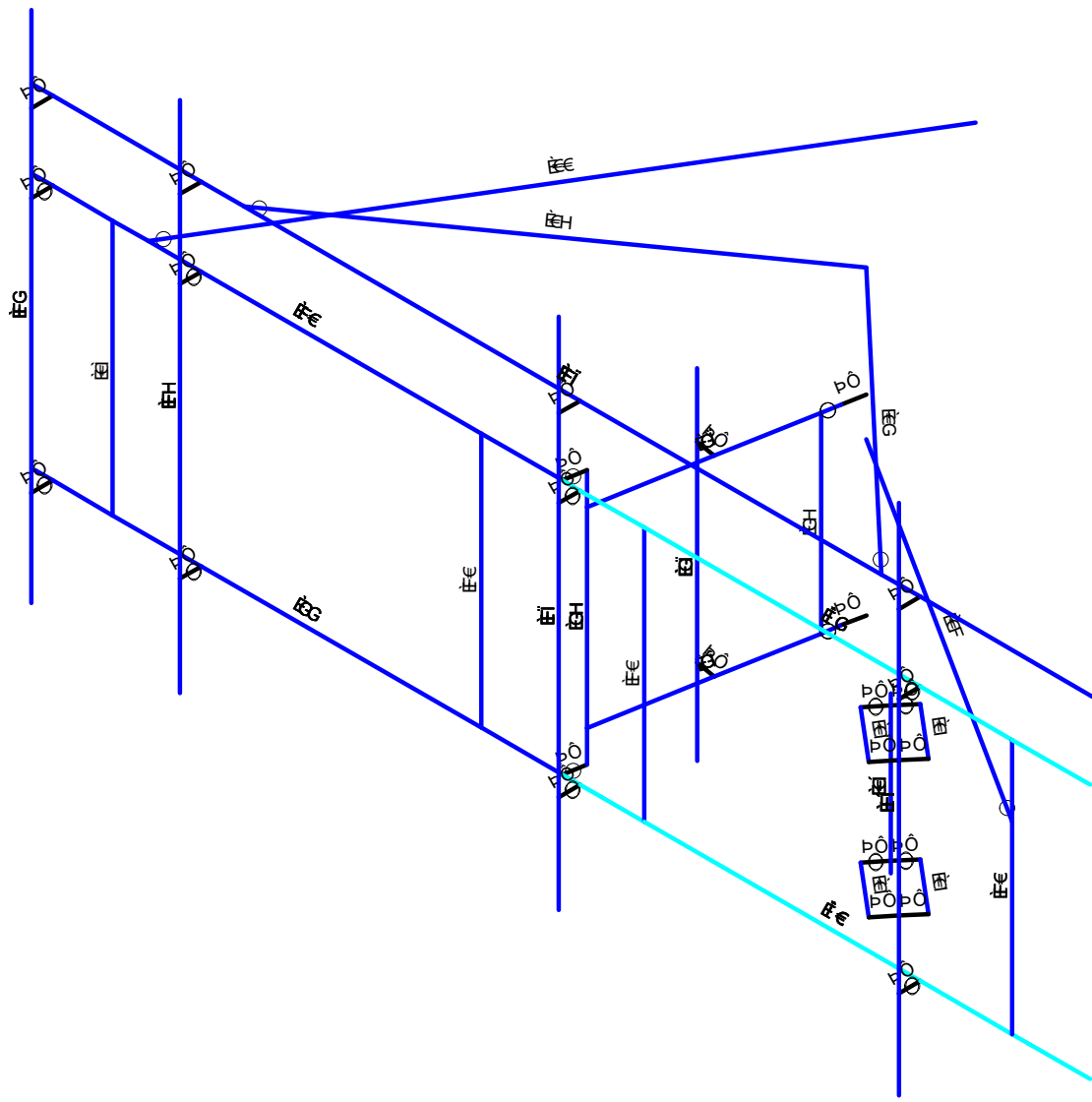
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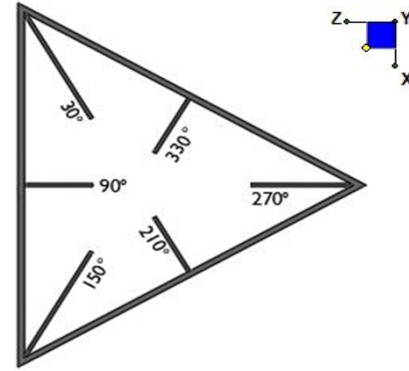
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I. Mount-to-Tower Connection Check

RISA Model Data

Nodes (labeled per RISA)	Orientation (per graphic of typical platform)
N45	90
N46A	90



TYPICAL PLATFORM

Tower Connection Bolt Checks

Any moment resistance?:

Bolt Quantity per Reaction:

d_x (in) (Delta X of typ. bolt config. sketch) :

d_y (in) (Delta Y of typ. bolt config. sketch) :

Bolt Type:

Bolt Diameter (in):

Required Tensile Strength (kips):

Required Shear Strength (kips):

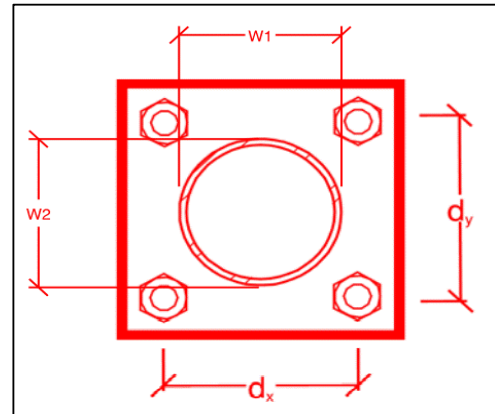
Tensile Strength / bolt (kips):

Shear Strength / bolt (kips):

Tensile Capacity Overall:

Shear Capacity Overall:

yes
4
11
3
A307
0.5
6.3
4.1
6.4
3.8
24.4%*
26.4%



*Note: Tension reduction not required if tension or shear capacity < 30%

Mount Desktop – Post Modification Inspection (PMI) Report Requirements

Documents & Photos Required from Contractor – Mount Modification

Purpose – to provide Maser Consulting Connecticut the proper documentation in order to complete the required Mount Desktop review of the Post Modification Inspection Report.

- Contractor is responsible for making certain the photos provided as noted below provide confirmation that the modification was completed in accordance with the modification drawings.
- Contractor shall relay any data that can impact the performance of the mount or the mount modification, this includes safety issues.

Base Requirements:

- Any special photos outside of the standard requirements will be indicated on the drawings
- Provide “as built drawings” showing contractor’s name, preparer’s signature, and date. Any deviations from the drawings (proposed modification) must be shown.
- Notation that all hardware was properly installed, and the existing hardware was inspected for any issues.
- Verification that loading is as communicated in the modification drawings. NOTE If loading is different than what is conveyed in the modification drawing contact Maser Consulting Connecticut immediately.
- Each photo should be time and date stamped
- Photos should be high resolution and submitted in a Zip File and should be organized in the file structure as depicted in Schedule A attached.
- Contractor shall ensure that the safety climb wire rope is supported and not adversely impacted by the install of the modification components. This may involve the install of wire rope guides, or other items to protect the wire rope.
- The photos in the file structure should be uploaded to <https://pmi.vzwsmart.com> as depicted on the drawings

Photo Requirements:

- Base and “During Installation Photos”
 - Base pictures include
 - Photo of Gate Signs showing the tower owner, site name, and number
 - Photo of carrier shelter showing the carrier site name and number if available
 - Photos of the galvanizing compound and/or paint used (if applicable), clearly showing the label and name
 - “During Installation Photos if provided - must be placed only in this folder
- Photos taken at ground level
 - Overall tower structure before and after installation of the modifications
 - Photos of the appropriate mount before and after installation of the modifications; if the mounts are at different rad elevations, pictures must be provided for all elevations that the modifications were installed

- Photos taken at Mount Elevation
 - Photos showing each individual sector before and also after installation of modifications. Each entire sector must be in one photo to show in the inter-connection of members.
 - These photos should also certify that the placement and geometry of the equipment on the mount is as depicted on the sketch and table in the mount analysis
 - Close-up photos of each installed modification per the modification drawings; pictures should also include connection hardware (U-bolts, bolts, nuts, all-threaded rods, etc.)
 - Photos showing the measurements of the installed modification member sizes (i.e. lengths, widths, depths, diameters, thicknesses)
 - Photos showing the elevation or distances of the installed modifications from the appropriate reference locations shown in the modification drawings
 - Photos showing the installed modifications onto the tower with tape drop measurements (if applicable) (i.e. ring/collar mounts, tie-backs, V-bracing kits, etc.); if the existing mount elevation needs to be changed according to the modification drawings, a tape drop measurement shall be provided before the elevation change
 - Photos showing the safety climb wire rope above and below the mount prior to modification.
 - Photos showing the climbing facility and safety climb if present.

Material Certification:

- Materials utilized must be as per specification on the drawings or the equivalent as validated by Maser Consulting Connecticut.
 - If the drawings are as specified on the drawings
 - The contractor should provide the packing list or the materials utilized to perform the mount modification
 - If an equivalent is utilized
 - It is required that the Maser Consulting Connecticut certification of such is included in the contractor submission package. There may be an additional charge for this certification if the equivalent submission doesn't meet specifications as prescribed in the drawings.
- The contractor must certify that the materials meet these specifications by one of these methods.

The Material utilized was as specified on the Maser Consulting Connecticut Mount Modification Drawings and included in the Material certification folder is a packing list or invoice for these materials


















The material utilized was an "equivalent" and included as part of the contractor submission is the TES certification, invoices, or specifications validating accepted status

Certifying Individual: Company _____

Name _____

Signature _____

Schedule A – Photo & Document File Structure

-  VzW Site Number / Name
 -  Base & “During Installation” Photos
 -  Pre-Installation Photos
 -  Alpha
 -  Beta
 -  Gamma
 -  Ground Level
 -  Tape Drop
 -  Post-Installation Photos
 -  Alpha
 -  Beta
 -  Gamma
 -  Ground Level
 -  Tape Drop
 -  Photos of climbing facility and safety climb – If Present
-  Certifications – Submission of this document including certifications
-  Specific Required Additional Photos

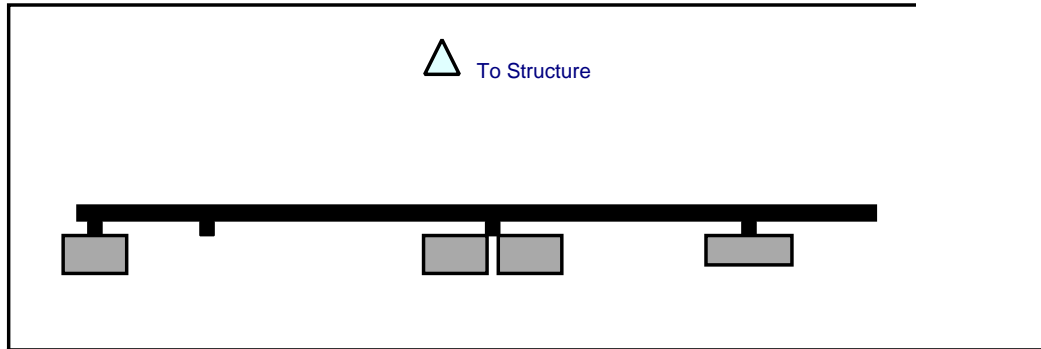
Sector: A
 Structure Type: Self Support
 Mount Elev: 79.00

3/10/2021

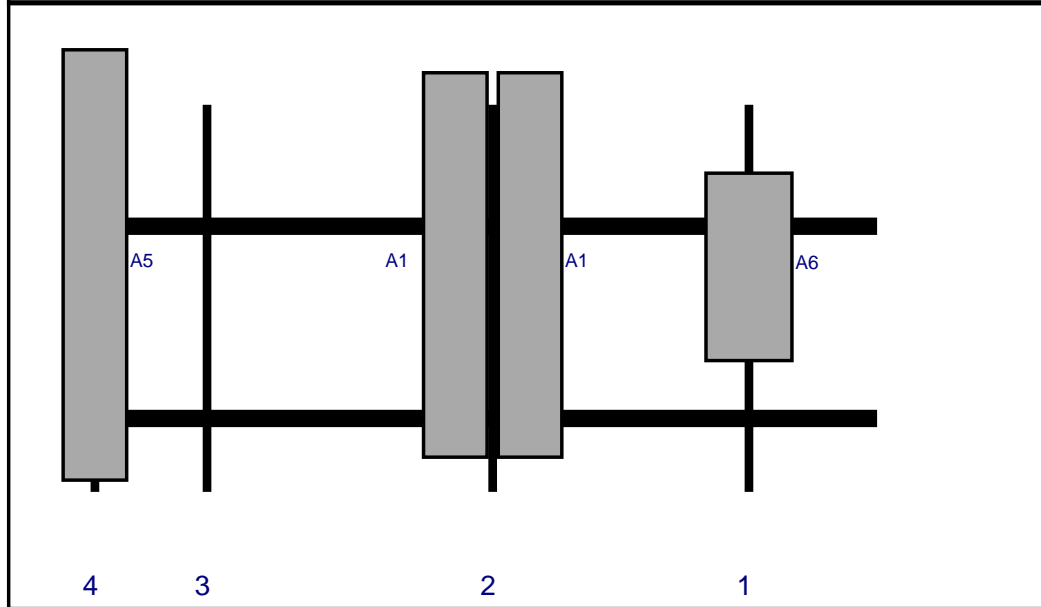
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Plan View

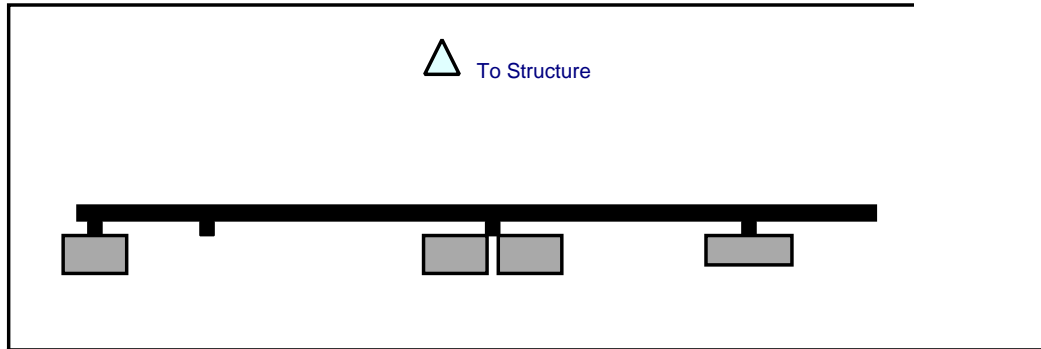


Front View
 Looking at Structure

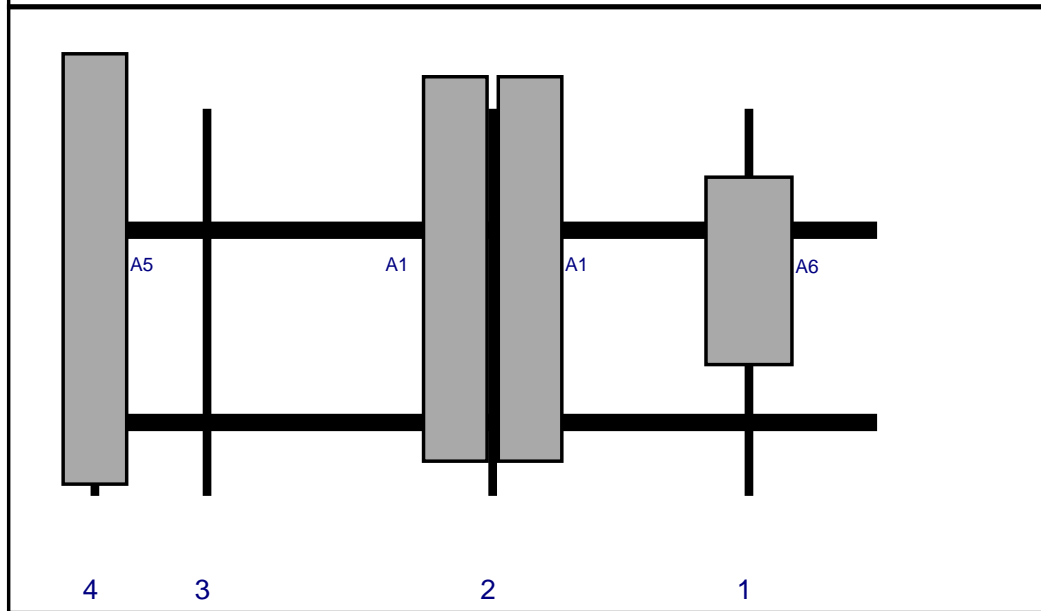


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A1	NHH-65B-R2B	72	11.9	78	2	a	Front	30	-7	Added	
A1	NHH-65B-R2B	72	11.9	78	2	b	Front	30	7	Added	
A5	LNK-6514DS-A1M	80.6	11.9	3.5	4	a	Front	30	0	Retained	01/10/2021

Plan View

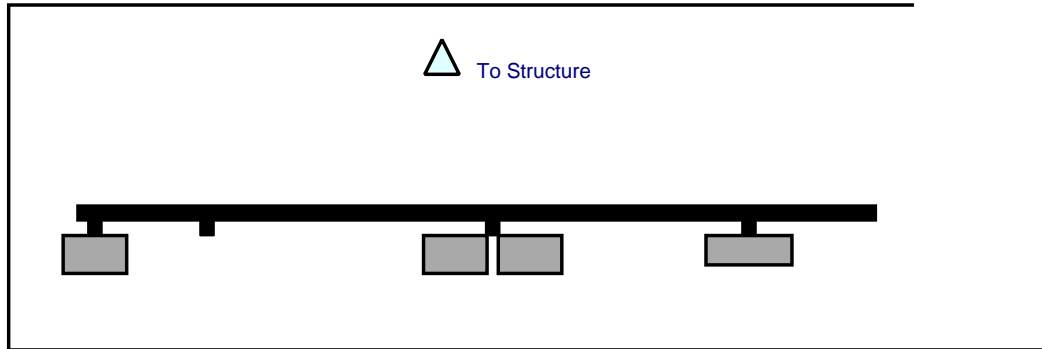


Front View
Looking at Structure

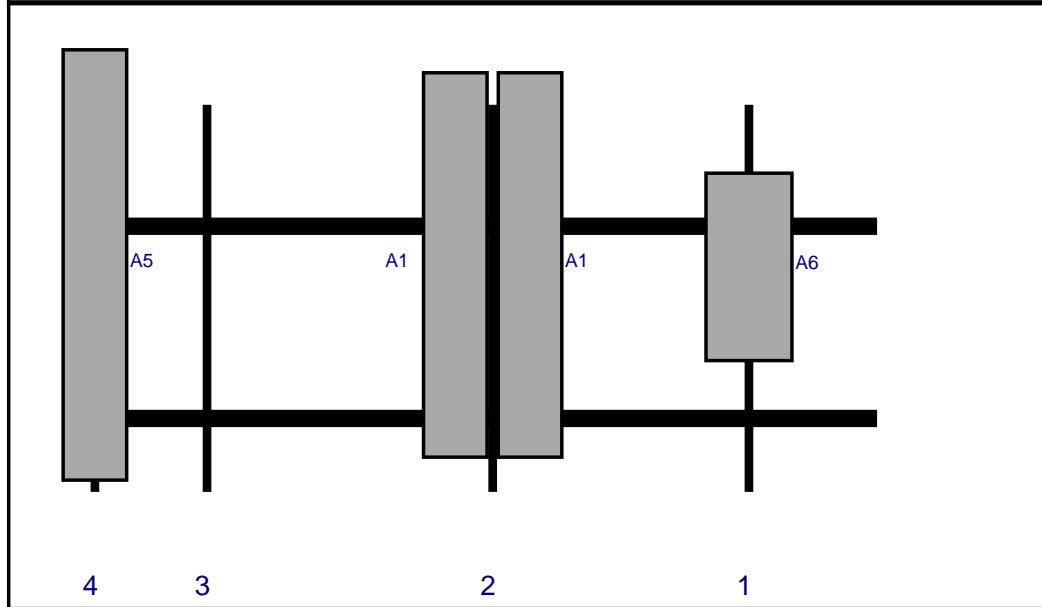


Ref#	Model	Height (in)	Width (in)	H Dist Frm L.	Pipe #	Pipe Pos V	Ant Pos	C. Ant Frm T.	Ant H Off	Status	Validation
A6	VZS01	35.1	16.1	126	1	a	Front	30.36	0	Added	
A1	NHH-65B-R2B	72	11.9	78	2	a	Front	30	-7	Added	
A1	NHH-65B-R2B	72	11.9	78	2	b	Front	30	7	Added	
A5	LNx-6514DS-A1M	80.6	11.9	3.5	4	a	Front	30	0	Retained	01/10/2021

Plan View



Front View
 Looking at Structure



Ref#	Model	Height (in)	Width (in)	H Dist Frm L.	Pipe #	Pipe Pos V	Ant Pos	C. Ant Frm T.	Ant H Off	Status	Validation
A6	VZS01	35.1	16.1	126	1	a	Front	30.36	0	Added	
A1	NHH-65B-R2B	72	11.9	78	2	a	Front	30	-7	Added	
A1	NHH-65B-R2B	72	11.9	78	2	b	Front	30	7	Added	
A5	LNX-6514DS-A1M	80.6	11.9	3.5	4	a	Front	30	0	Retained	01/10/2021

Maser Consulting Connecticut

Subject TIA-222-H Adoption and Wind Speed Usage

Site Information Site ID: 468258-VZW / Glastonbury West CT
Site Name: Glastonbury West CT
Carrier Name: Verizon Wireless
Address: 2577 Main St.
Glastonbury, Connecticut 06033
Hartford County

Latitude: 41.7143289°
Longitude: -72.613028°

Structure Information Tower Type: Self Support
Mount Type: 12.50-Ft T-Frame

To Whom It May Concern,

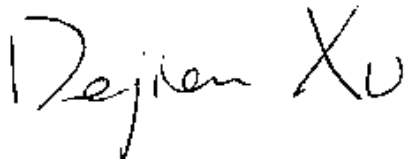
We respectfully submit the above referenced Antenna Mount Structural Analysis report in conformance with ANSI/TIA-222-H, Structural Standard for Antenna Supporting Structures and Antennas and Small Wind Turbine Support Structures.

The 2015 International Building Code states that, in Section 3108, telecommunication towers shall be designed and constructed in accordance with the provisions of TIA-222. The TIA-222-H is the latest revision of the TIA-222 Standard, effective as of January 01, 2018.

As with all ANSI standards and engineering best practice is to apply the most current revision of the standard. This ensures the engineer is applying all updates. As an example, the TIA-222-H standard includes updates to bring it in line with the latest AISC and ACI standards and it also incorporates the latest wind speed maps by ASCE 7 based on updated studies of the wind data.

The TIA-222-H standard clarifies these specific requirements for the antenna mount analysis such as modeling methods, seismic analysis, 30-degree increment wind directions and maintenance loading. Therefore, it is our opinion that TIA-222-H is the most appropriate standard for antenna mount structural analysis and is acceptable for use at this site to ensure the engineer is taking into account the most current engineering standard available.

Sincerely,



Dejian Xu, PE
Technical Specialist

March 29, 2021

Mr. Andrew Leone
Verizon Wireless
20 Alexander Dr.
Wallingford, CT 06492

Re: Verizon Wireless antenna Model Clarification for CT Siting Council

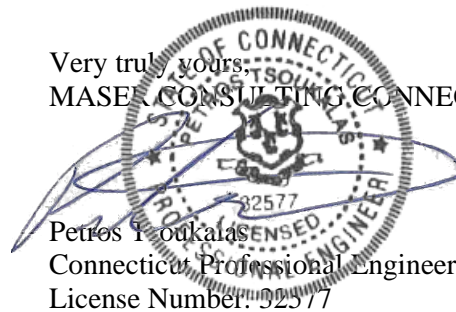
Dear Mr. Leone,

This letter is intended to clarify and confirm the antenna naming convention used by Verizon Wireless as a part of an antenna upgrade project on numerous wireless facilities.

The antenna naming convention “Licensed Sub-6, L-Sub6, nL-Sub6, VZS01” and any other slight variants refer to the 64T64RMMU antenna manufactured by Samsung Electronics. These names are interchangeable and are used in various documents, including but not limited to the “Antenna Mount Analysis”.

If you have any questions or comments, or require additional information, please do not hesitate to contact me.

Very truly yours,
MASER CONSULTING CONNECTICUT



Petros I. Ioukalis
Connecticut Professional Engineer
License Number: 32577

PROJECT NOTES

1. SEE MODIFICATION NOTES
2. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE CODES, ORDINANCES, LAWS AND REGULATIONS OF ALL MUNICIPALITIES, UTILITY COMPANIES OR OTHER PUBLIC/GOVERNING AUTHORITIES.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS THAT MAY BE REQUIRED BY ANY FEDERAL, STATE, COUNTY OR MUNICIPAL AUTHORITIES.
4. THE CONTRACTOR SHALL NOTIFY THE CONSTRUCTION MANAGER, IN WRITING, OF ANY CONFLICTS, ERRORS OR OMISSIONS PRIOR TO THE SUBMISSION OF BIDS OR PERFORMANCE OF WORK.
5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING ALL EXISTING SITE IMPROVEMENTS PRIOR TO COMMENCING CONSTRUCTION. THE CONTRACTOR SHALL REPAIR ANY DAMAGE AS A RESULT OF CONSTRUCTION OF THIS FACILITY AT THE CONTRACTOR'S EXPENSE TO THE SATISFACTION OF THE OWNER.
6. THE SCOPE OF WORK FOR THIS PROJECT SHALL INCLUDE PROVIDING ALL MATERIALS, EQUIPMENT AND LABOR REQUIRED TO COMPLETE THIS PROJECT. ALL EQUIPMENT SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
7. THE CONTRACTOR SHALL VISIT THE PROJECT SITE PRIOR TO SUBMITTING THE BID TO VERIFY THAT THE PROJECT CAN BE CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND CONSTRUCTION DRAWINGS.
8. THE CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THESE DRAWINGS MUST BE VERIFIED. THE CONTRACTOR SHALL NOTIFY THE CONSTRUCTION MANAGER OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH CONSTRUCTION.
9. SINCE THE CELL SITE MAY BE ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE REQUIRED TO BE WORN TO ALERT OF ANY POTENTIALLY DANGEROUS EXPOSURE LEVELS.
10. NO NOISE, SMOKE, DUST OR ODOR WILL RESULT FROM THIS FACILITY AS TO CAUSE A NUISANCE.
11. THE FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION (NO HANDICAP ACCESS IS REQUIRED).



**MOUNT MODIFICATION DRAWINGS
EXISTING 12.50' T-FRAME**

**SITE NAME: GLASTONBURY WEST CT
SITE NUMBER: 468258**

**2577 MAIN ST.
GLASTONBURY, CT 06033
HARTFORD COUNTY**

PROJECT INFORMATION	
SITE INFORMATION	
LATITUDE:	41.7143289° N
LONGITUDE:	72.613028° W
JURISDICTION:	HARTFORD COUNTY
APPLICANT/LESSEE	
COMPANY:	VERIZON WIRELESS
CLIENT REPRESENTATIVE	
COMPANY:	VERIZON WIRELESS
ADDRESS:	118 FLANDERS ROAD, THIRD FLOOR
CITY, STATE, ZIP:	WESTBOROUGH, MA 01581
CONTACT:	ANDREW CANDIELLO
EMAIL:	ANDREW.CANDIELLO@VERIZONWIRELESS.COM
PROJECT MANAGER	
COMPANY:	MASER CONSULTING CONNECTICUT
CONTACT:	GREG DULNIK
PHONE:	(615) 686-2575
E-MAIL:	GREG.DULNIK@COLLIERSENGINEERING.COM

SHEET INDEX	
SHEET	DESCRIPTION
T-1	TITLE SHEET
S-1	BILL OF MATERIALS
S-2	MODIFICATION NOTES
S-3	MODIFICATION NOTES
S-4	MODIFICATION DETAILS
S-5	MODIFICATION DETAILS
S-6	MOUNT PHOTOS
	SPECIFICATION SHEETS

CONTRACTOR PMI REQUIREMENTS	
PMI LOCATION:	HTTPS://PMI.VZWSMART.COM
SMART TOOL PROJECT #:	10044065
VZW LOCATION CODE (PSLC):	468258
FUZE ID:	16232043

CONTRACTOR PMI REQUIREMENTS	
FAILING MOUNT ANALYSIS REPORT	
SMART TOOL PROJECT #:	10032203
MASER CONSULTING PROJECT #:	20777629A
ANALYSIS DATE:	2/4/2021

PMI REQUIREMENTS EMBEDDED WITHIN MOUNT MODIFICATION REPORT

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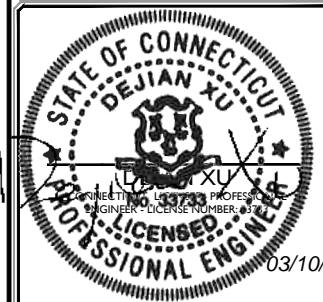
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HARTFORD COUNTY

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Mount Laurel, NJ 08054
Phone: 856.797.0412
Fax: 856.722.1120

SHEET TITLE:
TITLE SHEET

SHEET NUMBER:
T-1

BILL OF MATERIALS

VZWSMART KITS					
QUANTITY	MANUFACTURER	PART NUMBER	DESCRIPTION	NOTES	
3	VZWSMART	VZWSMART-SFK1	TIE BACK ASSEMBLY	FINAL LENGTH TO BE DETERMINED IN FIELD, TRIM AS REQUIRED	
3		VZWSMART-SFK3	V-BRACING KIT	FINAL LENGTH TO BE DETERMINED IN FIELD, TRIM AS REQUIRED	
12		VZWSMART-MSK1	CROSSOVER PLATE		
		-			
		-			
		-			

OTHER REQUIRED PARTS				
QUANTITY	MANUFACTURER	PART NUMBER	DESCRIPTION	NOTES
3	-	-	150" LONG, P2.5 STD PIPE	GALVANIZED
1	-	-	48" LONG, P2.0 STD PIPE	GALVANIZED
2	PERFECT VISION	PV-XP-30ST20	SQUARE TUBE CROSSOVER PLATE	OR EOR APPROVED EQUAL, CONTACT MASER CONSULTING TO OBTAIN APPROVAL OF SUBSTITUTION

NOTE: ALL MATERIALS REQUIRED FOR THE DESIGNED MODIFICATIONS BUT NOT LISTED IN THIS SHEET ARE ASSUMED TO BE PROVIDED BY THE CONTRACTOR

VZWSMART KITS - APPROVED VENDORS	
COMMSCOPE	
CONTACT	SALVADOR ANGUIANO
PHONE	(817) 304-7492
EMAIL	SALVADOR.ANGUIANO@COMMSCOPE.COM
WEBSITE	WWW.COMMSCOPE.COM
METROSITE FABRICATORS, LLC	
CONTACT	KENT RAMEY
PHONE	(706) 335-7045 (O), (706) 982-9788 (M)
EMAIL	KENT@METROSITELLC.COM
WEBSITE	METROSITEFABRICATORS.COM
PERFECTVISION	
CONTACT	WIRELESS SALES
PHONE	(844) 887-6723
EMAIL	WWW.PERFECT-VISION.COM
WEBSITE	WIRELESSSALES@PERFECT-VISION.COM
SABRE INDUSTRIES, INC.	
CONTACT	ANGIE WELCH
PHONE	(866) 428-6937
EMAIL	AKWELCH@SABREINDUSTRIES.COM
WEBSITE	WWW.SABRESITESOLUTIONS.COM
SITE PRO 1	
CONTACT	PAULA BOSWELL
PHONE	(972) 236-9843
EMAIL	PAULA.BOSWELL@VALMONT.COM
WEBSITE	WWW.SITEPRO1.COM

NOTE: WHEN SPECIFIED, VZWSMART KITS SHALL BE REQUIRED AND WILL BE VERIFIED DURING THE DESKTOP PMI



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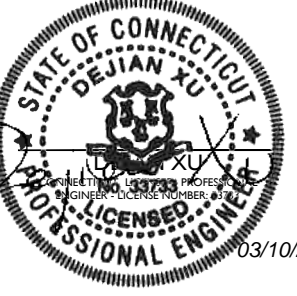
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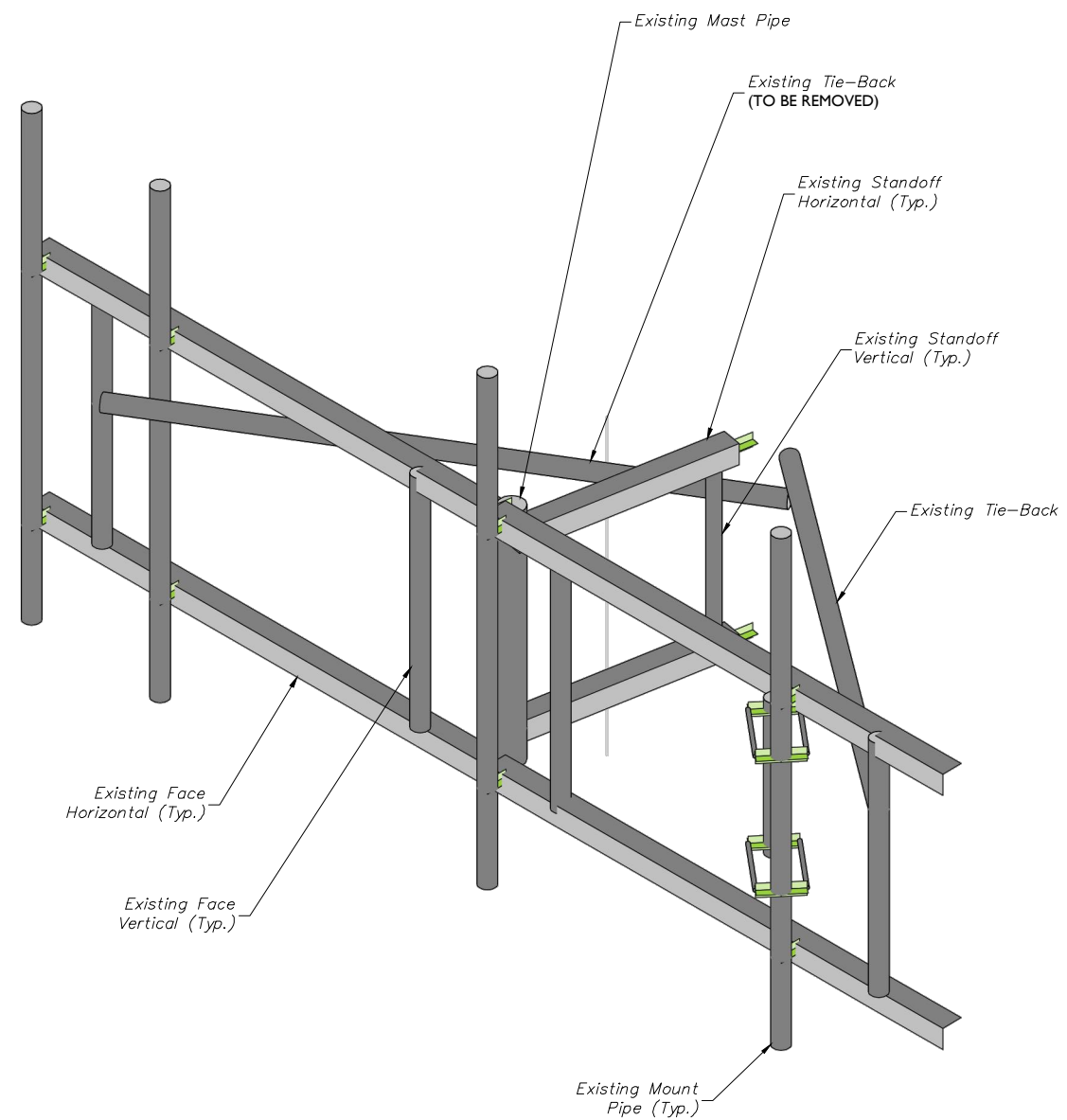
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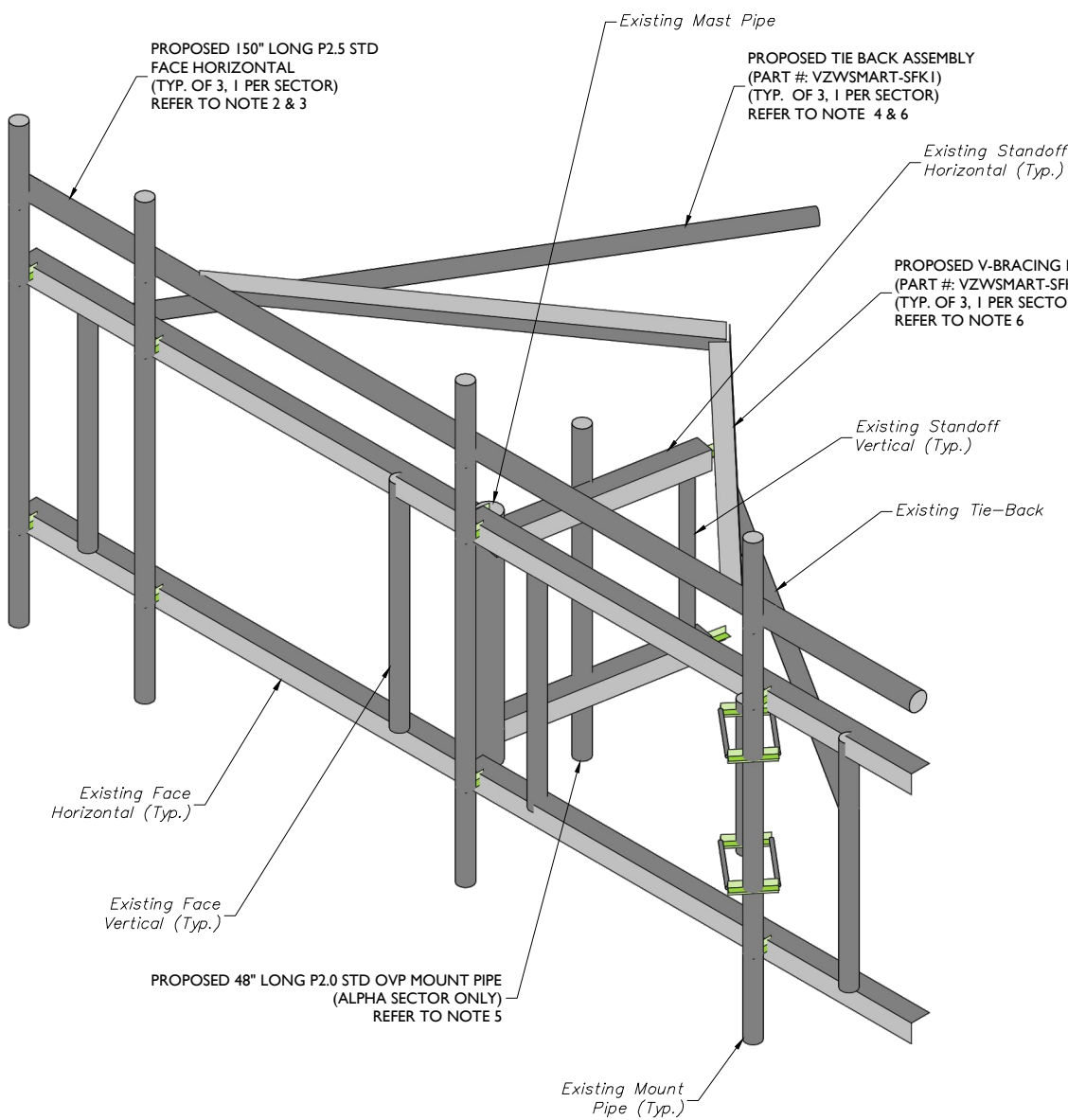
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S-4



1 EXISTING T-FRAME ISOMETRIC VIEW (TYP. ALL SECTORS)
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2 PROPOSED T-FRAME ISOMETRIC VIEW (TYP. ALL SECTORS)
SCALE : N.T.S.

STRUCTURAL NOTES:

- PER THE MOUNT MAPPING COMPLETED BY RKS DESIGN & ENGINEERING, LLC ON 1/10/2021, THE SAFETY CLIMB AND CLIMBING FACILITIES UP TO THE VERIZON MOUNT ELEVATION (79'-0") ARE IN GOOD CONDITION. MASER DOES NOT WARRANT THIS INFORMATION.
- INSTALL SHALL NOT CAUSE HARM TO THE STRUCTURE, CLIMBING FACILITY, SAFETY CLIMB, OR ANY SYSTEM INSTALLED ON THE STRUCTURE. TIMELY NOTICE AND DOCUMENTATION SHALL BE PROVIDED BY CONTRACTORS TO THE EOR (OF STRUCTURAL DESIGN) IF AN OBSTRUCTION WAS REQUIRED TO MEET THE RF SYSTEM DESIGN REQUIREMENTS AND PERFORMANCES.

MODIFICATION NOTES:

- MOUNT MEMBERS NOT SHOWN FOR CLARITY U.N.O.
- RADIO AND/OR TME POSITIONS SHALL BE ADJUSTED VERTICALLY AS NEEDED IN ORDER TO ACHIEVE INSTALLATION OF HORIZONTAL AS SHOWN. EOR SHALL BE NOTIFIED IF EQUIPMENT NEEDS TO BE RELOCATED TO ANOTHER MOUNT PIPE.
- CONNECT NEW HORIZONTAL TO ALL EXISTING VERTICAL MOUNT PIPES WITH CROSSOVER PLATES (PART #: VZWSMART-MSK1).
- CONNECT OTHER END OF TIEBACK TO ADJACENT TOWER LEG.
- CONNECT NEW OVP MOUNT PIPE TO EXISTING STANDOFF WITH SQUARE TUBE CROSSOVER PLATES (PERFECT VISION PART #: PV-XP-30ST20, OR EOR APPROVED EQUAL).
- FINAL LENGTH TO BE DETERMINED IN FIELD, CONTRACTOR TO TRIM AS REQUIRED.

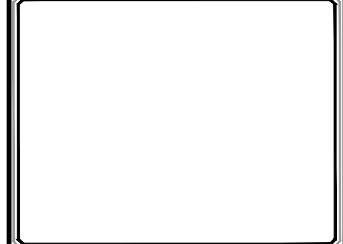
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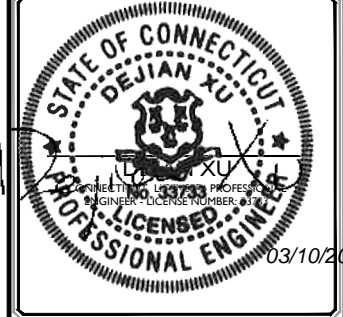
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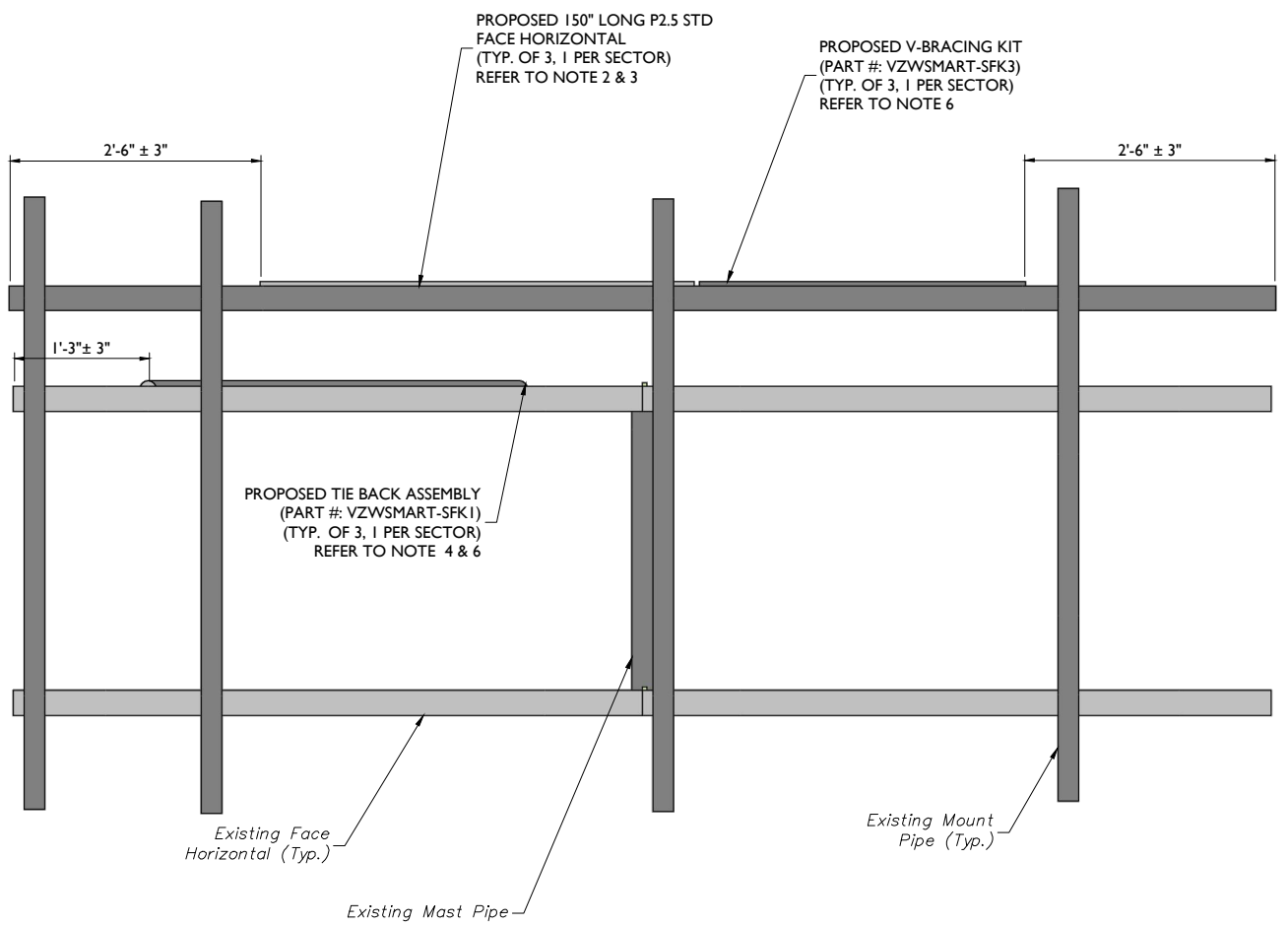
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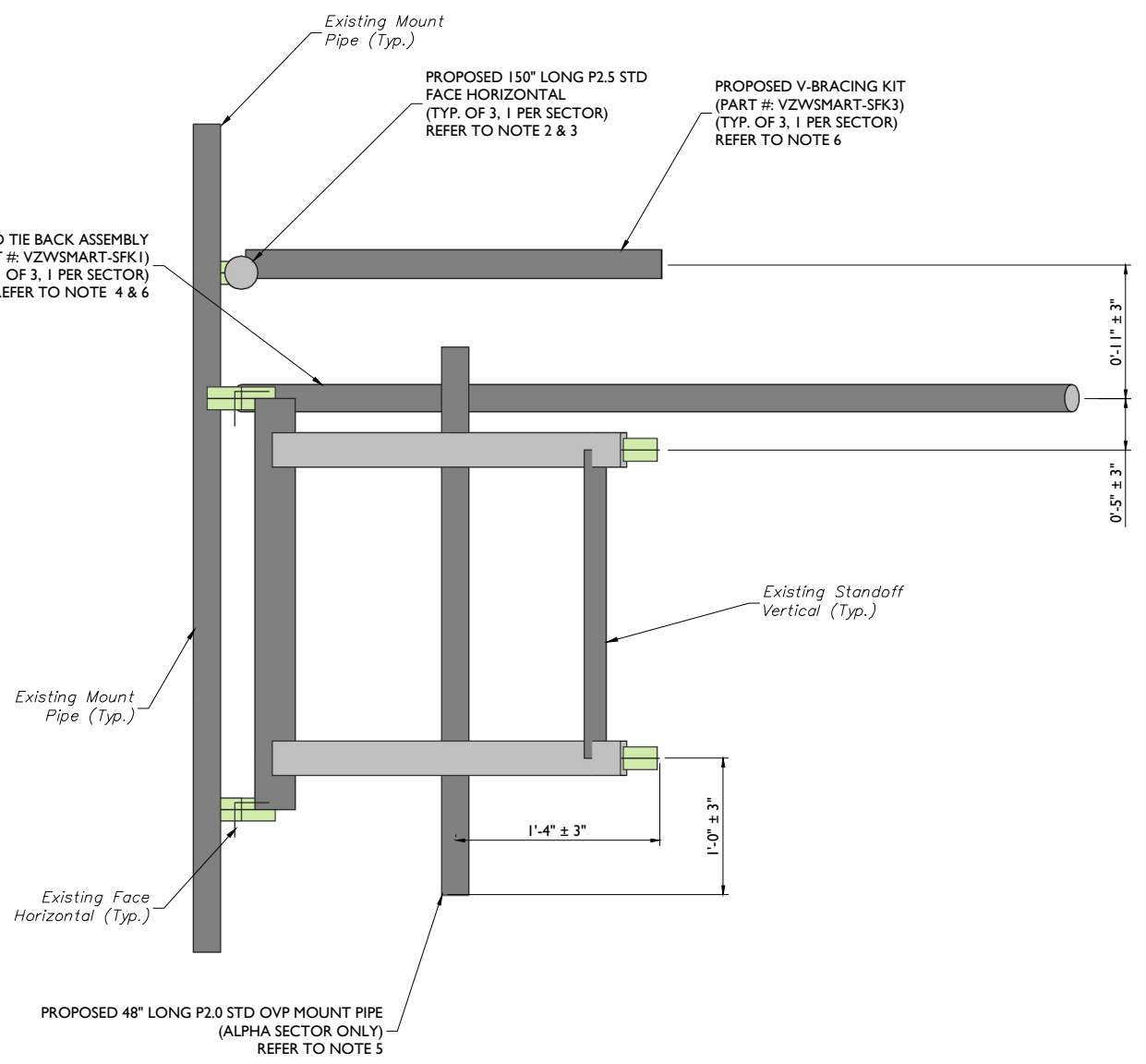
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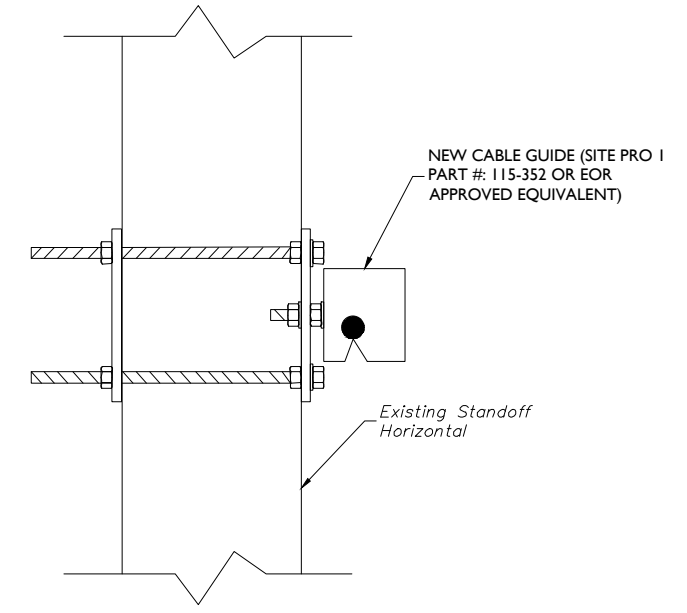
1 PROPOSED FRONT ELEVATION (TYP. ALL SECTORS)
SCALE : N.T.S.

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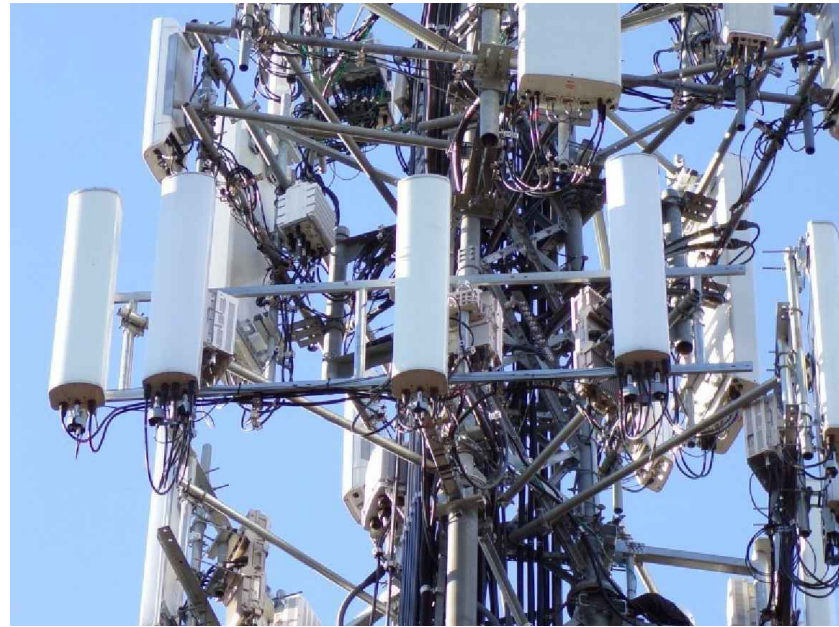
1. MOUNT MEMBERS NOT SHOWN FOR CLARITY U.N.O.
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6. FINAL LENGTH TO BE DETERMINED IN FIELD, CONTRACTOR TO TRIM AS REQUIRED.



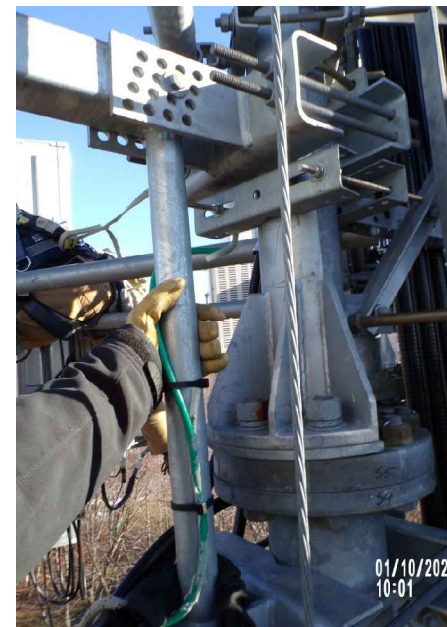
2 PROPOSED SIDE ELEVATION (TYP. ALL SECTORS)
SCALE : N.T.S.



3 PROPOSED CABLE GUIDE STANDOFF PIPE ATTACHMENT - PLAN VIEW
SCALE : N.T.S.



MOUNT PHOTO 1



MOUNT PHOTO 2



MOUNT PHOTO 3



MOUNT PHOTO 4

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STATE OF CONNECTICUT
DEJIAN XU
REGISTERED PROFESSIONAL ENGINEER
LICENSE NUMBER: 031030003
03/10/2021

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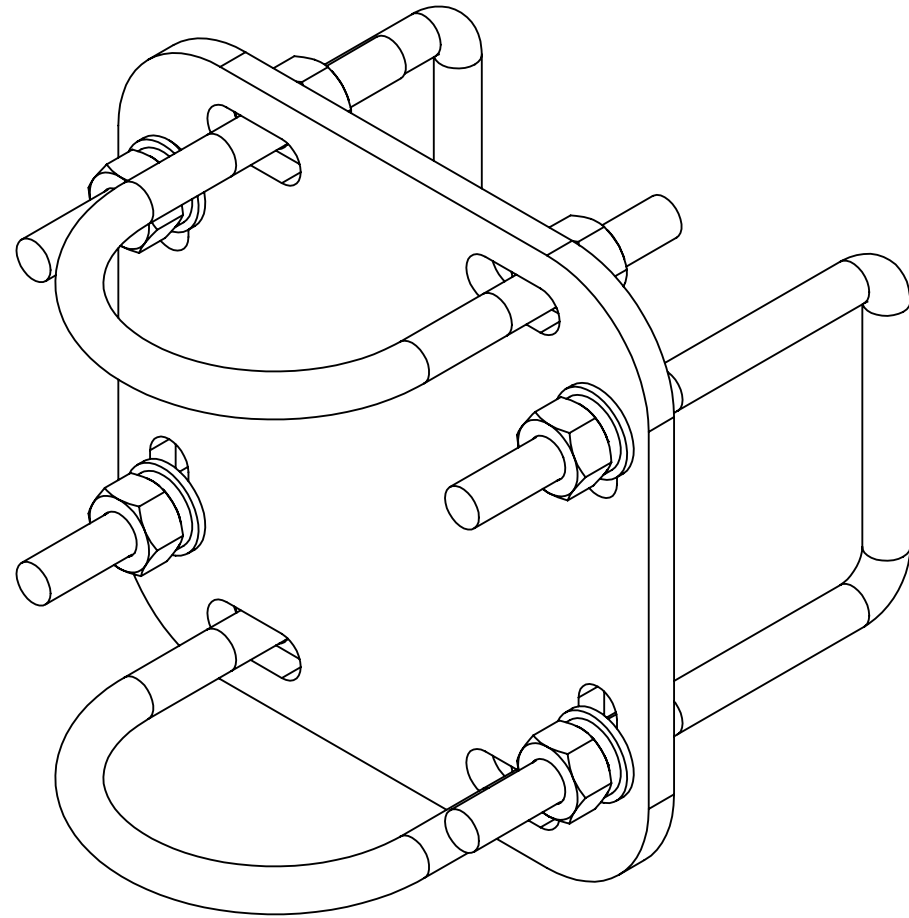
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468258
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HARTFORD COUNTY

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2000 Pilidian Drive
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Mount Laurel, NJ 08054
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Fax: 856.722.1120

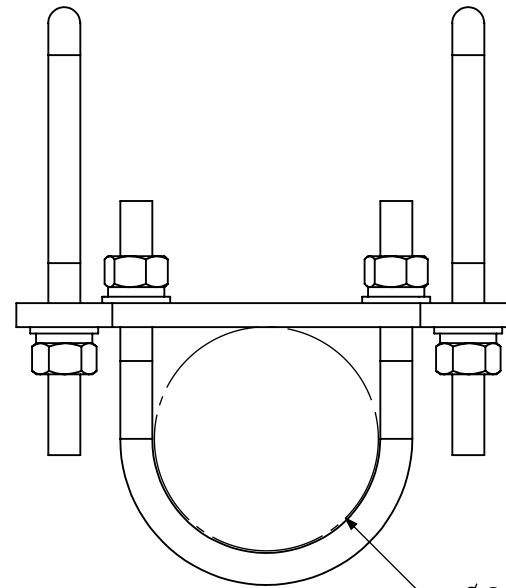
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MOUNT PHOTOS

SHEET NUMBER:
S-6

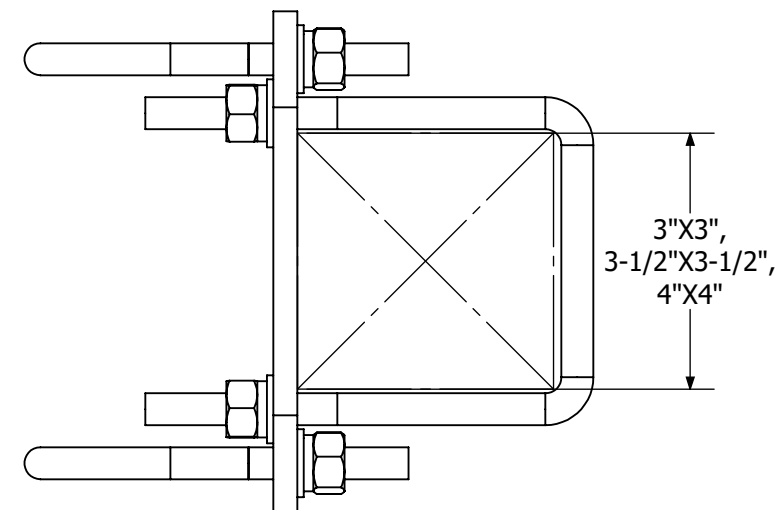
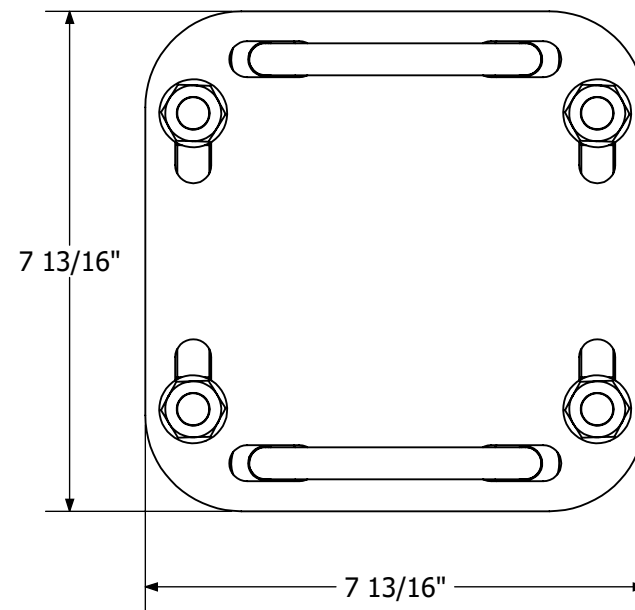
**PV-XP-ST
SQUARE TUBE TO ROUND PIPE CROSSOVER**



**PV-XP-ST
SQUARE TUBE TO ROUND PIPE CROSSOVER
WEIGHT: SEE TABLE 1**



$\phi 2 \frac{3}{8}$ ", $\phi 2 \frac{7}{8}$ ", $\phi 3 \frac{1}{2}$ "



Part Number	Square Tube	Round Pipe	Weight (lbs)
PV-XP-30ST20	3"	NPS 2 (2-3/8" OD)	10
PV-XP-30ST25	3"	NPS 2-1/2 (2-7/8" OD)	10
PV-XP-30ST30	3"	NPS 3 (3-1/2" OD)	10
PV-XP-35ST20	3-1/2"	NPS 2 (2-3/8" OD)	10
PV-XP-35ST25	3-1/2"	NPS 2-1/2 (2-7/8" OD)	10
PV-XP-35ST30	3-1/2"	NPS 3 (3-1/2" OD)	10
PV-XP-40ST20	4"	NPS 2 (2-3/8" OD)	10
PV-XP-40ST25	4"	NPS 2-1/2 (2-7/8" OD)	10
PV-XP-40ST30	4"	NPS 3 (3-1/2" OD)	10



16101 La Grande Dr.
Little Rock, AR 72223
1-800-205-8620

STAMP:

The information contained in this set of documents is proprietary by nature, any use or disclosure other than that which relates to the client named is strictly prohibited.

REVISIONS:

NO.	DATE	INITIAL RELEASE	DESCRIPTION	BY	CHK	APD
5					SS	
4					AM	
3					DJN	
2						
1						
0	9/27/16					

SITE INFORMATION:

DESIGN TYPE:

**SQUARE TUBE
CROSSOVER PLATE**

SHEET TITLE:

ENGINEERING DETAIL

SHEET NO.:

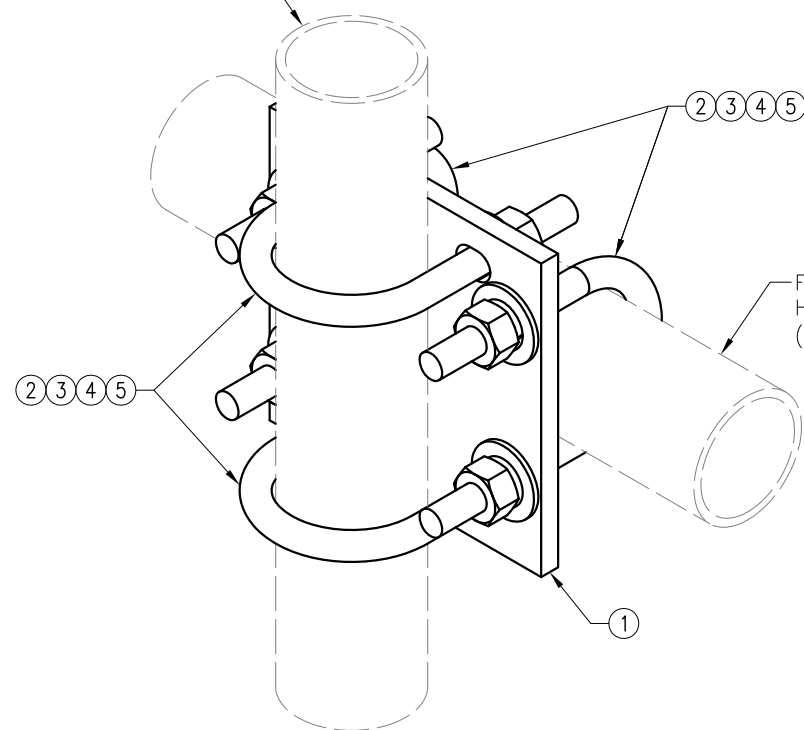
E-1

REVISION:

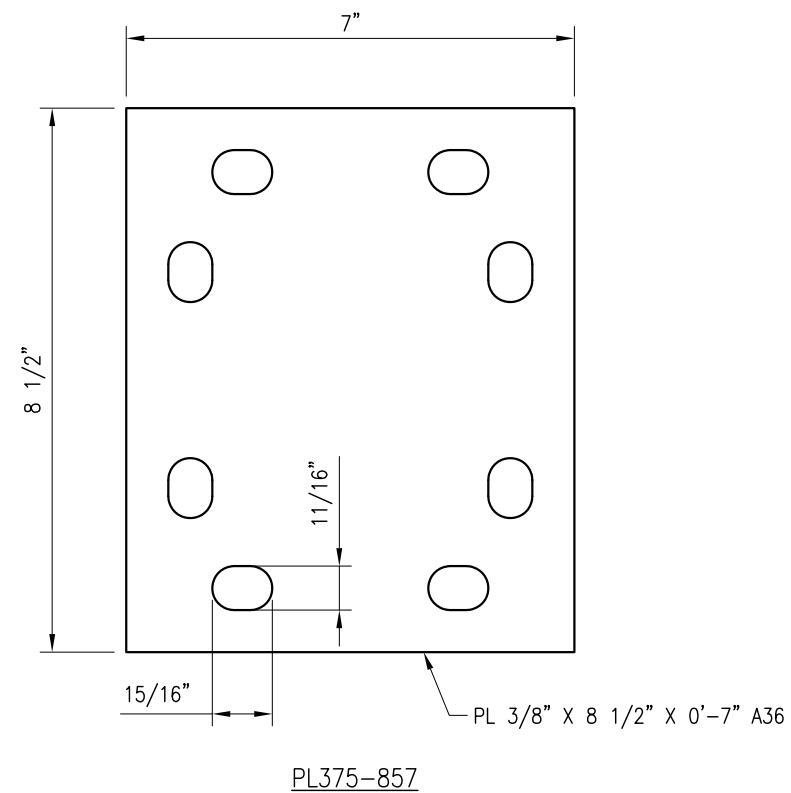
0



FITS 2.375" O.D. AND 2.875" O.D.
 VERTICAL PIPE.
 (NOT INCLUDED IN THIS KIT)



FITS 2.375" O.D. AND 2.875" O.D.
 HORIZONTAL PIPE.
 (NOT INCLUDED IN THIS KIT)



PL375-857

DRAWN BY: H.R. CHECKED BY: HMA

REV.	DESCRIPTION	BY	DATE
1	FIRST ISSUE	H.R.	05/08/20

SHEET TITLE:

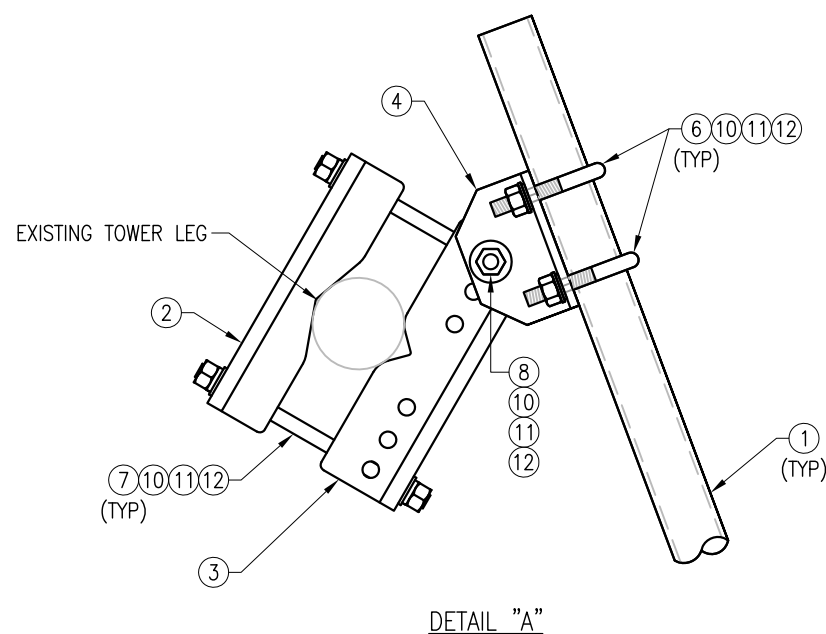
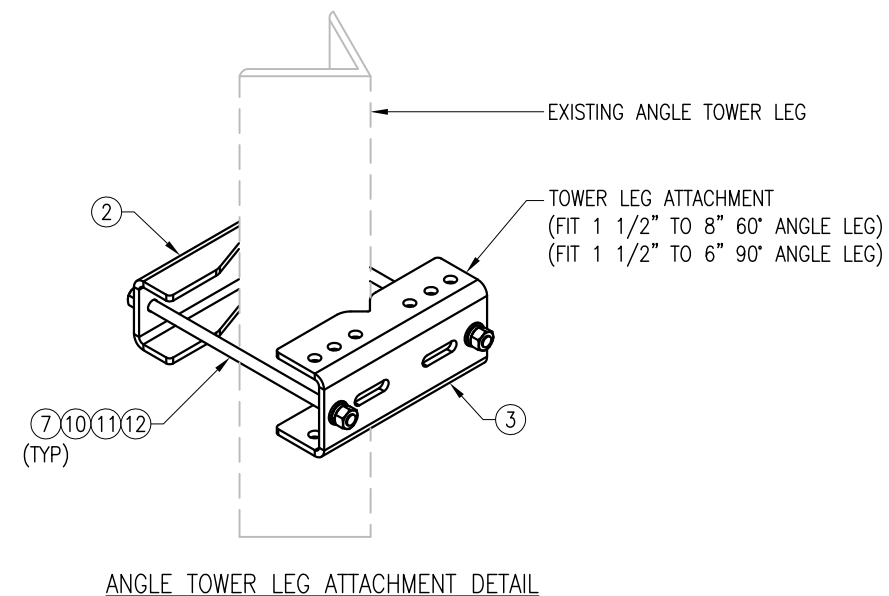
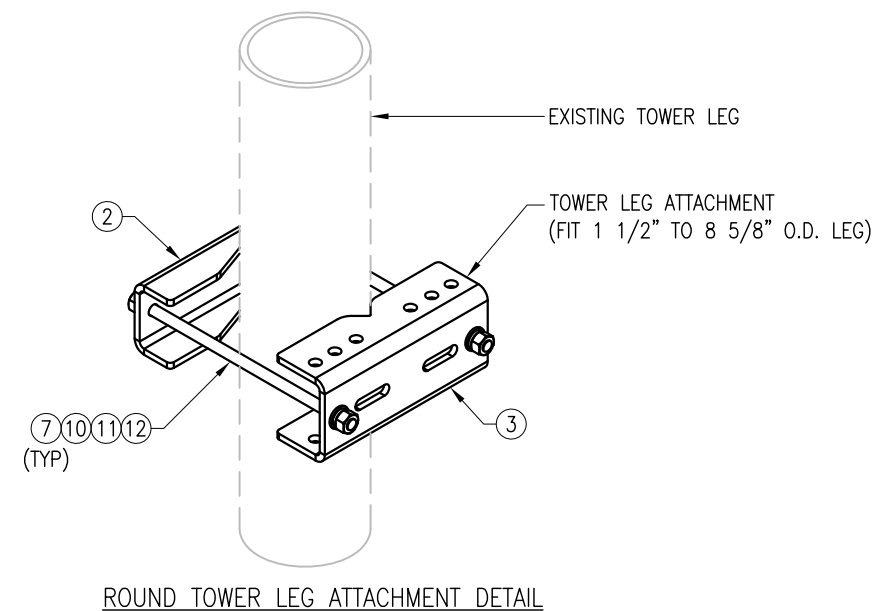
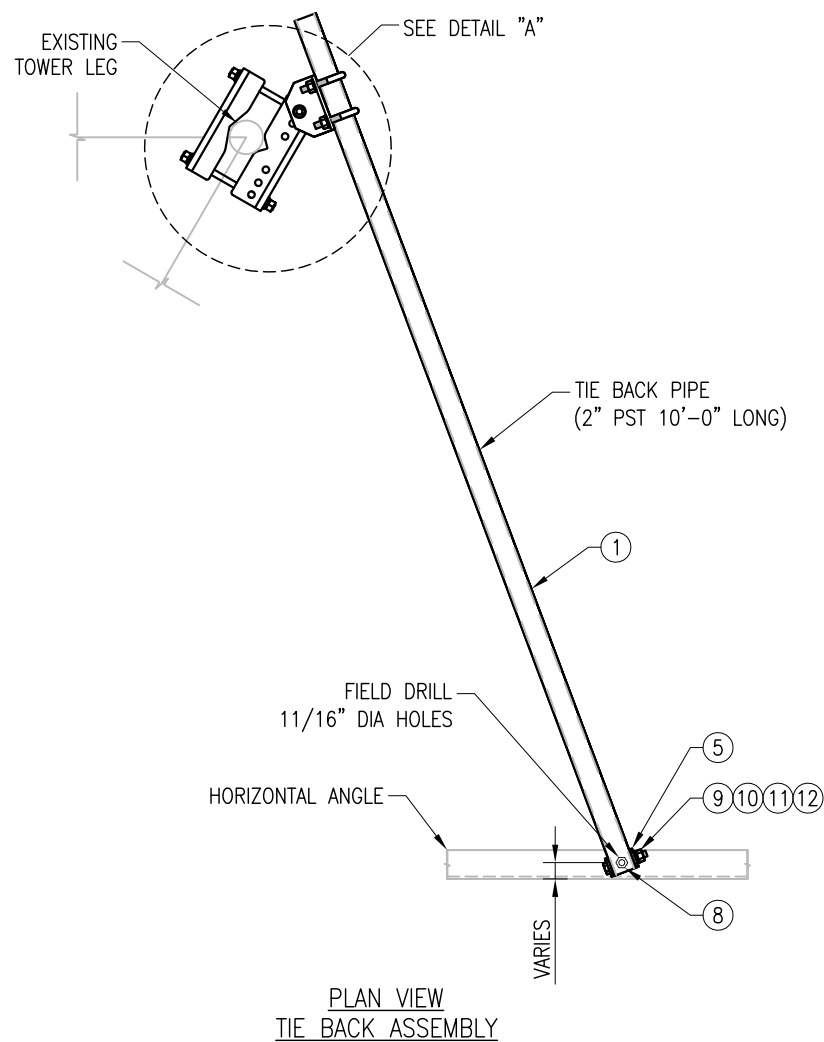
VZSMART-MSK1
 CROSSOVER PLATE

SHEET NUMBER: REV #:

VZSMART-MSK1 0

NOTES:
 1. HOT-DIPPED GALVANIZED PER ASTM A123.

VZSMART-MSK1 (CROSSOVER PLATE)					
ITEM NO.	QTY.	PART NO.	DESCRIPTION	SHEET #	WT
1	1	PL375-857	PL 3/8" X 8 1/2" X 0'-7" A36	MSK1-F1	6
2	4	MS02-625-300-500	RU-BOLT 5/8" X 3" I.W. X 5" I.L. A36 (OR EQUIV.)	RBC-1	5
3	8	FW-625	5/8" HDG USS FLAT WASHER	---	1
4	8	LW-625	5/8" HDG LOCK WASHER	---	0
5	8	NUT-625	5/8" HDG HEX NUT	---	1
GALVANIZED WT					14



VZWSMART-SFK1 (TIE BACK ASSEMBLY)					
ITEM NO.	QTY.	PART NO.	DESCRIPTION	SHEET #	WT
1	1	PST2375-10	2" PST (2.375" O.D. X 0.154" THK) X 10'-0" A53 GR-B 35KSI	SFK1-F1	38
2	1	BP825-12	PL 3/8" X 8 1/4" X 1'-0" A36 BENT PLATE	SFK1-F2	11
3	1	BP11125-12	PL 3/8" X 11 1/8" X 1'-0" A36 BENT PLATE	SFK1-F3	14
4	1	BP6-9375	PL 3/8" X 6" X 9 3/8" A36 BENT PLATE	SFK1-F4	6
5	1	BP2-875	PL 1/4" X 2" X 8 3/4" A36 BENT PLATE	SFK1-F4	1
6	2	MS02-625-300-500	RU-BOLT 5/8" X 3" I.W. X 5" I.L. A36 (OR EQUIV.)	RBC-1	2
7	2	---	THREADED ROD 5/8" DIA. X 1'-6" F1554-36 HDG	---	0
8	2	---	BOLT 5/8" X 2" A325	---	0
9	1	---	BOLT 5/8" X 4 1/4" A325	---	0
10	11	FW-625	5/8" HDG USS FLAT WASHER	---	1
11	11	LW-625	5/8" HDG LOCK WASHER	---	0
12	11	NUT-625	5/8" HDG HEX NUT	---	1
GALVANIZED WT					72

NOTES:
 1. HOT-DIPPED GALVANIZED PER ASTM A123.

DRAWN BY: BT CHECKED BY: HMA/KW

REV. DESCRIPTION BY DATE
 △ FIRST ISSUE BT 05/08/20

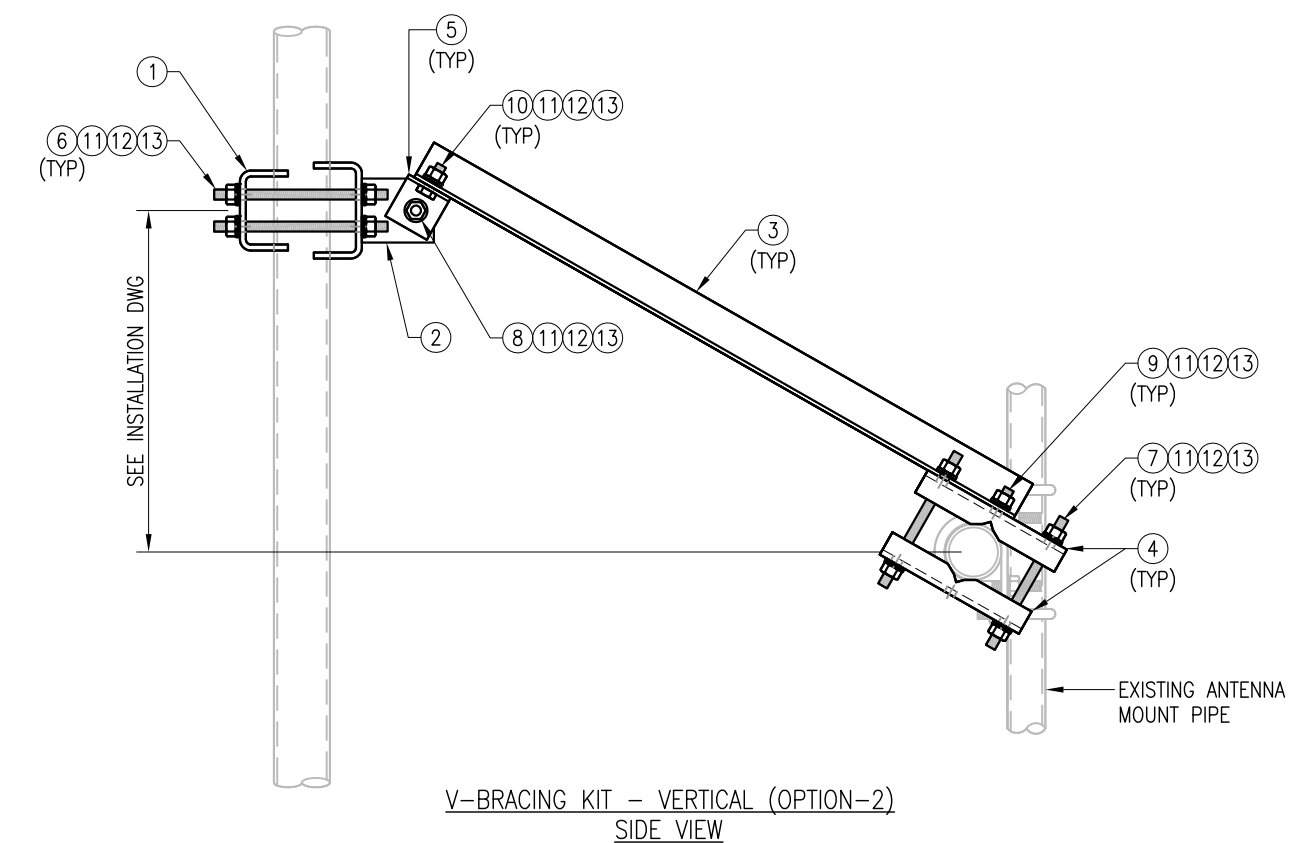
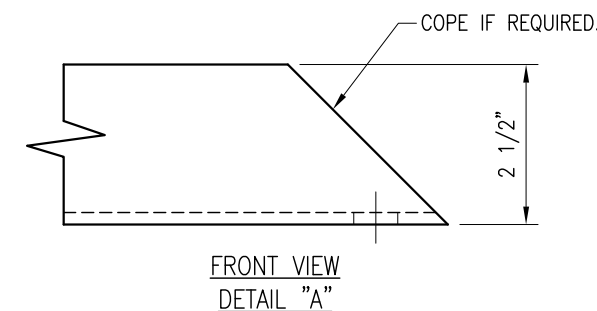
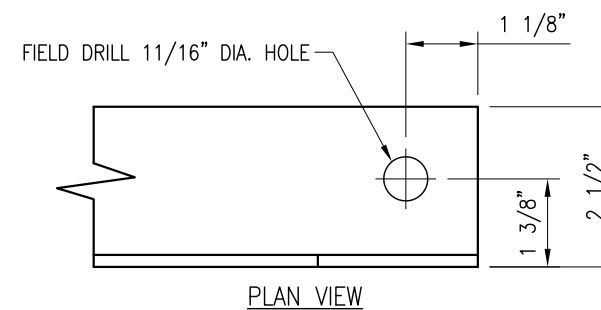
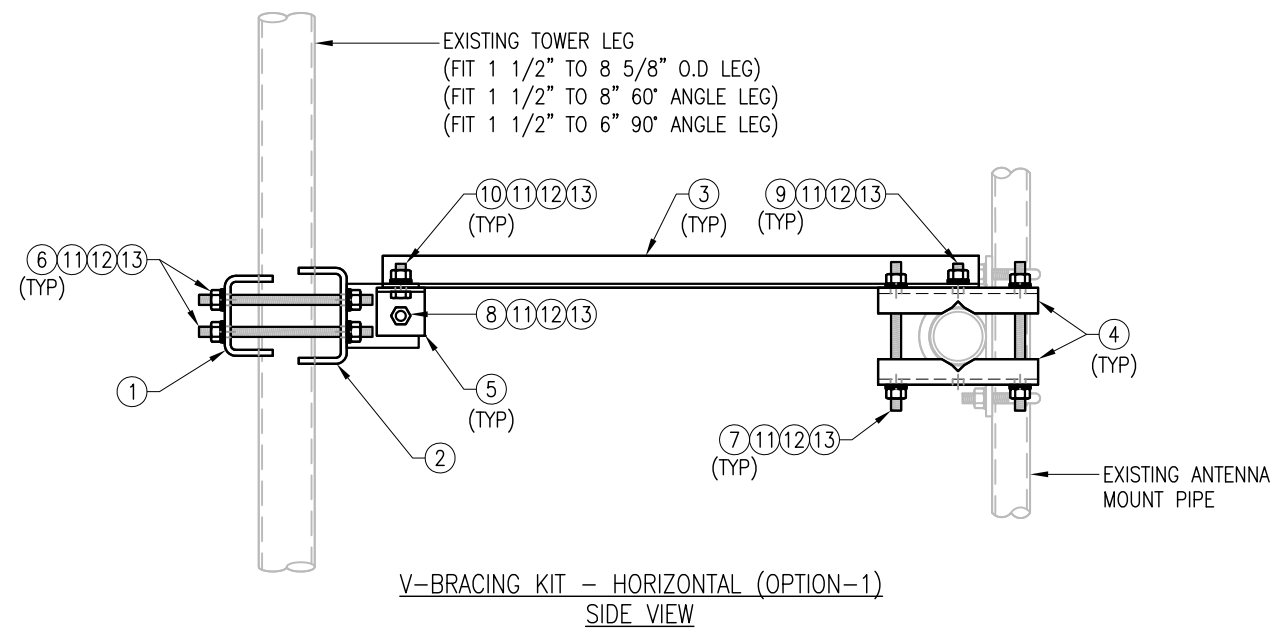
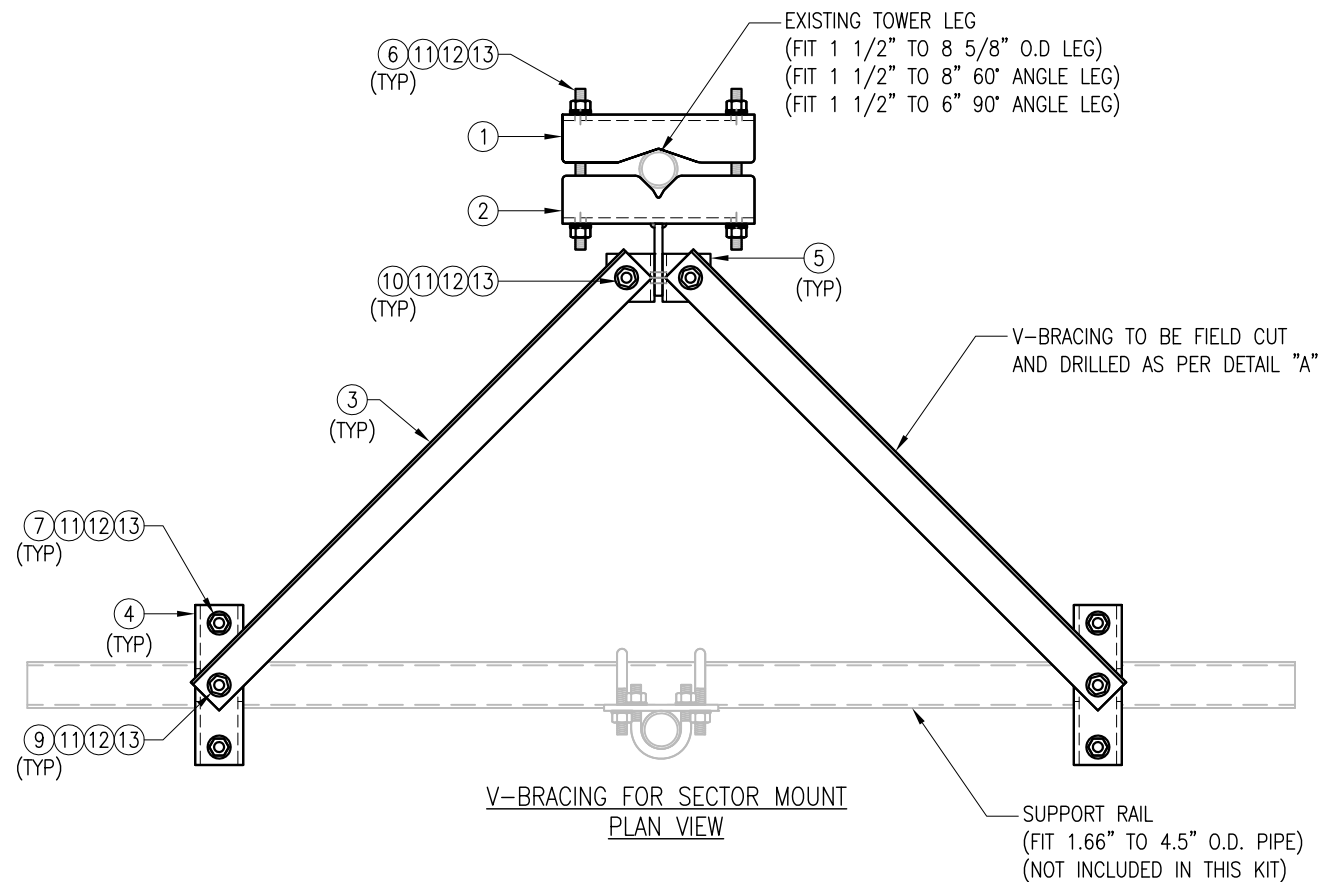
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SHEET TITLE:

VZWSMART-SFK1
 TIE BACK ASSEMBLY

SHEET NUMBER: REV #:

VZWSMART-SFK1 0



VZSMART-SFK3 (V-BRACING KIT)					
ITEM NO.	QTY.	PART NO.	DESCRIPTION	SHEET #	WT
1	1	BP9625-12	PL 3/8" X 9 5/8" X 1'-0" A36 BENT PLATE	VBSM-F1	12
2	1	BRKW-VBSM	WELDMENT BRACKET	VBSM-F3	16
3	2	L252525-8	L 2 1/2" X 2 1/2" X 1/4" X 8'-0" A36	VBSM-F5	67
4	4	BP6875-10	PL 3/8" X 6 7/8" X 10" A36 BENT PLATE	VBSM-F2	20
5	2	AL-333	L 3" X 3" X 1/4" X 3" A36	VBSM-F2	3
6	4	---	THREADED ROD 5/8" DIA. X 1'-6" F1554-36 HDG	---	---
7	4	---	THREADED ROD 5/8" DIA. X 10" F1554-36 HDG	---	---
8	1	---	BOLT 5/8" X 2 1/4" A325	---	---
9	2	---	BOLT 5/8" X 2" A325	---	---
10	2	---	BOLT 5/8" X 1 3/4" A325	---	---
11	21	FW-625	5/8" HDG USS FLAT WASHER	---	2
12	21	LW-625	5/8" HDG LOCK WASHER	---	0
13	21	NUT-625	5/8" HDG HEX NUT	---	2
GALVANIZED WT					122

NOTES:
 1. HOT-DIPPED GALVANIZED PER ASTM A123.

DRAWN BY: H.R. CHECKED BY: HMA

REV. DESCRIPTION BY DATE
 △ FIRST ISSUE H.R. 05/08/20

SHEET TITLE:

VZSMART-SFK3
 V-BRACING KIT

SHEET NUMBER: REV #:

VZSMART-SFK3 0

ATTACHMENT 5



2615-2639

MAIN STREET

2384

2577

2534

2510

2493

WELLES STREET

2401

0 50 100ft

Owner of Record

GIS ID: 41402577
Owner: SAINTS ISIDORE AND MARIA PARISH CORP
Co-Owner:
Address: 2577 MAIN ST
City, State ZIP: GLASTONBURY, CT 06033-2023

Account Number: 41402577

Property Address: 2577 MAIN ST

Parcel Information

Map/Street/Lot D5 / 4140 / W0038 **Property ID:** 12111
Developer Lot ID: **Water:** Public-MDC
Parcel Acreage: 4.07 **Sewer:** Sewer Tax Rec
Zoning Code: PBD **Census:** 5203

Valuation Summary

Item	Appraised Value	Assessed Value
Buildings	2155600	1508900
Land	734900	514400
Appurtenances	104700	73400
Total	3015200	2110700



Property highlighted in blue

Owner of Record

SAINTE ISIDORE AND MARIA PARISH CORP
 ST PAULS ROMAN CATHOLIC CHURCH

Deed / Page Sale Date Sale Price

3570/0272 2019-07-18 0
 0450/0069 1988-10-04 0

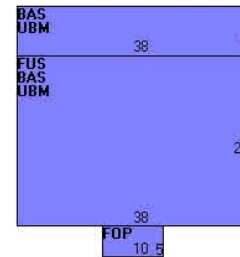
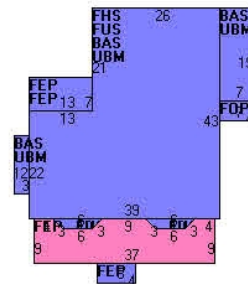
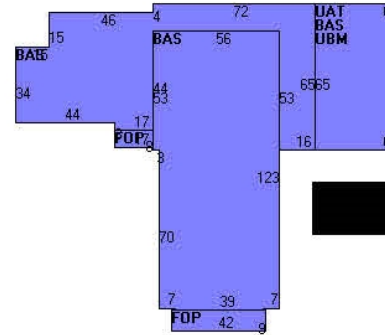




Building Information

Building ID 14170

Year Constructed : 1988	Number of Rooms :
Building Type : Ind/Comm	Number of Bedrooms :
Style : Churches	Number of Bathrooms :
Occupancy : Exempt MDL-96	Number of Half-Baths :
Stories : 1	Exterior Wall : Brick
Building Zone : PBD	Interior Wall : Drywall
Roof Type : Gable	Interior Floor : Carpet
Roof Material : Asphalt Shingl	Interior Floor #2 : No entry
Est. Gross S.F. : 23623	Air Conditioning Type : Partial A/C
Est. Living S.F. : 15149	Heat Type : Forced Air
	Fuel Type : Gas



Subarea Type	Est. Gross S.F.	Est. Living S.F.	Outbuilding Type	Est. Gross S.F.	Comments
First Floor	1573	1573	Garage	924.00	
Porch, Enclosed	547	0	LIGHT 1	1.00	
Half Story, Finished	1404	702	Paving	96515.00	
Porch, Open	28	0	Air Condition	10000.00	
Upper Story, Finished	1436	1436			
Basement	1573	0			
First Floor	15149	15149			

This data & map is a user generated static output from an Internet mapping site and is for reference only. Data that appears on this form may or may not be accurate, current, or otherwise reliable. Any questions on the data provided above should be directed to the Town of Glastonbury Property Assessment Office 860-652-7600.



Town of Glastonbury GIS Parcel Report


Report Generated 6/2/2021 8:27:19 AM

Basement, Finished	3965	0
Porch, Open	664	0
Attic, Unfinished	3905	0
Basement	-60	0
First Floor	1368	1368
Porch, Open	50	0
Upper Story, Finished	1064	1064
Basement	1368	0

ATTACHMENT 6



GLASTONBURY WEST
Certificate of Mailing — Firm

Name and Address of Sender Kenneth C. Baldwin, Esq. Robinson & Cole LLP 280 Trumbull Street Hartford, CT 06103	TOTAL NO. of Pieces Listed by Sender <div style="font-size: 2em; text-align: center;">3</div>	TOTAL NO. of Pieces Received at Post Office™ <div style="font-size: 2em; text-align: center;">3</div>	Affix Stamp Here Postmark with Date of Receipt. <div style="text-align: right;"> <p>neopost[®] 06/03/2021 US POSTAGE \$002.89⁰</p>  <p>ZIP 06103 041L12203937</p> </div>
	Postmaster, per (name of receiving employee) <div style="font-size: 2em; text-align: center;">V.P</div>		

USPS® Tracking Number Firm-specific Identifier	Address (Name, Street, City, State, and ZIP Code™)	Postage	Fee	Special Handling	Parcel Airlift
1.	Richard J. Johnson, Town Manager Town of Glastonbury 2155 Main Street Glastonbury, CT 06033				
2.	Rebecca Augur, Director of Planning and Land Use Services Town of Glastonbury 2155 Main Street Glastonbury, CT 06033				
3.	Saint Isidore and Maria Parish Corporation 2577 Main Street Glastonbury, CT 06033				
4.					
5.					
6.					

