



Crown Castle
3530 Toringdon Way Suite 300
Charlotte NC 28277

Tel (704) 405-6600

March 10, 2015

Melanie A. Bachman
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

RE: T-Mobile-Exempt Modification - Crown Site BU: 871584
T-Mobile Site ID: CT11189E
Located at: 115 Birch Mountain Road, Glastonbury, CT 06033

Dear Ms. Bachman:

This letter and exhibits are submitted on behalf of T-Mobile. T-Mobile is making modifications to certain existing sites in its Connecticut system in order to implement their 700MHz technology. Please accept this letter and exhibits as notification, pursuant to § 16-50j-73 of the Regulations of Connecticut State Agencies (“R.C.S.A.”), of construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In compliance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Mr. Richard J. Johnson, Town Manager for the Town of Glastonbury, and Carolyn R. Scarrone Revocable Trust, Property Owner.

T-Mobile plans to modify the existing wireless communications facility owned by Crown Castle and located at **115 Birch Mountain Road, Glastonbury, CT 06033**. Attached are a compound plan and elevation depicting the planned changes (Exhibit-1), and documentation of the structural sufficiency of the structure to accommodate the revised antenna configuration (Exhibit-2). Also included is a power density table report reflecting the modification to T-Mobile’s operations at the site (Exhibit-3).

The changes to the facility do not constitute a modification as defined in Connecticut General Statutes (“C.G.S.”) § 16-50i(d) because the general physical characteristics of the facility will not be significantly changed. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in the R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing tower. T-Mobile’s replacement antennas will be located at the same elevation on the existing tower.
2. There will be no proposed modifications to the ground and no extension of boundaries.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more.

4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative General Power Density table report for T-Mobile's modified facility is included as Exhibit-3.
5. A Structural Modification Report confirming that the tower and foundation can support T-Mobile's proposed modifications is included as Exhibit-2.

For the foregoing reasons, T-Mobile respectfully submits the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

Jerry Feathers
Real Estate Specialist

Enclosure

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes

Tab 2: Exhibit-2: Structural Modification Report

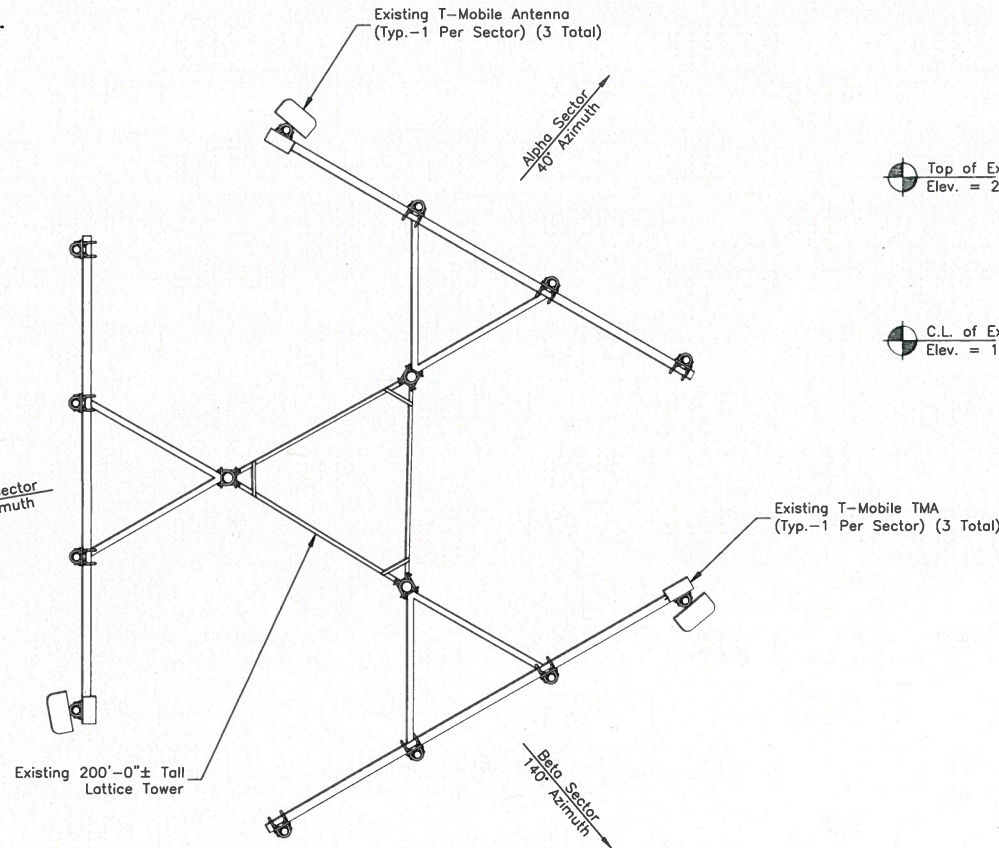
Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: Mr. Richard J. Johnson, Town Manager
Glastonbury Town Hall
2155 Main Street
Glastonbury, CT 06033

Carolyn R. Scarrone Revocable Trust
c/o Dr. Louis Scarrone
Carolyn R. Scarrone, Trustee
Glastonbury, CT 06033



Gamma Sector
260° Azimuth



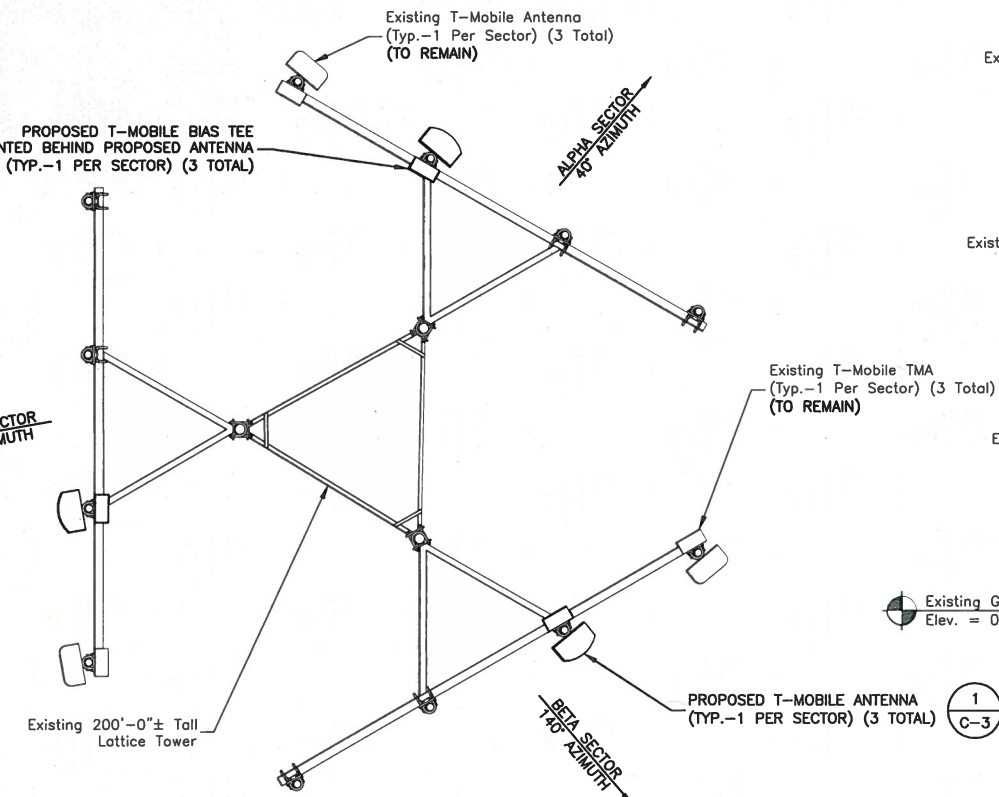
EXISTING ANTENNA LAYOUT

SCALE: N.T.S.

1



GAMMA SECTOR
260° AZIMUTH



PROPOSED ANTENNA LAYOUT

SCALE: N.T.S.

2

Top of Existing Lattice Tower
Elev. = 200'-0"± A.G.L.

C.L. of Existing T-Mobile Antennas
Elev. = 183'-0"± A.G.L.

Existing Whip Antennas
(By Others) (Typ.)

Existing T-Mobile Antenna
(Typ.-1 Per Sector) (3 Total)

Existing Antennas
(By Others) (Typ.)

Existing T-Mobile TMA
(Typ.-1 Per Sector) (3 Total)

Existing 200'-0"± Tall
Lattice Tower

Existing T-Mobile (6) 1-5/8"Ø
Coax Cables Routed Along
Lattice Tower to Antennas

Existing T-Mobile TMA
(Typ.-1 Per Sector) (3 Total)
(TO REMAIN)

Existing Chain Link Fence
w/ (3) Strands of
Barbed Wire

Existing Grade
Elev. = 0'-0" A.G.L.

EXISTING ELEVATION

SCALE: 1"=20' FOR 11"x17"
1"=10' FOR 22"x34"



3

NOTES:

- ALL PROPOSED EQUIPMENT, INCLUDING ANTENNAS, BIAS TEES, COAX, ETC., SHALL BE MOUNTED IN ACCORDANCE WITH THE TOWER STRUCTURAL ANALYSIS BY JACOBS ENGINEERING GROUP, INC. DATED FEBRUARY 12, 2015.
- DEWBERRY HAS NOT BEEN CONTRACTED TO PERFORM A STRUCTURAL ANALYSIS ON THE EXISTING ANTENNA MOUNT AND THEREFORE ASSUMES NO RESPONSIBILITY FOR THE STRUCTURAL CAPACITY.

Top of Existing Lattice Tower
Elev. = 200'-0"± A.G.L.

1
C-3
PROPOSED T-MOBILE ANTENNA
(TYP.-1 PER SECTOR) (3 TOTAL)

C.L. OF EXISTING & PROPOSED
T-MOBILE ANTENNAS
ELEV. = 183'-0"± A.G.L.

Existing Whip Antennas
(By Others) (Typ.)

Existing T-Mobile Antenna
(Typ.-1 Per Sector) (3 Total)
(TO REMAIN)

Existing Antennas
(By Others) (Typ.)

PROPOSED T-MOBILE (6) 1-5/8"Ø
COAX CABLES ROUTED WITH EXISTING
COAX CABLES ALONG LATTICE TOWER
TO ANTENNAS

Existing 200'-0"± Tall
Lattice Tower

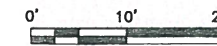
Existing T-Mobile (6) 1-5/8"Ø
Coax Cables Routed Along
Lattice Tower to Antennas
(TO REMAIN)

Existing Chain Link Fence
w/ (3) Strands of
Barbed Wire

Existing Grade
Elev. = 0'-0" A.G.L.

PROPOSED ELEVATION

SCALE: 1"=20' FOR 11"x17"
1"=10' FOR 22"x34"



4

T-Mobile

T-MOBILE NORTHEAST LLC
4 SYLVAN WAY
PARSIPPANY, NJ 07054



CROWN CASTLE
500 WEST CUMMINGS PARK, SUITE 3600
WOBURN, MA 01801

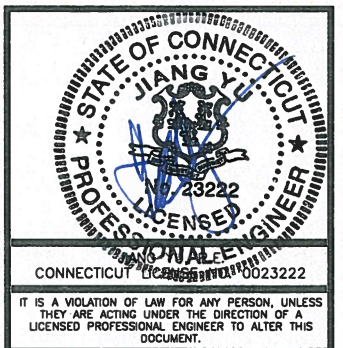
CT11189E
JOHN TOM HILL

CONSTRUCTION DRAWINGS

0	03/06/15	ISSUED AS FINAL
B	03/03/15	REVISED PER COMMENTS
A	03/02/15	ISSUED FOR REVIEW



Dewberry Engineers Inc.
600 PARSIPPANY ROAD
SUITE 301
PARSIPPANY, NJ 07054
PHONE: 973.739.9400
FAX: 973.739.9710



DRAWN BY: JC

REVIEWED BY: BSH

CHECKED BY: GHN

PROJECT NUMBER: 50066258

JOB NUMBER: 50071487

SITE ADDRESS:

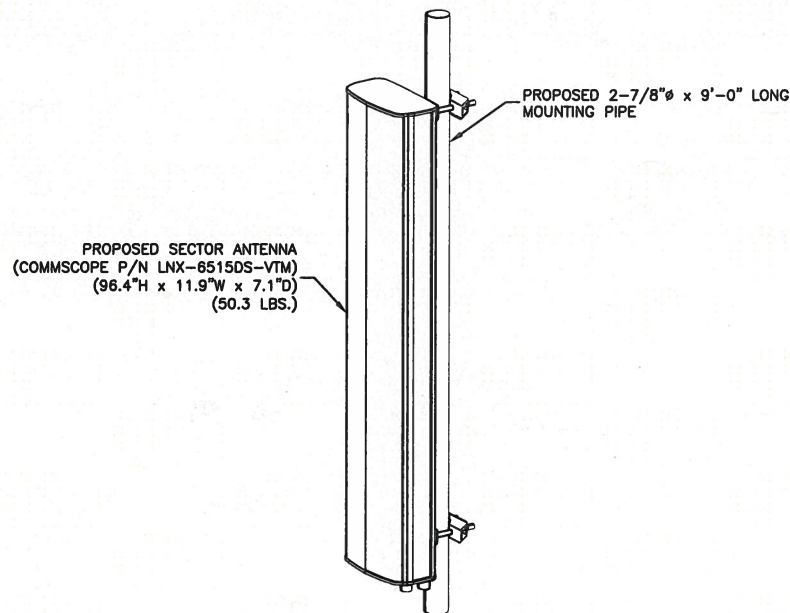
115 BIRCH MTN. ROAD
GLASTONBURY, CT 06033
HARTFORD COUNTY

SHEET TITLE

ANTENNA LAYOUTS &
ELEVATIONS

SHEET NUMBER

C-2

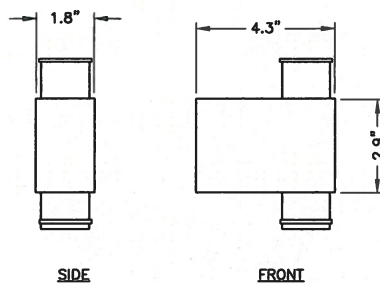


NOTES:

1. MOUNT ANTENNAS PER MANUFACTURER'S RECOMMENDATIONS.
2. GROUND ANTENNAS AND MOUNTS PER MANUFACTURER'S RECOMMENDATIONS AND T-MOBILE STANDARDS.
3. CONFIRM REQUIRED ANTENNAS WITH THE LATEST RFDS.

ISOMETRIC ANTENNA DETAIL
SCALE: N.T.S.

1



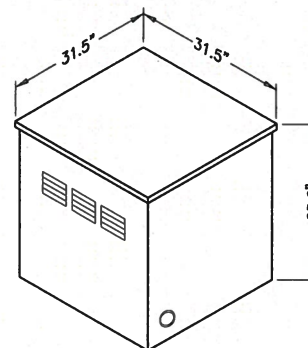
ANDREW ATBT-BOTTOM-24V

NOTES:

1. MOUNT EQUIPMENT PER MANUFACTURER'S RECOMMENDATIONS.
2. GROUND EQUIPMENT AND MOUNTS PER MANUFACTURER'S RECOMMENDATIONS AND T-MOBILE STANDARDS.
3. CONFIRM REQUIRED EQUIPMENT WITH THE LATEST RFDS.

BIAS TEE DETAIL
SCALE: N.T.S.

2



ALCATEL-LUCENT EZBF₀ BATTERY BACKUP SYSTEM

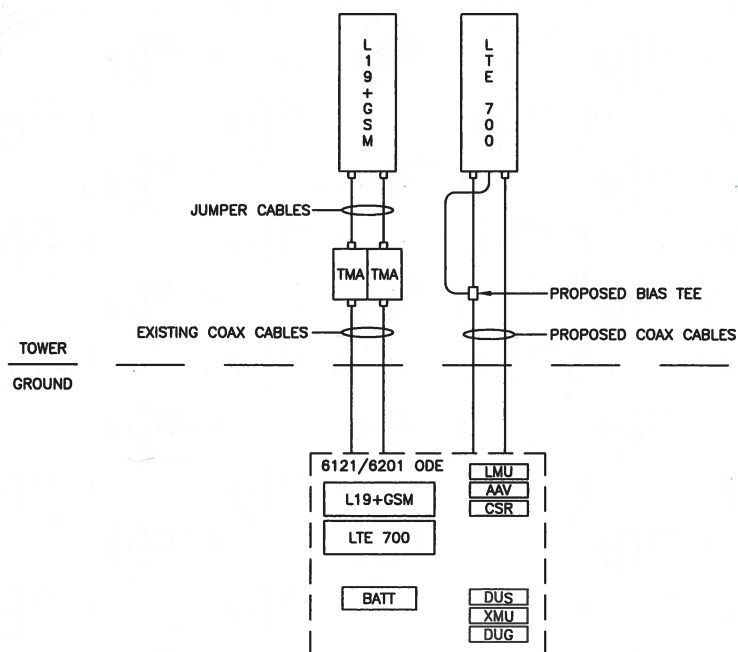
MATERIAL:	ANCHOR:
CONCRETE	3/8" HILTI KWIK BOLT 3 W/2-1/2" MIN. EMBED.
STRUCTURAL STEEL	1/2" STRUCTURAL BOLTS

NOTE:

1. CONTRACTOR SHALL ANCHOR CABINET IN ACCORDANCE WITH MANUFACTURER RECOMMENDATIONS.

BBU CABINET DETAIL
SCALE: N.T.S.

3



SITE CONFIGURATION 704G
SCALE: N.T.S.

4

DESIGN CONFIGURATION					
	ANTENNAS		COAX		COAX LENGTH
	EXISTING	PROPOSED	EXISTING	PROPOSED	
ALPHA	EMS RR90-17-02DP	EXISTING TO REMAIN	(2) 1-5/8"	(2) 1-5/8"	233'-0"
	-	COMMSCOPE LNX-6515DS-VTM			
BETA	EMS RR90-17-02DP	EXISTING TO REMAIN	(2) 1-5/8"	(2) 1-5/8"	233'-0"
	-	COMMSCOPE LNX-6515DS-VTM			
GAMMA	EMS RR90-17-02DP	EXISTING TO REMAIN	(2) 1-5/8"	(2) 1-5/8"	233'-0"
	-	COMMSCOPE LNX-6515DS-VTM			



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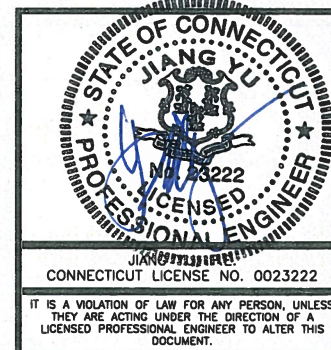
CT11189E
JOHN TOM HILL

CONSTRUCTION DRAWINGS

REV	DATE	DESCRIPTION
0	03/06/15	ISSUED AS FINAL
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HARTFORD COUNTY

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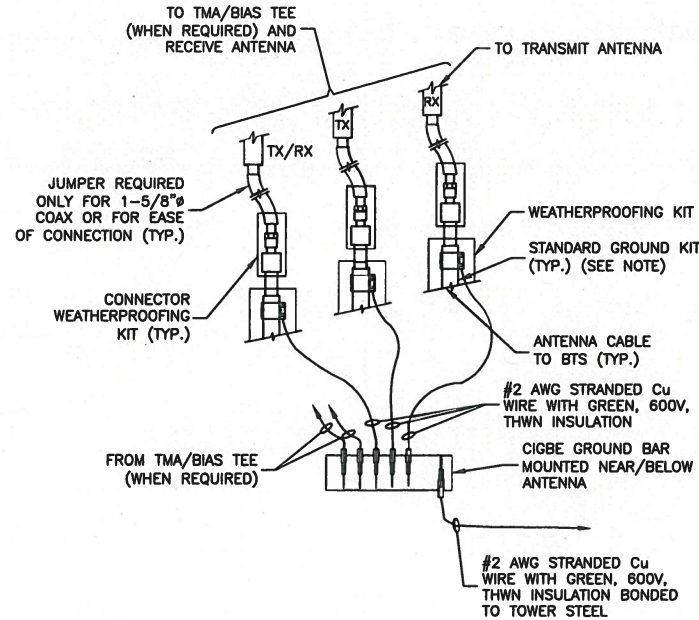
CONSTRUCTION
DETAILS

SHEET NUMBER

C-3

GROUNDING NOTES:

1. THE CONTRACTOR SHALL REVIEW AND INSPECT THE EXISTING FACILITY GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRICT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ), THE SITE-SPECIFIC (UL, LPI, OR NFPA) LIGHTNING PROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND TIA GROUNDING STANDARDS. THE CONTRACTOR SHALL REPORT ANY VIOLATIONS OR ADVERSE FINDINGS TO THE ENGINEER FOR RESOLUTION.
2. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GESS'S) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS. ALL AVAILABLE GROUNDING ELECTRODES SHALL BE CONNECTED TOGETHER IN ACCORDANCE WITH THE NEC.
3. THE CONTRACTOR SHALL PERFORM IEEE FALL-OFF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS. USE OF OTHER METHODS MUST BE PRE-APPROVED BY THE ENGINEER IN WRITING.
4. THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS ON TOWER SITES AND 10 OHMS OR LESS ON ROOFTOP SITES. WHEN ADDING ELECTRODES, CONTRACTOR SHALL MAINTAIN A MINIMUM DISTANCE BETWEEN THE ADDED ELECTRODE AND ANY OTHER EXISTING ELECTRODE EQUAL TO THE BURIED LENGTH OF THE ROD. IDEALLY, CONTRACTOR SHALL STRIVE TO KEEP THE SEPARATION DISTANCE EQUAL TO TWICE THE BURIED LENGTH OF THE RODS.
5. THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT.
6. METAL CONDUIT AND TRAY SHALL BE GROUNDING AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH 6 AWG COPPER WIRE AND UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
7. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO TRANSMISSION EQUIPMENT.
8. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED. BACK-TO-BACK CONNECTIONS ON OPPOSITE SIDES OF THE GROUND BUS ARE PERMITTED.
9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
10. USE OF 90° BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45° BENDS CAN BE ADEQUATELY SUPPORTED. IN ALL CASES, BENDS SHALL BE MADE WITH A MINIMUM BEND RADIUS OF 8 INCHES.
11. EACH INTERIOR TRANSMISSION CABINET FRAME/PLINTH SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH 6 AWG STRANDED, GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRE UNLESS NOTED OTHERWISE IN THE DETAILS. EACH OUTDOOR CABINET FRAME/PLINTH SHALL BE DIRECTLY CONNECTED TO THE BURIED GROUND RING WITH 2 AWG SOLID TIN-PLATED COPPER WIRE UNLESS NOTED OTHERWISE IN THE DETAILS.
12. ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING, SHALL BE 2 AWG SOLID TIN-PLATED COPPER UNLESS OTHERWISE INDICATED.
13. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE. CONNECTIONS TO ABOVE GRADE UNITS SHALL BE MADE WITH EXOTHERMIC WELDS WHERE PRACTICAL OR WITH 2 HOLE MECHANICAL TYPE BRASS CONNECTORS WITH STAINLESS STEEL HARDWARE, INCLUDING SET SCREWS. HIGH PRESSURE CRIMP CONNECTORS MAY ONLY BE USED WITH WRITTEN PERMISSION FROM T-MOBILE MARKET REPRESENTATIVE.
14. EXOTHERMIC WELDS SHALL BE PERMITTED ON TOWERS ONLY WITH THE EXPRESS APPROVAL OF THE TOWER MANUFACTURER OR THE CONTRACTORS STRUCTURAL ENGINEER.
15. ALL WIRE TO WIRE GROUND CONNECTIONS TO THE INTERIOR GROUND RING SHALL BE FORMED USING HIGH PRESS CRIMPS OR SPLIT BOLT CONNECTORS WHERE INDICATED IN THE DETAILS.
16. ON ROOFTOP SITES WHERE EXOTHERMIC WELDS ARE A FIRE HAZARD COPPER COMPRESSION CAP CONNECTORS MAY BE USED FOR WIRE TO WIRE CONNECTIONS. 2 HOLE MECHANICAL TYPE BRASS CONNECTORS WITH STAINLESS STEEL HARDWARE, INCLUDING SET SCREWS SHALL BE USED FOR CONNECTION TO ALL ROOFTOP TRANSMISSION EQUIPMENT AND STRUCTURAL STEEL.
17. COAX BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR USING TWO-HOLE MECHANICAL TYPE BRASS CONNECTORS AND STAINLESS STEEL HARDWARE.
18. APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
19. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.
20. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
21. BOND ALL METALLIC OBJECTS WITHIN 6 FT OF THE BURIED GROUND RING WITH 2 AWG SOLID TIN-PLATED COPPER GROUND CONDUCTOR. DURING EXCAVATION FOR NEW GROUND CONDUCTORS, IF EXISTING GROUND CONDUCTORS ARE ENCOUNTERED, BOND EXISTING GROUND CONDUCTORS TO NEW CONDUCTORS.
22. GROUND CONDUCTORS USED IN THE FACILITY GROUND AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC PLASTIC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (E.G., NON-METALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT WITH LISTED BONDING FITTINGS.



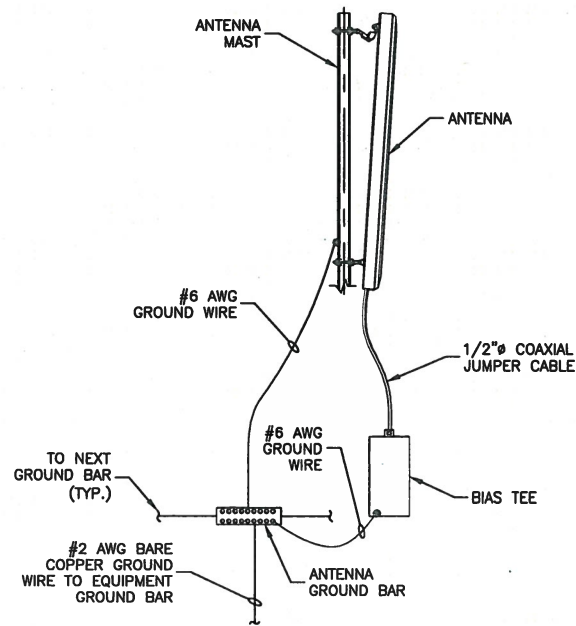
NOTE:

1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO CIGBE.

CONNECTION OF GROUND WIRES TO GROUNDING BAR (CIGBE)

SCALE: N.T.S.

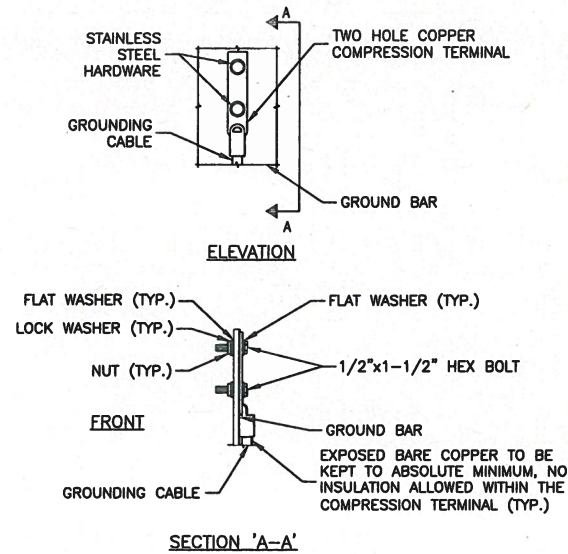
1



TYPICAL ANTENNA GROUNDING DETAIL

SCALE: N.T.S.

3



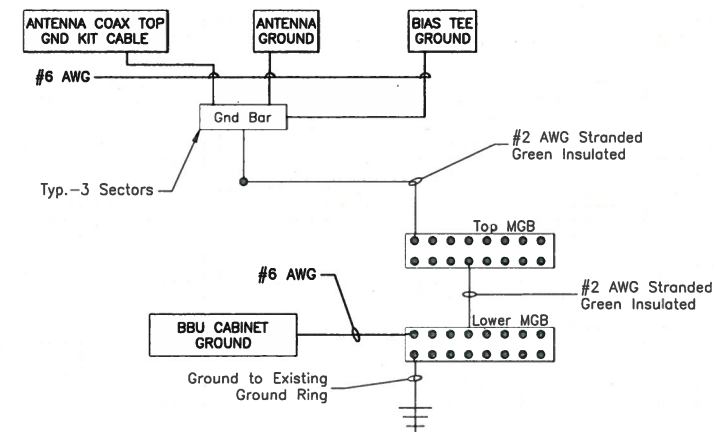
NOTES:

1. DOUBLING UP OR STACKING OF CONNECTIONS IS NOT PERMITTED.
2. OXIDE INHIBITING COMPOUND TO BE USED AT ALL LOCATIONS.

TYPICAL GROUND BAR MECHANICAL CONNECTION DETAIL

SCALE: N.T.S.

2



NOTES:

1. BOND ANTENNA GROUNDING KIT CABLE TO TOP CIGBE
2. BOND ANTENNA GROUNDING KIT CABLE TO BOTTOM CIGBE.
3. SCHEMATIC GROUNDING DIAGRAM IS TYPICAL FOR EACH SECTOR.
4. VERIFY EXISTING GROUND SYSTEM IS INSTALLED PER T-MOBILE STANDARDS.

SCHEMATIC GROUNDING DIAGRAM

SCALE: N.T.S.

4

T-Mobile

T-MOBILE NORTHEAST LLC
4 SYLVAN WAY
PARSIPPANY, NJ 07054

CROWN CASTLE

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500 WEST CUMMINGS PARK, SUITE 3600
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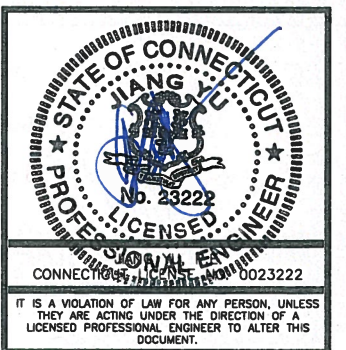
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JOHN TOM HILL**

CONSTRUCTION DRAWINGS

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Dewberry

Dewberry Engineers Inc.
800 PARSIPPANY ROAD
SUITE 301
PARSIPPANY, NJ 07054
PHONE: 973.739.8400
FAX: 973.739.8710



DRAWN BY:	JC
REVIEWED BY:	BSH
CHECKED BY:	GHN
PROJECT NUMBER:	50066258
JOB NUMBER:	50071487
SITE ADDRESS:	

115 BIRCH MTN. ROAD
GLASTONBURY, CT 06033
HARTFORD COUNTY

SHEET TITLE

GROUNDING NOTES & DETAILS

SHEET NUMBER

Date: February 12, 2015

Holly Haas
Crown Castle
3530 Toringdon Way Suite 300
Charlotte, NC 28277



Jacobs Engineering Group, Inc.

5449 Bells Ferry Road
Acworth, GA 301102
(770) 701-2500

Subject: Structural Analysis Report

Carrier Designation: T-Mobile Co-Locate
Carrier Site Number: CT11189E
Carrier Site Name: Glastonbury/ Rt 94 & Fern

Crown Castle Designation:
Crown Castle BU Number: 871584
Crown Castle Site Name: John Tom Hill
Crown Castle JDE Job Number: 322291
Crown Castle Work Order Number: 1005912
Crown Castle Application Number: 282547 Rev. 0

Engineering Firm Designation: Jacobs Engineering Group, Inc. Project Number: 1005912

Site Data: 115 Birch Mtn. Road, GLASTONBURY, Hartford County, CT
Latitude 41° 42' 32.24", Longitude -72° 28' 24.41"
200 Foot - Self Support Tower

Dear Holly Haas,

Jacobs Engineering Group, Inc. is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 754645, in accordance with application 282547, revision 0.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Existing + Proposed Equipment

Sufficient Capacity

Note: See Table I and Table II for the proposed and existing loading, respectively.

The analysis has been performed in accordance with the TIA/EIA-222-F standard and the 2005 CT State Building Code based upon a wind speed of 80 mph fastest mile.

All modifications and equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

We at Jacobs Engineering Group, Inc. appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted by:

Kristi Holder, EI
Structural Engineer



Reviewed By:

Matthew Watkins, P.E.
Engineering Project Manager

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3.2) Assumptions

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1) INTRODUCTION

This tower is a 200 ft Self Support tower designed by SABRE COMMUNICATIONS in November of 1998. The tower was originally designed for a wind speed of 85 mph per TIA/EIA-222-F.

2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 80 mph with no ice, 37.6 mph with 1.00 inch ice thickness and 50 mph under service loads.

Table 1 - Proposed Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
182.0	183.0	3	commscope	ATBT-BOTTOM-24V	6	1-5/8	-
		3	commscope	LNx-6515DS-VTM w/ Mount Pipe			

Table 2 - Existing Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note	
198.0	208.0	1	rfs celwave	ALR10-O	4	7/8	1	
	205.0	1	decibel	DB225-A	1	1/2		
		1	rfs celwave	PD1107-1	-	-	2	
	1	rfs celwave	PD201-7					
	204.0	1	scala	OGB6-928N	-	-	1	
	198.0	1	crown mounts	Sector Mount [SM 602-3]				
182.0	183.0	3	ems wireless	RR90-17-02DP w/ Mount Pipe	6	1-5/8	1	
	182.0	3	ericsson	KRY 112 71				
		1	crown mounts	Sector Mount [SM 602-3]				
170.0	171.0	6	decibel	DB980H90E-M w/ Mount Pipe	6	1-5/8	1	
	170.0	1	crown mounts	Sector Mount [SM 506-3]	1	1/2		
163.0	163.0	1	kathrein	PR-850	1	1/2	1	
		1	crown mounts	Pipe Mount [PM 601-1]				
144.0	155.0	1	sinclair	SRL480N1DT4	3	1/2	2	
	152.0	2	rfs celwave	PD1109-1				
	150.0	1	rfs celwave	PD156S-4			1	7/8
	144.0	1	crown mounts	Sector Mount [SM 602-3]				
53.0	55.0	1	lucent	KS24019-L112A	1	1/2	1	
	53.0	1	crown mounts	Side Arm Mount [SO 202-1]				

Notes:

- 1) Existing Equipment
- 2) Abandoned Equipment; Considered In This Analysis

Table 3 - Design Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
180.0	180.0	9	Dapa	DB809	9	7/8
160.0	160.0	9	Dapa	DB809	9	7/8
140.0	140.0	12	Dapa	2980	12	1-5/8
120.0	120.0	12	Dapa	2980	12	1-5/8
100.0	100.0	12	Dapa	2980	12	1-5/8
80.0	80.0	1	Generic	6' Dish	1	EW64
70.0	70.0	1	Generic	1 Meter Dish	1	7/8
60.0	60.0	1	Generic	1 Meter Dish	1	7/8
50.0	50.0	1	Generic	1 Meter Dish	1	7/8

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Clarence Welti Assoc., Inc.	1404208	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Sabre Communications	1333892	CCISITES
4-TOWER MANUFACTURER DRAWINGS	Sabre Communications	1403674	CCISITES

3.1) Analysis Method

tnxTower (version 6.1.4.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by TIA/EIA-222-F.

This analysis may be affected if any assumptions are not valid or have been made in error. Jacobs Engineering Group, Inc. should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail	
T1	200 - 180	Leg	Pipe 2.875" x 0.375" (2.5 #160)	3	-14.50	84.26	17.2	Pass	
T2	180 - 160	Leg	Pipe 3.5" x 0.300" (3 XS)	33	-41.41	96.06	43.1	Pass	
T3	160 - 140	Leg	Pipe 4" x 0.318" (3.5 XS)	60	-68.21	122.40	55.7	Pass	
T4	140 - 120	Leg	Pipe 4.5" x 0.438" (4 #120)	87	-94.22	174.90	53.9	Pass	
T5	120 - 100	Leg	Pipe 5.563" x 0.375" (5 XS)	108	-119.69	206.29	58.0	Pass	
T6	100 - 80	Leg	Pipe 5.563" x 0.375" (5 XS)	129	-144.59	206.29	70.1	Pass	
T7	80 - 60	Leg	Pipe 6.625" x 0.432" (6 EH)	150	-167.20	264.31	63.3	Pass	
T8	60 - 40	Leg	Pipe 8.625" x 0.322" (8 STD)	165	-191.59	287.78	66.6	Pass	
T9	40 - 20	Leg	Pipe 8.625" x 0.500" (8 XS)	180	-216.08	435.22	49.6	Pass	
T10	20 - 0	Leg	Pipe 8.625" x 0.500" (8 XS)	195	-247.25	435.64	56.8	Pass	
T1	200 - 180	Diagonal	L1 3/4x1 3/4x3/16	10	-2.77	9.23	30.1 34.2 (b)	Pass	
T2	180 - 160	Diagonal	L1 3/4x1 3/4x3/16	37	-3.13	6.30	49.6	Pass	
T3	160 - 140	Diagonal	L1 3/4x1 3/4x3/16	64	-3.83	4.35	88.0	Pass	
T4	140 - 120	Diagonal	L2 1/2x2 1/2x3/16	91	-4.45	8.21	54.2	Pass	
T5	120 - 100	Diagonal	L2 1/2x2 1/2x3/16	112	-4.79	6.38	75.0	Pass	
T6	100 - 80	Diagonal	L3x3x3/16	133	-5.15	8.76	58.8	Pass	
T7	80 - 60	Diagonal	L3 1/2x3 1/2x1/4	154	-6.22	12.65	49.1	Pass	
T8	60 - 40	Diagonal	L3 1/2x3 1/2x1/4	169	-6.88	10.81	63.6	Pass	
T9	40 - 20	Diagonal	L3 1/2x3 1/2x1/4	184	-7.32	9.14	80.1	Pass	
T10	20 - 0	Diagonal	L4x4x1/4	199	-8.27	11.79	70.2	Pass	
T1	200 - 180	Top Girt	L1 3/4x1 3/4x3/16	4	-0.06	5.17	1.1	Pass	
							Summary		
							Leg (T6)	70.1	Pass
							Diagonal (T3)	88.0	Pass
							Top Girt (T1)	1.1	Pass
							Bolt Checks	49.3	Pass
							RATING =	88.0	Pass

Table 6 - Tower Component Stresses vs. Capacity – LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	49.3	Pass
1	Base Foundation Structural	0	73.9	Pass
1	Base Foundation Soil Interaction	0	68.2	Pass

Structure Rating (max from all components) =	88.0%
---	--------------

Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the existing and proposed loads. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
Beacon	200	(2) 6' x 2" Mount Pipe	182
DB225-A	198	Sector Mount [SM 602-3]	182
ALR10-O	198	(2) DB980H90E-M w/ Mount Pipe	170
OGB6-928N	198	(2) DB980H90E-M w/ Mount Pipe	170
PD1107-1	198	(2) DB980H90E-M w/ Mount Pipe	170
PD201-7	198	6' x 2" Mount Pipe	170
(4) 6' x 2" Mount Pipe	198	6' x 2" Mount Pipe	170
(4) 6' x 2" Mount Pipe	198	6' x 2" Mount Pipe	170
(4) 6' x 2" Mount Pipe	198	Sector Mount [SM 506-3]	170
Sector Mount [SM 602-3]	198	Pipe Mount [PM 601-1]	163
RR90-17-02DP w/ Mount Pipe	182	PR-850	163
RR90-17-02DP w/ Mount Pipe	182	PD156S-4	144
RR90-17-02DP w/ Mount Pipe	182	SRL480N1DT4	144
KRY 112 71	182	(2) PD1109-1	144
KRY 112 71	182	(4) 6' x 2" Mount Pipe	144
KRY 112 71	182	(4) 6' x 2" Mount Pipe	144
LNx-6515DS-VTM w/ Mount Pipe	182	(4) 6' x 2" Mount Pipe	144
LNx-6515DS-VTM w/ Mount Pipe	182	Sector Mount [SM 602-3]	144
LNx-6515DS-VTM w/ Mount Pipe	182	Sidemarker	100
ATBT-BOTTOM-24V	182	Sidemarker	100
ATBT-BOTTOM-24V	182	Sidemarker	100
ATBT-BOTTOM-24V	182	Side Arm Mount [SO 202-1]	53
(2) 6' x 2" Mount Pipe	182	KS24019-L112A	53
(2) 6' x 2" Mount Pipe	182		

SYMBOL LIST

MARK	SIZE	MARK	SIZE
A	Pipe 2.875" x 0.375" (2.5 #160)	D	Pipe 4.5" x 0.438" (4 #120)
B	Pipe 3.5" x 0.300" (3 XS)	E	Pipe 6.625" x 0.432" (6 EH)
C	Pipe 4" x 0.318" (3.5 XS)	F	Pipe 8.625" x 0.322" (8 STD)

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

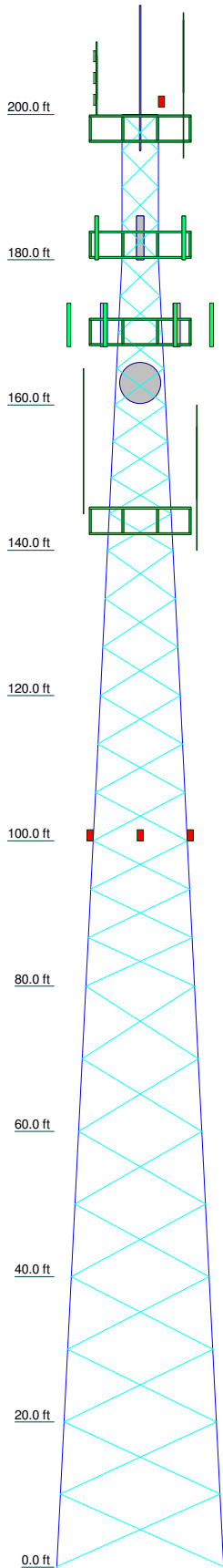
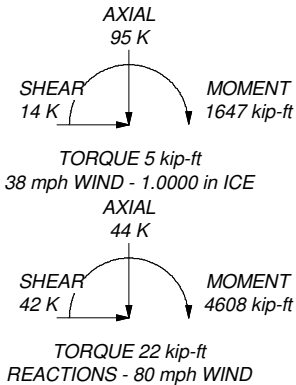
TOWER DESIGN NOTES

1. Tower is located in Hartford County, Connecticut.
2. Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 38 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 88%

MAX. CORNER REACTIONS AT BASE:

DOWN: 246 K
SHEAR: 26 K

UPLIFT: -198 K
SHEAR: 22 K



Section	T1	T2	T3	T4	T5	T6	T7	T8	T9	T10
Legs	A	B	C	D	(5 XS)	Pipe 5.563" x 0.375"	E	F	(8 XS)	Pipe 8.625" x 0.500"
Leg Grade	A572-50									
Diagonals	L1 3/4x1 3/4x3/16									
Diagonal Grade	A36									
Top Girts	N.A.									
Face Width (ft)	5	7	9	11	13	15	17	19	21	23
# Panels @ (ft)	12 @ 5	9 @ 6.66667	6 @ 10	4.2	4.5	2.0	2.3	3.1	3.0	3.1
Weight (K)	1.0	1.0	1.2	1.8	2.0	2.3	3.0	3.1	4.2	4.5

<p>JACOBS</p> <p>5449 Bells Ferry Road Acworth, GA 301102 Phone: (770) 701-2500 FAX: (770) 701-2501</p>	<p>Job: John Tom Hill BU 871584</p> <p>Project: 1005912</p>	
	<p>Client: Crown Castle</p> <p>Code: TIA/EIA-222-F</p> <p>Path:</p>	<p>Drawn by: holderkg</p> <p>Date: 02/11/15</p>
	<p>App'd:</p> <p>Scale: NTS</p> <p>Dwg No. E-1</p>	
	<p><small>C:\Users\holderkg\Desktop\Project Storage\871584 John Tom Hill 1005912 Analysis\CG Model\John Tom Hill BU 871584.dwg</small></p>	

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Tower Input Data

The main tower is a 3x free standing tower with an overall height of 200.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 5.00 ft at the top and 23.00 ft at the base.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in Hartford County, Connecticut.

Basic wind speed of 80 mph.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 38 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

Pressures are calculated at each section.

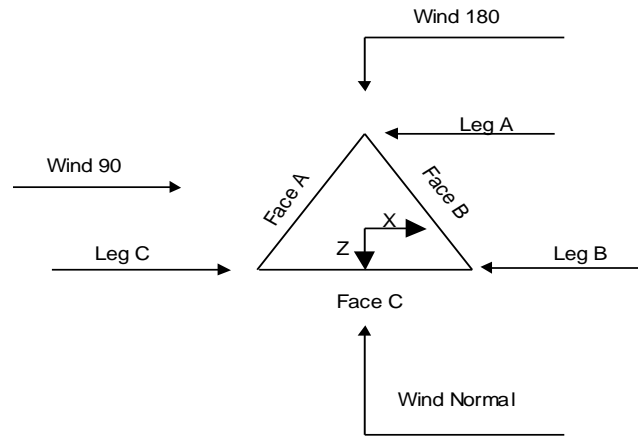
Stress ratio used in tower member design is 1.333.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Treat Feedline Bundles As Cylinder
Consider Moments - Horizontals	Assume Legs Pinned	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Diagonals	√ Assume Rigid Index Plate	√ Calculate Redundant Bracing Forces
Use Moment Magnification	√ Use Clear Spans For Wind Area	Ignore Redundant Members in FEA
√ Use Code Stress Ratios	√ Use Clear Spans For KL/r	√ SR Leg Bolts Resist Compression
√ Use Code Safety Factors - Guys	Retension Guys To Initial Tension	√ All Leg Panels Have Same Allowable
√ Escalate Ice	√ Bypass Mast Stability Checks	√ Offset Girt At Foundation
Always Use Max Kz	√ Use Azimuth Dish Coefficients	√ Consider Feedline Torque
Use Special Wind Profile	√ Project Wind Area of Appurt.	√ Include Angle Block Shear Check
√ Include Bolts In Member Capacity	Autocalc Torque Arm Areas	Poles
Leg Bolts Are At Top Of Section	SR Members Have Cut Ends	Include Shear-Torsion Interaction
√ Secondary Horizontal Braces Leg	√ Sort Capacity Reports By Component	Always Use Sub-Critical Flow
Use Diamond Inner Bracing (4 Sided)	Triangulate Diamond Inner Bracing	Use Top Mounted Sockets
Add IBC .6D+W Combination	Use TIA-222-G Tension Splice Capacity	
	Exemption	

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Triangular Tower

Tower Section Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	<i>ft</i>			<i>ft</i>		<i>ft</i>
T1	200.00-180.00			5.00	1	20.00
T2	180.00-160.00			5.00	1	20.00
T3	160.00-140.00			7.00	1	20.00
T4	140.00-120.00			9.00	1	20.00
T5	120.00-100.00			11.00	1	20.00
T6	100.00-80.00			13.00	1	20.00
T7	80.00-60.00			15.00	1	20.00
T8	60.00-40.00			17.00	1	20.00
T9	40.00-20.00			19.00	1	20.00
T10	20.00-0.00			21.00	1	20.00

Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	<i>ft</i>	<i>ft</i>				<i>in</i>	<i>in</i>
T1	200.00-180.00	5.00	X Brace	No	No	0.0000	0.0000
T2	180.00-160.00	5.00	X Brace	No	No	0.0000	0.0000
T3	160.00-140.00	5.00	X Brace	No	No	0.0000	0.0000
T4	140.00-120.00	6.67	X Brace	No	No	0.0000	0.0000
T5	120.00-100.00	6.67	X Brace	No	No	0.0000	0.0000

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	Client	Crown Castle	Designed by	holderkg

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T6	100.00-80.00	6.67	X Brace	No	No	0.0000	0.0000
T7	80.00-60.00	10.00	X Brace	No	No	0.0000	0.0000
T8	60.00-40.00	10.00	X Brace	No	No	0.0000	0.0000
T9	40.00-20.00	10.00	X Brace	No	No	0.0000	0.0000
T10	20.00-0.00	9.96	X Brace	No	No	0.0000	1.0000

Tower Section Geometry (cont'd)

Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
ft						
T1 200.00-180.00	Pipe	Pipe 2.875" x 0.375" (2.5 #160)	A572-50 (50 ksi)	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T2 180.00-160.00	Pipe	Pipe 3.5" x 0.300" (3 XS)	A572-50 (50 ksi)	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T3 160.00-140.00	Pipe	Pipe 4" x 0.318" (3.5 XS)	A572-50 (50 ksi)	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T4 140.00-120.00	Pipe	Pipe 4.5" x 0.438" (4 #120)	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T5 120.00-100.00	Pipe	Pipe 5.563" x 0.375" (5 XS)	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T6 100.00-80.00	Pipe	Pipe 5.563" x 0.375" (5 XS)	A572-50 (50 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)
T7 80.00-60.00	Pipe	Pipe 6.625" x 0.432" (6 EH)	A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A36 (36 ksi)
T8 60.00-40.00	Pipe	Pipe 8.625" x 0.322" (8 STD)	A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A36 (36 ksi)
T9 40.00-20.00	Pipe	Pipe 8.625" x 0.500" (8 XS)	A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A36 (36 ksi)
T10 20.00-0.00	Pipe	Pipe 8.625" x 0.500" (8 XS)	A572-50 (50 ksi)	Equal Angle	L4x4x1/4	A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
ft						
T1 200.00-180.00	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft ²	in					in	in

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Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A_f	Adjust. Factor A_r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in
ft	ft ²	in						
T1 200.00-180.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T2 180.00-160.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T3 160.00-140.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T4 140.00-120.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T5 120.00-100.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T6 100.00-80.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T7 80.00-60.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T8 60.00-40.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T9 40.00-20.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T10 20.00-0.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000

Tower Section Geometry (cont'd)

Tower Elevation	Calc K Single Angles	Calc K Solid Rounds	Legs	K Factors ¹							
				X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace	
											X
ft			Y	Y	Y	Y	Y	Y	Y	Y	
T1 200.00-180.00	Yes	No	1	1	1	1	1	1	1	1	1
T2 180.00-160.00	Yes	No	1	1	1	1	1	1	1	1	1
T3 160.00-140.00	Yes	No	1	1	1	1	1	1	1	1	1
T4 140.00-120.00	Yes	No	1	1	1	1	1	1	1	1	1
T5 120.00-100.00	Yes	No	1	1	1	1	1	1	1	1	1
T6 100.00-80.00	Yes	No	1	1	1	1	1	1	1	1	1
T7 80.00-60.00	Yes	No	1	1	1	1	1	1	1	1	1
T8 60.00-40.00	Yes	No	1	1	1	1	1	1	1	1	1
T9 40.00-20.00	Yes	No	1	1	1	1	1	1	1	1	1
T10 20.00-0.00	Yes	No	1	1	1	1	1	1	1	1	1

¹Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

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	Client	Crown Castle	Designed by	holderkg

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 200.00-180.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T2 180.00-160.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T3 160.00-140.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T4 140.00-120.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T5 120.00-100.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 100.00-80.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 80.00-60.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 60.00-40.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T9 40.00-20.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T10 20.00-0.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 200.00-180.00	Flange	0.7500	4	0.6250	1	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325X		A325N		A325N		A325N		A325N	
T2 180.00-160.00	Flange	1.0000	4	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T3 160.00-140.00	Flange	1.0000	4	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T4 140.00-120.00	Flange	1.2500	4	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T5 120.00-100.00	Flange	1.2500	4	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T6 100.00-80.00	Flange	1.2500	6	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T7 80.00-60.00	Flange	1.2500	6	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T8 60.00-40.00	Flange	1.3750	6	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T9 40.00-20.00	Flange	1.3750	6	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T10 20.00-0.00	Flange	1.5000	8	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A572-50		A325X		A325N		A325N		A325N		A325N		A325N	

Feed Line/Linear Appurtenances - Entered As Round Or Flat

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Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
** A Face **												
** B Face **												
AVA7-50(1-5/8)	B	Yes	Ar (CfAe)	182.00 - 0.00	0.0000	-0.4	12	6	0.5000	2.0100		0.70
Feedline Ladder (Af)	B	Yes	Af (CfAe)	182.00 - 0.00	0.0000	-0.4	1	1	3.0000	3.0000	12.0000	8.40
Feedline Ladder (Af)	B	Yes	Af (CfAe)	200.00 - 0.00	0.0000	0	1	1	3.0000	3.0000	12.0000	8.40
** FLC 12-50J(1/2")	B	Yes	Ar (CfAe)	163.00 - 0.00	0.0000	-0.2	1	1	0.5000	0.6400		0.17
Feedline Ladder (Af)	B	Yes	Af (CfAe)	163.00 - 0.00	0.0000	-0.2	1	1	3.0000	3.0000	12.0000	8.40
** LDF4-50A(1/2")	B	Yes	Ar (CfAe)	144.00 - 0.00	0.0000	0.4	4	4	0.5000	0.6300		0.15
LDF4-50A(1/2")	B	Yes	Ar (CfAe)	198.00 - 144.00	0.0000	0.4	1	1	0.5000	0.6300		0.15
LDF5-50A(7/8")	B	Yes	Ar (CfAe)	144.00 - 0.00	0.0000	0.35	5	3	0.5000	1.0900		0.33
LDF5-50A(7/8")	B	Yes	Ar (CfAe)	198.00 - 144.00	0.0000	0.35	4	2	0.5000	1.0900		0.33
Feedline Ladder (Af)	B	Yes	Af (CfAe)	198.00 - 0.00	0.0000	0	1	1	3.0000	3.0000	12.0000	8.40
** C Face **												
LDF7-50A(1-5/8")	C	Yes	Ar (CfAe)	170.00 - 0.00	-1.5000	0	6	6	0.5000	1.9800		0.82
LDF4-50A(1/2")	C	Yes	Ar (CfAe)	53.00 - 0.00	-1.5000	0.08	1	1	0.5000	0.6300		0.15
Feedline Ladder (Af)	C	Yes	Af (CfAe)	170.00 - 0.00	-1.5000	0	1	1	3.0000	3.0000	12.0000	8.40
** Safety Line 3/8 Climbing Ladder	C	Yes	Ar (CfAe)	200.00 - 0.00	0.0000	0	1	1	0.3750	0.3750		0.22
	C	Yes	Af (CfAe)	200.00 - 0.00	0.0000	0	1	1	2.5000	2.5000	10.0000	7.90

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	C _{AA} ft ² /ft	Weight plf
** A Face **							
**							
**							

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
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Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
T1	200.00-180.00	A	0.000	0.000	0.000	0.000	0.00
		B	6.225	10.000	0.000	0.000	0.38
		C	0.625	4.167	0.000	0.000	0.16
T2	180.00-160.00	A	0.000	0.000	0.000	0.000	0.00
		B	24.943	15.750	0.000	0.000	0.73
		C	10.525	6.667	0.000	0.000	0.30
T3	160.00-140.00	A	0.000	0.000	0.000	0.000	0.00
		B	26.843	20.000	0.000	0.000	0.88
		C	20.425	9.167	0.000	0.000	0.43
T4	140.00-120.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	20.425	9.167	0.000	0.000	0.43
T5	120.00-100.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	20.425	9.167	0.000	0.000	0.43
T6	100.00-80.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	20.425	9.167	0.000	0.000	0.43
T7	80.00-60.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	20.425	9.167	0.000	0.000	0.43
T8	60.00-40.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	21.108	9.167	0.000	0.000	0.43
T9	40.00-20.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	21.475	9.167	0.000	0.000	0.43
T10	20.00-0.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	21.475	9.167	0.000	0.000	0.43

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
T1	200.00-180.00	A	1.234	0.000	0.000	0.000	0.000	0.00
		B		10.729	19.960	0.000	0.000	0.95
		C		4.738	6.908	0.000	0.000	0.35
T2	180.00-160.00	A	1.217	0.000	0.000	0.000	0.000	0.00
		B		19.159	47.838	0.000	0.000	2.03
		C		8.362	21.058	0.000	0.000	0.76
T3	160.00-140.00	A	1.199	0.000	0.000	0.000	0.000	0.00
		B		23.273	55.887	0.000	0.000	2.36
		C		11.920	35.163	0.000	0.000	1.16
T4	140.00-120.00	A	1.179	0.000	0.000	0.000	0.000	0.00
		B		23.001	62.345	0.000	0.000	2.42
		C		11.784	35.073	0.000	0.000	1.15
T5	120.00-100.00	A	1.155	0.000	0.000	0.000	0.000	0.00
		B		22.689	62.137	0.000	0.000	2.39
		C		11.628	34.969	0.000	0.000	1.13
T6	100.00-80.00	A	1.128	0.000	0.000	0.000	0.000	0.00
		B		22.323	61.893	0.000	0.000	2.36
		C		11.445	34.846	0.000	0.000	1.11
T7	80.00-60.00	A	1.094	0.000	0.000	0.000	0.000	0.00
		B		21.876	61.595	0.000	0.000	2.31
		C		11.221	34.697	0.000	0.000	1.09
T8	60.00-40.00	A	1.051	0.000	0.000	0.000	0.00	

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Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A_R ft ²	A_F ft ²	C_{AA} In Face ft ²	C_{AA} Out Face ft ²	Weight K
T9	40.00-20.00	B		21.298	61.210	0.000	0.000	2.26
		C		13.892	34.505	0.000	0.000	1.09
		A	1.000	0.000	0.000	0.000	0.000	0.00
T10	20.00-0.00	B		20.617	60.756	0.000	0.000	2.19
		C		14.975	34.278	0.000	0.000	1.07
		A	1.000	0.000	0.000	0.000	0.000	0.00
		B		20.617	60.756	0.000	0.000	2.19
		C		14.975	34.278	0.000	0.000	1.07

Feed Line Shielding

Section	Elevation ft	Face	A_R ft ²	A_R Ice ft ²	A_F ft ²	A_F Ice ft ²
T1	200.00-180.00	A	0.000	0.000	0.000	0.000
		B	0.000	4.232	1.457	3.002
		C	0.000	1.648	0.430	1.169
T2	180.00-160.00	A	0.000	0.000	0.000	0.000
		B	0.000	7.563	3.104	5.436
		C	0.000	3.338	1.311	2.399
T3	160.00-140.00	A	0.000	0.000	0.000	0.000
		B	0.000	7.980	3.228	5.823
		C	0.000	4.699	2.039	3.428
T4	140.00-120.00	A	0.000	0.000	0.000	0.000
		B	0.000	6.425	3.822	6.812
		C	0.000	3.509	2.225	3.721
T5	120.00-100.00	A	0.000	0.000	0.000	0.000
		B	0.000	5.949	3.636	6.436
		C	0.000	3.251	2.117	3.517
T6	100.00-80.00	A	0.000	0.000	0.000	0.000
		B	0.000	5.576	4.223	7.415
		C	0.000	3.049	2.459	4.055
T7	80.00-60.00	A	0.000	0.000	0.000	0.000
		B	0.000	3.801	3.497	6.079
		C	0.000	2.081	2.036	3.327
T8	60.00-40.00	A	0.000	0.000	0.000	0.000
		B	0.000	3.495	3.392	5.819
		C	0.000	2.034	2.021	3.386
T9	40.00-20.00	A	0.000	0.000	0.000	0.000
		B	0.000	3.199	3.315	5.598
		C	0.000	1.919	1.999	3.358
T10	20.00-0.00	A	0.000	0.000	0.000	0.000
		B	0.000	3.140	3.719	6.281
		C	0.000	1.884	2.243	3.767

Feed Line Center of Pressure

Section	Elevation ft	CP_x in	CP_z in	CP_x Ice in	CP_z Ice in
T1	200.00-180.00	3.4651	-0.4625	2.0800	0.2993
T2	180.00-160.00	3.9589	-5.1825	2.7190	-2.4222

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Section	Elevation	CP _x	CP _z	CP _x	CP _z
	ft	in	in	Ice in	Ice in
T3	160.00-140.00	4.9408	-4.4982	3.5930	-2.6912
T4	140.00-120.00	6.4598	-4.2753	4.4918	-2.9657
T5	120.00-100.00	6.9684	-4.5573	4.9955	-3.2447
T6	100.00-80.00	7.2794	-4.7194	5.3018	-3.4309
T7	80.00-60.00	8.0650	-5.1938	6.3955	-4.0271
T8	60.00-40.00	7.9729	-4.9685	6.4325	-3.6130
T9	40.00-20.00	8.5702	-5.2387	6.9087	-3.6366
T10	20.00-0.00	8.6861	-5.2884	7.0572	-3.7110

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
			Horz Lateral ft	Vert ft					
Beacon	B	From Leg	0.50	0.0000	200.00	No Ice	2.00	2.00	0.02
						1/2" Ice	2.50	2.50	0.03
						1" Ice	3.00	3.00	0.04
						2" Ice	4.00	4.00	0.06
						4" Ice	6.00	6.00	0.10
Sidemarker	A	From Leg	0.50	0.0000	100.00	No Ice	0.60	0.60	0.01
						1/2" Ice	0.72	0.72	0.02
						1" Ice	0.86	0.86	0.03
						2" Ice	1.16	1.16	0.05
						4" Ice	1.89	1.89	0.13
Sidemarker	B	From Leg	0.50	0.0000	100.00	No Ice	0.60	0.60	0.01
						1/2" Ice	0.72	0.72	0.02
						1" Ice	0.86	0.86	0.03
						2" Ice	1.16	1.16	0.05
						4" Ice	1.89	1.89	0.13
Sidemarker	C	From Leg	0.50	0.0000	100.00	No Ice	0.60	0.60	0.01
						1/2" Ice	0.72	0.72	0.02
						1" Ice	0.86	0.86	0.03
						2" Ice	1.16	1.16	0.05
						4" Ice	1.89	1.89	0.13
**									
** 198 **									
DB225-A	A	From Leg	4.00	0.0000	198.00	No Ice	3.21	3.21	0.04
						1/2" Ice	5.78	5.78	0.05
						1" Ice	8.35	8.35	0.06
						2" Ice	13.48	13.48	0.08
						4" Ice	23.75	23.75	0.13
ALR10-O	B	From Leg	4.00	0.0000	198.00	No Ice	13.25	13.25	0.09
						1/2" Ice	15.31	15.31	0.18
						1" Ice	17.39	17.39	0.28
						2" Ice	20.79	20.79	0.52
						4" Ice	25.72	25.72	1.17
OGB6-928N	B	From Leg	4.00	0.0000	198.00	No Ice	0.97	0.97	0.01
						1/2" Ice	1.33	1.33	0.02
						1" Ice	1.63	1.63	0.03
						2" Ice	2.26	2.26	0.06
						4" Ice	3.67	3.67	0.17
PD1107-1	C	From Leg	4.00	0.0000	198.00	No Ice	2.18	2.18	0.01

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Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight
			Horz	Lateral	Vert					
							1/2" Ice	3.29	3.29	0.02
							1" Ice	4.43	4.43	0.05
							2" Ice	6.42	6.42	0.12
							4" Ice	9.18	9.18	0.35
PD201-7	C	From Leg	4.00	0.0000	198.00		No Ice	1.02	1.02	0.00
			0.00				1/2" Ice	1.81	1.81	0.01
			7.00				1" Ice	2.62	2.62	0.03
							2" Ice	3.76	3.76	0.07
							4" Ice	5.78	5.78	0.22
(4) 6' x 2" Mount Pipe	A	From Leg	4.00	0.0000	198.00		No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.50	1.50	0.03
			0.00				1" Ice	1.57	1.57	0.04
							2" Ice	1.71	1.71	0.05
							4" Ice	1.99	1.99	0.07
(4) 6' x 2" Mount Pipe	B	From Leg	4.00	0.0000	198.00		No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.50	1.50	0.03
			0.00				1" Ice	1.57	1.57	0.04
							2" Ice	1.71	1.71	0.05
							4" Ice	1.99	1.99	0.07
(4) 6' x 2" Mount Pipe	C	From Leg	4.00	0.0000	198.00		No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.50	1.50	0.03
			0.00				1" Ice	1.57	1.57	0.04
							2" Ice	1.71	1.71	0.05
							4" Ice	1.99	1.99	0.07
Sector Mount [SM 602-3]	C	None		0.0000	198.00		No Ice	33.11	33.11	1.54
							1/2" Ice	44.90	44.90	2.16
							1" Ice	56.69	56.69	2.78
							2" Ice	80.27	80.27	4.01
							4" Ice	127.43	127.43	6.49
**										
** 182 **										
RR90-17-02DP w/ Mount Pipe	A	From Leg	4.00	0.0000	182.00		No Ice	4.59	3.32	0.03
			0.00				1/2" Ice	5.09	4.09	0.07
			1.00				1" Ice	5.58	4.78	0.12
							2" Ice	6.59	6.23	0.22
							4" Ice	8.73	9.31	0.56
RR90-17-02DP w/ Mount Pipe	B	From Leg	4.00	0.0000	182.00		No Ice	4.59	3.32	0.03
			0.00				1/2" Ice	5.09	4.09	0.07
			1.00				1" Ice	5.58	4.78	0.12
							2" Ice	6.59	6.23	0.22
							4" Ice	8.73	9.31	0.56
RR90-17-02DP w/ Mount Pipe	C	From Leg	4.00	0.0000	182.00		No Ice	4.59	3.32	0.03
			0.00				1/2" Ice	5.09	4.09	0.07
			1.00				1" Ice	5.58	4.78	0.12
							2" Ice	6.59	6.23	0.22
							4" Ice	8.73	9.31	0.56
KRY 112 71	A	From Leg	4.00	0.0000	182.00		No Ice	0.68	0.45	0.01
			0.00				1/2" Ice	0.80	0.56	0.02
			0.00				1" Ice	0.93	0.68	0.03
							2" Ice	1.22	0.94	0.04
							4" Ice	1.90	1.57	0.11
KRY 112 71	B	From Leg	4.00	0.0000	182.00		No Ice	0.68	0.45	0.01
			0.00				1/2" Ice	0.80	0.56	0.02
			0.00				1" Ice	0.93	0.68	0.03
							2" Ice	1.22	0.94	0.04
							4" Ice	1.90	1.57	0.11
KRY 112 71	C	From Leg	4.00	0.0000	182.00		No Ice	0.68	0.45	0.01

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight
			Horz	Lateral					
			0.00						
			0.00			1/2" Ice	0.80	0.56	0.02
						1" Ice	0.93	0.68	0.03
						2" Ice	1.22	0.94	0.04
						4" Ice	1.90	1.57	0.11
LNx-6515DS-VTM w/ Mount Pipe	A	From Leg	4.00	0.0000	182.00	No Ice	11.68	9.84	0.08
			0.00			1/2" Ice	12.40	11.37	0.17
			1.00			1" Ice	13.14	12.91	0.27
						2" Ice	14.60	15.27	0.51
						4" Ice	17.87	20.14	1.15
LNx-6515DS-VTM w/ Mount Pipe	B	From Leg	4.00	0.0000	182.00	No Ice	11.68	9.84	0.08
			0.00			1/2" Ice	12.40	11.37	0.17
			1.00			1" Ice	13.14	12.91	0.27
						2" Ice	14.60	15.27	0.51
						4" Ice	17.87	20.14	1.15
LNx-6515DS-VTM w/ Mount Pipe	C	From Leg	4.00	0.0000	182.00	No Ice	11.68	9.84	0.08
			0.00			1/2" Ice	12.40	11.37	0.17
			1.00			1" Ice	13.14	12.91	0.27
						2" Ice	14.60	15.27	0.51
						4" Ice	17.87	20.14	1.15
ATBT-BOTTOM-24V	A	From Leg	4.00	0.0000	182.00	No Ice	0.12	0.08	0.00
			0.00			1/2" Ice	0.17	0.12	0.00
			1.00			1" Ice	0.23	0.17	0.01
						2" Ice	0.38	0.30	0.01
						4" Ice	0.77	0.67	0.04
ATBT-BOTTOM-24V	B	From Leg	4.00	0.0000	182.00	No Ice	0.12	0.08	0.00
			0.00			1/2" Ice	0.17	0.12	0.00
			1.00			1" Ice	0.23	0.17	0.01
						2" Ice	0.38	0.30	0.01
						4" Ice	0.77	0.67	0.04
ATBT-BOTTOM-24V	C	From Leg	4.00	0.0000	182.00	No Ice	0.12	0.08	0.00
			0.00			1/2" Ice	0.17	0.12	0.00
			1.00			1" Ice	0.23	0.17	0.01
						2" Ice	0.38	0.30	0.01
						4" Ice	0.77	0.67	0.04
(2) 6' x 2" Mount Pipe	A	From Leg	4.00	0.0000	182.00	No Ice	1.43	1.43	0.02
			0.00			1/2" Ice	1.50	1.50	0.03
			0.00			1" Ice	1.57	1.57	0.04
						2" Ice	1.71	1.71	0.05
						4" Ice	1.99	1.99	0.07
(2) 6' x 2" Mount Pipe	B	From Leg	4.00	0.0000	182.00	No Ice	1.43	1.43	0.02
			0.00			1/2" Ice	1.50	1.50	0.03
			0.00			1" Ice	1.57	1.57	0.04
						2" Ice	1.71	1.71	0.05
						4" Ice	1.99	1.99	0.07
(2) 6' x 2" Mount Pipe	C	From Leg	4.00	0.0000	182.00	No Ice	1.43	1.43	0.02
			0.00			1/2" Ice	1.50	1.50	0.03
			0.00			1" Ice	1.57	1.57	0.04
						2" Ice	1.71	1.71	0.05
						4" Ice	1.99	1.99	0.07
Sector Mount [SM 602-3]	C	None		0.0000	182.00	No Ice	33.11	33.11	1.54
						1/2" Ice	44.90	44.90	2.16
						1" Ice	56.69	56.69	2.78
						2" Ice	80.27	80.27	4.01
						4" Ice	127.43	127.43	6.49
**									
** 170 **									
(2) DB980H90E-M w/ Mount	A	From Leg	5.00	0.0000	170.00	No Ice	4.04	3.62	0.03

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K	
			Horz ft	Lateral ft						
Pipe			0.00			1/2" Ice	4.50	4.48	0.07	
			1.00			1" Ice	4.95	5.22	0.11	
						2" Ice	5.87	6.74	0.22	
						4" Ice	8.05	10.00	0.55	
(2) DB980H90E-M w/ Mount Pipe	B	From Leg	5.00		0.0000	170.00	No Ice	4.04	3.62	0.03
			0.00				1/2" Ice	4.50	4.48	0.07
			1.00				1" Ice	4.95	5.22	0.11
							2" Ice	5.87	6.74	0.22
							4" Ice	8.05	10.00	0.55
(2) DB980H90E-M w/ Mount Pipe	C	From Leg	5.00		0.0000	170.00	No Ice	4.04	3.62	0.03
			0.00				1/2" Ice	4.50	4.48	0.07
			1.00				1" Ice	4.95	5.22	0.11
							2" Ice	5.87	6.74	0.22
							4" Ice	8.05	10.00	0.55
6' x 2" Mount Pipe	A	From Leg	5.00		0.0000	170.00	No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.50	1.50	0.03
			0.00				1" Ice	1.57	1.57	0.04
							2" Ice	1.71	1.71	0.05
							4" Ice	1.99	1.99	0.07
6' x 2" Mount Pipe	B	From Leg	5.00		0.0000	170.00	No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.50	1.50	0.03
			0.00				1" Ice	1.57	1.57	0.04
							2" Ice	1.71	1.71	0.05
							4" Ice	1.99	1.99	0.07
6' x 2" Mount Pipe	C	From Leg	5.00		0.0000	170.00	No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.50	1.50	0.03
			0.00				1" Ice	1.57	1.57	0.04
							2" Ice	1.71	1.71	0.05
							4" Ice	1.99	1.99	0.07
Sector Mount [SM 506-3]	C	None			0.0000	170.00	No Ice	35.47	35.47	1.74
							1/2" Ice	50.60	50.60	2.35
							1" Ice	65.73	65.73	2.95
							2" Ice	95.99	95.99	4.16
							4" Ice	156.51	156.51	6.59
** ** 163 **										
Pipe Mount [PM 601-1]	A	From Leg	0.50		0.0000	163.00	No Ice	3.00	0.90	0.07
			0.00				1/2" Ice	3.74	1.12	0.08
			0.00				1" Ice	4.48	1.34	0.09
							2" Ice	5.96	1.78	0.12
							4" Ice	8.92	2.66	0.18
** ** 144 **										
PD156S-4	B	From Leg	4.00		0.0000	144.00	No Ice	0.44	0.44	0.01
			0.00				1/2" Ice	0.79	0.79	0.01
			6.00				1" Ice	1.14	1.14	0.01
							2" Ice	1.85	1.85	0.01
							4" Ice	3.26	3.26	0.02
SRL480N1DT4	C	From Leg	4.00		0.0000	144.00	No Ice	3.81	3.81	0.03
			0.00				1/2" Ice	5.37	5.37	0.06
			11.00				1" Ice	6.94	6.94	0.10
							2" Ice	10.13	10.13	0.20
							4" Ice	14.61	14.61	0.54
(2) PD1109-1	B	From Leg	4.00		0.0000	144.00	No Ice	2.83	2.83	0.02
			0.00				1/2" Ice	3.89	3.89	0.04
			8.00				1" Ice	4.97	4.97	0.07
							2" Ice	6.37	6.37	0.14

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA}		Weight
			Horz	Lateral			Front	Side	
			ft	ft	°	ft	ft ²	ft ²	K
(4) 6' x 2" Mount Pipe	A	From Leg	4.00	0.0000	144.00	4" Ice	9.00	9.00	0.38
			0.00	No Ice		1.43	1.43	0.02	
			0.00	1/2" Ice		1.50	1.50	0.03	
			0.00	1" Ice		1.57	1.57	0.04	
			0.00	2" Ice		1.71	1.71	0.05	
(4) 6' x 2" Mount Pipe	B	From Leg	4.00	0.0000	144.00	4" Ice	1.99	1.99	0.07
			0.00	No Ice		1.43	1.43	0.02	
			0.00	1/2" Ice		1.50	1.50	0.03	
			0.00	1" Ice		1.57	1.57	0.04	
			0.00	2" Ice		1.71	1.71	0.05	
(4) 6' x 2" Mount Pipe	C	From Leg	4.00	0.0000	144.00	4" Ice	1.99	1.99	0.07
			0.00	No Ice		1.43	1.43	0.02	
			0.00	1/2" Ice		1.50	1.50	0.03	
			0.00	1" Ice		1.57	1.57	0.04	
			0.00	2" Ice		1.71	1.71	0.05	
Sector Mount [SM 602-3]	C	None		0.0000	144.00	4" Ice	1.99	1.99	0.07
				No Ice		33.11	33.11	1.54	
				1/2" Ice		44.90	44.90	2.16	
				1" Ice		56.69	56.69	2.78	
				2" Ice		80.27	80.27	4.01	
					4" Ice	127.43	127.43	6.49	
** ** 53 ** KS24019-L112A	A	From Leg	1.00	0.0000	53.00	No Ice	0.16	0.16	0.01
			0.00			1/2" Ice	0.22	0.22	0.01
			2.00			1" Ice	0.30	0.30	0.01
						2" Ice	0.48	0.48	0.02
						4" Ice	0.95	0.95	0.06
Side Arm Mount [SO 202-1]	A	From Leg	2.00	0.0000	53.00	No Ice	2.96	2.53	0.11
			0.00	1/2" Ice		4.10	3.51	0.13	
			0.00	1" Ice		5.24	4.49	0.16	
				2" Ice		7.52	6.45	0.20	
				4" Ice		12.08	10.37	0.30	
**									

Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets:		Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weight	
				Horz	Lateral							
				ft	ft	°	°	ft	ft	ft ²	K	
** 163 ** PR-850	A	Grid	From Leg	1.00	0.0000	60.0000		163.00	5.67	No Ice	25.22	0.04
				0.00						1/2" Ice	25.97	0.17
				0.00						1" Ice	26.71	0.30
										2" Ice	28.21	0.57
										4" Ice	31.20	1.10

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Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 90 deg - No Ice
4	Dead+Wind 180 deg - No Ice
5	Dead+Ice+Temp
6	Dead+Wind 0 deg+Ice+Temp
7	Dead+Wind 90 deg+Ice+Temp
8	Dead+Wind 180 deg+Ice+Temp
9	Dead+Wind 0 deg - Service
10	Dead+Wind 90 deg - Service
11	Dead+Wind 180 deg - Service

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	200 - 180	6.549	9	0.3126	0.0569
T2	180 - 160	5.241	9	0.3004	0.0443
T3	160 - 140	4.027	9	0.2611	0.0325
T4	140 - 120	2.989	9	0.2140	0.0240
T5	120 - 100	2.134	9	0.1770	0.0184
T6	100 - 80	1.430	9	0.1400	0.0132
T7	80 - 60	0.886	9	0.1007	0.0090
T8	60 - 40	0.500	9	0.0710	0.0066
T9	40 - 20	0.236	9	0.0405	0.0042
T10	20 - 0	0.075	9	0.0204	0.0020

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
200.00	Beacon	9	6.549	0.3126	0.0569	201465
198.00	DB225-A	9	6.417	0.3120	0.0557	201465
182.00	RR90-17-02DP w/ Mount Pipe	9	5.370	0.3027	0.0455	55966
170.00	(2) DB980H90E-M w/ Mount Pipe	9	4.616	0.2836	0.0381	33642
163.00	PR-850	9	4.199	0.2682	0.0341	26908
144.00	PD156S-4	9	3.181	0.2228	0.0254	24901
100.00	Sidemarker	9	1.430	0.1400	0.0132	29546
53.00	KS24019-L112A	9	0.395	0.0600	0.0058	43231

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	200 - 180	16.660	2	0.7952	0.1458

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T2	180 - 160	13.334	2	0.7639	0.1133
T3	160 - 140	10.245	2	0.6637	0.0832
T4	140 - 120	7.606	2	0.5440	0.0613
T5	120 - 100	5.433	2	0.4500	0.0472
T6	100 - 80	3.641	2	0.3559	0.0339
T7	80 - 60	2.256	2	0.2561	0.0230
T8	60 - 40	1.273	2	0.1804	0.0168
T9	40 - 20	0.603	2	0.1030	0.0109
T10	20 - 0	0.193	2	0.0519	0.0051

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
200.00	Beacon	2	16.660	0.7952	0.1458	81818
198.00	DB225-A	2	16.323	0.7937	0.1425	81818
182.00	RR90-17-02DP w/ Mount Pipe	2	13.660	0.7698	0.1165	22716
170.00	(2) DB980H90E-M w/ Mount Pipe	2	11.744	0.7209	0.0976	13230
163.00	PR-850	2	10.684	0.6817	0.0873	10602
144.00	PD156S-4	2	8.095	0.5663	0.0649	9795
100.00	Sidemarker	2	3.641	0.3559	0.0339	11624
53.00	KS24019-L112A	2	1.007	0.1525	0.0148	17023

Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load/Allowable	Allowable Ratio	Criteria	
T1	200	Leg	A325N	0.7500	4	2.76	19.44	0.142	✓	1.333	Bolt Tension
		Diagonal	A325X	0.6250	1	2.60	5.71	0.456	✓	1.333	Member Block Shear
		Top Girt	A325X	0.6250	1	0.06	8.16	0.007	✓	1.333	Member Bearing
T2	180	Leg	A325N	1.0000	4	8.35	34.56	0.242	✓	1.333	Bolt Tension
		Diagonal	A325X	0.6250	1	3.18	5.71	0.557	✓	1.333	Member Block Shear
T3	160	Leg	A325N	1.0000	4	14.01	34.56	0.406	✓	1.333	Bolt Tension
		Diagonal	A325X	0.6250	1	3.61	5.71	0.633	✓	1.333	Member Block Shear
T4	140	Leg	A325N	1.2500	4	19.57	54.00	0.362	✓	1.333	Bolt Tension
		Diagonal	A325X	0.6250	1	4.26	7.75	0.550	✓	1.333	Member Block Shear
T5	120	Leg	A325N	1.2500	4	24.91	54.00	0.461	✓	1.333	Bolt Tension
		Diagonal	A325X	0.6250	1	4.63	7.75	0.598	✓	1.333	Member Block Shear
T6	100	Leg	A325N	1.2500	6	20.02	54.00	0.371	✓	1.333	Bolt Tension
		Diagonal	A325X	0.7500	1	5.14	7.88	0.652	✓	1.333	Member Block Shear

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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T7	80	Leg	A325N	1.2500	6	23.03	54.00	0.427	✓	1.333 Bolt Tension
		Diagonal	A325X	0.7500	1	6.09	12.69	0.480	✓	1.333 Member Bearing
T8	60	Leg	A325N	1.3750	6	26.20	65.34	0.401	✓	1.333 Bolt Tension
		Diagonal	A325X	0.7500	1	6.88	13.05	0.527	✓	1.333 Member Bearing
T9	40	Leg	A325N	1.3750	6	29.34	65.34	0.449	✓	1.333 Bolt Tension
		Diagonal	A325X	0.7500	1	7.32	13.05	0.561	✓	1.333 Member Bearing
T10	20	Leg	A572-50	1.5000	8	24.91	37.91	0.657	✓	1.333 Bolt Tension
		Diagonal	A325X	0.7500	1	8.27	13.05	0.634	✓	1.333 Member Bearing

Compression Checks

Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	200 - 180	Pipe 2.875" x 0.375" (2.5 #160)	20.00	5.00	67.1 K=1.00	21.46	2.9452	-14.50	63.21	0.229 ✓
T2	180 - 160	Pipe 3.5" x 0.300" (3 XS)	20.03	5.01	52.9 K=1.00	23.89	3.0159	-41.41	72.06	0.575 ✓
T3	160 - 140	Pipe 4" x 0.318" (3.5 XS)	20.03	5.01	46.0 K=1.00	24.96	3.6784	-68.21	91.82	0.743 ✓
T4	140 - 120	Pipe 4.5" x 0.438" (4 #120)	20.03	6.68	55.5 K=1.00	23.47	5.5894	-94.22	131.21	0.718 ✓
T5	120 - 100	Pipe 5.563" x 0.375" (5 XS)	20.03	6.68	43.6 K=1.00	25.32	6.1120	-119.69	154.76	0.773 ✓
T6	100 - 80	Pipe 5.563" x 0.375" (5 XS)	20.03	6.68	43.6 K=1.00	25.32	6.1120	-144.59	154.76	0.934 ✓
T7	80 - 60	Pipe 6.625" x 0.432" (6 EH)	20.03	10.02	54.8 K=1.00	23.59	8.4049	-167.20	198.28	0.843 ✓
T8	60 - 40	Pipe 8.625" x 0.322" (8 STD)	20.03	10.02	40.9 K=1.00	25.70	8.3993	-191.59	215.89	0.887 ✓
T9	40 - 20	Pipe 8.625" x 0.500" (8 XS)	20.03	10.02	41.8 K=1.00	25.58	12.7627	-216.08	326.50	0.662 ✓
T10	20 - 0	Pipe 8.625" x 0.500" (8 XS)	20.03	9.97	41.6 K=1.00	25.61	12.7627	-247.25	326.82	0.757 ✓

Diagonal Design Data (Compression)

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Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	200 - 180	L1 3/4x1 3/4x3/16	7.07	3.20	113.8 K=1.02	11.15	0.6211	-2.77	6.92	0.401
T2	180 - 160	L1 3/4x1 3/4x3/16	8.40	4.01	140.0 K=1.00	7.61	0.6211	-3.13	4.73	0.661
T3	160 - 140	L1 3/4x1 3/4x3/16	10.08	4.82	168.6 K=1.00	5.25	0.6211	-3.83	3.26	1.174
T4	140 - 120	L2 1/2x2 1/2x3/16	12.58	6.10	147.9 K=1.00	6.83	0.9020	-4.45	6.16	0.722
T5	120 - 100	L2 1/2x2 1/2x3/16	14.32	6.92	167.7 K=1.00	5.31	0.9020	-4.79	4.79	1.000
T6	100 - 80	L3x3x3/16	16.11	7.82	157.4 K=1.00	6.03	1.0900	-5.15	6.57	0.784
T7	80 - 60	L3 1/2x3 1/2x1/4	19.30	9.43	163.1 K=1.00	5.62	1.6900	-6.22	9.49	0.655
T8	60 - 40	L3 1/2x3 1/2x1/4	21.03	10.20	176.4 K=1.00	4.80	1.6900	-6.88	8.11	0.848
T9	40 - 20	L3 1/2x3 1/2x1/4	22.81	11.10	191.9 K=1.00	4.06	1.6900	-7.32	6.86	1.068
T10	20 - 0	L4x4x1/4	24.60	11.99	181.0 K=1.00	4.56	1.9400	-8.27	8.84	0.936

Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	200 - 180	L1 3/4x1 3/4x3/16	5.00	4.43	154.7 K=1.00	6.24	0.6211	-0.06	3.88	0.015

Tension Checks

Leg Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	200 - 180	Pipe 2.875" x 0.375" (2.5 #160)	20.00	5.00	67.1	30.00	2.9452	11.04	88.36	0.125
T2	180 - 160	Pipe 3.5" x 0.300" (3 XS)	20.03	5.01	52.9	30.00	3.0159	33.40	90.48	0.369
T3	160 - 140	Pipe 4" x 0.318" (3.5 XS)	20.03	5.01	46.0	30.00	3.6784	56.06	110.35	0.508
T4	140 - 120	Pipe 4.5" x 0.438" (4 #120)	20.03	6.68	55.5	30.00	5.5894	78.26	167.68	0.467
T5	120 - 100	Pipe 5.563" x 0.375" (5 XS)	20.03	6.68	43.6	30.00	6.1120	99.65	183.36	0.543

tnxTower Jacobs Engineering Group, Inc. 5449 Bells Ferry Road Acworth, GA 301102 Phone: (770) 701-2500 FAX: (770) 701-2501	Job	John Tom Hill BU 871584	Page	18 of 19
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	Client	Crown Castle	Designed by	holderkg

Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T6	100 - 80	Pipe 5.563" x 0.375" (5 XS)	20.03	6.68	43.6	30.00	6.1120	120.11	183.36	0.655
T7	80 - 60	Pipe 6.625" x 0.432" (6 EH)	20.03	10.02	54.8	30.00	8.4049	138.20	252.15	0.548
T8	60 - 40	Pipe 8.625" x 0.322" (8 STD)	20.03	10.02	40.9	30.00	8.3993	157.20	251.98	0.624
T9	40 - 20	Pipe 8.625" x 0.500" (8 XS)	20.03	10.02	41.8	30.00	12.7627	176.01	382.88	0.460
T10	20 - 0	Pipe 8.625" x 0.500" (8 XS)	20.03	9.97	41.6	30.00	12.7627	199.29	382.88	0.521

Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	200 - 180	L1 3/4x1 3/4x3/16	7.07	3.20	75.2	29.00	0.3604	2.60	10.45	0.249
T2	180 - 160	L1 3/4x1 3/4x3/16	8.40	4.01	93.3	29.00	0.3604	3.18	10.45	0.304
T3	160 - 140	L1 3/4x1 3/4x3/16	10.08	4.82	111.6	29.00	0.3604	3.61	10.45	0.346
T4	140 - 120	L2 1/2x2 1/2x3/16	12.58	6.10	96.6	29.00	0.5710	4.26	16.56	0.258
T5	120 - 100	L2 1/2x2 1/2x3/16	14.32	6.92	109.3	29.00	0.5710	4.63	16.56	0.280
T6	100 - 80	L3x3x3/16	16.11	7.82	102.0	29.00	0.6945	5.14	20.14	0.255
T7	80 - 60	L3 1/2x3 1/2x1/4	19.30	9.43	105.9	29.00	1.1034	6.09	32.00	0.190
T8	60 - 40	L3 1/2x3 1/2x1/4	21.03	10.20	114.4	29.00	1.1034	6.45	32.00	0.201
T9	40 - 20	L3 1/2x3 1/2x1/4	21.92	10.65	119.3	29.00	1.1034	6.85	32.00	0.214
T10	20 - 0	L4x4x1/4	24.60	11.99	116.9	29.00	1.2909	7.36	37.44	0.197

Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	200 - 180	L1 3/4x1 3/4x3/16	5.00	4.43	106.4	29.00	0.3604	0.03	10.45	0.003

tnxTower Jacobs Engineering Group, Inc. 5449 Bells Ferry Road Acworth, GA 301102 Phone: (770) 701-2500 FAX: (770) 701-2501	Job	John Tom Hill BU 871584	Page	19 of 19
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	Client	Crown Castle	Designed by	holderkg

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
T1	200 - 180	Leg	Pipe 2.875" x 0.375" (2.5 #160)	3	-14.50	84.26	17.2	Pass
T2	180 - 160	Leg	Pipe 3.5" x 0.300" (3 XS)	33	-41.41	96.06	43.1	Pass
T3	160 - 140	Leg	Pipe 4" x 0.318" (3.5 XS)	60	-68.21	122.40	55.7	Pass
T4	140 - 120	Leg	Pipe 4.5" x 0.438" (4 #120)	87	-94.22	174.90	53.9	Pass
T5	120 - 100	Leg	Pipe 5.563" x 0.375" (5 XS)	108	-119.69	206.29	58.0	Pass
T6	100 - 80	Leg	Pipe 5.563" x 0.375" (5 XS)	129	-144.59	206.29	70.1	Pass
T7	80 - 60	Leg	Pipe 6.625" x 0.432" (6 EH)	150	-167.20	264.31	63.3	Pass
T8	60 - 40	Leg	Pipe 8.625" x 0.322" (8 STD)	165	-191.59	287.78	66.6	Pass
T9	40 - 20	Leg	Pipe 8.625" x 0.500" (8 XS)	180	-216.08	435.22	49.6	Pass
T10	20 - 0	Leg	Pipe 8.625" x 0.500" (8 XS)	195	-247.25	435.64	56.8	Pass
T1	200 - 180	Diagonal	L1 3/4x1 3/4x3/16	10	-2.77	9.23	30.1	Pass
							34.2 (b)	
T2	180 - 160	Diagonal	L1 3/4x1 3/4x3/16	37	-3.13	6.30	49.6	Pass
T3	160 - 140	Diagonal	L1 3/4x1 3/4x3/16	64	-3.83	4.35	88.0	Pass
T4	140 - 120	Diagonal	L2 1/2x2 1/2x3/16	91	-4.45	8.21	54.2	Pass
T5	120 - 100	Diagonal	L2 1/2x2 1/2x3/16	112	-4.79	6.38	75.0	Pass
T6	100 - 80	Diagonal	L3x3x3/16	133	-5.15	8.76	58.8	Pass
T7	80 - 60	Diagonal	L3 1/2x3 1/2x1/4	154	-6.22	12.65	49.1	Pass
T8	60 - 40	Diagonal	L3 1/2x3 1/2x1/4	169	-6.88	10.81	63.6	Pass
T9	40 - 20	Diagonal	L3 1/2x3 1/2x1/4	184	-7.32	9.14	80.1	Pass
T10	20 - 0	Diagonal	L4x4x1/4	199	-8.27	11.79	70.2	Pass
T1	200 - 180	Top Girt	L1 3/4x1 3/4x3/16	4	-0.06	5.17	1.1	Pass
							Summary	
							Leg (T6)	70.1 Pass
							Diagonal (T3)	88.0 Pass
							Top Girt (T1)	1.1 Pass
							Bolt Checks	49.3 Pass
							RATING =	88.0 Pass

APPENDIX B
BASE LEVEL DRAWING

Date: **February 11, 2015**

Holly Haas
Crown Castle
3530 Toringdon Way Suite 300
Charlotte, NC 28277

JACOBS[®]

Jacobs Engineering Group, Inc.

5449 Bells Ferry Road
Acworth, GA 301102
(770) 701-2500

Subject: **Structural Analysis Report**

Carrier Designation:

T-Mobile Co-Locate

Carrier Site Number:

CT11189E

Carrier Site Name:

Glastonbury/ Rt 94 & Fern

Crown Castle Designation:

Crown Castle BU Number:

871584

Crown Castle Site Name:

John Tom Hill

Crown Castle JDE Job Number:

322291

Crown Castle Work Order Number:

1005912

Crown Castle Application Number:

282547 Rev. 0

Engineering Firm Designation:

Jacobs Engineering Group, Inc. Project Number: 1005912

Site Data:

115 Birch Mtn. Road, GLASTONBURY, Hartford County, CT

Latitude 41° 42' 32.24", Longitude -72° 28' 24.41"

200 Foot - Self Support Tower

Dear Holly Haas,

Jacobs Engineering Group, Inc. is pleased to submit this "**Structural Analysis Report**" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 754645, in accordance with application 282547, revision 0.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Existing + Proposed Equipment

Sufficient Capacity

Note: See Table I and Table II for the proposed and existing loading, respectively.

The analysis has been performed in accordance with the TIA/EIA-222-F standard and The 2005 CT State Building Code based upon a wind speed of 80 mph fastest mile.

All modifications and equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

We at Jacobs Engineering Group, Inc. appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted by:

Reviewed By:



Kristi Holder, EI
Structural Engineer

Matthew Watkins, P.E.
Engineering Project Manager

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1) INTRODUCTION

This tower is a 200 ft Self Support tower designed by SABRE COMMUNICATIONS in November of 1998. The tower was originally designed for a wind speed of 85 mph per TIA/EIA-222-F.

2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 80 mph with no ice, 37.6 mph with 1.00 inch ice thickness and 50 mph under service loads.

Table 1 - Proposed Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
182.0	183.0	3	commscope	ATBT-BOTTOM-24V	6	1-5/8	-
		3	commscope	LNx-6515DS-VTM w/ Mount Pipe			

Table 2 - Existing Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note	
198.0	208.0	1	rfs celwave	ALR10-O	4	7/8	1	
	205.0	1	decibel	DB225-A	1	1/2		
		1	rfs celwave	PD1107-1	-	-	2	
	1	rfs celwave	PD201-7					
	204.0	1	scala	OGB6-928N	-	-	1	
	198.0	1	crown mounts	Sector Mount [SM 602-3]				
182.0	183.0	3	ems wireless	RR90-17-02DP w/ Mount Pipe	6	1-5/8	1	
	182.0	3	ericsson	KRY 112 71				
		1	crown mounts	Sector Mount [SM 602-3]				
170.0	171.0	6	decibel	DB980H90E-M w/ Mount Pipe	9	1-5/8	1	
	170.0	1	crown mounts	Sector Mount [SM 506-3]	1	1/2		
163.0	163.0	1	kathrein	PR-850	1	1/2	1	
		1	crown mounts	Pipe Mount [PM 601-1]				
144.0	155.0	1	sinclair	SRL480N1DT4	3	1/2	2	
	152.0	2	rfs celwave	PD1109-1				
	150.0	1	rfs celwave	PD156S-4			1	7/8
	144.0	1	crown mounts	Sector Mount [SM 602-3]				
53.0	55.0	1	lucent	KS24019-L112A	1	1/2	1	
	53.0	1	crown mounts	Side Arm Mount [SO 202-1]				

Notes:

- 1) Existing Equipment
- 2) Abandoned Equipment; Considered In This Analysis

Table 3 - Design Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
180.0	180.0	9	Dapa	DB809	9	7/8
160.0	160.0	9	Dapa	DB809	9	7/8
140.0	140.0	12	Dapa	2980	12	1-5/8
120.0	120.0	12	Dapa	2980	12	1-5/8
100.0	100.0	12	Dapa	2980	12	1-5/8
80.0	80.0	1	Generic	6' Dish	1	EW64
70.0	70.0	1	Generic	1 Meter Dish	1	7/8
60.0	60.0	1	Generic	1 Meter Dish	1	7/8
50.0	50.0	1	Generic	1 Meter Dish	1	7/8

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Clarence Welti Assoc., Inc.	1404208	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Sabre Communications	1333892	CCISITES
4-TOWER MANUFACTURER DRAWINGS	Sabre Communications	1403674	CCISITES

3.1) Analysis Method

tnxTower (version 6.1.4.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by TIA/EIA-222-F.

This analysis may be affected if any assumptions are not valid or have been made in error. Jacobs Engineering Group, Inc. should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	200 - 180	Leg	Pipe 2.875" x 0.375" (2.5 #160)	3	-14.50	84.26	17.2	Pass
T2	180 - 160	Leg	Pipe 3.5" x 0.300" (3 XS)	33	-41.41	96.06	43.1	Pass
T3	160 - 140	Leg	Pipe 4" x 0.318" (3.5 XS)	60	-68.21	122.40	55.7	Pass
T4	140 - 120	Leg	Pipe 4.5" x 0.438" (4 #120)	87	-94.22	174.90	53.9	Pass
T5	120 - 100	Leg	Pipe 5.563" x 0.375" (5 XS)	108	-119.69	206.29	58.0	Pass
T6	100 - 80	Leg	Pipe 5.563" x 0.375" (5 XS)	129	-144.59	206.29	70.1	Pass
T7	80 - 60	Leg	Pipe 6.625" x 0.432" (6 EH)	150	-167.20	264.31	63.3	Pass
T8	60 - 40	Leg	Pipe 8.625" x 0.322" (8 STD)	165	-191.59	287.78	66.6	Pass
T9	40 - 20	Leg	Pipe 8.625" x 0.500" (8 XS)	180	-216.08	435.22	49.6	Pass
T10	20 - 0	Leg	Pipe 8.625" x 0.500" (8 XS)	195	-247.25	435.64	56.8	Pass
T1	200 - 180	Diagonal	L1 3/4x1 3/4x3/16	10	-2.77	9.23	30.1 34.2 (b)	Pass
T2	180 - 160	Diagonal	L1 3/4x1 3/4x3/16	37	-3.13	6.30	49.6	Pass
T3	160 - 140	Diagonal	L1 3/4x1 3/4x3/16	64	-3.83	4.35	88.0	Pass
T4	140 - 120	Diagonal	L2 1/2x2 1/2x3/16	91	-4.45	8.21	54.2	Pass
T5	120 - 100	Diagonal	L2 1/2x2 1/2x3/16	112	-4.79	6.38	75.0	Pass
T6	100 - 80	Diagonal	L3x3x3/16	133	-5.15	8.76	58.8	Pass
T7	80 - 60	Diagonal	L3 1/2x3 1/2x1/4	154	-6.22	12.65	49.1	Pass
T8	60 - 40	Diagonal	L3 1/2x3 1/2x1/4	169	-6.88	10.81	63.6	Pass
T9	40 - 20	Diagonal	L3 1/2x3 1/2x1/4	184	-7.32	9.14	80.1	Pass
T10	20 - 0	Diagonal	L4x4x1/4	199	-8.27	11.79	70.2	Pass
T1	200 - 180	Top Girt	L1 3/4x1 3/4x3/16	4	-0.06	5.17	1.1	Pass
							Summary	
							Leg (T6)	70.1 Pass
							Diagonal (T3)	88.0 Pass
							Top Girt (T1)	1.1 Pass
							Bolt Checks	49.3 Pass
							RATING =	88.0 Pass

Table 6 - Tower Component Stresses vs. Capacity – LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	48.9	Pass
1	Base Foundation Structural	0	73.9	Pass
1	Base Foundation Soil Interaction	0	68.2	Pass

Structure Rating (max from all components) =	88.0%
---	--------------

Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the existing and proposed loads. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
Beacon	200	(2) 6' x 2" Mount Pipe	182
DB225-A	198	Sector Mount [SM 602-3]	182
ALR10-O	198	(2) DB980H90E-M w/ Mount Pipe	170
OGB6-928N	198	(2) DB980H90E-M w/ Mount Pipe	170
PD1107-1	198	(2) DB980H90E-M w/ Mount Pipe	170
PD201-7	198	6' x 2" Mount Pipe	170
(4) 6' x 2" Mount Pipe	198	6' x 2" Mount Pipe	170
(4) 6' x 2" Mount Pipe	198	6' x 2" Mount Pipe	170
(4) 6' x 2" Mount Pipe	198	Sector Mount [SM 506-3]	170
Sector Mount [SM 602-3]	198	Pipe Mount [PM 601-1]	163
RR90-17-02DP w/ Mount Pipe	182	PR-850	163
RR90-17-02DP w/ Mount Pipe	182	PD156S-4	144
RR90-17-02DP w/ Mount Pipe	182	SRL480N1DT4	144
KRY 112 71	182	(2) PD1109-1	144
KRY 112 71	182	(4) 6' x 2" Mount Pipe	144
KRY 112 71	182	(4) 6' x 2" Mount Pipe	144
LNx-6515DS-VTM w/ Mount Pipe	182	(4) 6' x 2" Mount Pipe	144
LNx-6515DS-VTM w/ Mount Pipe	182	Sector Mount [SM 602-3]	144
LNx-6515DS-VTM w/ Mount Pipe	182	Sidemarker	100
ATBT-BOTTOM-24V	182	Sidemarker	100
ATBT-BOTTOM-24V	182	Sidemarker	100
ATBT-BOTTOM-24V	182	Side Arm Mount [SO 202-1]	53
(2) 6' x 2" Mount Pipe	182	KS24019-L112A	53
(2) 6' x 2" Mount Pipe	182		

SYMBOL LIST

MARK	SIZE	MARK	SIZE
A	Pipe 2.875" x 0.375" (2.5 #160)	D	Pipe 4.5" x 0.438" (4 #120)
B	Pipe 3.5" x 0.300" (3 XS)	E	Pipe 6.625" x 0.432" (6 EH)
C	Pipe 4" x 0.318" (3.5 XS)	F	Pipe 8.625" x 0.322" (8 STD)

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

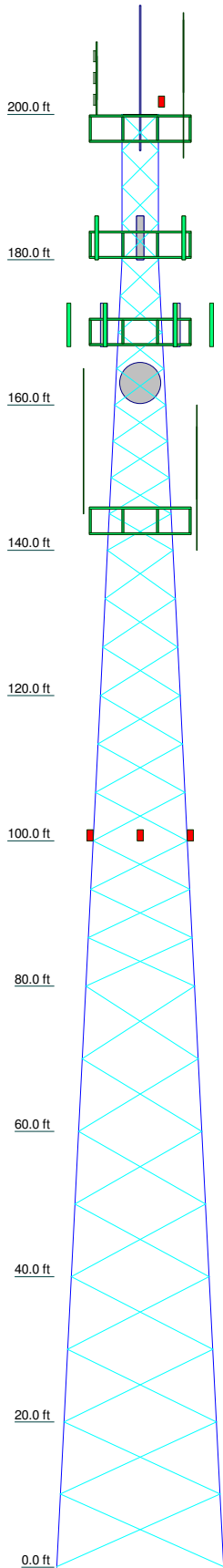
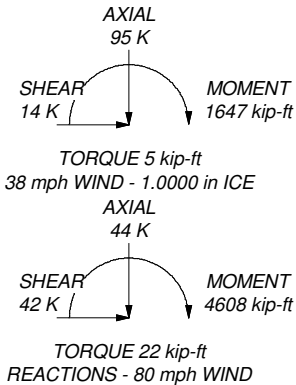
TOWER DESIGN NOTES

1. Tower is located in Hartford County, Connecticut.
2. Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 38 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 88%

MAX. CORNER REACTIONS AT BASE:

DOWN: 246 K
SHEAR: 26 K

UPLIFT: -198 K
SHEAR: 22 K



Section	T1	T2	T3	T4	T5	T6	T7	T8	T9	T10
Legs	A	B	C	D	Pipe 5.563" x 0.375" (5 XS)	Pipe 5.563" x 0.375" (5 XS)	Pipe 5.563" x 0.375" (5 XS)	Pipe 5.563" x 0.375" (5 XS)	Pipe 5.563" x 0.375" (5 XS)	Pipe 8.625" x 0.500" (8 XS)
Leg Grade	A572-50									
Diagonals	L1 3/4x1 3/4x3/16									
Diagonal Grade	A36									
Top Girts	N.A.									
Face Width (ft)	5	7	9	11	13	15	17	19	21	23
# Panels @ (ft)	12 @ 5	9 @ 6.66667	6 @ 10	4 @ 10	2 @ 9.95633					
Weight (K)	1.0	1.2	1.8	2.0	2.3	3.0	3.1	4.2	4.5	24.2

<p>JACOBS</p> <p>5449 Bells Ferry Road Acworth, GA 301102 Phone: (770) 701-2500 FAX: (770) 701-2501</p>	<p>Job: John Tom Hill BU 871584</p> <p>Project: 1005912</p>	
	<p>Client: Crown Castle</p> <p>Code: TIA/EIA-222-F</p> <p>Path:</p>	<p>Drawn by: holderkg</p> <p>Date: 02/11/15</p> <p>Dwg No. E-1</p>
	<p>App'd:</p> <p>Scale: NTS</p>	
	<p><small>C:\Users\holderkg\Desktop\Project Storage\871584 John Tom Hill 1005912 Analysis\CG Model\John Tom Hill BU 871584.dwg</small></p>	

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	Client Crown Castle	Designed by holderkg

Tower Input Data

The main tower is a 3x free standing tower with an overall height of 200.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 5.00 ft at the top and 23.00 ft at the base.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in Hartford County, Connecticut.

Basic wind speed of 80 mph.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 38 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

Pressures are calculated at each section.

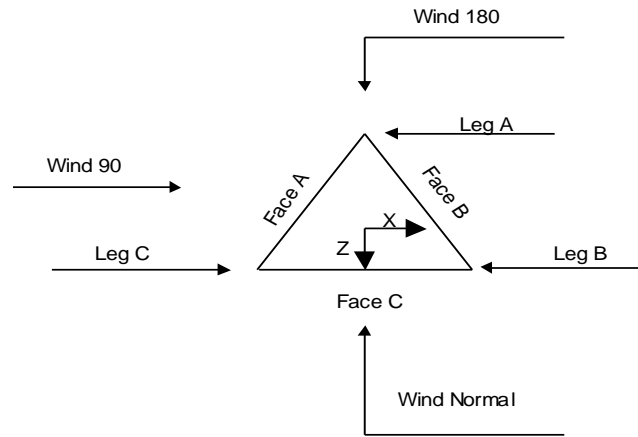
Stress ratio used in tower member design is 1.333.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

<ul style="list-style-type: none"> Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification √ Use Code Stress Ratios √ Use Code Safety Factors - Guys √ Escalate Ice Always Use Max Kz Use Special Wind Profile √ Include Bolts In Member Capacity Leg Bolts Are At Top Of Section √ Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) Add IBC .6D+W Combination 	<ul style="list-style-type: none"> Distribute Leg Loads As Uniform Assume Legs Pinned √ Assume Rigid Index Plate √ Use Clear Spans For Wind Area √ Use Clear Spans For KL/r Retension Guys To Initial Tension √ Bypass Mast Stability Checks √ Use Azimuth Dish Coefficients √ Project Wind Area of Appurt. Autocalc Torque Arm Areas SR Members Have Cut Ends √ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Use TIA-222-G Tension Splice Capacity Exemption 	<ul style="list-style-type: none"> Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules √ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA √ SR Leg Bolts Resist Compression √ All Leg Panels Have Same Allowable √ Offset Girt At Foundation √ Consider Feedline Torque √ Include Angle Block Shear Check <div style="text-align: center; background-color: #e0e0e0; padding: 2px;">Poles</div> <ul style="list-style-type: none"> Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets
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Triangular Tower

Tower Section Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	<i>ft</i>			<i>ft</i>		<i>ft</i>
T1	200.00-180.00			5.00	1	20.00
T2	180.00-160.00			5.00	1	20.00
T3	160.00-140.00			7.00	1	20.00
T4	140.00-120.00			9.00	1	20.00
T5	120.00-100.00			11.00	1	20.00
T6	100.00-80.00			13.00	1	20.00
T7	80.00-60.00			15.00	1	20.00
T8	60.00-40.00			17.00	1	20.00
T9	40.00-20.00			19.00	1	20.00
T10	20.00-0.00			21.00	1	20.00

Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	<i>ft</i>	<i>ft</i>				<i>in</i>	<i>in</i>
T1	200.00-180.00	5.00	X Brace	No	No	0.0000	0.0000
T2	180.00-160.00	5.00	X Brace	No	No	0.0000	0.0000
T3	160.00-140.00	5.00	X Brace	No	No	0.0000	0.0000
T4	140.00-120.00	6.67	X Brace	No	No	0.0000	0.0000
T5	120.00-100.00	6.67	X Brace	No	No	0.0000	0.0000

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Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T6	100.00-80.00	6.67	X Brace	No	No	0.0000	0.0000
T7	80.00-60.00	10.00	X Brace	No	No	0.0000	0.0000
T8	60.00-40.00	10.00	X Brace	No	No	0.0000	0.0000
T9	40.00-20.00	10.00	X Brace	No	No	0.0000	0.0000
T10	20.00-0.00	9.96	X Brace	No	No	0.0000	1.0000

Tower Section Geometry (cont'd)

Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
ft						
T1 200.00-180.00	Pipe	Pipe 2.875" x 0.375" (2.5 #160)	A572-50 (50 ksi)	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T2 180.00-160.00	Pipe	Pipe 3.5" x 0.300" (3 XS)	A572-50 (50 ksi)	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T3 160.00-140.00	Pipe	Pipe 4" x 0.318" (3.5 XS)	A572-50 (50 ksi)	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T4 140.00-120.00	Pipe	Pipe 4.5" x 0.438" (4 #120)	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T5 120.00-100.00	Pipe	Pipe 5.563" x 0.375" (5 XS)	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T6 100.00-80.00	Pipe	Pipe 5.563" x 0.375" (5 XS)	A572-50 (50 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)
T7 80.00-60.00	Pipe	Pipe 6.625" x 0.432" (6 EH)	A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A36 (36 ksi)
T8 60.00-40.00	Pipe	Pipe 8.625" x 0.322" (8 STD)	A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A36 (36 ksi)
T9 40.00-20.00	Pipe	Pipe 8.625" x 0.500" (8 XS)	A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A36 (36 ksi)
T10 20.00-0.00	Pipe	Pipe 8.625" x 0.500" (8 XS)	A572-50 (50 ksi)	Equal Angle	L4x4x1/4	A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
ft						
T1 200.00-180.00	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft ²	in					in	in

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Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A_f	Adjust. Factor A_r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in
ft	ft ²	in						
T1 200.00-180.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T2 180.00-160.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T3 160.00-140.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T4 140.00-120.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T5 120.00-100.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T6 100.00-80.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T7 80.00-60.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T8 60.00-40.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T9 40.00-20.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T10 20.00-0.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000

Tower Section Geometry (cont'd)

Tower Elevation	Calc K Single Angles	Calc K Solid Rounds	Legs	K Factors ¹							
				X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace	
											X Y
ft											
T1 200.00-180.00	Yes	No	1	1	1	1	1	1	1	1	1
T2 180.00-160.00	Yes	No	1	1	1	1	1	1	1	1	1
T3 160.00-140.00	Yes	No	1	1	1	1	1	1	1	1	1
T4 140.00-120.00	Yes	No	1	1	1	1	1	1	1	1	1
T5 120.00-100.00	Yes	No	1	1	1	1	1	1	1	1	1
T6 100.00-80.00	Yes	No	1	1	1	1	1	1	1	1	1
T7 80.00-60.00	Yes	No	1	1	1	1	1	1	1	1	1
T8 60.00-40.00	Yes	No	1	1	1	1	1	1	1	1	1
T9 40.00-20.00	Yes	No	1	1	1	1	1	1	1	1	1
T10 20.00-0.00	Yes	No	1	1	1	1	1	1	1	1	1

¹Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

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Tower Section Geometry (cont'd)

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 200.00-180.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T2 180.00-160.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T3 160.00-140.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T4 140.00-120.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T5 120.00-100.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 100.00-80.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 80.00-60.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 60.00-40.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T9 40.00-20.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T10 20.00-0.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 200.00-180.00	Flange	0.7500	4	0.6250	1	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325X		A325N		A325N		A325N		A325N	
T2 180.00-160.00	Flange	1.0000	4	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T3 160.00-140.00	Flange	1.0000	4	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T4 140.00-120.00	Flange	1.2500	4	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T5 120.00-100.00	Flange	1.2500	4	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T6 100.00-80.00	Flange	1.2500	6	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T7 80.00-60.00	Flange	1.2500	6	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T8 60.00-40.00	Flange	1.3750	6	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T9 40.00-20.00	Flange	1.3750	6	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325X		A325N		A325N		A325N		A325N		A325N	
T10 20.00-0.00	Flange	1.5000	8	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A572-50		A325X		A325N		A325N		A325N		A325N		A325N	

Feed Line/Linear Appurtenances - Entered As Round Or Flat

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Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
** A Face **												
** B Face **												
AVA7-50(1-5/8)	B	Yes	Ar (CfAe)	182.00 - 0.00	0.0000	-0.4	12	6	0.5000	2.0100		0.70
Feedline	B	Yes	Af (CfAe)	182.00 - 0.00	0.0000	-0.4	1	1	3.0000	3.0000	12.0000	8.40
Ladder (Af)	B	Yes	Af (CfAe)	200.00 - 0.00	0.0000	0	1	1	3.0000	3.0000	12.0000	8.40
**												
FLC	B	Yes	Ar (CfAe)	163.00 - 0.00	0.0000	-0.2	1	1	0.5000	0.6400		0.17
12-50J(1/2")	B	Yes	Af (CfAe)	163.00 - 0.00	0.0000	-0.2	1	1	3.0000	3.0000	12.0000	8.40
Feedline	B	Yes	Af (CfAe)	163.00 - 0.00	0.0000	-0.2	1	1	3.0000	3.0000	12.0000	8.40
Ladder (Af)												
**												
LDF4-50A(1/2")	B	Yes	Ar (CfAe)	144.00 - 0.00	0.0000	0.4	4	4	0.5000	0.6300		0.15
LDF4-50A(1/2")	B	Yes	Ar (CfAe)	198.00 - 144.00	0.0000	0.4	1	1	0.5000	0.6300		0.15
LDF5-50A(7/8")	B	Yes	Ar (CfAe)	144.00 - 0.00	0.0000	0.35	5	3	0.5000	1.0900		0.33
LDF5-50A(7/8")	B	Yes	Ar (CfAe)	198.00 - 144.00	0.0000	0.35	4	2	0.5000	1.0900		0.33
Feedline	B	Yes	Af (CfAe)	198.00 - 0.00	0.0000	0	1	1	3.0000	3.0000	12.0000	8.40
Ladder (Af)												
**												
** C Face **												
LDF7-50A(1-5/8")	C	Yes	Ar (CfAe)	170.00 - 0.00	-1.5000	0	6	6	0.5000	1.9800		0.82
LDF4-50A(1/2")	C	Yes	Ar (CfAe)	53.00 - 0.00	-1.5000	0.08	1	1	0.5000	0.6300		0.15
Feedline	C	Yes	Af (CfAe)	170.00 - 0.00	-1.5000	0	1	1	3.0000	3.0000	12.0000	8.40
Ladder (Af)												
**												
Safety Line 3/8	C	Yes	Ar (CfAe)	200.00 - 0.00	0.0000	0	1	1	0.3750	0.3750		0.22
Climbing Ladder	C	Yes	Af (CfAe)	200.00 - 0.00	0.0000	0	1	1	2.5000	2.5000	10.0000	7.90

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	C _{AA} ft ² /ft	Weight plf
** A Face **							
**							
**							

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
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Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
T1	200.00-180.00	A	0.000	0.000	0.000	0.000	0.00
		B	6.225	10.000	0.000	0.000	0.38
		C	0.625	4.167	0.000	0.000	0.16
T2	180.00-160.00	A	0.000	0.000	0.000	0.000	0.00
		B	24.943	15.750	0.000	0.000	0.73
		C	10.525	6.667	0.000	0.000	0.30
T3	160.00-140.00	A	0.000	0.000	0.000	0.000	0.00
		B	26.843	20.000	0.000	0.000	0.88
		C	20.425	9.167	0.000	0.000	0.43
T4	140.00-120.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	20.425	9.167	0.000	0.000	0.43
T5	120.00-100.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	20.425	9.167	0.000	0.000	0.43
T6	100.00-80.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	20.425	9.167	0.000	0.000	0.43
T7	80.00-60.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	20.425	9.167	0.000	0.000	0.43
T8	60.00-40.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	21.108	9.167	0.000	0.000	0.43
T9	40.00-20.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	21.475	9.167	0.000	0.000	0.43
T10	20.00-0.00	A	0.000	0.000	0.000	0.000	0.00
		B	30.817	20.000	0.000	0.000	0.89
		C	21.475	9.167	0.000	0.000	0.43

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
T1	200.00-180.00	A	1.234	0.000	0.000	0.000	0.000	0.00
		B		10.729	19.960	0.000	0.000	0.95
		C		4.738	6.908	0.000	0.000	0.35
T2	180.00-160.00	A	1.217	0.000	0.000	0.000	0.000	0.00
		B		19.159	47.838	0.000	0.000	2.03
		C		8.362	21.058	0.000	0.000	0.76
T3	160.00-140.00	A	1.199	0.000	0.000	0.000	0.000	0.00
		B		23.273	55.887	0.000	0.000	2.36
		C		11.920	35.163	0.000	0.000	1.16
T4	140.00-120.00	A	1.179	0.000	0.000	0.000	0.000	0.00
		B		23.001	62.345	0.000	0.000	2.42
		C		11.784	35.073	0.000	0.000	1.15
T5	120.00-100.00	A	1.155	0.000	0.000	0.000	0.000	0.00
		B		22.689	62.137	0.000	0.000	2.39
		C		11.628	34.969	0.000	0.000	1.13
T6	100.00-80.00	A	1.128	0.000	0.000	0.000	0.000	0.00
		B		22.323	61.893	0.000	0.000	2.36
		C		11.445	34.846	0.000	0.000	1.11
T7	80.00-60.00	A	1.094	0.000	0.000	0.000	0.000	0.00
		B		21.876	61.595	0.000	0.000	2.31
		C		11.221	34.697	0.000	0.000	1.09
T8	60.00-40.00	A	1.051	0.000	0.000	0.000	0.00	

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Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A_R ft ²	A_F ft ²	C_{AA} In Face ft ²	C_{AA} Out Face ft ²	Weight K
T9	40.00-20.00	B		21.298	61.210	0.000	0.000	2.26
		C		13.892	34.505	0.000	0.000	1.09
		A	1.000	0.000	0.000	0.000	0.000	0.00
T10	20.00-0.00	B		20.617	60.756	0.000	0.000	2.19
		C		14.975	34.278	0.000	0.000	1.07
		A	1.000	0.000	0.000	0.000	0.000	0.00
		B		20.617	60.756	0.000	0.000	2.19
		C		14.975	34.278	0.000	0.000	1.07

Feed Line Shielding

Section	Elevation ft	Face	A_R ft ²	A_R Ice ft ²	A_F ft ²	A_F Ice ft ²
T1	200.00-180.00	A	0.000	0.000	0.000	0.000
		B	0.000	4.232	1.457	3.002
		C	0.000	1.648	0.430	1.169
T2	180.00-160.00	A	0.000	0.000	0.000	0.000
		B	0.000	7.563	3.104	5.436
		C	0.000	3.338	1.311	2.399
T3	160.00-140.00	A	0.000	0.000	0.000	0.000
		B	0.000	7.980	3.228	5.823
		C	0.000	4.699	2.039	3.428
T4	140.00-120.00	A	0.000	0.000	0.000	0.000
		B	0.000	6.425	3.822	6.812
		C	0.000	3.509	2.225	3.721
T5	120.00-100.00	A	0.000	0.000	0.000	0.000
		B	0.000	5.949	3.636	6.436
		C	0.000	3.251	2.117	3.517
T6	100.00-80.00	A	0.000	0.000	0.000	0.000
		B	0.000	5.576	4.223	7.415
		C	0.000	3.049	2.459	4.055
T7	80.00-60.00	A	0.000	0.000	0.000	0.000
		B	0.000	3.801	3.497	6.079
		C	0.000	2.081	2.036	3.327
T8	60.00-40.00	A	0.000	0.000	0.000	0.000
		B	0.000	3.495	3.392	5.819
		C	0.000	2.034	2.021	3.386
T9	40.00-20.00	A	0.000	0.000	0.000	0.000
		B	0.000	3.199	3.315	5.598
		C	0.000	1.919	1.999	3.358
T10	20.00-0.00	A	0.000	0.000	0.000	0.000
		B	0.000	3.140	3.719	6.281
		C	0.000	1.884	2.243	3.767

Feed Line Center of Pressure

Section	Elevation ft	CP_x in	CP_z in	CP_x Ice in	CP_z Ice in
T1	200.00-180.00	3.4651	-0.4625	2.0800	0.2993
T2	180.00-160.00	3.9589	-5.1825	2.7190	-2.4222

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Section	Elevation	CP _x	CP _z	CP _x	CP _z
	ft	in	in	Ice in	Ice in
T3	160.00-140.00	4.9408	-4.4982	3.5930	-2.6912
T4	140.00-120.00	6.4598	-4.2753	4.4918	-2.9657
T5	120.00-100.00	6.9684	-4.5573	4.9955	-3.2447
T6	100.00-80.00	7.2794	-4.7194	5.3018	-3.4309
T7	80.00-60.00	8.0650	-5.1938	6.3955	-4.0271
T8	60.00-40.00	7.9729	-4.9685	6.4325	-3.6130
T9	40.00-20.00	8.5702	-5.2387	6.9087	-3.6366
T10	20.00-0.00	8.6861	-5.2884	7.0572	-3.7110

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
			Horz Lateral ft	Vert ft					
Beacon	B	From Leg	0.50	0.0000	200.00	No Ice	2.00	2.00	0.02
						1/2" Ice	2.50	2.50	0.03
						1" Ice	3.00	3.00	0.04
						2" Ice	4.00	4.00	0.06
						4" Ice	6.00	6.00	0.10
Sidemarker	A	From Leg	0.50	0.0000	100.00	No Ice	0.60	0.60	0.01
						1/2" Ice	0.72	0.72	0.02
						1" Ice	0.86	0.86	0.03
						2" Ice	1.16	1.16	0.05
						4" Ice	1.89	1.89	0.13
Sidemarker	B	From Leg	0.50	0.0000	100.00	No Ice	0.60	0.60	0.01
						1/2" Ice	0.72	0.72	0.02
						1" Ice	0.86	0.86	0.03
						2" Ice	1.16	1.16	0.05
						4" Ice	1.89	1.89	0.13
Sidemarker	C	From Leg	0.50	0.0000	100.00	No Ice	0.60	0.60	0.01
						1/2" Ice	0.72	0.72	0.02
						1" Ice	0.86	0.86	0.03
						2" Ice	1.16	1.16	0.05
						4" Ice	1.89	1.89	0.13
**									
** 198 **									
DB225-A	A	From Leg	4.00	0.0000	198.00	No Ice	3.21	3.21	0.04
						1/2" Ice	5.78	5.78	0.05
						1" Ice	8.35	8.35	0.06
						2" Ice	13.48	13.48	0.08
						4" Ice	23.75	23.75	0.13
ALR10-O	B	From Leg	4.00	0.0000	198.00	No Ice	13.25	13.25	0.09
						1/2" Ice	15.31	15.31	0.18
						1" Ice	17.39	17.39	0.28
						2" Ice	20.79	20.79	0.52
						4" Ice	25.72	25.72	1.17
OGB6-928N	B	From Leg	4.00	0.0000	198.00	No Ice	0.97	0.97	0.01
						1/2" Ice	1.33	1.33	0.02
						1" Ice	1.63	1.63	0.03
						2" Ice	2.26	2.26	0.06
						4" Ice	3.67	3.67	0.17
PD1107-1	C	From Leg	4.00	0.0000	198.00	No Ice	2.18	2.18	0.01

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Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight
			Horz	Lateral	Vert					
							1/2" Ice	3.29	3.29	0.02
							1" Ice	4.43	4.43	0.05
							2" Ice	6.42	6.42	0.12
							4" Ice	9.18	9.18	0.35
PD201-7	C	From Leg	4.00	0.0000	198.00		No Ice	1.02	1.02	0.00
			0.00				1/2" Ice	1.81	1.81	0.01
			7.00				1" Ice	2.62	2.62	0.03
							2" Ice	3.76	3.76	0.07
							4" Ice	5.78	5.78	0.22
(4) 6' x 2" Mount Pipe	A	From Leg	4.00	0.0000	198.00		No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.50	1.50	0.03
			0.00				1" Ice	1.57	1.57	0.04
							2" Ice	1.71	1.71	0.05
							4" Ice	1.99	1.99	0.07
(4) 6' x 2" Mount Pipe	B	From Leg	4.00	0.0000	198.00		No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.50	1.50	0.03
			0.00				1" Ice	1.57	1.57	0.04
							2" Ice	1.71	1.71	0.05
							4" Ice	1.99	1.99	0.07
(4) 6' x 2" Mount Pipe	C	From Leg	4.00	0.0000	198.00		No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.50	1.50	0.03
			0.00				1" Ice	1.57	1.57	0.04
							2" Ice	1.71	1.71	0.05
							4" Ice	1.99	1.99	0.07
Sector Mount [SM 602-3]	C	None		0.0000	198.00		No Ice	33.11	33.11	1.54
							1/2" Ice	44.90	44.90	2.16
							1" Ice	56.69	56.69	2.78
							2" Ice	80.27	80.27	4.01
							4" Ice	127.43	127.43	6.49
**										
** 182 **										
RR90-17-02DP w/ Mount Pipe	A	From Leg	4.00	0.0000	182.00		No Ice	4.59	3.32	0.03
			0.00				1/2" Ice	5.09	4.09	0.07
			1.00				1" Ice	5.58	4.78	0.12
							2" Ice	6.59	6.23	0.22
							4" Ice	8.73	9.31	0.56
RR90-17-02DP w/ Mount Pipe	B	From Leg	4.00	0.0000	182.00		No Ice	4.59	3.32	0.03
			0.00				1/2" Ice	5.09	4.09	0.07
			1.00				1" Ice	5.58	4.78	0.12
							2" Ice	6.59	6.23	0.22
							4" Ice	8.73	9.31	0.56
RR90-17-02DP w/ Mount Pipe	C	From Leg	4.00	0.0000	182.00		No Ice	4.59	3.32	0.03
			0.00				1/2" Ice	5.09	4.09	0.07
			1.00				1" Ice	5.58	4.78	0.12
							2" Ice	6.59	6.23	0.22
							4" Ice	8.73	9.31	0.56
KRY 112 71	A	From Leg	4.00	0.0000	182.00		No Ice	0.68	0.45	0.01
			0.00				1/2" Ice	0.80	0.56	0.02
			0.00				1" Ice	0.93	0.68	0.03
							2" Ice	1.22	0.94	0.04
							4" Ice	1.90	1.57	0.11
KRY 112 71	B	From Leg	4.00	0.0000	182.00		No Ice	0.68	0.45	0.01
			0.00				1/2" Ice	0.80	0.56	0.02
			0.00				1" Ice	0.93	0.68	0.03
							2" Ice	1.22	0.94	0.04
							4" Ice	1.90	1.57	0.11
KRY 112 71	C	From Leg	4.00	0.0000	182.00		No Ice	0.68	0.45	0.01

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K	
			Horz ft	Lateral ft						
Pipe			0.00			1/2" Ice	4.50	4.48	0.07	
			1.00			1" Ice	4.95	5.22	0.11	
						2" Ice	5.87	6.74	0.22	
						4" Ice	8.05	10.00	0.55	
(2) DB980H90E-M w/ Mount Pipe	B	From Leg	5.00		0.0000	170.00	No Ice	4.04	3.62	0.03
			0.00				1/2" Ice	4.50	4.48	0.07
			1.00				1" Ice	4.95	5.22	0.11
							2" Ice	5.87	6.74	0.22
							4" Ice	8.05	10.00	0.55
(2) DB980H90E-M w/ Mount Pipe	C	From Leg	5.00		0.0000	170.00	No Ice	4.04	3.62	0.03
			0.00				1/2" Ice	4.50	4.48	0.07
			1.00				1" Ice	4.95	5.22	0.11
							2" Ice	5.87	6.74	0.22
							4" Ice	8.05	10.00	0.55
6' x 2" Mount Pipe	A	From Leg	5.00		0.0000	170.00	No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.50	1.50	0.03
			0.00				1" Ice	1.57	1.57	0.04
							2" Ice	1.71	1.71	0.05
							4" Ice	1.99	1.99	0.07
6' x 2" Mount Pipe	B	From Leg	5.00		0.0000	170.00	No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.50	1.50	0.03
			0.00				1" Ice	1.57	1.57	0.04
							2" Ice	1.71	1.71	0.05
							4" Ice	1.99	1.99	0.07
6' x 2" Mount Pipe	C	From Leg	5.00		0.0000	170.00	No Ice	1.43	1.43	0.02
			0.00				1/2" Ice	1.50	1.50	0.03
			0.00				1" Ice	1.57	1.57	0.04
							2" Ice	1.71	1.71	0.05
							4" Ice	1.99	1.99	0.07
Sector Mount [SM 506-3]	C	None			0.0000	170.00	No Ice	35.47	35.47	1.74
							1/2" Ice	50.60	50.60	2.35
							1" Ice	65.73	65.73	2.95
							2" Ice	95.99	95.99	4.16
							4" Ice	156.51	156.51	6.59
** ** 163 **										
Pipe Mount [PM 601-1]	A	From Leg	0.50		0.0000	163.00	No Ice	3.00	0.90	0.07
			0.00				1/2" Ice	3.74	1.12	0.08
			0.00				1" Ice	4.48	1.34	0.09
							2" Ice	5.96	1.78	0.12
							4" Ice	8.92	2.66	0.18
** ** 144 **										
PD156S-4	B	From Leg	4.00		0.0000	144.00	No Ice	0.44	0.44	0.01
			0.00				1/2" Ice	0.79	0.79	0.01
			6.00				1" Ice	1.14	1.14	0.01
							2" Ice	1.85	1.85	0.01
							4" Ice	3.26	3.26	0.02
SRL480N1DT4	C	From Leg	4.00		0.0000	144.00	No Ice	3.81	3.81	0.03
			0.00				1/2" Ice	5.37	5.37	0.06
			11.00				1" Ice	6.94	6.94	0.10
							2" Ice	10.13	10.13	0.20
							4" Ice	14.61	14.61	0.54
(2) PD1109-1	B	From Leg	4.00		0.0000	144.00	No Ice	2.83	2.83	0.02
			0.00				1/2" Ice	3.89	3.89	0.04
			8.00				1" Ice	4.97	4.97	0.07
							2" Ice	6.37	6.37	0.14

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA}		Weight
			Horz Lateral	Vert			Front	Side	
			ft	ft	°	ft	ft ²	ft ²	K
(4) 6' x 2" Mount Pipe	A	From Leg	4.00	0.0000	144.00	4" Ice	9.00	9.00	0.38
			0.00	No Ice		1.43	1.43	0.02	
			0.00	1/2" Ice		1.50	1.50	0.03	
			0.00	1" Ice		1.57	1.57	0.04	
			0.00	2" Ice		1.71	1.71	0.05	
(4) 6' x 2" Mount Pipe	B	From Leg	4.00	0.0000	144.00	4" Ice	1.99	1.99	0.07
			0.00	No Ice		1.43	1.43	0.02	
			0.00	1/2" Ice		1.50	1.50	0.03	
			0.00	1" Ice		1.57	1.57	0.04	
			0.00	2" Ice		1.71	1.71	0.05	
(4) 6' x 2" Mount Pipe	C	From Leg	4.00	0.0000	144.00	4" Ice	1.99	1.99	0.07
			0.00	No Ice		1.43	1.43	0.02	
			0.00	1/2" Ice		1.50	1.50	0.03	
			0.00	1" Ice		1.57	1.57	0.04	
			0.00	2" Ice		1.71	1.71	0.05	
Sector Mount [SM 602-3]	C	None		0.0000	144.00	4" Ice	1.99	1.99	0.07
				No Ice		33.11	33.11	1.54	
				1/2" Ice		44.90	44.90	2.16	
				1" Ice		56.69	56.69	2.78	
				2" Ice		80.27	80.27	4.01	
		4" Ice	127.43	127.43	6.49				
** ** 53 ** KS24019-L112A	A	From Leg	1.00	0.0000	53.00	No Ice	0.16	0.16	0.01
0.00			1/2" Ice	0.22		0.22	0.01		
2.00			1" Ice	0.30		0.30	0.01		
			2" Ice	0.48		0.48	0.02		
			4" Ice	0.95		0.95	0.06		
Side Arm Mount [SO 202-1]	A	From Leg	2.00	0.0000	53.00	No Ice	2.96	2.53	0.11
			0.00	1/2" Ice		4.10	3.51	0.13	
			0.00	1" Ice		5.24	4.49	0.16	
				2" Ice		7.52	6.45	0.20	
				4" Ice		12.08	10.37	0.30	
**									

Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets:		Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weight	
				Horz Lateral	Vert							
				ft	ft	°	°	ft	ft	ft ²	K	
** 163 ** PR-850	A	Grid	From Leg	1.00	60.0000			163.00	5.67	No Ice	25.22	0.04
0.00				1/2" Ice	25.97					0.17		
0.00				1" Ice	26.71					0.30		
				2" Ice	28.21					0.57		
				4" Ice	31.20					1.10		

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Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 90 deg - No Ice
4	Dead+Wind 180 deg - No Ice
5	Dead+Ice+Temp
6	Dead+Wind 0 deg+Ice+Temp
7	Dead+Wind 90 deg+Ice+Temp
8	Dead+Wind 180 deg+Ice+Temp
9	Dead+Wind 0 deg - Service
10	Dead+Wind 90 deg - Service
11	Dead+Wind 180 deg - Service

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	200 - 180	6.549	9	0.3126	0.0569
T2	180 - 160	5.241	9	0.3004	0.0443
T3	160 - 140	4.027	9	0.2611	0.0325
T4	140 - 120	2.989	9	0.2140	0.0240
T5	120 - 100	2.134	9	0.1770	0.0184
T6	100 - 80	1.430	9	0.1400	0.0132
T7	80 - 60	0.886	9	0.1007	0.0090
T8	60 - 40	0.500	9	0.0710	0.0066
T9	40 - 20	0.236	9	0.0405	0.0042
T10	20 - 0	0.075	9	0.0204	0.0020

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
200.00	Beacon	9	6.549	0.3126	0.0569	201465
198.00	DB225-A	9	6.417	0.3120	0.0557	201465
182.00	RR90-17-02DP w/ Mount Pipe	9	5.370	0.3027	0.0455	55966
170.00	(2) DB980H90E-M w/ Mount Pipe	9	4.616	0.2836	0.0381	33642
163.00	PR-850	9	4.199	0.2682	0.0341	26908
144.00	PD156S-4	9	3.181	0.2228	0.0254	24901
100.00	Sidemarker	9	1.430	0.1400	0.0132	29546
53.00	KS24019-L112A	9	0.395	0.0600	0.0058	43231

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	200 - 180	16.660	2	0.7952	0.1458

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T2	180 - 160	13.334	2	0.7639	0.1133
T3	160 - 140	10.245	2	0.6637	0.0832
T4	140 - 120	7.606	2	0.5440	0.0613
T5	120 - 100	5.433	2	0.4500	0.0472
T6	100 - 80	3.641	2	0.3559	0.0339
T7	80 - 60	2.256	2	0.2561	0.0230
T8	60 - 40	1.273	2	0.1804	0.0168
T9	40 - 20	0.603	2	0.1030	0.0109
T10	20 - 0	0.193	2	0.0519	0.0051

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
200.00	Beacon	2	16.660	0.7952	0.1458	81818
198.00	DB225-A	2	16.323	0.7937	0.1425	81818
182.00	RR90-17-02DP w/ Mount Pipe	2	13.660	0.7698	0.1165	22716
170.00	(2) DB980H90E-M w/ Mount Pipe	2	11.744	0.7209	0.0976	13230
163.00	PR-850	2	10.684	0.6817	0.0873	10602
144.00	PD156S-4	2	8.095	0.5663	0.0649	9795
100.00	Sidemarker	2	3.641	0.3559	0.0339	11624
53.00	KS24019-L112A	2	1.007	0.1525	0.0148	17023

Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load/Allowable	Allowable Ratio	Criteria	
T1	200	Leg	A325N	0.7500	4	2.76	19.44	0.142	✓	1.333	Bolt Tension
		Diagonal	A325X	0.6250	1	2.60	5.71	0.456	✓	1.333	Member Block Shear
		Top Girt	A325X	0.6250	1	0.06	8.16	0.007	✓	1.333	Member Bearing
T2	180	Leg	A325N	1.0000	4	8.35	34.56	0.242	✓	1.333	Bolt Tension
		Diagonal	A325X	0.6250	1	3.18	5.71	0.557	✓	1.333	Member Block Shear
T3	160	Leg	A325N	1.0000	4	14.01	34.56	0.406	✓	1.333	Bolt Tension
		Diagonal	A325X	0.6250	1	3.61	5.71	0.633	✓	1.333	Member Block Shear
T4	140	Leg	A325N	1.2500	4	19.57	54.00	0.362	✓	1.333	Bolt Tension
		Diagonal	A325X	0.6250	1	4.26	7.75	0.550	✓	1.333	Member Block Shear
T5	120	Leg	A325N	1.2500	4	24.91	54.00	0.461	✓	1.333	Bolt Tension
		Diagonal	A325X	0.6250	1	4.63	7.75	0.598	✓	1.333	Member Block Shear
T6	100	Leg	A325N	1.2500	6	20.02	54.00	0.371	✓	1.333	Bolt Tension
		Diagonal	A325X	0.7500	1	5.14	7.88	0.652	✓	1.333	Member Block Shear

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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T7	80	Leg	A325N	1.2500	6	23.03	54.00	0.427 ✓	1.333	Bolt Tension
		Diagonal	A325X	0.7500	1	6.09	12.69	0.480 ✓	1.333	Member Bearing
T8	60	Leg	A325N	1.3750	6	26.20	65.34	0.401 ✓	1.333	Bolt Tension
		Diagonal	A325X	0.7500	1	6.88	13.05	0.527 ✓	1.333	Member Bearing
T9	40	Leg	A325N	1.3750	6	29.34	65.34	0.449 ✓	1.333	Bolt Tension
		Diagonal	A325X	0.7500	1	7.32	13.05	0.561 ✓	1.333	Member Bearing
T10	20	Leg	A572-50	1.5000	8	24.91	37.91	0.657 ✓	1.333	Bolt Tension
		Diagonal	A325X	0.7500	1	8.27	13.05	0.634 ✓	1.333	Member Bearing

Compression Checks

Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	200 - 180	Pipe 2.875" x 0.375" (2.5 #160)	20.00	5.00	67.1 K=1.00	21.46	2.9452	-14.50	63.21	0.229 ✓
T2	180 - 160	Pipe 3.5" x 0.300" (3 XS)	20.03	5.01	52.9 K=1.00	23.89	3.0159	-41.41	72.06	0.575 ✓
T3	160 - 140	Pipe 4" x 0.318" (3.5 XS)	20.03	5.01	46.0 K=1.00	24.96	3.6784	-68.21	91.82	0.743 ✓
T4	140 - 120	Pipe 4.5" x 0.438" (4 #120)	20.03	6.68	55.5 K=1.00	23.47	5.5894	-94.22	131.21	0.718 ✓
T5	120 - 100	Pipe 5.563" x 0.375" (5 XS)	20.03	6.68	43.6 K=1.00	25.32	6.1120	-119.69	154.76	0.773 ✓
T6	100 - 80	Pipe 5.563" x 0.375" (5 XS)	20.03	6.68	43.6 K=1.00	25.32	6.1120	-144.59	154.76	0.934 ✓
T7	80 - 60	Pipe 6.625" x 0.432" (6 EH)	20.03	10.02	54.8 K=1.00	23.59	8.4049	-167.20	198.28	0.843 ✓
T8	60 - 40	Pipe 8.625" x 0.322" (8 STD)	20.03	10.02	40.9 K=1.00	25.70	8.3993	-191.59	215.89	0.887 ✓
T9	40 - 20	Pipe 8.625" x 0.500" (8 XS)	20.03	10.02	41.8 K=1.00	25.58	12.7627	-216.08	326.50	0.662 ✓
T10	20 - 0	Pipe 8.625" x 0.500" (8 XS)	20.03	9.97	41.6 K=1.00	25.61	12.7627	-247.25	326.82	0.757 ✓

Diagonal Design Data (Compression)

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Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	200 - 180	L1 3/4x1 3/4x3/16	7.07	3.20	113.8 K=1.02	11.15	0.6211	-2.77	6.92	0.401
T2	180 - 160	L1 3/4x1 3/4x3/16	8.40	4.01	140.0 K=1.00	7.61	0.6211	-3.13	4.73	0.661
T3	160 - 140	L1 3/4x1 3/4x3/16	10.08	4.82	168.6 K=1.00	5.25	0.6211	-3.83	3.26	1.174
T4	140 - 120	L2 1/2x2 1/2x3/16	12.58	6.10	147.9 K=1.00	6.83	0.9020	-4.45	6.16	0.722
T5	120 - 100	L2 1/2x2 1/2x3/16	14.32	6.92	167.7 K=1.00	5.31	0.9020	-4.79	4.79	1.000
T6	100 - 80	L3x3x3/16	16.11	7.82	157.4 K=1.00	6.03	1.0900	-5.15	6.57	0.784
T7	80 - 60	L3 1/2x3 1/2x1/4	19.30	9.43	163.1 K=1.00	5.62	1.6900	-6.22	9.49	0.655
T8	60 - 40	L3 1/2x3 1/2x1/4	21.03	10.20	176.4 K=1.00	4.80	1.6900	-6.88	8.11	0.848
T9	40 - 20	L3 1/2x3 1/2x1/4	22.81	11.10	191.9 K=1.00	4.06	1.6900	-7.32	6.86	1.068
T10	20 - 0	L4x4x1/4	24.60	11.99	181.0 K=1.00	4.56	1.9400	-8.27	8.84	0.936

Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	200 - 180	L1 3/4x1 3/4x3/16	5.00	4.43	154.7 K=1.00	6.24	0.6211	-0.06	3.88	0.015

Tension Checks

Leg Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	200 - 180	Pipe 2.875" x 0.375" (2.5 #160)	20.00	5.00	67.1	30.00	2.9452	11.04	88.36	0.125
T2	180 - 160	Pipe 3.5" x 0.300" (3 XS)	20.03	5.01	52.9	30.00	3.0159	33.40	90.48	0.369
T3	160 - 140	Pipe 4" x 0.318" (3.5 XS)	20.03	5.01	46.0	30.00	3.6784	56.06	110.35	0.508
T4	140 - 120	Pipe 4.5" x 0.438" (4 #120)	20.03	6.68	55.5	30.00	5.5894	78.26	167.68	0.467
T5	120 - 100	Pipe 5.563" x 0.375" (5 XS)	20.03	6.68	43.6	30.00	6.1120	99.65	183.36	0.543

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Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T6	100 - 80	Pipe 5.563" x 0.375" (5 XS)	20.03	6.68	43.6	30.00	6.1120	120.11	183.36	0.655
T7	80 - 60	Pipe 6.625" x 0.432" (6 EH)	20.03	10.02	54.8	30.00	8.4049	138.20	252.15	0.548
T8	60 - 40	Pipe 8.625" x 0.322" (8 STD)	20.03	10.02	40.9	30.00	8.3993	157.20	251.98	0.624
T9	40 - 20	Pipe 8.625" x 0.500" (8 XS)	20.03	10.02	41.8	30.00	12.7627	176.01	382.88	0.460
T10	20 - 0	Pipe 8.625" x 0.500" (8 XS)	20.03	9.97	41.6	30.00	12.7627	199.29	382.88	0.521

Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	200 - 180	L1 3/4x1 3/4x3/16	7.07	3.20	75.2	29.00	0.3604	2.60	10.45	0.249
T2	180 - 160	L1 3/4x1 3/4x3/16	8.40	4.01	93.3	29.00	0.3604	3.18	10.45	0.304
T3	160 - 140	L1 3/4x1 3/4x3/16	10.08	4.82	111.6	29.00	0.3604	3.61	10.45	0.346
T4	140 - 120	L2 1/2x2 1/2x3/16	12.58	6.10	96.6	29.00	0.5710	4.26	16.56	0.258
T5	120 - 100	L2 1/2x2 1/2x3/16	14.32	6.92	109.3	29.00	0.5710	4.63	16.56	0.280
T6	100 - 80	L3x3x3/16	16.11	7.82	102.0	29.00	0.6945	5.14	20.14	0.255
T7	80 - 60	L3 1/2x3 1/2x1/4	19.30	9.43	105.9	29.00	1.1034	6.09	32.00	0.190
T8	60 - 40	L3 1/2x3 1/2x1/4	21.03	10.20	114.4	29.00	1.1034	6.45	32.00	0.201
T9	40 - 20	L3 1/2x3 1/2x1/4	21.92	10.65	119.3	29.00	1.1034	6.85	32.00	0.214
T10	20 - 0	L4x4x1/4	24.60	11.99	116.9	29.00	1.2909	7.36	37.44	0.197

Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	200 - 180	L1 3/4x1 3/4x3/16	5.00	4.43	106.4	29.00	0.3604	0.03	10.45	0.003

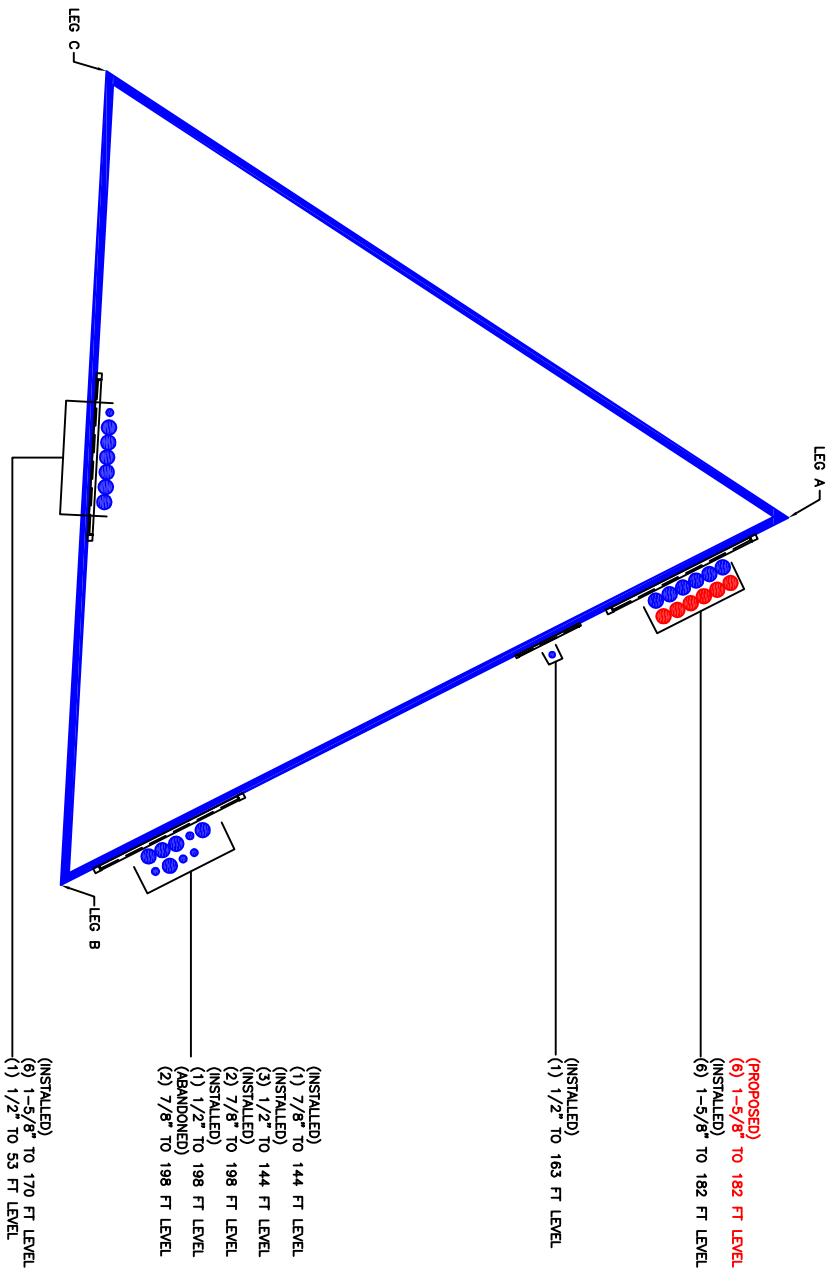
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Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
T1	200 - 180	Leg	Pipe 2.875" x 0.375" (2.5 #160)	3	-14.50	84.26	17.2	Pass
T2	180 - 160	Leg	Pipe 3.5" x 0.300" (3 XS)	33	-41.41	96.06	43.1	Pass
T3	160 - 140	Leg	Pipe 4" x 0.318" (3.5 XS)	60	-68.21	122.40	55.7	Pass
T4	140 - 120	Leg	Pipe 4.5" x 0.438" (4 #120)	87	-94.22	174.90	53.9	Pass
T5	120 - 100	Leg	Pipe 5.563" x 0.375" (5 XS)	108	-119.69	206.29	58.0	Pass
T6	100 - 80	Leg	Pipe 5.563" x 0.375" (5 XS)	129	-144.59	206.29	70.1	Pass
T7	80 - 60	Leg	Pipe 6.625" x 0.432" (6 EH)	150	-167.20	264.31	63.3	Pass
T8	60 - 40	Leg	Pipe 8.625" x 0.322" (8 STD)	165	-191.59	287.78	66.6	Pass
T9	40 - 20	Leg	Pipe 8.625" x 0.500" (8 XS)	180	-216.08	435.22	49.6	Pass
T10	20 - 0	Leg	Pipe 8.625" x 0.500" (8 XS)	195	-247.25	435.64	56.8	Pass
T1	200 - 180	Diagonal	L1 3/4x1 3/4x3/16	10	-2.77	9.23	30.1	Pass
							34.2 (b)	
T2	180 - 160	Diagonal	L1 3/4x1 3/4x3/16	37	-3.13	6.30	49.6	Pass
T3	160 - 140	Diagonal	L1 3/4x1 3/4x3/16	64	-3.83	4.35	88.0	Pass
T4	140 - 120	Diagonal	L2 1/2x2 1/2x3/16	91	-4.45	8.21	54.2	Pass
T5	120 - 100	Diagonal	L2 1/2x2 1/2x3/16	112	-4.79	6.38	75.0	Pass
T6	100 - 80	Diagonal	L3x3x3/16	133	-5.15	8.76	58.8	Pass
T7	80 - 60	Diagonal	L3 1/2x3 1/2x1/4	154	-6.22	12.65	49.1	Pass
T8	60 - 40	Diagonal	L3 1/2x3 1/2x1/4	169	-6.88	10.81	63.6	Pass
T9	40 - 20	Diagonal	L3 1/2x3 1/2x1/4	184	-7.32	9.14	80.1	Pass
T10	20 - 0	Diagonal	L4x4x1/4	199	-8.27	11.79	70.2	Pass
T1	200 - 180	Top Girt	L1 3/4x1 3/4x3/16	4	-0.06	5.17	1.1	Pass
							Summary	
							Leg (T6)	70.1 Pass
							Diagonal (T3)	88.0 Pass
							Top Girt (T1)	1.1 Pass
							Bolt Checks	49.3 Pass
							RATING =	88.0 Pass

APPENDIX B
BASE LEVEL DRAWING

APPENDIX C
ADDITIONAL CALCULATIONS



BUSINESS UNIT: 871584 TOWER ID: C_BASLEVEL

APPENDIX C
ADDITIONAL CALCULATIONS

Project Name:	John Tom Hill
Project Number:	871584
Job Number:	1005912
Date:	2/12/2015



Created On:	6/3/2014
Checked By:	SMR / DW
Revised On:	9/26/2014
Revision No.:	5.4

Self Support Tower Pad-Pier Foundation

Load	
Code	F
Compression	246 kips
Shear (comp)	26 kips
Uplift	198 kips
Shear (uplift)	22 kips
Soil Unit Weight	135 pcf
Friction Angle	36
Cohesion	0 psf

Pad	
Thickness	1.75 ft
Bearing Depth	10 ft
Width	15 ft
Top Rebar Size	7
Top Rebar Quantity	20
Bottom Rebar Size	7
Bottom Rebar Quantity	20

Pier	
Pier type	Circle
Diameter	3.5 ft
Height above Grade	0.5 ft
Rebar Size	7
Rebar Quantity	14
Tie Size	3
Tie C/C Spacing	12 in

Material	
Concrete Strength (F'c)	3000 psi
Concrete Density	150 pcf
Rebar Tensile (Fy)	60 ksi
Clear Cover	3 in

Soil	
Ult. Bearing Capacity	8 ksf
Frost Depth	40 in
Water Table Depth	99 ft

Structural Capacity	73.9%	Pass
Soil Interaction Capacity	68.2%	Pass
Overall Foundation Capacity	73.9%	Pass

Compression		
Pad Beam Shear Capacity	246.8	kips
Pad Beam Shear	182.3	kips
Pad Beam Shear Check	73.9%	Pass
Steel Punching Resistance	648.0	kips
Punching Shear	326.1	kips
Punching Shear Check	50.3%	Pass
Pad Bending Moment Capacity	858.8	k-ft
Pad Bending Moment	542.2	k-ft
Pad Bending Moment Check	63.1%	Pass
Pier Beam Shear Capacity	92.2	kips
Pier Beam Shear	35.1	kips
Pier Beam Shear Check	38.1%	Pass
Pier Bending Moment Capacity	952.2	k-ft
Pier Bending Moment	191.5	k-ft
Pier Bending Moment Check	20.1%	Pass
Pad-Pier Bearing Capacity	4592.7	kips
Pad-Pier Bearing	643.7	kips
Pad-Pier Bearing Check	14.0%	Pass

Uplift		
Steel Rupture Capacity	453.6	kips
Steel Rupture Force	257.2	kips
Steel Rupture Check	56.7%	Pass
Pad Bending Moment Capacity	858.8	k-ft
Pad Bending Moment	58.9	k-ft
Pad Bending Moment Check	6.9%	Pass
Pier Beam Shear Capacity	92.2	kips
Pier Beam Shear	29.7	kips
Pier Beam Shear Check	32.2%	Pass
Pier Bending Moment Capacity	309.0	k-ft
Pier Bending Moment	153.9	k-ft
Pier Bending Moment Check	49.8%	Pass

Soil Interaction		
Compression Capacity	911.87	kips
Compression Force	253.82	kips
Compression Check	27.8%	Pass
Uplift Capacity	290.13	kips
Uplift Force	198	kips
Uplift Check	68.2%	Pass
Lateral Capacity	124.56	kips
Lateral Force	26	kips
Lateral Check	20.9%	Pass

Project Name:	John Tom Hill
Project Number:	871584
Job Number:	1005912
Date:	2/12/2015
Leg:	
Boring Number:	



Created On:	5/28/2014
Checked By:	
Revised On:	
Revision No.:	0

Self Support Tower Pad - Pier Foundation

Code :	F
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UPLIFT CHECK		
Uplift Force (k)	198	
	(Ultimate)	(Allowable)
Wt. Concrete (k):	71.69	57.35
Wt. Soil (k):	441.82	220.91
Friction (k):	23.74	11.87
Total (k)		290.13
% Capacity	68.2%	PASS

COMPRESSION CHECK		
Comp. Force (k)	246	
	(Ultimate)	(Allowable)
Friction (k):	23.74	11.87
Bearing Cap. (ksf):	8.00	4.00
Gross/Net Bearing?:	N	
	Weights	Factored Wt.
Gross Concrete (k):	N/A	7.82
Net Concrete (k):	7.82	
Soil (k):	N/A	0.00
% Capacity	27.8%	PASS

FOUNDATION INFORMATION	
Density Concrete (pcf):	150
Pier Shape (R or S):	R
Pier Diameter/Width (ft):	3.5
Pier Area (ft^2):	9.62
Pier Height Above Grade (ft):	0.50
Pad Width/Length (ft):	15.00
Pad Thickness (ft):	1.75
Pad Bearing Depth (ft):	10.00
Frost Depth (ft):	3.33
Water Table Depth (ft):	99.00

SOIL INFORMATION	
Number of Layers Above Pad:	2
Number of Layers Adjacent Pad:	1
Total Number of Layers:	3
Number of Layers Check	OK
Pad Check	OK
Frost Depth Check	OK
Water Table Check	OK

Soil Profile						
Soil Layer Number.	Layer Thickness (ft)	Depth at Bot. (ft)	Soil Type (C/S)	Unit Weight. (pcf)	Frict. Angle (°)	Cohesion (psf)
1	3.33	3.33	S	135	0	0
2	4.92	8.25	S	135	36	0
3	1.75	10.00	S	135	36	0
4	N/A	N/A	N/A	N/A	N/A	N/A
5	N/A	N/A	N/A	N/A	N/A	N/A
6	N/A	N/A	N/A	N/A	N/A	N/A

RADIO FREQUENCY EMISSIONS ANALYSIS REPORT
EVALUATION OF HUMAN EXPOSURE POTENTIAL
TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CT11189E

Glastonbury / Rt-94 & Fern
115 Birch Mountain Road
Glastonbury, CT 06033

March 4, 2015

EBI Project Number: 6215001321

Site Compliance Summary	
Compliance Status:	COMPLIANT
Site total MPE% of FCC general public allowable limit:	34.45 %

March 4, 2015

T-Mobile USA
Attn: Jason Overbey, RF Manager
35 Griffin Road South
Bloomfield, CT 06002

Emissions Analysis for Site: **CT11189E – Glastonbury / Rt-94 & Fern**

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **115 Birch Mountain Road, Glastonbury, CT**, for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limit for the 700 MHz Band is $467 \mu\text{W}/\text{cm}^2$, and the general population exposure limit for the PCS and AWS bands is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at **115 Birch Mountain Road, Glastonbury, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 GSM channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel
- 2) 2 UMTS channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 3) 2 LTE channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 4) 1 LTE channel (700 MHz Band) was considered for each sector of the proposed installation. This channel has a transmit power of 30 Watts.
- 5) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.

- 6) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antennas used in this modeling are the **EMS RR90_17_02DP** for 1900 MHz (PCS) channels and the **Commscope LNX-6515DS-VTM** for 700 MHz channels. This is based on feedback from the carrier with regards to anticipated antenna selection. The **EMS RR90_17_02DP** has a maximum gain of **14.4 dBd** at its main lobe. The **Commscope LNX-6515DS-VTM** has a maximum gain of **14.6 dBd** at its main lobe. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antenna mounting height centerline of the proposed antennas is **183 feet** above ground level (AGL).
- 9) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general public threshold limits.

T-Mobile Site Inventory and Power Data

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	EMS RR90_17_02DP	Make / Model:	EMS RR90_17_02DP	Make / Model:	EMS RR90_17_02DP
Gain:	14.4 dBd	Gain:	14.4 dBd	Gain:	14.4 dBd
Height (AGL):	183	Height (AGL):	183	Height (AGL):	183
Frequency Bands	1900 MHz(PCS)	Frequency Bands	1900 MHz(PCS)	Frequency Bands	1900 MHz(PCS)
Channel Count	6	Channel Count	6	# PCS Channels:	6
Total TX Power:	240	Total TX Power:	240	# AWS Channels:	240
ERP (W):	6,610.15	ERP (W):	6,610.15	ERP (W):	6,610.15
Antenna A1 MPE%	0.76	Antenna B1 MPE%	0.76	Antenna C1 MPE%	0.76
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	Commscope LNX- 6515DS-VTM	Make / Model:	Commscope LNX- 6515DS-VTM	Make / Model:	Commscope LNX- 6515DS-VTM
Gain:	14.6 dBd	Gain:	14.6 dBd	Gain:	14.6 dBd
Height (AGL):	183	Height (AGL):	183	Height (AGL):	183
Frequency Bands	700 MHz	Frequency Bands	700 MHz	Frequency Bands	700 MHz
Channel Count	1	Channel Count	1	Channel Count	1
Total TX Power:	30	Total TX Power:	30	Total TX Power:	30
ERP (W):	865.21	ERP (W):	865.21	ERP (W):	865.21
Antenna A2 MPE%	0.21	Antenna B2 MPE%	0.21	Antenna C2 MPE%	0.21

Site Composite MPE%	
Carrier	MPE%
T-Mobile	2.91
Sprint	1.50 %
Tilcon Tomasso	1.12 %
Arch Cmens	0.82 %
SkyTel	0.75 %
Arch Cmens	1.48 %
US Drug	0.78 %
Internal Revenue	0.77 %
Connecticut Radio	1.08 %
Federal Express	0.72 %
Northeast Paging	4.16 %
Stamm Const.	1.38 %
Unknown	1.40 %
Unknown	1.12 %
Unknown	7.64 %
Unknown	6.79 %
Site Total MPE %:	34.45 %

T-Mobile Sector 1 Total:	0.97 %
T-Mobile Sector 2 Total:	0.97 %
T-Mobile Sector 3 Total:	0.97 %
Site Total:	34.45 %

Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general public exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general public exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector 1:	0.97 %
Sector 2:	0.97 %
Sector 3 :	0.97 %
T-Mobile Total:	2.91 %
Site Total:	34.45 %
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **34.45%** of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.



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