

Northeast Site Solutions Denise Sabo 4 Angela's Way, Burlington CT 06013 203-435-3640 denise@northeastsitesolutions.com

October 7, 2021

Members of the Siting Council Connecticut Siting Council Ten Franklin Square New Britain, CT 06051

RE: Tower Share Application 115 Birch Mountain Road, Glastonbury, CT 06033 Latitude: 41.708956 Longitude: -72.473447 Site# 871584_Crown_Dish

Dear Ms. Bachman:

This letter and attachments are submitted on behalf of Dish Wireless LLC. Dish Wireless LLC plans to install antennas and related equipment to the tower site located at 115 Birch Mountain Road in Glastonbury, Connecticut.

Dish Wireless LLC proposes to install three (3) 600/1900 5G MHz antenna and six (6) RRUs, at the 134-foot level of the existing 200foot self-support tower, one (1) Fiber cables will also be installed. Dish Wireless LLC equipment cabinets will be placed within 7x5 lease area. Included are plans by Infinigy, dated October 8, 2021 Exhibit C. Also included is a structural analysis prepared by Crown Castle, dated June 4, 2021, confirming that the existing tower is structurally capable of supporting the proposed equipment. Attached as Exhibit D. The facility was approved by the Town of Glastonbury Planning and Zoning on April 25, 2001. Please see attached Exhibit A.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies 16-50aa, of Dish Wireless LLC intent to share a telecommunications facility pursuant to R.C.S.A. 16-50j-88. In accordance with R.C.S.A., a copy of this letter is being sent to Richard J. Johnson, Town Manager and Peter R Carey, Building Official for the Town of Glastonbury, as well as the tower owner (Crown Castle) and property owner (Scarrone Park LLC)

The planned modifications of the facility fall squarely within those activities explicitly provided for in R.C.S.A. 16-50j-89.

1. The proposed modification will not result in an increase in the height of the existing structure. The top of the tower is 200-feet; Dish Wireless LLC proposed antennas will be located at a center line height of 134-feet.

2. The proposed modifications will not result in the increase of the site boundary as depicted on the attached site plan.

3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed local and state criteria. The incremental effect of the proposed changes will be negligent.

54 Main Street Unit 3 | Sturbridge Ma 01566 | f: 413-521-0558 | www.northeastsitesolutions.com



4. The operation of the proposed antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard. As indicated in the attached power density calculations, the combined site operations will result in a total power density of 15.05% as evidenced by Exhibit F.

Connecticut General Statutes 16-50aa indicates that the Council must approve the shared use of a telecommunications facility provided it finds the shared use is technically, legally, environmentally, and economically feasible and meets public safety concerns. As demonstrated in this letter, Dish Wireless LLC respectfully indicates that the shared use of this facility satisfies these criteria.

A. Technical Feasibility. The existing monopole has been deemed structurally capable of supporting Dish Wireless LLC proposed loading. The structural analysis is included as Exhibit D.

B. Legal Feasibility. As referenced above, C.G.S. 16-50aa has been authorized to issue orders approving the shared use of an existing tower such as this support tower in Glastonbury. Under the authority granted to the Council, an order of the Council approving the requested shared use would permit Dish Wireless LLC to obtain a building permit for the proposed installation. Further, a Letter of Authorization is included as Exhibit G, authorizing Dish Wireless LLC to file this application for shared use.

C. Environmental Feasibility. The proposed shared use of this facility would have a minimal environmental impact. The installation of Dish Wireless LLC equipment at the 134-foot level of the existing 200-foot tower would have an insignificant visual impact on the area around the tower. Dish Wireless LLC ground equipment would be installed within the existing facility compound. Dish Wireless LLC shared use would therefore not cause any significant alteration in the physical or environmental characteristics of the existing site. Additionally, as evidenced by Exhibit F, the proposed antennas would not increase radio frequency emissions to a level at or above the Federal Communications Commission safety standard.

D. Economic Feasibility. Dish Wireless LLC will be entering into an agreement with the owner of this facility to mutually agreeable terms. As previously mentioned, the Letter of Authorization has been provided by the owner to assist Dish Wireless LLC with this tower sharing application.

E. Public Safety Concerns. As discussed above, the tower is structurally capable of supporting Dish Wireless LLC proposed loading. Dish Wireless LLC is not aware of any public safety concerns relative to the proposed sharing of the existing guyed tower. Dish Wireless LLC intentions of providing new and improved wireless service through the shared use of this facility is expected to enhance the safety and welfare of local residents and individuals traveling through Glastonbury.

Sincerely,

Deníse Sabo

Denise Sabo Mobile: 203-435-3640 Fax: 413-521-0558 Office: 4 Angela's Way, Burlington CT 06013 Email: denise@northeastsitesolutions.com



Attachments cc:

Richard J. Johnson, Town Manager Glastonbury Town Hall 2155 Main Street Glastonbury, CT 06033

Peter R Carey, Building Official Glastonbury Town Hall 2155 Main Street Glastonbury, CT 06033

Scarrone Park LLC, Property Owner C/O Maria A. Toczyska 3385 Hebron Avenue Glastonbury, CT 06033

Crown Castle, Tower Owner

Exhibit A

Original Facility Approval

0425-01'

TOWN OF GLASTONBURY APPLICATION FOR BUILDING PERMIT CONNECTICUT STATE BUILDING CODE (SBC111.0)

DEPARTMENT D	ECISION	ESTIMATED	COSTS	FEES
Approved	Disapproved	Plumbing Electrical	, <i>8.70</i> , .000	C.O. & Use Structural Plumbing Electrical Heating/AC
Date	Inspector	Fire Protection	0, 870	Fire Protection
	(Please Print or "	Type All Entries	5)	N92
		SKEWCOD A	UPEL, SHE	LOIH CINCOD & JANENDA, 1-1-C
701 HEBRON AVE. Street Address			CT State	
Home Phone#		<u>860.652</u>	- 4022-	Mobile Phone#
MOTTRAD NORDY PMELICAN	ANTENNA SITES - Applicant's Name	T PAUL BENN	TL M	LIEIT MANAGER
<u>C349 FORGE TVEN</u> Street Address	BENSALK	,	PA State	 Zip
Home Phone#	215-252-4955 Work Phone#	215-257	- 6152	Mobile Phone#
C.E.R., TOWERS, L.L.C.	or/General Contractor			$\frac{16-148-6446}{\text{Registration #}}$
7693 WEST STATE ST Street Address	LOWVILLE	NY State	13367 Zip	- <u>3/5 - 376 - 0056</u> Telephone#
Home Phone#	Work Phone#	<u></u>	<u>6 - 8139</u> =x#	Mobile Phone#
ZONING INFORMATION: <u>Distance From</u> : Street Line	Rear Line	75		e. <i>RISOL RESUMACE</i> Permit
Project Type: a) S New Constru- b) Addition c) Alteration d) Repair/Repla e) Demolition	uction f) 🗌 Relow g) 🔲 Char h) 🛄 Articl	cation nge of Use		· • • • • • • • • • • • • • • • • • • •
Construction Type: 1 A 1 Use Group(s): A-1 B A-2 A-3 F- A-4 F-2 A-5	☐ H-1 ☐ H-2 1 ☐ H-3 2 ☐ H-4	□ I-2 □ I-3 □	☐ 3B]M R-1 R-2 R-3	□ 4 □ 5A □ 5B □ S-1 □ S-2 □ U
Mixed Use: Yes No	Separated (Ove	Nonseparated		

Exhibit B

Property Card

115 BIRCH MOUNTAIN RD

Location	115 BIRCH MOUNTAIN RD	Mblu	N6/ 2920/ E0001C/ /
Acct#	29203387	Owner	SCARRONE PARK LLC
Assessment	\$566,600	Appraisal	\$809,400
PID	13487	Building Count	1

Current Value

Appraisal					
Valuation Year	Improvements	Land	Total		
2019	\$800	\$808,600	\$809,400		
Assessment					
Valuation Year	Improvements	Land	Total		
2019	\$600	\$566,000	\$566,600		

Owner of Record

Owner	SCARRONE PARK LLC	Sale Price	\$0
Co-Owner	C/O TOCZYSKA MARIA A	Certificate	
Address	3385 HEBRON AVE	Book & Page	3525/0218
	GLASTONBURY, CT 06033-2806	Sale Date	11/15/2018
		Instrument	79

Ownership History

Ownership History						
Owner	Sale Price	Certificate	Book & Page	Instrument	Sale Date	
TOCZYSKA MARIA A	\$0		3525/0216	81	11/15/2018	
SCARRONE CAROLYN R REVOCABLE TRUST	\$0		3468/0328	25	01/22/2018	
SCARRONE CAROLYN R REVOCABLE TRUST	\$0		1829/0101	79	06/03/2003	
SCARRONE CAROLYN R	\$0		1261/0312		07/29/1999	

Building Information

Building 1 : Section 1

Year Built:	
Living Area:	0
Replacement Cost:	\$0

Replacement Cost

Less Depreciation:

:

\$0

Building Attributes				
Field	Description			
Style	Vacant Land			
Model				
Occupancy				
Exterior Wall 1				
Roof Structure:				
Roof Cover				
Interior Wall 1				
Floor/Cover 1				
Floor/Cover 2				
Heat Fuel				
Heat Type:				
АС Туре:				
Total Bedrooms:				
Total Bthrms:				
Total Half Baths:				
Total Rooms:				
Extra Kitchens				
Style Sub Class				
Bsmt Garages				
Fireplaces				

Building Photo



(http://images.vgsi.com/photos/GlastonburyCTPhotos//\02\02\02\02.jpg)

Building Layout

(http://images.vgsi.com/photos/GlastonburyCTPhotos//Sketches/13487_13

Building Sub-Areas (sq ft)

No Data for Building Sub-Areas

.

Extra Features

Extra Features	<u>Legend</u>
No Data for Extra Features	

Land

Land Use		Land Line Valuation	
Use Code	350V	Size (Acres)	11.54
Description	Cell Tower 00 MDL	Assessed Value	\$566,000
Zone	RR	Appraised Value	\$808,600
Category			

Outbuildings

		Outbui	ldings			<u>Legend</u>
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #

SHD2Shed-Metal-Storage168 S.F.\$8001

Valuation History

Appraisal				
Valuation Year	Improvements	Land	Total	
4000	\$800	\$808,600	\$809,400	

		Assessment						
Valuation Year Improvements Land Total								
	4000	\$600	\$566,000	\$566,600				

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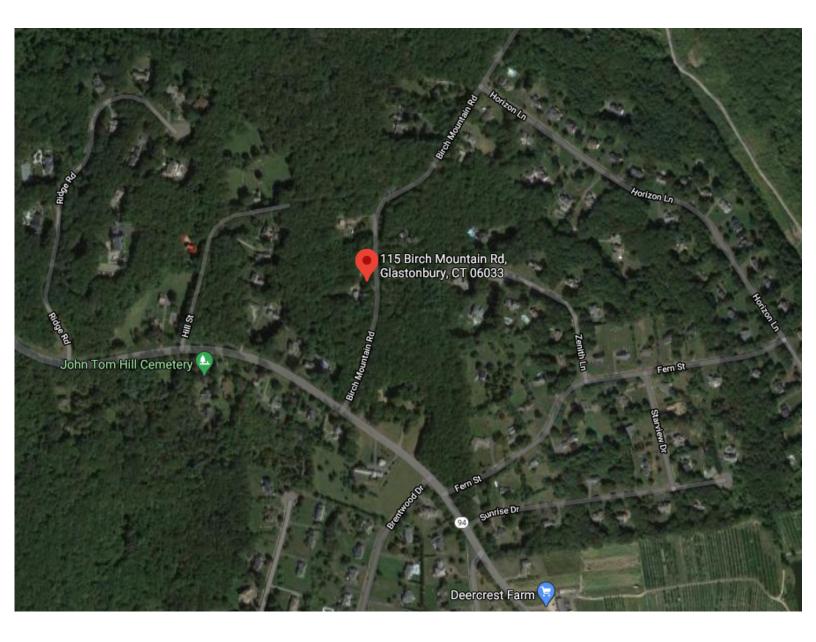


Exhibit C

Construction Drawings

		SITE INF	ORMATION	
		PROPERTY OWNER: ADDRESS:	SCARRONE PARK LLC 3385 HEBRON AVE. GLASTONBURY, CT 06033	•
		TOWER TYPE:	SELF SUPPORT TOWER	
		TOWER CO SITE ID:	871584	T
	SCOPE OF WORK	TOWER APP NUMBER:	556617	
	THIS IS NOT AN ALL INCLUSIVE LIST. CONTRACTOR SHALL UTILIZE SPECIFIED EQUIPMENT PART OR ENGINEER APPROVED EQUIVALENT. CONTRACTOR SHALL VERIFY ALL NEEDED EQUIPMENT TO PROVIDE A FUNCTIONAL SITE.	COUNTY:	HARTFORD	5
wireless	THE PROJECT GENERALLY CONSISTS OF THE FOLLOWING: TOWER SCOPE OF WORK:	LATITUDE (NAD 83):	41° 42' 32.24" N 41.708956 N	
	INSTALL (3) PROPOSED PANEL ANTENNAS (1 PER SECTOR) INSTALL (3) PROPOSED ANTENNA MOUNTS (1 PER SECTOR) INSTALL PROPOSED JUMPERS INSTALL PROPOSED JUMPERS INSTALL (3) DEPOSED PRIME (2 PER SECTOR)	LONGITUDE (NAD 83): ZONING JURISDICTION:	72° 28' 24.41" W 72.473447 W CT – CONNECTICUT	
DISH Wireless L.L.C. SITE ID:	INSTALL (6) PROPOSED RRUS (2 PER SECTOR) INSTALL (1) PROPOSED OVER VOLTAGE PROTECTION DEVICE (OVP) INSTALL (1) PROPOSED HYBRID CABLE	ZONING DISTRICT:	SITTING COUNCIL TBD	
BOBDL00076A	GROUND SCOPE OF WORK: • INSTALL (1) PROPOSED METAL PLATFORM • INSTALL (1) PROPOSED ICE BRIDGE	PARCEL NUMBER:	GLAS-002920-003387	0
DISH Wireless L.L.C. SITE ADDRESS:	INSTALL (1) PROPOSED PC CABINET INSTALL (1) PROPOSED EQUIPMENT CABINET INSTALL (1) PROPOSED POWER CONDUIT	OCCUPANCY GROUP:	U	F
115 BIRCH MTN, ROAD	INSTALL (1) PROPOSED TELCO CONDUIT INSTALL (1) PROPOSED TELCO-FIBER BOX	CONSTRUCTION TYPE:	∥−В	
	INSTALL (1) PROPOSED GPS UNIT INSTALL (1) PROPOSED SAFETY SWITCH (IF REQUIRED) INSTALL (1) PROPOSED CIENA BOX (IF REQUIRED) INSTALL (1) PROPOSED CIENA BOX (IF REQUIRED)	POWER COMPANY:	CONNECTICUT LIGHT & POWER	;
GLASTONBURY, CT 06033	INSTALL (1) PROPOSED METER SOCKET	TELEPHONE COMPANY:	AT&T	
CONNECTICUT CODE COMPLIANCE	SITE PHOTO		DIREC	;TIC
ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES: <u>CODE TYPE</u> <u>CODE</u> BUILDING 2018 CT STATE BUILDING CODE/2015 IBC W/ CT AMENDMENTS MECHANICAL 2018 CT STATE BUILDING CODE/2015 IMC W/ CT AMENDMENTS ELECTRICAL 2018 CT STATE BUILDING CODE/2017 NEC W/ CT AMENDMENTS		DEPART AND HEAD TOW. THE RAMP ON THE RIG SPRINGFIELD, HEAD RIG ON THE RAMP FOR HEE AVE TOWARD IRISH AME	MANCHESTER BOSTON R ARD MAXIM RD, TURN LEFT ONT IT FOR US-5 N / CT-15 N / IT ON THE RAMP FOR US-5 N IRON AVE TOWARD IRISH AMERIC RICAN HOME SOCIETY / HEBROI DAD, GLASTONBURY, CT 06033.	' Will Orth Can F
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A-3 EQUIPMENT PLATFORM AND H-FRAME DETAILS			Rd.	
A-4 EQUIPMENT DETAILS A-5 EQUIPMENT DETAILS		\sum	Mountain	
A-6 EQUIPMENT DETAILS			Birch	
E-1 ELECTRICAL/FIBER ROUTE PLAN AND NOTES E-2 ELECTRICAL DETAILS				
E-3 ELECTRICAL ONE-LINE, FAULT CALCS & PANEL SCHEDULE	UNDERGROUND SERVICE ALERT CBYD 811			/
G-1 GROUNDING PLANS AND NOTES G-2 GROUNDING DETAILS	UTILITY NOTIFICATION CENTER OF CONNECTICUT (800) 922-4455	SITE		
G-3 GROUNDING DETAILS	WWW.CBYD.COM		Duntain F	
RF-1 RF CABLE COLOR CODE RF-2 RF PLUMBING DIAGRAM			Birch M	
GN-1 LEGEND AND ABBREVIATIONS	GENERAL NOTES	lebron Ave		
GN-2 GENERAL NOTES GN-3 GENERAL NOTES	THE FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION. A TECHNICIAN WILL VISIT THE SITE AS REQUIRED FOR ROUTINE MAINTENANCE. THE PROJECT WILL NOT RESULT IN ANY SIGNIFICANT DISTURBANCE OR EFFECT ON		ain Rd	
GN-4 GENERAL NOTES	DRAINAGE. NO SANITARY SEWER SERVICE, POTABLE WATER, OR TRASH DISPOSAL IS REQUIRED AND NO COMMERCIAL SIGNAGE IS PROPOSED.		Brech	
	11"x17" PLOT WILL BE HALF SCALE UNLESS OTHERWISE NOTED		(94)	
	CONTRACTOR SHALL VERIFY ALL PLANS, EXISTING DIMENSIONS, AND CONDITIONS ON THE JOB SITE, AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK.	NO SCALE		
		INO JUALE		

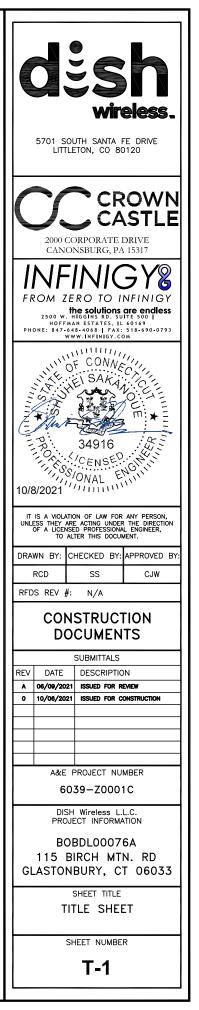
PROJE	ECT [DIRECTORY				
APPLICANT:	JCANT: DISH Wireless L.L.C. 5701 South Santa fe Drive Littleton, co 80120					
TOWER OWNER:	CANONS	CASTLE ORPORATE DRIVE BURG, PA 15317 -86-9377				
SITE DESIGNER:	HOFFMA	. Higgins RD. Ste. 500 N Estates, IL 60169 148–4068				
SITE ACQUISITION:		JEANNE CONTTRELL (203) 927–4317				
CONSTRUCTION M	ANAGER:	JAVIER SOTO JAVIER.SOTO©DISH.COM				
RF ENGINEER:		BOSSENER CHARLES BOSSENER.CHARLES@DISH.COM				
ONS						

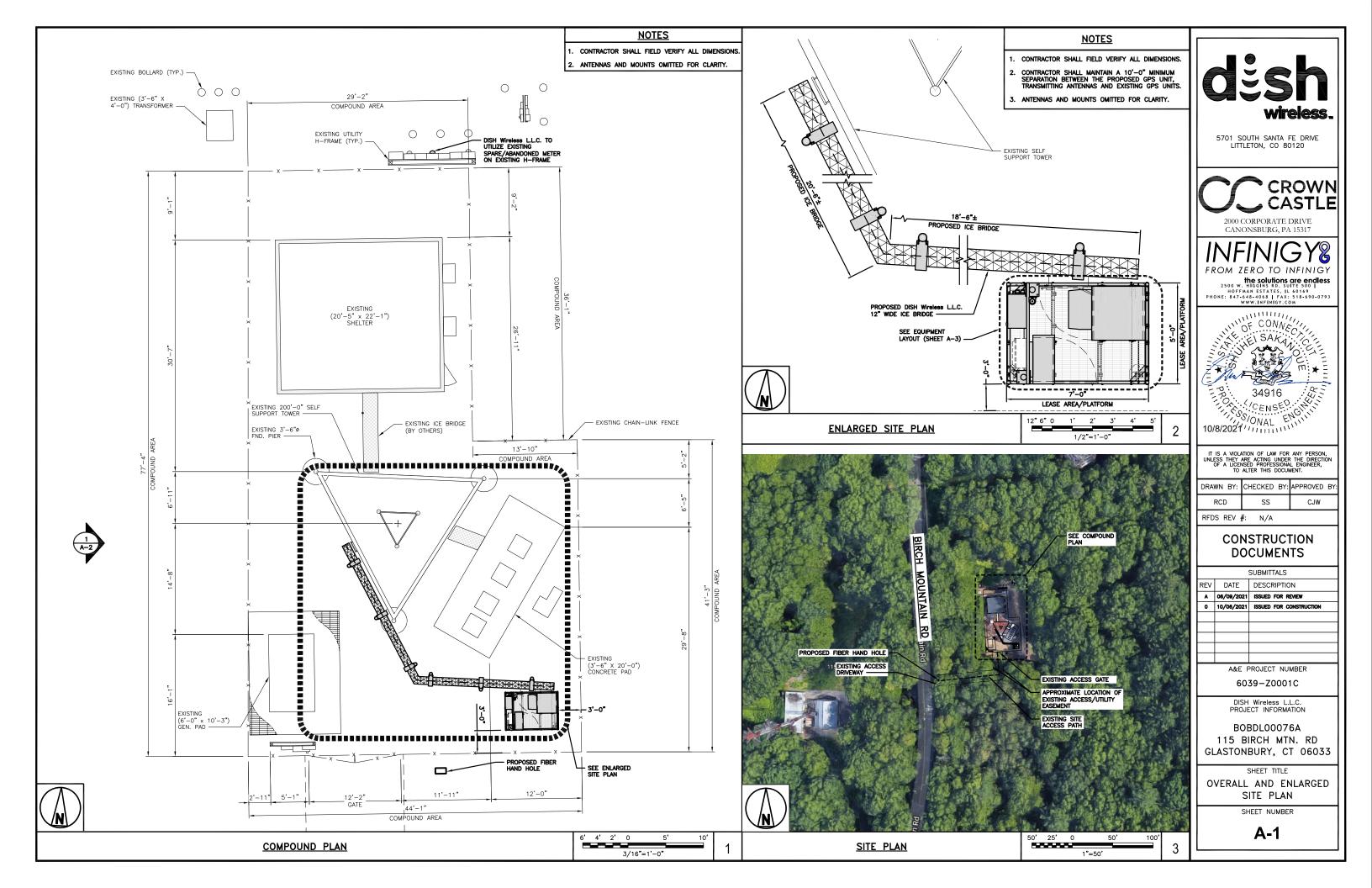
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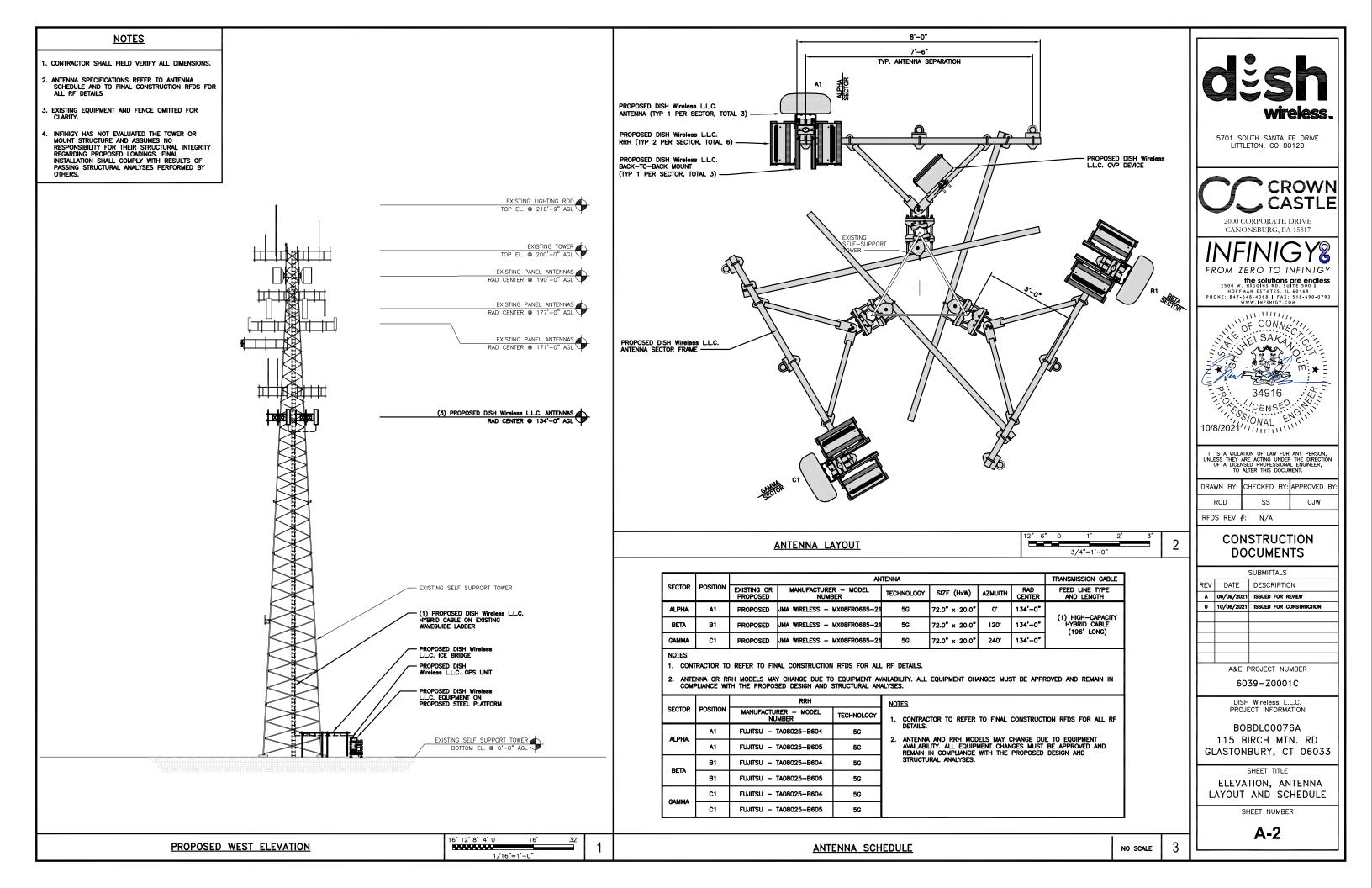
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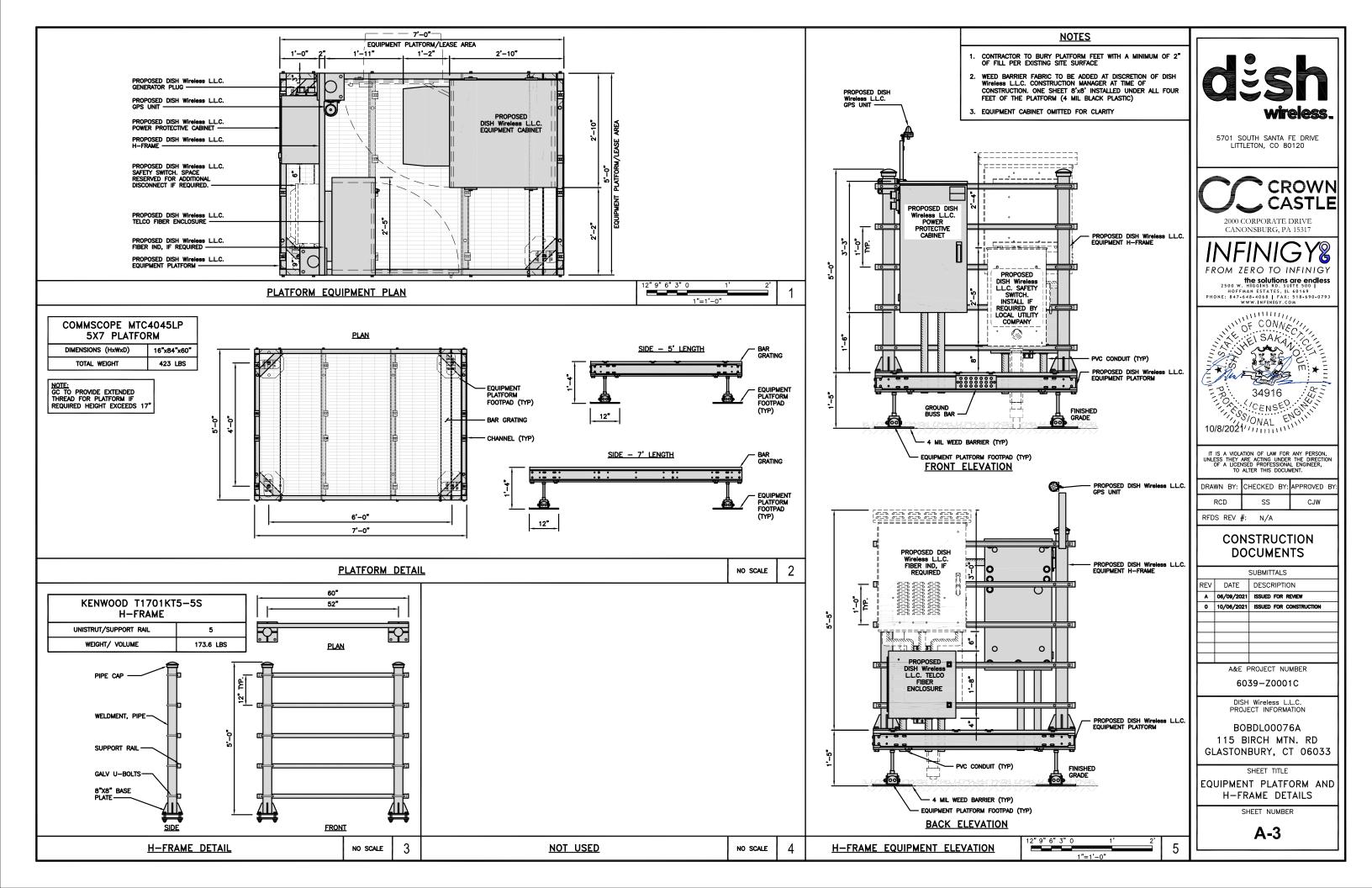
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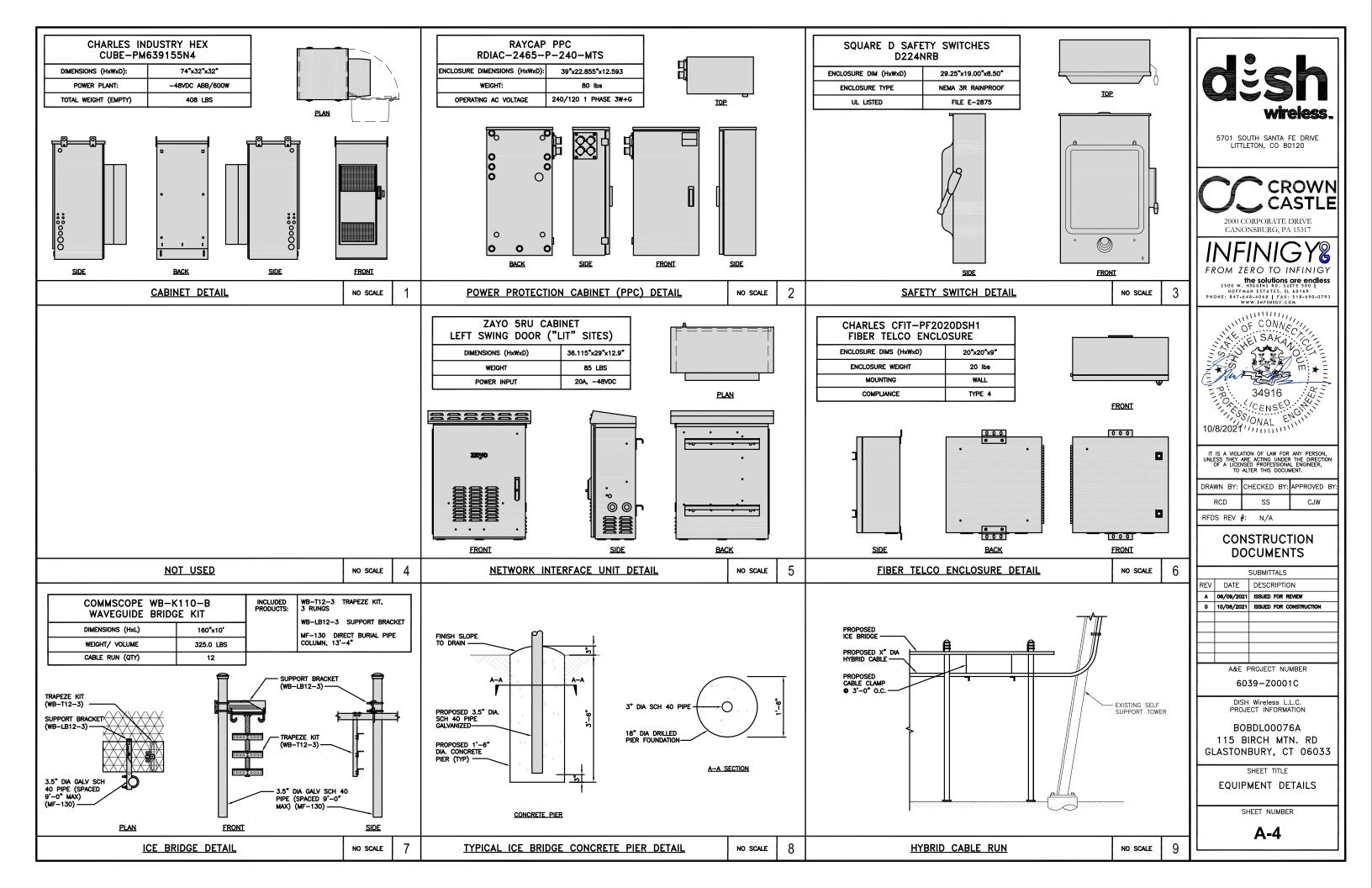




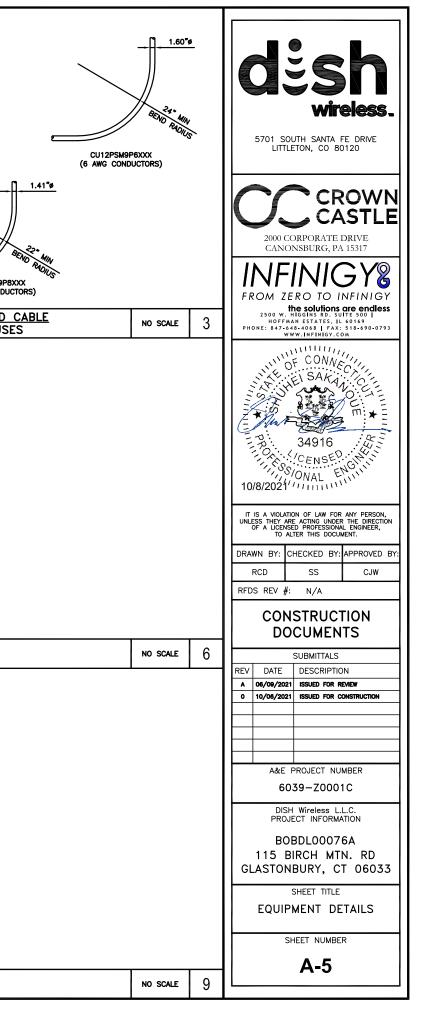


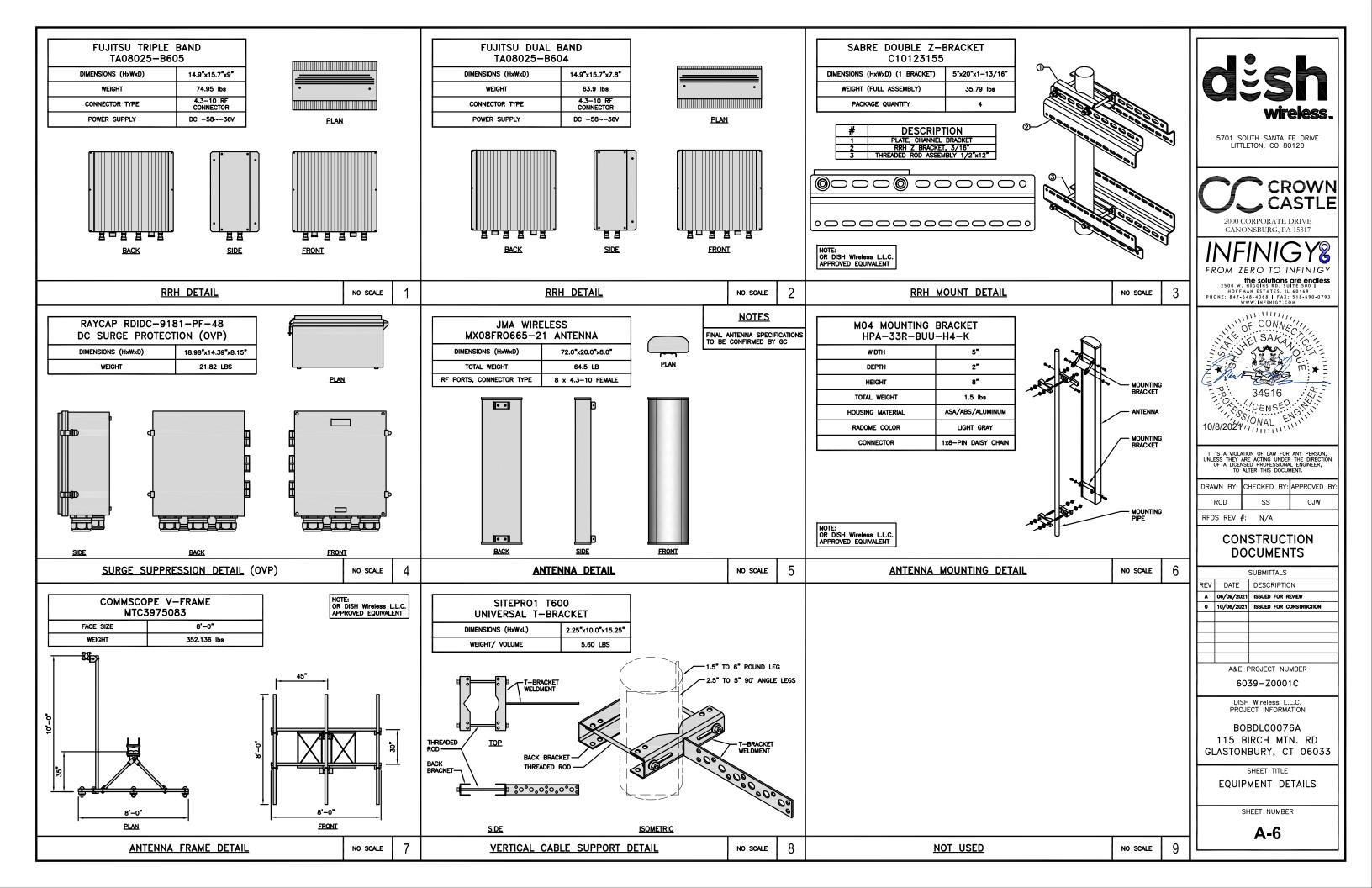


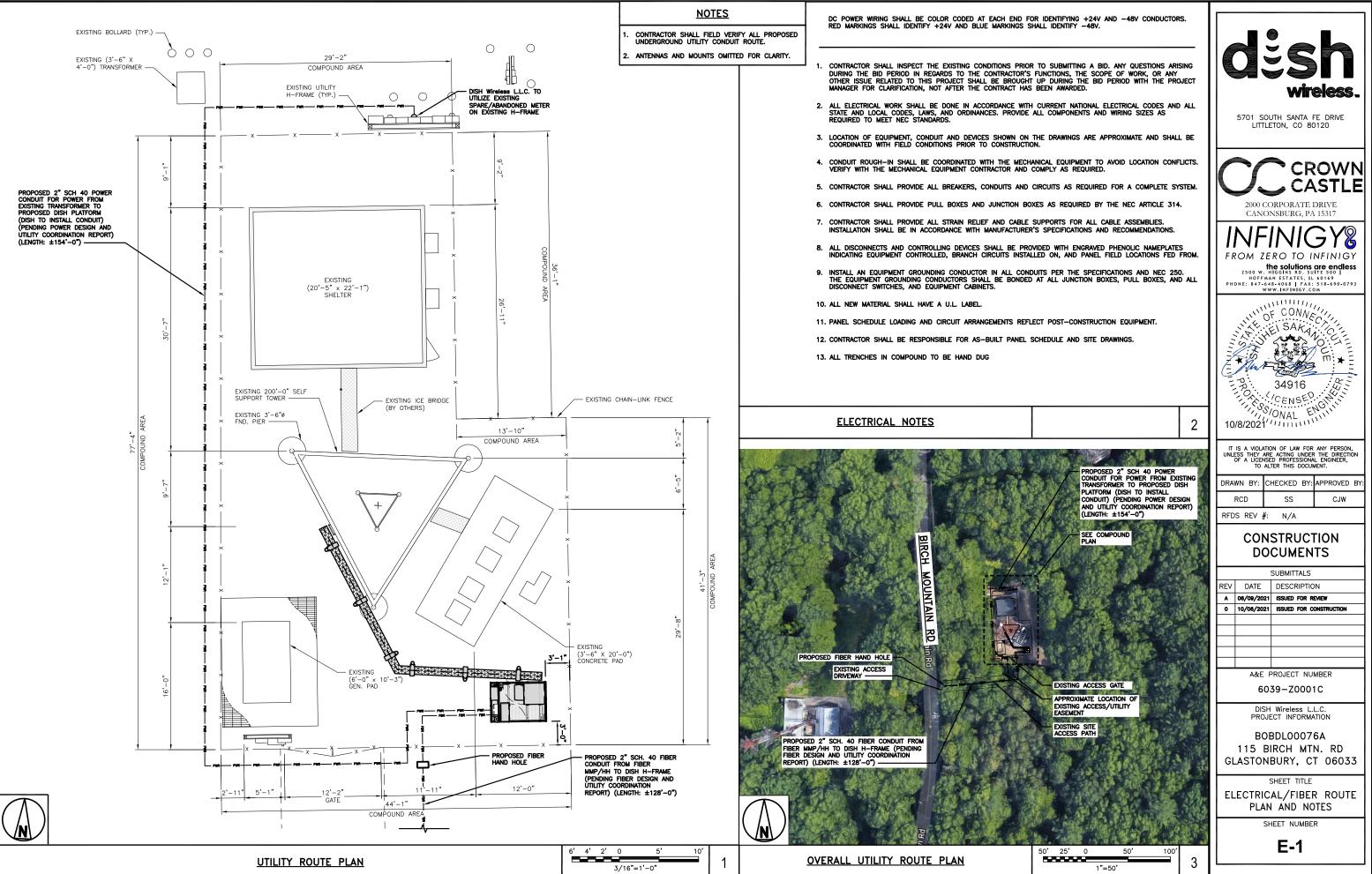


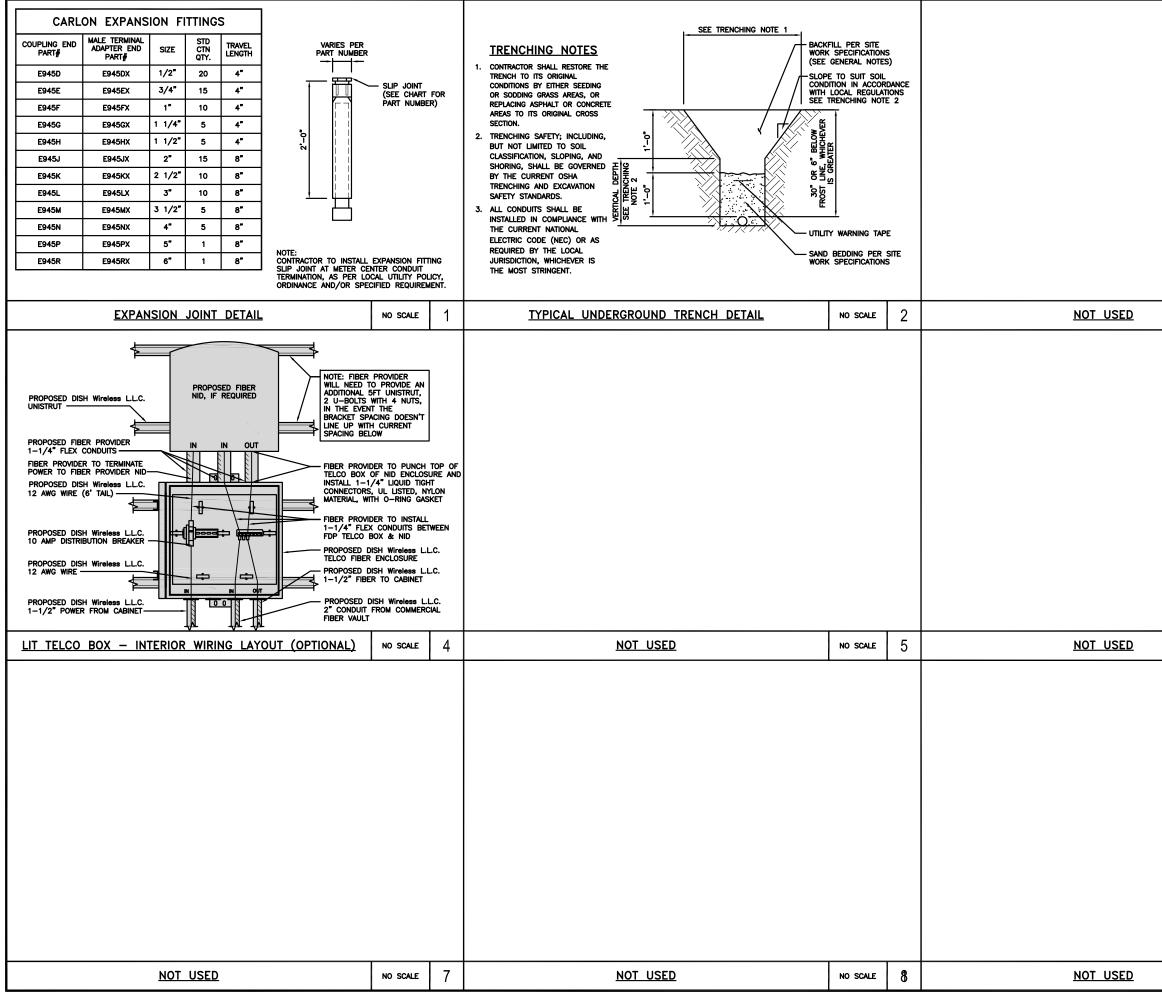


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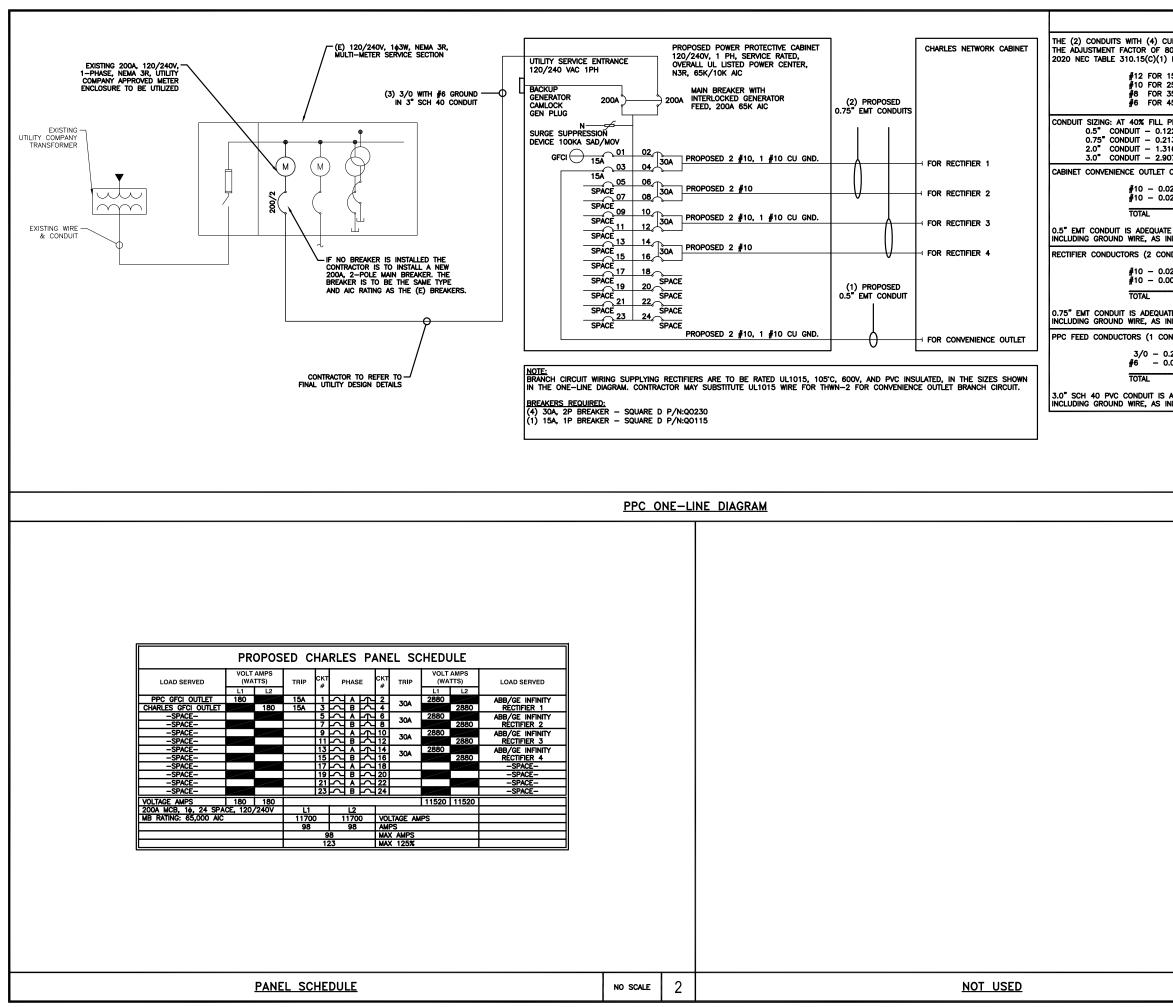




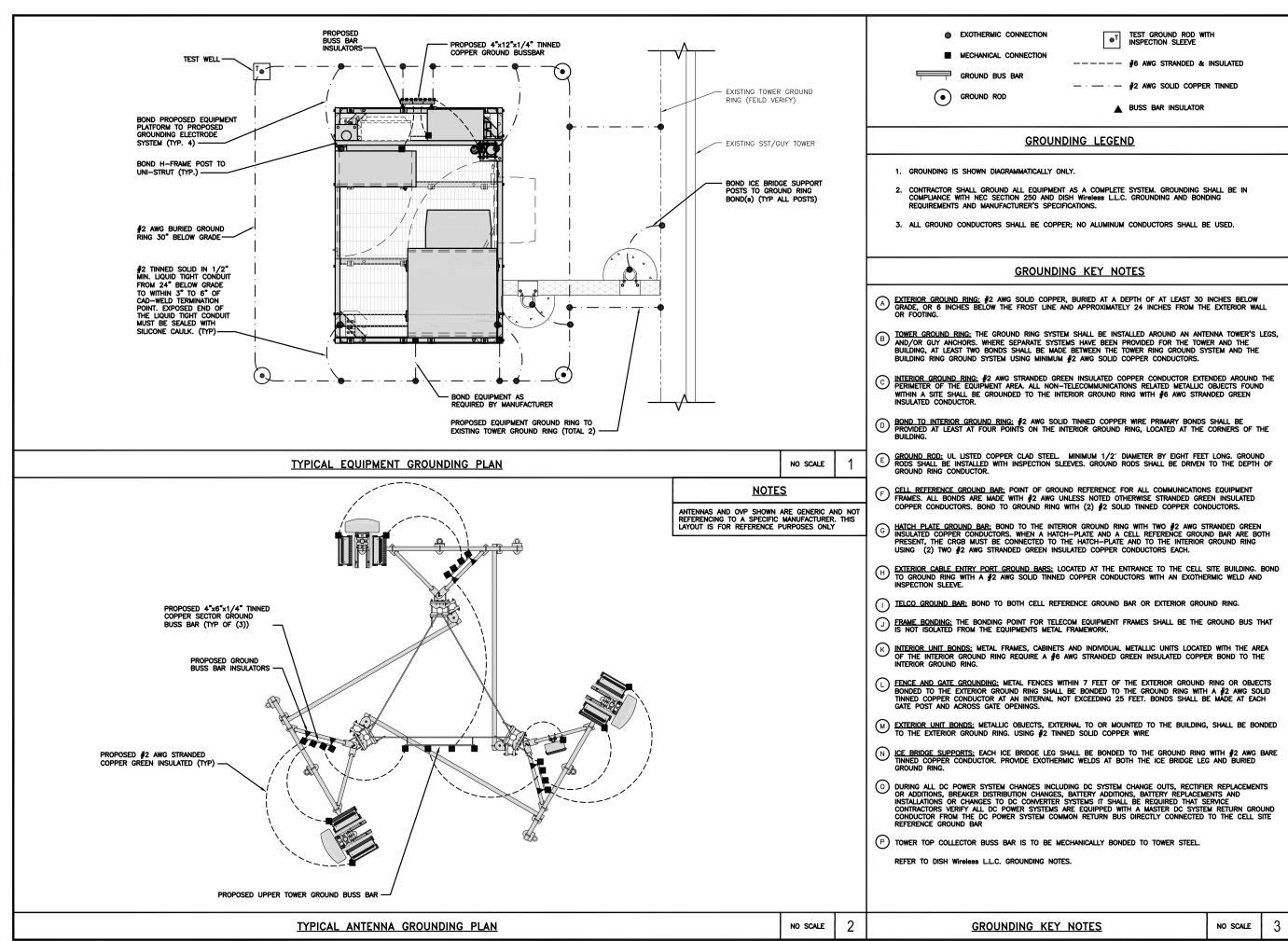




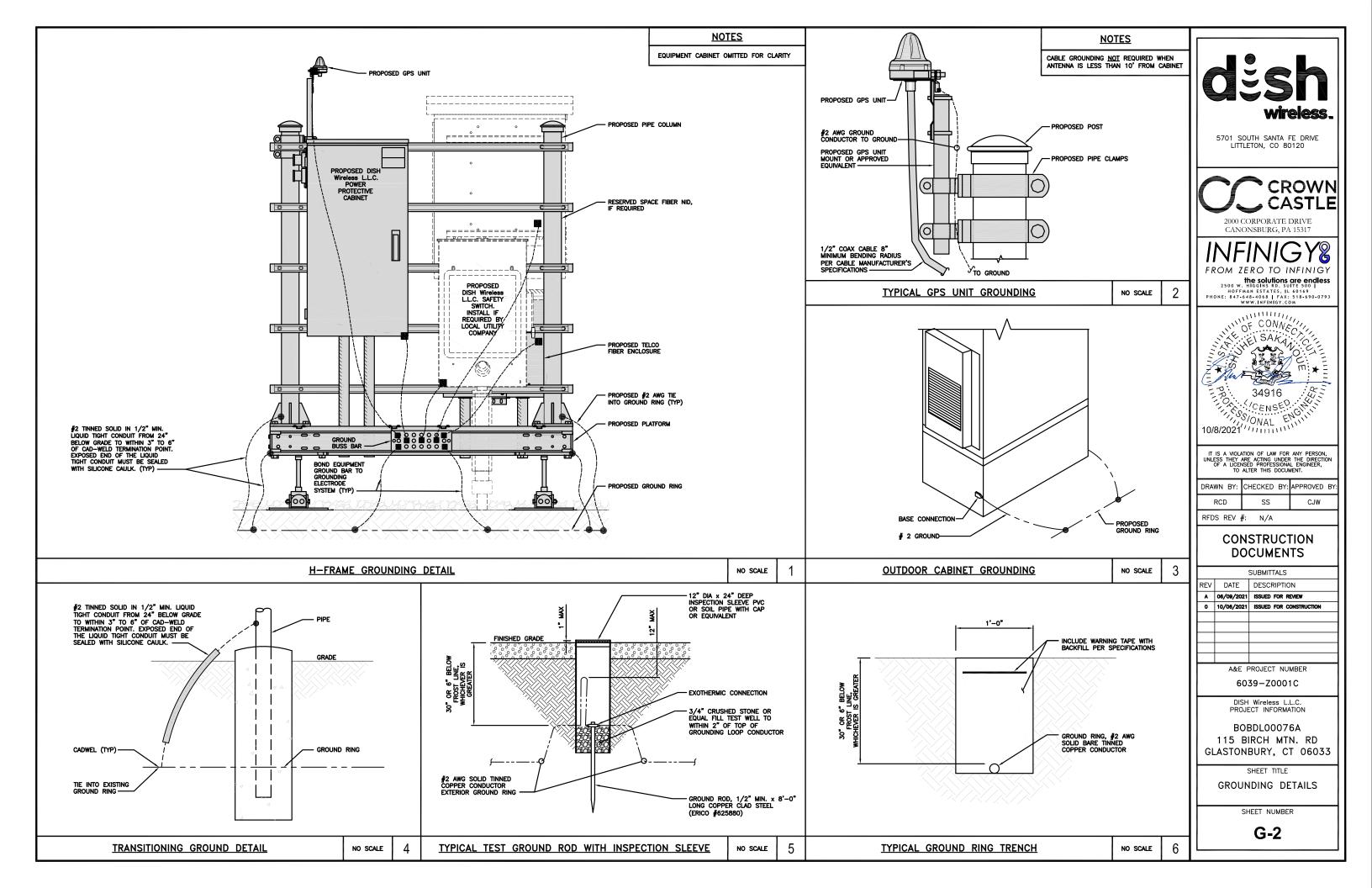
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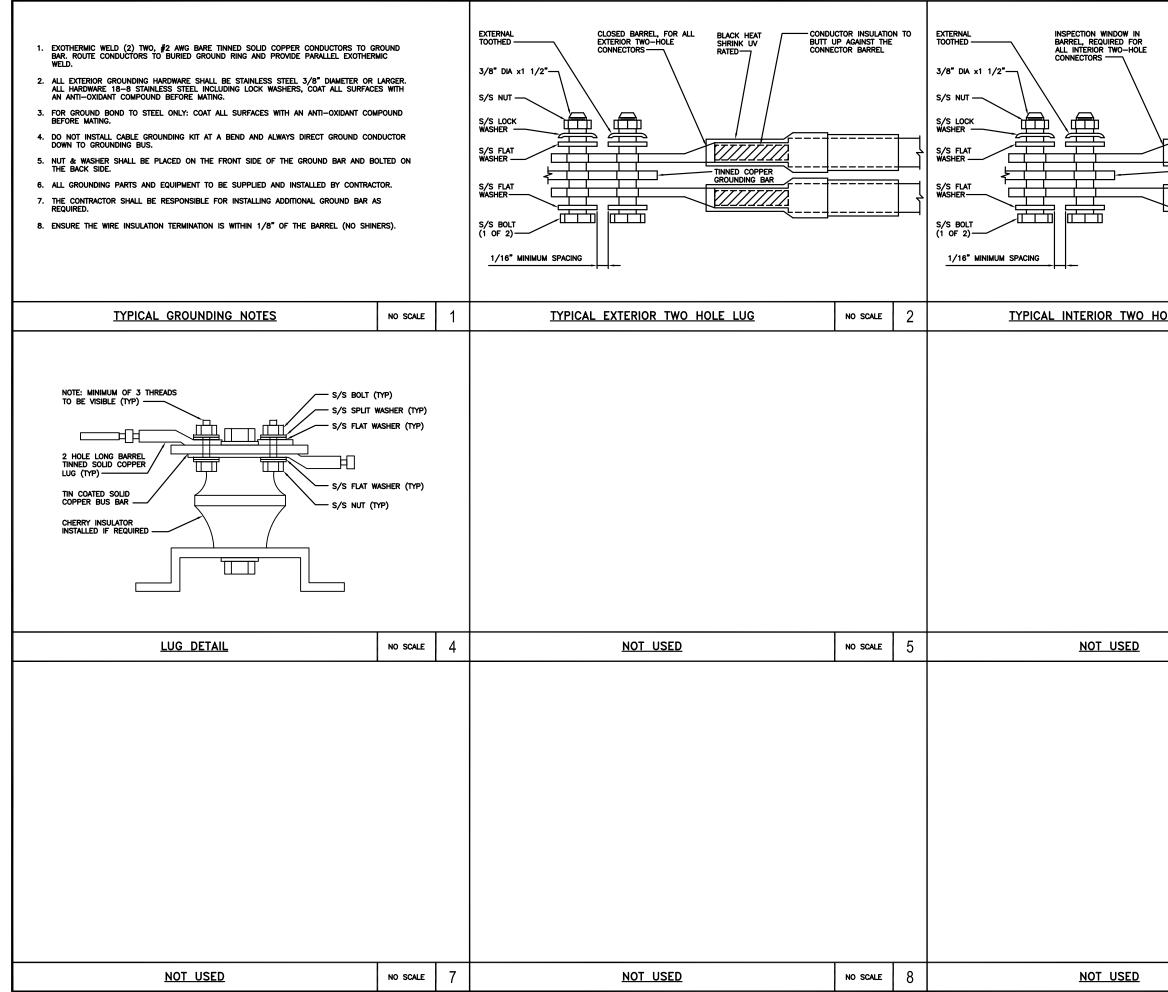


<u>NOTES</u>			
CURRENT CARRYING CONDUCTORS 80% PER 2014/17 NEC TABLE 3 1) FOR UL1015 WIRE.			
15A-20A/1P BREAKER: 0.8 × 3 25A-30A/2P BREAKER: 0.8 × 4 35A-40A/2P BREAKER: 0.8 × 5 45A-60A/2P BREAKER: 0.8 × 7	0A = 32.0A 5A = 44.0A		aisn
PER NEC CHAPTER 9, TABLE 4, 122 SQ. IN AREA 213 SQ. IN AREA 316 SQ. IN AREA	ARTICLE 358.		5701 SOUTH SANTA FE DRIVE
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= 0.0633 SQ.	ĪN		
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0.0266 SQ. IN X 4 = 0.1064 SQ. 0.0082 SQ. IN X 1 = 0.0082 SQ.		UND	INFINIGY [®]
= 0.1146 SQ.	ĪN		FROM ZERO TO INFINIGY
JATE TO HANDLE THE TOTAL OF (5 INDICATED ABOVE. CONDUIT): USING THWN, CU.	// ₩IREO,		the solutions are endless 2500 W. HIG GINS RO. SUITE 500 J HOFFMAN ESTATES, IL 60169 PHONE: 847-648-4068 J FAX: 518-630-0793 WWW.INFINICY.COM
0.2679 SQ. IN X 3 = 0.8037 SQ 0.0507 SQ. IN X 1 = 0.0507 SQ			NUCE CONNOUS
= 0.8544 SC			OF SAKA
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			- Mut the materia
			177 34916 (J)
			S/ONAL ENGLIN
	NO SCALE	1	10/8/2021
			IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.
			DRAWN BY: CHECKED BY: APPROVED BY:
			RCD SS CJW
			RFDS REV #: N/A
			CONSTRUCTION DOCUMENTS
			REV DATE DESCRIPTION A 06/09/2021 ISSUED FOR REVIEW
			0 10/06/2021 ISSUED FOR CONSTRUCTION
			A&E PROJECT NUMBER 6039-Z0001C
			DISH Wireless L.L.C.
			PROJECT INFORMATION BOBDL00076A
			115 BIRCH MTN. RD GLASTONBURY, CT 06033
			SHEET TITLE ELECTRICAL ONE-LINE, FAULT CALCS & PANEL SCHEDULE
			SHEET NUMBER
			E-3
	NO SCALE	3	



L									
	dësh wireless.								
1	5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120								
	CC CROWN CASTLE								
	CANONSBURG, PA 15317								
	FROM ZERO TO INFINIGY the solutions are endless 2500 W. HIGGINS RD, SUITE 500 I								
I	2500 W. HIGGINS RD. SUITE 500 I HOFFMAN ESTATES, LL 60169 PHONE: 847-648-4068 FAX: 518-690-0793 WWW.INFINIGY.COM								
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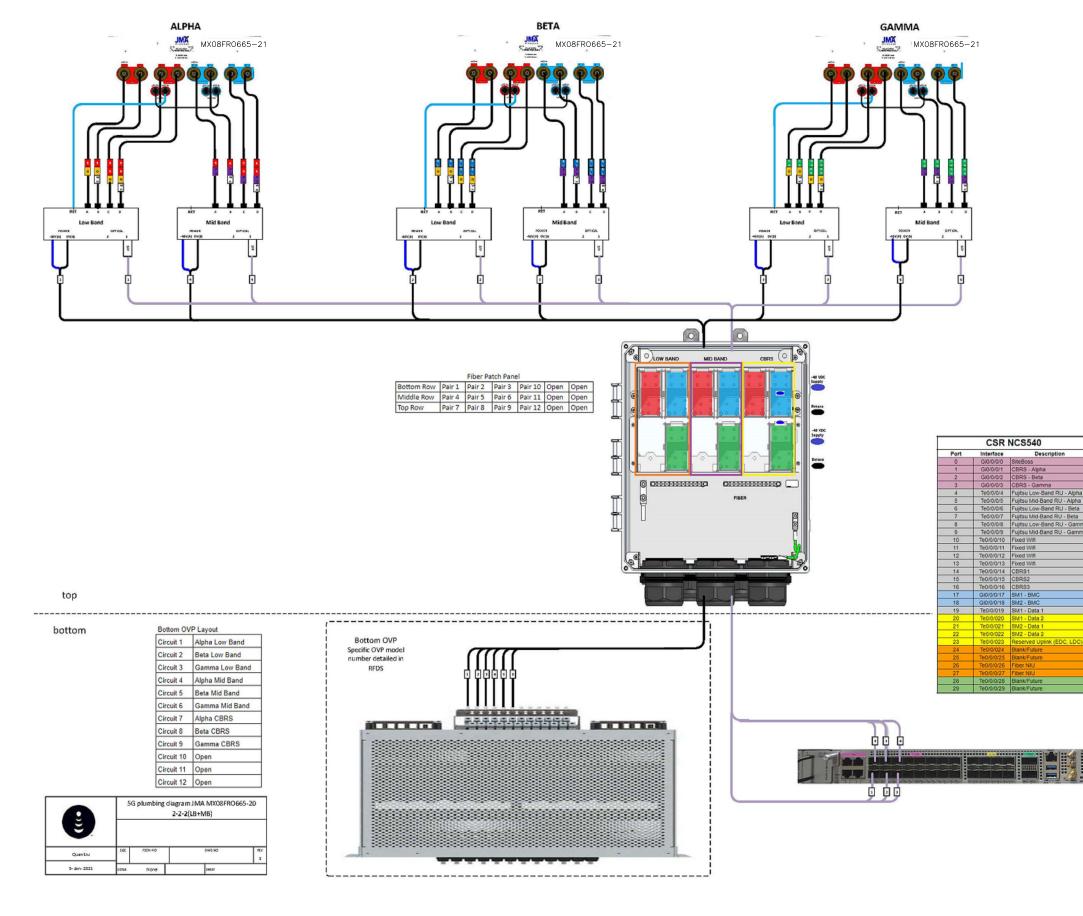




SHRINK / BUTT L	CTOR INSULATION IP AGAINST THE CTOR BARREL	, ∎ 1	STOL SOUTH SANTA FE DRIVE LITTLETON, CO 80120
- TINNED COPPER		- 	
		4	CROWN
		╧╧┙	
			INFINIGY [®]
			FROM ZERO TO INFINIGY
LE LUG	NO SCALE	3	the solutions are endless 2500 W. HIGGINS RD. SUITE 500 HOFFMAN ESTATES, IL 60169 PHONE: 847-648-4068 FAX: 518-690-0793 WWW.INFINIGY.COM
			DE 34916
			IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.
			DRAWN BY: CHECKED BY: APPROVED BY:
			RCD SS CJW
			RFDS REV #: N/A
			CONSTRUCTION DOCUMENTS
	NO SCALE	6	SUBMITTALS
			REV DATE DESCRIPTION
			A 06/09/2021 ISSUED FOR REVIEW 0 10/06/2021 ISSUED FOR CONSTRUCTION
			A&E PROJECT NUMBER
			6039-Z0001C
			DISH Wireless L.L.C. PROJECT INFORMATION
			BOBDL00076A 115 BIRCH MTN. RD GLASTONBURY, CT 06033
			SHEET TITLE GROUNDING DETAILS
			SHEET NUMBER
			G-3
	NO SCALE	9	

					1			
RF JUMPER COLOR CODING		3/4" TAPE WIDTHS WITH 3/4" SPACING						
LOW-BAND RRH - (600MHz N71 BASEBAND) + (850MHz N26 BAND) + (700MHz N29 BAND) - OPTIONAL PER MARKET	ALPHA RRH PORT 1 + SLANT - SLANT + SLANT - SLANT RED RED RED RED RED RED	BETA RRH PORT 1 PORT 2 PORT 3 PORT 4 + SLANT - SLANT + SLANT - SLANT BLUE BLUE BLUE BLUE	PORT 1 PORT 2	AA RRH PORT 3 PORT 4 + SLANT GREEN GREEN		LOW BANDS (N71+N OPTIONAL - (N29 ORANGE		AWS (N66+N70+H-BLO PURPLE
ADD FREQUENCY COLOR TO SECTOR BAND (CBRS WILL USE YELLOW BANDS)	ORANGE ORANGE RED RED WHITE () PORT ORANGE ORANGE ORANGE WHITE () PORT WHITE () PORT ORANGE WHITE () PORT	ORANGE ORANGE BLUE WHITE (-) PORT ORANGE ORANGE WHITE (-) PORT ORANGE WHITE (-) PORT	ORANGE ORANGE	GREEN GREEN ORANGE ORANGE WHITE (-) PORT		CBRS TECH (3 GHz) YELLOW		NEGATIVE SLANT PO ON ANT/RH WHITE
MID-BAND RRH – (AWS BANDS N66+N70)	RED RED RED RED PURPLE PURPLE RED RED	BLUE BLUE BLUE BLUE PURPLE BLUE	GREEN GREEN PURPLE PURPLE			ALPHA SECTOR	BETA SECTOR	САММ
ADD FREQUENCY COLOR TO SECTOR BAND (CBRS WILL USE YELLOW BANDS)	(-) PORT PURPLE PURPLE WHITE (-) PORT	WHITE		PURPLE PURPLE		<u>COLOR IDENTIF</u>	IER	NO SCAL
HYBRID/DISCREET CABLES	EXAMPLE 1 EXAMPLE 2	EXAMPLE 3						
NCLUDE SECTOR BANDS BEING SUPPORTED ALONG WITH FREQUENCY BANDS	RED RED BLUE BLUE	RED						
EXAMPLE 1 – HYBRID, OR DISCREET, SUPPORTS ALL SECTORS, BOTH LOW-BANDS AND MID-BANDS	GREEN GREEN	ORANGE PURPLE						
EXAMPLE 2 – HYBRID, OR DISCREET, SUPPORTS SBRS ONLY, ALL SECTORS	PURPLE							
FIBER JUMPERS TO RRHS LOW-BAND RRH FIBER CABLES HAVE SECTOR STRIPE ONLY	LOW BAND RRH HIGH BAND RRH RED RED PURPLE	LOW BAND RRH HIGH BAND RRH BLUE BLUE PURPLE	LOW BAND RRH	GREEN				
POWER CABLES TO RRHs	LOW BAND RRH HIGH BAND RRH	LOW BAND RRH HIGH BAND RRH	LOW BAND RRH	HIGH BAND RRH				
LOW-BAND RRH POWER CABLES HAVE SECTOR STRIPE ONLY	RED RED PURPLE	BLUE BLUE PURPLE	GREEN	GREEN PURPLE		<u>NOT USED</u>		NO SCAI
RET MOTORS AT ANTENNAS	ANTENNA 1 ANTENNA 1 LOW BAND/ HIGH BAND/ "IN" "IN" RED RED PURPLE	ANTENNA 1 LOW BAND/ "IN" BLUE BLUE PURPLE	ANTENNA 1 ANTEN LOW BAND/ HIGH B "N" "IN GREEN GRE	EN				
			ARD AZIMUTH OF 240-3					
LINKS WILL HAVE A 1.5-2 INCH WHITE WRAP WITH THE AZIMUTH COLOR OVERLAPPING IN THE MIDDLE. ADD ADDITIONAL SECTOR COLOR BANDS FOR EACH ADDITIONAL MW RADIO. MICROWAVE CABLES WILL REQUIRE P-TOUCH LABELS INSIDE THE CABINET TO IDENTIFY THE LOCAL AND REMOTE SITE ID'S	PRIMARY SECONDARY WHITE WHITE RED RED WHITE RED RED RED RED RED RED	PRIMARY SECONDARY WHITE WHITE BLUE BLUE WHITE BLUE BLUE BLUE	PRIMARY SECON WHITE WHI GREEN GRE WHITE WHI	TE N TE EN				
	WHITE	WHITE	WHI	TE				

LOW BANDS (N71+N26) OPTIONAL - (N29) ORANGE CBRS TECH (3 GHz) YELLOW PHA SECTOR BETA SECTOR RED BLUE	AWS (N66+N70+H-BLOCK) PURPLE NEGATIVE SLANT PORT ON ANT/RRH WHITE OR GAMMA SECTOR	_	CORPORATE DRIVE CONCORPORATE DRIVE CONCORPORATE DRIVE CANONSBURG, PA 15317
COLOR IDENTIFIER	NO SCALE	2	FROM ZERO TO INFINIGY the solutions are enalless 2500 w. HigGins RD. Sulte 500 I HOFFMAN ESTATES, IL 60169 PHONE: 847-648-068 J FAX: 518-690-0793
			IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A UIOANIA ENGINEER, 10/8/2021
NOT USED	NO SCALE	3	SUBMITTALS
			REV DATE DESCRIPTION A 06/09/2021 ISSUED FOR REVIEW 0 10/06/2021 ISSUED FOR CONSTRUCTION 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
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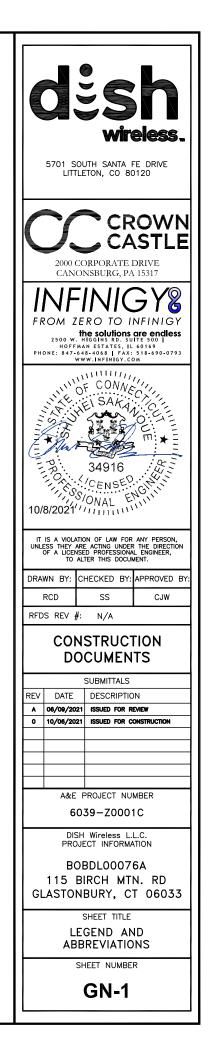
PLUMBING DIAGRAM

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EXOTHERMIC CONNECTION	•
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BUSS BAR INSULATOR	
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SINGLE POLE SWITCH	\$
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FLUORESCENT LIGHTING FIXTURE (2) TWO LAMPS 48	Б-ТВ Г Г Г С Г Г Г Г С Г Г Г Г
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WATER LINE	— w — w — w — w — w —
UNDERGROUND POWER	UGP UGP UGP UGP
UNDERGROUND TELCO	UGT UGT UGT UGT
OVERHEAD POWER	OHP OHP OHP
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UNDERGROUND TELCO/POWER	UGT/P UGT/P UGT/P
ABOVE GROUND POWER	AGP AGP AGP AGP
ABOVE GROUND TELCO	AGT AGT AGT AGT
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AB	ANCHOR BOLT	IN
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AC	ALTERNATING CURRENT	LB(S)
ADDL	ADDITIONAL	LF
AFF AFG	ABOVE FINISHED FLOOR ABOVE FINISHED GRADE	LTE
AGL	ABOVE FINISHED GRADE	MAS MAX
AIC	AMPERAGE INTERRUPTION CAPACITY	MB
ALUM	ALUMINUM	MECH
ALT	ALTERNATE	MFR
ANT	ANTENNA	MGB
APPROX ARCH	APPROXIMATE ARCHITECTURAL	MIN MISC
ATS	AUTOMATIC TRANSFER SWITCH	MISC
AWG	AMERICAN WIRE GAUGE	MTS
BATT	BATTERY	MW
BLDG	BUILDING	NEC
BLK BLKG	BLOCK BLOCKING	NM NO.
BM	BEAM	NU. #
BTC	BARE TINNED COPPER CONDUCTOR	# NTS
BOF	BOTTOM OF FOOTING	OC
CAB	CABINET	OSHA
CANT CHG	CANTILEVERED CHARGING	OPNG
CLG	CEILING	P/C
CLR	CLEAR	PCS PCU
COL	COLUMN	PRC
COMM	COMMON	PP
CONC CONSTR	CONCRETE CONSTRUCTION	PSF
DBL	DOUBLE	PSI
DC	DIRECT CURRENT	PT
DEPT	DEPARTMENT	PWR QTY
DF	DOUGLAS FIR	RAD
DIA DIAG	DIAMETER	RECT
DIAG	DIAGONAL DIMENSION	REF
DWG	DRAWING	REINF
DWL	DOWEL	REQ'D
EA	EACH	RET RF
EC		RMC
el. Elec	ELEVATION ELECTRICAL	RRH
EMT	ELECTRICAL METALLIC TUBING	RRU
ENG	ENGINEER	RWY
EQ	EQUAL	SCH SHT
EXP EXT	EXPANSION EXTERIOR	SIAD
EW	EACH WAY	SIM
FAB	FABRICATION	SPEC
FF	FINISH FLOOR	SQ SS
FG	FINISH GRADE	STD
FIF	FACILITY INTERFACE FRAME	STL
fin Flr	FINISH(ED) FLOOR	TEMP
FDN	FOUNDATION	THK
FOC	FACE OF CONCRETE	TMA TN
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FT	FOOT	TOP
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GFCI	GROUND FAULT CIRCUIT INTERRUPTER	UG
GLB GLV	GLUE LAMINATED BEAM GALVANIZED	UL
GPS	GLOBAL POSITIONING SYSTEM	UNO
GND	GROUND	UMTS
GSM	GLOBAL SYSTEM FOR MOBILE	UPS VIF
HDG	HOT DIPPED GALVANIZED	W
hdr Hgr	HEADER HANGER	 W/
HVAC	HANGER HEAT/VENTILATION/AIR CONDITIONING	WD
нт	HEIGHT	WP
IGR	INTERIOR GROUND RING	WT

INCH INTERIOR POUND(S) LINEAR FEET LONG TERM EVOLUTION MASONRY MAXIMUM MACHINE BOLT MECHANICAL MANUFACTURER MASTER GROUND BAR MINIMUM MISCELLANEOUS METAL MANUAL TRANSFER SWITCH MICROWAVE NATIONAL ELECTRIC CODE NEWTON METERS NUMBER NUMBER NOT TO SCALE ON-CENTER OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION OPENING PRECAST CONCRETE PERSONAL COMMUNICATION SERVICES PRIMARY CONTROL UNIT PRIMARY RADIO CABINET POLARIZING PRESERVING POUNDS PER SQUARE FOOT POUNDS PER SQUARE INCH PRESSURE TREATED POWER CABINET QUANTITY RADIUS RECTIFIER REFERENCE REINFORCEMENT REQUIRED REMOTE ELECTRIC TILT RADIO FREQUENCY RIGID METALLIC CONDUIT REMOTE RADIO HEAD REMOTE RADIO UNIT RACEWAY SCHEDULE SHEET SMART INTEGRATED ACCESS DEVICE SIMILAR SPECIFICATION SQUARE STAINLESS STEEL STANDARD STEEL TEMPORARY THICKNESS TOWER MOUNTED AMPLIFIER TOE NAIL TOP OF ANTENNA TOP OF CURB TOP OF FOUNDATION TOP OF PLATE (PARAPET) TOP OF STEEL TOP OF WALL TRANSIENT VOLTAGE SURGE SUPPRESSION TYPICAL UNDERGROUND UNDERWRITERS LABORATORY UNLESS NOTED OTHERWISE UNIVERSAL MOBILE TELECOMMUNICATIONS SYSTEM UNITERRUPTIBLE POWER SYSTEM (DC POWER PLANT) VERIFIED IN FIELD WIDE WITH WOOD WEATHERPROOF WEIGHT



SITE ACTIVITY REQUIREMENTS:

1. NOTICE TO PROCEED - NO WORK SHALL COMMENCE PRIOR TO CONTRACTOR RECEIVING A WRITTEN NOTICE TO PROCEED (NTP) AND THE ISSUANCE OF A PURCHASE ORDER. PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE DISH Wireless L.L.C. AND TOWER OWNER NOC & THE DISH Wireless L.L.C. AND TOWER OWNER CONSTRUCTION MANAGER.

2. "LOOK UP" - DISH Wireless L.L.C. AND TOWER OWNER SAFETY CLIMB REQUIREMENT:

THE INTEGRITY OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY ON THE STRUCTURE. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO: PINCHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM ITS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTIONAL WEAR, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO IMPEDE/BLOCK ITS INTENDED USE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGGED OUT AND REPORTED TO YOUR DISH Wireless L.L.C. AND DISH Wireless L.L.C. AND TOWER OWNER POC OR CALL THE NOC TO GENERATE A SAFETY CLIMB MAINTENANCE AND CONTRACTOR NOTICE TICKET.

3. PRIOR TO THE START OF CONSTRUCTION, ALL REQUIRED JURISDICTIONAL PERMITS SHALL BE OBTAINED. THIS INCLUDES, BUT IS NOT LIMITED TO, BUILDING, ELECTRICAL, MECHANICAL, FIRE, FLOOD ZONE, ENVIRONMENTAL, AND ZONING. AFTER ONSITE ACTIVITIES AND CONSTRUCTION ARE COMPLETED, ALL REQUIRED PERMITS SHALL BE SATISFIED AND CLOSED OUT ACCORDING TO LOCAL JURISDICTIONAL REQUIREMENTS.

4. ALL CONSTRUCTION MEANS AND METHODS; INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS, AND RESCUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONTAINED HEREIN, AND SHALL MEET ANSI/ASSE A10.48 (LATEST EDITION); FEDERAL, STATE, AND LOCAL REGULATIONS; AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL ADHERE TO ANSI/ASSE A10.48 (LATEST EDITION) AND DISH WIRELESS L.L.C. AND TOWER OWNER STANDARDS, INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION, TO CERTIFY THE SUPPORTING STRUCTURE(S) IN ACCORDANCE WITH ANSI/TIA-322 (LATEST EDITION).

5. ALL SITE WORK TO COMPLY WITH DISH Wireless L.L.C. AND TOWER OWNER INSTALLATION STANDARDS FOR CONSTRUCTION ACTIVITIES ON DISH Wireless L.L.C. AND TOWER OWNER TOWER SITE AND LATEST VERSION OF ANSI/TIA-1019-A-2012 "STANDARD FOR INSTALLATION, ALTERATION, AND MAINTENANCE OF ANTENNA SUPPORTING STRUCTURES AND ANTENNAS."

6. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY DISH Wireless L.L.C. AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.

7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.

8. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.

9. THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES INCLUDING PRIVATE LOCATES SERVICES PRIOR TO THE START OF CONSTRUCTION.

10. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION E) CONSTRUCTION SAFETY PROCEDURES.

11. ALL SITE WORK SHALL BE AS INDICATED ON THE STAMPED CONSTRUCTION DRAWINGS AND DISH PROJECT SPECIFICATIONS, LATEST APPROVED REVISION.

12. CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH AT THE COMPLETION OF THE WORK. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.

13. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF DISH WIRELESS LL.C. AND TOWER OWNER, AND/OR LOCAL UTILITIES.

14. THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE TECHNICAL SPECIFICATION FOR SITE SIGNAGE REQUIRED BY LOCAL JURISDICTION AND SIGNAGE REQUIRED ON INDIVIDUAL PIECES OF EQUIPMENT, ROOMS, AND SHELTERS.

15. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE CARRIER'S EQUIPMENT AND TOWER AREAS.

16. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.

17. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION AS SPECIFIED ON THE CONSTRUCTION DRAWINGS AND/OR PROJECT SPECIFICATIONS.

18. CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.

19. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.

20. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS AND RADIOS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.

21. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.

22. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.

GENERAL NOTES:

1.FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:

CONTRACTOR:GENERAL CONTRACTOR RESPONSIBLE FOR CONSTRUCTION

CARRIER:DISH Wireless L.L.C.

TOWER OWNER:TOWER OWNER

2. THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE ENGINEERS IN THIS OR SIMILAR LOCALITIES. IT IS ASSUMED THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKPEOPLE WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.

3. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY FOR PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, FORMWORK, SHORING, ETC. SITE VISITS BY THE ENGINEER OR HIS REPRESENTATIVE WILL NOT INCLUDE INSPECTION OF THESE ITEMS AND IS FOR STRUCTURAL OBSERVATION OF THE FINISHED STRUCTURE ONLY.

4. NOTES AND DETAILS IN THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE GREATER, MORE STRICT REQUIREMENTS, SHALL GOVERN. IF FURTHER CLARIFICATION IS REQUIRED CONTACT THE ENGINEER OF RECORD.

5. SUBSTANTIAL EFFORT HAS BEEN MADE TO PROVIDE ACCURATE DIMENSIONS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.

6. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CARRIER POC AND TOWER OWNER.

7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.

8. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.

9. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.

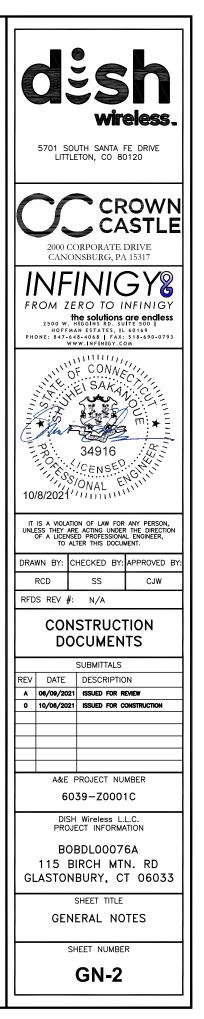
10. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CARRIER AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.

11. CONTRACTOR IS TO PERFORM A SITE INVESTIGATION, BEFORE SUBMITTING BIDS, TO DETERMINE THE BEST ROUTING OF ALL CONDUITS FOR POWER, AND TELCO AND FOR GROUNDING CABLES AS SHOWN IN THE POWER, TELCO, AND GROUNDING PLAN DRAWINGS.

12. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF DISH Wireless L.L.C. AND TOWER OWNER

13. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.

14. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.



CONCRETE, FOUNDATIONS, AND REINFORCING STEEL:

ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST-IN-PLACE CONCRETE.

UNLESS NOTED OTHERWISE, SOIL BEARING PRESSURE USED FOR DESIGN OF SLABS AND FOUNDATIONS IS ASSUMED TO BE 1000 psf.

ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (I'c) OF 3000 psi AT 28 DAYS, UNLESS NOTED OTHERWISE. NO 3. MORE THAN 90 MINUTES SHALL ELAPSE FROM BATCH TIME TO TIME OF PLACEMENT UNLESS APPROVED BY THE ENGINEER OF RECORD. TEMPERATURE OF CONCRETE SHALL NOT EXCEED 90'F AT TIME OF PLACEMENT.

CONCRETE EXPOSED TO FREEZE-THAW CYCLES SHALL CONTAIN AIR ENTRAINING ADMIXTURES, AMOUNT OF AIR ENTRAINMENT TO BE BASED ON SIZE OF AGGREGATE AND F3 CLASS EXPOSURE (VERY SEVERE). CEMENT USED TO BE TYPE II PORTLAND CEMENT WITH A MAXIMUM WATER-TO-CEMENT RATIO (W/C) OF 0.45.

ALL STEEL REINFORCING SHALL CONFORM TO ASTM A615. ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185. ALL SPLICES SHALL BE CLASS "B" TENSION SPLICES, UNLESS NOTED OTHERWISE. ALL HOOKS SHALL BE STANDARD 90 DEGREE HOOKS, UNLESS NOTED OTHERWISE. YIELD STRENGTH (Fy) OF STANDARD DEFORMED BARS ARE AS FOLLOWS:

#4 BARS AND SMALLER 40 ksi

#5 BARS AND LARGER 60 ksi

THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON 6. DRAWINGS:

- CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3"
- CONCRETE EXPOSED TO EARTH OR WEATHER:
- #6 BARS AND LARGER 2"
- #5 BARS AND SMALLER 1-1/2"
- · CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
- SLAB AND WALLS 3/4"
- BEAMS AND COLUMNS 1-1/2*

A TOOLED EDGE OR A 3/4" CHAMFER SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNLESS NOTED OTHERWISE, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.

ELECTRICAL INSTALLATION NOTES:

ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES/ORDINANCES.

CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED AND TRIP HAZARDS ARE ELIMINATED.

- WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC. 3.
- ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC.

ALL EQUIPMENT SHALL BEAR THE UNDERWRITERS LABORATORIES LABEL OF APPROVAL, AND SHALL CONFORM TO REQUIREMENT OF THE NATIONAL ELECTRICAL CODE.

ALL OVERCURRENT DEVICES SHALL HAVE AN INTERRUPTING CURRENT RATING THAT SHALL BE GREATER THAN THE SHORT CIRCUIT CURRENT TO WHICH THEY ARE SUBJECTED, 22,000 AIC MINIMUM. VERIFY AVAILABLE SHORT CIRCUIT CURRENT DOES NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRENT ADOPTED CODE PRE THE GOVERNING JURISDICTION.

EACH END OF EVERY POWER PHASE CONDUCTOR, GROUNDING CONDUCTOR, AND TELCO CONDUCTOR OR CABLE SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2" PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL), THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA.

ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH LAMICOID TAGS SHOWING THEIR RATED VOLTAGE, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING AND BRANCH CIRCUIT ID NUMBERS (i.e. PANEL BOARD AND CIRCUIT ID'S).

7. PANEL BOARDS (ID NUMBERS) SHALL BE CLEARLY LABELED WITH PLASTIC LABELS.

TIE WRAPS ARE NOT ALLOWED.

ALL POWER AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE COPPER CONDUCTOR (#14 OR LARGER) WITH TYPE THHW. THWN. THWN-2. XHHW. XHHW-2. THW. THW-2. RHW. OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.

SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE COPPER CONDUCTOR (#6 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.

POWER AND CONTROL WIRING IN FLEXIBLE CORD SHALL BE MULTI-CONDUCTOR, TYPE SOOW CORD (#14 OR LARGER) UNLESS OTHERWISE SPECIFIED.

POWER AND CONTROL WIRING FOR USE IN CABLE TRAY SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#14 OR LARGER), WITH 12 TYPE THHW. THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.

ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRE NUTS BY THOMAS AND 13 BETTS (OR EQUAL). LUGS AND WIRE NUTS SHALL BE RATED FOR OPERATION NOT LESS THAN 75" C (90" C IF AVAILABLE).

RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.

ELECTRICAL METALLIC TUBING (EMT), INTERMEDIATE METAL CONDUIT (IMC), OR RIGID METAL CONDUIT (RMC) SHALL BE USED FOR 15 EXPOSED INDOOR LOCATIONS.

ELECTRICAL METALLIC TUBING (EMT) OR METAL-CLAD CABLE (MC) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS. 16.

17 SCHEDULE 40 PVC UNDERGROUND ON STRAIGHTS AND SCHEDULE 80 PVC FOR ALL ELBOWS/90s AND ALL APPROVED ABOVE GRADE PVC CONDUIT.

LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION 18. OCCURS OR FLEXIBILITY IS NEEDED.

CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION-TYPE AND APPROVED FOR THE LOCATION USED. SET 19. SCREW FITTINGS ARE NOT ACCEPTABLE.

CABINETS, BOXES AND WIRE WAYS SHALL BE LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND THE 20 NEC.

21 WIREWAYS SHALL BE METAL WITH AN ENAMEL FINISH AND INCLUDE A HINGED COVER. DESIGNED TO SWING OPEN DOWNWARDS (WIREMOLD SPECMATE WIREWAY).

22. SLOTTED WIRING DUCT SHALL BE PVC AND INCLUDE COVER (PANDUIT TYPE E OR EQUAL).

23. CONDUITS SHALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-PERFORATED STRAPS AND HANGERS. EXPLOSIVE DEVICES (i.e. POWDER-ACTUATED) FOR ATTACHING HANGERS TO STRUCTURE WILL NOT BE PERMITTED. CLOSELY FOLLOW THE LINES OF THE STRUCTURE, MAINTAIN CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES. CHANGES IN DIRECTION TO ROUTE AROUND OBSTACLES SHALL BE MADE WITH CONDUIT OUTLET BODIES. CONDUIT SHALL BE INSTALLED IN A NEAT AND WORKMANLIKE MANNER. PARALLEL AND PERPENDICULAR TO STRUCTURE WALL AND CEILING LINES. ALL CONDUIT SHALL BE FISHED TO CLEAR OBSTRUCTIONS. ENDS OF CONDUITS SHALL BE TEMPORARILY CAPPED FLUSH TO FINISH GRADE TO PREVENT CONCRETE, PLASTER OR DIRT FROM ENTERING. CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED MALLEABLE IRON BUSHING ON INSIDE AND GALVANIZED MALLEABLE IRON LOCKNUT ON OUTSIDE AND INSIDE.

EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES AND PULL BOXES SHALL BE GALVANIZED OR EPOXY-COATED SHEET 24. STEEL. SHALL MEET OR EXCEED UL 50 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND NEMA 3 (OR BETTER) FOR EXTERIOR LOCATIONS.

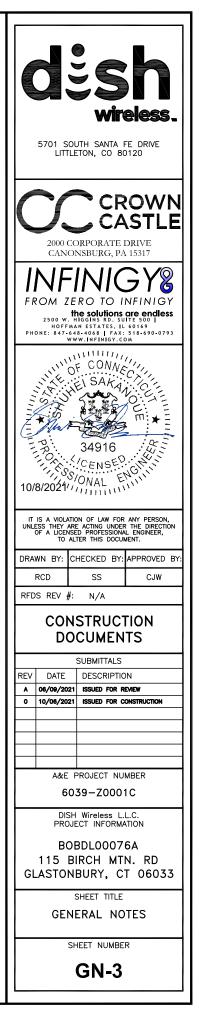
25. METAL RECEPTACLE, SWITCH AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY-COATED OR NON-CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.

NONMETALLIC RECEPTACLE, SWITCH AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2 (NEWEST REVISION) AND BE RATED 26. NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.

THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CARRIER AND/OR DISH Wireless L.L.C. AND 27 TOWER OWNER BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.

THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE 28 WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD LIFE AND PROPERTY.

- 29. INSTALL LAMICOID LABEL ON THE METER CENTER TO SHOW "DISH Wireless L.L.C.".
- 30. ALL EMPTY/SPARE CONDUITS THAT ARE INSTALLED ARE TO HAVE A METERED MULE TAPE PULL CORD INSTALLED.



GROUNDING NOTES:

1. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION AND AC POWER GES'S) SHALL BE BONDED TOGETHER AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.

2. THE CONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS, THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.

3. THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT AND PROVIDE TESTING RESULTS.

4. METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.

5. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.

6. EACH CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 STRANDED COPPER OR LARGER FOR INDOOR BTS; #2 BARE SOLID TINNED COPPER FOR OUTDOOR BTS.

7. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED BACK TO BACK CONNECTIONS ON OPPOSITE SIDE OF THE GROUND BUS ARE PERMITTED.

8. ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING SHALL BE #2 SOLID TINNED COPPER UNLESS OTHERWISE INDICATED.

9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.

10. USE OF 90° BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45° BENDS CAN BE ADEQUATELY SUPPORTED.

11. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.

12. ALL GROUND CONNECTIONS ABOVE GRADE (INTERIOR AND EXTERIOR) SHALL BE FORMED USING HIGH PRESS CRIMPS.

13. COMPRESSION GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.

14. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR.

15. APPROVED ANTIOXIDANT COATINGS (i.e. CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.

16. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.

17. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.

18. BOND ALL METALLIC OBJECTS WITHIN 6 ft OF MAIN GROUND RING WITH (1) #2 BARE SOLID TINNED COPPER GROUND CONDUCTOR.

19. GROUND CONDUCTORS USED FOR THE FACILITY GROUNDING AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (i.e., NONMETALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT.

20. ALL GROUNDS THAT TRANSITION FROM BELOW GRADE TO ABOVE GRADE MUST BE #2 BARE SOLID TINNED COPPER IN 3/4" NON-METALLIC, FLEXIBLE CONDUIT FROM 24" BELOW GRADE TO WITHIN 3" TO 6" OF CAD-WELD TERMINATION POINT. THE EXPOSED END OF THE CONDUIT MUST BE SEALED WITH SILICONE CAULK. (ADD TRANSITIONING GROUND STANDARD DETAIL AS WELL).

21. BUILDINGS WHERE THE MAIN GROUNDING CONDUCTORS ARE REQUIRED TO BE ROUTED TO GRADE, THE CONTRACTOR SHALL ROUTE TWO GROUNDING CONDUCTORS FROM THE ROOFTOP, TOWERS, AND WATER TOWERS GROUNDING RING, TO THE EXISTING GROUNDING SYSTEM, THE GROUNDING CONDUCTORS SHALL NOT BE SMALLER THAN 2/0 COPPER. ROOFTOP GROUNDING RING SHALL BE BONDED TO THE EXISTING GROUNDING SYSTEM, THE BUILDING STEEL COLUMNS, LIGHTNING PROTECTION SYSTEM, AND BUILDING MAIN WATER LINE (FERROUS OR NONFERROUS METAL PIPING ONLY). DO NOT ATTACH GROUNDING TO FIRE SPRINKLER SYSTEM PIPES.

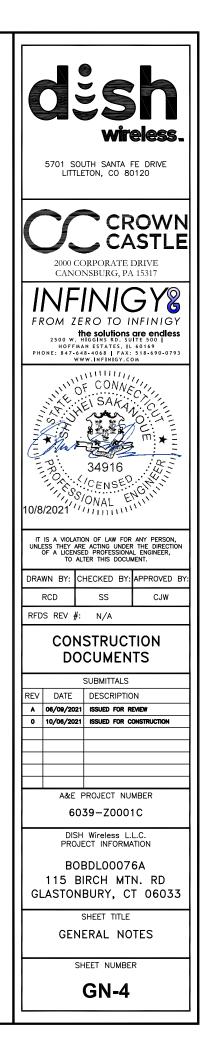


Exhibit D

Structural Analysis Report

Date: June 04, 2021



Crown Castle 2000 Corporate Drive Canonsburg, PA 15317 724-416-2000

Subject:	Structural Analysis Report			
Carrier Designation:	<i>DISH Network</i> Co-Locate Site Number: Site Name:	BOBDL00076A CT-CCI-T-871584		
Crown Castle Designation:	BU Number: Site Name: JDE Job Number: Work Order Number: Order Number:	871584 John Tom Hill 650066 1968782 556617 Rev. 1		
Engineering Firm Designation:	Crown Castle Project Number:	1968782		
Site Data:	115 Birch Mtn. Road, GLASTONBURY, HARTFORD County, CT Latitude <i>41° 42' 32.24"</i> , Longitude -72° 28' 24.41" 200 Foot - Self Support Tower			

Crown Castle is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Proposed Equipment Configuration

Sufficient Capacity-95.9%

This analysis utilizes an ultimate 3-second gust wind speed of 125 mph as required by the 2018 Connecticut Building Code. Applicable Standard references and design criteria are listed in Section 2 - "Analysis Criteria".

Structural analysis prepared by: Subhash Mandal

Respectfully submitted by:

Maham Barimani, P.E. Senior Project Engineer



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1) INTRODUCTION

This tower is a 200 ft Self Support tower designed by SABRE COMMUNICATIONS. The tower has been modified in the past to accommodate additional loading.

2) ANALYSIS CRITERIA

TIA-222 Revision: Risk Category:	TIA-222-H II
Wind Speed:	125 mph
Exposure Category:	С
Topographic Factor:	1
Ice Thickness:	2 in
Wind Speed with Ice:	50 mph
Service Wind Speed:	60 mph

Table 1 - Proposed Equipment Configuration

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
		3	fujitsu	TA08025-B604		
		3	fujitsu	TA08025-B605		
134.0	134.0	3	jma wireless	MX08FRO665-21 w/ Mount Pipe	1	1-1/2
		1	raycap	RDIDC-9181-PF-48		
		1	tower mounts	Commscope MTC3975083 (3)		

Table 2 - Other Considered Equipment

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	
	208.0	1	rfs celwave	ALR10-O			
		1	decibel	DB225-A			
198.0	205.0	1	rfs celwave	PD1107-1	2	1/2	
190.0		1	rfs celwave	PD201-7	3	7/8	
	204.0	1	scala	OGB6-928N			
	198.0	1	tower mounts	Sector Mount [SM 702-3]			
	190.0	190.0	3	commscope	USX6-6W-6GR	10	
190.0			190.0	6	saf	MXM REPEATER MK2	18
		3 tower mounts Pipe Mount [PM 601-1]			1/2		
) 183.0	3	ericsson	AIR6449 B41_T-MOBILE w/ Mount Pipe			
		3	ericsson	RADIO 4415 B66A_CCIV3			
		3	ericsson	RADIO 4424 B25_TMO	-		
182.0		3	ericsson	RADIO 4449 B12/B71	6	1-5/8	
102.0		3	rfs celwave	APX16DWV-16DWV-S-E-A20 w/ Mount Pipe		1-5/6	
		3	rfs celwave	APXVAARR24_43-U-NA20 w/ Mount Pipe			
	182.0	3	-	SitePro STK-U Stiff Arm Kit			

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)			
		1	tower mounts	Sector Mount [SM 702-3]					
		3	alcatel lucent	PCS 1900MHZ 4X45W-65MHZ					
		6	alcatel lucent	RRH2X50-800					
	171.0	3	alcatel lucent	TD-RRH8X20-25					
170.0	171.0	171.0	171.0	171.0	3	commscope	NNVV-65B-R4 w/ Mount Pipe	4	1-1/4
		3	rfs celwave	APXVTM14-ALU-I20 w/ Mount Pipe	-				
	170.0	1	tower mounts	Sector Mount [SM 506-3]	-				
163.0	400.0		kathrein	PR-850	4	1/2			
103.0	163.0	1	tower mounts	Pipe Mount [PM 601-1]		1/2			
	155.0	1	sinclair	SRL480N1DT4		4.10			
144.0	152.0	152.0 2 rfs celwave PD1109-1		3	1/2 7/8				
	144.0	1	tower mounts	Sector Mount [SM 702-3]	_ _	170			
E2 0	55.0 1 lucent		lucent	KS24019-L112A	1	1/2			
53.0	53.0	1	tower mounts	mounts Side Arm Mount [SO 202-1]		1/2			

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Reference	Source
4-GEOTECHNICAL REPORTS	1404208	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	2068370	CCISITES
4-TOWER MANUFACTURER DRAWINGS	1403674	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	9122283	CCISITES
4-POST-MODIFICATION INSPECTION	9366487	CCISITES

3.1) Analysis Method

tnxTower (version 8.0.9.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

3.2) Assumptions

- 1) Tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Section No.		Component Type	Size	Critical Element	Р (К)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	200 - 180	Leg	Sabre 2.875x.375	3	-28.43	100.37	28.3	Pass
T2	180 - 160	Leg	Sabre 3.5 x .3	33	-79.49	116.34	68.3	Pass
Т3	160 - 140	Leg	Sabre 4 x .318	60	-122.80	149.09	82.4	Pass
T4	140 - 120	Leg	Sabre 4.5 x .438	87	-164.75	211.28	78.0	Pass
T5	120 - 100	Leg	Sabre 5.5625 x .375	108	-204.37	251.62	81.2	Pass
Т6	100 - 80	Leg	Sabre 5.5625 x .375	129	-241.22	251.62	95.9	Pass
T7	80 - 60	Leg	Sabre 6.625 x .432	150	-273.59	319.52	85 <u>.</u> 6	Pass
Т8	60 - 40	Leg	Sabre 8.625 x .322	165	-307.43	351.50	87.5	Pass
Т9	40 - 20	Leg	Sabre 8.625 x .5	180	-340.99	531.40	64.2	Pass
T10	20 - 0	Leg	Sabre 8.625 x .5	195	-373.44	531.40	70.3	Pass
T1	200 - 180	Diagonal	L1 3/4x1 3/4x3/16	10	-5.37	13.85	38.8	Pass
T2	180 - 160	Diagonal	L1 3/4x1 3/4x3/16	37	-5.78	9.47	61.0	Pass
Т3	160 - 140	Diagonal	L1 3/4x1 3/4x3/16	64	-5.95	6.54	91.0	Pass
T4	140 - 120	Diagonal	L2 1/2x2 1/2x3/16	91	-7.23	12.36	58.5	Pass
T5	120 - 100	Diagonal	L2 1/2x2 1/2x3/16	113	-7.32	9.61	76.2	Pass
Т6	100 - 80	Diagonal	L3x3x3/16	134	-7.66	13.18	58.1	Pass
T7	80 - 60	Diagonal	L3 1/2x3 1/2x1/4	155	-8.88	18.99	46.7	Pass
Т8	60 - 40	Diagonal	L3 1/2x3 1/2x1/4	170	-9.50	16.23	58.5	Pass
Т9	40 - 20	Diagonal	L3 1/2x3 1/2x1/4	185	-9.94	13.73	72.4	Pass
T10	20 - 0	Diagonal	L4x4x1/4	199	-10.76	17.67	60.9	Pass
T1	200 - 180	Top Girt	L1 3/4x1 3/4x3/16	4	-0.40	7.66	5.3	Pass
							Summary	
						Leg (T6)	95.9	Pass
						Diagonal (T3)	91.0	Pass
						Top Girt (T1)	5.3	Pass
						Bolt Checks	71.7	Pass
						Rating =	95.9	Pass

Table 5 - Towe	r Component Stresses	vs. Capacity - LC7
----------------	----------------------	--------------------

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	58.8	Pass
1	Base Foundation (Structure)	0	73.5	Pass
1	Base Foundation (Soil Interaction)	0	69.2	Pass

Structure Rating (max from all components) =	95.9%

Notes:

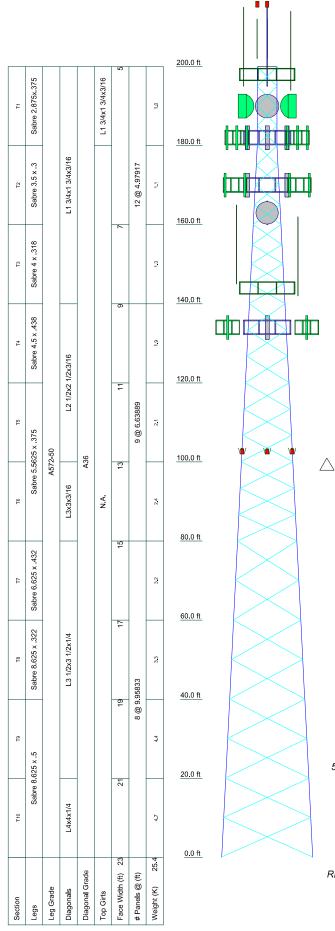
1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

APPENDIX A

TNXTOWER OUTPUT



		MATERIAL	STRENG	тн	
GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi
	•				

TOWER DESIGN NOTES

- Tower is located in Hartford County, Connecticut.
 Tower designed for Exposure C to the TIA-222-H Standard.

3. Tower designed for a 125 mph basic wind in accordance with the TIA-222-H Standard. 4. Tower is also designed for a 50 mph basic wind with 2.00 in ice. Ice is considered to increase in thickness with height.

Deflections are based upon a 60 mph wind.
 Tower Risk Category II.

Topographic Category 1 with Crest Height of 0.00 ft
 TOWER RATING: 95.9%

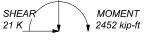
	Crown Castle	^{Job:} B	U 871584			
CROWN		Project				
CASILE	Canonsburg, PA 15317	Client:	Crown Castle	^{Drawn by:} SMandal	App'd:	
The Pathway to Possible	Phone: 724-416-2000	Code:	TIA-222-H	^{Date:} 06/04/21	Scale:	NTS
The Full may to Foodblo	FAX: -	Path:	:\Users\smandal\Desktop\WIP\8715	34\WO 1968782 - SA\Prod\871584 RPA.er	Dwg N	^{o.} E-1

ALL REACTIONS ARE FACTORED

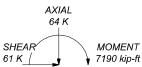
MAX. CORNER REACTIONS AT BASE: DOWN: 381 K SHEAR: 38 K

UPLIFT: -328 K SHEAR: 34 K





TORQUE 9 kip-ft 50 mph WIND - 2.000 in ICE





Tower Input Data

The main tower is a 3x free standing tower with an overall height of 200.00 ft above the ground line. The base of the tower is set at an elevation of 0.00 ft above the ground line. The face width of the tower is 5.000 ft at the top and 23.000 ft at the base. This tower is designed using the TIA-222-H standard. The following design criteria apply: Tower is located in Hartford County, Connecticut. Tower base elevation above sea level: 878.00 ft. • • Basic wind speed of 125 mph. **Risk Category II.** • Exposure Category C. Simplified Topographic Factor Procedure for wind speed-up calculations is used. • Topographic Category: 1. • Crest Height: 0.00 ft. • Nominal ice thickness of 2.000 in. • Ice thickness is considered to increase with height. Ice density of 56 pcf. • A wind speed of 50 mph is used in combination with ice. • Temperature drop of 50 °F. Deflections calculated using a wind speed of 60 mph. • Pressures are calculated at each section. • Stress ratio used in tower member design is 1. • Tower analysis based on target reliabilities in accordance with Annex S. •

- Load Modification Factors used: Kes(Fw) = 0.95, Kes(ti) = 0.85.
- Maximum demand-capacity ratio is: 1.05.
- Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

 √ Use Code Stress Ratios
 √ Use Code Safety Factors - Guys Escalate Ice
 Always Use Max Kz
 Use Special Wind Profile

Include Bolts In Member Capacity

Leg Bolts Are At Top Of Section √ Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area
- √ Use Clear Spans For KL/r
- Retension Guys To Initial Tension √ Bypass Mast Stability Checks
- $\sqrt{}$ Bypass Mast Stability Checks $\sqrt{}$ Use Azimuth Dish Coefficients
- √ Use Azimuth Dish Coefficients √ Project Wind Area of Appurt.

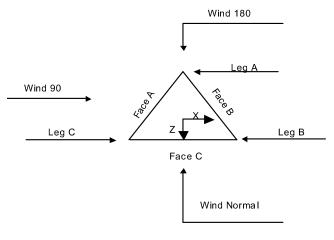
Autocalc Torque Arm Areas

Add IBC .6D+W Combination √ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs Use ASCE 10 X-Brace Ly Rules

- √ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA
- √ SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation
- $\sqrt{\text{Consider Feed Line Torque}}$
- ✓ Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice Exemption

Poles

Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known



<u>Triangular Tower</u>

	Tower Section Geometry											
Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of	Section Length						
	ft			ft	Sections	ft						
T1	200.00-180.00			5.000	1	20.00						
T2	180.00-160.00			5.000	1	20.00						
T3	160.00-140.00			7.000	1	20.00						
T4	140.00-120.00			9.000	1	20.00						
T5	120.00-100.00			11.000	1	20.00						
Т6	100.00-80.00			13.000	1	20.00						
Τ7	80.00-60.00			15.000	1	20.00						
Т8	60.00-40.00			17.000	1	20.00						
Т9	40.00-20.00			19.000	1	20.00						
T10	20.00-0.00			21,000	1	20.00						

Tower	Tower	Diagonal	Bracing	Has	Has	Top Girt	Bottom Girt
Section	Elevation	Spacing	Type	K Brace	Horizontals	Offset	Offset
				End			
	ft	ft		Panels		in	in
T1	200.00-180.00	4.979	X Brace	No	No	0.000	1.000
T2	180.00-160.00	4.979	X Brace	No	No	0.000	1.000
Т3	160.00-140.00	4.979	X Brace	No	No	0.000	1.000
T4	140.00-120.00	6.639	X Brace	No	No	0.000	1.000
T5	120.00-100.00	6.639	X Brace	No	No	0.000	1.000
T6	100.00-80.00	6.639	X Brace	No	No	0.000	1.000
T7	80.00-60.00	9.958	X Brace	No	No	0.000	1.000
T8	60.00-40.00	9.958	X Brace	No	No	0.000	1.000
Т9	40.00-20.00	9.958	X Brace	No	No	0.000	1.000
T10	20.00-0.00	9.958	X Brace	No	No	0.000	1.000

Tower Section Geometry (cont'd)

Tower	Leg	Leg	Leg	Diagonal	Diagonal	Diagonal
Elevation ft	Type	Size	Grade	Туре	Size	Grade
T1 200.00-	Pipe	Sabre 2.875x.375	A572-50	Equal Angle	L1 3/4x1 3/4x3/16	A36
180.00	•		(50 ksi)			(36 ksi)
T2 180.00-	Pipe	Sabre 3.5 x .3	À572-50	Equal Angle	L1 3/4x1 3/4x3/16	`A36 ´
160.00	•		(50 ksi)			(36 ksi)
T3 160.00-	Pipe	Sabre 4 x .318	A572-50	Equal Angle	L1 3/4x1 3/4x3/16	A36
140.00	•		(50 ksi)			(36 ksi)
T4 140.00-	Pipe	Sabre 4.5 x .438	À572-50	Equal Angle	L2 1/2x2 1/2x3/16	`A36 ´
120.00	•		(50 ksi)			(36 ksi)
T5 120.00-	Pipe	Sabre 5.5625 x .375	À572-50	Equal Angle	L2 1/2x2 1/2x3/16	`A36 ´
100.00	•		(50 ksi)			(36 ksi)
T6 100.00-	Pipe	Sabre 5.5625 x .375	A572-50	Equal Angle	L3x3x3/16	A36
80.00	•		(50 ksi)			(36 ksi)
T7 80.00-60.00	Pipe	Sabre 6.625 x .432	À572-50	Equal Angle	L3 1/2x3 1/2x1/4	`A36 ´
	•		(50 ksi)			(36 ksi)
T8 60.00-40.00	Pipe	Sabre 8.625 x .322	À572-50	Equal Angle	L3 1/2x3 1/2x1/4	`A36 ´
	•		(50 ksi)			(36 ksi)
T9 40.00-20.00	Pipe	Sabre 8.625 x .5	À572-50	Equal Angle	L3 1/2x3 1/2x1/4	`A36 ´
	•		(50 ksi)			(36 ksi)
T10 20.00-0.00	Pipe	Sabre 8.625 x .5	À572-50	Equal Angle	L4x4x1/4	`A36 ´
	-		(50 ksi)			(36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 200.00-	Equal Angle	L1 3/4x1 3/4x3/16	A36	Solid Round		A36
180.00			(36 ksi)			(36 ksi)

Tower Section Geometry (cont'd)

Tower	Gusset	Gusset	Gusset Grade	Adjust. Factor	Adjust.	Weight Mult.	Double Angle	Double Angle	Double Angle
Elevation	Area (per face)	Thickness		A_f	Factor Ar		Stitch Bolt Spacing Diagonals	Stitch Bolt Spacing Horizontals	Stitch Bolt Spacing Redundants
ft	ft²	in					in	in	in
T1 200.00- 180.00	0.00	0.375	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T2 180.00- 160.00	0.00	0.375	`A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T3 160.00- 140.00	0.00	0.375	`A36 ´ (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T4 140.00- 120.00	0.00	0.375	`A36 ´ (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T5 120.00- 100.00	0.00	0.375	`A36 ´ (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T6 100.00- 80.00	0.00	0.375	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T7 80.00- 60.00	0.00	0.375	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T8 60.00- 40.00	0.00	0.375	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T9 40.00- 20.00	0.00	0.375	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T10 20.00- 0.00	0.00	0.375	`A36 [′] (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt

I ower Section Geometry (cont'd)											
						K Fac	ctors ¹				
Tower Elevation	Calc K Single	Calc K Solid	Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace	
ft	Angles	Rounds		X Y	X Y	X Y	X Y	X Y	X Y	X Y	
T1 200.00- 180.00	Yes	No	1	1	1	1	1	1	1	1	
T2 180.00- 160.00	Yes	No	1	1 1	1 1	1 1	, 1 1	1 1	1	1 1	
T3 160.00- 140.00	Yes	No	1	1	1	1	1	1	1	1	
T4 140.00- 120.00	Yes	No	1	1	1	1	1	1	1	1	
T5 120.00 T5 120.00- 100.00	Yes	No	1	1	1	1	1 1	1	1	1	
T6 100.00- 80.00	Yes	No	1	1	1	1	1	1	1	1	
T7 80.00- 60.00	Yes	No	1	1 1	1 1	1	1	1	1 1	1	
T8 60.00- 40.00	Yes	No	1	1	1 1	1	1	1	1	1	
T9 40.00-	Yes	No	1	1	1	1	1	1	1	1	
20.00 T10 20.00- 0.00	Yes	No	1	1	1	1 1	1 1	1	1	1 1	

Tower Section Geometry (cont'd)

¹Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-ofplane direction applied to the overall length.

				ſow	er Sec	tior	ו Geo	metr	'y (col	nťď)				
Tower Elevation ft	Leg		Diago	onal	Top G	ìirt	Botton	n Girt	Mid	Girt	Long Ho	rizontal	Short Ho	orizontal
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 200.00- 180.00	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T2 180.00- 160.00	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T3 160.00- 140.00	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T4 140.00- 120.00	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T5 120.00- 100.00	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T6 100.00- 80.00	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T7 80.00- 60.00	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T8 60.00- 40.00	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T9 40.00- 20.00	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T10 20.00- 0.00	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75

tnxTower Report - version 8.0.9.0

Tower Elevation ft	Reduno Horizoi		Redun Diago		Redundar Diagoi		Redunda Horizo		Redui Vert		Redund	ant Hip	Redund Diag	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 200.00- 180.00	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T2 180.00- 160.00	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T3 160.00- 140.00	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T4 140.00- 120.00	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T5 120.00- 100.00	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T6 100.00- 80.00	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T7 80.00- 60.00	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T8 60.00- 40.00	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T9 40.00- 20.00	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T10 20.00- 0.00	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75

Tower Section Geometry (cont'd)

Tower Elevation	Leg Connection	Leg		Diagor	nal	Top G	irt	Bottom	Girt	Mid G	irt	Long Hori	zontal	Shor Horizor	-
ft	Type														ilai
	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.						
		in		in		in		in		in		in		in	
T1 200.00-	Flange	0.750	4	0.625	1	0.625	1	0.000	0	0.625	0	0.000	0	0.625	0
180.00	-	A325X		A325X		A325X		A325N		A325N		A325N		A325N	
T2 180.00-	Flange	1.000	4	0.625	1	0.000	0	0.000	0	0.625	0	0.000	0	0.625	0
160.00		A325X		A325X		A325N		A325N		A325N		A325N		A325N	
T3 160.00-	Flange	1.000	4	0.625	1	0.000	0	0.000	0	0.625	0	0.000	0	0.625	0
140.00		A325X		A325X		A325N		A325N		A325N		A325N		A325N	
T4 140.00-	Flange	1.250	4	0.625	1	0.000	0	0.000	0	0.625	0	0.000	0	0.625	0
120.00		A325X		A325X		A325N		A325N		A325N		A325N		A325N	
T5 120.00-	Flange	1.250	4	0.625	1	0.000	0	0.000	0	0.625	0	0.000	0	0.625	0
100.00		A325X		A325X		A325N		A325N		A325N		A325N		A325N	
T6 100.00-	Flange	1.250	6	0.750	1	0.000	0	0.000	0	0.625	0	0.000	0	0.625	0
80.00		A325X		A325X		A325N		A325N		A325N		A325N		A325N	
T7 80.00-	Flange	1.250	6	0.750	1	0.000	0	0.000	0	0.625	0	0.000	0	0.625	0
60.00		A325X		A325X		A325N		A325N		A325N		A325N		A325N	
T8 60.00-	Flange	1.375	6	0.750	1	0.000	0	0.000	0	0.625	0	0.000	0	0.625	0
40.00		A325X		A325X		A325N		A325N		A325N		A325N		A325N	
T9 40.00-	Flange	1.375	6	0.750	1	0.000	0	0.000	0	0.625	0	0.000	0	0.625	0
20.00	-	A325X		A325X		A325N		A325N		A325N		A325N		A325N	
T10 20.00-	Flange	0.000	0	0.750	1	0.000	0	0.000	0	0.625	0	0.000	0	0.625	0
0.00	-	A572-50		A325X		A325N		A325N		A325N		A325N		A325N	

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield		Componen t Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacin g in	Width or Diameter in	Perimete r in	Weight plf
*** LDF4-	в	No	No	Ar (CaAa)	198.00 -	0.000	0.18	1	1	0.500	0.630		0.150
nxTower R	eport	- versio	on 8.0.9.0										

Description	or	Allow Shield	Exclude From	Componen t		Offset	Lateral Offset	#			Width or Diameter	Perimete r	Weight
	Leg		Torque Calculation	Туре	ft	in	(Frac FW)		Row	g in	in	in	plf
50A(1/2) LDF4- 50A(1/2)	С	No	No	Ar (CaAa)	163.00 198.00 - 0.00	-2.000	0.03	1	1	0.500	0.630		0.150
LDF5- 50A(7/8)	В	No	No	Ar (CaAa)	198.00 - 144.00	0.000	0.16	3	2	0.500	1.090		0.330
760178129(1 /4)	В	No	No	Ar (CaAa)	190.00 - 182.00	1.000	-0.08	18	9	0.330	0.330		0.044
LDF4- 50A(1/2)	В	No	No	Ar (CaAa)	190.00 - 182.00	0.000	-0.08	9	9	0.625	0.625		0.150
760178129(1 /4)	В	No	No	Ar (CaAa)	- 182.00 0.00	3.000	-0.08	18	9	0.330	0.001		0.040
LDF4- 50A(1/2)	В	No	No	Ar (CaAa)	182.00 - 0.00	2.000	-0.08	9	9	0.625	0.001		0.150
AVA7-50(1- 5/8)	В	No	No	Ar (CaAa)	- 182.00 0.00	0.000	-0.1	6	6	0.500	2.010		0.700
Feedline Ladder (Af)	В	No	No	Af (CaAa)	168.00 - 0.00	0.000	-0.1	1	1	1.500	1.500		8.400
HB114-1- 0813U4-	С	No	No	Ar (CaAa)	170.00 - 0.00	-2.000	0.02	4	4	0.500	1.540		1.200
M5J(1-1/4) Feedline Ladder (Af)	С	No	No	Af (CaAa)	170.00 - 0.00	-1.000	0.005	1	1	3.000	3.000		8.400
FLC 12- 50J(1/2) ***	В	No	No	Ar (CaAa)	163.00 - 144.00	0.000	0.14	2	2	0.500	0.640		0.170
LDF5- 50A(7/8)	В	No	No	Ar (CaAa)	144.00 - 0.00	0.000	0.15	5	5	0.500	1.090		0.330
LDF4- 50A(1/2)	В	No	No	Ar (CaAa)	144.00 - 56.00	0.000	0.12	5	5	0.625	0.630		0.150
LDF4- 50A(1/2) ***	С	No	No	Ar (CaAa)	53.00 - 0.00	-1.500	0.03	1	1	0.630	0.630		0.150
LDF4- 50A(1/2)	В	No	No	Ar (CaAa)	56.00 - 0.00	1.000	0.15	6	6	0.630	0.630		0.150
LDF2- 50(3/8'')	В	No	No	Ar (CaAa)	100.00 - 0.00	0.000	0.04	1	1	0.500	0.440		0.080
50-AC-208- 8SM(3/4")	В	No	No	Ar (CaAa)	200.00 - 0.00	0.000	0.05	1	1	0.740	0.740		0.290
Feedline Ladder (Af)	В	No	No	Af (CaAa)	200.00 - 0.00	0.000	0.05	1	1	3.000	3.000		8.400
Feedline Ladder (Af)	В	No	No	Af (CaAa)	200.00 - 0.00	0.000	0.15	1	1	3.000	3.000		8.400
Thin Flat Bar Climbing Ladder	С	No	No	Af (CaAa)	200.00 - 0.00	0.000	0	1	1	2.000	2.000		4.000
Safety Line 3/8	С	No	No	Ar (CaAa)	200.00 - 0.00	0.000	0	1	1	0.375	0.375		0.220
1 1/2" Rigid Conduit	В	No	No	Ar (CaAa)	200.00 - 0.00	0.000	0.06	1	1	1.500	1.500		1.000
Feedline Ladder (Af)	А	No	No	Af (CaAa)	- 180.00 0.00	0.000	-0.13	1	1	3.000	3.000		8.400
Feedline Ladder (Af)	A	No	No	Af (CaAa)	140.00 - 0.00	0.000	-0.03	1	1	3.000	3.000		8.400
CU12PSM9P 6XXX(1-1/2)	А	No	No	Ar (CaAa)	134.00 - 0.00	0.000	0	1	1	1.600	1.600		2.350
Feedline Ladder (Af)	А	No	No	Af (CaAa)	134.00 - 0.00	0.000	0	1	1	3.000	3.000		8.400

Feed Line/Linear Appurtenances Section Area	nances Section Areas
---	----------------------

Tower Sectio	Tower Elevation	Face	A _R	A _F	C _A A _A In Face	C _A A _A Out Face	Weight
n	ft		ft ²	ft²	ft ²	ft ²	к
	200.00-180.00	A	0.000	0.000	0.000	0.000	0.00
11	200.00-160.00	B	0.000	0.000	43.169	0.000	0.00
		C	0.000	0.000	8.551	0.000	0.41
T2	180.00-160.00	A	0.000	0.000	10.000	0.000	0.09
12	100.00-100.00	B	0.000	0.000	58.649	0.000	0.17
то	100 00 140 00	C	0.000	0.000	19.837	0.000	0.22
Т3	160.00-140.00	A	0.000	0.000	10.000	0.000	0.17
		В	0.000	0.000	64.374	0.000	0.69
T 4	4 4 0 0 0 4 0 0 0 0	C	0.000	0.000	30.997	0.000	0.35
T4	140.00-120.00	A	0.000	0.000	29.240	0.000	0.49
		В	0.000	0.000	70.854	0.000	0.70
TC	400.00.400.00	С	0.000	0.000	30.997	0.000	0.35
Т5	120.00-100.00	A	0.000	0.000	33.200	0.000	0.55
		В	0.000	0.000	70.854	0.000	0.70
		С	0.000	0.000	30.997	0.000	0.35
Т6	100.00-80.00	Α	0.000	0.000	33.200	0.000	0.55
		В	0.000	0.000	71.734	0.000	0.70
		С	0.000	0.000	30.997	0.000	0.35
T7	80.00-60.00	Α	0.000	0.000	33.200	0.000	0.55
		В	0.000	0.000	71.734	0.000	0.70
		С	0.000	0.000	30.997	0.000	0.35
T8	60.00-40.00	Α	0.000	0.000	33.200	0.000	0.55
		В	0.000	0.000	72.742	0.000	0.71
		С	0.000	0.000	31.816	0.000	0.35
Т9	40.00-20.00	Α	0.000	0.000	33.200	0.000	0.55
		В	0.000	0.000	72.994	0.000	0.71
		С	0.000	0.000	32.257	0.000	0.35
T10	20.00-0.00	А	0.000	0.000	33.200	0.000	0.55
	-	В	0.000	0.000	72.994	0.000	0.71
		Ċ	0.000	0.000	32.257	0.000	0.35

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	A_R	AF	$C_A A_A$	$C_A A_A$	Weight
Sectio	Elevation	or	Thickness			In Face	Out Face	
n	ft	Leg	in	ft²	ft²	ft²	ft²	ĸ
T 1	200.00-180.00	А	2.025	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	122.399	0.000	2.13
		С		0.000	0.000	32.043	0.000	0.57
T2	180.00-160.00	А	2.003	0.000	0.000	18.011	0.000	0.47
		В		0.000	0.000	189.341	0.000	3.08
		С		0.000	0.000	57.862	0.000	1.06
Т3	160.00-140.00	А	1.978	0.000	0.000	17.912	0.000	0.47
		В		0.000	0.000	206.883	0.000	3.35
		С		0.000	0.000	82.449	0.000	1.52
T4	140.00-120.00	А	1.950	0.000	0.000	55.757	0.000	1.40
		В		0.000	0.000	218.156	0.000	3.52
		С		0.000	0.000	81.812	0.000	1.49
T5	120.00-100.00	А	1.918	0.000	0.000	63.880	0.000	1.58
		В		0.000	0.000	216.443	0.000	3.46
		С		0.000	0.000	81.081	0.000	1.47
T6	100.00-80.00	Α	1.879	0.000	0.000	63.271	0.000	1.55
		В		0.000	0.000	222.823	0.000	3.49
		С		0.000	0.000	80.218	0.000	1.44
T7	80.00-60.00	А	1.833	0.000	0.000	62.524	0.000	1.51
		В		0.000	0.000	220.167	0.000	3.40
		С		0.000	0.000	79.162	0.000	1.40
T8	60.00-40.00	А	1.772	0.000	0.000	61.554	0.000	1.47
		В		0.000	0.000	219.074	0.000	3.32
		С		0.000	0.000	83.217	0.000	1.42
Т9	40.00-20.00	А	1.684	0.000	0.000	60.142	0.000	1.41

Tower Sectio	Tower Elevation	Face or	lce Thickness	A _R	A _F	C _A A _A In Face	C _A A _A Out Face	Weight
n	ft	Leg	in	ft²	ft²	ft ²	ft ²	к
		В		0.000	0.000	214.662	0.000	3.16
		С		0.000	0.000	83.790	0.000	1.38
T10	20.00-0.00	А	1.509	0.000	0.000	57.339	0.000	1.29
		В		0.000	0.000	204.766	0.000	2.84
		С		0.000	0.000	79.134	0.000	1.24

Feed Line Center of Pressure

Section	Elevation	CPx	CPz	CPx	CPz
				lce	lce
	ft	in	in	in	in
T1	200.00-180.00	5.772	-1.242	6.623	-0.410
T2	180.00-160.00	4.863	-2.309	6.752	-2.095
Т3	160.00-140.00	5.765	-2.107	8.067	-1.956
Τ4	140.00-120.00	3.698	-3.421	6.058	-3.478
T5	120.00-100.00	3.562	-4.025	6.098	-4.158
T6	100.00-80.00	3.813	-4.215	7.235	-4.743
T7	80.00-60.00	4.362	-4.769	8.198	-5.375
Т8	60.00-40.00	4.744	-4.652	8.656	-4.899
Т9	40.00-20.00	5.086	-4.810	9.143	-4.906
T10	20.00-0.00	5.069	-4.825	9.232	-5.258

Shielding Factor Ka

Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.		Segment	No Ice	lce
			Elev.		
T1	2	LDF4-50A(1/2)	180.00 -	0.6000	0.5372
			198.00		
T1	3	LDF4-50A(1/2)	180.00 -	0.6000	0.5372
T 4			198.00	0 0000	0 5070
T1	4	LDF5-50A(7/8)	180.00 -	0.6000	0.5372
T1	6	760178129(1/4)	198.00 182.00 -	0.6000	0.5372
1 1	0	700178129(1/4)	190.00	0.0000	0.5572
T1	7	LDF4-50A(1/2)	182.00 -	0.6000	0.5372
	'	LDI 4 30A(172)	190.00	0.0000	0.0072
T1	8	760178129(1/4)	180.00	0.6000	0.5372
	Ű		182.00	010000	010072
T1	9	LDF4-50A(1/2)	180.00 -	0.6000	0.5372
		· · · ·	182.00		
T1	11	AVA7-50(1-5/8)	180.00 -	0.6000	0.5372
			182.00		
T1	26	50-AC-208-8SM(3/4")	180.00 -	0.6000	0.5372
			200.00		
T1	27	Feedline Ladder (Af)	180.00 -	0.6000	0.5372
			200.00		
T1	29	Feedline Ladder (Af)	180.00 -	0.6000	0.5372
T 4	20		200.00	0 0000	0 5070
T1	30	Thin Flat Bar Climbing Ladder	180.00 - 200.00	0.6000	0.5372
T1	31	Safety Line 3/8	180.00	0.6000	0.5372
1 '	31	Salety Liffe 5/6	200.00	0.0000	0.5572
T1	33	1 1/2" Rigid Conduit	180.00	0.6000	0.5372
		i ii z rugia conduit	200.00	0.0000	0.0072
Т2	2	LDF4-50A(1/2)	163.00	0.6000	0.5940
. –	_	()	180.00		
T2	3	LDF4-50A(1/2)	160.00 -	0.6000	0.5940
		、 <i>、</i> ,	180.00		

Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.		Segment Elev.	No Ice	lce
T2	4	LDF5-50A(7/8)	160.00 - 180.00	0.6000	0.5940
Т2	8	760178129(1/4)	160.00 - 180.00	0.6000	0.5940
Т2	9	LDF4-50A(1/2)	160.00 - 180.00	0.6000	0.5940
Т2	11	AVA7-50(1-5/8)	160.00 - 180.00	0.6000	0.5940
Т2	12	Feedline Ladder (Af)	160.00 -	0.6000	0.5940
T2	14	HB114-1-0813U4-M5J(1-	168.00 160.00 -	0.6000	0.5940
Т2	15	1/4) Feedline Ladder (Af)	170.00 160.00 -	0.6000	0.5940
Т2	17	FLC 12-50J(1/2)	170.00 160.00 -	0.6000	0.5940
Т2	26	50-AC-208-8SM(3/4")	163.00 160.00 -	0.6000	0.5940
Т2	27	Feedline Ladder (Af)	180.00 160.00 -	0.6000	0.5940
Т2	29	Feedline Ladder (Af)	180.00 160.00 -	0.6000	0.5940
Т2	30	Thin Flat Bar Climbing	180.00 160.00 -	0.6000	0.5940
Т2	31	Ladder Safety Line 3/8	180.00 160.00 -	0.6000	0.5940
Т2	33	1 1/2" Rigid Conduit	180.00 160.00 -	0.6000	0.5940
T2	35	Feedline Ladder (Af)	180.00 160.00 -	0.6000	0.5940
тз	3	LDF4-50A(1/2)	180.00 140.00 -	0.6000	0.6000
тз	4	LDF5-50A(7/8)	160.00 144.00 -	0.6000	0.6000
Т3	8	760178129(1/4)	160.00 140.00 -	0.6000	0.6000
Т3	9	LDF4-50A(1/2)	160.00 140.00 -	0.6000	0.6000
тз	11	AVA7-50(1-5/8)	160.00 140.00 -	0.6000	0.6000
тз	12	Feedline Ladder (Af)	160.00 140.00 -	0.6000	0.6000
тз	14	HB114-1-0813U4-M5J(1-	160.00 140.00 -	0.6000	0.6000
тз	15	1/4) Feedline Ladder (Af)	160.00 140.00 -	0.6000	0.6000
тз	17	FLC 12-50J(1/2)	160.00 144.00 -	0.6000	0.6000
тз	19	LDF5-50A(7/8)	160.00 140.00 -	0.6000	0.6000
Т3	20	LDF4-50A(1/2)	144.00 140.00 -	0.6000	0.6000
Т3	26	50-AC-208-8SM(3/4")	144.00 140.00 -	0.6000	0.6000
тз	27	Feedline Ladder (Af)	160.00 140.00 -	0.6000	0.6000
тз	29	Feedline Ladder (Af)	160.00 140.00 -	0.6000	0.6000
тз	30	Thin Flat Bar Climbing	160.00 140.00 -	0.6000	0.6000
тз	31	Ladder Safety Line 3/8	160.00 140.00 -	0.6000	0.6000
Т3	33	1 1/2" Rigid Conduit	160.00 140.00 -	0.6000	0.6000
тз	35	Feedline Ladder (Af)	160.00 140.00 -	0.6000	0.6000
Т4	3	LDF4-50A(1/2)	160.00 120.00 -	0.6000	0.6000
Т4		760178129(1/4)	140.00 120.00 -	0.6000	0.6000

Tower Section	Feed Line Record No.	Description	Feed Line Segment	K _a No Ice	K _a Ice
			Elev.		
T4	9	LDF4-50A(1/2)	140.00 120.00 - 140.00	0.6000	0.6000
Т4	11	AVA7-50(1-5/8)	120.00 - 140.00	0.6000	0.6000
Т4	12	Feedline Ladder (Af)	120.00 - 140.00	0.6000	0.6000
T4	14	HB114-1-0813U4-M5J(1- 1/4)	120.00 - 140.00	0.6000	0.6000
Т4	15	Feedline Ladder (Af)	120.00 - 140.00	0.6000	0.6000
Т4	19	LDF5-50A(7/8)	120.00 - 140.00	0.6000	0.6000
T4	20	LDF4-50A(1/2)	120.00 - 140.00	0.6000	0.6000
T4	26	50-AC-208-8SM(3/4")	120.00 - 140.00	0.6000	0.6000
T4	27	Feedline Ladder (Af)	120.00 - 140.00	0.6000	0.6000
T4	29	Feedline Ladder (Af)	120.00 - 140.00	0.6000	0.6000
T4	30	Thin Flat Bar Climbing Ladder	120.00 - 140.00	0.6000	0.6000 0.6000
T4	31	Safety Line 3/8	120.00 - 140.00	0.6000	0.6000
T4 T4	33 35	1 1/2" Rigid Conduit	120.00 - 140.00	0.6000	
14 T4	36	Feedline Ladder (Af) Feedline Ladder (Af)	120.00 - 140.00 120.00 -	0.6000 0.6000	0.6000 0.6000
T4	38	CU12PSM9P6XXX(1-1/2)	140.00 140.00 120.00 -	0.6000	0.6000
T4	39	Feedline Ladder (Af)	134.00 120.00 -	0.6000	0.6000
T5	3	LDF4-50A(1/2)	134.00 100.00 -	0.6000	0.6000
T5	8	760178129(1/4)	120.00 100.00 -	0.6000	0.6000
Т5	9	LDF4-50A(1/2)	120.00 100.00 -	0.6000	0.6000
Т5	11	AVA7-50(1-5/8)	120.00 100.00 -	0.6000	0.6000
Т5	12	Feedline Ladder (Af)	120.00 100.00 -	0.6000	0.6000
Т5	14	HB114-1-0813U4-M5J(1-	120.00 100.00 -	0.6000	0.6000
Т5	15	1/4) Feedline Ladder (Af)	120.00 100.00 -	0.6000	0.6000
Т5	19	LDF5-50A(7/8)	120.00 100.00 -	0.6000	0.6000
Т5	20	LDF4-50A(1/2)	120.00 100.00 -	0.6000	0.6000
Т5	26	50-AC-208-8SM(3/4")	120.00 100.00 - 120.00	0.6000	0.6000
Т5	27	Feedline Ladder (Af)	120.00 100.00 - 120.00	0.6000	0.6000
Т5	29	Feedline Ladder (Af)	100.00 - 120.00 120.00	0.6000	0.6000
Т5	30	Thin Flat Bar Climbing Ladder	100.00 - 120.00	0.6000	0.6000
Т5	31	Safety Line 3/8	100.00 - 120.00	0.6000	0.6000
Т5	33	1 1/2" Rigid Conduit	100.00 - 120.00	0.6000	0.6000
Т5	35	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
Т5	36	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000

Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.		Segment Elev.	No Ice	lce
Т5	38	CU12PSM9P6XXX(1-1/2)	100.00 -	0.6000	0.6000
T5	39	Feedline Ladder (Af)	120.00 100.00 - 120.00	0.6000	0.6000
Т6	3	LDF4-50A(1/2)	80.00 -	0.6000	0.6000
т6	8	760178129(1/4)	100.00 80.00 -	0.6000	0.6000
Т6	9	LDF4-50A(1/2)	100.00 80.00 - 100.00	0.6000	0.6000
т6	11	AVA7-50(1-5/8)	80.00 - 100.00	0.6000	0.6000
т6	12	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
т6	14	HB114-1-0813U4-M5J(1- 1/4)	80.00 - 100.00	0.6000	0.6000
т6	15	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
т6	19	LDF5-50A(7/8)	80.00 - 100.00	0.6000	0.6000
т6	20	LDF4-50A(1/2)	80.00 - 100.00	0.6000	0.6000
т6	25	LDF2-50(3/8")	80.00 - 100.00	0.6000	0.6000
т6	26	50-AC-208-8SM(3/4")	80.00 - 100.00	0.6000	0.6000
т6	27	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
т6	29	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
Т6	30	Thin Flat Bar Climbing Ladder	80.00 - 100.00	0.6000	0.6000
т6	31	Safety Line 3/8	80.00 -	0.6000	0.6000
Т6	33	1 1/2" Rigid Conduit	100.00 80.00 - 100.00	0.6000	0.6000
т6	35	Feedline Ladder (Af)	80.00 -	0.6000	0.6000
т6	36	Feedline Ladder (Af)	100.00 80.00 - 100.00	0.6000	0.6000
т6	38	CU12PSM9P6XXX(1-1/2)	80.00 - 100.00	0.6000	0.6000
т6	39	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
Т7	3	LDF4-50A(1/2)	60.00 - 80.00	0.6000	0.6000
Т7	8	760178129(1/4)	60.00 - 80.00	0.6000	0.6000
Т7	9	LDF4-50A(1/2)	60.00 - 80.00	0.6000	0.6000
Т7	11	AVA7-50(1-5/8)	60.00 - 80.00	0.6000	0.6000
Т7	12	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
Т7	14	HB114-1-0813U4-M5J(1- 1/4)	60.00 - 80.00	0.6000	0.6000
Т7	15	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
Т7	19	LDF5-50A(7/8)	60.00 - 80.00	0.6000	0.6000
Т7	20	LDF4-50A(1/2)	60.00 - 80.00	0.6000	0.6000
Т7	25	LDF2-50(3/8")	60.00 - 80.00	0.6000	0.6000
Т7	26	50-AC-208-8SM(3/4")	60.00 - 80.00	0.6000	0.6000
Т7	27	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
Т7	29	Feedline Ladder (Af)		0.6000	0.6000

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	Kª No Ice	K _a Ice
Т7	30	Thin Flat Bar Climbing	80.00 60.00 -	0.6000	0.6000
Т7	31	Ladder Safety Line 3/8	80.00 60.00 -	0.6000	0.6000
Τ7	33	1 1/2" Rigid Conduit	80.00 - 60.00 80.00	0.6000	0.6000
Τ7	35	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
Τ7	36	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
Τ7	38	CU12PSM9P6XXX(1-1/2)	60.00 - 80.00	0.6000	0.6000
Τ7	39	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
Т8	3	LDF4-50A(1/2)	40.00 - 60.00	0.6000	0.6000
T8	8	760178129(1/4)	40.00 - 60.00	0.6000	0.6000
T8	9	LDF4-50A(1/2)	40.00 - 60.00	0.6000	0.6000
Т8 Т8	11 12	AVA7-50(1-5/8) Feedline Ladder (Af)	40.00 - 60.00 40.00 -	0.6000 0.6000	0.6000 0.6000
T8	14	HB114-1-0813U4-M5J(1-	40.00 - 60.00 40.00 -	0.6000	0.6000
Т8	15	1/4) Feedline Ladder (Af)	60.00 40.00 -	0.6000	0.6000
T8	19	LDF5-50A(7/8)	60.00 40.00 -	0.6000	0.6000
Т8	20	LDF4-50A(1/2)	60.00 56.00 -	0.6000	0.6000
Т8	22	LDF4-50A(1/2)	60.00 40.00 -	0.6000	0.6000
Т8	24	LDF4-50A(1/2)	53.00 40.00 -	0.6000	0.6000
Т8	25	LDF2-50(3/8")	56.00 40.00 -	0.6000	0.6000
Т8	26	50-AC-208-8SM(3/4")	60.00 40.00 - 60.00	0.6000	0.6000
Т8	27	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
Т8	29	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
Т8	30	Thin Flat Bar Climbing Ladder	40.00 - 60.00	0.6000	0.6000
Т8	31	Safety Line 3/8	40.00 - 60.00	0.6000	0.6000
T8	33	1 1/2" Rigid Conduit	40.00 - 60.00	0.6000	0.6000
Т8	35	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
Т8	36	Feedline Ladder (Af)	40.00 - 60.00	0.6000 0.6000	0.6000
Т8 Т8	38 39	CU12PSM9P6XXX(1-1/2) Feedline Ladder (Af)	40.00 - 60.00 40.00 -	0.6000	0.6000 0.6000
T9	3	LDF4-50A(1/2)	40.00 - 60.00 20.00 -	0.6000	0.6000
Т9	8	760178129(1/4)	40.00 20.00 -	0.6000	0.6000
Т9	9	LDF4-50A(1/2)	40.00 20.00 -	0.6000	0.6000
Т9	11	AVA7-50(1-5/8)	40.00 20.00 -	0.6000	0.6000
Т9	12	Feedline Ladder (Af)	40.00 20.00 -	0.6000	0.6000
		l	40.00	l	

Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.		Segment	No Ice	lce
Т9	14	HB114-1-0813U4-M5J(1-	<i>Elev.</i> 20.00 -	0.6000	0.6000
19	14	1/4)	40.00	0.6000	0.0000
Т9	15	Feedline Ladder (Af)	20.00 -	0.6000	0.6000
			40.00	0,0000	010000
Т9	19	LDF5-50A(7/8)	20.00 -	0.6000	0.6000
			40.00		
Т9	22	LDF4-50A(1/2)	20.00 -	0.6000	0.6000
то	04		40.00	0 0000	0,0000
Т9	24	LDF4-50A(1/2)	20.00 40.00	0.6000	0.6000
Т9	25	LDF2-50(3/8")	20.00	0.6000	0.6000
10	20	221 2 00(0/0)	40.00	010000	010000
Т9	26	50-AC-208-8SM(3/4")	20.00 -	0.6000	0.6000
			40.00		
Т9	27	Feedline Ladder (Af)	20.00 -	0.6000	0.6000
Т9	29	Feedline Ladder (Af)	40.00 20.00 -	0.6000	0.6000
19	29	i eediine Ladder (Ar)	40.00	0.0000	0.0000
Т9	30	Thin Flat Bar Climbing	20.00	0.6000	0.6000
		Ladder	40.00		
Т9	31	Safety Line 3/8	20.00 -	0.6000	0.6000
T 0			40.00	0.0000	0,0000
Т9	33	1 1/2" Rigid Conduit	20.00 - 40.00	0.6000	0.6000
Т9	35	Feedline Ladder (Af)	20.00	0.6000	0.6000
			40.00	010000	010000
Т9	36	Feedline Ladder (Af)	20.00 -	0.6000	0.6000
			40.00		
Т9	38	CU12PSM9P6XXX(1-1/2)	20.00 -	0.6000	0.6000
Т9	39	Feedline Ladder (Af)	40.00 20.00 -	0.6000	0.6000
13	00		40.00	0.0000	0.0000
T10	3	LDF4-50A(1/2)	0.00 - 20.00	0.6000	0.6000
T10	8	760178129(1/4)	0.00 - 20.00	0.6000	0.6000
T10	9	LDF4-50A(1/2)	0.00 - 20.00	0.6000	0.6000
T10	11	AVA7-50(1-5/8)	0.00 - 20.00	0.6000	0.6000
T10	12	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	14	HB114-1-0813U4-M5J(1-	0.00 - 20.00	0.6000	0.6000
T 10	4.5	1/4)	0.00.00.00	0 0000	0,0000
T10	15	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	19	LDF5-50A(7/8)	0.00 - 20.00 0.00 - 20.00	0.6000	0.6000
T10	22	LDF4-50A(1/2)		0.6000	0.6000 0.6000
T10	24	LDF4-50A(1/2)	0.00 - 20.00	0.6000	
T10 T10	25	LDF2-50(3/8") 50-AC-208-8SM(3/4")	0.00 - 20.00 0.00 - 20.00	0.6000 0.6000	0.6000
	26 27				0.6000
T10 T10	27 29	Feedline Ladder (Af) Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000 0.6000
T10	30	Thin Flat Bar Climbing	0.00 - 20.00	0.6000	0.6000
.10		Ladder	2.00 20.00	0.0000	0.0000
T10	31	Safety Line 3/8	0.00 - 20.00	0.6000	0.6000
T10	33	1 1/2" Rigid Conduit	0.00 - 20.00	0.6000	0.6000
T10	35	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	36	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	38	CU12PSM9P6XXX(1-1/2)	0.00 - 20.00	0.6000	0.6000
T10	39	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000

Discrete Tower Loads

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement
	Leg		Lateral	-	
			Vert	٥	
			ft ft	-	ft
			ft		
15" Dia. x 15" Beacon	А	From Leg	0.00	0.000	200.00
			0.000 15.000		
2.4" x 16' Mount Pipe	А	From Leg	0.00	0.000	200.00
			0.000		
			7.000	0.000	
15" Dia. x 15" Beacon	С	From Leg	0.00 0.000	0.000	200.00
			15.000		
2.4" x 16' Mount Pipe	С	From Leg	0.00	0.000	200.00
			0.000		
2" x 6" Cidal ight	^	From Log	7.000	0.000	102.00
3" x 6" SideLight	A	From Leg	0.00 0.000	0.000	102.00
			0.000		
3" x 6" SideLight	В	From Leg	0.00	0.000	102.00
			0.000		
3" x 6" SideLight	С	From Leg	0.000 0.00	0.000	102.00
	Ũ	1 Iom Eog	0.000	0.000	102.00
			0.000		
** DB225-A	^	From Log	4.00	0.000	198.00
DB225-A	А	From Leg	0.000	0.000	198.00
			7.000		
ALR10-O	В	From Leg	4.00	0.000	198.00
			0.000		
OGB6-928N	В	From Leg	10.000 4.00	0.000	198.00
0000-9201	D	TIONILeg	0.000	0.000	190.00
			6.000		
PD1107-1	С	From Leg	4.00	0.000	198.00
			0.000 7.000		
PD201-7	С	From Leg	4.00	0.000	198.00
	-		0.000		
			7.000		
(4) 6' x 2" Mount Pipe	А	From Leg	4.00	0.000	198.00
			0.000 0.000		
(4) 6' x 2" Mount Pipe	В	From Leg	4.00	0.000	198.00
			0.000		
(4) Cly Of Mount Ding	0	From Log	0.000	0.000	109.00
(4) 6' x 2" Mount Pipe	С	From Leg	4.00 0.000	0.000	198.00
			0.000		
(2) 4' x 2" Pipe Mount	А	From Leg	4.00	0.000	198.00
			0.000		
(2) 4' x 2" Pipe Mount	В	From Leg	0.000 4.00	0.000	198.00
	D	r toin Log	0.000	0.000	100.00
			0.000		
(2) 4' x 2" Pipe Mount	С	From Leg	4.00	0.000	198.00
			0.000 0.000		
Sector Mount [SM 702-3]	С	None	0.000	0.000	198.00

(2) MXM REPEATER MK2	А	From Leg	1.00	0.000	190.00
			0.000		
(2) MXM REPEATER MK2	В	From Leg	0.000 1.00	0.000	190.00
	-		0.000		
			0.000		
(2) MXM REPEATER MK2	С	From Leg	1.00	0.000	190.00

Description	Face	Offset	Offsets:	Azimuth	Placemen
	or Leg	Туре	Horz Lateral	Adjustment	
	LUG		Vert		
			ft	٥	ft
			ft ft		
			0.000		
Pipe Mount [PM 601-1]	А	From Leg	0.50	0.000	190.00
			0.000 0.000		
Pipe Mount [PM 601-1]	В	From Leg	0.50	0.000	190.00
	D	Troin Log	0.000	0.000	100.00
			0.000		
Pipe Mount [PM 601-1]	С	From Leg	0.50	0.000	190.00
			0.000		
***			0.000		
APXVAARR24_43-U-NA20 w/ Mount Pipe	А	From Leg	4.00	0.000	182.00
			0.000		
	_		1.000		
APXVAARR24_43-U-NA20 w/ Mount Pipe	В	From Leg	4.00	0.000	182.00
			0.000 1.000		
APXVAARR24_43-U-NA20 w/ Mount Pipe	С	From Leg	4.00	0.000	182.00
	0	1 Iom Eog	0.000	0.000	102.00
			1.000		
AIR6449 B41_T-MOBILE w/ Mount Pipe	А	From Leg	4.00	0.000	182.00
			0.000		
	_	_ .	1.000		
AIR6449 B41_T-MOBILE w/ Mount Pipe	В	From Leg	4.00	0.000	182.00
			0.000 1.000		
AIR6449 B41_T-MOBILE w/ Mount Pipe	С	From Leg	4.00	0.000	182.00
Antorra by 1_1 MOBILE W Mount Tipe	0	1 Iom Eog	0.000	0.000	102.00
			1.000		
PX16DWV-16DWV-S-E-A20 w/ Mount Pipe	А	From Leg	4.00	0.000	182.00
		-	0.000		
	_		1.000		
PX16DWV-16DWV-S-E-A20 w/ Mount Pipe	В	From Leg	4.00	0.000	182.00
			0.000 1.000		
PX16DWV-16DWV-S-E-A20 w/ Mount Pipe	С	From Leg	4.00	0.000	182.00
	0	Troin Log	0.000	0.000	102.00
			1.000		
RADIO 4449 B12/B71	A	From Leg	4.00	0.000	182.00
			0.000		
RADIO 4449 B12/B71	В	From Loc	1.000	0.000	182.00
	D	From Leg	4.00 0.000	0.000	102.00
			1.000		
RADIO 4449 B12/B71	С	From Leg	4.00	0.000	182.00
		5	0.000		
		_ ·	1.000	0.005	100.05
RADIO 4415 B66A_CCIV3	A	From Leg	4.00	0.000	182.00
			0.000 1.000		
RADIO 4415 B66A_CCIV3	В	From Leg	4.00	0.000	182.00
	-		0.000		
			1.000		
RADIO 4415 B66A_CCIV3	С	From Leg	4.00	0.000	182.00
			0.000		
	۸	From Log	1.000	0.000	100 00
RADIO 4424 B25_TMO	А	From Leg	4.00 0.000	0.000	182.00
			1.000		
RADIO 4424 B25_TMO	В	From Leg	4.00	0.000	182.00
		5	0.000	-	
			1.000		
RADIO 4424 B25_TMO	С	From Leg	4.00	0.000	182.00
			0.000		
			1.000		

Description	Face	Offset	Offsets:	Azimuth	Placemer
	or Leg	Туре	Horz Lateral	Adjustment	
	Leg		Vert		
			ft	٥	ft
			ft ft		
Sector Mount [SM 702-3]	С	None	п.	0.000	182.00
(3) 6' x 2" Mount Pipe	A	From Leg	4.00	0.000	182.00
			0.000		
	_		0.000	0.000	100.00
(3) 6' x 2" Mount Pipe	В	From Leg	4.00	0.000	182.00
			0.000 0.000		
(3) 6' x 2" Mount Pipe	С	From Leg	4.00	0.000	182.00
	Ũ	1 Iom Eog	0.000	0.000	102.00
			0.000		
SitePro STK-U Stiff Arm Kit	А	From Leg	2.00	0.000	182.00
		-	0.000		
			0.000		
SitePro STK-U Stiff Arm Kit	В	From Leg	2.00	0.000	182.00
			0.000		
	0	Energy Law	0.000	0.000	400.00
SitePro STK-U Stiff Arm Kit	С	From Leg	2.00 0.000	0.000	182.00
			0.000		
**			0.000		
APXVTM14-ALU-I20 w/ Mount Pipe	А	From Leg	4.00	0.000	170.00
F -		5	0.000		
			1.000		
APXVTM14-ALU-I20 w/ Mount Pipe	В	From Leg	4.00	0.000	170.00
			0.000		
	0	Example a	1.000	0.000	170.00
APXVTM14-ALU-I20 w/ Mount Pipe	С	From Leg	4.00 0.000	0.000	170.00
			1.000		
NNVV-65B-R4 w/ Mount Pipe	А	From Leg	4.00	0.000	170.00
	<i>,</i> ,	1 Iom Log	0.000	01000	
			1.000		
NNVV-65B-R4 w/ Mount Pipe	В	From Leg	4.00	0.000	170.00
			0.000		
			1.000		470.00
NNVV-65B-R4 w/ Mount Pipe	С	From Leg	4.00	0.000	170.00
			0.000 1.000		
PCS 1900MHZ 4X45W-65MHZ	А	From Leg	4.00	0.000	170.00
		1 Ioni Log	0.000	0.000	110.00
			1.000		
PCS 1900MHZ 4X45W-65MHZ	В	From Leg	4.00	0.000	170.00
		-	0.000		
		_ .	1.000		
PCS 1900MHZ 4X45W-65MHZ	С	From Leg	4.00	0.000	170.00
			0.000		
TD-RRH8X20-25	А	From Leg	1.000 4.00	0.000	170.00
	~	r totti Leg	0.000	0.000	170.00
			1.000		
TD-RRH8X20-25	В	From Leg	4.00	0.000	170.00
		0	0.000		
		_	1.000		
TD-RRH8X20-25	С	From Leg	4.00	0.000	170.00
			0.000		
	۸	Eromlar	1.000	0.000	170.00
(2) RRH2X50-800	A	From Leg	4.00 0.000	0.000	170.00
			1.000		
(2) RRH2X50-800	В	From Leg	4.00	0.000	170.00
	D	1.5m Log	0.000	0.000	110.00
			1.000		
(2) RRH2X50-800	С	From Leg	1.000 4.00	0.000	170.00
(2) RRH2X50-800	С	From Leg		0.000	170.00

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement
	Leg	51	Lateral	, i i i i i i i i i i i i i i i i i i i	
			Vert ft	٥	ft
			ft		п
			ft		170.00
10' horizontal x 2" Pipe Mount	A	From Leg	2.00 0.000	0.000	170.00
			0.000		
10' horizontal x 2" Pipe Mount	В	From Leg	2.00	0.000	170.00
			0.000 0.000		
10' horizontal x 2" Pipe Mount	С	From Leg	2.00	0.000	170.00
· ·		5	0.000		
		Ensue Line	0.000	0.000	170.00
6' x 2" Mount Pipe	A	From Leg	4.00 0.000	0.000	170.00
			0.000		
6' x 2" Mount Pipe	В	From Leg	4.00	0.000	170.00
			0.000 0.000		
6' x 2" Mount Pipe	С	From Leg	4.00	0.000	170.00
	-		0.000		
	0		0.000	0.000	170.00
Sector Mount [SM 506-3]	С	None		0.000	170.00
Pipe Mount [PM 601-1]	А	From Leg	0.50	0.000	163.00
			0.000		
***			0.000		
(2) PD1109-1	В	From Leg	4.00	0.000	144.00
ζ,		0	0.000		
	C	Enormal e a	8.000	0.000	144.00
SRL480N1DT4	С	From Leg	4.00 0.000	0.000	144.00
			11.000		
(4) 6' x 2" Mount Pipe	A	From Leg	4.00	0.000	144.00
			0.000 0.000		
(4) 6' x 2" Mount Pipe	В	From Leg	4.00	0.000	144.00
		C C	0.000		
(4) 6' x 2" Mount Pino	С	From Leg	0.000 4.00	0.000	144.00
(4) 6' x 2" Mount Pipe	C	FIGHTLEG	0.000	0.000	144.00
			0.000		
(2) 4' x 2" Pipe Mount	A	From Leg	4.00	0.000	144.00
			0.000 0.000		
(2) 4' x 2" Pipe Mount	В	From Leg	4.00	0.000	144.00
		C C	0.000		
(2) 4' x 2" Pipe Mount	С	From Leg	0.000 4.00	0.000	144.00
	0	1 Ioni Leg	0.000	0.000	144.00
	_		0.000		
Sector Mount [SM 702-3]	С	None		0.000	144.00
KS24019-L112A	С	From Leg	2.00	-30.000	53.00
		0	0.000		
Side Arm Mount ISO 202 11	C	From Log	2.000	-30.000	53.00
Side Arm Mount [SO 202-1]	С	From Leg	1.00 0.000	-30.000	55.00
			0.000		
	٨	Frem Law	4.00	0.000	404.00
MX08FRO665-21 w/ Mount Pipe	А	From Leg	4.00 0.000	0.000	134.00
			0.000		
MX08FRO665-21 w/ Mount Pipe	В	From Leg	4.00	0.000	134.00
			0.000 0.000		
MX08FRO665-21 w/ Mount Pipe	С	From Leg	4.00	0.000	134.00
	0				

Description	Face or	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement
	Leg		Vert		
			ft	٥	ft
			ft		
			ft		
			0.000		
TA08025-B604	А	From Leg	4.00	0.000	134.00
			0.000		
		Enter Lan	0.000	0.000	404.00
TA08025-B604	В	From Leg	4.00 0.000	0.000	134.00
			0.000		
TA08025-B604	С	From Leg	4.00	0.000	134.00
17100020 8004	0	Troin Log	0.000	0.000	104.00
			0.000		
TA08025-B605	А	From Leg	4.00	0.000	134.00
		0	0.000		
			0.000		
TA08025-B605	В	From Leg	4.00	0.000	134.00
			0.000		
	_		0.000		
TA08025-B605	С	From Leg	4.00	0.000	134.00
			0.000		
RDIDC-9181-PF-48	В	Erom Log	0.000 4.00	0.000	134.00
RDIDC-9101-PF-40	D	From Leg	4.00 0.000	0.000	134.00
			0.000		
(2) 8' x 2" Mount Pipe	А	From Leg	4.00	0.000	134.00
	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	Troin Log	0.000	0.000	101.00
			0.000		
(2) 8' x 2'' Mount Pipe	В	From Leg	4.00	0.000	134.00
		0	0.000		
			0.000		
(2) 8' x 2" Mount Pipe	С	From Leg	4.00	0.000	134.00
			0.000		
	•		0.000	0.000	404.00
Commscope MTC3975083 (3)	С	None		0.000	134.00

	Dishes										
Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter			
				ft	0	٥	ft	ft			
USX6-6W-6GR	A	Paraboloid w/Shroud (HP)	From Leg	1.00 0.000 0.000	-6.000		190.00	6.00			
USX6-6W-6GR	В	Paraboloid w/Shroud (HP)	From Leg	1.00 0.000 0.000	53.000		190.00	6.00			
USX6-6W-6GR	С	Paraboloid w/Shroud (HP)	From Leg	1.00 0.000 0.000	-49.000		190.00	6.00			
PR-850	A	Grid	From Leg	1.00 0.000 0.000	30.000		163.00	5.67			
***				0.000							

## **Load Combinations**

Comb.	Description
<u>No.</u> 1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
2	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 50 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1,2 Dead+1,0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43 44	Dead+Wind 120 deg - Service
44 45	Dead+Wind 150 deg - Service
45 46	Dead+Wind 180 deg - Service Dead+Wind 210 deg - Service
40 47	Dead+Wind 210 deg - Service
47 48	Dead+Wind 270 deg - Service
40 49	Dead+Wind 300 deg - Service
49 50	Dead+Wind 300 deg - Service

# **Maximum Member Forces**

Sectio n No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	200 - 180	Leg	Max Tension	15	28.63	-0.02	-0.65
			Max. Compression	2	-35.52	0.04	1.29
			Max. Mx	20	-1.77	-1.12	-0.02
			Max. My	2	-35.52	0.04	1.29
			Max Vy	18	-5.61	1.09	-0.58
			Max. Vx	2	-7.12	0.04	1.29
		Diagonal	Max Tension	25	5.38	0.00	0.00
		-	Max. Compression	24	-5.37	0.00	0.00
			Max. Mx	31	0.83	0.03	0.00
			Max. My	17	-4.52	-0.01	0.01

Sectio n	Elevation ft	Component Type	Condition	Gov. Load	Axial	Major Axis Moment	Minor Axis Moment
No.				Comb.	ĸ	kip-ft	kip-ft
			Max. Vy	31	-0.02	0.03	0.00
			Max. Vx	17	-0.00	0.01	0.01
		Top Girt	Max Tension	3	0.38	0.00	0.00
			Max. Compression	14	-0.40	0.00	0.00
			Max. Mx	26	-0.09	-0.04	0.00
			Max. Vy	26	-0.03	0.00	0.00
T2	180 - 160	Leg	Max Tension	15	73.31	-0.10	0.00
			Max. Compression	2	-85.66	0.68	-0.01
			Max. Mx	2	-41.38	1.29	-0.04
			Max. My	16	-6.57	0.08	0.62
			Max. Vy	3	-6.64	0.68	-0.01
			Max. Vx	16	-2.60	0.05	0.26
		Diagonal	Max Tension	24	5.83	0.00	0.00
			Max. Compression	24	-5.88	0.00	0.00
			Max. Mx	31	1.45	0.03	0.00
			Max. My	24	-5.62	-0.01	-0.01
			Max. Vy	31	-0.03	0.03	0.00
			Max. Vx	24	0.00	0.00	0.00
Т3	160 - 140	Leg	Max Tension	15	111.78	-0.35	0.02
			Max. Compression	2	-128.57	0.97	-0.04
			Max. Mx	14	109.95	-1.00	0.04
			Max. My	17	-13.09	0.01	1.01
			Max. Vy	2	-7.95	0.97	-0.04
			Max. Vx	16	-3.08	0.00	1.01
		Diagonal	Max Tension	24	5.93	0.00	0.00
			Max. Compression	24	-5.95	0.00	0.00
			Max. Mx	27	1.46	0.04	-0.01
			Max. My	27	-2.04	0.03	-0.01
			Max. Vy	29	0.04	0.04	-0.00
			Max. Vy Max. Vx	27	0.00	0.00	0.00
Т4	140 - 120	Leg	Max Tension	15	150.02	-0.22	0.00
17	140 120	Log	Max. Compression	2	-172.26	1.01	-0.03
			Max. Oompression Max. Mx	2	-172.26	1.01	-0.03
			Max. My	17	-13.36	0.01	1.01
			Max. Wy	18	-9.39	0.99	-0.03
			Max. Vy Max. Vx	16	-3.57	0.99	0.63
		Diagonal	Max Tension	24	7 25	0.04	0.00
		Diagonal	Max Compression	24	7.30	0.00	0.00
			•	24 27	1.96	0.08	-0.01
			Max. Mx Max. My	27	1.90	0.08	-0.01
			,	29	0.06	0.08	0.01
			Max. Vy	29 27	0.00	0.08	0.01
TE	100 100	1.00	Max. Vx				
Т5	120 - 100	Leg	Max Tension	15	184.78	-0.38	0.01
			Max. Compression	2	-211.27	1.24	-0.03
			Max. Mx	2	-211.27	1.24	-0.03
			Max. My	4	-1.44	-0.03	-0.88
			Max. Vy	18	-10.24	1.22	-0.03
		D' '	Max. Vx	16	-3.77	0.05	0.87
		Diagonal	Max Tension	4	7.28	0.00	0.00
			Max. Compression	4	-7.32	0.00	0.00
			Max. Mx	29	2.05	0.10	-0.01
			Max. My	27	-2.66	0.09	-0.01
			Max. Vy	29	0.07	0.10	-0.01
			Max. Vx	27	0.00	0.00	0.00
Т6	100 - 80	Leg	Max Tension	15	216.57	-0.74	0.03
			Max. Compression	2	247.66	1.67	-0.06
			Max. Mx	3	-244.46	1.67	-0.06
			Max. My	4	-2.43	-0.05	-1.39
			Max. Vy	18	-11.06	1.66	-0.03
			Max. Vx	16	-4.35	0.04	1.39
			Max Tension	4	7.68	0.00	0.00
		Diagonal					0.00
		Diagonal	Max. Compression	4	-7.66	0.00	0.00
		Diagonal	Max. Compression				
		Diagonal	Max. Compression Max. Mx	27	2.29	0.15	-0.02
		Diagonal	Max. Compression Max. Mx Max. My	27 27	2.29 -2.30	0.15 0.13	-0.02 -0.02
		Diagonal	Max. Compression Max. Mx Max. My Max. Vy	27 27 29	2.29 -2.30 0.09	0.15 0.13 0.15	-0.02 -0.02 0.02
Τ7	80 - 60	-	Max. Compression Max. Mx Max. My Max. Vy Max. Vx	27 27 29 27	2.29 -2.30 0.09 0.00	0.15 0.13 0.15 0.00	-0.02 -0.02 0.02 0.00
Τ7	80 - 60	Diagonal Leg	Max. Compression Max. Mx Max. My Max. Vy	27 27 29	2.29 -2.30 0.09	0.15 0.13 0.15	-0.02 -0.02 0.02

Sectio	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
n	ft	Туре		Load		Moment	Moment
No.				Comb.	K	kip-ft	kip-ft
			Max. My	4	-2.56	-0.05	-1.39
			Max. Vy	18	-12.24	2.14	-0.03
			Max. Vx	16	-4.53	0.10	1.19
		Diagonal	Max Tension	4	8.85	0.00	0.00
			Max. Compression	4	-8.88	0.00	0.00
			Max. Mx	27	3.12	0.25	0.03
			Max. My	27	2 <u>.</u> 17	0.24	-0.03
			Max. Vy	29	0.12	0.25	-0.03
			Max. Vx	27	0.01	0.00	0.00
T8	60 - 40	Leg	Max Tension	15	274.90	-1.19	0.01
			Max. Compression	2	-316.75	2.42	-0.04
			Max. Mx	2	-316.75	2.42	-0.04
			Max. My	4	-4.42	-0.15	-2.36
			Max. Vy	18	-13.71	2.41	-0.05
			Max. Vx	16	-4.55	0.11	1.68
		Diagonal	Max Tension	4	9.38	0.00	0.00
			Max. Compression	4	-9.50	0.00	0.00
			Max. Mx	29	2.81	0.29	-0.04
			Max. My	28	2.97	0.28	-0.04
			Max. Vy	29	0.13	0.29	-0.04
			Max. Vx	28	0.01	0.00	0.00
Т9	40 - 20	Leg	Max Tension	7	302.22	-1.28	0.04
		-	Max. Compression	2	-350.15	2.69	-0.05
			Max. Mx	29	57.17	-4.28	0.00
			Max. My	4	-5.71	-0.02	-1.70
			Max. Vy	18	-14.83	2.69	-0.03
			Max. Vx	16	-4.75	0.16	1.36
		Diagonal	Max Tension	4	9.75	0.00	0.00
		-	Max. Compression	4	-9.94	0.00	0.00
			Max. Mx	29	2.07	0.31	-0.04
			Max. My	27	-2.64	0.28	-0.04
			Max. Vy	29	0.14	0.31	-0.04
			Max. Vx	27	0.01	0.00	0.00
T10	20 - 0	Leg	Max Tension	7	329.54	1.23	0.00
		Ū	Max. Compression	2	-382.99	0.00	0.00
			Max, Mx	35	-167.13	4.49	-0.04
			Max, My	4	-8.42	-0.21	-3,26
			Max. Vy	18	-16.45	0.00	-0.00
			Max, Vx	16	-4.36	0.00	-0.00
		Diagonal	Max Tension	16	10.45	0.00	0.00
		3	Max. Compression	2	-10.76	0.00	0.00
			Max. Mx	29	0.79	0.42	0.04
			Max, My	28	5.94	0.30	-0.05
			Max. Vy	29	0.15	0.42	0.04
			Max. Vy Max. Vx	28	0.01	0.00	0.00
				_0		-100	-100

## **Maximum Reactions**

Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, Z
		Load	K	K	K
		Comb.			
Leg C	Max. Vert	18	377.18	33.19	-19.03
	Max. H _x	18	377.18	33.19	-19.03
	Max. H _z	7	-328.14	-29.18	16.79
	Min. Vert	7	-328.14	-29.18	16.79
	Min. H _x	7	-328.14	-29.18	16.79
	Min. H _z	18	377.18	33.19	-19.03
Leg B	Max. Vert	10	363.39	-31.54	-18.49
-	Max. H _x	23	-311.90	27.43	16.19
	Max. H _z	23	-311.90	27.43	16.19
	Min. Vert	23	-311.90	27.43	16.19
	Min. H _x	10	363.39	-31.54	-18.49
	Min. H _z	10	363.39	-31.54	-18.49
Leg A	Max. Vert	2	381.43	0.40	38.00
5	Max. H _x	20	21.68	4.08	1.64

Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, Z
		Load	K	K	K
		Comb.			
	Max. H _z	2	381.43	0.40	38.00
	Min. Vert	15	-326.87	-0.40	-33.10
	Min. H _x	9	16.32	-4.07	1.23
	Min. H _z	15	-326.87	-0.40	-33.10

<b>Tower Mast Reaction Sun</b>	nmary
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Load Combination	Vertical	Shear _x	Shearz	Overturning Moment, M _x	Overturning Moment, Mz	Torque
	K	ĸ	K	kip-ft	kip-ft	kip-ft
Dead Only	53.59	0.00	-0.00	-5.32	-5.20	0.00
1.2 Dead+1.0 Wind 0 deg - No Ice	64.31	0.04	-60.32	-7170.52	-14.23	15.78
0.9 Dead+1.0 Wind 0 deg -	48.23	0.04	-60.32	-7168.92	-12.67	15.78
No Ice						
1 2 Dead+1 0 Wind 30 deg -	64.31	29.18	-52.99	-6338.93	-3393.17	17.17
No Ice 0.9 Dead+1.0 Wind 30 deg -	48.23	29.18	-52.99	-6337.33	-3391.61	17.17
No Ice	40.23	29.10	-52.99	-0337.33	-3391.01	17.17
1.2 Dead+1.0 Wind 60 deg -	64.31	49.34	-30.41	-3706.61	-5779.59	1.91
No Ice						
0.9 Dead+1.0 Wind 60 deg -	48.23	49.34	-30.41	-3705.02	-5778.02	1.91
No Ice 1.2 Dead+1.0 Wind 90 deg -	64,31	57.49	0.00	-6.35	-6748,48	-13.01
No Ice	04.01	07.40	0.00	0.00	07-10.10	10.01
0.9 Dead+1.0 Wind 90 deg -	48.23	57.49	0.00	-4.76	-6746.92	-13.01
Nolce						
1.2 Dead+1.0 Wind 120 deg - No Ice	64.31	49.54	30.30	3641.43	-5762.44	-9.62
0.9 Dead+1.0 Wind 120 deg	48,23	49.54	30.30	3643.02	-5760.88	-9.62
- No Ice						
1.2 Dead+1.0 Wind 150 deg	64.31	26.46	48.02	5819.58	-3127.13	-3.41
- No Ice	48.23	26.46	48.02	5821.17	-3125.57	-3.41
0.9 Dead+1.0 Wind 150 deg - No Ice	40.23	20.40	40.02	3021.17	-3125.57	-3.41
1.2 Dead+1.0 Wind 180 deg	64.31	0.04	56.83	6829.52	-12.10	-16.10
- No Ice						
0.9 Dead+1.0 Wind 180 deg	48.23	0.04	56.83	6831.12	-10.54	-16.10
- No Ice 1.2 Dead+1.0 Wind 210 deg	64,31	-29,07	52,66	6263,82	3361,23	-19,23
- No Ice	01.01	20.07	02.00	0200.02	0001.20	10.20
0.9 Dead+1.0 Wind 210 deg	48.23	-29.07	52.66	6265.41	3362.79	-19.23
- No Ice	64.24	50.00	24 75	2702.02	5008.04	-4.60
1.2 Dead+1.0 Wind 240 deg - No Ice	64.31	-52.08	31.75	3782.82	5998.01	-4.00
0.9 Dead+1.0 Wind 240 deg	48.23	-52.08	31.75	3784.42	5999.57	-4.60
- No Ice						
1.2 Dead+1.0 Wind 270 deg	64.31	-57.51	0.00	-4.80	6740.28	12.86
- No Ice 0.9 Dead+1.0 Wind 270 deg	48.23	-57.51	0.00	-3.21	6741.85	12.86
- No Ice	10.20	01.01	0.00	0.21	0111.00	12.00
1.2 Dead+1.0 Wind 300 deg	64.31	-46.66	-28.89	-3551.38	5492.43	12.19
	40.00	10.00		0540.70	E 400.00	40.40
0.9 Dead+1.0 Wind 300 deg - No Ice	48.23	-46.66	-28.89	-3549.79	5493.99	12.19
1.2 Dead+1.0 Wind 330 deg	64.31	-26.44	-48.30	-5883.58	3112.40	5.12
- No Ice						
0.9 Dead+1.0 Wind 330 deg	48.23	-26.44	-48.30	-5881.99	3113.96	5.12
- No Ice 1.2 Dead+1.0 Ice+1.0 Temp	184,06	0.00	-0.00	-22.10	-64.20	0.00
1.2 Dead+1.0 Wind 0	184.06	0.00	-19.65	-2367.75	-65.21	9.00
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 30	184.06	9.92	-17.62	-2121.90	-1228.56	8.87
deg+1.0 Ice+1.0 Temp	104 06	17 20	10.07	1060 07	2101 55	0 10
1.2 Dead+1.0 Wind 60	184.06	17.38	-10.37	-1263.27	-2101.55	2.13

Load Combination	Vertical	Shearx	Shear₂	Overturning	Overturning	Torque
Complination	к	к	К	Moment, M _x kip-ft	Moment, Mz kip-ft	kip-ft
dea+1.0 lce+1.0 Temp	<u>^</u>	<u>^</u>	<u> </u>	кір-п	кір-п	кір-п
1.2 Dead+1.0 Wind 90	184.06	19.60	-0.01	-24.15	-2376.77	-4.66
	184.06	19.60	-0.01	-24.15	-23/0.//	-4.00
deg+1.0 Ice+1.0 Temp	404.00	40.00	0.75		4000 40	F 00
1.2 Dead+1.0 Wind 120	184.06	16.36	9.75	1152.50	-1996.40	-5.80
deg+1.0 Ice+1.0 Temp	404.00	0.07	40.05	4000.04	4400.40	5.04
1.2 Dead+1.0 Wind 150	184.06	8.97	16.35	1960.34	-1126.13	-5.24
deg+1.0 Ice+1.0 Temp	101.00	0.40	10.04	0000.04	44.05	0.07
1.2 Dead+1.0 Wind 180	184.06	-0.12	19.24	2290.91	-44.05	-8.27
deg+1.0 Ice+1.0 Temp		<b>-</b>				
1.2 Dead+1.0 Wind 210	184.06	-9.95	17.65	2079.77	1104.30	-9.01
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 240	184.06	-17.75	10.70	1253.49	2003.24	-2.75
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 270	184.06	-19.68	0.21	12.29	2260.70	4.76
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 300	184.06	-15.97	-9.56	-1183.06	1834.49	6.29
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 330	184.06	-9.11	-16.24	-1987.62	1019.30	6.42
deg+1.0 Ice+1.0 Temp						
Dead+Wind 0 deg - Service	53.59	0.01	-15.09	-1780.31	-7.14	4.19
Dead+Wind 30 deg - Service	53.59	7.31	-13.25	-1573.60	-845.37	4.59
Dead+Wind 60 deg - Service	53.59	12.37	-7.61	-921.47	-1437.87	0.83
Dead+Wind 90 deg - Service	53.59	14.41	0.00	-5.31	-1677.88	-2.94
Dead+Wind 120 deg -	53.59	12.42	7.58	898.13	-1433.71	-2.33
Service						
Dead+Wind 150 deg -	53.59	6.65	12.05	1440.11	-780.85	-1.04
Service						
Dead+Wind 180 deg -	53.59	0.01	14.25	1690.07	-6.62	-4.27
Service						
Dead+Wind 210 deg -	53,59	7.28	13,17	1547.85	830.25	-5.09
Service						
Dead+Wind 240 deg -	53.59	-13.03	7.93	932.42	1483.46	-1.48
Service						
Dead+Wind 270 deg -	53.59	-14.41	0.00	-4.93	1668.51	2.91
Service	00100		0.00			2.01
Dead+Wind 300 deg -	53.59	-11.72	-7.24	-883.82	1360.85	2.96
Service	00.00	11.72	7.27	000.02	1000.00	2.00
Dead+Wind 330 deg -	53.59	-6.65	-12.12	-1463.17	769.90	1.45
Service	00.00	0.00	12.12	1400.17	100.00	1.40

# **Solution Summary**

	Sun	n of Applied Force	es		Sum of Reactio	ns	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	ĸ	K	ĸ	
1	0.00	-53.59	0.00	-0.00	53.59	0.00	0.000%
2	0.04	-64.31	-60.32	-0.04	64.31	60.32	0.000%
3	0.04	-48.23	-60.32	-0.04	48.23	60.32	0.000%
4	29.18	-64.31	-52.99	-29.18	64.31	52.99	0.000%
5	29.18	-48.23	-52.99	-29.18	48.23	52,99	0.000%
6	49.34	-64.31	-30.41	-49.34	64.31	30.41	0.000%
7	49.34	-48.23	-30.41	-49.34	48.23	30.41	0.000%
8	57.49	-64.31	0.00	-57.49	64.31	-0.00	0.000%
9	57.49	-48.23	0.00	-57.49	48.23	-0.00	0.000%
10	49.54	-64.31	30.30	-49.54	64.31	-30.30	0.000%
11	49.54	-48.23	30.30	-49.54	48.23	-30.30	0.000%
12	26.46	-64.31	48.02	-26.46	64.31	-48.02	0.000%
13	26.46	-48.23	48.02	-26.46	48.23	-48.02	0.000%
14	0.04	-64.31	56.83	-0.04	64.31	-56.83	0.000%
15	0.04	-48.23	56.83	-0.04	48.23	-56.83	0.000%
16	-29.07	-64.31	52.65	29.07	64.31	-52.66	0.000%
17	-29.07	-48.23	52.65	29.07	48.23	-52.66	0.000%
18	-52.08	-64.31	31.75	52.08	64.31	-31.75	0.000%
19	-52.08	-48.23	31.75	52.08	48.23	-31.75	0.000%
20	-57.51	-64.31	0.00	57.51	64.31	-0.00	0.000%
21	-57.51	-48.23	0.00	57.51	48.23	-0.00	0.000%

	Sun	n of Applied Force	s		Sum of Reactions			
Load	PX	PY	PZ	PX	PY	PZ	% Error	
Comb.	ĸ	K	K	ĸ	K	ĸ		
22	-46.66	-64.31	-28.89	46.66	64.31	28.89	0.000%	
23	-46.66	-48.23	-28.89	46.66	48.23	28.89	0.000%	
24	-26.44	-64.31	-48.30	26.44	64.31	48.30	0.000%	
25	-26.44	-48.23	-48.30	26.44	48.23	48.30	0.000%	
26	0.00	-184.06	0.00	-0.00	184.06	0.00	0.000%	
27	0.00	-184.06	-19.65	-0.00	184.06	19.65	0.000%	
28	9.92	-184.06	-17.62	-9.92	184.06	17.62	0.000%	
29	17.38	-184.06	-10.37	-17.38	184.06	10.37	0.000%	
30	19.60	-184.06	-0.01	-19.60	184.06	0.01	0.000%	
31	16.36	-184.06	9.75	-16.36	184.06	-9.75	0.000%	
32	8.97	-184.06	16.35	-8.97	184.06	-16.35	0.000%	
33	-0.12	-184.06	19.24	0.12	184.06	-19.24	0.000%	
34	-9.95	-184.06	17.65	9.95	184.06	-17.65	0.000%	
35	-17.75	-184.06	10.70	17.75	184.06	-10.70	0.000%	
36	-19.68	-184.06	0.21	19.68	184.06	-0.21	0.000%	
37	-15.97	-184.06	-9.56	15.97	184.06	9.56	0.000%	
38	-9.11	-184.06	-16.24	9.11	184.06	16.24	0.000%	
39	0.01	-53.59	-15.09	-0.01	53.59	15.09	0.000%	
40	7.31	-53.59	-13.25	-7.31	53.59	13.25	0.000%	
41	12.37	-53.59	-7.61	-12.37	53.59	7.61	0.000%	
42	14.41	-53.59	0.00	-14.41	53.59	-0.00	0.000%	
43	12.42	-53.59	7.58	-12.42	53.59	-7.58	0.000%	
44	6.65	-53.59	12.05	-6.65	53.59	-12.05	0.000%	
45	0.01	-53.59	14.25	-0.01	53.59	-14.25	0.000%	
46	-7.28	-53.59	13.17	7.28	53.59	-13.17	0.000%	
47	-13.03	-53.59	7.93	13.03	53.59	-7.93	0.000%	
48	-14.41	-53.59	0.00	14.41	53.59	-0.00	0.000%	
49	-11.72	-53.59	-7.24	11.72	53.59	7.24	0.000%	
50	-6.65	-53.59	-12.12	6.65	53.59	12.12	0.000%	

## **Maximum Tower Deflections - Service Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	o
T1	200 - 180	6.946	39	0.346	0.048
T2	180 - 160	5.497	40	0.332	0.037
Т3	160 - 140	4.170	40	0.282	0.024
Т4	140 - 120	3.064	40	0.226	0.017
T5	120 - 100	2.168	40	0.185	0.013
Т6	100 - 80	1.438	40	0.145	0.009
Τ7	80 - 60	0.881	40	0.103	0.006
Т8	60 - 40	0.491	40	0.072	0.004
Т9	40 - 20	0.229	40	0.041	0.003
T10	20 - 0	0.071	47	0.020	0.001

## Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	0	ft
200.00	15" Dia. x 15" Beacon	39	6.946	0.346	0.048	158913
198.00	DB225-A	39	6.799	0.346	0.047	158913
190.00	USX6-6W-6GR	40	6.214	0.342	0.043	79457
182.00	APXVAARR24_43-U-NA20 w/ Mount Pipe	40	5.638	0.335	0.038	44105
170.00	APXVTM14-ALU-I20 w/ Mount Pipe	40	4.811	0.310	0.030	25761
163.00	PR-850	40	4.356	0.291	0.026	20713
144.00	(2) PD1109-1	40	3.267	0.236	0.018	21571
134.00	MX08FRO665-21 w/ Mount Pipe	40	2.776	0.212	0.016	24466

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Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	٥	ft
102.00	3" x 6" SideLight	40	1.503	0.149	0.009	27286
53.00	KS24019-L112A	40	0.386	0.061	0.004	41686

# Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
T1	200 - 180	28.057	2	1.393	0.139
T2	180 - 160	22.210	2	1.338	0.114
Т3	160 - 140	16.843	4	1.138	0.080
T4	140 - 120	12.377	4	0.914	0.059
T5	120 - 100	8.754	4	0.747	0.044
Т6	100 - 80	5.802	4	0.584	0.031
Τ7	80 - 60	3.552	4	0.415	0.021
Т8	60 - 40	1.976	4	0.290	0.015
Т9	40 - 20	0.919	4	0.164	0.010
T10	20 - 0	0.285	19	0.082	0.005

# Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	0	ft
200.00	15" Dia. x 15" Beacon	2	28.057	1.393	0.139	41669
198.00	DB225-A	2	27.465	1.391	0.137	41669
190.00	USX6-6W-6GR	2	25.104	1.379	0.128	20834
182.00	APXVAARR24_43-U-NA20 w/	2	22.781	1.349	0.117	11552
	Mount Pipe					
170.00	APXVTM14-ALU-I20 w/ Mount	2	19.432	1.252	0.096	6528
	Pipe					
163.00	PR-850	4	17.595	1.174	0.084	5185
144.00	(2) PD1109-1	4	13.197	0.955	0.062	5350
134.00	MX08FRO665-21 w/ Mount Pipe	4	11 <u>.</u> 211	0.859	0.055	6053
102.00	3" x 6" SideLight	4	6.066	0.602	0.032	6728
53.00	KS24019-L112A	4	1.554	0.244	0.013	10297

## **Bolt Design Data**

Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of	Maximum Load	Allowable Load	Ratio Load	Allowable Ratio	Criteria
	ft			in	Bolts	per Bolt K	per Bolt K	Allowable		
T1	200	Leg	A325X	0.750	4	7.16	30.10	0.238	1.05	Bolt Tension
		Diagonal	A325X	0.625	1	5.38	7.88	0.684	1.05	Member Block Shear
		Top Girt	A325X	0.625	1	0.38	7.88	0.049	1.05	Member Block Shear
T2	180	Leg	A325X	1.000	4	18.33	54.52	0.336	1.05	Bolt Tension
		Diagonal	A325X	0.625	1	5.83	7.88	0.740	1.05	Member Block Shear
Т3	160	Leg	A325X	1.000	4	27.94	54.52	0.513	1.05	Bolt Tension
		Diagonal	A325X	0.625	1	5.93	7.88	0.753	1.05	Member Block Shear
T4	140	Leg	A325X	1.250	4	37.50	87.22	0.430	1.05	Bolt Tension
		Diagonal	A325X	0.625	1	7.25	9.91	0.731	1.05	Member Block

Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of	Maximum Load	Allowable Load	Ratio Load	Allowable Ratio	Criteria
	ft			in	Bolts	per Bolt K	per Bolt K	Allowable		
										Shear
T5	120	Leg	A325X	1.250	4	46.20	87.22	0.530	1.05	Bolt Tension
		Diagonal	A325X	0.625	1	7.28	9.91	0.734	1.05	Member Block Shear
T6	100	Leg	A325X	1.250	6	36.09	87.22	0.414	1.05	Bolt Tension
		Diagonal	A325X	0.750	1	7.68	10.42	0.736	1.05	Member Block Shear
T7	80	Leg	A325X	1.250	6	41.07	87.22	0.471	1.05	Bolt Tension
		Diagonal	A325X	0.750	1	8.85	14.36	0.617	1.05	Member Bearing
T8	60	Leg	A325X	1.375	6	45.82	103.94	0.441	1.05	Bolt Tension
		Diagonal	A325X	0.750	1	9.38	14.36	0.653	1.05	Member Bearing
Т9	40	Leg	A325X	1.375	6	50.37	103.94	0.485	1.05	Bolt Tension
		Diagonal	A325X	0.750	1	9.75	14.36	0.679	1.05	Member Bearing
T10	20	Diagonal	A325X	0.750	1	10.45	14.36	0.728	1.05	Member Bearing

# **Compression Checks**

		Leg De	esign [	Data	(Comp	oressi	on)		
Section No.	Elevation	Size	L	Lu	Kl/r	Α	Pu	φPn	Ratio Pu
	ft		ft	ft		in²	ĸ	К	$\phi P_n$
T1	200 - 180	Sabre 2.875x.375	20.00	4.98	66.9 K=1.00	2.945	-28.43	95.59	0.297 ¹
T2	180 - 160	Sabre 3.5 x .3	20.03	4.99	52.7 K=1.00	3.016	-79.49	110.80	0.717 ¹
Т3	160 - 140	Sabre 4 x .318	20.03	4.99	45.8 K=1.00	3.678	-122.80	141.99	0.865 ¹
T4	140 - 120	Sabre 4.5 x .438	20.03	6.65	55.2 K=1.00	5.589	-164.75	201.22	0.819 ¹
T5	120 - 100	Sabre 5.5625 x .375	20.03	6.65	43.4 K=1.00	6.111	-204.37	239.63	0.853 ¹
Т6	100 - 80	Sabre 5.5625 x .375	20.03	6.65	43.4 K=1.00	6.111	-241.22	239.63	1.007 ¹
Τ7	80 - 60	Sabre 6.625 x .432	20.03	9.97	54.5 K=1.00	8.405	-273.59	304.30	0.899 ¹
Т8	60 - 40	Sabre 8.625 x .322	20.03	9.97	40.7 K=1.00	8.399	-307.43	334.76	0.918 ¹
Т9	40 - 20	Sabre 8.625 x .5	20.03	9.97	41.6 K=1.00	12.763	-340.99	506.09	0.674 ¹
T10	20 - 0	Sabre 8.625 x .5	20.03	9.97	41.6 K=1.00	12.763	-373.44	506.09	0.738 ¹

¹  $P_u$  /  $\phi P_n$  controls

	Diagonal Design Data (Compression)											
Section No.	Elevation	Size	L	Lu	Kl/r	A	Pu	$\phi P_n$	Ratio Pu			
	ft		ft	ft		in²	К	K	$\phi P_n$			
T1	200 - 180	L1 3/4x1 3/4x3/16	7.06	3.21	114.2 K=1.02	0.621	-5.37	13.19	0.407 ¹			

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Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio Pu
	ft		ft	ft		in²	ĸ	K	$\phi P_n$
Т2	180 - 160	L1 3/4x1 3/4x3/16	8.38	4.02	140.4 K=1.00	0.621	-5.78	9.01	0.641 ¹
Т3	160 - 140	L1 3/4x1 3/4x3/16	10.06	4.84	169.0 K=1.00	0.621	-5.95	6.22	0.956 ¹
T4	140 - 120	L2 1/2x2 1/2x3/16	12.56	6.11	148.1 K=1.00	0.902	-7.23	11.77	0.614 ¹
Т5	120 - 100	L2 1/2x2 1/2x3/16	14.30	6.93	168.0 K=1.00	0.902	-7.32	9.15	0.800 ¹
Т6	100 - 80	L3x3x3/16	16.09	7.83	157.6 K=1.00	1.090	-7.66	12.56	0.610 ¹
Τ7	80 - 60	L3 1/2x3 1/2x1/4	19.27	9 <u>.</u> 46	163.5 K=1.00	1.690	-8.88	18.09	0.491 ¹
Т8	60 - 40	L3 1/2x3 1/2x1/4	21.01	10.23	176.9 K=1.00	1.690	-9.50	15.45	0.615 ¹
Т9	40 - 20	L3 1/2x3 1/2x1/4	22.79	11.12	192.4 K=1.00	1.690	-9.94	13.07	0.761 ¹
T10	20 - 0	L4x4x1/4	24.60	12.03	181.6 K=1.00	1.940	-10.76	16.83	0.639 ¹

¹  $P_u$  /  $\phi P_n$  controls

	Top Girt Design Data (Compression)											
Section No.	Elevation	Size	L	Lu	Kl/r	A	Pu	$\phi P_n$	Ratio P _u			
	ft		ft	ft		in²	ĸ	К	$\phi P_n$			
<b>T</b> 1	200 - 180	L1 3/4x1 3/4x3/16	5.00	4.47	156.1 K=1.00	0.621	-0.40	7.29	0.055 ¹			

¹  $P_u$  /  $\phi P_n$  controls

## **Tension Checks**

	Leg Design Data (Tension)												
Section No.	Elevation	Size	L	Lu	Kl/r	А	Pu	$\phi P_n$	Ratio Pu				
	ft		ft	ft		in²	ĸ	ĸ	$\phi P_n$				
T1	200 - 180	Sabre 2.875x.375	20.00	0.08	1.1	2.945	28.63	132.54	0.216 1				
T2	180 - 160	Sabre 3.5 x .3	20.03	0.08	0.9	3.016	73.31	135.72	0.540 ¹				
Т3	160 - 140	Sabre 4 x .318	20.03	0.08	0.8	3.678	111.78	165.53	0.675 ¹				
T4	140 - 120	Sabre 4.5 x .438	20.03	0.08	0.7	5.589	150.02	251.52	0.596 ¹				
T5	120 - 100	Sabre 5.5625 x .375	20.03	0.08	0.5	6.111	184.78	275.01	0.672 ¹				
T6	100 - 80	Sabre 5.5625 x .375	20.03	0.08	0.5	6.111	216.57	275.01	0.787 ¹				
T7	80 - 60	Sabre 6.625 x .432	20.03	0.08	0.5	8.405	246.42	378.22	0.652 ¹				
Т8	60 - 40	Sabre 8.625 x .322	20.03	0.08	0.3	8.399	274.90	377.97	0.727 ¹				
Т9	40 - 20	Sabre 8.625 x .5	20.03	0.08	0.3	12.763	302.23	574.32	0.526 ¹				
T10	20 - 0	Sabre 8.625 x .5	20.03	0.08	0.3	12.763	329.54	574.32	0.574 ¹				

¹  $P_u$  /  $\phi P_n$  controls

# Diagonal Design Data (Tension)

Section No.	Elevation	Size	L	Lu	Kl/r	A	Pu	$\phi P_n$	Ratio Pu
	ft		ft	ft		in²	K	К	$\phi P_n$
T1	200 - 180	L1 3/4x1 3/4x3/16	7.06	3.21	75.1	0.360	5.38	15.68	0.343 ¹
T2	180 - 160	L1 3/4x1 3/4x3/16	8.38	4.02	93.1	0.360	5.83	15.68	0.372 ¹
Т3	160 - 140	L1 3/4x1 3/4x3/16	10.06	4.84	111.4	0.360	5.93	15.68	0.378 ¹
T4	140 - 120	L2 1/2x2 1/2x3/16	12.00	5.83	92.2	0.571	7.25	24.84	0.292 ¹
T5	120 - 100	L2 1/2x2 1/2x3/16	14.30	6.93	109.1	0.571	7.28	24.84	0.293 ¹
T6	100 - 80	L3x3x3/16	16.09	7.83	101.9	0.694	7.68	30.21	0.254 ¹
Τ7	80 - 60	L3 1/2x3 1/2x1/4	19.27	9.46	105.7	1.103	8.85	48.00	0.184 ¹
Т8	60 - 40	L3 1/2x3 1/2x1/4	21.01	10.23	114.3	1.103	9.38	48.00	0.195 ¹
Т9	40 - 20	L3 1/2x3 1/2x1/4	22.79	11.12	124.1	1.103	9.75	48.00	0.203 ¹
T10	20 - 0	L4x4x1/4	24.60	12.03	116.9	1.291	10.45	56.16	0.186 ¹

¹  $P_u$  /  $\phi P_n$  controls

	Top Girt Design Data (Tension)										
Section No.	Elevation	Size	L	Lu	Kl/r	A	Pu	φPn	Ratio Pu		
	ft		ft	ft		in²	К	К	$\phi P_n$		
T1	200 - 180	L1 3/4x1 3/4x3/16	5.00	4.47	106.4	0.360	0.38	15.68	0.024 1		

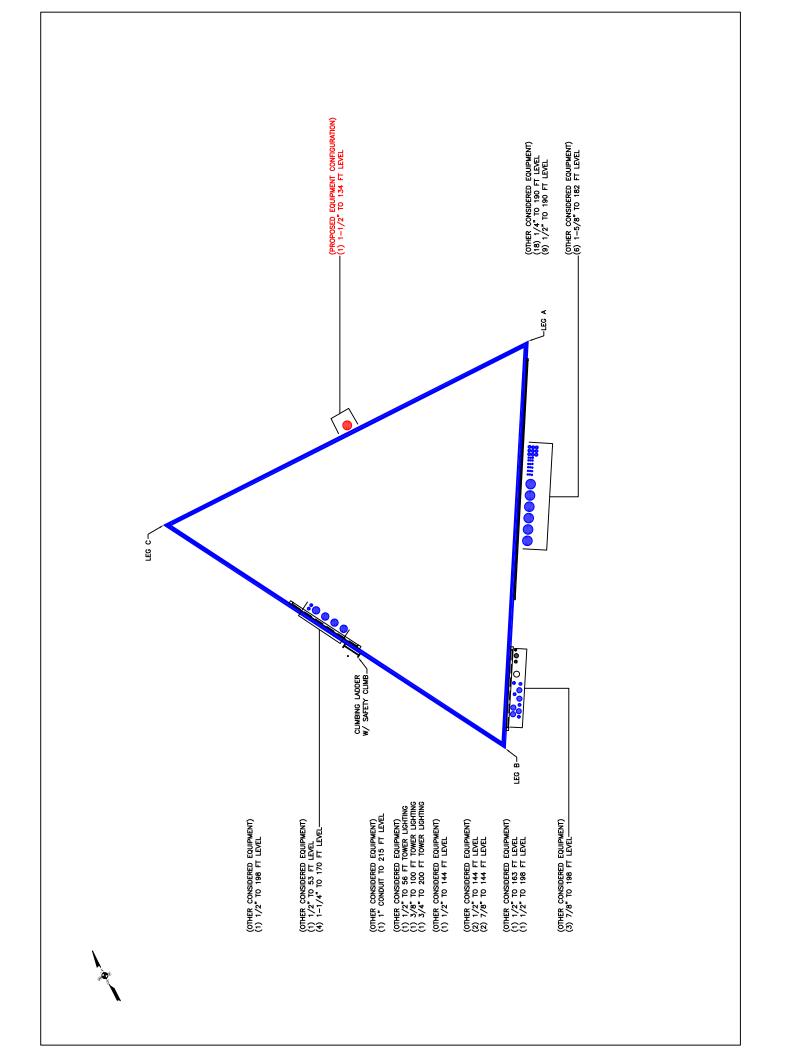
¹  $P_u$  /  $\phi P_n$  controls

# **Section Capacity Table**

Section	Elevation	Component	Size	Critical	Р		%	Pass
No.	ft	Type		Element	K	K	Capacity	Fail
T1	200 - 180	Leg	Sabre 2.875x.375	3	-28.43	100.37	28.3	Pass
T2	180 - 160	Leg	Sabre 3.5 x .3	33	-79.49	116.34	68.3	Pass
Т3	160 - 140	Leg	Sabre 4 x .318	60	-122.80	149.09	82.4	Pass
T4	140 - 120	Leg	Sabre 4.5 x .438	87	-164.75	211.28	78.0	Pass
T5	120 - 100	Leg	Sabre 5.5625 x .375	108	-204.37	251.62	81.2	Pass
<b>T</b> 6	100 - 80	Leg	Sabre 5.5625 x .375	129	-241.22	251.62	95.9	Pass
Τ7	80 - 60	Leg	Sabre 6.625 x .432	150	-273.59	319.52	85.6	Pass
Т8	60 - 40	Leg	Sabre 8.625 x .322	165	-307.43	351.50	87.5	Pass
Т9	40 - 20	Leg	Sabre 8.625 x .5	180	-340.99	531.40	64.2	Pass
T10	20 - 0	Leg	Sabre 8.625 x .5	195	-373.44	531 <u>.</u> 40	70.3	Pass
T1	200 - 180	Diagonal	L1 3/4x1 3/4x3/16	10	-5.37	13.85	38.8	Pass
T2	180 - 160	Diagonal	L1 3/4x1 3/4x3/16	37	-5.78	9.47	61.0	Pass
Т3	160 - 140	Diagonal	L1 3/4x1 3/4x3/16	64	-5.95	6.54	91.0	Pass
Τ4	140 - 120	Diagonal	L2 1/2x2 1/2x3/16	91	-7.23	12.36	58.5	Pass
T5	120 - 100	Diagonal	L2 1/2x2 1/2x3/16	113	-7.32	9.61	76.2	Pass
Т6	100 - 80	Diagonal	L3x3x3/16	134	-7.66	13.18	58.1	Pass
Τ7	80 - 60	Diagonal	L3 1/2x3 1/2x1/4	155	-8.88	18.99	46.7	Pass
Т8	60 - 40	Diagonal	L3 1/2x3 1/2x1/4	170	-9.50	16.23	58.5	Pass
Т9	40 - 20	Diagonal	L3 1/2x3 1/2x1/4	185	-9.94	13.73	72.4	Pass
T10	20 - 0	Diagonal	L4x4x1/4	199	-10.76	17.67	60.9	Pass
T1	200 - 180	Top Girt	L1 3/4x1 3/4x3/16	4	-0.40	7.66	5.3	Pass
							Summary	
						Leg (T6)	95.9	Pass
						Diagonal	91.0	Pass
						(T3)		
						Top Girt	5.3	Pass
						(T1)		
						Bolt	71.7	Pass
						Checks		
						RATING =	95.9	Pass

#### **APPENDIX B**

#### **BASE LEVEL DRAWING**



#### **APPENDIX C**

#### ADDITIONAL CALCULATIONS

#### Self Support Anchor Rod Capacity



Site Info	
BU #	871584
Site Name	John Tom Hill
Order #	556617 Rev.1

Analysis Considerations		
TIA-222 Revision	Н	
Grout Considered:	No	
I _{ar} (in)	1.125	

Applied Loads			
	Comp.	Uplift	
Axial Force (kips)	381.43	328.14	
Shear Force (kips)	38.00	33.67	
*TIA-222-H Section 15.5 Applied			

Considered Eccentricity	
Leg Mod Eccentricity (in)	0.000
Anchor Rod N.A Shift (in)	0.000
Total Eccentricity (in)	0.000

*Anchor Rod Eccentricity Applied

Connection Prope	rtioc
connection riope	i ues

#### Anchor Rod Data

(8) 1-1/2" ø bolts (A572-50 N; Fy=50 ksi, Fu=65 ksi) I _{ar} (in): 1.125

000

#### Analysis Results

_	Anchor Rod Summary		(units of kips, kip-in)
_	Pu_c = 47.68	φPn_c = 79.52	Stress Rating
	Vu = 4.75	φVn = 35.78	58.8%
	Mu = n/a	φMn = n/a	Pass

# CROWN

## **Pier and Pad Foundation**

BU # :	<b>BU #</b> : 871584	
	John Tom Hill	
App. Number:	556617 Rev.1	

TIA-222 Revision: Н Self Support Tower Type:

Top & Bot. Pad Rein. Different?:	
Block Foundation?:	
Rectangular Pad?:	

Superstructure Analysis Reactions		
Compression, P _{comp} :	381.43	kips
Compression Shear, Vu_comp:	38	kips
Uplift, <b>P_{uplift}:</b>	328.14	kips
Uplift Shear, <b>V_{u_uplift}:</b>	20.3333333	kips
Tower Height, H:	200	ft
Base Face Width, <b>BW</b> :	23	ft
BP Dist. Above Fdn, <b>bp_{dist}:</b>	2.625	in

Pier Properties		
Pier Shape:	Circular	
Pier Diameter, <b>dpier</b> :	3.5	ft
Ext. Above Grade, E:	0.4167	ft
Pier Rebar Size, <b>Sc</b> :	7	
Pier Rebar Quantity, <b>mc</b> :	14	
Pier Tie/Spiral Size, <b>St</b> :	3	
Pier Tie/Spiral Quantity, <b>mt</b> :	10	
Pier Reinforcement Type:	Tie	
Pier Clear Cover, <b>cc_{pier}:</b>	3	in

Pad Properties		
Depth, D:	10.5	ft
Pad Width, <b>W</b> ₁ :	15	ft
Pad Thickness, <b>T</b> :	1.75	ft
Pad Rebar Size (Bottom dir. 2), Sp ₂ :	7	
Pad Rebar Quantity (Bottom dir. 2), mp ₂ :	20	
Pad Clear Cover, <b>cc_{pad}:</b>	3	in

Material Properties					
Rebar Grade, <b>Fy</b> :	60	ksi			
Concrete Compressive Strength, F'c:	3	ksi			
Dry Concrete Density, $\delta c$ :	150	pcf			

Soil Properties			
Total Soil Unit Weight, $\gamma$ :	125	pcf	
Ultimate Gross Bearing, Qult:	16.000	ksf	
Cohesion, <b>Cu</b> :	0.000	ksf	
Friction Angle, $\varphi$ :	36	degrees	
SPT Blow Count, N _{blows} :	28		
Base Friction, $\mu$ :	0.6		
Neglected Depth, N:	3.33	ft	
Foundation Bearing on Rock?	No		
Groundwater Depth, gw:	8	ft	

Foundation Analysis Checks					
	Capacity	Demand	Rating*	Check	
Uplift (kips)	451.48	328.14	69.2%	Pass	
Lateral (Sliding) (kips)	124.75	20.33	15.5%	Pass	
Bearing Pressure (ksf)	12.00	3.15	25.0%	Pass	
Pier Flexure (Comp.) (kip*ft)	990.29	348.33	33.5%	Pass	
Pier Flexure (Tension) (kip*ft)	241.44	186.39	73.5%	Pass	
Pier Compression (kip)	4592.74	396.76	8.2%	Pass	
Pad Flexure (kip*ft)	858.77	423.95	47.0%	Pass	
Pad Shear - 1-way (kips)	246.78	111.80	43.1%	Pass	
Pad Shear - 2-way (Comp) (ksi)	0.164	0.119	68.7%	Pass	
Flexural 2-way (Comp) (kip*ft)	990.20	209.00	20.1%	Pass	
Pad Shear - 2-way (Uplift) (ksi)	0.164	0.114	66.0%	Pass	
Flexural 2-way (Tension) (kip*ft)	990.20	111.83	10.8%	Pass	

*Rating per TIA-222-H Section 15.5

Structural Rating*:	73.5%
Soil Rating*:	69.2%

<--Toggle between Gross and Net



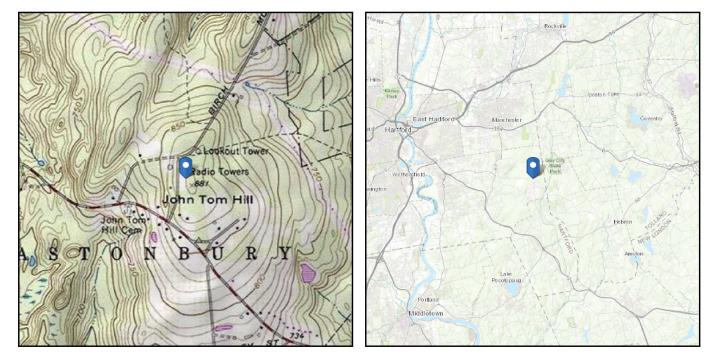
No Address at This

Location

# ASCE 7 Hazards Report

Standard:ASCE/SEI 7-10Risk Category:IISoil Class:D - Stiff Soil

Elevation: 877.95 ft (NAVD 88) Latitude: 41.708956 Longitude: -72.473447



# Wind

### **Results**:

Wind Speed:	125 Vmph
10-year MRI	77 Vmph
25-year MRI	87 Vmph
50-year MRI	94 Vmph
100-year MRI	102 Vmph

### Date Socessed:

**ASOLE/(SEI 2020**, Fig. 26.5-1A and Figs. CC-1–CC-4, and Section 26.5.2, incorporating errata of March 12, 2014

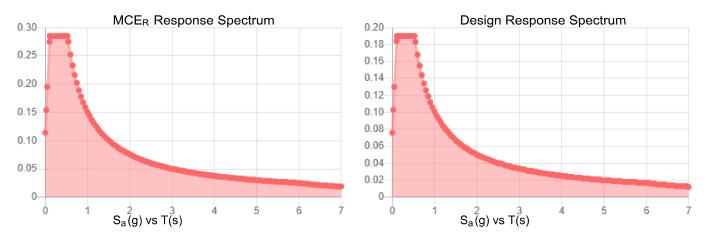
Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.



Site Soil Class:	D - Stiff Soil			
Results:				
S _s :	0.178	S _{DS} :	0.19	
<b>S</b> ₁ :	0.063	<b>S</b> _{D1} :	0.101	
F _a :	1.6	Τ _L :	6	
F _v :	2.4	PGA :	0.09	
S _{MS} :	0.285	PGA M :	0.143	
S _{M1} :	0.151	F _{PGA} :	1.6	
		e :	1	

### Seismic Design Category B



Data Accessed: Date Source:

#### Fri Jun 04 2021

USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-10 Ch. 21 are available from USGS.



### Ice

#### Results:

Ice Thickness:	1.00 in.
Concurrent Temperature:	5 F
Gust Speed:	50 mph
Data Source:	Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8
Date Accessed:	Fri Jun 04 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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# Exhibit E

**Mount Analysis** 

Darcy Tarr Crown Castle 3530 Toringdon Way, Suite 300 Charlotte, NC 28277 (704) 405-6589		Trylon 1825 W. Walnut Hill Lane, Suite 302 Irving, TX 75038 214-930-1730
Subject:	Mount Replacement Analysis Report	
Carrier Designation:	DISH Network Equipment Change-Out Carrier Site Number: Carrier Site Name:	BOBDL00076A CT-CCI-T-871584
Crown Castle Designation:	Crown Castle BU Number: Crown Castle Site Name: Crown Castle JDE Job Number: Crown Castle Order Number:	871584 John Tom Hill 650066 556617 Rev. <i>1</i>
Engineering Firm Designation:	Trylon Report Designation:	189057
Site Data:	115 Birch Mtn. Road, Glastonbury, Har Latitude 41°42'32.24" Longitude -72°28'	• • • •
Structure Information:	Tower Height & Type: Mount Elevation: Mount Type:	200.0 ft Self-Support 134.0 ft 8.0 ft Sector Frame
Dear Darcy Tarr,		

. N. / .

Trylon is pleased to submit this **"Mount Replacement Analysis Report"** to determine the structural integrity of DISH Network's antenna mounting system with the proposed appurtenance and equipment addition on the abovementioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

# Sector Frame Sufficient *Sufficient upon completion of the changes listed in the 'Recommendations' section of this report.

This analysis utilizes an ultimate 3-second gust wind speed of 125 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount analysis prepared by: Trevor Leahy, E.I.T.

Respectfully Submitted by: Jinshan Wang, P.E.



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Supplemental Drawings

### 1) INTRODUCTION

This is a proposed 3 sector 8.0 ft Sector Frame Mount, designed by Commscope.

### 2) ANALYSIS CRITERIA

Building Code:	2015 IBC / 2018 CTSBC
TIA-222 Revision:	TIA-222-H
Risk Category:	II
Ultimate Wind Speed:	125 mph
Exposure Category:	С
Topographic Factor at Base:	1.0
Topographic Factor at Mount:	1.0
Ice Thickness:	2.00 in
Wind Speed with Ice:	50 mph
Seismic S _s :	0.180
Seismic S ₁ :	0.063
Live Loading Wind Speed:	30 mph
Man Live Load at Mid/End-Points:	250 lb
Man Live Load at Mount Pipes:	500 lb

### Table 1 - Proposed Equipment Configuration

Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount / Modification Details
		3	JMA Wireless	MX08FRO665-21	9.0 ft Sector Frame
134.0	134.0	3	Fujitsu	TA08025-B604	8.0 ft Sector Frame
134.0	134.0	3	Fujitsu	TA08025-B605	[Commscope MTC3975083]
		1	Raycap	RDIDC-9181-PF-48	WTC3975085]

#### 3) ANALYSIS PROCEDURE

#### Table 2 - Documents Provided

Document	Remarks	Reference	Source
Crown Application	DISH Network Application	556617 Rev. 1	CCI Sites
Mount Manufacturer Drawings	Commscope	MTC3975083	Trylon
Tower Analysis	Crown Castle	9810957	CCI Sites

#### 3.1) Analysis Method

RISA-3D (Version 17.0.4), a commercially available analysis software package, was used to create a threedimensional model of the antenna mounting system and calculate member stresses for various loading cases.

A tool internally developed, using Microsoft Excel, by Trylon was used to calculate wind loading on all appurtenances, dishes, and mount members for various load cases. Selected output from the analysis is included in Appendix B.

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 *Tower Mount Analysis* (Revision B).

### 3.2) Assumptions

- 1) The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- The configuration of antennas, mounts, and other appurtenances are as specified in Table 1 and 2) the referenced drawings.
- All member connections are assumed to have been designed to meet or exceed the load carrying 3) capacity of the connected member unless otherwise specified in this report.
- The analysis will be required to be revised if the existing conditions in the field differ from those 4) shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
- Prior structural modifications to the tower mounting system are assumed to be installed as shown 5) per available data. 6)

Steel grades have been assumed as follows, ur	less noted otherwise:
Channel, Solid Round, Angle, Plate	ASTM A36 (GR 36)
HSS (Rectangular)	ASTM A500 (GR B-46)
Pipe	ASTM A53 (GR 35)
Connection Bolts	ASTM A325

This analysis may be affected if any assumptions are not valid or have been made in error. Trylon should be notified to determine the effect on the structural integrity of the antenna mounting system.

#### 4) ANALYSIS RESULTS

Notes	Component	Critical Member	Centerline (ft)	% Capacity	Pass / Fail
	Mount Pipe(s)	MP2		32.5	Pass
	Horizontal(s)	M5		14.4	Pass
1, 2	Standoff(s)	M1	134.0	16.1	Pass
1, ∠	Bracing(s)	M23	134.0	31.8	Pass
	Tieback(s)	M31A		10.2	Pass
	Mount Connection(s)	-		15.2	Pass

#### Table 3 - Mount Component Stresses vs. Capacity (Sector Frame, All Sectors)

Structure Rating (max from all components) =	32.5%
Notes:	

1) See additional documentation in "Appendix C - Software Analysis Output" for calculations supporting the % capacity consumed

2) Rating per TIA-222-H, Section 15.5

#### Table 4 - Tieback Connection Data Table

Tower Connectio Node No.	Existing / Proposed	Resultant End Reaction (Ib)	Connected Member Type	Connected Member Size	Member Compressive Capacity (lb) ³	Notes
N52A	Proposed	651.0	Leg	Pipe 4.5" x 0.438"	10,061.0	1

Notes:

Tieback connection point is within 25% of either end of the connected tower member 1)

2) Tieback connection point is NOT within 25% of either end of the connected tower member

3) Reduced member compressive capacity according to CED-STD-10294 Standard for Installation of Mounts and Appurtenances

### 4.1) Recommendations

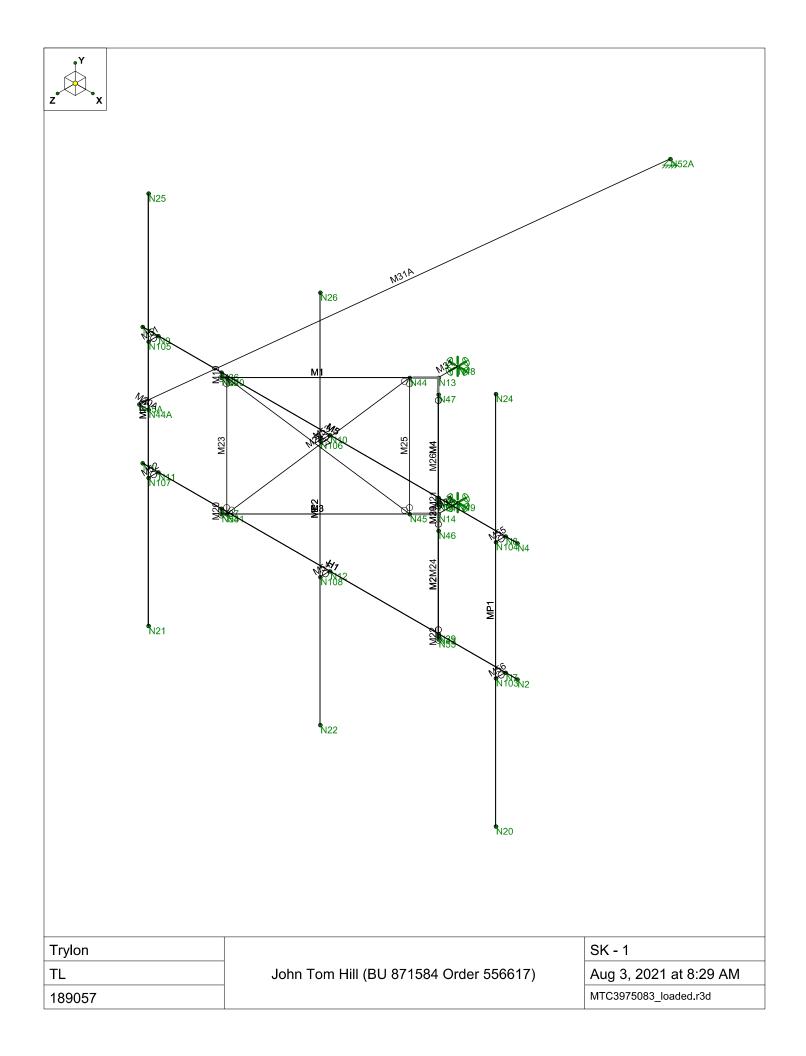
The mount has sufficient capacity to carry the proposed loading configuration. In order for the results of the analysis to be considered valid, the proposed mount listed below must be installed.

1. Commscope MTC3975083. Install tieback connection point within 25% of either end of tower leg.

No structural modifications are required at this time, provided that the above-listed changes are implemented.

**APPENDIX A** 

WIRE FRAME AND RENDERED MODELS



Trylon TL Jol 189057	hn Tom Hill (BU 871584 Order 556617)	SK - 2 Aug 3, 2021 at 8:29 AM MTC3975083_loaded.r3d

# APPENDIX B

# SOFTWARE INPUT CALCULATIONS



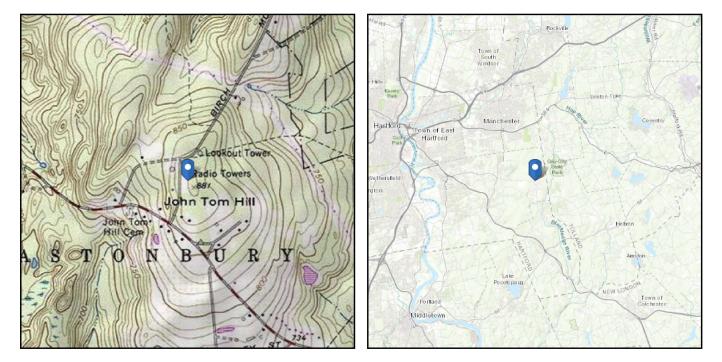
No Address at This

Location

# ASCE 7 Hazards Report

Standard:ASCE/SEI 7-10Risk Category:IISoil Class:D - Stiff Soil

Elevation: 877.95 ft (NAVD 88) Latitude: 41.708956 Longitude: -72.473447



# lce

### **Results:**

Ice Thickness:	1.00 in.
Concurrent Temperature:	5 F
Gust Speed:	50 mph
Data Source:	Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8
Date Accessed:	Tue Aug 03 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.



# TIA LOAD CALCULATOR 2.0

PROJECT	DATA
Job Code:	189057
Carrier Site ID:	BOBDL00076A
Carrier Site Name:	CT-CCI-T-871584

CODES AND S	TANDARDS
Building Code:	2015 IBC
Local Building Code:	2018 CTSBC
Design Standard:	TIA-222-H

STRUCTURE DETAILS		
Mount Type:	Sector Frame	
Mount Elevation:	134.0	ft.
Number of Sectors:	3	
Structure Type:	Self Support Tower	
Structure Height:	200.0	ft.

ANALYSIS	CRITERIA	
Structure Risk Category:	II	
Exposure Category:	С	
Site Class:	D - Stiff Soil	
Ground Elevation:	877.95	ft.

TOPOGRAP	HIC DATA	
Topographic Category:	1.00	
Topographic Feature:	N/A	
Crest Point Elevation:	0.00	ft.
Base Point Elevation:	0.00	ft.
Crest to Mid-Height (L/2):	0.00	ft.
Distance from Crest (x):	0.00	ft.
Base Topo Factor (K _{zt} ):	1.00	
Mount Topo Factor (K _{zt} ):	1.00	

WIND PARAM	IETERS	
Design Wind Speed:	125	mph
Wind Escalation Factor (K _s ):	1.00	
Velocity Coefficient (Kz):	1.35	
Directionality Factor (K _d ):	0.95	
Gust Effect Factor (Gh):	1.00	
Shielding Factor (K _a ):	0.90	
Velocity Pressure (q _z ):	49.55	psf

	ETERS	
Design Ice Wind Speed:	50	mph
Design Ice Thickness (t _i ):	2.00	in
Importance Factor (I _i ):	1.00	
Ice Velocity Pressure (q _{zi} ):	49.55	psf
Mount Ice Thickness (t _{iz} ):	2.30	in

WIND STRUCTURE C	ALCULATIONS	
Flat Member Pressure:	89.19	psf
Round Member Pressure:	53.51	psf
Ice Wind Pressure:	7.32	psf

SEISMIC PARA	METERS	
Importance Factor (I _e ):	1.00	
Short Period Accel $(S_s)$ :	0.18	g
1 Second Accel (S ₁ ):	0.06	g
Short Period Des. (S _{DS} ):	0.19	g
1 Second Des. (S _{D1} ):	0.10	g
Short Period Coeff. (F _a ):	1.60	
1 Second Coeff. ( $F_v$ ):	2.40	
Response Coefficient (Cs):	0.10	
Amplification Factor (A _S ):	1.20	

# LOAD COMBINATIONS [LRFD]

#	Description
1	1.4DL
2	1.2DL + 1WL 0 AZI
3	1.2DL + 1WL 30 AZI
4	1.2DL + 1WL 45 AZI
5	1.2DL + 1WL 60 AZI
6	1.2DL + 1WL 90 AZI
7	1.2DL + 1WL 120 AZI
8	1.2DL + 1WL 135 AZI
9	1.2DL + 1WL 150 AZI
10	1.2DL + 1WL 180 AZI
11	1.2DL + 1WL 210 AZI
12	1.2DL + 1WL 225 AZI
13	1.2DL + 1WL 240 AZI
14	1.2DL + 1WL 270 AZI
15	1.2DL + 1WL 300 AZI
16	1.2DL + 1WL 315 AZI
17	1.2DL + 1WL 330 AZI
18	0.9DL + 1WL 0 AZI
19	0.9DL + 1WL 30 AZI
20	0.9DL + 1WL 45 AZI
21	0.9DL + 1WL 60 AZI
22	0.9DL + 1WL 90 AZI
23	0.9DL + 1WL 120 AZI
24	0.9DL + 1WL 135 AZI
25	0.9DL + 1WL 150 AZI
26	0.9DL + 1WL 180 AZI
27	0.9DL + 1WL 210 AZI
28 29	0.9DL + 1WL 225 AZI 0.9DL + 1WL 240 AZI
30	0.9DL + 1WL 270 AZI
31	0.9DL + 1WL 300 AZI
32	0.9DL + 1WL 315 AZI
33	0.9DL + 1WL 330 AZI
34	1.2DL + 1DLi + 1WLi 0 AZI
35	1.2DL + 1DLi + 1WLi 30 AZI
	1.2DL + 1DLi + 1WLi 30 AZI
36	
37	1.2DL + 1DLi + 1WLi 60 AZI
38	1.2DL + 1DLi + 1WLi 90 AZI
39	1.2DL + 1DLi + 1WLi 120 AZI
40	1.2DL + 1DLi + 1WLi 135 AZI
41	1.2DL + 1DLi + 1WLi 150 AZI

#	Description
"	Bessinption
42	1.2DL + 1DLi + 1WLi 180 AZI
43	1.2DL + 1DLi + 1WLi 210 AZI
44	1.2DL + 1DLi + 1WLi 225 AZI
45	1.2DL + 1DLi + 1WLi 240 AZI
46	1.2DL + 1DLi + 1WLi 270 AZI
47	1.2DL + 1DLi + 1WLi 300 AZI
48	1.2DL + 1DLi + 1WLi 315 AZI
49	1.2DL + 1DLi + 1WLi 330 AZI
50	(1.2+0.2Sds) + 1.0E 0 AZI
51	(1.2+0.2Sds) + 1.0E 30 AZI
52	(1.2+0.2Sds) + 1.0E 45 AZI
53	(1.2+0.2Sds) + 1.0E 60 AZI
54	(1.2+0.2Sds) + 1.0E 90 AZI
55	(1.2+0.2Sds) + 1.0E 120 AZI
56	(1.2+0.2Sds) + 1.0E 135 AZI
57	(1.2+0.2Sds) + 1.0E 150 AZI
58	(1.2+0.2Sds) + 1.0E 180 AZI
59	(1.2+0.2Sds) + 1.0E 210 AZI (1.2+0.2Sds) + 1.0E 225 AZI
60 61	(1.2+0.2Sds) + 1.0E 225 AZI (1.2+0.2Sds) + 1.0E 240 AZI
62	(1.2+0.2Sds) + 1.0E 240 AZI (1.2+0.2Sds) + 1.0E 270 AZI
63	(1.2+0.2Sds) + 1.0E 270 AZI (1.2+0.2Sds) + 1.0E 300 AZI
64	(1.2+0.2Sds) + 1.0E 300 AZI (1.2+0.2Sds) + 1.0E 315 AZI
65	(1.2+0.2Sds) + 1.0E 310 AZI (1.2+0.2Sds) + 1.0E 330 AZI
66	(0.9-0.2Sds) + 1.0E 0 AZI
67	(0.9-0.2Sds) + 1.0E 30 AZI
68	(0.9-0.2Sds) + 1.0E 45 AZI
69	(0.9-0.2Sds) + 1.0E 60 AZI
70	(0.9-0.2Sds) + 1.0E 90 AZI
71	(0.9-0.2Sds) + 1.0E 120 AZI
72	(0.9-0.2Sds) + 1.0E 135 AZI
73	(0.9-0.2Sds) + 1.0E 150 AZI
74	(0.9-0.2Sds) + 1.0E 180 AZI
75	(0.9-0.2Sds) + 1.0E 210 AZI
76	(0.9-0.2Sds) + 1.0E 225 AZI
77	(0.9-0.2Sds) + 1.0E 240 AZI
78	(0.9-0.2Sds) + 1.0E 270 AZI
79	(0.9-0.2Sds) + 1.0E 200 AZI
80	(0.9-0.2Sds) + 1.0E 300 AZI (0.9-0.2Sds) + 1.0E 315 AZI
81	(0.9-0.2Sds) + 1.0E 330 AZI
82-88	1.2D + 1.5 Lv1

#	Description	#	Description
89	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP1	121	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP3
90	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP1	122	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP3
91	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP1	123	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP3
92	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP1	124	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP3
93	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP1	125	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP3
94	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP1	126	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP3
95	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP1	127	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP3
96	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP1	128	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP3
97	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP1	129	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP3
98	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP1	130	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP3
99	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP1	131	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP3
100	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP1	132	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP3
101	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP1	133	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP3
102	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP1	134	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP3
103	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP1	135	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP3
104	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP1	136	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP3
105	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP2	137	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP4
106	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP2	138	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP4
107	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP2	139	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP4
108	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP2	140	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP4
109	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP2	141	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP4
110	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP2	142	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP4
111	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP2	143	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP4
112	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP2	144	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP4
113	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP2	145	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP4
114	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP2	146	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP4
115	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP2	147	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP4
116	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP2	148	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP4
117	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP2	149	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP4
118	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP2	150	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP4
119	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP2	151	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP4
120	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP2	152	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP4

*This page shows an example of maintenance loads for (4) pipes, the number of mount pipe LCs may vary per site

# **EQUIPMENT LOADING**

Appurtenance Name/Location	Qty.	Elevation [ft]		<b>EPA</b> _N (ft2)	<b>EPA</b> _T (ft2)	Weight (lbs)
MX08FRO665-21	1	134	No Ice	12.49	5.87	82.50
MP2/5/8, 0/140			w/ Ice	14.14	7.37	396.29
TA08025-B604	1	134	No Ice	1.96	0.98	63.90
MP2/5/8, 0/140			w/ Ice	2.55	1.44	100.81
TA08025-B605	1	134	No Ice	1.96	1.13	75.00
MP2/5/8, 0/140			w/ Ice	2.55	1.61	107.08
RDIDC-9181-PF-48	1	134	No Ice	2.01	1.17	21.85
MP2, 0			w/ Ice	2.60	1.66	105.62
			No Ice			
			w/ Ice			
			No Ice			
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			No Ice			
			w/ Ice			

# EQUIPMENT LOADING [CONT.]

Appurtenance Name/Location	Qty.	Elevation [ft]		EPA _N (ft2)	<b>EPA</b> _T (ft2)	Weight (lbs)
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
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			No Ice			
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			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			

# **EQUIPMENT WIND CALCULATIONS**

Appurtenance Name	Qty.	Elevation [ft]	<b>K</b> _{zt}	Kz	K _d	<b>t</b> _d	<b>q</b> _z [psf]	<b>q</b> _{zi} [psf]
MX08FRO665-21	1	134	1.00	1.35	0.95	2.30	49.55	7.93
TA08025-B604	1	134	1.00	1.35	0.95	2.30	49.55	7.93
TA08025-B605	1	134	1.00	1.35	0.95	2.30	49.55	7.93
RDIDC-9181-PF-48	1	134	1.00	1.35	0.95	2.30	49.55	7.93

# **EQUIPMENT LATERAL WIND FORCE CALCULATIONS**

Appurtenance Name	Qty.		0° 180°	30° 210°	60° 240°	90° 270°	120° 300°	150° 330°
MX08FRO665-21	1	No Ice	556.94	335.45	483.11	261.62	483.11	335.45
MP2/5/8, 0/140		w/ Ice	100.86	64.67	88.80	52.60	88.80	64.67
TA08025-B604	1	No Ice	87.56	54.71	76.61	43.75	76.61	54.71
MP2/5/8, 0/140		w/ Ice	18.17	12.24	16.19	10.26	16.19	12.24
TA08025-B605	1	No Ice	87.56	59.67	78.26	50.37	78.26	59.67
MP2/5/8, 0/140		w/ Ice	18.17	13.13	16.49	11.45	16.49	13.13
RDIDC-9181-PF-48	1	No Ice	89.72	61.50	80.31	52.10	80.31	61.50
MP2, 0		w/ Ice	18.57	13.54	16.89	11.86	16.89	13.54
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
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		w/ Ice						
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		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						

# **EQUIPMENT LATERAL WIND FORCE CALCULATIONS [CONT.]**

Appurtenance Name	Qty.		0° 180°	30° 210°	60° 240°	90° 270°	120° 300°	150° 330°
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						

# **EQUIPMENT SEISMIC FORCE CALCULATIONS**

Appurtenance Name	Qty.	Elevation [ft]	Weight	F _p
			[lbs]	[lbs]
MX08FRO665-21	1	134	82.5	9.50
TA08025-B604	1	134	63.9	7.36
TA08025-B605	1	134	75	8.64
RDIDC-9181-PF-48	1	134	21.85	2.52

# APPENDIX C

### SOFTWARE ANALYSIS OUTPUT

## (Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in ² )	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	24
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Υ
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver
L	·
Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISAConnection Code	AISC 15th(360-16): LRFD
Cold Formed Steel Code	AISI S100-16: LRFD
Wood Code	AWC NDS-18: ASD
Wood Temperature	< 100F
Concrete Code	ACI 318-14
Masonry Code	TMS 402-16: Strength
Aluminum Code	AA ADM1-15: LRFD - Building
Stainless Steel Code	AISC 14th(360-10): LRFD
Adjust Stiffness?	Yes(Iterative)
Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parme Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	Yes
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR_SET_ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8

# (Global) Model Settings, Continued

Seismic Code	ASCE 7-10
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
TZ (sec)	Not Entered
RX	3
RZ	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	l or ll
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	1
Cd X	1
Rho Z	1
Rho X	1

# Hot Rolled Steel Properties

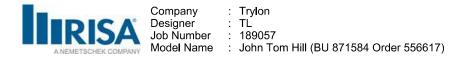
	Label	E [ksi]	G [ksi]	Nu	Therm (/1E	Density[k/ft	. Yield[ksi]	Ry	Fu[ksi]	Rt
1	A992	29000	11154	.3	.65	.49	50	1.1	65	1.1
2	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
3	A572 Gr.50	29000	11154	.3	.65	.49	50	1.1	65	1.1
4	A500 Gr.B RND	29000	11154	.3	.65	.527	42	1.4	58	1.3
5	A500 Gr.B Rect	29000	11154	.3	.65	.527	46	1.4	58	1.3
6	A53 Gr.B	29000	11154	.3	.65	.49	35	1.6	60	1.2
7	A1085	29000	11154	.3	.65	.49	50	1.4	65	1.3
8	A500 Gr. C - 46	29000	11154	.3	.65	.49	46	1.3	62	1.4
9	A529 Gr. 50	29000	11154	.3	.65	.49	50	1.3	65	1.4

# **Cold Formed Steel Properties**

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E5 F)	Density[k/ft^3]	Yie <b>l</b> d[ksi]	Fu[ksi]
1	A653 SS Gr33	29500	11346	.3	.65	.49	33	45
2	A653 SS Gr50/1	29500	11346	.3	.65	.49	50	65

### Hot Rolled Steel Section Sets

	Label	Shape	Туре	Design List	Material	Design	A [in2]	lyy [in4]	Izz [in4]	J [in4]
1	Horizontals	PIPE 2.5	Beam	None	A500 Gr. C - 46	Typical	1.61	1 45	1.45	2.89
2	Standoffs	PIPE 1.5	Beam	None	A500 Gr. C - 46	Typical	.749	.293	.293	.586
3	Tie Backs	PIPE 2.0	Beam	None	A500 Gr. C - 46	Typical	1.02	.627	.627	1.25
4	Mount Pipes	PIPE 2.0	Beam	None	A500 Gr. C - 46	Typical	1.02	.627	.627	1.25
5	Standoff Bracing (V	SR 5/8_HRA	Beam	None	A529 Gr. 50	Typical	.307	.007	.007	.015
6	Vertical pipes	PIPE 3.0	Beam	None	A500 Gr. C - 46	Typical	2.07	2.85	2.85	5.69
7	Standoff Bracing (D	SR 1/2"	Beam	None	A529 Gr. 50	Typical	.196	.003	.003	.006



# **Cold Formed Steel Section Sets**

	Label	Shape	Type	Design List	Material	Design Rul	A [in2]	lyy [in4]	Izz [in4]	J [in4]
1	CF1A	8CU1.25X057	Beam	None	A653 SS Gr33	Typical	.581	.057	4 4 1	00063

# Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot [k-ft/rad]
1	N13						
2	N14						
3	N48	Reaction	Reaction	Reaction	Reaction		Reaction
4	N49	Reaction	Reaction	Reaction	Reaction		Reaction
5	N52A	Reaction	Reaction	Reaction			

# **Basic Load Cases**

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed	Area(Me	.Surface(
1	Self Weight	DĹ		-1			7			
2	Structure Wind Z	WLZ						31		
3	Structure Wind X	WLX						31		
4	Wind Load 0 AZI	WLZ					14			
5	Wind Load 30 AZI	None					14			
6	Wind Load 45 AZI	None					14			
7	Wind Load 60 AZI	None					14			
8	Wind Load 90 AZI	WLX					14			
9	Wind Load 120 AZI	None					14			
10	Wind Load 135 AZI	None					14			
11	Wind Load 150 AZI	None					14			
12	Ice Weight	OL1					7	31		
13	Ice Structure Wind Z	OL2						31		
14	Ice Structure Wind X	OL3						31		
15	Ice Wind Load 0 AZ	OL2					14			
16	Ice Wind Load 30 AZI	None					14			
17	Ice Wind Load 45 AZI	None					14			
18	Ice Wind Load 60 AZI	None					14			
19	Ice Wind Load 90 AZI	OL3					14			
20	Ice Wind Load 120 AZI	None					14			
21	Ice Wind Load 135 AZI	None					14			
22	Ice Wind Load 150 AZI	None					14			
23	Seismic Load Z	ELZ			115		7			
24	Seismic Load X	ELX	115				7			
25	Live Load 1 (Lv)	None					1			
26	Live Load 2 (Lv)	None					1			
27	Live Load 3 (Lv)	None					1			
28	Maintenance Load 1 (Lm)	None					1			
	Maintenance Load 2 (Lm)	None					1			
	Maintenance Load 3 (Lm)	None					1			

### Load Combinations

	Description	S	P	S	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa
1	1.4DL	Yes	Y		DL	1.4																		
2	1.2DL + 1WL 0 AZI	Yes	Y		DL	1.2	2	1	3		4	1												
3	1.2DL + 1WL 30 AZI	Yes	Y		DL	1.2	2	.866	3	.5	5	1												
4	1.2DL + 1WL 45 AZI	Yes	Y		DL	1.2	2	.707	3	.707	6	1												
5	1.2DL + 1WL 60 AZI	Yes	Y		DL	1.2	2	.5	3	.866	7	1												
6	1.2DL + 1WL 90 AZI	Yes	Y		DL	1.2	2		3	1	8	1												

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# Load Combinations (Continued)

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					-	_	_	_	_	_	-	_	_	_	_	_	_	_	_	_	_	_	_
7	Description 1.2DL + 1WL 120 AZI		P S E									. В	⊦a	. В	<u>⊦a</u>	В	<u>⊦a</u>	<u>. В</u>	<u>⊦a</u>	<u>В</u>	<u>⊦a</u>	<u>. В</u>	<u>⊦a</u>
7					1.2				.866														
8	1.2DL + 1WL 135 AZI						8		.707														
9	1.2DL + 1WL 150 AZI 1.2DL + 1WL 180 AZI	Yes					o -1		.5														
10		Yes Yes							5	4	-1												
11	1.2DL + 1WL 210 AZI						8			5													
12	1.2DL + 1WL 225 AZI 1.2DL + 1WL 240 AZI	Yes Yes			1.2				7 8														
13	1.2DL + 1WL 240 AZI	Yes				2	5	3		8	-1												
14 15	1.2DL + 1WL 270 AZI	Yes			1.2		.5																
16	1.2DL + 1WL 300 AZI	Yes							7	_													
17	1.2DL + 1WL 330 AZI	Yes					.866		5														
18	0.9DL + 1WL 0 AZI	Yes			.9		.000	3	5	4	1												
19	0.9DL + 1WL 30 AZI	Yes			. <u>ə</u> a	2	.866		.5	5	1												
20	0.9DL + 1WL 45 AZI	Yes							.707														
21	0.9DL + 1WL 60 AZI	Yes					.5	3	.866		1												
22	0.9DL + 1WL 90 AZI	Yes			.9			3	1	8	1												
23	0.9DL + 1WL 120 AZI	Yes			.9		- 5		.866														
24	0.9DL + 1WL 135 AZI	Yes							.707														
25	0.9DL + 1WL 150 AZI	Yes					8			11													
26	0.9DL + 1WL 180 AZI	Yes				2	-1	3		4	-1												
27	0.9DL + 1WL 210 AZI	Yes		DL	.9	2	8		5														
28	0.9DL + 1WL 225 AZI	Yes							7														
29	0.9DL + 1WL 240 AZI	Yes					5		8														
30	0.9DL + 1WL 270 AZI	Yes				2		3		8													
31	0.9DL + 1WL 300 AZI	Yes			.9		.5		8														
32	0.9DL + 1WL 315 AZI	Yes		DL	.9	2	.707	3	7	10	-1												
33		Yes					.866																
34	1.2DL + 1DLi + 1WLi 0 AZI				1.2					14		15	1										
35	1.2DL + 1DLi + 1WLi 30 AZI								.866														
36	1.2DL + 1DLi + 1WLi 45 AZI	Yes	Y	)L 1	1.2	0	1	13	.707	14	.707	17	1										
37	1.2DL + 1DLi + 1WLi 60 AZI		Y [		1.2			13	.5	14	.866	18	1										
38	1.2DL + 1DLi + 1WLi 90 AZI			)L ^	1.2	0	1	13			1	19											
39	1.2DL + 1DLi + 1WLi 120 AZI			)L ′	1.2	0	1	13	5	14	.866	20	1										
40	1.2DL + 1DLi + 1WLi 135 AZI			)L	1.2	0	1	13	7	14	.707	21	1										
41	1.2DL + 1DLi + 1WLi 150 AZI			)L ^	1.2	0	1	13	8	14	.5	22	1										
42	1.2DL + 1DLi + 1WLi 180 AZI			)L ⁻	1.2	0				14		15											
43	1.2DL + 1DLi + 1WLi 210 AZI				1.2				8														
44	1.2DL + 1DLi + 1WLi 225 AZI		-		1.2				7														
45	1.2DL + 1DLi + 1WLi 240 AZI	Yes	Y [						5														
46	1.2DL + 1DLi + 1WLi 270 AZI	Yes			1.2			13		14	-1	19	-1										
47	1.2DL + 1DLi + 1WLi 300 AZI								.5														
48	1.2DL + 1DLi + 1WLi 315 AZI								.707														
	1.2DL + 1DLi + 1WLi 330 AZI				1.2				.866	14	5	22	-1										
50	(1.2+0.2Sds)DL + 1E 0 AZI				.2			24															
51	(1.2+0.2Sds)DL + 1E 30 AZI						.866			_													
52	(1.2+0.2Sds)DL + 1E 45 AZI								.707														
53	(1.2+0.2Sds)DL + 1E 60 AZI								.866														
54	(1.2+0.2Sds)DL + 1E 90 AZI				.2			24															
55	(1.2+0.2Sds)DL + 1E 120 AZI								.866														
	(1.2+0.2Sds)DL + 1E 135 AZI								.707														
57	(1.2+0.2Sds)DL + 1E 150 AZI						8																
58	(1.2+0.2Sds)DL + 1E 180 AZI						-1									-							
	(1.2+0.2Sds)DL + 1E 210 AZI						8																
	(1.2+0.2Sds)DL + 1E 225 AZI								7 o							-							
	(1.2+0.2Sds)DL + 1E 240 AZI		Y L		.2	23	5		8														
62	(1.2+0.2Sds)DL + 1E 270 AZI				.2				-1							-							
63	(1.2+0.2Sds)DL + 1E 300 AZI	res	Y L	ר ∟ <i>ו</i>	.2	23	.5	24	8														
	$A_3D$ Version 17.0.4																					Done	

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# Load Combinations (Continued)

	Description			S B	Fa	B	Fa	B	Fa	B	Fa	B	Fa	B	Fa	B	Fa	в	Fa	B	Fa	B	Fa
64	(1.2+0.2Sds)DL + 1E 315 AZI	Yes	V		1.2					D	1 a	U	<u>1 a</u>	D	1 a	D	1 a	D	1 a	D	1 a	D	<u>a</u>
65	(1.2+0.2Sds)DL + 1E 330 AZI				1.2																		
66	(0.9-0.2Sds)DL + 1E 0 AZI				.862			24															
67	(0.9-0.2Sds)DL + 1E 30 AZI				.862																		
68	(0.9-0.2Sds)DL + 1E 45 AZI				.862																		
69	(0.9-0.2Sds)DL + 1E 60 AZI								.866														
70	(0.9-0.2Sds)DL + 1E 90 AZI				.862			24															
71	(0.9-0.2Sds)DL + 1E 120 AZI	_							.866														
72	(0.9-0.2Sds)DL + 1E 135 AZI	_		DL					.707														
73	(0.9-0.2Sds)DL + 1E 150 AZI				.862																		
74	(0.9-0.2Sds)DL + 1E 180 AZI				.862			24															
75	(0.9-0.2Sds)DL + 1E 210 AZI				.862										-								
76	(0.9-0.2Sds)DL + 1E 225 AZI				.862																		
77	(0.9-0.2Sds)DL + 1E 240 AZI				.862																		
78	(0.9-0.2Sds)DL + 1E 270 AZI				.862			24															
79	(0.9-0.2Sds)DL + 1E 300 AZI				.862										-								
80	(0.9-0.2Sds)DL + 1E 315 AZI				.862																		
81	(0.9-0.2Sds)DL + 1E 330 AZI				.862																		
82	1.2DL + 1Lv1	Yes			1.2				5														
83	1.2DL + 1Lv1	Yes			1.2																		
84		Yes			1.2																		
	1.2DL + 1.5Lm + 1Wm 0 AZI				1.2				.058	3		Δ	.058										
	1.2DL + 1.5Lm + 1Wm 30 AZI.				1.2	28	1.5	2			.029	_	.058										
	1.2DL + 1.5Lm + 1Wm 45 AZI.				1.2																		
	1.2DL + 1.5Lm + 1Wm 60 AZI.				12	28	1.5	2	.029				.058										
	1.2DL + 1.5Lm + 1Wm 90 AZI.				1.2				1020				.058										
	1.2DL + 1.5Lm + 1Wm 120 A				1.2	28	1.5	2	0		.05		.058										
	1.2DL + 1.5Lm + 1Wm 135 A								0				.058										
	1.2DL + 1.5Lm + 1Wm 150 A				1.2				05														
	1.2DL + 1.5Lm + 1Wm 180 A				1.2				0				0										
	1.2DL + 1.5Lm + 1Wm 210 A			DI	1.2	28	1.5	2	05		0												
	1.2DL + 1.5Lm + 1Wm 225 A				1.2				0		0												
	1.2DL + 1.5Lm + 1Wm 240 A								0		05												
	1.2DL + 1.5Lm + 1Wm 270 A				1.2						0												
	1.2DL + 1.5Lm + 1Wm 300 A								.029														
	1.2DL + 1.5Lm + 1Wm 315 A				1.2																		
	1.2DL + 1.5Lm + 1Wm 330 A				1.2						0												
101	1.2DL + 1.5Lm + 1Wm 0 AZI	.Yes	Ý	DL	1.2	29	1.5	2	.058				.058										
	1.2DL + 1.5Lm + 1Wm 30 AZI.				1.2						.029		.058										
	1.2DL + 1.5Lm + 1Wm 45 AZI.			DL	1.2	29	1.5	2	.041														
	1.2DL + 1.5Lm + 1Wm 60 AZI.								.029				.058										
	1.2DL + 1.5Lm + 1Wm 90 AZI.				1.2						.058		.058										
	1.2DL + 1.5Lm + 1Wm 120 A	-							0				.058										
107	1.2DL + 1.5Lm + 1Wm 135 A	Yes	Y		1.2				0				.058										
	1.2DL + 1.5Lm + 1Wm 150 A	_	-		1.2				05		.029		.058										
	1.2DL + 1.5Lm + 1Wm 180 A	_	-		1.2				0			4	0										
110	1.2DL + 1.5Lm + 1Wm 210 A	Yes	Y		1.2				05	-	0		0										
	1.2DL + 1.5Lm + 1Wm 225 A				1.2				0		0		0										
112	1.2DL + 1.5Lm + 1Wm 240 A	. Yes	Y		1.2				0		05		0										
	1.2DL + 1.5Lm + 1Wm 270 A				1.2						0		0										
	1.2DL + 1.5Lm + 1Wm 300 A			DL	1.2	29	1.5	2	.029	3	05	9	0										
115	1.2DL + 1.5Lm + 1Wm 315 A	Yes	Υ		1.2				.041		0												
116	1.2DL + 1.5Lm + 1Wm 330 A	Yes	Y		1.2								0										
117	1.2DL + 1.5Lm + 1Wm 0 AZI	.Yes	Y		1.2				.058			4	.058										
	1.2DL + 1.5Lm + 1Wm 30 AZI.				1.2				.05		.029	5	.058										
	1.2DL + 1.5Lm + 1Wm 45 AZI.			DL	1.2	30	1.5	2	.041				.058										
120	1.2DL + 1.5Lm + 1Wm 60 AZI.	.Yes	Y	DL	1.2	30	1.5	2	.029	3	.05	7	.058										
_		_			-		_										_	_		_			

RISA-3D Version 17.0.4 [C:\...\...\...\MA_8-3-2021\RISA or TNX\MTC3975083_loaded.r3d]

### Load Combinations (Continued)

Description	S	P	S	B F	a	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa
121 1.2DL + 1.5Lm + 1Wm 90 AZI.	.Yes	Y		DL 1	1.2	30	1.5	2		3	.058	8	.058										
122 1.2DL + 1.5Lm + 1Wm 120 A	Yes	Y		DL 1	.2	30	1.5	2	0	3	.05	9	.058										
123 1.2DL + 1.5Lm + 1Wm 135 A	Yes	Y		DL 1	.2	30	1.5	2	0	3	.041	10	.058										
124 1.2DL + 1.5Lm + 1Wm 150 A	Yes	Y		DL 1	.2	30	1.5	2	05	3	.029	11	.058										
125 1.2DL + 1.5Lm + 1Wm 180 A	.Yes	Y		DL 1	.2	30	1.5	2	0	3		4	0										
126 1.2DL + 1.5Lm + 1Wm 210 A	. Yes	Y		DL 1	1.2	30	1.5	2	05	<u> </u>	0	<u> </u>	0										
127 1.2DL + 1.5Lm + 1Wm 225 A	.Yes	Y		DL 1	.2	30	1.5	2	0	3	0	6	0										
128 1.2DL + 1.5Lm + 1Wm 240 A	.Yes	Y		DL 1	.2	30	1.5	2	0	3	05	7	0										
129 1.2DL + 1.5Lm + 1Wm 270 A	Yes	Y		DL 1	.2	30	1.5	2		3	0	8	0										
130 1.2DL + 1.5Lm + 1Wm 300 A	Yes	Y		DL 1	.2	30	1.5	2	.029	3	05	9	0										
131 1.2DL + 1.5Lm + 1Wm 315 A	Yes	Y		DL 1	.2	30	1.5	2	.041	3	0	10	0										
132 1.2DL + 1.5Lm + 1Wm 330 A	Yes	Y		DL 1	1.2	30	1.5	2	.05	3	0	11	0										

# Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-ft]	LC	MY [lb-ft]	LC	MZ [lb-ft]	LC
1	N48	max	895.402	89	1273.018		650.736	33	-135.063	33	Ö	132	496.931	131
2		min	-865.475	129	139.431	33	-1960.176	41	-914.211	41	0	1	-509.869	87
3	N49	max	855.992	121	1142.384	49	1875.455	49	-18.738	124	0	132	166.041	84
4		min	-885.693	97	55.527	93	-249.567	25	-920.603	49	0	1	-184.257	82
5	N52A	max	25.21	12	88.903	46	638.723	22	0	132	0	132	0	132
6		min	-26.314	4	15.356	70	-641.222	30	0	1	0	1	0	1
7	Totals:	max	863.246	6	2330.655	44	1414.315	2						
8		min	-863.245	30	430.751	70	-1414.314	26						

### Envelope AISC 15th(360-16): LRFD Steel Code Checks

	Member	Shape	Code	Loc[in]	LC	Shear.	.Loc[in]	DirL	Cphi*Pnc	phi*Pnt	phi*Mn	phi*Mn	Cb	Egn
1	MP2	PIPE 2.0	.341	33	2	.076	33	8	7 15369.6	42228	2459.85	2459.85	1.368	H1-1b
2	M23	SR 5/8_HRA_H	.334	24.5	92	.012	30.25	4	1849.145	13805.82	143.808	143.808	1	H1-1a
3	M24	SR 5/8_HRA_H	.332	24.5	124	.012	30.25	8	1849.145	13805.82	143.808	143.808	1	H1-1a
4	M28	SR 1/2"	.233	22.4	48	.013	44.8	3	7 1432.022	8835.75	73.632	73.632	1	H1-1b
5	M29	SR 1/2"	.229	22.4	120	.011	0	4	7 1432.022	8835.75	73.632	73.632	1	H1-1a
6	M1	PIPE_1.5	.169	34.81	90	.146	34.81	8	6 23485.28	31008.6	1452.45	1452.45	1	H1-1b
7	MP3	PIPE 2.0	.169	48	5	.092	33	7	15369.6	42228	2459.85	2459.85	1.427	H1-1b
8	M4	PIPE 1.5	.164	34.81	128	.146	34.81	1.	. 23485.28	31008.6	1452.45	1452.45	1	H1-1b
9	M5	PIPE 2.5	.151	48	3	.039	20	1.	. 45255.2	66654	4726.5	4726.5	1	H1-1b
10	M3	PIPE_1.5	.151	34.81	98	.117	34.81	44	1 23485.28	31008.6	1452.45	1452.45	1.326	H1-1b
11	M2	PIPE 1.5	.145	34.81	120	.115	34.81	39	23485.28	31008.6	1452.45	1452.45	1.367	H1-1b
12	H1	PIPE 2.5	.126	76	130	.043	20	92	2 45255 <mark>.</mark> 2	66654	4726.5	4726.5	2.424	H1-1b
13	M31A	PIPE 2.0	.107	61.6	38	.007	123	40	6 9324.69	42228	2459.85	2459.85	1.136	H1-1b
14	M25	SR 5/8_HRA_H	.028	30.25	91	.042	0	8	7 1849.145	13805.82	143.808	143.808	1	H1-1b*
15	M26	SR 5/8_HRA_H	.028	30.25	124	.042	0	8	7 1849.145	13805.82	143.808	143.808	1	H1-1b*
16	MP1	PIPE_2.0	.026	63	125	.049	33	8	7 15369.6	42228	2459.85	2459.85	1.149	H1-1b*
17	M27	SR 1/2"	.000	0	132	.012	44.8	42	2 1432.022	8835.75	73.632	73.632	1	H1-1a
18	M30	SR 1/2"	.000	0	132	.010	0	42	2 1432.022	8835.75	73.632	73.632	1	H1-1a

# Envelope AISI S100-16: LRFD Cold Formed Steel Code Checks

Member Shape Code Check Loc[in]LC Shea...Loc[i..DirLC phi*Pn[..phi*Tn[..phi*Mn...phi*Mn...phi*... phi*... Cb Eqn No Data to Print ...

# APPENDIX D

# ADDITIONAL CALCULATIONS

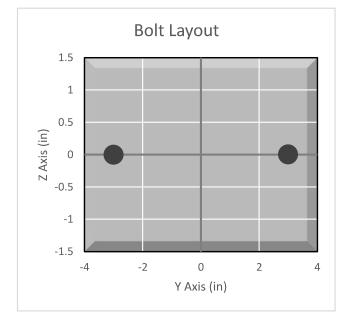


### **BOLT TOOL 1.5.2**

Projec	et Data
Job Code:	189057
Carrier Site ID:	BOBDL00076A
Carrier Site Name:	CT-CCI-T-871584

Co	de
Design Standard:	TIA-222-H
Slip Check:	Yes
Pretension Standard:	TIA-222-H

Bolt Pro	operties	
Connection Type:	Thread	led Rod
Diameter:	0.75	in
Grade:	A529	
Yield Strength (Fy):	50	ksi
Ultimate Strength (Fu):	65	ksi
Number of Bolts:	2	
Threads Included:	Yes	
Double Shear:	No	
Distance Between Rods:	6	in



### **Connection Description**

Mount Standoff to Tower Leg

Bolt Check*								
Tensile Capacity (φT _n ):	16304.9	lbs						
Shear Capacity (φV _n ):	10768.5	lbs						
Tension Force (T _u ):	592.5	lbs						
Shear Force (V _u ):	1713.3	lbs						
Tension Usage:	3.5%							
Shear Usage:	15.2%							
Interaction:	15.2%	Pass						
Controlling Member:	M31							
Controlling LC:	90							
*Rating per TIA-222-H Section 15.5								

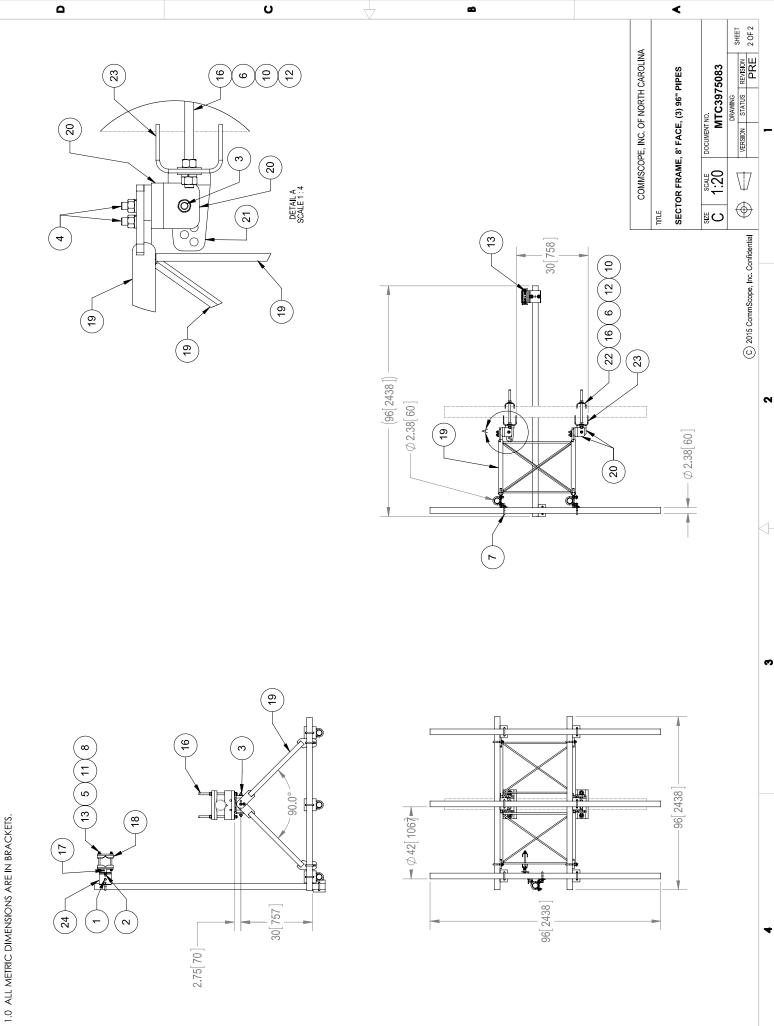
Slip Check* Sliding Capacity ( $\phi R_{ns}$ ): 9883.2 lbs Torsion Capacity ( $\phi R_{nr}$ ): 2470.8 lb-ft Sliding Force (V_{us}): 1273.0 lbs Torsional Force (Tur): 0.0 lb-ft Sliding Usage: 12.3% Torsion Usage: 0.0% Interaction: 12.3% Pass Controlling Member: M31 ---Controlling LC: 93 ---

*Rating per TIA-222-H Section 15.5

# APPENDIX E

## SUPPLEMENTAL DRAWINGS

	۵	U V	£							1			1	<	1			
REVISIONS BY DATE DESCRIPTION BY DATE REVIEW DRH 01/28/21								NORTH	SAP MATERIAL MASTER	M1C39/2083	RIAL	A1011/A1018, A500, A529		SECTOR FRAME, 8' FACE, (3) 96" PIPES	NT NO.	MTC3975083	ION VERSION STATUS REVISION 1052	-
REV. ECN D	18							COMMSCOPE, INC. OF	TOLERANCES 2 PLACE _X	.12	MAT		NAME DATE T			1:12	WORK AREA MODEL	- - 7 <del>)</del>
									0 PLACE X	щ	FINISH	GAI	ON (2002) WINCS' NGDS SLEES HED	-	ST 82 1 82 THE PORT	AOFONE HANDBC INLEGE DIWENZI DIWENZI DIWENZI DIWENZI INLEZZ	SIZE	)
7			24											DENSITY 0.28	400.61 1421.66	SURFACE AREA	HEIGHT LENGTH	2 3
Ð																	C 2015 CommScope Inc Confidential	
m	079		NOTE NO.															
	800.949.7079	(23)	WEICHT 0.12 LBS 0.20 LBS	0.28 LBS 0.35 LBS	0.04 LBS	0.56 LBS	0.03 LBS	0.10 LBS	0.04 LBS	0.44 LBS 17.29 LBS	23.05 LBS	1.99 LBS 0.14 LBS	1.35 LBS	6.70 LBS	7.49 LBS	12.15 LBS	2.65 LBS	
				8 4	4	19	4 4	8	· ∞ .	7 7		4 -	2	3	- 0	5	6	
<ul> <li>4</li> <li>NOTES:</li> <li>1.0 ALL METRIC DIMENSIONS ARE IN BRACKETS.</li> </ul>	www.Talleycom.com   Sales@Talleycom.com		DESCRIPTION 1/2"X 1-1/4" GALV BOLT KIT 1/2"X 2-3/4" GALV BOLT KIT	5/8" X 2-1/4" GALV BOLT KIT 5/8" X 3" GALV BOLT KIT	1/2" GALV HEX NUT	3/4 GALY REA NUI 1/2" X 2-1/2" X 4" GALV U-BOLT	1/2" GALV FLAT WASHER 5/8" GALV FLAT WASHER	3/4" GALV FLAT WASHER 1/2" GALV LOCK WASHER	3/4" GALV LOCK WASHER	1/2" X 8" GALV IHREAUED ROD 2.375" OD x 96" PIPE	Ø2.375" OD X 96" PIPE	Threaded Rod Galv 3/4" × 16" 3/4" X 1-1/2" OFFSET COLLAR		SFV AZIMUTH BRACKET	SFV TAPER BRACKET CI AMP PI ATE	MOUNT	ANIENNA MOUNI ANGLE	
TES: 0 ALL METRIC DIMEN	www.Talleycon		ITEM PART NO. 1 GB-04125 2 GB-04265	3 GB-05225 4 GB-05305			8 GWF-04 9 GWF-05	10 GWF-06 11 GWI-04		13 MI-3/9-8 14 MT-651-96		16 MT38416 17 OS15034			21 SFV03 22 SMU2080.06			4
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NOTES:

# Exhibit F

**Power Density/RF Emissions Report** 



# RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

**Dish Wireless Existing Facility** 

# Site ID: BOBDL00076A

871584 115 Birch Mtn. Road Glastonbury, Connecticut 06033

September 28, 2021

EBI Project Number: 6221005700

Site Compliance Summary				
Compliance Status:	COMPLIANT			
Site total MPE% of FCC general population allowable limit:	15.05%			



environmental | engineering | due diligence

September 28, 2021

**Dish Wireless** 

Emissions Analysis for Site: BOBDL00076A - 871584

EBI Consulting was directed to analyze the proposed Dish Wireless facility located at **115 Birch Mtn. Road** in **Glastonbury, Connecticut** for the purpose of determining whether the emissions from the Proposed Dish Wireless Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu$ W/cm²). The number of  $\mu$ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits; therefore, it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

<u>General population/uncontrolled exposure</u> limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu$ W/cm²). The general population exposure limits for the 600 MHz and 700 MHz frequency bands are approximately 400  $\mu$ W/cm² and 467  $\mu$ W/cm², respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 11 GHz frequency bands is 1000  $\mu$ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

<u>Occupational/controlled exposure</u> limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure.



Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

## CALCULATIONS

Calculations were done for the proposed Dish Wireless Wireless antenna facility located at 115 Birch Mtn. Road in Glastonbury, Connecticut using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since Dish Wireless is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 20 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was focused at the base of the tower. For this report, the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 4 n71 channels (600 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) 4 n70 channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
- 3) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 4) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 20 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.



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- 5) The antennas used in this modeling are the JMA MX08FRO665-21 for the 600 MHz / 1900 MHz channel(s) in Sector A, the JMA MX08FRO665-21 for the 600 MHz / 1900 MHz channel(s) in Sector B, the JMA MX08FRO665-21 for the 600 MHz / 1900 MHz channel(s) in Sector C. This is based on feedback from the carrier with regard to anticipated antenna selection. All Antenna gain values and associated transmit power levels are shown in the Site Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 20 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 6) The antenna mounting height centerline of the proposed antennas is 134 feet above ground level (AGL).
- 7) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 8) All calculations were done with respect to uncontrolled / general population threshold limits.



# **Dish Wireless Site Inventory and Power Data**

Sector:	А	Sector:	В	Sector:	С
Antenna #:	I	Antenna #:	I	Antenna #:	I
Make / Model:	JMA MX08FRO665- 21	Make / Model:	JMA MX08FRO665- 21	Make / Model:	JMA MX08FRO665- 21
Frequency Bands:	600 MHz / 1900 MHz	Frequency Bands:	600 MHz / 1900 MHz	Frequency Bands:	600 MHz / 1900 MHz
Gain:	17.45 dBd / 22.65 dBd	Gain:	17.45 dBd / 22.65 dBd	Gain:	17.45 dBd / 22.65 dBd
Height (AGL):	134 feet	Height (AGL):	134 feet	Height (AGL):	134 feet
Channel Count:	8	Channel Count:	8	Channel Count:	8
Total TX Power (W):	280 Watts	Total TX Power (W):	280 Watts	Total TX Power (W):	280 Watts
ERP (W):	3,065.51	ERP (W):	3,065.51	ERP (VV):	3,065.51
Antenna AI MPE %:	0.97%	Antenna BI MPE %:	0.97%	Antenna CI MPE %:	0.97%



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Site Composite MPE %				
Carrier	MPE %			
Dish Wireless (Max at Sector A):	0.97%			
DRW NX	I.38%			
Various Others	1.41%			
Unknown	2.16%			
T-Mobile	7.08%			
Sprint	2.05%			
Site Total MPE % :	15.05%			

Dish Wireless MPE % Per Sector					
Dish Wireless Sector A Total:	0.97%				
Dish Wireless Sector B Total:	0.97%				
Dish Wireless Sector C Total:	0.97%				
Site Total MPE % :	15.05%				

Dish Wireless Maximum MPE Power Values (Sector A)							
Dish Wireless Frequency Band / Technology (Sector A)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm ² )	Frequency (MHz)	Allowable MPE (µW/cm²)	Calculated % MPE
Dish Wireless 600 MHz n71	4	223.68	134.0	1.96	600 MHz n71	400	0.49%
Dish Wireless 1900 MHz n70	4	542.70	134.0	4.76	1900 MHz n70	1000	0.48%
		•	·	•		Total:	0.97%

• NOTE: Totals may vary by approximately 0.01% due to summation of remainders in calculations.



#### Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the Dish Wireless facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

Dish Wireless Sector	Power Density Value (%)
Sector A:	0.97%
Sector B:	0.97%
Sector C:	0.97%
Dish Wireless Maximum MPE % (Sector A):	0.97%
Site Total:	15.05%
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **15.05%** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

# Exhibit G

Letter of Authorization



4545 E River Rd, Suite 320 West Henrietta, NY 14586 Phone: (585) 445-5896 Fax: (724) 416-4461 www.crowncastle.com

#### Crown Castle Letter of Authorization

**CT - CONNECTICUT SITING COUNCIL** 

Melanie A. Bachman Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

#### Re: Tower Share Application Crown Castle telecommunications site at: 115 BIRCH MTN. ROAD, GLASTONBURY, CT 06033

PINNACLE TOWERS LLC ("Crown Castle") hereby authorizes DISH Wireless, LLC, including their Agent, to act as our Agent in the processing of all zoning applications, building permits and approvals through the CT - CONNECTICUT SITING COUNCIL for the existing wireless communications site described below:

Crown Site ID/Name: 871584/John Tom Hill Customer Site ID: BOBDL00076A/CT-CCI-T-871584 Site Address: 115 Birch Mtn. Road, GLASTONBURY, CT 06033

Date:

Crown Castle

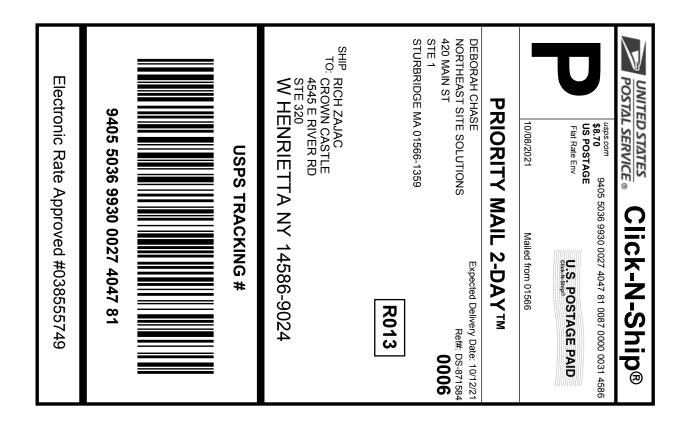
By:

10/4/2021

Richard Zajac Site Acquisition Specialist

# Exhibit H

**Recipient Mailings** 

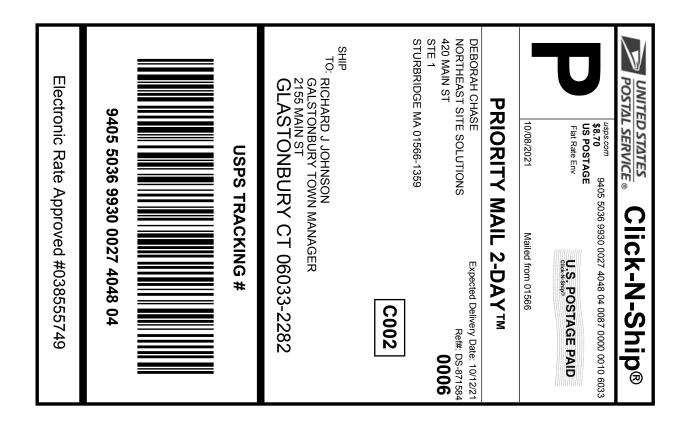


#### Instructions

- 1. Each Click-N-Ship® label is unique. Labels are to be used as printed and used only once. DO NOT PHOTO COPY OR ALTER LABEL.
- 2. Place your label so it does not wrap around the edge of the package.
- 3. Adhere your label to the package. A self-adhesive label is recommended. If tape or glue is used, DO NOT TAPE OVER BARCODE. Be sure all edges are secure.
- 4. To mail your package with PC Postage®, you may schedule a Package Pickup online, hand to your letter carrier, take to a Post Office™, or drop in a USPS collection box.
- 5. Mail your package on the "Ship Date" you selected when creating this label.

## Click-N-Ship® Label Record



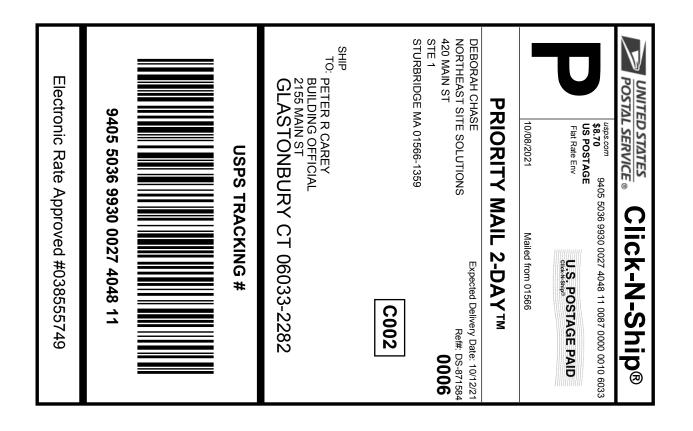


#### Instructions

- 1. Each Click-N-Ship® label is unique. Labels are to be used as printed and used only once. DO NOT PHOTO COPY OR ALTER LABEL.
- 2. Place your label so it does not wrap around the edge of the package.
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- 5. Mail your package on the "Ship Date" you selected when creating this label.

# Click-N-Ship® Label Record



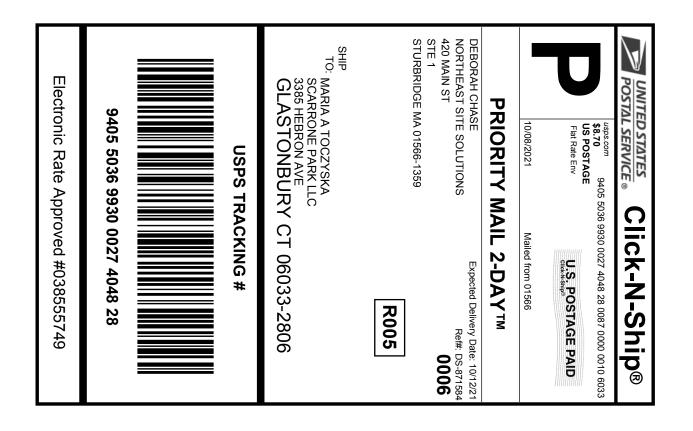


#### Instructions

- 1. Each Click-N-Ship® label is unique. Labels are to be used as printed and used only once. DO NOT PHOTO COPY OR ALTER LABEL.
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- 5. Mail your package on the "Ship Date" you selected when creating this label.

## Click-N-Ship® Label Record





#### Instructions

- 1. Each Click-N-Ship® label is unique. Labels are to be used as printed and used only once. DO NOT PHOTO COPY OR ALTER LABEL.
- 2. Place your label so it does not wrap around the edge of the package.
- 3. Adhere your label to the package. A self-adhesive label is recommended. If tape or glue is used, DO NOT TAPE OVER BARCODE. Be sure all edges are secure.
- 4. To mail your package with PC Postage®, you may schedule a Package Pickup online, hand to your letter carrier, take to a Post Office™, or drop in a USPS collection box.
- 5. Mail your package on the "Ship Date" you selected when creating this label.

## Click-N-Ship® Label Record



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	<u>VITEI</u> ISTAL	<u>2 STA</u> SERV	TES //CF.
UNIONVIL { 10/12/2021	UNIONVIL 24 MILL 9 LE, CT 00 800)275-8	_E ST 5085-9998 3777	
Product	Qty	Unit Price	Price
Prepaid Mai) Glastonbury, C Weight: O Ib Acceptance Dat Tue 10/12/ Tracking #: 9405 5036	1 T 06033 11.70 oz e: 2021	*	\$0.00
Prepaid Mail Glastonbury, C Weight: O lb Acceptance Date Tue 10/12/2 Tracking #: 9405 5036 g	9: 2021	4048 28	\$0.00
Prepaid Mail Glastonbury, CT Weight: O lb 1 Acceptance Date Tue 10/12/20 Tracking #: 9405 5036 95	1.70 oz	4048 04	\$0.00
Prepaid Mail West Henrietta, Weight: O lb 2. Acceptance Date: Tue 10/12/20 Tracking #: 9405 5036 99	1 NY 14586 OO oz 21		\$0.00
Grand Total:			<u></u>