

March 4, 2015

Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Re: **Notice of Exempt Modification – Facility Modification
577 Bell Street, Glastonbury, Connecticut**

Dear Ms. Bachman:

Cellco Partnership d/b/a Verizon Wireless (“Cellco”) currently maintains twelve (12) antennas at the top of the existing 104-foot self-supporting lattice tower at 577 Bell Street in Glastonbury, Connecticut (the “Property”). The tower is owned by Insite Towers, LLC. The Council approved Cellco’s shared use of this tower in 2011 (Petition No. 990). Cellco now intends to modify its facility by replacing all of its existing antennas with three (3) model BXA-70063-6CF, 700 MHz antennas; three (3) model LNX-8514DS-VTM, 850 MHz antennas; three (3) model HBXX-6517DS-VTM, 1900 MHz antennas; and three (3) model HBXX-6517DS-VTM, 2100 MHz antennas, all at the same level on the tower. Cellco also intends to install three (3) remote radio heads (“RRHs”) behind its 2100 MHz antennas and two (2) HYBRIFLEX™ antenna cables. Included in Attachment 1 are specifications for Cellco’s replacement antennas, RRHs and HYBRIFLEX™ cables.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent Richard M. Johnson, Town Manager of the Town of Glastonbury. A copy of this letter is also being sent to John B. Spencer Trust, the owner of the Property.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

Robinson+Cole

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1. The proposed modifications will not result in an increase in the height of the existing tower. Cellco's replacement antennas and RRHs will be installed on its existing antenna platform at the top of the tower.
2. The proposed modifications will not involve any change to ground-mounted equipment and, therefore, will not require the extension of the site boundary.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) safety standard. A cumulative General Power Density table for Cellco's modified facility is included in Attachment 2.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The tower and its foundation can support Cellco's proposed modifications. (*See Structural Analysis included in Attachment 3*).

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,



Kenneth C. Baldwin

Enclosures

Copy to:

Richard M. Johnson, Glastonbury Town Manager
John B. Spencer Trust
Timothy Parks

ATTACHMENT 1

BXA-70063-6CF-EDIN-X

X-Pol | FET Panel | 63° | 14.5 dBd

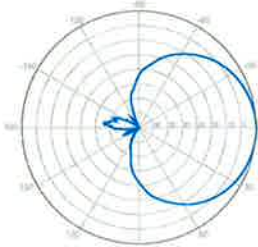
Replace "X" with desired electrical downtilt.

Antenna is also available with NE connector(s).
Replace 'EDIN' with 'NE' in the model number when ordering.

Electrical Characteristics	696-900 MHz		
Frequency bands	696-806 MHz	806-900 MHz	
Polarization	±45°		
Horizontal beamwidth	65°	63°	
Vertical beamwidth	13°	11°	
Gain	14.0 dBd (16.1 dBi)	14.5 dBd (16.6 dBi)	
Electrical downtilt (X)	0, 2, 3, 4, 5, 6, 8, 10		
Impedance	50Ω		
VSWR	≤1.35:1		
Upper sidelobe suppression (0°)	-18.3 dB	-18.2 dB	
Front-to-back ratio (+/-30°)	-33.4 dB	-36.3 dB	
Null fill	5% (-26.02 dB)		
Isolation between ports	< -25 dB		
Input power with EDIN connectors	500 W		
Input power with NE connectors	300 W		
IM3 (2x20W carriers)	< -153 dBc		
Lightning protection	Direct Ground		
Connector(s)	2 Ports / EDIN or NE / Female / Center (Back)		
Mechanical Characteristics			
Dimensions Length x Width x Depth	1804 x 285 x 132 mm	71.0 x 11.2 x 5.2 in	
Depth with z-brackets	172 mm	6.8 in	
Weight without mounting brackets	7.9 kg	17 lbs	
Survival wind speed	> 201 km/hr		
Wind area	Front: 0.51 m ² Side: 0.24 m ²	Front: 5.5 ft ² Side: 2.6 ft ²	
Wind load @ 161 km/hr (100 mph)	Front: 759 N Side: 391 N	Front: 169 lbf Side: 89 lbf	
Mounting Options	Part Number	Fits Pipe Diameter	Weight
3-Point Mounting & Downtilt Bracket Kit	36210008	40-115 mm 1.57-4.5 in	6.9 kg 15.2 lbs
Concealment Configurations	For concealment configurations, order BXA-70063-6CF-EDIN-X-FP		

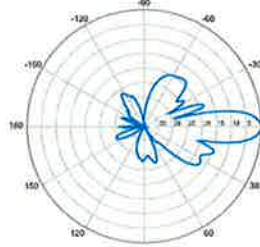


BXA-70063-6CF-EDIN-X



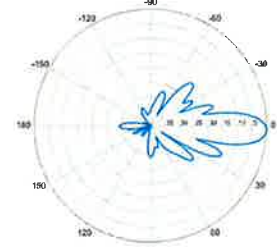
Horizontal | 750 MHz

BXA-70063-6CF-EDIN-0

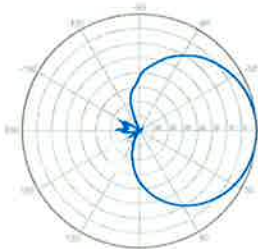


0° | Vertical | 750 MHz

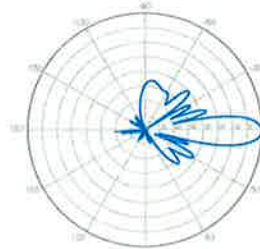
BXA-70063-6CF-EDIN-2



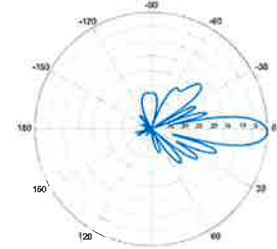
2° | Vertical | 750 MHz



Horizontal | 850 MHz



0° | Vertical | 850 MHz



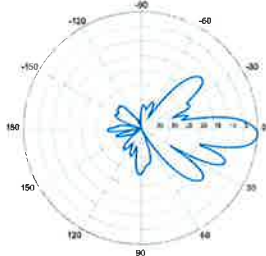
2° | Vertical | 850 MHz

Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.

BXA-70063-6CF-EDIN-X

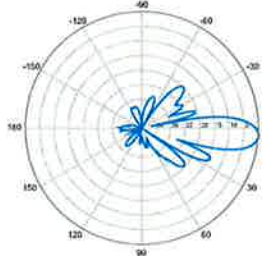
X-Pol | FET Panel | 63° | 14.5 dBd

BXA-70063-6CF-EDIN-3



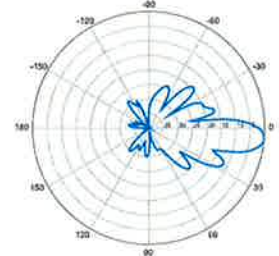
3° | Vertical | 750 MHz

BXA-70063-6CF-EDIN-4

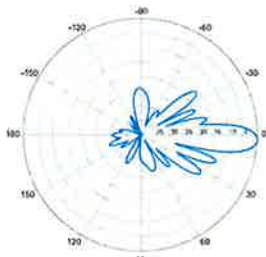


4° | Vertical | 750 MHz

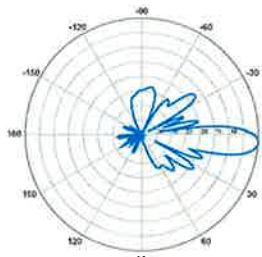
BXA-70063-6CF-EDIN-5



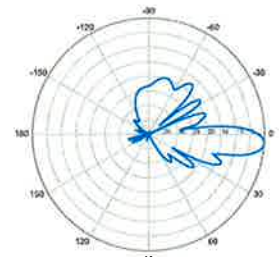
5° | Vertical | 750 MHz



3° | Vertical | 850 MHz

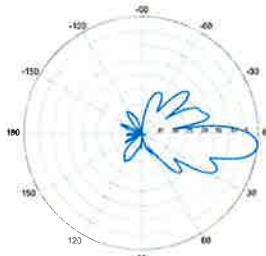


4° | Vertical | 850 MHz



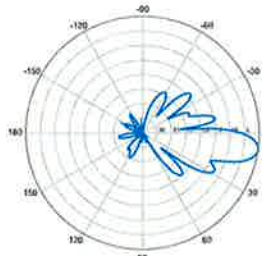
5° | Vertical | 850 MHz

BXA-70063-6CF-EDIN-6



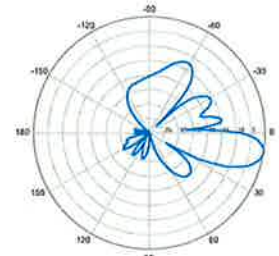
6° | Vertical | 750 MHz

BXA-70063-6CF-EDIN-8

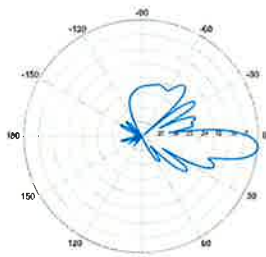


8° | Vertical | 750 MHz

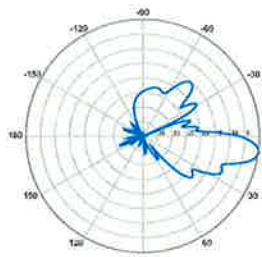
BXA-70063-6CF-EDIN-10



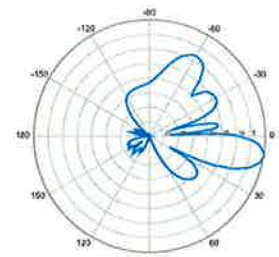
10° | Vertical | 750 MHz



6° | Vertical | 850 MHz



8° | Vertical | 850 MHz



10° | Vertical | 850 MHz

Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.

Product Specifications

COMMScope®

LNX-8514DS-VTM

Andrew® Teletilt® Antenna, 698–896 MHz, 85° horizontal beamwidth, RET compatible

POWERED BY



Electrical Specifications

Frequency Band, MHz	698–806	806–896
Gain by all Beam Tilts, average, dBi	15.7	16.2
Gain by all Beam Tilts Tolerance, dB	±0.3	±0.2
Gain by Beam Tilt, average, dBi	0 ° 15.7	0 ° 16.3
	4 ° 15.7	4 ° 16.3
	8 ° 15.5	8 ° 16.1
Beamwidth, Horizontal, degrees	85	84
Beamwidth, Horizontal Tolerance, degrees	±1.2	±1.3
Beamwidth, Vertical, degrees	8.6	7.8
Beamwidth, Vertical Tolerance, degrees	±0.5	±0.4
Beam Tilt, degrees	0–8	0–8
USLS, dB	20	22
Front-to-Back Total Power at 180° ± 30°, dB	22	23
CPR at Boresight, dB	18	18
CPR at Sector, dB	12	11
Isolation, dB	30	30
VSWR Return Loss, dB	1.4 15.6	1.4 15.6
PIM, 3rd Order, 2 x 20 W, dBc	-153	-153
Input Power per Port, maximum, watts	400	400
Polarization	±45°	±45°

*Values calculated using NGMN Alliance N-P-BASTA v9.6

Mechanical Specifications

Color Radome Material	Light gray Fiberglass, UV resistant
Connector Interface Location Quantity	7-16 DIN Female Bottom 2
Wind Loading, maximum	879.0 N @ 150 km/h 197.6 lbf @ 150 km/h
Wind Speed, maximum	241.0 km/h 149.8 mph
Antenna Dimensions, L x W x D	2449.0 mm x 301.0 mm x 181.0 mm 96.4 in x 11.9 in x 7.1 in
Net Weight	23.1 kg 50.9 lb
Model with factory installed AISG 2.0 RET LNX-8514DS-A1M	



Product Specifications

COMMSCOPE®

HBXX-6517DS-VTM

Andrew® Quad Port Teletilt® Antenna, 1710–2180 MHz, 65° horizontal beamwidth, RET compatible

POWERED BY



Electrical Specifications

Frequency Band, MHz	1710–1880	1850–1990	1920–2180
Gain by all Beam Tilts, average, dBi	18.5	18.6	18.8
Gain by all Beam Tilts Tolerance, dB	±0.4	±0.3	±0.4
Gain by Beam Tilt, average, dBi	0 ° 18.4	0 ° 18.4	0 ° 18.7
	3 ° 18.7	3 ° 18.7	3 ° 18.9
	6 ° 18.4	6 ° 18.5	6 ° 18.6
Beamwidth, Horizontal, degrees	67	66	65
Beamwidth, Horizontal Tolerance, degrees	±2.4	±1.7	±2.9
Beamwidth, Vertical, degrees	5.0	4.7	4.4
Beamwidth, Vertical Tolerance, degrees	±0.3	±0.3	±0.3
Beam Tilt, degrees	0–6	0–6	0–6
USLS, dB	18	19	19
Front-to-Back Total Power at 180° ± 30°, dB	25	26	26
CPR at Boresight, dB	22	23	22
CPR at Sector, dB	10	10	9
Isolation, dB	30	30	30
VSWR Return Loss, dB	1.4 15.6	1.4 15.6	1.4 15.6
PIM, 3rd Order, 2 x 20 W, dBc	-153	-153	-153
Input Power per Port, maximum, watts	350	350	350
Polarization	±45°	±45°	±45°

*Values calculated using NGMN Alliance N-P-BASTA v9.6

Mechanical Specifications

Color Radome Material	Light gray PVC, UV resistant
Connector Interface Location Quantity	7-16 DIN Female Bottom 4
Wind Loading, maximum	668.0 N @ 150 km/h 150.2 lbf @ 150 km/h
Wind Speed, maximum	241.0 km/h 149.8 mph
Antenna Dimensions, L x W x D	1903.0 mm x 305.0 mm x 166.0 mm 74.9 in x 12.0 in x 6.5 in
Net Weight	19.5 kg 43.0 lb
Model with factory installed AISG 2.0 RET	HBXX-6517DS-A2M



ALCATEL-LUCENT WIRELESS PRODUCT DATASHEET RRH2X60-AWS FOR BAND 4 APPLICATIONS

The Alcatel-Lucent RRH2x60-AWS is a high power, small form factor Remote Radio Head operating in the AWS frequency band (3GPP Band 4) for LTE technology. It is designed with an eco-efficient approach, providing operators with the means to achieve high quality and high capacity coverage with minimum site requirements and efficient operation.



A distributed Node B expands the deployment options by using two components, a Base Band Unit (BBU) containing the digital assets and a separate RRH containing the radio-frequency (RF) elements. This modular design optimizes available space and allows the main components of a Node B to be installed separately, within the same site or several kilometers apart.

The Alcatel-Lucent RRH2x60-AWS is linked to the BBU by an optical-fiber connection carrying downlink and uplink digital radio signals

along with operations, administration and maintenance (OA&M) information.

SUPERIOR RF PERFORMANCE

The Alcatel-Lucent RRH2x60-AWS integrates all the latest technologies. This allows to offer best-in-class characteristics.

It delivers an outstanding 120 watts of total RF power thanks to its two transmit RF paths of 60 W each.

It is ideally suited to support multiple-input multiple-output (MIMO) 2x2 operation.

It includes four RF receivers to natively support 4-way uplink reception diversity. This improves the radio uplink coverage and this can be used to extend the cell radius commensurate with 2x2MIMO 2x60 W for the downlink.

It supports multiple discontinuous LTE carriers within an instantaneous bandwidth of 45 MHz corresponding to the entire AWS B4 spectrum.

The latest generation power amplifiers (PA) used in this product achieve high efficiency (>40%), resulting in improved power consumption figures.

OPTIMIZED TCO

The Alcatel-Lucent RRH2x60-AWS is designed to make available all the benefits of a distributed Node B, with excellent RF characteristics, with low capital expenditures (CAPEX) and low operating expenditures (OPEX).

The Alcatel-Lucent RRH2x60-AWS is a very cost-effective solution to deploy LTE MIMO.

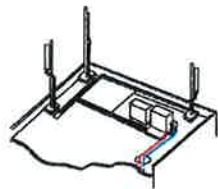
EASY INSTALLATION

The RRH2x60-AWS includes a reversible mounting bracket which allows for ease of installation behind an antenna, or on a rooftop knee wall while providing easy access to the mid body RF connectors.

The limited space available in some sites may prevent the installation of traditional single-cabinet BTS equipment. However, many of these sites can host an Alcatel-Lucent RRH2x60-AWS installation, providing more flexible site selection and improved network quality along with greatly reduced installation time and costs.

The Alcatel-Lucent RRH2x60-AWS is a zero-footprint solution and is convection cooled without fans for silent operation, simplifying negotiations with site property owners and minimizing environmental impacts.

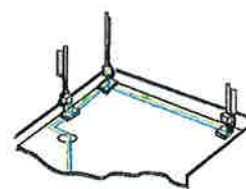
Installation can easily be done by a single person as the Alcatel-Lucent RRH2x60-AWS is compact and weighs about 20 kg, eliminating the need for a crane to hoist the BTS cabinet to the rooftop. A site can be in operation in less than one day.



Macro



RRH for space-constrained cell sites



Distributed

FEATURES

- RRH2x60-AWS integrates two power amplifiers of 60W rating (at each antenna connector)
- Support multiple carriers over the entire 3GPP band 4
- RRH2x60-AWS is optimized for LTE operation
- RRH2x60-AWS is a very compact and lightweight product
- Advanced power management techniques are embedded to provide power savings, such as PA bias control

BENEFITS

- MIMO LTE operation with only one single unit per sector
- Improved uplink coverage with built-in 4-way receive diversity capability
- RRH can be mounted close to the antenna, eliminating nearly all losses in RF cables and thus reducing power consumption by 50% compared to conventional solutions
- Distributed configurations provide easily deployable and cost-effective solutions, near zero footprint and

silent solutions, with minimum impact on the neighborhood, which ease the deployment

- RETA and TMA support without additional hardware thanks to the AISG v2.0 port and the integrated Bias-Tees. Bias-Tees support AISG DC supply and signaling.

TECHNICAL SPECIFICATIONS

Specifications listed are hardware capabilities. Some capabilities depend on support in a specific software release or future release.

Dimensions and weights

- HxWxD : 510x285x186mm (27 l with solar shield)
- Weight : 20 kg (44 lbs)

Electrical Data

- Power Supply : -48V DC (-40.5 to -57V)
- Power Consumption (ETSI average traffic load reference) : 250W @2x60W

RF Characteristics

- Frequency band: 1710-1755, UL / 2110-2155 MHz, DL (3GPP band 4)
- Output power: 2x60W at antenna connectors
- Technology supported: LTE
- Instantaneous bandwidth: 45 MHz
- Rx diversity: 2-way and 4-way uplink reception
- Typical sensitivity without Rx diversity: -105 dBm for LTE

Connectivity

- Two CPRI optical ports for daisy chaining and up to six RRHs per fiber
- Type of optical fiber: Single-Mode (SM) and Multi-Mode (MM) SFPs
- Optical fiber length: up to 500m using MM fiber, up to 20km using SM fiber
- TMA/RETA : AISG 2.0 (RS485 connector and internal Bias-Tee)
- Six external alarms
- Surge protection for all external ports (DC and RF)

Safety and Regulatory Data

- EMC : 3GPP 25113, EN 301 489-1, EN 301 489-23, GR 1089, GR 3108, OET-65
- Safety : IEC60950-1, EN 60825-1, UL, ANSI/NFPA 70, CAN/CSA-C22.2
- Regulatory : FCC Part 15 Class B, CE Mark – European Directive : 2002/95/EC (ROHS); 2002/96/EC (WEEE); 1999/5/EC (R&TTE)
- Health : EN 50385

Environmental specifications

- Operating temperature: -40°C to 55°C including solar load
- Operating relative humidity: 8% to 100%
- Environmental Conditions : ETS 300 019-1-4 class 4.1E
- Ingress Protection : IEC 60529 IP65
- Acoustic Noise : Noiseless (natural convection cooling)

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.....Alcatel-Lucent

AT THE SPEED OF IDEAS™

Alcatel-Lucent 

HYBRIFLEX™ RRH Hybrid Feeder Cabling Solution, 1-5/8", Single-Mode Fiber

Product Description

RFS' HYBRIFLEX Remote Radio Head (RRH) hybrid feeder cabling solution combines optical fiber and DC power for RRHs in a single lightweight aluminum corrugated cable, making it the world's most innovative solution for RRH deployments.

It was developed to reduce installation complexity and costs at Cellular sites. HYBRIFLEX allows mobile operators deploying an RRH architecture to standardize the RRH installation process and eliminate the need for and cost of cable grounding. HYBRIFLEX combines optical fiber (multi-mode or single-mode) and power in a single corrugated cable. It eliminates the need for junction boxes and can connect multiple RRHs with a single feeder. Standard RFS CELLFLEX® accessories can be used with HYBRIFLEX cable. Both pre-connectorized and on-site options are available.

Features/Benefits

- Aluminum corrugated armor with outstanding bending characteristics - minimizes installation time and enables mechanical protection and shielding
- Same accessories as 1 5/8" coaxial cable
- Outer conductor grounding - Eliminates typical grounding requirements and saves on installation costs
- Lightweight solution and compact design - Decreases tower loading
- Robust cabling - Eliminates need for expensive cable trays and ducts
- Installation of tight bundled fiber optic cable pairs directly to the RRH - Reduces CAPEX and wind load by eliminating need for interconnection
- Optical fiber and power cables housed in single corrugated cable - Saves CAPEX by standardizing RRH cable installation and reducing installation requirements
- Outdoor polyethylene jacket - Ensures long-lasting cable protection

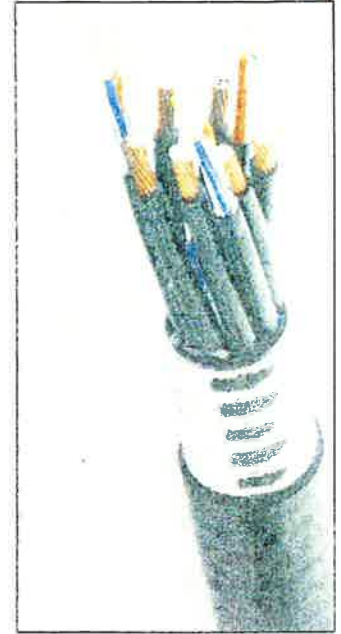


Figure 1: HYBRIFLEX Series

Technical Specifications

Structure			
Outer Conductor Armor	Corrugated Aluminum	[mm (in.)]	46.5 (1.83)
Jacket	Polyethylene, PE	[mm (in.)]	50.3 (1.98)
UV-Protection	Individual and External Jacket		Yes
Mechanical Properties			
Weight, Approximate		[kg/m (lb/ft)]	1.9 (1.30)
Minimum Bending Radius, Single Bending		[mm (in.)]	200 (8)
Minimum Bending Radius, Repeated Bending		[mm (in.)]	500 (20)
Recommended/Maximum Clamp Spacing		[m (ft)]	1.0 / 1.2 (3.25 / 4.0)
Electrical Properties			
DC-Resistance Outer Conductor Armor		[Ω/km (Ω/1000ft)]	0.68 (0.205)
DC-Resistance Power Cable, 8.4mm ² (8AWG)		[Ω/km (Ω/1000ft)]	2.1 (0.307)
Optical Properties			
Version			Single-mode OM3
Quantity, Fiber Count			16 (8 pairs)
Core/Clad		[μm]	50/125
Primary Coating (Acrylate)		[μm]	245
Buffer Diameter, Nominal		[μm]	900
Secondary Protection, Jacket, Nominal		[mm (in.)]	2.0 (0.08)
Minimum Bending Radius		[mm (in.)]	104 (4.1)
Insertion Loss @ wavelength 850nm		dB/km	3.0
Insertion Loss @ wavelength 1310nm		dB/km	1.0
Standards (Meets or exceeds)			UL34-V0, UL1666 RoHS Compliant
DC Power Cable Properties			
Size (Power)		[mm (AWG)]	8.4 (8)
Quantity, Wire Count (Power)			16 (8 pairs)
Size (Alarm)		[mm (AWG)]	0.8 (18)
Quantity, Wire Count (Alarm)			4 (2 pairs)
Type			UV protected
Strands			19
Primary Jacket Diameter, Nominal		[mm (in.)]	6.8 (0.27)
Standards (Meets or exceeds)			NFPA 130, ICEA S-95-658 UL Type XHHW-2, UL 44 UL-LS Limited Smoke, UL VW-1 IEEE-383 (1974), IEEE1202/FT4 RoHS Compliant
Environment			
Installation Temperature		[°C (°F)]	-40 to +65 (-40 to 149)
Operation Temperature		[°C (°F)]	-40 to +65 (-40 to 149)

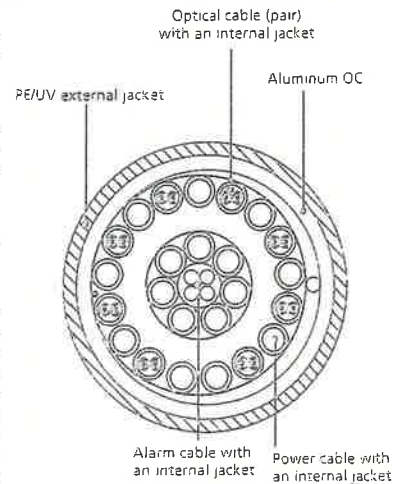


Figure 2: Construction Detail

All information contained in the present datasheet is subject to confirmation at time of ordering

* This data is provisional and subject to change

ATTACHMENT 2

		General		Power		Density							
Site Name: Manchester S (Glastonbury)													
Tower Height: 104Ft.													
CARRIER	# OF CHAN.	WATTS ERP	HEIGHT	CALC. POWER DENS	FREQ.	MAX. PERMISS. EXP.	FRACTION MPE	Total					
*Town	1	16.829	75	0.0011	960	0.6400	0.17%						
*Clearwire	2	153	55	0.0364	2496	1.0000	3.64%						
*Clearwire	1	211	55	0.0251	11 GHz	1.0000	2.51%						
*MetroPCS/T-Mobile LTE	2	24	65	0.0041	2100	1.0000	0.41%						
*MetroPCS/T-Mobile GSM/	2	12	65	0.0020	1950	1.0000	0.20%						
*MetroPCS/T-Mobile UMTS	2	12	65	0.0020	2100	1.0000	0.20%						
*AT&T UMTS	2	565	89	0.0513	880	0.5867	0.87%						
*AT&T UMTS	2	1077	89	0.0978	1900	1.0000	0.98%						
*AT&T GSM	1	647	89	0.0294	880	0.5867	0.50%						
*AT&T GSM	4	934	89	0.1696	1900	1.0000	1.70%						
*AT&T LTE	1	1615	89	0.0733	734	0.4893	1.50%						
*Cox	1	100	94	0.0041	451	0.3007	1.35%						
*Cox	6	100	70	0.0440	452	0.3013	14.61%						
Verizon PCS	11	457	102	0.1737	1970	1.0000	17.37%						
Verizon Cellular	9	414	102	0.1288	869	0.5793	22.23%						
Verizon AWS	1	1750	102	0.0605	2145	1.0000	6.05%						
Verizon 700	1	1050	102	0.0363	698	0.4973	7.30%						
								81.59%					
* Source: Siting Council													

ATTACHMENT 3

August 28, 2014

Ms. Tracy Lee
 Insite Towers, LLC
 1199 N. Fairfax St., Ste. 700
 Alexandria, VA 22314

Re: Tower Structural Analysis- Verizon Antenna Installation

Site Number:	CT901	Site Address:	577 Bell Street Glastonbury, CT
Site Name:	Glastonbury		
Tower Owner:	N/A	Lat/Long:	41.7338/-72.5497
Tower Type:	104-ft Self-Support Tower	B&P Job No:	14013.003
Tower Status:	Passed (99.3% capacity)	Foundation Status:	Passed

We have completed our structural analysis of the proposed equipment installation on the foregoing tower to determine its ability to support the new loads proposed by Verizon Antennas.

The following information was provided for our tower structural analysis:

- Tower: Member sizes and configuration were obtained from the previous structural analysis by Bennett & Pless dated 5/29/2014.
- Foundation: Foundation details were obtained from the previous structural analysis listed above.
- Geotechnical: A geotechnical report was not available for this site.
- Antennas: Proposed antenna loading was obtained from the tenant application provided by Insite Towers, LLC dated 08/05/2014. Existing antenna loading was obtained from the structural analysis listed above.
- Other: General photographs of the tower

Table 1 summarizes the antenna, attachment, and transmission line loading proposed and Table 2 summarizes the design criteria used for our structural analysis. Attached is a copy of the structural calculations, which in addition to detailed results of the analysis also includes a tower profile with member sizes and configuration, and the existing/proposed equipment list with types and location.

Table 1 – Proposed Equipment Loading

Status	Antennas/Attachments				Transmission Lines ^{1,2}	
	Rad Center	Quantity	Manufacturer	Model	# of Feed lines	Feed line Size (in)
New Antenna	102	6	Andrew	HBXX-6517DS-A2M	2	1 5/8" Fiber
New Antenna	102	3	Antel	BXA-70063-6CF		
New Antenna	102	3	Andrew	LNx-8514DS		
New Antenna	102	3	ALU	RRH 2x60-AW		
New Antenna	102	1	Raycap	RC2DC-3315-PF-48		

¹Note: See attachment for transmission line layout

²Note: Existing (18) 1 5/8" Verizon feed lines to remain

Table 2 – Design Criteria Used for Structural Analysis

Criterion	Information Used
State Building Code	Connecticut (IBC 2003)
Tower Standard	EIA/TIA-222-F
County	Montgomery
Basic Wind Speed	80 mph, no ice 69 mph, 1/2" ice
Steel Grade Assumed	50 ksi SR legs, 36 ksi all others, A325 bolts
Tower Analysis Software	tnxTower (version 6.1.4.1)

Based on the foregoing information, our structural analysis determined that **the existing tower is structurally capable of supporting the proposed equipment loads without modification.** The foundation reactions as a result of the proposed installation are less than the designed foundation modification reactions and as such the existing foundation is structurally capable of supporting the proposed Verizon loads.

The following assumptions were made in conducting our structural analysis:

1. The existing tower has been maintained to manufacturer's specifications and is in good condition.
2. All member connections are assumed to have been designed to meet the load carrying capacity of the connected member.
3. Antenna mount loads have been estimated based on typical industry standards.
4. The new feed lines for the proposed Verizon antennas are assumed to be mounted across one tower face opposite of the existing feed lines.
5. The mounts for the proposed antennas have been analyzed and designed by others.
6. **Existing antennas (6 Antel LPA-80063/6CF and 6 Antel LPA-171063-12CF) at 100' have been removed prior to the Verizon Antennas installation.**
7. See additional assumptions contained in the report attached.

Bennett & Pless, Inc. makes no warranties, expressed or implied, in connection with this report, and disclaims any liability arising from material, fabrication and erection of this tower. Bennett & Pless, Inc. will not be responsible whatsoever for or on account of, consequential or incidental damages sustained by any person, firm, or organization as a result of any data or conclusions contained in this report. The maximum liability of Bennett & Pless, Inc. pursuant to this report will be limited to the total fee received for preparation of this report.

We appreciate the opportunity of providing our continuing professional services to you. If you have any questions or need further assistance on this please call us anytime.

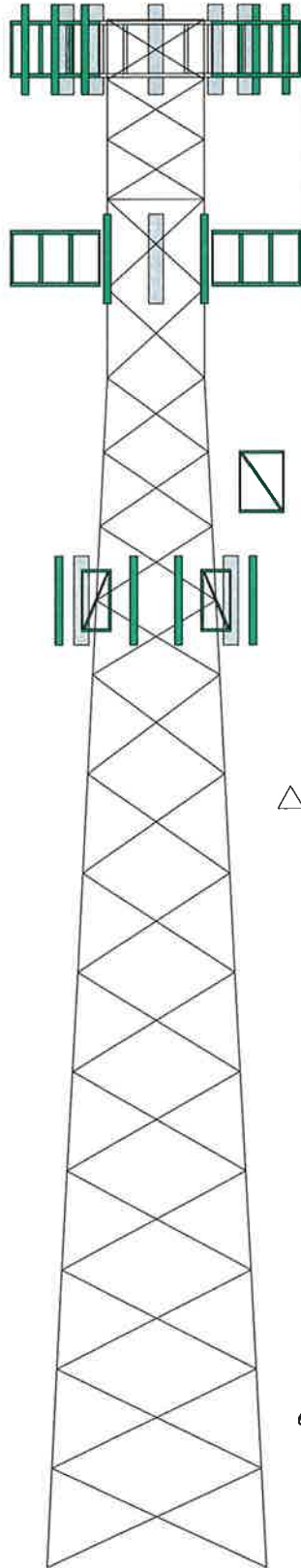
Yours very truly,
Bennett & Pless, Inc.



Mike De Boer, P.E.
 Senior Technical Director, Telecom



Section	T1	T2	T3	T4	T5	T6	
Legs	P2x154	P2.5x203	Pipe 2.875 x 2.03 w/ 3/8 Plate	Pipe 2.875 x 2.03 w/ 3/8 Plate	Pipe 2.875 x 0.276 w/ 3/8 Plate	Pipe 3 x 0.3 w/ 3/8 Plate	
Leg Grade	L1 1/2x1 1/2x3/16	L1 1/2x1 1/2x1/4	A618-50	A618-50	L2 1/2x2 1/2x3/16	L2 1/2x2 1/2x3/16	
Diagonals							
Diagonal Grade	L2x2x3/16		A36	A36			
Top Girts				N.A.			
Face Width (ft)	6.52	6.56	8.56	10.56	12.6	14.85	
# Panels @ (ft)	3 @ 4	2 @ 8	4 @ 5	9 @ 6.66667	0.7	0.6	
Weight (K)	0.3	0.3	0.5	0.6	0.7	0.6	



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
T-Frame Sector (Verizon)	102	Andrew SBNH-1D6565C (ATI)	88
T-Frame Sector (Verizon)	102	Andrew SBNH-1D6565C (ATI)	88
T-Frame Sector (Verizon)	102	(2) TMA (ATI)	88
BXA-70063-6CF (Verizon)	102	(2) TMA (ATI)	88
BXA-70063-6CF (Verizon)	102	(2) TMA (ATI)	88
BXA-70063-6CF (Verizon)	102	(2) RRU-11 (ATI)	88
(2) HBXX-6517DS-A2M (Verizon)	102	(2) RRU-11 (ATI)	88
(2) HBXX-6517DS-A2M (Verizon)	102	(2) RRU-11 (ATI)	88
(2) HBXX-6517DS-A2M (Verizon)	102	Demarcation Box DC6-4860-188F (ATI)	88
LNK-8514DS (Verizon)	102		
LNK-8514DS (Verizon)	102	T-Frame Sector (ATI)	88
LNK-8514DS (Verizon)	102	T-Frame Sector (ATI)	88
LNK-8514DS (Verizon)	102	DB806-XT (Town of Glastonbury)	79
RC2DC-3315-PF-48 (Verizon)	102	PR-950 (Town of Glastonbury)	73
RRH-2x60-AW (Verizon)	102	PIROD 6' Side Mount Standoff (Town of Glastonbury)	73
RRH-2x60-AW (44lb) (Verizon)	102		
RRH-2x60-AW (44lb) (Verizon)	102	3' Stand-Off (Metro PCS)	65
Powerwave P65-17-XLH-RR (ATI)	88	3' Stand-Off (Metro PCS)	65
Powerwave P65-17-XLH-RR (ATI)	88	3' Stand-Off (Metro PCS)	65
Powerwave P65-17-XLH-RR (ATI)	88	(2) AIR 21 (Metro PCS)	65
KMW AX-X-CD-1665-OOT (ATI)	88	(2) AIR 21 (Metro PCS)	65
KMW AX-X-CD-1665-OOT (ATI)	88	(2) AIR 21 (Metro PCS)	65
KMW AX-X-CD-1665-OOT (ATI)	88		
Andrew SBNH-1D6565C (ATI)	88		

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A618-50	50 ksi	70 ksi	A36	36 ksi	58 ksi

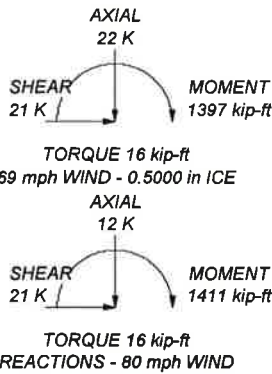
TOWER DESIGN NOTES

1. Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
2. Tower is also designed for a 69 mph basic wind with 0.50 in ice.
3. Deflections are based upon a 60 mph wind.
4. Weld together tower sections have flange connections.
5. Connections use galvanized A325 bolts, nuts and locking devices. Installation per TIA/EIA-222 and AISC Specifications.
6. Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
7. Welds are fabricated with ER-70S-6 electrodes.
8. TOWER RATING: 99.3%

MAX. CORNER REACTIONS AT BASE:

DOWN: 117 K
SHEAR: 12 K

UPLIFT: -104 K
SHEAR: 12 K



Bennett & Pless		Job: CT901 Glastonbury	
3395 Northeast Expressway NE		Project: SST Analysis	
Atlanta, GA 30341		Client: Insite Towers, LLC	
Phone: 678-990-8700		Drawn by: J. Turner	
FAX: 678-990-8701		Date: 08/28/14	
		Scale: NTS	
		Path: Dwg No. E-1	

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Tower Input Data

The main tower is a 3x free standing tower with an overall height of 104.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 6.52 ft at the top and 14.65 ft at the base.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Basic wind speed of 80 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 69 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

Weld together tower sections have flange connections..

Connections use galvanized A325 bolts, nuts and locking devices. Installation per TIA/EIA-222 and AISC Specifications..

Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards..

Welds are fabricated with ER-70S-6 electrodes..

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

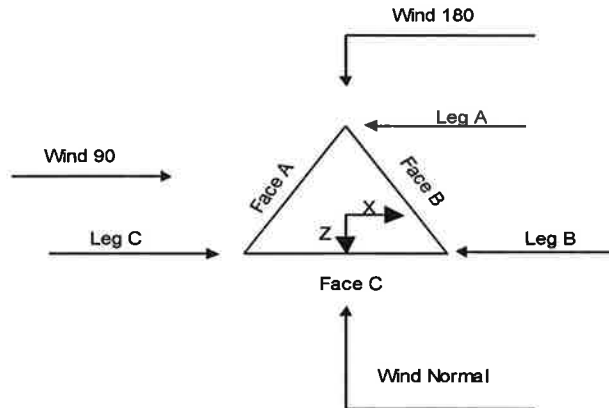
Stress ratio used in tower member design is 1.333.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Treat Feedline Bundles As Cylinder
Consider Moments - Horizontals	Assume Legs Pinned	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Diagonals	√ Assume Rigid Index Plate	√ Calculate Redundant Bracing Forces
Use Moment Magnification	√ Use Clear Spans For Wind Area	Ignore Redundant Members in FEA
√ Use Code Stress Ratios	√ Use Clear Spans For KL/r	SR Leg Bolts Resist Compression
√ Use Code Safety Factors - Guys	Retension Guys To Initial Tension	√ All Leg Panels Have Same Allowable
Escalate Ice	Bypass Mast Stability Checks	Offset Girt At Foundation
Always Use Max Kz	√ Use Azimuth Dish Coefficients	√ Consider Feedline Torque
Use Special Wind Profile	√ Project Wind Area of Appurt.	Include Angle Block Shear Check
√ Include Bolts In Member Capacity	Autocalc Torque Arm Areas	Poles
√ Leg Bolts Are At Top Of Section	SR Members Have Cut Ends	Include Shear-Torsion Interaction
Secondary Horizontal Braces Leg	√ Sort Capacity Reports By Component	Always Use Sub-Critical Flow
Use Diamond Inner Bracing (4 Sided)	Triangulate Diamond Inner Bracing	Use Top Mounted Sockets
Add IBC .6D+W Combination	Use TIA-222-G Tension Splice Capacity	
	Exemption	

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Triangular Tower

Tower Section Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	ft			ft		ft
T1	104.00-92.00			6.52	1	12.00
T2	92.00-80.00			6.52	1	12.00
T3	80.00-60.00			6.56	1	20.00
T4	60.00-40.00			8.56	1	20.00
T5	40.00-20.00			10.56	1	20.00
T6	20.00-0.00			12.60	1	20.00

Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T1	104.00-92.00	4.00	X Brace	No	No	0.0000	0.0000
T2	92.00-80.00	6.00	X Brace	No	No	0.0000	0.0000
T3	80.00-60.00	5.00	X Brace	No	No	0.0000	0.0000
T4	60.00-40.00	6.67	X Brace	No	No	0.0000	0.0000
T5	40.00-20.00	6.67	X Brace	No	No	0.0000	0.0000
T6	20.00-0.00	6.67	X Brace	No	No	0.0000	0.0000

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Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 104.00-92.00	Pipe	P2x.154	A618-50 (50 ksi)	Single Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T2 92.00-80.00	Pipe	P2x.154	A618-50 (50 ksi)	Single Angle	L1 1/2x1 1/2x1/4	A36 (36 ksi)
T3 80.00-60.00	Pipe	P2.5x.203	A618-50 (50 ksi)	Single Angle	L1 1/2x1 1/2x1/4	A36 (36 ksi)
T4 60.00-40.00	Arbitrary Shape	Pipe 2.875 x .203 w/ 3/8 Plate	A618-50 (50 ksi)	Single Angle	L2x2x3/16	A36 (36 ksi)
T5 40.00-20.00	Arbitrary Shape	Pipe 2.875 x 0.276 w/ 3/8 Plate	A618-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T6 20.00-0.00	Arbitrary Shape	Pipe 3 x 0.3 w/ 3/8 Plate	A618-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 104.00-92.00	Single Angle	L2x2x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T2 92.00-80.00	Single Angle	L2x2x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in
T1 104.00-92.00	0.00	0.2500	A36 (36 ksi)	1.02	1	1	36.0000	36.0000
T2 92.00-80.00	0.00	0.2500	A36 (36 ksi)	1.02	1	1	36.0000	36.0000
T3 80.00-60.00	0.00	0.2500	A36 (36 ksi)	1.02	1	1	36.0000	36.0000
T4 60.00-40.00	0.00	0.2500	A36 (36 ksi)	1.02	1	1	36.0000	36.0000
T5 40.00-20.00	0.00	0.2500	A36 (36 ksi)	1.02	1	1	36.0000	36.0000
T6 20.00-0.00	0.00	0.2500	A36 (36 ksi)	1.02	1	1	36.0000	36.0000

Tower Section Geometry (cont'd)

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Tower Elevation ft	Leg Connection Type	Leg Bolt Size in	Leg No.	Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
				Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T6 20.00-0.00	Flange	0.7500 A325N	4	0.5000 A325N	1	0.0000 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
Feedline Ladder (Tower)	C	Yes	Ar (CfAe)	65.00 - 6.00	0.0000	0.43	1	1	0.5000	3.0000		8.40
LDF7-50A (1 5/8 FOAM)	B	Yes	Ar (CfAe)	100.00 - 6.00	-6.0000	-0.25	18	9	0.5000	1.9800		0.82
Feedline Ladder (Tower)	B	Yes	Ar (CfAe)	100.00 - 6.00	-2.0000	-0.25	1	1	0.5000	3.0000		8.40
Hybrid Flex (1 5/8 Fiber) (Metro PCS)	A	Yes	Ar (CfAe)	65.00 - 6.00	0.0000	0.43	1	1	1.9800	1.9800		0.82
1 5/8" (1.63", 41.3 mm) Fiber (Verizon)	C	Yes	Ar (CfAe)	102.00 - 6.00	0.0000	0	1	1	1.6300	1.6300		0.82
1 5/8" (1.63", 41.3 mm) Fiber (Verizon)	A	Yes	Ar (CfAe)	102.00 - 6.00	0.0000	0	1	1	1.6300	1.6300		0.82

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	C _{AA}	Weight plf	
LDF7-50A (1 5/8 FOAM)	A	No	CaAa (In Face)	92.00 - 6.00	-4.0000	0.43	12	No Ice 1/2" Ice	0.16 0.26	0.82 2.05

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
T1	104.00-92.00	A	1.358	0.000	0.000	0.000	0.01
		B	13.880	0.000	0.000	0.000	0.19
		C	1.358	0.000	0.000	0.000	0.01
T2	92.00-80.00	A	1.630	0.000	23.401	0.000	0.13
		B	20.820	0.000	0.000	0.000	0.28
		C	1.630	0.000	0.000	0.000	0.01

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Tower Section	Tower Elevation ft	Face	A_R ft ²	A_F ft ²	C_{AA} In Face ft ²	C_{AA} Out Face ft ²	Weight K
T3	80.00-60.00	A	3.542	0.000	39.001	0.000	0.22
		B	34.700	0.000	0.000	0.000	0.46
		C	3.967	0.000	0.000	0.000	0.06
T4	60.00-40.00	A	6.017	0.000	39.001	0.000	0.23
		B	34.700	0.000	0.000	0.000	0.46
		C	7.717	0.000	0.000	0.000	0.18
T5	40.00-20.00	A	6.017	0.000	39.001	0.000	0.23
		B	34.700	0.000	0.000	0.000	0.46
		C	7.717	0.000	0.000	0.000	0.18
T6	20.00-0.00	A	4.212	0.000	27.301	0.000	0.16
		B	24.290	0.000	0.000	0.000	0.32
		C	5.402	0.000	0.000	0.000	0.13

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A_R ft ²	A_F ft ²	C_{AA} In Face ft ²	C_{AA} Out Face ft ²	Weight K
T1	104.00-92.00	A	0.500	2.192	0.000	0.000	0.000	0.02
		B		4.653	13.227	0.000	0.000	0.41
		C		2.192	0.000	0.000	0.000	0.02
T2	92.00-80.00	A	0.500	2.630	0.000	37.800	0.000	0.32
		B		6.980	19.840	0.000	0.000	0.62
		C		2.630	0.000	0.000	0.000	0.03
T3	80.00-60.00	A	0.500	5.625	0.000	63.000	0.000	0.55
		B		11.633	33.067	0.000	0.000	1.03
		C		6.050	0.000	0.000	0.000	0.10
T4	60.00-40.00	A	0.500	9.350	0.000	63.000	0.000	0.58
		B		11.633	33.067	0.000	0.000	1.03
		C		11.050	0.000	0.000	0.000	0.25
T5	40.00-20.00	A	0.500	9.350	0.000	63.000	0.000	0.58
		B		11.633	33.067	0.000	0.000	1.03
		C		11.050	0.000	0.000	0.000	0.25
T6	20.00-0.00	A	0.500	6.545	0.000	44.100	0.000	0.41
		B		8.143	23.147	0.000	0.000	0.72
		C		7.735	0.000	0.000	0.000	0.18

Feed Line Shielding

Section	Elevation ft	Face	A_R ft ²	A_R Ice ft ²	A_F ft ²	A_F Ice ft ²
T1	104.00-92.00	A	0.000	0.122	0.118	0.191
		B	0.000	0.998	1.211	1.559
		C	0.000	0.122	0.118	0.191
T2	92.00-80.00	A	0.000	0.117	0.115	0.185
		B	0.000	1.197	1.466	1.889
		C	0.000	0.117	0.115	0.185
T3	80.00-60.00	A	0.000	0.225	0.213	0.338
		B	0.000	1.790	2.085	2.686
		C	0.000	0.242	0.238	0.363
T4	60.00-40.00	A	0.000	0.285	0.367	0.571
		B	0.000	1.364	2.118	2.728
		C	0.000	0.337	0.471	0.674

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Section	Elevation	Face	A_R	$A_{R\ Ice}$	A_F	$A_{F\ Ice}$
	ft		ft ²	ft ²	ft ²	ft ²
T5	40.00-20.00	A	0.000	0.270	0.434	0.675
		B	0.000	1.290	2.504	3.226
		C	0.000	0.319	0.557	0.797
T6	20.00-0.00	A	0.000	0.182	0.293	0.456
		B	0.000	0.871	1.691	2.178
		C	0.000	0.215	0.376	0.538

Feed Line Center of Pressure

Section	Elevation	CP_x	CP_z	$CP_x\ Ice$	$CP_z\ Ice$
	ft	in	in	in	in
T1	104.00-92.00	0.7906	-6.2203	0.4207	-4.4099
T2	92.00-80.00	0.5791	-9.4112	0.3840	-8.6655
T3	80.00-60.00	0.4551	-10.7402	0.2402	-9.9641
T4	60.00-40.00	-0.1229	-14.7086	-0.3502	-13.7276
T5	40.00-20.00	-0.1223	-16.8463	-0.4301	-15.9509
T6	20.00-0.00	-0.1137	-17.3403	-0.4607	-16.5629

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement	$C_{AA\ Front}$	$C_{AA\ Side}$	Weight	
			ft ft ft	°	ft	ft ²	ft ²	K	
T-Frame Sector (Verizon)	A	From Leg	4.00	0.0000	102.00	No Ice	9.00	9.00	0.47
			0.00			1/2" Ice	9.30	9.30	0.61
			0.00						
T-Frame Sector (Verizon)	B	From Leg	4.00	0.0000	102.00	No Ice	9.00	9.00	0.47
			0.00			1/2" Ice	9.30	9.30	0.61
			0.00						
T-Frame Sector (Verizon)	C	From Leg	4.00	0.0000	102.00	No Ice	9.00	9.00	0.47
			0.00			1/2" Ice	9.30	9.30	0.61
			0.00						
BXA-70063-6CF (Verizon)	A	From Leg	4.00	0.0000	102.00	No Ice	7.59	4.04	0.02
			0.00			1/2" Ice	8.13	4.48	0.06
			0.00						
BXA-70063-6CF (Verizon)	B	From Leg	4.00	0.0000	102.00	No Ice	7.59	4.04	0.02
			0.00			1/2" Ice	8.13	4.48	0.06
			0.00						
BXA-70063-6CF (Verizon)	C	From Leg	4.00	0.0000	102.00	No Ice	7.59	4.04	0.02
			0.00			1/2" Ice	8.13	4.48	0.06
			0.00						
(2) HBXX-6517DS-A2M (Verizon)	A	From Leg	4.00	0.0000	102.00	No Ice	8.74	5.24	0.04
			-6.00			1/2" Ice	9.31	5.71	0.09
			0.00						
(2) HBXX-6517DS-A2M (Verizon)	A	From Leg	4.00	0.0000	102.00	No Ice	8.74	5.24	0.04
			6.00			1/2" Ice	9.31	5.71	0.09
			0.00						

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									J. Turner	

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K	
(2) HBXX-6517DS-A2M (Verizon)	A	From Leg	4.00 -4.00 0.00	0.0000	102.00	No Ice 1/2" Ice	8.74 9.31	5.24 5.71	0.04 0.09
LNX-8514DS (Verizon)	B	From Leg	4.00 -4.00 0.00	0.0000	102.00	No Ice 1/2" Ice	11.45 12.06	7.70 8.29	0.05 0.12
LNX-8514DS (Verizon)	C	From Leg	4.00 -4.00 0.00	0.0000	102.00	No Ice 1/2" Ice	11.45 12.06	7.70 8.29	0.05 0.12
LNX-8514DS (Verizon)	A	From Leg	4.00 4.00 0.00	0.0000	102.00	No Ice 1/2" Ice	11.45 12.06	7.70 8.29	0.05 0.12
RC2DC-3315-PF-48 (Verizon)	C	From Leg	4.00 4.00 0.00	0.0000	102.00	No Ice 1/2" Ice	2.29 2.51	3.52 3.77	0.04 0.07
T-Frame Sector (AT&T)	B	From Leg	4.00 0.00 0.00	0.0000	88.00	No Ice 1/2" Ice	9.00 9.30	9.00 9.30	0.47 0.61
T-Frame Sector (AT&T)	C	From Leg	4.00 0.00 0.00	0.0000	88.00	No Ice 1/2" Ice	9.00 9.30	9.00 9.30	0.47 0.61
DB806-XT (Town of Glastonbury)	B	From Leg	4.00 0.00 0.00	0.0000	79.00	No Ice 1/2" Ice	1.14 1.68	1.14 1.68	0.02 0.03
PR-950 (Town of Glastonbury)	B	From Leg	4.00 0.00 0.00	0.0000	73.00	No Ice 1/2" Ice	6.35 11.43	6.35 11.43	0.04 0.05
PiROD 6' Side Mount Standoff (Town of Glastonbury)	B	From Leg	4.00 0.00 0.00	0.0000	73.00	No Ice 1/2" Ice	4.97 6.12	4.97 6.12	0.07 0.13
(2) AIR 21 (Metro PCS)	A	From Leg	0.00 0.00 0.00	0.0000	65.00	No Ice 1/2" Ice	6.53 6.98	5.72 6.42	0.12 0.18
(2) AIR 21 (Metro PCS)	B	From Leg	0.00 0.00 0.00	0.0000	65.00	No Ice 1/2" Ice	6.53 6.98	5.72 6.42	0.12 0.18
(2) AIR 21 (Metro PCS)	C	From Leg	0.00 0.00 0.00	0.0000	65.00	No Ice 1/2" Ice	6.53 6.98	5.72 6.42	0.12 0.18
3' Stand-Off (Metro PCS)	A	From Leg	0.00 0.00 0.00	0.0000	65.00	No Ice 1/2" Ice	4.00 4.25	8.00 8.50	0.05 0.07
3' Stand-Off (Metro PCS)	B	From Leg	0.00 0.00 0.00	0.0000	65.00	No Ice 1/2" Ice	4.00 4.25	8.00 8.50	0.05 0.07
3' Stand-Off (Metro PCS)	C	From Leg	0.00 0.00 0.00	0.0000	65.00	No Ice 1/2" Ice	4.00 4.25	8.00 8.50	0.05 0.07
Powerwave P65-17-XLH-RR (AT&T)	A	From Leg	0.00 0.00 0.00	0.0000	88.00	No Ice 1/2" Ice	11.70 12.42	8.94 10.45	0.09 0.12
Powerwave P65-17-XLH-RR (AT&T)	B	From Leg	0.00 0.00 0.00	0.0000	88.00	No Ice 1/2" Ice	11.70 12.42	8.94 10.45	0.09 0.12
Powerwave P65-17-XLH-RR (AT&T)	C	From Leg	0.00 0.00 0.00	0.0000	88.00	No Ice 1/2" Ice	11.70 12.42	8.94 10.45	0.09 0.12

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A _A		Weight	
			Horz	Lateral			Front	Side		
			ft	ft	°	ft	ft ²	ft ²	K	
KMW AX-X-CD-1665-OOT (AT&T)	A	From Leg	0.00	0.00	0.0000	88.00	No Ice	8.50	6.30	0.07
			0.00	0.00			1/2" Ice	9.15	7.48	0.09
			0.00	0.00						
KMW AX-X-CD-1665-OOT (AT&T)	B	From Leg	0.00	0.00	0.0000	88.00	No Ice	8.50	6.30	0.07
			0.00	0.00			1/2" Ice	9.15	7.48	0.09
			0.00	0.00						
KMW AX-X-CD-1665-OOT (AT&T)	C	From Leg	0.00	0.00	0.0000	88.00	No Ice	8.50	6.30	0.07
			0.00	0.00			1/2" Ice	9.15	7.48	0.09
			0.00	0.00						
Andrew SBNH-1D6565C (AT&T)	A	From Leg	0.00	0.00	0.0000	88.00	No Ice	11.64	9.84	0.09
			0.00	0.00			1/2" Ice	12.37	11.37	0.18
			0.00	0.00						
Andrew SBNH-1D6565C (AT&T)	B	From Leg	0.00	0.00	0.0000	88.00	No Ice	11.64	9.84	0.09
			0.00	0.00			1/2" Ice	12.37	11.37	0.18
			0.00	0.00						
Andrew SBNH-1D6565C (AT&T)	C	From Leg	0.00	0.00	0.0000	88.00	No Ice	11.64	9.84	0.09
			0.00	0.00			1/2" Ice	12.37	11.37	0.18
			0.00	0.00						
(2) TMA (AT&T)	A	From Leg	0.00	0.00	0.0000	88.00	No Ice	1.69	0.85	0.02
			0.00	0.00			1/2" Ice	1.87	0.98	0.03
			0.00	0.00						
(2) TMA (AT&T)	B	From Leg	0.00	0.00	0.0000	88.00	No Ice	1.69	0.85	0.02
			0.00	0.00			1/2" Ice	1.87	0.98	0.03
			0.00	0.00						
(2) TMA (AT&T)	C	From Leg	0.00	0.00	0.0000	88.00	No Ice	1.69	0.85	0.02
			0.00	0.00			1/2" Ice	1.87	0.98	0.03
			0.00	0.00						
(2) RRU-11 (AT&T)	A	From Leg	0.00	0.00	0.0000	88.00	No Ice	4.42	1.19	0.06
			0.00	0.00			1/2" Ice	4.71	1.35	0.08
			0.00	0.00						
(2) RRU-11 (AT&T)	B	From Leg	0.00	0.00	0.0000	88.00	No Ice	4.42	1.19	0.06
			0.00	0.00			1/2" Ice	4.71	1.35	0.08
			0.00	0.00						
(2) RRU-11 (AT&T)	C	From Leg	0.00	0.00	0.0000	88.00	No Ice	4.42	1.19	0.06
			0.00	0.00			1/2" Ice	4.71	1.35	0.08
			0.00	0.00						
Demarcation Box DC6-4860-188F (AT&T)	C	From Leg	0.00	0.00	0.0000	88.00	No Ice	4.45	0.89	0.02
			0.00	0.00			1/2" Ice	4.76	1.04	0.05
			0.00	0.00						
RRH-2x60-AW (Verizon)	C	From Leg	4.00	0.00	0.0000	102.00	No Ice	2.52	1.59	0.03
			0.00	0.00			1/2" Ice	2.75	1.80	0.04
			0.00	0.00						
RRH-2x60-AW (44lb) (Verizon)	C	From Leg	4.00	0.00	0.0000	102.00	No Ice	2.52	1.59	0.04
			0.00	0.00			1/2" Ice	2.75	1.80	0.06
			0.00	0.00						
RRH-2x60-AW (44lb) (Verizon)	C	From Leg	4.00	0.00	0.0000	102.00	No Ice	2.52	1.59	0.04
			0.00	0.00			1/2" Ice	2.75	1.80	0.06
			0.00	0.00						

Tower Pressures - No Ice

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$$G_H = 1.159$$

Section Elevation	z	K _Z	q _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
ft	ft		psf	ft ²		ft ²	ft ²	ft ²			
T1 104.00-92.00	98.00	1.365	22	80.615	A	6.630	6.108	4.750	37.29	0.000	0.000
					B	5.538	18.630		19.65	0.000	0.000
					C	6.630	6.108		37.29	0.000	0.000
T2 92.00-80.00	86.00	1.315	22	80.855	A	5.349	6.380	4.750	40.50	23.401	0.000
					B	3.998	25.570		16.06	0.000	0.000
					C	5.349	6.380		40.50	0.000	0.000
T3 80.00-60.00	70.00	1.24	20	155.998	A	8.752	13.141	9.599	43.85	39.001	0.000
					B	6.880	44.299		18.76	0.000	0.000
					C	8.726	13.566		43.06	0.000	0.000
T4 60.00-40.00	50.00	1.126	18	197.250	A	11.196	6.017	0.000	0.00	39.001	0.000
					B	9.445	34.700		0.00	0.000	0.000
					C	11.092	7.717		0.00	0.000	0.000
T5 40.00-20.00	30.00	1	16	237.659	A	16.208	6.017	0.000	0.00	39.001	0.000
					B	14.138	34.700		0.00	0.000	0.000
					C	16.086	7.717		0.00	0.000	0.000
T6 20.00-0.00	10.00	1	16	279.593	A	18.604	4.212	0.000	0.00	27.301	0.000
					B	17.207	24.290		0.00	0.000	0.000
					C	18.522	5.402		0.00	0.000	0.000

Tower Pressure - With Ice

$$G_H = 1.159$$

Section Elevation	z	K _Z	q _z	t _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
ft	ft		psf	in	ft ²		ft ²	ft ²	ft ²			
T1 104.00-92.00	98.00	1.365	17	0.5000	81.615	A	6.558	13.055	6.750	34.42	0.000	0.000
						B	18.416	14.640		20.42	0.000	0.000
						C	6.558	13.055		34.42	0.000	0.000
T2 92.00-80.00	86.00	1.315	16	0.5000	81.855	A	5.279	12.658	6.750	37.63	37.800	0.000
						B	23.415	15.928		17.16	0.000	0.000
						C	5.279	12.658		37.63	0.000	0.000
T3 80.00-60.00	70.00	1.24	15	0.5000	157.666	A	8.627	24.197	12.938	39.42	63.000	0.000
						B	39.346	28.640		19.03	0.000	0.000
						C	8.601	24.605		38.96	0.000	0.000
T4 60.00-40.00	50.00	1.126	14	0.5000	198.919	A	10.992	14.733	0.000	0.00	63.000	0.000
						B	41.901	15.937		0.00	0.000	0.000
						C	10.888	16.381		0.00	0.000	0.000
T5 40.00-20.00	30.00	1	12	0.5000	239.328	A	15.968	15.607	0.000	0.00	63.000	0.000
						B	46.483	16.869		0.00	0.000	0.000
						C	15.845	17.257		0.00	0.000	0.000
T6 20.00-0.00	10.00	1	12	0.5000	281.262	A	18.442	13.774	0.000	0.00	44.100	0.000
						B	39.866	14.683		0.00	0.000	0.000
						C	18.359	14.930		0.00	0.000	0.000

Tower Pressure - Service

$$G_H = 1.159$$

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Section Elevation ft	z ft	K _Z	q _z psf	A _G ft ²	F a c e	A _F ft ²	A _R ft ²	A _{leg} ft ²	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
T1 104.00-92.00	98.00	1.365	13	80.615	A	6.630	6.108	4.750	37.29	0.000	0.000
					B	5.538	18.630		19.65	0.000	0.000
					C	6.630	6.108		37.29	0.000	0.000
T2 92.00-80.00	86.00	1.315	12	80.855	A	5.349	6.380	4.750	40.50	23.401	0.000
					B	3.998	25.570		16.06	0.000	0.000
					C	5.349	6.380		40.50	0.000	0.000
T3 80.00-60.00	70.00	1.24	11	155.998	A	8.752	13.141	9.599	43.85	39.001	0.000
					B	6.880	44.299		18.76	0.000	0.000
					C	8.726	13.566		43.06	0.000	0.000
T4 60.00-40.00	50.00	1.126	10	197.250	A	11.196	6.017	0.000	0.00	39.001	0.000
					B	9.445	34.700		0.00	0.000	0.000
					C	11.092	7.717		0.00	0.000	0.000
T5 40.00-20.00	30.00	1	9	237.659	A	16.208	6.017	0.000	0.00	39.001	0.000
					B	14.138	34.700		0.00	0.000	0.000
					C	16.086	7.717		0.00	0.000	0.000
T6 20.00-0.00	10.00	1	9	279.593	A	18.604	4.212	0.000	0.00	27.301	0.000
					B	17.207	24.290		0.00	0.000	0.000
					C	18.522	5.402		0.00	0.000	0.000

Tower Forces - No Ice - Wind Normal To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
T1 104.00-92.00	0.20	0.29	A	0.158	2.742	0.583	1	1	10.190	1.01	84.39	B
			B	0.3	2.297	0.616	1	1	17.011			
			C	0.158	2.742	0.583	1	1	10.190			
T2 92.00-80.00	0.42	0.30	A	0.145	2.79	0.581	1	1	9.054	1.67	139.00	B
			B	0.366	2.136	0.638	1	1	20.317			
			C	0.145	2.79	0.581	1	1	9.054			
T3 80.00-60.00	0.74	0.51	A	0.14	2.807	0.58	1	1	16.374	2.73	136.41	B
			B	0.328	2.224	0.625	1	1	34.562			
			C	0.143	2.798	0.58	1	1	16.600			
T4 60.00-40.00	0.88	0.51	A	0.087	3.016	0.574	1	1	14.648	2.46	122.80	B
			B	0.224	2.518	0.596	1	1	30.110			
			C	0.095	2.983	0.575	1	1	15.526			
T5 40.00-20.00	0.88	0.74	A	0.094	2.99	0.574	1	1	19.665	2.44	121.90	B
			B	0.205	2.578	0.592	1	1	34.665			
			C	0.1	2.963	0.575	1	1	20.524			
T6 20.00-0.00	0.61	0.84	A	0.082	3.039	0.573	1	1	21.019	2.17	108.55	B
			B	0.148	2.777	0.581	1	1	31.325			
			C	0.086	3.023	0.574	1	1	21.621			
Sum Weight:	3.72	3.19						OTM	651.31 kip-ft	12.47		

Tower Forces - No Ice - Wind 60 To Face

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Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1 104.00-92.00	0.20	0.29	A	0.158	2.742	0.583	0.8	1	8.864	0.95	78.89	B
			B	0.3	2.297	0.616	0.8	1	15.904			
			C	0.158	2.742	0.583	0.8	1	8.864			
T2 92.00-80.00	0.42	0.30	A	0.145	2.79	0.581	0.8	1	7.985	1.63	135.45	B
			B	0.366	2.136	0.638	0.8	1	19.517			
			C	0.145	2.79	0.581	0.8	1	7.985			
T3 80.00-60.00	0.74	0.51	A	0.14	2.807	0.58	0.8	1	14.624	2.66	132.81	B
			B	0.328	2.224	0.625	0.8	1	33.186			
			C	0.143	2.798	0.58	0.8	1	14.855			
T4 60.00-40.00	0.88	0.51	A	0.087	3.016	0.574	0.8	1	12.409	2.35	117.71	B
			B	0.224	2.518	0.596	0.8	1	28.221			
			C	0.095	2.983	0.575	0.8	1	13.308			
T5 40.00-20.00	0.88	0.74	A	0.094	2.99	0.574	0.8	1	16.423	2.30	114.97	B
			B	0.205	2.578	0.592	0.8	1	31.837			
			C	0.1	2.963	0.575	0.8	1	17.306			
T6 20.00-0.00	0.61	0.84	A	0.082	3.039	0.573	0.8	1	17.298	1.99	99.47	B
			B	0.148	2.777	0.581	0.8	1	27.884			
			C	0.086	3.023	0.574	0.8	1	17.916			
Sum Weight:	3.72	3.19						OTM	625.08 kip-ft	11.87		

Tower Forces - No Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1 104.00-92.00	0.20	0.29	A	0.158	2.742	0.583	0.85	1	9.195	0.96	80.27	B
			B	0.3	2.297	0.616	0.85	1	16.181			
			C	0.158	2.742	0.583	0.85	1	9.195			
T2 92.00-80.00	0.42	0.30	A	0.145	2.79	0.581	0.85	1	8.252	1.64	136.34	B
			B	0.366	2.136	0.638	0.85	1	19.717			
			C	0.145	2.79	0.581	0.85	1	8.252			
T3 80.00-60.00	0.74	0.51	A	0.14	2.807	0.58	0.85	1	15.061	2.67	133.71	B
			B	0.328	2.224	0.625	0.85	1	33.530			
			C	0.143	2.798	0.58	0.85	1	15.291			
T4 60.00-40.00	0.88	0.51	A	0.087	3.016	0.574	0.85	1	12.969	2.38	118.98	B
			B	0.224	2.518	0.596	0.85	1	28.693			
			C	0.095	2.983	0.575	0.85	1	13.862			
T5 40.00-20.00	0.88	0.74	A	0.094	2.99	0.574	0.85	1	17.233	2.33	116.70	B
			B	0.205	2.578	0.592	0.85	1	32.544			
			C	0.1	2.963	0.575	0.85	1	18.111			
T6 20.00-0.00	0.61	0.84	A	0.082	3.039	0.573	0.85	1	18.229	2.03	101.74	B
			B	0.148	2.777	0.581	0.85	1	28.744			
			C	0.086	3.023	0.574	0.85	1	18.842			
Sum Weight:	3.72	3.19						OTM	631.63 kip-ft	12.02		

Tower Forces - With Ice - Wind Normal To Face

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Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1 104.00-92.00	0.45	0.61	A	0.24	2.467	0.599	1	1	14.383	1.12	93.13	B
			B	0.405	2.054	0.654	1	1	27.986			
			C	0.24	2.467	0.599	1	1	14.383			
T2 92.00-80.00	0.96	0.56	A	0.219	2.533	0.594	1	1	12.804	1.95	162.34	B
			B	0.481	1.926	0.688	1	1	34.371			
			C	0.219	2.533	0.594	1	1	12.804			
T3 80.00-60.00	1.67	0.97	A	0.208	2.569	0.592	1	1	22.954	3.18	159.02	B
			B	0.431	2.006	0.665	1	1	58.387			
			C	0.211	2.561	0.593	1	1	23.183			
T4 60.00-40.00	1.86	0.92	A	0.129	2.849	0.579	1	1	19.515	2.93	146.71	B
			B	0.291	2.321	0.613	1	1	51.672			
			C	0.137	2.82	0.58	1	1	20.382			
T5 40.00-20.00	1.86	1.30	A	0.132	2.839	0.579	1	1	25.002	2.83	141.56	B
			B	0.265	2.394	0.606	1	1	56.702			
			C	0.138	2.815	0.58	1	1	25.850			
T6 20.00-0.00	1.30	1.47	A	0.115	2.906	0.577	1	1	26.385	2.44	121.82	B
			B	0.194	2.616	0.589	1	1	48.517			
			C	0.118	2.891	0.577	1	1	26.976			
Sum Weight:	8.12	5.83						OTM	755.69 kip-ft	14.45		

Tower Forces - With Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1 104.00-92.00	0.45	0.61	A	0.24	2.467	0.599	0.8	1	13.072	0.97	80.88	B
			B	0.405	2.054	0.654	0.8	1	24.303			
			C	0.24	2.467	0.599	0.8	1	13.072			
T2 92.00-80.00	0.96	0.56	A	0.219	2.533	0.594	0.8	1	11.748	1.78	148.26	B
			B	0.481	1.926	0.688	0.8	1	29.688			
			C	0.219	2.533	0.594	0.8	1	11.748			
T3 80.00-60.00	1.67	0.97	A	0.208	2.569	0.592	0.8	1	21.229	2.90	145.08	B
			B	0.431	2.006	0.665	0.8	1	50.518			
			C	0.211	2.561	0.593	0.8	1	21.462			
T4 60.00-40.00	1.86	0.92	A	0.129	2.849	0.579	0.8	1	17.317	2.62	131.11	B
			B	0.291	2.321	0.613	0.8	1	43.292			
			C	0.137	2.82	0.58	0.8	1	18.205			
T5 40.00-20.00	1.86	1.30	A	0.132	2.839	0.579	0.8	1	21.808	2.51	125.71	B
			B	0.265	2.394	0.606	0.8	1	47.405			
			C	0.138	2.815	0.58	0.8	1	22.681			
T6 20.00-0.00	1.30	1.47	A	0.115	2.906	0.577	0.8	1	22.697	2.14	106.96	B
			B	0.194	2.616	0.589	0.8	1	40.544			
			C	0.118	2.891	0.577	0.8	1	23.304			
Sum Weight:	8.12	5.83						OTM	679.16 kip-ft	12.93		

Tower Forces - With Ice - Wind 90 To Face

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Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1 104.00-92.00	0.45	0.61	A	0.24	2.467	0.599	0.85	1	13.400	1.01	83.94	B
			B	0.405	2.054	0.654	0.85	1	25.224			
			C	0.24	2.467	0.599	0.85	1	13.400			
T2 92.00-80.00	0.96	0.56	A	0.219	2.533	0.594	0.85	1	12.012	1.82	151.78	B
			B	0.481	1.926	0.688	0.85	1	30.859			
			C	0.219	2.533	0.594	0.85	1	12.012			
T3 80.00-60.00	1.67	0.97	A	0.208	2.569	0.592	0.85	1	21.660	2.97	148.57	B
			B	0.431	2.006	0.665	0.85	1	52.485			
			C	0.211	2.561	0.593	0.85	1	21.893			
T4 60.00-40.00	1.86	0.92	A	0.129	2.849	0.579	0.85	1	17.867	2.70	135.01	B
			B	0.291	2.321	0.613	0.85	1	45.387			
			C	0.137	2.82	0.58	0.85	1	18.749			
T5 40.00-20.00	1.86	1.30	A	0.132	2.839	0.579	0.85	1	22.607	2.59	129.67	B
			B	0.265	2.394	0.606	0.85	1	49.729			
			C	0.138	2.815	0.58	0.85	1	23.473			
T6 20.00-0.00	1.30	1.47	A	0.115	2.906	0.577	0.85	1	23.619	2.21	110.68	B
			B	0.194	2.616	0.589	0.85	1	42.537			
			C	0.118	2.891	0.577	0.85	1	24.222			
Sum Weight:	8.12	5.83						OTM	698.29 kip-ft	13.31		

Tower Forces - Service - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
T1 104.00-92.00	0.20	0.29	A	0.158	2.742	0.583	1	1	10.190	0.57	47.47	B
			B	0.3	2.297	0.616	1	1	17.011			
			C	0.158	2.742	0.583	1	1	10.190			
T2 92.00-80.00	0.42	0.30	A	0.145	2.79	0.581	1	1	9.054	0.94	78.19	B
			B	0.366	2.136	0.638	1	1	20.317			
			C	0.145	2.79	0.581	1	1	9.054			
T3 80.00-60.00	0.74	0.51	A	0.14	2.807	0.58	1	1	16.374	1.53	76.73	B
			B	0.328	2.224	0.625	1	1	34.562			
			C	0.143	2.798	0.58	1	1	16.600			
T4 60.00-40.00	0.88	0.51	A	0.087	3.016	0.574	1	1	14.648	1.38	69.07	B
			B	0.224	2.518	0.596	1	1	30.110			
			C	0.095	2.983	0.575	1	1	15.526			
T5 40.00-20.00	0.88	0.74	A	0.094	2.99	0.574	1	1	19.665	1.37	68.57	B
			B	0.205	2.578	0.592	1	1	34.665			
			C	0.1	2.963	0.575	1	1	20.524			
T6 20.00-0.00	0.61	0.84	A	0.082	3.039	0.573	1	1	21.019	1.22	61.06	B
			B	0.148	2.777	0.581	1	1	31.325			
			C	0.086	3.023	0.574	1	1	21.621			
Sum Weight:	3.72	3.19						OTM	366.36 kip-ft	7.02		

Tower Forces - Service - Wind 60 To Face

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Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K	e						ft ²	K	plf	
T1 104.00-92.00	0.20	0.29	A	0.158	2.742	0.583	0.8	1	8.864	0.53	44.38	B
			B	0.3	2.297	0.616	0.8	1	15.904			
			C	0.158	2.742	0.583	0.8	1	8.864			
T2 92.00-80.00	0.42	0.30	A	0.145	2.79	0.581	0.8	1	7.985	0.91	76.19	B
			B	0.366	2.136	0.638	0.8	1	19.517			
			C	0.145	2.79	0.581	0.8	1	7.985			
T3 80.00-60.00	0.74	0.51	A	0.14	2.807	0.58	0.8	1	14.624	1.49	74.70	B
			B	0.328	2.224	0.625	0.8	1	33.186			
			C	0.143	2.798	0.58	0.8	1	14.855			
T4 60.00-40.00	0.88	0.51	A	0.087	3.016	0.574	0.8	1	12.409	1.32	66.21	B
			B	0.224	2.518	0.596	0.8	1	28.221			
			C	0.095	2.983	0.575	0.8	1	13.308			
T5 40.00-20.00	0.88	0.74	A	0.094	2.99	0.574	0.8	1	16.423	1.29	64.67	B
			B	0.205	2.578	0.592	0.8	1	31.837			
			C	0.1	2.963	0.575	0.8	1	17.306			
T6 20.00-0.00	0.61	0.84	A	0.082	3.039	0.573	0.8	1	17.298	1.12	55.95	B
			B	0.148	2.777	0.581	0.8	1	27.884			
			C	0.086	3.023	0.574	0.8	1	17.916			
Sum Weight:	3.72	3.19						OTM	351.61 kip-ft	6.68		

Tower Forces - Service - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K	e						ft ²	K	plf	
T1 104.00-92.00	0.20	0.29	A	0.158	2.742	0.583	0.85	1	9.195	0.54	45.15	B
			B	0.3	2.297	0.616	0.85	1	16.181			
			C	0.158	2.742	0.583	0.85	1	9.195			
T2 92.00-80.00	0.42	0.30	A	0.145	2.79	0.581	0.85	1	8.252	0.92	76.69	B
			B	0.366	2.136	0.638	0.85	1	19.717			
			C	0.145	2.79	0.581	0.85	1	8.252			
T3 80.00-60.00	0.74	0.51	A	0.14	2.807	0.58	0.85	1	15.061	1.50	75.21	B
			B	0.328	2.224	0.625	0.85	1	33.530			
			C	0.143	2.798	0.58	0.85	1	15.291			
T4 60.00-40.00	0.88	0.51	A	0.087	3.016	0.574	0.85	1	12.969	1.34	66.93	B
			B	0.224	2.518	0.596	0.85	1	28.693			
			C	0.095	2.983	0.575	0.85	1	13.862			
T5 40.00-20.00	0.88	0.74	A	0.094	2.99	0.574	0.85	1	17.233	1.31	65.65	B
			B	0.205	2.578	0.592	0.85	1	32.544			
			C	0.1	2.963	0.575	0.85	1	18.111			
T6 20.00-0.00	0.61	0.84	A	0.082	3.039	0.573	0.85	1	18.229	1.14	57.23	B
			B	0.148	2.777	0.581	0.85	1	28.744			
			C	0.086	3.023	0.574	0.85	1	18.842			
Sum Weight:	3.72	3.19						OTM	355.29 kip-ft	6.76		

Force Totals

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Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, M_x kip-ft	Sum of Overturning Moments, M_z kip-ft	Sum of Torques kip-ft
Leg Weight	0.00					
Bracing Weight	3.19					
Total Member Self-Weight	3.19			-6.85	0.96	
Total Weight	12.09			-6.85	0.96	
Wind 0 deg - No Ice		0.06	-20.81	-1407.90	-4.27	1.20
Wind 90 deg - No Ice		19.88	-0.06	-12.08	-1330.37	-16.06
Wind 180 deg - No Ice		-0.06	20.21	1367.97	6.19	-1.19
Member Ice	2.64					
Total Weight Ice	21.67			-22.21	-0.03	
Wind 0 deg - Ice		0.04	-21.30	-1392.02	-4.10	1.38
Wind 90 deg - Ice		19.79	-0.04	-26.28	-1273.86	-15.64
Wind 180 deg - Ice		-0.04	19.78	1271.06	4.04	-1.39
Total Weight	12.09			-6.85	0.96	
Wind 0 deg - Service		0.03	-11.71	-785.29	-2.78	0.67
Wind 90 deg - Service		11.18	-0.03	-0.14	-748.72	-9.04
Wind 180 deg - Service		-0.03	11.37	776.14	3.10	-0.67

Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 90 deg - No Ice
4	Dead+Wind 180 deg - No Ice
5	Dead+Ice+Temp
6	Dead+Wind 0 deg+Ice+Temp
7	Dead+Wind 90 deg+Ice+Temp
8	Dead+Wind 180 deg+Ice+Temp
9	Dead+Wind 0 deg - Service
10	Dead+Wind 90 deg - Service
11	Dead+Wind 180 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	104 - 92	Leg	Max Tension	4	4.72	-0.01	0.24
			Max. Compression	6	-7.01	-0.01	0.43
			Max. Mx	3	-0.95	1.23	0.00
			Max. My	2	-0.56	-0.01	-1.01
			Max. Vy	3	0.93	-0.60	0.00
			Max. Vx	2	-0.77	-0.01	0.51
		Diagonal	Max Tension	4	1.73	0.00	0.00
			Max. Compression	2	-1.71	0.00	0.00
			Max. Mx	6	1.27	0.01	-0.00
			Max. My	3	-1.40	0.00	0.00
			Max. Vy	6	-0.01	0.01	-0.00
			Max. Vx	3	0.00	0.00	0.00
		Top Girt	Max Tension	4	0.17	0.00	0.00
			Max. Compression	2	-0.16	0.00	0.00
			Max. Mx	5	0.01	-0.02	0.00

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T2	92 - 80	Leg	Max. My	7	0.00	0.00	0.00
			Max. Vy	5	0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	4	16.51	-0.57	-0.01
			Max. Compression	2	-19.59	-0.15	0.00
			Max. Mx	2	-11.39	-0.94	0.00
			Max. My	3	-1.45	0.00	-0.83
			Max. Vy	4	0.76	-0.57	-0.01
		Diagonal	Max. Vx	3	-0.69	0.00	0.53
			Max Tension	4	3.97	0.00	0.00
			Max. Compression	2	-4.11	0.00	0.00
			Max. Mx	6	3.08	0.02	0.00
			Max. My	3	-1.56	0.00	0.00
			Max. Vy	6	-0.01	0.02	0.00
			Max. Vx	3	0.00	0.00	0.00
			Max Tension	2	0.24	0.00	0.00
T3	80 - 60	Leg	Max. Compression	4	-0.29	0.00	0.00
			Max. Mx	5	-0.06	-0.02	0.00
			Max. My	7	-0.06	0.00	0.00
			Max. Vy	5	0.01	0.00	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	4	42.48	-0.09	-0.01
			Max. Compression	2	-47.70	0.05	-0.00
			Max. Mx	8	19.14	-0.18	0.07
		Diagonal	Max. My	8	-17.12	0.05	-0.18
			Max. Vy	4	-0.47	-0.09	-0.01
			Max. Vx	3	0.44	0.00	0.02
			Max Tension	4	3.39	0.00	0.00
			Max. Compression	2	-3.45	0.00	0.00
			Max. Mx	8	2.90	0.02	0.00
			Max. My	7	-2.00	0.01	0.00
			Max. Vy	8	0.01	0.02	0.00
T4	60 - 40	Leg	Max. Vx	7	-0.00	0.00	0.00
			Max Tension	4	64.59	-0.06	-0.00
			Max. Compression	2	-71.71	0.12	-0.00
			Max. Mx	8	55.21	-0.22	-0.00
			Max. My	3	-2.97	-0.01	0.15
			Max. Vy	8	0.04	-0.22	-0.00
			Max. Vx	3	0.05	-0.01	0.15
			Max Tension	6	3.61	0.00	0.00
		Diagonal	Max. Compression	6	-3.74	0.00	0.00
			Max. Mx	8	3.12	0.03	0.00
			Max. My	7	-2.36	0.02	0.01
			Max. Vy	8	0.02	0.03	0.00
			Max. Vx	7	-0.00	0.00	0.00
			Max Tension	4	84.19	-0.10	-0.00
			Max. Compression	6	-93.26	0.42	0.00
			Max. Mx	6	-93.26	0.42	0.00
T5	40 - 20	Leg	Max. My	3	-4.32	-0.01	0.17
			Max. Vy	8	-0.09	-0.32	-0.00
			Max. Vx	3	-0.04	-0.01	0.17
			Max Tension	6	3.74	0.00	0.00
			Max. Compression	6	-4.05	0.00	0.00
			Max. Mx	6	3.12	0.06	-0.00
			Max. My	7	-2.81	0.04	0.01
			Max. Vy	8	0.03	0.06	0.00
		Diagonal	Max. Vx	7	-0.00	0.00	0.00
			Max Tension	4	101.43	-0.11	-0.00
			Max. Compression	6	-114.75	0.00	0.00
			Max. Mx	8	86.05	-0.71	-0.00
			Max. My	3	-4.88	-0.01	0.30

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
			Max. Vy	8	-0.18	-0.71	-0.00
			Max. Vx	3	-0.07	-0.01	0.30
		Diagonal	Max Tension	8	4.40	0.00	0.00
			Max. Compression	6	-4.31	0.00	0.00
			Max. Mx	8	2.64	0.07	0.00
			Max. My	7	-3.30	0.06	0.01
			Max. Vy	8	0.03	0.07	0.00
			Max. Vx	7	-0.00	0.00	0.00

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg C	Max. Vert	4	58.50	4.76	-4.27
	Max. H _x	4	58.50	4.76	-4.27
	Max. H _z	7	-81.09	-9.44	5.23
	Min. Vert	3	-87.48	-8.60	4.80
	Min. H _x	7	-81.09	-9.44	5.23
	Min. H _z	4	58.50	4.76	-4.27
Leg B	Max. Vert	3	94.58	-9.07	-5.05
	Max. H _x	6	-47.56	5.27	4.77
	Max. H _z	6	-47.56	5.27	4.77
	Min. Vert	2	-51.30	4.31	4.17
	Min. H _x	3	94.58	-9.07	-5.05
	Min. H _z	3	94.58	-9.07	-5.05
Leg A	Max. Vert	6	117.35	0.05	11.83
	Max. H _x	6	117.35	0.05	11.83
	Max. H _z	2	115.27	0.04	12.53
	Min. Vert	4	-104.06	-0.05	-11.62
	Min. H _x	3	4.99	-2.20	0.31
	Min. H _z	8	-93.34	-0.05	-12.32

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _y K	Overturning Moment, M _x kip-ft	Overturning Moment, M _y kip-ft	Torque kip-ft
Dead Only	12.09	-0.00	-0.00	-6.85	0.96	0.00
Dead+Wind 0 deg - No Ice	12.09	0.06	-20.81	-1411.34	-4.28	1.19
Dead+Wind 90 deg - No Ice	12.09	19.88	-0.06	-12.15	-1333.60	-16.08
Dead+Wind 180 deg - No Ice	12.09	-0.06	20.21	1371.33	6.20	-1.19
Dead+Ice+Temp	21.67	0.00	0.00	-22.28	-0.03	-0.00
Dead+Wind 0 deg+Ice+Temp	21.67	0.04	-21.30	-1397.19	-4.13	1.38
Dead+Wind 90 deg+Ice+Temp	21.67	19.79	-0.04	-26.42	-1278.60	-15.72
Dead+Wind 180 deg+Ice+Temp	21.67	-0.04	19.78	1275.84	4.06	-1.39
Dead+Wind 0 deg - Service	12.09	0.03	-11.71	-796.89	-1.99	0.67
Dead+Wind 90 deg - Service	12.09	11.18	-0.03	-9.83	-749.73	-9.05
Dead+Wind 180 deg - Service	12.09	-0.03	11.37	768.38	3.91	-0.67

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Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-12.09	0.00	0.00	12.09	0.00	0.000%
2	0.06	-12.09	-20.81	-0.06	12.09	20.81	0.001%
3	19.88	-12.09	-0.06	-19.88	12.09	0.06	0.002%
4	-0.06	-12.09	20.21	0.06	12.09	-20.21	0.002%
5	0.00	-21.67	0.00	-0.00	21.67	-0.00	0.000%
6	0.04	-21.67	-21.30	-0.04	21.67	21.30	0.002%
7	19.79	-21.67	-0.04	-19.79	21.67	0.04	0.002%
8	-0.04	-21.67	19.78	0.04	21.67	-19.78	0.002%
9	0.03	-12.09	-11.71	-0.03	12.09	11.71	0.001%
10	11.18	-12.09	-0.03	-11.18	12.09	0.03	0.001%
11	-0.03	-12.09	11.37	0.03	12.09	-11.37	0.001%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	6	0.00000001	0.00000001
2	Yes	8	0.00000001	0.00007668
3	Yes	8	0.00000001	0.00008540
4	Yes	8	0.00000001	0.00009461
5	Yes	7	0.00000001	0.00006683
6	Yes	8	0.00000001	0.00013258
7	Yes	8	0.00000001	0.00014119
8	Yes	8	0.00000001	0.00014928
9	Yes	8	0.00000001	0.00008034
10	Yes	8	0.00000001	0.00008506
11	Yes	8	0.00000001	0.00009064

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	104 - 92	4.090	9	0.3500	0.0850
T2	92 - 80	3.204	9	0.3389	0.0691
T3	80 - 60	2.372	9	0.2930	0.0552
T4	60 - 40	1.296	9	0.1898	0.0370
T5	40 - 20	0.585	9	0.1227	0.0222
T6	20 - 0	0.165	9	0.0553	0.0110

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
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Elevation	Appurtenance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
ft			in	°	°	ft
102.00	T-Frame Sector	9	3.942	0.3494	0.0823	103515
88.00	T-Frame Sector	9	2.916	0.3274	0.0642	21397
79.00	DB806-XT	9	2.308	0.2879	0.0542	10869
73.00	PR-950	9	1.948	0.2561	0.0483	11261
65.00	(2) AIR 21	9	1.528	0.2135	0.0411	12377

Maximum Tower Deflections - Design Wind

Section No.	Elevation	Horz. Deflection	Gov. Load Comb.	Tilt	Twist
	ft	in		°	°
T1	104 - 92	7.255	2	0.6203	0.1512
T2	92 - 80	5.683	2	0.6017	0.1229
T3	80 - 60	4.205	2	0.5200	0.0982
T4	60 - 40	2.298	2	0.3363	0.0657
T5	40 - 20	1.037	2	0.2172	0.0395
T6	20 - 0	0.293	6	0.0980	0.0195

Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
ft			in	°	°	ft
102.00	T-Frame Sector	2	6.991	0.6195	0.1464	61298
88.00	T-Frame Sector	2	5.172	0.5813	0.1141	12173
79.00	DB806-XT	2	4.091	0.5110	0.0963	6156
73.00	PR-950	2	3.453	0.4543	0.0858	6364
65.00	(2) AIR 21	2	2.707	0.3784	0.0731	6963

Bolt Design Data

Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of Bolts	Maximum Load per Bolt	Allowable Load	Ratio Load Allowable	Allowable Ratio	Criteria	
	ft			in		K	K				
T1	104	Leg	A325N	0.6250	4	0.06	13.49	0.004	✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	1	1.73	4.08	0.425	✓	1.333	Member Bearing
		Top Girt	A325N	0.5000	1	0.17	4.08	0.042	✓	1.333	Member Bearing
T2	92	Leg	A325N	0.6250	4	2.15	13.49	0.160	✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	1	4.11	4.12	0.997	✓	1.333	Bolt Shear
		Top Girt	A325N	0.5000	1	0.29	4.12	0.069	✓	1.333	Bolt Shear
T3	80	Leg	A325N	0.6250	4	6.15	13.50	0.456	✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	1	3.45	4.12	0.837	✓	1.333	Bolt Shear
T4	60	Leg	A325N	0.6250	4	12.40	13.50	0.918	✓	1.333	Bolt Tension

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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T5	40	Diagonal	A325N	0.5000	1	3.74	4.12	0.907 ✓	1.333	Bolt Shear
		Leg	A325N	0.6250	4	17.86	13.50	1.323 ✓	1.333	Bolt Tension
T6	20	Diagonal	A325N	0.5000	1	4.05	4.12	0.981 ✓	1.333	Bolt Shear
		Leg	A325N	0.7500	4	22.54	19.44	1.159 ✓	1.333	Bolt Tension
		Diagonal	A325N	0.5000	1	4.40	4.08	1.079 ✓	1.333	Member Bearing

Compression Checks

Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	104 - 92	P2x.154	12.00	4.00	61.0 K=1.00	22.549	1.0745	-7.01	24.23	0.289 ✓
T2	92 - 80	P2x.154	12.00	6.00	91.5 K=1.00	16.619	1.0745	-19.59	17.86	1.097 ✓
T3	80 - 60	P2.5x.203	20.03	5.01	63.4 K=1.00	22.122	1.7040	-47.70	37.70	1.265 ✓
T4	60 - 40	Pipe 2.875 x .203 w/ 3/8 Plate	20.03	6.68	88.5 K=1.00	17.258	3.5180	-71.71	60.71	1.181 ✓
T5	40 - 20	Pipe 2.875 x 0.276 w/ 3/8 Plate	20.03	6.68	87.0 K=1.00	17.572	4.0080	-93.26	70.43	1.324 ✓
T6	20 - 0	Pipe 3 x 0.3 w/ 3/8 Plate	20.03	6.68	71.4 K=1.00	20.671	5.0840	-114.75	105.09	1.092 ✓

Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	104 - 92	L1 1/2x1 1/2x3/16	7.65	3.60	147.4 K=1.00	6.869	0.5273	-1.71	3.62	0.471 ✓
T2	92 - 80	L1 1/2x1 1/2x1/4	8.88	4.21	173.0 K=1.00	4.990	0.6875	-4.11	3.43	1.198 ✓
T3	80 - 60	L1 1/2x1 1/2x1/4	9.70	4.75	195.3 K=1.00	3.917	0.6875	-3.45	2.69	1.282 ✓
T4	60 - 40	L2x2x3/16	12.21	6.04	183.9 K=1.00	4.416	0.7150	-3.56	3.16	1.128 ✓
T5	40 - 20	L2 1/2x2 1/2x3/16	13.96	6.91	167.6 K=1.00	5.316	0.9020	-4.05	4.80	0.844 ✓
T6	20 - 0	L2 1/2x2 1/2x3/16	15.17	7.49	181.6	4.526	0.9020	-4.31	4.08	1.057 ✓

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Section No.	Elevation	Size	L	L _u	Kl/r	F _a	A	Actual P	Allow. P _a	Ratio P
	ft		ft	ft		ksi	in ²	K	K	$\frac{P}{P_n}$
K=1.00										✓

Top Girt Design Data (Compression)

Section No.	Elevation	Size	L	L _u	Kl/r	F _a	A	Actual P	Allow. P _a	Ratio P
	ft		ft	ft		ksi	in ²	K	K	$\frac{P}{P_n}$
T1	104 - 92	L2x2x3/16	6.52	6.11	186.2	4.307	0.7150	-0.16	3.08	0.053
					K=1.00					✓
T2	92 - 80	L2x2x3/16	6.52	6.11	186.2	4.307	0.7150	-0.29	3.08	0.093
					K=1.00					✓

Tension Checks

Leg Design Data (Tension)

Section No.	Elevation	Size	L	L _u	Kl/r	F _a	A	Actual P	Allow. P _a	Ratio P
	ft		ft	ft		ksi	in ²	K	K	$\frac{P}{P_n}$
T1	104 - 92	P2x.154	12.00	4.00	61.0	30.000	1.0745	4.72	32.24	0.146
T2	92 - 80	P2x.154	12.00	6.00	91.5	30.000	1.0745	16.51	32.24	0.512
T3	80 - 60	P2.5x.203	20.03	5.01	63.4	30.000	1.7040	42.22	51.12	0.826
T4	60 - 40	Pipe 2.875 x .203 w/ 3/8 Plate	20.03	6.68	88.5	30.000	3.5180	64.59	105.54	0.612
T5	40 - 20	Pipe 2.875 x 0.276 w/ 3/8 Plate	20.03	6.68	87.0	30.000	4.0080	84.19	120.24	0.700
T6	20 - 0	Pipe 3 x 0.3 w/ 3/8 Plate	20.03	6.68	71.4	30.000	5.0840	101.43	152.52	0.665

Diagonal Design Data (Tension)

Section No.	Elevation	Size	L	L _u	Kl/r	F _a	A	Actual P	Allow. P _a	Ratio P
	ft		ft	ft		ksi	in ²	K	K	$\frac{P}{P_n}$
T1	104 - 92	L1 1/2x1 1/2x3/16	7.65	3.60	97.4	29.000	0.3076	1.73	8.92	0.194
T2	92 - 80	L1 1/2x1 1/2x1/4	8.88	4.21	115.3	29.000	0.3984	3.97	11.55	0.343
T3	80 - 60	L1 1/2x1 1/2x1/4	9.70	4.75	129.8	29.000	0.3984	3.39	11.55	0.293

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T4	60 - 40	L2x2x3/16	12.21	6.04	119.4	29,000	0.4484	3.61	13.00	0.277 ✓
T5	40 - 20	L2 1/2x2 1/2x3/16	13.36	6.62	103.7	29,000	0.5886	3.74	17.07	0.219 ✓
T6	20 - 0	L2 1/2x2 1/2x3/16	15.79	7.80	121.9	29,000	0.5886	4.40	17.07	0.258 ✓

Top Girt Design Data (Tension)

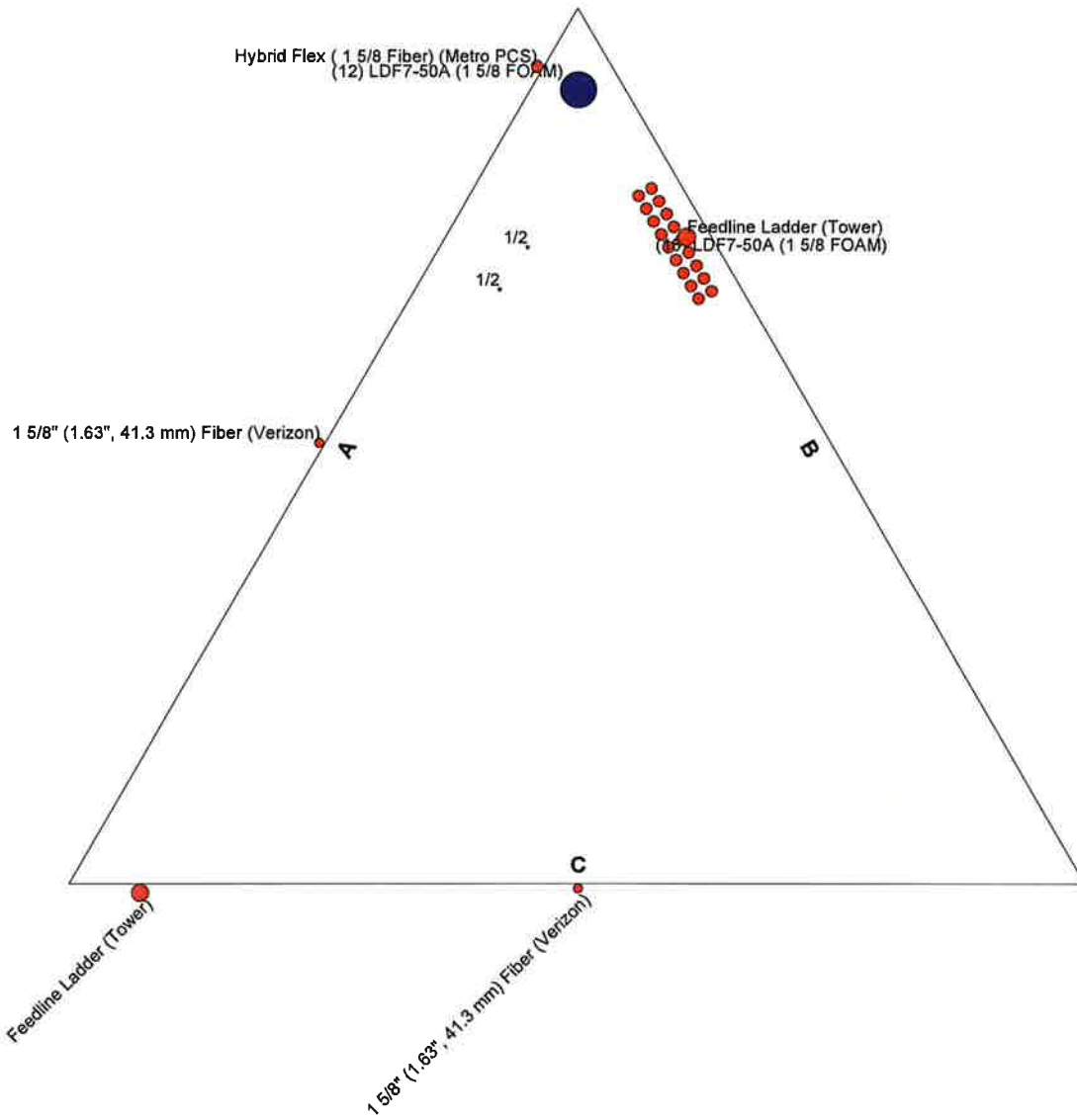
Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
T1	104 - 92	L2x2x3/16	6.52	6.11	123.0	29,000	0.4484	0.17	13.00	0.013 ✓
T2	92 - 80	L2x2x3/16	6.52	6.11	123.0	29,000	0.4484	0.24	13.00	0.019 ✓

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
T1	104 - 92	Leg	P2x.154	3	-7.01	32.30	21.7	Pass
T2	92 - 80	Leg	P2x.154	27	-19.59	23.80	82.3	Pass
T3	80 - 60	Leg	P2.5x.203	45	-47.70	50.25	94.9	Pass
T4	60 - 40	Leg	Pipe 2.875 x .203 w/ 3/8 Plate	72	-71.71	80.93	88.6	Pass
T5	40 - 20	Leg	Pipe 2.875 x 0.276 w/ 3/8 Plate	93	-93.26	93.88	99.3	Pass
T6	20 - 0	Leg	Pipe 3 x 0.3 w/ 3/8 Plate	114	-114.75	140.09	81.9	Pass
							87.0 (b)	
T1	104 - 92	Diagonal	L1 1/2x1 1/2x3/16	17	-1.71	4.83	35.3	Pass
T2	92 - 80	Diagonal	L1 1/2x1 1/2x1/4	35	-4.11	4.57	89.9	Pass
T3	80 - 60	Diagonal	L1 1/2x1 1/2x1/4	49	-3.45	3.59	96.2	Pass
T4	60 - 40	Diagonal	L2x2x3/16	76	-3.56	4.21	84.6	Pass
T5	40 - 20	Diagonal	L2 1/2x2 1/2x3/16	97	-4.05	6.39	63.3	Pass
							73.6 (b)	
T6	20 - 0	Diagonal	L2 1/2x2 1/2x3/16	124	-4.31	5.44	79.3	Pass
							80.9 (b)	
T1	104 - 92	Top Girt	L2x2x3/16	4	-0.16	4.10	4.0	Pass
T2	92 - 80	Top Girt	L2x2x3/16	28	-0.29	4.10	6.9	Pass
						Summary	ELC:	Existing + Proposed
						Leg (T5)	99.3	Pass
						Diagonal (T3)	96.2	Pass
						Top Girt (T2)	6.9	Pass
						Bolt Checks Rating =	99.2	Pass
							99.3	Pass

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Feed Line Plan



Bennett & Pless		Job: CT901 Glastonbury	
3395 Northeast Expressway NE		Project: SST Analysis	
Atlanta, GA 30341		Client: Insite Towers, LLC	Drawn by: J. Turner
Phone: 678-990-8700		Code: TIA/EIA-222-F	Date: 08/28/14
FAX: 678-990-8701		Path:	Scale: NTS
			Dwg No. E-7