#### EM-CING-052-110210





New Cingular Wireless PCS, LLC

500 Enterprise Drive

Rocky Hill, Connecticut 06067-3900

Phone: (860) 463-5511 Fax: (860) 513-7190

**Douglas L. Culp** Real Estate Consultant

#### HAND DELIVERED

February 10, 2011



Honorable Daniel F. Caruso, Chairman, and Members of the Connecticut Siting Council Connecticut Siting Council 10 Franklin Square New Britain, Connecticut 06051



CONNECTICUT SITING COUNCIL

Re: New Cingular Wireless PCS, LLC notice of intent to modify an existing telecommunications facility located at 82 Lovely Street Farmington, CT (owner Verizon Wireless)

Dear Chairman Caruso and Members of the Council:

In order to accommodate technological changes, implement Uniform Mobile Telecommunications System ("UMTS") capability, and enhance system performance in the State of Connecticut, New Cingular Wireless PCS, LLC ("AT&T") plans to modify the equipment configurations at many of its existing cell sites. Please accept this letter and attachments as notification, pursuant to R.C.S.A. Section 16-50j-73, of construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2). In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter and attachments is being sent to the chief elected official of the municipality in which the affected cell site is located.

UMTS technology offers services to mobile computer and phone users anywhere in the world. Based on the Global System for Mobile (GSM) communication standard, UMTS is the planned worldwide standard for mobile users. UMTS, fully implemented, gives computer and phone users high-speed access to the Internet as they travel. They have the same capabilities even when they roam, through both terrestrial wireless and satellite transmissions.

Attached is a summary of the planned modifications, including power density calculations reflecting the change in AT&T's operations at the site. Also included is documentation of the structural sufficiency of the tower to accommodate the revised antenna configuration.

The changes to the facility do not constitute modifications as defined in Connecticut General Statutes ("C.G.S.") Section 16-50i(d) because the general physical characteristics of the facility will not be significantly changed or altered. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in R.C.S.A. Section 16-50j-72(b)(2).

- 1. The height of the overall structure will be unaffected.
- 2. The proposed changes will not extend the site boundaries. There will be no effect on the site compound other than some enlarged equipment pads as may be noted in the attachments.
- 3. The proposed changes will not increase the noise level at the existing facility by six decibels or more.
- 4. Radio frequency power density may increase due to use of one or more GSM channel for UMTS transmissions. However, the changes will not increase the calculated "worst case" power density for the combined operations at the site to a level at or above the applicable standard for uncontrolled environments as calculated for a mixed frequency site.

For the foregoing reasons, New Cingular Wireless respectfully submits that the proposed changes at the referenced site constitute exempt modifications under R.C.S.A. Section 16-50j-72(b)(2).

Please feel free to call me at (860) 463-5511 with questions concerning this matter. Thank you for your consideration.

Sincerely,

Douglas L. Culp

Real Estate Consultant

Attachments

## NEW CINGULAR WIRELESS PCS, LLC Equipment Modification

82 Lovely Street Farmington, CT

Site Number 1061

Exempt Mods - 5/94, 6/00 and 9/02

Tower Owner/Manager: AT&T Mobility

**Equipment configuration:** Monopole

Current and/or approved:

Three EMS panel antennas @ 102 ft

Six TMA's @ 100 ft Nine runs (9) 1 1/4" Coax

**Equipment Shelter** 

**Planned Modifications:** Remove existing antennas and TMA's

Install six Powerwave P65-15XLH antennas (or equiv.) @ 102 ft Install six Powerwave TMA's (TT19-08BP111-001) @ 100 ft

#### **Power Density:**

Calculations for current operations at the site indicate a cumulative radio frequency electromagnetic radiation power density, measured at the tower base, of approximately 25.8 % of the standard adopted by the FCC. As depicted in the second table below, the total radio frequency electromagnetic radiation power density following AT&T's planned modifications would be approximately 39.7 % of the standard.

#### **Existing**

Total							25.8%
AT&T GSM *	102	880 - 894	2	296	0.0205	0.5867	3.49
AT&T GSM *	102	1900 Band	2	427	0.0295	1.0000	2.95
AT&T TDMA *	102	880 - 894	16	100	0.0553	0.5867	9.43
Other Users *							9.94

<sup>\*</sup> Per CSC records.

#### Proposed

Company	Centerline Ht (feet)	Frequency (MHz)	Number of Channels	Power Per Channel (Watts)	Power Density (mW/cm²)	Standard Limits (mW/cm²)	Percent of Limit
Other Users *							25.08
AT&T GSM	102	880 - 894	4	296	0.0409	0.5867	6.97
AT&T GSM	102	1900 Band	2	427	0.0295	1.0000	2.95
AT&T UMTS	102	1900 Band	1	500	0.0173	1.0000	1.73
AT&T UMTS	102	880 - 894	1	500	0.0173	0.5867	2.95
Total							39.7%

<sup>\*</sup> Per CSC records.

#### Structural information:

The attached structural analysis (GPD Associates, 2/11) demonstrates that the tower and foundation have sufficient structural capacity to accommodate the proposed equipment modifications.

## **GN-1 GENERAL NOTES** Ξ LONGITUDE: JURISDICTION: CURRENT USE: PROPOSED USE: NOC# SITE ADDRESS: LATITUDE: SCOPE OF WORK: GROUNDING, ONE-LINE DIAGRAM & DETAILS ANTENNA LAYOUT AND ELEVATION COMPOUND & EQUIPMENT PLAN TITLE SHEET 41.761389 N 41" 45" 41" N -72.867328 W -72" 53" 15.1" W NATIONAL, STATE & LOCAL CODES OR ORDINANCES TELECOMMUNICATIONS FACILITY TELECOMMUNICATIONS FACILITY 866-915-5600 UNMANNED TELECOMMUNICATIONS FACILITY MODIFICATIONS DRAWING INDEX PROJECT INFORMATION REV FROM MOCKY HILL: MERGE ONTO 1-91 S VA THE RAMP ON THE LETT TOWARD NEW HAVEN, 4.1 MILES. MERGE ONTO CT-9 N VA EXIT 22N TOWARD NEW BRITAIN, 11.0 MILES. MERGE ONTO CT-4 W / US-6 W VA EXIT 32 ON THE LET TOWARD WATERBURY, 1.2 MILES. MERGE ONTO CT-4 W / FARMINGTON AME VIA EXIT 39 TOWARD FARMINGTON. 5.8 MILES. TURN RIGHT ONTO LOVELY ST / CT-177. 0.2 MILES. END AT 82 LOVELY ST. VICINITY MAP RIMAN ST. SYLVAN AVE. SITE NAME: UNIONVILLE SITE NUMBER: CT1061 at&t CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ATALT WIRELESS REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME. THE FACILITY IS AN UNMANNED PRIVATE AND SECURED EQUIPMENT INSTALLATION. IT IS ONLY ACCESSED BY TAKED TECHNICHAIS FOR PERIODIC ROUTINE MAINTENANCE. AND THEREFORE DOES NOT REQUIRE MY MATER OR SANIFAK SEVER SERVICE. THE FACILITY IS NOT GOVERNED BY REGULATIONS REQUIRING PUBLIC ACCESS PER ADA REQUIREMENTS. THIS DOCUMENT IS THE CREATION, DESIGN, PROPERTY AND COPYRIGHTED WORK OF AT&T WRELESS, ANY DUPLICATION OR USE WITHOUT EXPRESS OWN THEN CONSIGN IS STRONGY. CONDUCTION THEM FAMOLIES OF COPYRIGHT RESIDES FOR THE PURPOSES OF SECURIOLITY ALLOWED. **GENERAL NOTES**

BUILDING 20 NORTH, SUITE 2-101 N. ANDOVER, MA 01845

TEL: [978] 557-5553 FAX: [978] 336-5586 Hudson Design Groupus

Communications

SITE NUMBER: CT1061
SITE NAME: UNIONVILLE
82 LOVELY STREET
FARMINGTON, CT 06085
HARTFORD COUNTY

500 ENTERPRISE DRIVE, SUITE 3A ROCKY HILL, CT 06067

| 1 01/28/11 | CONSTRUCTION FINAL | 0 12/03/10 | ISSUED FOR CONSTRUCTION | NO. DATE | REVISIONS | REVI

BY CHK APP'

1061.01

AT&T
TITLE SHEET
(2ND CARRIER)

at&t

ARMINGTON RIVER

CALL TOLL FREE 800-922-4455

BEFORE YOU DIG

CALL

UNDERGROUND SERVICE ALERT

# **GROUNDING NOTES**

- EXISTING FACILITY GROUNDING SYSTEM AND LIGHTINING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRECT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ), THE SITE—SPECIFIC (LIL, LIP, OR NETPA) LIGHTING FROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND ITA GROUNDING STANDARDS. THE SUBCONTRACTOR SHALL REPORT MAY MOLITIONS OR ADVERSE FINDINGS TO THE CONTRACTOR FOR RESOLUTION. THE SUBCONTRACTOR SHALL REVIEW AND INSPECT THE
- THE SUBCOMPACTOR SHALL PERFORM IEEE
  FALL-OF-POIDTHAL RESISTANCE TO EARTH TESTING (PER IEEE
  1100 AND 81) FOR REW GROUND ELECTRODE SYSTEMS, THE
  SUBCOMPACTOR SHALL FURNISH AND INSTALL
  SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO
  ACHIDIE A TEST RESULT OF 5 OHMS OR LESS. . ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, AND LIGHTINKS PROTECTION, AND AC POWERS CESS) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
- METAL RACEMAY SHALL NOT BE USED AS THE NEC RECOURED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN NISJUATION, SZED IN ACCORDANCE WITH THE NEC, SHALL BE FUNNISHED AND NISTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
- EACH BTS CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, 6 AWG STRANDED COPPER OR LARGER FOR INDOOR BTS 2 AWG STRANDED COPPER FOR OUTDOOR BTS.
- EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
- APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE OR PASTE) SHALL BE USED ON ALL COMPRESSION BOLTED GROUND CONNECTIONS. AND
- ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR.
- ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS
- MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
- 11. METAL COMDUIT SHALL BE MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ATTENDED THE DESCRIPTION OF A MY COPPER WIFE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- 12. ALL NEW STRUCTURES WITH A FOUNDATION AND/OR FOOTING HANNG 20 FT. OR NORE OF 1/2 IN. OR GREATER ELECTROPALLY CONDUCTIVE REFFORMS STEEL MUST HAVE IT BONGED TO THE GROUND RING USING AN EXOTHERMIC WELD CONNECTION USING #2 AWG SOLID BARE TINNED COPPER GROUND WIRE, PER NEC 250.50

00 OSG-OOD STREET ILIDNIG 20 NORTH, SUITE 2-101 ANDOVER, MA 01845 Hudson Design Groupus

L: (978) 557-5553 X: (978) 326-5586

22 KEEWAYDIN DRIVE SALEM, NH 03079

communicati

# **GENERAL NOTES**

1. FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:

CONTRACTOR - SAI

SUBCONTRACTOR - GENERAL CONTRACTOR (CONSTRUCTION)

OWNER - AT&T MOBILITY

2. PROR TO THE SUBMISSION OF BDIS, THE BIDDING SUBCONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIABLE WITH THE STRING CONSTRUCTION BANK GAN BE ACCOMPILISHED AS SHOWN ON THE CONSTRUCTION DEPARMICS, ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CONTRACTOR.

3. ALL MATERIAS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND DORINANCES, SUBCONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AND LAWFUL ORDERS ANY PUBLIC AUTHORITY RECREADING THE PERFORMANCE OF THE WORK, ALL WOF ANY COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AN APPLICABLE REGULATIONS. AND

DRAWINGS PROVIDED HERE ARE NOT TO BE SCALED AND ARE INTENDED TO SHOW OUTLINE ONLY.

UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.

6. "KITING LIST" SUPPLIED WITH THE BID PACKAGE IDENTIFIES ITEMS THAT WILL BE SUPPLIED BY CONTRACTOR, ITEMS NOT NOCLUDED IN THE BILL OF MATERIALS AND KITTING LIST SHALL BE SUPPLIED BY THE SUBCONTRACTOR.

7. THE SUBCONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.

8. IF THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE SUBCONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION SPACE FOR APPROVAL BY THE CONTRACTOR.

9. SUBCONTRACTOR SHALL DETERMINE ACTIM, ROUTING OF CONDUIT, POWER AND TI. CARLES, GROUNDING CLEUIS AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING. SUBCONTRACTOR SHALL UNLIZE EXISTING GROUNDING AND/OR SHALL NO NEW TRAYS OF RECESSING, SUBCONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH THE CONTRACTOR.

10. THE SUBCONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PARTMENTS, CURBS, LANDSCAPING AND STRUCTURES, ANY DAMAGED PART SHALL BE REPAIRED AT SUBCONTRACTOR'S EXPENSE TO THE SATISFACTION OF

11. SUBCONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAPMATERALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.

13. ALL CONCRETE REPAIR WORK SHALL BE DONE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301. SUBCONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION.

14. ANY NEW CONCRETE NEEDED FOR THE CONSTRUCTION SHALL BE ARR-ENTRAINED AND SHALL HAVE 4000 PSI STRENGTH AT 28 DAYS. ALL CONCRETE WORK SHALL BE DONE IN ACCORDANCE WITH ACI 318 CODE REQUIREMENTS.

15. ALL STRUCTURAL STEEL WORK SHALL BE DETAILED, FABRICATED AND ERECTED IN ACOGNOMICE WITH AGGS SPECIFICATIONS. ALL STRUCTURAL STEEL SHALL BE ASTN A36 (F) = 36 ks)) UNLESS OTHERWISE NOTED, PIPES SHALL BE KATN A57 FREE (F) = 36 ks)). ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPED GALWARDED. TOUCH PALL SCRATCHES AND OTHER MARCS IN THE FIELD AFTER STEEL IS ERECTED USING A COMPATIBLE ZING RICH PANN.

116. CONSTRUCTION SHALL COMPLY WITH UMTS SPECIFICATIONS AND "OENERA". CONSTRUCTION SERVICES FOR CONSTRUCTION OF AT&T MOBILITY SITES."

17. SUBCONTRACTOR SHALL WEBFY ALL EXISTING DIMENSIONS AND CONDITIONS PROOF TO COMMENCING MY WEAK, ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE WEAK OF WEBFIELD SUBCONTRACTORS SHALL NOTIFY THE CONTRACTORS AND DISPERSANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING MIT CONSTRUCTION.

18. THE EXISTING CELL SITE IS IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION MOVER BY SUBGOUNTANCED OF MALL NOT DEFENTION. ANY MORK ON EXISTING EQUIPMENT MUST BE SOFTED THE EXISTING SOFTED THE SOFTE

19. SINCE THE CELL SITE IS ACTIVE. ALL SAFETY PRESAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROWNER AND ADMINIST. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERSONN WORK THAT COULD EXPOSE THE WORKERS TO DANCER PRESSONN REPORTING MONITORS, ARE ADVISED TO BE WORN TO ALERT OF ANY DANCEROUSE ELYPICIA.

20. APPLICABLE BUILDING CODES:
SUBCONTRACTOR'S WORK SHALL COMPLY WITH ALL APPLICABLE NATIONAL,
STATE, AND LOCAL CODES AS ADDRED BY THE LOCAL AUTHORITY HAWNER
UMRSDICTION (AHJ) FOR THE LOCATION, THE EDITION OF THE ALL ANDPIED
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SHALL CODE: 2003 BC WITH 2005 CT SUPPLEMENT & 2009 CT
LECTIFICAL CODE: REFER TO ELECTRICAL DRAWINGS

SUBCONTRACTOR'S WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING STANDARDS:

AMERICAN CONCRETE INSTITUTE (ACI) 318; BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE;

AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC)

MANUAL OF STEEL CONSTRUCTION, ASD, NINTH EDITION;

TELECOMMUNICATIONS INDUSTRY ASSOCIATION (TIA) 222-F, STRUCTURAL STANDARDS FOR STEEL

ANTENNA TOWER AND ANTENNA SUPPORTING STRUCTURES; REFER TO ELECTRICAL DRAWINGS FOR SPECIFIC ELECTRICAL STANDARDS.

FOR ANY COMPLICTS BETWEEN SECTIONS OF LISTED CODES AND STANDARDS RECARDING MATERIAL METHODS FOR PROMOTIONAL OR OTHER RECUIRDLAINTS, THE MOST RESTRICTION RECRUIRDLAINTS THE MOST RESTRICTION RECRUIRDLAINTS AND A SPECIFIC RECUIRDLAINT SHALL GOVERN.

SPECIFIC REQUIRDLAINT, THE SPECIFIC RECUIRDLAINT SHALL GOVERN.

G SSB BCW

EQUIPMENT GROUND RING EQUIPMENT GROUND BASE TRANSCEIVER STATION AMERICAN WIRE GAUGE BARE COPPER WIRE ABOVE GRADE LEVEL

REQ REF N.T.S. PROPOSED X Z MGB G.C.

REQUIRED REFERENCE NOT TO SCALE MINIMUM MASTER GROUND BUS GENERAL CONTRACTOR

AT&T

EXISTING EXISTING

NEW

TBRR TBR

TO BE REMOVED
TO BE REMOVED
AND REPLACED TYPICAL

TO BE DETERMINED

AGL

**ABBREVIATIONS** 

굒 BB BB

RADIO

FREQUENCY

at&t

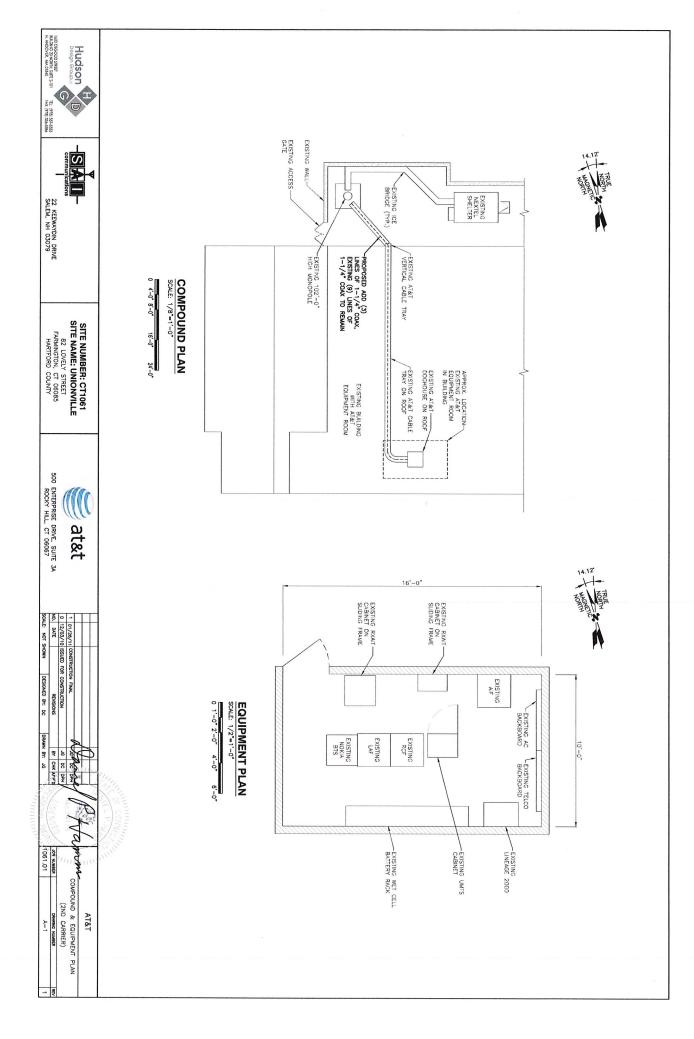
SITE NAME: UNIONVILLE SITE NUMBER: CT1061

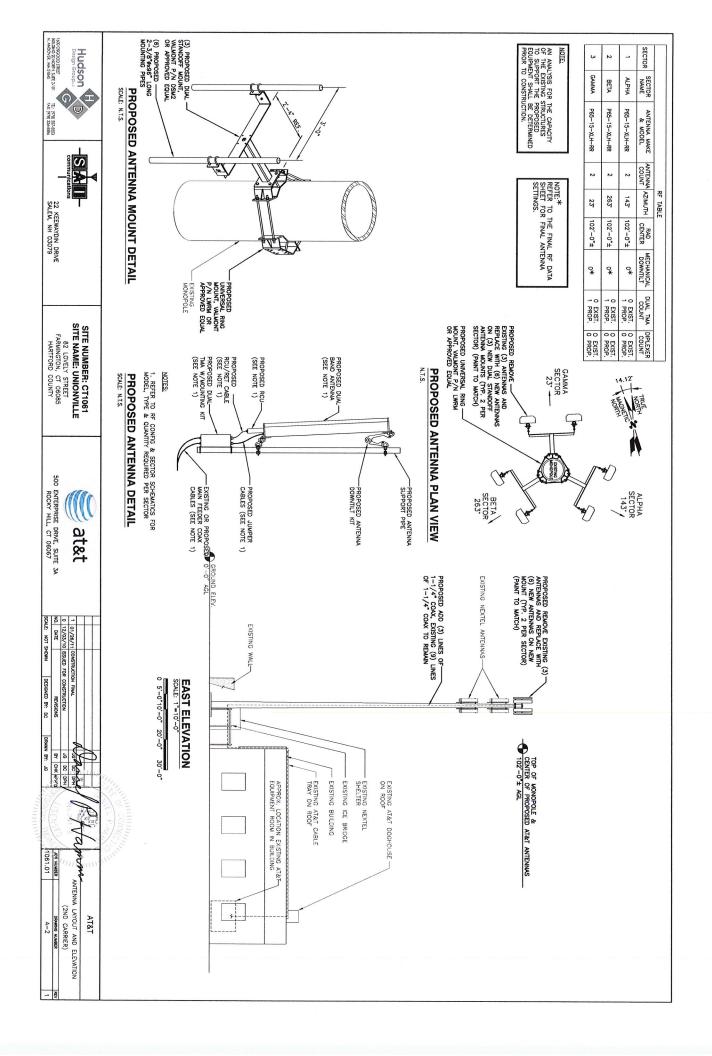
82 LOVELY STREET FARMINGTON, CT 06085 HARTFORD COUNTY

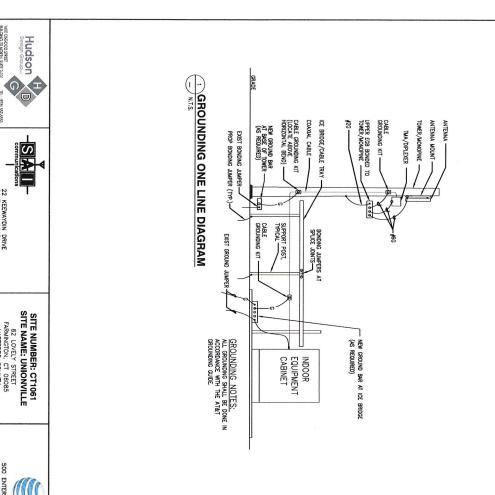
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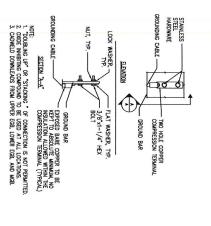
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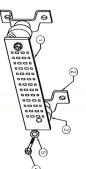






2 N.T.S. TYPICAL GROUND BAR CONNECTION DETAIL





EACH GROWND CONDUCTOR TERMINATING ON ANY GROUND BAR SHALL HAVE AN IDENTIFICATION TAG ATTACHED AT EACH END THAT MILL IDENTIFY ITS ORIGIN AND DESTINATION.

SECTION "P" - SURGE PRODUCERS

MRELESS SOLUTIONS INC.

CABLE ENTRY PORTS (HATCH PLATES) (#2)
SIDERATIOR FRAMEWORKS (IF ANNUABLE) (#2)
TARGENIA DAME TO COMMON NEUTRAL/GROUND BOND (#2)
1-24V POWER SUPPLY RETURN BAR (#2)
RECIPIER RANGES
RECIPIER RA

SECTION "A" - SURGE ABSORBERS

INTERIOR CROUND BING (#2)
EXTERNAL EARTH GROWN FELD (BURED GROUND RING) (#2)
METALLIC COLD MATE PIPE (IF AVAILABLE) (#2)
BUILDING STEEL (IF AVAILABLE) (#2)



GROUND BAR - DETAIL

at&t

SITE NUMBER: CT1061
SITE NAME: UNIONVILLE
82 LOVELY STREET
FARMINGTON, CT 06085
HARTFORD COUNTY

500 ENTERPRISE DRIVE, SUITE 3A ROCKY HILL, CT 06067

1400 OSGOOD STREET BUILDING 20 NORTH, SUITE 2-101 N. ANDGVER, MA 01845

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DRAWN BY: JG DPH (2ND CARRIER) 1061.01 AT&T G-1



Karen Couture Real Estate Consultant 500 Enterprise Drive, Ste. 3-A Rocky Hill, CT 06067 (860) 389-4924



**Kevin Clements** 12600 Deerfield Pkwy, Suite 2039 Alpharetta, GA 30004 (678) 762-3305 kclements@gpdgroup.com

GPD# 2011007.04 February 11, 2011

#### STRUCTURAL ANALYSIS REPORT

**AT&T DESIGNATION:** 

**Site USID:** 

59358

Site FA:

10035037

**Site Name:** 

**UNIONVILLE SBC CO** 

**AT&T Project:** 

**MOD-2nd Carrier Add 2011** 

**SAI DESIGNATION:** 

**Site Name:** 

CT 1061

ANALYSIS CRITERIA:

Codes:

TIA/EIA-222-F & 2003 IBC

80-mph with 0" ice 69-mph with 1/2" ice

SITE DATA:

82 Lovely Street, Unionville, CT 06085, Hartford County Latitude 41° 45' 40.971" N, Longitude 72° 53' 15.097" W

100' Monopole

Ms. Couture,

GPD is pleased to submit this Structural Analysis Report to determine the structural integrity of the aforementioned tower. The purpose of the analysis is to determine the suitability of the tower with the addition of the following proposed loading configuration:

Elev. 102'

(6) P65-15-XH-RR Antennas on (3) Dual Standoffs w/ (9) existing and (3) proposed 1-1/4" internal coax

Elev. 99'

(3) TT19-08BP111-001 Tower Mounted Amplifiers on the same mounts

Based on our analysis we have determined the designs of the tower and its foundation are sufficient for the proposed, existing, and reserved loadings as referenced in Appendix A.

We at GPD appreciate the opportunity of providing our continuing professional services to you and SAI. If you have any questions please do not hesitate to call.

Respectfully submitted,

David Granger, P.E.

Connecticut #: 17557

#### **SUMMARY & RESULTS**

The purpose of this analysis was to verify whether the existing structure is capable of carrying the proposed loading configuration as specified by AT&T to SAI. This report was commissioned by Ms. Karen Couture of SAI.

### TOWER SUMMARY AND RESULTS

Member	Capacity	Results		
Monopole	57.3%	<del></del>		
Base Plate Anchor Rods	79.9%	Pass Pass		
Autorior Rous	25.2%	Pass		
Foundation				
	27.2%	Pass		

#### **ANALYSIS METHOD**

RISA Tower (Version 5.4.2.0), a commercially available software program, was used to create a three-dimensional model of the tower and calculate primary member stresses for various dead, live, wind, and ice load cases. Selected output from the analysis is included in Appendix B. The following table details the information provided to complete this structural analysis. This analysis is solely based on this information and is being provided without the benefit of a

#### **DOCUMENTS PROVIDED**

Equipment Modification Form Internal Modification Document	1_
Construction Drawings RL Construction Drawings, dated 12/03/10 Tower Mapping GPD Associates & STG, dated 2/18/2010 Geotechnical Report WEI, Project #: 2010-1010, dated 2/16/2010 Foundation Exploration Report WEI, Project #: 2010-1010, dated 2/16/2010 Previous Analysis GPD Job #: 2010261.35 Rev. 1, dated 2/24/10	Siterra SAI Siterra Siterra Siterra Siterra Siterra

2/9/2011

#### **ASSUMPTIONS**

This structural analysis is based on the theoretical capacity of the members and is not a condition assessment of the tower. This analysis is from information supplied, and therefore, its results are based on and are as accurate as that supplied data. GPD has made no independent determination, nor is it required to, of its accuracy. The following assumptions were made for this structural analysis.

- 1. The tower shaft sizes and shape are considered accurate as supplied. The material grade is as per data supplied and/or as assumed and as stated in the materials section.
- 2. The antenna configuration is as supplied and/or as modeled in the analysis. It is assumed to be complete and accurate. All antennas, mounts, coax and waveguides are assumed to be properly installed and supported as per manufacturer requirements
- 3. Some assumptions are made regarding antennas and mount sizes and their projected areas based on best interpretation of data supplied and of best knowledge of antenna type and industry practice.
- 4. All mounts, if applicable, are considered adequate to support the loading. No actual analysis of the mount(s) is performed. This analysis is limited to analyzing the tower only.
- 5. The soil parameters are as per data supplied or as assumed and stated in the calculations. If no data is available, the foundation system is not verified. In the case of absent foundation data, it is the tower owner's responsibility to insure that the foundation system is adequate to support the structure with its new reactions.
- 6. The tower and structures have been properly maintained in accordance with TIA Standards and/or with manufacturer's specifications.
- 7. All welds and connections are assumed to develop at least the member capacity, unless determined otherwise and explicitly stated in this report.
- 8. All prior structural modifications, if any, are assumed to be as per data supplied/available, to have been properly installed and to be fully effective.
- 9. All proposed coax/ waveguide are assumed to be internal to the monopole.
- 10. All existing loading was obtained from the most recent structural analysis, by GPD Job #: 2010261.35 Rev. 1, dated 2/24/10, site photos, RL Construction Drawings, dated 12/03/10, and the provided internal modification document and is assumed to be accurate.
- 11. The proposed TMA's were assumed to be mounted at 99', below the proposed antennas, based on the RL Construction Drawings, dated 12/03/10.
- 12. The proposed TMA model is based on email correspondence with Karen Couture, dated 2/9/11.

If any of these assumptions are not valid or have been made in error, this analysis may be affected, and GPD Associates should be allowed to review any new information to determine its effect on the structural integrity of the tower.

#### **DISCLAIMER OF WARRANTIES**

GPD ASSOCIATES has not performed a recent site visit to the tower to verify the member sizes or antenna/coax loading. If the existing conditions are not as represented on the tower elevation contained in this report, we should be contacted immediately to evaluate the significance of the discrepancy. This is not a condition assessment of the tower or foundation. This report does not replace a full tower inspection. The tower and foundations are assumed to have been properly fabricated, erected, maintained, in good condition, twist free, and plumb.

The engineering services rendered by GPD ASSOCIATES in connection with this Structural Analysis are limited to a computer analysis of the tower structure and theoretical capacity of its main structural members. All tower components have been assumed to only resist dead loads when no other loads are applied. No allowance was made for any damaged, bent, missing, loose, or rusted members (above and below ground). No allowance was made for loose bolts or cracked welds.

GPD ASSOCIATES does not analyze the fabrication of the structure (including welding). It is not possible to have all the very detailed information needed to perform a thorough analysis of every structural sub-component and connection of an existing tower. GPD ASSOCIATES provides a limited scope of service in that we cannot verify the adequacy of every weld, plate connection detail, etc. The purpose of this report is to assess the feasibility of adding appurtenances usually accompanied by transmission lines to the structure.

It is the owner's responsibility to determine the amount of ice accumulation, if any, that should be considered in the structural analysis.

The attached sketches are a schematic representation of the analyzed tower. If any material is fabricated from these sketches, the contractor shall be responsible for field verifying the existing conditions, proper fit, and clearance in the field. Any mentions of structural modifications are reasonable estimates and should not be used as a precise construction document. Precise modification drawings are obtainable from GPD ASSOCIATES, but are beyond the scope of this report.

Miscellaneous items such as antenna mounts, etc., have not been designed or detailed as a part of our work. We recommend that material of adequate size and strength be purchased from a reputable tower manufacturer.

GPD ASSOCIATES makes no warranties, expressed and/or implied, in connection with this report and disclaims any liability arising from material, fabrication, and erection of this tower. GPD ASSOCIATES will not be responsible whatsoever for, or on account of, consequential or incidental damages sustained by any person, firm, or organization as a result of any data or conclusions contained in this report. The maximum liability of GPD ASSOCIATES pursuant to this report will be limited to the total fee received for preparation of this report.

2/9/2011

#### **APPENDIX A**

**Tower Analysis Summary Form** 

# Tower Analysis Summary Form

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•	ı

General Info	
Site Name	SO COS SI INNOMILI
Site Number	CHICHWILLE SBC CO
DO N	CT1061 (59358)
umber	10035037
ate of Analysis	Topogo.
Don't Don't and a series	2/3/2011
Chipany Performing Analysis	GPD

OHII ISHO:	Description	
Tower Type (G. SST, MP)	02	Date
I ower Height (top of steel AGL)	100.	
Tower Manufacturer	n/a	
Tower Model	n/a	
Tower Design	n/a	
Foundation Design	n/a	
Geotech Report	WEI Project #: 2040 4040	
Tower Mapping	GBD A	2/16/2010
Previous Structural Application	OFD ASSOCIATES & STG	2/18/2010
Modification As-Built Desirance	GPD Job #: 2010261.35 Rev. 1	2/24/2010
Spillings and a second	INS	
roundation Mapping	WEI Broken and and and and and and and and and an	

Design Code Used	
	TIA/FIA-222 E 8 2000 12.0
Location of Tower (County, State)	Hartford C. 2003 IBC
Basic Wind Speed (mph)	narriold, connecticut
Ice Thickness (in)	80-fastest
Structure O	0.5
Suructure Classification (I, II, III)	
Exposure Category (B, C, D)	
Topographic Category (1 to 5)	

The information contained in this summary report is not to be used independently from the PE stamped tower analysis.

lesign Code Used	
	TIA/EIA-222-F & 2003 IBC
ocation of Tower (County, State)	Harfford Connection
asic Wind Speed (mph)	80 feet -
e Thickness (in)	00-1451651
ructure Classification (1 11 111)	0.5
(1, 11, 11)	
Apusure Category (B, C, D)	
pographic Category (1 to 5)	

	TIA/EIA-222-F & 2003 IBC
ocation of Tower (County State)	Delicate a rough
(Simo)	narriord, Connecticut
asic Wind Speed (mph)	80-fastest
ce Thickness (in)	0.5
	6.0
uncure classification (I, II, III)	
osure Category (B, C, D)	
ographic Category (1 to 5)	

# Steel Yield Strength (ksi)

	09	OF THE PERSON NAMED IN COLUMN 1	8	75
Pole		Base Plate		Anchor Rods

## Note: Steel strengths assumed. Existing / Reserved Loading

		Transmission Line		Size Attachment	Internal/External		I-1/4 Internal				7/8" Internal	
		Transı		Quantity Model		Inchange					Unknown	
		MANAGEMENT OF THE PARTY OF THE		Quantity		6						
	Mount	Tipou.	T.			Circiowii Pipe mounts	on same mounts	on same mounts		Pipe mounts		Pipe mounts
			Quantity Manufacturer		Intracum	IIMOIIVIIO				Unknown		Unknown Pipe mounts
			Quantity		3					8		3
			Azimuth		23,143,263					30,150,270		30,150,270
			Model		COUDPBL	12"x9"x2.5"	12"x9"x2.5"		AP11-880/090D/XP		AP11-880/090D/VD	
AND CHARGO CHARGOS		Manufacturer	5	FMS	1	UNKNOWN	Unknown		Kathrein		Kathrein	
C. C	STATE OF STA	Type		Panel	Dintour	Т	Diplexer		Panel		Panel P	
Antenna		Quantity										e same elevation.
		Antenna CL (ft)		3	3	·	2		20		3	loading at the sam
	Mount	Height (ft)		8	66	99		88	2	2		place the existing
	Antenna Ounce		AT&T Mobility	AT&T Mobility	, month	AI&I Mobility		Nextel		Nextel	Note: The proposed loading shall age.	I III III III III III III III III III

# ised loading shall replace the existing loading at the same elevation.

## Proposed Loading

	September 1980 Septem	Attachment	Internal/External	
	mission Line	i	9710	* ****
	Trans	Model		I DES. SOA
		Quantity		12
		Type		andoffs
Moun	_	turer	-	Dual St
		ity Manufac		vaimont
	,	Guant	e	,
	Amino	Unilling.	23.143.263	
	Model		H-RR	P111-001
_	-a		P65-15-XI	TT19-08B
	Manufactur		Powerwave	Powerwave
2	Туре	, , ,	rane	IMA
	Quantity			
	(£)	9	~	londing at the
-	Antenna	102	66	ting londing
Mount	Height (ft)	61	6	solace the exic
Jer		6	6	ading shall re
Antenna Ow		Mobility	lobility	ne proposed lo
		T&TA	ŏ	ote: Th

	Attended to	Internal/External
	mission Line	Size
	Transi	Model
	Ousnette	ć na
	Type	
	Mount	
	Manufacture	
	Quantity	
	Azimuth	
#Maleurara announce	Model	
Thurst outstand outside		
	Manufacturer	
	Туре	
Antenna	Quantity	
	a CL (ft)	
	Antenna	
	Mount Height (ft)	
	enna Owner	
	Ant	

#### **APPENDIX B**

RISA Tower Output File

#### **RISATower**

GPD Associates 520 South Main St. Suite 2531 Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101

Job		Page
	CT1061 (59358) UNIONVILLE SBC CO	1 of 2
Project		Date
	2011007.04	14:09:41 02/09/11
Client		Designed by
	SAI	mhoudeshell

#### **Tower Input Data**

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in Hartford County, Connecticut.

Basic wind speed of 80 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 69 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

#### Feed Line/Linear Appurtenances - Entered As Area

Description	Face	Allow	Component	Placement	Total		$C_A A_A$	Weight
	or Leg	Shield	Туре	ft	Number		ft²/ft	plf
LDF6-50A (1-1/4	С	No	Inside Pole	99.00 - 2.00	12	No Ice	0.00	0.66
FOAM)						1/2" Ice	0.00	0.66
LDF5-50A (7/8 FOAM)	Α	No	Inside Pole	88.00 - 2.00	6	No Ice	0.00	0.33
						1/2" Ice	0.00	0.33
LDF5-50A (7/8 FOAM)	Α	No	Inside Pole	81.00 - 2.00	6	No Ice	0.00	0.33
						1/2" Ice	0.00	0.33
5/8" Step Bolts	C	No	CaAa (Out Of	100.00 - 8.00	1	No Ice	0.04	1.00
			Face)			1/2" Ice	0.14	1.56
Safety Line 3/8	C	No	CaAa (Out Of	100.00 - 8.00	1	No Ice	0.04	0.22
			Face)			1/2" Ice	0.14	0.75

#### **Discrete Tower Loads**

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ft ft ft	o	ft		ft²	ft²	lb
Valmont Dual Standoff Mount	С	None		0.0000	99.00	No Ice 1/2" Ice	4.85 4.96	2.67 3.22	150.000
(2) P65-15-XLH-RR w/ Mount Pipe	Α	From Leg	2.76 1.17 3.00	23.0000	99.00	No Ice 1/2" Ice	6.13 6.59	4.27 4.90	59.250 104.856
(2) P65-15-XLH-RR w/ Mount Pipe	В	From Leg	2.76 1.17 3.00	23.0000	99.00	No Ice 1/2" Ice	6.13 6.59	4.27 4.90	59.250 104.856
(2) P65-15-XLH-RR w/ Mount Pipe	С	From Leg	2.76 1.17 3.00	23.0000	99.00	No Ice 1/2" Ice	6.13 6.59	4.27 4.90	59.250 104.850
TT19-08BP111-001	A	From Leg	2.76 1.17 0.00	23.0000	99.00	No Ice 1/2" Ice	0.64 0.76	0.52 0.62	16.000 21.800

#### **RISATower**

GPD Associates 520 South Main St. Suite 2531 Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101

Job		Page
	CT1061 (59358) UNIONVILLE SBC CO	2 of 2
Project		Date
	2011007.04	14:09:41 02/09/11
Client		Designed by
	SAI	mhoudeshell

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	$C_A A_A$ Side	Weight
			Vert ft ft	o	ft		$ft^2$	$ft^2$	lb
			ft						
TT19-08BP111-001	В	From Leg	2.76	23.0000	99.00	No Ice	0.64	0.52	16.000
			1.17			1/2" Ice	0.76	0.62	21.800
			0.00						
TT19-08BP111-001	C	From Leg	2.76	23.0000	99.00	No Ice	0.64	0.52	16.000
			1.17			1/2" Ice	0.76	0.62	21.800
			0.00						
Ring Mount	C	None		0.0000	88.00	No Ice	2.50	2.50	150.000
						1/2" Ice	3.20	3.20	200.000
AP11-880/090/XP w/Mount	Α	From Leg	0.87	30.0000	88.00	No Ice	5.81	4.17	43.150
Pipe			0.50			1/2" Ice	6.50	5.21	87.331
			0.00						
AP11-880/090/XP w/Mount	В	From Leg	0.87	30.0000	88.00	No Ice	5.81	4.17	43.150
Pipe			0.50 0.00			1/2" Ice	6.50	5.21	87.331
AP11-880/090/XP w/Mount	С	From Leg	0.00	30.0000	88.00	No Ice	5.81	4.17	43.150
Pipe	C	From Leg	0.50	30.0000	88.00	1/2" Ice	6.50	5.21	87.331
ripe			0.00			1/2 100	0.50	3.21	07.331
Ring Mount	C	None	0.00	0.0000	81.00	No Ice	2.50	2.50	150.000
King Mount	C	None		0.0000	01.00	1/2" Ice	3.20	3.20	200.000
AP11-880/090/XP w/Mount	Α	From Leg	0.87	30.0000	81.00	No Ice	5.81	4.17	43.150
Pipe		110111 208	0.50	20.000	01.00	1/2" Ice	6.50	5.21	87.331
p-			0.00				0.00		
AP11-880/090/XP w/Mount	В	From Leg	0.87	30.0000	81.00	No Ice	5.81	4.17	43.150
Pipe		8	0.50			1/2" Ice	6.50	5.21	87.331
•			0.00						
AP11-880/090/XP w/Mount	C	From Leg	0.87	30.0000	81.00	No Ice	5.81	4.17	43.150
Pipe			0.50			1/2" Ice	6.50	5.21	87.331
			0.00						

### Critical Deflections and Radius of Curvature - Service Wind

Elevation	vation Appurtenance		Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	0	ft
99.00	Valmont Dual Standoff Mount	35	17.669	1.4835	0.0002	19163
88.00	Ring Mount	35	14.289	1.4373	0.0002	8406
81.00	Ring Mount	35	12.218	1.3723	0.0002	5764

### **Section Capacity Table**

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SF*P <sub>allow</sub> lb	% Capacity	Pass Fail
L1	100 - 87.1667	Pole	TP16.2634x14.4933x0.2188	1	-868.972	520799.079	11.2	Pass
L2	87.1667 - 45.5	Pole	TP21.6355x15.4121x0.25	2	-3996.680	795758.311	51.0	Pass
L3	45.5 - 0	Pole	TP27.4112x20.6478x0.3438	3	-9285.810	1417192.22	57.3	Pass
						1	Summary	
						Pole (L3)	57.3	Pass
						RATING =	57.3	Pass

#### **APPENDIX C**

**Tower Elevation Drawing** 

#### 14.4933 16.2634 12.83 3.00 459.6 18 87.2 ft 44.67 3.50 15.4121 21.6355 0.2500 2203.9 18 A572-60 45.5 ft 49.00 4308.9 9 AXIAL 11394 lb SHEAR MOMENT 5664 lb 395158 lb-ft TORQUE 88 lb-ft 69 mph WIND - 0.5000 in ICE AXIAL 9292 lb SHEAR MOMENT 6417 lb 444493 lb-ft 0.0 ft TORQUE 32 lb-ft REACTIONS - 80 mph WIND 6972.5 Socket Length (ft) Number of Sides Thickness (in) Top Dia (in) Length (ft) Bot Dia (in) Weight (Ib)

#### **DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION
Valmont Dual Standoff Mount	99	AP11-880/090/XP w/Mount Pipe	88
(2) P65-15-XLH-RR w/ Mount Pipe	99	AP11-880/090/XP w/Mount Pipe	88
(2) P65-15-XLH-RR w/ Mount Pipe	99	AP11-880/090/XP w/Mount Pipe	88
(2) P65-15-XLH-RR w/ Mount Pipe	99	Ring Mount	81
TT19-08BP111-001	99	AP11-880/090/XP w/Mount Pipe	81
TT19-08BP111-001	99	AP11-880/090/XP w/Mount Pipe	81
TT19-08BP111-001	99	AP11-880/090/XP w/Mount Pipe	81
Ring Mount	88		

#### **MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-60	60 ksi	75 ksi			

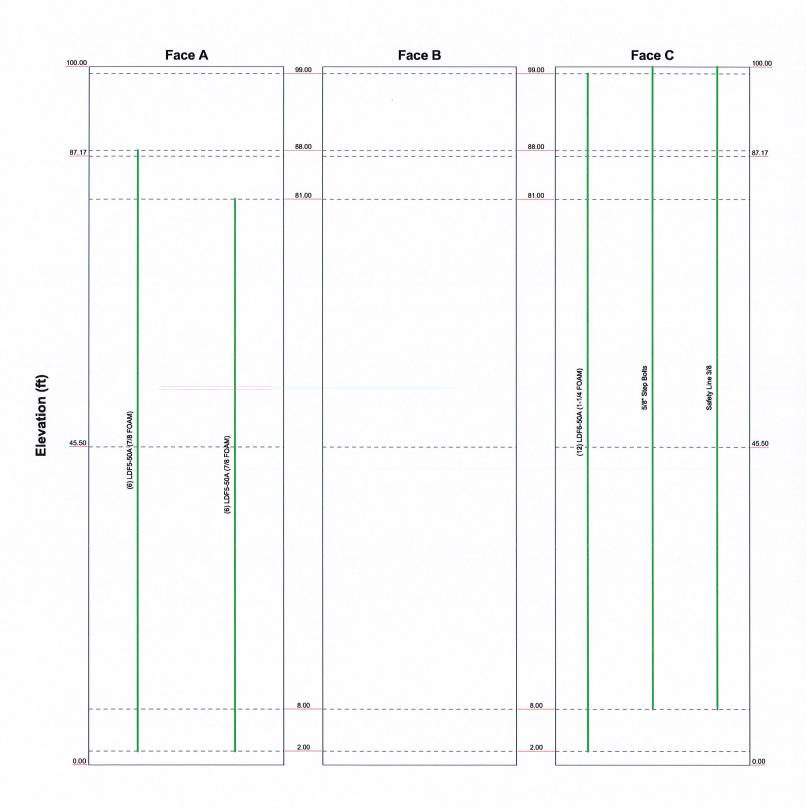
#### **TOWER DESIGN NOTES**

- 1. Tower is located in Hartford County, Connecticut.
  2. Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
  3. Tower is also designed for a 69 mph basic wind with 0.50 in ice.
  4. Deflections are based upon a 50 mph wind.
  5. TOWER RATING: 57.3%

GPD Associates	<sup>Job:</sup> CT1061 (5935	8) UNIONVILLE S	вс со
	Project: 2011007.04		
Akron, OH 44311	Client: SAI	Drawn by: mhoudeshell	App'd:
Phone: (330) 572-2100	Code: TIA/EIA-222-F	Date: 02/09/11	Scale: NTS
	Path: N:\2011\2011007\04\CT106	51 Unionville (59358)\RISA\59358.er	Dwg No. E-1

### Feedline Distribution Chart 0' - 100'

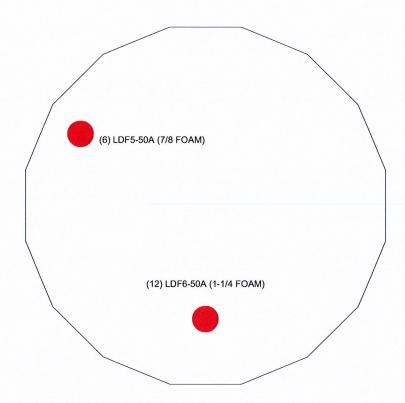
\_\_\_\_\_\_ Round \_\_\_\_\_\_ Flat \_\_\_\_\_ App In Face \_\_\_\_\_ App Out Face \_\_\_\_\_ Truss Le



GPD Associates	Job: CT1061 (59358	B) UNIONVILLE S	вс со
520 South Main St. Suite 2531	Project: 2011007.04		
Akron, OH 44311	Client: SAI	Drawn by: mhoudeshell	App'd:
Phone: (330) 572-2100	Code: TIA/EIA-222-F	Date: 02/09/11	Scale: NTS
FAX: (330) 572-2101	Path: N:\2011\2011007\04\CT106	1 Unionville (59358)\RISA\59358.er	Dwg No. E-7

#### Feedline Plan

\_\_\_\_\_\_ Round \_\_\_\_\_\_ Flat \_\_\_\_\_ App In Face \_\_\_\_\_ App Out Face



GPD Associates	Job: CT1061 (5935	8) UNIONVILLE S	вс со
1020 Coulii Maiii Ct. Cuite 200 i	Project: 2011007.04		
Akron, OH 44311	Client: SAI	Drawn by: mhoudeshell	App'd:
Phone: (330) 572-2100	Code: TIA/EIA-222-F	Date: 02/09/11	Scale: NTS
FAX: (330) 572-2101	Path: N:\2011\2011007\04\CT10	61 Unionville (59358)\RISA\59358.er	Dwg No. E-7

#### **APPENDIX D**

Anchor Rod & Base Plate Analysis

# CT1061 (59358) UNIONVILLE SBC CO

444.493 Kip-ft 6.417 Kip 9.292 Kip Below ft TOWER REACTIONS
total overturning moment =
total shear = ground water table = axial =

# PAD DIMENSIONS

3533.41 k-ft 508.663 k-ft 15.21 k 8.1 ksf 0.124 kcf 0.150 kcf 17 ft 9.5 ft 4.5 ft 383.547 k height = Y<sub>soil</sub> = Mr= Mot = Yconc = П Wwedge width = depth of conc =

# LOAD PERPENDICULAR TO PAD

Allowable Bearing =

0.44711246 1.94835681 0.70594769 2.20719204  $Q_{MAX} = P/A+M/S = Q_{MIN} = P/A-M/S =$  $Q_{MAX} = P/A+M/S=$ Q<sub>MIN</sub>= P/A-M/S= LOAD AT 45 DEGREES TO PAD

1.33 M/P width/6 2.83

ok (width/6 > M/P

22.69 ft 22.69 ft B,= <u>ا</u>۔  $Q_{ALL} = 2084.5 \text{ kips}$ 

IF e/W > 1/6

 $Q_{Max} = 568.01 \text{ kips}$ 27.2% OK

0.055 ok (e/W < 1/6) 0.055 ok (e/W < 1/6)

e<sub>x</sub>/W =

e<sub>y</sub>/W =

ey=

0.938 0.938

359.679 359.679

M M X

QMAX/QALL =

충 27.2% Foundation Capacity:

6" 🗡 🦳 finish grade				width
	,	height	depth of conc.	Spreadsheat Input PCAMATS Input

F.S. OVERTURNING / F.S. ALLOWABLE 21.6%

1.5

F.S. OVERTURNING = 6.94647034 OK >

IF M/P>width/6

ksf ksf Qmax = 2.097Qmin = 0.000

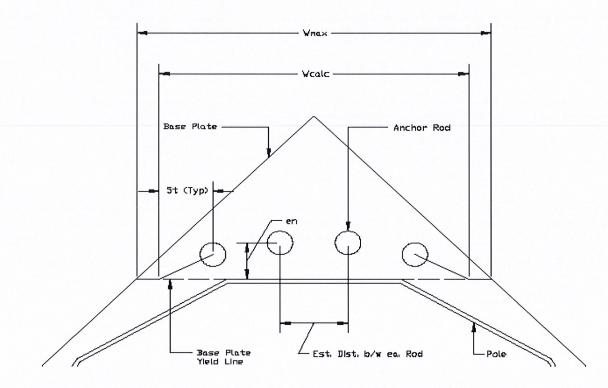
Q<sub>MAX</sub>/Q<sub>ALL</sub> 25.9% OK

## Anchor Rod and Base Plate Stresses CT1061 (59358) UNIONVILLE SBC CO 2011007.04

Overturning Moment =	444.49 k*ft
Axial Force =	9.29 k
Shear Force =	6.42 k

Anchor Rods				
Pole Diameter =	27.4112	in		
Number of Rods =	8			
Type =	Upset Rod			
Rod Yield Strength (Fy) =	75	ksi		
ASIF =	1.333			
Rod Circle =	53	in		
Rod Diameter =	2.25	in		
Net Tensile Area =	3.25	in <sup>2</sup>		
Max Tension on Rod =	49.11	kips		
Max Compression on Rod =	51.44	kips		
Allow. Rod Force =	195.00	kips		
Anchor Rod Capacity =	25.2%	OK		

Base Plate		
Plate Strength (Fy) =	50	ksi
Plate Thickness =	2.5	in
Plate Width =	48.5	in
Est. Dist. b/w ea. Rod =	6	in
$W_{calc} =$	31.000	in
w <sub>max</sub> =	41.178	in
w =	31.00	in
S =	32.29	in <sup>3</sup>
fb =	39.96	ksi
Fb =	50	ksi
Base Plate Capacity =	79.9%	OK



GPD Unstiffened Square Base Plate Stress (Rev F) - V2.05

#### **APPENDIX E**

Foundation Analysis

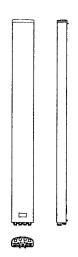
P65-15-XLH-RR

#### **Dual Broadband Antennas**

POLARIZATION: Dual linear ±45° FREQUENCY (MHz): 698-894, 1710-2170 HORIZONTAL BEAM WIDTH (°): 65, 65 GAIN (dBi/dBd): 14.7/12.6, 17.0/14.9 TILT: 0-13, 0-9 LENGTH: 51"

ELECTRICAL SPECIFICATIONS*				22.10111. 01		
Frequency range (MHz)	698	-894		1710-2170		
Frequency band (MHz)	698-806	806-894	1710-1880	1850-1990	1900-2170	
Gain (dBi/dBd)	14/11.9	14.7/12.6	16.4/14.3	16.7/14.6	17.0/14.9	
Polarization	Dual Line	ear +/- 45	Dual Linear +/- 45			
Nominal Impedance (Ω)	5	60		50		
VSWR	< 1	.5:1		< 1.5:1		
Horizontal beam width, -3 dB (°)	73	63	65	61	60	
Vertical beam width, -3 dB (°)	1	7		7.5	, 00	
Electrical down tilt (°)	0-	13	0-9			
Side lobe suppression, vertical 1st upper (dB)	> 14		>20			
Isolation between inputs (dB)	>30		> 30			
Inter band Isolation (dB)	> 40		> 40			
Tracking, horizontal plane ±60° (dB)	< 2		< 2			
Vertical beam squint (°)	<1.25		< 0.5			
Front to back ratio (dB) 180°±30° copolar	> ;		> 28			
Front to back ratio (dB) 180°±30° total power	>	25	> 25			
Cross polar discrimination (XPD) 0° (dB)	> 15		> 15			
Cross polar discrimination (XPD) ±60° (dB)	> 10		> 10			
IM3, 2xTx@43dBm (dBc)	<-153		<-153			
Power handling, average per input (W)	500		300			
Power handling, average total (W)	1000		600			

MECHANICAL SPECIFICATIONS*	
Connector	4 X 7/16 DIN Female, IP67
Connector position	Bottom
Dimensions, HxWxD, in (mm)	51" x 12" x 6" (1295 x 305 x 152)
Mounting	Pre-mounted Tilt Brackets
Weight, with brackets, lbs (kg)	41 (19)
Weight, without brackets, lbs (kg)	30 (14)
Wind load, frontal/lateral/rear side 42 m/s Cd=1.0 (N)	920
Maximum operational wind speed, mph (m/s)	100 (45)
Survival wind speed, mph (m/s)	150 (67)
Lightning protection	DC Ground
Operating Temperature	-40°C to +60°C
Radome material	PVC, IP55
Packet size, HxWxD, in (mm)	60" x 16" x 10" (1524 x 400 x 255)
Radome colour	Light Grey
Shipping weight, lbs (kg)	52 (24)
RET	iRET AISGv1.1, MET and AISGv2.0
Brackets	7256.00, 7454.00



<sup>\*</sup>All specifications subject to change without notice. Please contact your Powerwave representative for complete performance data.

#### ANTENNA PATTERNS\*

For detailed patterns visit http://www.powerwave.com/rpa/.

#### TT19-08BP111-001

#### TMA Twin 1900 with 850 Bypass 12 dB AISG 1.1

ELECTRICAL SPECIFICATIONS	
UL Frequency Range (MHz)	1850-1910 with 824-894 bypass
UL Rejection	>77 dB
UL Gain(dB)	12
UL Return Loss	>18
UL Noise Figure	<1.7 dB, Typical
UL Output 3rd Order Intercept Point(dBm)	>+23
UL Bypass Loss(dB)	2.5, Typical
UL Max Input Power (dBm)	+14 dBm
DL Frequency Range (MHz)	1930-1990 with 824-894 bypass
DL Return Loss	>18
DL Insertion Loss (dB)	850 MHz, <0.3; 1900 MHz, <0.5
Intermodulation	@ 2 x +43 dBm TX carriers, in receive band, <160 dBc, reffered to antenna port
Input Voltage (V)	AISG Mode: 10-30; Current alarm mode: 8 -17
Alarm Functionality	AISG compatible or in case of no AISG command received, current alarm mode 170-190 mA
Power Consumption	<1.1W @12V
Power Handling, RMS	850: >57 dBm; 1900: >55 dBm
AISG Compatibility	AISG 1.1 fully upgreadable to AISG 2.0 (AISG version only dependent on loaded SW version) TT19-08BP112-001 has AISG 2.0 loaded from factory

#### MECHANICAL SPECIFICATIONS

Dimension HxWxD mm(ft)	250x169x137 mm (9.9"x6.7"x5.4")
Weight(lbs)	<16
Colors	Off white (NCS 1502-R)
RF Connectors	DIN 7/16 female, long neck
Mounting Kit	Mounting kit for pole and wall is included

#### **ENVIRONMENTAL SPECIFICATIONS**

Temperature Range	-40° C to +65° C (-40° F to +149° F)
Operational	ETS 300 019-1-4
Transportation	ETS 300 019-1-2
Storage	ETS 300 019-1-1
Lightning Protection	3 kA 10/350 μs; 20 kA (Shield)
Housing	Aluminum
MTBF	>1 million hours per TMA
Ingress Protection	IP65 and IP68

#### APPROVAL AND TESTS

Safety	EN60950
EMC	3GPP: TS 25.113



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