

March 25, 2015

Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Re: **Notice of Exempt Modification – Facility Modification
190 Colt Highway (aka Rattlesnake Mountain), Farmington, Connecticut**

Dear Ms. Bachman:

Cellco Partnership d/b/a Verizon Wireless (“Cellco”) currently maintains eight (8) wireless telecommunications antennas at the 120-foot level on the existing 1,339-foot guyed-lattice tower at 190 Colt Highway in Farmington, Connecticut (the “Property”). The tower and underlying property are owned by Communications Site Management LLC. Cellco’s use of the tower was approved by the Council in 1988. Cellco now intends to modify its facility by replacing two (2) of its existing antennas with two (2) model X7C-FRO-660-V, 700 MHz antennas at the same level on the tower. Included in Attachment 1 are specifications for Cellco’s replacement antennas.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is also being sent to Kathleen A. Eagen, Town Manager for the Town of Farmington. A copy of this letter is also being sent to Communications Site Management, LLC, the owner of the Property.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing tower. Cellco’s replacement antennas are located at the 120-foot level of the 1,339-foot tower.

13535351-v1

Robinson+Cole

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Page 2

2. The proposed modifications will not involve any change to ground-mounted equipment and, therefore, will not require the extension of the site boundary.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) safety standard. A cumulative General Power Density table for Cellco's modified facility is included in Attachment 2.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The tower, guy wires and foundation can support Cellco's proposed modifications. (See Structural Analysis included in Attachment 3).

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,



Kenneth C. Baldwin

Enclosures

Copy to:

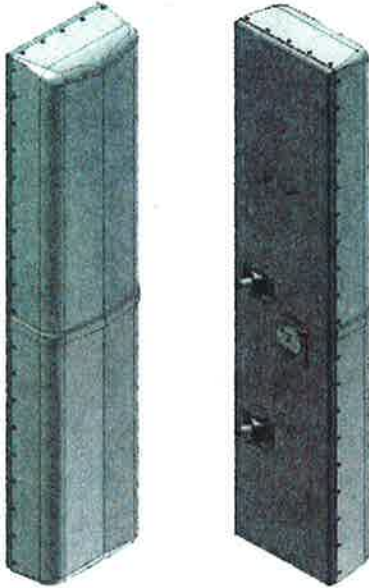
Kathleen A. Eagen, Farmington Town Manager
Communications Site Management, LLC
Tim Parks

ATTACHMENT 1



X7C-FRO-660-V

X-Pol Antenna, 698-896MHz, Fast-Roll-Off 60° H-Beam
RET/MET



- Designed to improve SNR
- Greatly increases LTE data rates
- Broadband radiator
- Macro Cell, high gain antenna
- Suitable for LTE/CDMA/UMTS/GSM
- AISG 2.0 RET or manual MET tilt control

Electrical Specifications

Frequency Band, MHz	698-824	824-896
Horizontal Beamwidth, 3dB points	62	58
Gain, dBi	15.9	16.0
Vertical Beamwidth, 3dB points	12.0	10.5
Front-to-Back at 180°, dB	>28	
Upper Sidelobe Suppression, Typical, dB	<-18	
Polarization	+/-45°	
Electrical Downtilt	0-10° or 4-14°	
VSWR/Return Loss, dB, Maximum	1.5:1/14.0	
Isolation Between Ports, dB, Mimimum	-28	
Intermodulation (2x20w), IM3, dBc, Maximum	-150	
Impedance, ohms	50	
Maximum Power Per Connector, CW	500	

www.cssantenna.com

410-612-0080

All Specifications are subject to change.

Refer to www.cssantenna.com for the most current information

customerservice@cssantenna.com

ATTACHMENT 2



X7C-FRO-660-V

X-Pol Antenna, 698-896MHz, Fast-Roll-Off 60° H-Beam
RET/MET

Mechanical Specifications

Dimensions, Length/Width/Depth	72.0/14.6/8.0 in (1829/372/204 mm)
Connector (Quantity) Type	(2) 7-16 DIN Female
Connector Torque	220-265 lbf-in (25-30 N-m)
Connector Location	Back
Antenna Weight	35.0 lbs
Bracket Weight	13.2 lbs (6.0 kg)
Standard Bracket Kit	CSS P/N 919011
Mechanical Downtilt Range	0-12°
Radome Material	Ultra High Strength Luran, UV Stabilized, ASTM D1925
Wind Survival	150 mph (241 km/h)
Front Wind Load	205.39 lbf (913.65 N) @100mph
Equivalent Flat Plate	4.09 sq-ft (c=2) @ 100mph

RET Information

Model	CSS-RET-200
Mounting Location	Rear of Antenna
Weight	1.2 lb (0.54 kg)
Communication Standard	AISG 2.0
Control System	CSS-PCU-220



Order Information

Model	Description
X7C-FRO-660-VR0	Antenna with manual RET adjust electrical downtilt 0-10°
X7C-FRO-660-VR4	Antenna with manual RET adjust electrical downtilt 4-14°
X7C-FRO-660-VM0	Antenna with remote MET adjust electrical downtilt 0-10°
X7C-FRO-660-VM4	Antenna with remote MET adjust electrical downtilt 4-14°

Optional Bracket Kit

919036	Bracket Kit, 2-Point, 12 deg D-tilt, For 4.5" OD Pole
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www.cssantenna.com

410-612-0080

All Specifications are subject to change.

Refer to www.cssantenna.com for the most current information

customerservice@cssantenna.com

ATTACHMENT 3



PROJECT:
STRUCTURAL ANALYSIS
of
Existing 1339ft
LRM3700 Guyed Mast

CUSTOMER:
VitalSite Services, Inc

SITE:
Rattlesnake
(aka Farmington), CT

TURRIS FILE:
14-0846
January 26, 2015



Turriss Project: 14-0846
January 26, 2015

STRUCTURAL ANALYSIS OF
Existing 1339 Ft. LRM3700 Guyed Mast
at Rattlesnake (aka Farmington), CT

FOR:

VitalSite Services, Inc

Attention: Stephen Schadler
VitalSite Services, Inc.
Real Estate Consultant / Verizon Wireless
99 East River Drive, 9th Floor
East Hartford, CT 06810

CC: Joe Legere
Communications Site Management LLC.
Goodwin Square
225 Asylum Street, 29th Floor
Hartford, CT 06103

Prepared by: Meimei Lam
TURRIS CORP.
70 Todd Road, Georgetown, ON, Canada L7G 4R7
Phone: (905) 877-8885 Fax: (905) 877-8835

Reviewed By: Tony Fonseca, P.E.
Turris Engineering Inc.
9 Apple Lane, Moorestown, NJ 08057
Mob: (803) 873-1562



Introduction

We have completed the structural analysis of the existing 1339ft LRM3700 guyed mast at Rattlesnake (aka Farmington), CT, and are pleased to submit our report for your attention.

The purpose of this analysis is to evaluate the tower for compliance with ANSI/TIA-222-G-2009 Add. 2 with modifications to existing antenna on the tower based on information provided by VitalSite Services, Inc on December 23, 2014. Table 1 tabulates Verizon’s current inventory on the tower. Table 2 tabulates the removed equipment from the tower.

Table 1 –Verizon’s Current Inventory

ID	Pos	Description	Qty	Elev (ft)	Tx Line	Qty	AZ	Comments
81a		Swedcom SCE-6016 Rev 2	1	116	1 5/8"	12	0	
81b		Swedcom SACP 2x5516	1	116			0	
81c		CSS / X7C-FRO-660-VR4	1	116			0	
81d		Swedcom SCE-6016 Rev 2	1	116			0	
82		NA						
83a		Swedcom SCE-6016 Rev 2	1	116			270	2 existing Andrew CBC721-DF
83b		CSS / X7C-FRO-660-VR4	1	116			270	
83c		Swedcom SACP 2x5516	1	116			270	
83d		Swedcom SCE-6016 Rev 2	1	116			270	

Table 2 – Removed Equipment

ID	Pos	Description	Qty	Elev (ft)	Tx Line	Qty	AZ	Comments
77		Hyperlink 3ft dish w/ radome	1	210	Cat 5 Cable	1	39	Computer hospital
78		Proxim 5054R-LR Base panels	2	210	Cat 5 Cable	3	159	Computer hospital

As requested by Communications Site Management, LLC, the current analysis does not include the AT&T’s previously proposed antenna in Structural Analysis Report (Job: 14-0660) by Turriss dated October 9, 2014.

We trust the analysis and recommendations presented in the report will meet your requirements. However, please do not hesitate to contact us if you have any questions, or require any further information regarding this study.



1.0 Terms of Reference

The following documents and drawings were examined:

Tower Profile: Radian dwg. No. 37-1030-E01-01 Rev. 2 dated Jan/10/2005.
Tower Foundations: LeBlanc dwg. No. 3.7A1001-FE10 Issue 2 dated Aug/31/84.
LeBlanc dwg. No. 3.7A1001-FE1 Issue 1 dated May/7/84.
LeBlanc dwg. No. 3.7A1001-FE2 Issue 1 dated May/1/84.
LeBlanc dwg. No. 3.7A1001-FE3 Issue 1 dated Apr/30/84.
LeBlanc dwg. No. 3.7A1001-FE4 Issue 1 dated Apr/30/84.
LeBlanc dwg. No. 3.7A1001-FE5 Issue 1 dated May/1/84.
LeBlanc dwg. No. 3.7A1001-FE6 Issue 1 dated Apr/30/84.
Radian dwg. No. 37-1030-F01-01 Rev. 0 dated Oct/4/2004.
Radian dwg. No. 37-1030-F02-01 Rev. 0 dated Oct/5/2004.
Radian dwg. No. 37-1030-F03-01 Rev. 0 dated Oct/5/2004.
Antenna Inventory: Mapping by Communications Site Management, LLC dated December 22, 2014 and refer to Appendix A.
Soil Report: Dr. Clarence Welti, Geotechnical Engineering
Report dated January 30, 2004

A tower inspection was not performed in conjunction with this analysis. The tower and loading data used in this analysis are based on and is as accurate as the data furnished/obtained.

2.0 Analysis Parameters

- Standard: ANSI/TIA-222-G-2009 Add. 2
- County: Hartford, CT
- Basic Wind Speed: 100.00(mph)
- Basic Wind Speed With Ice: 50.00(mph)
- Design Ice Thickness: 1.00(in)
- Structure Class: II
- Exposure Category: C
- Topographic Category: 1

3.0 Assumptions

1. The tower is in good, non-corroded conditions.
2. The tower and its foundation system have been properly constructed as per the original design drawings and specifications and able to resist the original design loads.
3. This analysis assumes that all previous reinforcing recommendations and antenna rearrangement have been implemented.
4. All existing/future tx lines less than 3" in diameter are considered grouped together in blocks based on an assumed arrangement for this analysis.
5. This analysis assumes that the back-to-back diagonals at sections 6, 7, 12, 13, 19, 20, 21, and 33 had been upgraded with (1) 5/8" stitch bolt on each side of the existing middle stitch bolt.
6. This analysis assumes that the antenna mount at elevation 120' has the structural capacities to support the equipment at elev. 120'.
7. The base foundation was analyzed based on the soil parameters as stated in the original foundation drawing (Dwg. 3.7A1001-FE10 Issue 2). Allowable bearing capacity = 50 ksf.
8. The inner and outer anchors were analyzed based on the following assumed dry soil parameters: density = 130.5 pcf, angle of internal friction = 30 deg, $K_p = 3$.

4.0 Analysis Results

Appendix A shows the tower profile, along with the antennas, transmission lines and ancillary loading considered in this analysis. The existing structure was analyzed using the comprehensive computer program "TSTower". Graphical and tabular results are presented in Appendix B.

5.0 Conclusions & Recommendations

The existing 1339 ft LRM3700 guyed tower at Rattlesnake (aka Farmington), CT, was examined for compliance with American standard ANSI/TIA-222-G-2009 Add. 2. A summary of member stresses are listed below:

Summary of member stress ratios

Member Type	Section	Panel	Member size	Ratio	Comment
Leg	15	2	SR 6	0.96	Acceptable
Diagonal	6	4	2L3x2x1/4	0.64	Acceptable

Summary of base reactions

Axial (Kips)	Shear (Kips)
3623.98	62.1

Base foundation is acceptable.

5.0 Conclusions & Recommendations (Cont'd)

Summary of anchor reactions

Anchor #	Azimuth (deg)	Radius (ft)	Elevation (ft)	Horizontal Load (Kips)	Vertical Load (Kips)	Axial Load (Kips)
1C	39.0	685.00	-25.0	467.32	330.66	527.47
2C	159.0	645.00	-30.0	469.32	353.33	587.46
3C	279.0	729.00	-120.0	458.11	362.29	584.05
1B	39.0	845.00	-140.0	118.84	150.27	191.58
2B	159.0	735.00	-38.0	123.08	161.91	203.38
3B	279.0	827.00	-130.0	121.04	154.94	196.61
1A	39.0	875.00	-150.0	232.85	352.51	422.47
2A	159.0	765.00	-33.0	238.75	377.05	446.28
3A	279.0	857.00	-129.0	233.83	355.71	425.69

Soil report for the inner and outer anchors was not available at the time of study. Foundations were checked against soil conditions as stated in Section 3.0 and were considered to be structurally acceptable. The customer is advised to inform the engineer the actual soil conditions differ than the stated soil conditions. Turriss recommends that a geotechnical report for the base, inner and outer anchors should be made available for future study.

The tower and the waveguide bridge are considered in compliance with ANSI/TIA-222-G-2009 Add. 2 with the assumptions and listed documentations as stated in this report.

Prepared by:

Meimei Lam
Project Designer

Reviewed by:



John Wahba, Ph.D, P.E., P.Eng.
Principal Engineer



Turriss Project: 14-0846
January 26, 2015

SCOPE & LIMITATIONS FOR THE PROVISION OF PROFESSIONAL ENGINEERING SERVICES FOR STRUCTURES

All engineering services performed by Turriss Corp. (Turriss) in connection with the structural analysis of the tower is limited to the strength of the members and does not account for any variations due fabrication, including welding and connection capacities and installations, except as outlined in this Report.

This analysis report is based on assumptions that the information below, but is not necessarily limited to:

- information supplied by the client regarding the structure and its components, foundations, soil conditions, appurtenances loading on the structure, and other site-specific information.
- information from documents and/or drawings in the possession of Turriss Corporation, or acquired from field inspections.

It is the responsibility of the client to ensure that the information provided to Turriss, and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications provided, and are in non-corroded condition and have not deteriorated. Therefore, we assume that the member capacities have not changed from the “as new” condition.

All services will be performed to meet the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed to in writing. If wind and ice loads or other relevant parameters are to be different than the minimum values recommended by the standards, the client shall specify the requirement.

All services are performed in accordance with generally accepted engineering principles and practices. Turriss is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

Furthermore, Turriss assumes no obligations to revise any of the information or conclusions contained in this Report in the event that such engineering and analysis procedures and formulas are hereafter modified or revised. In addition, under no circumstances will Turriss have any obligations or responsibility whatsoever for or on account of consequential or incidental damages sustained by any person, firm or organization as a result of any information or conclusions contained in the report and the maximum liability of Turriss Corp., if any, pursuant to this Report shall be limited to the total funds actually received by Turriss Corp. for preparation of this Report.



Turris Project: 14-0846
January 26, 2015

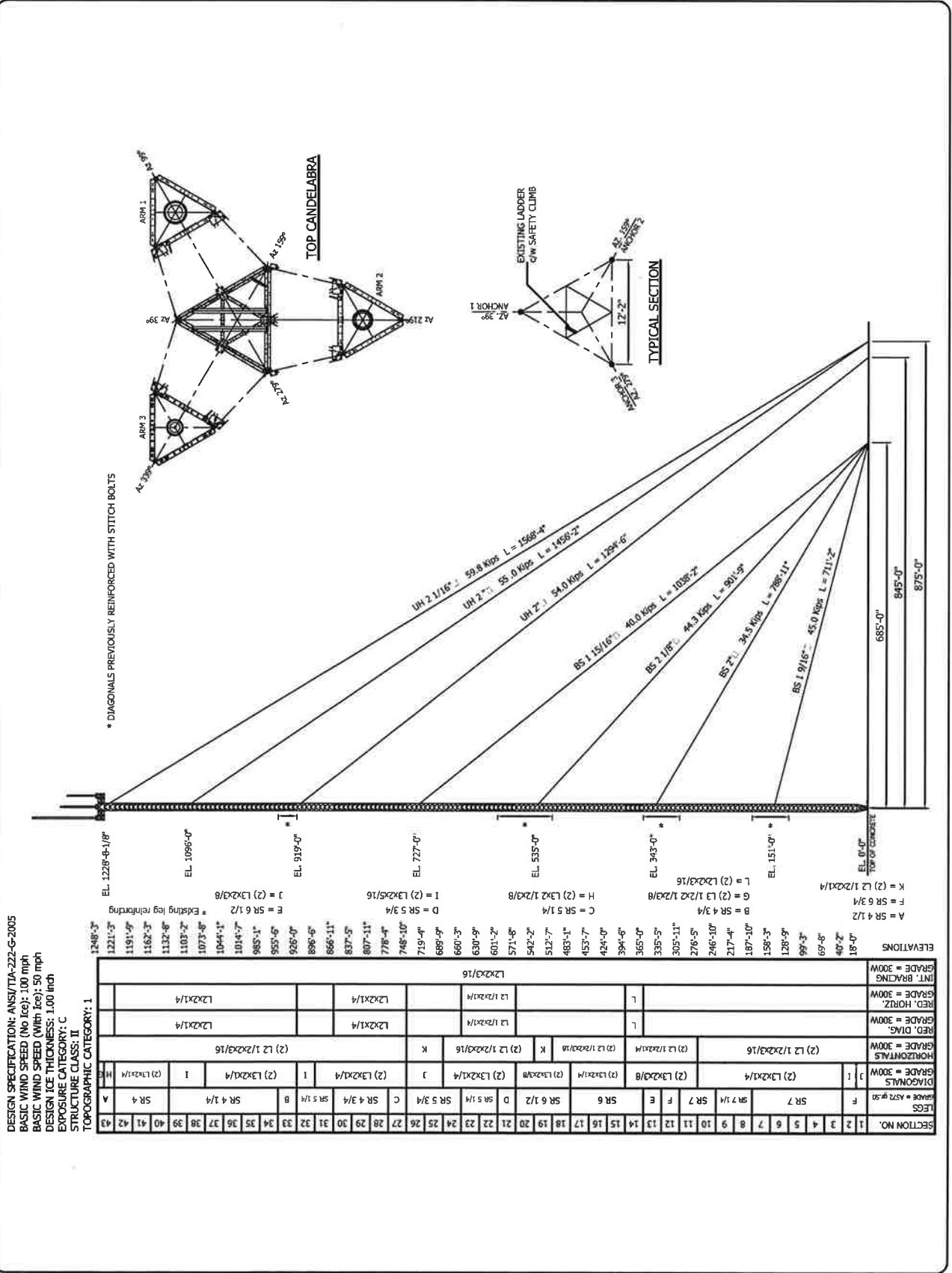
APPENDIX A
Tower Profile and Antenna Loading Chart

REV.	DESCRIPTION	DWN	CHK	APP

DWG REFERENCE	

PROJECT	EXISTING 1339FT URM7700
CLIENT	New Cellular Wireless PCS, LLC
LOCATION	RATTLENAKE (HARTFORD CT), CT

TOWER PROFILE	
DATE	10/11/07
DRAWN BY	JL
CHECKED BY	
SCALE	1:1
PROJECT NO.	14086-PROJ-01

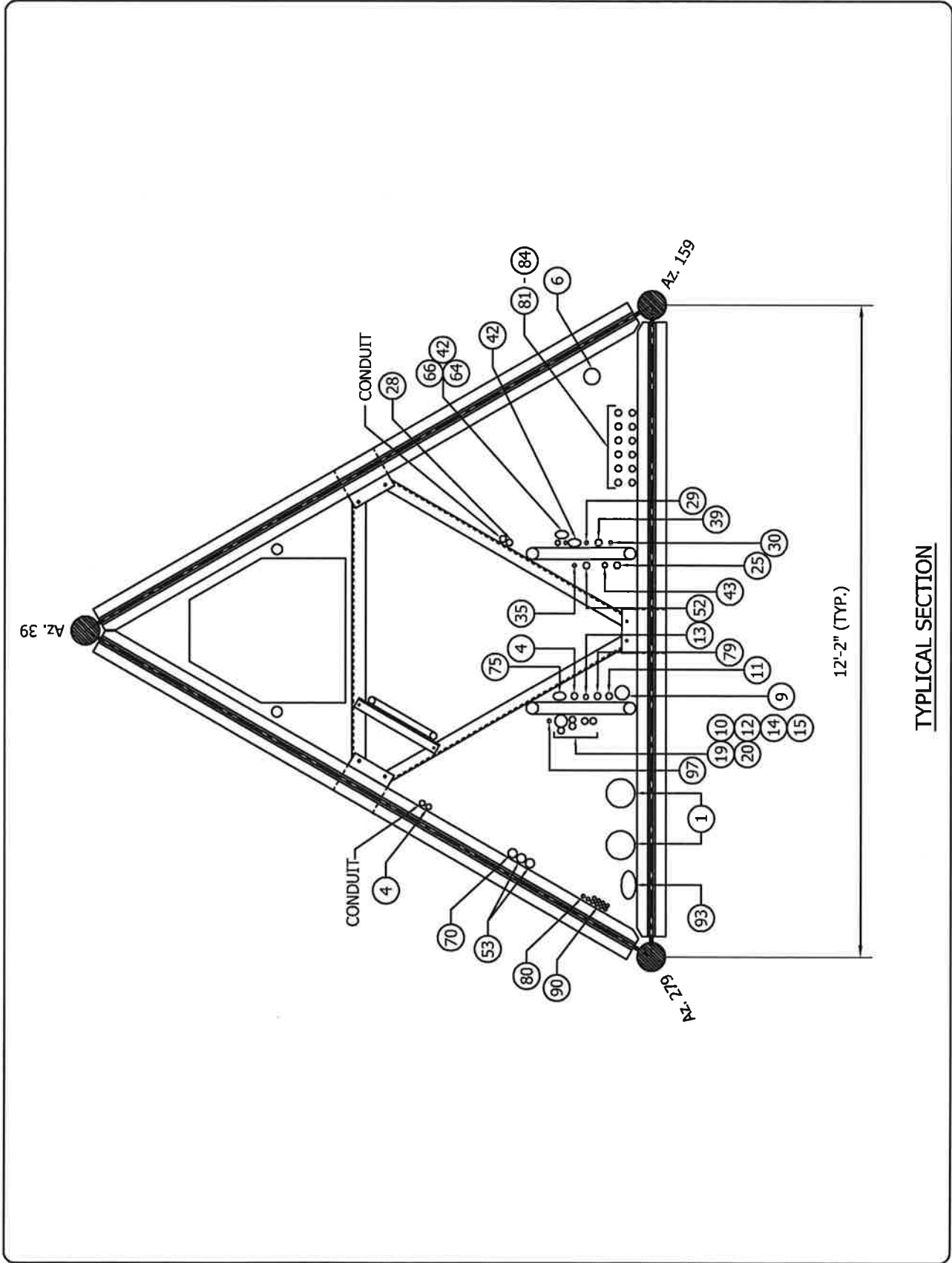


DESIGN SPECIFICATION: ANSII/TIA-222-G-2005
 BASIC WIND SPEED (No Ice): 100 mph
 BASIC WIND SPEED (With Ice): 50 mph
 DESIGN ICE THICKNESS: 1.00 inch
 EXPOSURE CATEGORY: C
 STRUCTURE CLASS: II
 TOPOGRAPHIC CATEGORY: 1

REV.	DESCRIPTION	DWN.	CHK.	APP.

PROJECT	EXISTING 1339FT LRM3700
CLIENT	VERIZON
LOCATION	RATTLESNAKE (HARTFORD CT), CT

TOWER PROFILE			
DATE	SCALE	PROJECT NO.	REV.
2014.03.15	1/8" = 1'-0"	14-0866	0



TYPICAL SECTION



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January 26, 2015

APPENDIX A Antenna Loading Chart

ID	Pos	Description	Qty	Elev (ft)	Tx Line	Qty	AZ	Comments	Status
Top Candelabra loading									
1	2b	TFU-16DSC-R C170	1	1273.00	7 3/16" **	** Shared		East Arm Bottom Up	E
1	3a	TFU-18JTH/VP-R-04	1	1315.55	7 3/16" *	* Shared		NW Arm Top Up	E
1	3b	TFU-18DSC/VP-R C170	1	1266.70	7 3/16" *	* Shared		NW Arm Bottom Up	E
				1248.25	6-1/8"	Spare			F
Miscellaneous loading on tower mast									
4	4	Radio Waves PR09-DRB-2C	1	1221.16	1 5/8" + 1"	1 Each	39	ProscanIII	E
6		TLP24A	1	1096.42	4 1/16"	1	39	Leg mounted	E
9	7	FM ERI-1183-1CP	1	847.81	3 1/2" + 1 5/8"	1	1 Bay Each Face	1 5/8" Future	E
10		DB809-H	1	802.9	3 1/8"	1	39		E
11	9	DB413	1	779.8	1 5/8"	1	39, S Face		E
			1	778.38	1 5/8"	1	39, S Face		E
12	10	DB413	1	761.65	1 5/8"	1	39, S Face		E
			1	756.23	1 5/8"	1	39, S Face		E
13				750	1 1/4"	1			
14	13	ANT150D6-9	1	747.75	1 5/8"	1	159		E
15		DB809-H	1	729.78	1 5/8"	1	39		E
16	14	DB254C	1	716.15	None	None	S Face		E
			1	713.69	None	None	S Face		E
17	15	DB8983P	1	715	None	None	279	Mounted to leg inside tower	E
18	16	DB420B	1	704.63	None	None	39		E
19		DB809K	1	690.47	1 5/8"	1	39		E
20		DB224	1	670.48	1 5/8"	1	39		E
25	25	Scala OGB9-900K	1	519.38	1 5/8"	1	39		E
26	26	Dish Mounts & I/G	3	510.8	None	None	39, 159, 279		E
28	30	BA80-67	1	440.81	1 5/8"	1	39		E
29	32	DB Dipole, 12' Whip	1	410.45	7/8"	1	39		E
30	33	SHPXA-4BC-HW-SP (without radome)	1	384.74	3 1/8"	1	159	WRCH	F
35	51	Scala OGB9-900N	1	323.45	7/8"	1	279		E
36	52	I/G	3	309.11	None	None	1 Each Face		E
39	55	Scala MF-950M	1	304.3	7/8"	1	279		E
42		PXL8	1	285.12	EW 63	2	279		E
43		MF900B	1	271.02	1 1/4"	1	279		E
52	64	BMR 10A	1	174	1 5/8"	1	39		E
53		DB950F65T4E-M	2	160.3	2 1/4"	2	279		E
54		DB950F65T4E-M	1	160	2 1/4"	1	39		F
64	66	PD400	1	120.51	7/8"	1	159		E
66		PD1110	1	107.46	1 1/4"	1	159		E
67	70	Ice Guards	1	99	None	None	39	RELOCATED	E
70		A-18A24	1	73.89	2 1/4"	1	39		E
71		Dish Mount	1	60	None	None	39		E
72		Dish Mount	1	54	None	None	39		E
73		Dish Mount	1	43	None	None	39		E
74	73	Ice Guards	1	40.2	None	None	39		E
75	74	PL6	1	29.05	EW63	1	39		E



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79	Scala PR-950U	1	330.08	1 5/8"	1	159	WJMJ Radio and TV	E
80	Scala PR-950U	1	48.07	7/8"	1	279	WJMJ Radio and TV	E
81a	Swedcom SCE-6016 Rev 2	1	116	1 5/8"	12	0		E
81b	Swedcom SACP 2x5516	1	116			0		E
81c	CSS / X7C-FRO-660-VR4	1	116			0		E
81d	Swedcom SCE-6016 Rev 2	1	116			0		E
83a	Swedcom SCE-6016 Rev 2	1	116			270	2 existing Andrew CB721-DF	E
83b	CSS / X7C-FRO-660-VR4	1	116			270		E
83c	Swedcom SACP 2x5516	1	116			270		E
83d	Swedcom SCE-6016 Rev 2	1	116			270		E
84	Andrew CBC721-DF	2	116				E	
86	12' lightweight T-frame	1	116			0		E
88	12' lightweight T-frame	1	116			270		E
90	Andrew VHLP 2.5 (30")	6	198.71	7/8"	9	39, 159, 279	Two Dishes on Each Leg	E
91	Ice Shield (2'6"x 2'6")	1	37.6	None	None	159		E
92	Camera Sony SNC-RZ50N	1	35.27	Cat 5 cable + RG 6	1+2	159		E
93	TA-2355-DAB-M-T2	1	457.58	EW20	1	39	REUSE EXISTING EW20	E
94	Prodelin VSAT #1183 (1.8m dish)	1	35	RG6-QS	1	Leg 8 of WG Bridge	.298" dia. coax cable	E
95	Trimble GPS Unit #57860-30	1	34	RG6-QS	1	Leg 7 of WG Bridge	.298" dia. coax cable	E
96	Scala MF-950M	1	230	7/8"	1	279	Backup STL	F
97	-	-	217	7/8"	1	-	-	E

Notes:

- 1) All measurements are to the base of the antennas.
- 2) ID 75 line is not attached to the cable ladder. The line goes from the waveguide bridge to the dish.

Verizon's Current Inventory on the Turriss Antenna Loading Chart

**Communications Site Management LLC.
Rattlesnake Mountain Tower**

ID	Pos	Description	Qty	Elev (ft)	Tx Line	Qty	AZ	Comments
81a		Swedcom SCE-6016 Rev 2	1	120	1 5/8"	12	0	
81b		Swedcom SACP 2x5516	1	120			0	
81c		CSS / X7C-FRO-660-VR4	1	120			0	
81d		Swedcom SCE-6016 Rev 2	1	120			0	
82		NA						
83a		Swedcom SCE-6016 Rev 2	1	120			270	2 existing Andrew CBC721-DF
83b		CSS / X7C-FRO-660-VR4	1	120			270	
83c		Swedcom SACP 2x5516	1	120			270	
83d		Swedcom SCE-6016 Rev 2	1	120			270	

Updated: December 11, 2014

**Communications Site Management LLC.
Rattlesnake Tower
Farmington CT.
Date: 12/23/2014**

Tower Changes:

Remove:

ID	Pos	Description	Qty	Elev (ft)	Tx Line	Qty	AZ	Comments	Status
77		Hyperlink 3ft dish w/ radome	1	210'	Cat 5 Cable	1	39	Computer hospital	E
78		Proxim 5054R-LR Base panels	2	210'	Cat 5 Cable	3	159	Computer hospital	E



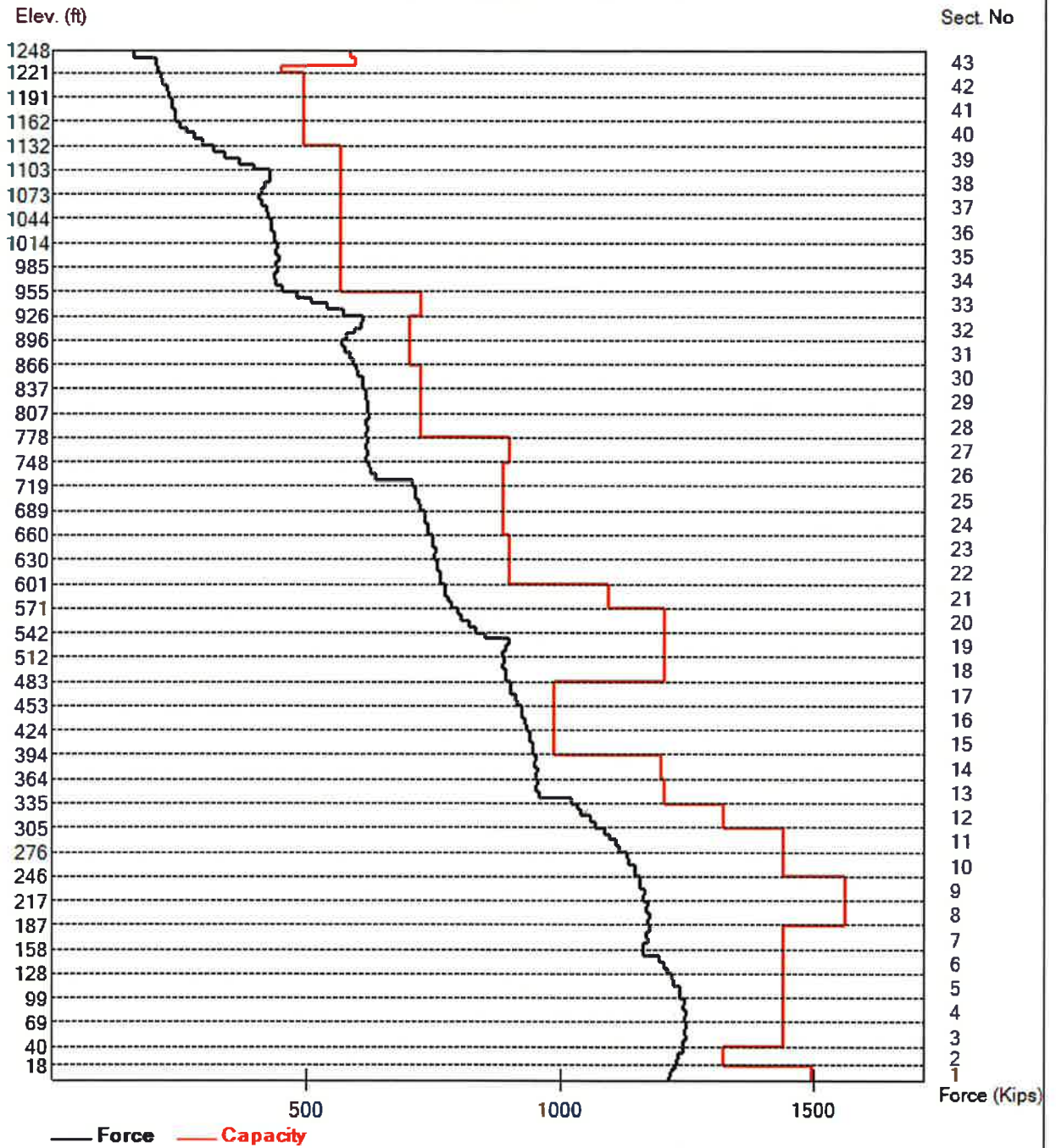
Turriss Project: 14-0846
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APPENDIX B
Results of Analysis

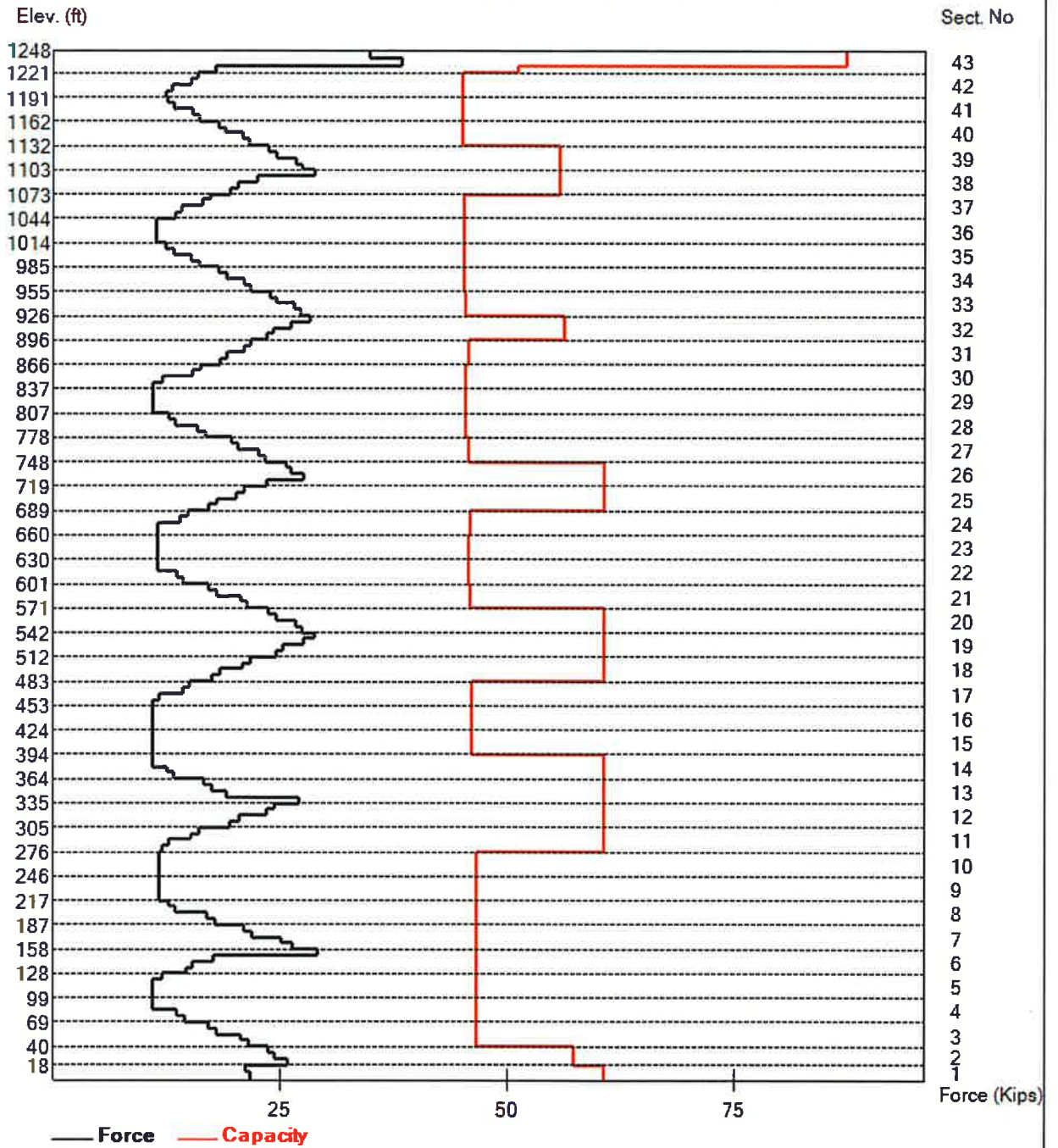
Guy Elevation (ft)	Guy Maximum Stress Levels (% of Rated Capacity)
1228.68	69
1096.00	66
919.00	63
727.00	65
535.00	58
343.00	49
151.00	59

Elevation (ft)	Maximum Beam Rotation (Degrees) for Serviceability Conditions
1221.16	1.08
802.90	0.49
747.75	0.52
729.78	0.52
690.47	0.54
670.48	0.55
410.45	0.57
330.08	0.55
285.12	0.54
198.71	0.44
116.00	0.31
107.46	0.29
48.07	0.20
29.05	0.19

Leg Load Compression Diagram
Max. Envelope (All Loading Cases)



Diag. Load Compression Diagram
Max. Envelope (All Loading Cases)





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Horiz. Load Compression Diagram
Max. Envelope (All Loading Cases)

