

AFF Consulting, LLC  
21 Ridgecrest Drive  
Napa, California 94558  
Tel: 813-220-0077

February 14, 2018

Melanie A. Bachman, Esq.  
Executive Director/Staff Attorney  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

RE: **Notice of Modification Exemption – Facility Modification  
Rattlesnake Mountain (near 200 Colt Highway, Rte. 6) Farmington, CT 06032**

Dear Ms. Bachman,

NBC Telemundo License LLC c/o (“NBC Universal”) currently maintains a single broadcast antenna located on the 1000’ guyed tower of Highway 166 in Farmington, Connecticut (“the property”). The tower is owned by: Outlet Broadcasting Inc c/o NBC Universal, Washington DC 20001. It is unknown if this was previously approved by the Council as it first went on air in 1953. NBC Universal intends to replace the broadcast antenna with a like-for-like shape and size broadcast antenna at the same level on the tower. This is simply a technology change as the Federal Communication Commission (“FCC”) has asked for the frequency currently used to be returned and reassigned to another wireless provider. Included in Attachment 1 are the specifications for NBC Universal’s replacement antenna.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes and exemption pursuant to the R.C.S.A. § 16-50j-72 (b)(2)). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is sent to Kathleen A. Eagen, Town Manager of Farmington, William Warner, Farmington Planning Manager, Outlet Broadcasting Inc, the tower and property owner.

The planned modification to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72 (b)(2).

1. The proposed modifications **will not** result in an increase in height of the existing tower. NBC Universal’s replacement antenna will be installed at the same elevation at the top of the existing tower.

Melanie A Bachman, Esq.

February 14, 2018

Page 2

2. The proposed modifications will not involve any change to ground based equipment as the equipment swap will be like-for-like in the same existing equipment building and, therefore, will not require the extension of the site boundary.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antenna will not increase radio frequency (RF) emissions at the facility to a level at or above the "FCC" safety standards. NO General Power Density study is being provided as the antennas are so far removed from the ground, levels are far below federal or local standards
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The tower and its foundation, with certain modifications described in the Structural Analysis Report included in Attachment 2, can support NBC Universal's proposed modifications.

A copy of the parcel map and owner information for the "Property" is included in Attachment 3. A Certification of mailing verifying that this filing was sent to municipal officials and the owner of the "Property" is included in Attachment 4.

For the foregoing reasons, NBC Universal submits that this proposed modification to the above reference broadcast communications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72 (b)(2).

Sincerely,



Anthony F. Flores

Enclosures

Copy to:

Kathleen A Eagen, Farmington Town Manager  
William Warner, Farmington Planning Manager  
Outlet Broadcasting, Inc., Tower and Property Owner

# ANTENNA MODIFICATIONS AT 200 COLT HIGHWAY FARMINGTON, CT 06032 FOR WVIT TV

ISSUED FOR PERMIT  
FEBRUARY 13, 2018



30 GALESI DRIVE,  
SUITE 202B  
WAYNE, NJ 07470  
(973) 785-4545

CONSULTANTS



1422 New Britain Ave,  
West Hartford, CT 06107

SEAL

## WVIT TV ANTENNA MODIFICATIONS

200 Colt Highway  
Farmington, CT 06032

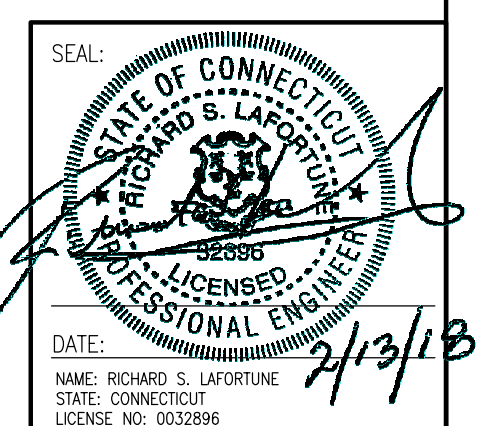
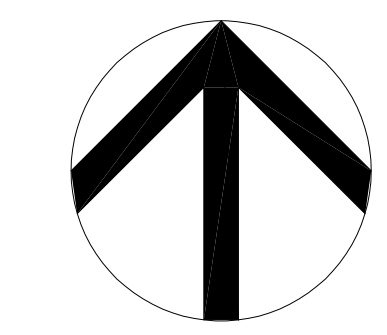
MARK	DATE	DESCRIPTION	
		DWN	DSG   REV

ISSUE: 02/09/2018  
PROJECT NO: 18C0001  
CAD FILE: T-101.00.DWG  
DRAWN BY: HPJ  
CHECKED BY: RSL  
REVIEWED BY: RSL

SHEET TITLE

TITLE SHEET  
LOCATION MAP  
DRAWING LIST

T-101.00  
SHEET 1 OF 6



DRAWINGS LIST		N.T.S.	
DRAWING NO.	SHT. NO.	BUILDING(S) DRAWINGS TITLE	SUBMISSION DATE: (2018)
T-101.00	1	TITLE SHEET LOCATION MAP & DRAWING LIST	PI - - - -
T-001.00	2	GENERAL NOTES	PI - - - -
STRUCTURAL			
S-100.00	3	SITE SURVEY SHEET 1	PI - - - -
S-200.00	4	SITE SURVEY SHEET 2	PI - - - -
S-300.00	5	SITE SURVEY SHEET 3	PI - - - -
S-101.00	6	ANTENNA PLAN AND ELEVATIONS	PI - - - -

**SYMBOLS:**

- NOTATION INDICATING TOP OF FINISHED FLOOR, TOP OF SLAB OR OTHER ELEVATIONS.
- SECTION LETTER/NUMBER SHEET REFERENCE NUMBER
- ELEVATION NUMBER SHEET REFERENCE NUMBER
- PLAN DETAIL SHEET REFERENCE NUMBER
- DISCONTINUITY
- ROOM NAME & NUMBER
- CLOUD AROUND CHANGE DRAWINGS

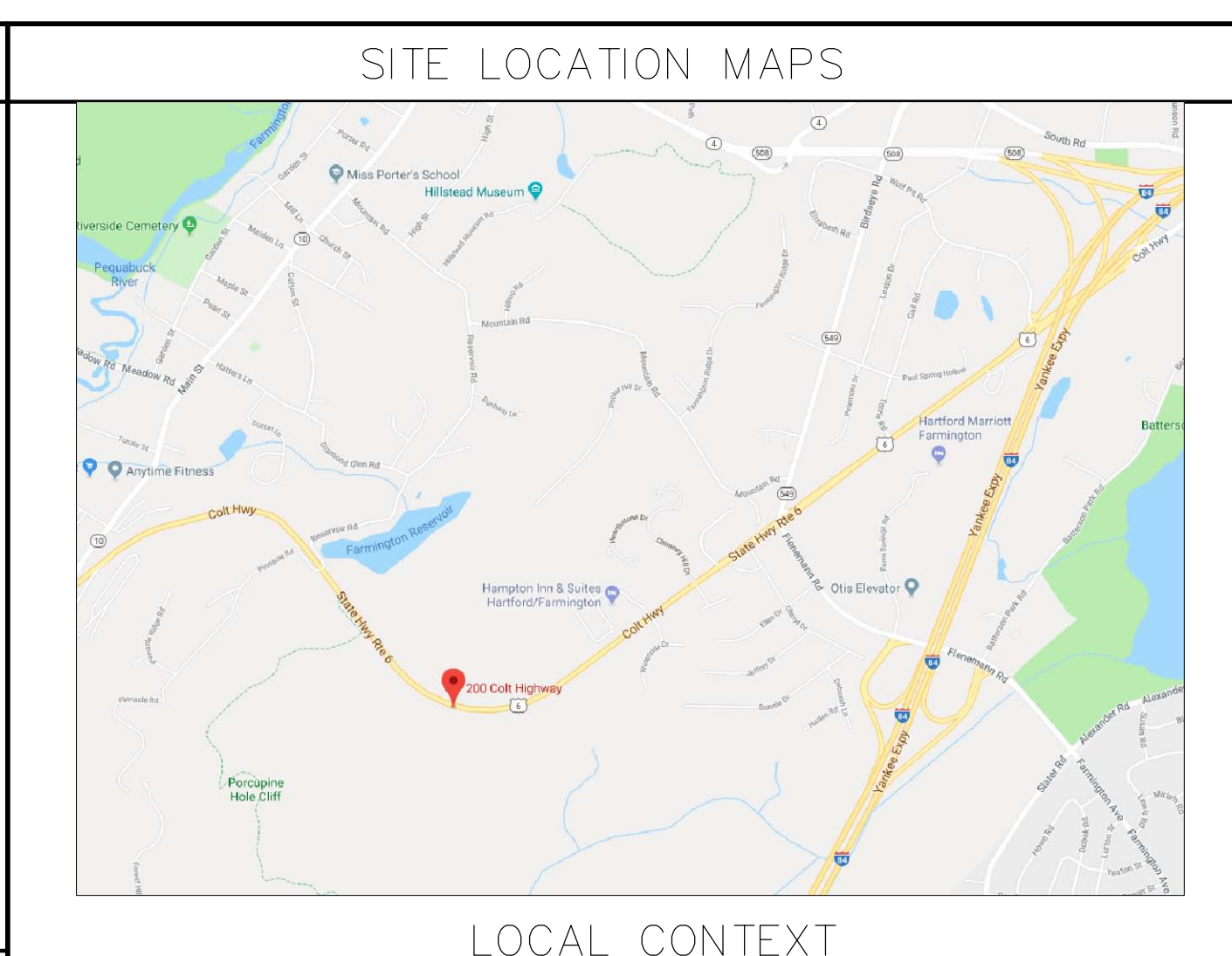
**ABBREVIATIONS:**

A, AMP A/C, AC ACOUST. ADJ. A.F.F. AFC ALUM. APPROX. ARCH. AUTO AWG BD B.O.D. B.O.D.F. B.O.P. BLDG BLK. BTU/HR CB CER. CFM C.J. C.J.O. CKT C CL CLR. CLG CLR. CLG CLR. C CMU CONC. COND. CONF. CONST. CONTIN. CPT. CRAC	AMPERE AIR CONDITIONING ACOUSTICAL ACCESS DOOR ADJUSTABLE ABOVE FINISH FLOOR ABOVE FINISHED GRADE ALUMINUM APPROXIMATE ARCHITECTURAL AUTOMATIC AMERICAN WIRE GAUGE BOARD BOTTOM OF DUCT BOTTOM OF DIFFUSER BOTTOM OF PIPE BUILDING BLOCKING BRITISH THERMAL UNITS PER HOUR CIRCUIT BREAKER CERAMIC CUBIC FEET PER MINUTE CONTROL JOINT CLEAR JAMB OPNG. CIRCUIT CONDUIT CEILING CLEAR CENTER LINE CONC. MAS. UNIT CONC CONDENSED/CONDENSATE CONFERENCE CONSTRUCTION CONTINUOUS CARPET COMPUTER ROOM AIR CONDITIONING CERAMIC TILE CONDENSING UNIT DISCONNECT DEGREE DRAWING DIAMETER DIM. DOWN D.N. DITTO DR. DWG. EXIST. EA. EL. ELEC. EMT ENCL. ENTR. EQ. EXPAN.	EMT EQUIP ESP EXIST EXT EVAP F.D. FIN. FL. FLOOR F.O.C. FT. FTG. FURRR. F F FLOOR FLEX FT G (GND) GA GALV. G.C. GL. GYP. HC HDWE. HT. HM. HORIZ. HVAC HWH HOA HP HZ IFP IN INSUL. INT. INFO. INDIC. INT. JB JAN. JT. KCMIL KW KWH LB LAV. LOC LOUV. LOUV. MACH. MAINT. MATL. MAX MECH	ELEC METAL TUBING EQUIPMENT EXTERNAL STATIC PRESSURE EXISTING EXTERIOR EVAPORATOR FIRE DAMPER FINISH FLOOR FACE OF CONCRETE FOOT OR FEET FOOTING FURRING DEGREE FAHRENHEIT FLOOR FLEXIBLE FEET OR FOOT GROUND GUAGE GALVANIZED GEN. CONTRACTOR GLASS OR GLAZING GYPSUM HANDICAPPED HARDWARE HEIGHT HOLLOW METAL HORIZONTAL HEATING, VENTING & AIR CONDITIONING HOT WATER HEATER HAND-OFF-AUTO HORSEPOWER HERTZ ISSUED FOR REVIEW ISSUED FOR PERMIT INCH OR INCHES INSULATION INTERIOR INFORMATION INDICATOR INTERNAL JUNCTION BOX JANITOR JOINT KILO (1000 CIRC. MILLS) KILOWATT KILOWATT HOUR POUND LAVATORY LOCATION LOUVER MACHINE MAINTENANCE MATERIAL MAXIMUM MECHANICAL	M MIN MM MOIST. MTL. MFT. M.F. M.O. MOUNTED MUL. MILLION NM. NC NORMALY CLOSED NATIONAL ELEC CODE NIC NOT IN CONTRACT NO NORMALLY OPEN NTS NOT TO SCALE #/C MULTI-COND. CABLE OBD OPPOSED BLADE DAMPER O.C. O.A.I. OFF. O.H. OPG. OPENING P PART. PAVE. PAVE. PDU PL. PLATE PMT PVC. PH PHASE PNL PANEL PWR R R.A.G. R.A.G. RAD. RADIUS REBAR. REF. REFL. REFL. REIN. REINFORCED REQD. REQUIRED RGS RIGID GALVANIZED STL RM ROOM S.A. SUPPLY AIR SWBD SWGB SWGB SWGR SW SYM SYMMETRICAL TEMP TEMPERATURE TYP TYPICAL UNON UNLESS OTHERWISE NOTED UTIL UTILITY W.W.F. WELDED WIRE FABRIC V VOLT VINYL BASE VCT TILE VINYL COMPOSITION W.G. WATER GAUGE XMFR TRANSFORMER
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**PROJECT DESCRIPTION**

**THIS PROJECT CONSISTS OF:**

1. MOBILIZATION OF TOWER CREW TO THE PROJECT SITE.
2. RIGGING OF TOWER.
3. PERFORM ALL REQUIRED TOWER SHAFT MODIFICATION.
4. PERFORM ALL TOWER BASE MODIFICATIONS.
5. REMOVE EXISTING TOP MOUNTED DIELECTRIC MODEL TFU-28G BROADCAST ANTENNA.
6. INSTALL NEW TOP MOUNTED DIELECTRIC NEW TOP MOUNTED MODEL TFU-20ETT / VP-R 06 BROADCAST ANTENNA.
7. COMPLETION OF ALL WORK IN ACCORDANCE WITH DESIGN DRAWINGS BY STAINLESS DATED NOVEMBER 30, 2017 (DESIGN DRAWINGS 1106.0' GUYED G7 TOWER, NEW BRITAIN, CT).
8. DE-RIG AND DEMOBILIZE FROM SITE.



**PROJECT TEAM**

**ARCHITECT/ENGINEER:**  
EJC INTERNATIONAL SERVICES  
30 GALESI DRIVE,  
SUITE 202B  
WAYNE, NJ 07470  
CONTACT: RICHARD LaFORTUNE  
(973) 785-4545  
EMAIL: rlafortune@tse-ejc.com

**ENTITLEMENTS & PERMITTING:**  
AFF CONSULTING, LLC  
CONTACT: ANTHONY FLORES  
TEL: (813) 220-0077  
EMAIL: aflores@affconsultingllc.com

**STRUCTURAL ENGINEER**  
STAINLESS  
200 NORTH WARNER ROAD  
SUITE 215  
KING OF PRUSSIA, PA  
CONTACT: DENNIA ABEL

**APPLICANT/LESSEE:**  
WVIT TV Hartford  
WVIT TV-NBC Connecticut  
1422 New Britain Ave.  
West Hartford, CT 06110  
CONTACT: JAMES MOYER  
EMAIL: james.moyer@nbcuni.com

**CONSTRUCTION MANAGER:**  
WVIT TV Hartford  
WVIT TV-NBC Connecticut  
1422 New Britain Ave.  
West Hartford, CT 06110  
CONTACT: JAMES MOYER  
EMAIL: james.moyer@nbcuni.com

**CODE COMPLIANCE**

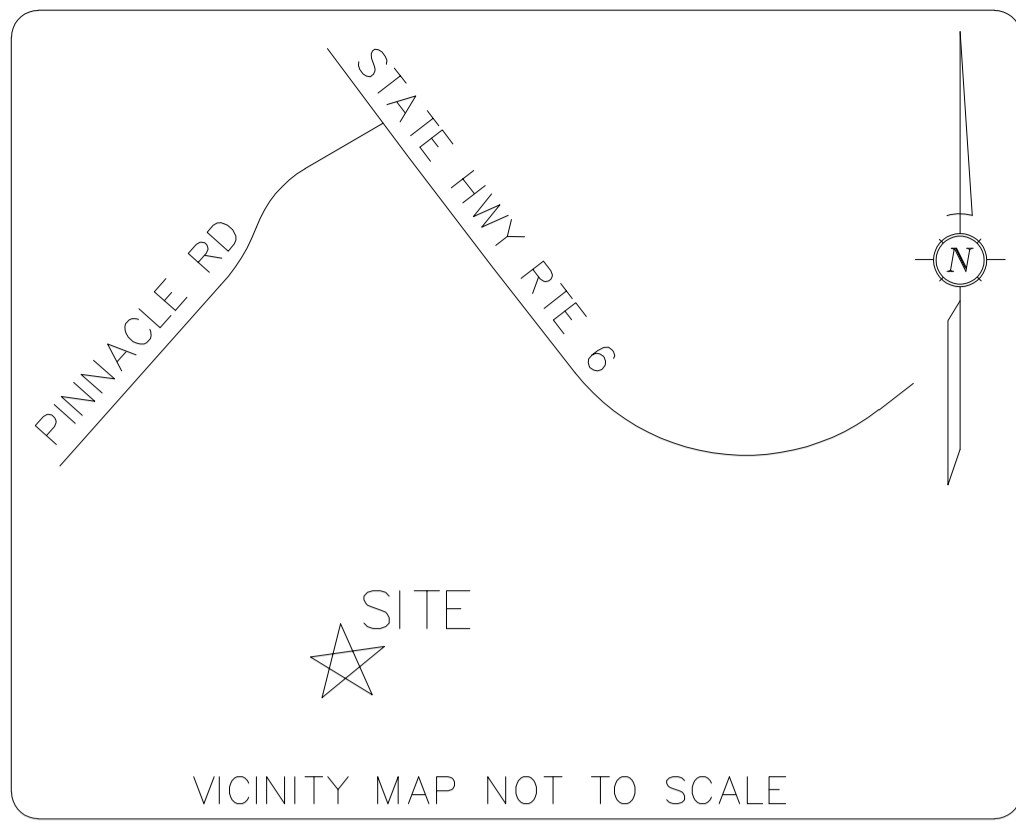
PURSUANT TO CONNECTICUT GENERAL STATUTE 29-252, AS AMENDED BY PUBLIC ACT 16-215, THE FOLLOWING NATIONAL MODEL CODES, AS AMENDED HEREIN, ARE ADOPTED AND SHALL BE KNOWN AS THE 2016 CONNECTICUT STATE BUILDING CODE.

2012 INTERNATIONAL BUILDING CODE  
2009 ICC/ANSI A117.1 ACCESSIBLE AND USABLE BUILDINGS AND FACILITIES  
2012 INTERNATIONAL EXISTING BUILDING CODE  
2012 INTERNATIONAL PLUMBING CODE  
2012 INTERNATIONAL MECHANICAL CODE  
2012 INTERNATIONAL ENERGY CONSERVATION CODE  
2012 NFPA 70, NATIONAL ELECTRICAL CODE, OF THE NATIONAL FIRE PROTECTION ASSOCIATION INC.  
2012 INTERNATIONAL RESIDENTIAL CODE OF THE INTERNATIONAL CODE COUNCIL INC.  
ANSI/TIA 222 REV G TOWER STANDARD AND THE ANSI/TIA 322 RIGGING STANDARD

ISSUED FOR PERMIT

FEB 13, 2018 5:42 PM PLMCKSON  
I:\80805\18C0001\_WVIT\_TV\_HARTFORD\_CT\CADD\GEN\TITLE-T-101.00.DWG





**PARENT PARCEL INFORMATION**  
 OWNER: OUTLET BROADCASTING INC  
 200 COLT HWY RTE 6, FARMINGTON, CT  
 DEED BOOK 554 PAGE 608  
 PARCEL NO 141 7B

ZONING: R80 (RESIDENTIAL)

THIS PARCEL OF LAND LIES WITHIN FLOOD ZONE X WHICH IS NOT A SPECIAL FLOOD HAZARD AREA AS PER F.I.R.M. PANEL NUMBER: 09003C0479F EFFECTIVE DATE: September 26, 2008

**LEGEND**

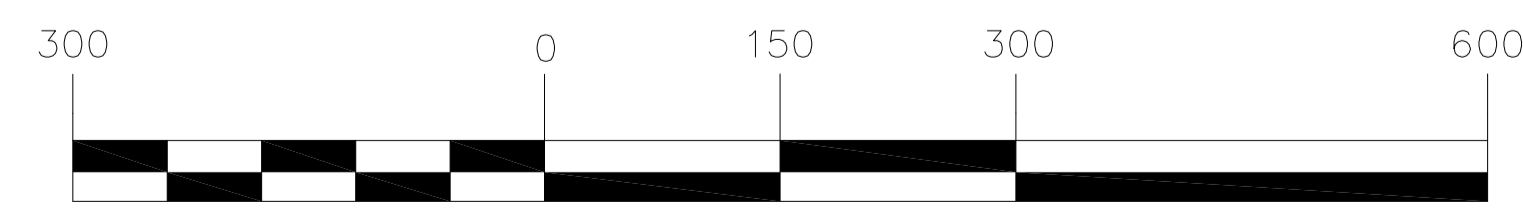
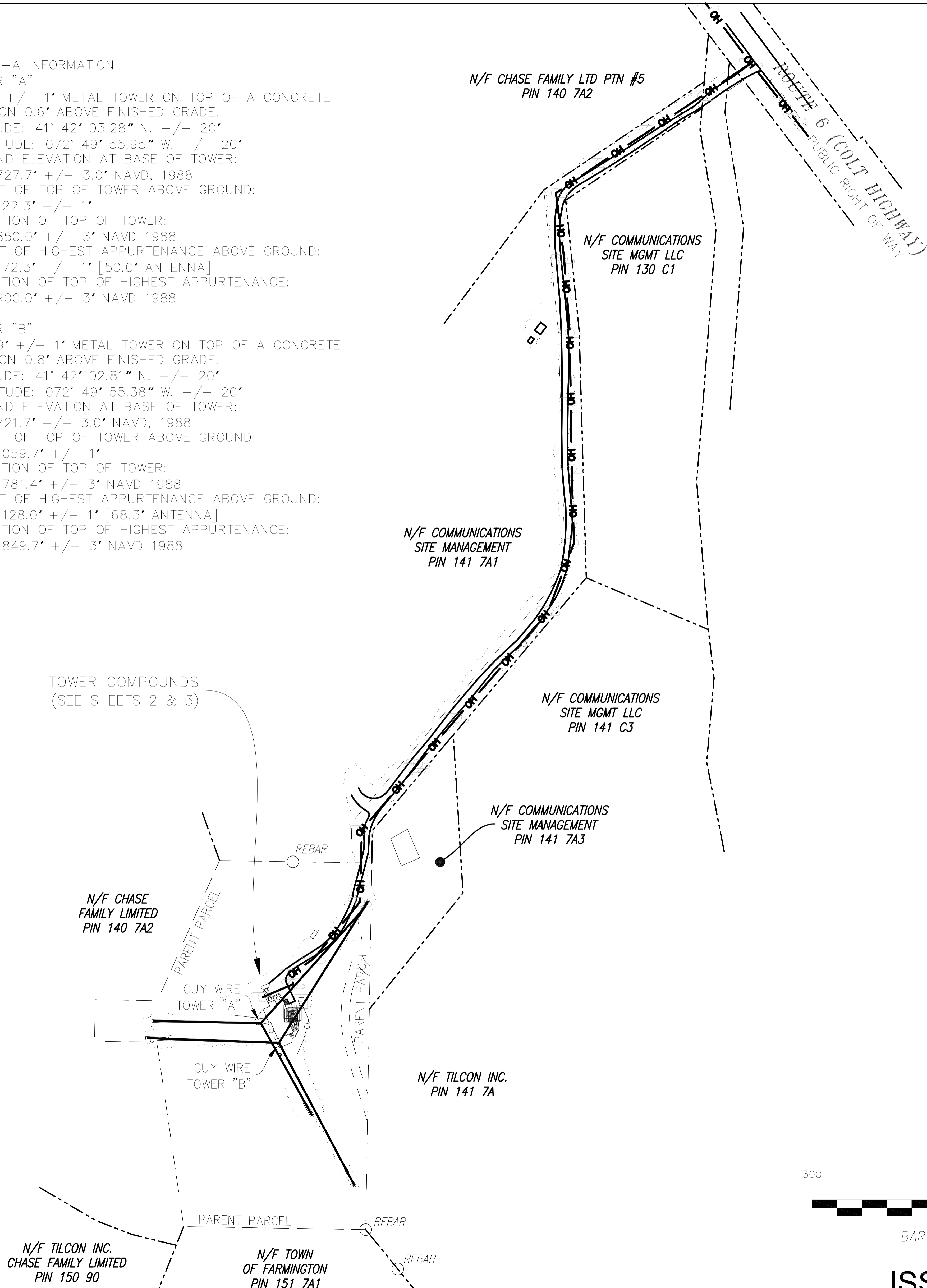
- :SET 5/8" REBAR, OR AS NOTED.
- :FOUND 1/2" REBAR, OR AS NOTED.
- :FOUND MONUMENT, OR AS NOTED.
- (---) :RECORD DESCRIPTION DATA.
- P.O.T. :POINT OF TERMINUS.
- P.O.B. :POINT OF BEGINNING.
- P.O.C. :POINT OF COMMENCEMENT.
- :FENCE AS NOTED.
- OH— :OVER HEAD UTILITY LINES.
- :WOOD UTILITY POLE.
- :ELECTRIC TRANSFORMER.
- ⊞ :TELCO PEDESTAL.
- ⊕ :HAND HOLE.
- N/A :NOT AVAILABLE
- ▼ :FLOOD LIGHT

AREA	SQUARE FEET	ACRE
PARENT PARCEL	435600	10
TOWER EASEMENT	TBD	TBD
TOWER COMPOUND		
ACCESS/UTILITY EASEMENT	TBD	TBD

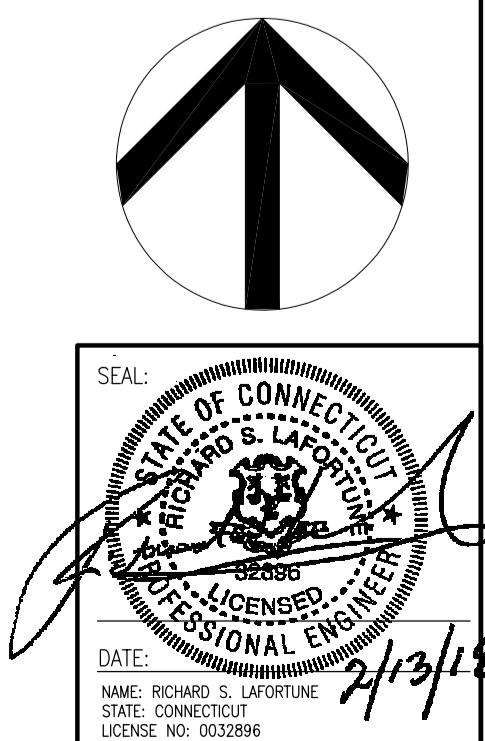
**FAA 1-A INFORMATION**  
**TOWER "A"**  
 121.7' +/- 1' METAL TOWER ON TOP OF A CONCRETE CAISSON 0.6' ABOVE FINISHED GRADE.  
 LATITUDE: 41° 42' 03.28" N. +/- 20'  
 LONGITUDE: 072° 49' 55.95" W. +/- 20'  
 GROUND ELEVATION AT BASE OF TOWER:  
 727.7' +/- 3.0' NAVD, 1988  
 HEIGHT OF TOP OF TOWER ABOVE GROUND:  
 122.3' +/- 1'  
 ELEVATION OF TOP OF TOWER:  
 850.0' +/- 3' NAVD 1988  
 HEIGHT OF HIGHEST APPURTENANCE ABOVE GROUND:  
 172.3' +/- 1' [50.0' ANTENNA]  
 ELEVATION OF TOP OF HIGHEST APPURTENANCE:  
 900.0' +/- 3' NAVD 1988

**TOWER "B"**  
 1058.9' +/- 1' METAL TOWER ON TOP OF A CONCRETE CAISSON 0.8' ABOVE FINISHED GRADE.  
 LATITUDE: 41° 42' 02.81" N. +/- 20'  
 LONGITUDE: 072° 49' 55.38" W. +/- 20'  
 GROUND ELEVATION AT BASE OF TOWER:  
 721.7' +/- 3.0' NAVD, 1988  
 HEIGHT OF TOP OF TOWER ABOVE GROUND:  
 1059.7' +/- 1'  
 ELEVATION OF TOP OF TOWER:  
 1781.4' +/- 3' NAVD 1988  
 HEIGHT OF HIGHEST APPURTENANCE ABOVE GROUND:  
 1128.0' +/- 1' [68.3' ANTENNA]  
 ELEVATION OF TOP OF HIGHEST APPURTENANCE:  
 1849.7' +/- 3' NAVD 1988

TOWER COMPOUNDS  
 (SEE SHEETS 2 & 3)



**ISSUED FOR PERMIT**



NATIONAL SURVEY SERVICES COORDINATION BY:

**GEOLINE SURVEYING, INC.**

13430 NW 104th Terrace, Suite A Alachua, FL 32615  
 Office:(386) 418-0500 Fax:(386) 462-9986  
 WWW.GEOLINEINC.COM

SURVEY WORK PERFORMED BY:

**JONATHAN MURPHY**

**Professional Land Surveying**

10505 Leafwood Place (919) 280-8189  
 Raleigh NC 27613 FAX 995-9616  
 E-MAIL : raleigh@murphygeomatics.com FIRM C-2757

**SURVEYOR'S NOTES**

1. BASIS OF BEARING:  
CT GRID NAD83
2. NO SUBSURFACE INVESTIGATION WAS PERFORMED TO LOCATE UNDERGROUND UTILITIES. UTILITIES SHOWN HEREON ARE LIMITED TO AND ARE PER OBSERVED EVIDENCE ONLY.
3. THIS SURVEY DOES NOT REPRESENT A BOUNDARY SURVEY OF THE PARENT PARCEL.
4. ALL VISIBLE TELECOM EQUIPMENT AND IMPROVEMENTS ARE CONTAINED WITHIN THE DESCRIBED AREA.
5. ALL SYMBOLS SHOWN HEREON NOT DEPICTED TO SCALE.



30 GALESI DRIVE,  
 SUITE 202B  
 WAYNE, NJ 07470  
 (973) 785-4545

CONSULTANTS



1422 New Britain Ave,  
 West Hartford, CT 06107

SEAL

**WVIT TV ANTENNA MODIFICATIONS**

200 Colt Highway  
 Farmington, CT 06032

MARK	DATE	DESCRIPTION		
		DWN	DSG	REV

ISSUE: 02/09/2018  
 PROJECT NO: 18C0001  
 CAD FILE: S-100.00.DWG  
 DRAWN BY: HPJ  
 CHECKED BY: RSL  
 REVIEWED BY: TC

SHEET TITLE  
**SITE SURVEY SHEET 1**

S-100-00

SHEET 3 OF 6

1

2

3

4

5

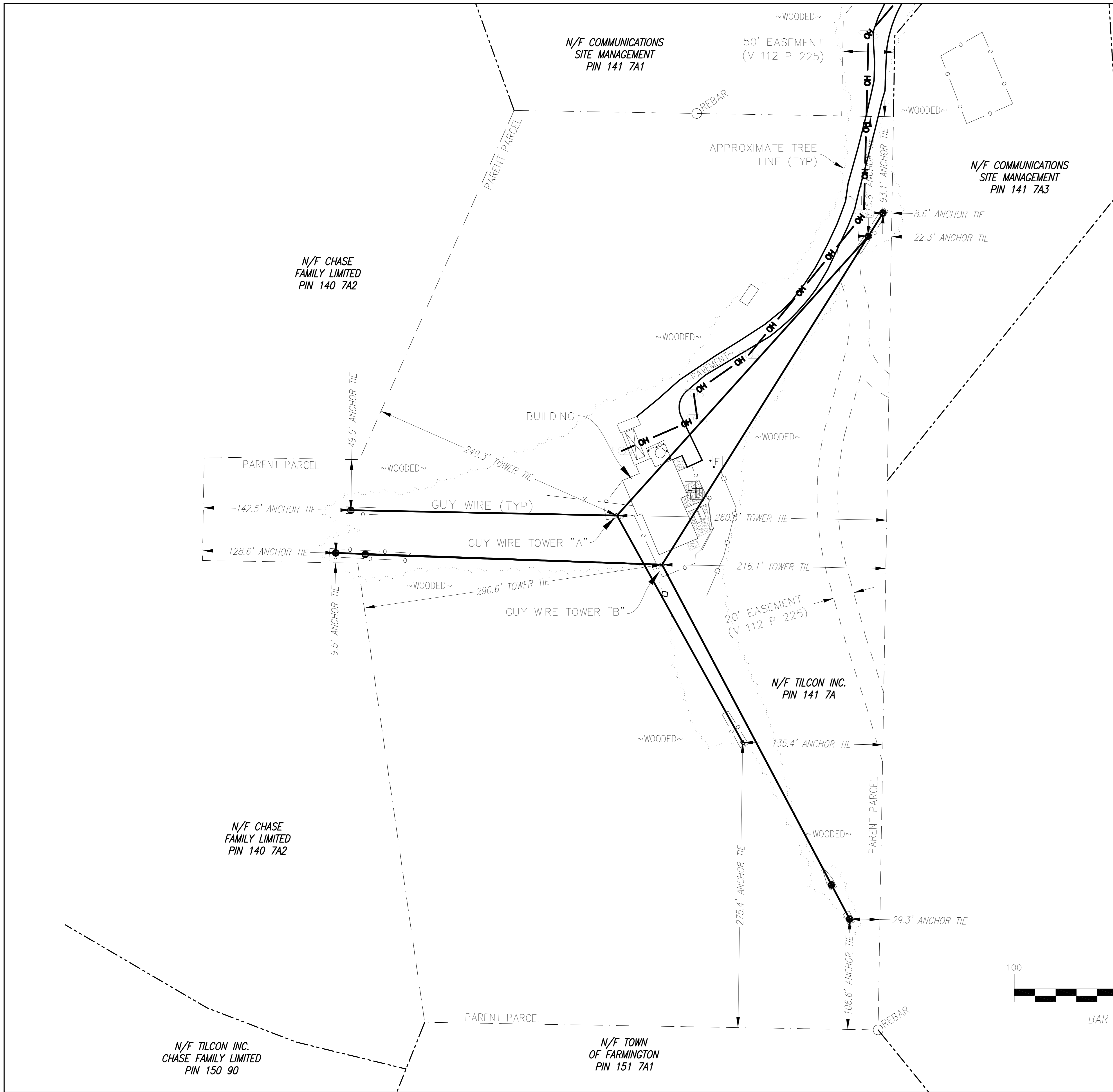
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NATIONAL SURVEY SERVICES COORDINATION BY:

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SURVEY WORK PERFORMED BY:

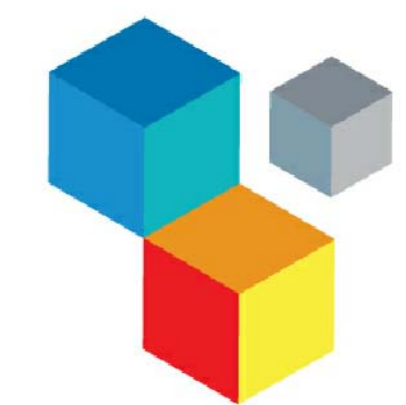
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**EJC**  
INTERNATIONAL SERVICES, LLC

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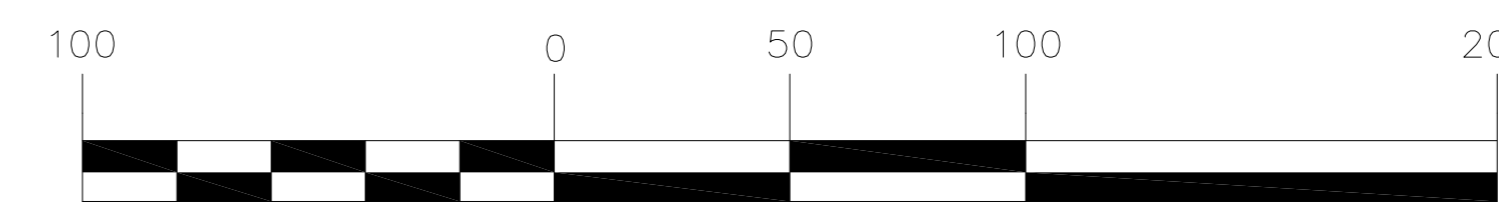
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 PROJECT NO: 18C0001  
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 DRAWN BY: HPIJ  
 CHECKED BY: RSL  
 REVIEWED BY: TC

SHEET TITLE  
**SITE SURVEY  
SHEET 2**

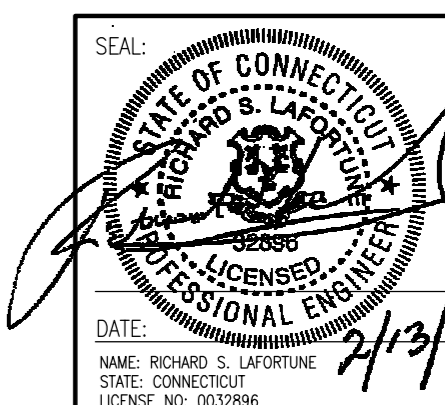
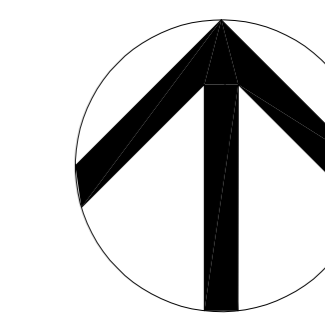
S-200-00

SHEET 4 OF 6



BAR GRAPH 1 inch = 100 ft.  
S-100.00.DWG

**ISSUED FOR PERMIT**



1

2

3

4

5

D

C

B

A

ELECTRICAL APPARATUS

CENTERLINE FOR SCHEDULED

GAS TANK

~PAVEMENT~

CONC PAD

BOLLARD (TYP)

GUARD RAIL (TYP)

~BUILDING~

~HVAC~

CONC PAD

GUY WIRE TOWER "B"

~WOODED~

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**EJC**  
INTERNATIONAL SERVICES, LLC

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ANTENNA  
MODIFICATIONS

200 Colt Highway  
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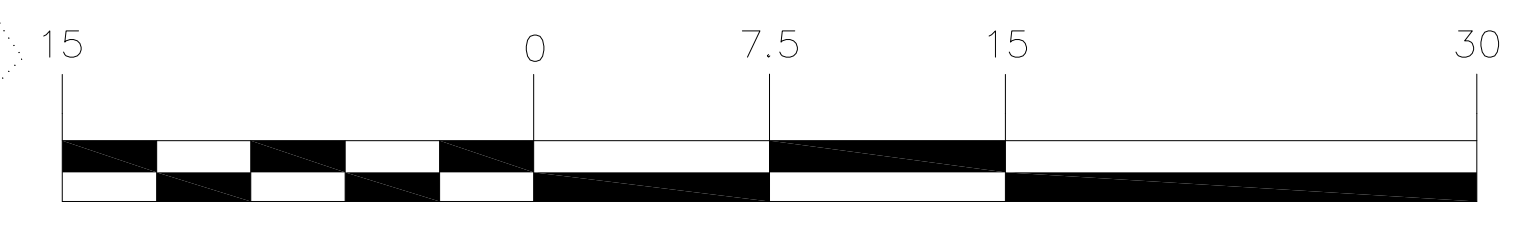
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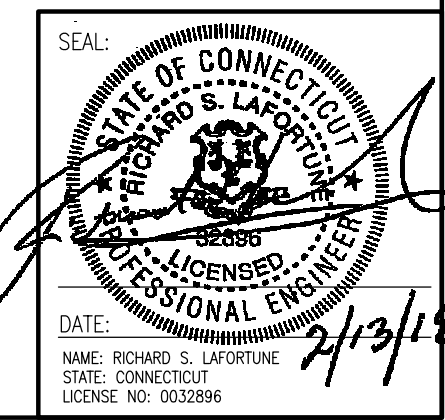
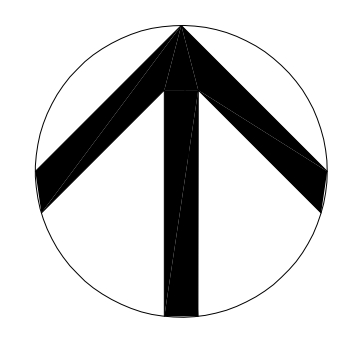
SHEET TITLE  
SITE SURVEY  
SHEET 3

S-300-00  
SHEET 5 OF 6

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BAR GRAPH 1 inch = 15 ft.  
S-100.00.DWG  
**ISSUED FOR PERMIT**

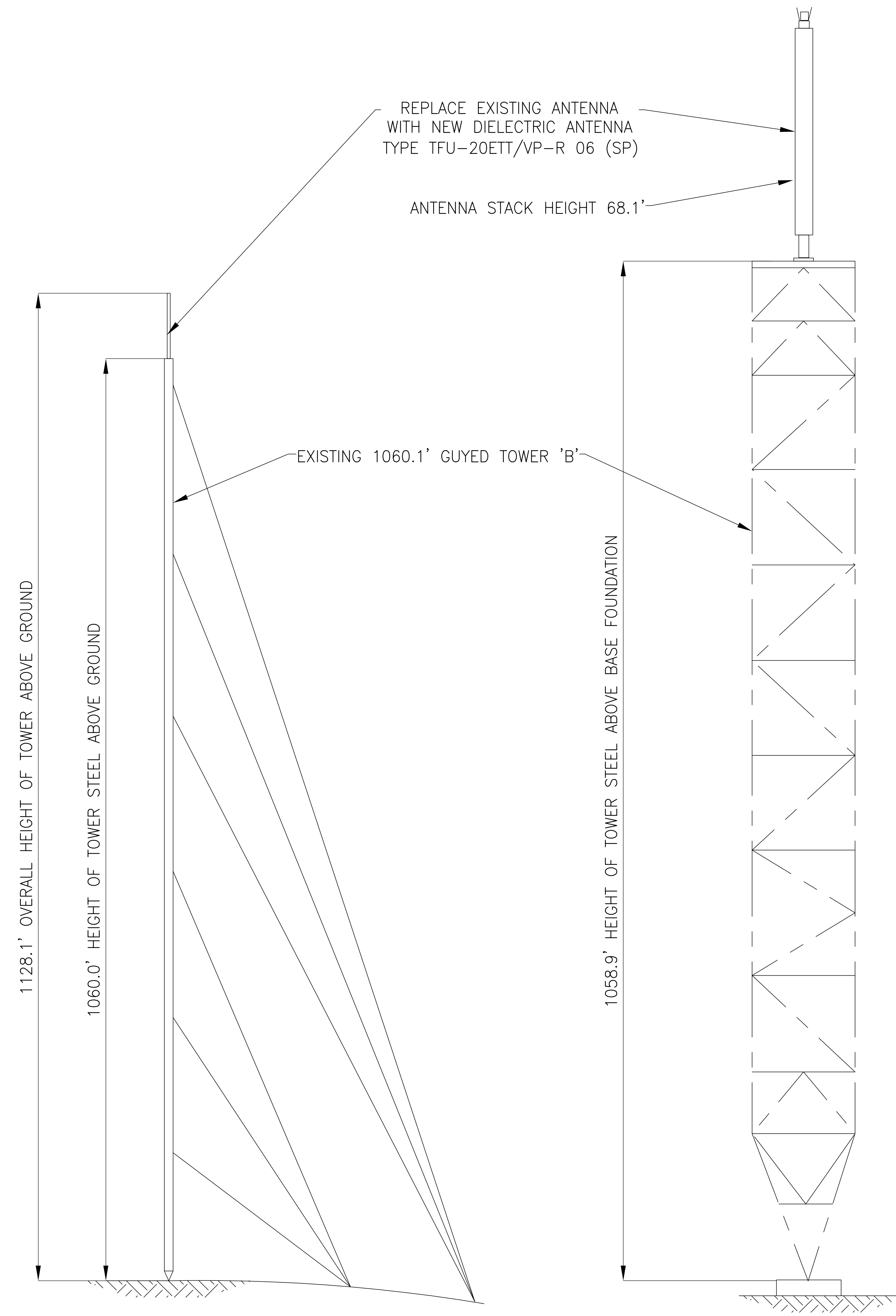


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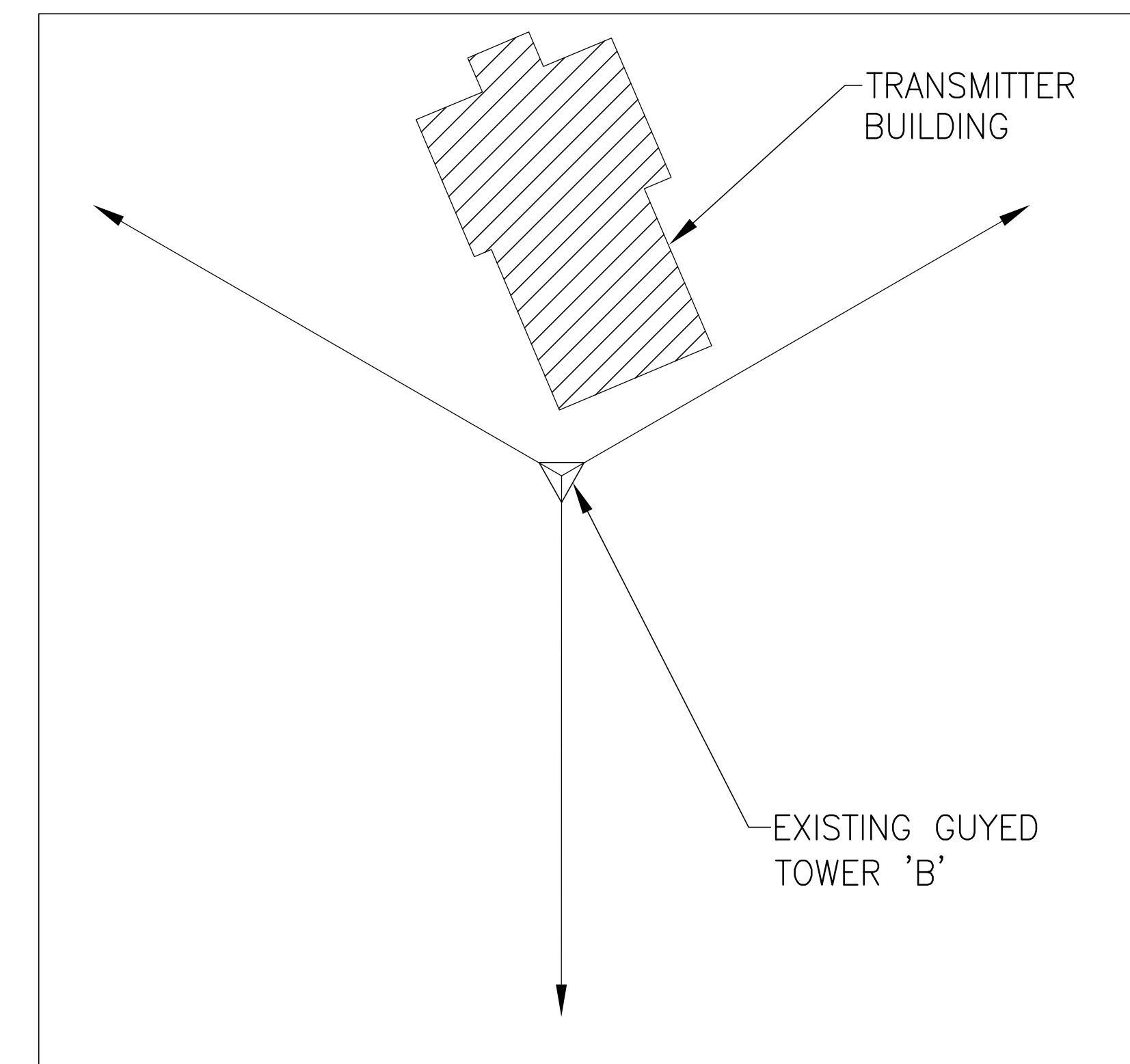
B

A



1 TOWER ELEVATION  
S-101.00  
NTS

2 TOWER ENLARGED ELEVATION  
S-101.00  
NTS



3 TOWER 'B' PLAN VIEW  
S-101.00  
NTS

**Dielectric** TFU-20ETT/VP-R 06

Proposal Number: C-70211-7  
Date: 15-Feb-17  
Customer: NBC Universal  
Location: New Britain, CT

**Electrical Specifications**

Polarization	Elliptical
Antenna Input	6-1/8" 75 Ohm EIA/DCA
VSWR	Channel 1.08 : 1
Bandwidth	6 MHz
Rated Input Power	50 kW (16.99 dBk) Maximum Average Power

**Mechanical Specifications**

Mounting	Bottom of a Stack
Environmental Protection	Full Radome
Height	39.1 ft (11.9m)
Weight	8900 lb (30)
Effective Projected Area	42.7 ft² (4m²) TIA-222-G Basic Wind Speed 97 m/h (156.1 km/h)

**Channel Specifications**

Call	CH	Freq	Hpol ERP	Vpol ERP	TPO	RMS Main Lobe Hpol Gain	RMS Main Lobe Vpol Gain	RMS at Horizontal Hpol Gain	RMS at Horizontal Vpol Gain
WVIT	31	57.6 MHz	374.0 kW (25.73 dBk)	187.0 kW (22.72 dBk)	38.0 kW (15.80 dBk)	13.07 (11.16dB)	6.53 (8.15dB)	8.79 (9.44dB)	4.39 (6.43dB)

Andre J Skalina  
I have reviewed this document  
16:11:44 2018.02.06  
90105

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**Dielectric**

**MECHANICALS**

Proposal No. C-70211-7  
Date 15-Feb-17  
Call Letters WVIT 31  
Frequency 57.6 MHz  
Antenna Type TFU-20ETT/VP-R 06

**Preliminary Specifications**

**Top Mounted**

**Mechanical Specification without ice TIA-222-G**

Height AGL(z)	1057.4 ft (322.3 m)
Basic Wind Speed	97 m/h (156.1 km/h)

Structure Class II  
Exposure Category C  
Topography Category 1

**Mechanical Specifications with ice TIA-222-G**

Design Ice	1 in. $t_{ice} = 2.80$ in
Wind Speed w/ice	50 m/h (90.5 km/h)

**Mechanical Specifications**

	without ice	with ice	full stack	full stack with ice
Height with Lightning Protector	H4 39.1 ft (11.9m)		68.1 ft (20.8m)	
Height less Lightning Protector	H2 19.55 ft (6m)		64.1 ft (19.5m)	
Height of Center of Radiation	H3 19.55 ft (6m)		19.55 ft (6m)	
Effective Projected Area	(EPAG) 42.7 ft² (4m²)	107.2 ft² (10m²)	61.1 ft² (5.7m²)	190.5 ft² (17.7m²)
Moment Arm	D1 19.5 ft (5.9m)	19.5 ft (5.9m)	25.1 ft (8.9m)	32.8 ft (9.9m)

Weight W 6800 lb (30) 10800 lb (4.8) 8750 lb (40) 13700 lb (6.2)

Antenna designed in accordance with AISC specifications for design of structural steel as prescribed by TIA-222-G

Prepared by: KLP Date: 15-Feb-17 ME: SPJC EE:  
Rev. No.7 by: SPJC Date: 2-Feb-18

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WAYNE, NJ 07470  
(973) 785-4545

CONSULTANTS



1422 New Britain Ave,  
West Hartford, CT 06107

SEAL

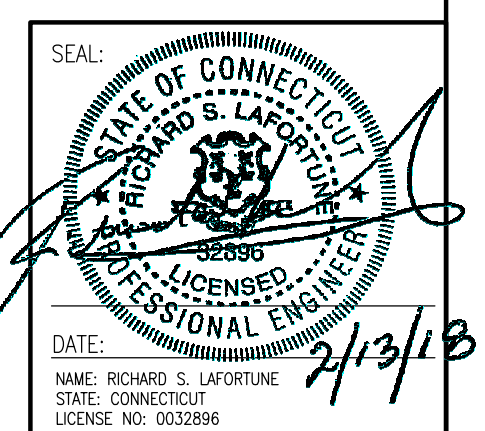
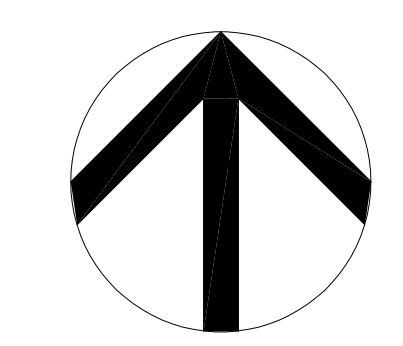
WVIT TV ANTENNA MODIFICATIONS

200 Colt Highway  
Farmington, CT 06032

MARK	DATE	DESCRIPTION		
		DWN	DSG	REV

ISSUE: 02/09/2018  
PROJECT NO: 18C0001  
CAD FILE: S-101.00.DWG  
DRAWN BY: HPJ  
CHECKED BY: RSL  
REVIEWED BY: RSL

SHEET TITLE  
ANTENNA PLAN AND ELEVATIONS



ISSUED FOR PERMIT

S-101.00  
SHEET 6 OF 6





A BUSINESS OF FDH VELOCITEL  
 200 North Warner Road, Suite 215  
 King of Prussia, PA 19406

# DESIGN DRAWINGS

## 1106.0' GUYED G7 TOWER

### NEW BRITAIN, CT

#### INDEX

DESCRIPTION	DWG	REV	DATE	DESCRIPTION	DWG	REV	DATE
GENERAL ARRANGEMENT	D01.00		11/8/2017	BASE ASSEMBLY SUB-BRACING DETAILS	D05.01		11/8/2017
GENERAL NOTES	D01.01		11/8/2017	SUB-BRACING DETAILS	D05.02		11/8/2017
GENERAL NOTES	D01.02		11/8/2017	DIAGONAL REPLACEMENT	D05.03		11/8/2017
BASE FOUNDATION MODIFICATION	D02.00		11/8/2017	VERTICAL LEG REINFORCEMENT DETAILS	D05.04		11/8/2017
FOUNDATION NOTES	D03.00		11/8/2017	VERTICAL LEG REINFORCEMENT DETAILS	D05.05		11/8/2017
TOWER PROFILE	D04.00		11/8/2017	TOP ANTENNA ADAPTER PLATE DETAILS	D05.06		11/8/2017
TOWER PROFILE	D04.01		11/8/2017	COAX ARRANGEMENT	D06.00		11/8/2017
LINEAR APPURTENANCES	D05.00		11/8/2017	INTERCEPTS & ERECTION TENSIONS	D08.00		11/8/2017

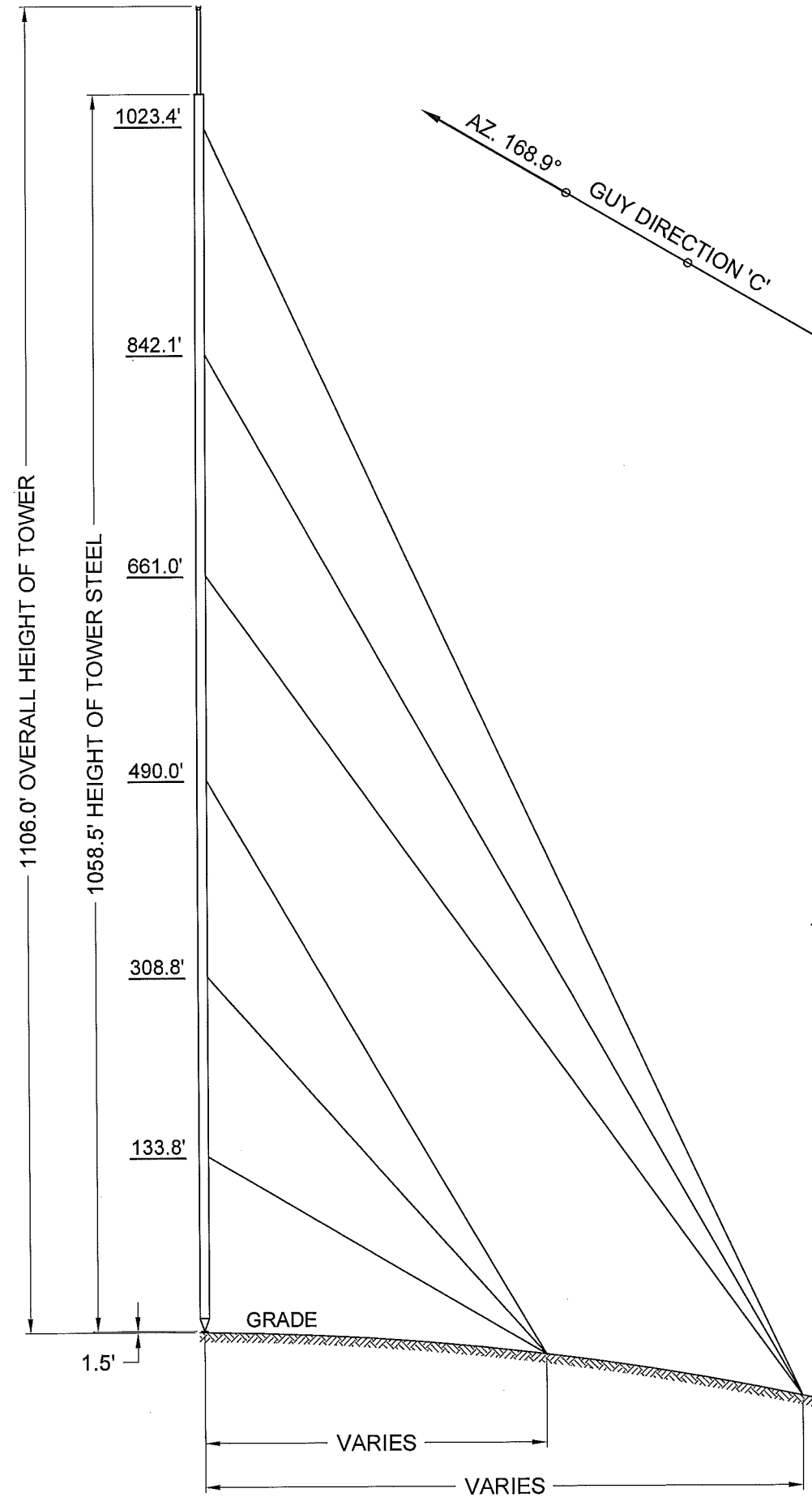
Rev	Description	Drawn by	Date	Checked by	Date	Reviewed by	Date	Approved by	Date
--	Initial Release	RE	11/8/2017	SA	11/28/17	SA	11/28/17	Free	11/24/17

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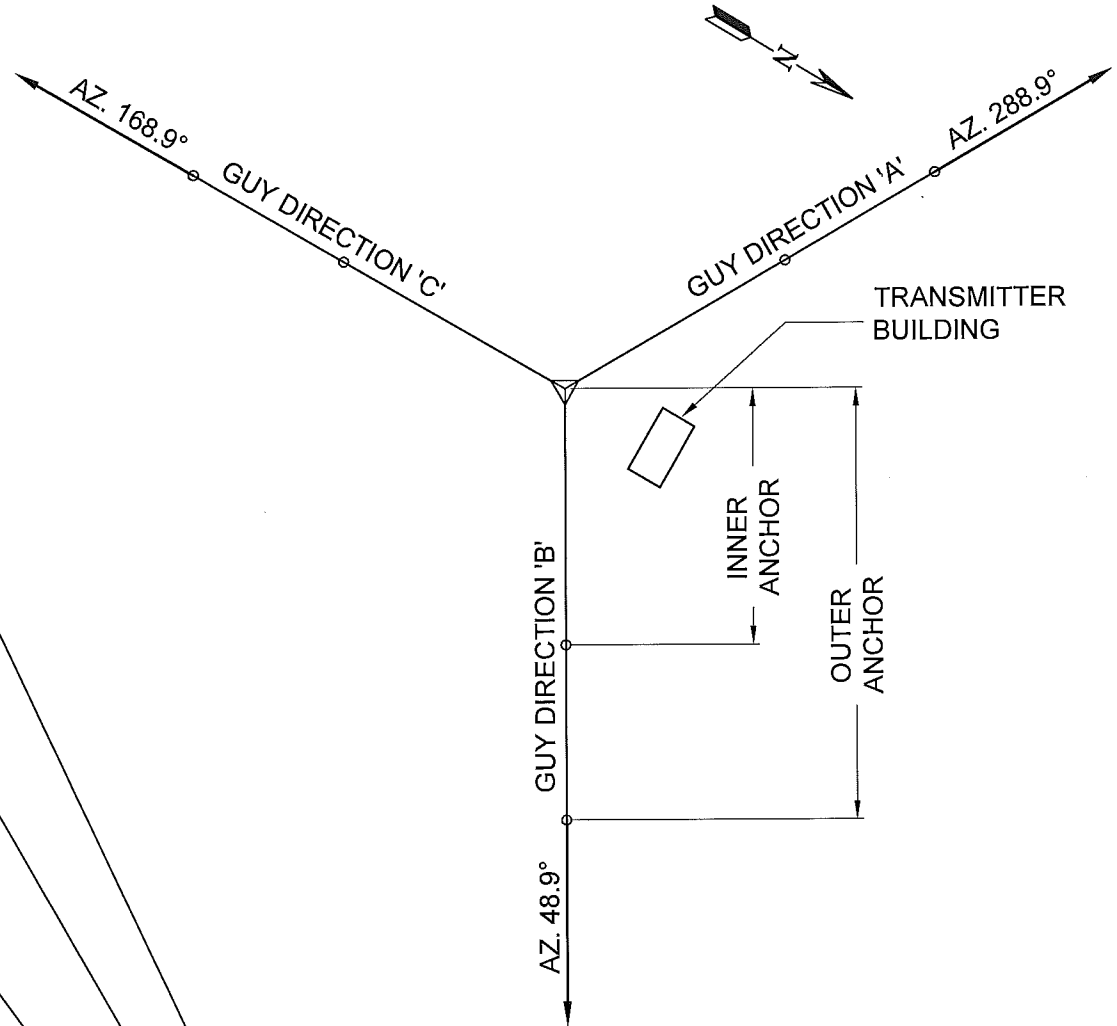


Project No.: 258114

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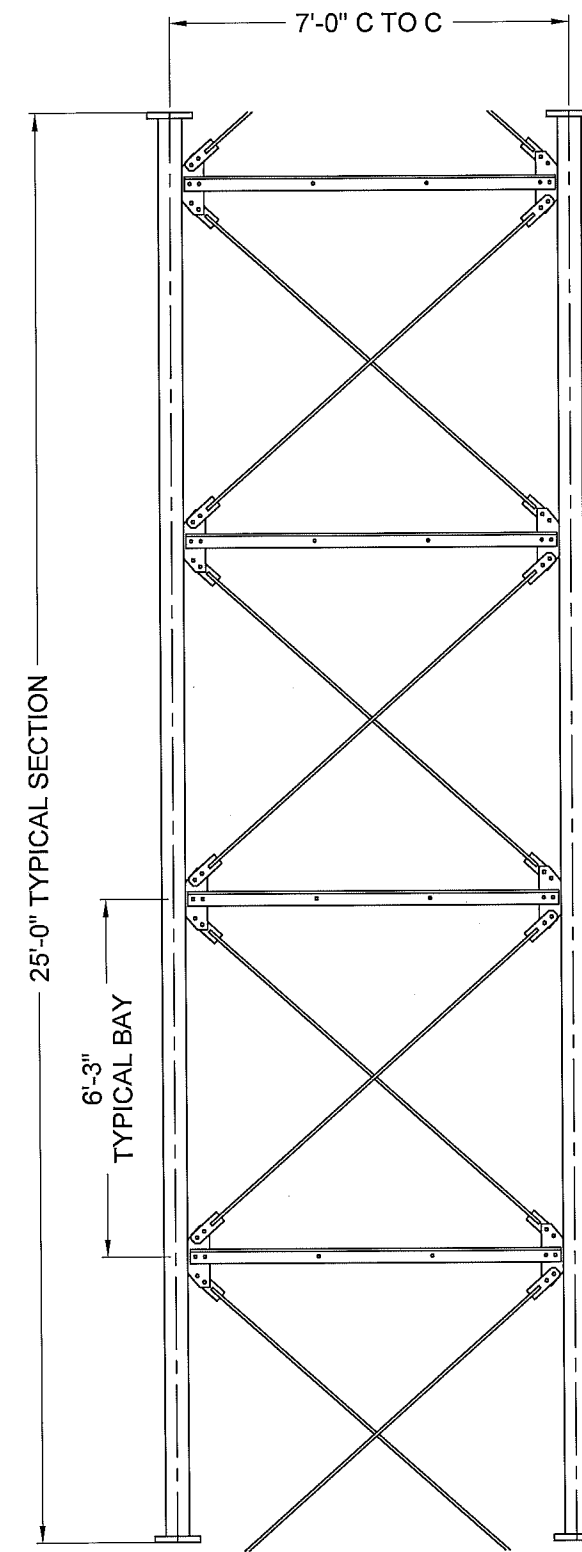


**ELEVATION VIEW**



**PLOT PLAN**

**NOTE:**  
1. SEE PAGE D01.01 AND D01.02 FOR GENERAL NOTES.



**TOWER DETAIL**



**GENERAL ARRANGEMENT**  
NEW BRITAIN, CT

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PREPARED BY	RE	11/8/17
CHECKED BY	GA	11/28/17
ENGINEER REVIEW	PEC	11/28/17
PROJECT NUMBER	258114	
DRAWING NUMBER	D01.00	

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1. The tower is a guyed, triangular, non-insulated, open face structure.
2. The tower was analyzed per Stainless Rigorous Structural Analysis Report 258113 Rev. A, dated 6/30/2017 in accordance with the 2016 Connecticut State Building Code and ANSI/TIA 222-G-2005, Structural Standard for Antenna Supporting Structures and antennas, including addenda 1 and 2, dated 2007 and 2009 respectively, for the following analysis parameters while supporting equipment as listed below:

- Structure Classification II
- 125 mph ultimate wind speed with no ice
- 50 mph normal design wind speed with 1" design ice thickness
- Exposure Category B
- Topographic Category 5 (Flat topped hill, H=220', L=1670', x=0')
- 0.18 earthquake spectral response acceleration at short periods (S<sub>s</sub>)
- Earthquake Site Class D

- a. One (1) top mounted TFU-20ETT/VP-R 06 antenna (NB: replaces existing TFU-28G antenna), fed by one (1) 6-1/8" rigid coax (NB: replaces existing 8-3/16" rigid coax). **(Proposed)**
- b. One (1) TFU-22GTH/VP-R 4C140 antenna at the 1025' level, fed by one (1) 4-1/16" rigid coax.
- c. One (1) Proscan III ENG antenna at the 1020' level, fed by one (1) 1-5/8" line and one (1) 1" control cable.
- d. One (1) Outside transfer platform at the 1000' level.
- e. One (1) Step down transformer at the 945' level. **(Proposed)**
- f. Three (3) 3'x 4' ice shields at the 945' level. **(Proposed)**
- g. Six (6) ENGensis panel antennas and three (3) ENGensis radios at the 940' level, fed by one (1) 1/2" line. **(Proposed)**
- h. One (1) Station Master omni antenna at the 520' level, fed by one (1) 7/8" line.
- i. One (1) 8'x 9' ice shield at the 360' level.
- j. One (1) PA4-65 dish with radome at the 350' level, fed by one (1) WEP65 line.
- k. One (1) ENGensis ENG antenna at the 335' level, fed by one (1) 7/8" line and one (1) 1" control cable.
- l. One (1) DB-408 omni antenna at the 330' level, fed by one (1) 7/8" line.
- m. One (1) PA4-65 dish with radome at the 320' level, fed by one (1) WEP65 line.
- n. One (1) Diamond X-50A omni antenna at the 140' level, fed by one (1) 7/8" line.
- o. One (1) 6'x 7' ice shield at the 110' level.
- p. One (1) PA6-65 dish with radome at the 100' level, fed by one (1) WEP65 line.
- q. One (1) inside ladder w/cable safety device for the full height of the tower.
- r. One (1) 1-1/4" existing conduit to top.
- s. One (1) TechnoStrobe LED lighting system with armored cable, diameters vary up the tower (NB: replaces existing strobes). Full height of tower. **(Proposed)**

3. In order for the tower to achieve a 125 mph ultimate wind speed with no ice and a 50 mph normal design wind speed with 1" design ice thickness in accordance with the 2016 Connecticut State Building Code and ANSI/TIA 222-G for a maximum rating of 100%, the following modifications are required:

- a. Reinforce the tower base. It is assumed there are no physical obstructions, both above and below grade, preventing the installation of the foundation modification.
- b. Remove existing tower leg sub-horizontal bracing members between levels 704.6' thru 717.1' and 935.9' thru 948.4'
- c. Reinforce tower legs with full pipe sleeves at the following bays:

Location	No. of bays
704.0' - 718.0'	2
935.3' - 949.0'	2

- d. Install additional horizontal sub-horizontal bracing members at the midpoints of the following bays:

Location	No. of bays
0.0' - 10.0'	2
158.8' - 183.8'	4
233.8' - 271.3'	6
358.8' - 365.0'	1
598.4' - 604.6'	1
648.4' - 654.6'	1
679.6' - 698.4'	3
729.6' - 754.6'	4
760.9' - 779.6'	3

- e. Replace the existing diagonal braces with new, higher capacity members at the following locations:

Location	No. of bays
290.0' - 271.3'	3
998.4' - 948.4'	2

- f. Adjust initial tensions in all guy levels.
- g. Provide new top adapter plate assembly to accept new proposed antenna.

4. The design of the tower modifications above has been based upon Stainless Report 258113 Rev A, dated 6/30/2017. The details contained within this design drawing package are included for information and are not intended to be used as shop or final fabrication drawings. The Contractor shall field verify all dimensions, elevations and existing site conditions and notify Stainless immediately of any site discrepancies or variances. Contractor shall not scale dimensions from the design drawings. It shall be the responsibility of the Contractor to ensure proper fit-up of the tower modification materials.

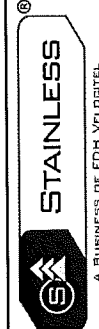

5. All work shown on this design drawing package shall be performed by qualified contractor (s) with a minimum of 5 years experience in tower and foundation construction.

6. All material shall be in accordance with the notes, specifications and drawings. All deviations and substitutions must be approved by a registered Professional Engineer in the state where the work is being done and submitted to Stainless for approval prior to installation. The Contractor shall furnish satisfactory evidence as to the kind and quality of the materials and equipment being substituted. Contractor shall also be responsible for obtaining all necessary permits, licenses and any other requirements for the construction. Submit all necessary calculations for substitutions and design details.

7. Contractor shall observe safe construction practices and shall be responsible for all methods of construction, including proper and adequate bracing to the tower and excavation work during the installation process. Adequately designed temporary support shall be installed before any tower component is removed and replaced. All means and methods of construction, including construction and soil pressure loads, shall be properly calculated and documented by the Contractor.

8. If the construction activities require a rigging plan per the requirements of ANSI/ASSE A10.48 and ANSI/TIA-322-2016, a rigging plan shall be developed by a Qualified Person, submitted to the Owner for review and implemented by a competent rigger. The Qualified Person shall coordinate Class IV rigging plans with a Qualified Engineer for a structural analysis of the structure considering the construction loading. A properly detailed rigging plan shall include, as a minimum, a review of the following:

- Operational and non-operational construction loads.
- Equipment used, and Supporting structure
- Construction sequence and durations

RE	11/9/17	CHECKED BY	GH	ENGINEER REVIEW	PCC 11/30/17	PROJECT NUMBER	258114	DRAWING NUMBER	D01.01
PREPARED BY		CHECKED BY		ENGINEER REVIEW		PROJECT NUMBER		DRAWING NUMBER	
DATE		DATE		DATE		DATE		DATE	
REV		DATE		DATE		DATE		DATE	
REVISION DESCRIPTION									
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 <b>STAINLESS</b> <small>A BUSINESS OF FDM VELOCITY</small> <small>200 North Warner Road, Suite 215</small> <small>King of Prussia, Pa 19406</small>									
<b>GENERAL NOTES</b> <b>NEW BRITAIN, CT</b>									
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9. Stainless assumes no responsibility for the structural adequacy of the tower if non-conforming modification materials are supplied and/or installed by others, and shall have no liability whatsoever to owner or to others for any work performed by any persons other than Stainless in connection with the implementation of any structural changes or modifications not specifically addressed within this design drawing package. Owner acknowledges and agrees that any riggers, erectors or subcontractors retained or employed by owner shall be solely responsible to owner and to others for the quality of work performed by them and that Stainless shall have no liability or responsibility whatsoever as a result of any negligence or breach of contract by such rigger, erector or subcontractor.

10. The modification drawings contained herein are based on the assumption that the tower has been properly installed and maintained, including, but not limited to the following:

- Proper alignment and plumbness.
- Correct bolt tightness.
- No significant deterioration or damage to any component.

### APPLICABLE CODES AND STANDARDS

Use latest editions of the following Codes and Standards unless noted otherwise.

- ANSI/TIA-222-G 2005 Structural Standards for Antenna Supporting Structures and Antennas including Addenda 1 & 2, dated 2007 and 2009.
- ANSI/ASSE A10.48 Criteria for Safety Practices Related to the Installation, Alteration, and Maintenance of Communication Structures. ANSI/TIA-322 Loading, Analysis and Design Criteria Related to the Installation, Alteration and Maintenance Communication Structures.
- AISC Manual of Steel Construction.
- RCSC Specification for Structural Joints Using ASTM A325 Bolts.
- ACI 301 Specifications for Structural Concrete.
- ACI 318 Building Code Requirements for Structural Concrete.
- ACI 315 Details and Detailing of Concrete Reinforcement.
- CRSI Manual of Standard Practice.
- ASTM C260 Standard Specification for Air-Entraining Admixtures for Concrete.
- ASTM C494 Standard Specification for Chemical Admixtures for Concrete.
- ASTM A36 Standard Specification for Carbon Structural Steel.
- ASTM A572 Standard Specification for High-Strength Low-Alloy Columbium-Vanadium Structural Steel.
- ASTM A325 Standard Specification for Structural Bolts, Steel, Heat Treated, 120/105 ksi Minimum Tensile Strength.
- ASTM A194 Standard Specification for Carbon and Alloy Steel Nuts for Bolts for High Pressure or High Temperature Service, or Both.
- ASTM F436 Standard Specification for Hardened Steel Washers.
- ASTM A123 Standard Specification for Zinc (Hot-Dip Galvanized) Coatings on Iron and products.
- ASTM A153 Standard Specification for Zinc Coating (Hot-Dip) on Iron and Steel Hardware.
- ASTM A780 Standard Practice for Repair of Damage and Uncoated Areas of Hot-Dip Galvanized Coatings.
- ASTM A615 Standard Specification for Deformed and Plain Carbon Steel Bars for Concrete Reinforcement.

### STRUCTURAL STEEL

- The fabrication and erection of structural steel shall conform to the AISC Manual of Steel Construction.
- Repair all damaged or uncoated areas of galvanized coatings in accordance with ASTM A780.
- Locking ANCO style nuts shall be installed on all bolts unless noted otherwise.
- All A325 high strength bolts shall be tightened by the "snug tightening" method as specified in the RCSC Specification for Structural Joints Using ASTM A325 Bolts unless noted otherwise on the design drawings.
- Material grades shall be as follows:
  - Bolts - A325X
  - U-Bolts - A307 min.
  - HSS - A500 Gr.B (min.42ksi)
  - Plates and angles - A36
  - Channels and Round Bars - A572 Grade 50

### PLUMBING LINES

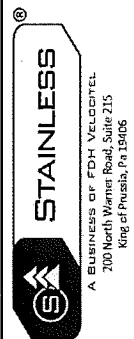
- The tower is designed for initial tension as specified in the erection drawings. It is important that the guys be tensioned accurately to assure the stiffness of the tower.
- Uneven terrain, temperature, plumbness of tower and wind are factors which affect guy tensions. If the tower site is level and anchor distances are equal, the tensions in all three guys at a level will be equal when the tower is plumb. If the terrain of the tower site is uneven, the guys are not perfectly symmetrical and tensions in guys vary in the three directions. For this reason initial guy tensions are specified in one direction only. The tower should be plumbed with the specified tensions in the given guy direction.
- Wind load on tower and guys changes the tension in all guys; therefore, plumb the tower in calm weather only.
- The plumbing of a tower or checking alignment of a tower should be performed in accordance with Annex J of ANSI/TIA 222-G.

### REINFORCED CONCRETE

- All concrete shall be in accordance with ACI 318 and ACI 301 and have a minimum compressive strength of 4000 psi after 28 days.
- All concrete shall be sampled and tested in accordance with ACI 301. Testing shall be carried out by an independent testing laboratory.
- Concrete shall not contain calcium chloride or any admixtures that contain chlorides. All admixtures used shall conform t ASTM C260 (air-entraining) and ASTM C494 (water reducing and/or accelerating)
- All reinforcing bars shall be Grade 60 deformed bars in accordance with ASTM A615, and shall be fabricated and placed in accordance with ASTM 315, ACI 318 and CRSI's Manual of Standard Practice.
- Minimum cover of the reinforcing bars for foundation concrete shall be 3".
- All formwork shall conform to ACI 318. No rough lumber shall be used where the concrete surface is visible.
- Concrete shall be placed monolithically unless noted otherwise on the drawings.
- All exposed concrete corners shall be beveled neatly with approximately 1" chamfer.
- Reinforcing bars shall be positioned as shown on the drawings and shall be adequately supported against displacement during concreting. Tack welding shall not be used.
- Backfill near and around the foundation with a reasonably well graded fill and compact to original density.

### WELDING

- All welding shall be in accordance with AWS D1.1 Structural Welding Code-Steel.
- All welding shall be performed by welders certified by the AWS in both type of weld and position of welding matching the details contained within this design drawings package.
- All weld electrodes shall be low hydrogen E70XX or equal.
- Prepare weld areas - Areas to be welded are to be free of scale, rust, galvanizing, and slag. All base metals shall be prepared in accordance to AWS D1.1.
- Remove any galvanizing finish completely within a 2" perimeter of any weld zone.
- Preheat material to 70 degree (F) if air temperature is below 32 degree (F). Material should be heat soaked through or a 3" minimum in every direction.
- Refer to AWS D1.1 for general workmanship and technique.
- No starts and stops are allowed at the end of the pipe corners. Weld wraps must begin and end at a 1" minimum away from the corners.



GENERAL NOTES  
NEW BRITAIN, CT

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RE	11/9/17			
CHECKED BY	CA	11/28/17		
ENGINEER REVIEW	PC	11/30/17		
PROJECT NUMBER			258114	
DRAWING NUMBER				D01.02
DATE				
E. OK				
DATE				
D. CK				
REVISION DESCRIPTION				
REV. BY				
DATE				

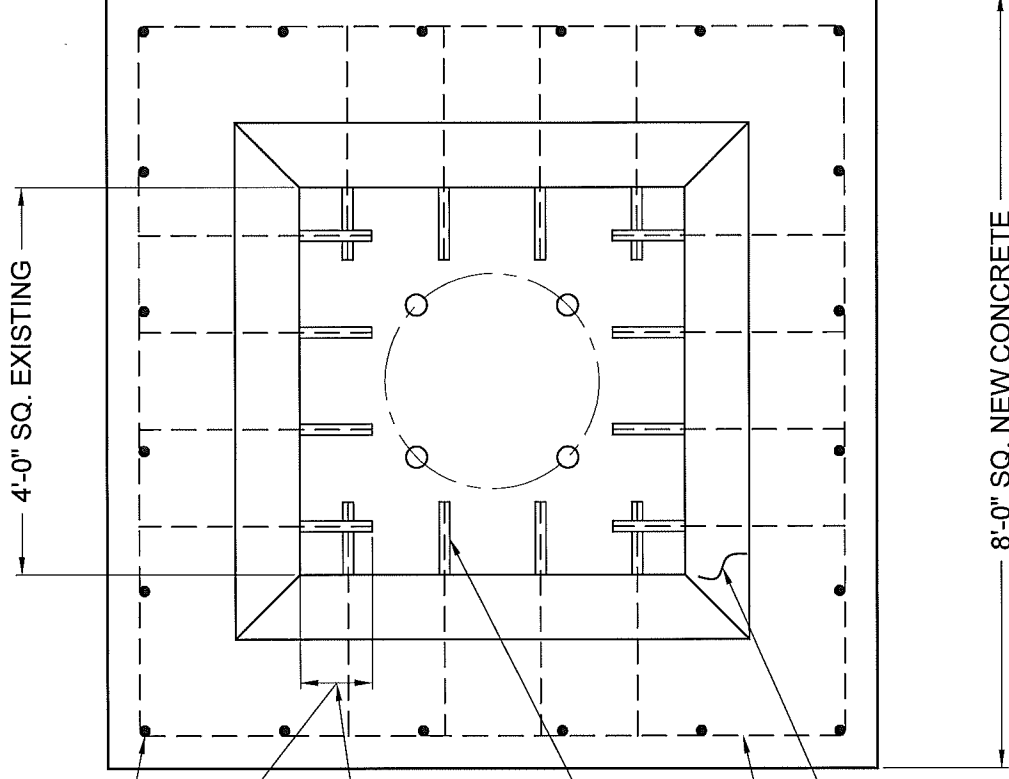
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**NOTES:**

- SEE PAGE D03.00 FOR FOUNDATION NOTES.
- EXCAVATE AROUND PERIMETER OF EXISTING BASE PIER.
- CLEAN AND ROUGHEN ALL INTERFACES BETWEEN OLD AND NEW CONCRETE. APPLY BONDING AGENT SIKADUR 32, HI-MOD LPL OR EQUIVALENT BONDING AGENT PRIOR TO NEW CONCRETE PLACEMENT. BONDING AGENT SHALL BE APPLIED IN ACCORDANCE WITH MANUFACTURER APPLICATION SPECIFICATIONS AND GUIDELINES.
- SECURE DOWELED IN REBAR WITH REBAR ADHESIVE (HILTI-HIT-HY 200 EPOXY ADHESIVE OR EQUIVALENT).

**BILL OF MATERIAL**

QTY.	NAME	DESCRIPTION
120'	REINFORCING BARS	#9 - ASTM A615 GRADE 60
100'	REINFORCING BARS	#6 - ASTM A615 GRADE 60
200'	REINFORCING TIES	#3 - ASTM A615 GRADE 60
12.0 CU. YDS.	CONCRETE	4000 PSI AFTER 28 DAYS
AS REQUIRED	HILTI-HIT-HY 200 ADHESIVE	----



(20) #9 VERTICAL BARS EMBEDDED EQUALLY SPACED AS SHOWN

9" TYP.

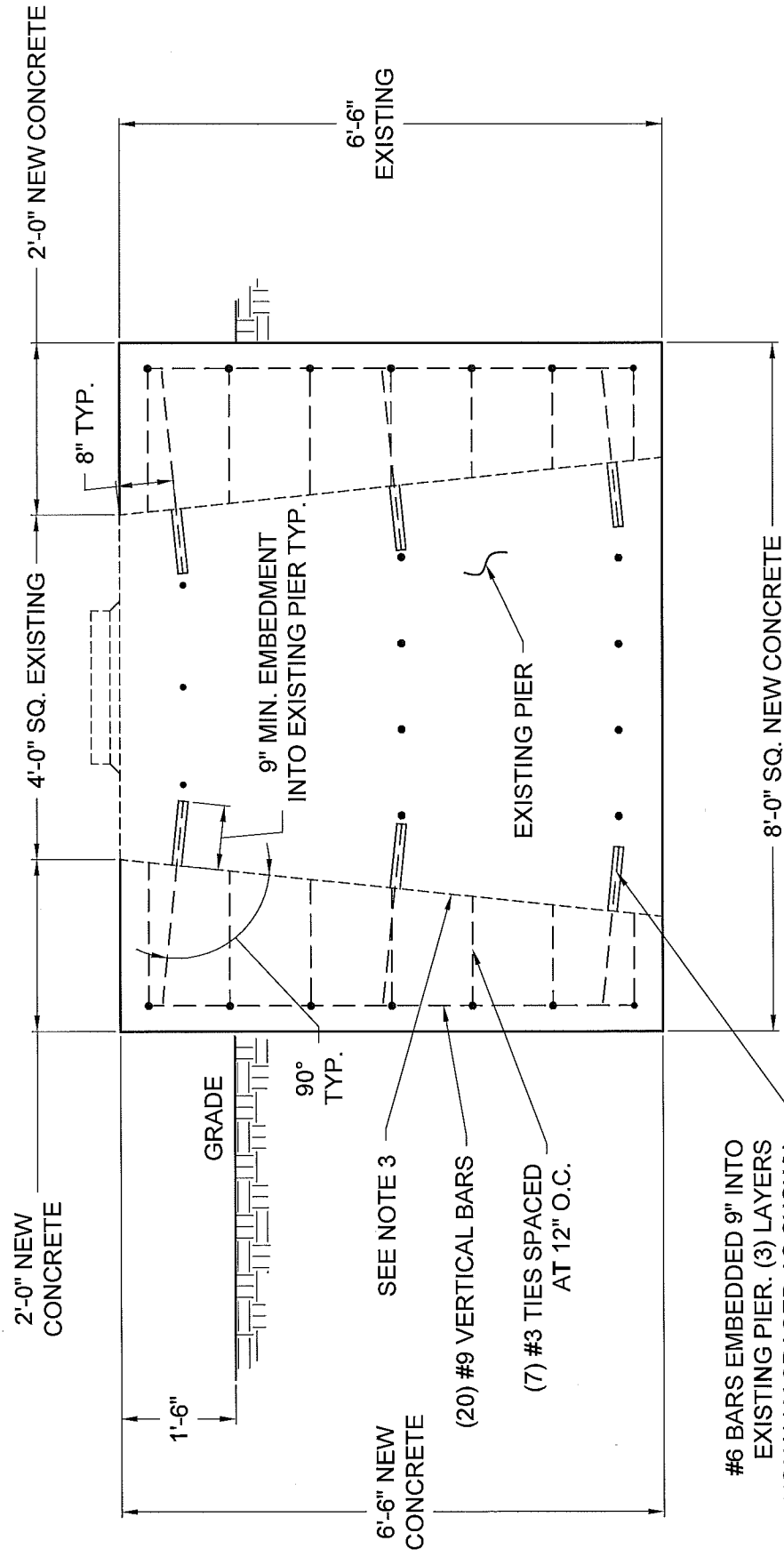
MIN. EMBEDMENT INTO EXISTING FOUNDATION.

(12) #6 BARS FOR THE TOP LAYER AND (16) #6 BARS FOR THE MIDDLE AND BOTTOM LAYERS EMBEDDED 9" INTO EXISTING PIER EQUALLY SPACED AS SHOWN, LENGTH VARIES

#3 TIES SPACED AT 12" O.C.

EXISTING PIER

**PLAN VIEW**



2'-0" NEW CONCRETE

4'-0" SQ. EXISTING

2'-0" NEW CONCRETE

1'-6"

GRADE

8" TYP.

6'-6" EXISTING

6'-6" NEW CONCRETE

SEE NOTE 3

(20) #9 VERTICAL BARS

(7) #3 TIES SPACED AT 12" O.C.

9" MIN. EMBEDMENT INTO EXISTING PIER TYP.

EXISTING PIER

#6 BARS EMBEDDED 9" INTO EXISTING PIER. (3) LAYERS EQUALLY SPACED AS SHOWN. LENGTH VARIES, SEE PLAN VIEW. START AT 8" FROM TOP OF PIER.

**ELEVATION VIEW**



**BASE FOUNDATION MODIFICATION**  
NEW BRITAIN, CT

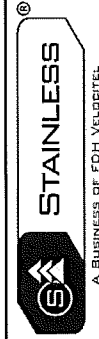
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PREPARED BY	RE	11/10/17
CHECKED BY	Git	11/22/17
ENGINEER REVIEW	Pcc	11/30/17
PROJECT NUMBER	258114	
DRAWING NUMBER	D02.00	

FOUNDATION NOTES

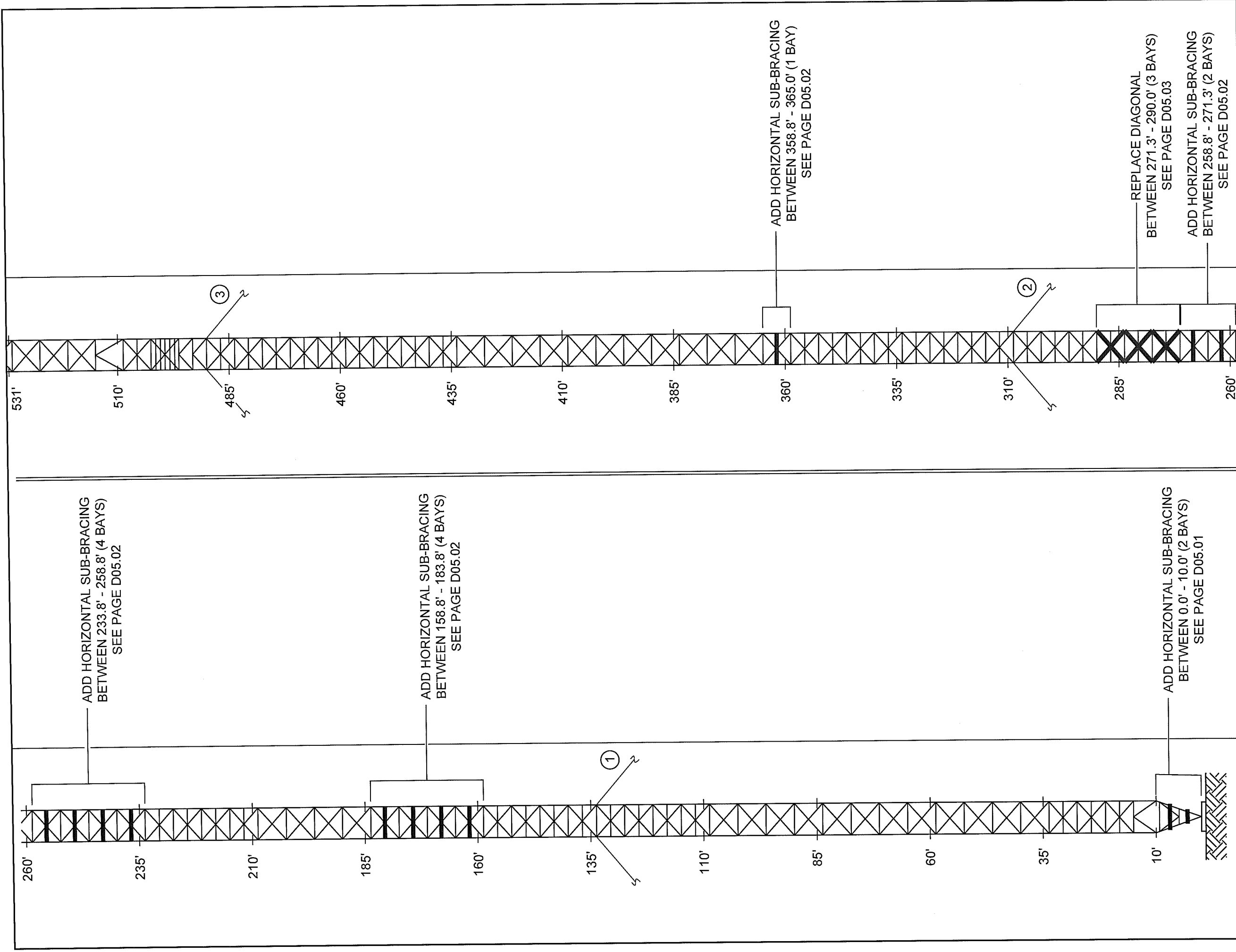
1. All concrete to be in accordance with the current edition of ACI 318 and ACI 301 and have a minimum compressive strength of 4000 PSI after 28 days.
2. Concrete to be sampled and tested in accordance with ACI 301, paragraphs 16.3, 16.4, and 16.5. Testing to be performed by an independent testing laboratory.
3. All reinforcing shall be deformed steel bars in accordance with ASTM A615, Grade 60.
4. Concrete and reinforcing bars furnished by foundation contractor.
5. No rough lumber to be used where concrete surface is visible.
6. All exposed concrete corners shall be beveled neatly with approximately 1" chamfer.
7. Reinforcing shall be positioned as shown and adequately supported against displacement. Tack welding is not permitted.
8. Bend all reinforcing cold and remove all scale.
9. Minimum cover for reinforcing bars is 3".
10. The foundation must rest on undisturbed soil.
11. Backfill near and around all foundations with a reasonable well graded fill and compact to original density.
12. Elevation and flatness of base foundation top to be within plus or minus 1/4".
13. Foundation design is based upon the subsurface analysis performed by Clarence Welte Associates, Inc. dated July 19, 1976 for Connecticut Television Inc. tower project. Test boring data was forwarded to Stainless Inc. for job #2581.
14. If the actual subsurface conditions deviate from those described in the subsurface investigation or any other soil information provided, contact Stainless immediately.
15. Bill of Material is approximate and for reference only. Contractor must verify all quantities.



FOUNDATION NOTES  
NEW BRITAIN, CT

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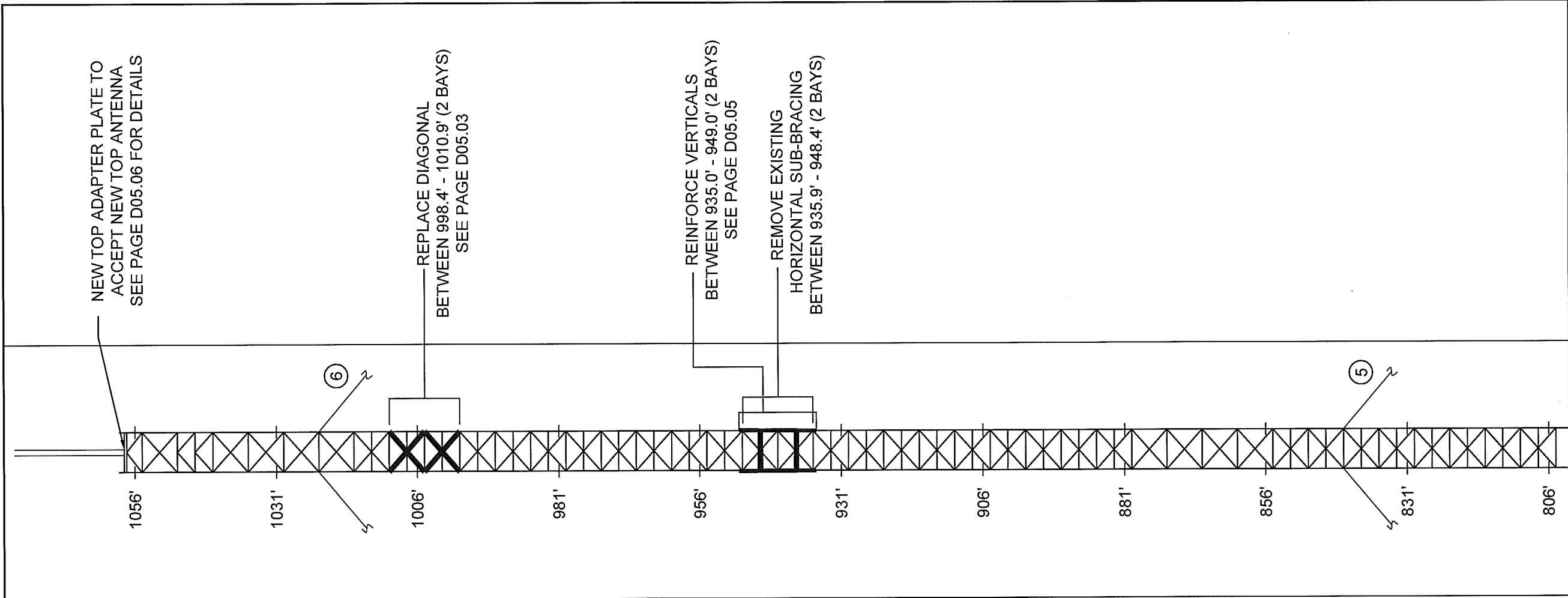
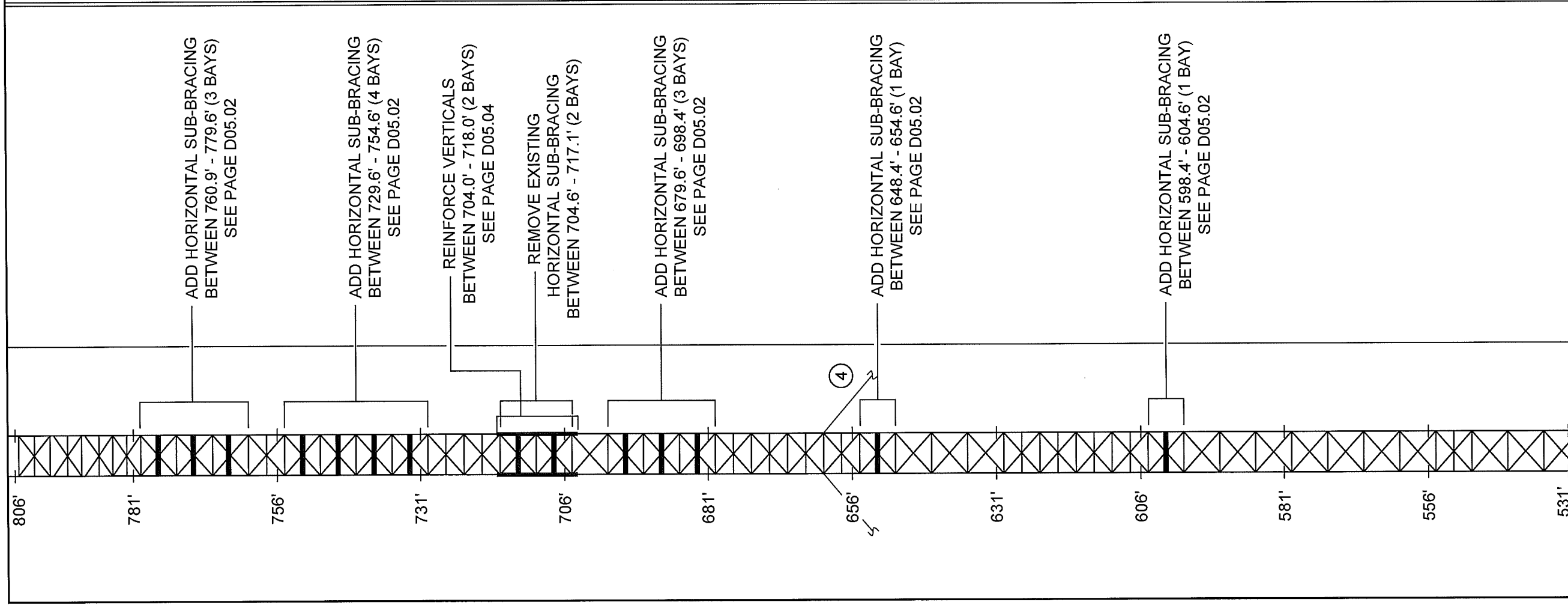
RE	11/9/17					
PREPARED BY	GA	11/28/17				
CHECKED BY	PCC	11/30/17				
ENGINEER REVIEW						
PROJECT NUMBER	258114					
DRAWING NUMBER	D03.01					
REV	BY	DATE	REVISION DESCRIPTION	D.CK	DATE	E. CK



TOWER ELEVATION		TOWER MODIFICATIONS		TOWER ELEVATION		TOWER MODIFICATIONS	
REV	BY	DATE	REVISION DESCRIPTION	D.CK	DATE	E.CK	DATE
*							
*							
*							
*							
*							
*							

<p><b>STAINLESS</b> A BUSINESS OF FDH VELOCITEL 200 North Warner Road, Suite 215 King of Prussia, PA 19406</p>	<p>RE 11/8/17</p>
<p>TOWER PROFILE NEW BRITAIN, CT</p>	<p>CHECKED BY GAT 11/29/17</p>
<p>STATE OF CONNECTICUT DENNIS D. FEE No. 23247 LICENSED PROFESSIONAL ENGINEER</p>	<p>ENGINEER REVIEW PCC 11/30/17</p>
<p>THIS DRAWING IS THE PROPERTY OF STAINLESS AND TRANSMITTED IN CONFIDENCE. THE DESIGN AND DETAILS CONTAINED HEREIN ARE PROHIBITED IN PART OR IN WHOLE FROM BEING REPRODUCED OR USED IN ANY MANNER WITHOUT THE PRIOR WRITTEN PERMISSION OF STAINLESS.</p>	<p>PROJECT NUMBER 258114</p>
<p>12-17</p>	<p>DRAWING NUMBER D04.00</p>

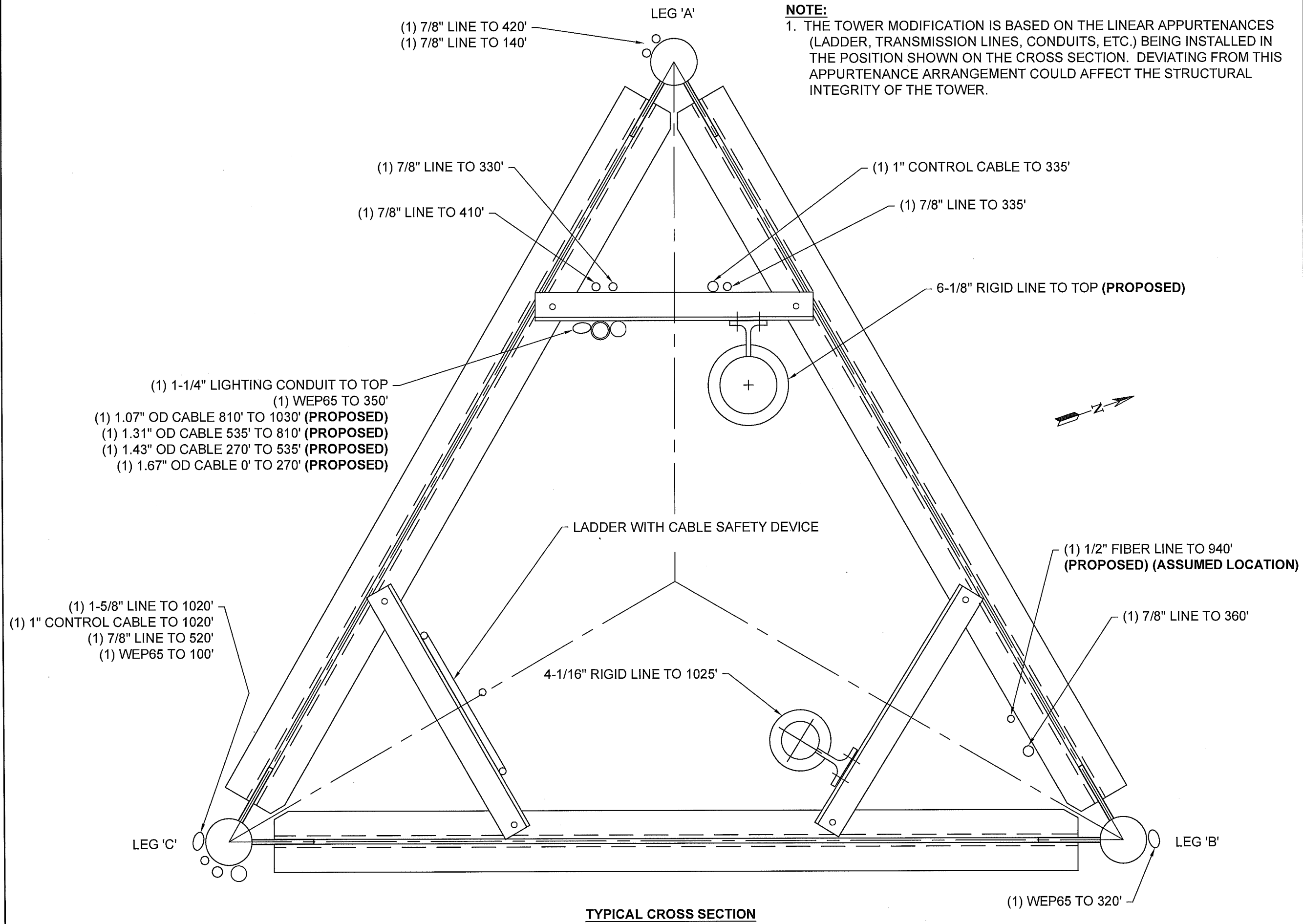


TOWER ELEVATION		TOWER MODIFICATIONS		TOWER ELEVATION		TOWER MODIFICATIONS	
806'			*				
781'			*				
756'			*				
731'			*				
706'			*				
681'			*				
656'			*				
631'			*				
606'			*				
581'			*				
556'			*				
531'			*				
						<b>TOWER PROFILE</b> <b>NEW BRITAIN, CT</b>	
						PREPARED BY: 11/8/17 CHECKED BY: GA 11/29/17 ENGINEER REVIEW: PCC 11/30/17 PROJECT NUMBER: 258114 DRAWING NUMBER: D04.01	



**NOTE:**

1. THE TOWER MODIFICATION IS BASED ON THE LINEAR APPURTENANCES (LADDER, TRANSMISSION LINES, CONDUITS, ETC.) BEING INSTALLED IN THE POSITION SHOWN ON THE CROSS SECTION. DEVIATING FROM THIS APPURTENANCE ARRANGEMENT COULD AFFECT THE STRUCTURAL INTEGRITY OF THE TOWER.



**TYPICAL CROSS SECTION**

RE	11/0017
PREPARED BY	EA
CHECKED BY	EA
ENGINEER REVIEW	PEC
PROJECT NUMBER	258114
DRAWING NUMBER	D05.00

REV	BY	DATE	DESCRIPTION
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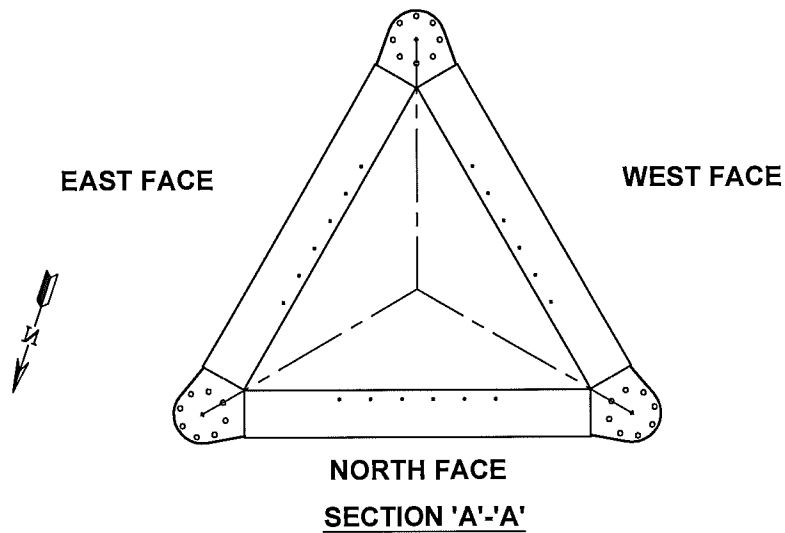
**STAINLESS**  
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 King of Prussia, PA 19406

**LINEAR APPURTENANCES**  
 NEW BRITAIN, CT

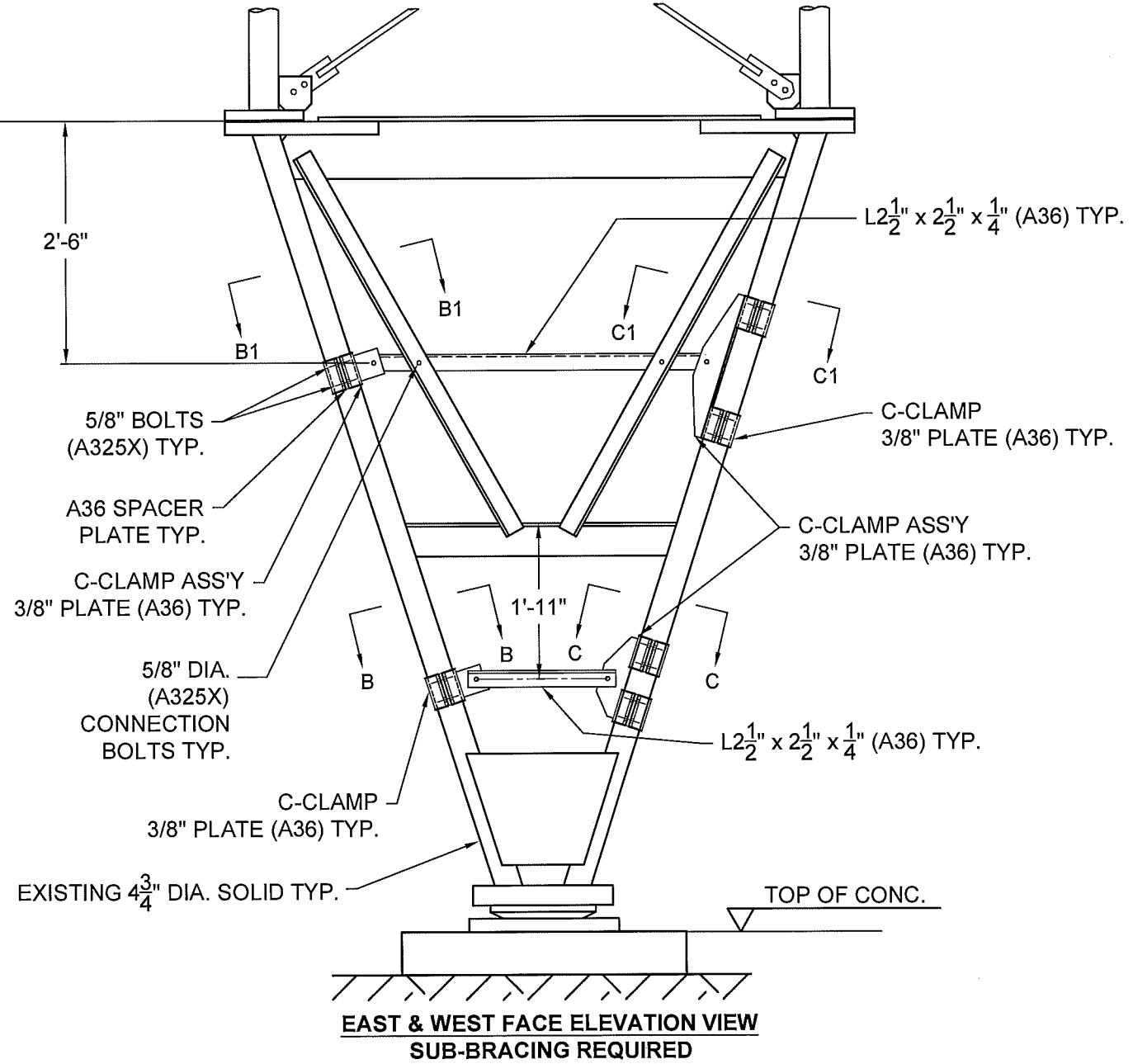
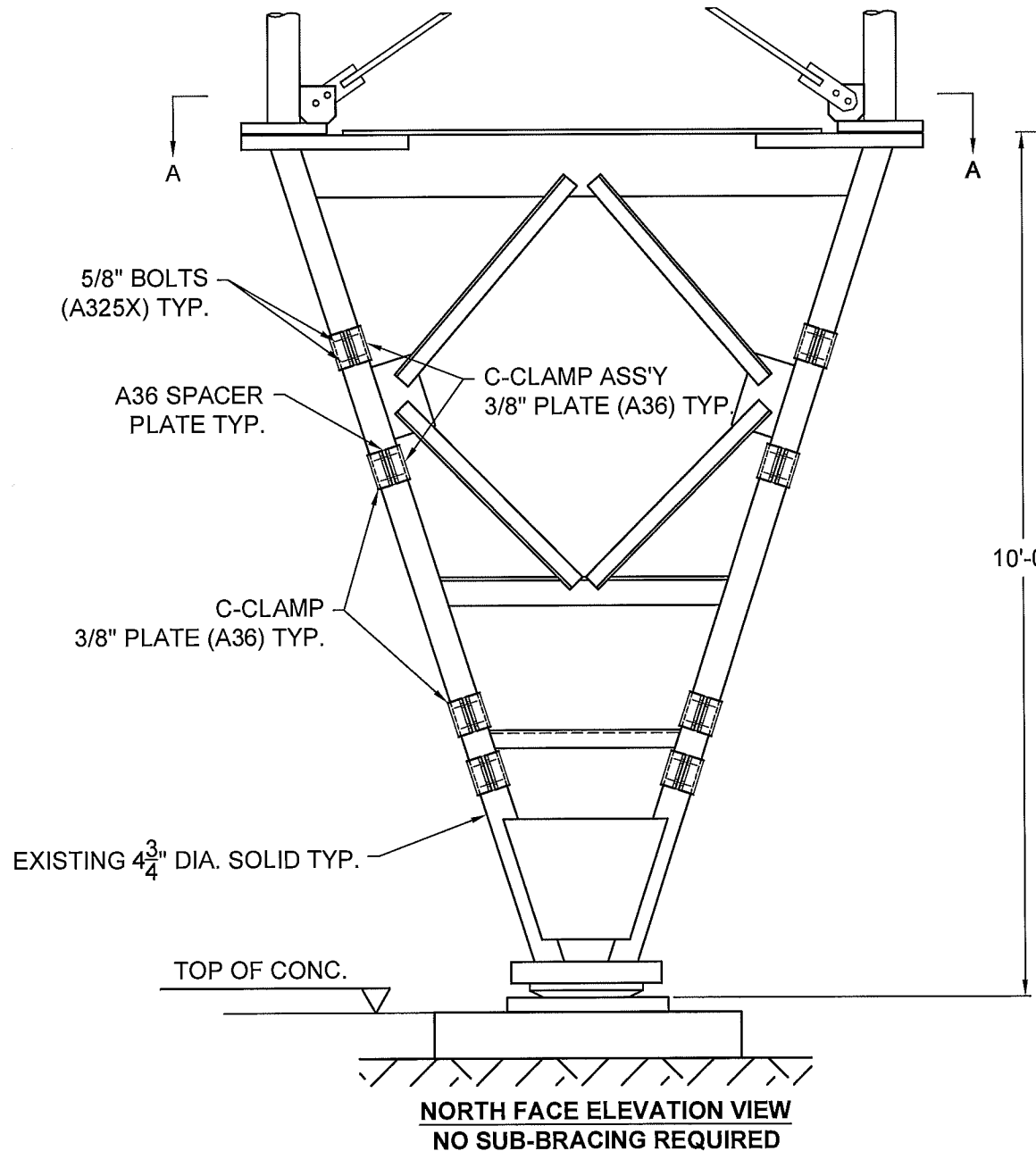
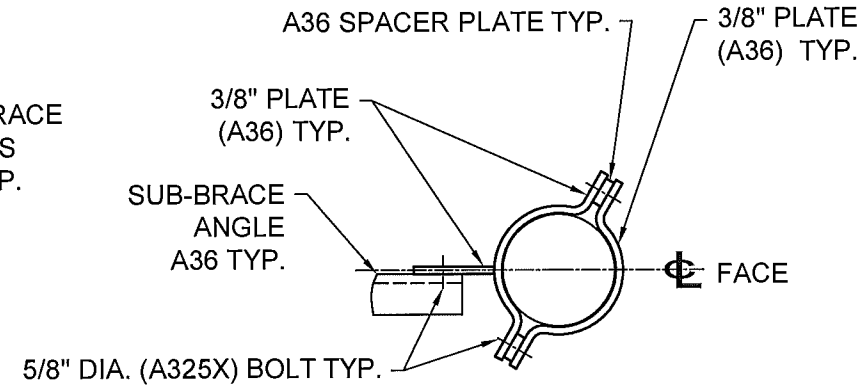
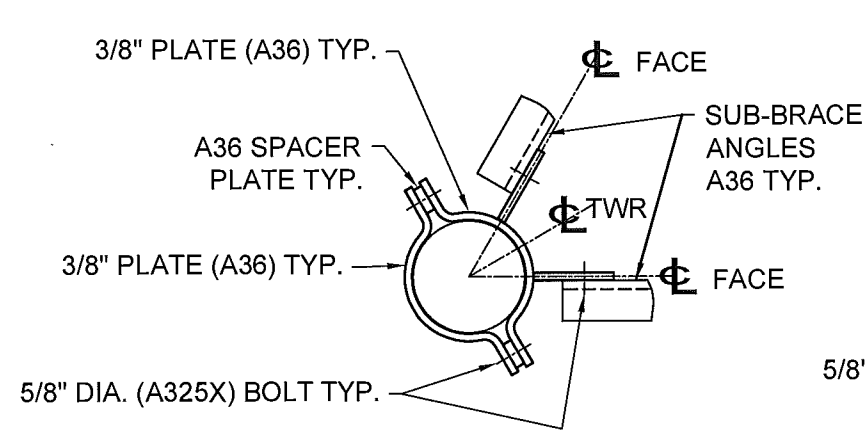
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STATE OF CONNECTICUT  
 DENNIS D. ABEL  
 No. 23247  
 LICENSED PROFESSIONAL ENGINEER  
 12-1-17

K:\258114.dwg\258114\_D05.00.dwg



**NOTE:**  
 DESIGN SUB-BRACING CONNECTIONS PER ANSI/TIA 222-G BASED UPON  
 THE MAXIMUM FACTORED LEG COMPRESSION LOAD (537 KIPS).



11/8/17	RE	258114	D05.01
11/29/17	CHECKED BY	4A	
11/30/17	ENGINEER REVIEW	PEC	
	PROJECT NUMBER	258114	
	DRAWING NUMBER		D05.01

REV	BY	DATE	REVISION DESCRIPTION
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K:\258114\dwg\258114\_D05.01\_BASE\_SUB\_BRACING.dwg

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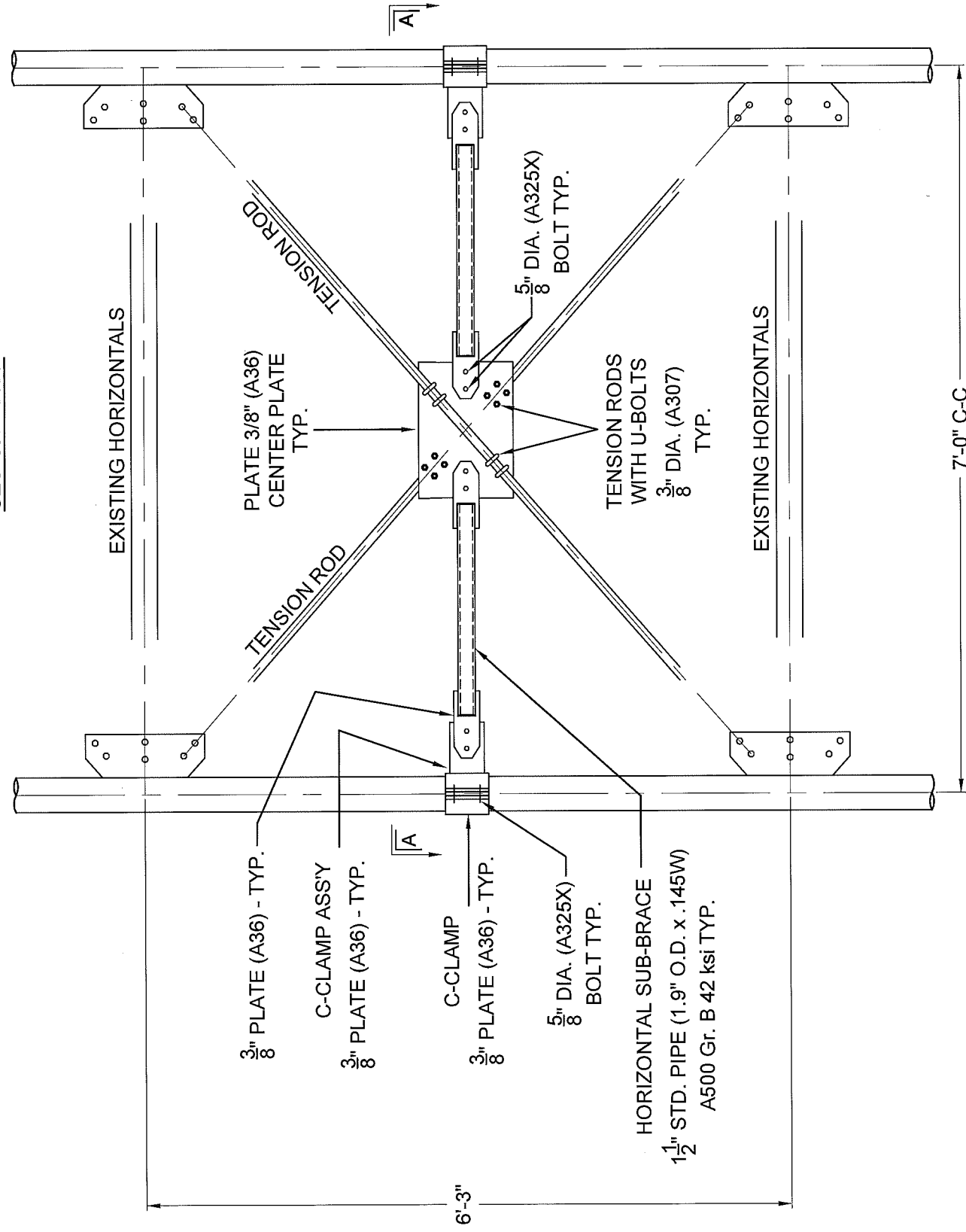
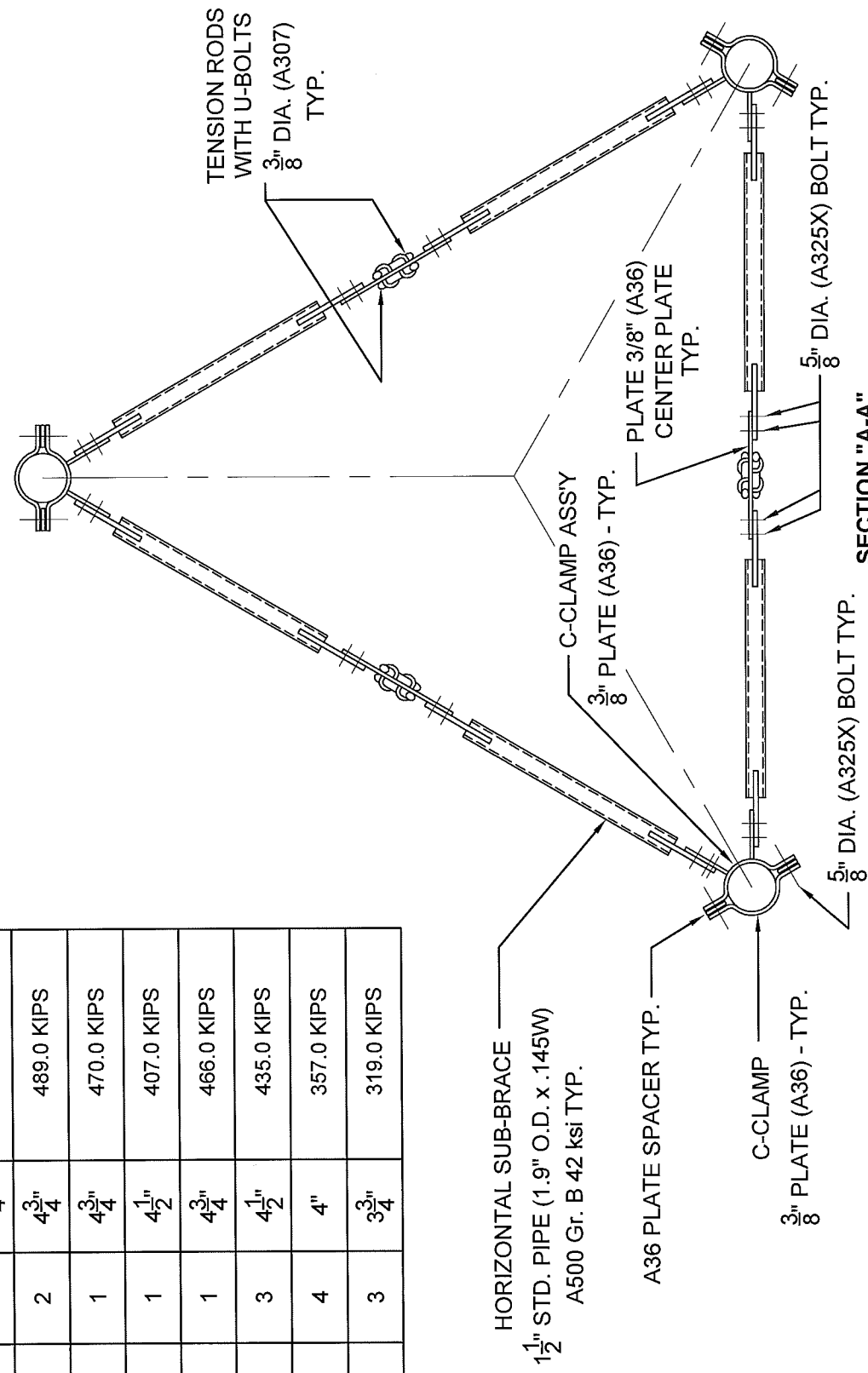
**BASE ASSEMBLY SUB-BRACING DETAILS**  
 NEW BRITAIN, CT

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STATE OF CONNECTICUT  
 DENNIS D. ABEL  
 No. 23247  
 LICENSED PROFESSIONAL ENGINEER  
 12-1-17

ELEVATIONS	# BAYS	LEG Ø	MAX. FACTORED COMPRESSION LEG LOADS
158.8' - 183.8'	4	4 $\frac{3}{4}$ "	483.0 KIPS
233.8' - 258.8'	4	4 $\frac{3}{4}$ "	489.0 KIPS
258.8' - 271.3'	2	4 $\frac{3}{4}$ "	489.0 KIPS
358.8' - 365.0'	1	4 $\frac{3}{4}$ "	470.0 KIPS
598.4' - 604.6'	1	4 $\frac{1}{2}$ "	407.0 KIPS
648.4' - 654.6'	1	4 $\frac{3}{4}$ "	466.0 KIPS
679.6' - 698.4'	3	4 $\frac{1}{2}$ "	435.0 KIPS
729.6' - 754.6'	4	4"	357.0 KIPS
760.9' - 779.6'	3	3 $\frac{3}{4}$ "	319.0 KIPS

**NOTE:** DESIGN SUB-BRACING CONNECTIONS PER ANSI/TIA 222-G BASED UPON THE MAXIMUM COMPRESSION LEG LOADS SHOWN.



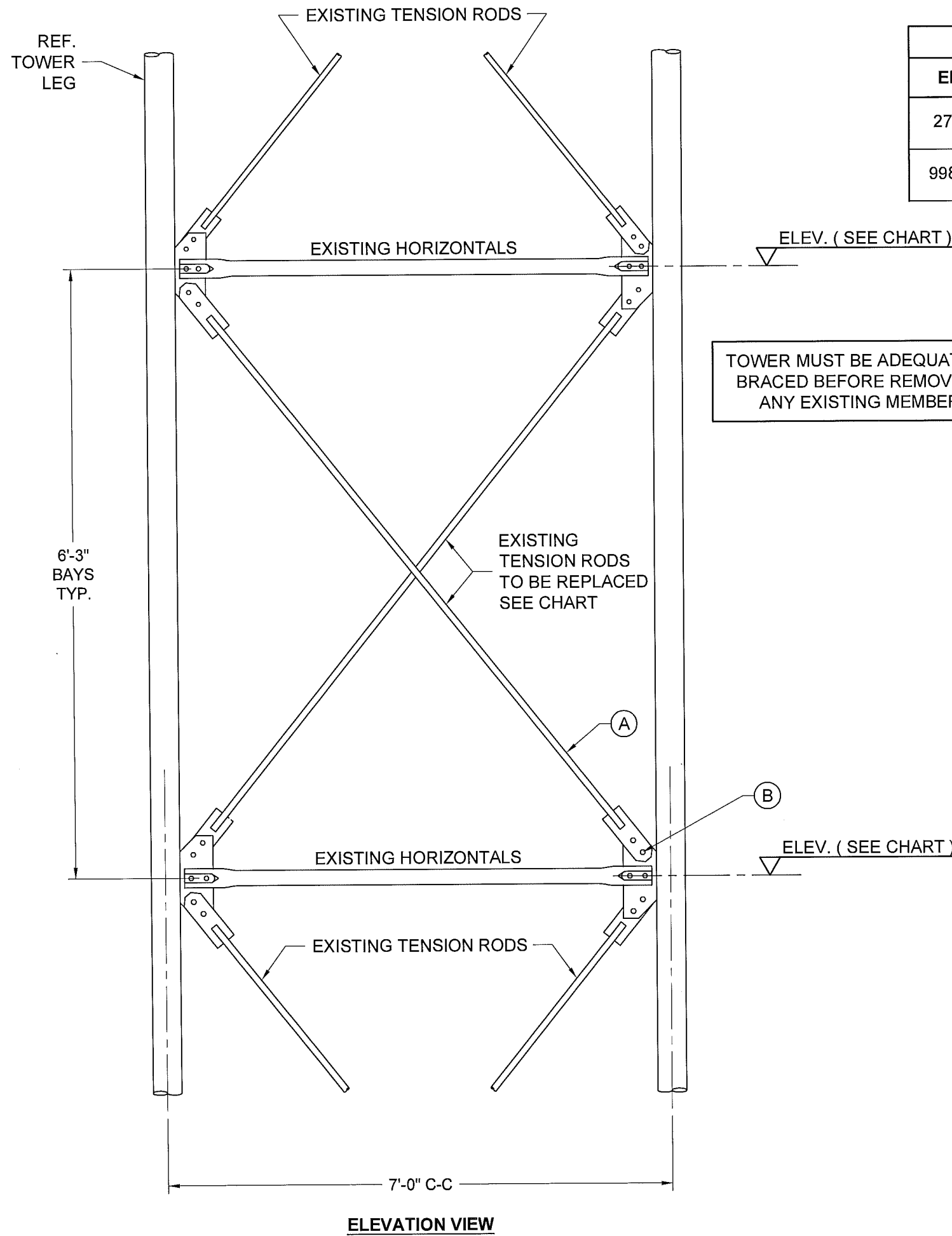
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SUB-BRACING DETAILS  
NEW BRITAIN, CT

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REV	BY	DATE	REVISION DESCRIPTION	D.CK	DATE	E.CK	DATE
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*							
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*							

PREPARED BY	RE	11/8/17
CHECKED BY	gib	11/28/17
ENGINEER REVIEW	pec	11/30/17
PROJECT NUMBER		258114
DRAWING NUMBER		D05.02



DIAGONAL REPLACEMENTS				
ELEVATION	BAYS	(A)	(B)	MAXIMUM FACTORED LOADS
271.3' - 290.0'	3	3/4" DIAMETER (A572 GRADE 50)	(2) 5/8" DIA. BOLTS (A325X)	19.88 KIPS SEE NOTE 2
998.4' - 1010.9'	2	3/4" DIAMETER (A572 GRADE 50)	(2) 5/8" DIA. BOLTS (A325X)	19.88 KIPS SEE NOTE 2

- NOTES:**
1. REPLACE ONLY ONE TENSION ROD MEMBER ON ONE FACE AT A TIME.
  2. DESIGN END PAD CONNECTIONS TO DEVELOP THE DESIGN TENSILE STRENGTH OF THE TENSION RODS PER ANSI/TIA 222-G.

TOWER MUST BE ADEQUATELY BRACED BEFORE REMOVING ANY EXISTING MEMBER

RE	11/8/17	DATE	
CHECKED BY	GH	11/29/17	
ENGINEER REVIEW	PEC	11/30/17	
PROJECT NUMBER	258114		
DRAWING NUMBER	D05.03		

REV	BY	DATE	REVISION DESCRIPTION
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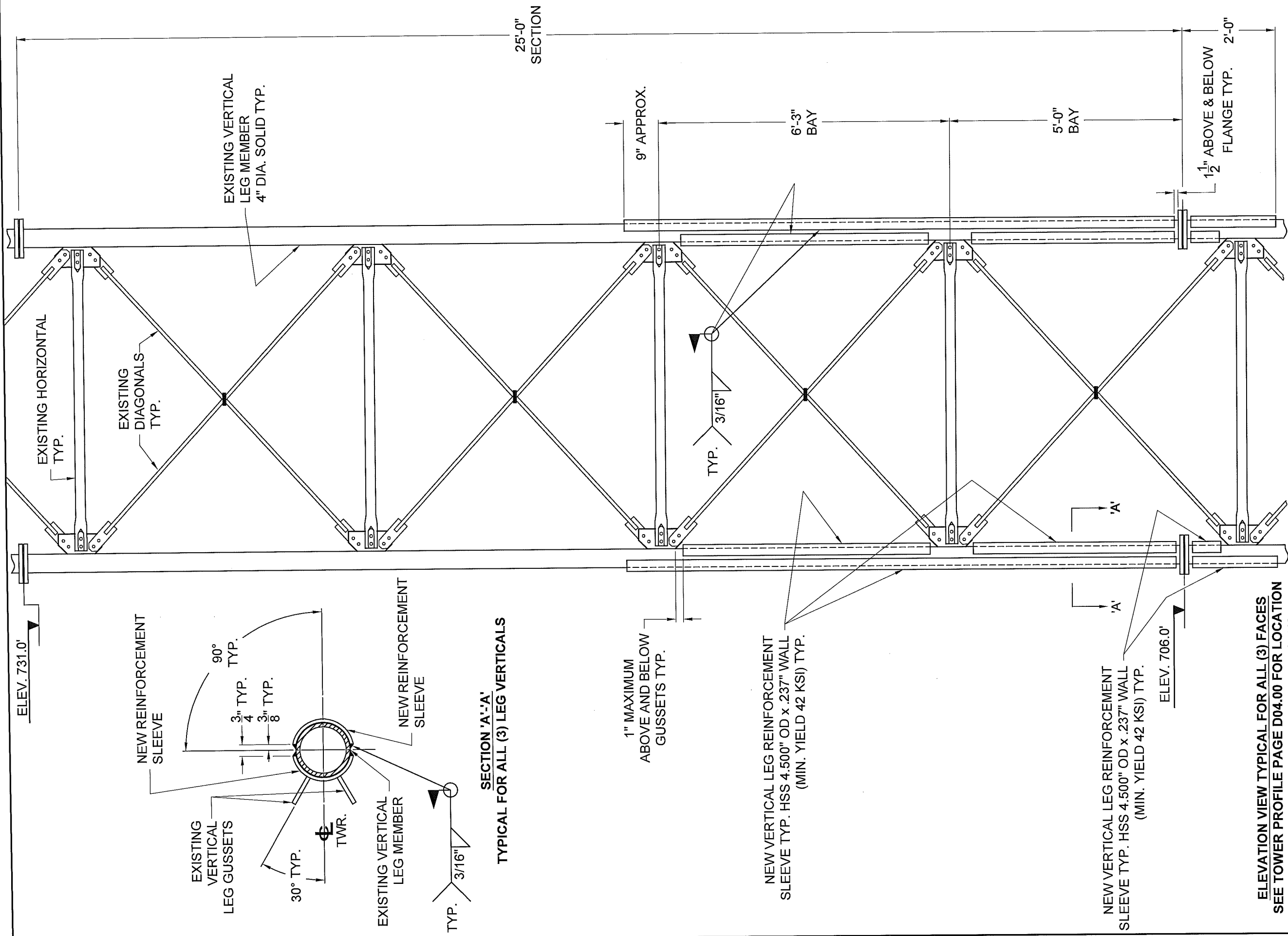
  

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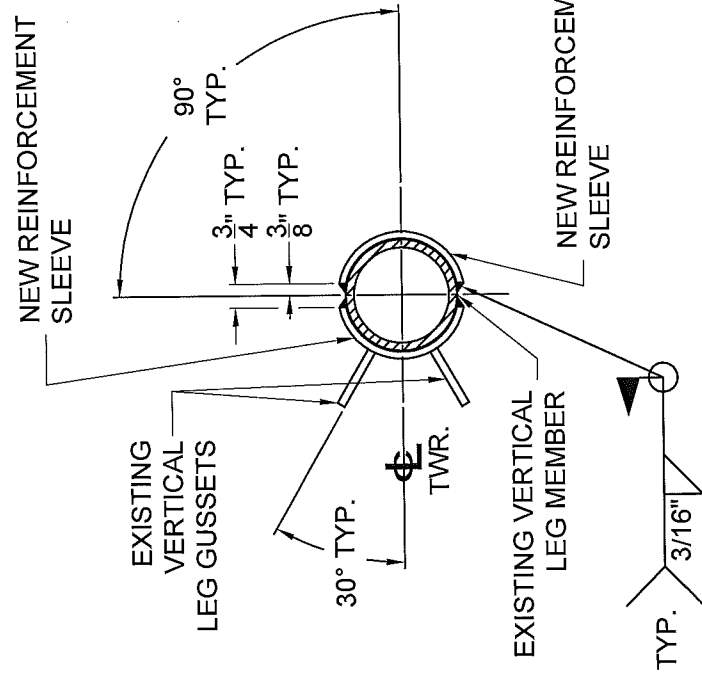
**DIAGONAL REPLACEMENT**  
 NEW BRITAIN, CT

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STATE OF CONNECTICUT  
 DENNIS D. ABEL  
 No. 23247  
 LICENSED PROFESSIONAL ENGINEER  
 12-117



ELEV. 731.0'



SECTION 'A-A'  
TYPICAL FOR ALL (3) LEG VERTICALS

1" MAXIMUM  
ABOVE AND BELOW  
GUSSETS TYP.

NEW VERTICAL LEG REINFORCEMENT  
SLEEVE TYP. HSS 4.500" OD x .237" WALL  
(MIN. YIELD 42 KSI) TYP.

NEW VERTICAL LEG REINFORCEMENT  
SLEEVE TYP. HSS 4.500" OD x .237" WALL  
(MIN. YIELD 42 KSI) TYP.

ELEV. 706.0'

ELEVATION VIEW TYPICAL FOR ALL (3) FACES  
SEE TOWER PROFILE PAGE D04.00 FOR LOCATION  
AND PAGE D01.02 FOR WELDING NOTES

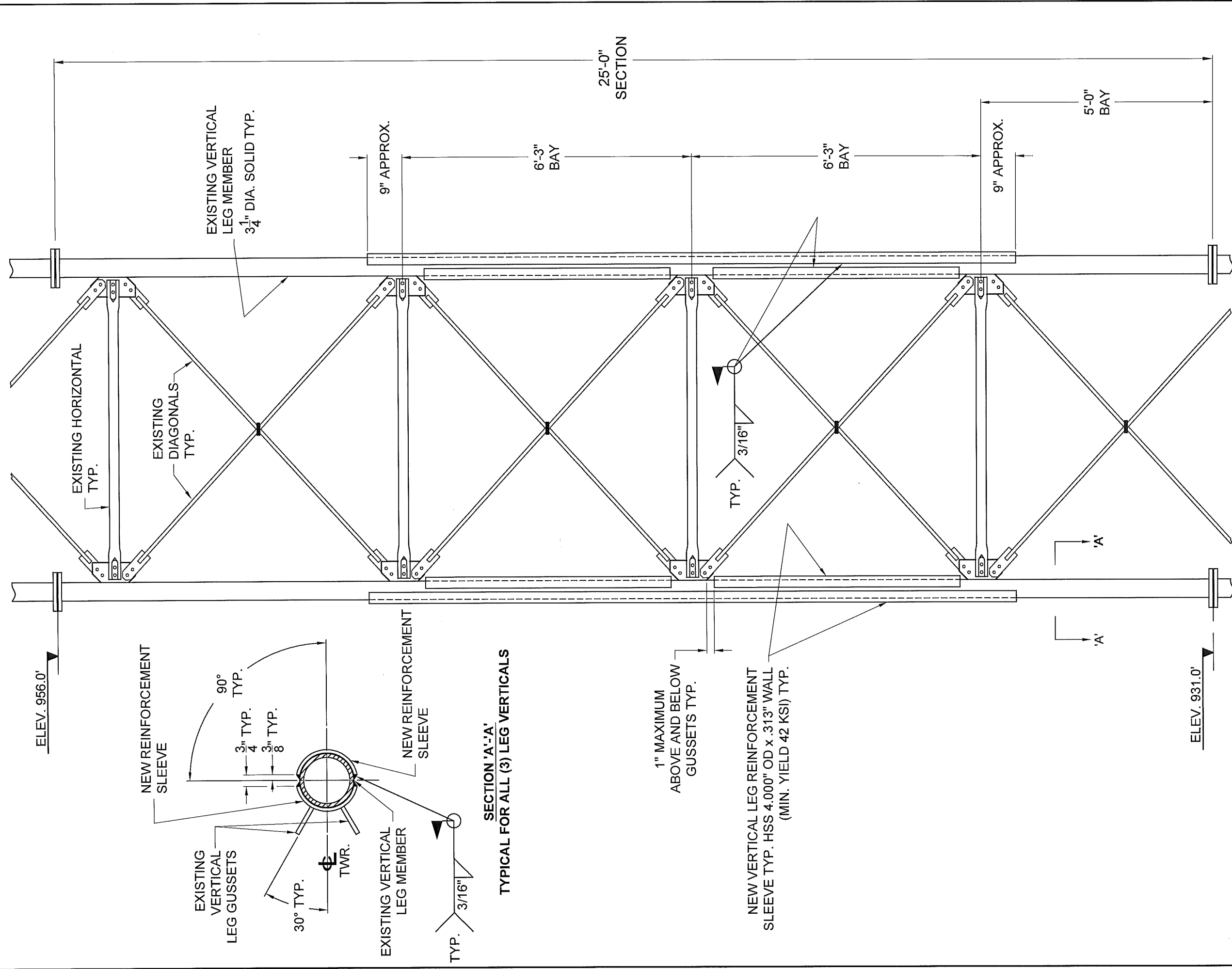


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**VERTICAL LEG REINFORCEMENT DETAILS**  
NEW BRITAIN, CT

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PREPARED BY	GA		
CHECKED BY	11/28/17		
ENGINEER REVIEW	pec 11/30/17		
PROJECT NUMBER	258114		
DRAWING NUMBER	D05.04		
REV	BY	DATE	REVISION DESCRIPTION
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ELEVATION VIEW TYPICAL FOR ALL (3) FACES  
 SEE TOWER PROFILE PAGE D04.00 FOR LOCATION  
 AND PAGE D01.02 FOR WELDING NOTES

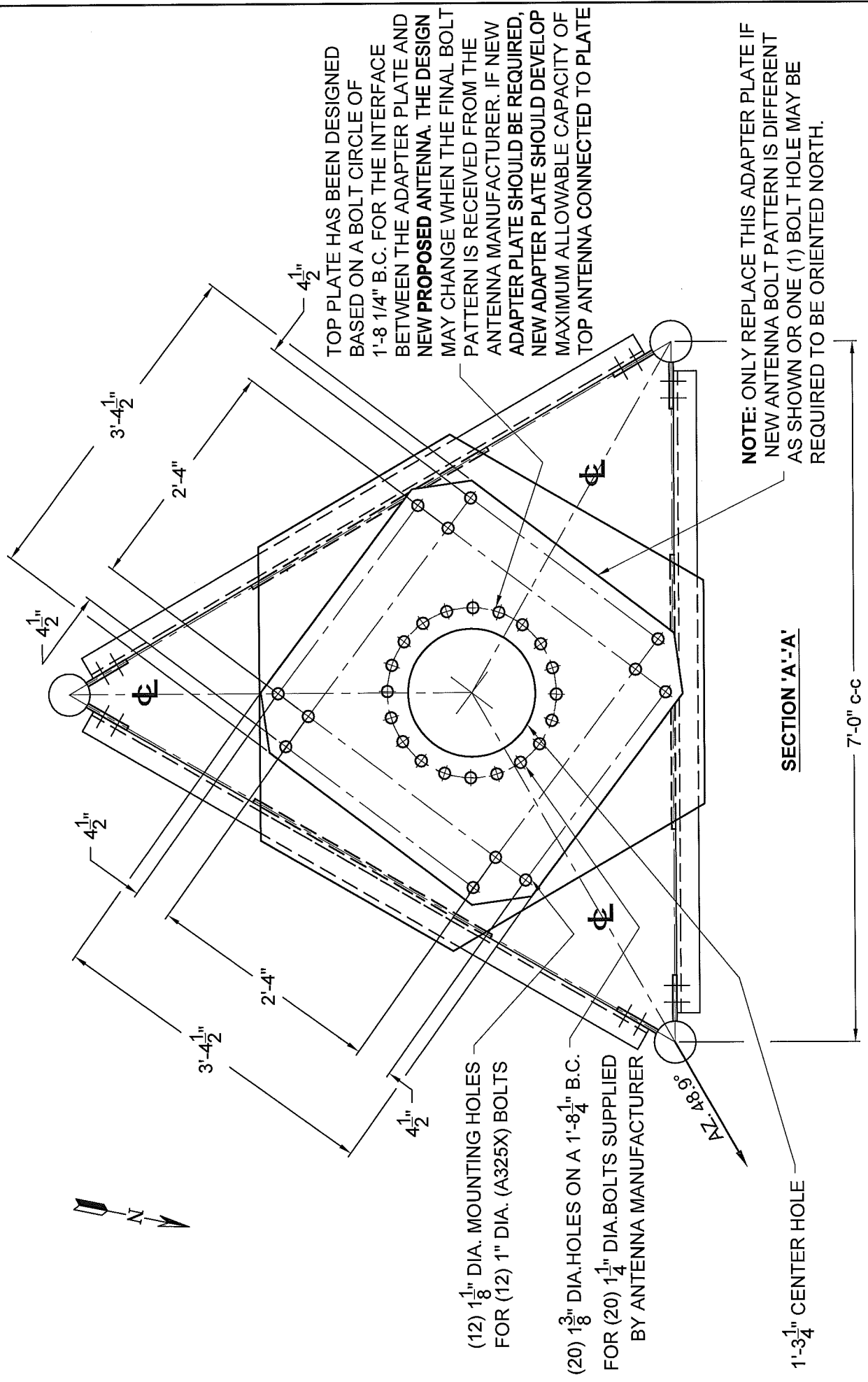


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**VERTICAL LEG REINFORCEMENT DETAILS**  
 NEW BRITAIN, CT

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RE	11/8/17		
PREPARED BY			
CHECKED BY	GH		
ENGINEER REVIEW	pac 11/30/17		
PROJECT NUMBER	258114		
DRAWING NUMBER	D05.05		
REV	BY	DATE	REVISION DESCRIPTION
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*			
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*			
D.C.K	DATE	E.C.K	DATE



TOP PLATE HAS BEEN DESIGNED BASED ON A BOLT CIRCLE OF 1'-8 1/4" B.C. FOR THE INTERFACE BETWEEN THE ADAPTER PLATE AND NEW PROPOSED ANTENNA. THE DESIGN MAY CHANGE WHEN THE FINAL BOLT PATTERN IS RECEIVED FROM THE ANTENNA MANUFACTURER. IF NEW ADAPTER PLATE SHOULD BE REQUIRED, NEW ADAPTER PLATE SHOULD DEVELOP MAXIMUM ALLOWABLE CAPACITY OF TOP ANTENNA CONNECTED TO PLATE

**NOTE:** ONLY REPLACE THIS ADAPTER PLATE IF NEW ANTENNA BOLT PATTERN IS DIFFERENT AS SHOWN OR ONE (1) BOLT HOLE MAY BE REQUIRED TO BE ORIENTED NORTH.

**SECTION 'A'-'A'**

7'-0" c-c

(12) 1 1/8" DIA. MOUNTING HOLES FOR (12) 1" DIA. (A325X) BOLTS  
 (20) 1 3/8" DIA. HOLES ON A 1'-8 1/4" B.C. FOR (20) 1 1/4" DIA. BOLTS SUPPLIED BY ANTENNA MANUFACTURER

1'-3 1/4" CENTER HOLE

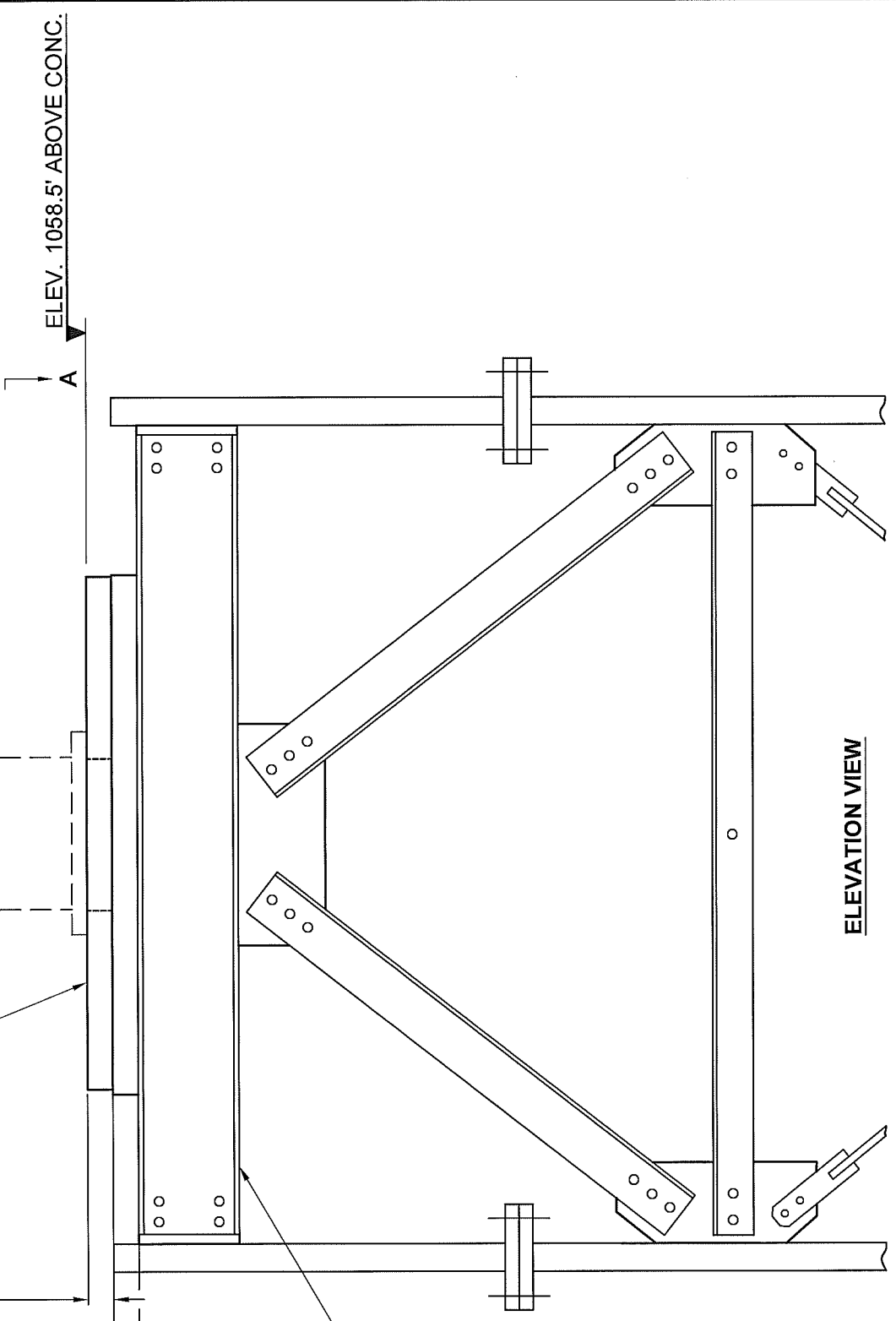
**PROPOSED ANTENNA:**  
 DIELECTRIC TOP MOUNTED ANTENNA  
 ANTENNA TYPE - TFU-20ETTAP-R O6 (SP)  
 CH 31 CALL LETTERS - WMIT  
 WEIGHT = 6.5 KIPS  
 LENGTH = 42.5 FT. OVERALL

EXISTING ANTENNA ADAPTER PLATE DETAILED CONFIGURATION SHOWN ON SECTION 'A'-'A'

EXISTING 2" THICK (A36) ADAPTER PLATE

EXISTING 2" THICK (A36) PLATE

EXISTING TOP ANTENNA PLATFORM ASSEMBLY



**ELEVATION VIEW**



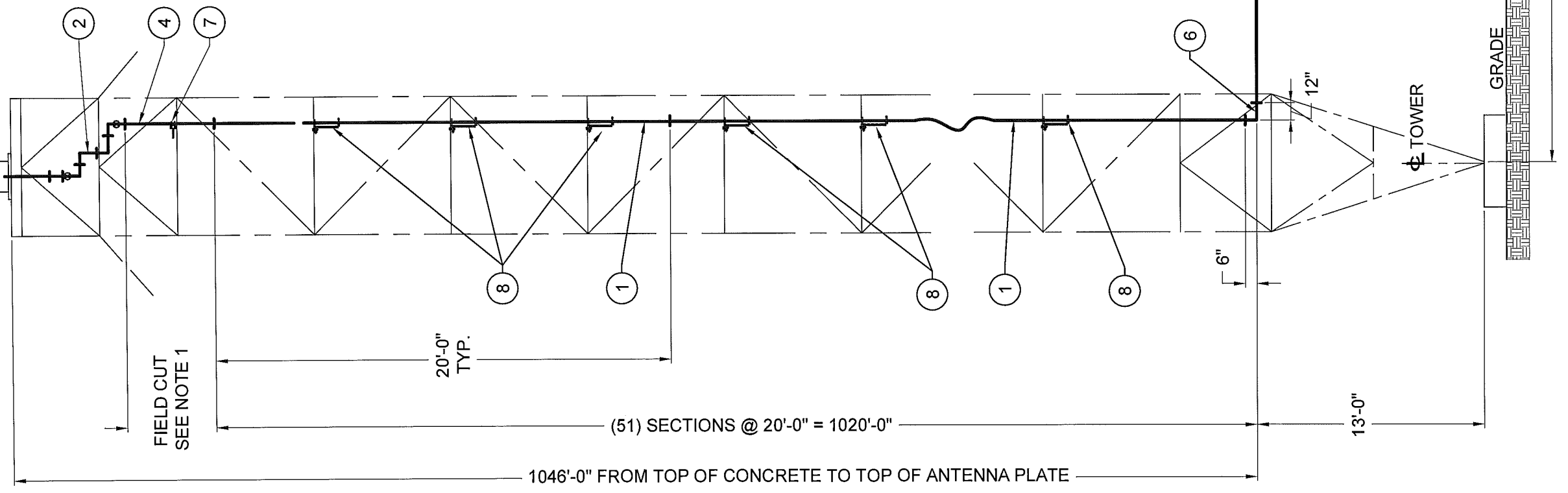
**TOP ANTENNA ADAPTER PLATE DETAILS**  
 NEW BRITAIN, CT

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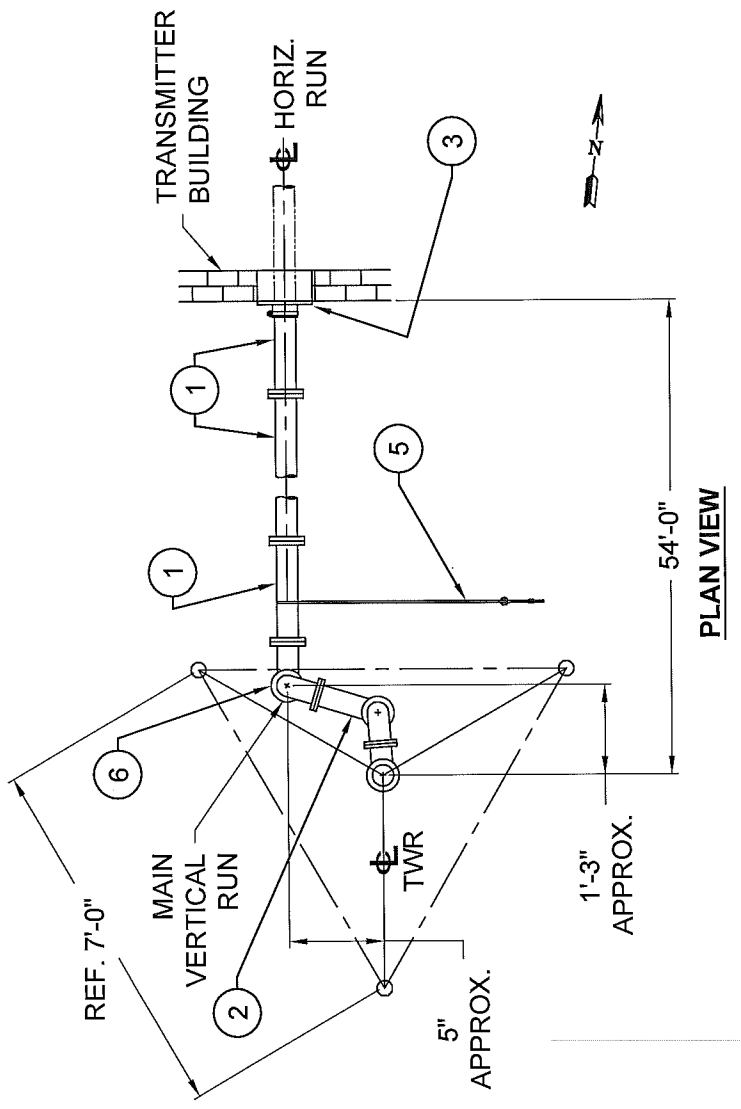
REV	BY	DATE	REVISION DESCRIPTION	D.C.K	DATE	E.C.K	DATE
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PREPARED BY	RE	11/8/17
CHECKED BY	GA	11/28/17
ENGINEER REVIEW	PEC	11/30/17
PROJECT NUMBER		258114
DRAWING NUMBER		D05.06

DIELECTRIC TOP MOUNTED ANTENNA  
 ANTENNA TYPE - TFU-20ETT/VP-R 06  
 CH 31 CALL LETTERS - WWIT, SEE  
 DIELECTRIC DRAWINGS FOR DETAILS



ELEVATION VIEW

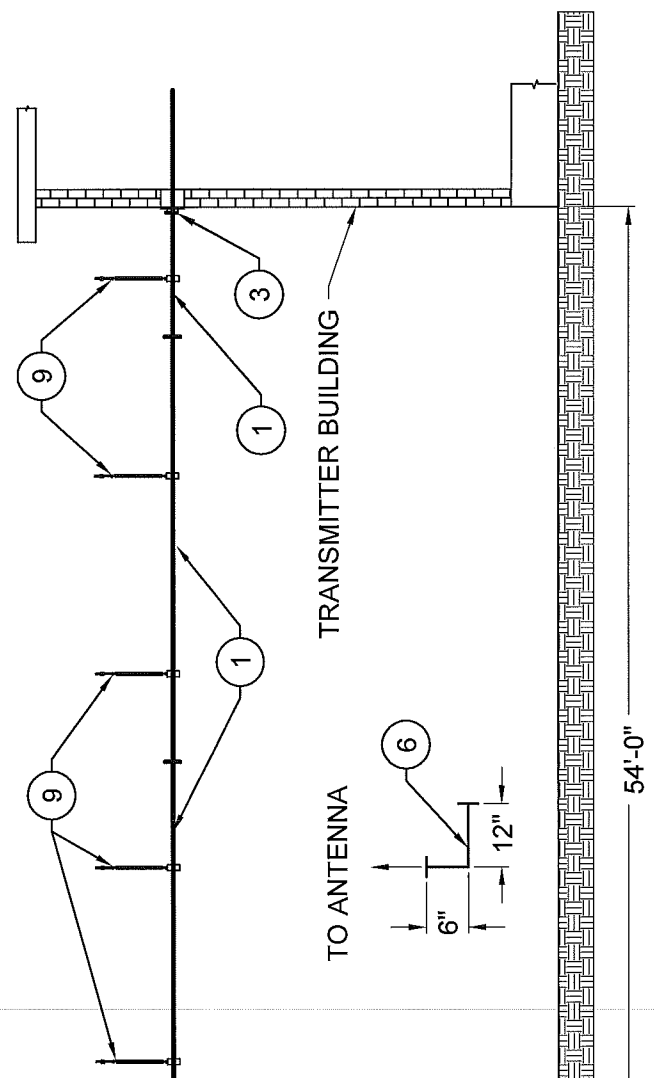


PLAN VIEW

- BILL OF MATERIAL -	
ITEM #	DESCRIPTION
1	4 1/16" COAX x 20'-0" STRAIGHT SECTION
2	ANTENNA INPUT COMPLEX, SEE NOTE 6
3	4 1/16" COAX WALL ANCHOR
4	4 1/16" COAX x * (FIELD CUT) STRAIGHT SECTION
5	4 1/16" LATERAL BRACE
6	4 1/16" COAX 6" x 12" 90° REINFORCED ELBOW
7	4 1/16" COAX LIGHT DUTY FIXED HANGER
8	4 1/16" COAX VERTICAL EXPANSION HANGER
9	4 1/16" COAX 3 POINT SUSPENSION HANGER

**NOTES:**

- FIELD CUT SECTIONS ARE TO BE FIELD MEASURED PRIOR TO CUTTING. THE FIELD CUT DIMENSIONS SHOWN ARE FOR REFERENCE ONLY.
- LINES SHOWN OUT OF POSITION IN ELEVATION VIEW FOR CLARITY. SEE PLAN VIEW FOR TRUE ORIENTATION.
- TOWER SPRING HANGERS (SPACED AT 6'-3") MAY BE OMITTED FROM SOME CROSS MEMBERS TO MAINTAIN AN AVERAGE OF 10 HANGERS PER 100' OF TOWER, HOWEVER HANGERS SHOULD NOT BE OMITTED FROM TWO CONSECUTIVE TOWER MEMBERS.
- SPRING HANGER LOCATIONS TO BE PLANNED TO AVOID INTERFERENCE BETWEEN TRANSMISSION LINE FLANGES AND HANGERS.
- TWO FIXED HANGERS IS TO BE INSTALLED AT THE TOP OF MAIN VERTICAL RUN.
- FOR ANTENNA INPUT COMPLEX SEE DIELECTRIC DRAWINGS.



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 King of Prussia, PA 19606  
**COAX ARRANGEMENT - WWIT - CH 31**  
**NEW BRITAIN, CT**

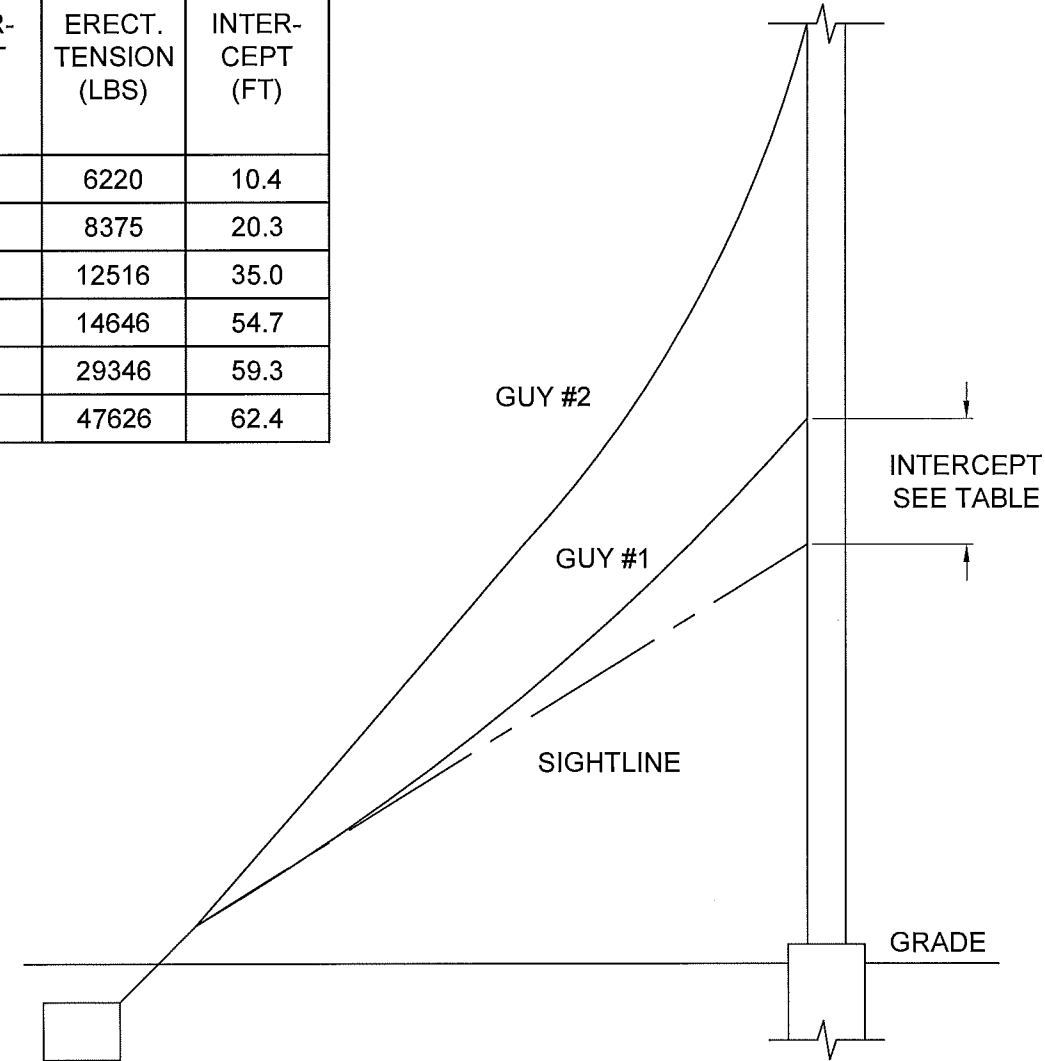
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REV	BY	DATE	REVISION DESCRIPTION	D.CK	DATE	E.CK	DATE
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PREPARED BY	RE	11/8/17
CHECKED BY	GA	11/23/17
ENGINEER REVIEW	PRC	11/30/17
PROJECT NUMBER		258114
DRAWING NUMBER		D06.00



	0 DEG. F		20 DEG. F		40 DEG. F		60 DEG. F		80 DEG. F		100 DEG. F	
	ERECT. TENSION (LBS)	INTER-CEPT (FT)	ERECT. TENSION (LBS)	INTER-CEPT (FT)	ERECT. TENSION (LBS)	INTER-CEPT (FT)	ERECT. TENSION (LBS)	INTER-CEPT (FT)	ERECT. TENSION (LBS)	INTER-CEPT (FT)	ERECT. TENSION (LBS)	INTER-CEPT (FT)
1A	10451	6.2	9522	6.9	8616	7.6	7760	8.4	6958	9.3	6220	10.4
2A	11499	14.9	10815	15.8	10154	16.8	9520	17.9	8934	19.1	8375	20.3
3A	14913	29.7	14410	30.6	13915	31.7	13420	32.8	12968	33.9	12516	35.0
4A	16544	48.8	16151	49.9	15758	51.0	15360	52.3	15011	53.4	14646	54.7
5A	31608	55.3	31143	56.1	30678	56.9	30200	57.7	29780	58.5	29346	59.3
6A	49621	60.0	49222	60.5	48823	60.9	48360	61.5	48025	61.9	47626	62.4



**ELEVATION VIEW**

**NOTES:**

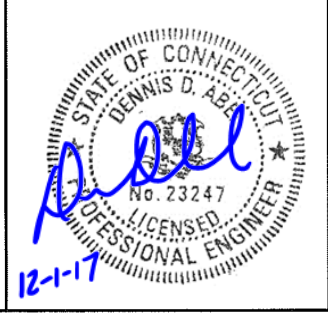
1. DURING THE INITIAL GUY TENSIONING PROCEDURES AND AT THE TIME OF INSPECTION, THE GUY TENSIONS AND/OR INTERCEPTS SHOULD BE IN ACCORDANCE WITH THE VALUES SHOWN ABOVE. USE THE TEMPERATURE WHICH ACTUALLY EXISTS AT THE TIME THE TENSION IS BEING CHECKED. FOR TEMPERATURES OTHER THAN THOSE SHOWN ABOVE, INTERPOLATE OR EXTRAPOLATE OTHER VALUES.
2. TOWER PLUMBING AND INITIAL TENSIONING OF GUYS SHOULD BE DONE ONLY IN CALM WEATHER AND WITH NO ICE ON GUYS.
3. USE INTERCEPTS AND TENSIONS IN GUY DIRECTION "A" ONLY.
4. GUY #1 IS BOTTOM GUY; GUY #2 IS NEXT, ETC.
5. USE SIGHT BAR FOR DETERMINING GUY INTERCEPTS.
6. TENSION AND/OR INTERCEPT TOLERANCES +/- 5%.

RE	11/8/17											
PREPARED BY	GA											
CHECKED BY	11/29/17											
ENGINEER REVIEW	Pce	11/30/17										
PROJECT NUMBER	258114											
DRAWING NUMBER	D08.00											
DATE												
E.C.K.												
DATE												
D.C.K.												
REVISION DESCRIPTION												
DATE												
REV BY												

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**INTERCEPTS & ERECTION TENSIONS**  
 NEW BRITAIN, CT

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# Attachment 1





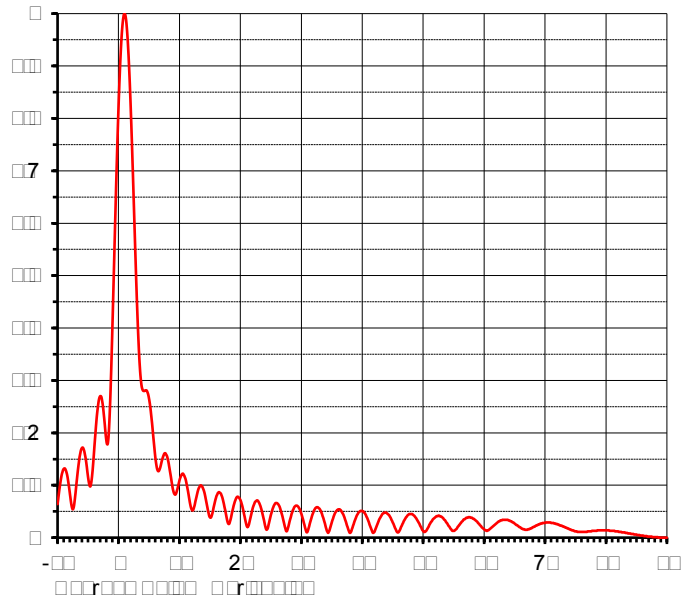
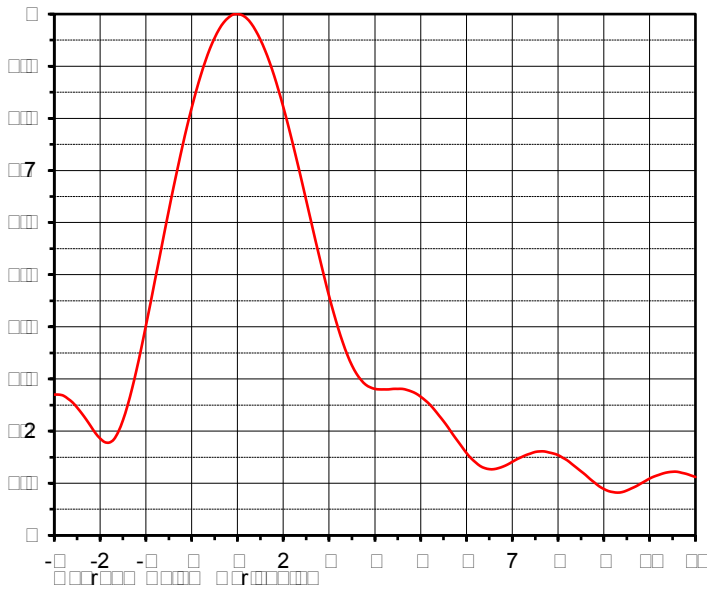


## ELEVATION PATTERN

C-70211-7  
 15-Feb-17  
 WVIT 31  
 575 MHz  
 TFU-20ETT/VP-R O6

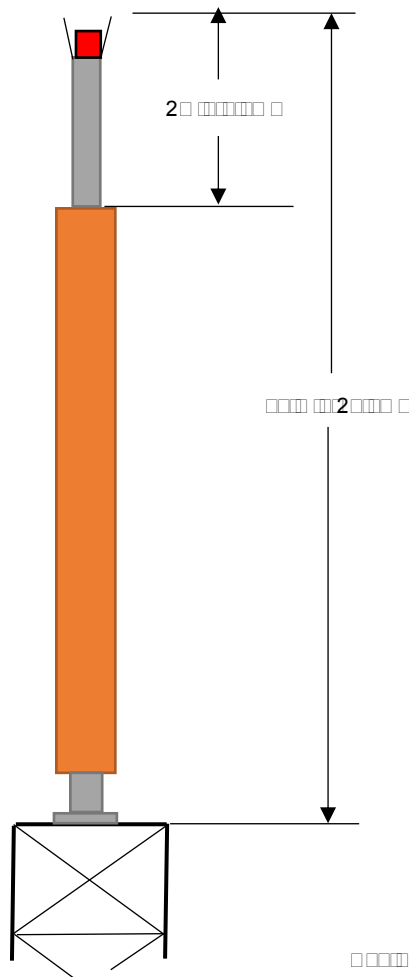
19.60 ( 12.92 dB )  
 13.20 ( 11.21 dB )  
 Calculated

1.00 deg  
 20E196100



Angle	Field	Angle	Field	Angle	Field	Angle	Field	Angle	Field
-10	13.20	0	19.60	10	13.20	7	12.92		
-9	13.20	1	19.60	9	13.20	6	12.92		
-8	13.20	2	19.60	8	13.20	5	12.92		
-7	13.20	3	19.60	7	13.20	4	12.92		
-6	13.20	4	19.60	6	13.20	3	12.92		
-5	13.20	5	19.60	5	13.20	2	12.92		
-4	13.20	6	19.60	4	13.20	1	12.92		
-3	13.20	7	19.60	3	13.20	0	12.92		
-2	13.20	8	19.60	2	13.20	-1	12.92		
-1	13.20	9	19.60	1	13.20	-2	12.92		
0	13.20	10	19.60	0	13.20	-3	12.92		
1	13.20	11	19.60	1	13.20	-4	12.92		
2	13.20	12	19.60	2	13.20	-5	12.92		
3	13.20	13	19.60	3	13.20	-6	12.92		
4	13.20	14	19.60	4	13.20	-7	12.92		
5	13.20	15	19.60	5	13.20	-8	12.92		
6	13.20	16	19.60	6	13.20	-9	12.92		
7	13.20	17	19.60	7	13.20	-10	12.92		
8	13.20	18	19.60	8	13.20	-11	12.92		
9	13.20	19	19.60	9	13.20	-12	12.92		
10	13.20	20	19.60	10	13.20	-13	12.92		

This document contains confidential information and is intended for the use of the individual named. If you have received this document by mistake, please notify the sender immediately by e-mail at [email address]. Do not disseminate, distribute, or copy this document.



## MECHANICALS

r... C-70211-7  
 15-Feb-17  
 r... WVIT 31  
 r... 575 MHz  
 r... TFU-20ETT/VP-R O6

r... r... r...

### Top Mounted

#### Mechanical Specification without ice TIA-222-G

Height AGL(z) 7m 22m  
 Basic Wind Speed 7 m/s

Structure Class II  
 Exposure Category  
 Topography Category

#### Mechanical Specifications with ice TIA-222-G

t<sub>iz</sub> = 2m  
 d d

#### Mechanical Specifications

	without ice	with ice	full stack	full stack with ice
...	...	...	2m	...
...	2m	...	...	...
...	...	...	...	...
...	2.7m	7.2m	7m	7.7m
Mod ...	...	...	2m	2m

...

...

Prepared by: ... Date: 02-02-17 ME: SPJC EE:  
 Rev. No.7 by: ... Date: 2-02-17

...

## Summary

Dielectric Corporation      **C-70211-7**  
15-Feb-17  
WVIT      31  
575 MHz  
TFU-20ETT/VP-R 06

## Antenna

	Hpol	Vpol
ERP:	<b>374.0 kW ( 25.73 dBk )</b>	<b>187.0 kW ( 22.72 dBk )</b>
M	7	

**Antenna Input Power**      28.6 kW ( 14.57 dBk )

## Transmission Line

Loss      d                2 d  
           "                7  
d      7  
           2

## Transmitter Output

**38.0 kW ( 15.80 dBk )**

Dielectric Corporation      15-Feb-17

Dielectric Corporation      15-Feb-17



# Attachment 2



A BUSINESS OF FDH VELOCITEL

# REPORT 258113

DATE: 5/12/2017

RIGOROUS STRUCTURAL ANALYSIS  
FOR A 1057' STEEL HEIGHT G-7 GUYED TOWER  
NEW BRITAIN, CT

PREPARED BY: AP  
CHECKED BY: PCC

APPROVED: DDA



□ □ □ □ □ □ □ □ 2 □ □ 7	□ □ □ □ □ □ 2-7	□ □ □ □ r □ □ □ □ □ □ □ □ □ □ □ □ □ d □ □ □ □ □ □ □ □ □ □
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Rev.	Date	Description
□	□□□□7	□□□□d □□□□ □□ □ □

<u>SECTION</u>	<u>PAGE</u>
A. AUTHORIZATION/PURPOSE .....	1
B. TOWER HISTORY .....	1
C. CONDITIONS INVESTIGATED .....	2
D. LOADS AND STRESSES .....	3
E. METHOD OF ANALYSIS .....	4
F. RESULTS .....	4
G. CONCLUSIONS AND RECOMMENDATIONS .....	5
H. PROVISIONS OF ANALYSIS .....	7
 <u>APPENDIX</u>	
GENERAL ARRANGEMENT .....	E-1
LINEAR APPURTENANCES .....	A-2



Rev.	Date	Description
0	05/12/17	000000d 0000 000 0 0

**A. AUTHORIZATION/PURPOSE**

As authorized by Joe DiMaggio of WVIT-TV, a structural analysis was performed to investigate the adequacy of a 1057' steel height Stainless G-7 guyed tower near New Britain, Connecticut to support specified equipment.

**B. TOWER HISTORY**

The tower was originally designed and furnished in 1976 by Stainless, Inc. It was designed in accordance with EIA Standard RS-222-C for a wind load rating of 50 psf with no ice, 40 psf with 1/2" of radial ice, and 30 psf with 1" of radial ice while supporting the following equipment:

1. One (1) top mounted RCA Channel 30 Zee panel antenna with radome, fed by one (1) 8-3/16" rigid coax.
2. Two (2) 8'x12' reflectors at the first and second guy levels.
3. One (1) high intensity strobe lighting system for the full height of the tower.
4. One (1) inside climbing ladder for the full height of the tower.

In December 1978, the tower was extended to 1057' per Stainless Inc. Report 258102. The extended tower was designed to support the following:

1. One (1) top mounted RCA TFU 28G Channel 30 antenna, fed by one (1) 8-3/16" rigid coax.
2. Two (2) 8'x12' reflectors at the first and second guy levels.
3. One (1) high intensity strobe lighting system for the full height of the tower.
4. One (1) inside climbing ladder for the full height of the tower.

❖ In 2004, the tower was modified by Stainless LLC per Report 258108. The modifications consisted of the following:

- ◆ Replaced Levels 5 and 6 (topmost) guys with new, higher capacity guys.
- ◆ Adjusted initial tensions in all guy levels.
- ◆ Replaced existing diagonals with new, higher capacity members at the following bay:

Location	No of bays
1035.9' – 1010.9'	4
910.9' – 842.1'	11
517.1' – 511.5'	1

Rev.	Date	Description
0	0000007	000000d 0000 000 0 0

- ◆ Replaced existing horizontals with new, higher capacity members at the following levels:

Location	No of levels
1023.4'	1
885.9' – 848.4'	7
842.1'	1

- ◆ Installed additional horizontal sub-bracing at the midpoints of the following bays:

Location	No of bays
1023.4' – 879.6'	23
860.9' – 779.6'	13
760.9' – 754.6'	1
729.6' – 704.6'	4
679.6' – 654.6'	4
629.6' – 604.6'	4
560.9' – 554.6'	1
508.8' – 496.3'	2
490.0' – 433.8'	9
358.8' – 271.3'	14
233.8' – 208.8'	4
158.8' – 108.8'	8
15.0' – 33.8'	3

**C. CONDITION INVESTIGATED**

The analysis was performed for the tower supporting equipment based upon the following sources:

- ) Stainless Proposal P16\_2581\_001 dated 10/28/2016.
- ) Stainless LLC Report 258110 dated 6/4/2003.
- ) Stainless LLC Report 258108 dated March 2002.
- ) Email from Rick Smart of Dielectric dated 3/17/2017 with mechanical specifications for the proposed top antenna.
- ) Equipment schedule WVIT Stainless G-7 Tower 2016, undated.
- ) Emails from Joe DiMaggio of WVIT dated 11/2, 11/18, and 11/23/2016 with details of existing tower loading.
- ) Email from Joe DiMaggio dated 6/15/2017 with details of original tower geotechnical design information.
- ) Email from Joe DiMaggio dated 6/27/2017 with details of final tower loading condition.
- ) Email from David Sheppard of Drake Lighting dated 6/28/2017 with details of TechnoStrobe high intensity LED lighting system.

Rev.	Date	Description
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APPURTENANCE	ELEVATION, ft.	FEED LINES
TFU-22GTH/VP-R 4C140	1025	4-1/16" rigid
Proscan III ENG	1020	1-5/8" & 1" control cable
Outside transfer platform	1000	--
Station Master omni	520	7/8"
8' x 9' ice shield	360	--
PA4-65 dish/radome	350	WEP65
ENGensis ENG	335	7/8" & 1" control cable
DB-408 omni	330	7/8"
PA4-65 dish/radome	320	WEP65
Diamond X-50A omni	140	7/8"
6' x 7' ice shield	110	--
PA6-65 dish/radome	100	WEP65
Ladder with cable safety device	Full height of tower	3/8" safety cable
<b>Proposed equipment:</b>		
<b>TFU-20ETT/VP-R O6 (NB: replaces existing TFU-28G)</b>	<b>Tower top</b>	<b>6-1/8" rigid (NB: replaces existing 8-3/16" rigid)</b>
<b>Step down transformer</b>	<b>945</b>	--
<b>(3) 3' x 4' ice shields</b>	<b>945</b>	--
<b>(6) ENGensis panel antennas (3) ENGensis radios</b>	<b>940</b>	<b>1/2" fiber 1-1/4" conduit (existing)</b>
<b>TechnoStrobe LED lighting system (NB: replaces existing strobes)</b>	<b>Full height of tower</b>	<b>Armored cable, diameters vary up the tower</b>

The transmission line arrangement was based upon Stainless LLC Report 258110 dated 6/4/2003. Lines with unknown locations on the cross section are conservatively assumed to be fully exposed to wind. The locations of all existing and proposed transmission lines are shown on page A-2 of the report. Deviating from this arrangement may invalidate the results of the analysis presented in this report.

#### D. LOADS AND STRESSES

The analysis was performed using the following design parameters in accordance with the 2016 Connecticut State Building Code and ANSI/TIA 222-G-2005, Structural Standard for Antenna Supporting Structures and Antennas, including Addenda 1 and 2 dated 2007 and 2009 respectively:

- ) Risk Category II
- ) 125 mph ultimate design wind speed with no ice
- ) 50 mph nominal design wind speed with 1" design ice thickness
- ) Exposure Category B
- ) Topographic Category 5 (Flat topped hill, H=220', L=1670', x=0')
- ) 0.18 earthquake spectral response acceleration at short periods (S<sub>s</sub>)
- ) Earthquake Site Class D

Rev.	Date	Description
0	000007	00000d 0000 0000 0 0

The ultimate design wind speed is converted to a nominal design wind speed for use in ANSI/TIA 222-G based upon the following formula:

$$\begin{aligned}
 V_{asd} &= V_{ult} * (0.6)^{1/2} \\
 &= 125 * (0.6)^{1/2} \\
 &= 97 \text{ mph}
 \end{aligned}$$

Seismic effects need not be considered as the value of  $S_s$  is less than 1.0 per Section 2.7.3 of ANSI/TIA 222-G. Load and resistance factors used to evaluate the adequacy of the structure were in accordance with ANSI/TIA 222-G.

**E. METHOD OF ANALYSIS**

The analysis was performed using tnxTower, a computerized program which idealizes the tower as a structure consisting of finite elements, and subjected to simultaneous transverse and axial loads.

**F. RESULTS**

The results of the analysis show the following ratings:

COMPONENT	SPAN	RATING %
Tower top	--	30
Leg compression	Cantilever	33
	6	105
	5	<b>154</b>
	4	105
	3	92
	2	103
	1	103
Leg tension	Cantilever	30
	6	46
	5	11
	4	--
	3	--
	2	--
	1	--
Diagonals	Cantilever	52
	6	105
	5	87
	4	80
	3	88
	2	<b>107</b>
	1	92

Rev.	Date	Description
□	□□□□7	□□□□d □□□□ □□ □ □

Horizontal	Cantilever	52
	6	93
	5	87
	4	65
	3	62
	2	69
	1	67
Guys	6	87
	5	82
	4	86
	3	75
	2	72
	1	70
Foundations	Tower base	<b>147</b>
	Inner anchors	94
	Outer anchors	46

The rating is defined as the percentage of the component design capacity that is used up in supporting itself and the loading from the antennas and transmission lines under the design wind and ice loading conditions. Ratings of up to 105% are considered acceptable due to tolerances in calculating the applied loads on the tower as well as component design capacities.

A second set of tower and foundation modifications have also been shown in the following section for a maximum acceptance rating of 100%.

Foundations have been reviewed based upon the original geotechnical information dated 7/19/1976 and 4/23/1979 by Clarence Welti Associates, Inc., and a geotechnical report dated 19/1/199 by Haley and Aldrich, Inc. for an adjacent tower site.

## G. CONCLUSIONS AND RECOMMENDATIONS

Based on the preceding results, the following conclusions may be drawn:

1. The tower, supporting the equipment as specified in section C of this report, is not adequate to achieve an ultimate design wind speed of 125 mph with no ice, and 50 mph nominal design wind speed with 1” design ice thickness in accordance with the 2016 Connecticut State Building Code, and ANSI/TIA 222-G with the analysis parameters of Section D.
2. In order for the tower to achieve an ultimate design wind speed of 125 mph with no ice, and 50 mph nominal design wind speed with 1” design ice thickness in accordance with the 2016 Connecticut State Building Code, and ANSI/TIA 222-G with the analysis parameters of Section D for a maximum rating of 105%, the following modifications are required:



Rev.	Date	Description
0	05/12/17	000000d 0000 000 0 0

- a. Strengthen the tower base. It is assumed there are no physical obstructions preventing the tower base remediation.
- b. Adjust the initial tension in all guy levels.
- c. Install additional horizontal sub-bracing at the midpoints of the following bays:

Location	No. of bays
779.6' – 760.9'	3
754.6' – 729.6'	4
692.1' – 679.6'	2
271.3' – 265.0'	1

- d. Replace the existing diagonal braces at the following locations with higher capacity members:

Location	No. of bays
1010.9' – 998.4'	2
290.0' – 277.5'	2

3. In order for the tower to achieve an ultimate design wind speed of 125 mph with no ice, and 50 mph nominal design wind speed with 1" design ice thickness in accordance with the 2016 Connecticut State Building Code, and ANSI/TIA 222-G with the analysis parameters of Section D for a maximum rating of 100%, the following modifications are required:

- a. Strengthen the tower base. It is assumed there are no physical obstructions preventing the tower base remediation.
- b. Adjust the initial tension in all guy levels.
- c. Install additional horizontal sub-bracing at the midpoints of the following bays:

Location	No. of bays
779.6' – 760.9'	3
754.6' – 729.6'	4
698.4' – 679.6'	3
654.6' – 648.4'	1
604.6' – 598.4'	1
365.0' – 358.8'	1
271.3' – 233.8'	6
183.8' – 158.8'	4
10.0' – 0.0'	1

- d. Remove existing sub-bracing and install reinforcing to the legs at the following bays:

Location	No. of bays
948.4' – 935.9'	2
717.1' – 704.6'	2

- e. Replace the existing diagonal braces at the following locations with new, higher capacity members:

Location	No. of bays
1010.9' – 998.4'	2
290.0' – 271.3'	3

Rev.	Date	Description
0	000007	00000d 0000 000 0

4. After the modifications are completed, the tower twist and sway at the elevations of the proposed dish under a service wind speed of 60 mph are as follows:

Dish	Elev, ft.	Twist, degrees		Sway, degrees	
		<b>105%</b>	<b>100%</b>	<b>105%</b>	<b>100%</b>
4' dish/radome	350	0.59	0.61	0.10	0.10
4' dish/radome	320	0.54	0.56	0.08	0.09
6' dish/radome	100	0.24	0.24	0.10	0.10

**H. PROVISIONS OF ANALYSIS**

The analysis performed and the conclusions contained herein are based on the assumption that the tower has been properly installed and maintained, including, but not limited to the following:

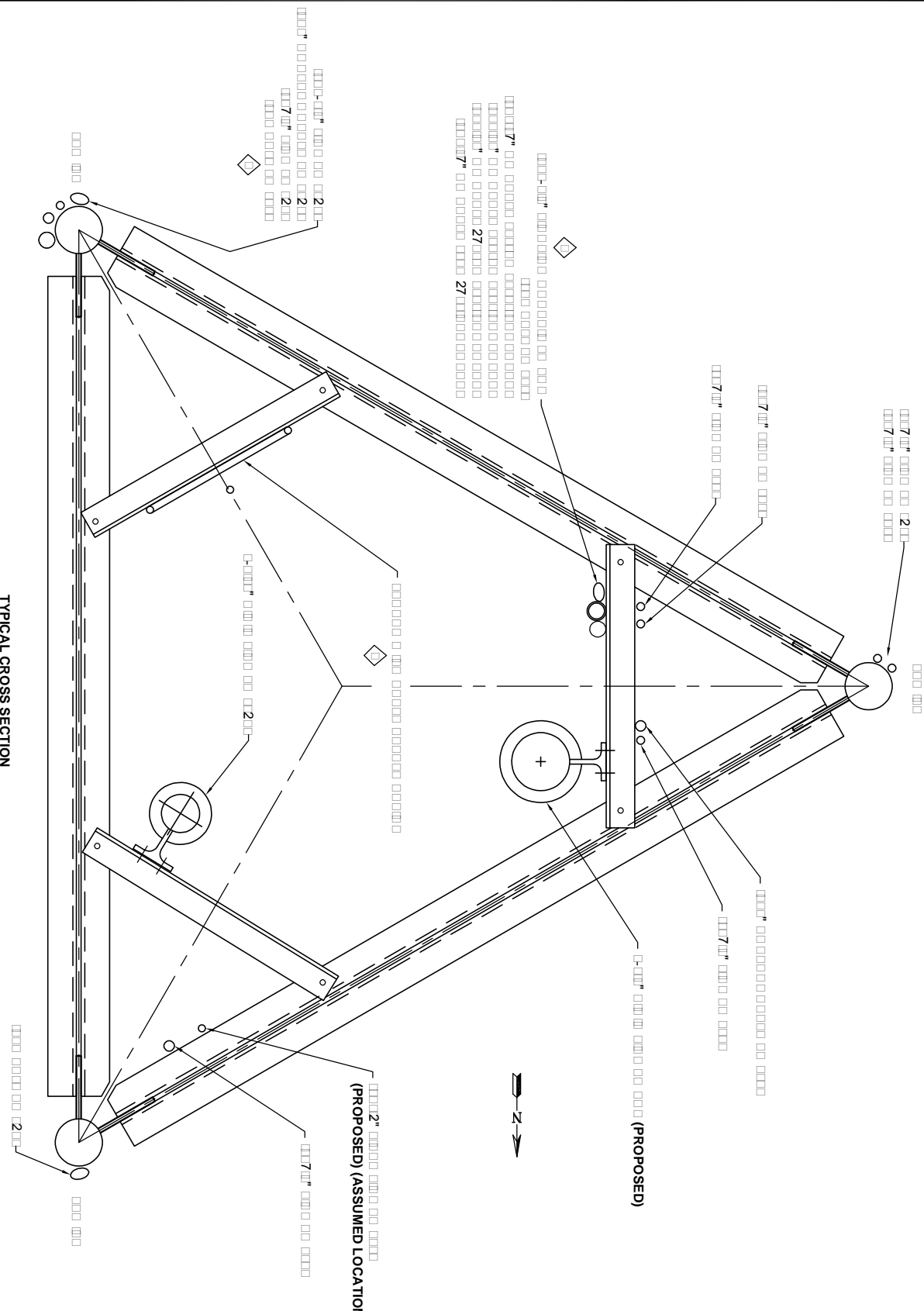
1. Proper alignment and plumbness.
2. Correct guy tensions.
3. Correct bolt tightness.
4. No significant deterioration or damage to any component.

Furthermore, the information and conclusions contained in this Report were determined by application of the current "state-of-the-arts" engineering and analysis procedures and formulae, and Stainless assumes no obligations to revise any of the information or conclusions contained in this Report in the event that such engineering and analysis procedures and formulae are hereafter modified or revised. In addition, under no circumstances will Stainless have any obligation or responsibility whatsoever for or on account of consequential or incidental damages sustained by any person, firm or organization as a result of any information or conclusions contained in the Report, and the maximum liability of Stainless, if any, pursuant to this Report shall be limited to the total funds actually received by Stainless for preparation of this Report.


Customer has requested Stainless to prepare and submit to Customer an engineering analysis with respect to the Subject Tower and has further requested Stainless to make appropriate recommendations regarding suggested structural modifications and changes to the Subject Tower. In making such request of Stainless, Customer has informed Stainless that Customer will make a determination as to whether or not to implement any of the changes or modifications which may be suggested by Stainless and that Customer will have any such changes or modifications made by riggers, erectors and other subcontractors of Customer's choice.

Customer hereby agrees and acknowledges that Stainless shall have no liability whatsoever to Customer or to others for any work or services performed by any persons other than Stainless in connection with the implementation of any structural changes or modifications recommended by Stainless including but not limited to any services rendered for Customer or for others by riggers, erectors or other subcontractors. Customer acknowledges and agrees that any riggers, erectors or subcontractors retained or employed by Customer shall be solely responsible to Customer and to others for the quality of work performed by them and that Stainless shall have no liability or responsibility whatsoever as a result of any negligence or breach of contract by any such rigger, erector or subcontractor.

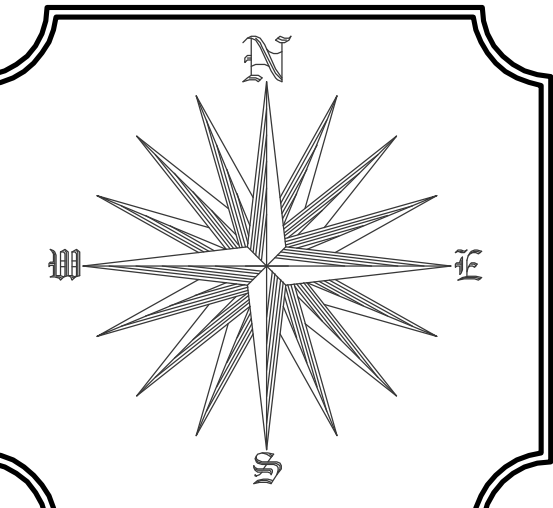




TYPICAL CROSS SECTION

 <p>STAINLESS A BUSINESS OF FDH VELOCITEL</p>			42/117
			2
	830/17	REMOVED LADDER EXTENSION, 7/8" AND 1" CTRL CABLE, LT. COND. WAS 1-1/4", ADDED 1.07", 1.31", 1.43" & 1.67 CABLES	-2

# Attachment 3



THESE ASSESSOR MAPS ARE NOT LAND RECORD MAPS AND SHOULD NOT BE USED FOR DEED DESCRIPTION OR REFERENCE. REPORT ANY INACCURACIES TO THE OFFICE OF THE TOWN ENGINEER. ALL AVENUES, STREETS, ROADS AND LANES ARE SHOWN WHETHER ACCEPTED, PROPOSED OR DEDICATED BY DEED.

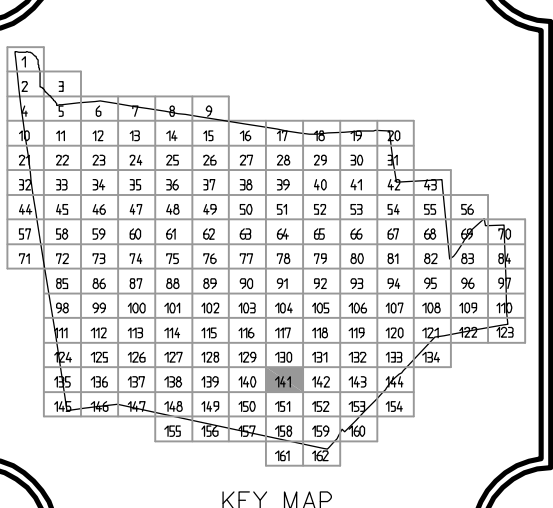
THE 500 FOOT GRID IS BASED ON THE CONNECTICUT STATE PLANE COORDINATE SYSTEM (N.A. DATUM OF 1927)

NATIONAL GEODETIC DATUM OF 1929

MAPPING CONFORMS TO NATIONAL MAP ACCURACY STANDARDS

DIGITAL PHOTOGRAMMATIC MAPPING BY: QUINN ASSOCIATES, HORSHAM, PA. DATE OF PHOTOGRAPHY MARCH 19, 1990. CONTOUR INTERVALS ARE 2 FEET.

- LEGEND**
- ⊙ - ASSESSOR NUMBER
  - x 385.6 SPOT ELEV.
  - ≡ LEDGE
  - ⊕ TRAFFIC SIGNAL
  - ⊙ IRON PIN
  - ⊙ VALVE
  - ⊙ CATCH BASIN
  - ⊙ FP FLAG POLE
  - ⊙ HYDRANT
  - ↑ FLOW ARROW
  - ⊙ R.R. SWITCH
  - ⊙ LIGHT POLE
  - ⊙ POLE
  - ⊙ SWAMP
  - ⊙ SHRUB
  - ⊙ CTREE
  - ⊙ DTREE
  - ⊙ CULVERT
  - ⊙ LIGHT PEDESTAL
  - ⊙ R.R. CATENARY SUPPORT
  - EDGE OF PAVEMENT
  - DRIVEWAY
  - SIDEWALK
  - G — G GUARD RAIL
  - X — X FENCE
  - RETAINING WALL
  - RAILROAD
  - WATERCOURSE
  - TREE LINE
  - INDEX CONTOUR
  - INTERMEDIATE CONTOUR
  - DEPRESSION CONTOUR
  - 300 CONTOUR LABEL
  - WETLAND
  - ⊙ MON
  - ⊙ BENCHMARK
  - ⊙ HAND HOLE
  - ⊙ MANHOLE
  - ⊙ WELL

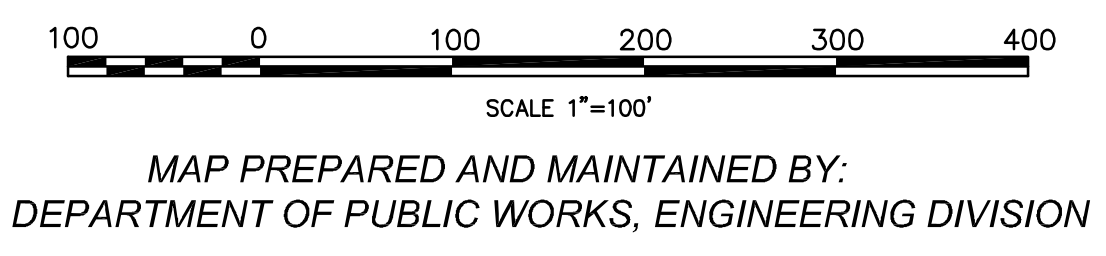


**TOWN OF FARMINGTON**

ASSESSOR'S OFFICE

1 MONTEITH DRIVE, FARMINGTON, CONNECTICUT 06032

PHONE: (860) 675-2370 FAX: (860) 675-2376



**ASSESSOR'S MAP**

OF THE

TOWN OF FARMINGTON

HARTFORD COUNTY, CONNECTICUT

SHEET NO:

**141 OF 162**

DATE PRINTED: OCTOBER 2016

The Assessor's office is responsible for the maintenance of records on the ownership of properties. Assessments are computed at 70% of the estimated market value of real property at the time of the last revaluation which was 2012.



Information on the Property Records for the Municipality of Farmington was last updated on 2/13/2018.

## Property Summary Information

Parcel Data And Values   Building ▾   Outbuildings   Sales   Google Map

### Parcel Information

Location:	200 COLT HIGHWAY	Property Use:	Industrial	Primary Use:	Utility Building
Unique ID:	03750200	Map Block Lot:	0141 7B	Acres:	10.00
490 Acres:	0.00	Zone:	EE	Volume / Page:	0554/0608
Developers Map / Lot:		Census:	4602-02		

### Value Information

	Appraised Value	Assessed Value
Land	600,000	420,000
Buildings	291,886	204,320

	Appraised Value	Assessed Value
Detached Outbuildings	0	0
Total	891,886	624,320

### Owner's Information

#### Owner's Data

OUTLET BROADCASTING INC  
E-PROPERTY TAX DEPT 201  
ONE COMCAST CENTER,32ND FL  
PHILADELPHIA, PA 19103

[Back To Search \(JavaScript:window.history.back\(1\);\)](#)

[Print View \(PrintPage.aspx?towncode=052&uniqueid=03750200\)](#)

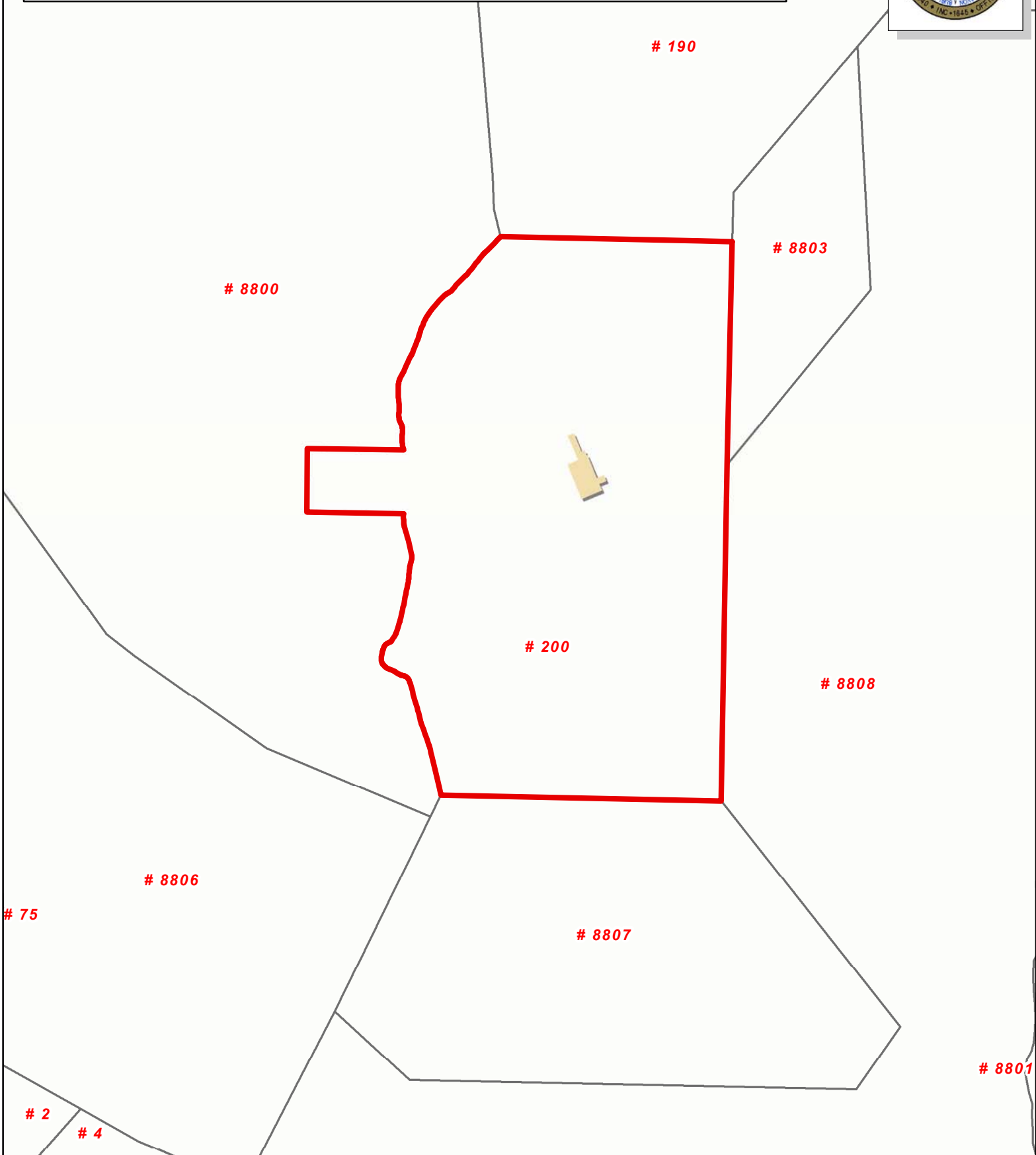
Information Published With Permission From The Assessor



# Town of Farmington, Connecticut - Assessment Parcel Map

UNIQUE ID: 03750200

Address: 200 COLT HIGHWAY



Approximate Scale: 1 inch = 200 feet

Map Produced Aug 2017

Disclaimer: This map is for informational purposes only. All information is subject to verification by any user. The Town of Farmington and its mapping contractors assume no legal responsibility for the information contained herein.

# Attachment 4



## Shipment Receipt

### Address Information

#### Ship to:

c/o NBC Universal - WVIT  
Outlet Broadcasting Inc  
Attn: Jim Moyer  
1422 New Britain Ave  
WEST HARTFORD, CT  
06110  
US  
2025246401

#### Ship from:

Anthony Flores  
  
21 Ridgecrest Drive  
  
Napa, CA  
94558  
US  
7078121311

### Shipment Information:

Tracking no.: 771487090551  
Ship date: 02/15/2018  
Estimated shipping charges: 63.78 USD

### Package Information

Pricing option: FedEx Standard Rate  
Service type: Standard Overnight  
Package type: FedEx Box  
Number of packages: 1  
Total weight: 2 LBS  
Declared Value: 0.00 USD  
Special Services:  
Pickup/Drop-off: Drop off package at FedEx location

### Billing Information:

Bill transportation to:  
Your reference:  
P.O. no.:  
Invoice no.:  
Department no.:

Thank you for shipping online with FedEx ShipManager at [fedex.com](http://fedex.com).

### Please Note

FedEx will not be responsible for any claim in excess of \$100 per package, whether the result of loss, damage, delay, non-delivery, misdelivery, or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim. Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including intrinsic value of the package, loss of sales, income interest, profit, attorney's fees, costs, and other forms of damage whether direct, incidental, consequential, or special is limited to the greater of \$100 or the authorized declared value. Recovery cannot exceed actual documented loss. Maximum for items of extraordinary value is \$1000, e.g., jewelry, precious metals, negotiable instruments and other items listed in our Service Guide. Written claims must be filed within strict time limits; Consult the applicable FedEx Service Guide for details. The estimated shipping charge may be different than the actual charges for your shipment. Differences may occur based on actual weight, dimensions, and other factors. Consult the applicable [FedEx Service Guide](#) or the FedEx Rate Sheets for details on how shipping charges are calculated.



## Shipment Receipt

### Address Information

**Ship to:**

Melanie Bachman  
Connecticut Siting Council  
Ten Franklin Square

NEW BRITAIN, CT  
06051  
US  
8608272935

**Ship from:**

Anthony Flores  
21 Ridgcrest Drive

Napa, CA  
94558  
US  
7078121311

### Shipment Information:

Tracking no.: 771487143033  
Ship date: 02/15/2018  
Estimated shipping charges: 63.78 USD

### Package Information

Pricing option: FedEx Standard Rate  
Service type: Standard Overnight  
Package type: FedEx Box  
Number of packages: 1  
Total weight: 2 LBS  
Declared Value: 0.00 USD  
Special Services:  
Pickup/Drop-off: Drop off package at FedEx location

### Billing Information:

Bill transportation to:  
Your reference:  
P.O. no.:  
Invoice no.:  
Department no.:

Thank you for shipping online with FedEx ShipManager at [fedex.com](http://fedex.com).

### Please Note

FedEx will not be responsible for any claim in excess of \$100 per package, whether the result of loss, damage, delay, non-delivery, misdelivery, or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim. Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including intrinsic value of the package, loss of sales, income interest, profit, attorney's fees, costs, and other forms of damage whether direct, incidental, consequential, or special is limited to the greater of \$100 or the authorized declared value. Recovery cannot exceed actual documented loss. Maximum for items of extraordinary value is \$1000, e.g., jewelry, precious metals, negotiable instruments and other items listed in our Service Guide. Written claims must be filed within strict time limits; Consult the applicable FedEx Service Guide for details. The estimated shipping charge may be different than the actual charges for your shipment. Differences may occur based on actual weight, dimensions, and other factors. Consult the applicable FedEx Service Guide or the FedEx Rate Sheets for details on how shipping charges are calculated.



Shipment Receipt

**Address Information**

**Ship to:**

William Warner  
Town of Farmington  
1 Monteith Drive

FARMINGTON, CT  
06032  
US  
8606752325

**Ship from:**

Anthony Flores  
21 Ridgecrest Drive

Napa, CA  
94558  
US  
7078121311

**Shipment Information:**

Tracking no.: 771487139968  
Ship date: 02/15/2018  
Estimated shipping charges: 63.78 USD

**Package Information**

Pricing option: FedEx Standard Rate  
Service type: Standard Overnight  
Package type: FedEx Box  
Number of packages: 1  
Total weight: 2 LBS  
Declared Value: 0.00 USD  
Special Services:  
Pickup/Drop-off: Drop off package at FedEx location

**Billing Information:**

Bill transportation to:  
Your reference:  
P.O. no.:  
Invoice no.:  
Department no.:

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## Shipment Receipt

### Address Information

**Ship to:**

Kathleen Eagen  
City of Farmington  
1 Monteith Drive

FARMINGTON, CT  
06032  
US  
8606752325

**Ship from:**

Anthony Flores  
21 Ridgecrest Drive

Napa, CA  
94558  
US  
7078121311

### Shipment Information:

Tracking no.: 771487164991

Ship date: 02/15/2018

Estimated shipping charges: 63.78 USD

### Package Information

Pricing option: FedEx Standard Rate

Service type: Standard Overnight

Package type: FedEx Box

Number of packages: 1

Total weight: 2 LBS

Declared Value: 0.00 USD

Special Services:

Pickup/Drop-off: Drop off package at FedEx location

### Billing Information:

Bill transportation to:

Your reference:

P.O. no.:

Invoice no.:

Department no.:

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