

EM-POCKET-052-080911

ORIGINAL

CARRIE L. LARSON
90 State House Square
Hartford, CT 06103-3702
p (860) 424-4312
f (860) 424-4370

www.pullcom.com

September 10, 2008

Via Federal Express

S. Derek Phelps, Executive Director
Connecticut Siting Council
Ten Franklin Square
New Britain, CT 06051

RECEIVED
SEP 11 2008

**CONNECTICUT
SITING COUNCIL**

**Re: Notice of Exempt Modification
Crown Castle Telecommunications Facility
130 Birdseye Road, Farmington, Connecticut**

Dear Mr. Phelps:

Youghiogheny Communications-Northeast, LLC, doing business as Pocket Communications ("Pocket"), intends to install antennas and appurtenant equipment at the existing 140-foot monopole facility owned by Crown Castle, USA, Inc. and located at 130 Birdseye Road, Farmington, Connecticut ("Facility"). Pocket Communications provides prepaid, flat rate wireless voice and data services to more than a quarter of a million subscribers. Pocket is licensed by the Federal Communications Commission (FCC) to provide PCS wireless telecommunications service in the State of Connecticut, which includes the area to be served by the proposed installation. This installation constitutes an exempt modification pursuant to the Public Utility Environmental Standards Act, Connecticut General Statutes Section 16-50g et. seq. (PUESA), and Section 16-50j-72(b)(2) of the Regulations of the Connecticut State Agencies adopted pursuant to PUESA. In accordance with R.C.S.A. Section 16-50j-73, a copy of this notice has been sent to Kathleen Eagen, Farmington Town Manager.

The existing Facility consists of a 140-foot self-supporting monopole tower capable of supporting multiple carriers within a fenced compound. The coordinates for the Facility are **Lat: 41°-42'-57"** and **Long: 72°- 48'-39"**. The tower is located about 700 feet east of Birdseye Road, roughly 2,600 feet due west of Route 6, and 3,300 feet due west of Interstate 84. The Facility is in the eastern portion of Farmington, roughly a mile northwest of the New Britain town line (see Site Map, attached as Exhibit A). The tower currently supports T-Mobile antennas at the one hundred foot level (100') centerline AGL (above ground level), Verizon antennas at the one hundred ten foot level (110') AGL, Nextel antennas at the one hundred twenty foot level (120') AGL, AT&T antennas at the one hundred thirty foot level (130') AGL and Sprint antennas at the one hundred thirty nine foot level (139') AGL. Pocket proposes to install three Kathrein 742-213 flush mount antennas on the tower at the ninety foot centerline (90') AGL, and a Nortel CDMA Micro BTS 3231 cabinet, mounted on an "H-Frame," contained

Page 2

within a six foot by six foot (6'-0" x 6'-0") lease area. A small GPS antenna will be mounted to the ice bridge which will run from the lease area to the tower. Utilities will be run via a proposed underground conduit from an existing utility backboard and transformer just east of the compound (See Design Drawings and Equipment Specifications, attached as Exhibits B and C respectively).

For the following reasons, the proposed modifications to the Birdseye Road Facility meet the exempt modification criteria set forth in R.C.S.A. Section 16-50j-72(b)(2):

1. The proposed modification will not increase the height of the tower as Pocket's antennas will be installed at a center line height of approximately 90 feet .
2. The installation of Pocket's equipment and shelter will not require an extension of the site boundaries.
3. The proposed modifications will not increase the noise levels at the existing Facility by six decibels or more.
4. The operation of the additional antennas will not increase the total radio frequency (RF) power density, measured at the site boundary, to a level at or above the standard adopted by the Connecticut Department of Environmental Protection as set forth in Section 22a-162 of the Connecticut General Statutes and MPE limits established by the Federal Communications Commission. The worst-case RF power density calculations for the proposed Pocket antennas would be 28.34% of the FCC standard (see general power density calculations table, attached as Exhibit D).

Also attached, Exhibit E, is a structural analysis confirming that the tower can support the existing and proposed antennas and associated equipment.

For the foregoing reasons, Pocket respectfully submits that the proposed antenna installation and equipment at the Farmington Facility constitutes an exempt modification under R.C.S.A. Section 16-50j-72(b)(2)

Respectfully Submitted,



Carrie L. Larson

cc: Kathleen Eagen, Farmington Town Manager.
Media Park Realty Inc., underlying property owner.

Exhibit A

Site Map

Pocket Site NHCT0371E

**130 Birdseye Road,
Farmington, Connecticut**

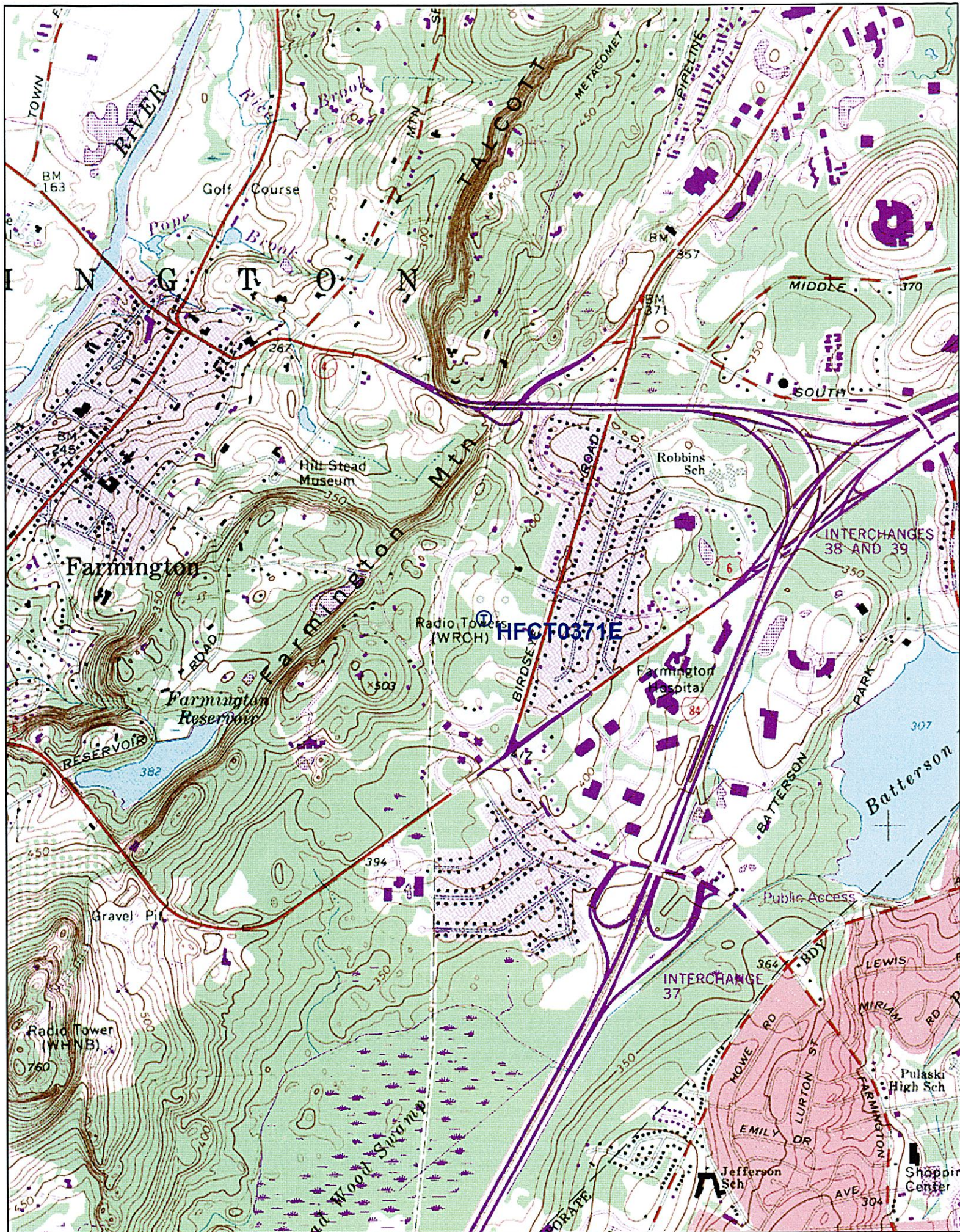


Exhibit B

Design Drawings

Pocket Site NHCT0371E

**130 Birdseye Road,
Farmington, Connecticut**



SHEET NO. T-1 Sheet No. 1 of 14

TRUE NORTH

30° ALPHA SECTOR

150° BETA SECTOR

270° GAMMA SECTOR

RFS UNIVERSAL RING MOUNT (P/N 917010).
CONTRACTOR TO SUPPLY
MOUNTING PIPES &
OTHER ASSOCIATED
HARDWARE FOR
COMPLETE ANTENNA
INSTALLATION.

PROPOSED POCKET COMMUNICATIONS
PANEL ANTENNA TYP.
OF THREE (3)
KATHREIN 742 213

RFS UNIVERSAL RING MOUNT (P/N 917010).

RAD CENTER ELEVATION
(REFER TO 1/C-2)

EXISTING MONOPOLE TOWER

COAX CABLES MOUNTED TO
EXISTING MONOPOLE TOWER @ 4'
O.C. VERTICAL MAX. W/ RFS
UNIVERSAL EXPANSION
COMPRESSION BAND (P/N
917028)

NOTE:

1. VERIFY COMPATIBILITY OF THIS COAX MOUNTING DETAIL AND
COAX ROUTING REQUIREMENTS WITH TOWER STRUCTURAL
ANALYSIS REPORT PRIOR TO INSTALLATION OF COAX.

1
C-3

ANTENNA MOUNTING CONFIGURATION

NOT TO SCALE

PLAN VIEW

TYPICAL SECTOR ELEVATION

2
C-3

VERTICAL COAX MOUNTING
CONFIGURATION DETAIL

NOT TO SCALE

TRUE NORTH

VERTICAL COAX MOUNTING
CONFIGURATION DETAIL

ANTENNA KEY

	# ANTENNAS PER SECTOR	ANTENNA NUMBER	COAX COLOR CODE	ANTENNA VENDOR	MODEL NUMBER	AZIMUTH	C/L HEIGHT	MECHANICAL DOWNTILT	COAX SIZE	# COAX PER ANTENNA	COAX MODEL NUMBER
ALPHA FACE	1	A-1	(1)-RED BAND	KATHREIN	742 213	30°	90'-0"	0°	1 5/8"	2	COMMSCOPE
BETA FACE	1	B-1	(1)-GREEN BAND	KATHREIN	742 213	150°	90'-0"	0°	1 5/8"	2	COMMSCOPE
GAMMA FACE	1	C-1	(1)-BROWN BAND	KATHREIN	742 213	270°	90'-0"	0°	1 5/8"	2	COMMSCOPE

TOWER NOTES:

1. FOR DETAILED TOWER INFORMATION, REFER TO TOWER
ERECTION DRAWINGS BY OTHERS. THE TOWER ON SHEET
C-2 IS SHOWN GENERAL CONFIGURATION PURPOSES ONLY.

2. ANTENNA CONFIGURATION IS SUBJECT TO CHANGE. VERIFY
ANTENNA HEIGHT, DOWN-TILT, AND AZIMUTH WITH PROJECT
MANAGER PRIOR TO CONSTRUCTION.

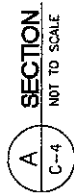
3. THE TOWER STRUCTURAL SHALL BE REVISED TO REFLECT
ANY REVISION IN ANTENNA OR COAX CONFIGURATION.

ANTENNA NOTES:

1. ALL COAX SHALL BE COLOR CODED AT (2) PLACES EACH:
AT ANTENNA AND AT CABINET.

2. (2) COLOR BANDS DENOTES TRANSMIT.

3. PRIOR TO ORDERING ANY ANTENNAS OR COAX, CONTRACTOR
SHALL CONTACT POCKET COMMUNICATIONS CONSTRUCTION
MANAGER AND OBTAIN APPROVAL FOR MATERIALS LISTED.
CONTRACTOR IS SOLELY RESPONSIBLE FOR THIS
COORDINATION.



BILL OF MATERIALS		
ITEM	DESCRIPTION	QUANTITY
①	1"ø O.D. x 3'-0" LG. ALUMINUM MOUNTING PIPE	1
②	UNIVERSAL CLAMP SET.	1

1. THE ELEVATION AND LOCATION OF THE GPS ANTENNA SHALL BE IN ACCORDANCE WITH THE FINAL RF REPORT.
2. GPS ANTENNA LOCATION MUST BE ABLE TO RECEIVE CLEAR SIGNALS FROM A MINIMUM OF FOUR (4) SATELLITES. CONFIRM WITH HANDHELD GPS PRIOR TO ESTABLISHING FINAL LOCATION OF GPS ANTENNAS.
3. THE GPS ANTENNA MOUNT IS DESIGNED TO FASTEN TO A 1" OUTSIDE DIAMETER ALUMINUM PIPE. THE PIPE MUST NOT BE THREADED AT THE ANTENNA MOUNT END. THE PIPE SHALL BE CUT TO THE REQUIRED LENGTH (MINIMUM OF 24 INCHES) USING A HAND OR ROTARY PIPE CUTTER TO ASSURE A SMOOTH AND PERPENDICULAR CUT. A HACK SAW SHALL NOT BE USED. THE CUT PIPE END SHALL BE DEBURRED AND SMOOTH IN ORDER TO SEAL AGAINST THE NEOPRENE GASKET ATTACHED TO THE ANTENNA MOUNT.

NOT TO SCALE



1. THE CONTRACTOR SHALL CALL UTILITIES PRIOR TO THE START OF CONSTRUCTION.
2. ACTIVE EXISTING UTILITIES, WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES. THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY, PRIOR TO PROCEEDING, SHOULD ANY UNCOVERED EXISTING UTILITY PRECLUDE COMPLETION OF THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
3. ALL RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED OFF SITE AND BE LEGALLY DISPOSED, AT NO ADDITIONAL COST.
4. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE EQUIPMENT AND TOWER AREAS.
5. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.
6. THE AREAS OF THE COMPOUND DISTURBED BY THE WORK SHALL BE RETURNED TO THEIR ORIGINAL CONDITION.
7. CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.
8. IF ANY FIELD CONDITIONS EXIST WHICH PRECLUDE COMPLIANCE WITH THE DRAWINGS, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER AND SHALL PROCEED WITH AFFECTED WORK AFTER CONFLICT IS SATISFACTORILY RESOLVED.
9. DIMENSIONS AND DETAILS SHALL BE CHECKED AGAINST THE PRE MANUFACTURED EQUIPMENT SHOP DRAWINGS.
10. THE CONTRACTOR SHALL VERIFY AND COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS, SLEEVES AND ANCHOR BOLTS AS REQUIRED BY ALL TRADES.
11. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318.
12. ALL CONCRETE SHALL BE NORMAL WEIGHT, 6% AIR ENTRAINED WITH A MAXIMUM SLUMP OF 4", AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS, UNLESS NOTED OTHERWISE ON THE DRAWINGS.
13. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, DEFORMED BARS.
14. PROVIDE 1 1/2 IN. MINIMUM COVER FOR REINFORCING STEEL UNLESS OTHERWISE NOTED ON THE DRAWINGS.
15. ALL STRUCTURAL STEEL IS DESIGNED BY ALLOWABLE STRESS DESIGN (ASD) AND SHALL CONFORM TO ASTM A36, (FY = 36 KSI).
16. ALL STEEL PIPE SHALL CONFORM TO ASTM A53, (FY = 35 KSI).
17. ALL STEEL MATERIAL (EXPOSED TO WEATHER) SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT DIPPED GALVANIZED) COATINGS" ON IRONS AND STEEL PRODUCTS.
18. ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC COATING (HOT-DIP)" ON IRON AND STEEL HARDWARE".

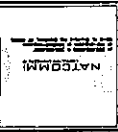
ICE BRIDGE
DETAIL AND
NOTES



- 1 ELECTRICAL DEMARCATION POINT 120/208V OR 240V. 1ø. 3W. ELECTRICAL CONTRACTOR TO COORDINATE WITH LOCAL POWER FOR SERVICE TO METER.
- 2 CONTRACTOR TO SUPPLY AND INSTALL A 100A, 120/208/240V 1ø, 3W. METER BASE. METER BASE TO BE NEMA 3R RATED AND ACCEPTABLE TO LOCAL UTILITY. PROVIDE WITH MECHANICALLY ATTACHED ENGRAVED IDENTIFICATION LABEL INDICATING "POCKET COMMUNICATIONS METER".
- 3 MAIN DISCONNECT SWITCH, CONTRACTOR TO SUPPLY AND INSTALL NEMA 3R 100A FUSIBLE DISCONNECT SWITCH WITH LOCKABLE HANDLE. PROVIDE WITH 100A FUSES, AIC RATING TO BE PER LOCAL UTILITY REQUIREMENTS. PROVIDE WITH MECHANICALLY ATTACHED ENGRAVED IDENTIFICATION LABEL INDICATING "POCKET COMMUNICATIONS SERVICE DISCONNECT".
- 4 WEATHER TIGHT JUNCTION BOX (IF REQUIRED). SIZE TO NEC CODE FOR APPLICATION.
- 5 LIQUID TIGHT FLEXIBLE METALLIC CONDUIT W/W WEATHER TIGHT FITTINGS AND SUPPORTS. SIZE AND CONTENTS TO MATCH ASSOCIATED USE (POWER OR TELCO).
- 6 UTILITY POWER ENTRY INTO CABINET. COORDINATE TERMINATION WITH CABINET MANUFACTURER.
- 7 CONTRACTOR TO SUPPLY AND INSTALL 1 1/2" GRC. AFG AND PVC 36" BFG C/W (3) #1 AWG THHN & (1) #6 GRND FOR UTILITY SERVICE. (INSTALL EXPANSION COUPLINGS AT EACH END OF RUN ABOVE GRADE).
- 8 TELCO DEMARCATION POINT. ELECTRICAL CONTRACTOR TO COORDINATE WITH LOCAL TELCO FOR SERVICE TO TELCOBOX OR CABINET.
- 9 CONTRACTOR TO SUPPLY AND INSTALL (1) 2" GRC AFG AND PVC 36" BFG C/W PULL CORDS AND T1 CABLE FOR TELCO SERVICE TO CABINET TERMINATION POINT. VERIFY WITH TELCO REQUIREMENTS (INSTALL EXPANSION COUPLINGS AT EACH END OF RUN ABOVE GRADE).
- 10 TELCO SERVICE ENTRY INTO CABINET. COORDINATE TERMINATION IN WITH CABINET MANUFACTURER.
- 11 CONTRACTOR TO ARRANGE AND PAY FOR UNDERGROUND UTILITY LOCATION SURVEYS FOR ALL TRENCHING. REUSE NATIVE BACKFILL AND RETURN TO ORIGINAL CONDITION. INSTALL 6" WIDE METALLIC LINED RED PLASTIC MARKER TAPE 8" ABOVE ALL BURIED CONDUIT.
- 12 BURIED GROUND RING.
- 13 (1) #2 SOLID BARE TINNED CU GROUNDING ELECTRODE CONDUCTOR BONDED TO 5/8"x10" COPPER CLAD STEEL GROUNDING ELECTRODES. LOCATE GROUNDING ELECTRODE ADJACENT TO "CABINET". BOND GROUNDING ELECTRODE SYSTEM TO CABINET GROUND RING.
- 14 #2 AWG GND.
- 15 #2 AWG SERVICE GROUND.
- 16 TO UTILITY SOURCE PER UTILITY REQUIREMENTS (COORDINATE WITH ELECTRIC UTILITY).
- 17 NEMA 3R TELCO BOX.
- 18 2" PVC (RGS ABOVE GROUND) W/PULL STRING AND T1 CABLE.

[illegible]

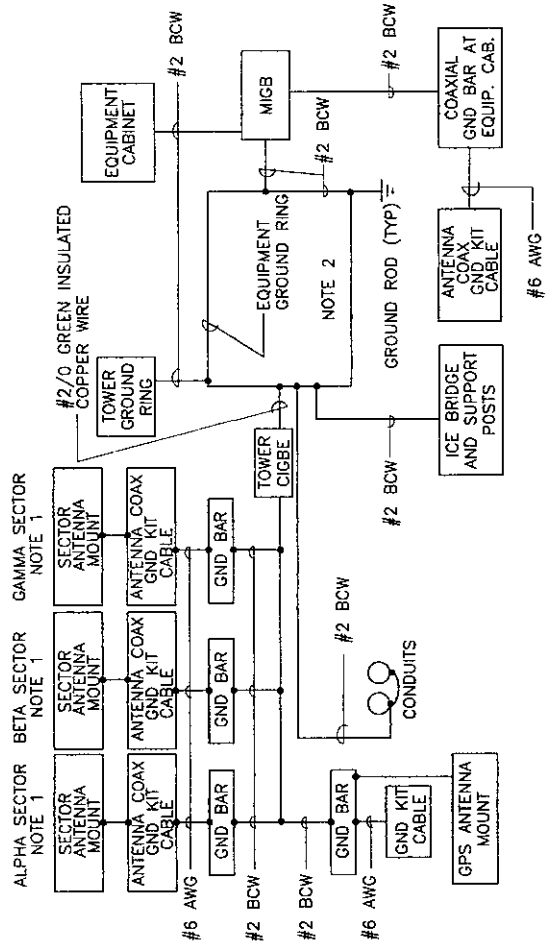
DESIGNED BY:	CTC
DRAWN BY:	DES
CHECKED BY:	CTC
DATE:	08/22/08
SCALE:	AS SHOWN
JOB NO.:	0817.0002
PROJECT:	AS SHOWN
REVISION:	
DATE:	
BY:	
DESCRIPTION:	



POCKET COMMUNICATIONS
130A BIRDSEYE ROAD
EAST FARMINGTON
FARMINGTON, CT
DATE: 08/22/08
SCALE: AS SHOWN
JOB NO.: 0817.0002

ANTENNA CABLE
GROUNDING
DETAIL

E-4
Sheet No. 11 of 15



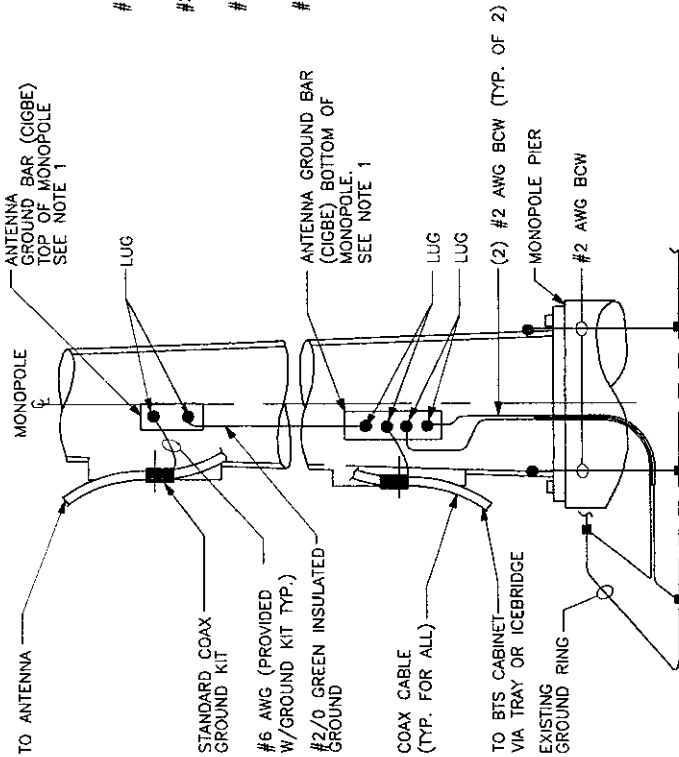
NOTES:

1. BOND ANTENNA GROUNDING KIT CABLE AND ANY OTHER ELEMENTS REQUIRING A GROUND CONNECTION (TMA'S, ETC.) TO GROUND BAR.
2. FOR GROUNDING CONNECTION & DETAILS, SEE LAYOUT DRAWINGS.

2 SCHEMATIC DIAGRAM - GROUNDING SYSTEM

NOT TO SCALE

2
E-4



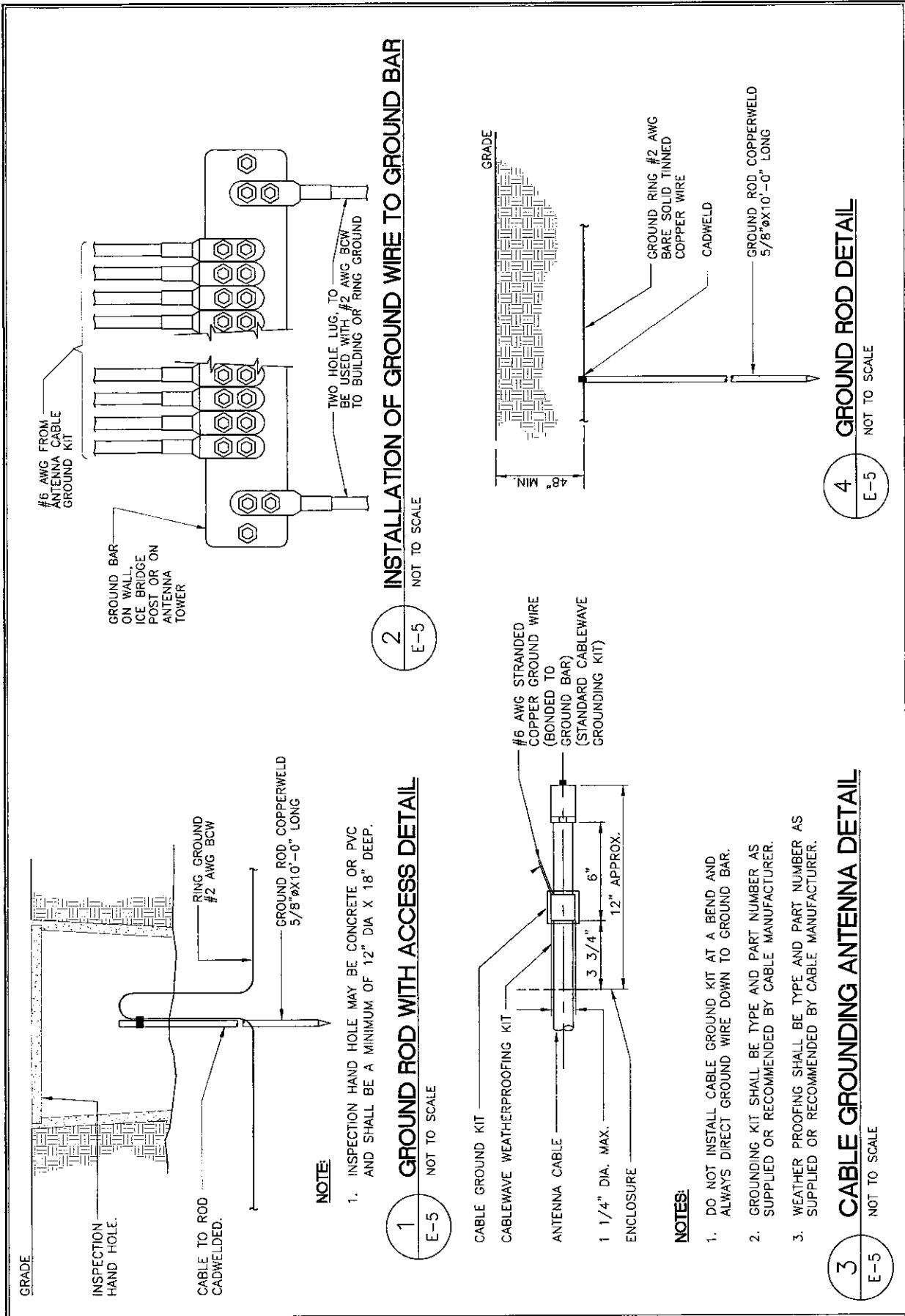
NOTES:

1. NUMBER OF GROUND BARS MAY VARY DEPENDING ON THE TYPE OF TOWER, LOCATION AND CONNECTION ORIENTATION. PROVIDE AS REQUIRED.
2. A SEPARATE GROUND BAR TO BE USED FOR GPS ANTENNA IF REQUIRED.

1 ANTENNA CABLE GROUNDING DETAIL

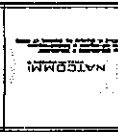
NOT TO SCALE

1
E-4





DESIGNED BY:	CTC	
DRAWN BY:	STB	
CHECKED BY:	CTC	
DATE:	03/27/08	
SCALE:	AS SHOWN	
JOB NO.:	00172002	
PROJECT NO.:		
REVISIONS:		
NO.	DATE	DESCRIPTION
1	03/27/08	ISSUED FOR REVIEW
2	04/01/08	ISSUED FOR REVIEW



POCKET COMMUNICATIONS
HFC10371E
EAST FARMINGTON
130A BIRDSEYE ROAD
FARMINGTON, CT

GROUNDING
RISER

E-7
Sheet No. 7 of 11

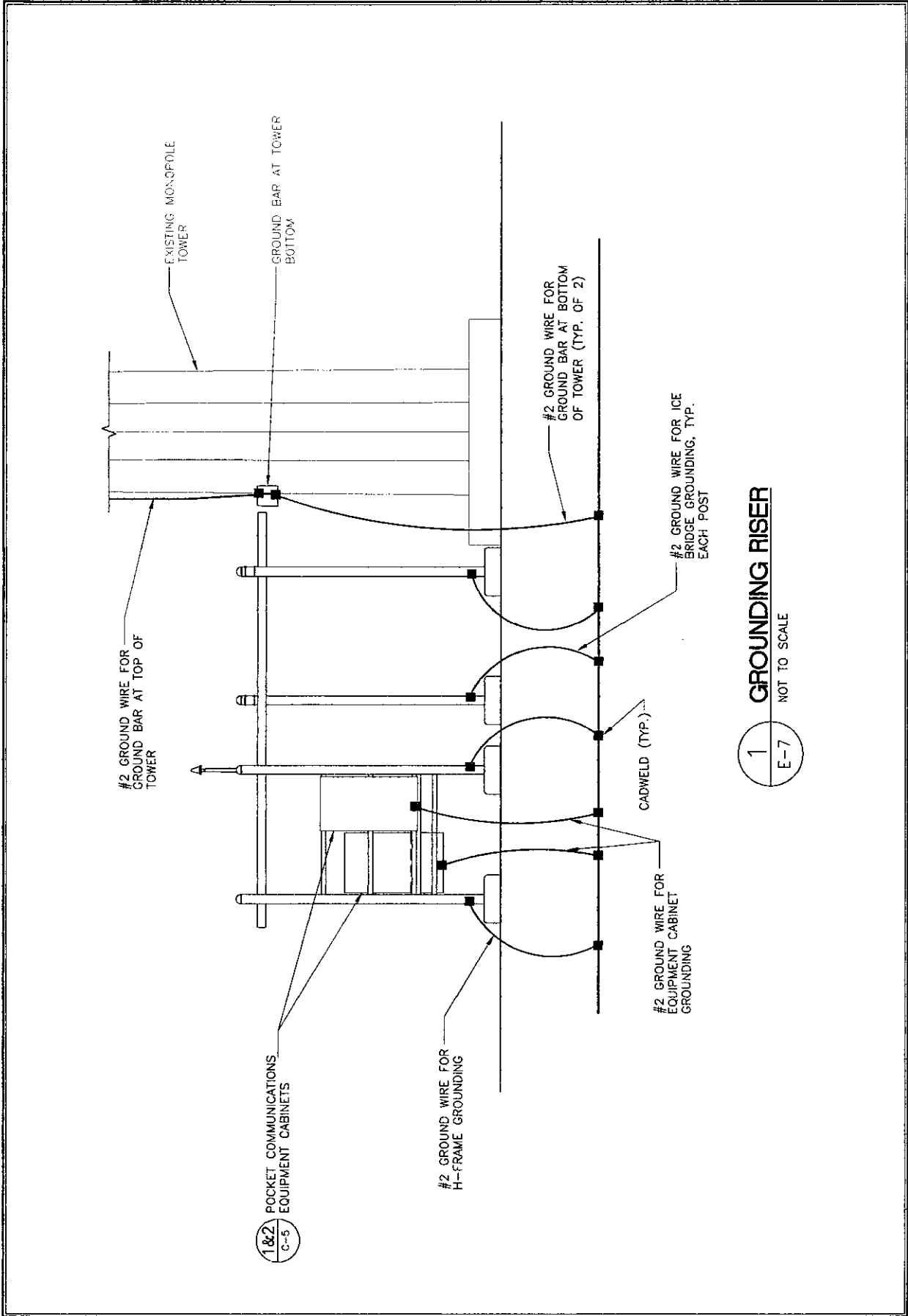


Exhibit C

Equipment Specifications

Pocket Site NHCT0371E

**130 Birdseye Road,
Farmington, Connecticut**

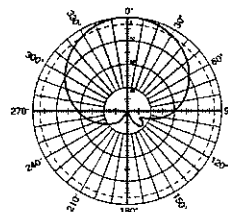
Kathrein's X-polarized adjustable electrical downtilt antennas offer the wireless carrier the ability to tailor polarization diversity sites for optimum performance. Using variable downtilt, only a few models need be procured to accommodate the needs of widely varying conditions. Remotely controlled downtilt is available as a retrofitable option.

- 0-6° downtilt range.
- UV resistant pultruded fiberglass radome.
- DC Grounded metallic parts for impulse suppression.
- No moving electrical connections.
- Wideband vector dipole technology.
- Optional remote downtilt Control.
- Will accommodate future 3G / UMTS applications.

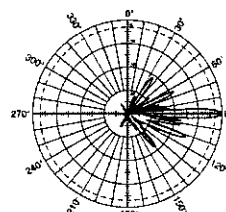
General specifications:

Frequency range	1710-2170 MHz
VSWR	< 1.5:1
Impedance	50 ohms
Intermodulation (2x20w)	IM3: < -150 dBc
Polarization	+45° and -45°
Front-to-back ratio (180°±30°)	>30 dB (co-polar) >25 dB (total power)
Maximum input power	300 watts per input (at 50°C)
Electrical downtilt continuously adjustable	0-6 degrees
Connector	2 x 7/16 DIN female
Isolation	>30 dB
Cross polar ratio	
Main direction	0° 25 dB (typical)
Sector	±60° >10 dB
Weight	22 lb (10 kg)
Dimensions	76.5 x 6.1 x 2.7 inches (1942 x 155 x 69 mm)
Equivalent flat plate area	4.62 ft² (0.429 m²)
Wind survival rating*	120 mph (200 kph)
Shipping dimensions	87.2 x 6.8 x 3.6 inches (2214 x 172 x 92 mm)
Shipping weight	24.3 lb (11 kg)
Mounting	Fixed and tilt mount options are available for 2 to 4.6 inch (50 to 115 mm) OD masts.

See reverse for order information.



Horizontal pattern
±45°- polarization



Vertical pattern
±45°- polarization



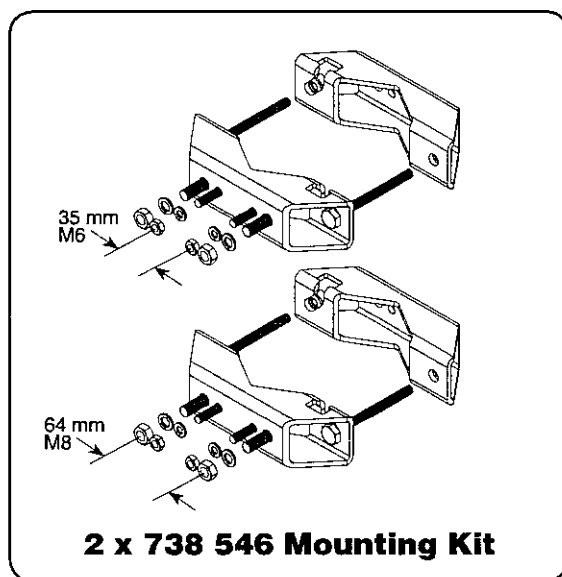
Specifications:	1710-1880 MHz	1850-1990 MHz	1920-2170 MHz
Gain	19 dBi	19.2 dBi	19.5 dBi
+45° and -45° polarization horizontal beamwidth	67° (half-power)	65° (half-power)	63° (half-power)
+45° and -45° polarization vertical beamwidth	4.7° (half-power)	4.5° (half-power)	4.3° (half-power)
Vertical Pattern—sidelobe suppression for first side- lobe above main beam	0° 2° 4° 6° T 18 17 15 15 dB	0° 2° 4° 6° T 18 18 17 15 dB	0° 2° 4° 6° T 18 18 17 15 dB



10642-H
936.2074/h

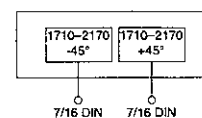
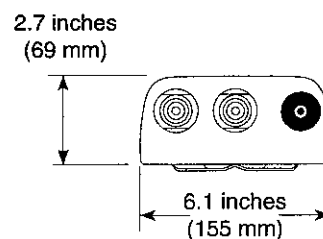
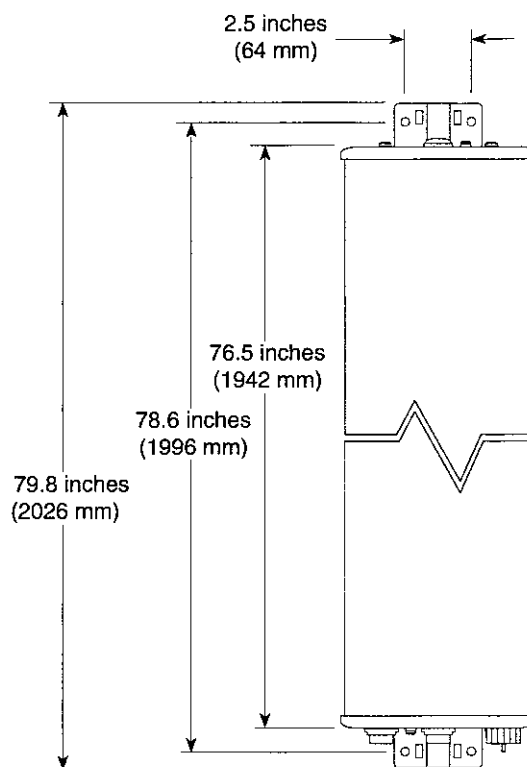


*Mechanical design is based on environmental conditions as stipulated in EIA-222-F (June 1996) and/or ETS 300 019-1-4 which include the static mechanical load imposed on an antenna by wind at maximum velocity. See the Engineering Section of the catalog for further details.



Mounting Options:

Model	Description
2 x 738 546	Mounting Kit for 2 to 4.6 inch (50 to 115 mm) OD mast.
737 978	Tilt Kit for use with the above mounting kit, 0–11 degrees downtilt angle. (requires 2 x 738 546 Mounting Kit)
742 263	Three-panel Sector Mounting Kit (120 deg. ea.) for 3.5 inch (89 mm) OD mast.



Order Information:

Model	Description
742 213	Antenna with 7/16 DIN connectors 0°–6° adjustable electrical downtilt

All specifications are subject to change without notice. The latest specifications are available at www.kathrein-scala.com.

Kathrein Inc., Scala Division Post Office Box 4580 Medford, OR 97501 (USA) Phone: (541) 779-6500 Fax: (541) 779-3991
Email: communications@kathrein.com Internet: www.kathrein-scala.com



CDMA BTS 3231 AWS 1.7/2.1 GHz (Outdoor/Indoor)

CDMA BTS 3231

Industry's Highest Capacity AWS Micro BTS

The CDMA BTS 3231 is the latest extension to Nortel Networks BTS (Base Transceiver Station) portfolio providing the ideal solution for urban, sub-urban and rural deployments. The CDMA BTS 3231 is a 3-carrier, 3-sector outdoor/indoor BTS operating at the AWS band of 1.7/2.1 GHz supporting IS-95, 1XRTT and 1xEV-DO simultaneously. BTS 3231 provides flexible deployments solutions including floor, rack, and wall mount options. The power consumption of BTS3231 is industry leading consuming only 630W for 3C3S. The BTS 3231 is also very light at 240lbs making it easy

to transport to hard to reach locations such as the top of a high rise building.

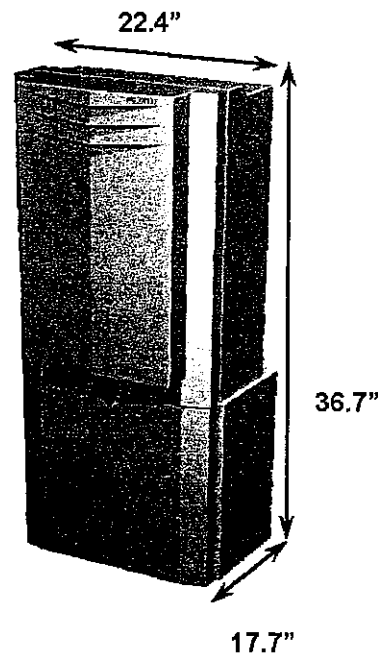


Exhibit D

Power Density Calculations

Pocket Site NHCT0371E

**130 Birdseye Road,
Farmington, Connecticut**



C Squared Systems, LLC
920 Candia Road
Manchester, NH 03109
Phone: (603) 657 9702
E-mail:

support@csquaredsystems.com

Calculated Radio Frequency Emissions



CT-0371E

130 Birdseye Road, Farmington, CT

Table of Contents

1. Introduction	1
2. FCC Guidelines for Evaluating RF Radiation Exposure Limits	2
3. RF Exposure Prediction Methods	2
4. Calculation Results	3
5. Conclusion	4
6. Statement of Certification	5
Attachment A: References	6
Attachment B: FCC Limits For Maximum Permissible Exposure (MPE)	7

List of Tables

Table 1: Proposed Carrier Information	3
---	---

1. Introduction

The purpose of this report is to investigate compliance with applicable FCC regulations for the proposed Pocket antennas to be installed on the existing tower at 130 Birdseye Road, Farmington, CT.

These calculations assume that the antennas are operating at 100 percent capacity, that all antenna channels are transmitting simultaneously, and that the radio transmitters are operating at full power. Obstructions (trees, buildings etc.) that would normally attenuate the signal are not taken into account. As a result, the predicted signal levels are much more conservative (higher) than the actual signal levels will be from the finished installation.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter (mW/cm^2). The number of mW/cm^2 emitted is called the power density. The general population exposure limit for the cellular band is $0.567\text{--}0.593 \text{ mW}/\text{cm}^2$, and the general population exposure limit for the PCS/AWS band is $1.0 \text{ mW}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

The FCC general population / uncontrolled limits set the maximum exposure to which most people may be subjected. General population / uncontrolled exposures apply in situations in which the general public may be exposed, or in which persons that are exposed as a consequence of their employment may not be fully aware of the potential for exposure or cannot exercise control over their exposure.

Higher exposure limits are permitted under the occupational / controlled exposure category, but only for persons who are exposed as a consequence of their employment and who have been made fully aware of the potential for exposure (through training), and they must be able to exercise control over their exposure. General population / uncontrolled limits are five times more stringent than the levels that are acceptable for occupational, or radio frequency trained individuals.”

The FCC describes exposure to radio frequency (RF) energy in terms of percentage of maximum permissible exposure (MPE) with 100% being the maximum allowed. Rather than the FCC presenting the user specification in terms of complex power density figures over a specified surface area, this MPE measure is particularly useful, and even more so when considering that power density limits actually vary by frequency because of the different absorptive properties of the human body at different frequencies.

MPE limits are specified as time-averaged exposure limits. This means that exposure can be averaged over 30 minutes for general population / uncontrolled exposure (or 6 minutes for occupational / controlled exposure). However, for the case of exposure of the general public, time averaging is usually not applied because of uncertainties over exact exposure conditions and difficulty in controlling time of exposure. Therefore, the typical conservative approach is to assume that any RF exposure to the general public will be continuous.

Finally, it should be noted that the MPE limits adopted by the FCC for both general population / uncontrolled exposure and for occupational / controlled exposure incorporate a substantial margin of safety and have been established to be well below levels generally accepted as having the potential to cause adverse health effects.

2. FCC Guidelines for Evaluating RF Radiation Exposure Limits

In 1985, the FCC established rules to regulate radio frequency (RF) exposure from FCC licensed antenna facilities. In 1996, the FCC updated these rules, which were further amended in August 1997 by OET Bulletin 65 Edition 97-01. These new rules include limits for Maximum Permissible Exposure (MPE) for transmitters operating between 300 kHz and 100 GHz. The FCC MPE limits are based on exposure limits recommended by the National Council on Radiation Protection and Measurements (NCRP), the exposure limits developed by the Institute of Electrical and Electronics Engineers, Inc., (IEEE) and adopted by the American National Standards Institute (ANSI).

Attachment B contains excerpts from OET Bulletin 65 and defines the Maximum Exposure Limit. As shown in these excerpts, each frequency band has different exposure limits, requiring power density to be reported as a percent of Maximum Permissible Exposure (MPE) when dealing with carriers transmitting in different frequency bands.

3. RF Exposure Prediction Methods

The emission field calculation results displayed in the following figures were generated using the following formula as outlined in FCC bulletin OET 65:

$$\text{Power Density} = \left(\frac{EIRP}{\pi \times R^2} \right) \times \text{Off Beam Loss}$$

Where:

EIRP = Effective Isotropic Radiated Power

R = Radial Distance = $\sqrt{H^2 + V^2}$

H = Horizontal Distance from antenna

V = Vertical Distance from bottom of antenna

Off Beam Loss is determined by the selected antenna patterns

4. Calculation Results

Table 1 below outlines the power assumptions used to compute the power density.

Carrier	Operating Frequency (MHz)	Antenna Height (Feet)	Number of Trans.	Effective Radiated Power (ERP) Per Transmitter (Watts)	Total ERP (Watts)
Nextel	851	120	9	100	900
Sprint	1957.5	140	11	122	1342
Verizon	869	110	9	200	1800
Verizon	1900	110	3	200	600
T-Mobile	1935	100	8	189	1512
Cingular GSM	1900	131	2	646	1292
Cingular UMTS	880	131	1	500	500
Pocket	2130-2133.75	90	3	631	1,893

Table 1: Proposed Carrier Information

The calculated result for the cumulative %MPE is 28.34%.

Please note that for distances of 100 feet or less, a nominal 10 dB of attenuation due to antenna pattern is assumed. Also, as noted in the introduction, obstructions (trees, buildings etc.) that would normally attenuate the signal are not taken into account. As a result, the predicted signal levels are more conservative (higher) than the actual signal levels will be from the finished installation.

5. Conclusion

The above analysis verifies that emissions from the proposed site will be well below the maximum power density levels as outlined by the FCC in the OET Bulletin 65 Ed. 97-01. Even when using conservative methods, the cumulative power density from the proposed transmit antennas at the existing facility is well below the limits for the general public. The highest expected percent of Maximum Permissible Exposure at the base of the tower is 28.34% of the FCC limit.

Please note that for distances of 100 feet or less, a nominal 10 dB of attenuation due to antenna pattern is assumed. Also, as noted in the introduction, obstructions (trees, buildings etc.) that would normally attenuate the signal are not taken into account. As a result, the predicted signal levels are more conservative (higher) than the actual signal levels will be from the finished installation.

6. Statement of Certification

I certify to the best of my knowledge that the statements in this report are true and accurate. The calculations follow guidelines set forth in ANSI/IEEE Std. C95.3, ANSI/IEEE Std. C95.1 and FCC OET Bulletin 65 Edition 97-01.



Daniel L. Goulet
C Squared Systems, LLC

August 26, 2008
Date

Attachment A: References

OET Bulletin 65 - Edition 97-01 - August 1997 Federal Communications Commission Office of Engineering & Technology

ANSI C95.1-1982, American National Standard Safety Levels With Respect to Human Exposure to Radio Frequency Electromagnetic Fields, 300 kHz to 100 GHz. IEEE-SA Standards Board

IEEE Std C95.3-1991 (Reaff 1997), IEEE Recommended Practice for the Measurement of Potentially Hazardous Electromagnetic Fields - RF and Microwave. IEEE-SA Standards Board

Attachment B: FCC Limits For Maximum Permissible Exposure (MPE)

(A) Limits for Occupational/Controlled Exposure

Frequency Range (MHz)	Electric Field Strength (E) (V/m)	Magnetic Field Strength (E) (A/m)	Power Density (S) (mW/cm ²)	Averaging Time E ² , H ² or S (minutes)
0.3-3.0	614	1.63	(100)*	6
3.0-30	1842/f	4.89/f	(900/f ²)*	6
30-300	61.4	0.163	1.0	6
300-1500	-	-	f/300	6
1500-100,000	-	-	5	6

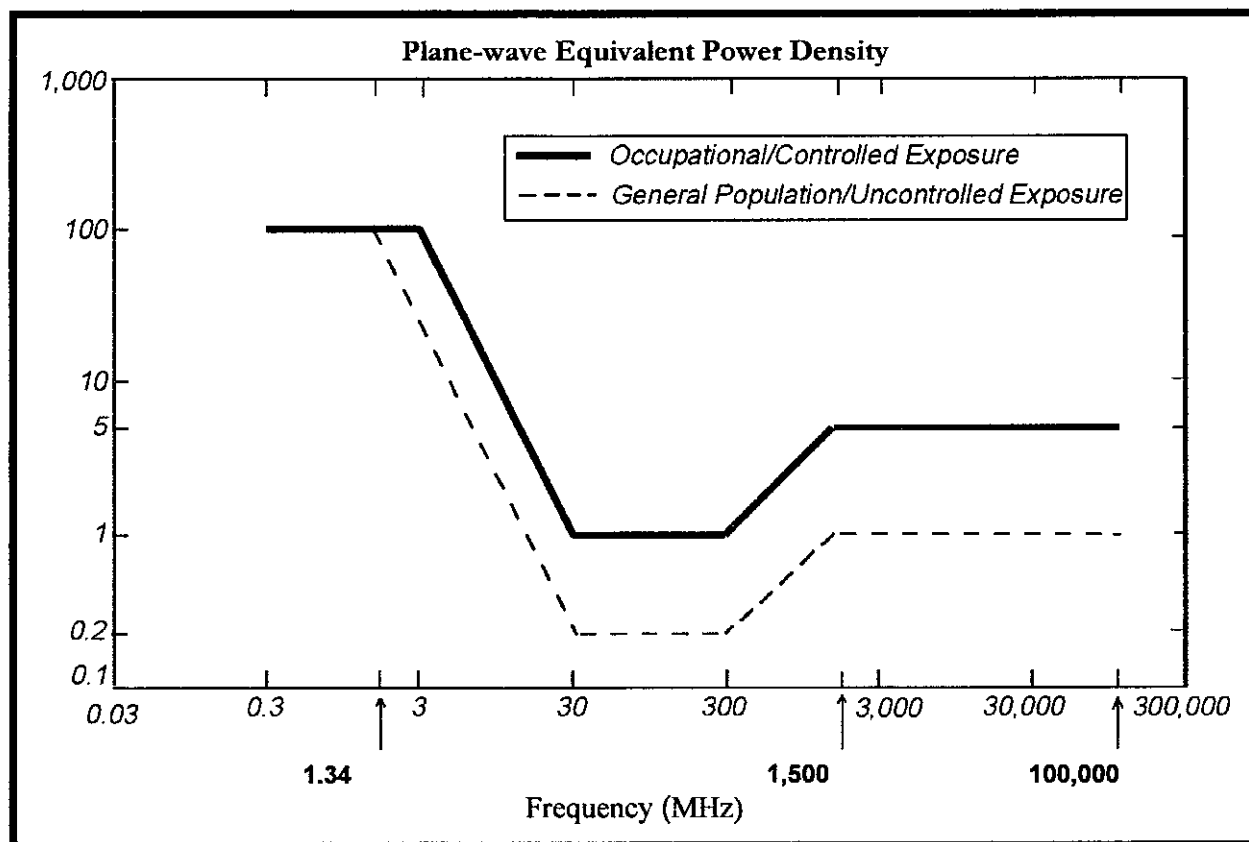
(B) Limits for General Population/Uncontrolled Exposure

Frequency Range (MHz)	Electric Field Strength (E) (V/m)	Magnetic Field Strength (E) (A/m)	Power Density (S) (mW/cm ²)	Averaging Time E ² , H ² or S (minutes)
0.3-1.34	614	1.63	(100)*	30
1.34-30	824/f	2.19/f	(180/f ²)*	30
30-300	27.5	0.073	0.2	30
300-1500	-	-	f/1500	30
1500-100,000	-	-	1.0	30

f = frequency in MHz * Plane-wave equivalent power density

NOTE 1: Occupational/controlled limits apply in situations in which persons are exposed as a consequence of their employment provided those persons are fully aware of the potential for exposure and can exercise control over their exposure. Limits for occupational/controlled exposure also apply in situations when an individual is transient through a location where occupational/controlled limits apply provided he or she is made aware of the potential for exposure.

NOTE 2: General population/uncontrolled exposures apply in situations in which the general public may be exposed, or in which persons that are exposed as a consequence of their employment may not be fully aware of the potential for exposure or can not exercise control over their exposure.



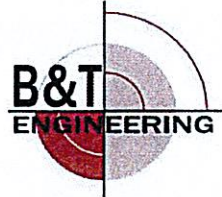
• FCC Limits for Maximum Permissible Exposure (MPE)

Exhibit E

Structural Analysis

Pocket Site NHCT0371E

**130 Birdseye Road,
Farmington, Connecticut**



July 31, 2008

Ms. Veronica Harris
Crown Castle USA Inc.
1200 MacArthur Blvd
Mahwah, NJ 07430

B&T Engineering, Inc.
1717 S. Boulder, Suite 300
Tulsa, OK 74119
(918) 587-4630
ctuttle@btengineering.com

Subject: Structural Analysis Report

Carrier Designation: Youghiogheny Co-Locate
Carrier Site Number: CT-0371E
Carrier Site Name: N/A

Crown Castle Designation: Crown Castle BU Number: 876335
Crown Castle Site Name: East Farmington
Crown Castle JDE Job Number: 108553

Engineering Firm Designation: B&T Engineering Project Number: 77969

Site Data: 3 A Birdseye Road, Farmington, CT, Hartford County
Latitude 41°-42'-56.58", Longitude -72°-48'-39.08"
140 Foot – Monopole Tower

Dear Ms. Harris,

B&T Engineering is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the aforementioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 297272, in accordance with Application 67286, Revision 2.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC1: Existing + Reserved + Proposed Equipment

Sufficient Capacity

Note: See Table 1 and Table 2 for the proposed and existing/reserved loading.

The analysis has been performed in accordance with the TIA/EIA-222-F standard and the 2005 Connecticut Building Code based upon a wind speed of 80 mph fastest mile.

All equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

We at B&T Engineering appreciate the opportunity of providing our continuing professional services to you and Crown Castle USA, Inc. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted,

Jerod Dotson
Project Engineer

Chad E. Tuttle, P.E.
President

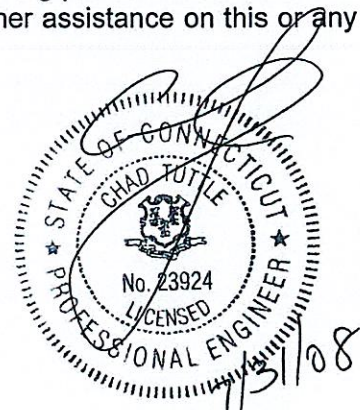


TABLE OF CONTENTS

1) INTRODUCTION

2) ANALYSIS CRITERIA

Table 1 – Proposed Antenna and Cable Information

Table 2 – Existing and Reserved Antenna and Cable Information

Table 3 – Design Antenna and Cable Information

3) ANALYSIS PROCEDURE

Table 4 – Documents Provided

3.1) Analysis Method

3.2) Assumptions

4) ANALYSIS RESULTS

Table 5 – Tower Component Stresses vs. Capacity

4.1) Recommendations

5) APPENDIX A

RISA Tower Output

6) APPENDIX B

Base Level Drawing

7) APPENDIX C

Additional Calculations

1) INTRODUCTION

The subject structure is a 140 foot tapered monopole manufactured by Summit Manufacturing, Inc.

2) ANALYSIS CRITERIA

Specific code

- TIA/EIA-222-F – 80 mph fastest mile wind speed
- Connecticut Building Code – 100 mph 3-second gust

The controlling wind loads for this analysis were derived from TIA/EIA-222-F therefore the tower was analyzed for a fastest mile wind speed of 80 mph with no ice and 69 mph with ½" of radial ice.

Table 1 – Proposed Antenna and Cable Information

Center Line Elev. (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount	Number of Feed Lines	Feed Line Size (in)
90	3	Kathrein	742-213	(3) Flush Mounts	6	1 5/8

Table 2 – Existing and Reserved Antenna and Cable Information

Center Line Elev. (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount	Number of Feed Lines	Feed Line Size (in)
139 [#]	6	Decibel	DB980H90A-M	LP Platform	6	1 5/8
	9 (MLA)	--	6' x 1'x 6" Panel		9 (MLA)	1 5/8
130	3 6	Powerwave	7770.00 LGP21401 TMA	(3) T-Arms	9	7/8
120	12	Swedcom	ALP 9212-N	LP Platform	12	1 1/4
110	6 6	Antel	LPD-6513 LPA 185063/12CF	LP Platform	12	1 5/8
100 ^{##}	3 6	Celwave Remec	APX16PV-16PVL-E S20057A1 TMA	Flush Mounts	6 12 (SLA)	7/8 1 5/8
72	1	Lucent	KS24019-L112A	Standoff	1	1/2
51	1	--	GPS	Standoff	1	1/2

*Refer to Cable Routing Drawing in Appendix B for Feedline Placement.

Structural Analysis performed using MLA loading rather than existing.

Structural Analysis performed using existing antennas and SLA feedlines.

Table 3 – Design Antenna and Cable Information

Center Line Elev. (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount	Number of Feed Lines	Feed Line Size (in)
Information Not Available						

3) ANALYSIS PROCEDURE

Table 4 – Documents Provided

Document	Remarks	Reference	Source
Tower Manufacturer Drawings	Summit Manufacturing, Inc	Crown Doc ID# 1615361	Crown OTG
Foundation Drawings	Summit Manufacturing, Inc	Crown Doc ID# 1440555	Crown OTG
Geotech Report	Dr. Clarence Welti Geotechnical Engineering	Crown Doc ID# 1850446	Crown OTG
Antenna Configuration	Crown CAD Package	Date: 07/28/08	Crown OTG

3.1) Analysis Method

RISA Tower (version 5.2.0.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various dead, live, wind, and ice load cases. All loads were computed in accordance with the TIA/EIA-222-F or the local building code requirements. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

1. This structural analysis **does not** include a grouted base plate.
2. Tower and structures were built in accordance with the manufacturer's specifications.
3. The tower and structures have been maintained in accordance with manufacturer's specifications.
4. The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
5. When applicable, transmission cables are considered to be structural components for calculating wind loads, as allowed by TIA/EIA-222-F.

If any of these assumptions are not valid or have been made in error, this analysis may be affected, and B&T Engineering, Inc. should be allowed to review any new information to determine its effect on the structural integrity of the tower.

Slab Size

Program: Pad and Pedestal Date: 7/31/2008
 File Name: 77969
 Location: East Farmington(BU# 876335), CT

INPUT Bold Values Only are Input

20.00 Length, L (ft.)	Conc. Unit Weight	0.15 kcf
20.00 Width, B (ft.)	Soil Unit Weight	0.1 kcf
4.00 Thickness, T (ft)		
0.50 Height of Pier Above Grade, (ft.)		
9.00 Depth to Base, D (ft.)		
1.00 Round Pedestal =0, Else Square Pedestal		
8.00 Pedestal Diameter or Width, (ft.)		
30.00 Phi Angle, (deg.)	0.5236 Phi in Radians	
0.00 Cohesion, c (ksf)		
3000 Conc. f'c, (psi)		
1.30 USD Load Factor		
12.00 Net Allow Bearing Stress, include Code Increase when approp., (ksf)		
1.50 Allow O.T. Factor of Safety		
2391 Tower O.T. Moment, (ft.-kips)	O.T. Moment @ fdn Bas	2628.50 ft.-kips
29 Leg Download, kips		
0 Leg Uplift, kips		
25 Leg Shear, kips		

Check of Punching Shear

43.50 d, top of conc. To middle of bottom rebar mat, (in.)
 558.00 Bo, The circumference at d/2 from face of pier, (in.)
 37.7 Factored Leg Download, (kips)
 4520.3 ΦV_n , Conc. 2-way Shear Resistance, (kips)

Check of One Way Shear Shear

293.0 Factored Shear at Face of Pier
 972.1 ΦV_n , Conc. 1-way Shear Resistance, (kips)

Mat Thickness is acceptable

72.30 Concrete Volume (c.y.)

4) ANALYSIS RESULTS

Table 5 – Tower Component Stresses vs. Capacity – LC1

Section Capacity Table								
Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
L1	140 - 91.75	Pole	TP25.89x16x0.25	1	-8.945	964.748	85.7	Pass
L2	91.75 - 77.1667	Pole	TP28.38x24.724x0.313	2	-11.909	1468.259	86.7	Pass
L3	77.1667 - 46.5	Pole	TP34.67x28.38x0.313	3	-16.616	1749.029	104.0	OK
L4	46.5 - 0	Pole	TP43.58x33.122x0.375	4	-28.423	2712.162	101.3	OK
							Summary	
							Pole (L3)	OK
							RATING =	OK
Individual Components:								
Notes:	Component		Elevation	% Capacity		Pass/Fail		
1	Base Plate		Base	74.5		Pass		
1	Anchor Rods		Base	95.2		Pass		
1	Base Foundation		Base	71.4		Pass		
Structure Rating (max from all components) =							104.0 %	

*Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity listed.
- 2) Capacities up to 105% are considered acceptable based on analysis procedures used.
- 3) The percent capacities shown above (excluding foundations) include the 1/3 increase in allowable stresses as allowed by TIA/EIA-222-F.

4.1) Recommendations

N/A

Slab Size

Program: Pad and Pedestal

File Name: 77969

Location: East Farmington(BU# 876335), CT

<u>Check Net Vert. Soil Pressure</u>	Eccentricity =	5.37	>	3.33 ft. (L/6)
Vert. Loads in kips				
29.00 Tower				RESULTANT IS OUTSIDE MIDDLE THIRD OF THE PAD
168.00 Soil	L-PRIME=	13.90		
292.80 Conc				
489.80 Sum of the Verticals, (kips)				

0.00 Net q-min. (ksf)

2.62 Net q-max, (ksf)

Status: OK

21.9%

2.10 O.T. Factor of Safety

Status: OK

71.4%

2.42 q-Bottom Design, (ksf)

The Net q-max minus the opposing effect of conc DW

1.10 q-TopDesign, (ksf)

Deadweight TO BE USED FOR CONCRETE SLAB DESIGN
of top mat reinforcing steel

Conservatively assume maximum moment at the pier centerline with maximum mat edge distance

Pier Design, Max Moment = 30342 in.-kips

Pad Bottom Design, Moment Arm: 6.00 ft.

Pad Bottom Design, Max Moment: 2617 in.-kips

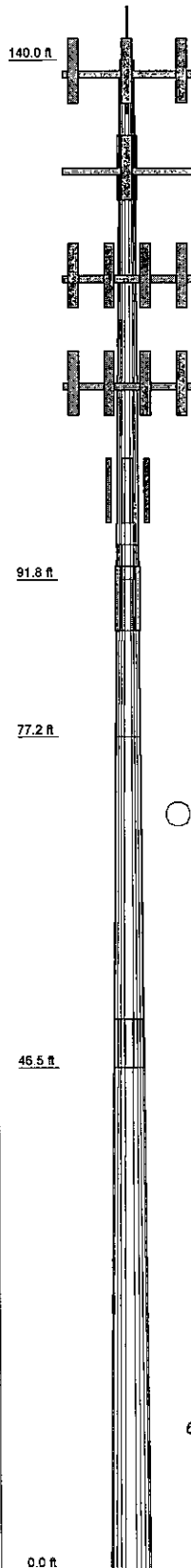
Pad Top Design, Moment Arm: 6.00 ft.

Pad Top Design, Max Moment: 1188 in.-kips

Use the above moments in the Pier, Pad Bottom & Top design sheets
in this Workbook to determine reinforcing steel requirements.

APPENDIX A
RISA TOWER OUTPUT

Section	1	2	3	4
Length (ft)	48.250	17.833	30.867	51.000
Number of Sides	12	12	12	12
Thickness (in)	0.250	0.313	0.313	0.375
Lap Splice (ft)	3.250		4.500	
Top Dia (in)	16.000	24.724	28.380	33.122
Bot Dia (in)	25.890	28.380	34.670	43.580
Grade	A607-60		A607-65	
Weight (K)	2.7	1.6	3.3	8.0



DESIGNED APPURTENANCE LOADING

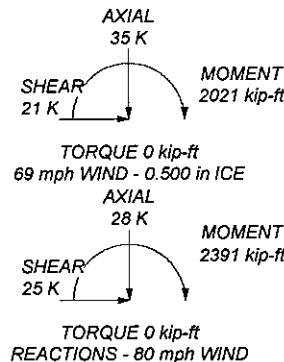
TYPE	ELEVATION	TYPE	ELEVATION
Lightning Rod (E)	140	(2) LPA-185063/12CF (E)	110
(3) MLA ANTENNA (6' x 1' x 0.5' 35 LB) (MLA)	139	(2) LPA-185063/12CF (E)	110
(3) MLA ANTENNA (6' x 1' x 0.5' 35 LB) (MLA)	139	(2) LPA-185063/12CF (E)	110
(3) MLA ANTENNA (6' x 1' x 0.5' 35 LB) (MLA)	139	Low Profile Platform (E)	110
(3) MLA ANTENNA (6' x 1' x 0.5' 35 LB) (MLA)	139	APX16PV-16PVL-E (E)	100
Low Profile Platform (E)	139	APX16PV-16PVL-E (E)	100
7770.00 (E)	130	APX16PV-16PVL-E (E)	100
7770.00 (E)	130	Flush Mount (E)	100
7770.00 (E)	130	Flush Mount (E)	100
(2) LGP21401 (E)	130	Flush Mount (E)	100
(2) LGP21401 (E)	130	(2) Remec S20057A1 TMA (E)	100
(2) LGP21401 (E)	130	(2) Remec S20057A1 TMA (E)	100
(3) T-Arms (E)	130	(2) Remec S20057A1 TMA (E)	100
(4) ALP 9212-N (E)	120	742 213 (P)	90
(4) ALP 9212-N (E)	120	742 213 (P)	90
(4) ALP 9212-N (E)	120	742 213 (P)	90
Low Profile Platform (E)	120	(3) Flush Mounts (P)	90
(2) LPD-6513 (E)	110	KS24019-L112A (E)	72
(2) LPD-6513 (E)	110	2' Standoff (E)	70
(2) LPD-6513 (E)	110	GPS (E)	51
(2) LPD-6513 (E)	110	2' Standoff (E)	51

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A607-60	60 ksi	75 ksi	A607-65	65 ksi	80 ksi

TOWER DESIGN NOTES

1. Tower is located in Hartford County, Connecticut.
2. Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 69 mph basic wind with 0.50 in ice.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 104%

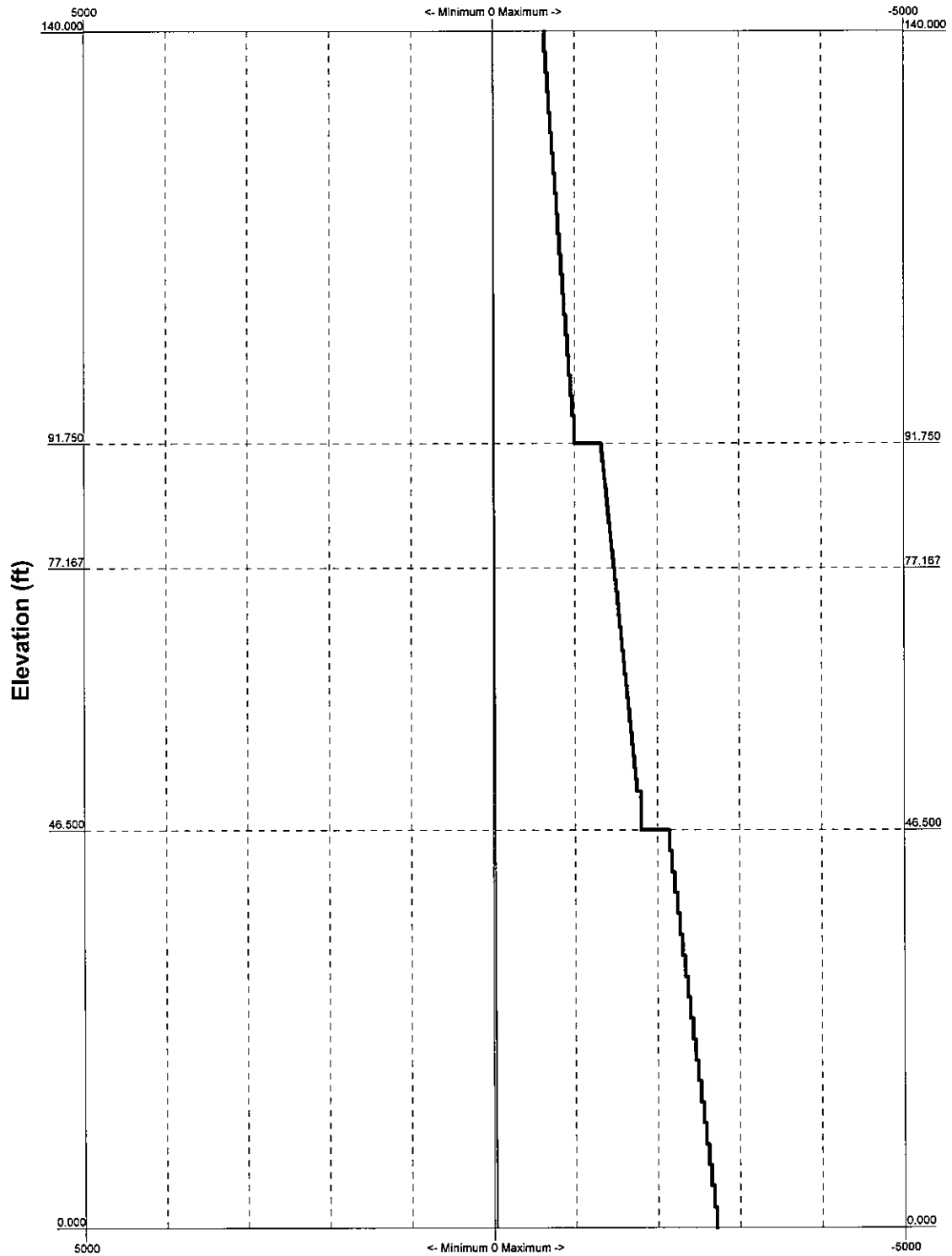


B&T Engineering
 1717 S. Boulder, Suite 300
 Tulsa, OK 74119
 Phone: (918) 587-4630
 FAX: (918) 295-0265

Job: **77969 - East Farmington, CT (BU# 876335)**
 Project: **140' Summit Monopole / App ID: 67286; Rev: 2**
 Client: **Crown Castle USA, Inc.** Drawn by: **dotson** App'd:
 Code: **TIA/EIA-222-F** Date: **07/31/08** Scale: **NTS**
 Path: **E-1** Dwg No. **E-1**

TIA/EIA-222-F - 80 mph/69 mph 0.500 in Ice

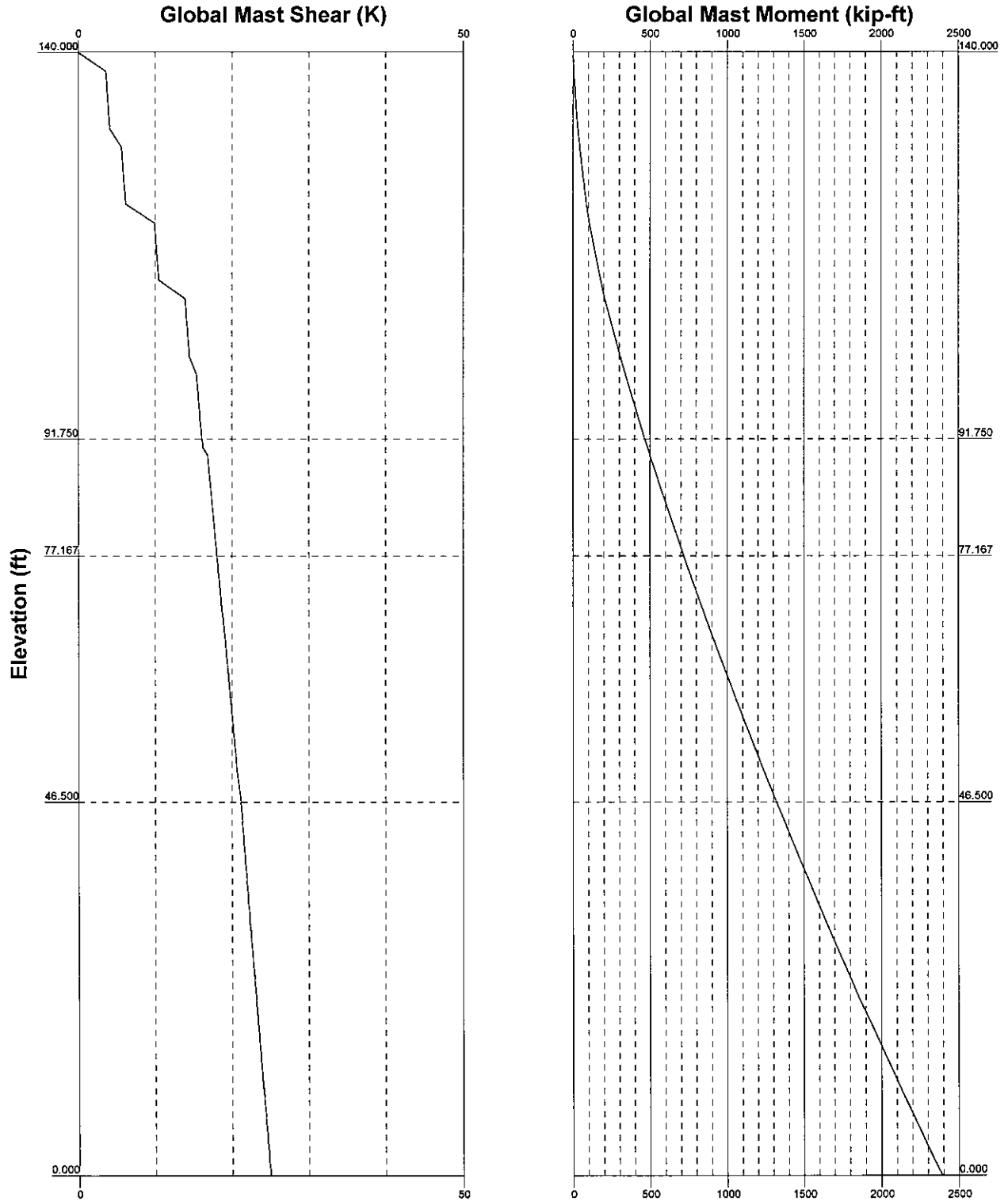
Leg Capacity ——— Leg Compression (K)



B&T Engineering		Job: 77969 - East Farmington, CT (BU# 876335)	
1717 S. Boulder, Suite 300		Project: 140' Summit Monopole / App ID: 67286; Rev: 2	
Tulsa, OK 74119		Client: Crown Castle USA, Inc.	Drawn by: jdotson
Phone: (918) 587-4630		Code: TIA/EIA-222-F	Date: 07/31/08
FAX: (918) 295-0265		Path:	Scale: NTS
		Dwg No. E-3	

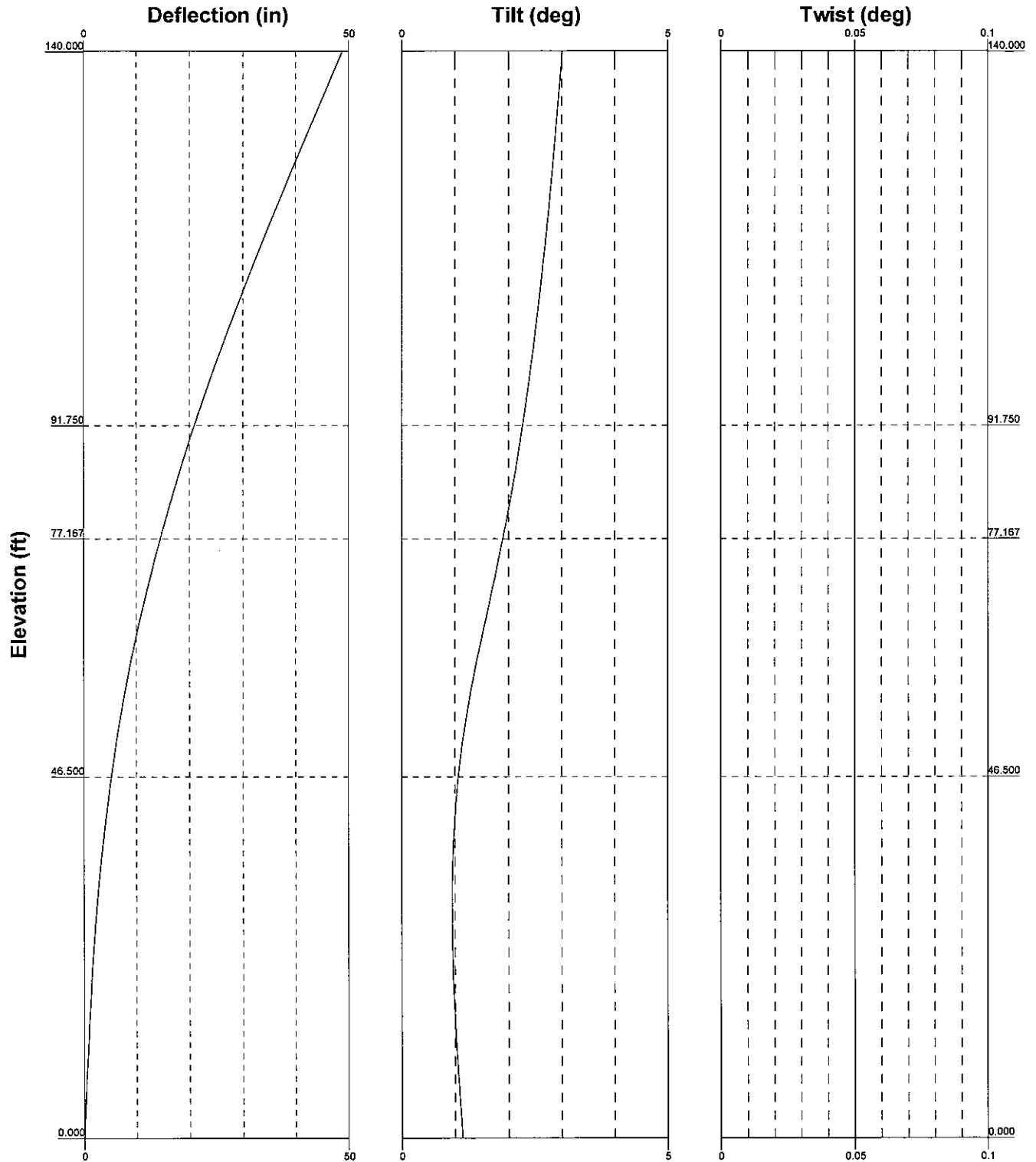
—— Vx Vz

—— Mx Mz



B&T Engineering
 1717 S. Boulder, Suite 300
 Tulsa, OK 74119
 Phone: (918) 587-4630
 FAX: (918) 295-0265

Job: **77969 - East Farmington, CT (BU# 876335)**
 Project: **140' Summit Monopole / App ID: 67286; Rev: 2**
 Client: **Crown Castle USA, Inc.** Drawn by: **jdotson** App'd:
 Code: **TIA/EIA-222-F** Date: **07/31/08** Scale: **NTS**
 Path: **E-4**



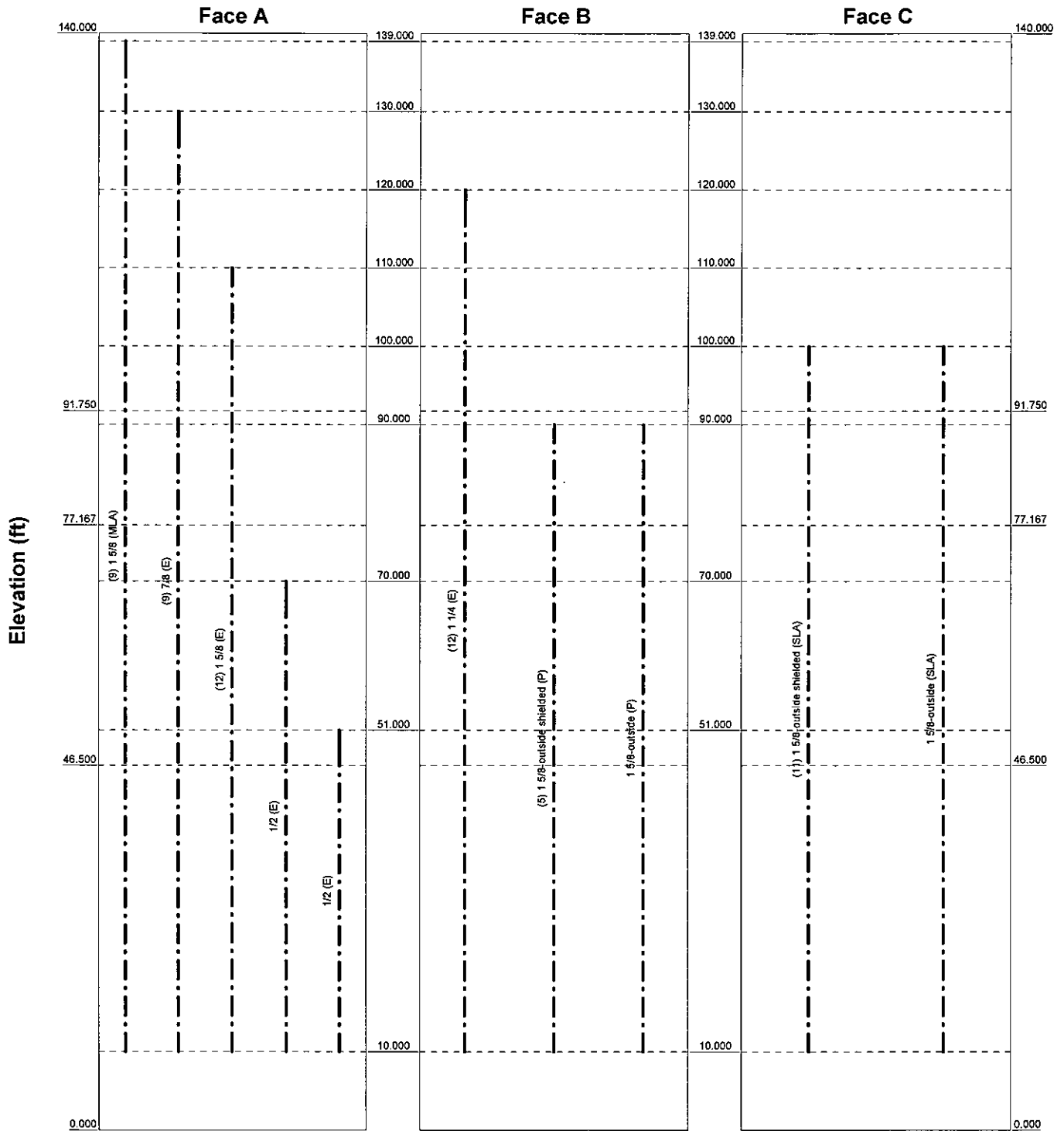
B&T Engineering
1717 S. Boulder, Suite 300
Tulsa, OK 74119
Phone: (918) 587-4630
FAX: (918) 295-0265

Job: 77969 - East Farmington, CT (BU# 876335)		
Project: 140' Summit Monopole / App ID: 67286; Rev: 2		
Client: Crown Castle USA, Inc.	Drawn by: jdotson	App'd:
Code: TIA/EIA-222-F	Date: 07/31/08	Scale: NTS
Path:	Dwg No. E-5	

Feedline Distribution Chart

0' - 140'

Round Flat App In Face App Out Face Truss Leg



B&T Engineering		Job: 77969 - East Farmington, CT (BU# 876335)	
1717 S. Boulder, Suite 300		Project: 140' Summit Monopole / App ID: 67286; Rev: 2	
Tulsa, OK 74119		Client: Crown Castle USA, Inc.	Drawn by: jdotson
Phone: (918) 587-4630		Code: TIA/EIA-222-F	Date: 07/31/08
FAX: (918) 295-0265		Scale: NTS	Dwg No. E-7

RISATower B&T Engineering 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job	77969 - East Farmington, CT (BU# 876335)	Page	1 of 7
	Project	140' Summit Monopole / App ID: 67286; Rev: 2	Date	14:18:55 07/31/08
	Client	Crown Castle USA, Inc.	Designed by	jdotson

Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in Hartford County, Connecticut.

Basic wind speed of 80 mph.

Nominal ice thickness of 0.500 in.

Ice density of 56 pcf.

A wind speed of 69 mph is used in combination with ice.

Deflections calculated using a wind speed of 50 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Treat Feedline Bundles As Cylinder
Consider Moments - Horizontals	Assume Legs Pinned	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Diagonals	√ Assume Rigid Index Plate	√ Calculate Redundant Bracing Forces
Use Moment Magnification	√ Use Clear Spans For Wind Area	Ignore Redundant Members in FEA
√ Use Code Stress Ratios	√ Use Clear Spans For KL/r	SR Leg Bolts Resist Compression
√ Use Code Safety Factors - Guys	√ Retension Guys To Initial Tension	√ All Leg Panels Have Same Allowable
Escalate Ice	√ Bypass Mast Stability Checks	Offset Girt At Foundation
Always Use Max Kz	√ Use Azimuth Dish Coefficients	√ Consider Feedline Torque
Use Special Wind Profile	√ Project Wind Area of Appurt.	Include Angle Block Shear Check
Include Bolts In Member Capacity	√ Autocalc Torque Arm Areas	Poles
Leg Bolts Are At Top Of Section	SR Members Have Cut Ends	Include Shear-Torsion Interaction
√ Secondary Horizontal Braces Leg	Sort Capacity Reports By Component	Always Use Sub-Critical Flow
Use Diamond Inner Bracing (4 Sided)	√ Triangulate Diamond Inner Bracing	Use Top Mounted Sockets
Add IBC .6D+W Combination		

Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft		in	in	in	in	
L1	140.000-91.750	48.250	3.250	12	16.000	25.890	0.250	1.000	A607-60 (60 ksi)
L2	91.750-77.167	17.833	0.000	12	24.724	28.380	0.313	1.250	A607-65 (65 ksi)
L3	77.167-46.500	30.667	4.500	12	28.380	34.670	0.313	1.250	A607-65 (65 ksi)
L4	46.500-0.000	51.000		12	33.122	43.580	0.375	1.500	A607-65 (65 ksi)

RISATower

B&T Engineering
1717 S. Boulder, Suite 300
Tulsa, OK 74119
Phone: (918) 587-4630
FAX: (918) 295-0265

Job

77969 - East Farmington, CT (BU# 876335)

Page

2 of 7

Project

140' Summit Monopole / App ID: 67286; Rev: 2

Date

14:18:55 07/31/08

Client

Crown Castle USA, Inc.

Designed by

jdotson

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	Iu/Q in ²	w in	w/t
L1	16.564	12.679	401.443	5.638	8.288	48.437	813.432	6.240	3.618	14.472
	26.803	20.640	1731.952	9.179	13.411	129.144	3509.405	10.158	6.269	25.074
L2	26.286	24.564	1868.385	8.739	12.807	145.888	3785.855	12.090	5.788	18.523
	29.381	28.243	2839.900	10.048	14.701	193.179	5754.407	13.900	6.768	21.659
L3	29.381	28.243	2839.900	10.048	14.701	193.179	5754.407	13.900	6.768	21.659
	35.893	34.572	5209.026	12.300	17.959	290.050	10554.899	17.015	8.454	27.053
L4	35.246	39.542	5412.378	11.723	17.157	315.458	10966.945	19.461	7.872	20.991
	45.117	52.170	12430.114	15.467	22.574	550.628	25186.781	25.677	10.674	28.465

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft ²	in					in	in
L1 140.000-91.75 0				1	1	1		
L2 91.750-77.167				1	1	1		
L3 77.167-46.500				1	1	1		
L4 46.500-0.000				1	1	1		

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	C _A A _A	Weight
						ft ² /ft	klf
1 5/8 (MLA) **	A	No	Inside Pole	139.000 - 10.000	9	No Ice 1/2" Ice	0.000 0.001
7/8 (E) **	A	No	Inside Pole	130.000 - 10.000	9	No Ice 1/2" Ice	0.000 0.001
1 1/4 (E) **	B	No	Inside Pole	120.000 - 10.000	12	No Ice 1/2" Ice	0.000 0.001
1 5/8 (E) **	A	No	Inside Pole	110.000 - 10.000	12	No Ice 1/2" Ice	0.000 0.001
1 5/8-outside shielded (SLA)	C	No	Inside Pole	100.000 - 10.000	11	No Ice 1/2" Ice	0.000 0.001
1 5/8-outside (SLA) **	C	No	CaAa (Out Of Face)	100.000 - 10.000	1	No Ice 1/2" Ice	0.198 0.003
1 5/8-outside shielded (P)	B	No	Inside Pole	90.000 - 10.000	5	No Ice 1/2" Ice	0.000 0.001
1 5/8-outside (P) **	B	No	CaAa (Out Of Face)	90.000 - 10.000	1	No Ice 1/2" Ice	0.198 0.003
1/2 (E)	A	No	Inside Pole	70.000 - 10.000	1	No Ice 1/2" Ice	0.000 0.000
1/2 (E)	A	No	Inside Pole	51.000 - 10.000	1	No Ice 1/2" Ice	0.000 0.000

RISATower B&T Engineering 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job	77969 - East Farmington, CT (BU# 876335)	Page 3 of 7
	Project	140' Summit Monopole / App ID: 67286; Rev: 2	Date 14:18:55 07/31/08
	Client	Crown Castle USA, Inc.	Designed by jdotson

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A_R ft ²	A_F ft ²	$C_A A_A$ In Face ft ²	$C_A A_A$ Out Face ft ²	Weight K
L1	140.000-91.750	A	0.000	0.000	0.000	0.000	0.856
		B	0.000	0.000	0.000	0.000	0.224
		C	0.000	0.000	0.000	1.634	0.103
L2	91.750-77.167	A	0.000	0.000	0.000	0.000	0.389
		B	0.000	0.000	0.000	2.541	0.196
		C	0.000	0.000	0.000	2.887	0.182
L3	77.167-46.500	A	0.000	0.000	0.000	0.000	0.826
		B	0.000	0.000	0.000	6.072	0.434
		C	0.000	0.000	0.000	6.072	0.383
L4	46.500-0.000	A	0.000	0.000	0.000	0.000	0.993
		B	0.000	0.000	0.000	7.227	0.517
		C	0.000	0.000	0.000	7.227	0.456

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A_R ft ²	A_F ft ²	$C_A A_A$ In Face ft ²	$C_A A_A$ Out Face ft ²	Weight K
L1	140.000-91.750	A	0.500	0.000	0.000	0.000	0.000	0.856
		B		0.000	0.000	0.000	0.000	0.224
		C		0.000	0.000	0.000	2.458	0.115
L2	91.750-77.167	A	0.500	0.000	0.000	0.000	0.000	0.389
		B		0.000	0.000	0.000	3.824	0.215
		C		0.000	0.000	0.000	4.346	0.204
L3	77.167-46.500	A	0.500	0.000	0.000	0.000	0.000	0.826
		B		0.000	0.000	0.000	9.139	0.481
		C		0.000	0.000	0.000	9.139	0.429
L4	46.500-0.000	A	0.500	0.000	0.000	0.000	0.000	0.993
		B		0.000	0.000	0.000	10.877	0.572
		C		0.000	0.000	0.000	10.877	0.511

Feed Line Center of Pressure

Section	Elevation ft	CP_X in	CP_Z in	CP_X Ice in	CP_Z Ice in
L1	140.000-91.750	-0.052	0.030	-0.073	0.042
L2	91.750-77.167	-0.025	0.240	-0.034	0.327
L3	77.167-46.500	0.000	0.258	0.000	0.355
L4	46.500-0.000	0.000	0.207	0.000	0.292

RISATower

B&T Engineering
 1717 S. Boulder, Suite 300
 Tulsa, OK 74119
 Phone: (918) 587-4630
 FAX: (918) 295-0265

Job

77969 - East Farmington, CT (BU# 876335)

Page

4 of 7

Project

140' Summit Monopole / App ID: 67286; Rev: 2

Date

14:18:55 07/31/08

Client

Crown Castle USA, Inc.

Designed by

jdotson

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K
Lightning Rod (E) **	C	None		0.000	140.000	No Ice 1/2" Ice	0.500 0.750	0.500 0.750	0.100 0.200
(3) MLA ANTENNA (6' x 1' x 0.5' 35 LB) (MLA)	C	From Face	0.000 0.000 0.000	0.000	139.000	No Ice 1/2" Ice	8.400 8.949	4.700 5.147	0.035 0.082
(3) MLA ANTENNA (6' x 1' x 0.5' 35 LB) (MLA)	B	From Face	0.000 0.000 0.000	0.000	139.000	No Ice 1/2" Ice	8.400 8.949	4.700 5.147	0.035 0.082
(3) MLA ANTENNA (6' x 1' x 0.5' 35 LB) (MLA)	A	From Face	0.000 0.000 0.000	0.000	139.000	No Ice 1/2" Ice	8.400 8.949	4.700 5.147	0.035 0.082
Low Profile Platform (E) **	C	None		0.000	139.000	No Ice 1/2" Ice	17.000 20.000	17.000 20.000	1.500 2.000
7770.00 (E)	C	From Face	0.000 0.000 0.000	0.000	130.000	No Ice 1/2" Ice	5.882 6.314	2.928 3.273	0.035 0.068
7770.00 (E)	B	From Face	0.000 0.000 0.000	0.000	130.000	No Ice 1/2" Ice	5.882 6.314	2.928 3.273	0.035 0.068
7770.00 (E)	A	From Face	0.000 0.000 0.000	0.000	130.000	No Ice 1/2" Ice	5.882 6.314	2.928 3.273	0.035 0.068
(2) LGP21401 (E)	C	From Face	0.000 0.000 0.000	0.000	130.000	No Ice 1/2" Ice	0.500 0.700	0.233 0.313	0.014 0.021
(2) LGP21401 (E)	B	From Face	0.000 0.000 0.000	0.000	130.000	No Ice 1/2" Ice	0.500 0.700	0.233 0.313	0.014 0.021
(2) LGP21401 (E)	A	From Face	0.000 0.000 0.000	0.000	130.000	No Ice 1/2" Ice	0.500 0.700	0.233 0.313	0.014 0.021
(3) T-Arms (E) **	C	None		0.000	130.000	No Ice 1/2" Ice	15.000 18.000	15.000 18.000	0.900 1.200
(4) ALP 9212-N (E)	C	From Face	0.000 0.000 0.000	0.000	120.000	No Ice 1/2" Ice	5.784 6.199	5.784 6.199	0.017 0.062
(4) ALP 9212-N (E)	B	From Face	0.000 0.000 0.000	0.000	120.000	No Ice 1/2" Ice	5.784 6.199	5.784 6.199	0.017 0.062
(4) ALP 9212-N (E)	A	From Face	0.000 0.000 0.000	0.000	120.000	No Ice 1/2" Ice	5.784 6.199	5.784 6.199	0.017 0.062
Low Profile Platform (E) **	C	None		0.000	120.000	No Ice 1/2" Ice	15.000 18.000	15.000 18.000	1.500 2.000
(2) LPD-6513 (E)	C	From Face	0.000 0.000 0.000	0.000	110.000	No Ice 1/2" Ice	6.415 6.816	5.149 5.532	0.028 0.051
(2) LPD-6513 (E)	B	From Face	0.000 0.000 0.000	0.000	110.000	No Ice 1/2" Ice	6.415 6.816	5.149 5.532	0.028 0.051

RISATower

B&T Engineering
 1717 S. Boulder, Suite 300
 Tulsa, OK 74119
 Phone: (918) 587-4630
 FAX: (918) 295-0265

Job

77969 - East Farmington, CT (BU# 876335)

Page

5 of 7

Project

140' Summit Monopole / App ID: 67286; Rev: 2

Date

14:18:55 07/31/08

Client

Crown Castle USA, Inc.

Designed by

jdotson

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
(2) LPD-6513 (E)	A	From Face	0.000 0.000 0.000	0.000	110.000	No Ice 1/2" Ice	6.415 6.816	5.149 5.532	0.028 0.051
(2) LPA-185063/12CF (E)	C	From Face	0.000 0.000 0.000	0.000	110.000	No Ice 1/2" Ice	4.972 5.417	4.511 4.952	0.014 0.046
(2) LPA-185063/12CF (E)	B	From Face	0.000 0.000 0.000	0.000	110.000	No Ice 1/2" Ice	4.972 5.417	4.511 4.952	0.014 0.046
(2) LPA-185063/12CF (E)	A	From Face	0.000 0.000 0.000	0.000	110.000	No Ice 1/2" Ice	4.972 5.417	4.511 4.952	0.014 0.046
Low Profile Platform (E) **	C	None		0.000	110.000	No Ice 1/2" Ice	15.000 18.000	15.000 18.000	1.500 2.000
APX16PV-16PVL-E (E)	C	From Leg	1.000 0.000 0.000	0.000	100.000	No Ice 1/2" Ice	6.699 7.131	2.003 2.326	0.040 0.071
APX16PV-16PVL-E (E)	B	From Leg	1.000 0.000 0.000	0.000	100.000	No Ice 1/2" Ice	6.699 7.131	2.003 2.326	0.040 0.071
APX16PV-16PVL-E (E)	A	From Leg	1.000 0.000 0.000	0.000	100.000	No Ice 1/2" Ice	6.699 7.131	2.003 2.326	0.040 0.071
Flush Mount (E)	C	From Leg	0.000 0.000 0.000	0.000	100.000	No Ice 1/2" Ice	1.000 2.000	1.000 2.000	0.100 0.150
Flush Mount (E)	B	From Leg	0.000 0.000 0.000	0.000	100.000	No Ice 1/2" Ice	1.000 2.000	1.000 2.000	0.100 0.150
Flush Mount (E)	A	From Leg	0.000 0.000 0.000	0.000	100.000	No Ice 1/2" Ice	1.000 2.000	1.000 2.000	0.100 0.150
(2) Remec S20057A1 TMA (E)	C	From Leg	0.000 0.000 0.000	0.000	100.000	No Ice 1/2" Ice	0.400 0.700	0.410 0.510	0.011 0.016
(2) Remec S20057A1 TMA (E)	B	From Leg	0.000 0.000 0.000	0.000	100.000	No Ice 1/2" Ice	0.400 0.700	0.410 0.510	0.011 0.016
(2) Remec S20057A1 TMA (E)	A	From Leg	0.000 0.000 0.000	0.000	100.000	No Ice 1/2" Ice	0.400 0.700	0.410 0.510	0.011 0.016
** 742 213 (P)	C	From Leg	0.000 0.000 0.000	0.000	90.000	No Ice 1/2" Ice	5.135 5.609	2.869 3.483	0.022 0.047
742 213 (P)	B	From Leg	0.000 0.000 0.000	0.000	90.000	No Ice 1/2" Ice	5.135 5.609	2.869 3.483	0.022 0.047
742 213 (P)	A	From Leg	0.000 0.000 0.000	0.000	90.000	No Ice 1/2" Ice	5.135 5.609	2.869 3.483	0.022 0.047
(3) Flush Mounts (P) **	C	None		0.000	90.000	No Ice 1/2" Ice	2.000 4.000	2.000 4.000	0.100 0.180
KS24019-L112A (E)	A	From Leg	0.000 0.000	0.000	72.000	No Ice 1/2" Ice	0.100 0.180	0.100 0.180	0.005 0.006

RISATower

B&T Engineering
 1717 S. Boulder, Suite 300
 Tulsa, OK 74119
 Phone: (918) 587-4630
 FAX: (918) 295-0265

Job

77969 - East Farmington, CT (BU# 876335)

Page

6 of 7

Project

140' Summit Monopole / App ID: 67286; Rev: 2

Date

14:18:55 07/31/08

Client

Crown Castle USA, Inc.

Designed by

jdotson

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _{A,A} Front ft ²	C _{A,A} Side ft ²	Weight K
2' Standoff (E)	A	From Leg	0.000 0.000 0.000	0.000	70.000	No Ice 1/2" Ice 4.000	2.000 4.000	0.100 0.180
GPS (E)	A	From Leg	0.000 0.000 0.000	0.000	51.000	No Ice 1/2" Ice 0.180	0.100 0.180	0.005 0.006
2' Standoff (E)	A	From Leg	0.000 0.000 0.000	0.000	51.000	No Ice 1/2" Ice 4.000	2.000 4.000	0.100 0.180

Compression Checks**Pole Design Data**

Section No.	Elevation ft	Size	L ft	L _a ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
L1	140 - 91.75 (1)	TP25.89x16x0.25	48.250	0.000	0.0	36.0000	20.104	-8.945	723.742	0.012
L2	91.75 - 77.1667 (2)	TP28.38x24.724x0.313	17.833	0.000	0.0	39.0000	28.243	-11.909	1101.470	0.011
L3	77.1667 - 46.5 (3)	TP34.67x28.38x0.313	30.667	0.000	0.0	39.0000	33.644	-16.616	1312.100	0.013
L4	46.5 - 0 (4)	H1-3 (1.39 CR) - 3 TP43.58x33.122x0.375 H1-3 (1.35 CR) - 4	51.000	0.000	0.0	39.0000	52.170	-28.423	2034.630	0.014

Pole Bending Design Data

Section No.	Elevation ft	Size	Actual M _x kip-ft	Actual f _{bx} ksi	Allow. F _{bx} ksi	Ratio f _{bx} F _{bx}	Actual M _y kip-ft	Actual f _{by} ksi	Allow. F _{by} ksi	Ratio f _{by} F _{by}
L1	140 - 91.75 (1)	TP25.89x16x0.25	415.253	-40.6815	36.0000	1.130	0.000	0.0000	36.0000	0.000
L2	91.75 - 77.1667 (2)	TP28.38x24.724x0.313	719.081	-44.6682	39.0000	1.145	0.000	0.0000	39.0000	0.000
L3	77.1667 - 46.5 (3)	TP34.67x28.38x0.313	1225.35 8	-53.5467	39.0000	1.373	0.000	0.0000	39.0000	0.000
L4	46.5 - 0 (4)	TP43.58x33.122x0.375	2391.17 5	-52.1116	39.0000	1.336	0.000	0.0000	39.0000	0.000

RISATower B&T Engineering 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job	77969 - East Farmington, CT (BU# 876335)	Page	7 of 7
	Project	140' Summit Monopole / App ID: 67286; Rev: 2	Date	14:18:55 07/31/08
	Client	Crown Castle USA, Inc.	Designed by	jdotson

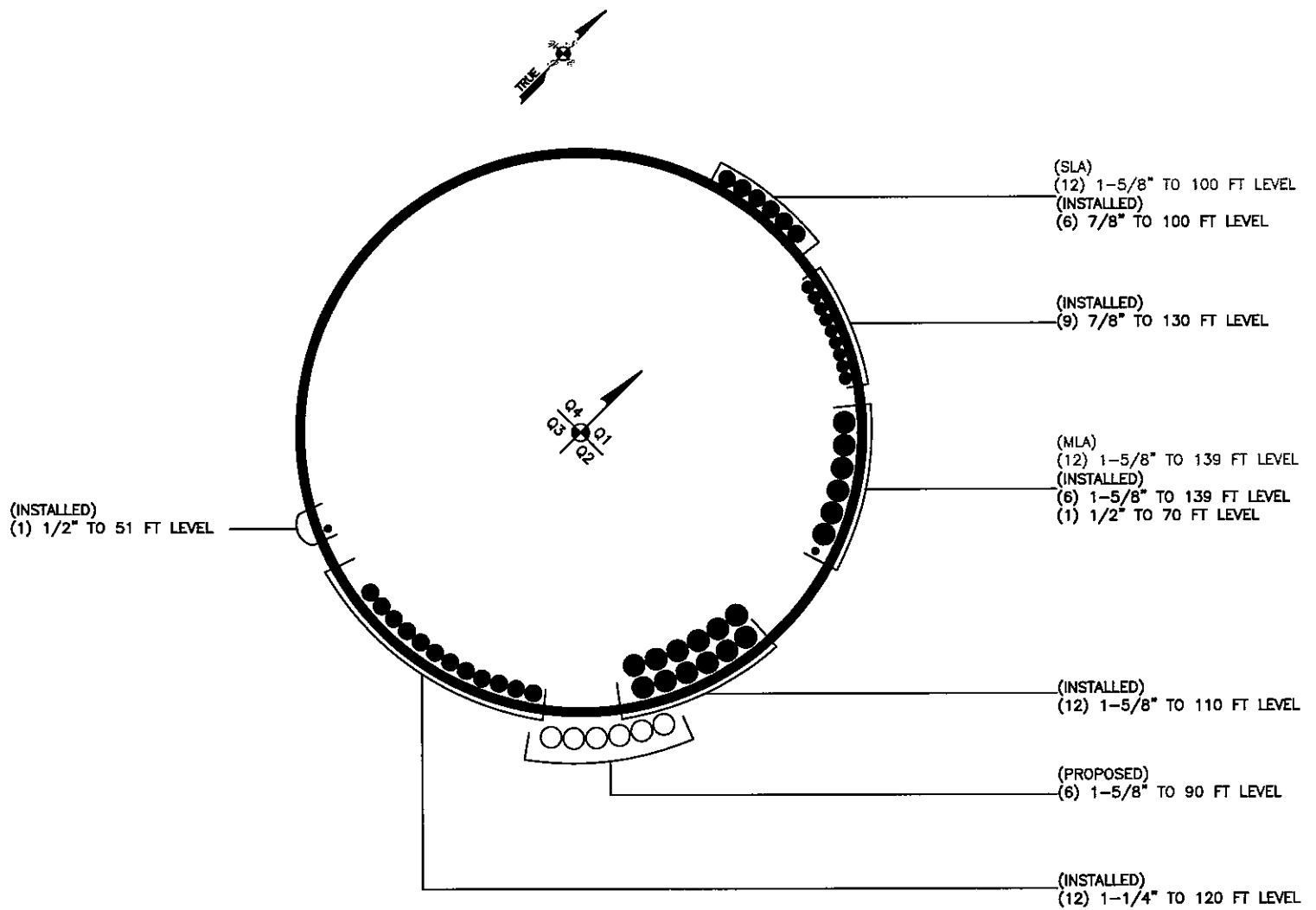
Pole Interaction Design Data

Section No.	Elevation ft	Size	Ratio $\frac{P}{P_a}$	Ratio $\frac{f_{bx}}{F_{bx}}$	Ratio $\frac{f_{by}}{F_{by}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	140 - 91.75 (1)	TP25.89x16x0.25	0.012	1.130	0.000	1.142	1.333	H1-3 ✓
L2	91.75 - 77.1667 (2)	TP28.38x24.724x0.313	0.011	1.145	0.000	1.156	1.333	H1-3 ✓
L3	77.1667 - 46.5 (3)	TP34.67x28.38x0.313	0.013	1.373	0.000	1.386 ✗	1.333	H1-3 ✗
L4	46.5 - 0 (4)	TP43.58x33.122x0.375	0.014	1.336	0.000	1.350 ✗	1.333	H1-3 ✗

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
L1	140 - 91.75	Pole	TP25.89x16x0.25	1	-8.945	964.748	85.7	Pass
L2	91.75 - 77.1667	Pole	TP28.38x24.724x0.313	2	-11.909	1468.259	86.7	Pass
L3	77.1667 - 46.5	Pole	TP34.67x28.38x0.313	3	-16.616	1749.029	104.0	Fail ✗
L4	46.5 - 0	Pole	TP43.58x33.122x0.375	4	-28.423	2712.162	101.3	Fail ✗
							Summary	
							Pole (L3)	104.0 Fail ✗
							RATING =	104.0 Fail ✗

APPENDIX B
BASE LEVEL DRAWING



BUSINESS UNIT: 876335

APPENDIX C
ADDITIONAL CALCULATIONS

PROJECT **77969 - East Farmington, CT (BU 876335)**
 SUBJECT **Base Plate Analysis**
 DATE **07/31/08** BY **CT/jr**



B&T Engineering, Inc.
 1717 S. Boulder, Suite 300
 Tulsa, OK 74119
 (918) 587-4630

Base Plate / Anchor Bolt Check - Square Plate No Stiffeners

Input:

Tower Reactions:

Moment: **2391 k-ft**
 Axial: **29.0 kips**
 Shear: **25.0 kips**

Allowable Stress Increase

Stress Incr.: **1.33**

Pole Information:

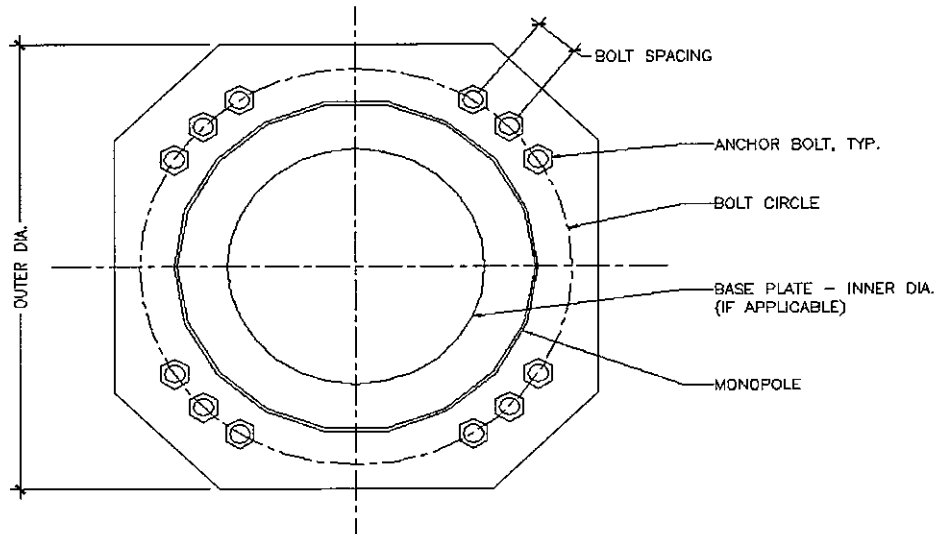
Outer Dia.: **43.58 in**

Bolt Information:

of Bolts: **12**
 Bolt diameter: **2.25 in**
 Bolt Grade: **A615 GR75**
 Bolt Fy: **75 ksi**
 Bolt Circle dia: **51.00 in**
 Clustered? **Y** Y/N
 Spacing: **6.00 in** Only if Clustered

Plate Information:

Thickness: **3.00 in**
 Plate Fy: **50 ksi**
 Outer Dia.: **49.50 in**
 Inner Dia.: **24.00 in**
 Grouted? **N**
 Grout Height: **2.00 in** If Grouted



Anchor Bolt Check:

	Actual	Allowable	Stress Ratio	Result
Tension:	185.1 k	194.4 k	95.2%	PASS
Compression	189.9 k	238.0 k	79.8%	PASS

Base Plate Check:

Plate Moment:	1473.03 k-in	
Eff. Width at Pole:	26.42 in	
S plate	39.64 in ³	
Actual Stress:	37.16 ksi	
Allowable Stress:	49.88 ksi	Result
Stress Ratio:	74.5%	PASS

Slab Size

Program: Pad and Pedestal Date: 7/31/2008
 File Name: 77969
 Location: East Farmington(BU# 876335), CT

INPUT Bold Values Only are Input

20.00 Length, L (ft.)	Conc. Unit Weight	0.15 kcf
20.00 Width, B (ft.)	Soil Unit Weight	0.1 kcf
4.00 Thickness, T (ft)		
0.50 Height of Pier Above Grade, (ft.)		
9.00 Depth to Base, D (ft.)		
1.00 Round Pedestal =0, Else Square Pedestal		
8.00 Pedestal Diameter or Width, (ft.)		
30.00 Phi Angle, (deg.)	0.5236 Phi in Radians	
0.00 Cohesion, c (ksf)		
3000 Conc. f'c, (psi)		
1.30 USD Load Factor		
12.00 Net Allow Bearing Stress, include Code Increase when approp., (ksf)		
1.50 Allow O.T. Factor of Safety		
2391 Tower O.T. Moment, (ft.-kips)	O.T. Moment @ fdn Bas	2628.50 ft.-kips
29 Leg Download, kips		
0 Leg Uplift, kips		
25 Leg Shear, kips		

Check of Punching Shear

43.50 d, top of conc. To middle of bottom rebar mat, (in.)
 558.00 Bo, The circumference at d/2 from face of pier, (in.)
 37.7 Factored Leg Download, (kips)
 4520.3 $\Phi \cdot V_n$, Conc. 2-way Shear Resistance, (kips)

Check of One Way Shear Shear

293.0 Factored Shear at Face of Pier
 972.1 $\Phi \cdot V_n$, Conc. 1-way Shear Resistance, (kips)

Mat Thickness is acceptable

72.30 Concrete Volume (c.y.)

Slab Size

Program: Pad and Pedestal

File Name: 77969

Location: East Farmington(BU# 876335), CT

Check Net Vert. Soil Pressure Eccentricity = 5.37 > 3.33 ft. (L/6)

Vert. Loads in kips

29.00 Tower

RESULTANT IS OUTSIDE MIDDLE THIRD OF THE PAD

168.00 Soil

L-PRIME= 13.90

292.80 Conc

489.80 Sum of the Verticals, (kips)

0.00 Net q-min. (ksf)

2.62 Net q-max, (ksf)

Status: OK

21.9%

2.10 O.T. Factor of Safety

Status: OK

71.4%

2.42 q-Bottom Design, (ksf)

The Net q-max minus the opposing effect of conc DW

1.10 q-TopDesign, (ksf)

Deadweight TO BE USED FOR CONCRETE SLAB DESIGN
of top mat reinforcing steel

Conservatively assume maximum moment at the pier centerline with maximum mat edge distance

Pier Design, Max Moment =

30342 in.-kips

Pad Bottom Design, Moment Arm:

6.00 ft.

Pad Bottom Design, Max Moment:

2617 in.-kips

Pad Top Design, Moment Arm:

6.00 ft.

Pad Top Design, Max Moment:

1188 in.-kips

Use the above moments in the Pier, Pad Bottom & Top design sheets
in this Workbook to determine reinforcing steel requirements.