



January 28, 2015

Melanie A. Bachman
Executive Director
Connecticut Siting Council
10 Franklin Street
New Britain, CT 06051

Regarding: Notice of Exempt Modification – Addition of 3 radio heads previously approved
Property Address: 281 Wood House Road, Fairfield, CT (the “Property”)
Applicant: AT&T Mobility (“AT&T”)

Dear Ms. Bachman:

AT&T currently maintains a wireless telecommunications facility on an existing 171 foot Monopole (“tower”) location on the Property. AT&T’s facility consists of nine (9) wireless telecommunications antenna at 150 feet. The tower is controlled by Crown Castle, LLC. The Council approved the previous application on June 20th 2011 reference number EM-CING-051-110527. This application (attached) granted AT&T the use of 6 radio heads at this location. The approval expired one year from the issue date. During that time AT&T made the changes to the site per the approval but only installed three (3) of the six (6) radio heads that they received approval. AT&T would now like to install the additional three (3) radio heads that were originally approved under EM-CING-051-110527.

Please accept this application as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72 (b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to the First Selectman and Planning Director for the Town of Fairfield. A copy of this letter is also being sent to Crown Castle, LLC, the owner of the structure that AT&T is located.

The planned modifications to AT&T’s facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The planned modifications will not result in an increase in the height of the existing structure. AT&T’s additional, previously approved 3 radio heads will be installed at 150 foot level of the 171 foot monopole.
2. The proposed modifications will not involve any changes to ground-mounted equipment and, therefore will not require an extension of the site boundary.
3. The proposed modification will not increase the noise level at the facility by six decibel or more, or to levels that exceed state and local criteria.
4. The operation of the modified facility will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) safety



standard. An RF emissions calculation (attached) for AT&T's modified facility was provided in the application which led to the June 20th 2011 Decision.

5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The tower and its foundation can support AT&T's proposed modifications. (Please see attached Structural analysis completed by Crown Castle, LLC. dated April 21, 2011).

For the foregoing reasons AT&T respectfully requests that the proposed addition of 3 radio heads previously approved be allowed within the exempt modifications under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

A handwritten signature in cursive script that reads "David P. Cooper".

David P. Cooper
Director of Site Acquisition
Empire Telecom

CC: Michael C. Tetreau, First Selectman, Town of Fairfield
Joe Devonshuk, Planning Director, Town of Fairfield
Crown Castle, LLC



STATE OF CONNECTICUT

CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: siting.council@ct.gov

www.ct.gov/csc

CT2105

June 20, 2011

Douglas L. Culp, Real Estate Consultant
New Cingular Wireless PCS, LLC
500 Enterprise Drive
Rocky Hill, CT 06067-3900

RE: **EM-CING-051-110527** - New Cingular Wireless PCS, LLC notice of intent to modify an existing telecommunications facility located at 281 Wood House Road, Fairfield, Connecticut

Dear Mr. Culp:

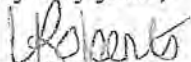
The Connecticut Siting Council (Council) hereby acknowledges your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies with the following conditions:

- Any deviation from the proposed modification as specified in this notice and supporting materials with Council shall render this acknowledgement invalid;
- Any material changes to this modification as proposed shall require the filing of a new notice with the Council;
- Not less than 45 days after completion of construction, the Council shall be notified in writing that construction has been completed;
- The validity of this action shall expire one year from the date of this letter; and
- The applicant may file a request for an extension of time beyond the one year deadline provided that such request is submitted to the Council not less than 60 days prior to the expiration;

The proposed modifications including the placement of all necessary equipment and shelters within the tower compound are to be implemented as specified here and in your notice dated May 27, 2011. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Thank you for your attention and cooperation.

Very truly yours,


Linda Roberts
Executive Director

LR/CDM/laf

c: The Honorable Kenneth A. Flatto, First Selectman, Town of Fairfield
Joseph E. Devonshuk, Town Planner, Town of Fairfield
Crown Castle USA, Inc.





STATE OF CONNECTICUT
CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

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June 6, 2011

The Honorable Kenneth A. Flatto
First Selectman
Town of Fairfield
Town Hall
725 Old Post Road
Fairfield, CT 06430

RE: **EM-CING-051-110527** - New Cingular Wireless PCS, LLC notice of intent to modify an existing telecommunications facility located at 281 Wood House Road, Fairfield, Connecticut.

Dear First Selectman Flatto:

The Connecticut Siting Council (Council) received this request to modify an existing telecommunications facility, pursuant to Regulations of Connecticut State Agencies Section 16-50j-72.

If you have any questions or comments regarding this proposal, please call me or inform the Council by June 20, 2011.

Thank you for your cooperation and consideration.

Very truly yours,

Linda Roberts NAB

Linda Roberts
Executive Director

LR/jbw

Enclosure: Notice of Intent

c: Joseph E. Devonshuk, Town Planner, Town of Fairfield



New Cingular Wireless PCS, LLC
500 Enterprise Drive
Rocky Hill, Connecticut 06067-3900
Phone: (860) 463-5511
Fax: (860) 513-7190

Douglas L. Culp
Real Estate Consultant

HAND DELIVERED

May 27, 2011



Ms. Linda Roberts
Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, Connecticut 06051

Re: New Cingular Wireless PCS, LLC notice of intent to modify an existing tele-communications facility located at 281 Wood House Fairfield, CT (owner Crown Castle)

Dear Ms. Roberts:

In order to accommodate technological changes, implement Uniform Mobile Telecommunications System ("UMTS") and/or Long Term Evolution ("LTE") capabilities, and enhance system performance in the State of Connecticut, New Cingular Wireless PCS, LLC ("AT&T") plans to modify the equipment configurations at many of its existing cell sites. Please accept this letter and attachments as notification, pursuant to R.C.S.A. Section 16-50j-73, of construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2). In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter and attachments is being sent to the chief elected official of the municipality in which the affected cell site is located.

UMTS technology offers services to mobile computer and phone users anywhere in the world. Based on the Global System for Mobile ("GSM") communication standard, UMTS is the planned worldwide standard for mobile users. UMTS, fully implemented, gives computer and phone users high-speed access to the Internet as they travel. They have the same capabilities even when they roam, through both terrestrial wireless and satellite transmissions.

LTE is a new high-performance air interface for cellular mobile communications. It is designed to increase the capacity and speed of mobile telephone networks.

Attached is a summary of the planned modifications, including power density calculations reflecting the change in AT&T's operations at the site. Also included is documentation of the structural sufficiency of the tower to accommodate the revised antenna configuration.

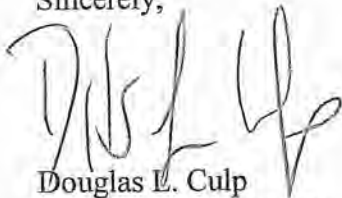
The changes to the facility do not constitute modifications as defined in Connecticut General Statutes ("C.G.S.") Section 16-50i(d) because the general physical characteristics of the facility will not be significantly changed or altered. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in R.C.S.A. Section 16-50j-72(b)(2).

1. The height of the overall structure will be unaffected.
2. The proposed changes will not extend the site boundaries. There will be no effect on the site compound other than some enlarged equipment pads as may be noted in the attachments.
3. The proposed changes will not increase the noise level at the existing facility by six decibels or more.
4. Radio frequency power density may increase due to use of one or more GSM channel for UMTS transmissions. Moreover, LTE will utilize additional radio frequencies newly-licensed by the FCC for cellular mobile communications. However, the changes will not increase the calculated "worst case" power density for the combined operations at the site to a level at or above the applicable standard for uncontrolled environments as calculated for a mixed frequency site.

For the foregoing reasons, New Cingular Wireless respectfully submits that the proposed changes at the referenced site constitute exempt modifications under R.C.S.A. Section 16-50j-72(b)(2).

Please feel free to call me at (860) 463-5511 with questions concerning this matter. Thank you for your consideration.

Sincerely,



Douglas L. Culp
Real Estate Consultant

Attachments

**NEW CINGULAR WIRELESS PCS, LLC
Equipment Modification**

281 Wood House Road Fairfield, CT
Site Number CT2105
Exempt Mod

Tower Owner/Manager: Crown Castle

Equipment configuration: Monopole

Current and/or approved: Six PowerWave antennas @ 150 ft
Twelve PowerWave TMA's @ 150 ft
Twelve runs 1 1/4 inch coax to 150 ft
Equipment Shelter

Planned Modifications: Retain existing PowerWave Antenna's, TMA's at 150 ft
Retain all Coax Cabling
Install three PowerWave P65-16 antennas or equivalent @ 150 ft
Install six remote radio heads and surge arrestor @ 150 ft
Install one fiber and two DC power cables to 150 ft

Power Density:

Worst-case calculations for existing wireless operations at the site, using standard parameters for other carriers, indicate a radio frequency electromagnetic radiation power density, measured at ground level beside the Tower, of 51.3% of the standard adopted by the FCC. As depicted in the second table below, the total radio frequency electromagnetic radiation power density following proposed modifications would be approximately 52.5% of the standard.

Existing

Company	Centerline Ht (feet)	Frequency (MHz)	Number of Channels	Power Per Channel (Watts)	Power Density (mW/cm ²)	Standard Limits (mW/cm ²)	Percent of Limit
Other Users							37.69
AT&T UMTS	150	1900 Band	1	500	0.0080	1.0000	0.80
AT&T UMTS	150	800 Band	2	500	0.0160	0.5867	2.72
AT&T GSM	150	800Band	10	296	0.0473	0.5867	8.06
AT&T UMTS	150	1900 Band	3	427	0.0205	1.0000	2.05
Total							51.3%

* Data for other users are from Siting Council records.

Proposed

Company	Centerline Ht (feet)	Frequency (MHz)	Number of Channels	Power Per Channel (Watts)	Power Density (mW/cm ²)	Standard Limits (mW/cm ²)	Percent of Limit
Other Users							37.69
AT&T UMTS	150	800 Band	1	500	0.0080	0.5867	1.36
AT&T UMTS	150	1900 Band	2	500	0.0160	1.0000	1.60
AT&T GSM	150	880 - 894	10	296	0.0473	0.5867	8.06
AT&T GSM	150	1900 Band	3	427	0.0205	1.0000	2.05
AT&T LTE	150	740 - 746	1	500	0.0080	0.4933	1.62
Total							52.4%

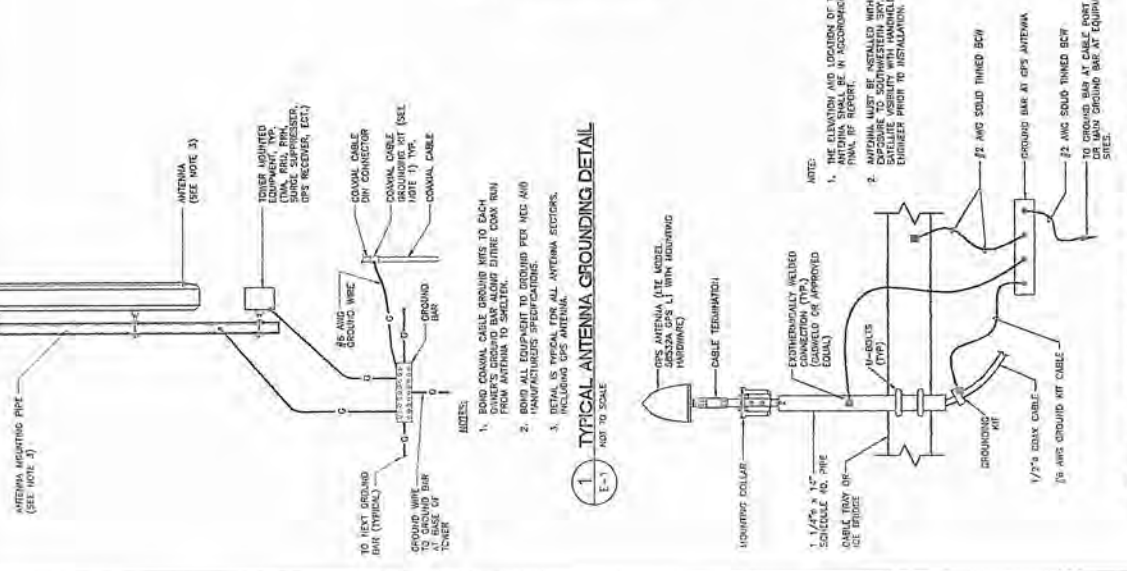
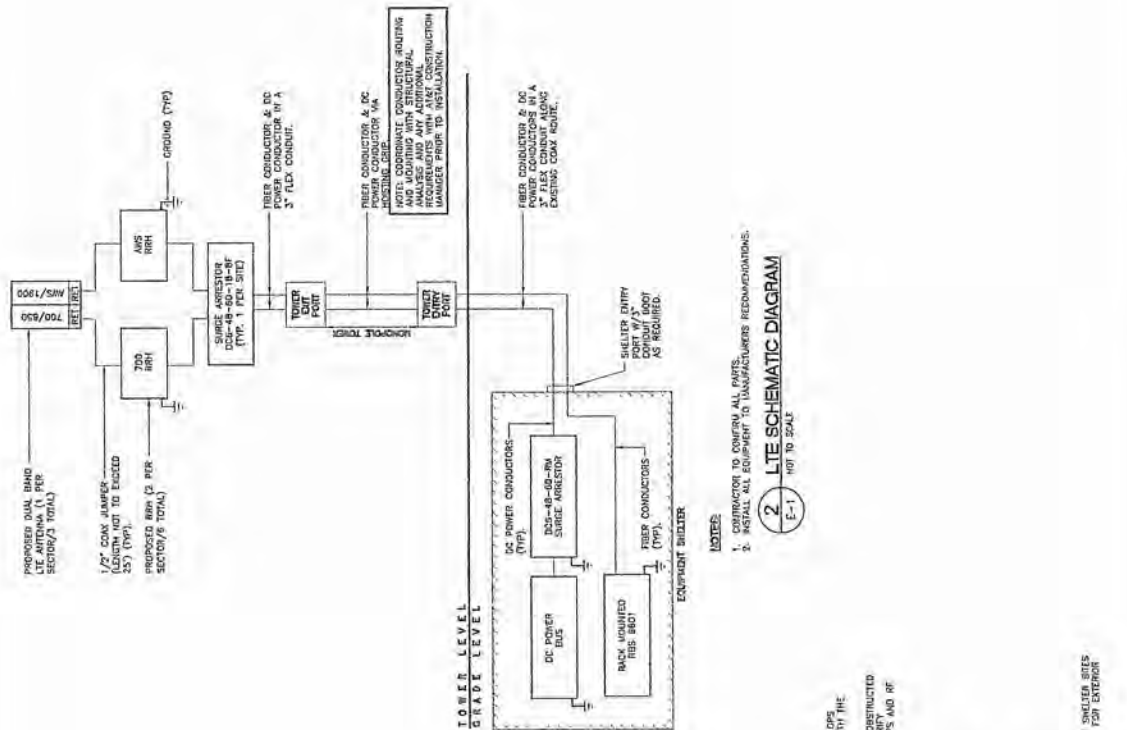
* Data for other users are from Siting Council records.

Structural information:

The attached structural analysis demonstrates that the monopole and foundation have adequate structural capacity to accommodate the proposed modifications. (Crown Castle, dated 4-21-11).

ELECTRICAL NOTES

1. PRIOR TO START OF CONSTRUCTION CONTRACTOR SHALL COORDINATE WITH OWNER FOR ALL CONSTRUCTION STANDARDS, AND SPECIFICATIONS, AND ALL MANUFACTURER SPECIALS, ALL EQUIPMENT, ACCESSORIES, AND SPECIFICATIONS, AND ALL MANUFACTURER ELECTRICAL CODE, OTHER AND MANUFACTURER'S SPECIFICATIONS.
2. VERIFY ALL NEW EQUIPMENT TO BE INSTALLED AS REQUIRED BY MANUFACTURER.
3. VERIFY ALL NEW EQUIPMENT TO BE INSTALLED AS REQUIRED BY MANUFACTURER.
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20. VERIFY ALL NEW EQUIPMENT TO BE INSTALLED AS REQUIRED BY MANUFACTURER.



NO.	DESCRIPTION	QUANTITY	REMARKS
1	CONDUCTOR - CLEAR PVC		
2	CONDUCTOR - CLEAR PVC		
3	CONDUCTOR - CLEAR PVC		
4	CONDUCTOR - CLEAR PVC		
5	CONDUCTOR - CLEAR PVC		
6	CONDUCTOR - CLEAR PVC		
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18	CONDUCTOR - CLEAR PVC		
19	CONDUCTOR - CLEAR PVC		
20	CONDUCTOR - CLEAR PVC		

Date: April 21, 2011

Veronica Harris
Crown Castle
1200 McArthur Blvd
Mahwah, NJ 07430



Crown Castle
2000 Corporate Dr.
Canonsburg, PA 15317
(724) 416-2000

Subject: Structural Analysis Report

Carrier Designation: **AT&T Mobility Co-Locate**
Carrier Site Number: 2105
Carrier Site Name: Fairfield-Woodhouse Road

Crown Castle Designation: **Crown Castle BU Number:** 806355
Crown Castle Site Name: BRG 126 943086
Crown Castle JDE Job Number: 154916
Crown Castle Work Order Number: 402472

Engineering Firm Designation: **Crown Castle Project Number:** 402472

Site Data: 281 WOODHOUSE ROAD, FAIRFIELD, Fairfield County, CT
Latitude 41° 11' 45.3", Longitude -73° 16' 52.9"
171 Foot - Monopole Tower

Dear Veronica Harris,

Crown Castle is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 402472, in accordance with application 121276, revision 1.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC1: Existing + Reserved + Proposed Equipment **Sufficient Capacity**
Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

The analysis has been performed in accordance with the TIA/EIA-222-F standard and local code requirements based upon a wind speed of 85 mph fastest mile.

All modifications and equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

We at Crown Castle appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Structural analysis prepared by: Alex Mrkajic / CSV

Respectfully submitted by:



Kenton C. Weber, P.E.
Engineering Supervisor



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1) INTRODUCTION

This tower is a 171 ft Monopole tower designed by EEI, Inc. in May of 1998. The tower was originally designed for a wind speed of 85 mph per TIA/EIA-222-E & F.

2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 85 mph with no ice, 37.6 mph with 0.75 inch ice thickness and 50 mph under service loads.

Table 1 - Proposed Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
146	148	6	ericsson	RRUS-11	2	3/8	-
		6	powerwave technologies	LGP2140X			
		3	powerwave technologies	P65-16-XLH-RR w/ Mount Pipe	1	5/8	
		1	raycap	DC6-48-60-18-8F			

Table 2 - Existing and Reserved Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note			
166	167	3	ems wireless	RV90-17-02DP	-	-	2			
	166	1	tower mounts	Pipe Mount [PM 601-3]						
155	158	3	andrew	LNx-6514DS-T4M w/ Mount Pipe	-	-	1			
		6	decibel	DB844G65ZAXY w/ Mount Pipe						
		6	rfs celwave	FD9R6004/2C-3E				12	1 5/8	3
		3	rymsa wireless	MG D3-800TV w/ Mount Pipe				-	-	1
	155	1	tower mounts	Platform Mount [LP 602-1]	-	-	-			
146	148	6	powerwave technologies	7770.00 w/ Mount Pipe	12	1 5/8	1			
		6	powerwave technologies	LGP2140X						
	146	-	-	-	1	1/2	4			
138	140	1	tower mounts	Platform Mount [LP 602-1]	-	-	1			
		3	ems wireless	DR65-18-02DP w/ Mount Pipe	12	1 5/8	1			
		3	rfs celwave	APX16DWW-16DWW-S-E-A20 w/Mount Pipe						
		3	rfs celwave	ATMAA1412D-1A20						
		3	rfs celwave	ATMPP1412D-1CWA						
3	rfs celwave	ATMPP1412D-1CWA								

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
	138	1	tower mounts	Platform Mount [LP 602-1]			
128	128	1	andrew	VHLP800-11	3 1 3	5/16 1/2 1/4	1
		3	kathrein	840 10054 w/ Mount Pipe			
		1	tower mounts	Side Arm Mount [SO 101-3]			
118	118	1	tower mounts	Side Arm Mount [SO 301-1]	-	-	2
		2	tower mounts	Side Arm Mount [SO 701-1]			
		1	tower mounts	T-Arm Mount [TA 602-1]			

Notes:

- 1) Existing Equipment
- 2) Abandoned Equipment, considered in this analysis
- 3) Reserved Equipment
- 4) Equipment to be Removed

Table 3 - Design Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
160	160	12	Allgon	ALP 9212	-	-
148	148	12	Allgon	ALP 11011	-	-
138	138	6	Celwave	APN 199015	-	-
128	128	12	Allgon	ALP 9212	-	-
118	118	12	Allgon	ALP 9212	-	-

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Clarence Welti Assoc., Inc.	1099974	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	EEL, Inc.	1098364	CCISITES
4-TOWER MANUFACTURER DRAWINGS	EEL, Inc.	653293	CCISITES

3.1) Analysis Method

RISATower (version 5.4.2.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by TIA/EIA-222-F.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail	
L1	171 - 156.5	Pole	TP10.75x10.75x0.365	1	-0.70	333.35	13.9	Pass	
L2	156.5 - 156	Pole	TP19.5x10.75x0.365	2	-0.70	625.46	8.2	Pass	
L3	156 - 132.669	Pole	TP24.79x19.5x0.1875	3	-6.58	735.45	64.7	Pass	
L4	132.669 - 87.0859	Pole	TP34.63x23.5836x0.375	4	-14.55	2052.54	85.4	Pass	
L5	87.0859 - 43	Pole	TP43.75x32.7959x0.4375	5	-25.02	3029.76	89.0	Pass	
L6	43 - 0	Pole	TP52.5x41.5315x0.5	6	-37.50	4081.19	83.2	Pass	
							Summary		
							Pole (L5)	89.0	Pass
							Rating =	89.0	Pass

Table 6 - Tower Component Stresses vs. Capacity - LC1

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	74.8	Pass
1	Base Plate	0	90.6	Pass
1	Base Foundation	0	87.0	Pass
1	Exterior Flange bolts	156	1.7	Pass
1	Exterior Flange plate	156	2.4	Pass

Structure Rating (max from all components) =	90.6%
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Notes:

- 1) See additional documentation in "Appendix C - Additional Calculations" for calculations supporting the % capacity consumed.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the existing, reserved, and proposed loads. No modifications are required at this time.

APPENDIX A
RISA TOWER OUTPUT

Section	1	2	3	4	5	6
Length (ft)	14'9"	6'	23'3-1/32"	48'3"	48'11-1/32"	48'
Number of Sides	18	18	18	18	18	18
Thickness (in)	0.3650	0.3650	0.1875	0.3750	0.4375	0.5000
Socket Length (ft)	0.3650	0.3650	38-1/32"	4'9-3/132"	6'	
Top Dia (in)	10.7500	10.7500	19.5000	23.6836	32.7959	41.5315
Bot Dia (in)	10.7500	19.5000	24.7800	34.6300	43.7500	52.5000
Grade	A53-B-35			A572-65		
Weight (K)	0.0	0.0	1.0	-5.7	8.7	12.3

171.0 ft
159.5 ft
132.7 ft
87.1 ft
43.0 ft
0.0 ft



DESIGNED APPURTENANCE LOADING

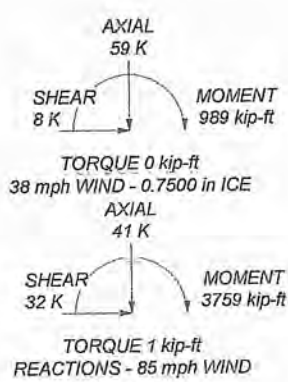
TYPE	ELEVATION	TYPE	ELEVATION
RV90-17-02DP	166	6' x 2" Mount Pipe	146
RV90-17-02DP	166	Platform Mount [LP 602-1]	146
RV90-17-02DP	166	APX16DWV-16DWV-S-E-A20 w/Mount Pipe	138
Pipe Mount [PM 601-3]	166	DR65-18-02DP w/ Mount Pipe	138
LNX-6514DS-T4M w/ Mount Pipe	155	ATMAA1412D-1A20	138
(2) DB844G65ZAXY w/ Mount Pipe	155	ATMPP1412D-1CWA	138
MG D3-800TV w/ Mount Pipe	155	APX16DWV-16DWV-S-E-A20 w/Mount Pipe	138
(2) FD9R6004/2C-3L	155	DR65-18-02DP w/ Mount Pipe	138
LNX-6514DS-T4M w/ Mount Pipe	155	ATMAA1412D-1A20	138
(2) DB844G65ZAXY w/ Mount Pipe	155	ATMPP1412D-1CWA	138
MG D3-800TV w/ Mount Pipe	155	DR65-18-02DP w/ Mount Pipe	138
(2) FD9R6004/2C-3L	155	ATMAA1412D-1A20	138
LNX-6514DS-T4M w/ Mount Pipe	155	ATMPP1412D-1CWA	138
(2) DB844G65ZAXY w/ Mount Pipe	155	APX16DWV-16DWV-S-E-A20 w/Mount Pipe	138
MG D3-800TV w/ Mount Pipe	155	DR65-18-02DP w/ Mount Pipe	138
(2) FD9R6004/2C-3L	155	ATMAA1412D-1A20	138
Platform Mount [LP 602-1]	155	ATMPP1412D-1CWA	138
P65-16-XLH-RR w/ Mount Pipe	146	6' x 2" Mount Pipe	138
(2) RRUS-11	146	6' x 2" Mount Pipe	138
(2) LGP2140X	146	Platform Mount [LP 602-1]	138
P65-16-XLH-RR w/ Mount Pipe	146	840 10054 w/ Mount Pipe	128
(2) RRUS-11	146	840 10054 w/ Mount Pipe	128
(2) LGP2140X	146	840 10054 w/ Mount Pipe	128
P65-16-XLH-RR w/ Mount Pipe	146	6' x 2" Mount Pipe	128
(2) RRUS-11	146	6' x 2" Mount Pipe	128
DC6-48-60-18-8F	146	Side Arm Mount [SO 101-3]	128
(2) LGP2140X	146	VHLP800-11	128
(2) 7770.00 w/ Mount Pipe	146	Side Arm Mount [SO 301-1]	118
(2) LGP2140X	146	Side Arm Mount [SO 701-1]	118
(2) 7770.00 w/ Mount Pipe	146	Side Arm Mount [SO 701-1]	118
(2) LGP2140X	146	(3) 6' x 2" Mount Pipe	118
(2) 7770.00 w/ Mount Pipe	146	6' x 2" Mount Pipe	118
(2) LGP2140X	146	6' x 2" Mount Pipe	118
6' x 2" Mount Pipe	146	T-Arm Mount [TA 602-1]	118
6' x 2" Mount Pipe	146		

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A53-B-35	35 ksi	63 ksi	A572-65	65 ksi	80 ksi

TOWER DESIGN NOTES

1. Tower is located in Fairfield County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 38 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 89%



<p>Crown Castle 2000 Corporate Dr. Canonsburg, PA 15317 Phone: (724) 416-2000 FAX: (724) 416-2254</p>	Job: BU# 806355
	Project:
	Client: Crown Castle
	Code: TIA/EIA-222-F
	Path:
Drawn by: cvolk	App'd:
Date: 04/21/11	Scale: NTS
Dwg No: E-1	

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Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in Fairfield County, Connecticut.

Basic wind speed of 85 mph.

Nominal ice thickness of 0.7500 in.

Ice thickness is considered to increase with height.

Ice density of 56.00 pcf.

A wind speed of 38 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Options

<ul style="list-style-type: none"> Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification √ Use Code Stress Ratios √ Use Code Safety Factors - Guys √ Escalate Ice Always Use Max Kz Use Special Wind Profile Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) Add IBC .6D+W Combination 	<ul style="list-style-type: none"> Distribute Leg Loads As Uniform Assume Legs Pinned √ Assume Rigid Index Plate √ Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension √ Bypass Mast Stability Checks √ Use Azimuth Dish Coefficients √ Project Wind Area of Appurt. Autocalc Torque Arm Areas SR Members Have Cut Ends Sort Capacity Reports By Component Triangulate Diamond Inner Bracing 	<ul style="list-style-type: none"> Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation √ Consider Feedline Torque Include Angle Block Shear Check Poles √ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets
--	--	--

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	171'-156'6"	14'6"	0'	Round	10.7500	10.7500	0.3650		A53-B-35 (35 ksi)
L2	156'6"-156'	6"	0'	18	10.7500	19.5000	0.3650	1.4600	A572-65 (65 ksi)
L3	156'-132'8"-1/32"	23'3-31/32"	3'8-1/32"	18	19.5000	24.7900	0.1875	0.7500	A572-65 (65 ksi)
L4	132'8"-1/32"-87'1-3/32"	49'3"	4'9-31/32"	18	23.5836	34.6300	0.3750	1.5000	A572-65 (65 ksi)
L5	87'1-3/32"-43'	48'11-1/32"	6'	18	32.7959	43.7500	0.4375	1.7500	A572-65 (65 ksi)

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Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft		in	in	in	in	
L6	43'-0'	49'		18	41.5315	52.5000	0.5000	2.0000	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia.	Area	I	r	C	I/C	J	I/Q	w	w/t
	in	in ²	in ⁴	in	in	in ³	in ⁴	in ²	in	
L1	10.7500	11.9083	160.6589	3.6763	5.3750	29.8900	320.8802	5.9506	0.0000	0
L2	10.7500	11.9083	160.6589	3.6763	5.3750	29.8900	320.8802	5.9506	0.0000	0
L3	10.9158	12.0311	163.9294	3.6867	5.4610	30.0182	328.0744	6.0167	1.2496	3.424
L4	19.8008	22.1681	1025.4692	6.7929	9.9060	103.5200	2052.2878	11.0862	2.7896	7.643
L5	19.8008	11.4934	541.5782	6.8559	9.9060	54.6717	1083.8689	5.7478	3.1020	16.544
L6	25.1724	14.6416	1119.6528	8.7339	12.5933	88.9085	2240.7788	7.3222	4.0330	21.51
L7	24.7825	27.6241	1879.8522	8.2391	11.9805	156.9096	3762.1780	13.8147	3.4907	9.309
L8	35.1642	40.7720	6044.3215	12.1605	17.5920	343.5828	12096.5963	20.3899	5.4349	14.493
L9	34.4008	44.9337	5944.0771	11.4872	16.6603	356.7803	11895.9756	22.4711	5.0021	11.433
L10	44.4249	60.1448	14254.8347	15.3759	22.2250	641.3874	28528.4263	30.0781	6.9300	15.84
L11	43.5360	65.1169	13850.5059	14.5662	21.0980	656.4847	27719.2367	32.5647	6.4295	12.859
L12	53.3099	82.5240	28191.9040	18.4600	26.6700	1057.0643	56420.9036	41.2698	8.3600	16.72

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft ²	in					in	in
L1 171'-156'6"				1	1	1		
L2 156'6"-156'				1	1	1		
L3				1	1	1		
156'-132'8"-1/3 2"								
L4				1	1	1		
132'8"-1/32"-87' 1-3/32"								
L5				1	1	1		
87'1-3/32"-43' L6 43'-0"				1	1	1		

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement	Face Offset	Lateral Offset (Frac FW)	#	C _A A _A	Weight
				ft	in			ft ² /ft	klf

561(1-5/8")	C	No	Inside Pole	155' - 0'	0.0000	0	12	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.00 0.00 0.00 0.00 0.00
FB-L98B-002-75000(3/8")	C	No	Inside Pole	146' - 0'	0.0000	0	2	No Ice 1/2" Ice 1" Ice 2" Ice	0.00 0.00 0.00 0.00

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Description	Face or Shield Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	C _A A _A ft ² /ft	Weight klf	
WR-VG82ST-BRDA (5/8")	C	No	Inside Pole	146' - 0'	0.0000	0	1	4" Ice	0.00	0.00
								No Ice	0.00	0.00
								1/2" Ice	0.00	0.00
								1" Ice	0.00	0.00
								2" Ice	0.00	0.00
CR 50 1873PE(1-5/8")	C	No	Inside Pole	146' - 0'	0.0000	0	12	4" Ice	0.00	0.00
								No Ice	0.00	0.00
								1/2" Ice	0.00	0.00
								1" Ice	0.00	0.00
								2" Ice	0.00	0.00
LCF158-50JA -A0(1 5/8")	C	No	Inside Pole	138' - 0'	0.0000	0	6	4" Ice	0.00	0.00
								No Ice	0.00	0.00
								1/2" Ice	0.00	0.00
								1" Ice	0.00	0.00
								2" Ice	0.00	0.00
LCF158-50JA -A0(1 5/8")	C	No	CaAa (Out Of Face)	138' - 0'	1.0000	0	1	4" Ice	0.00	0.00
								No Ice	0.20	0.00
								1/2" Ice	0.30	0.00
								1" Ice	0.40	0.00
								2" Ice	0.60	0.01
LCF158-50JA -A0(1 5/8")	C	No	CaAa (Out Of Face)	138' - 0'	1.0000	0	5	4" Ice	1.00	0.03
								No Ice	0.00	0.00
								1/2" Ice	0.00	0.00
								1" Ice	0.00	0.00
								2" Ice	0.00	0.01
7983A(1/2")	C	No	CaAa (Out Of Face)	128' - 0'	1.0000	0	1	4" Ice	0.00	0.00
								No Ice	0.00	0.00
								1/2" Ice	0.00	0.00
								1" Ice	0.00	0.00
								2" Ice	0.00	0.01
9207(5/16")	C	No	CaAa (Out Of Face)	128' - 0'	1.0000	0	3	4" Ice	0.00	0.02
								No Ice	0.00	0.00
								1/2" Ice	0.00	0.00
								1" Ice	0.00	0.00
								2" Ice	0.00	0.01
9258(1/4")	C	No	CaAa (Out Of Face)	128' - 0'	1.0000	0	3	4" Ice	0.00	0.02
								No Ice	0.00	0.00
								1/2" Ice	0.00	0.00
								1" Ice	0.00	0.00
								2" Ice	0.00	0.01
2" Rigid Conduit	C	No	CaAa (Out Of Face)	128' - 0'	1.0000	0	1	4" Ice	0.00	0.02
								No Ice	0.00	0.00
								1/2" Ice	0.00	0.00
								1" Ice	0.00	0.01
								2" Ice	0.00	0.01
2" Rigid Conduit	C	No	CaAa (Out Of Face)	128' - 0'	1.0000	0	1	4" Ice	0.00	0.03
								No Ice	0.20	0.00
								1/2" Ice	0.30	0.00
								1" Ice	0.40	0.01
								2" Ice	0.60	0.01
* Climbing Ladder (Flat)	C	No	CaAa (Out Of Face)	155' - 147'	48.0000	0	1	4" Ice	1.00	0.03
								No Ice	0.58	0.00
								1/2" Ice	1.03	0.01
								1" Ice	1.48	0.01
								2" Ice	2.37	0.02
Climbing Ladder (Flat)	C	No	CaAa (Out Of Face)	146' - 138'	48.0000	0	1	4" Ice	4.15	0.05
								No Ice	0.58	0.00
								1/2" Ice	1.03	0.01
								1" Ice	1.48	0.01
								2" Ice	2.37	0.02

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Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	C _A A _A ft ² /ft	Weight klf
Climbing Ladder (Flat)	C	No	CaAa (Out Of Face)	138' - 130'	48.0000	0	1	4" Ice 4.15	0.05
								No Ice 0.58	0.00
								1/2" Ice 1.03	0.01
								1" Ice 1.48	0.01
								2" Ice 2.37	0.02
							4" Ice 4.15	0.05	

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L1	171'-156'6"	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.00
L2	156'6"-156'	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.00
L3	156'-132'8"-1/32"	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	13.521	0.61
L4	132'8"-1/32"-87'1-3/32"	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	18.768	1.58
L5	87'1-3/32"-43'	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	17.546	1.55
L6	43'-0'	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	17.114	1.51

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L1	171'-156'6"	A	0.909	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.00
L2	156'6"-156'	A	0.904	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.00
L3	156'-132'8"-1/32"	A	0.895	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	31.495	0.81
L4	132'8"-1/32"-87'1-3/32"	A	0.866	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	36.380	3.11
L5	87'1-3/32"-43'	A	0.813	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	32.809	3.03
L6	43'-0'	A	0.750	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	31.098	2.83

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Feed Line Center of Pressure

Section	Elevation	CP _X	CP _Z	CP _X Ice	CP _Z Ice
	ft	in	in	in	in
L1	171'-156'6"	0.0000	0.0000	0.0000	0.0000
L2	156'6"-156'	0.0000	0.0000	0.0000	0.0000
L3	156'-132'8"-1/32"	-0.5804	0.3351	-0.9791	0.5653
L4	132'8"-1/32"-87'1'-3/32"	-0.4555	0.2630	-0.7418	0.4283
L5	87'1'-3/32"-43'	-0.4604	0.2658	-0.7584	0.4379
L6	43'-0'	-0.4700	0.2713	-0.7725	0.4460

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement	C _A A _{Front}	C _A A _{Side}	Weight	
			ft ft ft	°	ft	ft ²	ft ²	K	
** RV90-17-02DP	A	From Leg	1.00 0' 1'	0.000	166'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.36 4.77 5.20 6.08 7.95	1.97 2.31 2.66 3.37 4.89	0.02 0.04 0.07 0.14 0.33
RV90-17-02DP	B	From Leg	1.00 0' 1'	0.000	166'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.36 4.77 5.20 6.08 7.95	1.97 2.31 2.66 3.37 4.89	0.02 0.04 0.07 0.14 0.33
RV90-17-02DP	C	From Leg	1.00 0' 1'	0.000	166'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.36 4.77 5.20 6.08 7.95	1.97 2.31 2.66 3.37 4.89	0.02 0.04 0.07 0.14 0.33
Pipe Mount [PM 601-3]	C	None		0.000	166'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.39 5.48 6.57 8.75 13.11	4.39 5.48 6.57 8.75 13.11	0.20 0.24 0.28 0.36 0.53
* LNX-6514DS-T4M w/ Mount Pipe	A	From Leg	4.00 0' 3'	0.000	155'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	8.57 9.22 9.84 11.10 13.75	7.00 8.19 9.08 10.90 14.93	0.06 0.12 0.20 0.38 0.89
(2) DB844G65ZAXY w/ Mount Pipe	A	From Leg	4.00 0' 3'	0.000	155'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.90 5.35 5.80 6.73 8.73	4.92 5.60 6.28 7.71 10.83	0.03 0.08 0.13 0.26 0.62
MG D3-800TV w/ Mount Pipe	A	From Leg	4.00 0'	0.000	155'	No Ice 1/2" Ice	3.57 3.98	3.42 4.12	0.04 0.07



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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A ₁ Front	C _A A ₁ Side	Weight
			Horz	Lateral					
				3'			1" Ice 4.39	4.78	0.11
							2" Ice 5.33	6.16	0.21
							4" Ice 7.34	9.18	0.52
(2) FD9R6004/2C-3L	A	From Leg	4.00		0.000	155'	No Ice 0.37	0.08	0.00
			0'				1/2" Ice 0.45	0.14	0.01
			3'				1" Ice 0.54	0.20	0.01
							2" Ice 0.75	0.34	0.02
							4" Ice 1.28	0.74	0.06
LNX-6514DS-T4M w/ Mount Pipe	B	From Leg	4.00		0.000	155'	No Ice 8.57	7.00	0.06
			0'				1/2" Ice 9.22	8.19	0.12
			3'				1" Ice 9.84	9.08	0.20
							2" Ice 11.10	10.90	0.38
							4" Ice 13.75	14.93	0.89
(2) DB844G65ZAXY w/ Mount Pipe	B	From Leg	4.00		0.000	155'	No Ice 4.90	4.92	0.03
			0'				1/2" Ice 5.35	5.60	0.08
			3'				1" Ice 5.80	6.28	0.13
							2" Ice 6.73	7.71	0.26
							4" Ice 8.73	10.83	0.62
MG D3-800TV w/ Mount Pipe	B	From Leg	4.00		0.000	155'	No Ice 3.57	3.42	0.04
			0'				1/2" Ice 3.98	4.12	0.07
			3'				1" Ice 4.39	4.78	0.11
							2" Ice 5.33	6.16	0.21
							4" Ice 7.34	9.18	0.52
(2) FD9R6004/2C-3L	B	From Leg	4.00		0.000	155'	No Ice 0.37	0.08	0.00
			0'				1/2" Ice 0.45	0.14	0.01
			3'				1" Ice 0.54	0.20	0.01
							2" Ice 0.75	0.34	0.02
							4" Ice 1.28	0.74	0.06
LNX-6514DS-T4M w/ Mount Pipe	C	From Leg	4.00		0.000	155'	No Ice 8.57	7.00	0.06
			0'				1/2" Ice 9.22	8.19	0.12
			3'				1" Ice 9.84	9.08	0.20
							2" Ice 11.10	10.90	0.38
							4" Ice 13.75	14.93	0.89
(2) DB844G65ZAXY w/ Mount Pipe	C	From Leg	4.00		0.000	155'	No Ice 4.90	4.92	0.03
			0'				1/2" Ice 5.35	5.60	0.08
			3'				1" Ice 5.80	6.28	0.13
							2" Ice 6.73	7.71	0.26
							4" Ice 8.73	10.83	0.62
MG D3-800TV w/ Mount Pipe	C	From Leg	4.00		0.000	155'	No Ice 3.57	3.42	0.04
			0'				1/2" Ice 3.98	4.12	0.07
			3'				1" Ice 4.39	4.78	0.11
							2" Ice 5.33	6.16	0.21
							4" Ice 7.34	9.18	0.52
(2) FD9R6004/2C-3L	C	From Leg	4.00		0.000	155'	No Ice 0.37	0.08	0.00
			0'				1/2" Ice 0.45	0.14	0.01
			3'				1" Ice 0.54	0.20	0.01
							2" Ice 0.75	0.34	0.02
							4" Ice 1.28	0.74	0.06
Platform Mount [LP 602-1]	C	None			0.000	155'	No Ice 32.03	32.03	1.34
							1/2" Ice 38.71	38.71	1.80
							1" Ice 45.39	45.39	2.26
							2" Ice 58.75	58.75	3.17
							4" Ice 85.47	85.47	5.00
* P65-16-XLH-RR w/ Mount Pipe	A	From Leg	4.00		0.000	146'	No Ice 8.64	6.36	0.08
			0'				1/2" Ice 9.29	7.54	0.14
			2'				1" Ice 9.91	8.43	0.22

RISA Tower <i>Crown Castle</i> 2000 Corporate Dr. Canonsburg, PA 15317 Phone: (724) 416-2000 FAX: (724) 416-2254	Job	BU# 806355	Page	7 of 18
	Project		Date	11:39:29 04/21/11
	Client	Crown Castle	Designed by	cvolk

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft. ft. ft.	Azimuth Adjustment °	Placement ft	C _A A _F Front ft ²	C _A A _S Side ft ²	Weight K	
(2) RRUS-11	A	From Leg	4.00 0' 2'	0.000	146'	2" Ice	11.18	10.24	0.39
						4" Ice	13.83	14.10	0.89
						No Ice	4.42	1.19	0.06
						1/2" Ice	4.71	1.35	0.08
						1" Ice	5.00	1.53	0.11
						2" Ice	5.61	1.90	0.18
(2) LGP2140X	A	From Leg	4.00 0' 2'	0.000	146'	4" Ice	6.94	2.75	0.37
						No Ice	1.26	0.38	0.02
						1/2" Ice	1.42	0.49	0.03
						1" Ice	1.58	0.62	0.04
						2" Ice	1.94	0.89	0.06
						4" Ice	2.75	1.54	0.14
P65-16-XLH-RR w/ Mount Pipe	B	From Leg	4.00 0' 2'	0.000	146'	No Ice	8.64	6.36	0.08
						1/2" Ice	9.29	7.54	0.14
						1" Ice	9.91	8.43	0.22
						2" Ice	11.18	10.24	0.39
						4" Ice	13.83	14.10	0.89
						No Ice	4.42	1.19	0.06
(2) RRUS-11	B	From Leg	4.00 0' 2'	0.000	146'	1/2" Ice	4.71	1.35	0.08
						1" Ice	5.00	1.53	0.11
						2" Ice	5.61	1.90	0.18
						4" Ice	6.94	2.75	0.37
						No Ice	1.26	0.38	0.02
						1/2" Ice	1.42	0.49	0.03
(2) LGP2140X	B	From Leg	4.00 0' 2'	0.000	146'	1" Ice	1.58	0.62	0.04
						2" Ice	1.94	0.89	0.06
						4" Ice	2.75	1.54	0.14
						No Ice	8.64	6.36	0.08
						1/2" Ice	9.29	7.54	0.14
						1" Ice	9.91	8.43	0.22
P65-16-XLH-RR w/ Mount Pipe	C	From Leg	4.00 0' 2'	0.000	146'	2" Ice	11.18	10.24	0.39
						4" Ice	13.83	14.10	0.89
						No Ice	4.42	1.19	0.06
						1/2" Ice	4.71	1.35	0.08
						1" Ice	5.00	1.53	0.11
						2" Ice	5.61	1.90	0.18
(2) RRUS-11	C	From Leg	4.00 0' 2'	0.000	146'	4" Ice	6.94	2.75	0.37
						No Ice	1.27	1.27	0.02
						1/2" Ice	1.46	1.46	0.04
						1" Ice	1.66	1.66	0.05
						2" Ice	2.09	2.09	0.10
						4" Ice	3.10	3.10	0.21
DC6-48-60-18-8F	C	From Leg	4.00 0' 2'	0.000	146'	No Ice	1.26	0.38	0.02
						1/2" Ice	1.42	0.49	0.03
						1" Ice	1.58	0.62	0.04
						2" Ice	1.94	0.89	0.06
						4" Ice	2.75	1.54	0.14
						No Ice	6.12	4.25	0.06
(2) 7770.00 w/ Mount Pipe	A	From Leg	4.00 0' 2'	0.000	146'	1/2" Ice	6.63	5.01	0.10
						1" Ice	7.13	5.71	0.16
						2" Ice	8.16	7.16	0.29
						4" Ice	10.36	10.41	0.66
						No Ice	1.26	0.38	0.02
						1/2" Ice	1.42	0.49	0.03
(2) LGP2140X	A	From Leg	4.00 0' 2'	0.000	146'	1" Ice	1.58	0.62	0.04
						2" Ice	1.94	0.89	0.06
						4" Ice	2.75	1.54	0.14
						No Ice	1.26	0.38	0.02
						1/2" Ice	1.42	0.49	0.03
						1" Ice	1.58	0.62	0.04

RISA Tower

Crown Castle
 2000 Corporate Dr.
 Canonsburg, PA 15317
 Phone: (724) 416-2000
 FAX: (724) 416-2254

Job	BU# 806355	Page	8 of 18
Project		Date	11:39:29 04/21/11
Client	Crown Castle	Designed by	cvolk

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A _{Front}	C _A A _{Side}	Weight
			Horz	Lateral					
(2) 7770.00 w/ Mount Pipe	B	From Leg	4.00	0.000	146'	No Ice	6.12	4.25	0.06
			0'			1/2" Ice	6.63	5.01	0.10
			2'			1" Ice	7.13	5.71	0.16
						2" Ice	8.16	7.16	0.29
						4" Ice	10.36	10.41	0.66
(2) LGP2140X	B	From Leg	4.00	0.000	146'	No Ice	1.26	0.38	0.02
			0'			1/2" Ice	1.42	0.49	0.03
			2'			1" Ice	1.58	0.62	0.04
						2" Ice	1.94	0.89	0.06
						4" Ice	2.75	1.54	0.14
(2) 7770.00 w/ Mount Pipe	C	From Leg	4.00	0.000	146'	No Ice	6.12	4.25	0.06
			0'			1/2" Ice	6.63	5.01	0.10
			2'			1" Ice	7.13	5.71	0.16
						2" Ice	8.16	7.16	0.29
						4" Ice	10.36	10.41	0.66
(2) LGP2140X	C	From Leg	4.00	0.000	146'	No Ice	1.26	0.38	0.02
			0'			1/2" Ice	1.42	0.49	0.03
			2'			1" Ice	1.58	0.62	0.04
						2" Ice	1.94	0.89	0.06
						4" Ice	2.75	1.54	0.14
6' x 2" Mount Pipe	A	From Leg	4.00	0.000	146'	No Ice	1.43	1.43	0.02
			0'			1/2" Ice	1.92	1.92	0.03
			0'			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
						4" Ice	4.70	4.70	0.23
6' x 2" Mount Pipe	B	From Leg	4.00	0.000	146'	No Ice	1.43	1.43	0.02
			0'			1/2" Ice	1.92	1.92	0.03
			0'			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
						4" Ice	4.70	4.70	0.23
6' x 2" Mount Pipe	C	From Leg	4.00	0.000	146'	No Ice	1.43	1.43	0.02
			0'			1/2" Ice	1.92	1.92	0.03
			0'			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
						4" Ice	4.70	4.70	0.23
Platform Mount [LP 602-1]	C	None		0.000	146'	No Ice	32.03	32.03	1.34
						1/2" Ice	38.71	38.71	1.80
						1" Ice	45.39	45.39	2.26
						2" Ice	58.75	58.75	3.17
						4" Ice	85.47	85.47	5.00
APX16DWV-16DWV-S-E-A 20 w/Mount Pipe	A	From Leg	4.00	0.000	138'	No Ice	7.27	3.29	0.06
			0'			1/2" Ice	7.73	3.92	0.10
			2'			1" Ice	8.21	4.57	0.16
						2" Ice	9.18	5.92	0.28
						4" Ice	11.23	8.88	0.65
DR65-18-02DP w/ Mount Pipe	A	From Leg	4.00	0.000	138'	No Ice	10.40	6.73	0.07
			0'			1/2" Ice	11.19	8.86	0.14
			2'			1" Ice	11.99	11.02	0.22
						2" Ice	13.55	14.26	0.42
						4" Ice	16.72	19.69	1.02
ATMAA1412D-1A20	A	From Leg	4.00	0.000	138'	No Ice	1.17	0.47	0.01
			0'			1/2" Ice	1.31	0.57	0.02
			2'			1" Ice	1.47	0.69	0.03
						2" Ice	1.81	0.95	0.06
						4" Ice	2.58	1.57	0.14
ATMPP1412D-1CWA	A	From Leg	4.00	0.000	138'	No Ice	1.17	0.42	0.01

RISA Tower <i>Crown Casile</i> 2000 Corporate Dr. Canonsburg, PA 15317 Phone: (724) 416-2000 FAX: (724) 416-2254	Job	BU# 806355	Page	9 of 18
	Project		Date	11:39:29 04/21/11
	Client	Crown Casile	Designed by	cvolk

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A ₁ Front	C _A A ₁ Side	Weight
			Horz	Lateral					
APX16DWV-16DWV-S-E-A 20 w/Mount Pipe	B	From Leg	4.00	0.000	138'	1/2" Ice	1.32	0.53	0.02
			0'			1" Ice	1.48	0.65	0.03
			2'			2" Ice	1.82	0.92	0.05
						4" Ice	2.61	1.57	0.13
						No Ice	7.27	3.29	0.06
						1/2" Ice	7.73	3.92	0.10
						1" Ice	8.21	4.57	0.16
						2" Ice	9.18	5.92	0.28
						4" Ice	11.23	8.88	0.65
DR65-18-02DP w/ Mount Pipe	B	From Leg	4.00	0.000	138'	No Ice	10.40	6.73	0.07
			0'			1/2" Ice	11.19	8.86	0.14
			2'			1" Ice	11.99	11.02	0.22
						2" Ice	13.55	14.26	0.42
						4" Ice	16.72	19.69	1.02
ATMAA1412D-1A20	B	From Leg	4.00	0.000	138'	No Ice	1.17	0.47	0.01
			0'			1/2" Ice	1.31	0.57	0.02
			2'			1" Ice	1.47	0.69	0.03
						2" Ice	1.81	0.95	0.06
						4" Ice	2.58	1.57	0.14
ATMPP1412D-1CWA	B	From Leg	4.00	0.000	138'	No Ice	1.17	0.42	0.01
			0'			1/2" Ice	1.32	0.53	0.02
			2'			1" Ice	1.48	0.65	0.03
						2" Ice	1.82	0.92	0.05
						4" Ice	2.61	1.57	0.13
DR65-18-02DP w/ Mount Pipe	C	From Leg	4.00	0.000	138'	No Ice	10.40	6.73	0.07
			0'			1/2" Ice	11.19	8.86	0.14
			2'			1" Ice	11.99	11.02	0.22
						2" Ice	13.55	14.26	0.42
						4" Ice	16.72	19.69	1.02
ATMAA1412D-1A20	C	From Leg	4.00	0.000	138'	No Ice	1.17	0.47	0.01
			0'			1/2" Ice	1.31	0.57	0.02
			2'			1" Ice	1.47	0.69	0.03
						2" Ice	1.81	0.95	0.06
						4" Ice	2.58	1.57	0.14
ATMPP1412D-1CWA	C	From Leg	4.00	0.000	138'	No Ice	1.17	0.42	0.01
			0'			1/2" Ice	1.32	0.53	0.02
			2'			1" Ice	1.48	0.65	0.03
						2" Ice	1.82	0.92	0.05
						4" Ice	2.61	1.57	0.13
APX16DWV-16DWV-S-E-A 20 w/Mount Pipe	C	From Leg	4.00	0.000	138'	No Ice	7.27	3.29	0.06
			0'			1/2" Ice	7.73	3.92	0.10
			2'			1" Ice	8.21	4.57	0.16
						2" Ice	9.18	5.92	0.28
						4" Ice	11.23	8.88	0.65
6' x 2" Mount Pipe	A	From Leg	4.00	0.000	138'	No Ice	1.43	1.43	0.02
			0'			1/2" Ice	1.92	1.92	0.03
			0'			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
						4" Ice	4.70	4.70	0.23
6' x 2" Mount Pipe	B	From Leg	4.00	0.000	138'	No Ice	1.43	1.43	0.02
			0'			1/2" Ice	1.92	1.92	0.03
			0'			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
						4" Ice	4.70	4.70	0.23
6' x 2" Mount Pipe	C	From Leg	4.00	0.000	138'	No Ice	1.43	1.43	0.02
			0'			1/2" Ice	1.92	1.92	0.03
			0'			1" Ice	2.29	2.29	0.05

RISATower

Crown Castle
 2000 Corporate Dr.
 Canonsburg, PA 15317
 Phone: (724) 416-2000
 FAX: (724) 416-2254

Job	BU# 806355	Page	10 of 18
Project		Date	11:39:29 04/21/11
Client	Crown Castle	Designed by	cvolk

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A _F Front	C _A A _S Side	Weight	
			Horz	Lateral						Vert
Platform Mount [LP 602-1]	C	None			0.000	138'	2" Ice	3.06	3.06	0.09
							4" Ice	4.70	4.70	0.23
							No Ice	32.03	32.03	1.34
							1/2" Ice	38.71	38.71	1.80
							1" Ice	45.39	45.39	2.26
							2" Ice	58.75	58.75	3.17
* 840 10054 w/ Mount Pipe	A	From Leg	2.00 0' 0'		0.000	128'	No Ice	5.41	2.39	0.05
							1/2" Ice	5.83	2.92	0.09
							1" Ice	6.26	3.47	0.13
							2" Ice	7.16	4.61	0.23
							4" Ice	9.09	7.32	0.53
							No Ice	5.41	2.39	0.05
840 10054 w/ Mount Pipe	B	From Leg	2.00 0' 0'		0.000	128'	1/2" Ice	5.83	2.92	0.09
							1" Ice	6.26	3.47	0.13
							2" Ice	7.16	4.61	0.23
							4" Ice	9.09	7.32	0.53
							No Ice	5.41	2.39	0.05
							1/2" Ice	5.83	2.92	0.09
840 10054 w/ Mount Pipe	C	From Leg	2.00 0' 0'		0.000	128'	1" Ice	6.26	3.47	0.13
							2" Ice	7.16	4.61	0.23
							4" Ice	9.09	7.32	0.53
							No Ice	5.41	2.39	0.05
							1/2" Ice	5.83	2.92	0.09
							1" Ice	6.26	3.47	0.13
6' x 2" Mount Pipe	B	From Leg	2.00 0' 0'		0.000	128'	2" Ice	7.16	4.61	0.23
							4" Ice	9.09	7.32	0.53
							No Ice	1.43	1.43	0.02
							1/2" Ice	1.92	1.92	0.03
							1" Ice	2.29	2.29	0.05
							2" Ice	3.06	3.06	0.09
Side Arm Mount [SO 101-3]	C	None			0.000	128'	4" Ice	4.70	4.70	0.23
							No Ice	7.50	7.50	0.25
							1/2" Ice	8.90	8.90	0.33
							1" Ice	10.30	10.30	0.41
							2" Ice	13.10	13.10	0.58
							4" Ice	18.70	18.70	0.90
T-Arm Mount [TA 602-1]	A	From Leg	3.00 0' 0'		0.000	118'	No Ice	7.28	3.02	0.26
							1/2" Ice	9.52	4.20	0.33
							1" Ice	11.76	5.38	0.40
							2" Ice	16.24	7.74	0.55
							4" Ice	25.20	12.46	0.83
							No Ice	1.00	0.90	0.02
Side Arm Mount [SO 301-1]	B	From Leg	3.00 0' 0'		0.000	118'	1/2" Ice	1.39	1.42	0.03
							1" Ice	1.78	1.94	0.04
							2" Ice	2.56	2.98	0.06
							4" Ice	4.12	5.06	0.10
							No Ice	0.85	1.67	0.07
							1/2" Ice	1.14	2.34	0.08
Side Arm Mount [SO 701-1]	B	From Leg	3.00 0' 0'		0.000	118'	1" Ice	1.43	3.01	0.09
							2" Ice	2.01	4.35	0.12
							4" Ice	3.17	7.03	0.18
							No Ice	0.85	1.67	0.07
							1/2" Ice	1.14	2.34	0.08
							1" Ice	1.43	3.01	0.09
Side Arm Mount [SO 701-1]	C	From Leg	3.00 0' 0'		0.000	118'	2" Ice	2.01	4.35	0.12
							4" Ice	3.17	7.03	0.18
							No Ice	0.85	1.67	0.07
							1/2" Ice	1.14	2.34	0.08
							1" Ice	1.43	3.01	0.09
							2" Ice	2.01	4.35	0.12
(3) 6' x 2" Mount Pipe	A	From Leg	3.00 0' 0'		0.000	118'	4" Ice	3.17	7.03	0.18
							No Ice	1.43	1.43	0.02
							1/2" Ice	1.92	1.92	0.03
							1" Ice	2.29	2.29	0.05

RISA Tower Crown Castle 2000 Corporate Dr. Canonsburg, PA 15317 Phone: (724) 416-2000 FAX: (724) 416-2254	Job	BU# 806355	Page	11 of 18
	Project		Date	11:39:29 04/21/11
	Client	Crown Castle	Designed by	cvolk

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight	
			ft ft ft	°	ft	ft ²	ft ²	K	
6' x 2" Mount Pipe	B	From Leg	3.00 0' 0'	0.000	118'	2" Ice	3.06	3.06	0.09
						4" Ice	4.70	4.70	0.23
						No Ice	1.43	1.43	0.02
						1/2" Ice	1.92	1.92	0.03
						1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
6' x 2" Mount Pipe	C	From Leg	3.00 0' 0'	0.000	118'	4" Ice	4.70	4.70	0.23
						No Ice	1.43	1.43	0.02
						1/2" Ice	1.92	1.92	0.03
						1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
						4" Ice	4.70	4.70	0.23

Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weight	
				ft ft ft	°	°	ft	ft	ft ²	K	
VHLP800-11	B	Paraboloid w/Shroud (HP)	From Leg	2.00 0' 0'	-61.000		128'	2.92	No Ice	6.68	0.02
									1/2" Ice	7.07	0.03
									1" Ice	7.46	0.03
									2" Ice	8.23	0.07
									4" Ice	9.78	0.23

Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 60 deg - No Ice
5	Dead+Wind 90 deg - No Ice
6	Dead+Wind 120 deg - No Ice
7	Dead+Wind 150 deg - No Ice
8	Dead+Wind 180 deg - No Ice
9	Dead+Wind 210 deg - No Ice
10	Dead+Wind 240 deg - No Ice
11	Dead+Wind 270 deg - No Ice
12	Dead+Wind 300 deg - No Ice
13	Dead+Wind 330 deg - No Ice
14	Dead+Ice+Temp
15	Dead+Wind 0 deg+Ice+Temp
16	Dead+Wind 30 deg+Ice+Temp
17	Dead+Wind 60 deg+Ice+Temp
18	Dead+Wind 90 deg+Ice+Temp

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Comb. No.	Description
19	Dead+Wind 120 deg+Ice+Temp
20	Dead+Wind 150 deg+Ice+Temp
21	Dead+Wind 180 deg+Ice+Temp
22	Dead+Wind 210 deg+Ice+Temp
23	Dead+Wind 240 deg+Ice+Temp
24	Dead+Wind 270 deg+Ice+Temp
25	Dead+Wind 300 deg+Ice+Temp
26	Dead+Wind 330 deg+Ice+Temp
27	Dead+Wind 0 deg - Service
28	Dead+Wind 30 deg - Service
29	Dead+Wind 60 deg - Service
30	Dead+Wind 90 deg - Service
31	Dead+Wind 120 deg - Service
32	Dead+Wind 150 deg - Service
33	Dead+Wind 180 deg - Service
34	Dead+Wind 210 deg - Service
35	Dead+Wind 240 deg - Service
36	Dead+Wind 270 deg - Service
37	Dead+Wind 300 deg - Service
38	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	171 - 156.5	Pole	Max Tension	8	0.00	-0.00	0.00
			Max. Compression	14	-1.24	0.00	-0.00
			Max. Mx	11	-0.70	10.53	-0.00
			Max. My	8	-0.70	0.00	-10.53
			Max. Vy	11	-1.16	10.53	-0.00
			Max. Vx	8	1.16	0.00	-10.53
			Max. Torque	8			0.00
			Max Tension	1	0.00	0.00	0.00
L2	156.5 - 156	Pole	Max. Compression	14	-1.27	0.00	-0.00
			Max. Mx	11	-0.73	11.11	-0.00
			Max. My	8	-0.73	0.01	-11.11
			Max. Vy	11	-1.19	11.11	-0.00
			Max. Vx	8	1.19	0.01	-11.11
			Max. Torque	8			0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-15.46	0.47	-0.25
L3	156 - 132.669	Pole	Max. Mx	11	-6.59	228.63	-0.13
			Max. My	8	-6.59	0.17	-228.57
			Max. Vy	11	-17.85	228.63	-0.13
			Max. Vx	8	17.85	0.17	-228.57
			Max. Torque	13			0.38
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-26.80	2.14	0.04
			Max. Mx	11	-14.57	1177.04	-4.21
L4	132.669 - 87.0859	Pole	Max. My	2	-14.56	-4.30	1178.64
			Max. Vy	11	-23.90	1177.04	-4.21
			Max. Vx	8	24.02	7.74	-1178.24
			Max. Torque	11			1.25
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-39.80	4.69	-1.40
			Max. Mx	11	-25.03	2286.91	-10.10
			Max. My	8	-25.02	16.84	-2293.06

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L6	43 - 0	Pole	Max. Vy	11	-27.70	2286.91	-10.10
			Max. Vx	8	27.82	16.84	-2293.06
			Max. Torque	9			0.77
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-59.00	7.91	-3.26
			Max. Mx	11	-41.19	3738.20	-16.77
			Max. My	8	-41.19	27.19	-3749.93
			Max. Vy	11	-31.44	3738.20	-16.77
			Max. Vx	8	31.56	27.19	-3749.93
			Max. Torque	11			0.84

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	22	59.00	3.93	-6.81
	Max. H _x	11	41.21	31.41	-0.13
	Max. H _z	2	41.21	-0.12	31.51
	Max. M _x	2	3747.14	-0.12	31.51
	Max. M _z	5	3725.28	-31.34	0.10
	Max. Torsion	11	0.84	31.41	-0.13
	Min. Vert	1	41.21	0.00	0.00
	Min. H _x	5	41.21	-31.34	0.10
	Min. H _z	8	41.21	0.19	-31.53
	Min. M _x	8	-3749.93	0.19	-31.53
	Min. M _z	11	-3738.20	31.41	-0.13
	Min. Torsion	5	-0.56	-31.34	0.10

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead Only	41.21	0.00	0.00	0.09	1.45	0.00
Dead+Wind 0 deg - No Ice	41.21	0.12	-31.51	-3747.14	-14.69	0.37
Dead+Wind 30 deg - No Ice	41.21	15.75	-27.29	-3245.11	-1872.92	0.44
Dead+Wind 60 deg - No Ice	41.21	27.16	-15.80	-1879.24	-3229.28	0.51
Dead+Wind 90 deg - No Ice	41.21	31.34	-0.10	-13.40	-3725.28	0.56
Dead+Wind 120 deg - No Ice	41.21	27.15	15.65	1859.93	-3227.56	0.45
Dead+Wind 150 deg - No Ice	41.21	15.65	27.25	3239.55	-1859.03	0.08
Dead+Wind 180 deg - No Ice	41.21	-0.19	31.53	3749.93	27.19	-0.44
Dead+Wind 210 deg - No Ice	41.21	-15.81	27.34	3252.52	1883.69	-0.58
Dead+Wind 240 deg - No Ice	41.21	-27.23	15.84	1884.55	3240.90	-0.71
Dead+Wind 270 deg - No Ice	41.21	-31.41	0.13	16.77	3738.20	-0.84
Dead+Wind 300 deg - No Ice	41.21	-27.20	-15.60	-1852.68	3236.92	-0.76
Dead+Wind 330 deg - No Ice	41.21	-15.59	-27.28	-3243.75	1854.93	0.08
Dead+Ice+Temp	59.00	-0.00	0.00	3.26	7.91	-0.00
Dead+Wind 0 deg+Ice+Temp	59.00	0.03	-7.85	-977.01	4.34	0.02
Dead+Wind 30 deg+Ice+Temp	59.00	3.92	-6.80	-845.73	-481.25	0.05
Dead+Wind 60 deg+Ice+Temp	59.00	6.76	-3.94	-488.21	-835.81	0.10
Dead+Wind 90 deg+Ice+Temp	59.00	7.79	-0.02	0.21	-965.44	0.15
Dead+Wind 120 deg+Ice+Temp	59.00	6.75	3.90	490.32	-835.31	0.15

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Load Combination	Vertical K	Shear _x K	Shear _y K	Overturning Moment, M _x kip-ft	Overturning Moment, M _y kip-ft	Torque kip-ft
Dead+Wind 150 deg+Ice+Temp	59.00	3.89	6.79	850.96	-477.97	0.09
Dead+Wind 180 deg+Ice+Temp	59.00	-0.04	7.85	984.21	13.84	-0.03
Dead+Wind 210 deg+Ice+Temp	59.00	-3.93	6.81	853.96	499.04	-0.08
Dead+Wind 240 deg+Ice+Temp	59.00	-6.77	3.94	495.97	853.80	-0.14
Dead+Wind 270 deg+Ice+Temp	59.00	-7.81	0.03	7.12	983.71	-0.21
Dead+Wind 300 deg+Ice+Temp	59.00	-6.76	-3.89	-482.13	852.79	-0.22
Dead+Wind 330 deg+Ice+Temp	59.00	-3.88	-6.80	-845.31	492.47	-0.05
Dead+Wind 0 deg - Service	41.21	0.04	-10.93	-1302.72	-4.11	0.13
Dead+Wind 30 deg - Service	41.21	5.46	-9.46	-1128.18	-650.18	0.16
Dead+Wind 60 deg - Service	41.21	9.42	-5.48	-653.30	-1121.75	0.18
Dead+Wind 90 deg - Service	41.21	10.87	-0.04	-4.60	-1294.19	0.20
Dead+Wind 120 deg - Service	41.21	9.41	5.43	646.68	-1121.15	0.16
Dead+Wind 150 deg - Service	41.21	5.43	9.45	1126.33	-645.36	0.03
Dead+Wind 180 deg - Service	41.21	-0.07	10.93	1303.77	10.40	-0.16
Dead+Wind 210 deg - Service	41.21	-5.48	9.48	1130.84	655.88	-0.20
Dead+Wind 240 deg - Service	41.21	-9.44	5.49	655.23	1127.75	-0.25
Dead+Wind 270 deg - Service	41.21	-10.89	0.04	5.85	1300.64	-0.29
Dead+Wind 300 deg - Service	41.21	-9.43	-5.41	-644.09	1126.35	-0.27
Dead+Wind 330 deg - Service	41.21	-5.41	-9.46	-1127.70	645.90	0.03

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-41.21	0.00	0.00	41.21	0.00	0.000%
2	0.12	-41.21	-31.51	-0.12	41.21	31.51	0.000%
3	15.75	-41.21	-27.29	-15.75	41.21	27.29	0.000%
4	27.16	-41.21	-15.80	-27.16	41.21	15.80	0.000%
5	31.34	-41.21	-0.10	-31.34	41.21	0.10	0.000%
6	27.15	-41.21	15.65	-27.15	41.21	-15.65	0.000%
7	15.65	-41.21	27.25	-15.65	41.21	-27.25	0.000%
8	-0.19	-41.21	31.53	0.19	41.21	-31.53	0.000%
9	-15.81	-41.21	27.34	15.81	41.21	-27.34	0.000%
10	-27.23	-41.21	15.84	27.23	41.21	-15.84	0.000%
11	-31.41	-41.21	0.13	31.41	41.21	-0.13	0.000%
12	-27.20	-41.21	-15.60	27.20	41.21	15.60	0.000%
13	-15.59	-41.21	-27.28	15.59	41.21	27.28	0.000%
14	0.00	-59.00	0.00	0.00	59.00	-0.00	0.000%
15	0.03	-59.00	-7.85	-0.03	59.00	7.85	0.000%
16	3.92	-59.00	-6.80	-3.92	59.00	6.80	0.000%
17	6.76	-59.00	-3.94	-6.76	59.00	3.94	0.000%
18	7.79	-59.00	-0.02	-7.79	59.00	0.02	0.000%
19	6.75	-59.00	3.90	-6.75	59.00	-3.90	0.000%
20	3.89	-59.00	6.79	-3.89	59.00	-6.79	0.000%
21	-0.04	-59.00	7.85	0.04	59.00	-7.85	0.000%
22	-3.93	-59.00	6.81	3.93	59.00	-6.81	0.000%
23	-6.77	-59.00	3.94	6.77	59.00	-3.94	0.000%
24	-7.81	-59.00	0.03	7.81	59.00	-0.03	0.000%
25	-6.76	-59.00	-3.89	6.76	59.00	3.89	0.000%
26	-3.88	-59.00	-6.80	3.88	59.00	6.80	0.000%
27	0.04	-41.21	-10.93	-0.04	41.21	10.93	0.000%
28	5.46	-41.21	-9.46	-5.46	41.21	9.46	0.000%
29	9.42	-41.21	-5.48	-9.42	41.21	5.48	0.000%
30	10.87	-41.21	-0.04	-10.87	41.21	0.04	0.000%
31	9.41	-41.21	5.43	-9.41	41.21	-5.43	0.000%
32	5.43	-41.21	9.45	-5.43	41.21	-9.45	0.000%
33	-0.07	-41.21	10.93	0.07	41.21	-10.93	0.000%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
34	-5.48	-41.21	9.48	5.48	41.21	-9.48	0.000%
35	-9.44	-41.21	5.49	9.44	41.21	-5.49	0.000%
36	-10.89	-41.21	0.04	10.89	41.21	-0.04	0.000%
37	-9.43	-41.21	-5.41	9.43	41.21	5.41	0.000%
38	-5.41	-41.21	-9.46	5.41	41.21	9.46	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.0000001	0.0000001
2	Yes	5	0.0000001	0.00004457
3	Yes	6	0.0000001	0.00008801
4	Yes	6	0.0000001	0.00008647
5	Yes	4	0.0000001	0.00035367
6	Yes	6	0.0000001	0.00008759
7	Yes	6	0.0000001	0.00008690
8	Yes	4	0.0000001	0.00059813
9	Yes	6	0.0000001	0.00008662
10	Yes	6	0.0000001	0.00008861
11	Yes	5	0.0000001	0.00006950
12	Yes	6	0.0000001	0.00008584
13	Yes	6	0.0000001	0.00008680
14	Yes	4	0.0000001	0.00003004
15	Yes	5	0.0000001	0.00028196
16	Yes	5	0.0000001	0.00055540
17	Yes	5	0.0000001	0.00055118
18	Yes	5	0.0000001	0.00027910
19	Yes	5	0.0000001	0.00055617
20	Yes	5	0.0000001	0.00055100
21	Yes	5	0.0000001	0.00028336
22	Yes	5	0.0000001	0.00057094
23	Yes	5	0.0000001	0.00057432
24	Yes	5	0.0000001	0.00028435
25	Yes	5	0.0000001	0.00055545
26	Yes	5	0.0000001	0.00056423
27	Yes	4	0.0000001	0.00015997
28	Yes	5	0.0000001	0.00022107
29	Yes	5	0.0000001	0.00021449
30	Yes	4	0.0000001	0.00013738
31	Yes	5	0.0000001	0.00021751
32	Yes	5	0.0000001	0.00021502
33	Yes	4	0.0000001	0.00009679
34	Yes	5	0.0000001	0.00021669
35	Yes	5	0.0000001	0.00022399
36	Yes	4	0.0000001	0.00026700
37	Yes	5	0.0000001	0.00021146
38	Yes	5	0.0000001	0.00021605

Maximum Tower Deflections - Service Wind

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	171 - 156.5	48.552	34	2.529	0.001
L2	156.5 - 156	40.901	34	2.495	0.001
L3	156 - 132.669	40.639	34	2.495	0.001
L4	136.336 - 87.0859	30.725	34	2.257	0.001
L5	91.9193 - 43	13.172	34	1.440	0.001
L6	49 - 0	3.549	34	0.677	0.000

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
166'	RV90-17-02DP	34	45.902	2.515	0.002	32524
155'	LNx-6514DS-T4M w/ Mount Pipe	34	40.118	2.493	0.002	10510
146'	P65-16-XLH-RR w/ Mount Pipe	34	35.488	2.414	0.002	5272
138'	APX16DWV-16DWV-S-E-A20	34	31.525	2.286	0.002	3614
128'	w/Mount Pipe VHLP800-11	34	26.869	2.108	0.002	3289
118'	T-Arm Mount [TA 602-1]	34	22.578	1.925	0.001	3211

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	171 - 156.5	138.947	9	7.241	0.004
L2	156.5 - 156	117.124	9	7.148	0.004
L3	156 - 132.669	116.379	9	7.147	0.004
L4	136.336 - 87.0859	88.070	9	6.472	0.004
L5	91.9193 - 43	37.815	9	4.134	0.001
L6	49 - 0	10.199	9	1.945	0.000

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
166'	RV90-17-02DP	9	131.391	7.201	0.007	12135
155'	LNx-6514DS-T4M w/ Mount Pipe	9	114.891	7.141	0.007	3865
146'	P65-16-XLH-RR w/ Mount Pipe	9	101.676	6.918	0.006	1906
138'	APX16DWV-16DWV-S-E-A20	9	90.355	6.556	0.006	1299
128'	w/Mount Pipe VHLP800-11	9	77.045	6.047	0.005	1176
118'	T-Arm Mount [TA 602-1]	9	64.768	5.526	0.004	1144

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Compression Checks

Pole Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P/P _a
L1	171 - 156.5 (1)	TP10.75x10.75x0.365	14'6"	0'	0.0	21.00	11.9083	-0.70	250.07	0.003
L2	156.5 - 156 (2)	TP19.5x10.75x0.365	6"	0'	0.0	39.00	12.0311	-0.70	469.21	0.001
L3	156 - 132.669 (3)	TP24.79x19.5x0.1875	23'3"-31/3"	0'	0.0	39.00	14.1468	-6.58	551.73	0.012
L4	132.669 - 87.0859 (4)	TP34.63x23.5836x0.375	49'3"	0'	0.0	39.00	39.4817	-14.55	1539.79	0.009
L5	87.0859 - 43 (5)	TP43.75x32.7959x0.4375	48'11"-1/3"	0'	0.0	39.00	58.2792	-25.02	2272.89	0.011
L6	43 - 0 (6)	TP52.5x41.5315x0.5	49'	0'	0.0	39.00	78.5041	-37.50	3061.66	0.012

Pole Bending Design Data

Section No.	Elevation ft	Size	Actual M _x kip-ft	Actual f _{bx} ksi	Allow. F _{bx} ksi	Ratio f _{bx} /F _{bx}	Actual M _y kip-ft	Actual f _{by} ksi	Allow. F _{by} ksi	Ratio f _{by} /F _{by}
L1	171 - 156.5 (1)	TP10.75x10.75x0.365	10.53	4.23	23.10	0.183	0.00	0.00	23.10	0.000
L2	156.5 - 156 (2)	TP19.5x10.75x0.365	10.53	4.21	39.00	0.108	0.00	0.00	39.00	0.000
L3	156 - 132.669 (3)	TP24.79x19.5x0.1875	228.66	33.07	39.00	0.848	0.00	0.00	39.00	0.000
L4	132.669 - 87.0859 (4)	TP34.63x23.5836x0.375	1181.17	44.01	39.00	1.128	0.00	0.00	39.00	0.000
L5	87.0859 - 43 (5)	TP43.75x32.7959x0.4375	2298.68	45.82	39.00	1.175	0.00	0.00	39.00	0.000
L6	43 - 0 (6)	TP52.5x41.5315x0.5	3405.82	42.75	39.00	1.096	0.00	0.00	39.00	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V K	Actual f _v ksi	Allow. F _v ksi	Ratio f _v /F _v	Actual T kip-ft	Actual f _{vt} ksi	Allow. F _{vt} ksi	Ratio f _{vt} /F _{vt}
L1	171 - 156.5 (1)	TP10.75x10.75x0.365	1.16	0.10	14.00	0.014	0.00	0.00	14.00	0.000
L2	156.5 - 156 (2)	TP19.5x10.75x0.365	1.19	0.10	26.00	0.004	0.00	0.00	26.00	0.000
L3	156 - 132.669 (3)	TP24.79x19.5x0.1875	17.85	1.26	26.00	0.097	0.19	0.01	26.00	0.001
L4	132.669 - 87.0859 (4)	TP34.63x23.5836x0.375	24.08	0.61	26.00	0.047	0.78	0.01	26.00	0.001
L5	87.0859 - 43 (5)	TP43.75x32.7959x0.4375	27.88	0.48	26.00	0.037	0.69	0.01	26.00	0.000
L6	43 - 0 (6)	TP52.5x41.5315x0.5	30.94	0.39	26.00	0.030	0.61	0.00	26.00	0.000

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Pole Interaction Design Data

Section No.	Elevation ft	Ratio P P_o	Ratio f_{bx} F_{bx}	Ratio f_{by} F_{by}	Ratio f_v F_v	Ratio f_{vt} F_{vt}	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	171 - 156.5 (1)	0.003	0.183	0.000	0.014	0.000	0.186	1.333	H1-3+VT ✓
L2	156.5 - 156 (2)	0.001	0.108	0.000	0.004	0.000	0.109	1.333	H1-3+VT ✓
L3	156 - 132.669 (3)	0.012	0.848	0.000	0.097	0.001	0.862	1.333	H1-3+VT ✓
L4	132.669 - 87.0859 (4)	0.009	1.128	0.000	0.047	0.001	1.138	1.333	H1-3+VT ✓
L5	87.0859 - 43 (5)	0.011	1.175	0.000	0.037	0.000	1.186	1.333	H1-3+VT ✓
L6	43 - 0 (6)	0.012	1.096	0.000	0.030	0.000	1.109	1.333	H1-3+VT ✓

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail	
L1	171 - 156.5	Pole	TP10.75x10.75x0.365	1	-0.70	333.35	13.9	Pass	
L2	156.5 - 156	Pole	TP19.5x10.75x0.365	2	-0.70	625.46	8.2	Pass	
L3	156 - 132.669	Pole	TP24.79x19.5x0.1875	3	-6.58	735.45	64.7	Pass	
L4	132.669 - 87.0859	Pole	TP34.63x23.5836x0.375	4	-14.55	2052.54	85.4	Pass	
L5	87.0859 - 43	Pole	TP43.75x32.7959x0.4375	5	-25.02	3029.76	89.0	Pass	
L6	43 - 0	Pole	TP52.5x41.5315x0.5	6	-37.50	4081.19	83.2	Pass	
							Summary		
							Pole (L5)	89.0	Pass
							RATING =	89.0	Pass

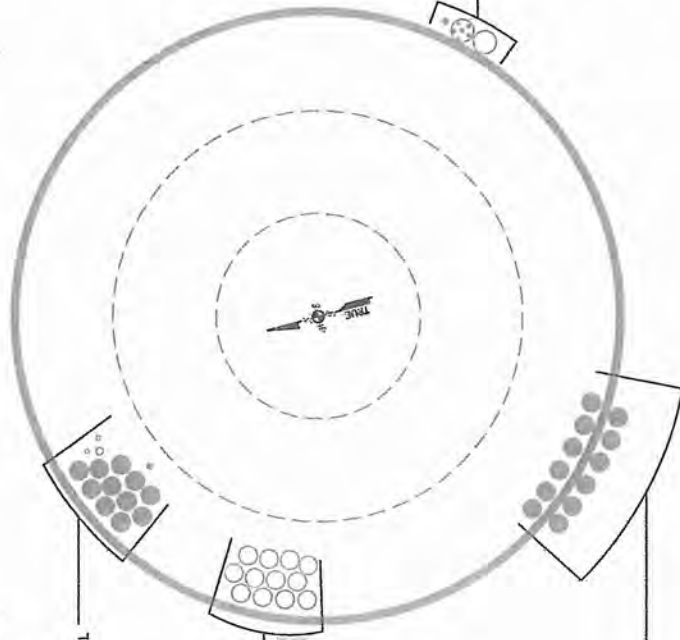
APPENDIX B
BASE LEVEL DRAWING

(PROPOSED—IF ADDITION TO INSTALLED)
 (2) 3/8" TO 146 FT LEVEL
 (1) 5/8" TO 148 FT LEVEL
 (INSTALLED)
 (12) 1-5/8" TO 146 FT LEVEL
 (INSTALLED—TO BE REMOVED)
 (1) 1/2" TO GPS AT 146 FT LEVEL

(RESERVED)
 (12) 1-5/8" TO 155 FT LEVEL

(INSTALLED)
 (12) 1-5/8" TO 138 FT LEVEL

(INSTALLED)
 (1) 1/2" TO 128 FT LEVEL
 (INSTALLED—BUNDLED IN (2) 2" CONDUIT)
 (3) 1/4" TO 128 FT LEVEL
 (3) 5/16" TO 128 FT LEVEL



APPENDIX C
ADDITIONAL CALCULATIONS

Stiffened or Unstiffened, Exterior Flange Plate - Any Bolt Material TIA Rev F

Site Data

BU#: 806355	
Site Name: BRG 126 943086	
App #: 121276	
Connection Type:	Butt
Pole Manufacturer:	Other

Reactions

Moment:	10.53	ft-kips
Axial:	0.7	kips
Shear:	1.19	kips
Elevation:	156	feet

Bolt Data

Qty:	24			
Diameter (in.):	1	Bolt Fu:	120	
Bolt Material:	A325	Bolt Fy:	92	
N/A:	75	<-- Disregard	Bolt Fty:	44.00
N/A:	55	<-- Disregard		
Circle (in.):	25.75			

If No stiffeners, Criteria: **AISC ASD** <-Only Applicable to Unstiffened Cases

Flange Bolt Results

Bolt Tension Capacity, B:	46.07 kips
Max Bolt directly applied T:	0.79 Kips
Min. PL "tc" for B cap. w/o Pry:	2.177 in
Min PL "treq" for actual T w/ Pry:	0.226 in
Min PL "t1" for actual T w/o Pry:	0.285 in
T allowable with Prying:	31.95 kips
Prying Force, Q:	0.00 kips
Total Bolt Tension=T+Q:	0.79 kips
Prying Bolt Stress Ratio=(T+Q)/(B):	1.7%

Rigid
Service, ASD
Fty*ASIF

Plate Data

Diam:	28.5	in
Thick, t:	1.5	in
Grade (Fy):	60	ksi
Strength, Fu:	75	ksi
Single-Rod B-eff:	2.58	in

Exterior Flange Plate Results

Flexural Check	
Compression Side Plate Stress:	1.4 ksi
Allowable Plate Stress:	60.0 ksi
Compression Plate Stress Ratio:	2.4%
No Prying	
Tension Side Stress Ratio, (treq/t)^2:	2.3%

Rigid
Service ASD
0.75*Fy*ASIF
Comp. Y.L. Length:
16.82

Stiffener Data (Welding at Both Sides)

Config:	0	*
Weld Type:		
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		<-- Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

n/a

Stiffener Results

Horizontal Weld :	n/a
Vertical Weld:	n/a
Plate Flex+Shear, fb/Fb+(fv/Fv)^2:	n/a
Plate Tension+Shear, ft/Ft+(fv/Fv)^2:	n/a
Plate Comp. (AISC Bracket):	n/a

Pole Results

Pole Punching Shear Check:	n/a
----------------------------	-----

Pole Data

Diam:	19.5	in
Thick:	0.365	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None

Stress Increase Factor

ASIF:	1.333
-------	-------



* 0 = none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

** Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

Stiffened or Unstiffened, Ungrouted, Circular Base Plate - Any Rod Material

TIA Rev F

Site Data

BU#: 806355
Site Name: BRG 126 943086
App #: 121276 Rev. 1
Pole Manufacturer: Other

Reactions		
Moment:	3759	ft-kips
Axial:	41	kips
Shear:	32	kips

Anchor Rod Data

Qty:	20	
Diam:	2.25	in
Rod Material:	A615-J	
Strength (Fu):	100	ksi
Yield (Fy):	75	ksi
Bolt Circle:	61	in

If No stiffeners, Criteria: AISC ASD <-Only Applicable to Unstiffened Cases

Anchor Rod Results

Maximum Rod Tension:	145.8 Kips
Allowable Tension:	195.0 Kips
Anchor Rod Stress Ratio:	74.8% $F_{t,ASD}$

Rigid
Service, ASD
Fty*ASIF

Plate Data

Diam:	67	in
Thick:	2.25	in
Grade:	60	ksi
Single-Rod B-eff:	8.33	in

Base Plate Results

Base Plate Stress:	54.3 ksi	Flexural Check
Allowable Plate Stress:	60.0 ksi	
Base Plate Stress Ratio:	90.6% $F_{t,ASD}$	

Rigid
Service ASD
0.75*Fy*ASIF
Y.L. Length:
31.06

Stiffener Data (Welding at both sides)

Config:	0	*
Weld Type:		
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		<-- Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

n/a

Stiffener Results

Horizontal Weld :	n/a
Vertical Weld:	n/a
Plate Flex+Shear, $f_b/F_b+(f_v/F_v)^2$:	n/a
Plate Tension+Shear, $f_t/F_t+(f_v/F_v)^2$:	n/a
Plate Comp. (AISC Bracket):	n/a

Pole Results

Pole Punching Shear Check:	n/a
----------------------------	-----

Pole Data

Diam:	52.5	in
Thick:	0.5	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None

Stress Increase Factor

ASIF:	1.333
-------	-------



* 0 = none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

** Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

Monopole Pier and Pad Foundation



BU #: 806355
 Site Name: BRG 126 943086
 App. Number: 121276

Design Reactions	
Shear, S:	32 kips
Moment, M:	3759 ft-kips
Tower Height, H:	171 ft
Tower Weight, Wt:	41 kips
Base Diameter, BD:	4.375 ft

Foundation Dimensions	
Depth, D:	9 ft
Pad Width, W:	22 ft
Neglected Depth, N:	4 ft
Thickness, T:	3.00 ft
Pier Diameter, Pd:	7.00 ft
Ext. Above Grade, E:	1.00 ft
Clear Cover, Cc:	3.0 in

Soil Properties	
Soil Unit Weight, γ :	0.125 kcf
Bearing Capacity, Bc:	12.0 ksf
Angle of Friction, ϕ :	35 deg
Cohesion, Cc:	0.000 ksf
Passive Pressure, Pp:	0.000 kcf
Base Friction, μ :	0.60

Material Properties	
Rebar Yield Strength, Fy:	60000 psi
Concrete Strength, F'c:	4000 psi
Concrete Unit Weight, γ_c :	0.150 kcf
Seismic Zone, z:	1

Rebar Properties	
Pier Rebar Size, Sp:	8
Pier Rebar Quantity, mp:	46
Pad Rebar Size, Spad:	8
Pad Rebar Quantity, mpad:	96
Pier Tie Size, St:	4
Tie Quantity, mt:	7

Design Checks			
	Capacity/Availability	Demand/Limits	Check
Reqd'd Pier Diam. (ft)	7	5.875	OK
Overturning (ft-kips)	5997.72	4079.00	OK
Shear Capacity (kips)	238.86	32.00	OK
Bearing (ksf)	12.00	4.11	OK
Pad Shear - 1-way (kips)	1085.29	794.46	OK
Pad Shear - 2-way (kips)	3009.19	1242.18	OK
Pier Rebar Area (in ²)	36.34	27.71	OK
Pad Rebar Area (in ²)	28.44	19.82	OK
Pier Moment Capacity (k-ft)	4577.56	3983.00	OK
Pier Bar Spacing (in)	4.33	18 > s > 2	OK
Pad Bar Spacing (in)	6.34	18 > s > 2	OK
Pier Development Length (in)	81	28.21	OK
Pad Development Length (in)	33	28.21	OK
Hook Development Length (in)	129.00	13.28	OK
Rebar Hook Length (in)	90.00	16.00	OK

Modification Checks			
	Capacity/Availability	Demand/Limits	Check
Sleeve Rebar Area (in ²):	15.6	0.00	Not Used
Sleeve Moment Capacity (k-ft):	4577.56	3983.00	Not Used
Sleeve Rebar Spacing (in):	N/A	18 > s > 2	Not Used
Sleeve Tie Spacing (in):	N/A	8 > s > 4.5	Not Used
Minimum Extra Thickness (in):	0	0	Not Used
Pad Rebar Area-short (in ²):	0.44	0.00	Not Used
Pad Rebar Area-long (in ²):	0.44	0.00	Not Used
Pad Rebar Spacing-short (in):	85.5	18 > s > 2	Not Used
Pad Rebar Spacing-long (in):	85.5	18 > s > 2	Not Used
End Cap Width (ft):	0	0	Not Used
End Cap Rebar Area (in ²):	3.16	0	Not Used
Rebar Spacing (in):	-3.00	18 > s > 2	Not Used
Tie Spacing (in):	17.93	258 > s > 4.5	Not Used
Dowel Area (in ²):	2.2	0.00	Not Used
Dowel Embedment (in):	9	6	Not Used
Core Shear Strength (kips):	25.15	23.76	Not Used
Dowel Edge Dist (in):	12.00	4.78	Not Used
Dowel Spacing (in):	60.00	18.00	Not Used
Dowel Edge Dist (vert) (in):	18.00	4.78	Not Used
Dowel Devel. Length (in):	-3.00	13.32	Not Used

Modifications	
	ft
Pier Sleeve, ds:	0
Revised Pier Diameter, dk:	22
PS Rebar Size, Ss:	8
Rebar Quantity, ms:	4
Tie Size, Sst:	4
Tie Quantity, mst:	15
Pad Thickness, Tc:	6
Revised Pier Thickness, Tc:	5
Rebar Size, Sc:	1
Rebar Quantity (long), me:	9
Rebar Quantity (short), mscd:	12
Dowel Size, Ssd:	
Dowel Quantity, mcd:	

P55-15-XLH-RR

Dual Broadband Antennas

POLARIZATION Dual Feed 90°
 FREQUENCY RANGE 500-500, 500-500
 HORIZONTAL BEAM WIDTH 60 dB
 PART NUMBER P55-15-XLH-RR
 YLT 0-15 2-0
 LENGTH 5"

ELECTRICAL SPECIFICATIONS*

	500-500		1710-1000	1710-2170	1000-2170
Frequency range (MHz)	500-500	500-500	1710-1000	1710-2170	1000-2170
Frequency band (MHz)	100-100	100-100	100-100	100-100	100-100
Gain (dBi/dBs)	10/11.5	10/11.5	10/11.5	10/11.5	10/11.5
Polarization	Dual Linear +/- 45			Dual Linear +/- 45	
Nominal impedance (Ohm)	50			50	
VSWR	< 1.5:1			< 1.5:1	
Horizontal beam width -3 dB (°)	75	65	65	61	60
Vertical beam width -3 dB (°)	17			7.5	
Elemental beam (°)	9-10			9-9	
Gain lobe suppression, vertical 1st upper (dB)	> 14			> 20	
Isolation between inputs (dB)	> 20			> 20	
Intra band isolation (dB)	> 40			> 40	
Tracking, horizontal plane 45° (dB)	< 2			< 2	
Vertical beam skew (°)	< 1.25			< 0.5	
Front to back ratio (dB) 100° x 30° upper	> 25			> 28	
Front to back ratio (dB) 100° x 30° total power	> 25			> 25	
Cross polar discrimination (XPD) 0° (dB)	> 15			> 15	
Cross polar discrimination (XPD) 45° (dB)	> 10			> 10	
IM3, 2x @ 10dBm (dBc)	-133			-133	
Power handling, average per input (W)	500			300	
Power handling, average total (W)	1000			600	

MECHANICAL SPECIFICATIONS*

Connector	4 X 7140 DIN Female, IP67
Connector position	Bottom
Dimensions, HxWxD, in (mm)	51" x 12" x 6" (1285 x 305 x 152)
Mounting	Pre-mounted Tilt Brackets
Weight, with brackets, lbs (kg)	41 (18)
Weight, without brackets, lbs (kg)	20 (9)
Wind load resistance (average sea level @ 100 ft)	630
Maximum operational wind speed, mph (m/s)	100 (45)
Survival wind speed, mph (m/s)	150 (67)
Lightning protection	DC Ground
Operating Temperature	-40°C to +80°C
Radome material	PVC, IP68
Radome size, HxWxD, in (mm)	60" x 16" x 16" (1524 x 400 x 255)
Radome colour	Light Gray
Shipping weight, lbs (kg)	52 (24)
REIT	REIT 400/1, NET and MISG-2.0
Brackets	T205 00, T400 00



*All specifications subject to change without notice. Please contact your Representative for complete performance data.

ANTENNA PATTERNS*

For detailed patterns visit <http://www.farnell.com>

POWER

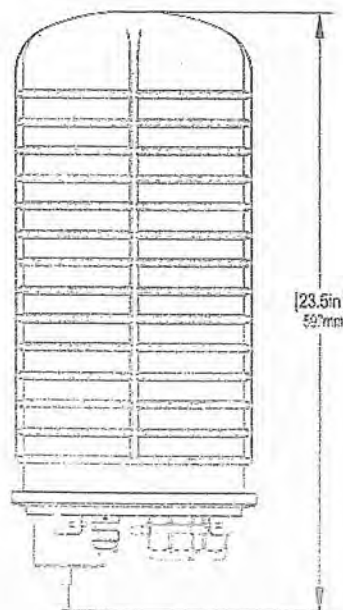
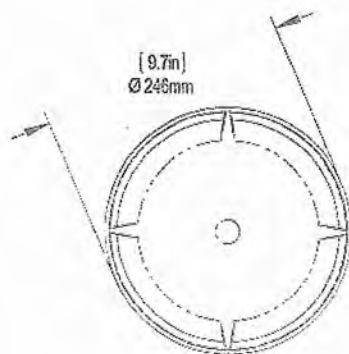
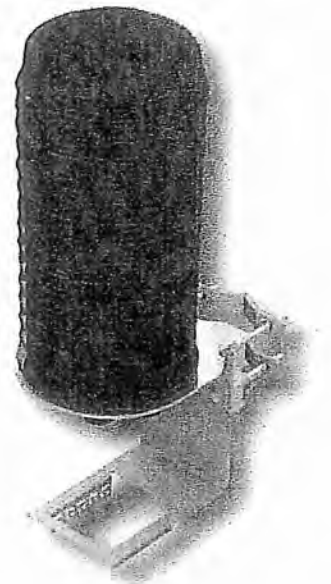
DC6-48-60-18-8F

DC Surge Suppression Solution

The DC6-48-60-18 is a dual chambered, DC surge suppression system for use in multi-circuit, Distributed Antenna Systems. The system will protect up to 6 Remote Radio Heads from voltage surges and lightning, and connect up to 18 fiber pairs. The system is enclosed in a NEMA 4 rated, waterproof enclosure.

FEATURES

- Protects up to 6 Remote Radio Heads, each with its own protection circuit.
- Flexible design allows for installation at the top of a tower for Remote Radio Head protection.
- Includes fiber connections for up to 18 pairs of fiber.
- LED indicators on individual circuits provide visual indication of suppressor status.
- Form 'C' relays allow for remote monitoring of the suppressor status.
- Patented Strikesorb technology provides over 60 kA of surge current capacity per circuit.
- Strikesorb suppression modules are fully recognized to UL 1449-3rd Edition Safety Standard, meeting all intermediate and high current fault requirements to facilitate use in OEM applications.
- Raycap recommends that DC protection system be installed within 2 meters or 6 feet of the radio.
- Dome design is lightweight and aerodynamic providing maximum flexibility for installation on top of towers.



Raycap

DC6-48-60-18-8F

DC Power Surge Protection

Electrical Specifications	
Model Number	DC6-48-60-18-8F
Nominal Operating Voltage	48 VDC
Nominal Discharge Current (I_n)	20 kA 8/20 μ s
Maximum Discharge Current (I_{max}) per NEMA LS-1	60 kA 8/20 μ s
Maximum Continuous Operating Voltage (U_c)	75 VDC
Voltage Protection Rating	400 V

Mechanical Specifications	
Suppression Connection Method	Compression Lug, #2-#14 AWG Copper, #2-#12 Aluminum
Fiber Connection Method	LC-LC Single mode duplex
Environmental Rating	IP 68, 7m 72hrs
Operating Temperature	-40° C to + 80° C
Storage Temperature	-70° C to + 80° C
Cold Temperature Cycling	IEC 61300-2-22e -30° C to + 60° C 200 hrs @ 5 psi
Resistance to Aggressive Materials	CEI IEC 61073-2 including acids and bases
UV Protection	ISO 4892-2 Method A Xenon-Arc 2160 hrs
Weight	20 lbs without Mounting Bracket

STANDARDS

Strikesorb modules are compliant to the following Surge Protection Device (SPD) Standards:

- ANSI/UL 1449 - 3rd Edition
- IEEE C62.41
- NEMA LS-1, IEC 61643-1:2005 2nd Edition:2005
- IEC 61643-12
- EN 61643-11:2002 (including A11:2007)



Raycap

G02-00-068 REV 050610



GS-07F-0435V



Certified to
ISO 9001:2000



TUV Rheinland
of North America

RRUS 11 – Dual PA RRU.

Technical Data



RBS6000

- > Multi standard
- > RF: 2x30 Watts
- > Carrier BW: 1.4 – 20 MHz
- > Alarms: 2
- > Dimensions (with sunshield):
 - Width: 17.0 in
 - Height: 17.8 in
 - Depth: 7.2 in
 - Weight: 55 lbs (Band 12)
 - Weight: 50 lbs (Band 4)
- > Temperature: -40 to +131 F
- > Cooling: Self convection
- > Power: -48 VDC
- > Rec. fuse size 20 Amp
 - Rec. DC cable:
 - 6 mm² up to 60 meters
 - 10 mm² over 60 meters
- > Power Cons: 200 Watts typ. Shielded

