



# STATE OF CONNECTICUT

## CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: [siting.council@ct.gov](mailto:siting.council@ct.gov)

[www.ct.gov/csc](http://www.ct.gov/csc)

June 20, 2011

Douglas L. Culp, Real Estate Consultant  
New Cingular Wireless PCS, LLC  
500 Enterprise Drive  
Rocky Hill, CT 06067-3900

RE: **EM-CING-051-110527** - New Cingular Wireless PCS, LLC notice of intent to modify an existing telecommunications facility located at 281 Wood House Road, Fairfield, Connecticut

Dear Mr. Culp:

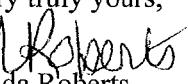
The Connecticut Siting Council (Council) hereby acknowledges your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies with the following conditions:

- Any deviation from the proposed modification as specified in this notice and supporting materials with Council shall render this acknowledgement invalid;
- Any material changes to this modification as proposed shall require the filing of a new notice with the Council;
- Not less than 45 days after completion of construction, the Council shall be notified in writing that construction has been completed;
- The validity of this action shall expire one year from the date of this letter; and
- The applicant may file a request for an extension of time beyond the one year deadline provided that such request is submitted to the Council not less than 60 days prior to the expiration;

The proposed modifications including the placement of all necessary equipment and shelters within the tower compound are to be implemented as specified here and in your notice dated May 27, 2011. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Thank you for your attention and cooperation.

Very truly yours,



Linda Roberts  
Executive Director

LR/CDM/laf

c: The Honorable Kenneth A. Flatto, First Selectman, Town of Fairfield  
Joseph E. Devonshuk, Town Planner, Town of Fairfield  
Crown Castle USA, Inc.





New Cingular Wireless PCS, LLC  
500 Enterprise Drive  
Rocky Hill, Connecticut 06067-3900  
Phone: (860) 463-5511  
Fax: (860) 513-7190

Douglas L. Culp  
Real Estate Consultant

HAND DELIVERED

May 27, 2011

Ms. Linda Roberts  
Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, Connecticut 06051

RECEIVED  
MAY 27 2011  
CONNECTICUT  
SITING COUNCIL

Re: New Cingular Wireless PCS, LLC notice of intent to modify an existing telecommunications facility located at 281 Wood House Fairfield, CT (owner Crown Castle)

Dear Ms. Roberts:

In order to accommodate technological changes, implement Uniform Mobile Telecommunications System (“UMTS”) and/or Long Term Evolution (“LTE”) capabilities, and enhance system performance in the State of Connecticut, New Cingular Wireless PCS, LLC (“AT&T”) plans to modify the equipment configurations at many of its existing cell sites. Please accept this letter and attachments as notification, pursuant to R.C.S.A. Section 16-50j-73, of construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2). In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter and attachments is being sent to the chief elected official of the municipality in which the affected cell site is located.

UMTS technology offers services to mobile computer and phone users anywhere in the world. Based on the Global System for Mobile (“GSM”) communication standard, UMTS is the planned worldwide standard for mobile users. UMTS, fully implemented, gives computer and phone users high-speed access to the Internet as they travel. They have the same capabilities even when they roam, through both terrestrial wireless and satellite transmissions.

LTE is a new high-performance air interface for cellular mobile communications. It is designed to increase the capacity and speed of mobile telephone networks.

Attached is a summary of the planned modifications, including power density calculations reflecting the change in AT&T’s operations at the site. Also included is documentation of the structural sufficiency of the tower to accommodate the revised antenna configuration.

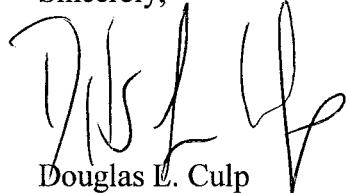
The changes to the facility do not constitute modifications as defined in Connecticut General Statutes ("C.G.S.") Section 16-50i(d) because the general physical characteristics of the facility will not be significantly changed or altered. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in R.C.S.A. Section 16-50j-72(b)(2).

1. The height of the overall structure will be unaffected.
2. The proposed changes will not extend the site boundaries. There will be no effect on the site compound other than some enlarged equipment pads as may be noted in the attachments.
3. The proposed changes will not increase the noise level at the existing facility by six decibels or more.
4. Radio frequency power density may increase due to use of one or more GSM channel for UMTS transmissions. Moreover, LTE will utilize additional radio frequencies newly-licensed by the FCC for cellular mobile communications. However, the changes will not increase the calculated "worst case" power density for the combined operations at the site to a level at or above the applicable standard for uncontrolled environments as calculated for a mixed frequency site.

For the foregoing reasons, New Cingular Wireless respectfully submits that the proposed changes at the referenced site constitute exempt modifications under R.C.S.A. Section 16-50j-72(b)(2).

Please feel free to call me at (860) 463-5511 with questions concerning this matter. Thank you for your consideration.

Sincerely,



Douglas L. Culp  
Real Estate Consultant

Attachments

**NEW CINGULAR WIRELESS PCS, LLC**  
**Equipment Modification**

281 Wood House Road Fairfield, CT  
Site Number CT2105  
Exempt Mod

**Tower Owner/Manager:** Crown Castle

**Equipment configuration:** Monopole

**Current and/or approved:** Six PowerWave antennas @ 150 ft  
Twelve PowerWave TMA's @ 150 ft  
Twelve runs 1 1/4 inch coax to 150 ft  
Equipment Shelter

**Planned Modifications:** Retain existing PowerWave Antenna's, TMA's at 150 ft  
Retain all Coax Cabling  
Install three PowerWave P65-16 antennas or equivalent @ 150 ft  
Install six remote radio heads and surge arrestor @ 150 ft  
Install one fiber and two DC power cables to 150 ft

**Power Density:**

Worst-case calculations for existing wireless operations at the site, using standard parameters for other carriers, indicate a radio frequency electromagnetic radiation power density, measured at ground level beside the Tower, of 51.3% of the standard adopted by the FCC. As depicted in the second table below, the total radio frequency electromagnetic radiation power density following proposed modifications would be approximately 52.5% of the standard.

**Existing**

Company	Centerline Ht (feet)	Frequency (MHz)	Number of Channels	Power Per Channel (Watts)	Power Density (mW/cm <sup>2</sup> )	Standard Limits (mW/cm <sup>2</sup> )	Percent of Limit
Other Users							37.69
AT&T UMTS	150	1900 Band	1	500	0.0080	1.0000	0.80
AT&T UMTS	150	800 Band	2	500	0.0160	0.5867	2.72
AT&T GSM	150	800Band	10	296	0.0473	0.5867	8.06
AT&T UMTS	150	1900 Band	3	427	0.0205	1.0000	2.05
<b>Total</b>							<b>51.3%</b>

\* Data for other users are from Siting Council records.

## Proposed

Company	Centerline Ht (feet)	Frequency (MHz)	Number of Channels	Power Per Channel (Watts)	Power Density (mW/cm <sup>2</sup> )	Standard Limits (mW/cm <sup>2</sup> )	Percent of Limit
Other Users							37.69
AT&T UMTS	150	800 Band	1	500	0.0080	0.5867	1.36
AT&T UMTS	150	1900 Band	2	500	0.0160	1.0000	1.60
AT&T GSM	150	880 - 894	10	296	0.0473	0.5867	8.06
AT&T GSM	150	*1900 Band	3	427	0.0205	1.0000	2.05
AT&T LTE	150	740 - 746	1	500	0.0080	0.4933	1.62
<b>Total</b>							<b>52.4%</b>

\* Data for other users are from Siting Council records.

### **Structural information:**

The attached structural analysis demonstrates that the monopole and foundation have adequate structural capacity to accommodate the proposed modifications. (Crown Castle. dated 4-21-11).



# WIRELESS COMMUNICATIONS FACILITY

## CT2105

## FAIRFIELD WOODHOUSE

## 281 WOOD HOUSE ROAD

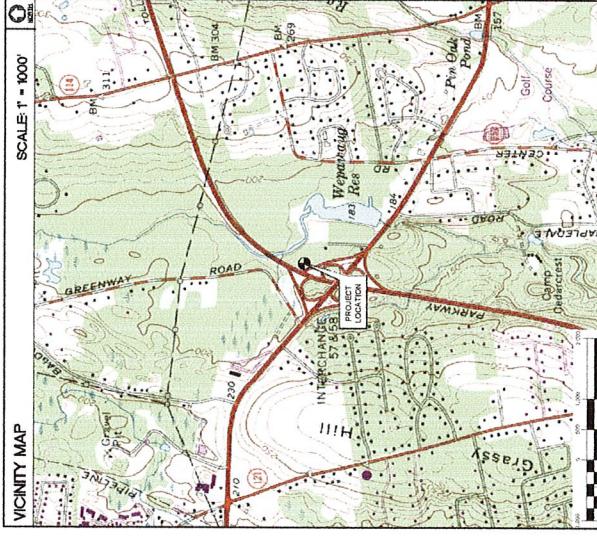
## FAIRFIELD, CT 06430

SITE DIRECTIONS	
FROM:	281 WOOD HOUSE ROAD FAIRFIELD, CT 06430
TO:	281 WOOD HOUSE ROAD FAIRFIELD, CT 06430
<p>1. Depart Enterprise Dr./Garden Capital Blvd 2. Turn left onto Gable St. 3. Turn left at 1st for I-91 South 4. Take ramp off I-91 South 5. At exit 17, take ramp right for SR-15 South / Wilbur Cross Hwy toward E. Main St 6. Turn left onto Wilbur Cross Hwy and continue straight, heading toward E. Main St 7. Turn left onto Ringwood Rd 8. Turn left onto Ringwood Rd and then immediately turn right onto SR-38 / Black Rock Turnpike 9. Turn right onto Black Rock Turnpike 10. Turn right onto Woodhouse Rd 11. Arrive at 281 Wood House Rd, Fairfield, CT 06430 on the left</p>	

### GENERAL NOTES

1. ALL WORK SHALL BE IN ACCORDANCE WITH THE 2005 INTERNATIONAL BUILDING CODE AS AMENDED BY THE 2009 CONNECTICUT EDITION, INCLUDING THE 2009 CONNECTICUT EDITION OF THE STRUCTURAL STANDARDS FOR CONNECTICUT FIRE SAFETY CODE AND 2008 APARTMENTS, NATIONAL ELECTRICAL CODE AND LOCAL CODES.
2. THE COMPOUND, TOWER, PRIMARY GROUND RING, ELECTRICAL SERVICE TO THE METER BANK, AND LANDLINE SERVICE TO THE DEMONSTRATION HOME PROVIDED BY THE OWNER SHALL BE PROVIDED BY THE CONTRACTOR. SHOULD ANY FIELD CONDITIONS PRECIPITATE A CHANGE IN THE DRAWINGS, THE CONTRACTOR SHALL NOTIFY THE OWNER IMMEDIATELY.
3. CONTRACTOR SHALL REVIEW ALL DRAWINGS AND SPECIFICATIONS IN THE CONTRACT DOCUMENT SET. CONTRACTOR SHALL COORDINATE WITH ALL RELATED PARTIES. THE SUBCONTRACTORS, SUBSUPPLIERS AND ALL RELATED PARTIES SHALL EXAMINE ALL THE DRAWINGS AND SPECIFICATIONS FOR THE INFORMATION THAT AFFECTS THEIR WORK.
4. CONTRACTOR SHALL PROVIDE A COMPLETE BUILD-OUT WITH ALL FINISHES, STRUCTURAL, MECHANICAL, ELECTRICAL, PLUMBING, ELECTRICAL AND AIR CONDITIONING SYSTEMS. CONTRACTOR SHALL NOT USE ANY LOCAL SUPPLIERS.
5. CONTRACTOR SHALL DEDUCE ALL NEEDED MATERIALS, EQUIPMENT AND TO COMPLETE THE WORK. CONTRACTOR SHALL MAINTAIN A CURRENT SET OF DRAWINGS AND SPECIFICATIONS IN ACCORDANCE WITH LOCAL AND STATE GOVERNING AUTHORITIES AND OTHER AUTHORITIES HAVING LAWFUL JURISDICTION OVER THE WORK.
6. CONTRACTOR SHALL SECURE AND PAY FOR ALL PERMITS AND ALL INSPECTIONS REQUIRED AND SHALL ALSO PAY FEES REQUIRED FOR THE GENERAL CONSTRUCTION, PLUMBING, ELECTRICAL AND AIR CONDITIONING SYSTEMS. CONTRACTOR SHALL NOT USE ANY LOCAL SUPPLIERS.
7. CONTRACTOR SHALL MAINTAIN A CURRENT SET OF DRAWINGS AND SPECIFICATIONS ON SITE AT ALL TIMES AND SHALL MAINTAIN A CURRENT SET OF DRAWINGS AND SPECIFICATIONS FOR THE PROJECT. CONTRACTOR SHALL MARK NEW DRAWINGS AS SOON AS THEY ARE MADE AVAILABLE. ALL OLD DRAWINGS SHALL BE MARKED VOID AND REMOVED FROM THE CONTRACT AREA. THE CONTRACTOR SHALL REMOVE ALL OLD DRAWINGS AS SOON AS POSSIBLE UPON COMPLETION OF THE PROJECT.
8. LOCATION DATA INDICATED IN THE DRAWINGS SHALL NOT BE DETERMINED BY THE CONTRACTOR. THE CONTRACTOR SHALL DETERMINE LOCATIONS AND DIMENSIONS SUBJECT TO STRUCTURAL CONDITIONS AND WORK OF THE SUBCONTRACTORS.
9. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND REQUIREMENTS TO ENSURE THE SAFETY OF THE EXISTING PROPERTY AND TO RELOCATE OR REMOVE THE EXISTING PROPERTY AS NECESSARY. CONTRACTOR SHALL NOT ADD TO THE COST OF THE PROJECT FOR THE ADDITION OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY. MAINTAIN EXISTING BUILDINGS/PROPERTY OWNERS OPERATIONS. COORDINATE WORK WITH BUILDING/PROPERTY OWNER.
10. DRAWINGS INDICATE THE MINIMUM STANDARDS. BUT IF ANY WORK SHOULD BE INDICATED TO BE SUBSTANTIALLY TO ANY ORDINANCES, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE WORK CORRELLY IN ACCORDANCE WITH THE APPROPRIATE LAW, THE CONTRACTOR SHALL COORDINATE WITH THE APPROPRIATE AUTHORITY AND MAINTAIN EXCELSIOR COPIES OF THE APPROPRIATE LAW, THE CODES, RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
11. ALL UTILITY WORK SHALL BE IN ACCORDANCE WITH LOCAL UTILITY COMPANY REQUIREMENTS AND SPECIFICATIONS.
12. ALL EQUIPMENT AND PRODUCTS PURCHASED ARE TO BE PROVIDED BY CONTRACTOR AND ALL APPLICABLE SUBCONTRACTORS FOR ANY CONDITION PER MRPs. RECOMMENDATIONS, CONTRACTOR TO SUPPLY THESE ITEMS AT NO COST OWNERS OR CONTRACTOR TO PURCHASE ANY AND ALL EXPENSES DISCHARGED. CONTRACTOR SHALL NOT BE ALLOWED TO DIRECTLY BILL THE OWNER. CONTRACTOR SHALL NOT BILL THE OWNER FOR THESE ITEMS DURING THE BIDDING PROCESS BY THE CONTRACTOR. ALL THESE ITEMS ARE TO BE INCLUDED IN THE BID. NO EXTRA WILL BE ALLOWED FOR MISSED ITEMS.
13. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ON-SITE SAFETY FROM THE DATE OF BIDDING UNTIL ALL WORK IS COMPLETE AND ACCEPTED BY THE OWNER.
14. CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY COPIES TO OWNER. CONTRACTOR TO SUBMIT DRAWINGS TO THE OWNER'S CHECKER'S INITIALS BEFORE SUBMITTING TO THE CONTRACTOR MANAGER FOR REVIEW.
15. THE CONTRACTOR SHALL FABRICATE, VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES, AND Existing CONDITIONS AT THE SITE, PRIOR TO FABRICATION AND/or INSTALLATION OF ANY ITEM IN THE CONTRACT.
16. THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES, AND Existing CONDITIONS AT THE SITE, PRIOR TO FABRICATION AND/or INSTALLATION OF ANY ITEM IN THE CONTRACT.
17. COORDINATION, LAYOUT, PUNCHING AND INSTALLATION OF CONDUIT, ELECTRICAL AND TELECOMMUNICATION SERVICES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
18. ALL EQUIPMENT AND PRODUCTS PURCHASED ARE TO BE PROVIDED BY CONTRACTOR AND ALL APPLICABLE SUB-CONTRACTORS. FOR ANY CONDITION PER THE MANUFACTURER'S SPECIFICATIONS, THE CONTRACTOR SHALL BE HELD RESPONSIBLE. NO COST TO OWNER FOR CONTRACTOR'S SUBCONTRACTORS.
19. RESPONSIBILITY OF THE CONTRACTOR FOR ANY DAMAGE, STRUCTURE, SHALL BE THE SOLE LIABILITY FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.
20. THE CONTRACTOR SHALL CONTACT CALL BEFORE YOU DIG™ AT LEAST 48 HOURS PRIOR TO ANY EXCAVATIONS AT 1-800-922-4455. ALL EXCAVATION WORK SHALL BE CONDUCTED BY THE CONTRACTOR'S MANAGED UTILITIES THROUGHOUT PROJECT COMPLETION.
21. CONTRACTOR SHALL COPY AND PROVIDE TO OWNERS ENVIRONMENTAL ENGINEER ON ALL METHODS AND PROVISIONS FOR ALL EXCAVATION ACTIVITIES INCLUDING SOIL DISPOSAL, ALL BACKFILL MATERIALS TO BE PROVIDED BY THE CONTRACTOR.

### VICINITY MAP



### PROJECT SUMMARY

1. THE PROPOSED SCOPE OF WORK GENERALLY CONSISTS OF THE INSTALLATION OF ONE (1) LT4 ANTENNA PER SECTOR FOR A TOTAL OF (3) LT4 ANTENNA'S. THE EXISTING ANTENNA WILL BE REMOVED AND THE NEW ANTENNA WILL BE INSTALLED IN THE EXISTING ANTENNA POLE. THE NEW ANTENNA WILL BE INSTALLED WITHIN THE EXISTING LT4 EQUIPMENT SHELF.
2. ADDITIONALLY, (2) SIGHT ROAD UNITS (SRU) PER SECTOR WILL BE INSTALLED. SRU'S ARE ANTENNA'S WHICH WILL BE INSTALLED AT BOTH ANTENNA TOWER AND EQUIPMENT LOCATIONS REFER TO THESE ACCOMPANYING DRAWINGS FOR FURTHER INFORMATION.

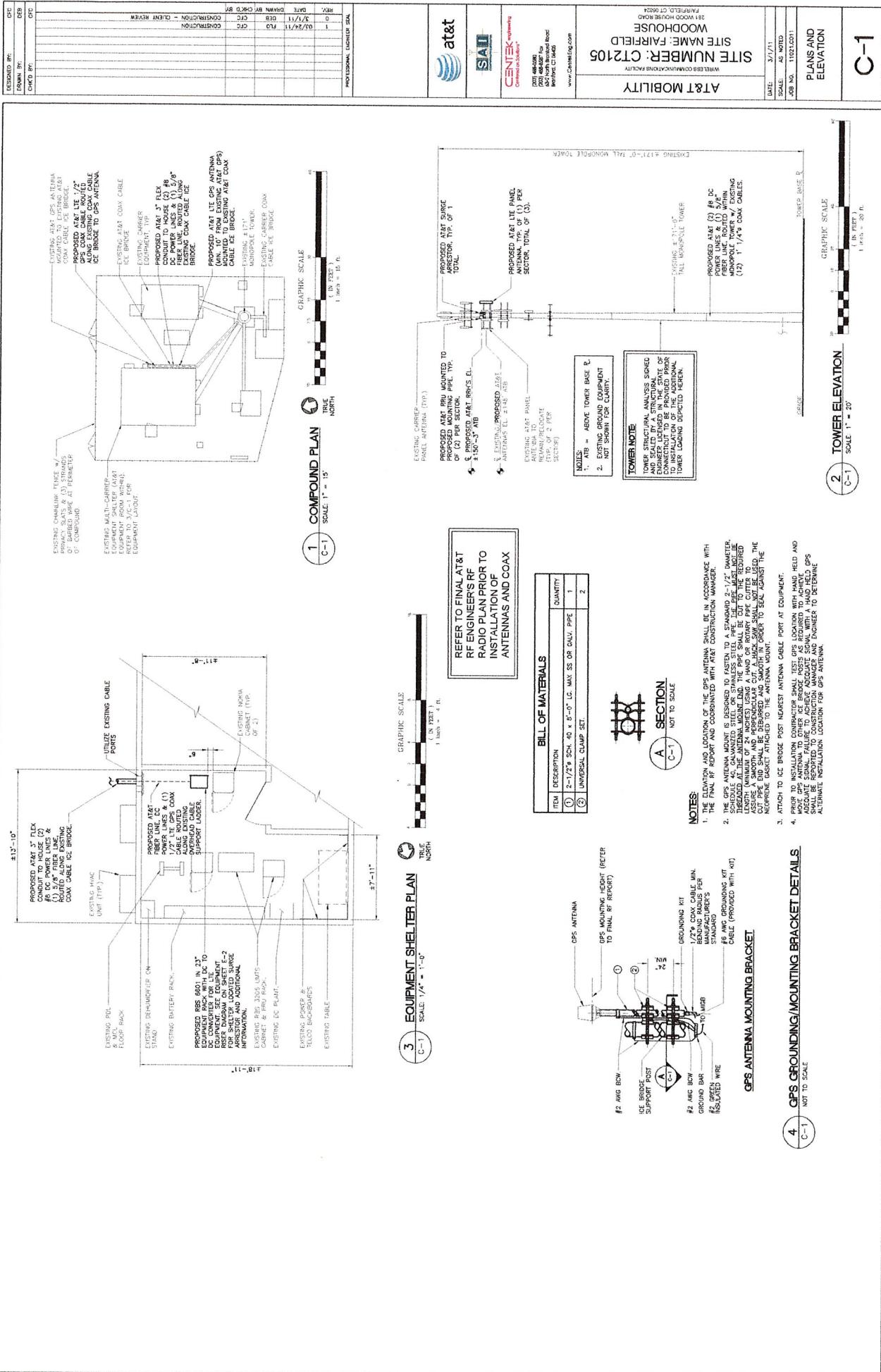
AT&T MOBILITY  
SITE NAME: WOODHOUSE  
SITE NUMBER: CT2105  
WEB: ESS COMMUNICATIONS PROJECT

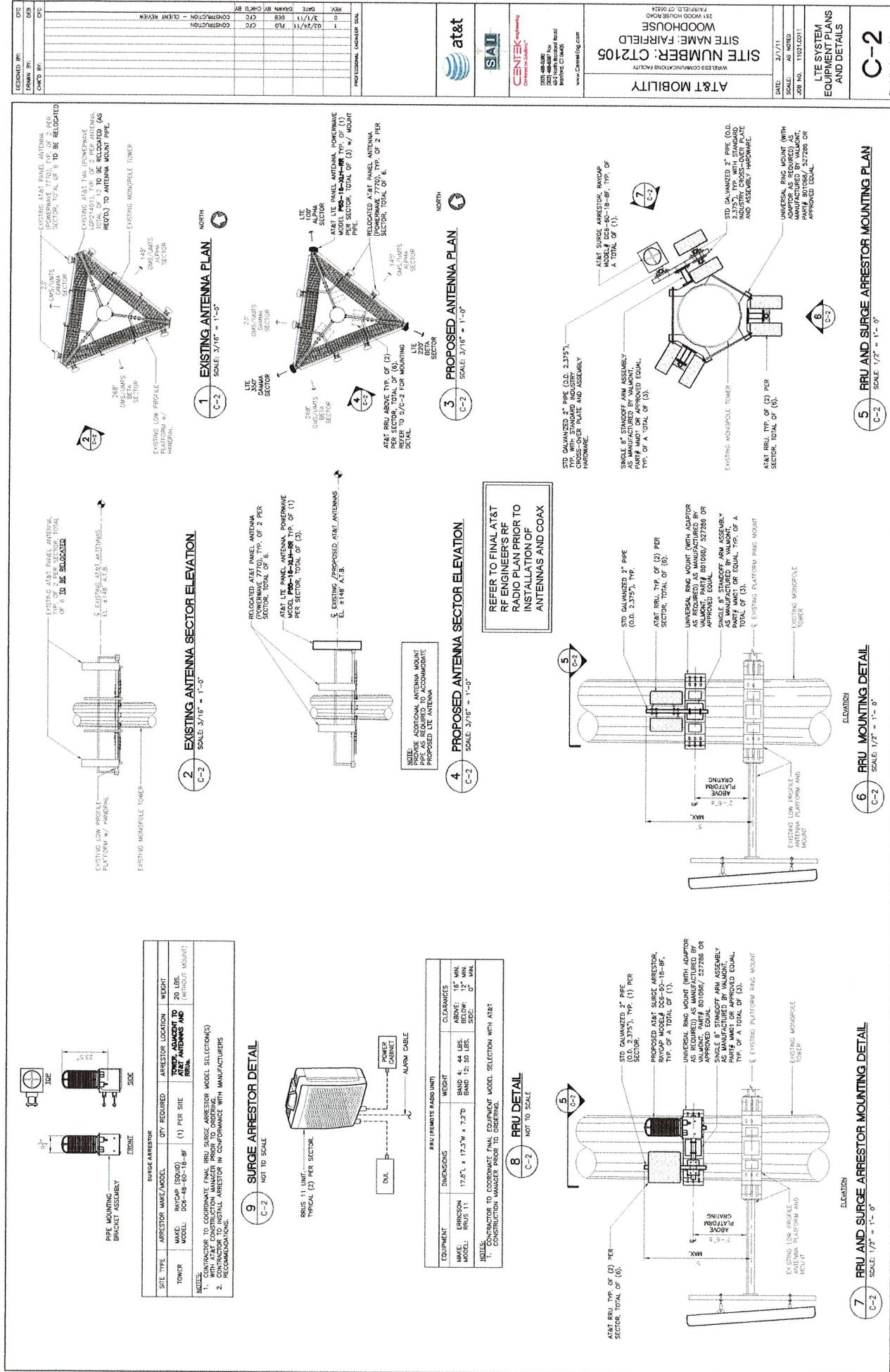
DESIGNED BY:	CTE
DRAWN BY:	CTE
CHECKED BY:	CTE
APPROVED BY:	CTE
DATE:	8/24/11
DESIGNATION:	CLIENT REVIEW

T-1

Sheet No. 1 of 5

REF ID:	C-000																	
DRAWING NO.:	C-00																	
CHARGE BY:																		
STRUCTURAL SPECIFICATIONS																		
<b>FOUNDATION CONSTRUCTION NOTES</b> <p>1. FOOTINGS SHALL BE PLACED ON SORBITE, COMPACTED SOIL HAVING ADEQUATE BEARING CAPACITY AND FREE OF ORGANIC CONTENT, CLAY OR OTHER UNSUITABLE MATERIAL. ADDITIONAL EXCAVATION MAY BE REQUIRED BELOW FLOOR LEVELS INDICATED IF UNSUITABLE MATERIAL IS ENCOUNTERED.</p> <p>2. SUBGRADE PREPARATION: IF UNSUITABLE SOIL IS ENCOUNTERED, REMOVE ALL UNSUITABLE MATERIALS AND APPROVED GRAVEL FILLS. ACTUAL SOIL TESTS AND LABORATORY TESTS SHALL BE CONDUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. ALL SUBGRADE SHALL BE COMPACTED IN LAYERS, NOT TO EXCEED 12" THICK, AND MAKE ONE COMPACTION TEST. CONTRACT DOCUMENTS REQUIRE THAT AS A MAXIMUM, DENSITY SHALL BE 95% OF DRY DENSITY. ACCORDINGLY, DENSITY TESTS SHALL BE CONDUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. ONE TEST PER LAYER, TO INSURE COMPACTION TO 95% OF MAX. DRY DENSITY.</p> <p>3. ALL SOIL SURROUNDING AND UNDER ALL FOOTINGS SHALL BE KEPT REASONABLY DRY AND PROTECTED FROM FREEZING AND THAWING DURING THE COURSE OF CONSTRUCTION.</p> <p>4. WHERE GROUNDOVER IS ENCOUNTERED, REMOVING SHALL BE ACCOMPLISHED CONTINUOUSLY AND COMPLETELY DURING FOUNDATION CONSTRUCTION. PROVIDE CRUSHED STONE AS REQUIRED TO STABILIZE COATING SURFACE.</p> <p>5. ALL FOOTINGS ARE TO REST ON FIRM SOIL, REGARDLESS OF ELEVATIONS SHOWN ON THE DRAWINGS, UNLESS SPECIFICALLY DIRECTED BY THE ENGINEER. HIGHER THAN INDICATED ON THE FOUNDATION PLAN, UNLESS SPECIFICALLY DIRECTED BY THE ENGINEER.</p>																		
<b>CONCRETE CONSTRUCTION NOTES</b> <p>1. CONCRETE STRENGTH SHALL CONFORM TO THE FOLLOWING STANDARDS:</p> <p>ACI 211 - STANDARD PRACTICE FOR SELECTING PROPORTIONS FOR NORMAL AND HEAVYWEIGHT CONCRETE.</p> <p>ACI 301 - SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS.</p> <p>ACI 302 - GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION</p> <p>ACI 304 - RECOMMENDED PRACTICE FOR MEASURING, MIXING, TRANSPORTING, AND PLACING CONCRETE.</p> <p>ACI 306.1 - STANDARD SPECIFICATION FOR COLD WEATHER CONCRETE</p> <p>ACI 318 - BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE</p> <p>2. CONCRETE SLAB, DEVELOP COMPRESSIVE STRENGTH IN 24 HOURS AS FOLLOWING:</p> <p>SLABS ON GROOVE ALL OTHER CONCRETE ACI 318 1,000 PSI 1,000 PSI</p> <p>PORTLAND CEMENT ASTM C-150, TYPE I, TYPICAL AGGREGATE: ASTM No. 87 WATER: POTABLE, WITH NO AWWAR WATER CEMENT RATIO OF .55</p> <p>ADDITIONAL TO USE OF AIR-ENTERING AGENT, CONFORMING TO ASTM C-450, TYPE A, IN ALL CONCRETE, CALCIUM CHLORIDE MAY NOT BE USED TO ACCELERATE THE CONCRETE SETTING TIME.</p> <p>3. REINFORCING STEEL SHALL CONFORM TO ASTM A-105.</p> <p>4. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-105.</p> <p>5. TIE DRAILING, FABRICATION, AND EJECTION OF REINFORCING BARS, UNLESS OTHERWISE NOTED, MUST BE CONDUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.</p> <p>6. CONCRETE OVER REINFORCING SHALL CONFORM TO THE FOLLOWING, UNLESS OTHERWISE SHOWN: C-000 CENTERLINE Contractor's responsibility</p> <p>BOTTOM OF FOOTINGS SURFACES NOT EXPOSED TO EARTH OR WEATHER 3 INCHES 1-1/2 INCHES</p> <p>7. NO STEEL, IRON, FORM TIES, OR ANY OTHER METAL SHALL REMAIN WITHIN THE REINFORCED COVER OF ANY CONCRETE SURFACE.</p> <p>8. ALL REINFORCEMENT SHALL BE CONTINUOUS, UNLESS OTHERWISE NOTED. SPlices SHALL BE WELL, STAGED; ADDITIONAL BARS AND SPECIAL BENDS ARE REQUIRED AT INTRICATE WALLS AND AT JOINTS, SUCH DETAILS SHALL COMPLY WITH ACI 318 RECOMMENDATIONS UNLESS OTHERWISE SHOWN.</p> <p>9. NO TACK WELDING OF REINFORCING WILL BE PERMITTED.</p> <p>10. NO CALCIUM CHLORIDE OR ANADAMIDES CONTAINING MORE THAN 1% CHLORIDE BY WEIGHT OF ADMIXTURE SHALL BE USED IN THE CONCRETE.</p> <p>11. UNLESS OTHERWISE NOTED, ALL LAP SPLICES SHALL BE 4B BAR DIAMETERS.</p> <p>12. SLAB ON GRADE FINISHES:</p> <p>EXTERIOR SLAB: NON-SLIP BROOM FINISH INTERIOR SLAB: STEEL TROWEL FINISH</p>																		
<b>EARTHWORK NOTES</b> <p>1. COMPACTED GRAVEL FILL SHALL BE FURNISHED AND PLACED AS A FOUNDATION FOR STRUCTURES, WHERE SHOWN ON THE CONTRACT DRAWINGS OR DIRECTED BY THE ENGINEER.</p> <p>2. CRUSHED STONE TILL SHALL BE PLACED IN 12" MAX. LIFTS AND CONSOLIDATED USING A HAND OPERATED VIBRATORY STONE COMPACTOR WITH A MINIMUM OF 2 PASSES OF COMPACTOR PER LIFT.</p> <p>3. COMPACTED GRAVEL TILL TO BE WELL GRADED BANK RUN GRAVEL, MEETING THE FOLLOWING GRADATION REQUIREMENTS.</p> <table border="1"> <thead> <tr> <th>SIZE DESIGNATION</th> <th>% PASSING</th> </tr> </thead> <tbody> <tr> <td>No. 100</td> <td>40-70</td> </tr> <tr> <td>No. 200</td> <td>4-8</td> </tr> </tbody> </table> <p>4. CRUSHED STONE TO BE UNIFORMLY graded, CLEAN, HARD, PROCESS AGGREGATE MEETING THE FOLLOWING GRADATION REQUIREMENTS.</p> <table border="1"> <thead> <tr> <th>SIZE DESIGNATION</th> <th>% PASSING</th> </tr> </thead> <tbody> <tr> <td>1"</td> <td>100</td> </tr> <tr> <td>¾"</td> <td>90-100</td> </tr> <tr> <td>½"</td> <td>0-15</td> </tr> <tr> <td>¼"</td> <td>0-5</td> </tr> </tbody> </table> <p>5. SELECT BACKFILL FOR FOUNDATION WALLS SHALL BE FREE OF ORGANIC MATERIAL, TOPSOIL, DEBRIS AND BOULDERS LARGER THAN 6".</p> <p>6. GRAVEL AND GRANULAR FILL SHALL BE INSTALLED IN 6" MAX. LIFTS, COMPACTED TO 95% MIN. AT MAX. 39% DENSITY.</p> <p>7. NON WOVEN GEOTEXTILE, FOR SEPARATION PURPOSES SHALL BE MIRAFI 140N, OR ENGINEER APPROVED EQUAL.</p>			SIZE DESIGNATION	% PASSING	No. 100	40-70	No. 200	4-8	SIZE DESIGNATION	% PASSING	1"	100	¾"	90-100	½"	0-15	¼"	0-5
SIZE DESIGNATION	% PASSING																	
No. 100	40-70																	
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SIZE DESIGNATION	% PASSING																	
1"	100																	
¾"	90-100																	
½"	0-15																	
¼"	0-5																	
<b>GENERAL NOTES (REFERENCE)</b> CONTRACTOR TO REFER TO THE GENERAL NOTES ON SHEET T-1 FOR ADDITIONAL INFORMATION.																		
<b>ATT&amp;T MOBILITY</b> <b>SITE NAME: FAIRFIELD</b> <b>SITE NUMBER: CT2105</b> WIRELESS COMMUNICATIONS FACILITY 281 WOOD HOLLOW ROAD NEW YORK, NY 10524 PRINTED 04/11/2011 DATE: 3/1/11 SCALE: AS BUILT Job No. 11012011 <b>N-1</b> <small>Sheet No. 2 of 6</small>																		







RECORD BY:	CH
PRINT BY:	LS
SERIAL BY:	CS
DATE: 3/1/11	
JOB NO. 1101011	
CITY/STATE/COUNTRY - CLIENT NAME	
ATT&T MOBILITY	
SITE NAME: FAIRFIELD, CT2105	
WBES COMMUNICATIONS AUTOMATION	
24 VACO WOOD HOUSE	
FAX: 203-644-0204	
at&t	
SIA	
CENTEK Engineering Centek@centek.com	
DOL-ELCO P.O. Box 8400 800 North Elmwood Road Norwalk, CT 06854 www.CentekEng.com	
WIRELESS COMMUNICATIONS AUTOMATION	
ATT&T MOBILITY	
E-2	
Sheet No. 3 of 5	

**1 ANTENNA CABLE GROUNDING - MONPOLE**

NOTES:  
1. NUMBER OF GROUND BARS MAY VARY DEPENDING ON THE TYPE OF TOWER, LOCATION AND CONNECTION. PROVIDE AS REQUIRED.  
2. A SEPARATE GROUND BAR TO BE USED FOR GPS ANTENNA IF REQUIRED.

E-2 NOT TO SCALE

**2 GROUND BAR DETAIL**

NOT TO SCALE

LEGEND

1. TINED COPPER GROUND BAR 1/4" x 4" x 20", NEUTON INSTRUMENT CO. HOLE CENTERS TO MATCH NEUTON DOUBLE LUG.
2. INSULATORS, NEUTON INSTRUMENT CAT. NO. 2. 3081-4.
3. 1/4" LOCK WASHERS, NEUTON INSTRUMENT CO. CAT. NO. 3015-8.
4. NAIL HOLDING BRACKET, NEUTON INSTRUMENT CO. CAT. NO. 4-39346.
5. STAINLESS STEEL SECURITY SCREWS.

NOTE: FROM EARTH

E-2 NOT TO SCALE

**3 CONNECTION OF GROUND WIRES TO GROUND BAR**

NOT TO SCALE

NOTE:  
1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUPE

E-2 NOT TO SCALE

**4 ANTENNA CABLE GROUNDING DETAIL**

NOT TO SCALE

NOTE:  
1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUPE

E-2 NOT TO SCALE

Date: April 21, 2011

Veronica Harris  
Crown Castle  
1200 McArthur Blvd  
Mahwah, NJ 07430



Crown Castle  
2000 Corporate Dr.  
Canonsburg, PA 15317  
(724) 416-2000

**Subject: Structural Analysis Report**

<b>Carrier Designation:</b>	<b>AT&amp;T Mobility Co-Locate</b>	
	<b>Carrier Site Number:</b>	2105
	<b>Carrier Site Name:</b>	Fairfield-Woodhouse Road
<b>Crown Castle Designation:</b>	<b>Crown Castle BU Number:</b>	806355
	<b>Crown Castle Site Name:</b>	BRG 126 943086
	<b>Crown Castle JDE Job Number:</b>	154916
	<b>Crown Castle Work Order Number:</b>	402472
<b>Engineering Firm Designation:</b>	<b>Crown Castle Project Number:</b>	402472
<b>Site Data:</b>	<b>281 WOODHOUSE ROAD, FAIRFIELD, Fairfield County, CT</b>	
	Latitude 41° 11' 45.3", Longitude -73° 16' 52.9"	
	171 Foot - Monopole Tower	

Dear Veronica Harris,

Crown Castle is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 402472, in accordance with application 121276, revision 1.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

<b>LC1: Existing + Reserved + Proposed Equipment</b>	<b>Sufficient Capacity</b>
Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.	

The analysis has been performed in accordance with the TIA/EIA-222-F standard and local code requirements based upon a wind speed of 85 mph fastest mile.

All modifications and equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

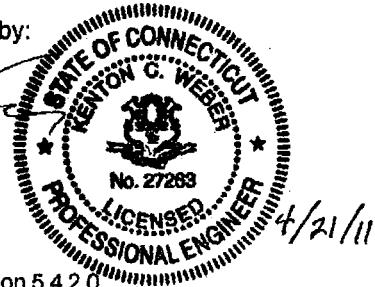
We at Crown Castle appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Structural analysis prepared by: Alex Mrkajic / CSV

Respectfully submitted by:

A handwritten signature of Kenton C. Weber.

Kenton C. Weber, P.E.  
Engineering Supervisor



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## 1) INTRODUCTION

This tower is a 171 ft Monopole tower designed by EEI, Inc. in May of 1998. The tower was originally designed for a wind speed of 85 mph per TIA/EIA-222-E & F.

## 2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 85 mph with no ice, 37.6 mph with 0.75 inch ice thickness and 50 mph under service loads.

**Table 1 - Proposed Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
146	148	6	ericsson	RRUS-11			
		6	powerwave technologies	LGP2140X	2	3/8	
		3	powerwave technologies	P65-16-XLH-RR w/ Mount Pipe	1	5/8	
		1	raycap	DC6-48-60-18-8F			

**Table 2 - Existing and Reserved Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
166	167	3	ems wireless	RV90-17-02DP	-	-	2
		1	tower mounts	Pipe Mount [PM 601-3]	-	-	
		3	andrew	LNX-6514DS-T4M w/ Mount Pipe	-	-	1
		6	decibel	DB844G65ZAXY w/ Mount Pipe	-	-	
155	158	6	rfs celwave	FD9R6004/2C-3L	12	1 5/8	3
		3	rymsa wireless	MG D3-800TV w/ Mount Pipe	-	-	1
		1	tower mounts	Platform Mount [LP 602-1]	-	-	
		6	powerwave technologies	7770.00 w/ Mount Pipe	-	-	
146	148	6	powerwave technologies	LGP2140X	12	1 5/8	1
		-	-	-	1	1/2	4
		1	tower mounts	Platform Mount [LP 602-1]	-	-	1
		3	ems wireless	DR65-18-02DP w/ Mount Pipe	-	-	
138	140	3	rfs celwave	APX16DWV-16DWV-S-E-A20 w/Mount Pipe	12	1 5/8	1
		3	rfs celwave	ATMAA1412D-1A20	-	-	
		3	rfs celwave	ATMPP1412D-1CWA	-	-	

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
128	128	1	tower mounts	Platform Mount [LP 602-1]			
		1	andrew	VHLP800-11			
		3	kathrein	840 10054 w/ Mount Pipe	3	5/16	
		1	tower mounts	Side Arm Mount [SO 101-3]	1	1/2	1
		1	tower mounts	Side Arm Mount [SO 301-1]	3	1/4	
		2	tower mounts	Side Arm Mount [SO 701-1]	-	-	2
		1	tower mounts	T-Arm Mount [TA 602-1]			

Notes:

- 1) Existing Equipment
- 2) Abandoned Equipment, considered in this analysis
- 3) Reserved Equipment
- 4) Equipment to be Removed

Table 3 - Design Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
160	160	12	Allgon	ALP 9212	-	-
148	148	12	Allgon	ALP 11011	-	-
138	138	6	Celwave	APN 199015	-	-
128	128	12	Allgon	ALP 9212	-	-
118	118	12	Allgon	ALP 9212	-	-

### 3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Clarence Welti Assoc., Inc.	1099974	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	EEI, Inc.	1098364	CCISITES
4-TOWER MANUFACTURER DRAWINGS	EEI, Inc.	653293	CCISITES

#### 3.1) Analysis Method

RISATower (version 5.4.2.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

### 3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by TIA/EIA-222-F.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

## 4) ANALYSIS RESULTS

**Table 5 - Section Capacity (Summary)**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	171 - 156.5	Pole	TP10.75x10.75x0.365	1	-0.70	333.35	13.9	Pass
L2	156.5 - 156	Pole	TP19.5x10.75x0.365	2	-0.70	625.46	8.2	Pass
L3	156 - 132.669	Pole	TP24.79x19.5x0.1875	3	-6.58	735.45	64.7	Pass
L4	132.669 - 87.0859	Pole	TP34.63x23.5836x0.375	4	-14.55	2052.54	85.4	Pass
L5	87.0859 - 43	Pole	TP43.75x32.7959x0.4375	5	-25.02	3029.76	89.0	Pass
L6	43 - 0	Pole	TP52.5x41.5315x0.5	6	-37.50	4081.19	83.2	Pass
						Summary		
						Pole (L5)	89.0	Pass
						Rating =	89.0	Pass

**Table 6 - Tower Component Stresses vs. Capacity - LC1**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	74.8	Pass
1	Base Plate	0	90.6	Pass
1	Base Foundation	0	87.0	Pass
1	Exterior Flange bolts	156	1.7	Pass
1	Exterior Flange plate	156	2.4	Pass

**Structure Rating (max from all components) =**

**90.6%**

Notes:

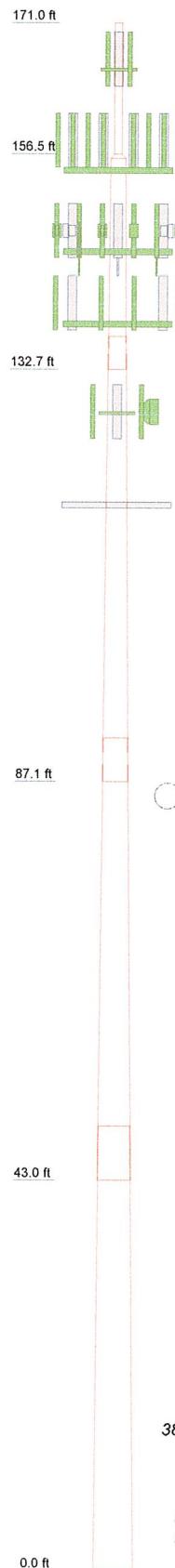
- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

### 4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the existing, reserved, and proposed loads. No modifications are required at this time.

**APPENDIX A**  
**RISA TOWER OUTPUT**

Section	6	49'					
Length (ft)	18						
Number of Sides							
Thickness (in)	0.4375						
Socket Length (ft)	0.5000						
Top Dia (in)	41.5315	32.7959					
Bot Dia (in)	52.5000	43.7500					
Grade			A572-65				
Weight (K)	12.3						
	8.7						



### DESIGNED APPURTENANCE LOADING

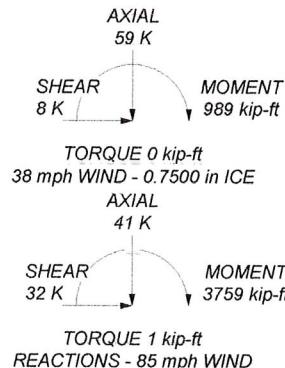
TYPE	ELEVATION	TYPE	ELEVATION
RV90-17-02DP	166	6' x 2" Mount Pipe	146
RV90-17-02DP	166	Platform Mount [LP 602-1]	146
RV90-17-02DP	166	APX16DWV-16DWV-S-E-A20 w/Mount Pipe	138
Pipe Mount [PM 601-3]	166	DR65-18-02DP w/ Mount Pipe	138
LNX-6514DS-T4M w/ Mount Pipe	155	ATMAA1412D-1A20	138
(2) DB844G652AXY w/ Mount Pipe	155	ATMPP1412D-1CWA	138
MG D3-800TV w/ Mount Pipe	155	APX16DWV-16DWV-S-E-A20 w/Mount Pipe	138
(2) FD9R6004/2C-3L	155	DR65-18-02DP w/ Mount Pipe	138
LNX-6514DS-T4M w/ Mount Pipe	155	ATMAA1412D-1A20	138
(2) DB844G652AXY w/ Mount Pipe	155	ATMPP1412D-1CWA	138
MG D3-800TV w/ Mount Pipe	155	APX16DWV-16DWV-S-E-A20 w/Mount Pipe	138
(2) FD9R6004/2C-3L	155	Platform Mount [LP 602-1]	138
P65-16-XLH-RR w/ Mount Pipe	146	6' x 2" Mount Pipe	138
(2) RRUS-11	146	6' x 2" Mount Pipe	138
(2) LGP2140X	146	6' x 2" Mount Pipe	138
P65-16-XLH-RR w/ Mount Pipe	146	Platform Mount [LP 602-1]	138
(2) RRUS-11	146	840 10054 w/ Mount Pipe	128
(2) LGP2140X	146	840 10054 w/ Mount Pipe	128
P65-16-XLH-RR w/ Mount Pipe	146	840 10054 w/ Mount Pipe	128
(2) RRUS-11	146	6' x 2" Mount Pipe	128
DC6-48-60-18-8F	146	Side Arm Mount [SO 101-3]	128
(2) LGP2140X	146	VHLP800-11	128
(2) 7770.00 w/ Mount Pipe	146	Side Arm Mount [SO 301-1]	118
(2) LGP2140X	146	Side Arm Mount [SO 701-1]	118
(2) 7770.00 w/ Mount Pipe	146	Side Arm Mount [SO 701-1]	118
(2) LGP2140X	146	(3) 6' x 2" Mount Pipe	118
(2) 7770.00 w/ Mount Pipe	146	6' x 2" Mount Pipe	118
(2) LGP2140X	146	6' x 2" Mount Pipe	118
6' x 2" Mount Pipe	146	T-Arm Mount [TA 602-1]	118
6' x 2" Mount Pipe	146		

### MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A53-B-35	35 ksi	63 ksi	A572-65	65 ksi	80 ksi

### TOWER DESIGN NOTES

1. Tower is located in Fairfield County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 38 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 89%



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## Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in Fairfield County, Connecticut.  
Basic wind speed of 85 mph.  
Nominal ice thickness of 0.7500 in.  
Ice thickness is considered to increase with height.  
Ice density of 56.00 pcf.  
A wind speed of 38 mph is used in combination with ice.  
Temperature drop of 50 °F.  
Deflections calculated using a wind speed of 50 mph.  
A non-linear (P-delta) analysis was used.  
Pressures are calculated at each section.  
Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

## Options

- |                                     |                                     |
|-------------------------------------|-------------------------------------|
| Consider Moments - Legs             | Distribute Leg Loads As Uniform     |
| Consider Moments - Horizontals      | Assume Legs Pinned                  |
| Consider Moments - Diagonals        | ✓ Assume Rigid Index Plate          |
| Use Moment Magnification            | ✓ Use Clear Spans For Wind Area     |
| ✓ Use Code Stress Ratios            | Use Clear Spans For KL/r            |
| ✓ Use Code Safety Factors - Guys    | Retension Guys To Initial Tension   |
| ✓ Escalate Ice                      | ✓ Bypass Mast Stability Checks      |
| Always Use Max Kz                   | ✓ Use Azimuth Dish Coefficients     |
| Use Special Wind Profile            | ✓ Project Wind Area of Appurt.      |
| Include Bolts In Member Capacity    | Autocalc Torque Arm Areas           |
| Leg Bolts Are At Top Of Section     | SR Members Have Cut Ends            |
| Secondary Horizontal Braces Leg     | Sort Capacity Reports By Component  |
| Use Diamond Inner Bracing (4 Sided) | Triangulate Diamond Inner Bracing   |
| Add IBC .6D+W Combination           |                                     |
|                                     | Treat Feedline Bundles As Cylinder  |
|                                     | Use ASCE 10 X-Brace Ly Rules        |
|                                     | Calculate Redundant Bracing Forces  |
|                                     | Ignore Redundant Members in FEA     |
|                                     | SR Leg Bolts Resist Compression     |
|                                     | All Leg Panels Have Same Allowable  |
|                                     | Offset Girt At Foundation           |
|                                     | ✓ Consider Feedline Torque          |
|                                     | Include Angle Block Shear Check     |
|                                     | Poles                               |
|                                     | ✓ Include Shear-Torsion Interaction |
|                                     | Always Use Sub-Critical Flow        |
|                                     | Use Top Mounted Sockets             |

## Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	171'-156'6"	14'6"	0'	Round	10.7500	10.7500	0.3650		A53-B-35 (35 ksi)
L2	156'6"-156'	6"	0'	18	10.7500	19.5000	0.3650	1.4600	A572-65 (65 ksi)
L3	156'-132'8-1/32"	23'3-31/32"	3'8-1/32"	18	19.5000	24.7900	0.1875	0.7500	A572-65 (65 ksi)
L4	132'8-1/32"-87'1 -3/32"	49'3"	4'9-31/32"	18	23.5836	34.6300	0.3750	1.5000	A572-65 (65 ksi)
L5	87'1-3/32"-43'	48'11-1/32"	6'	18	32.7959	43.7500	0.4375	1.7500	A572-65 (65 ksi)

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	Client	Crown Castle	Designed by cvolk

Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft		in	in	in	in	
L6	43'-0"	49'		18	41.5315	52.5000	0.5000	2.0000	A572-65 (65 ksi)

### Tapered Pole Properties

Section	Tip Dia. in	Area in <sup>2</sup>	I in <sup>4</sup>	r in	C in	I/C in <sup>3</sup>	J in <sup>4</sup>	It/Q in <sup>3</sup>	w in	w/t
L1	10.7500	11.9083	160.6589	3.6763	5.3750	29.8900	320.8802	5.9506	0.0000	0
	10.7500	11.9083	160.6589	3.6763	5.3750	29.8900	320.8802	5.9506	0.0000	0
L2	10.9158	12.0311	163.9294	3.6867	5.4610	30.0182	328.0744	6.0167	1.2496	3.424
	19.8008	22.1681	1025.4692	6.7929	9.9060	103.5200	2052.2878	11.0862	2.7896	7.643
L3	19.8008	11.4934	541.5782	6.8559	9.9060	54.6717	1083.8689	5.7478	3.1020	16.544
	25.1724	14.6416	1119.6528	8.7339	12.5933	88.9085	2240.7788	7.3222	4.0330	21.51
L4	24.7825	27.6241	1879.8522	8.2391	11.9805	156.9096	3762.1780	13.8147	3.4907	9.309
	35.1642	40.7720	6044.3215	12.1605	17.5920	343.5828	12096.5963	20.3899	5.4349	14.493
L5	34.4008	44.9337	5944.0771	11.4872	16.6603	356.7803	11895.9756	22.4711	5.0021	11.433
	44.4249	60.1448	14254.8347	15.3759	22.2250	641.3874	28528.4263	30.0781	6.9300	15.84
L6	43.5360	65.1169	13850.5059	14.5662	21.0980	656.4847	27719.2367	32.5647	6.4295	12.859
	53.3099	82.5240	28191.9040	18.4600	26.6700	1057.0643	56420.9036	41.2698	8.3600	16.72

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor $A_f$	Adjust. Factor $A_r$	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in
ft	ft <sup>2</sup>	in						
L1 171'-156'6"				1	1	1		
L2 156'6"-156'				1	1	1		
L3				1	1	1		
156'-132'8-1/3								
2"								
L4				1	1	1		
132'8-1/32"-87"								
1-3/32"								
L5				1	1	1		
87'1-3/32"-43"								
L6 43'-0"				1	1	1		

### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	$C_A A_A$	Weight
***								ft <sup>2</sup> /ft	k/lf
561(1-5/8")	C	No	Inside Pole	155' - 0'	0.0000	0	12	No Ice	0.00
								1/2" Ice	0.00
								1" Ice	0.00
								2" Ice	0.00
								4" Ice	0.00
FB-L98B-002-75000(3/8")	C	No	Inside Pole	146' - 0'	0.0000	0	2	No Ice	0.00
								1/2" Ice	0.00
								1" Ice	0.00
								2" Ice	0.00

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	Client	Crown Castle	Designed by cvolk

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	$C_A A_A$	Weight klf
WR-VG82ST-BRDA( 5/8")	C	No	Inside Pole	146' - 0'	0.0000	0	1	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.00 0.00 0.00 0.00 0.00 0.00
CR 50 1873PE(1-5/8" )	C	No	Inside Pole	146' - 0'	0.0000	0	12	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.00 0.00 0.00 0.00 0.00
LCF158-50JA -A0(1 5/8")	C	No	Inside Pole	138' - 0'	0.0000	0	6	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.00 0.00 0.00 0.00 0.00
LCF158-50JA -A0(1 5/8")	C	No	CaAa (Out Of Face)	138' - 0'	1.0000	0	1	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.20 0.30 0.40 0.60 1.00
LCF158-50JA -A0(1 5/8")	C	No	CaAa (Out Of Face)	138' - 0'	1.0000	0	5	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.00 0.00 0.00 0.01 0.03
7983A(1/2")	C	No	CaAa (Out Of Face)	128' - 0'	1.0000	0	1	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.00 0.00 0.00 0.01 0.03
9207(5/16")	C	No	CaAa (Out Of Face)	128' - 0'	1.0000	0	3	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.00 0.00 0.00 0.01 0.02
9258(1/4")	C	No	CaAa (Out Of Face)	128' - 0'	1.0000	0	3	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.00 0.00 0.00 0.01 0.02
2" Rigid Conduit	C	No	CaAa (Out Of Face)	128' - 0'	1.0000	0	1	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.00 0.00 0.00 0.01 0.02
2" Rigid Conduit	C	No	CaAa (Out Of Face)	128' - 0'	1.0000	0	1	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.20 0.30 0.40 0.60 1.00
* Climbing Ladder ( Flat)	C	No	CaAa (Out Of Face)	155' - 147'	48.0000	0	1	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.58 1.03 1.48 2.37 4.15
Climbing Ladder ( Flat)	C	No	CaAa (Out Of Face)	146' - 138'	48.0000	0	1	No Ice 1/2" Ice 1" Ice 2" Ice	0.58 1.03 1.48 2.37



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	Project		Date 11:39:29 04/21/11
	Client	Crown Castle	Designed by cvolk

### Feed Line Center of Pressure

Section	Elevation	CP <sub>X</sub> ft	CP <sub>Z</sub> in	CP <sub>X</sub> Ice in	CP <sub>Z</sub> Ice in
L1	171'-156'6"	0.0000	0.0000	0.0000	0.0000
L2	156'6"-156'	0.0000	0.0000	0.0000	0.0000
L3	156'-132'8"-1/32"	-0.5804	0.3351	-0.9791	0.5653
L4	132'8"-1/32"-87'1"-3/3 2"	-0.4555	0.2630	-0.7418	0.4283
L5	87'1"-3/32"-43"	-0.4604	0.2658	-0.7584	0.4379
L6	43'-0"	-0.4700	0.2713	-0.7725	0.4460

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight K	
**									
RV90-17-02DP	A	From Leg	1.00 0' 1'	0.000	166'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.36 4.77 5.20 6.08 7.95	1.97 2.31 2.66 3.37 4.89	0.02 0.04 0.07 0.14 0.33
RV90-17-02DP	B	From Leg	1.00 0' 1'	0.000	166'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.36 4.77 5.20 6.08 7.95	1.97 2.31 2.66 3.37 4.89	0.02 0.04 0.07 0.14 0.33
RV90-17-02DP	C	From Leg	1.00 0' 1'	0.000	166'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.36 4.77 5.20 6.08 7.95	1.97 2.31 2.66 3.37 4.89	0.02 0.04 0.07 0.14 0.33
Pipe Mount [PM 601-3]	C	None		0.000	166'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.39 5.48 6.57 8.75 13.11	4.39 5.48 6.57 8.75 13.11	0.20 0.24 0.28 0.36 0.53
*									
LNX-6514DS-T4M w/ Mount Pipe	A	From Leg	4.00 0' 3'	0.000	155'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	8.57 9.22 9.84 11.10 13.75	7.00 8.19 9.08 10.90 14.93	0.06 0.12 0.20 0.38 0.89
(2) DB844G65ZAXY w/ Mount Pipe	A	From Leg	4.00 0' 3'	0.000	155'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.90 5.35 5.80 6.73 8.73	4.92 5.60 6.28 7.71 10.83	0.03 0.08 0.13 0.26 0.62
MG D3-800TV w/ Mount Pipe	A	From Leg	4.00 0'	0.000	155'	No Ice 1/2" Ice	3.57 3.98	3.42 4.12	0.04 0.07

<p><b>RISATower</b></p> <p><b>Crown Castle</b> 2000 Corporate Dr. Canonsburg, PA 15317 Phone: (724) 416-2000 FAX: (724) 416-2254</p>	Job	BU# 806355	Page
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	Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	CAA Front ft²	CAA Side ft²	Weight K	
(2) RRUS-11		A	From Leg	4.00 0' 2'	0.000	146'	2" Ice 4" Ice No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	11.18 13.83 4.42 4.71 5.00 5.61 6.94	10.24 14.10 1.19 1.35 1.53 1.90 2.75	0.39 0.89 0.06 0.08 0.11 0.18 0.37
(2) LGP2140X		A	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.26 1.42 1.58 1.94 2.75	0.38 0.49 0.62 0.89 1.54	0.02 0.03 0.04 0.06 0.14
P65-16-XLH-RR w/ Mount Pipe		B	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	8.64 9.29 9.91 11.18 13.83	6.36 7.54 8.43 10.24 14.10	0.08 0.14 0.22 0.39 0.89
(2) RRUS-11		B	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.42 4.71 5.00 5.61 6.94	1.19 1.35 1.53 1.90 2.75	0.06 0.08 0.11 0.18 0.37
(2) LGP2140X		B	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.26 1.42 1.58 1.94 2.75	0.38 0.49 0.62 0.89 1.54	0.02 0.03 0.04 0.06 0.14
P65-16-XLH-RR w/ Mount Pipe		C	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	8.64 9.29 9.91 11.18 13.83	6.36 7.54 8.43 10.24 14.10	0.08 0.14 0.22 0.39 0.89
(2) RRUS-11		C	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.42 4.71 5.00 5.61 6.94	1.19 1.35 1.53 1.90 2.75	0.06 0.08 0.11 0.18 0.37
DC6-48-60-18-8F		C	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.27 1.46 1.66 2.09 3.10	1.27 1.46 1.66 2.09 3.10	0.02 0.04 0.05 0.10 0.21
(2) LGP2140X		C	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.26 1.42 1.58 1.94 2.75	0.38 0.49 0.62 0.89 1.54	0.02 0.03 0.04 0.06 0.14
(2) 7770.00 w/ Mount Pipe		A	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	6.12 6.63 7.13 8.16 10.36	4.25 5.01 5.71 7.16 10.41	0.06 0.10 0.16 0.29 0.66
(2) LGP2140X		A	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.26 1.42 1.58 1.94 2.75	0.38 0.49 0.62 0.89 1.54	0.02 0.03 0.04 0.06 0.14

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	CA_A Front	CA_A Side	Weight K	
(2) 7770.00 w/ Mount Pipe	B	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	6.12 6.63 7.13 8.16 10.36	4.25 5.01 5.71 7.16 10.41	0.06 0.10 0.16 0.29 0.66
(2) LGP2140X	B	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.26 1.42 1.58 1.94 2.75	0.38 0.49 0.62 0.89 1.54	0.02 0.03 0.04 0.06 0.14
(2) 7770.00 w/ Mount Pipe	C	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	6.12 6.63 7.13 8.16 10.36	4.25 5.01 5.71 7.16 10.41	0.06 0.10 0.16 0.29 0.66
(2) LGP2140X	C	From Leg	4.00 0' 2'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.26 1.42 1.58 1.94 2.75	0.38 0.49 0.62 0.89 1.54	0.02 0.03 0.04 0.06 0.14
6' x 2" Mount Pipe	A	From Leg	4.00 0' 0'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.43 1.92 2.29 3.06 4.70	1.43 1.92 2.29 3.06 4.70	0.02 0.03 0.05 0.09 0.23
6' x 2" Mount Pipe	B	From Leg	4.00 0' 0'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.43 1.92 2.29 3.06 4.70	1.43 1.92 2.29 3.06 4.70	0.02 0.03 0.05 0.09 0.23
6' x 2" Mount Pipe	C	From Leg	4.00 0' 0'	0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.43 1.92 2.29 3.06 4.70	1.43 1.92 2.29 3.06 4.70	0.02 0.03 0.05 0.09 0.23
Platform Mount [LP 602-1]	C	None		0.000	146'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	32.03 38.71 45.39 58.75 85.47	32.03 38.71 45.39 58.75 85.47	1.34 1.80 2.26 3.17 5.00
APX16DWV-16DWV-S-E-A 20 w/Mount Pipe	A	From Leg	4.00 0' 2'	0.000	138'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	7.27 7.73 8.21 9.18 11.23	3.29 3.92 4.57 5.92 8.88	0.06 0.10 0.16 0.28 0.65
DR65-18-02DP w/ Mount Pipe	A	From Leg	4.00 0' 2'	0.000	138'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	10.40 11.19 11.99 13.55 16.72	6.73 8.86 11.02 14.26 19.69	0.07 0.14 0.22 0.42 1.02
ATMAA1412D-1A20	A	From Leg	4.00 0' 2'	0.000	138'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.17 1.31 1.47 1.81 2.58	0.47 0.57 0.69 0.95 1.57	0.01 0.02 0.03 0.06 0.14
ATMPP1412D-1CWA	A	From Leg	4.00	0.000	138'	No Ice	1.17	0.42	0.01



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Description		Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	$C_A A_{Front}$	$C_A A_{Side}$	Weight
Platform Mount [LP 602-1]	C	None			0.000	138'	2" Ice 4" Ice No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	3.06 4.70 32.03 38.71 45.39 58.75 85.47	0.09 0.23 1.34 1.80 2.26 3.17 5.00
840 10054 w/ Mount Pipe	A	From Leg	2.00 0' 0'		0.000	128'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	5.41 5.83 6.26 7.16 9.09	0.05 0.09 0.13 0.23 0.53
840 10054 w/ Mount Pipe	B	From Leg	2.00 0' 0'		0.000	128'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	5.41 5.83 6.26 7.16 9.09	0.05 0.09 0.13 0.23 0.53
840 10054 w/ Mount Pipe	C	From Leg	2.00 0' 0'		0.000	128'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	5.41 5.83 6.26 7.16 9.09	0.05 0.09 0.13 0.23 0.53
6' x 2" Mount Pipe	B	From Leg	2.00 0' 0'		0.000	128'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.43 1.92 2.29 3.06 4.70	0.02 0.03 0.05 0.09 0.23
Side Arm Mount [SO 101-3]	C	None			0.000	128'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	7.50 8.90 10.30 13.10 18.70	0.25 0.33 0.41 0.58 0.90
T-Arm Mount [TA 602-1]	A	From Leg	3.00 0' 0'		0.000	118'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	7.28 9.52 11.76 16.24 25.20	0.26 0.33 0.40 0.55 0.83
Side Arm Mount [SO 301-1]	B	From Leg	3.00 0' 0'		0.000	118'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.00 1.39 1.78 2.56 4.12	0.02 0.03 0.04 0.06 0.10
Side Arm Mount [SO 701-1]	B	From Leg	3.00 0' 0'		0.000	118'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.85 1.14 1.43 2.01 3.17	0.07 0.08 0.09 0.12 0.18
Side Arm Mount [SO 701-1]	C	From Leg	3.00 0' 0'		0.000	118'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.85 1.14 1.43 2.01 3.17	0.07 0.08 0.09 0.12 0.18
(3) 6' x 2" Mount Pipe	A	From Leg	3.00 0' 0'		0.000	118'	No Ice 1/2" Ice 1" Ice	1.43 1.92 2.29	0.02 0.03 0.05

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	CA A Front	CA A Side	Weight
6' x 2" Mount Pipe	B	From Leg	3.00 0' 0'	0.000	118'	2" Ice 4" Ice No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	3.06 4.70 1.43 1.92 2.29 3.06 4.70	0.09 0.23 0.02 0.03 0.05 0.09 0.23
6' x 2" Mount Pipe	C	From Leg	3.00 0' 0'	0.000	118'	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.43 1.92 2.29 3.06 4.70	0.02 0.03 0.05 0.09 0.23

## Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert ft ° °	Azimuth Adjustment °	3 dB Beam Width ft	Elevation ft	Outside Diameter	Aperture Area ft²	Weight K	
VHLP800-11	B	Paraboloid w/Shroud (HP)	From Leg	2.00 0' 0'	-61.000		128'	2.92	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	6.68 7.07 7.46 8.23 9.78	0.02 0.03 0.03 0.07 0.23

## Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 60 deg - No Ice
5	Dead+Wind 90 deg - No Ice
6	Dead+Wind 120 deg - No Ice
7	Dead+Wind 150 deg - No Ice
8	Dead+Wind 180 deg - No Ice
9	Dead+Wind 210 deg - No Ice
10	Dead+Wind 240 deg - No Ice
11	Dead+Wind 270 deg - No Ice
12	Dead+Wind 300 deg - No Ice
13	Dead+Wind 330 deg - No Ice
14	Dead+Ice+Temp
15	Dead+Wind 0 deg+Ice+Temp
16	Dead+Wind 30 deg+Ice+Temp
17	Dead+Wind 60 deg+Ice+Temp
18	Dead+Wind 90 deg+Ice+Temp

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<i>Comb. No.</i>	<i>Description</i>
19	Dead+Wind 120 deg+Ice+Temp
20	Dead+Wind 150 deg+Ice+Temp
21	Dead+Wind 180 deg+Ice+Temp
22	Dead+Wind 210 deg+Ice+Temp
23	Dead+Wind 240 deg+Ice+Temp
24	Dead+Wind 270 deg+Ice+Temp
25	Dead+Wind 300 deg+Ice+Temp
26	Dead+Wind 330 deg+Ice+Temp
27	Dead+Wind 0 deg - Service
28	Dead+Wind 30 deg - Service
29	Dead+Wind 60 deg - Service
30	Dead+Wind 90 deg - Service
31	Dead+Wind 120 deg - Service
32	Dead+Wind 150 deg - Service
33	Dead+Wind 180 deg - Service
34	Dead+Wind 210 deg - Service
35	Dead+Wind 240 deg - Service
36	Dead+Wind 270 deg - Service
37	Dead+Wind 300 deg - Service
38	Dead+Wind 330 deg - Service

### Maximum Member Forces

<i>Section No.</i>	<i>Elevation ft</i>	<i>Component Type</i>	<i>Condition</i>	<i>Gov. Load Comb.</i>	<i>Force K</i>	<i>Major Axis Moment kip-ft</i>	<i>Minor Axis Moment kip-ft</i>
L1	171 - 156.5	Pole	Max Tension	8	0.00	-0.00	0.00
			Max. Compression	14	-1.24	0.00	-0.00
			Max. Mx	11	-0.70	10.53	-0.00
			Max. My	8	-0.70	0.00	-10.53
			Max. Vy	11	-1.16	10.53	-0.00
			Max. Vx	8	1.16	0.00	-10.53
			Max. Torque	8			0.00
L2	156.5 - 156	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-1.27	0.00	-0.00
			Max. Mx	11	-0.73	11.11	-0.00
			Max. My	8	-0.73	0.01	-11.11
			Max. Vy	11	-1.19	11.11	-0.00
			Max. Vx	8	1.19	0.01	-11.11
			Max. Torque	8			0.00
L3	156 - 132.669	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-15.46	0.47	-0.25
			Max. Mx	11	-6.59	228.63	-0.13
			Max. My	8	-6.59	0.17	-228.57
			Max. Vy	11	-17.85	228.63	-0.13
			Max. Vx	8	17.85	0.17	-228.57
			Max. Torque	13			0.38
L4	132.669 - 87.0859	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-26.80	2.14	0.04
			Max. Mx	11	-14.57	1177.04	-4.21
			Max. My	2	-14.56	-4.30	1178.64
			Max. Vy	11	-23.90	1177.04	-4.21
			Max. Vx	8	24.02	7.74	-1178.24
			Max. Torque	11			1.25
L5	87.0859 - 43	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-39.80	4.69	-1.40
			Max. Mx	11	-25.03	2286.91	-10.10
			Max. My	8	-25.02	16.84	-2293.06

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L6	43 - 0	Pole	Max. Vy	11	-27.70	2286.91	-10.10
			Max. Vx	8	27.82	16.84	-2293.06
			Max. Torque	9			0.77
			Max. Tension	1	0.00	0.00	0.00
			Max. Compression	14	-59.00	7.91	-3.26
			Max. Mx	11	-41.19	3738.20	-16.77
			Max. My	8	-41.19	27.19	-3749.93
			Max. Vy	11	-31.44	3738.20	-16.77
			Max. Vx	8	31.56	27.19	-3749.93
			Max. Torque	11			0.84

### Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	22	59.00	3.93	-6.81
	Max. Hx	11	41.21	31.41	-0.13
	Max. Hz	2	41.21	-0.12	31.51
	Max. Mx	2	3747.14	-0.12	31.51
	Max. Mz	5	3725.28	-31.34	0.10
	Max. Torsion	11	0.84	31.41	-0.13
	Min. Vert	1	41.21	0.00	0.00
	Min. Hx	5	41.21	-31.34	0.10
	Min. Hz	8	41.21	0.19	-31.53
	Min. Mx	8	-3749.93	0.19	-31.53
	Min. Mz	11	-3738.20	31.41	-0.13
	Min. Torsion	5	-0.56	-31.34	0.10

### Tower Mast Reaction Summary

Load Combination	Vertical	Shear <sub>x</sub>	Shear <sub>z</sub>	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque
	K	K	K			kip-ft
Dead Only	41.21	0.00	0.00	0.09	1.45	0.00
Dead+Wind 0 deg - No Ice	41.21	0.12	-31.51	-3747.14	-14.69	0.37
Dead+Wind 30 deg - No Ice	41.21	15.75	-27.29	-3245.11	-1872.92	0.44
Dead+Wind 60 deg - No Ice	41.21	27.16	-15.80	-1879.24	-3229.28	0.51
Dead+Wind 90 deg - No Ice	41.21	31.34	-0.10	-13.40	-3725.28	0.56
Dead+Wind 120 deg - No Ice	41.21	27.15	15.65	1859.93	-3227.56	0.45
Dead+Wind 150 deg - No Ice	41.21	15.65	27.25	3239.55	-1859.03	0.08
Dead+Wind 180 deg - No Ice	41.21	-0.19	31.53	3749.93	27.19	-0.44
Dead+Wind 210 deg - No Ice	41.21	-15.81	27.34	3252.52	1883.69	-0.58
Dead+Wind 240 deg - No Ice	41.21	-27.23	15.84	1884.55	3240.90	-0.71
Dead+Wind 270 deg - No Ice	41.21	-31.41	0.13	16.77	3738.20	-0.84
Dead+Wind 300 deg - No Ice	41.21	-27.20	-15.60	-1852.68	3236.92	-0.76
Dead+Wind 330 deg - No Ice	41.21	-15.59	-27.28	-3243.75	1854.93	0.08
Dead+Ice+Temp	59.00	-0.00	0.00	3.26	7.91	-0.00
Dead+Wind 0 deg+Ice+Temp	59.00	0.03	-7.85	-977.01	4.34	0.02
Dead+Wind 30 deg+Ice+Temp	59.00	3.92	-6.80	-845.73	-481.25	0.05
Dead+Wind 60 deg+Ice+Temp	59.00	6.76	-3.94	-488.21	-835.81	0.10
Dead+Wind 90 deg+Ice+Temp	59.00	7.79	-0.02	0.21	-965.44	0.15
Dead+Wind 120 deg+Ice+Temp	59.00	6.75	3.90	490.32	-835.31	0.15



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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
34	-5.48	-41.21	9.48	5.48	41.21	-9.48	0.000%
35	-9.44	-41.21	5.49	9.44	41.21	-5.49	0.000%
36	-10.89	-41.21	0.04	10.89	41.21	-0.04	0.000%
37	-9.43	-41.21	-5.41	9.43	41.21	5.41	0.000%
38	-5.41	-41.21	-9.46	5.41	41.21	9.46	0.000%

### Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	5	0.00000001	0.00004457
3	Yes	6	0.00000001	0.00008801
4	Yes	6	0.00000001	0.00008647
5	Yes	4	0.00000001	0.00035367
6	Yes	6	0.00000001	0.00008759
7	Yes	6	0.00000001	0.00008690
8	Yes	4	0.00000001	0.00059813
9	Yes	6	0.00000001	0.00008662
10	Yes	6	0.00000001	0.00008861
11	Yes	5	0.00000001	0.00006950
12	Yes	6	0.00000001	0.00008584
13	Yes	6	0.00000001	0.00008680
14	Yes	4	0.00000001	0.00003004
15	Yes	5	0.00000001	0.00028196
16	Yes	5	0.00000001	0.00055540
17	Yes	5	0.00000001	0.00055118
18	Yes	5	0.00000001	0.00027910
19	Yes	5	0.00000001	0.00055617
20	Yes	5	0.00000001	0.00055100
21	Yes	5	0.00000001	0.00028336
22	Yes	5	0.00000001	0.00057094
23	Yes	5	0.00000001	0.00057432
24	Yes	5	0.00000001	0.00028435
25	Yes	5	0.00000001	0.00055545
26	Yes	5	0.00000001	0.00056423
27	Yes	4	0.00000001	0.00015997
28	Yes	5	0.00000001	0.00022107
29	Yes	5	0.00000001	0.00021449
30	Yes	4	0.00000001	0.00013738
31	Yes	5	0.00000001	0.00021751
32	Yes	5	0.00000001	0.00021502
33	Yes	4	0.00000001	0.00009679
34	Yes	5	0.00000001	0.00021669
35	Yes	5	0.00000001	0.00022399
36	Yes	4	0.00000001	0.00026700
37	Yes	5	0.00000001	0.00021146
38	Yes	5	0.00000001	0.00021605

### Maximum Tower Deflections - Service Wind

<b>RISA Tower</b>  <i>Crown Castle</i> 2000 Corporate Dr. Canonsburg, PA 15317 Phone: (724) 416-2000 FAX: (724) 416-2254	Job	BU# 806355	Page
	Project		Date 11:39:29 04/21/11
	Client	Crown Castle	Designed by cvolk

Section No.	Elevation	Horz. Deflection	Gov. Load Comb.	Tilt	Twist
	ft	in		°	°
L1	171 - 156.5	48.552	34	2.529	0.001
L2	156.5 - 156	40.901	34	2.495	0.001
L3	156 - 132.669	40.639	34	2.495	0.001
L4	136.336 - 87.0859	30.725	34	2.257	0.001
L5	91.9193 - 43	13.172	34	1.440	0.001
L6	49 - 0	3.549	34	0.677	0.000

### Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
ft			in	°	°	ft
166'	RV90-17-02DP	34	45.902	2.515	0.002	32524
155'	LNX-6514DS-T4M w/ Mount Pipe	34	40.118	2.493	0.002	10510
146'	P65-16-XLH-RR w/ Mount Pipe	34	35.488	2.414	0.002	5272
138'	APX16DWV-16DWV-S-E-A20 w/Mount Pipe	34	31.525	2.286	0.002	3614
128'	VHLP800-11	34	26.869	2.108	0.002	3289
118'	T-Arm Mount [TA 602-1]	34	22.578	1.925	0.001	3211

### Maximum Tower Deflections - Design Wind

Section No.	Elevation	Horz. Deflection	Gov. Load Comb.	Tilt	Twist
	ft	in		°	°
L1	171 - 156.5	138.947	9	7.241	0.004
L2	156.5 - 156	117.124	9	7.148	0.004
L3	156 - 132.669	116.379	9	7.147	0.004
L4	136.336 - 87.0859	88.070	9	6.472	0.004
L5	91.9193 - 43	37.815	9	4.134	0.001
L6	49 - 0	10.199	9	1.945	0.000

### Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
ft			in	°	°	ft
166'	RV90-17-02DP	9	131.391	7.201	0.007	12135
155'	LNX-6514DS-T4M w/ Mount Pipe	9	114.891	7.141	0.007	3865
146'	P65-16-XLH-RR w/ Mount Pipe	9	101.676	6.918	0.006	1906
138'	APX16DWV-16DWV-S-E-A20 w/Mount Pipe	9	90.355	6.556	0.006	1299
128'	VHLP800-11	9	77.045	6.047	0.005	1176
118'	T-Arm Mount [TA 602-1]	9	64.768	5.526	0.004	1144

<b>RISA Tower</b>  <i>Crown Castle</i> 2000 Corporate Dr. Canonsburg, PA 15317 Phone: (724) 416-2000 FAX: (724) 416-2254	Job BU# 806355	Page 17 of 18
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	Client Crown Castle	Designed by cvolk

## Compression Checks

### Pole Design Data

Section No.	Elevation	Size	L	L <sub>u</sub>	Kl/r	F <sub>a</sub>	A	Actual P	Allow. P <sub>a</sub>	Ratio P	
			ft	ft		ksi	in <sup>2</sup>	K	K		
L1	171 - 156.5 (1)	TP10.75x10.75x0.365	14'6"	0'	0.0	21.00	11.9083	-0.70	250.07	0.003	
L2	156.5 - 156 (2)	TP19.5x10.75x0.365		6"	0'	0.0	39.00	12.0311	-0.70	469.21	0.001
L3	156 - 132.669	TP24.79x19.5x0.1875	23'3-31/3	0'	0.0	39.00	14.1468	-6.58	551.73	0.012	
	(3)			2"							
L4	132.669 - 87.0859 (4)	TP34.63x23.5836x0.375	49'3"	0'	0.0	39.00	39.4817	-14.55	1539.79	0.009	
L5	87.0859 - 43 (5)	TP43.75x32.7959x0.4375	48'11-1/3	0'	0.0	39.00	58.2792	-25.02	2272.89	0.011	
	(5)			2"							
L6	43 - 0 (6)	TP52.5x41.5315x0.5	49'	0'	0.0	39.00	78.5041	-37.50	3061.66	0.012	

### Pole Bending Design Data

Section No.	Elevation	Size	Actual M <sub>c</sub>	Actual f <sub>bx</sub>	Allow. F <sub>bx</sub>	Ratio f <sub>bx</sub>	Actual M <sub>y</sub>	Actual f <sub>by</sub>	Allow. F <sub>by</sub>	Ratio f <sub>by</sub>
			kip-ft	ksi	ksi	$\frac{f_{bx}}{F_{bx}}$	kip-ft	ksi	ksi	$\frac{f_{by}}{F_{by}}$
L1	171 - 156.5 (1)	TP10.75x10.75x0.365	10.53	4.23	23.10	0.183	0.00	0.00	23.10	0.000
L2	156.5 - 156 (2)	TP19.5x10.75x0.365	10.53	4.21	39.00	0.108	0.00	0.00	39.00	0.000
L3	156 - 132.669	TP24.79x19.5x0.1875	228.66	33.07	39.00	0.848	0.00	0.00	39.00	0.000
	(3)									
L4	132.669 - 87.0859 (4)	TP34.63x23.5836x0.375	1181.17	44.01	39.00	1.128	0.00	0.00	39.00	0.000
L5	87.0859 - 43 (5)	TP43.75x32.7959x0.4375	2298.68	45.82	39.00	1.175	0.00	0.00	39.00	0.000
	(5)									
L6	43 - 0 (6)	TP52.5x41.5315x0.5	3405.82	42.75	39.00	1.096	0.00	0.00	39.00	0.000

### Pole Shear Design Data

Section No.	Elevation	Size	Actual V	Actual f <sub>v</sub>	Allow. F <sub>v</sub>	Ratio f <sub>v</sub>	Actual T	Actual f <sub>vt</sub>	Allow. F <sub>vt</sub>	Ratio f <sub>vt</sub>
			K	ksi	ksi	$\frac{f_v}{F_v}$	kip-ft	ksi	ksi	$\frac{f_{vt}}{F_{vt}}$
L1	171 - 156.5 (1)	TP10.75x10.75x0.365	1.16	0.10	14.00	0.014	0.00	0.00	14.00	0.000
L2	156.5 - 156 (2)	TP19.5x10.75x0.365	1.19	0.10	26.00	0.004	0.00	0.00	26.00	0.000
L3	156 - 132.669	TP24.79x19.5x0.1875	17.85	1.26	26.00	0.097	0.19	0.01	26.00	0.001
	(3)									
L4	132.669 - 87.0859 (4)	TP34.63x23.5836x0.375	24.08	0.61	26.00	0.047	0.78	0.01	26.00	0.001
L5	87.0859 - 43 (5)	TP43.75x32.7959x0.4375	27.88	0.48	26.00	0.037	0.69	0.01	26.00	0.000
	(5)									
L6	43 - 0 (6)	TP52.5x41.5315x0.5	30.94	0.39	26.00	0.030	0.61	0.00	26.00	0.000

<b>RISATower</b>  <i>Crown Castle</i> 2000 Corporate Dr. Canonsburg, PA 15317 Phone: (724) 416-2000 FAX: (724) 416-2254	Job	BU# 806355	Page
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	Client	Crown Castle	Designed by cvolk

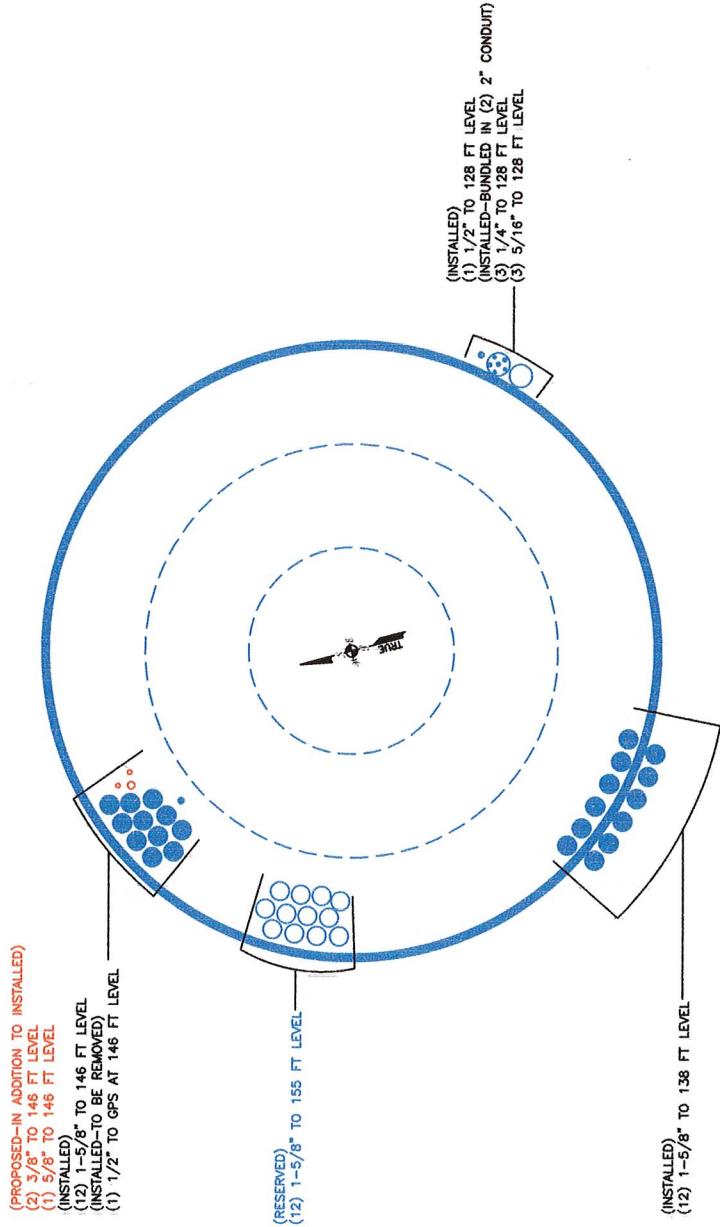
### Pole Interaction Design Data

Section No.	Elevation ft	Ratio P P <sub>a</sub>	Ratio f <sub>bx</sub> F <sub>bx</sub>	Ratio f <sub>by</sub> F <sub>bx</sub>	Ratio f <sub>v</sub> F <sub>v</sub>	Ratio f <sub>vt</sub> F <sub>vt</sub>	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	171 - 156.5 (1)	0.003	0.183	0.000	0.014	0.000	0.186	1.333	H1-3+VT ✓
L2	156.5 - 156 (2)	0.001	0.108	0.000	0.004	0.000	0.109	1.333	H1-3+VT ✓
L3	156 - 132.669 (3)	0.012	0.848	0.000	0.097	0.001	0.862	1.333	H1-3+VT ✓
L4	132.669 - 87.0859 (4)	0.009	1.128	0.000	0.047	0.001	1.138	1.333	H1-3+VT ✓
L5	87.0859 - 43 (5)	0.011	1.175	0.000	0.037	0.000	1.186	1.333	H1-3+VT ✓
L6	43 - 0 (6)	0.012	1.096	0.000	0.030	0.000	1.109	1.333	H1-3+VT ✓

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P <sub>allow</sub> K	% Capacity	Pass Fail
L1	171 - 156.5	Pole	TP10.75x10.75x0.365	1	-0.70	333.35	13.9	Pass
L2	156.5 - 156	Pole	TP19.5x10.75x0.365	2	-0.70	625.46	8.2	Pass
L3	156 - 132.669	Pole	TP24.79x19.5x0.1875	3	-6.58	735.45	64.7	Pass
L4	132.669 - 87.0859	Pole	TP34.63x23.5836x0.375	4	-14.55	2052.54	85.4	Pass
L5	87.0859 - 43	Pole	TP43.75x32.7959x0.4375	5	-25.02	3029.76	89.0	Pass
L6	43 - 0	Pole	TP52.5x41.5315x0.5	6	-37.50	4081.19	83.2	Pass
Summary								
Pole (L5) 89.0 Pass								
RATING = 89.0 Pass								

**APPENDIX B**  
**BASE LEVEL DRAWING**



**APPENDIX C**  
**ADDITIONAL CALCULATIONS**

## Stiffened or Unstiffened, Exterior Flange Plate - Any Bolt Material TIA Rev F

<b>Site Data</b>		<b>Reactions</b>																																					
BU#: 806355 Site Name: BRG 126 943086 App #: 121276 Connection Type: <b>Butt</b> Pole Manufacturer: Other		<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td>Moment:</td> <td>10.53</td> <td>ft-kips</td> </tr> <tr> <td>Axial:</td> <td>0.7</td> <td>kips</td> </tr> <tr> <td>Shear:</td> <td>1.19</td> <td>kips</td> </tr> <tr> <td>Elevation:</td> <td>156</td> <td>feet</td> </tr> </table>		Moment:	10.53	ft-kips	Axial:	0.7	kips	Shear:	1.19	kips	Elevation:	156	feet																								
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Axial:	0.7	kips																																					
Shear:	1.19	kips																																					
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<b>Bolt Data</b>		<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td>Qty:</td> <td>24</td> <td rowspan="5" style="vertical-align: middle; text-align: center; font-size: small;">&lt;- Disregard</td> </tr> <tr> <td>Diameter (in.):</td> <td>1</td> </tr> <tr> <td>Bolt Material:</td> <td>A325</td> </tr> <tr> <td>N/A:</td> <td>75</td> </tr> <tr> <td>N/A:</td> <td>55</td> </tr> <tr> <td>Circle (in.):</td> <td>25.75</td> </tr> </table>		Qty:	24	<- Disregard	Diameter (in.):	1	Bolt Material:	A325	N/A:	75	N/A:	55	Circle (in.):	25.75																							
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Bolt Material:	A325																																						
N/A:	75																																						
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<b>Plate Data</b>		<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td>Diam:</td> <td>28.5</td> <td>in</td> </tr> <tr> <td>Thick, t:</td> <td>1.5</td> <td>in</td> </tr> <tr> <td>Grade (Fy):</td> <td>60</td> <td>ksi</td> </tr> <tr> <td>Strength, Fu:</td> <td>75</td> <td>ksi</td> </tr> <tr> <td>Single-Rod B-eff:</td> <td>2.58</td> <td>in</td> </tr> </table>		Diam:	28.5	in	Thick, t:	1.5	in	Grade (Fy):	60	ksi	Strength, Fu:	75	ksi	Single-Rod B-eff:	2.58	in																					
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<b>Stiffener Data</b> (Welding at Both Sides)		<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td>Config:</td> <td>0</td> <td>*</td> </tr> <tr> <td>Weld Type:</td> <td></td> <td></td> </tr> <tr> <td>Groove Depth:</td> <td></td> <td>in **</td> </tr> <tr> <td>Groove Angle:</td> <td></td> <td>degrees</td> </tr> <tr> <td>Fillet H. Weld:</td> <td></td> <td>&lt;- Disregard</td> </tr> <tr> <td>Fillet V. Weld:</td> <td></td> <td>in</td> </tr> <tr> <td>Width:</td> <td></td> <td>in</td> </tr> <tr> <td>Height:</td> <td></td> <td>in</td> </tr> <tr> <td>Thick:</td> <td></td> <td>in</td> </tr> <tr> <td>Notch:</td> <td></td> <td>in</td> </tr> <tr> <td>Grade:</td> <td></td> <td>ksi</td> </tr> <tr> <td>Weld str.:</td> <td></td> <td>ksi</td> </tr> </table>		Config:	0	*	Weld Type:			Groove Depth:		in **	Groove Angle:		degrees	Fillet H. Weld:		<- Disregard	Fillet V. Weld:		in	Width:		in	Height:		in	Thick:		in	Notch:		in	Grade:		ksi	Weld str.:		ksi
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<b>Pole Data</b>		<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td>Diam:</td> <td>19.5</td> <td>in</td> </tr> <tr> <td>Thick:</td> <td>0.365</td> <td>in</td> </tr> <tr> <td>Grade:</td> <td>65</td> <td>ksi</td> </tr> <tr> <td># of Sides:</td> <td>18</td> <td>"0" IF Round</td> </tr> <tr> <td>Fu</td> <td>80</td> <td>ksi</td> </tr> <tr> <td>Reinf. Fillet Weld</td> <td>0</td> <td>"0" if None</td> </tr> </table>		Diam:	19.5	in	Thick:	0.365	in	Grade:	65	ksi	# of Sides:	18	"0" IF Round	Fu	80	ksi	Reinf. Fillet Weld	0	"0" if None																		
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<b>Stress Increase Factor</b>		<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td>ASIF:</td> <td>1.333</td> <td></td> </tr> </table>		ASIF:	1.333																																		
ASIF:	1.333																																						

If No stiffeners, Criteria: **AISC ASD** <- Only Applicable to Unstiffened Cases

Rigid
Service, ASD
Fly*ASIF

**Flange Bolt Results**

Bolt Tension Capacity, B:	46.07 kips
Max Bolt directly applied T:	0.79 Kips
Min. PL "tc" for B cap. w/o Pry:	2.177 in
Min PL "treq" for actual T w/ Pry:	0.226 in
Min PL "t1" for actual T w/o Pry:	0.285 in
T allowable with Prying:	31.95 kips
Prying Force, Q:	0.00 kips
Total Bolt Tension=T+Q:	0.79 kips
Prying Bolt Stress Ratio=(T+Q)/(B):	1.7% <b>Pass</b>

**Exterior Flange Plate Results**

Flexural Check	
Compression Side Plate Stress:	1.4 ksi
Allowable Plate Stress:	60.0 ksi
Compression Plate Stress Ratio:	2.4% <b>Pass</b>
<b>No Prying</b>	
Tension Side Stress Ratio, (treq/t)^2:	2.3% <b>Pass</b>

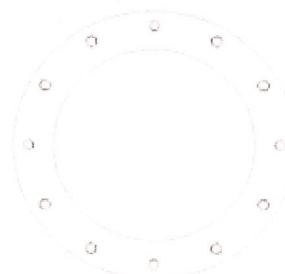
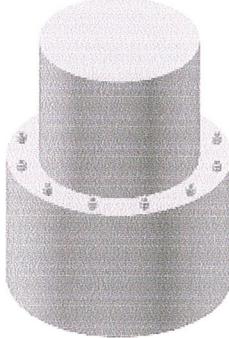
**n/a**

**Stiffener Results**

Horizontal Weld :	n/a
Vertical Weld:	n/a
Plate Flex+Shear, fb/Fb+(fv/Fv)^2:	n/a
Plate Tension+Shear, ft/Ft+(fv/Fv)^2:	n/a
Plate Comp. (AISC Bracket):	n/a

**Pole Results**

Pole Punching Shear Check:	n/a
----------------------------	-----

\* 0 = none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

\*\* Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

# Stiffened or Unstiffened, Ungrounded, Circular Base Plate - Any Rod Material

## TIA Rev F

### Site Data

BU#: 806355

Site Name: BRG 126 943086

App #: 121276 Rev. 1

Pole Manufacturer: Other

### Reactions

Moment:	3759	ft-kips
Axial:	41	kips
Shear:	32	kips

### Anchor Rod Data

Qty:	20	
Diam:	2.25	in
Rod Material:	A615-J	
Strength (Fu):	100	ksi
Yield (Fy):	75	ksi
Bolt Circle:	61	in

### Plate Data

Diam:	67	in
Thick:	2.25	in
Grade:	60	ksi
Single-Rod B-eff:	8.33	in

### Stiffener Data (Welding at both sides)

Config:	0	*
Weld Type:		
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		<-- Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

### Pole Data

Diam:	52.5	in
Thick:	0.5	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None

### Stress Increase Factor

ASIF:	1.333	
-------	-------	--

If No stiffeners, Criteria:

AISC ASD

<- Only Applicable to Unstiffened Cases

### Anchor Rod Results

Maximum Rod Tension: 145.8 Kips  
Allowable Tension: 195.0 Kips  
Anchor Rod Stress Ratio: 74.8% Pass

Rigid
Service, ASD
0.75*Fy*ASIF

### Base Plate Results

Flexural Check  
Base Plate Stress: 54.3 ksi  
Allowable Plate Stress: 60.0 ksi  
Base Plate Stress Ratio: 90.6% Pass

Rigid
Service ASD
0.75*Fy*ASIF
Y.L. Length: 31.06

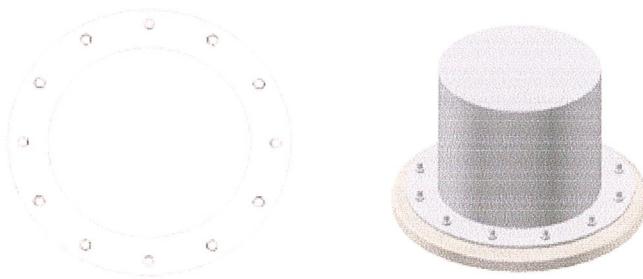
n/a

### Stiffener Results

Horizontal Weld : n/a  
Vertical Weld: n/a  
Plate Flex+Shear,  $f_b/F_b + (f_v/F_v)^2$ : n/a  
Plate Tension+Shear,  $f_t/F_t + (f_v/F_v)^2$ : n/a  
Plate Comp. (AISC Bracket): n/a

### Pole Results

Pole Punching Shear Check: n/a



\* 0 = none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

\*\* Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

## Monopole Pier and Pad Foundation

BU #: 806355

Site Name: BRG 126 943086

App. Number: 121276



Design Reactions		
Shear, S:	32 kips	
Moment, M:	3759 ft-kips	
Tower Height, H:	171 ft	
Tower Weight, Wt:	41 kips	
Base Diameter, BD:	4.375 ft	

Soil Properties		
Soil Unit Weight, Y:	0.125 kcf	
Bearing Capacity, Bc:	12.0 ksf	
Angle of Friction, Φ:	36 deg	
Cohesion, Co:	0.000 ksf	
Passive Pressure, Pp:	0.000 kcf	
Base Friction, μ:	0.60	

Material Properties		
Rebar Yield Strength, Fy:	60000 psi	
Concrete Strength, Fc:	4000 psi	
Concrete Unit Weight, δc:	0.150 kcf	
Seismic Zone, z:	1	

Rebar Properties		
Pier Rebar Size, Sp:	8	
Pier Rebar Quantity, mp:	46	36
Pad Rebar Size, Spad:	8	
Pad Rebar Quantity, mpad:	36	26
Pier Tie Size, St:	4	3
Tie Quantity, mt:	7	8

Design Checks			Modification Checks		
Capacity/ Availability	Demand/ Limits	Check	Capacity/ Availability	Demand/ Limits	Check
Req'd Pier Diam.(ft):	7	OK	Sleeve Rebar Area (in²):	15.8	0.00
Overturining (ft-kips)	5997.72	OK	Sleeve Moment Capacity (k-ft):	4577.56	3983.00
Shear Capacity (kips)	238.86	OK	Sleeve Rebar Spacing (in):	N/a	18 > s > 2
Beaming (ft-sd):	12.00	OK	Sleeve Tie Spacing (in):	N/A	9 > s > 2.5
Pad Shear - 1-way (kips):	1085.29	OK	Minimum Extra Thickness (in):	0	Not Used
Pad Shear - 2-way (kips):	3009.19	OK	Pad Rebar Area-short (in²):	0.44	0.00
Pier Rebar Area (in²)	36.34	OK	Pad Rebar Area-long (in²):	0.44	0.00
Pad Rebar Area (in²)	28.44	OK	Pad Rebar Spacing-short (in):	85.5	18 > s > 2
Pier Moment Capacity (k-ft):	4577.56	OK	Pad Rebar Spacing-long (in):	85.5	18 > s > 2
Pier Bar Spacing (in):	4.33	OK	End Cap Width (ft):	0	Not Used
Pad Bar Spacing (in):	6.34	OK	End Cap Rebar Area (in²):	3.16	0
Pier Development Length (in):	81	OK	Rebar Spacing (in):	-3.00	18 > s > 2
Pad Development Length (in):	33	OK	Tie Spacing (in):	17.93	2.58 > s > 4.5
Hook Development Length (in):	129.00	OK	Dowel Area (in²):	2.2	0.00
Rebar Hook Length (in):	90.00	OK	Dowel Embedment (in):	9	6
			Cone Shear Strength (kips):	25.15	23.76
			Dowel Edge Dist (in):	12.00	4.78
			Dowel Spacing (in):	60.00	18.00
			Dowel Edge Dist (vert) (in):	18.00	4.78
			Dowel Devel. Length (in):	-3.00	13.32

## P65-15-XLH-RR

## Dual Broadband Antennas

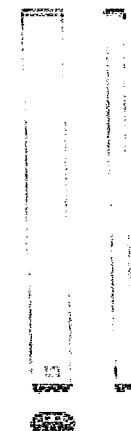
POLARISATION: Circular +/- 5°  
 FREQUENCY BAND: 1324-1400, 1626-1670  
 HORIZONTAL BEAM WIDTH (HP): 61.45°  
 VERT. BEAM WIDTH: 14.712.0° FWHM  
 TILT: ±10.0°  
 LEAD: 54°

ELECTRICAL SPECIFICATIONS<sup>1</sup>

	DB3-034	DB6-034	1710-1000	1626-1670	1324-1400
Frequency range (MHz)	905-920	826-844	1710-1860	1626-1670	1324-1400
Frequency band (MHz)	1324-1400	1626-1670	1710-1860	1626-1670	1324-1400
Gain (dBi/deg)	15.1±0.3	16.7±0.3	18.4±0.3	18.7±0.3	17.0±0.3
Polarization	Dual Linear +/- 45°			Dual Linear +/- 45°	
Normal Impedance (Ω)	50	50	50	50	50
VSWR	<1.5:1	<1.5:1	<1.5:1	<1.5:1	<1.5:1
Horizontal beam width, <3 dB (°)	F3	65	65	61	60
Vertical beam width, <3 dB (°)	17	17	17	16	16
Electrical down tilt (°)	0.10	0.10	0.09	0.09	0.09
Side lobe suppression, vertical 'hot' upper (dB)	>14	>14	>20	>20	>20
Isolation between inputs (dB)	>30	>30	>30	>30	>30
Inter-band isolation (dB)	>40	>40	>40	>40	>40
Tracking, horizontal plane ±60° (dB)	<2	<2	<2	<2	<2
Vertical beam equal. (°)	<1.25	<1.25	<0.5	<0.5	<0.5
Front-to-back ratio (dB) 180°+30° cap/tilt	>25	>25	>28	>28	>28
Front-to-back ratio (dB) 180°+30° full power	>25	>25	>28	>28	>28
Cross polar discrimination (+90°; 0°) (dB)	>15	>15	>15	>15	>15
Cross polar discrimination (XFD) ±60° (dB)	>10	>10	>10	>10	>10
IAR, 2xTx@13dBm (dBc)	<-153	<-153	<-153	<-153	<-153
Power handling, average per input (W)	500	500	300	300	300
Power handling, average total (W)	1000	1000	600	600	600

MECHANICAL SPECIFICATIONS<sup>1</sup>

Connector	4 X 7/16 DIN Female, IP67
Connector position	Bolt-on
Dimensions, HxWxD, in (mm)	51" x 12" x 6" (1285 x 305 x 152)
Mounting	Pre-mounted Til Brackets
Weight, with brackets, lbs (kg)	41 (19)
Weight, without brackets, lbs (kg)	39 (14)
Wind load, horizontal/year per 10 m/s (m/s)	620
Maximum operational wind speed, mph (m/s)	100 (45)
Survival wind speed, mph (m/s)	150 (67)
Lightning protection	DC Ground
Operating Temperature	-40°C to +60°C
Resin material	PVC, IP67
Packet size, HxWxD, in (mm)	60" x 16" x 10" (1524 x 406 x 254)
Plastic colour	Light Grey
Shipping weight, lbs (kg)	62 (28)
REI	REI A/B/Gv1.1, MET and AIBGv2.0
Brackets	7255.00, 7425.00



<sup>1</sup>All specifications subject to change without notice. Please contact your Powerwave representative for complete performance data.

ANTENNA PATTERNS<sup>1</sup>

For detailed patterns visit <http://www.powerwave.com/pat>.

# POWER

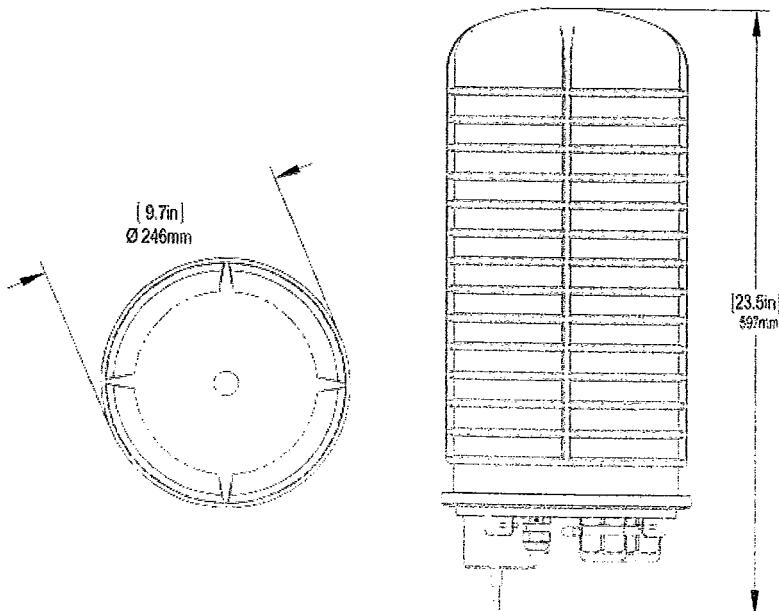
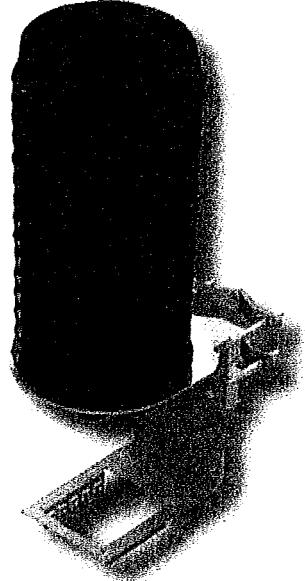
## DC6-48-60-18-8F

### DC Surge Suppression Solution

The DC6-48-60-18 is a dual chambered, DC surge suppression system for use in multi-circuit, Distributed Antenna Systems. The system will protect up to 6 Remote Radio Heads from voltage surges and lightning, and connect up to 18 fiber pairs. The system is enclosed in a NEMA 4 rated, waterproof enclosure.

#### FEATURES

- Protects up to 6 Remote Radio Heads, each with its own protection circuit.
- Flexible design allows for installation at the top of a tower for Remote Radio Head protection.
- Includes fiber connections for up to 18 pairs of fiber.
- LED indicators on individual circuits provide visual indication of suppressor status.
- Form 'C' relays allow for remote monitoring of the suppressor status.
- Patented Strikesorb technology provides over 60 kA of surge current capacity per circuit.
- Strikesorb suppression modules are fully recognized to UL 1449-3rd Edition Safety Standard, meeting all intermediate and high current fault requirements to facilitate use in OEM applications.
- Raycap recommends that DC protection system be installed within 2 meters or 6 feet of the radio.
- Dome design is lightweight and aerodynamic providing maximum flexibility for installation on top of towers.



# DC6-48-60-18-8F

## DC Power Surge Protection

Electrical Specifications	
Model Number	DC6-48-60-18-8F
Nominal Operating Voltage	48 VDC
Nominal Discharge Current ( $I_n$ )	20 kA 8/20 $\mu$ s
Maximum Discharge Current ( $I_{max}$ ) per NEMA LS-1	60 kA 8/20 $\mu$ s
Maximum Continuous Operating Voltage ( $U_o$ )	75 VDC
Voltage Protection Rating	400 V

Mechanical Specifications	
Suppression Connection Method	Compression lug, #2-#14 AWG Copper, #2-#12 Aluminum
Fiber Connection Method	LC-LC Single mode duplex
Environmental Rating	IP 68, 7m 72hrs
Operating Temperature	-40° C to + 80° C
Storage Temperature	-70° C to + 80° C
Cold Temperature Cycling	IEC 61300-2-22e -30° C to + 60° C 200 hrs @ 5 psi
Resistance to Aggressive Materials	CEI IEC 61073-2 including acids and bases
UV Protection	ISO 4892-2 Method A Xenon-Arc 2160 hrs
Weight	20 lbs without Mounting Bracket

### STANDARDS

Strikesorb modules are compliant to the following Surge Protection Device (SPD) Standards:

- ANSI/UL 1449 – 3rd Edition
- IEEE C62.41
- NEMA LS-1, IEC 61643-1:2005 2nd Edition:2005
- IEC 61643-12
- EN 61643-11:2002 (including A11:2007)



GS-07F-0435V



Certified to  
ISO 9001:2000



**Raycap**

G02-00-068 REV 050610

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Phone: 208.777.1166 • Fax: 800.890.2569 • Email: 208.777.4466 • www.raycapsurgeprotection.com

# RRUS 11 – Dual PA RRU

## Technical Data

- > Multi standard
- > RF: 2x30 Watts
- > Carrier BW: 1.4 – 20 MHz
- > Alarms: 2
- > Dimensions (with sunshield):
  - Width: 17.0 in
  - Height: 17.8 in
  - Depth: 7.2 in
  - Weight: 55 lbs (Band 12)
  - Weight: 50 lbs (Band 4)
- > Temperature: -40 to +131 F
- > Cooling: Self convection
- > Power: -48 VDC
- > Rec. fuse size 20 Amp
  - Rec. DC cable:
    - 6 mm<sup>2</sup> up to 60 meters
    - 10 mm<sup>2</sup> over 60 meters
- > Shielded
- > Power Cons: 200 Watts typ.



RBS6000



# RRUS-111/F

RBS6000

