

June 11th, 2021

Melanie A. Bachman Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

**RE:** Notice of Exempt Modification for T-Mobile:

842862 - T-Mobile Site ID: CT11623B

259 Commerce Street, East Haven, CT 06512

Latitude: 41° 15′ 22.88″/ Longitude: -72° 52′ 32.80″

Dear Ms. Bachman:

T-Mobile currently maintains six (6) total antennas at the 47-foot mount on the existing 58-foot monopole at 259 Commerce Street in East Haven, Connecticut. The tower is owned by Crown Castle and the property is owned by Stephen J. Viglione. T-Mobile now intends to replace three (3) existing antennas as well as add three (3) new antennas and ancillary equipment. The new antennas will be installed at the 47-ft level of the tower. This modification/proposal includes hardware that is both 4G(LTE) and 5G capable through remote software configuration and either or both services may be turned on or off at various times.

#### **Planned Modifications:**

#### **Tower:**

#### Remove:

(12) 1 5/8" Coax

(6) TMA

#### Remove and Replace:

(3) APX16DWV-16DWVS-C Antenna 1900/2100 MHz (**REMOVE**) - (3) AIR32 KRD901146-1 B66A B2A (**REPLACE**)

#### Install New:

- (2) 6x12 HCS 1 5/8" Hybrid Fiber Line
- (3) Radio 4415 B25
- (3) AIR6449 B41 5G antenna

#### Existing to Remain:

(3) RFS-APXVAARR24 43-U-NA20 Antenna 600/700 MHz

The Foundation for a Wireless World.

CrownCastle.com

#### Page 2

#### **Ground:**

#### Remove:

- (1) 6201 cabinet
- (6) RUS01 B12s
- (3) RUS01 B2s
- (6) RUS01 B4s

#### Install new:

- (1) 6160 equipment cabinet
- (1) B160 battery cabinet
- (1) BB 6630 baseband radio
- (1) BB 6648 baseband radio
- (1) PSU 4813

The facility was approved by the Siting Council in Petition Number 634 on July 8, 2003. No conditions were attached that would be impacted by this modification.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to the Honorable Joseph A. Carfora, Mayor of the Town of East Haven, as well as, the East Haven Planning & Zoning Department, and Stephen J. Viglione, the property owner.

- 1. The proposed modifications will not result in an increase in the height of the existing tower.
- 2. The proposed modifications will not require the extension of the site boundary.
- 3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
- 4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.
- 5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
- 6. The existing structure and its foundation can support the proposed loading.

#### Page 3

For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

Richard Zajac

Site Acquisition Specialist 4545 East River Road, Suite 320

West Henrietta, NY 14586

(585) 445-5896

richard.zajac@crowncastle.com

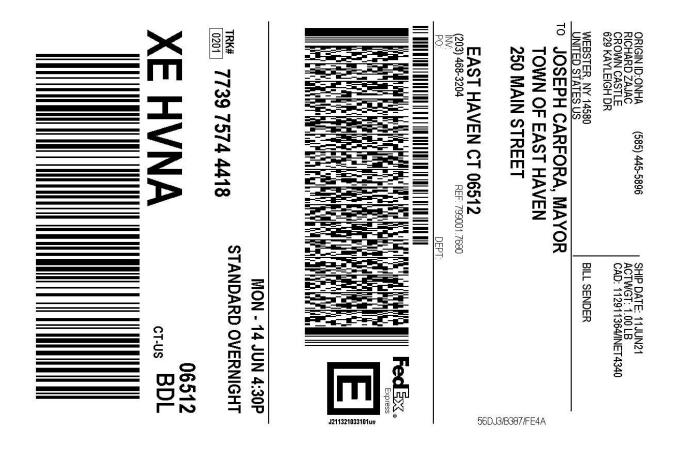
Attachments

cc:

The Honorable Joseph A. Carfora, Mayor Town of East Haven Town Hall – Upper Level 250 Main Street East Haven, CT 06512-3004 (203) 468-3204

Ellen Pellegrino, Deputy Zoning Officer (via email to: epellegrino@townofeasthavenct.org)
Town Hall – Lower Level
250 Main Street
East Haven, CT 06512-3004
(203) 468-3349

Stephen J. Viglione 259 Commerce Street East Haven, CT 06512-4147 203-467-8388



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Use of this system constitutes your agreement to the service conditions in the current FedEx Service Guide, available on fedex.com.FedEx will not be responsible for any claim in excess of \$100 per package, whether the result of loss, damage, delay, non-delivery,misdelivery,or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim.Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including intrinsic value of the package, loss of sales, income interest, profit, attorney's fees, costs, and other forms of damage whether direct, incidental,consequential, or special is limited to the greater of \$100 or the authorized declared value. Recovery cannot exceed actual documented loss.Maximum for items of extraordinary value is \$1,000, e.g. jewelry, precious metals, negotiable instruments and other items listed in our ServiceGuide. Written claims must be filed within strict time limits, see current FedEx Service Guide.

#### Zajac, Richard

From: Zajac, Richard

**Sent:** Friday, June 11, 2021 11:49 AM **To:** epellegrino@townofeasthavenct.org

**Subject:** Connecticut Siting Council exempt modification application notification

Attachments: CSC Exempt Modification Application - 259 Commerce St.pdf

Good morning Ms. Pellegrino,

Please see the attached application to the Connecticut Siting Council regarding antenna work on the existing cell tower located at 259 Commerce Street in East Haven.

Should you have any questions/comments/concerns regarding this application, please do not hesitate to contact me.

Thank you, **RICH ZAJAC** 

Site Acquisition Specialist

T: (585) 445-5896 M: (607) 346-7212

F: (724) 416-4461 CROWN CASTLE

4545 East River Road, Suite 320 West Henrietta, NY 14586



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# Exhibit A

**Original Facility Approval** 

Petition No. 634 AT&T Wireless East Haven, Connecticut Staff Report July 8, 2003

On June 10, 2003, Connecticut Siting Council (Council) member Philip T. Ashton and S. Derek Phelps of staff met with AT&T Wireless representatives at 259 Commerce Street in East Haven. Other persons in attendance were Lucia Chiocchio, Esq., of Cuddy & Feder LLP; Doug Frost, Engineering Technician, of NATCOMM, LLC; Kumar Rughoobur, RF Engineer, of WFI; Ray Vergati, Project Director, of Optasite, Inc.; and George Mingione, Planning and Zoning Administrator of the Town of East Haven. AT&T Wireless proposes to replace and expand an existing lattice tower and is petitioning the Council for a declaratory ruling that no Certificate of Environmental Compatibility and Public Need (Certificate) is required for the modification.

Specifically, AT&T Wireless proposes to replace and expand an existing 48' lattice tower (with a whip antenna extending to 61') with a 57' monopole to be relocated approximately 8' to 10' from the location of the existing tower. AT&T would attach six panel antennas on T-arms to the replacement tower. The property owner's whip antenna would not be reinstalled.

The existing lattice tower is located adjacent to the west side of the existing tower. The replacement monopole is 9' taller than the existing tower, but the overall height of the proposed facility will be approximately 1' lower in total height.

The proposed tower needs to be relocated approximately 8' to 10' from the location of the existing lattice tower for construction purposes. Associated equipment cabinets will be installed on a 7' x 13' concrete pad located at the base of the pole surrounded by an 8' vinyl stockade fence, which will be screened with 6' evergreen trees. The utilities will be installed underground.

At the request of the Council, AT&T Wireless wrote to six nearby residents on June 12, 2003, whose homes are within sight of the proposed tower location to advise them of the petition application. Those homeowners are: Antonio Rossano; Robert A. Esposito; Rita Compano; Phyllis Naqstri and Linda Lawson; Sebatiano and Maria DiBona; and Anne M. Fitzgerald. These persons were asked to forward comments to the Council by June 3, 2003. One resident, Rita Compano, sent a letter stating that she is not in favor of the petition primarily on the basis of concerns that it will adversely affect the property value of her home.

George Mingione, Planning and Zoning Administrator of the Town of East Haven, wrote to the Council in a letter dated June 11, 2003, stating that the town's preference is for vinyl fencing around the tower compound, not less than six feet tall, with evergreen plantings.

# Exhibit B

**Property Card** 

The Assessor's office is responsible for the maintenance of records on the ownership of properties. Assessments are computed at 70% of the estimated market value of real property at the time of the last revaluation which was 2016.



#### TOWN of EAST HAVEN ASSESSOR



Information on the Property Records for the Municipality of East Haven was last updated on 4/12/2019.

#### **Property Summary Information**

Parcel Data And Values Building Outbuildings Sales Permits

#### Parcel Information

Location:	259 COMMERCE ST	Property Use:	Industrial	Primary Use:	Light Industrial
Unique ID:	V0098600	Map Block Lot:	090 1013 005	Acres:	0.49
490 Acres:	0.00	Zone:	LI-2	Volume / Page:	0322/0838
Developers Map / Lot:	PT.4&7	Census:	1801000		

#### Value Information

	Appraised Value	Assessed Value
Land	114,000	79,800
Buildings	587,740	411,420

	Appraised Value	Assessed Value
Detached Outbuildings	54,682	38,280
Total	756,422	529,500

#### **Owner's Information**

#### Owner's Data

VIGLIONE STEPHEN J 259 COMMERCE ST EAST HAVEN CT 06512

Back To Search (JavaScript:window.history.back(1);)

Print View (PrintPage.aspx?towncode=044&uniqueid=V0098600)

Information Published With Permission From The Assessor

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#### TOWN of EAST HAVEN ASSESSOR



Information on the Property Records for the Municipality of East Haven was last updated on 4/12/2019.

#### **Property Summary Information**

Parcel Data And Values

Building •

Outbuildings

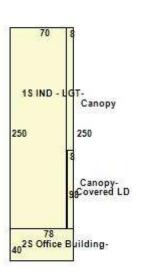
Sales

Permits

#### Building 1



(Images/Towns/EastHavenWeb/Pictures/V0098600-01.JPG)



#### (Images/Towns/EastHavenWeb/Sketches/V0098600\_01.jpg)

Category:	Industrial	Use:	Light Manu	GLA:	23,740
Stories:	1.00	Construction:	Masonry and Wood Frame	Year Built:	1956
Heating:	FHA	Fuel:	Gas	Cooling Percent:	20
Siding:	Concrete Block/B. V. Solid	Roof Material:		Beds/Units:	0

#### **Special Features**

Wet Sprinklers	3160
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#### **Attached Components**

Туре:	Year Built:	Area:
Canopy	1984	2,078
Covered Loading Dock	1984	783

Back To Search (JavaScript:window.history.back(1);)

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The Assessor's office is responsible for the maintenance of records on the ownership of properties. Assessments are computed at 70% of the estimated market value of real property at the time of the last revaluation which was 2016.



#### TOWN of EAST HAVEN ASSESSOR



Information on the Property Records for the Municipality of East Haven was last updated on 4/12/2019.

#### **Property Summary Information**

Parcel Data And Values

Building 🔻

Outbuildings

Sales

Permits

#### Building 2



(Images/Towns/EastHavenWeb/Pictures/V0098600-02.JPG)



(Images/Towns/EastHavenWeb/Sketches/V0098600\_02.jpg)

Category:	Cell Tower	Use:	Cell Site	GLA:	5
Stories:	0.00	Construction:	Metal	Year Built:	2011
Heating:		Fuel:		Cooling Percent:	0
Siding:		Roof Material:		Beds/Units:	1

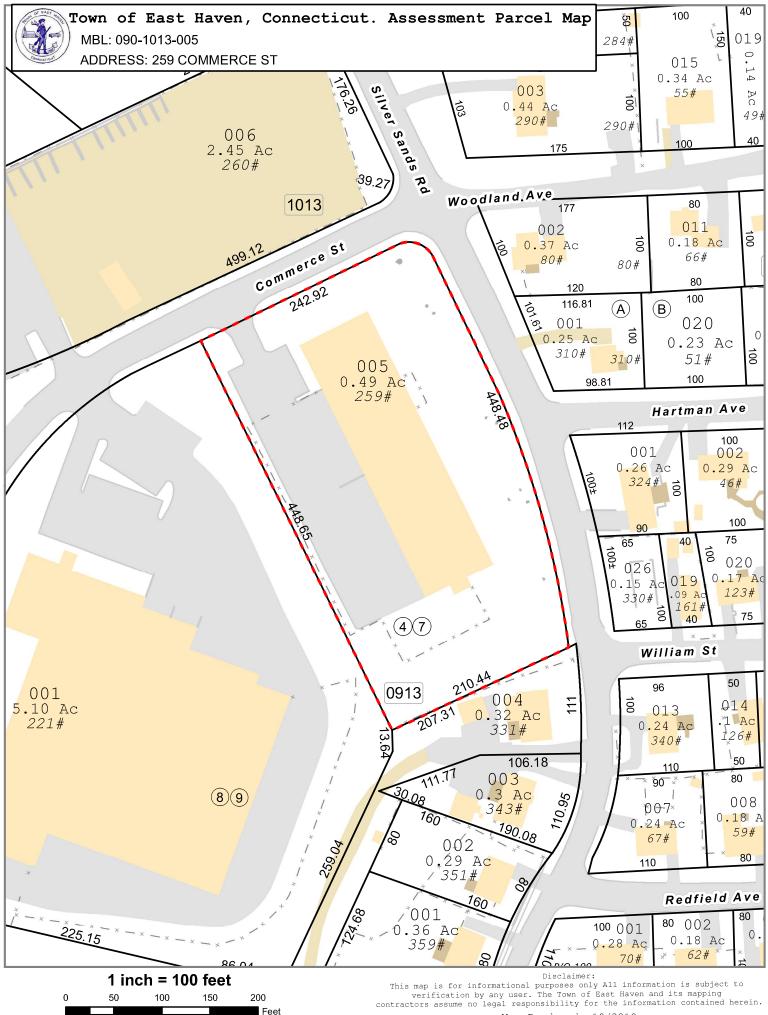
#### **Special Features**

#### **Attached Components**

Back To Search (JavaScript:window.history.back(1);)

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verification by any user. The Town of East Haven and its mapping contractors assume no legal responsibility for the information contained herein.

# Exhibit C

**Construction Drawings** 

# T--Mobile---

T-MOBILE SITE NUMBER: CT11623B

T-MOBILE SITE NAME: CT623/E. HAVEN ATT\_MP

T-MOBILE PROJECT: **ANCHOR**  **BUSINESS UNIT #:** 

**SITE ADDRESS:** 

**COUNTY:** 

SITE TYPE: **TOWER HEIGHT:**  842862

259 COMMERCE STREET EAST HAVEN, CT 06512

**NEW HAVEN MONOPOLE** 

59'-0"

#### **SITE INFORMATION**

EAST HAVEN SOUTH

CROWN CASTLE USA INC.

SITE NAME:

SITE ADDRESS: 259 COMMERCE STREET EAST HAVEN, CT 06512

COUNTY: NEW HAVEN MAP/PARCEL #:

0901013005 AREA OF CONSTRUCTION: EXISTING 41° 15' 22.88" N LATITUDE: LONGITUDE 72° 52' 32.80" W LAT/LONG TYPE: NAD83 GROUND ELEVATION:

CURRENT ZONING: EXEMPT NEW HAVEN COUNTY **IURISDICTION:** 

OCCUPANCY CLASSIFICATION: U TYPE OF CONSTRUCTION:

FACILITY IS UNMANNED AND NOT FOR A.D.A. COMPLIANCE: HUMAN HABITATION

STEPHEN I VIGLIONE PROPERTY OWNER:

259 COMMERCE STREET EAST HAVEN, CT 06512

TOWER OWNER:

A&E FIRM:

CONTACTS:

CROWN CASTLE

USA INC. DISTRICT

CROWN CASTLE 2000 CORPORATE DRIVE CANONSBURG, PA 15317

CARRIER/APPLICANT:

12920 SE 38TH STREET BELLEVUE, WA 98006

PROJECT TEAM

3875 EMBASSY PKWY, SUITE 280

KEVIN.CLEMENTS@KIMLEY-HORN.COM

CATHERINE COVINGTON - PROJECT MANAGER CATHERINE.COVINGTON@CROWNCASTLE.COM

3 CORPORATE PARK DRIVE, SUITE 101

AKRON, OH 44333

CLIFTON PARK, NY 12065

KIMLEY-HORN & ASSOCIATES, INC. COA: PEC.0000738

ELECTRIC PROVIDER: UNITED ILLUMINATING COMPANY

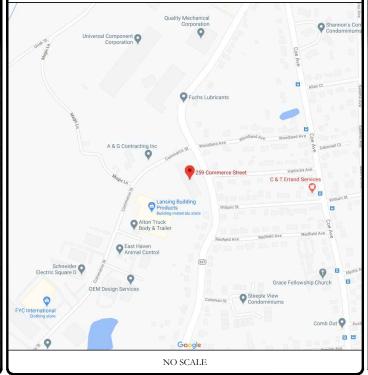
(800) 722-5584

TELCO PROVIDER: АТ&Т (800) 822-2020

#### **DRAWING INDEX**

SHEET#	SHEET DESCRIPTION	
T-1	TITLE SHEET	
T-2	GENERAL NOTES	
C-1.1	OVERALL SITE PLAN	
C-1.2	EXISTING EQUIPMENT PLAN	
C-1.3	FINAL EQUIPMENT PLAN	
C-2	TOWER ELEVATION & ANTENNA PLANS	Ш
C-3	FINAL EQUIPMENT SCHEDULE	
C-4	EQUIPMENT SPECS	11
C-5	EQUIPMENT SPECS	11
C-6	EQUIPMENT SPECS	11
E-1	PANEL SCHEDULE & ONE-LINE DIAGRAM	
G-1	GROUNDING DETAILS	Ш
		11
		11
		11
		11
		FYO
		1.15

#### **LOCATION MAP**



#### **SITE PHOTO**



#### **SCOPING NOTES**

- GROUND SCOPE OF WORK:
- REMOVE (1) 6201 CABINET
- REMOVE (6) RUS01 B12s
- REMOVE (3) RUS01 B2s • REMOVE (6) RUS01 B4s
- REMOVE (12) COAX CABLES
- INSTALL (1) 6160 SSC CABINET
- INSTALL (1) B160 BATTERY CABINET
- INSTALL (1) BB 6630 BASEBAND RADIOS • INSTALL (1) BB 6648 BASEBAND RADIO
- INSTALL (1) PSU 4813 INSTALL (2) HYBRID CABLES TOWER SCOPE OF WORK:
- REMOVE (6) TMAs
- REMOVE (3) ANTENNAS
- INSTALL (6) ANTENNAS
- INSTALL (3) RRH'S
- ALL DRAWINGS CONTAINED HEREIN ARE FORMATTED FOR 11X17. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME
- PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE CROWN NOC AT (800) 788-7011 & CROWN CONSTRUCTION MANAGER.



#### APPLICABLE CODES/REFERENCE DOCUMENTS

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES

BUILDING MECHANICAL ELECTRICAL

#### REFERENCE DOCUMENTS:

STRUCTURAL ANALYSIS: B+T GROUP (IOB#: 98372,006,01)

DATED: 4/27/19

MOUNT ANALYSIS: PAUL J FORD & COMPANY (JOB#: A37520-2250.001-7190)

RFDS REVISION: 7

DATED: 09/23/20

BELLEVUE, WA 98006



421 FAYETTEVILLE ST, SUITE 600 RALEIGH, NC 27601

T-MOBILE SITE NUMBER: CT11623B

BU #: **842862** EAST HAVEN SOUTH

259 COMMERCE STREET EAST HAVEN, CT 06512

EXISTING 59'-0" MONOPOLE

Г	ISSUED FOR:						
REV	DATE	DRWN	DESCRIPTION	DES./QA			
0	11/04/20	LMS	ISSUED FOR CONSTR.	MCK			
1	11/05/20	LMS	REVISED PER CLIENT	MCK			
2	12/15/20	ECW	REVISED PER CLIENT	MCK			



12/17/20 Exp. 01/31/21

IT IS A VIOLATION OF LAW FOR ANY PERSON, INLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER,

SHEET NUMBER:

#### CROWN CASTLE USA INC. SITE ACTIVITY REQUIREMENTS:

- NOTICE TO PROCEED- NO WORK SHALL COMMENCE PRIOR TO CROWN CASTLE USA INC. WRITTEN NOTICE TO PROCEED (NTP) AND THE ISSUANCE OF A PURCHASE ORDER. PRIOR TO ACCESSING/ENTERING THE SIDE OF THE CROWN CASTLE USA INC. NOC AT 800-788-7011 & THE CROWN CASTLE USA INC. CONSTRUCTION MANAGER.
- "LOOK LIP" CROWN CASTLE LISA INC. SAFETY CLIMB REQUIREMENT: THE INTEGRITY OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR FOLLIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR REINFORCEMENTS, AND/OW EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRAT OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY ON THE STRUCTURE. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO: PINCHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM ITS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTIONAL WEAR, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO IMPEDE/BLOCK ITS INTENDED USE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGGED OUT AND REPORTED TO YOUR GROWN CASTLE USA INC. POC OR CALL THE NOC TO GENERATE A SAFETY CLIMB MAINTENANCE AND CONTRACTOR NOTICE TICKET.
- PRIOR TO THE START OF CONSTRUCTION, ALL REQUIRED JURISDICTIONAL PERMITS SHALL BE OBTAINED. THIS INCLUDES, BUT IS NOT LIMITED TO, BUILDING, ELECTRICAL, MECHANICAL, FIRE, FLOOD ZONE, ENVIRONMENTAL, AND ZONING AFTER ONSITE ACTIVITIES AND CONSTRUCTION ARE COMPLETED, ALL REQUIRED PERMITS SHALL BE SATISFIED AND CLOSED OUT ACCORDING TO LOCAL JURISDICTIONAL
- ALL CONSTRUCTION MEANS AND METHODS; INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS, AND RESCUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONTAINED HEREIN, AND SHALL MEET ANSI/ASSE A10.48 (LATEST EDITION); FEDERAL, STATE, AND LOCAL REGULATIONS; AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL ADHERE TO ANSI/ASSE A10.48 (LATEST EDITION) AND CROWN CASTLE USA INC. STANDARD CED—STD—10253, INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION, TO CERTIFY THE SUPPORTING STRUCTURE(S) IN ACCORDANCE WITH ANSI/TIA—322
- ALL SITE WORK TO COMPLY WITH CAS-STD-10068 "INSTALLATION STANDARDS FOR CONSTRUCTION ACTIVITIES ON CROWN CASTLE USA INC. TOWER SITE" AND LATEST VERSION OF ANSI/TIA-1019-A-2012
  "STANDARD FOR INSTALLATION, ALTERATION, AND MAINTENANCE OF ANTENNA SUPPORTING STRUCTURES AND
- IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION. FOR APPROVAL BY CROWN CASTLE USA INC. PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
  ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE
  CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND
  COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC
  AUTHORITY REGARDING THE PERFORMANCE OF THE WORK, ALL WORK CARRIED OUT SHALL COMPLY WIT
  ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES PRIOR TO THE START OF CONSTRUCTION.
  ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHERE ENCOUNTERED IN THE
  WORK, SHALL BE PROTECTED AT ALL TIMES AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE
  WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION E) CONSTRUCTION SAFFTY PROCEDURES
- CONSTRUCTION SAFETY PROCEDURES.

  ALL SITE WORK SHALL BE AS INDICATED ON THE STAMPED CONSTRUCTION DRAWINGS AND PROJECT SPECIFICATIONS, LATEST APPROVED REVISION.

  CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH AT THE COMPLETION OF THE WORK. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.

  ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE
- EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF CONTRACTOR, TOWER OWNER, CROWN CASTLE USA INC., AND/OR LOCAL UTILITIES.
- THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE TECHNICAL SPECIFICATION FOR SITE SIGNAGE REQUIRED BY LOCAL JURISDICTION AND SIGNAGE REQUIRED ON INDIVIDUAL PIECES OF EQUIPMENT, ROOMS, AND SHELTERS.
- 15. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE CARRIER'S EQUIPMENT
- THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
- THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION AS SPECIFIED ON THE CONSTRUCTION DRAWINGS AND/OR PROJECT SPECIFICATIONS.
- CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION FROSION CONTROL SURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES EROSION AND SEDIMENT CONTROL.
- THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION
- OF UWNER.

  20. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.

  CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED.
- FROM SITE ON A DAILY BASIS.
- 22. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.

#### **GENERAL NOTES:**

- FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:
  CONTRACTOR: GENERAL CONTRACTOR RESPONSIBLE FOR CONSTRUCTION
  CARRIER: T-MOBILE
  TOWER OWNER: CROWN CASTLE USA INC.
- IUWER OWNER: CROWN CASTLE USA INC.
  THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY
  EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE ENGINEERS IN THIS OR SIMILAR LOCALITIES. IT IS
  ASSUMED THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKPEOPLE
  WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY
  ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN
  ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR
  MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.
- MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.

  MISCELLANEOUS WORK NOT EXPLICITLY SHOWN. RECEIVED STANDARD GOOD FRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION. HE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY FOR PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, FORMWORK, SHORING, ETC. SITE VISITS BY THE ENGINEER OR HIS REPRESENTATIVE WILL NOT INCLUDE INSPECTION OF THE FINISHED STRUCTURE ONLY. NOTES AND DETAILS IN THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE GREATER, MORE STRICT REQUIREMENTS, SHALL GOVERN. IF FURTHER CLARIFICATION IS REQUIRED CONTACT THE ENGINEER OF RECORD.

  SUBSTANTIAL EFFORT HAS BEEN MADE TO PROVIDE ACCURATE DIMENSIONS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND REPORTED TO PROVIDE ACCURATE DIMENSIONS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND CREATENT OF CONSTRUCTION PLEMENTS BUT IT IS THE SOLE RESPONSIBILITY.
- ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS.
- CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.

  PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CROWN CASTLE.

  ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORPANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATION AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS. UNLESS NOTION OF THE ORDER OF THE WORK ATTENDED AND APPLICABLE REGULATIONS. UNLESS NOTION FOR MATERIALS, EQUIPMENT, APPURTENANCES AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.

- THE CONTRACTOR SHALL INSTALLAL EQUIPMENT AND MATERIALS IN A ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CARRIER AND CROWN CASTLE PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.

  CONTRACTOR IS TO PEFFORM A SITE INVESTIGATION AND IS TO DETERMINE THE BEST ROUTING OF ALL CONDUITS FOR POWER, AND TELCO AND FOR GROUNDING CABLES AS SHOWN IN THE POWER, TELCO, AND GROUNDING PLAN DRAWINGS.
- THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEM<mark>ENTS, PAVEMENTS, CURBS, LANDS</mark>CAPING AND STRUCTURES, ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF GROWN CASTLE USA INC. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S
- DESIGNATED LOCATION CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON

#### CONCRETE, FOUNDATIONS, AND REINFORCING STEEL:

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST—IN—PLACE CONCRETE.
  UNLESS NOTED OTHERWISE, SOIL BEARING PRESSURE USED FOR DESIGN OF SLABS AND FOUNDATIONS IS ASSUMED
- ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f'c) OF 3000 psi AT 28 DAYS, UNLESS NOTED OTHERWISE. NO MORE THAN 90 MINUTES SHALL ELAPSE FROM BATCH TIME TO TIME OF PLACEMENT UNLESS APPROVED BY THE ENGINEER OF RECORD. TEMPERATURE OF CONCRETE SHALL NOT EXCEED 90'F AT TIME OF PLACEMENT.
- CONCRETE EXPOSED TO FREEZE—THAW CYCLES SHALL CONTAIN AIR ENTRAINING ADMIXTURES. AMOUNT OF AIR CONCRETE EXPOSED TO FREEZE-THAN TOLCES STALL CONTINUA AIR ENTRAINING ADMIXTURES. AMOUNT OF AIR ENTRAINMENT TO BE BASED ON SIZE OF AGGREGATE AND F3 CLASS EXPOSURE (VERY SEVERE). CEMENT USED TO BE TYPE II PORTLAND CEMENT WITH A MAXIMUM WATER-TO-CEMENT RATIO (W/C) OF 0.45. ALL STEEL REINFORCING SHALL CONFORM TO ASTM A615. ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185. ALL SPLICES SHALL BE CLASS "B" TENSION SPLICES, UNLESS NOTED OTHERWISE. ALL HOOKS SHALL BE
- TANDARD 90 DEGREE HOOKS, UNLESS NOTED OTHERWISE. YIELD STRENGTH (Fy) OF STANDARD DEFORMED BARS ARE AS FOLLOWS: #4 BARS AND SMALLER
- ## 5 BARS AND LARGER #5 BARS AND LARGER THE FOLLOWING MINIMUM, CONCRETE COVER. SHALL, BE, PROVIDED, FOR REINFORGING, STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:
- CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH CONCRETE EXPOSED TO EARTH OR WEATHER: #6 BARS AND LARGER 1-1/2" #5 BARS AND SMALLER
- CONCRETE NOT EXPOSED TO, EARTH, OR WEATHER: . . 3/4". . . . . . . . . . . . . . . BEAMS AND COLUMNS

#### GREENFIELD GROUNDING NOTES:

- ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION AND AC POWER GES'S) SHALL BE BONDED TOGETHER AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN
- THE CONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS, THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.

  THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT AND PROVIDE
- METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.

  CLAMPS.

  CLAMPS.

  METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.

  WITH THE POWER CIRCUITS TO BTS EQUIPMENT.

  FOR INDUCTOR AND THE WASTER COOLING BAD WITH CREEN INSULATED SLIPPIEMENTAL FOLLIPPIEMENT GROUND WIRES. #6 STRANDED COPPER OR LARGER FOR INDOOR BTS: #2 BARE SOLID TINNED
- EACH CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 STRANDED COPPER OR LARGER FOR INDOOR BTS; #2 BARE SOLID TINNED COPPER FOR OUTDOOR BTS. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED BACK TO BACK CONNECTIONS ON OPPOSITE SIDE OF THE GROUND BUS ARE PERMITTED.
- ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BASS AND THE GROUND RING SHALL BE #2 SOLID TINNED COPPER UNLESS OTHERWISE INDICATED.
  ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
  USE OF 90' BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45' BENDS CAN BE ADEQUATELY SUPPORTED.
  EXCITHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.

- ALL GROUND CONNECTIONS ABOVE GRADE (INTERIOR AND EXTERIOR) SHALL BE FORMED USING HIGH PRESS CRIMPS.

  COMPRESSION GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.

  ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR.
- APPROVED ANTIOXIDANT COATINGS (i.e. CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL
- ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.

  MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.

  BOND ALL METALLIC OBJECTS WITHIN 6 ft OF MAIN GROUND RING WITH (1) #2 BARE SOLID TINNED COPPER GROUND CONDUCTOR.

  GROUND CONDUCTORS USED FOR THE FACILITY GROUNDING AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC CONDUIT SHALL BE
- USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (i.e., NONMETALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT.

  ALL GROUNDS THAT TRANSITION FROM BELOW GRADE TO ABOVE GRADE MUST BE #2 BARE SOLID TINNED COPPER IN 3/4" NON-METALLIC, FLEXIBLE CONDUIT FROM 24" BELOW GRADE TO WITHIN 3" TO 6" OF CAD-WELD TERMINATION POINT. THE EXPOSED END OF THE CONDUIT MUST BE SEALED WITH SLICONE CAULK. (ADD TRANSITIONING GROUND STANDARD DETAIL AS WELL).

  BUILDINGS WHERE THE MAIN GROUNDING CONDUCTORS ARE REQUIRED TO BE ROUTED TO GRADE, THE CONTRACTOR SHALL ROUTE TWO GROUNDING CONDUCTORS FROM THE ROOFTOP, TOWERS, AND WATER TOWERS GROUNDING RING, TO
- THE EXISTING GROUNDING SYSTEM, THE GROUNDING CONDUCTORS SHALL NOT BE SMALLER THAN 2/0 COPPER. ROOFTOP GROUNDING RING SHALL BE BONDED TO THE EXISTING GROUNDING SYSTEM, THE BUILDING STEEL COLUMNS LIGHTNING PROTECTION SYSTEM, AND BUILDING MAIN WATER LINE (FERROUS OR NONFERROUS METAL PIPING ONLY).

#### **ELECTRICAL INSTALLATION NOTES:**

- ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE
- FEDERAL, STATE, AND LOCAL CODES/ORDINANCES.
  CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED
- AND TRIP HAZARDS ARE FLIMINATED

- AND TRIP HAZARDS ARE ELIMINATED.

  WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC.

  ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC.

  1. ALL EQUIPMENT SHALL BEAR THE UNDERWRITERS LABORATORIES LABEL OF APPROVAL, AND SHALL CONFORM TO REQUIREMENT OF THE NATIONAL ELECTRICAL CODE.

  2. ALL OVERCURRENT DEVICES SHALL HAVE AN INTERRUPTING CURRENT RATING THAT SHALL BE GREATER THAN THE SHORT CIRCUIT CURRENT TO WHICH THEY ARE SUBJECTED, 22,000 AIC MINIMUM. VERYIPY AVAILABLE SHORT CIRCUIT CURRENT DOES NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRENT APPORTER COOR DEET THE COVERNING HISPORTICAL.
- NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRY ADOPTED CODE PRE THE GOVERNING JURISDICTION.

  EACH END OF EVERY POWER PHASE CONDUCTOR, GROUNDING CONDUCTOR, AND TELCO CONDUCTOR OR CABLE SHALL BE LABELED WITH COLOR—CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2" PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA.

  ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH LAMICOID TAGS SHOWING THEIR RATED VOLTAGE, PHASE
- CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING AND BRANCH CIRCUIT ID NUMBERS (i.e. PANEL BOARD AND CIRCUIT ID'S).

- PANEL BOARDS (ID NUMBERS) SHALL BE CLEARLY LABELED WITH PLASTIC LABELS.
  ALL TIE WRAPS SHALL BE CUT FLUSH WITH APPROVED CUTTING TOOL TO REMOVE SHARP EDGES.
  ALL POWER AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE COPPER CONDUCTOR (#14 OR LARGER)
  WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE COPPER CONDUCTOR (#6 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.

  POWER AND CONTROL WIRING IN FLEXIBLE CORD SHALL BE MULTI-CONDUCTOR, TYPE SOOW CORD (#14 OR LARGER) UNLESS

- POWER AND CONTROL WIRING FOR USE IN CABLE TRAY SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#14 OR LARGER), WITH TYPE TIHM, THWN, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED. ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRE NUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRE NUTS SHALL BE RATED FOR OPERATION NOT LESS THAN 75 C (90° C IF AVAILABLE).
- RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL. ANSI/JEEE
- 15. ELECTRICAL METALLIC TUBING (EMT), INTERMEDIATE METAL CONDUIT (IMC), OR RIGID METAL CONDUIT (RMC) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.
- ELECTRICAL METALLIC TUBING (EMT) OR METAL—CLAD CABLE (MC) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.

  SCHEDULE 40 PVC UNDERGROUND ON STRAIGHTS AND SCHEDULE 80 PVC FOR ALL ELBOWS/90s AND ALL APPROVED ABOVE
- 18. LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION
- COURS OR FLEXIBILITY IS NEEDED.

  CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION—TYPE AND APPROVED FOR THE LOCATION USED. SET SCREW FITTINGS ARE NOT ACCEPTABLE.
- CABINETS, BOXES AND WIRE WAYS SHALL BE LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND 20.
- WIREWAYS SHALL BE METAL WITH AN ENAMEL FINISH AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARDS
- . WIREWAYS SHALL BE METAL WITH AN ENAMEL FINISH AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARDS (WIREWOLD SPECMATE WIREWAY).

  SLOTTED WIRING DUCT SHALL BE PVC AND INCLUDE COVER (PANDUIT TYPE E OR EQUAL).

  CONDUITS SHALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-PERFORATED STRAPS AND HANGERS. EXPLOSIVE DEVICES (i.e. POWDER-ACTUATED) FOR ATTACHING HANGERS TO STRUCTURE WILL NOT BE PERMITTED. CLOSELY FOLLOW THE LINES OF THE STRUCTURE, MAINTAIN CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES. CHANGES IN DIRECTION TO ROUTE AROUND OBSTACLES SHALL BE MEADE WITH CONDUIT OUTLET BODIES, CONDUIT SHALL BE RISTALLED IN A NEAT AND WORKMANLIKE MANNER. PARALLEL AND PERPENDICULAR TO STRUCTURE WALL AND CEILING LINES. ALL CONDUIT SHALL BE FISHED TO CLEAR OBSTRUCTIONS. ENDS OF CONDUITS SHALL BE RIGHORARILY CAPPED FLUSH TO FINISH GRADE TO PREVENT CONCRETE, PLASTER OR DIRT FROM ENTERING, CONDUITS SHALL BE RIGHOLY CLAMPED TO BOXES BY GALVANIZED MALLEABLE IRON BUSHING ON INSIDE AND GALVANIZED MALLEABLE RON OUTSIDE AND INSIDE.

  EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES AND PULL BOXES SHALL BE GALVANIZED OR EPOXY—COATED SHEET STEEL. SHALL MEET OR EXCEED UL 50 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND NEMA 3R (OR BETTER) FOR EXTERIOR LOCATIONS.
- BETTER) FOR EXTERIOR LOCATIONS. METAL RECEPTACLE, SWITCH AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY-COATED OR NON-CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- (WP OR BETTER) FOR EXTERIOR LOCATIONS.

  NONMETALLIC RECEPTACLE, SWITCH AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2 (NEWEST REVISION) AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.

  THE CONTRACTOR SHALL NOTIFY AND GETAIN NECESSARY AUTHORIZATION FROM THE CARRIER AND/OR CROWN CASTLE USA INC. BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.

  THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD LIFE AND PROPERTY.

  INSTALL LAMICOID LABEL ON THE METER CENTER TO SHOW "T—MOBILE". 27. 28.
- 29.
- 30. ALL EMPTY/SPARE CONDUITS THAT ARE INSTALLED ARE TO HAVE A METERED MULE TAPE PULL CORD INSTALLED.

00115	001011 002	011 0002	
SYSTEM	CONDUCTOR	COLOR	
	A PHASE	BLACK	
120/240V, 1Ø	B PHASE	RED	
120/2704, 10	NEUTRAL	WHITE	
	GROUND	GREEN	
	A PHASE	BLACK	
	B PHASE	RED	
120/208V, 3Ø	C PHASE	BLUE	
	NEUTRAL	WHITE	
	GROUND	GREEN	
	A PHASE	BROWN	
	B PHASE	ORANGE OR PURPLE	
277/480V, 3Ø	C PHASE	YELLOW	
	NEUTRAL	GREY	
	GROUND	GREEN	
DC VOLTAGE	POS (+)	RED**	
	NEG (-)	BLACK**	
* SEE NEC 210.5	5(C)(1) AND	(2)	

CONDUCTOR COLOR CODE

\*\* POLARITY MARKED AT TERMINATION

YELLOW GAS, OIL, STEAM, PETROLEUM, OR GASEOUS MATERIALS

#### **ABBREVIATIONS:**

UMTS W P

ΔΝΤΕΝΝΔ EXISTING FACILITY INTERFACE FRAME GENERATOR GLOBAL POSITIONING SYSTEM GEN GPS GSM LTE MGB MW (N) NEC GLOBAL SYSTEM FOR MOBILE LONG TERM EVOLUTION MASTER GROUND BAR MICROWAVE NATIONAL ELECTRIC CODE (P) PROPOSED POWER PLANT QTY RECT QUANTITY RECTIFIER RADIO BASE STATION RADIO BASE STATION
REMOTE ELECTRIC TILT
RADIO FREQUENCY DATA SHEET
REMOTE RADIO HEAD
REMOTE RADIO UNIT
SMART INTEGRATED DEVICE
TOWER NOUNTED AND INTER TOWER MOUNTED AMPLIFIER UNIVERSAL MOBILE TELECOMMUNICATIONS SYSTEM

#### APWA UNIFORM COLOR CODE:

WHITE PROPOSED EXCAVATION TEMPORARY SURVEY MARKINGS ELECTRIC POWER LINES, CABLES, CONDUIT, AND LIGHTING CABLES

COMMUNICATION, ALARM OR SIGNAL LINES, CABLES, OR CONDUIT AND TRAFFIC LOOPS POTABLE WATER

RECLAIMED WATER, IRRIGATION, AND

SLURRY LINES SEWERS AND DRAIN LINES

# 12920 SE 38TH STREET



BELLEVUE, WA 98006



421 FAYETTEVILLE ST, SUITE 600

RALEIGH, NC 27601

CLIFTON PARK, NY 12065

T-MOBILE SITE NUMBER: CT11623B

BU #: **842862** EAST HAVEN SOUTH

259 COMMERCE STREET EAST HAVEN, CT 06512

EXISTING 59'-0" MONOPOLE

Ε		ISSUED FOR:								
	REV	DATE	DRWN	DESCRIPTION	DES./Q					
	0	11/04/20	LMS	ISSUED FOR CONSTR.	MCK					
	1	11/05/20	LMS	REVISED PER CLIENT	MCK					
	2	12/15/20	ECW	REVISED PER CLIENT	MCK					



12/17/20 Exp. 01/31/21

IT IS A VIOLATION OF LAW FOR ANY PERSON UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER. TO ALTER THIS DOCUMENT

SHEET NUMBER:

#### SITE PLAN DISCLAIMER:

PROPERTY LINES AND STRUCTURES HAVE BEEN DIGITIZED FROM PREVIOUS PLAN SETS, CROWN CASTLE USA INC. HAS NOT COMPLETED A SITE SURVEY AND THEREFORE MAKES NO CLAIMS AS TO THE ACCURACY OF INFORMATION DEPICTED ON THIS SHEET.



12920 SE 38TH STREET BELLEVUE, WA 98006



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EXISTING 59'-0" MONOPOLE

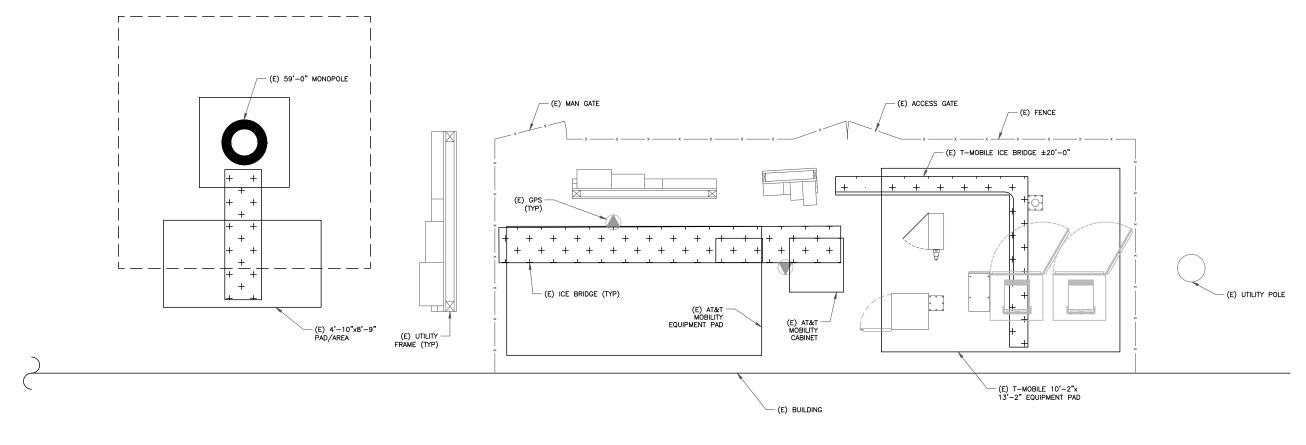
lſ	ISSUED FOR:							
REV	DATE	DRWN	DESCRIPTION	DES./QA				
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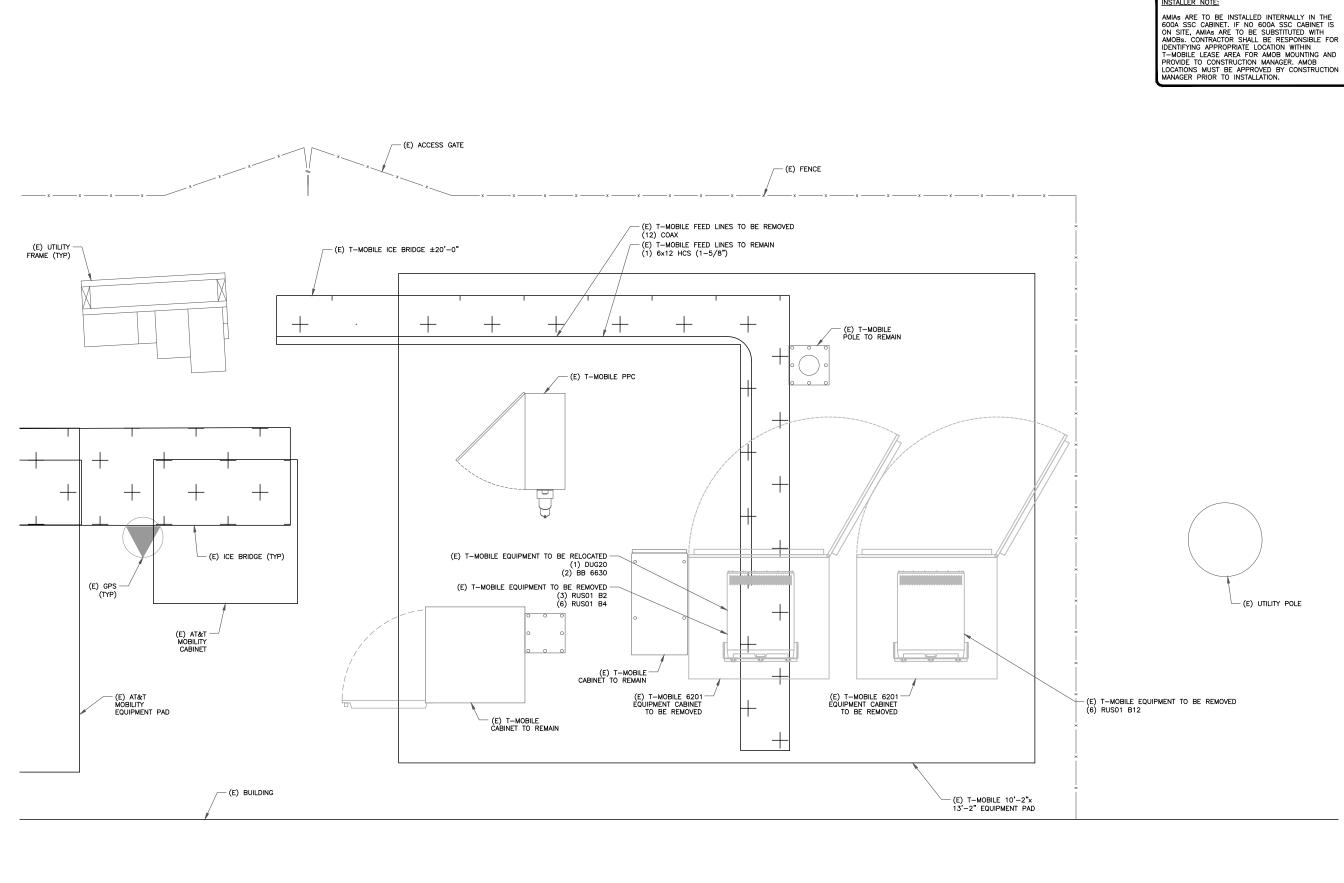
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EXISTING EQUIPMENT PLAN

#### INSTALLER NOTE:



12920 SE 38TH STREET BELLEVUE, WA 98006



CLIFTON PARK, NY 12065

421 FAYETTEVILLE ST, SUITE 600 RALEIGH, NC 27601

T-MOBILE SITE NUMBER: CT11623B

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1	11/05/20	LMS	REVISED PER CLIENT	MCK					
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12/17/20 Exp. 01/31/21

REVISION:

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SHEET NUMBER:



AMIAS ARE TO BE INSTALLED INTERNALLY IN THE 600A SSC CABINET. IF NO 600A SSC CABINET IS ON SITE, AMIAS ARE TO BE SUBSTITUTED WITH AMOBS. CONTRACTOR SHALL BE RESPONSIBLE FOR IDENTIFYING APPROPRIATE LOCATION WITHIN T-MOBILE LEASE AREA FOR AMO MOUNTING AND PROVIDE TO CONSTRUCTION MANAGER. AMOB LOCATIONS MUST BE APPROVED BY CONSTRUCTION MANAGER PRIOR TO INSTALLATION.



12920 SE 38TH STREET BELLEVUE, WA 98006



CLIFTON PARK, NY 12065

### Kimley»Horn

421 FAYETTEVILLE ST, SUITE 600 RALEIGH, NC 27601

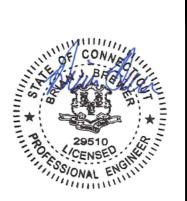
T-MOBILE SITE NUMBER: CT11623B

BU #: **842862** EAST HAVEN SOUTH

259 COMMERCE STREET EAST HAVEN, CT 06512

EXISTING 59'-0" MONOPOLE

ſ	ISSUED FOR:								
REV	DATE	DRWN	DESCRIPTION	DES./QA					
0	11/04/20	LMS	ISSUED FOR CONSTR.	MCK					
1	11/05/20	LMS	REVISED PER CLIENT	MCK					
2	12/15/20	ECW	REVISED PER CLIENT	MCK					

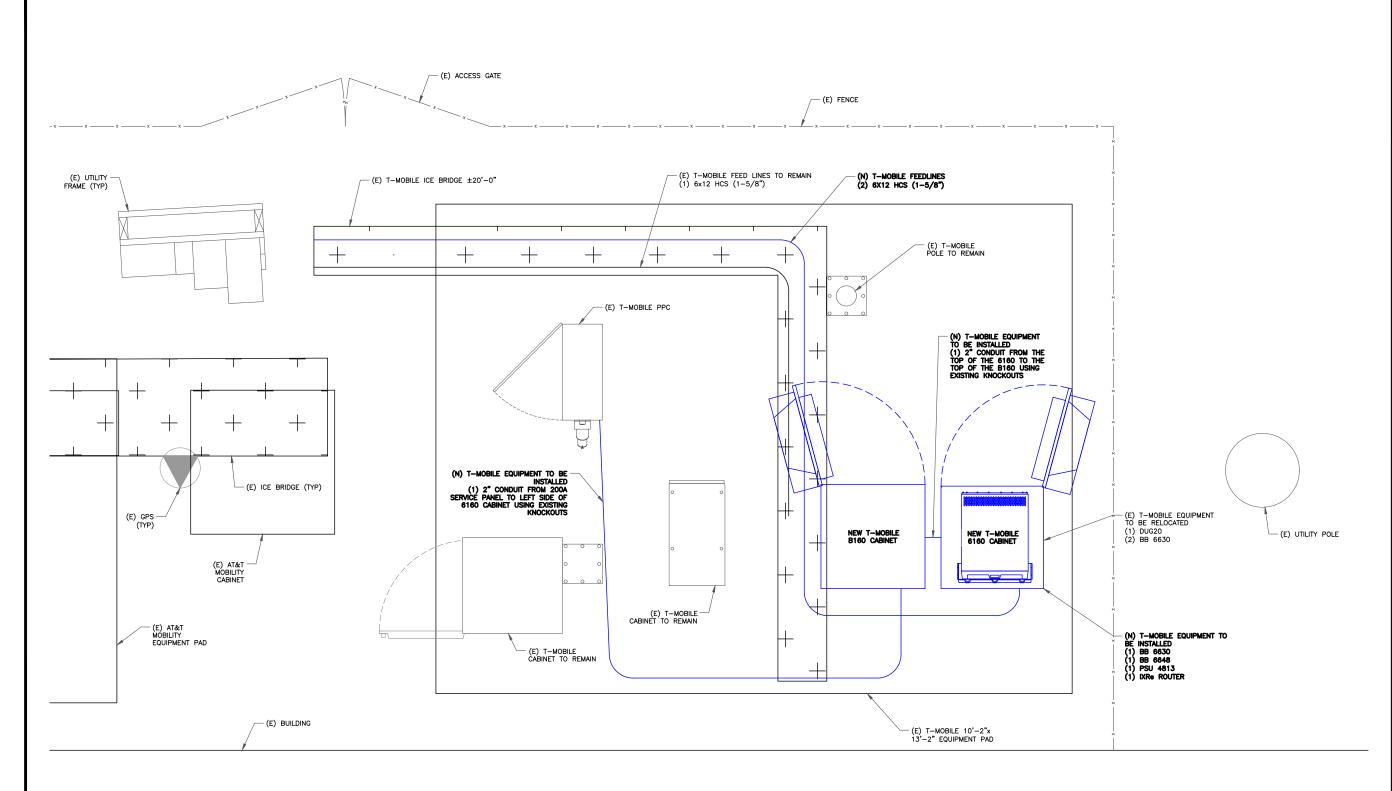


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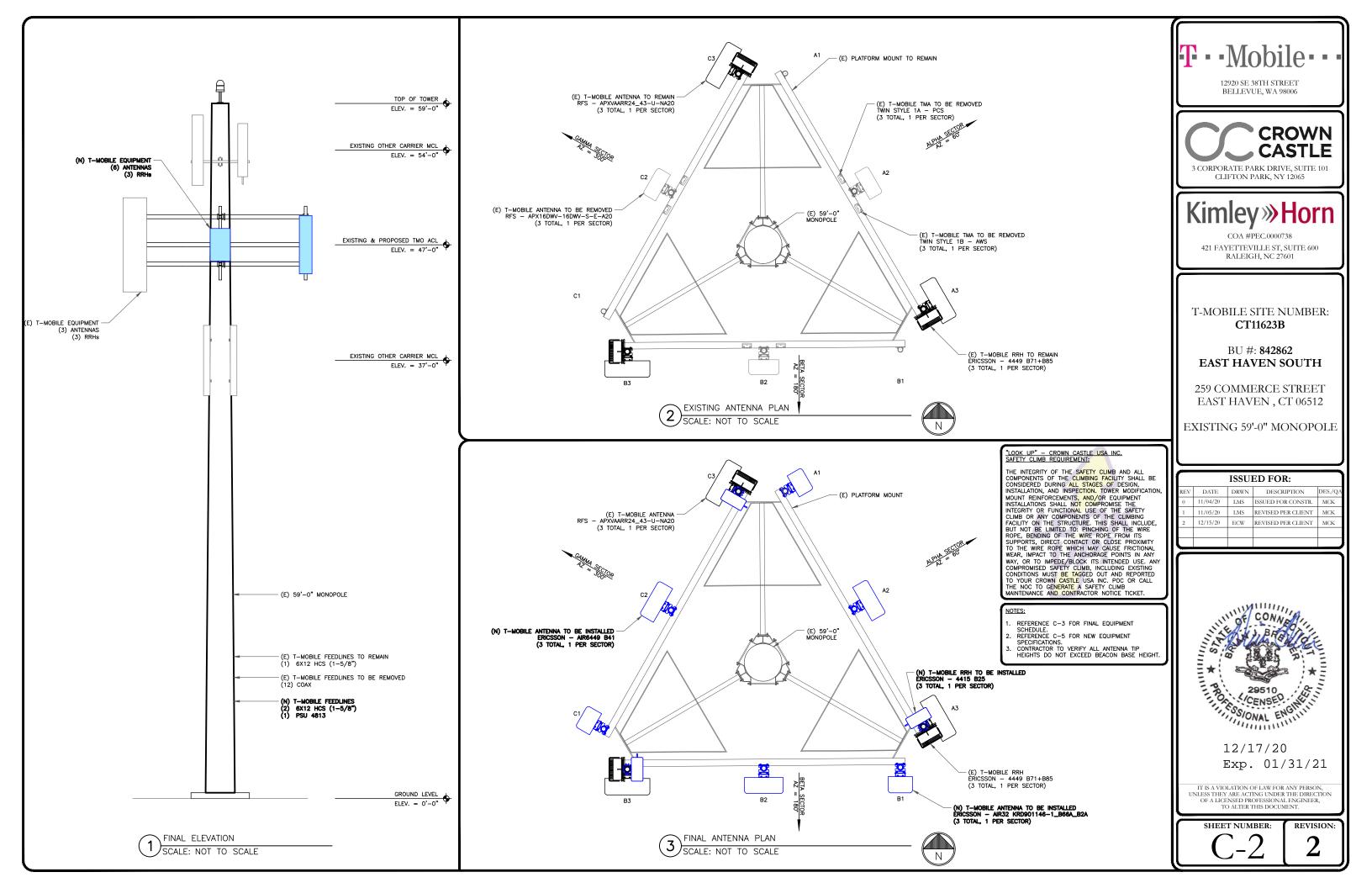
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FINAL EQUIPMENT PLAN



ALPHA	FINAL EQUIPMENT SCHEDULE  (VERIFY WITH CURRENT RFDS)																	
7.2.131		ANTENNA				RADIO			DIPLEXER		TMA SURGE PROTECTION			SURGE PROTECTION	CABLES			
POSITION	TECH.	STATUS/MANUFACTURER MODEL	AZIMUTH	RAD CENTER	Œ.	STATUS/MODEL	LOCATION	QTY.	STATUS	LOCATION	QTY.	STATUS	QTY.	STATUS/MODEL	QTY.	STATUS/TYPE	SIZE	LENGTH
A1	L2100/L1900/G1900	(N) ERICSSON — AIR32 KRD901146—1_B66A_B2A	60°	47'-0"	-	-	-	-	-	_	-	-	-	-	1	(N) HYBRID	1-5/8"	97'-0"
A2	L2500/N2500	(N) ERICSSON - AIR6449 B41	60*	47'-0"	1	-	-	-	ı	-	ı	_	-	-	1	(N) HYBRID	1-5/8"	97'-0"
A3	L700/L600/N600/ L1900	(E) RFS — APXVAARR24_43-U-NA20	60°	47'-0"	1 1	(N) 4415 B25 (E) 4449 B71+B85	TOWER	-	ı	_	1	_	-	-	1	(E) HYBRID	1-5/8"	97'-0"
BETA	DETA																	
B1	L2100/L1900/G1900	(N) ERICSSON – AIR32 KRD901146–1_B66A_B2A	180°	47'-0"	ı	-	-	1	ı	_	1	_	-	-				
82	L2500/N2500	(N) ERICSSON - AIR6449 B41	180°	47'-0"	1	-	-	-	-	-	-	-	-	-		SHARI	ED	
B3	L700/L600/N600/ L1900	(E) RFS - APXVAARR24_43-U-NA20	180°	47'-0"	<b>1</b>	(N) 4415 B25 (E) 4449 B71+B85	TOWER	-	-	-	-	-	-	-				
GAMMA												•						
C1	L2100/L1900/G1900	(N) ERICSSON — AIR32 KRD901146—1_B66A_B2A	300°	47°-0°	1	-	-	-	-	-	-	-	-	-				
C2	L2500/N2500	(N) ERICSSON - AIR6449 B41	300°	47'-0"	-	-	-	-	-	_	-	-	-	-		SHARI	ED	
C3	L700/L600/N600/ L1900	(E) RFS - APXVAARR24_43-U-NA20	300	47'-0"	1 1	(N) 4415 B25 (E) 4449 B71+B85	TOWER	-	-	-	-	-	-	-				
<u>NOTE:</u> (E) — E: (N) — N	XISTING EW											•			0	(E) UNUSED	-	-

T - Mobile - - 
12920 SE 38TH STREET
BELLEVUE, WA 98006





COA #PEC.0000/38 421 FAYETTEVILLE ST, SUITE 600 RALEIGH, NC 27601

T-MOBILE SITE NUMBER: **CT11623B** 

BU #: **842862** EAST HAVEN SOUTH

259 COMMERCE STREET EAST HAVEN, CT 06512

EXISTING 59'-0" MONOPOLE

ISSUED FOR:								
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1	11/05/20	LMS	REVISED PER CLIENT	MCK				
2	12/15/20	ECW	REVISED PER CLIENT	MCK				



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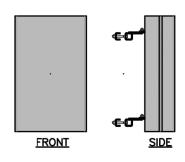
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 $\binom{-3}{}$ 

L25+N25 ANTENNA								
DIMENSIONS								
MODEL #	AIR 6449 B41							
MANUF.	ERICSSON							
HEIGHT	33.1"							
WIDTH	20.5"							
DEPTH	8.5"							
WEIGHT	103 LBS							





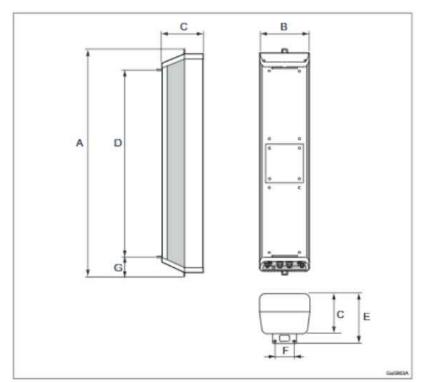


Figure 6 AIR Unit Dimensions

Table 7 AIR Unit Dimensions

AIR Unit Type	Height (A)	Width (B)	Depth (C)	Distance between mounting brackets (D)	Depth includin g mountin g brackets (E)	Distance between stud holes on mountin g brackets (F)	Distance between lower mountin g bracket and the lower end of the AIR unit (G)
AIR 32 B4A B2P (1.4 m unit)	1505 mm	327 mm	220 mm	1249 mm	271 mm	110 mm	128 mm
AIR 32 B7A HBP LBP (2.0 m unit)	2014 mm	389 mm	284 mm	1752 mm	335 mm	110 mm	131 mm
AIR 32 B7AA HBP LBP (2.0 m unit)	2014 mm	389 mm	284 mm	1752 mm	335 mm	110 mm	131 mm
AIR 32 B2A B56AA (1.4 m unit)	1505 mm	327 mm	220 mm	1249 mm	271 mm	110 mm	128 mm
AIR 32 866AA 87P LBP (2.0 m unit)	2014 mm	389 mm	284 mm	1752 mm	335 mm	110 mm	131 mm

ANTENNA SPECIFICATIONS
SCALE: NOT TO SCALE

12920 SE 38TH STREET BELLEVUE, WA 98006



## Kimley » Horn

421 FAYETTEVILLE ST, SUITE 600 RALEIGH, NC 27601

T-MOBILE SITE NUMBER: CT11623B

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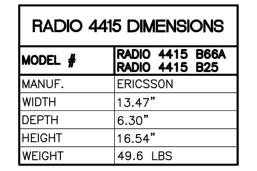
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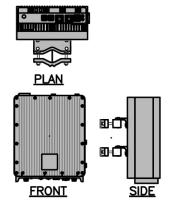
SHEET NUMBER:

REVISION:

NOT USED
SCALE: NOT TO SCALE

ANTENNA SPECIFICATIONS SCALE: NOT TO SCALE

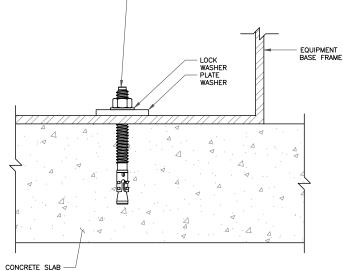




RADIO 4415 B25 DETAIL
SCALE: NOT TO SCALE

NOT USED
SCALE: NOT TO SCALE

PROPOSED 1/2" DIAMETER, SIMPSON STRONG—TIE STRONG—BOLT 2 WEDGE ANCHOR WITH 3-3/4" NOMINAL EMBEDMENT (ICC ESR-3037). TO BE INSTALLED A MINIMUM OF (1) IN EACH CORNER OF CABINET. SEE MANDEACTURER'S EQUIPMENT INSTALLATION MANUAL FOR ANCHOR LOCATIONS. IT IS ASSUMED THAT CABINET ANCHOR LOCATIONS HAVE BEEN DESIGNED/REINFORCED BY THE MANUFACTURET OW WITHSTAND SHEAR AND UPLIFT FORCES AS CALCULATED IN THIS DESIGN



CABINET ANCHOR DETAIL SCALE: NOT TO SCALE

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259 COMMERCE STREET EAST HAVEN, CT 06512

EXISTING 59'-0" MONOPOLE

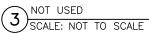
	ISSUED FOR:								
REV	DATE	DRWN	DESCRIPTION	DES./QA					
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1	11/05/20	LMS	REVISED PER CLIENT	MCK					
2	12/15/20	ECW	REVISED PER CLIENT	MCK					



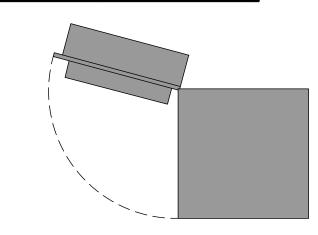
12/17/20 Exp. 01/31/21

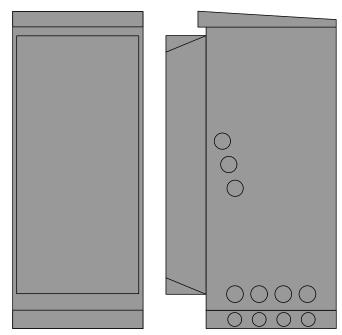
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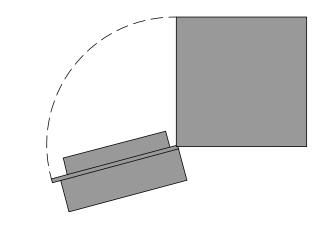


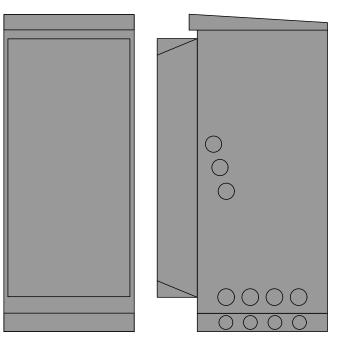
CABINET DIMENSIONS							
MODEL #	6160 SITE SUPPORT CABINET						
MANUF.	ERICSSON						
HEIGHT	63"						
WIDTH	25.6"						
DEPTH (W/ DOOR)	33.5"						
WEIGHT	1500 LBS MAX						
(INSTALL PER MANUINSTALLATION GUIDE							





CABINET DIMENSIONS							
MODEL #	B160 BATTERY CABINET						
MANUF.	ERICSSON						
HEIGHT	63"						
WIDTH	26"						
DEPTH (W/ DOOR)	34"						
WEIGHT	2000 LBS MAX						
(INSTALL PER MANUINSTALLATION GUIDE							











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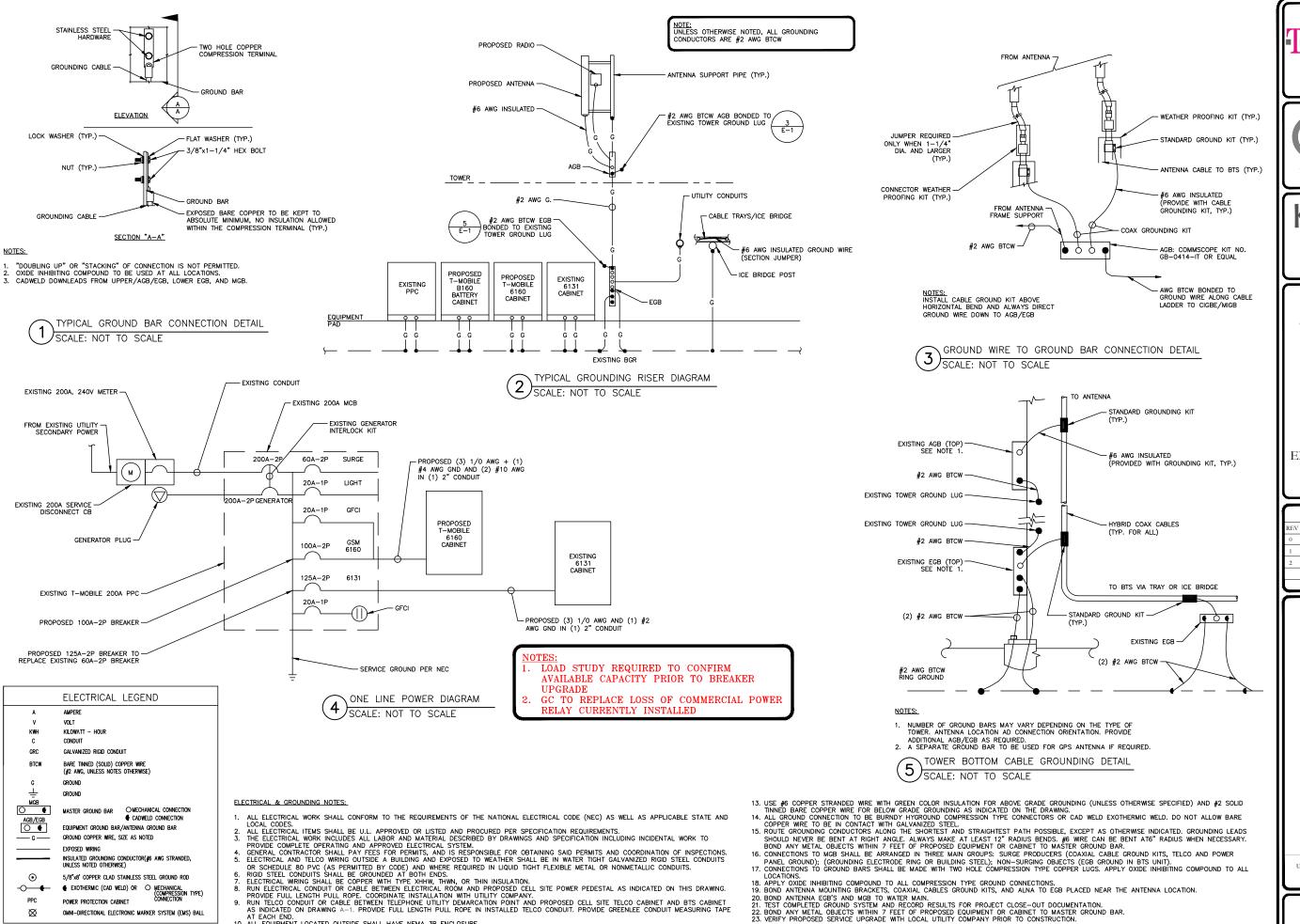


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C-6



AT EACH END.

10. ALL EQUIPMENT LOCATED OUTSIDE SHALL HAVE NEMA 3R ENCLOSURE.

11. GROUNDING SHALL COMPLY WITH NEC ART. 250.

12. GROUND COAXIAL CABLE SHIELDS MINIMUM AT BOTH ENDS USING MANUFACTURERS COAX CABLE GROUNDING KITS SUPPLIED BY PROJECT OWNER.

 $\boxtimes$ 

OMNI-DIRECTIONAL ELECTRONIC MARKER SYSTEM (EMS) BALL

BELLEVUE, WA 98006



CLIFTON PARK, NY 12065

421 FAYETTEVILLE ST, SUITE 600 RALEIGH, NC 27601

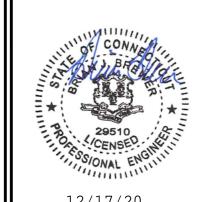
T-MOBILE SITE NUMBER: CT11623B

BU #: **842862** EAST HAVEN SOUTH

259 COMMERCE STREET EAST HAVEN, CT 06512

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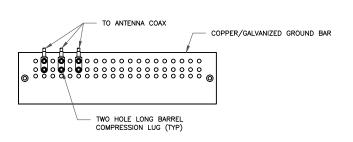
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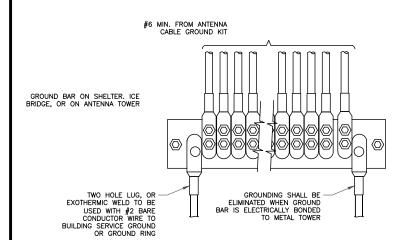
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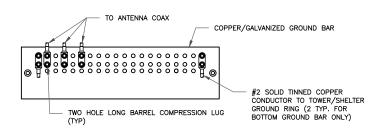


- DOUBLING UP "OR STACKING" OF CONNECTIONS IS NOT PERMITTED.
   EXTERIOR ANTIOXIDANT JOINT COMPOUND TO BE USED ON ALL EXTERIOR CONNECTIONS.
   GROUND BAR SHALL NOT BE ISOLATED FROM TOWER. MOUNT DIRECTLY TO TOWER STEEL.

ANTENNA GROUND BAR DETAIL SCALE: NOT TO SCALE

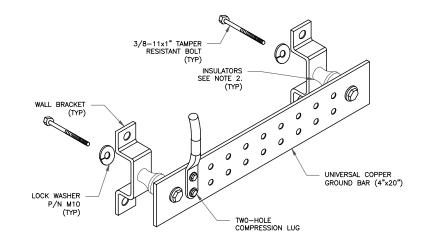


GROUNDWIRE INSTALLATION (4) SCALE: NOT TO SCALE



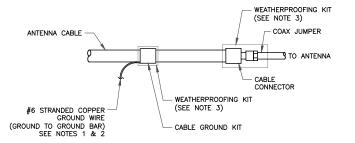
- EXTERIOR ANTIOXIDANT JOINT COMPOUND TO BE USED ON ALL EXTERIOR CONNECTIONS.
   GROUND BAR SHALL NOT BE ISOLATED FROM TOWER. MOUNT DIRECTLY TO TOWER STEEL (TOWER ONLY).
   GROUND BAR SHALL BE ISOLATED FROM BUILDING OR SHELTER.

TOWER/SHELTER GROUND BAR DETAIL TOWER/SHELTER GROUNDSCALE: NOT TO SCALE



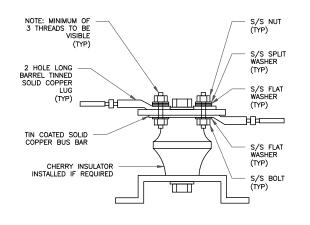
- 1. DOWN LEAD (HOME RUN) CONDUCTORS ARE <u>NOT</u> TO BE INSTALLED ON CROWN CASTLE USA INC. TOWER, PER THE GROUNDING DOWN CONDUCTOR POLICY QAS-STD-10091. NO MODIFICATION OR DRILLING TO TOWER STEEL IS ALLOWED IN ANY FORM OR FASHION, CAD-WELDING ON THE TOWER AND/OR IN THE AIR ARE NOT PERMITTED.
- 2. OMIT INSULATOR WHEN MOUNTING TO TOWER STEEL OR PLATFORM STEEL USE INSULATORS WHEN ATTACHING TO BUILDING OR SHELTERS.

GROUND BAR DETAIL 



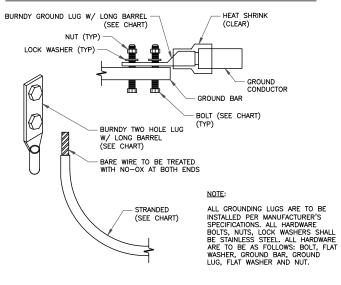
- DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.
   GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.
   WEATHER PROOFING SHALL BE TWO-PART TAPE KIT, COLD SHRINK SHALL NOT BE LISED.

CABLE GROUND KIT CONNECTION (6) SCALE: NOT TO SCALE

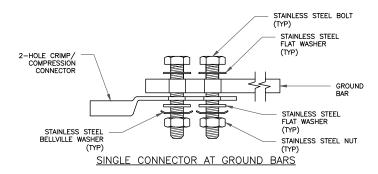


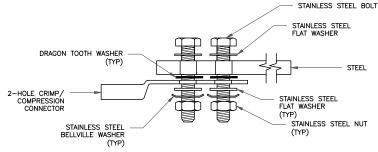
7) LUG DETAIL SCALE: NOT TO SCALE

WIRE SIZE BURNDY LUG BOLT SIZE #6 GREEN INSULATED YA6C-2TC38 3/8" - 16 NC SS 2 BOLT #2 SOLID TINNED YA3C-2TC38 3/8" - 16 NC SS 2 BOLT #2 STRANDED YA2C-2TC38 3/8" - 16 NC SS 2 BOLT YA26-2TC38 3/8" - 16 NC SS 2 BOLT #2/0 STRANDED #4/0 STRANDED YA28-2N 1/2" - 16 NC SS 2 BOLT

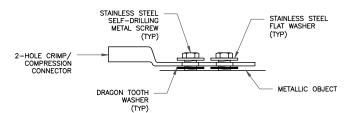


MECHANICAL LOS -SCALE: NOT TO SCALE MECHANICAL LUG CONNECTION





#### SINGLE CONNECTOR AT STEEL OBJECTS



SINGLE CONNECTOR AT METALLIC/STEEL OBJECTS

HARDWARE DETAIL FOR EXTERIOR CONNECTIONS SCALE: NOT TO SCALE







421 FAYETTEVILLE ST, SUITE 600 RALEIGH, NC 27601

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# Exhibit D

**Structural Analysis Report** 

Date: October 21, 2020

Cheryl Schultz Crown Castle 6325 Ardrey Kell Rd, Suite 600 Charlotte, NC



Crown Castle 2000 Corporate Drive Canonsburg, PA 15317 (724) 416-2000

Subject: Structural Analysis Report

Carrier Designation: T-Mobile Co-Locate

Carrier Site Number: CT11623B

Crown Castle BU Number: 842862 28277

Crown Castle Site Name: EAST HAVEN SOUTH

Crown Castle JDE Job Number: 620146
Crown Castle Work Order Number: 1892946
Crown Castle Order Number: 529726 Rev. 0

Engineering Firm Designation: Crown Castle Project Number: 1892946

Site Data: 259 COMMERCE STREET, EAST HAVEN, New Haven County, CT

Latitude 41° 15' 22.88", Longitude -72° 52' 32.8"

58 Foot - Monopole Tower

Dear Cheryl Schultz,

Crown Castle is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Proposed Equipment Configuration

**Sufficient Capacity – 84.5%** 

This analysis utilizes an ultimate 3-second gust wind speed of 130 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: Dolly Hsu, E.I.T.

Respectfully submitted by:

\* 31522 \* CENSEO CHIMINGS/ONAL EMILITATION

Terry P Styran 2020.10.23 11:08:44 -04'00'

Terry P. Styran, P.E. Senior Project Engineer

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Table 2 - Other Considered Equipment

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#### 4) ANALYSIS RESULTS

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Table 5 - Tower Component Stresses vs. Capacity - LC5
4.1) Recommendations

#### 5) APPENDIX A

tnxTower Output

#### 6) APPENDIX B

Base Level Drawing

#### 7) APPENDIX C

**Additional Calculations** 

#### 1) INTRODUCTION

This tower is a 58 ft Monopole tower designed by FWT INC.

#### 2) ANALYSIS CRITERIA

TIA-222 Revision: TIA-222-H

Risk Category:
Wind Speed:
130
Exposure Category:
C
Topographic Factor:
1ce Thickness:
1.5 in
Wind Speed with Ice:
50 mph
Service Wind Speed:
60 mph

Table 1 - Proposed Equipment Configuration

Table 1-1	•	i i	Jonngulation		1	1
Mounting Level (ft)	Elevation	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
		3	ericsson	AIR 32 B2A B66AA_T-MOBILE w/ Mount Pipe		
		3	ericsson	AIR6449 B41_T-MOBILE w/ Mount Pipe		
47.0	47.0	3	ericsson	RADIO 4449 B71 B85A_T- MOBILE	3	1-5/8
		3	ericsson	RRUS 4415 B25		
		3	rfs celwave	APXVAARR24_43-U-NA20 w/ Mount Pipe		
		1	tower mounts	Miscellaneous [NA 507-1]		
		1	tower mounts	Platform Mount [LP 303-1]		

Table 2 - Other Considered Equipment

Mounting Level (ft)	Flovation	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	
		3	ericsson	RRUS 32			
		3	ericsson	RRUS 4449 B5/B12		3/8 3/4 7/8 Conduit	
		3	ericsson	RRUS 8843 B2/B66A			
	55.0	3	kathrein	800 10121 w/ Mount Pipe			
		3	kathrein	80010965 w/ Mount Pipe			
54.0		6		powerwave technologies	LGP21401		6 6
		3	quintel technology	QS66512-6 w/ Mount Pipe	2		
		1	raycap	DC6-48-60-18-8C	_		
		3	raycap	DC6-48-60-18-8F			
		3	site pro 1	STK-U			
	54.0	1	tower mounts	Pipe Mount [PM 602-3]			
		1	tower mounts	Sector Mount [SM 502-3]			
37.0	37.0 3 rfs celwave APXV18-206517S-C w/ Mount Pipe		6	1-5/8			

#### 3) ANALYSIS PROCEDURE

**Table 3 - Documents Provided** 

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Jaworski Geotech Inc.	4291659	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	FWT, Inc.	4529325	CCISITES
4-TOWER MANUFACTURER DRAWINGS	FWT, Inc.	4291655	CCISITES

#### 3.1) Analysis Method

tnxTower (version 8.0.7.5), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 Standard.

#### 3.2) Assumptions

- 1) Tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

#### 4) ANALYSIS RESULTS

**Table 4 - Section Capacity (Summary)** 

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	58 - 50.5	Pole	TP19.078x17.393x0.188	1	-2.588	690.552	4.1	Pass
L2	50.5 - 0	Pole	TP30.05x18.141x0.188	2	-14.166	1091.643	84.5	Pass
							Summary	
						Pole (L2)	84.5	Pass
						Rating =	84.5	Pass

Table 5 - Tower Component Stresses vs. Capacity - LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail	
1	Anchor Rods	0	69.6	Pass	
1	Base Plate	0	72.2	Pass	
1	Base Foundation (Structure)	0	37.0	Pass	
1	Base Foundation (Soil Interaction)	0	56.5	Pass	

Structure Rating (max from all components) =	84.5%

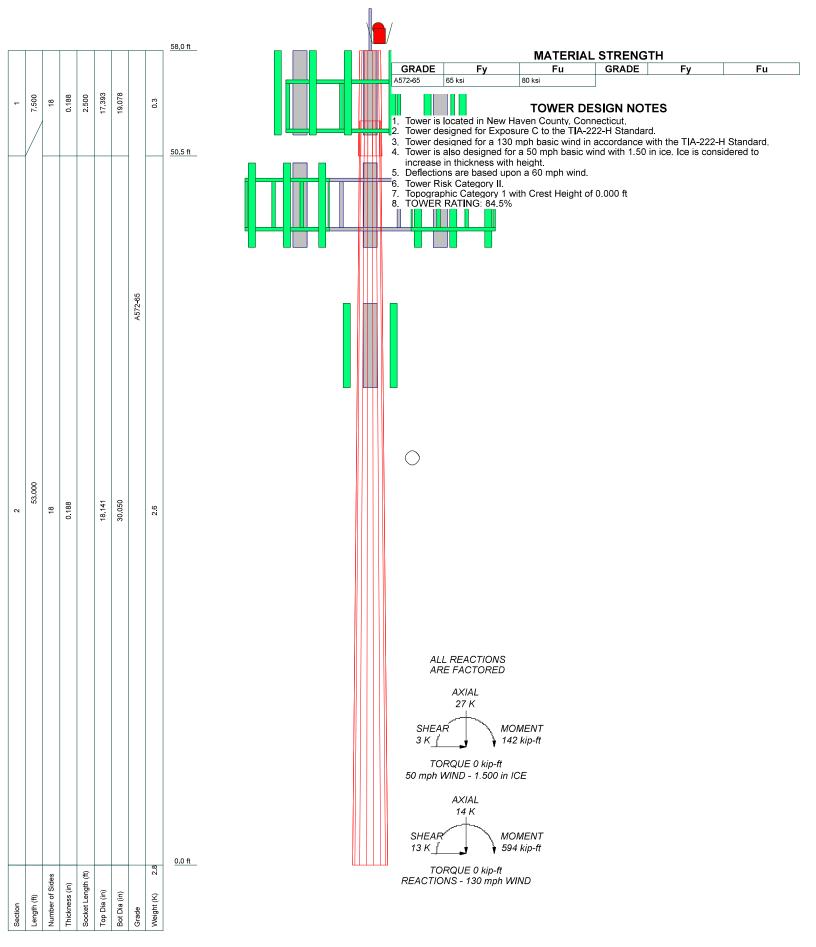
Notes:

<sup>1)</sup> See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

#### 4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

# APPENDIX A TNXTOWER OUTPUT



CROWN	Crown Castle	Job: BU# 842862		
CASTLE	. ZUUU GOLOOTAIE LIIVE I	Project: Client: Crown Castle	Drawn by: Dolly Hsu	App'd:
The Pathway to Possible	Phone: (724) 416-2000	11A-222-H	Date: 10/21/20	Scale: NTS
ŕ	`FAX:	Path: C:\Users\dhsu\Documents\Weekend WIF	1842862\VVO 1892946 - SA\Prod\842862_RPA.e	Dwg No. E-1

#### Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

- 3) Tower is located in New Haven County, Connecticut.
- 4) Tower base elevation above sea level: 35.000 ft.
- 5) Basic wind speed of 130 mph.
- 6) Risk Category II.
- 7) Exposure Category C.
- 8) Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- 9) Topographic Category: 1.
- 10) Crest Height: 0.000 ft.
- 11) Nominal ice thickness of 1.500 in.
- 12) Ice thickness is considered to increase with height.
- 13) Ice density of 56.000 pcf.
- 14) A wind speed of 50 mph is used in combination with ice.
- 15) Temperature drop of 50.000 °F.
- 16) Deflections calculated using a wind speed of 60 mph.
- 17) A non-linear (P-delta) analysis was used.
- 18) Pressures are calculated at each section.
- 19) Stress ratio used in pole design is 1.05.
- 20) Tower analysis based on target reliabilities in accordance with Annex S.
- 21) Load Modification Factors used:  $K_{es}(F_w) = 0.95$ ,  $K_{es}(t_i) = 0.85$ .
- 22) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

#### **Options**

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification Use Code Stress Ratios

V Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz

Use Special Wind Profile

Include Bolts In Member Capacity

Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt.

Autocalc Torque Arm Areas

Add IBC .6D+W Combination

√ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation

√ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice Exemption

#### Poles

✓ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known

### Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft	Sides	in	in	in	in	
L1	58.000-50.500	7.500	2.500	18	17.393	19.078	0.188	0.750	A572-65 (65 ksi)

Section	Elevation	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft	Sides	in	in	in	in	
L2	50.500-0.000	53.000		18	18.141	30.050	0.188	0.750	A572-65 (65 ksi)

Tapered Pole Properties										
Section	Tip Dia.	Area	1	r	С	VC	J	lt/Q	W	w/t
	in	in²	in⁴	in	in	in³	in⁴	in²	in	
L1	17.632	10.239	382.955	6.108	8.836	43.342	766.414	5.121	2.731	14.566
	19.343	11.242	506.846	6.706	9.692	52.297	1014.359	5.622	3.028	16.148
L2	18.963	10.685	435.128	6.374	9.216	47.215	870.829	5.343	2.863	15.269
	30.485	17.772	2002.277	10.601	15.265	131.164	4007.188	8.888	4.959	26.447

Tower	Gusset	Gusset	Gusset GradeAdjust. Factor	Adjust.	Weight Mult.	Double Angle	Double Angle	Double Angle
Elevation	Area (per face)	Thickness	$A_f$	Factor A,		Stitch Bolt Spacing	Stitch Bolt Spacing	Stitch Bolt Spacing
ft	ft <sup>2</sup>	in				Diagonals in	nonzontais in	Redundants in
L1 58.000-			1	1	1			
50.500								
L2 50.500- 0.000			1	1	1			

## Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Sector	Exclude From	Componen t	Placement	Total Number	Number PerRow	Start/En d	Width or Diamete	Perimete r	Weight
		Torque Calculation	Type	ft			Position	r in	in	pIf
FB-L98B-034-XXX(3/8)	С	No	Surface Ar (CaAa)	54.000 - 0.000	2	2	-0.400 -0.350	0.000		0.057
WR-VG86ST-BRD(3/4)	С	No	Surface Ar (CaAa)	54.000 <b>-</b> 0.000	6	4	-0.500 -0.350	0.000		0.584
2" Rigid Conduit	С	No	Surface Ar (CaAa)	54.000 - 0.000	2	2	-0.500 -0.350	2.000		2.800
***D*** ***			(2 1)							

## Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow	Exclude	Componen	Placement	Total		$C_AA_A$	Weight	
	Leg	Shield	From Torque Calculatior	Type 1	ft	Number		ft²/ft	plf	
*54*										
LDF5-50A(7/8)	С	No	No	Inside Pole	54.000 - 0.000	6	No Ice 1/2" Ice	0.000 0.000	0.330 0.330	
***D*** ***D***							1" Ice 2" Ice	0.000 0.000	0.330 0.330	
_DF7-50A(1-5/8)	С	No	No	Inside Pole	37.000 - 0.000	6	No Ice 1/2" Ice 1" Ice 2" Ice	0.000 0.000 0.000 0.000	0.820 0.820 0.820 0.820	
***D***										
.DF4P-50A(1/2")	В	No	No	Inside Pole	58.000 - 0.000	1	No Ice 1/2" Ice	0.000 0.000	0.150 0.150	

Description	Face	Allow Shield	Exclude From	Componen	Placement	Total Number		$C_AA_A$	Weight
	Leg	Siliela	Torque Calculation	Type	ft	Number		ft²/ft	pIf
							1" Ice	0.000	0.150
*s*							2" Ice	0.000	0.150
HCS 6X12	С	No	No	Inside Pole	47.000 - 0.000	3	No Ice	0.000	2.400
4AWG(1-5/8)							1/2" Ice	0.000	2.400
							1" Ice	0.000	2.400
***							2" Ice	0.000	2.400

## Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	$A_R$	$A_F$	$C_AA_A$	$C_A A_A$	Weight
Sectio	Elevation		_		In Face	Out Face	
n	ft		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	K
L1	58.000-50.500	Α	0.000	0.000	0.000	0.000	0.000
		В	0.000	0.000	0.000	0.000	0.001
		С	0.000	0.000	1.400	0.000	0.039
L2	50.500-0.000	Α	0.000	0.000	0.000	0.000	0.000
		В	0.000	0.000	0.000	0.000	0.008
		С	0.000	0.000	20.200	0.000	1.086

## Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	lce Tuit	$A_R$	$A_F$	$C_AA_A$	$C_AA_A$	Weight
Sectio n	Elevation ft	or Leg	Thickness in	ft <sup>2</sup>	ft <sup>2</sup>	In Face ft²	Out Face ft²	K
L1	58.000-50.500	Α	1.340	0.000	0.000	0.000	0.000	0.000
		В		0.000	0.000	0.000	0.000	0.001
		С		0.000	0.000	5.267	0.000	0.082
L2	50.500-0.000	Α	1.237	0.000	0.000	0.000	0.000	0.000
		В		0.000	0.000	0.000	0.000	0.008
		С		0.000	0.000	75.996	0.000	1.706

## **Feed Line Center of Pressure**

Section	Elevation	$CP_X$	CPz	CP <sub>×</sub> Ice	CP <sub>z</sub> Ice
	ft	in	in	in	in
L1	58.000-50.500	1.144	0.926	1.557	1.318
L2	50.500-0.000	2.153	1.744	2.668	2.260

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

## **Shielding Factor Ka**

Tower	Feed Line	Description	Feed Line	K <sub>a</sub>	K <sub>a</sub>
Section	Record No.		Seament	No Ice	Ice
Section	Necola No.		Elev.	NO ICE	100

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No <i>l</i> ce	K <sub>a</sub> Ice
L1	6	FB-L98B-034-XXX(3/8)	50.50 - 54.00	1.0000	1.0000
L1	7	WR-VG86ST-BRD(3/4)	50.50 -	1.0000	1.0000
L1	8	2" Rigid Conduit	54.00 50.50 -	1.0000	1.0000
L2	6	FB-L98B-034-XXX(3/8)	54.00 0.00 - 50.50	1.0000	1.0000
L2 L2	7 8	WR-VG86ST-BRD(3/4) 2" Rigid Conduit	0.00 - 50.50 0.00 - 50.50	1.0000 1.0000	1.0000 1.0000

			Disc	rete Tov	wer Loa	ds			
Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	۰	ft		ft²	ft²	К
(2) Side Lighting	В	From Leg	0.000 0.000 0.500	0.000	58.000	No Ice 1/2" Ice 1" Ice 2" Ice	0.108 0.170 0.233 0.389	0.108 0.170 0.233 0.389	0.005 0.007 0.010 0.019
Lightning Rod 1/2" x 2'	Α	From Leg	4.000 0.000 1.000	0.000	58.000	No Ice 1/2" Ice 1" Ice 2" Ice	0.100 0.264 0.395 0.685	0.100 0.264 0.395 0.685	0.020 0.021 0.024 0.034
*54* 800 10121 w/ Mount Pipe	Α	From Leg	4.000 0.000 1.000	0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	3.600 4.000 4.420 5.290	2.950 3.340 3.740 4.590	0.072 0.115 0.166 0.297
800 10121 w/ Mount Pipe	В	From Leg	4.000 0.000 1.000	0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	3.600 4.000 4.420 5.290	2.950 3.340 3.740 4.590	0.072 0.115 0.166 0.297
800 10121 w/ Mount Pipe	С	From Leg	4.000 0.000 1.000	0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	3.600 4.000 4.420 5.290	2.950 3.340 3.740 4.590	0.072 0.115 0.166 0.297
DC6-48-60-18-8F	Α	From Leg	4.000 0.000 1.000	0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.212 1.892 2.105 2.570	1.212 1.892 2.105 2.570	0.020 0.042 0.067 0.126
80010965 w/ Mount Pipe	Α	From Leg	4.000 0.000 1.000	0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	12.260 13.030 13.800 15.410	5.790 6.470 7.170 8.600	0.136 0.226 0.328 0.570
80010965 w/ Mount Pipe	В	From Leg	4.000 0.000 1.000	0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	12.260 13.030 13.800 15.410	5.790 6.470 7.170 8.600	0.136 0.226 0.328 0.570
80010965 w/ Mount Pipe	С	From Leg	4.000 0.000	0.000	54.000	No Ice 1/2"	12.260 13.030	5.790 6.470	0.136 0.226

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ven ft ft ft	۰	ft		ft²	ft²	K
			1.000			Ice 1" Ice	13.800 15.410	7.170 8.600	0.328 0.570
QS66512-6 w/ Mount Pipe	Α	From Leg	4.000 0.000 1.000	0.000	54.000	2" Ice No Ice 1/2" Ice	4.040 4.420 4.820	4.180 4.570 4.970	0.137 0.206 0.287
QS66512-6 w/ MountPipe	В	From Leg	4.000	0.000	54.000	1" Ice 2" Ice No Ice	5.630 4.040	5.790 4.180	0.482 0.137
Q = 0 = 0 = 0 = 0 = 0 = 0 = 0 = 0 = 0 =	_		0.000	0.000	0000	1/2" Ice 1" Ice 2" Ice	4.420 4.820 5.630	4.570 4.970 5.790	0.206 0.287 0.482
QS66512-6 w/ Mount Pipe	С	From Leg	4.000 0.000 1.000	0.000	54.000	No Ice 1/2" Ice 1" Ice	4.040 4.420 4.820 5.630	4.180 4.570 4.970 5.790	0.137 0.206 0.287 0.482
RRUS 8843 B2/B66A	Α	From Leg	4.000 0.000 1.000	0.000	54.000	2" Ice No Ice 1/2" Ice 1" Ice	1.639 1.799 1.966 2.323	1.353 1.500 1.655 1.986	0.072 0.090 0.110 0.159
RRUS 8843 B2/B66A	В	From Leg	4.000 0.000 1.000	0.000	54.000	2" Ice No Ice 1/2" Ice	1.639 1.799 1.966	1.353 1.500 1.655	0.072 0.090 0.110
RRUS 8843 B2/B66A	С	From Leg	4.000 0.000	0.000	54.000	1" Ice 2" Ice No Ice 1/2"	2.323 1.639 1.799	1.986 1.353 1.500	0.159 0.072 0.090
(2) 1 CP24 404	٨	From Log	1.000	0.000	F4 000	lce 1" lce 2" lce	1.966 2.323	1.655 1.986	0.110 0.159
(2) LGP21401	Α	From Leg	4.000 0.000 1.000	0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.104 1.239 1.381 1.688	0.207 0.274 0.348 0.521	0.014 0.021 0.030 0.055
(2) LGP21401	В	From Leg	4.000 0.000 1.000	0.000	54.000	No Ice 1/2" Ice 1" Ice	1.104 1.239 1.381 1.688	0.207 0.274 0.348 0.521	0.014 0.021 0.030 0.055
(2) LGP21401	С	From Leg	4.000 0.000 1.000	0.000	54.000	2" Ice No Ice 1/2" Ice 1" Ice	1.104 1.239 1.381 1.688	0.207 0.274 0.348 0.521	0.014 0.021 0.030 0.055
RRUS 32	Α	From Leg	4.000 0.000 1.000	0.000	54.000	2" Ice No Ice 1/2" Ice 1" Ice	2.857 3.083 3.316 3.805	1.777 1.968 2.166 2.583	0.055 0.077 0.103 0.165
RRUS 32	В	From Leg	4.000 0.000 1.000	0.000	54.000	2" Ice No Ice 1/2" Ice 1" Ice	2.857 3.083 3.316 3.805	1.777 1.968 2.166 2.583	0.055 0.077 0.103 0.165
RRUS 32	С	From Leg	4.000 0.000 1.000	0.000	54.000	2" Ice No Ice 1/2" Ice 1" Ice	2.857 3.083 3.316 3.805	1.777 1.968 2.166 2.583	0.055 0.077 0.103 0.165
RRUS 4449 B5/B12	В	From Leg	4.000 0.000	0.000	54.000	2" Ice No Ice 1/2"	1.968 2.144	1.408 1.564	0.071 0.090

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C₄A₄ Side	Weight
			ft ft ft	۰	ft		ft²	ft²	K
			1.000			Ice 1" Ice	2.328 2.718	1.727 2.075	0.111 0.163
(2) RRUS 4449 B5/B12	С	From Leg	4.000 0.000	0.000	54.000	2" Ice No Ice 1/2"	1.968 2.144	1.408 1.564	0.071 0.090
			1.000			Ice 1" Ice 2" Ice	2.328 2.718	1.727 2.075	0.111 0.163
(2) DC6-48-60-18-8F	Α	From Leg	4.000 0.000 1.000	0.000	54.000	No Ice 1/2" Ice 1" Ice	1.212 1.892 2.105	1.212 1.892 2.105	0.020 0.042 0.067
DC6-48-60-18-8C	В	From Leg	4.000 0.000	0.000	54.000	2" Ice No Ice 1/2"	2.570 1.145 1.792	2.570 1.145 1.792	0.126 0.026 0.047
			1.000			lce 1" lce 2" lce	2.002 2.451	2.002 2.451	0.070 0.125
12.5' horizontal x 2.375" Pipe Mount	Α	From Leg	6.000 0.000 0.000	0.000	54.000	No Ice 1/2" Ice 1" Ice	2.969 4.247 5.542 8.054	2.969 4.247 5.542 8.054	0.120 0.142 0.173 0.258
12.5' horizontal x 2.375" Pipe Mount	В	From Leg	6.000 0.000	0.000	54.000	2" Ice No Ice 1/2"	2.969 4.247	2.969 4.247	0.120 0.142
			0.000			Ice 1" Ice 2" Ice	5.542 8.054	5.542 8.054	0.173 0.258
12.5' horizontal x 2.375" Pipe Mount	С	From Leg	6.000 0.000 0.000	0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	2.969 4.247 5.542 8.054	2.969 4.247 5.542 8.054	0.120 0.142 0.173 0.258
Pipe Mount [PM 602-3]	С	None		0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	6.670 7.700 8.740 10.900	6.670 7.700 8.740 10.900	0.279 0.344 0.423 0.628
Sector Mount [SM 502-3]	С	None		0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	29.820 42.210 54.430 78.490	29.820 42.210 54.430 78.490	1.673 2.266 3.052 5.180
***D*** *47*						2 100			
AIR 32 B2A B66AA_T- MOBILE w/ Mount Pipe	Α	From Leg	4.000 0.000 0.000	0.000	47.000	No Ice 1/2" Ice 1" Ice	7.087 7.561 8.021 8.966	6.392 7.249 7.991 9.526	0.194 0.257 0.328 0.493
AIR 32 B2A B66AA_T- MOBILE w/ Mount Pipe	В	From Leg	4.000 0.000 0.000	0.000	47.000	2" Ice No Ice 1/2" Ice 1" Ice 2" Ice	7.087 7.561 8.021 8.966	6.392 7.249 7.991 9.526	0.194 0.257 0.328 0.493
AIR 32 B2A B66AA_T- MOBILE w/ Mount Pipe	С	From Leg	4.000 0.000 0.000	0.000	47.000	No Ice 1/2" Ice 1" Ice	7.087 7.561 8.021 8.966	6.392 7.249 7.991 9.526	0.194 0.257 0.328 0.493
AIR6449 B41_T-MOBILE w/ Mount Pipe	Α	From Leg	4.000 0.000 0.000	0.000	47.000	2" Ice No Ice 1/2" Ice 1" Ice 2" Ice	5.870 6.233 6.606 7.382	3.270 3.728 4.203 5.200	0.128 0.177 0.232 0.359

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ven ft ft ft	۰	ft		ft²	ft²	K
AIR6449 B41_T-MOBILE	В	From Leg	4.000	0.000	47.000	No Ice	5.870	3.270	0.128
w/ Mount Pipe		· ·	0.000			1/2"	6.233	3.728	0.177
·			0.000			Ice	6.606	4.203	0.232
						1" Ice	7.382	5.200	0.359
AIR6449 B41 T-MOBILE	С	From Leg	4.000	0.000	47.000	2" Ice No Ice	5.870	3.270	0.128
w/ Mount Pipe	Ü	1 Ioni Log	0.000	0.000	47.000	1/2"	6.233	3.728	0.177
•			0.000			Ice	6.606	4.203	0.232
						1" Ice	7.382	5.200	0.359
APXVAARR24 43-U-NA20	Α	From Leg	4.000	0.000	47.000	2" Ice No Ice	14.690	6.870	0.186
w/ Mount Pipe	^	r ioni Leg	0.000	0.000	47.000	1/2"	15.460	7.550	0.100
w would be			0.000			Ice	16.230	8.250	0.458
			0.000			1" Ice 2" Ice	17.820	9.670	0.788
APXVAARR24 43-U-NA20	В	From Leg	4.000	0.000	47.000	No Ice	14.690	6.870	0.186
w/ Mount Pipe			0.000			1/2"	15.460	7.550	0.315
			0.000			Ice	16.230	8.250	0.458
						1" Ice 2" Ice	17.820	9.670	0.788
APXVAARR24_43-U-NA20	С	From Leg	4.000	0.000	47.000	No Ice	14.690	6.870	0.186
w/ Mount Pipe	Ū	1 10111 209	0.000	0.000	17.000	1/2"	15.460	7.550	0.315
			0.000			Ice	16.230	8.250	0.458
						1" Ice 2" Ice	17.820	9.670	0.788
RADIO 4449 B71 B85A_T-	Α	From Leg	4.000	0.000	47.000	No Ice	1.970	1.587	0.073
MOBILE			0.000			1/2"	2.147	1.749	0.093
			0.000			lce 1" lce	2.331 2.721	1.918 2.280	0.116 0.170
						2" Ice	2.721	2.200	0.170
RADIO 4449 B71 B85A_T-	В	From Leg	4.000	0.000	47.000	No Ice	1.970	1.587	0.073
MOBILE		-	0.000			1/2"	2.147	1.749	0.093
			0.000			Ice	2.331	1.918	0.116
						1" Ice 2" Ice	2.721	2.280	0.170
RADIO 4449 B71 B85A_T-	С	From Leg	4.000	0.000	47.000	No Ice	1.970 2.147	1.587	0.073
MOBILE			0.000 0.000			1/2" Ice	2.147	1.749 1.918	0.093 0.116
			0.000			1" Ice	2.721	2.280	0.110
						2" Ice			00
RRUS 4415 B25	Α	From Leg	4.000	0.000	47.000	No Ice	1.644	0.679	0.044
			0.000			1/2"	1.804	0.791	0.056
			0.000			Ice 1" Ice	1.972 2.329	0.913 1.183	0.071 0.109
						2" Ice	2.329	1.103	0.109
RRUS 4415 B25	В	From Leg	4.000	0.000	47.000	No Ice	1.644	0.679	0.044
			0.000			1/2"	1.804	0.791	0.056
			0.000			Ice	1.972	0.913	0.071
						1" Ice 2" Ice	2.329	1.183	0.109
RRUS 4415 B25	С	From Leg	4.000	0.000	47.000	No Ice	1.644	0.679	0.044
	-	3	0.000			1/2"	1.804	0.791	0.056
			0.000			Ice	1.972	0.913	0.071
						1" Ice 2" Ice	2.329	1.183	0.109
8' x 2.375" Mount Pipe	Α	From Leg	4.000	0.000	47.000	No Ice	1.900	1.900	0.061
		9	0.000	<del>-</del>		1/2"	2.728	2.728	0.075
			0.000			Ice	3.401	3.401	0.095
						1" Ice 2" Ice	4.396	4.396	0.150
8' x 2.375" Mount Pipe	В	From Leg	4.000	0.000	47.000	No Ice	1.900	1.900	0.061
·		J	0.000			1/2"	2.728	2.728	0.075
			0.000			Ice	3.401	3.401	0.095
						1" Ice	4.396	4.396	0.150
						2" Ice			

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ft ft ft	۰	ft		ft <sup>2</sup>	ft²	K
8' x 2.375" MountPipe	С	From Leg	4.000 0.000 0.000	0.000	47.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.900 2.728 3.401 4.396	1.900 2.728 3.401 4.396	0.061 0.075 0.095 0.150
Miscellaneous [NA 507-1]	С	None		0.000	50.000	No Ice 1/2" Ice 1" Ice 2" Ice	4.560 6.390 8.180 11.660	4.560 6.390 8.180 11.660	0.245 0.311 0.402 0.657
Platform Mount [LP 303-1]  ***D***	С	None		0.000	47.000	No Ice 1/2" Ice 1" Ice 2" Ice	14.690 18.010 21.340 28.080	14.690 18.010 21.340 28.080	1.250 1.569 1.942 2.852
*37* APXV18-206517S-C w/ Mount Pipe	Α	From Leg	1.000 0.000 0.000	0.000	37.000	No Ice 1/2" Ice 1" Ice 2" Ice	3.790 4.380 4.990 6.250	3.160 3.750 4.350 5.590	0.053 0.094 0.145 0.281
APXV18-206517S-C w/ Mount Pipe	В	From Leg	1.000 0.000 0.000	0.000	37.000	No Ice 1/2" Ice 1" Ice 2" Ice	3.790 4.380 4.990 6.250	3.160 3.750 4.350 5.590	0.053 0.094 0.145 0.281
APXV18-206517S-C w/ Mount Pipe	С	From Leg	1.000 0.000 0.000	0.000	37.000	No Ice 1/2" Ice 1" Ice 2" Ice	3.790 4.380 4.990 6.250	3.160 3.750 4.350 5.590	0.053 0.094 0.145 0.281
*D*									

## **Load Combinations**

Comb.	Description
No.	Deed Only.
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice

Comb.	Description
No.	·
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg-Service
44	Dead+Wind 150 deg-Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg-Service
47	Dead+Wind 240 deg-Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg-Service
50	Dead+Wind 330 deg-Service

May	imum	Mam	hor	Forces
iviax	mmunn	wem	Dei	roices

Sectio n	Elevation ft	Component Type	Condition	Gov. Load	Axial	Major Axis Moment	Minor Axis Moment
No.				Comb.	K	kip-ft	kip-ft
L1	58 - 50.5	Pole	MaxTension	36	0.000	-0.001	-0.000
			Max. Compression	26	-10.837	0.185	0.148
			Max. Mx	20	-2.588	12.351	-0.164
			Max. My	14	-2.589	0.132	-12.300
			Max. Vý	20	-5.713	9.474	-0.282
			Max. Vx	14	5.680	0.232	-9.446
			Max. Torque	9			0.183
L2	50.5 - 0	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-26.800	0.195	-1.160
			Max. Mx	20	-14.166	593.528	-1.425
			Max. My	14	-14.166	0.757	-592.365
			Max. Vý	20	-12.862	593.528	-1.425
			Max. Vx	14	12.830	0.757	-592.365
			Max. Torque	9			0.183

## **Maximum Reactions**

Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, Z
		Load Comb.	K	Κ	K
Pole	Max. Vert	34	26.800	1.503	-2.596
	Max. H <sub>x</sub>	20	14.192	12.834	-0.009
	Max. H <sub>z</sub>	2	14.192	-0.009	12.802
	Max. M <sub>x</sub>	2	590.578	-0.009	12.802
	Max. M <sub>z</sub>	8	593.072	-12.834	0.009
	Max. Torsion	9	0.182	-12.834	0.009
	Min. Vert	25	10.644	6.409	11.082
	Min. H <sub>x</sub>	8	14.192	-12.834	0.009

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Min. H <sub>z</sub>	14	14.192	0.009	-12.802
	Min. M <sub>x</sub>	14	-592.365	0.009	-12.802
	Min. M <sub>z</sub>	20	-593.528	12.834	-0.009
	Min. Torsion	21	-0.181	12.834	-0.009

## **Tower Mast Reaction Summary**

Load Combination	Vertical	Shear <sub>x</sub>	Shearz	Overturning Moment, M <sub>x</sub>	Overturning Moment, Mz	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Dead Only	11.826	0.000	0.000	0.723	0.182	0.000
1.2 Dead+1.0 Wind 0 deg - No Ice	14.192	0.009	-12.802	-590.578	-0.301	-0.047
0.9 Dead+1.0 Wind 0 deg - No Ice	10.644	0.009	-12.802	-585.773	-0.355	-0.044
1.2 Dead+1.0 Wind 30 deg-	14.192	6.425	-11.091	-511.599	-296.883	-0.129
No Ice 0.9 Dead+1.0 Wind 30 deg-	10.644	6.425	-11.091	-507.466	-294.411	-0.128
No Ice 1.2 Dead+1.0 Wind 60 deg - No Ice	14.192	11.119	-6.409	-295.297	-513.852	-0.178
0.9 Dead+1.0 Wind 60 deg-	10.644	11.119	-6.409	-293.006	-509.533	-0.179
No Ice 1.2 Dead+1.0 Wind 90 deg- No Ice	14.192	12.834	-0.009	0.367	-593.072	-0.179
0.9 Dead+1.0 Wind 90 deg- No Ice	10.644	12.834	-0.009	0.141	-588.080	-0.182
1.2 Dead+1.0 Wind 120 deg - No Ice	14.192	11.110	6.393	296.173	-513.322	-0.132
0.9 Dead+1.0 Wind 120 deg - No Ice	10.644	11.110	6.393	293.428	-509.008	-0.136
1.2 Dead+1.0 Wind 150 deg - No Ice	14.192	6.409	11.082	512.860	-295.965	-0.049
0.9 Dead+1.0 Wind 150 deg - No Ice	10.644	6.409	11.082	508.271	-293.503	-0.053
1.2 Dead+1.0 Wind 180 deg - No Ice	14.192	-0.009	12.802	592.365	0.757	0.048
0.9 Dead+1.0 Wind 180 deg - No Ice	10.644	-0.009	12.802	587.099	0.692	0.044
1.2 Dead+1.0 Wind 210 deg - No Ice	14.192	-6.425	11.091	513.387	297.336	0.131
0.9 Dead+1.0 Wind 210 deg - No Ice	10.644	-6.425	11.091	508.793	294.746	0.130
1.2 Dead+1.0 Wind 240 deg - No Ice	14.192	-11.119	6.409	297.088	514.305	0.178
0.9 Dead+1.0 Wind 240 deg - No Ice	10.644	-11.119	6.409	294.335	509.868	0.180
1.2 Dead+1.0 Wind 270 deg - No Ice	14.192	-12.834	0.009	1.425	593.528	0.178
0.9 Dead+1.0 Wind 270 deg - No Ice	10.644	-12.834	0.009	1.189	588.416	0.181
1.2 Dead+1.0 Wind 300 deg - No Ice	14.192	-11.110	-6.393	-294.382	513.780	0.130
0.9 Dead+1.0 Wind 300 deg	10.644	-11.110	-6.393	-292.099	509.347	0.135
1.2 Dead+1.0 Wind 330 deg - No Ice	14.192	-6.409	-11.082	-511.071	296.423	0.048
0.9 Dead+1.0 Wind 330 deg - No Ice	10.644	-6.409	-11.082	-506.943	293.841	0.052
1.2 Dead+1.0 Ice+1.0Temp	26.800	-0.000	0.000	1.160	0.195	0.000
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	26.800	0.002	-2.997	-139.941	0.118	0.007
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	26.800	1.503	-2.596	-121.075	-70.599	-0.042
1.2 Dead+1.0 Wind 60	26.800	2.601	-1.500	-69.444	-122.344	-0.081

Load Combination	Vertical	Shear <sub>x</sub>	Shear₂	Overturning Moment, M <sub>x</sub>	Overturning Moment, M₂	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 90	26.800	3.002	-0.002	1.118	-141.251	-0.098
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 120	26.800	2.599	1.497	71.704	-122.254	-0.088
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 150	26.800	1.500	2.595	123.400	-70.444	-0.055
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 180	26.800	-0.002	2.997	142.355	0.296	-0.007
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 210	26.800	-1.503	2.596	123.490	71.013	0.043
deg+1.0 lce+1.0 Temp						
1.2 Dead+1.0 Wind 240	26.800	-2.601	1.500	71.859	122.757	0.081
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 270	26.800	-3.002	0.002	1.297	141.665	0.098
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 300	26.800	<b>-</b> 2.599	-1.497	-69.289	122.668	0.088
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 330	26.800	-1.500	-2.595	-120.986	70.859	0.055
deg+1.0 Ice+1.0 Temp						
Dead+Wind 0 deg - Service	11.826	0.002	-2.568	-117.328	0.083	-0.009
Dead+Wind 30 deg - Service	11.826	1.289	-2.225	-101.562	-59.120	-0.026
Dead+Wind 60 deg - Service	11.826	2.231	-1.286	<b>-</b> 58.385	-102.432	-0.036
Dead+Wind 90 deg - Service	11.826	2.575	-0.002	0.636	-118.246	-0.036
Dead+Wind 120 deg-	11.826	2.229	1.283	59.684	-102.326	-0.027
Service						
Dead+Wind 150 deg-	11.826	1.286	2.223	102.939	-58.937	-0.010
Service						
Dead+Wind 180 deg-	11.826	-0.002	2.568	118.810	0.294	0.009
Service						
Dead+Wind 210 deg-	11.826	-1.289	2.225	103.044	59.497	0.026
Service						
Dead+Wind 240 deg-	11.826	-2.231	1.286	59.867	102.808	0.036
Service						
Dead+Wind 270 deg-	11.826	<b>-</b> 2.575	0.002	0.847	118.622	0.036
Service						
Dead+Wind 300 deg-	11.826	-2.229	-1.283	-58.202	102.703	0.027
Service						
Dead+Wind 330 deg-	11.826	-1.286	-2.223	-101.457	59.314	0.010
Service						

## Solution Summary

	Sun	n of Applied Force	es		Sum of Reactio	ns	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
1	0.000	-11.826	0.000	0.000	11.826	0.000	0.000%
2	0.009	-14.192	-12.802	-0.009	14.192	12.802	0.000%
3	0.009	-10.644	-12.802	-0.009	10.644	12.802	0.000%
4	6.425	-14.192	-11.091	-6.425	14.192	11.091	0.000%
5	6.425	-10.644	-11.091	-6.425	10.644	11.091	0.000%
6	11.119	-14.192	-6.409	-11.119	14.192	6.409	0.000%
7	11.119	-10.644	-6.409	-11.119	10.644	6.409	0.000%
8	12.834	-14.192	-0.009	-12.834	14.192	0.009	0.000%
9	12.834	-10.644	-0.009	-12.834	10.644	0.009	0.000%
10	11.110	-14.192	6.393	-11.110	14.192	-6.393	0.000%
11	11.110	-10.644	6.393	-11.110	10.644	-6.393	0.000%
12	6.409	-14.192	11.082	-6.409	14.192	-11.082	0.000%
13	6.409	-10.644	11.082	-6.409	10.644	-11.082	0.000%
14	-0.009	-14.192	12.802	0.009	14.192	-12.802	0.000%
15	-0.009	-10.644	12.802	0.009	10.644	-12.802	0.000%
16	-6.425	-14.192	11.091	6.425	14.192	-11.091	0.000%
17	-6.425	-10.644	11.091	6.425	10.644	-11.091	0.000%
18	-11.119	-14.192	6.409	11.119	14.192	-6.409	0.000%
19	-11.119	-10.644	6.409	11.119	10.644	-6.409	0.000%
20	-12.834	-14.192	0.009	12.834	14.192	-0.009	0.000%
21	-12.834	-10.644	0.009	12.834	10.644	-0.009	0.000%

	Sun	of Applied Force	es		Sum of Reaction	ns	
Load	PX	· · PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
22	-11.110	-14.192	-6.393	11.110	14.192	6.393	0.000%
23	-11.110	-10.644	-6.393	11.110	10.644	6.393	0.000%
24	-6.409	-14.192	-11.082	6.409	14.192	11.082	0.000%
25	-6.409	-10.644	-11.082	6.409	10.644	11.082	0.000%
26	0.000	-26.800	0.000	0.000	26.800	-0.000	0.000%
27	0.002	-26.800	<b>-</b> 2.997	-0.002	26.800	2.997	0.000%
28	1.503	-26.800	<b>-</b> 2.596	-1.503	26.800	2.596	0.000%
29	2.601	-26.800	-1.500	<b>-</b> 2.601	26.800	1.500	0.000%
30	3.002	<b>-</b> 26.800	-0.002	-3.002	26.800	0.002	0.000%
31	2.599	-26.800	1.497	<b>-</b> 2.599	26.800	-1.497	0.000%
32	1.500	<b>-</b> 26.800	2.595	-1.500	26.800	<b>-</b> 2.595	0.000%
33	-0.002	-26.800	2.997	0.002	26.800	<b>-</b> 2.997	0.000%
34	-1.503	<b>-</b> 26.800	2.596	1.503	26.800	<b>-</b> 2.596	0.000%
35	<b>-</b> 2.601	<b>-</b> 26.800	1.500	2.601	26.800	<b>-</b> 1.500	0.000%
36	-3.002	-26.800	0.002	3.002	26.800	-0.002	0.000%
37	<b>-</b> 2.599	<b>-</b> 26.800	-1.497	2.599	26.800	1.497	0.000%
38	<b>-</b> 1.500	<b>-</b> 26.800	<b>-</b> 2.595	1.500	26.800	2.595	0.000%
39	0.002	-11.826	<b>-</b> 2.568	-0.002	11.826	2.568	0.000%
40	1.289	-11.826	<b>-</b> 2.225	-1.289	11.826	2.225	0.000%
41	2.231	-11.826	-1.286	-2.231	11.826	1.286	0.000%
42	2.575	-11.826	-0.002	-2.575	11.826	0.002	0.000%
43	2.229	-11.826	1.283	-2.229	11.826	-1.283	0.000%
44	1.286	-11.826	2.223	-1.286	11.826	-2.223	0.000%
45	-0.002	-11.826	2.568	0.002	11.826	<b>-</b> 2.568	0.000%
46	-1.289	-11.826	2.225	1.289	11.826	-2.225	0.000%
47	-2.231	-11.826	1.286	2.231	11.826	-1.286	0.000%
48	<b>-</b> 2.575	-11.826	0.002	2.575	11.826	-0.002	0.000%
49	-2.229	-11.826	-1.283	2.229	11.826	1.283	0.000%
50	-1.286	-11.826	-2.223	1.286	11.826	2.223	0.000%

## Non-Linear Convergence Results

Load	Converged?	Number	Displacement	Force
Combination		of Cycles	Tolerance	Tolerance
1	Yes	4	0.0000001	0.0000001
2	Yes	4	0.0000001	0.00008423
3	Yes	4	0.0000001	0.00003765
4	Yes	5	0.0000001	0.00018999
5	Yes	5	0.0000001	0.00008573
6	Yes	5	0.0000001	0.00019843
7	Yes	5	0.0000001	0.00008978
8	Yes	4	0.0000001	0.00022133
9	Yes	4	0.0000001	0.00014254
10	Yes	5	0.0000001	0.00019080
11	Yes	5	0.0000001	0.00008583
12	Yes	5	0.0000001	0.00019538
13	Yes	5	0.0000001	0.00008818
14	Yes	4	0.0000001	0.00010080
15	Yes	4	0.0000001	0.00005006
16	Yes	5	0.0000001	0.00019929
17	Yes	5	0.0000001	0.00008981
18	Yes	5	0.0000001	0.00019140
19	Yes	5	0.0000001	0.00008597
20	Yes	4	0.0000001	0.00019967
21	Yes	4	0.0000001	0.00012844
22	Yes	5	0.0000001	0.00019694
23	Yes	5 5	0.0000001	0.00008906
24	Yes		0.0000001	0.00019177
25	Yes	5	0.0000001	0.00008648
26	Yes	4	0.0000001	0.00000866
27	Yes	4	0.0000001	0.00070933
28	Yes	4	0.0000001	0.00086427
29	Yes	4	0.0000001	0.00087654
30	Yes	4	0.0000001	0.00071866
31	Yes	4	0.0000001	0.00087556

32	Yes	4	0.0000001	0.00088452
33	Yes	4	0.0000001	0.00072089
34	Yes	4	0.0000001	0.00089025
35	Yes	4	0.0000001	0.00088256
36	Yes	4	0.0000001	0.00072256
37	Yes	4	0.0000001	0.00088132
38	Yes	4	0.00000001	0.00086754
39	Yes	4	0.0000001	0.00000001
40	Yes	4	0.0000001	0.00004385
41	Yes	4	0.0000001	0.00005296
42	Yes	4	0.0000001	0.00000001
43	Yes	4	0.0000001	0.00004548
44	Yes	4	0.0000001	0.00005010
45	Yes	4	0.0000001	0.00000001
46	Yes	4	0.0000001	0.00005375
47	Yes	4	0.0000001	0.00004570
48	Yes	4	0.0000001	0.00000001
49	Yes	4	0.0000001	0.00005194
50	Yes	4	0.0000001	0.00004612

### **Maximum Tower Deflections - Service Wind**

Section No.	Elevation	Horz. Deflection	Gov. Load	Tilt	Twist
	ft	in	Comb.	•	0
L1	58 - 50.5	5.215	46	0.679	0.001
L2	53 - 0	4.504	46	0.678	0.001

## Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	۰	۰	ft
58.000	(2) Side Lighting	46	5.215	0.679	0.001	4959
54.000	800 10121 w/ Mount Pipe	46	4.642	0.679	0.001	4959
50.000	Miscellaneous [NA 507-1]	46	4.105	0.672	0.001	4351
47.000	AIR 32 B2A B66AA_T-MOBILE w/ Mount Pipe	46	3.731	0.659	0.001	4572
37.000	APXV18-206517S-C w/ Mount Pipe	46	2.648	0.583	0.001	5807

## Maximum Tower Deflections - Design Wind

Section No.	Elevation	Horz. Deflection	Gov. Load	Tilt	Twist
	ft	in	Comb.	۰	۰
L1	58 - 50.5	26.004	18	3.385	0.004
L2	53 - 0	22.464	18	3.380	0.004

## Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	۰	۰	ft
58.000	(2) Side Lighting	18	26.004	3.385	0.004	1003
54.000	800 10121 w/ Mount Pipe	18	23.154	3.385	0.004	1003

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	۰	۰	ft
50.000	Miscellaneous [NA 507-1]	18	20.478	3.347	0.004	880
47.000	AIR 32 B2A B66AA_T-MOBILE w/ Mount Pipe	18	18.617	3.286	0.004	924
37.000	APXV18-206517S-C w/ Mount Pipe	18	13.219	2.905	0.003	1172

## Compression Checks

	Pole Design Data								
Section No.	Elevation	Size	L	Lu	KI/r	Α	Pu	φPn	Ratio P.,
	ft		ft	ft		in <sup>2</sup>	K	K	$\frac{P_u}{\phi P_n}$
L1 L2	58 - 50.5 (1) 50.5 - 0 (2)	TP19.078x17.393x0.188 TP30.05x18.141x0.188	7.500 53.000	0.000 0.000	0.0 0.0	11.242 17.772	-2.588 -14.166	657.669 1039.660	0.004 0.014

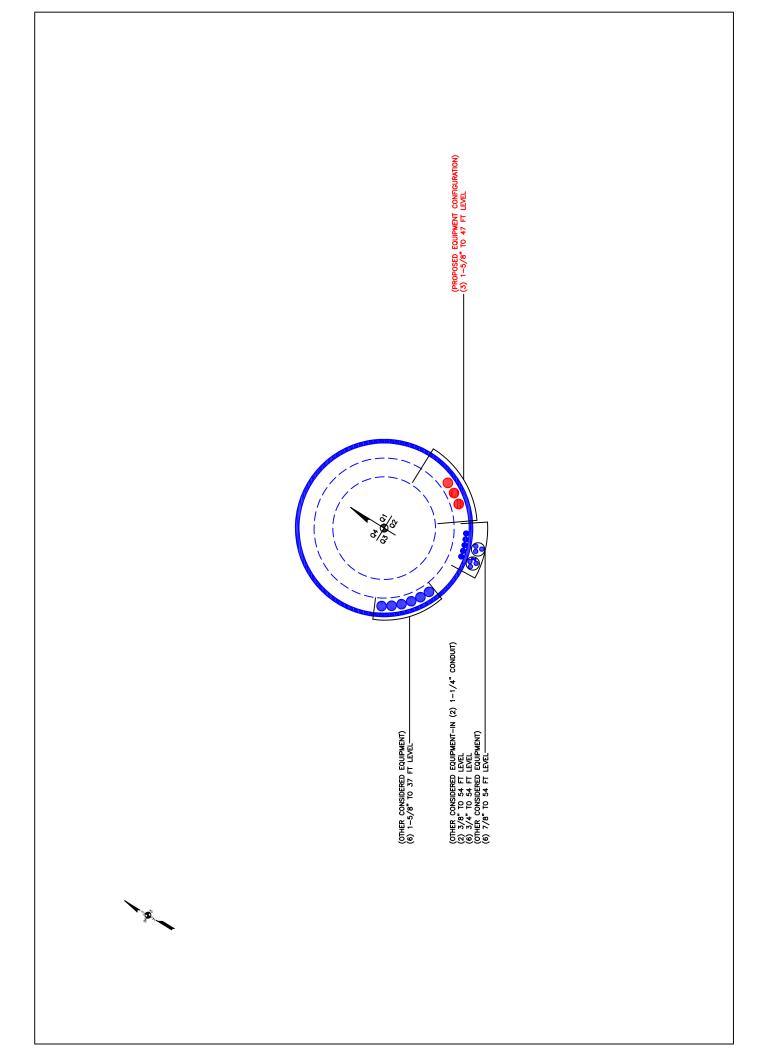
		Pole	Bendir	ng Desi	gn Da	ta		
Section	Elevation	Size	M <sub>ux</sub>	ф <b>М</b> лх	Ratio	Muy	ф <b>M</b> ny	Ratio
No.	ft		kip-ft	kip-ft	$\frac{M_{ux}}{\phi M_{nx}}$	kip-ft	kip-ft	$\frac{M_{uy}}{\phi M_{ny}}$
L1	58 - 50.5 (1)	TP19.078x17.393x0.188	12.405	319.168	0.039	0.000	319.168	0.000
L2	50.5 - 0 (2)	TP30.05x18.141x0.188	593.945	681.328	0.872	0.000	681.328	0.000

	Pol	e Snea	ir Desig	n Data			
Elevation	Size	Actual	$\phi V_n$	Ratio	Actual	<b>φ</b> <i>T</i> <sub>n</sub>	Ratio
		$V_u$		$V_u$	$T_u$		$T_u$
ft		K	K	$\phi V_n$	kip-ft	kip-ft	$\phi T_n$
58 - 50.5 (1)	TP19.078x17.393x0.188	3.055	197.301	0.015	0.092	326.402	0.000
50.5 - 0 (2)	TP30.05x18.141x0.188	12.862	311.897	0.041	0.178	815.676	0.000
	ft 58 - 50.5 (1)	Elevation Size  ft 58 - 50.5 (1) TP19.078x17.393x0.188	Elevation Size Actual V <sub>u</sub> ft K 58 - 50.5 (1) TP19.078x17.393x0.188 3.055	Elevation         Size         Actual $V_u$ $\psi V_n$ ft         K         K           58 - 50.5 (1)         TP19.078x17.393x0.188         3.055         197.301	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$V_u$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$

Pole Interaction Design Data									
Section No.	Elevation	Ratio P <sub>u</sub>	Ratio M <sub>ux</sub>	Ratio M <sub>uv</sub>	Ratio V <sub>u</sub>	Ratio T <sub>u</sub>	Comb. Stress	Allow. Stress	Criteria
	ft	$\phi P_n$	$\phi M_{nx}$	$\phi M_{ny}$	$\phi V_n$	$\phi T_n$	Ratio	Ratio	
L1	58 - 50.5 (1)	0.004	0.039	0.000	0.015	0.000	0.043	1.050	4.8.2
L2	50.5 - 0 (2)	0.014	0.872	0.000	0.041	0.000	0.887	1.050	4.8.2

Section Capacity Table								
Section No.	Elevation ft	Component Type	Size	Critical Element	P K	øP <sub>allow</sub> K	% Capacity	Pass Fail
L1	58 - 50.5	Pole	TP19.078x17.393x0.188	1	-2.588	690.552	4.1	Pass
L2	50.5 - 0	Pole	TP30.05x18.141x0.188	2	-14.166	1091.643	84.5	Pass
							Summary	
						Pole (L2)	84.5	Pass
						RATING =	84.5	Pass

# APPENDIX B BASE LEVEL DRAWING



# APPENDIX C ADDITIONAL CALCULATIONS

#### **Monopole Base Plate Connection**



(units of kips, kip-in)

**Stress Rating** 

69.6%

Pass

(Flexural)

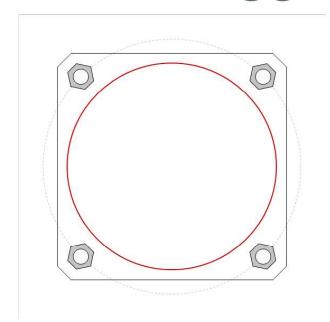
Pass

Site Info	
BU#	842862
Site Name	EAST HAVEN SOUTH
Order #	529726 Rev. 0

Analysis Considerations				
TIA-222 Revision	Н			
Grout Considered:	No			
I <sub>ar</sub> (in)	0.5			

Applied Loads					
Moment (kip-ft)	593.95				
Axial Force (kips)	14.17				
Shear Force (kips)	12.86				

<sup>\*</sup>TIA-222-H Section 15.5 Applied



#### **Connection Properties Analysis Results Anchor Rod Data Anchor Rod Summary** (4) 2-1/4" ø bolts (A615-75 N; Fy=75 ksi, Fu=100 ksi) on 37" BC Pu\_c = 195.88 $\phi$ Pn\_c = 268.39 Vu = 3.22 φVn = 120.77 **Base Plate Data** Mu = n/a φMn = n/a 33" OD x 2" Plate (A 633 Gr. E; Fy=60 ksi, Fu=70 ksi) **Base Plate Summary** Stiffener Data Max Stress (ksi): 40.96 N/A Allowable Stress (ksi): 54 Stress Rating: 72.2% Pole Data 30.05" x 0.1875" 18-sided pole (A572-65; Fy=65 ksi, Fu=80 ksi)

CCIplate - Version 3.7.2 Analysis Date: 10/21/2020

## **Pier and Pad Foundation**

BU #: 842862
Site Name: EAST HAVEN SOU
App. Number: 529726 Rev. 0



TIA-222 Revision: H
Tower Type: Monopole

Top & Bot. Pad Rein. Different?:	
Block Foundation?:	

Superstructure Analysis	Reaction	S
Compression, <b>P</b> <sub>comp</sub> :	14.19	kips
Base Shear, Vu_comp:	12.83	kips
Moment, <b>M</b> <sub>u</sub> :	593.95	ft-kips
Tower Height, <b>H</b> :	58	ft
BP Dist. Above Fdn, <b>bp</b> <sub>dist</sub> :	2.75	in

Found	Foundation Analysis Checks						
	Capacity	Demand	Rating*	Check			
Lateral (Sliding) (kips)	102.68	12.83	11.9%	Pass			
Bearing Pressure (ksf)	8.09	2.20	27.2%	Pass			
Overturning (kip*ft)	1214.61	686.70	56.5%	Pass			
Pier Flexure (Comp.) (kip*ft)	1679.58	651.69	37.0%	Pass			
Pier Compression (kip)	11934.00	34.44	0.3%	Pass			
Pad Flexure (kip*ft)	1145.25	213.96	17.8%	Pass			
Pad Shear - 1-way (kips)	351.97	53.53	14.5%	Pass			
Pad Shear - 2-way (Comp) (ksi)	0.164	0.016	9.1%	Pass			
Flexural 2-way (Comp) (kip*ft)	1971.95	391.01	18.9%	Pass			

Pier Properties					
Pier Shape:	Square				
Pier Diameter, <b>dpier</b> :	5	ft			
Ext. Above Grade, <b>E</b> :	0.5	ft			
Pier Rebar Size, <b>Sc</b> :	9				
Pier Rebar Quantity, <b>mc</b> :	15				
Pier Tie/Spiral Size, <b>St</b> :	4				
Pier Tie/Spiral Quantity, <b>mt</b> :	14				
Pier Reinforcement Type:	Tie				
Pier Clear Cover, <b>cc</b> <sub>pier</sub> :	3	in			

\*Rating per TIA-222-H Section 15.5

Pad Properties				
Depth, <b>D</b> :	6.5	ft		
Pad Width, <b>W</b> :	14	ft		
Pad Thickness, <b>T</b> :	2.5	ft		
Pad Rebar Size (Bottom), <b>Sp</b> :	8			
Pad Rebar Quantity (Bottom), <b>mp</b> :	13			
Pad Clear Cover, <b>cc<sub>pad</sub>:</b>	3	in		

Soil Rating*:	56.5%
Structural Rating*:	37.0%

Material Properties						
Rebar Grade, <b>Fy</b> :	60	ksi				
Concrete Compressive Strength, F'c:	3	ksi				
Dry Concrete Density, δ <b>c</b> :	150	pcf				

Soil Properties	S	
Total Soil Unit Weight, $\gamma$ :	120	pcf
Ultimate Net Bearing, Qnet:	10.000	ksf
Cohesion, <b>Cu</b> :	0.000	ksf
Friction Angle, $oldsymbol{arphi}$ :	30	degrees
SPT Blow Count, N <sub>blows</sub> :	21	
Base Friction, $\mu$ :	0.4	
Neglected Depth. N:	3.33	ft

Foundation Bearing on Rock?

Groundwater Depth, gw:

<--Toggle between Gross and Net



#### Address:

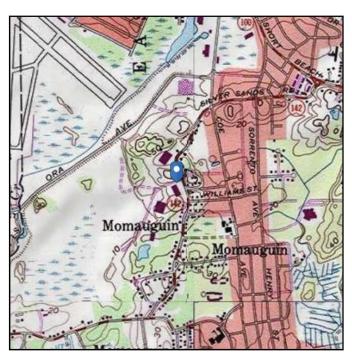
No Address at This Location

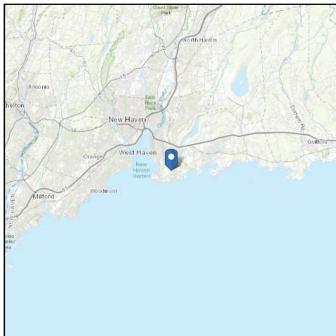
## ASCE 7 Hazards Report

Standard: ASCE/SEI 7-10 Elevation: 35.45 ft (NAVD 88)

Risk Category: || Latitude: 41.256356

Soil Class: D - Stiff Soil Longitude: -72.875778



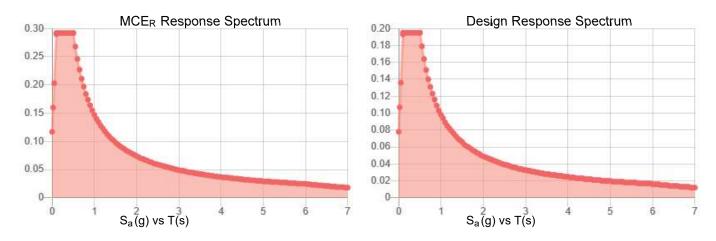




#### **Seismic**

Site Soil Class: Results:	D - Stiff Soil			
S <sub>s</sub> :	0.182	S <sub>DS</sub> :	0.195	
$S_1$ :	0.062	S <sub>D1</sub> :	0.098	
F <sub>a</sub> :	1.6	$T_L$ :	6	
F <sub>v</sub> :	2.4	PGA:	0.095	
S <sub>MS</sub> :	0.292	PGA <sub>M</sub> :	0.152	
S <sub>M1</sub> :	0.147	F <sub>PGA</sub> :	1.6	
		l. :	1	

#### Seismic Design Category B



Data Accessed: Tue Oct 20 2020

Date Source: USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating

Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with

ASCE/SEI 7-10 Ch. 21 are available from USGS.



#### **Ice**

#### Results:

Ice Thickness: 0.75 in.

Concurrent Temperature: 15 F

Gust Speed: 50 mph

**Data Source:** Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Tue Oct 20 2020

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

ASCE does not intend, nor should anyone interpret, the results provided by this Tool to replace the sound judgment of a competent professional, having knowledge and experience in the appropriate field(s) of practice, nor to substitute for the standard of care required of such professionals in interpreting and applying the contents of this Tool or the ASCE 7 standard.

In using this Tool, you expressly assume all risks associated with your use. Under no circumstances shall ASCE or its officers, directors, employees, members, affiliates, or agents be liable to you or any other person for any direct, indirect, special, incidental, or consequential damages arising from or related to your use of, or reliance on, the Tool or any information obtained therein. To the fullest extent permitted by law, you agree to release and hold harmless ASCE from any and all liability of any nature arising out of or resulting from any use of data provided by the ASCE 7 Hazard Tool.

## Exhibit E

**Mount Analysis** 



Date: October 19, 2020

Darcy Tarr Crown Castle

6325 Ardrey Kell Road, Suite 600

Charlotte, NC 28277

Paul J Ford and Company 250 E. Broad Street, Suite 600

Columbus, OH 43215 614.221.6679

Subject: **Mount Analysis Report** 

T-Mobile Equipment Change-out Carrier Designation:

Carrier Site Number: CT11623B N/A Carrier Site Name:

Crown Castle Designation: **Crown Castle BU Number:** 842862

> Crown Castle Site Name: East Haven South

**Crown Castle JDE Job Number:** 620146 Crown Castle Purchase Order Number: 1578176 **Crown Castle Order Number:** 529726 Rev. 0

Paul J Ford and Company Project Number: Engineering Firm Designation: A37520-2250.001.7190

Site Data: 259 Commerce Street, East Haven, New Haven County, CT 06512

Latitude41.256356°, Longitude-72.875778°

Structure Information: **Tower Height & Type:** 59 Foot Monopole

Mount Elevation: 47 Foot

MountType: (3)-Sector 12.5 Foot Platform

Dear Darcy Tarr,

Paul J Ford and Company is pleased to submit this "Mount Analysis Report" to determine the structural integrity of the T-Mobile antenna mounting system with the proposed appurtenance and equipment addition on the abovementioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point is not part of this document.

The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

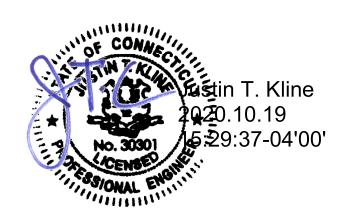
12.5' Platform SUFFICIENT

This analysis utilizes an ultimate 3-second gust wind speed of 130 mph as required by the 2018 Connecticut State Building Code and Appendix N. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount analysis prepared by: **MDS** 

Respectfully submitted by:

Deepesh Savla, PE ProiectManager dsavla@pauliford.com



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#### 2) ANALYSIS CRITERIA

Table 1 - Proposed Equipment Configuration

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Table 3 - Mount Component Capacity 4.1) Recommendations

#### 5) STANDARD CONDITIONS

#### 6) APPENDIX A

**SOFTWARE INPUT CALCULATIONS** 

#### 7) APPENDIX B

SOFTWAREANALYSISOUTPUT

#### 1) INTRODUCTION

The existing mount under consideration is a (3)-Sector 12.5' Platform mount, mapped by P-SEC on 04/16/2019.

#### 2) ANALYSIS CRITERIA

TIA-222Revision: TIA-222-H Risk Category: Ш **Ultimate Wind Speed:** 130 mph **ExposureCategory:** С Topographic Factor at Base: 1.00 **Topographic Factor at Mount:** 1.00 Ice Thickness: 1.5 in Wind Speed with Ice: 50 mph Seismic S<sub>s</sub>: 0.182 Seismic S<sub>1</sub>: 0.061 Maintenance Loading Wind Speed: 30 mph Maintenance Load at Mid/End-Points, L<sub>v</sub>: 250 lb Maintenance Load at Mount Pipes, L<sub>m</sub>: 500 lb

**Table 1 - Proposed Equipment Configuration** 

Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna <b>M</b> odel	Mount / Modification Details
		3	ERICSSON	AIR 32 B2A B66AA_T- MOBILE	
		3 ERICSSON		AIR6449 B41_T-MOBILE	(3) 12.5′
47	47	3	RFSCELWAVE	APXVAARR24-43-U-NA20	SECTOR
	3 ERICSSOI		ERICSSON	RADIO 4449 B71 B85A_T- MOBILE	MOUNTS
		3	ERICSSON	RRUS 4415 B25	

#### 3) ANALYSIS PROCEDURE

**Table 2 - Documents Provided** 

Document	Remarks	Reference	Source
Mount Mapping	P-Sec, 19651-21 Dated: 04/16/2019	8347391	CCISites
Photos	Dated: 09/24/2020	-	CCISites
Order	ID: 529726 Rev. 0 Dated: 10/13/2020	-	CCISites
Radio Frequency Data Sheet	RFDS ID #: CT11623B Version 7.00, Dated: 09/22/2020	-	Crown Castle

#### 3.1) Analysis Method

RISA-3D (version 17.0.4), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases.

A tool internally developed, using Microsoft Excel, by Paul J. Ford and Company was used to calculate wind loading on all appurtenances, dishes, and mount members for various load cases. Selected output from the analysis is included in Appendix B.

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 *Tower Mount Analysis* (Revision C).

#### 3.2) Assumptions

- 1) The analysis of the existing tower or the effect of the mount attachment to the tower is not within the current scope of work.
- 2) The antenna mounting system was properly fabricated, installed and maintained in good condition, twist free and plumb in accordance with its original design and manufacturer's specifications and all bolts are tightened as specified by the manufacturer and AISC requirements.
- The configuration of antennas, mounts, and other appurtenances are as specified in Table 1.
- All member connections have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report. All U-Bolt connections have been properly tightened. This analysis will be required to be revised if the existing conditions in the field differ from those shown in the above referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
- 5) Steel grades are as follows, unless noted otherwise:

a) Channel, Solid Round, Angle, Unistrut ASTM A53 (GR 35) b) Pipe ASTM A53 (GR 35)

c) HSS (Rectangular), Plate Q235 Gr B (Fy = 34 ksi, Fu = 58 ksi)

d) HSS (Round) ASTM A53 (GR 35)

e) Connection Bolts ASTM A325
f) Threaded Rods SAE J429 (GR2)
g) U-Bolts SAE J429 (GR2)

6) Proposed equipment is to be installed in the locations specified in Appendix A. Any changes to the proposed equipment locations will render this report invalid.

This analysis may be affected if any assumptions are not valid or have been made in error. Paul J Ford and Company should be notified to determine the effect on the structural integrity of the mount.

#### 4) ANALYSIS RESULTS

**Table 3- Mount Component Capacity** 

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1,2	Face Horizontals		20.5	Pass
1,2	Support Rails		24.4	Pass
1,2	Grating Support Members		24.9	Pass
1,2	StandoffMembers	47	60.2	Pass
1,2	Corner Plates		18.2	Pass
1,2	Mount Pipes		82.2	Pass
1,2	Mount to Tower Connection		45.8	Pass

Mount Rating (max from all components) = 82.2%
--

Notes:

#### 4.1) Recommendations

The mount has sufficient capacity to carry the proposed loading configuration. No modifications are required at this time.

<sup>1)</sup> See additional documentation in "Appendix B – Software Analysis Output" for calculations supporting the % capacity consumed.

<sup>2)</sup> Rating per TIA-222-H, Section 15.5

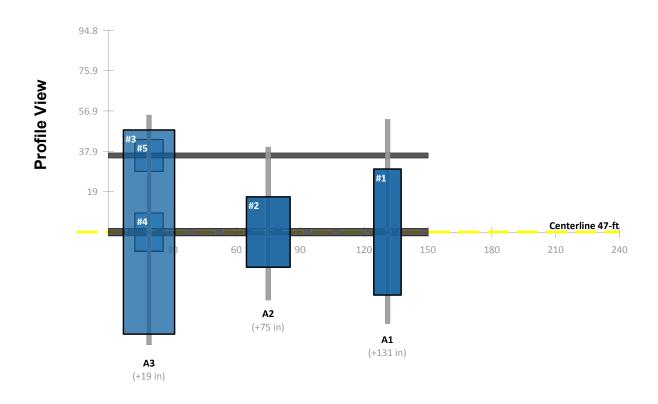
## STANDARD CONDITIONS FOR FURNISHING OF PROFESSIONAL ENGINEERING SERVICES ON EXISTING MOUNTS BY PAUL J. FORD AND COMPANY

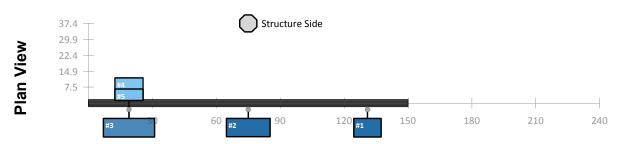
- 1) It is the responsibility of the client to ensure that the information provided to Paul J. Ford and Company is accurate and complete. Paul J. Ford and Company will rely on the accuracy and completeness of such information in performing or furnishing services under this project.
- 2) If the existing conditions are not as represented on the referenced drawings and/or documents, Paul J. Ford and Company should be contacted immediately to evaluate the significance of the deviation.
- 3) The mount has been analyzed according to the minimum design loads recommended by the Reference Standard. If additional design loads are required, Paul J. Ford and Company should be made aware of this prior to the start of the project.
- 4) The standard of care for all Professional Engineering Services performed or furnished by Paul J. Ford and Company under this project will be the skill and care used by members of the Consultant's profession practicing under similar circumstances at the same time and in the same locality.
- 5) All Services are performed, results obtained, and recommendations made in accordance with generally accepted engineering principles and practices. Paul J. Ford and Company is not responsible for the conclusions, opinions and/or recommendations made by others based on the information supplied herein.

\*

# APPENDIX A SOFTWARE INPUT CALCULATION

## **Sector A (typical)**





Ref ID	Туре	Manufacturer	Model	Height (in)	Width (in)	Depth (in)	Center Line (ft)	Mount Pipe	Tangential Offset (in)	Normal Offset (in)
#1	Antenna	ERICSSON	AIR 32 B2A B66AA_T-MOBILE	59.25	12.87	8.70	47.00	A1	0.00	3.00
#2	Antenna	ERICSSON	AIR6449 B41_T-MOBILE	33.11	20.51	8.54	47.00	A2	0.00	3.00
#3	Antenna	RFS CELWAVE	APXVAARR24-43-U-NA20	95.90	24.00	8.70	47.00	А3	0.00	3.00
#4	TME/RRH	ERICSSON	RADIO 4449 B71 B85A_T-MOBILE	17.91	13.20	10.63	47.00	А3	0.00	-3.00
#5	TME/RRH	ERICSSON	RRUS 4415 B25	14.96	13.19	5.39	50.00	A3	0.00	-3.00

<sup>1.</sup> A 6" tolerance for proposed equipment is

acceptable.

<sup>2.</sup> Contractor to verify location of existing equipment prior to installation of proposed equipment. Notify for any deviations.

<sup>3.</sup> Install shall not cause harm to the structure, climbing facility, safety climb, or any system installed on the structure



Mount Loading per TIA-222-H (version v3.1 - Effective 10/16/2020

(on all Appurt / Member h = 1.00 in (Bar Grating Height) (Table 2-3)
(Annex S - Ice)
(Section 2.6.11.6)
(Section 2.6.10)
in (Section 2.6.10) W<sub>i</sub> = 11.92 psf (Grating Ice Weight)  $K_{ss} = \begin{array}{c} 1.00 \\ K_{ss} = \\ R_{ss} = \begin{array}{c} 1.00 \\ R_{ss} = \\ 1.04 \\ R_{ss} = \begin{array}{c} 1.04 \\ 1.04 \\ R_{ss} = \\ 1.55 \\ R_{ss} = \end{array}$  $K_{\alpha} = 0.9$   $(q_{a}) (G_{b}) (K_{\alpha b}) = 44.32 \text{ psf}$   $(q_{a}) (G_{b}) (K_{\alpha b}) = 6.56 \text{ psf}$ Wind Pressure Ice Loading Override K<sub>c1</sub> = 1.00 (Section 2.6.6.2.1) K<sub>c2</sub> = 1.00 (Section 1.6.1) K<sub>c3</sub> = 1.00 (Section 2.6.8) K<sub>c4</sub> = 1.00 (Section 1.6.6) K<sub>c4</sub> = 1.00 (Section 1.6.6) K<sub>c4</sub> = 1.00 (Section 2.6.1)  $\begin{aligned} z_2 &= \frac{900 \, \text{ft}}{9.50} & \text{(Table 2-4)} \\ x_1 &= \frac{9.50}{1.08} & \text{(Section 2.6.5.2)} \\ K_{min} &= 0.95 \\ K_{man,8} &= 1.08 \\ K_{man,8} &= 2.01 \end{aligned}$ Velocity Pressure Coefficients L<sub>1</sub> = 250 lbs NS3 33 L<sub>2</sub> = 250 lbs NS8 58 L<sub>4</sub> = 250 lbs NS8 58 L<sub>m1</sub> = 600 lbs | Node # L<sub>m2</sub> = 500 lbs | N133 | 133 | 133 | 133 | 143 | 143 | 143 | 143 | 143 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 | 145 Maintenance Point Loads C:\PJF\MountTodProcessor\Files\Submission219\Risa219.r3d | Topography File Suffix= \_Client r3d Ice Thickness = 1.5 Analysis Scope = Client
Analysis Wind Direction Increment = 30°
EPA Calculation Method = TIA
Construction Duration = Risa3D Source: Mount Centerine (z) =  $\frac{3 \text{ Sectors}}{47.00}$  ft Risa  $\mathcal{G}_L$  Y-Coordinate =  $\frac{0.00}{1}$  in Structure Type = Mount Structure & Wind Speed Analysis Settings

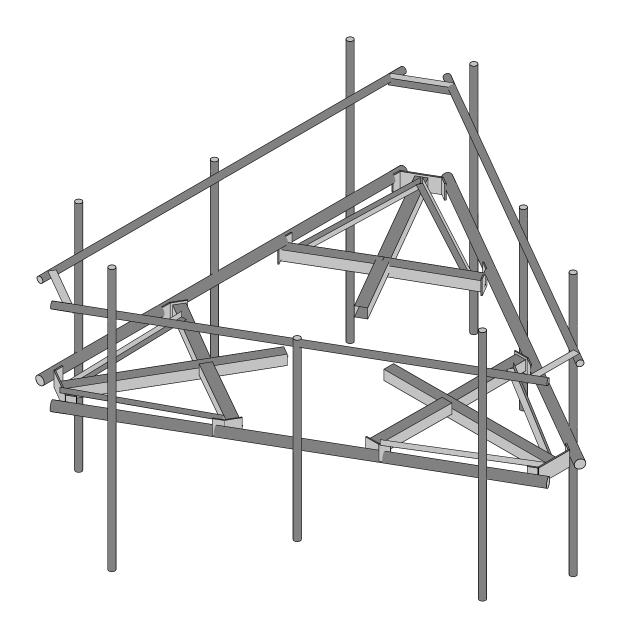
Antennas

ransverse Vind Force er Antenna (lbs)	198.684	98.776	354.521	63.279	27.075	198.684	98.776	354.521	63.279	27.075	98.684	98.776	354.521	63.279	27.075
->-											Ľ			H	
ide Normal Winc na Force per ing Antenna (lbs	273.202	225.713	807.383	78.578	65.586	273.202	225.713	807.383	78.578	65.586	273.202	225.713	807.383	78.578	65.586
ide Overri na Anten ing Mount on Locati (in)															
Overrid Top Antennoe Mountin Locatio															
Antenna Bottom Mount Location from Mount Pipe Bottom (in)	16.38	18.45	8.05	47.05	84.52	16.38	18.45	8.05	47.05	84.52	16.38	18.45	8.05	47.05	84.52
Antenna Top Mount Location from Mount Pipe Bottom (in)	69.63	45.56	97.95	58.96	93.48	69.63	45.56	97.95	58.96	93.48	69.63	45.56	97.95	58.96	93.48
Antenna C/L (ft)	47	47	47	47	20	47	47	47	47	20	47	47	47	47	20
Min Antenna C/L (ft)	45.635	45.463	46.329	43.080	42.957	45.635	45.463	46.329	43.080	42.957	45.635	45.463	46.329	43.080	42.957
Max Antenna C/L (ft)	49.198	49.204	47.838	51.087	51.210	49.198	49.204	47.838	51.087	51.210	49.198	49.204	47.838	51.087	51.210
Spacing (in)															
Top/Bottom C Mounting Point Spacing	53.25	27.11	99.90	11.91	96'8	53.25	27.11	06.68	11.91	96'8	53.25	27.11	06.68	11.91	8.96
Use tnxTower C <sub>s</sub> A <sub>s</sub> (CFD)	No	No	No	ON.	No	No	No	No	No	No	No	No	No	No	No
rientation	Normal	Normal	Normal	Normal	Normal	Normal	Normal	Normal	Normal	Normal	Normal	Normal	Normal	Normal	Normal
Quantity	1	-	-	-	1	-	-	-	-	-	-	-	-	-	1
Position	1	2	3	8	3	-	2	3	3	3	-	2	3	3	3
Sector / Face	٧	V	A	V	٧	8	8	8	8	8	ပ	ပ	ပ	ပ	ပ
Weight (lbs)	171.96	114.63	128	73.21	44	171.96	114.63	128	73.21	4	171.96	114.63	128	73.21	44
Flat or Round	Flat	Flat	Flat	Flat	Flat	Flat	Flat	Flat	Flat	Flat	Flat	Flat	Flat	Flat	Flat
) Depth (in)	8.7	8.54	8.7	10.63	5.39	8.7	8.54	8.7	10.63	5.39	8.7	8.54	8.7	10.63	5.39
Height (in) Width (in) Depth (in)	12.87	20.51	24	13.2	13.19	12.87	20.51	24	13.2	13.19	12.87	20.51	24	13.2	13.19
Height (	59.25	33.11	6.36	17.91	14.96	59.25	33.11	6.36	17.91	14.96	59.25	33.11	95.9	17.91	14.96
Automa	AIR 32 B2A B66AA_T-MOBILE	AIR6449 B41_T-MOBILE	APXVAARR24-43-U-NA20	RADIO 4449 B71 B85A_T-MOBILE	RRUS 4415 B25	AIR 32 B2A B66AA_T-MOBILE	AIR6449 B41_T-MOBILE	APXVAARR24-43-U-NA20	RADIO 4449 B71 B85A_T-MOBILE	RRUS 4415 B25	AIR 32 B2A B66AA_T-MOBILE	AIR6449 B41_T-MOBILE	APXVAARR24-43-U-NA20	RADIO 4449 B71 B85A_T-MOBILE	RRUS 4415 B25
Manufacturer	ERICSSON	ERICSSON	RFS CELWAVE	ERICSSON	ERICSSON	ERICSSON	ERICSSON	RFS CELWAVE	ERICSSON	ERICSSON	ERICSSON	ERICSSON	RFS CELWAVE	ERICSSON	ERICSSON
Mounting Location	Mount	Mount	Mount	Mount	Mount	Mount	Mount	Mount	Mount	Mount	Mount	Mount	Mount	Mount	Mount
	ъ	pe	pes	Proposed	Proposed	Proposed	Proposed	Proposed	Proposed	Proposed	Proposed	Proposed	Proposed	Proposed	Proposed
Status	Propose	Proposed	Proposed	Prop	Prop	Prop	Prop	Prop	Prop	Pro	Pro	Pro	Pro	Pro	P
Include Loading	Yes Proposed	Yes Propos	Yes Propo	Yes Prop	Yes Prop	Yes Prop	Yes Prop	Yes Prop	Yes Prop	Yes Pro	Yes Pro	Yes Pro	Yes Pro	Yes Pro	Yes Pro

Override Bottom Dish Mounting Location (in)
Override Top Dish Mounting Location (in)
Dish Bottom Mount Location from Mount Pipe Bottom
TopBootom Override Max Min Diela CL Location from Mount Location Mounting Specing Diela CL (ft) (ft) Mounting from Mounting Control Diela CL (ft) (ft) Mounting Control Diela Four Specing (in) CL (ft) CL (ft) (ft) Bottom
Dish C/L (ft)
Min Dish C/L (ft)
Max Dish C/L (ft)
Override Spacing (in)
TopBottom Override Max Min Mounting Specing Dish Dish Point Spacing (in) C/L (ft) C/L (ft)
Position
nt Sector/ Position Face
Weight Sector/ P. (lbs)
Dish Type
Dia (in)
Microwave Dish
Manufacturer
Mounting Location
Status
Include
Item

# APPENDIX B SOFTWARE ANALYSIS OUTPUT





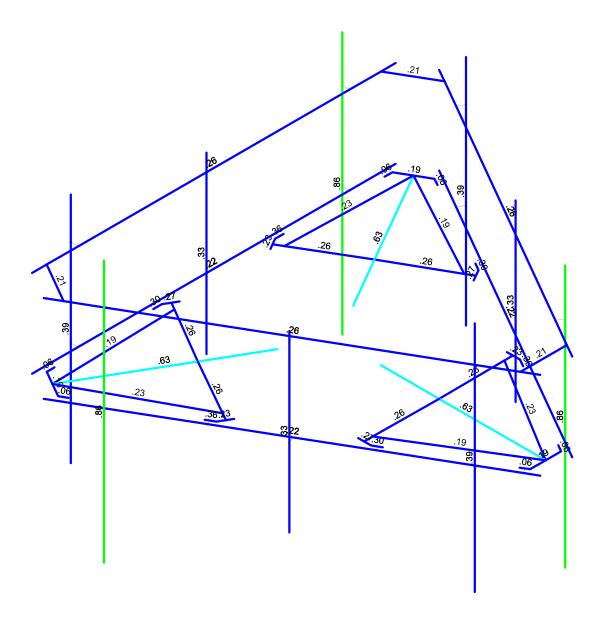
Envelope Only Solution

Paul J. Ford and Company
MDS
37520-2250.001.7190

SK	-	1	





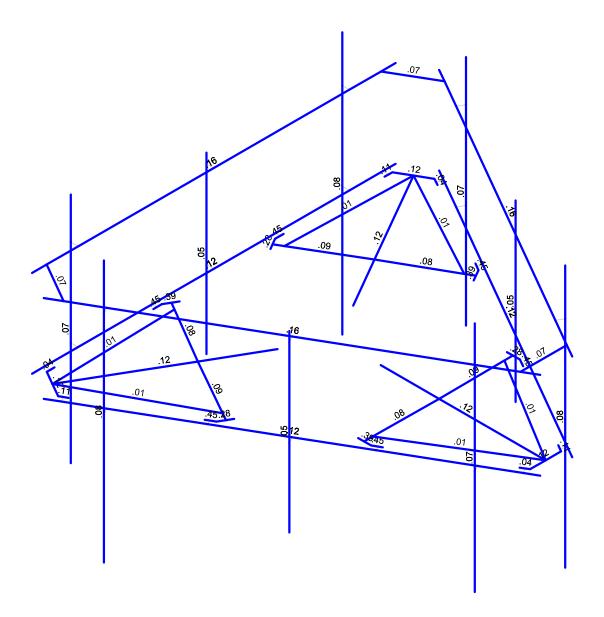


Member Code Checks Displayed (Enveloped) Envelope Only Solution

Paul J. Ford and Company		SK - 2
MDS	BU# 842862- East Haven South	Oct 19, 2020 at 2:35 PM
37520-2250.001.7190		Risa219_Client_2020-10-19T1139







Member Shear Checks Displayed (Enveloped) Envelope Only Solution

Paul J. Ford and Company		SK - 3
MDS	BU# 842862- East Haven South	Oct 19, 2020 at 2:35 PM
37520-2250.001.7190		Risa219_Client_2020-10-19T1139



Company : Paul J. Ford and Company
Designer : MDS
Job Number : 37520-2250.001.7190
Model Name : BU# 842862- East Haven S

: BU# 842862- East Haven South

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#### (Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	24
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Υ
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver

Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISAConnection Code	None
Cold Formed Steel Code	None
Wood Code	None
Wood Temperature	< 100F
Concrete Code	None
Masonry Code	None
Aluminum Code	None - Building
Stainless Steel Code	None

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parme Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	Yes
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR_SET_ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8

: Paul J. Ford and Company: MDS: 37520-2250.001.7190

BU# 842862- East Haven South

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## (Global) Model Settings, Continued

Seismic Code	ASCE 7-10
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
TX (sec)	Not Entered
T Z (sec)	Not Entered
RX	3
RZ	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	4
Cd X	4
Rho Z	1
Rho X	1

**Hot Rolled Steel Properties** 

		Label	E [ksi]	G [ksi]	Nu	Therm (/1E.	Density[k/ft	Yie <b>l</b> d[ksi]	Ry	Fu[ksi]	Rt
	1	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
	2	A53 Gr.B	29000	11154	.3	.65	.49	35	1.6	60	1.2
Ī	3	Q235 Gr B	29000	11154	3	65	527	34	1 4	58	1.3

#### Member Primary Data

	Label	I Joint	J Joint	K Joint	Rotate(deg)	Section/Shape	Туре	Design List	Material	Design Rules
1	M10	N11	N12			RIGID	None	None	RIGID	Typical
2	M14	N18	N19			RIGID	None	None	RIGID	Typical
3	M21	N29	N30			RIGID	None	None	RIGID	Typical
4	M22	N32	N31			RIGID	None	None	RIGID	Typical
5	M25	N35	N36			RIGID	None	None	RIGID	Typical
6	M29	N42	N43			RIGID	None	None	RIGID	Typical
7	M36	N54	N53			RIGID	None	None	RIGID	Typical
8	M37	N56	N55			RIGID	None	None	RIGID	Typical
9	M40	N60	N61			RIGID	None	None	RIGID	Typical
10	M43	N65	N66			RIGID	None	None	RIGID	Typical
11	M50	N77	N76			RIGID	None	None	RIGID	Typical
12	M51	N78	N79			RIGID	None	None	RIGID	Typical
13	M64	N93	N92			RIGID	None	None	RIGID	Typical
14	M65	N95	N94			RIGID	None	None	RIGID	Typical
15	M67	<b>N</b> 99	N98			RIGID	None	None	RIGID	Typical
16	M68	N101	N100			RIGID	None	None	RIGID	Typical
17	M70A	N105	N104			RIGID	None	None	RIGID	Typical
18	M71A	N107	N106			RIGID	None	None	RIGID	Typical
19	M15	N22	N20			PL6x0.375	None	None	Q235 Gr B	Typical
20	M16	N21	N22			PL6x0.375	None	None	Q235 Gr B	Typical
21	M18	N26	N24			PL6x0.375	None	None	Q235 Gr B	Typical
22	M19	N25	N26			PL6x0.375	None	None	Q235 Gr B	Typical
23	M30	N46	N44			PL6x0.375	None	None	Q235 Gr B	Typical



: Paul J. Ford and Company: MDS: 37520-2250.001.7190

BU# 842862- East Haven South

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#### Member Primary Data (Continued)

	oci i illilai		Oomana							
	Label	I Joint	J Joint	K Joint	Rotate(deg	Section/Shape	Type	Design List	Material	Design Rules
24	M31	N45	N46			PL6x0.375	None	None	Q235 Gr B	Typical
25	M33	N50	N48			PL6x0.375	None	None	Q235 Gr B	Typical
26	M34	N49	N50			PL6x0.375	None	None	Q235 Gr B	Typical
27	M44	N69	N67			PL6x0.375	None	None	Q235 Gr B	Typical
28	M45	N68	N69			PL6x0.375	None	None	Q235 Gr B	Typical
29	M47	N73	N71			PL6x0.375	None	None	Q235 Gr B	Typical
30	M48	N72	N73			PL6x0.375	None	None	Q235 Gr B	Typical
31	M9	N10	N15			PL 6" x 1/2"	None	None	Q235 Gr B	Typical
32	M12	N17	N15			PL 6" x 1/2"	None	None	Q235 Gr B	Typical
33	M13	N16	N17			PL 6" x 1/2"	None	None	Q235 Gr B	Typical
34	M24	N34	N39			PL 6" x 1/2"	None	None	Q235 Gr B	Typical
35	M27	N41	N39			PL 6" x 1/2"	None	None	Q235 Gr B	Typical
36	M28	N40	N41			PL 6" x 1/2"	None	None	Q235 Gr B	Typical
37	M39	N59	N62			PL 6" x 1/2"	None	None	Q235 Gr B	Typical
38	M41	N64	N62			PL 6" x 1/2"	None	None	Q235 Gr B	Typical
39	M42	N63	N64			PL 6" x 1/2"			Q235 Gr B	
	CBA1	N1	N2			PIPE 3.0	None	None	A53 Gr.B	Typical
40							None	None	A53 Gr.B	Typical
41	CBC1	N13	N14			PIPE 3.0	None	None	A53 Gr.B	Typical
42	CBB1	N37	N38			PIPE 3.0	None	None	A53 Gr.B	Typical
43	CBA2	N80	N81			PIPE 2.0	None	None	A53 Gr.B	Typical
44	CBC2	N82	N83			PIPE 2.0	None	None	A53 Gr.B	Typical
45	CBB2	N84	N85		070	PIPE 2.0	None	None	A53 Gr.B	Typical
46	<u>M2</u>	N9	N3		270	L2x2x3	None	None	A53 Gr.B	Typical
47	<u>M3</u>	N9	N4		070	L2x2x3	None	None	A53 Gr.B	Typical
48	<u>M4</u>	N33	N5		270	L2x2x3	None	None	A53 Gr.B	Typical
49	<u>M5</u>	N33	<u>N6</u>		070	L2x2x3	None	None	A53 Gr.B	Typical
50	<u>M6</u>	N58	N7		270	L2x2x3	None	None	A53 Gr.B	Typical
51	<u>M7</u>	N58	N8			L2x2x3	None	None	A53 Gr.B	Typical
52	<u>M73</u>	N87	N86		90	L2.5x2.5x4	None	None	A53 Gr.B	Typical
53	<u>M74</u>	N89	N88		90	L2.5x2.5x4	None	None	A53 Gr.B	Typical
54	M75	N91	N90		90	L2.5x2.5x4	None	None	A53 Gr.B	Typical
55	<u>M17</u>	N23	N28			HSS4X4X4	None	None	Q235 Gr B	Typical
56	M20	N27	N28			HSS4X4X4	None	None	Q235 Gr B	Typical
57	M32	N47	N52			HSS4X4X4	None	None	Q235 Gr B	Typical
58	M35	N51	N52			HSS4X4X4	None	None	Q235 Gr B	Typical
59	M38	N57	N58			HSS4X4X4	<u>None</u>	None	Q235 Gr B	Typical
60	M46	N70	N75			HSS4X4X4	None	None	Q235 Gr B	Typical
61	M49	N74	N75			HSS4X4X4	None	None	Q235 Gr B	Typical
62	M93	N110	N9			HSS4X4X4	None	None	Q235 Gr B	Typical
63	M95A	N111	N33			HSS4X4X4	None	None	Q235 Gr B	Typical
64	M67A	N116	N115			RIGID	None	None	RIGID	Typical
65	M68A	N118	N117			RIGID	None	None	RIGID	Typical
66	M69	N122	N121			RIGID	None	None	RIGID	Typical
67	M70	N124	N123			RIGID	None	None	RIGID	Typical
68	M71	N128	N127			RIGID	None	None	RIGID	Typical
69	M72	N130	N129			RIGID	None	None	RIGID	Typical
70	M76	N134	N133			RIGID	None	None	RIGID	Typical
71	M77	N136	N135			RIGID	None	None	RIGID	Typical
72	M78	N140	N139			RIGID	None	None	RIGID	Typical
73	<b>M</b> 79	N142	N141			RIGID	None	None	RIGID	Typical
74	M80	N146	N145			RIGID	None	None	RIGID	Typical
75	M81	N148	N147			RIGID	None	None	RIGID	Typical
76	C3	N96	N97			PIPE 2.0	None	None	A53 Gr.B	Typical
77	C2	N102	N103			PIPE 2.0	None	None	A53 Gr.B	Typical
78	C1	N108	N109			PIPE 2.0	None	None	A53 Gr.B	Typical
79	B1	N131	N132			PIPE 2.0	None	None	A53 Gr.B	Typical
80	A3	N137	N138			PIPE 2.0	None	None	A53 Gr.B	Typical



Company : Paul J. Ford and Company
Designer : MDS
Job Number : 37520-2250.001.7190
Model Name : BU# 842862- East Haven South

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#### Member Primary Data (Continued)

	Label	I Joint	J Joint	K Joint	Rotate(deg)	Section/Shape	Type	Design List	Material	Design Rules
81	A1	N149	N150		, ,	PIPE 2.0	None	None	A53 Gr.B	Typical
82	A2	N143	N144			PIPE 2.0	None	None	A53 Gr.B	Typical
83	B3	N119	N120			PIPE 2.0	None	None	A53 Gr.B	Typical
84	B2	N125	N126			PIPE 2.0	None	None	A53 Gr.B	Typical

#### Member Advanced Data

	Label	I Release	J Release	I Offset[in]	J Offset[in]	T/C Only	Physical	Defl Rat	Analysis	Inactive	Seismic
1 1	M10	BenPIN				·	Yes	** NA **	Ţ		None
2	M14	BenPIN					Yes	** NA **			None
3	M21		BenPIN				Yes	** NA **			None
4	M22	BenPIN					Yes	** NA **			None
5	M25	BenPIN					Yes	** NA **			None
6	M29	BenPIN					Yes	** NA **			None
7	M36	BenPIN					Yes	** NA **			None
8	M37	BenPIN					Yes	** NA **			None
9	M40	BenPIN					Yes	** NA **			None
10	M43	BenPIN					Yes	** NA **			None
11	M50	BenPIN					Yes	** NA **			None
12	M51	DeliFili	BenPIN				Yes	** NA **			None
13	M64		DeliFili				Yes	** NA **			None
14	M65						Yes	** NA **			None
15	M67						Yes	** NA **			
											None
16	M68						Yes	** NA **			None
17	M70A						Yes	** NA **			None
18	M71A						Yes	** NA **			None
19	<u>M15</u>						Yes	** NA **			None
20	M16						Yes	** NA **			None
21	<u>M18</u>						Yes	** NA **			None
22	M19						Yes	** NA **			None
23	<u>M30</u>						Yes	** NA **			None
24	M31						Yes	** NA **			None
25	M33						Yes	** NA **			None
26	M34						Yes	** NA **			None
27	M44						Yes	** NA **			None
28	M45						Yes	** NA **			None
29	M47						Yes	** NA **			None
30	M48						Yes	** NA **			None
31	M9						Yes	** NA **			None
32	M12						Yes	** NA **			None
33	M13						Yes	** NA **			None
34	M24						Yes	** NA **			None
35	M27						Yes	** NA **			None
36	M28						Yes	** NA **			None
37	M39						Yes	** NA **			None
38	M41						Yes	** NA **			None
39	M42						Yes	** NA **			None
40	CBA1						Yes	** NA **			None
41	CBC1						Yes	** NA **			None
42	CBB1						Yes	** NA **			None
43	CBA2						Yes	** NA **			None
44	CBC2						Yes	** NA **			None
45	CBB2						Yes	** NA **			None
46	M2	BenPIN	BenPIN				Yes	** NA **			None
47	M3	BenPIN	BenPIN				Yes	** NA **			None
48	M4	BenPIN	BenPIN				Yes	** NA **			None



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#### Member Advanced Data (Continued)

	Label	I Release	J Release	I Offset[in]	J Offset[in]	T/C Only	Physical	Defl RatAnalysis	Inactive	Seismic
49	M5	BenPIN	BenPIN				Yes	** NA **		None
50	M6	BenPIN	BenPIN				Yes	** NA **		None
51	M7	BenPIN	BenPIN				Yes	** NA **		None
52	M73						Yes	** NA **		None
53	M74						Yes	** NA **		None
54	M75						Yes	** NA **		None
55	M17						Yes	** NA **		None
56	M20						Yes	** NA **		None
57	M32						Yes	** NA **		None
58	M35						Yes	** NA **		None
59	M38						Yes	** NA **		None
60	M46						Yes	** NA **		None
61	M49						Yes	** NA **		None
62	M93						Yes	** NA **		None
63	M95A						Yes	** NA **		None
64	M67A						Yes	** NA **		None
65	M68A						Yes	** NA **		None
66	M69						Yes	** NA **		None
67	<b>M</b> 70						Yes	** NA **		None
68	M71						Yes	** NA **		None
69	M72						Yes	** NA **		None
70	M76						Yes	** NA **		None
71	M77						Yes	** NA **		None
72	M78						Yes	** NA **		None
73	M79						Yes	** NA **		None
74	M80						Yes	** NA **		None
75	M81						Yes	** NA **		None
76	C3						Yes	** NA **		None
77	C2						Yes	** NA **		None
78	C1						Yes	** NA **		None
79	B1						Yes	** NA **		None
80	A3						Yes	** NA **		None
81	A1						Yes	** NA **		None
82	A2						Yes	** NA **		None
83	B3						Yes	** NA **		None
84	B2						Yes	** NA **		None

#### Hot Rolled Steel Design Parameters

	Label	Shape	Length[in]	Lbyy[in]	Lbzz[in]	Lcomp top[in]Lcomp bot[in]L-t	torqu k	<b>C</b> yy	Kzz	Cb	<u>Function</u>
1	M15	PL6x0.37	5.363			Lbyy					Lateral
2	M16	PL6x0.37	5 3.499			Lbyy					Lateral
3	M18	PL6x0.37	5.363			Lbyy					Lateral
4	M19	PL6x0.37	5 3.499			Lbyy					Lateral
5	M30	PL6x0.37	5.363			Lbyy					Lateral
6	M31	PL6x0.37	5 3.499			Lbyy					Lateral
7	M33	PL6x0.37	5.363			Lbyy					Lateral
8	M34	PL6x0.37	5 3.499			Lbyy					Lateral
9	M44	PL6x0.37	5.363			Lbyy					Lateral
10	M45	PL6x0.37	5 3.499			Lbyy					Lateral
11	M47	PL6x0.37	5.363			Lbyy					Lateral
12	M48	PL6x0.37	5 3.499			Lbyy					Lateral
13	M9	PL 6" x 1/2'	3.184			Lbyy					Lateral
14	M12	PL 6" x 1/2"	12.707			Lbyy					Lateral
15	M13	PL 6" x 1/2'	3.184			Lbyy					Lateral
16	M24	PL 6" x 1/2"	3.184			Lbyy					Lateral

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#### Hot Rolled Steel Design Parameters (Continued)

	Label	Shape	Length[in]	Lbyy[in]	Lbzz[in]	Lcomp top[in]	Lcomp bot[in]L	torqu	Куу	Kzz	Cb	Function
17	M27	PL 6" x 1/2"	12,707			Lbyy						Lateral
18	M28	PL 6" x 1/2"	3.184			Lbyy						Lateral
19	M39	PL 6" x 1/2"	3.184			Lbyy						Lateral
20	M41	PL 6" x 1/2"	12.707			Lbyy						Lateral
21	M42	PL 6" x 1/2"	3.184			Lbyy						Lateral
22	CBA1	PIPE_3.0	150			Lbyy						Lateral
23	CBC1	PIPE 3.0	150			Lbyy						Lateral
24	CBB1	PIPE_3.0	150			Lbyy						Lateral
25	CBA2	PIPE 2.0	150			Lbyy						Lateral
26	CBC2	PIPE_2.0	150			Lbyy						Lateral
27	CBB2	PIPE 2.0	150			Lbyy						Lateral
28	M2	L2x2x3	51.837			Lbyy						Lateral
29	M3	L2x2x3	51.837			Lbyy						Lateral
30	M4	L2x2x3	51.837			Lbyy						Lateral
31	M5	L2x2x3	51.837			Lbyy						Lateral
32	M6	L2x2x3	51.837			Lbyy						Lateral
33	M7	L2x2x3	51.837			Lbyy						Lateral
34	M73	L2.5x2.5x4										Lateral
35	M74	L2.5x2.5x4										Lateral
36	M75	L2.5x2.5x4										Lateral
37	M17	HSS4X4X4	30.71			Lbyy						Lateral
38	M20	HSS4X4X4	30.71			Lbyy						Lateral
39	M32	HSS4X4X4	30.71			Lbyy						Lateral
40	M35	HSS4X4X4	30.71			Lbyy						Lateral
41	M38	HSS4X4X4	68.014			Lbyy						Lateral
42	M46	HSS4X4X4	30.71			Lbyy						Lateral
43	M49	HSS4X4X4	30.71			Lbyy						Lateral
44	M93	HSS4X4X4	68.014			Lbyy						Lateral
45	M95A	HSS4X4X4	68.014			Lbyy						Lateral
46	C3	PIPE 2.0	108									Lateral
47	C2	PIPE 2.0	72									Lateral
48	C1	PIPE 2.0	96									Lateral
49	B1	PIPE 2.0	96									Lateral
50	A3	PIPE_2.0	108									Lateral
51	A1	PIPE 2.0	96									Lateral
52	A2	PIPE_2.0	72									Lateral
53	B3	PIPE 2.0	108									Lateral
54	B2	PIPE 2.0	72									Lateral

#### **Basic Load Cases**

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut	.Area(Me	.Surface(
1	Dead	None		-1.1			30		3	
2	Wind 0	None					60	108		
3	Wind 30	None					60	108		
4	Wind 60	None					60	108		
5	Wind 90	None					60	108		
6	Wind 120	None					60	108		
7	Wind 150	None					60	108		
8	Ice Load	None					30	54	3	
9	Ice 0	None					60	108		
10	Ice 30	None					60	108		
11	Ice 60	None					60	108		
12	Ice 90	None					60	108		
13	Ice 120	None					60	108		
14	Ice 150	None					60	108		



Company : Paul J. Ford and Company
Designer : MDS
Job Number : 37520-2250.001.7190
Model Name : BU# 842862- East Haven South

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#### Basic Load Cases (Continued)

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut	Area(Me	Surface(
15	Lm1	None		•	,	1			,	,
16	Lm2	None				1				
17	Lm3	None				1				
18	Lm4	None				1				
19	Lv1	None				1				
20	Lv2	None				1				
21	Lv3	None				1				
22	Lv4	None				1				
23	BLC 1 Transient Area Loads	None						75		
24	BLC 8 Transient Area Loads	None						75		

#### **Load Combinations**

1	
3	
4	
5	
6	
7	
8	
9	
10	
11	
12	
13	
14       1.2 D + 1.0 Di + 1.0 Wi @ 0       Yes Y       1       1.2 8       1       9       1         15       1.2 D + 1.0 Di + 1.0 Wi @ 30       Yes Y       1       1.2 8       1       10       1         16       1.2 D + 1.0 Di + 1.0 Wi @ 60       Yes Y       1       1.2 8       1       11       1         17       1.2 D + 1.0 Di + 1.0 Wi @ 120       Yes Y       1       1.2 8       1       12       1         18       1.2 D + 1.0 Di + 1.0 Wi @ 150       Yes Y       1       1.2 8       1       14       1         20       1.2 D + 1.0 Di + 1.0 Wi @ 180       Yes Y       1       1.2 8       1       9 -1         21       1.2 D + 1.0 Di + 1.0 Wi @ 210       Yes Y       1       1.2 8       1       10 -1         22       1.2 D + 1.0 Di + 1.0 Wi @ 240       Yes Y       1       1.2 8       1       11 -1	
15  1.2 D + 1.0 Di + 1.0 Wi @ 30 Yes Y	
16 1.2 D + 1.0 Di + 1.0 Wi @ 60 Yes Y	
17 1.2 D + 1.0 Di + 1.0 Wi @ 90 Yes Y	
18 1.2 D + 1.0 Di + 1.0 Wi @ 120 Yes Y	
19 1.2 D + 1.0 Di + 1.0 Wi @ 150 Yes Y	
20 1.2 D + 1.0 Di + 1.0 Wi @ 180 Yes Y	
21 1.2 D + 1.0 Di + 1.0 Wi @ 210 Yes Y	
22   1.2 D + 1.0 Di + 1.0 Wi @ 240   Yes   Y     1   1.2   8   1   11   -1	
22   1.2 D + 1.0 Di + 1.0 Wi @ 240   Yes   Y     1   1.2   8   1   11   -1	
23   1.2 D + 1.0 Di + 1.0 Wi @ 270   Yes   Y     1   1.2   8   1   12   -1	
24   1.2 D + 1.0 Di + 1.0 Wi @ 300   Yes   Y     1   1.2   8   1   13   -1	
25   1.2 D + 1.0 Di + 1.0 Wi @ 330   Yes   Y     1   1.2   8   1   14   -1	
26 1.2 D + 1.5 Lm1 + 1.0 Wm @ 0 Yes Y	
27 1.2 D + 1.5 Lm1 + 1.0 Wm @ 30 Yes Y 1 1.2 15 1.5 3 .053	
28 1.2 D + 1.5 Lm1 + 1.0 Wm @ 60 Yes Y 1 1 1.2 15 1.5 4 .053	
29 1.2 D + 1.5 Lm1 + 1.0 Wm @ 90 Yes Y 1 1.2 15 1.5 5 .053	
30   1.2 D + 1.5 Lm1 + 1.0 Wm @ 120   Yes   Y     1   1.2   15   1.5   6   .053	
31 1.2 D + 1.5 Lm1 + 1.0 Wm @ 150 Yes Y 1 1.2 15 1.5 7 .053	
32 1.2 D + 1.5 Lm1 + 1.0 Wm @ 180 Yes Y 1 1.2 15 1.5 2 .053	
33 1.2 D + 1.5 Lm1 + 1.0 Wm @ 210 Yes Y 1 1.2 15 1.5 3 .053	
34 1.2 D + 1.5 Lm1 + 1.0 Wm @ 240 Yes Y 1 1.2 15 1.5 4 .053	
35   1.2 D + 1.5 Lm1 + 1.0 Wm @ 270   Yes Y     1   1.2   15   1.5   5   .053	
36 1.2 D + 1.5 Lm1 + 1.0 Wm @ 300 Yes Y 1 1.2 15 1.5 6 .053	
37   1.2 D + 1.5 Lm1 + 1.0 Wm @ 330   Yes Y     1   1.2   15   1.5   7   .053	
38 1.2 D + 1.5 Lm2 + 1.0 Wm @ 0 Yes Y	
39 1.2 D + 1.5 Lm2 + 1.0 Wm @ 30 Yes Y 1 1.2 16 1.5 3 .053	
40 1.2 D + 1.5 Lm2 + 1.0 Wm @ 60 Yes Y 1 1.2 16 1.5 4 .053	
41 1.2 D + 1.5 Lm2 + 1.0 Wm @ 90 Yes Y 1 1.2 16 1.5 5 .053	
42 1.2 D + 1.5 Lm2 + 1.0 Wm @ 120 Yes Y 1 1.2 16 1.5 6 .053	



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#### **Load Combinations (Continued)**

	Description	S P	S B	. FaB	. Fa	В	Fa	B F	-al	B Fa	.B	Fa	В	Fa	В	Fa	В	Fa	В	Fa
43	1.2 D + 1.5 Lm2 + 1.0 Wm @ 150	Yes Y	1	1.2 16	1.5	7	.053													
44	1.2 D + 1.5 Lm2 + 1.0 Wm @ 180	Yes Y	1	1.2 16			.053													
45	1.2 D + 1.5 Lm2 + 1.0 Wm @ 210	Yes Y	1	1.2 16	1.5	3	.053													
46	1.2 D + 1.5 Lm2 + 1.0 Wm @ 240	Yes Y	1	1.2 16	1.5	4	.053													
47	1.2 D + 1.5 Lm2 + 1.0 Wm @ 270	Yes Y	1	1.2 16	1.5	5	.053													
48	1.2 D + 1.5 Lm2 + 1.0 Wm @ 300	Yes Y	1	1.2 16	1.5	6	.053													
49	1.2 D + 1.5 Lm2 + 1.0 Wm @ 330	Yes Y	1	1.2 16			.053													
50	1.2 D + 1.5 Lm3 + 1.0 Wm @ 0		1	1.2 17			.053													
51	1.2 D + 1.5 Lm3 + 1.0 Wm @ 30	Yes Y	1	1.2 17			.053													
52	1.2 D + 1.5 Lm3 + 1.0 Wm @ 60	Yes Y	1	1.2 17			.053													
53	1.2 D + 1.5 Lm3 + 1.0 Wm @ 90	Yes Y	1	1.2 17			.053													
54	1.2 D + 1.5 Lm3 + 1.0 Wm @ 120	Yes Y	1	1.2 17			.053													
55	1.2 D + 1.5 Lm3 + 1.0 Wm @ 150	Yes Y	1	1.2 17			.053													
56	1.2 D + 1.5 Lm3 + 1.0 Wm @ 180	Yes Y	1	1.2 17			.053													
57	1.2 D + 1.5 Lm3 + 1.0 Wm @ 210	Yes Y	1	1.2 17			.053													
58	1.2 D + 1.5 Lm3 + 1.0 Wm @ 240	Yes Y	1	1.2 17			.053													
59	1.2 D + 1.5 Lm3 + 1.0 Wm @ 270	Yes Y	1	1.2 17			.053													
60	1.2 D + 1.5 Lm3 + 1.0 Wm @ 300	Yes Y	1	1.2 17			.053													
61	1.2 D + 1.5 Lm3 + 1.0 Wm @ 330	Yes Y	1	1.2 17			.053													
62	1.2 D + 1.5 Lm4 + 1.0 Wm @ 0		1				.053													
63	1.2 D + 1.5 Lm4 + 1.0 Wm @ 30	Yes Y	1	1.2 18			.053													
64	1.2 D + 1.5 Lm4 + 1.0 Wm @ 60	Yes Y	1				.053													
65	1.2 D + 1.5 Lm4 + 1.0 Wm @ 90	Yes Y	1	1.2 18			.053													
66	1.2 D + 1.5 Lm4 + 1.0 Wm @ 120	Yes Y	1	1.2 18			.053													
67	1.2 D + 1.5 Lm4 + 1.0 Wm @ 150	Yes Y	1	1.2 18			.053													
68	1.2 D + 1.5 Lm4 + 1.0 Wm @ 180	Yes Y	1				0													
69	1.2 D + 1.5 Lm4 + 1.0 Wm @ 210	Yes Y	1	1.2 18	1.5	3	0													
70	1.2 D + 1.5 Lm4 + 1.0 Wm @ 240	Yes Y	1	1.2 18			0													
71	1.2 D + 1.5 Lm4 + 1.0 Wm @ 270	Yes Y	1	1.2 18			0													
72	1.2 D + 1.5 Lm4 + 1.0 Wm @ 300	Yes Y	1	1.2 18	1.5	6	0													
73	1.2 D + 1.5 Lm4 + 1.0 Wm @ 330	Yes Y	1	1.2 18			0													
74	1.2 D + 1.5 Lv1	Yes Y	1																	
75	1.2 D + 1.5 Lv2	Yes Y		1.2 20																
76	1.2 D + 1.5 Lv3	Yes Y	1	1.2 21																
77	1.2 D + 1.5 Lv4	Yes Y	1	1.2 22	1.5															
78	1.0 D	Yes Y	1	1																

#### **Envelope Joint Reactions**

	Joint		X [lb]	LC	Y [ <b>l</b> b]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N57	max	2188.431	11	2821.009	17	1231 861	2	478	34	2.275	8	7 145	17
2		min	-2105.819	5	1015.191	78	-1233.156	8	218	55	-2.281	2	2.544	78
3	N110	max	1638.363	11	2821.074	25	1812.697	13	6.097	25	2.275	4	-1.193	6
4		min	-1680.747	5	1015.22	78	-1882.39	7	2.19	78	-2.281	10	-3.739	24
5	N111	max	1154.793	11	2821.055	21	2050.813	2	-2.217	78	2.275	12	-1.201	34
6		min	-1195.02	5	1015.209	78	-1974.327	8	-6.278	21	-2.281	6	-3.421	22
7	Totals:	max	4981.586	11	8300.359	22	4981.595	2						
8		min	-4981.587	5	3045.62	78	-4981.593	8						

#### Envelope AISC 15th(360-16): LRFD Steel Code Checks

	Member	Shape	Code Check	Loc[in] LC	Shear Check	Loc[in]	Dir LC	phi*Pnc	.phi*Pnt [	.phi*Mn y	.phi*Mn z	.Cb Eqn
1	C3	PIPE 2.0	.863	52.87511	.083	54	11	12143.9	32130	1.872	1.872	1H1-1b
2	B3	PIPE 2.0	.863	52.875 7	.083	54	7	12143.9	32130	1.872	1.872	1H1-1b
3	A3	PIPE 2.0	.863	52.875 3	.083	54	3	12143.9	32130	1.872	1.872	2H1-1b
4	M93	HSS4X4X4	.632	0 22	.120	0	v 44	93367.2	103122	11.96	11.96	2H1-1b

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#### Envelope AISC 15th(360-16): LRFD Steel Code Checks (Continued)

	Member	Shape	Code Check	Loc[in] LC	Shear Check	Loc[in]	Dir	LC phi*Pnc	phi*Pnt [	phi*Mn y.	phi*Mn zCb_ Eqn_
5	M95A	HSS4X4X4	.632	0 18	.120	0	У	61 93367.2	103122	11.96	11.96 2H1-1b
6	M38	HSS4X4X4	.632	0 14	.120	0	У	34 93367.2	103122	11.96	11.96 2H1-1b
7	C1	PIPE 2.0	.391	43 14	.072	43		24 14916.0	32130	1.872	1.872 2H1-1b
8	A1	PIPE 2.0	.391	43 18	.072	43		16 14916.0	32130	1.872	1.872 1H1-1b
9	B1	PIPE 2.0	.391	43 22	.072	43		20 14916.0	32130	1.872	1.872 1H1-1b
10	M45	PL6x0.375	.362	1.422 10	.455	3.499	v	17 65316.7	68850	.538	8.606 1H1-1b
11	M31	PL6x0.375	.362	1.422 2	.455	3.499	v	21 65316.7	68850	.538	8.606 1H1-1b
12	M16	PL6x0.375	.362	1.422 6	.455	3.499		25 65316.7	68850	.538	8.606 1H1-1b
13	A2	PIPE 2.0	.326	32.25 9	.049	32.25	,	3 20866.7	32130	1.872	1.872 1H1-1b
14	B2	PIPE 2.0	.326	32.25 13	.049	32.25		7 20866.7	32130	1.872	1.872 1H1-1b
15	C2	PIPE 2.0	.326	32.25 5	.049	32.25		11 20866.7	32130	1,872	1,872 1H1-1b
16	M19	PL6x0.375	.297	1.422 8	.446	3.499	v	25 65316.7	68850	.538	8.606 1H1-1b
17	M34	PL6x0.375	.297	1.422 4	.446	3,499		21 65316.7	68850	.538	8.606 1H1-1b
18	M48	PL6x0.375	.297	1.422 12	.446	3.499		17 65316.7	68850	.538	8.606 1H1-1b
19	M18	PL6x0.375	.272	2.849 2	.386	2.849		23 60835.0	68850	.538	8.606 1H1-1b
20	M33	PL6x0.375	.272	2.849 10	.386	2.849		19 60835.0	68850	.538	8.606 1H1-1b
21	M47	PL6x0.375	.272	2.849 6	.386	2,849		15 60835.0	68850	.538	8.606 1H1-1b
22	M20	HSS4X4X4	.263	30.71 14	.076	30.71		14 101053	103122	11.96	11.96 1H1-1b
23	M35	HSS4X4X4	.263	30.71 22	.076 .076	30.71		22 101053	103122	11.96	11.96 1H1-1b
24	M49	HSS4X4X4	.263	30.71 18	.076	30.71	V	18 101053	103122		11.96 1H1-1b
	M17	HSS4X4X4	.263 .263	30.71 24	<u>.076</u> _086	30.71		25 101053	103122	<u>11.96</u>	
25		HSS4X4X4				30.71		21 101053		<u>11.96</u> 11.96	11.96 1H1-1b
26	M32	HSS4X4X4	.263		<u>.086</u>	30.71		17 101053	103122		
27	M46		.263		.086		У		103122	11.96	
28	CBC2	PIPE 2.0	.259	75 19	.165	6.25				1.872	1.872 1H1-1b
29	CBA2	PIPE 2.0	.259	75 23	.165	6.25		9 6295.422		1.872	1.872 1H1-1b
30	CBB2	PIPE 2.0	.259	75 15	.165	6.25		13 6295.422		1.872	1.872 1H1-1b
31	<u>M30</u>	PL6x0.375	.233	2.849 8	.283	2.849		25 60835.0	68850	.538	8.606 1H1-1b
32	<u>M15</u>	PL6x0.375	.233	2.849 12	.283	2.849		17 60835.0	68850	.538	8.606 1H1-1b
33	<u>M44</u>	PL6x0.375	.233	2.849 4	.283	2.849		21 60835.0	68850	.538	8.606 1H1-1b
34	<u>M6</u>	L2x2x3	.230	25.918 3	.010			20 9165.131	22743	.542	1.04 1 H2-1
35	<u>M2</u>	L2x2x3	.230	25.91811	<u>.010</u>	51.837		16 9165.131	22743	542	1.04 1 H2-1
36	<u>M4</u>	L2x2x3	.230	25.918 7	.010	51.837		24 9165.131	22743	.542	1.04 1 H2-1
37	CBC1	PIPE 3.0	.217	100 22	.122	48.438	_	11 28250.5	65205	5.749	5.749 2H1-1b
38	CBA1	PIPE_3.0	.217	100 14	.122	48.438	_	3 28250.5	65205	5.749	5.749 2H1-1b
39	CBB1	PIPE 3.0	.217	100 18	.122	48.437		7 28250.5	65205	5.749	5.749 2H1-1b
40	<u>M74</u>	L2.5x2.5x4	.213	19.173 9	.066	0	Z	9 34569.0	37485	1.083	2.467 1 H2-1
41	M75	L2.5x2.5x4	.213	19.173 5	.066	0	z	5 34569.0	37485	<u> 1.083</u>	2.467 1 H2-1
42	M73	L2.5x2.5x4	.213	19.17313	.066	0	_	13 34569.0	37485	1.083	2.467 1 H2-1
43	<u> </u>	L2x2x3	.190	25.918 7	.012	51.837		15 9165.131	22743	542	1.04 1 H2-1
44	<u>M3</u>	L2x2x3	.190	25.918 3	.012	51.837		23 9165.131	22743	.542	1.04 1 H2-1
45	M5	L2x2x3	.190	25.91811	.012			19 9165.131		.542	1.04 1 H2-1
46	M12	PL 6" x 1/2"	.188	6.354 8	.118			22 62633.4		.956	11.475 1H1-1b
47	M27	PL 6" x 1/2"	.188	6.354 4	.118			60 62633.4		.956	11.475 1H1-1b
48	M41	PL 6" x 1/2"	.188	6.354 12	.118			32 62633.4		.956	11.475 1H1-1b
49	M39	PL 6" x 1/2"	.063	1.659 8	.114			32 89622.19		.956	11.475 2H1-1b
50	M9	PL 6" x 1/2"	.063	1.659 4	.112	3.184		49 89622.19		.956	11.475 2H1-1b
51	M24	PL 6" x 1/2"	.063	1.659 12	.114			60 89622.19		.956	11.475 2H1-1b
52	M13	PL 6" x 1/2"	.058	3.184 2	.041			10 89622.19		.956	11.475 2H1-1b
53	M28	PL 6" x 1/2"	.058	3.184 10	.041			60 89622.19		.956	11.475 2H1-1b
54	M42	PL 6" x 1/2"	.058	3.184 6	.041	1.659	у	32 89622.19	91800	.956	11.475 2H1-1b

37520-2250.001.7190 Project # 3

Date: 10/19/20 v3.6, Effective 08/10/2020

<b>MOUNT TO TOWER CONNECTION CHECKS-LRFD</b>		
IT TO TOW	15th	hecks Bolts & Welds
MOUN	AISC	Checks
	H-15.5	3-Sector
	TIA Rev.	Mount Type

	Moment about Vertical axis Torque (ft-k) (ft-k)	2.27 0.48
D	Moment about horizontal axis (ft-k)	7.15
<b>REACTIONS FROM RISA-3D</b>	Axial along member(k)	2.19
REACTIO	Vertical Shear (k)	2.82
	Horizontal Shear (k)	1.23
	רכ	Envelope
	NODE	<b>29N</b>

	Horizontal Bolt	Horizontal Bolt spacing (B) (in)
'ertical Bolt spacing   Horizonta		(D) (in) spacing (E
<u> </u>		Qualitity
:		Dia (III)
ŀ		adk-
41.0	<u>-</u>	Information

8	Note: Tension reduction not required if tension or shear canacity < 30%	ired if tension or	duction not requi	Note: Tension re
	8.9%	12.4	1.1.1	SHEAR (k)
	-	Reduced Tensile Rating	Reduce	
	48.1%	20.7	96.6	TENSION (K)
	Rating	Strength	Forces	CHECKS

Note: Lension reduction not required if tension of shear capacity < 30%

<u></u>	
	*

Standoff	Туре	Width (b) (in)	Depth (d) (in)	thickness (in)	Weld Size	Weld Assumed?	Stiffeners pres
Member	Rectangle	4	7	0.25	0.375	No	No

ffeners present

Rating	44.1%				
Strength (k)	8.35				
Resultant (k) 3.68					
WELDS CHECKS					

45.8% Controlling Rating per TIA-222-H Section 15.5:

# Exhibit F

**Power Density/RF Emissions Report** 



# RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CT11623B

CT623/E.Haven ATT\_MP
259 Commerce Street
East Haven, Connecticut 06512

May 27, 2021

EBI Project Number: 6220005731

Site Comp	liance Summary
Compliance Status:	COMPLIANT
Site total MPE% of FCC general population allowable limit:	98.08%



May 27, 2021

T-Mobile Attn: Jason Overbey, RF Manager 35 Griffin Road South Bloomfield, Connecticut 06002

Emissions Analysis for Site: CT11623B - CT623/E.Haven ATT\_MP

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **259 Commerce Street** in **East Haven, Connecticut** for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu$ W/cm²). The number of  $\mu$ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits; therefore, it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu$ W/cm²). The general population exposure limits for the 600 MHz and 700 MHz frequency bands are approximately 400  $\mu$ W/cm² and 467  $\mu$ W/cm², respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 11 GHz frequency bands is 1000  $\mu$ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

#### **CALCULATIONS**

Calculations were done for the proposed T-Mobile Wireless antenna facility located at 259 Commerce Street in East Haven, Connecticut using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was focused at the base of the tower. For this report, the sample point is the top of a 6-foot person standing at the base of the tower. For power density calculations, the broadcast footprint of the AIR6449 antenna has been considered. Due to the beamforming nature of this antenna, the actual beam locations vary depending on demand and are narrow in nature. Using the broadcast footprint accounts for the potential location of beams at any given time.

For all calculations, all equipment was calculated using the following assumptions:

- 1) I LTE channel (600 MHz Band) was considered for each sector of the proposed installation. This Channel has a transmit power of 40 Watts.
- 2) I NR channel (600 MHz Band) was considered for each sector of the proposed installation. This Channel has a transmit power of 80 Watts.
- 3) I LTE channel (700 MHz Band) was considered for each sector of the proposed installation. This Channel has a transmit power of 40 Watts.
- 4) I GSM channel (PCS Band 1900 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 10 Watts.



- 5) I LTE channel (PCS Band 1900 MHz) was considered for 2 antennas in each sector of the proposed installation. This Channel has a transmit power of 120 Watts.
- 6) I LTE channel (AWS Band 2100 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 120 Watts.
- 7) I LTE Traffic channel (LTE IC and 2C BRS Band 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 40 Watts.
- 8) I LTE Broadcast channel (LTE IC and 2C BRS Band 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 15 Watts.
- 9) I NR Traffic channel (BRS Band 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 40 Watts.
- 10) I NR Broadcast channel (BRS Band 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of I5 Watts.
- 11) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 12) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

- 13) The antennas used in this modeling are the Ericsson AIR 32 for the 1900 MHz / 1900 MHz / 2100 MHz channel(s), the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz channel(s), the RFS APXVAARR24 43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz / 1900 MHz channel(s) in Sector A, the Ericsson AIR 32 for the 1900 MHz / 1900 MHz / 2100 MHz channel(s), the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz channel(s), the RFS APXVAARR24 43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz / 1900 MHz channel(s) in Sector B, the Ericsson AIR 32 for the 1900 MHz / 1900 MHz / 2100 MHz channel(s), the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz channel(s), the RFS APXVAARR24 43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz / 1900 MHz channel(s) in Sector C. This is based on feedback from the carrier with regard to anticipated antenna selection. All Antenna gain values and associated transmit power levels are shown in the Site Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 14) The antenna mounting height centerline of the proposed antennas is 47 feet above ground level (AGL).
- 15) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 16) All calculations were done with respect to uncontrolled / general population threshold limits.



# **T-Mobile Site Inventory and Power Data**

Sector:	Α	Sector:	В	Sector:	С
Antenna #:	I	Antenna #:	I	Antenna #:	I
Make / Model:	Ericsson AIR 32	Make / Model:	Ericsson AIR 32	Make / Model:	Ericsson AIR 32
Frequency Bands:	1900 MHz / 1900 MHz / 2100 MHz	Frequency Bands:	1900 MHz / 1900 MHz / 2100 MHz	Frequency Bands:	1900 MHz / 1900 MHz / 2100 MHz
Gain:	15.35 dBd / 15.35 dBd / 15.85 dBd	Gain:	15.35 dBd / 15.35 dBd / 15.85 dBd	Gain:	15.35 dBd / 15.35 dBd / 15.85 dBd
Height (AGL):	47 feet	Height (AGL):	47 feet	Height (AGL):	47 feet
Channel Count:	3	Channel Count:	3	Channel Count:	3
Total TX Power (W):	190 Watts	Total TX Power (W):	190 Watts	Total TX Power (W):	190 Watts
ERP (W):	7,014.48	ERP (W):	7,014.48	ERP (W):	7,014.48
Antenna A1 MPE %:	15.00%	Antenna BI MPE %:	15.00%	Antenna CI MPE %:	15.00%
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	Ericsson AIR 6449	Make / Model:	Ericsson AIR 6449	Make / Model:	Ericsson AIR 6449
Frequency Bands:	2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz	Frequency Bands:	2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz	Frequency Bands:	2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz
Gain:	22.65 dBd / 17.3 dBd / 22.65 dBd / 17.3 dBd	Gain:	22.65 dBd / 17.3 dBd / 22.65 dBd / 17.3 dBd	Gain:	22.65 dBd / 17.3 dBd / 22.65 dBd / 17.3 dBd
Height (AGL):	47 feet	Height (AGL):	47 feet	Height (AGL):	47 feet
Channel Count:	4	Channel Count:	4	Channel Count:	4
Total TX Power (W):	110 Watts	Total TX Power (W):	110 Watts	Total TX Power (W):	110 Watts
ERP (W):	16,337.27	ERP (W):	16,337.27	ERP (W):	16,337.27
Antenna A2 MPE %:	34.94%	Antenna B2 MPE %:	34.94%	Antenna C2 MPE %:	34.94%
Antenna #:	3	Antenna #:	3	Antenna #:	3
Make / Model:	RFS APXVAARR24_43-U- NA20	Make / Model:	RFS APXVAARR24_43-U- NA20	Make / Model:	RFS APXVAARR24_43-U- NA20
Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz	Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz	Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz
Gain:	12.95 dBd / 12.95 dBd / 13.35 dBd / 15.65 dBd	Gain:	12.95 dBd / 12.95 dBd / 13.35 dBd / 15.65 dBd	Gain:	12.95 dBd / 12.95 dBd / 13.35 dBd / 15.65 dBd
Height (AGL):	47 feet	Height (AGL):			47 feet
Channel Count:	4	Channel Count:	4	Channel Count:	4
Total TX Power (W):	220 Watts	Total TX Power (W):	220 Watts	Total TX Power (W):	220 Watts
ERP (W):	5,435.69	ERP (W):	5,435.69	ERP (W):	5,435.69
Antenna A3 MPE %:	21.33%	Antenna B3 MPE %:	21.33%	Antenna C3 MPE %:	21.33%

#### environmental | engineering | due diligence

Site Composite MPE %					
Carrier	MPE %				
T-Mobile (Max at Sector A):	71.27%				
AT&T	26.81%				
Site Total MPE % :	98.08%				

T-Mobile MPE % F	er Sector
T-Mobile Sector A Total:	71.27%
T-Mobile Sector B Total:	71.27%
T-Mobile Sector C Total:	71.27%
Site Total MPE % :	98.08%

T-Mobile Maximum MPE Power Values (Sector A)							
T-Mobile Frequency Band / Technology (Sector A)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm²)	Frequency (MHz)	Allowable MPE (μW/cm²)	Calculated % MPE
T-Mobile 1900 MHz GSM	I	342.77	47.0	7.33	1900 MHz GSM	1000	0.73%
T-Mobile 1900 MHz LTE	I	2056.61	47.0	43.98	1900 MHz LTE	1000	4.40%
T-Mobile 2100 MHz LTE	1	4615.10	47.0	98.70	2100 MHz LTE	1000	9.87%
T-Mobile 2500 MHz LTE IC & 2C Traffic	I	7363.09	47.0	157.47	2500 MHz LTE IC & 2C Traffic	1000	15.75%
T-Mobile 2500 MHz LTE IC & 2C Broadcast	I	805.55	47.0	17.23	2500 MHz LTE IC & 2C Broadcast	1000	1.72%
T-Mobile 2500 MHz NR Traffic	1	7363.09	47.0	157.47	2500 MHz NR Traffic	1000	15.75%
T-Mobile 2500 MHz NR Broadcast	1	805.55	47.0	17.23	2500 MHz NR Broadcast	1000	1.72%
T-Mobile 600 MHz LTE	1	788.97	47.0	16.87	600 MHz LTE	400	4.22%
T-Mobile 600 MHz NR	1	1577.94	47.0	33.75	600 MHz NR	400	8.44%
T-Mobile 700 MHz LTE	I	865.09	47.0	18.50	700 MHz LTE	467	3.96%
T-Mobile 1900 MHz LTE	I	2203.69	47.0	47.13	1900 MHz LTE	1000	4.71%
	<b>'</b>		,			Total:	71.27%

<sup>•</sup> NOTE: Totals may vary by approximately 0.01% due to summation of remainders in calculations.



### **Summary**

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector A:	71.27%
Sector B:	71.27%
Sector C:	71.27%
T-Mobile Maximum	71.27%
MPE % (Sector A):	/1.2//6
Site Total:	98.08%
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **98.08**% of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were within the allowable 100% threshold standard per the federal government.