

# STATE OF CONNECTICUT

## CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: [siting.council@po.state.ct.us](mailto:siting.council@po.state.ct.us)

[www.ct.gov/csc](http://www.ct.gov/csc)

June 10, 2004

Wendell G. Davis, Esq.  
Blackwell, Davis, Spadaccini, LLC  
158 East Center Street  
Manchester, CT 06040

RE: **EM-SPRINT-043-040601** – Sprint Spectrum LP notice of intent to modify an existing telecommunications facility located at 148 Roberts Road, East Hartford, Connecticut.

Dear Attorney Davis:

At a public meeting held on June 9, 2004, the Connecticut Siting Council (Council) acknowledged your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies.

The proposed modifications are to be implemented as specified here and in your notice dated May 28, 2004. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Any deviation from this format may result in the Council implementing enforcement proceedings pursuant to General Statutes § 16-50u including, without limitation, imposition of expenses resulting from such failure and of civil penalties in an amount not less than one thousand dollars per day for each day of construction or operation in material violation.

Thank you for your attention and cooperation.

Very truly yours,

Pamela B. Katz, P.E.  
Chairman

PBK/cm

- c: Honorable Timothy D. Larson, Mayor, Town of East Hartford  
Michael J. Dayton, Town Planner, Town of East Hartford  
Stephen Marcus, The Marcus Group  
Thomas F. Flynn III, Nextel Communications, Inc.  
Christopher B. Fisher, Esq., Cuddy & Feder, LLP  
Kenneth C. Baldwin, Esq., Robinson & Cole, LLP

# Blackwell • Davis • Spadaccini LLC

Attorneys at Law

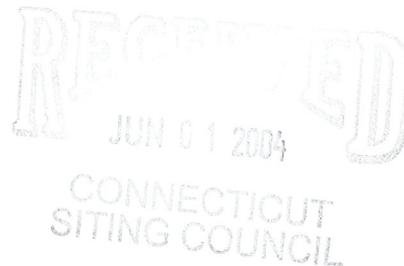
Manchester

Hartford

David H. Blackwell, Esq.  
Wendell G. Davis, Jr., Esq.  
Louis A. Spadaccini, Esq.

May 28, 2004

Connecticut Siting Council  
Ten Franklin Square  
New Britain, CT 06051



Re: SBA Network Services, Inc – Sprint Spectrum LP  
Notice of Exempt Modification  
- Roberts Road, East Hartford  
- Willard Avenue, Newington

To Whom It May Concern:

Enclosed please find one (1) original and twenty (20) copies of Sprint Spectrum LP's (by and through its agent, SBA Network Services, Inc.) Notice Of Intent To Modify An Existing Telecommunications Facility for the following locations:

1. 148 Roberts Road, East Hartford, CT
2. 605 Willard Avenue, Newington, CT

Also enclosed please find two (2) firm checks to cover the filing fee for these applications. If you should have any questions, please do not hesitate to contact me.

Respectfully,

A handwritten signature in blue ink, appearing to read "WGD".

Wendell G. Davis

**Blackwell • Davis • Spadaccini**  
Attorneys at  
*Manchester*

EM-SPRINT-043-040601

David H. Blackwell, Esq.  
Wendell G. Davis, Jr., Esq.  
Louis A. Spadaccini, Esq.

May 28, 2004

Hon. Pamela B. Katz,  
Chairperson  
Connecticut Siting Council  
10 Franklin Square  
New Britain, Connecticut 06051

RE: ***NOTICE OF INTENT TO MODIFY AN EXISTING  
TELECOMMUNICATIONS FACILITY AT  
148 Roberts Road, East Hartford, CT***

The Honorable Pamela B. Katz:

Sprint Spectrum LP (“Sprint”) by and through its agent SBA Network Services, Inc. hereby respectfully requests acknowledgment that the proposed co-location of Sprint on an existing telecommunications facility owned by The Marcus Group, LLC and located at 148 Roberts Road, East Hartford, Connecticut (the “Roberts Road Facility”) constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72. In accordance with R.C.S.A. Section 16-50j-73, a copy of this letter has been sent to the Honorable Timothy D. Larson, Mayor of the Town of East Hartford.

***The Roberts Road Facility***

The Roberts Road Facility consists of a 120 foot monopole located within a site compound surrounded by a chain link fence. The facility was approved by the Connecticut Siting Council on November 7, 2002 (Docket 228) and the facility currently supports the antenna arrays and related equipment of several carriers, already approved by the Connecticut Siting Council, including Verizon, AT&T and Nextel. Sprint and The Marcus Group, LLC have agreed to the shared use of the Roberts Road Facility as is more fully detailed below.

***Sprint’s Facility***

Sprint’s Facility will include the installation of 6 panel antennas at an antenna center line height of approximately 110 feet and 1 array antenna at an antenna center line of approximately 70 feet. A Structural Analysis Report, attached as Exhibit A, was generated by Tectonic/Keyes Associates and confirms that the tower is structurally

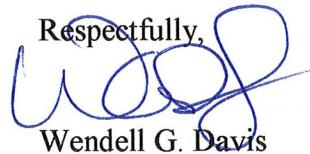
capable of supporting Sprint's proposed antennas.<sup>1</sup> Sprint will also install its equipment in a 15' x 25' area at the base of the tower all within the existing fenced compound.

### ***Sprints' Facility Constitutes An Exempt Modification***

For the following reasons, the proposed modifications to the Roberts Road Facility meet the exempt modification criteria set forth in R.C.S.A. Section 16-50j-72(b)(2):

1. As evidenced by the attached Tower Elevation Drawing (Exhibit B), the proposed modification will not increase the height of the tower as Sprint's antennas will be installed at a center line height of approximately 110' feet on an existing 120' monopole.
2. As evidenced by the attached Site Plan Drawing (Exhibit B), the installation of Sprint's equipment will not require an extension of the existing site boundaries.
3. The proposed modifications will not increase the noise levels at the existing facility by six decibels or more.
4. As set forth in the Power Density Report prepared by Sprint PCS attached as Exhibit C, the operation of the additional antennas will not increase the total radio frequency (RF) electromagnetic radiation power density to a level at or above the standards adopted by the Connecticut Department of Environmental Protection and the Federal Communications Commission.

For the foregoing reasons, Sprint respectfully submits that the proposed addition of Sprints' antenna and equipment at the Roberts Road Facility constitutes an exempt modification under R.C.S.A. Section 16-50j-72.

Respectfully,  
  
Wendell G. Davis

cc: The Honorable Timothy D. Larson, Mayor Town of East Hartford  
Aaron Cowher, SBA, Agent for Sprint

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<sup>1</sup> As the existing structure is expandable to 140', the Structural Analysis Report takes a conservative approach and assumes Sprint's proposed antennas are being added to a 140' structure with antennas located at the 140', 130', 120', 100' and 90' levels.

**EXHIBIT A**

**SBA NETWORK SERVICES  
MARCUS GROUP  
EXISTING 140' MONOPOLE  
EAST HARTFORD, CONNECTICUT  
STRUCTURAL ANALYSIS REPORT  
MAY 25, 2004**

## **1.0 INTRODUCTION**

The existing 140 foot monopole is located at 148 Roberts Road, East Hartford, CT. Sprint PCS has proposed installing additional antennas and a platform on the monopole at approximately elevation 110 feet.

Tectonic Engineering and Surveying Consultants, P.C. has performed a structural analysis of the monopole to verify its adequacy for supporting antennas, in accordance with current code requirements.

### **1.1 Information Provided**

Tectonic was furnished with the following information:

- "Monopole Details", Job No. MP1400800-0001 by Glen Martin Engineering, Inc., dated August 26, 2003
- "Foundation Design", Job No. GME-03309 by Glenn Martin Engineering, Inc., dated August 8, 2003

## **2.0 STRUCTURE DESCRIPTION**

### **2.1 General**

The existing 140 foot monopole is a 16 sided taper monopole, with four sections. Splices between the first three sections are slip joint type. The splice on the top section is a flange type connection. The monopole was designed by Glen Martin Engineering, Inc in 2003.

The monopole is 49.19" across the flats at the base, and 18.00" across the flats at the top. The wall thickness of the first (bottom) section is 3/8", the wall thickness of the second section is 5/16" and the wall thickness of the third section is 1/4". The wall thickness of the top section is 3/16". The monopole was designed with several feed line entrance ports and exit ports.

A diagram of the structure is presented in Figure 1, attached.

### **2.3. Monopole Foundation**

The existing foundation was designed by Glen Martin Engineering and the foundation design reactions were listed on their drawing that was furnished.

According to the Glen Martin Engineering drawing, the foundation consists of a 23 foot square reinforced concrete pad. The foundation design was based upon a geotechnical report prepared by Dr Clarence Welti, P.E., P.C. of Glastonbury, CT. dated April 25, 2003.

Anchorage of the monopole to the foundation is provided by twenty (20) 2-1/2" diameter x 6'-0" long ASTM A572, Grade 55 anchor bolts.

## **3.0 EXISTING CONDITION**

### **3.1 Field Visit**

The monopole was visual inspected from the ground on November 20, 2003. Photographs were taken to document the existing configuration and conditions.

Based on our limited inspection, the monopole is in good condition. The galvanizing on the monopole is intact. No damage or significant deformation of the monopole was observed. The exposed portion of the concrete foundation, the grout and anchor bolts are in good condition.

Based on these findings, we expect that the monopole is capable of supporting its original design loads.

## **4.0 PROPOSED INSTALLATION**

It is our understanding that all existing antennas and equipment will remain on the structure, and the following items are proposed to be added to the monopole by Sprint PCS and future telecommunication carriers:

Sprint PCS:

- 6 - Decibel DB950F40T2E-M panel antennas at elevation 110'
- 1 - 14' Summit low profile platform at elevation 110'
- 1 - CSA Two Foot Planar Array antenna at elevation 70'
- 7 - 1-5/8" diameter coaxial cables routed up the interior of the monopole to the 110' level

Future Telecommunication Carriers:

- Panel Antennas, with total CaAa = 34.40 ft<sup>2</sup>, at elevation 140'.
- 1 - 14' Summit low profile platform at elevation 140'.
- Panel Antennas, with total CaAa = 34.40 ft<sup>2</sup>, at elevation 130'.
- 1 - 14' Summit low profile platform at elevation 130'.
- Panel Antennas, with total CaAa = 34.40 ft<sup>2</sup>, at elevation 120'.

**Future Telecommunication Carriers (Continue):**

- 1 - 14' Summit low profile platform at elevation 120'.
- Panel Antennas, with total CaAa = 34.40 ft<sup>2</sup>, at elevation 100'.
- 1 - 14' Summit low profile platform at elevation 100'
- Panel Antennas, with total CaAa = 34.40 ft<sup>2</sup>, at elevation 90'.
- 1 - 14' Summit low profile platform at elevation 90'

## **5.0 STRUCTURAL ANALYSIS**

### **5.1 Current Loading Criteria and Procedure**

The Connecticut Building Code 2000 Supplement requires this structure to be analyzed in accordance with the provisions of ANSI/TIA/EIA-222-F-1996 "Structural Standards for Steel Antenna Towers and Antenna Supporting Structures". The basic wind speed of 80 mph applies to Hartford County, CT.

Appendix A of the Connecticut Building Code 2000 Supplemental State Building Code requires a design wind speed of 80 mph with an importance factor of 1.04 for East Hartford. This is equivalent to a basic wind speed of 83.2 mph and this value will be used throughout the analysis.

Ice loads have been established based on a 0.5" radial ice thickness in accordance with industry standard practice. A reduced wind speed of 72.1 mph is used in conjunction with ice.

A detailed analysis of the structure was performed using the geometry, material thickness and properties indicated in the Glen Martin Engineering drawings and confirm by our field inspection.

### **5.2 Results**

Under the loading conditions described above, the results of our analysis indicate the calculated stresses in the monopole are within the limits of the allowable values established by applicable codes. In addition, the foundation reactions are within their allowable design values.

### **5.2.1 Monopole Capacity**

The following chart depicts the location and percent of the tower capacity, with 100% representing full capacity;

Monopole Elevation (ft)	Combined Stress (ksi)	Allowable Stress (ksi)	Interaction %
139.00	4.96	52.00	9.5
135.00	7.39	52.00	14.2
131.00	12.30	52.00	23.7
127.00	15.66	52.00	30.1
123.00	18.90	52.00	36.3
119.00	24.17	52.00	46.5
111.20	24.44	52.00	47.0
103.40	30.71	52.00	59.1
95.60	36.14	52.00	69.5
87.80	41.65	52.00	80.1
80.00	46.49	52.00	89.4
72.60	40.35	52.00	77.6
65.20	42.75	52.00	82.2
57.80	44.72	52.00	86.0
50.40	46.31	52.00	89.1
43.00	47.61	52.00	91.6
34.40	40.87	52.00	78.6
25.80	41.66	52.00	80.1
17.20	42.26	52.00	81.3
8.60	42.72	52.00	82.2

### **5.2.1 Anchor Bolt Capacity**

The following chart depicts the location and percent of the anchor bolt capacity, with 100% representing full capacity;

Anchor Bolts	Actual Stress (ksi)	Allowable Stress (ksi)	Interaction %
	20.8	39.3	52.9%
Required Embedment			69.8%

### **5.2.1 Foundation Capacity**

The following chart depicts the location and percent of the foundation capacity, with 100% representing full capacity;

Foundation Footing	Actual Bearing Pressure (psf)	Allowable Bearing Pressure (psf)	Interaction %
	2303	3500	65.8%

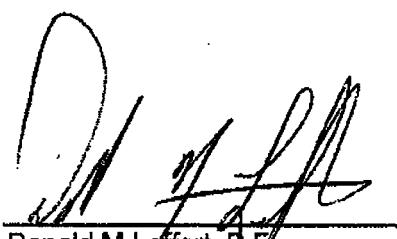
## **6.0 CONCLUSIONS AND RECOMMENDATIONS**

As a result of our analysis, we find that the existing monopole has sufficient capacity to permit the installation of 6 Sprint PCS antennas, 1 Sprint PCS Two Foot Planar Array antenna, low profile platform and related cables. The movement of the monopole will not exceed 3 degrees at the 50 mph operational wind speed. No structural problems for the monopole are anticipated, and no modifications are necessary.

The axial compression and overturning moment from the proposed installation will not exceed the original foundation design. The anchor bolts in the foundation will have sufficient capacity to permit the proposed installation.

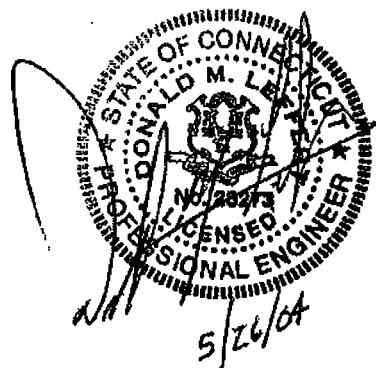
Any further changes to the antenna configuration at any of the levels or other appurtenances should be reviewed with respect to their effect on structural loads prior to implementation.

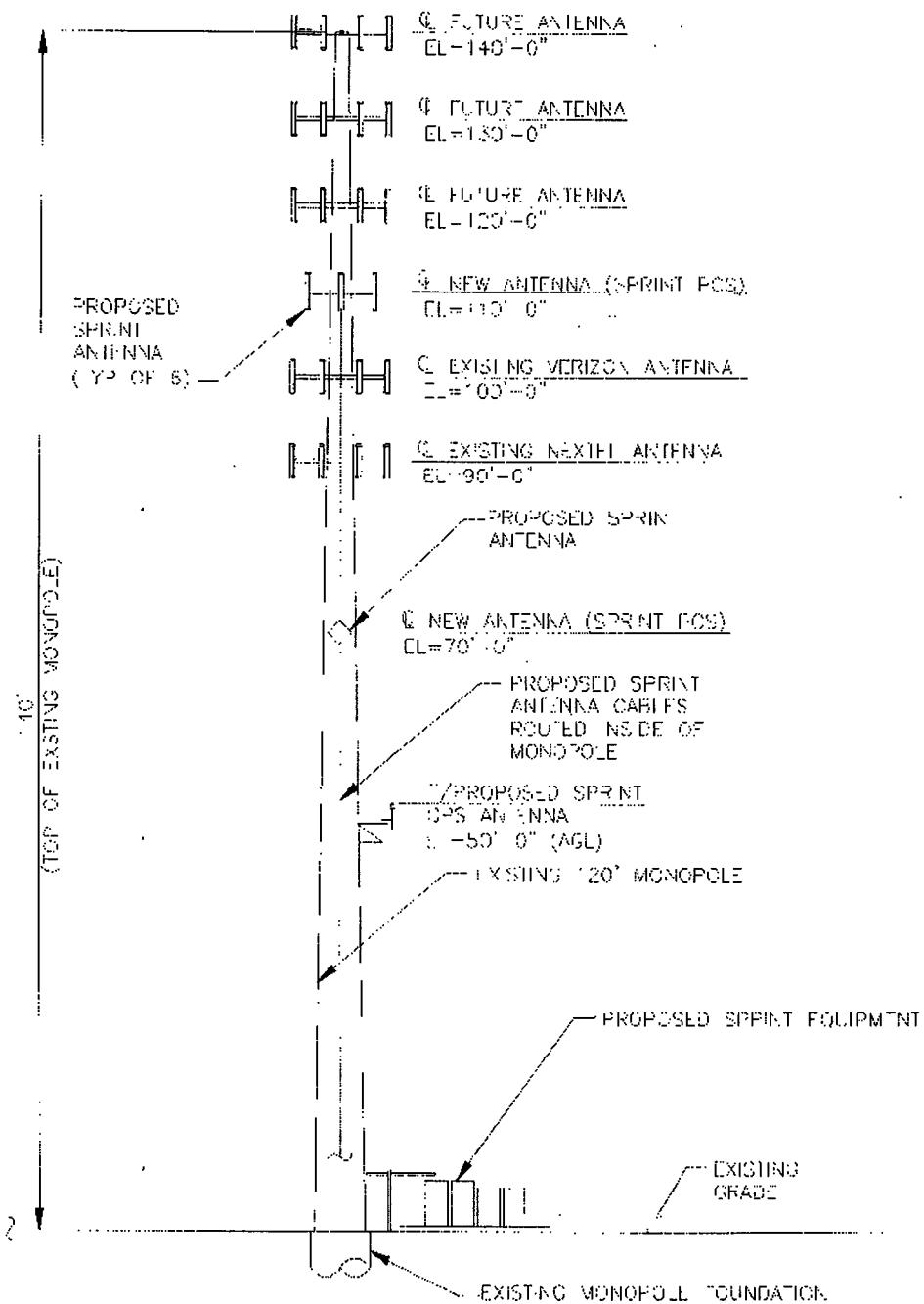
Prepared by:



Donald M Leffert, P.E.  
Senior Structural Engineer

Date: MAY 26, 2004





**FIGURE 1**

TOWER ANALYSIS  
FOR  
  
EXISTING MONOPOLE  
MARCUS GROUP  
148 ROBERTS ROAD  
EAST HARTFORD, CONNECTICUT

SPRINT SPECTRUM LP  
CASCADE NUMBER CT23XC640

PREPARED FOR  
SBA NETWORK SERVICES  
900 CUMMINGS CENTER SUITE 316U  
BEVERLY, MASSACHUSETTS 01915

PREPARED BY  
**TECTONIC ENGINEERING AND SURVEYING CONSULTANTS, PC**  
ROCKY HILL, CONNECTICUT  
May 25, 2004

**Monopole Properties:**

Pole Height	139 ft.
Top Width	.48-.19 in.
Bottom Width	.48-.00 in.
Increments Between Splices	5 (max=5)
Yield Strength	65 ksi
Unit Weight of Material	490 lbs/cf
Modulus of Elasticity	29000 ksi
Elevation of 5th Splice	(TOP)
Elevation of 4th Splice	Center line
Elevation of 3rd Splice	119,000 ft.
Elevation of 2nd Splice	80,000 ft.
Elevation of 1st (Bottom) Splice	43,000 ft.
Thickness of 6th Section	.0188 in.
Thickness of 5th Section	.0250 in.
Thickness of 4th Section	.0313 in.
Thickness of 3rd Section	.0375 in.
Thickness of 2nd Section	.0437 in.
Thickness of 1st (Bottom) Section	.0500 in.

Avg. Pole Taper  
Total Number of Segments

20

Length of 6th Section	20.00 ft.
Length of 5th Section	39.00 ft.
Length of 4th Section	37.00 ft.
Length of 3rd Section	43.00 ft.
Length of 2nd Section	0.00 ft.
Length of 1st (Bottom) Section	0.00 ft.
Increment in 6th Section	4.00 ft.
Increment in 5th Section	7.80 ft.
Increment in 4th Section	7.40 ft.
Increment in 3rd Section	8.60 ft.
Increment in 2nd Section	0.00 ft.
Increment in 1st (Bottom) Section	0.00 ft.

VxI = 83.2 mph  
1 (1=EIA-222-F)  
(2=prior to version EIA-222-F)

Gh= 1.690

**Antennas & Apertures:**

Item Number	Type	Z (ft.)	Total Weight (lbs.)	Quantity	Ca	Total CaAa (sq ft.)	Kz	Total Weight (lbs.)	FITEM (lbs.)	Total F (lbs.)	Elev. (ft.)	Total Item Weight (lbs.)	
1	Future 140	140	1955	1	1	60.8	1954.6	1,511	26,780	27,53	1 (top)	140	1955
2	Future 130	130	1955	1	1	60.8	1954.6	1,480	26,219	26,95	2	130	1955
3	Future 120	120	1955	1	1	60.8	1954.6	1,446	25,626	26,34	3	120	1955
4	Sprint	110	1721	1	1	63.2	1720.6	1,411	24,997	26,69	4	110	1721
5	Verizon	100	1751	1	1	60.8	1750.6	1,373	24,325	25,00	5	100	1751
6	Nexel	90	1751	1	1	60.8	1750.6	1,332	23,604	24,26	6	90	1751
7	Sprint Donor	70	15	1	1	6.6	15.0	1,240	21,968	24,4	7	70	15
8						0.0	0.000	0.000	0	8	0	0	0
9						0.0	0.000	0.000	0	9	0	0	0
10						0.0	0.000	0.000	0	10	0	0	0
								Total=	11100	Total=	15320		11100

## Pole properties &amp; weights:

Segment	Length (ft.)	Average I (in <sup>4</sup> )	Avg. Cross Sectional Area (in <sup>2</sup> )	Weight (lbs.)	Section Subtotal (lbs.)	Cable Weight (lb)
1	4.00	462	10.91	148	30.18	18
2	4.00	533	11.44	158	31.61	18
3	4.00	611	11.98	163	33.04	18
4	4.00	687	12.52	170	34.47	18
5	4.00	751	13.05	178	35.90	18
6	7.80	1238	18.32	486	38.18	0
7	7.80	1567	19.80	526	40.79	0
8	7.80	1908	21.19	563	43.58	0
9	7.80	2309	22.59	600	46.37	0
10	7.80	2782	23.98	637	48.16	0
11	7.40	4905	31.46	792	52.11	0
12	7.40	4715	33.27	838	54.52	0
13	7.40	5452	34.92	879	57.16	0
14	7.40	6283	36.57	921	59.81	0
15	7.40	7150	38.22	962	62.45	0
16	8.60	9696	47.71	1396	65.61	0
17	8.60	11279	50.24	1470	68.38	0
18	8.60	12902	52.56	1538	71.46	0
19	8.60	14673	54.85	1605	74.53	0
20	8.80	7796	28.00	819	77.23	0
0	0.00	0	0.00	0	0.00	0
0	0.00	0	0.00	0	0.00	0
0	0.00	0	0.00	0	0.00	0
0	0.00	0	0.00	0	0.00	0
0	0.00	0	0.00	0	0.00	0
0	0.00	0	0.00	0	0.00	0
0	0.00	0	0.00	0	0.00	0
0	0.00	0	0.00	0	0.00	0
0	0.00	0	0.00	0	0.00	0
0	0	0	0.00	0	0	0
					0	
					0	

THE ONLY CHANGE BET. ICE &amp; NO ICE ADD CABLE AND ICE

## Horizontal wind forces on pole:

Segment	Z (ft.)	Cables inside						Cum. F (lbs.)
		K <sub>Z</sub> [2.33]	q <sup>2</sup> ([lb/ft <sup>2</sup> ]) [2.33]	C [Table 1]	C <sub>f</sub> [Table 1]	A <sub>e</sub> ([ft <sup>2</sup> ])	F (lbs.)	
1	137.00	1.502	26.614	1.537	156.8	0.716	6.1	198
2	133.00	1.489	26.390	1.612	163.7	0.716	6.4	206
3	129.00	1.476	26.161	1.687	170.5	0.716	6.7	214
4	125.00	1.463	25.926	1.762	177.3	0.716	7.0	221
5	121.00	1.450	25.687	1.837	184.0	0.716	7.3	228
6	115.10	1.429	25.322	1.947	193.6	0.716	7.52	1067
7	107.30	1.401	24.820	2.093	206.1	0.716	16.3	490
8	99.50	1.371	24.290	2.239	218.1	0.716	17.5	513
9	91.70	1.339	23.730	2.384	229.6	0.716	18.6	534
10	83.90	1.306	23.135	2.530	240.5	0.716	19.7	3070
11	76.30	1.271	22.516	2.572	250.6	0.716	19.8	533
12	68.90	1.234	21.869	2.811	259.8	0.716	20.8	550
13	61.50	1.195	21.171	2.949	268.2	0.716	21.8	569
14	54.10	1.152	20.409	3.088	275.7	0.716	22.8	5835
15	46.70	1.104	19.569	3.226	282.0	0.716	23.9	6400
16	38.70	1.047	18.546	3.376	287.3	0.716	29.0	651
17	30.10	0.974	17.261	3.536	290.4	0.716	30.4	7052
18	21.50	0.885	15.679	3.697	289.3	0.716	31.8	635
19	12.90	0.765	13.550	3.858	280.7	0.716	33.2	8290
20	4.30	0.559	9.900	4.019	249.9	0.716	34.6	544
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0
0	0.00	0.000	4.099	1.8	0	0.0	0	0

**Monopole Stresses:**

	Elevation (ft.)	Section Thickness (in.)	Cross Sectional Area (in. <sup>2</sup> )	Total Cumulative Weight (lbs.)	Shear Force (lbs.)	Total Moment (in-lb.)	Combined Stress (ksi)	Allowable Stress (ksi)	Act/All Stress Ratio		
	139.00	0.1875	428	10.64	29.47	1955	16920	227	4.96	52.00	0.10
splice pt.	135.00	0.1875	496	11.17	30.89	2119	16118	378	7.39	52.00	0.14
	131.00	0.1875	570	11.71	32.32	2290	16324	698	12.30	52.00	0.24
splice pt.	127.00	0.1875	652	12.25	33.75	4456	16538	965	15.66	52.00	0.30
	123.00	0.1875	742	12.78	35.18	4673	16759	1274	18.90	52.00	0.36
splice pt.	119.00	0.1875	639	13.32	36.61	6853	16987	1765	24.17	52.00	0.46
	111.20	0.25	1393	19.10	39.40	7432	17453	2764	24.44	52.00	0.47
splice pt.	103.40	0.25	1720	20.50	42.19	9771	17943	4003	30.71	52.00	0.59
	95.60	0.25	2096	21.89	44.98	12176	18456	5377	36.14	52.00	0.69
splice pt.	87.80	0.25	2522	23.28	47.76	14819	18990	7017	41.65	52.00	0.80
	80.00	0.25	3002	24.64	50.55	15348	19543	8817	46.19	52.00	0.85
splice pt.	72.60	0.3125	4364	32.44	53.20	16229	20082	10572	40.35	52.00	0.76
	65.20	0.3125	5066	34.09	55.84	17169	20632	12386	42.75	52.00	0.82
splice pt.	57.80	0.3125	5839	36.74	58.49	18136	21191	14254	44.72	52.00	0.86
	50.40	0.3125	6687	37.40	61.13	19145	21755	16168	46.31	52.00	0.89
splice pt.	43.00	0.3125	7613	39.05	63.77	20195	22321	18133	47.61	52.00	0.92
	34.40	0.375	10504	49.09	66.85	22429	22872	20474	40.87	52.00	0.79
splice pt.	25.80	0.375	12054	51.40	69.92	24070	23607	22880	41.68	52.00	0.80
	17.20	0.375	13749	53.70	73.00	25781	24210	25348	42.26	52.00	0.81
splice pt.	8.60	0.375	15597	56.01	76.07	27563	24754	27874	42.72	52.00	0.82
	0.00	0	0	0.00	78.40	28562	25168	29485	0.00	52.00	0.65
Foundation Loads:											
Vertical=											
Shear=											
Moment=											

Foundation Loads:  
 Vertical= 28.6 kips  
 Shear= 25.2 kips  
 Moment= 29495 in.-kip (without sec. moments)

2457.10924

SUPPORTING CALCULATIONS:  
cumulative weights of appurtenances:

Elevation (ft.)	ITEM 1 weight	ITEM 2 weight	ITEM 3 weight	ITEM 4 weight	ITEM 5 weight	ITEM 6 weight	ITEM 7 weight	ITEM 8 weight	ITEM 9 weight	ITEM 10 weight	cum. weight
139.00	1954.56	0	0	0	0	0	0	0	0	0	1954.56
135.00	1954.56	0	0	0	0	0	0	0	0	0	1954.56
131.00	1954.56	0	0	0	0	0	0	0	0	0	1954.56
127.00	1954.56	1954.56	0	0	0	0	0	0	0	0	3909.12
123.00	1954.56	1954.56	0	0	0	0	0	0	0	0	3909.12
119.00	1954.56	1954.56	1954.56	0	0	0	0	0	0	0	5863.68
111.20	1954.56	1954.56	1954.56	1954.56	0	0	0	0	0	0	5863.68
103.40	1954.56	1954.56	1954.56	1954.56	1720.56	0	0	0	0	0	7584.24
95.60	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	0	0	0	0	9334.8
87.80	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	0	0	0	11085.36
80.00	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	0	0	0	11085.36
72.60	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	0	0	0	11085.36
65.20	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
57.80	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
50.40	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
43.00	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
34.40	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
25.80	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
17.20	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
8.60	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
0.00	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
0.00	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
0.00	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
0.00	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
0.00	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
0.00	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
0.00	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
0.00	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
0.00	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36
0.00	1954.56	1954.56	1954.56	1954.56	1720.56	1750.56	1750.56	15	0	0	11100.36

**SUPPORTING CALCULATIONS:**

combined bending and axial stresses:

Kerfless FOR 18 SIDED

18 SIDED	18 SIDED	ELEV-MIDPOINT ELEV	18 SIDED FOLIE									
elevation	(ft.)	check ( $\text{F}^{\text{v}} \times 5^{\text{y}}$ w/t)	moment of section midpoints (ft.)	moment arm segment (ft.)	wind force (lbs.)	wind moment (K-in)	moment due to apprt. (K-in)	secondary moment (K-in)	Dia Across Flats (in.)	w (min.-4) (in.)	Dia Across Flats (in.)	w (min.-4) (in.)
139.00	143	137.00	0	198	0	0	33	194	18.00	4.82	17.06	3.33
135.00	150	133.00	2.00	206	5	165	208	18.90	5.06	18.71	3.50	
131.00	157	129.00	2.00	214	19	297	381	19.80	5.31	19.61	3.66	
127.00	165	125.00	2.00	221	44	526	394	20.69	5.56	20.51	3.83	
123.00	172	121.00	2.00	228	79	785	407	21.59	5.79	21.40	3.99	
119.00	179	115.10	2.00	485	124	1081	560	22.49	6.03	22.30	4.16	
111.20	145	107.30	2.00	490	246	1637	680	24.24	6.50	23.99	4.48	
103.40	155	99.50	2.00	513	412	2805	785	25.99	6.96	25.74	4.81	
95.80	165	91.70	2.00	534	626	3943	808	27.74	7.43	27.49	5.13	
87.80	176	63.90	2.00	553	888	5248	881	29.49	7.90	29.24	5.46	
80.00	186	76.30	2.00	539	1201	6715	901	31.24	8.37	30.99	5.78	
72.80	157	68.90	3.90	550	1547	8107	918	32.90	8.82	32.59	6.09	
65.20	185	61.50	3.90	559	1941	9513	932	34.56	9.26	34.25	6.39	
57.80	173	54.10	3.90	564	2384	10927	943	36.22	9.71	35.91	6.70	
50.40	181	46.70	3.90	565	2877	12341	951	37.88	10.15	37.57	7.01	
43.00	189	38.70	3.90	651	3420	13754	958	39.54	10.60	39.23	7.32	
34.40	165	30.10	3.90	635	4114	15397	963	41.47	11.11	41.10	7.67	
25.80	173	21.50	3.90	603	4875	17040	965	43.40	11.63	43.03	8.03	
17.20	180	12.90	3.90	544	5699	18683	965	45.33	12.15	44.96	8.39	
8.60	188	4.30	3.90	414	6583	20326	965	47.26	12.67	46.89	8.74	
0.00	0	0.00	3.90	0	7516	21969	0	49.19	13.18	49.19	9.10	
0.00	0	0.00	3.70	0	7516	21969	0	49.19	13.18	49.19	9.10	
0.00	0	0.00	3.70	0	7516	21969	0	49.19	13.18	49.19	9.10	
0.00	0	0.00	3.70	0	7516	21969	0	49.19	13.18	49.19	9.10	
0.00	0	0.00	3.70	0	7516	21969	0	49.19	13.18	49.19	9.10	
0.00	0	0.00	3.70	0	7516	21969	0	49.19	13.18	49.19	9.10	
0.00	0	0.00	3.70	0	7516	21969	0	49.19	13.18	49.19	9.10	
0.00	0	0.00	3.70	0	7516	21969	0	49.19	13.18	49.19	9.10	
0.00	0	0.00	3.70	0	7516	21969	0	49.19	13.18	49.19	9.10	
0.00	0	0.00	3.70	0	7516	21969	0	49.19	13.18	49.19	9.10	

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**SUPPORTING CALCULATIONS:**  
combined bending and axial stresses [continued]:

elevation (ft.)	c (in.)	bending stress (ksi)	axial compr. (ksi)	unfactored allowable stress [Table 5] (ksi)	factor [3.1.1] (based on structure height)
138.00	9.0	4.78	0.184	39.00	1.333333
135.00	9.4	7.20	0.190	39.00	1.333333
131.00	9.9	12.11	0.196	39.00	1.333333
127.00	10.3	15.30	0.364	39.00	1.333333
123.00	10.8	18.53	0.366	39.00	1.333333
119.00	11.2	23.65	0.515	39.00	1.333333
111.20	12.1	24.05	0.389	39.00	1.333333
103.40	13.0	39.23	0.477	39.00	1.333333
95.60	13.9	35.58	0.556	39.00	1.333333
87.80	14.7	41.03	0.628	39.00	1.333333
80.00	15.6	45.87	0.622	39.00	1.333333
72.80	16.4	39.85	0.500	39.00	1.333333
65.20	17.3	42.25	0.504	39.00	1.333333
57.80	18.1	44.21	0.507	39.00	1.333333
50.40	18.9	45.80	0.512	39.00	1.333333
43.00	19.8	47.09	0.517	39.00	1.333333
34.40	20.7	40.42	0.457	39.00	1.333333
25.80	21.7	41.19	0.468	39.00	1.333333
17.20	22.7	41.78	0.480	39.00	1.333333
8.60	23.6	42.23	0.492	39.00	1.333333
0.00	24.6	0.00	0.000	39.00	1.333333
0.00	24.6	0.00	0.000	39.00	1.333333
0.00	24.6	0.00	0.000	39.00	1.333333
0.00	24.6	0.00	0.000	39.00	1.333333
0.00	24.6	0.00	0.000	39.00	1.333333
0.00	24.6	0.00	0.000	39.00	1.333333
0.00	24.6	0.00	0.000	39.00	1.333333
0.00	24.6	0.00	0.000	39.00	1.333333

**Monopole Properties:**

- Pole Height      139 ft.
- Top Width      18.00 in.
- Bottom Width      49.19 in.
- Increments Between Splices      5 (max=5)
- Yield Strength      65 ksi
- Unit Weight of Material      490 lbs/cf
- Modulus of Elasticity      29000 ksi
- Elevation of 5th Splice      (TOP)
- Elevation of 4th Splice      Center line
- Elevation of 3rd Splice      119,000 ft.
- Elevation of 2nd Splice      80,000 ft.
- Elevation of 1st (Bottom) Splice      43,000 ft.
- Thickness of 6th Section      fl.
- Thickness of 5th Section      fl.
- Thickness of 4th Section      fl.
- Thickness of 3rd Section      fl.
- Thickness of 2nd Section      fl.
- Thickness of 1st (Bottom) Section      fl.

0.22439 in./ft.

Avg. Pole Taper  
Total Number of Segments

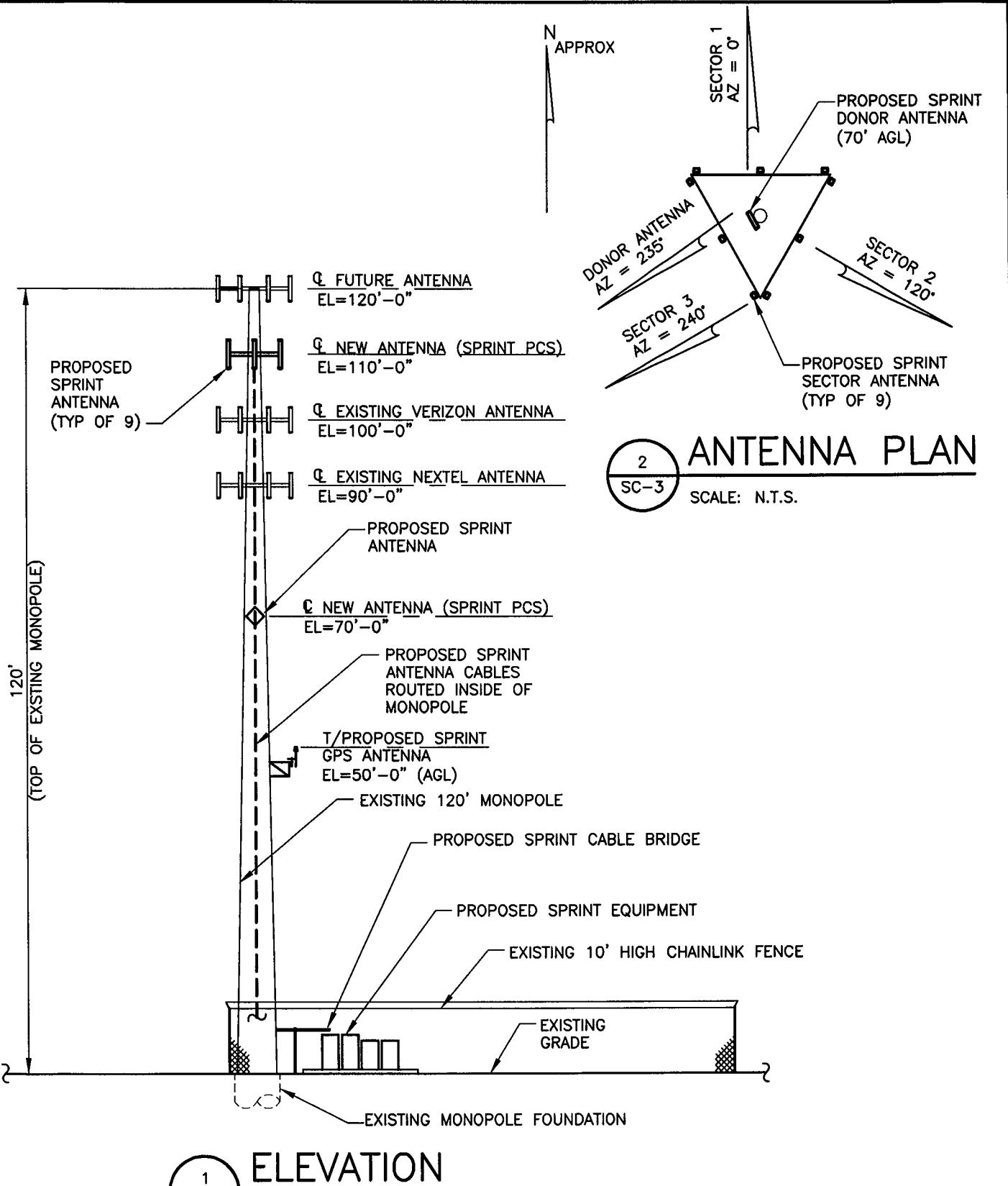
20

Loading Information:  
 Basic Wind Speed with Importance Factor      V<sub>w1</sub> =      72.1 mph  
 EI/A Version To Use      1 (1=EI/A-222-F)  
 (2=prior to version EI/A-222-F)

**Antennas & Appurtenances:**

Item Number	Type	Z (ft.)	Total Weight (lbs.)	Quantity	Total CaAa (sf)	Total Weight (lbs.)	Kz	qz	Total F (lbs.)	ITEM	Total Elev. (ft.)	Total Item Weight (lbs.)
1	Future 140	140	2872	1	1	74.2	2871.6	1,511	20,085	2548	1 (top)	140
2	Future 130	130	2872	1	1	74.2	2871.6	1,480	19,664	2486	2	130
3	Future 120	120	2872	1	1	74.2	2871.6	1,446	19,219	2410	3	120
4	Sprint	110	2556	1	1	75.1	2556.1	1,411	18,747	2379	4	110
5	Verizon	100	2668	1	1	74.2	2667.6	1,373	18,244	2288	5	100
6	Nexel	90	2668	1	1	74.2	2667.6	1,332	17,703	2220	6	90
7	Sprint Donor	70	42	1	1	71	41.7	1,240	16,476	197	7	70
8						0.0	0.000	0.000	0	8	0	0
9						0.0	0.000	0.000	0	9	0	0
10						0.0	0.000	0.000	0	10	0	0
									Total=	16548		16548
									Total=	14478		
												16548

**EXHIBIT B**



**TECTONIC**

- PLANNING
- ENGINEERING
- SURVEYING
- CONSTRUCTION MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.  
1344 Silas Deane Highway, Suite 500  
Rocky Hill, CT 06067

Phone: (860) 563-2341  
Fax: (860) 257-4882  
[www.tectonicengineering.com](http://www.tectonicengineering.com)



**Sprint**

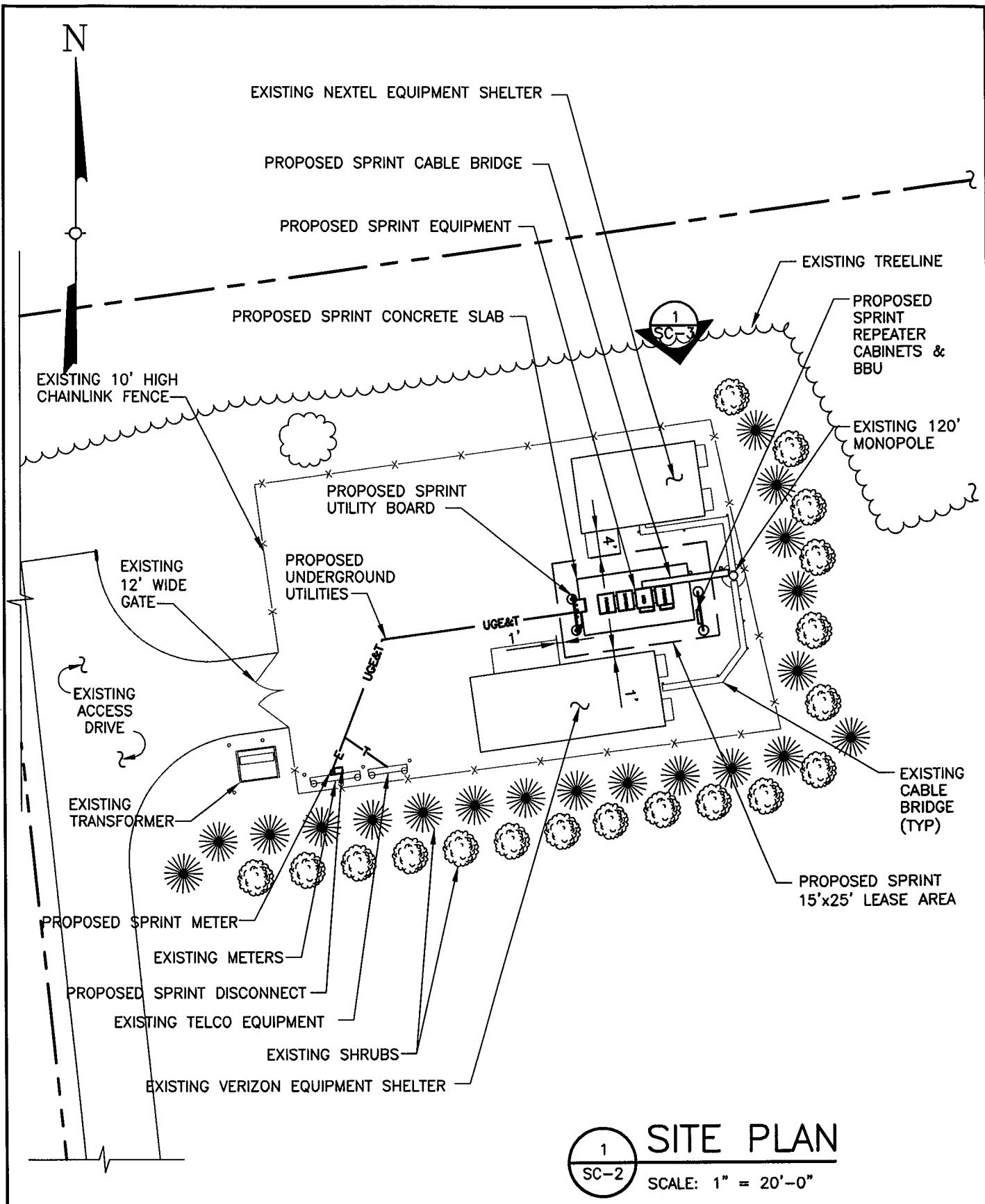
148 ROBERTS STREET  
EAST HARTFORD, CT. 06108

TEC WO:2850.CT540 ISSUED BY: DML DATE: 5/10/04

SITE NO: CT23XR540D

SHEET: SC-3

REV: 0



**TECTONIC**

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**Sprint**

148 ROBERTS STREET  
EAST HARTFORD, CT. 06108

TEC WO:2850.CT540 ISSUED BY: DML DATE: 5/10/04

SITE NO: CT23XR540D

SHEET: SC-2

REV: 0

## **EXHIBIT C**

**CT23XR540 - Marcus Communication Tower, 148 Roberts Road, East Hartford, CT 06108**  
**Cumulative Power Density Analysis of Sprint PCS and Existing Antennas**

Operator	Operating Frequency (MHz)	Distance to Target (feet)	Calculated Power Density (mW/cm <sup>2</sup> )	Maximum Permissible Exposure*	Fraction of MPE (%)
AT&T*	1900	120	0.033200	1.000000	3.32%
Verizon*	880	100	0.066178	0.580000	11.41%
Nextel*	851	90	0.013800	0.568000	2.43%
Sprint PCS	1962.5	110	0.100967	1.000000	10.10%
<b>Total Percentage of Maximum Permissible Exposure</b>				<b>27.26%</b>	

\* Technical data provided by respective operator

## Power Density Analysis - CT23XR540

CT23XR540 - Marcus Communications Tower, 148 Roberts Road, East Hartford, CT 06108

**Power Density Analysis of Sprint PCS Antennas to be mounted on Monopole. Assumes Maximum ERP and no antenna pattern adjustment.**

Operating Frequency (MHz)	Effective Radiated Power (ERP) (Watts)	Antenna Height (Feet)	Distance from Base of the tower (Feet)	Calculated Power Density (mV/cm)	Maximum Permissible Exposure*	%NTE
1962.5	11	308.51	3393.61	110	0	0.100967
1962.5	11	308.51	3393.61	110	50	0.083678
1962.5	11	308.51	3393.61	110	100	0.055281
1962.5	11	308.51	3393.61	110	150	0.035309
1962.5	11	308.51	3393.61	110	200	0.023449
1962.5	11	308.51	3393.61	110	250	0.016377
1962.5	11	308.51	3393.61	110	300	0.011966
1962.5	11	308.51	3393.61	110	400	0.007099
1962.5	11	308.51	3393.61	110	500	0.004661
1962.5	11	308.51	3393.61	110		0.4661%

\*Requirements set forth in OET Bulletin 65. Based on NCRP Report No. 86 and ANSI/IEEE C95.1-1992