



STATE OF CONNECTICUT

CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: siting.council@ct.gov

www.ct.gov/csc

VIA ELECTRONIC MAIL

August 1, 2019

Jeffrey Barbadora
Real Estate Specialist
Crown Castle
12 Gill Street, Suite 5800
Woburn, MA 01801

RE: **EM-SPRINT-040-190626** – Sprint notice of intent to modify an existing telecommunications facility located at 60 South Main Street, East Granby, Connecticut.

Dear Mr. Barbadora:

The Connecticut Siting Council (Council) is in receipt of your correspondence of July 31, 2019 submitted in response to the Council's June 27, 2019 notification of an incomplete request for exempt modification with regard to the above-referenced matter.

The submission renders the request for exempt modification complete and the Council will process the request in accordance with the Federal Communications Commission 60-day timeframe.

Thank you for your attention and cooperation.

Sincerely,

Melanie A. Bachman
Executive Director

MAB/IN/emr



Robidoux, Evan

From: Barbadora, Jeff <Jeff.Barbadora@crowncastle.com>
Sent: Wednesday, July 31, 2019 3:39 PM
To: Robidoux, Evan
Cc: CSC-DL Siting Council
Subject: RE: Council Extension Letter for EM-SPRINT-040-190626-SouthMainSt-EastGranby
Attachments: 876399_397084_MountModification_Pass_SprintPCS_94.0ft_07-23-2019.pdf; 876399_397084_MountModificationDrawings_SprintPCS_94.0ft_07-23-2019.pdf; BU876399 (F) E. Granby 4Q2000 Galasso_Sprint PCS CT43XC804_SA127643_006_01_07.29.19 WO1773228 (003).pdf

Hi Evan,

Attached are the updated MA and SA. I am overnighting three copies of each to your office for tomorrow delivery.

Thanks,

Jeffrey Barbadora

781-970-0053
12 Gill Street, Suite 5800, Woburn, MA 01801
CrownCastle.com

From: Robidoux, Evan <Evan.Robidoux@ct.gov>
Sent: Monday, July 15, 2019 12:00 PM
To: Barbadora, Jeff <Jeff.Barbadora@crowncastle.com>
Cc: CSC-DL Siting Council <Siting.Council@ct.gov>
Subject: Council Extension Letter for EM-SPRINT-040-190626-SouthMainSt-EastGranby

CAUTION: This email originated from outside of the organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.

Please see the attached correspondence.

Evan Robidoux
Clerk Typist
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

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Date: July 23, 2019

INFINIGY
FROM ZERO TO INFINIGY
the solutions are endless
Infinigy Engineering, PLLC
1033 Watervliet Shaker Road
Albany, NY 12205
518-690-0790
structural@infinigy.com

Kevin Morrow
Crown Castle
3530 Toringdon Way, Suite 300
Charlotte, NC 28277
704-405-6619

Subject: Mount Modification Report

Carrier Designation: Sprint PCS Equipment Change-Out
Carrier Site Number: CT43XC804
Carrier Site Name: CT43XC804

Crown Castle Designation: **Crown Castle BU Number:** 876399
Crown Castle Site Name: (F) E. GRANBY 4Q2000 / GALASSO
Crown Castle JDE Job Number: 447217
Crown Castle Order Number: 397084 Rev. 4

Engineering Firm Designation: Infinigy Engineering, PLLC Report Designation: 1039-B0002-B

Site Data: 60 South Main St., East Granby, Hartford County, CT, 06026
Latitude 41°56'29.59" Longitude -72°44'19.25"

Structure Information: **Tower Height & Type:** 98.0 ft Monopole
Mount Elevation: 94.0 ft
Mount Type: 14.0 ft Platform

Dear Kevin Morrow,

Infinigy Engineering, PLLC is pleased to submit this "Mount Modification Report" to determine the structural integrity of Sprint PCS's antenna mounting system with the proposed appurtenance and equipment addition on the abovementioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

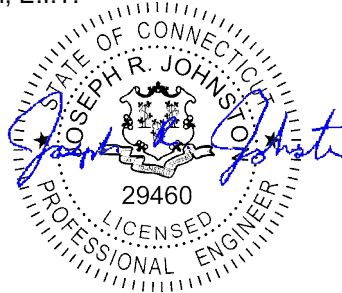
The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

Platform **Sufficient**
***Sufficient upon completion of the changes listed in the 'Recommendations' section of this report.**

This analysis utilizes an ultimate 3-second gust wind speed of 125 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount analysis prepared by: Mukunda Pokharel, E.I.T.

Respectfully Submitted by:
Joe Johnston, P.E.
VP Structural Engineering/Principal
518-690-0790
jjohnston@infinigy.com
CT PE License No. PEN.0029460



07-23-2019

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1) INTRODUCTION

This is a 14.0 ft Platform.

For the purpose of this analysis, the modifications detailed in Appendix E are considered to be installed.

2) ANALYSIS CRITERIA

Building Code:	2015 International Building Code / 2018 Connecticut State Building Code
TIA-222 Revision:	TIA-222-H
Risk Category:	II
Ultimate Wind Speed:	125 mph
Exposure Category:	C
Topographic Factor at Base:	1.000
Topographic Factor at Mount:	1.000
Ice Thickness:	2.0 in
Wind Speed with Ice:	50 mph
Live Loading Wind Speed:	30 mph
Man Live Load at Mid-Points:	250 lb
Man Live Load at Mount Pipes:	500 lb

Table 1 - Proposed Equipment Configuration

Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount / Modification Details
94.0	97.0	3	COMMSCOPE	NNVV-65B-R4	14.0 ft Platform
		3	RFS/CELWAVE	APXV/TM14-ALU-I20	
		3	ALCATEL LUCENT	PCS 1900MHZ 4X45W-65MHZ	
		6	ALCATEL LUCENT	RRH2X50-800	
		3	ALCATEL LUCENT	TD-RRH8X20-25	

3) ANALYSIS PROCEDURE

Table 2 - Documents Provided

Document	Remarks	Reference	Source
Crown Application	Sprint PCS Application	397084 Rev. 4	CCI Sites
Mount Analysis Documents	Infinigy Engineering, PLLC	7523327	CCI Sites
Mount Modification Drawings	Infinigy Engineering, PLLC	Appendix E	Infinigy

3.1) Analysis Method

RISA-3D (Version 17.0.4), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases.

Infinigy Mount Analysis Tool 4.0.5, a tool internally developed by Infinigy, was used to calculate wind loading on all appurtenances, dishes and mount members for various loading cases. Selected output from the analysis is included in Appendix B "Software Input Calculations".

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 *Tower Mount Analysis* (Revision B).

3.2) Assumptions

- 1) The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- 2) The configuration of antennas, mounts, and other appurtenances are as specified in Table 1 and the referenced drawings.
- 3) All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- 4) The analysis will be required to be revised if the existing conditions in the field differ from those shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
- 5) Steel grades have been assumed as follows, unless noted otherwise:

Channel, Solid Round, Angle, Plate	ASTM A36 (GR 36)
HSS (Rectangular)	ASTM 500 (GR B-46)
Pipe	ASTM A53 (GR 35)
Connection Bolts	ASTM A325

This analysis may be affected if any assumptions are not valid or have been made in error. Infinigy Engineering, PLLC should be notified to determine the effect on the structural integrity of the antenna mounting system.

4) ANALYSIS RESULTS

Table 3 - Mount Component Stresses vs. Capacity (Platform, All Sector)

Notes	Component	Critical Member	Centerline (ft)	% Capacity	Pass / Fail
1, 2	Mount Pipe(s)	MP8	94.0	57.8	Pass
	Horizontal(s)	M4		87.1	Pass
	Standoff(s)	M48A		93.4	Pass
	Plate(s)	P9		16.0	Pass
	Mount Connection(s)	--		71.0	Pass

Structure Rating (max from all components) =	93.4%
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Notes:

- 1) See additional documentation in "Appendix C – Software Analysis Output" for calculations supporting the % capacity consumed.
- 2) All sectors are typical.

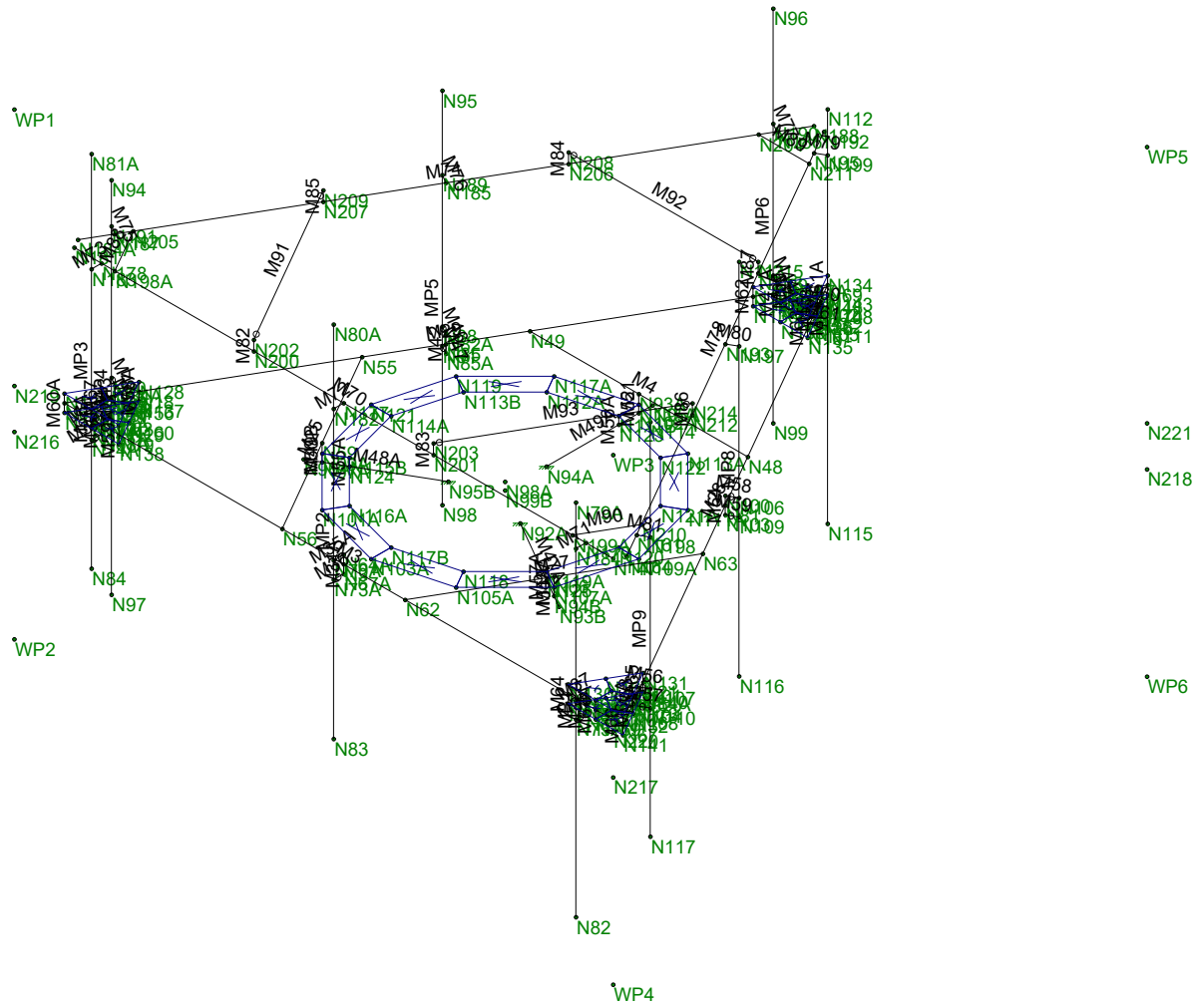
4.1) Recommendations

The mount has sufficient capacity to carry the proposed loading configuration. In order for the results of the analysis to be considered valid, the structural modification listed below must be completed.

1. Install a Site Pro 1 HRK14-3HD Handrail Kit.

Engineering detail drawings have been provided in Appendix E – Mount Modification Design Drawings. Connection from the mount to the tower and local stresses on the tower are sufficient.

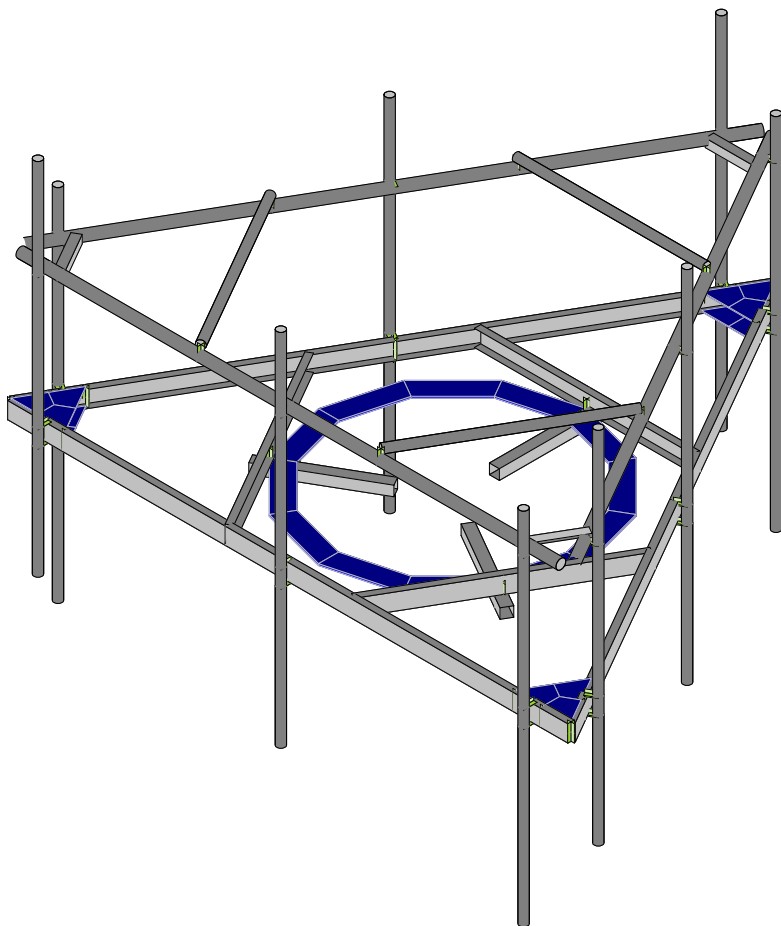
APPENDIX A
WIRE FRAME AND RENDERED MODELS



Infinigy Engineering, PLLC
MP
1039-B0002-B

876399

Wire Frame
July 22, 2019 at 10:34 AM
876399_loaded - MOD.r3d



Infinigy Engineering, PLLC

MP

1039-B0002-B

876399

Rendered

July 22, 2019 at 10:35 AM

876399_loaded - MOD.r3d

APPENDIX B
SOFTWARE INPUT CALCULATIONS

APPENDIX C
SOFTWARE ANALYSIS OUTPUT



Company : Infinigy Engineering, PLLC
 Designer : MP
 Job Number : 1039-B0002-B
 Model Name : 876399

July 22, 2019
 10:41 AM
 Checked By: _____

Member Primary Data

	Label	I Joint	J Joint	K Joint Ro...	Section/Shape	Type	Design List	Material	Design Rules
1	M3	N68	N67	180	Horizontal	Beam	RECT	A36 Gr.36	Typical
2	M4	N49	N48		Horizontal	Beam	RECT	A36 Gr.36	Typical
3	M25	N56	N55		Horizontal	Beam	RECT	A36 Gr.36	Typical
4	M27	N63	N62		Horizontal	Beam	RECT	A36 Gr.36	Typical
5	M28	N67	N69	180	Horizontal	Beam	RECT	A36 Gr.36	Typical
6	M29	N69	N68	180	Horizontal	Beam	RECT	A36 Gr.36	Typical
7	M30A	N65A	N68A		RIGID	None	None	RIGID	Typical
8	M31A	N64A	N67A		RIGID	None	None	RIGID	Typical
9	M32A	N66A	N69A		RIGID	None	None	RIGID	Typical
10	M33A	N71A	N65A		RIGID	None	None	RIGID	Typical
11	M34A	N74A	N68A		RIGID	None	None	RIGID	Typical
12	M35A	N70A	N64A		RIGID	None	None	RIGID	Typical
13	M36	N73A	N67A		RIGID	None	None	RIGID	Typical
14	M37	N72A	N66A		RIGID	None	None	RIGID	Typical
15	M38	N75A	N69A		RIGID	None	None	RIGID	Typical
16	MP3	N81A	N84		Mount Pipe	Beam	Single Angle	A53 Gr.B	Typical
17	MP2	N80A	N83		Mount Pipe	Beam	Single Angle	A53 Gr.B	Typical
18	MP1	N79A	N82		Mount Pipe	Beam	Single Angle	A53 Gr.B	Typical
19	M41A	N83A	N86		RIGID	None	None	RIGID	Typical
20	M42	N82A	N85A		RIGID	None	None	RIGID	Typical
21	M43	N84A	N87		RIGID	None	None	RIGID	Typical
22	M44	N89	N83A		RIGID	None	None	RIGID	Typical
23	M45	N92	N86		RIGID	None	None	RIGID	Typical
24	M46	N88	N82A		RIGID	None	None	RIGID	Typical
25	M47	N91	N85A		RIGID	None	None	RIGID	Typical
26	M48	N90	N84A		RIGID	None	None	RIGID	Typical
27	M49	N93	N87		RIGID	None	None	RIGID	Typical
28	MP6	N96	N99		Mount Pipe	Beam	Single Angle	A53 Gr.B	Typical
29	MP5	N95	N98		Mount Pipe	Beam	Single Angle	A53 Gr.B	Typical
30	MP4	N94	N97		Mount Pipe	Beam	Single Angle	A53 Gr.B	Typical
31	M53	N101	N104		RIGID	None	None	RIGID	Typical
32	M54	N100	N103		RIGID	None	None	RIGID	Typical
33	M55	N102	N105		RIGID	None	None	RIGID	Typical
34	M56	N107	N101		RIGID	None	None	RIGID	Typical
35	M57	N110	N104		RIGID	None	None	RIGID	Typical
36	M58	N106	N100		RIGID	None	None	RIGID	Typical
37	M59	N109	N103		RIGID	None	None	RIGID	Typical
38	M60	N108	N102		RIGID	None	None	RIGID	Typical
39	M61	N111	N105		RIGID	None	None	RIGID	Typical
40	MP9	N114	N117		Mount Pipe	Beam	Single Angle	A53 Gr.B	Typical
41	MP8	N113	N116		Mount Pipe	Beam	Single Angle	A53 Gr.B	Typical
42	MP7	N112	N115		Mount Pipe	Beam	Single Angle	A53 Gr.B	Typical
43	M49A	N94A	N95A		Standoff	Beam	Tube	A53 Gr.B	Typical
44	M47A	N92A	N93B		Standoff	Beam	Tube	A53 Gr.B	Typical
45	M48A	N95B	N96A		Standoff	Beam	Tube	A53 Gr.B	Typical
46	M49B	N59	N99A		RIGID	None	None	RIGID	Typical
47	M50	N99A	N97A		RIGID	None	None	RIGID	Typical
48	M51	N93A	N115A		RIGID	None	None	RIGID	Typical
49	M52	N115A	N102A		RIGID	None	None	RIGID	Typical
50	M53A	N66	N107A		RIGID	None	None	RIGID	Typical
51	M54A	N107A	N94B		RIGID	None	None	RIGID	Typical
52	M55A	N115B	N124		RIGID	None	None	RIGID	Typical
53	M56A	N111B	N123		RIGID	None	None	RIGID	Typical
54	M57A	N119A	N125		RIGID	None	None	RIGID	Typical
55	M58A	N128	N137		RIGID	None	None	RIGID	Typical
56	M59A	N129	N138		RIGID	None	None	RIGID	Typical



Member Primary Data (Continued)

Label	I Joint	J Joint	K Joint Ro...	Section/Shape	Type	Design List	Material	Design Rules
57	M60A	N133	N142		RIGID	None	RIGID	Typical
58	M61A	N134	N143		RIGID	None	RIGID	Typical
59	M62	N127	N136		RIGID	None	RIGID	Typical
60	M63	N126	N135		RIGID	None	RIGID	Typical
61	M64	N130	N139		RIGID	None	RIGID	Typical
62	M65	N131	N140		RIGID	None	RIGID	Typical
63	M66	N132	N141		RIGID	None	RIGID	Typical
64	M64A	N165	N169		RIGID	None	RIGID	Typical
65	M65A	N164A	N168		RIGID	None	RIGID	Typical
66	M66A	N157	N161A		RIGID	None	RIGID	Typical
67	M67	N155	N159		RIGID	None	RIGID	Typical
68	M68	N148	N152		RIGID	None	RIGID	Typical
69	M69	N147A	N151		RIGID	None	RIGID	Typical
70	M70	N181	N147	180	Handrail	Beam	Q235-GB	Typical
71	M71	N184	N179		RIGID	None	RIGID	Typical
72	M72	N182	N177		RIGID	None	RIGID	Typical
73	M73	N183	N178		RIGID	None	RIGID	Typical
74	M74	N188	N184A	180	Handrail	Beam	Q235-GB	Typical
75	M75	N191	N187		RIGID	None	RIGID	Typical
76	M76	N189	N185		RIGID	None	RIGID	Typical
77	M77	N190	N186		RIGID	None	RIGID	Typical
78	M78	N164	N192	180	Handrail	Beam	Q235-GB	Typical
79	M79	N199	N195		RIGID	None	RIGID	Typical
80	M80	N197	N193		RIGID	None	RIGID	Typical
81	M81	N198	N161		RIGID	None	RIGID	Typical
82	M82	N202	N200		RIGID	None	RIGID	Typical
83	M83	N203	N201		RIGID	None	RIGID	Typical
84	M84	N208	N206		RIGID	None	RIGID	Typical
85	M85	N209	N207		RIGID	None	RIGID	Typical
86	M86	N214	N212		RIGID	None	RIGID	Typical
87	M87	N215	N213		RIGID	None	RIGID	Typical
88	M88	N198A	N205	180	Handrail Corner	Beam	Q345	Typical
89	M89	N204	N211	180	Handrail Corner	Beam	Q345	Typical
90	M90	N199A	N210	90	Handrail Corner	Beam	Q345	Typical
91	M91	N202	N209		Handrail Ties	Beam	Q235-GB	Typical
92	M92	N208	N215		Handrail Ties	Beam	Q235-GB	Typical
93	M93	N203	N214		Handrail Ties	Beam	Q235-GB	Typical

Material Takeoff

	Material	Size	Pieces	Length[in]	Weight[K]
1	General				
2	RIGID		66	240.9	0
3	Total General		66	240.9	0
4					
5	Hot Rolled Steel				
6	A36 Gr.36	C5X9	6	700.6	.524
7	A53 Gr.B	HSS3X3X5	3	95.6	.08
8	A53 Gr.B	PIPE 2.0	9	972	.281
9	Q235-GB	PIPE 2.0	3	171	.049
10	Q235-GB	PIPE 2.5	3	486	.222
11	Q345	L2.5x2.5x3	3	45.5	.012
12	Total HR Steel		27	2470.7	1.168
13					
14	Plate Elements	Thickness (in)		Volume (yds^3)	
15	gen_Steel	.5	12	0	.191



Company : Infinigy Engineering, PLLC
 Designer : MP
 Job Number : 1039-B0002-B
 Model Name : 876399

July 22, 2019
 10:41 AM
 Checked By: _____

Material Takeoff (Continued)

	Material	Size	Pieces	Length[in]	Weight[K]
16	gen Steel	.3	18	0	.05
17	Total Plates		30	0	.24

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Self Weight	DL		-1			21		3	
2	Wind Load AZI 000	WLZ					21		3	
3	Wind Load AZI 090	WLX					21		3	
4	Ice Weight	OL1					21	48	3	
5	Wind + Ice Load AZI 000	OL2					21		1	
6	Wind + Ice Load AZI 090	OL3					21		1	
7	Service Live 1	LL					3			
8	BLC 1 Transient Area Loads	None								
9	BLC 2 Transient Area Loads	None						26		
10	Maintenance Load 1	None				1				
11	Maintenance Load 2	None				1				
12	Maintenance Load 3	None				1				
13	Maintenance Load 4	None				1				
14	Maintenance Load 5	None				1				
15	Maintenance Load 6	None				1				
16	Maintenance Load 7	None				1				
17	Maintenance Load 8	None				1				
18	Maintenance Load 9	None				1				
19	BLC 3 Transient Area Loads	None						106		
20	BLC 4 Transient Area Loads	None						26		
21	BLC 5 Transient Area Loads	None						83		
22	BLC 6 Transient Area Loads	None						88		

Load Combinations

	Description	...	Y	BLC Factor	BLC Factor	BLC Factor	BLC Factor	...	F.....	F.....	F.....	F.....	F.....	F.....
1	1.4DL	...	Y	DL	1.4									
2	1.2DL + 1WL AZI 0	...	Y	DL	1.2	WLZ	1	WLX						
3	1.2DL + 1WL AZI 30	...	Y	DL	1.2	WLZ	.866	WLX	.5					
4	1.2DL + 1WL AZI 60	...	Y	DL	1.2	WLZ	.5	WLX	.866					
5	1.2DL + 1WL AZI 90	...	Y	DL	1.2	WLZ		WLX	1					
6	1.2DL + 1WL AZI 120	...	Y	DL	1.2	WLZ	-.5	WLX	.866					
7	1.2DL + 1WL AZI 150	...	Y	DL	1.2	WLZ	-.866	WLX	.5					
8	1.2DL + 1WL AZI 180	...	Y	DL	1.2	WLZ	-1	WLX						
9	1.2DL + 1WL AZI 210	...	Y	DL	1.2	WLZ	-.866	WLX	-.5					
10	1.2DL + 1WL AZI 240	...	Y	DL	1.2	WLZ	-.5	WLX	-.866					
11	1.2DL + 1WL AZI 270	...	Y	DL	1.2	WLZ		WLX	-1					
12	1.2DL + 1WL AZI 300	...	Y	DL	1.2	WLZ	.5	WLX	-.866					
13	1.2DL + 1WL AZI 330	...	Y	DL	1.2	WLZ	.866	WLX	-.5					
14	0.9DL + 1WL AZI 0	...	Y	1	.9	WLZ	1	WLX						
15	0.9DL + 1WL AZI 30	...	Y	1	.9	WLZ	.866	WLX	.5					
16	0.9DL + 1WL AZI 60	...	Y	1	.9	WLZ	.5	WLX	.866					
17	0.9DL + 1WL AZI 90	...	Y	1	.9	WLZ		WLX	1					
18	0.9DL + 1WL AZI 120	...	Y	1	.9	WLZ	-.5	WLX	.866					
19	0.9DL + 1WL AZI 150	...	Y	1	.9	WLZ	-.866	WLX	.5					
20	0.9DL + 1WL AZI 180	...	Y	1	.9	WLZ	-1	WLX						
21	0.9DL + 1WL AZI 210	...	Y	1	.9	WLZ	-.866	WLX	-.5					
22	0.9DL + 1WL AZI 240	...	Y	1	.9	WLZ	-.5	WLX	-.866					
23	0.9DL + 1WL AZI 270	...	Y	1	.9	WLZ		WLX	-1					
24	0.9DL + 1WL AZI 300	...	Y	1	.9	WLZ	.5	WLX	-.866					



Load Combinations (Continued)

Description	...	Y	BLC	Factor	BLC	Factor	BLC	Factor	BLC	Factor	...	F.....	F.....	F.....	F.....	F.....	F.....
25 0.9DL + 1WL AZI 330	...	Y	1	.9	WLZ	.866	WLX	-5									
26 1.2D + 1.0Di	...	Y	DL	1.2	OL1	1											
27 1.2D + 1.0Di + 1.0Wi AZI 0	...	Y	DL	1.2	OL1	1	OL2	1	OL3								
28 1.2D + 1.0Di + 1.0Wi AZI 30	...	Y	DL	1.2	OL1	1	OL2	.866	OL3	.5							
29 1.2D + 1.0Di + 1.0Wi AZI 60	...	Y	DL	1.2	OL1	1	OL2	.5	OL3	.866							
30 1.2D + 1.0Di + 1.0Wi AZI 90	...	Y	DL	1.2	OL1	1	OL2		OL3	1							
31 1.2D + 1.0Di + 1.0Wi AZI 120	...	Y	DL	1.2	OL1	1	OL2	-.5	OL3	.866							
32 1.2D + 1.0Di + 1.0Wi AZI 150	...	Y	DL	1.2	OL1	1	OL2	-.866	OL3	.5							
33 1.2D + 1.0Di + 1.0Wi AZI 180	...	Y	DL	1.2	OL1	1	OL2	-1	OL3								
34 1.2D + 1.0Di + 1.0Wi AZI 210	...	Y	DL	1.2	OL1	1	OL2	-.866	OL3	-.5							
35 1.2D + 1.0Di + 1.0Wi AZI 240	...	Y	DL	1.2	OL1	1	OL2	-.5	OL3	-.866							
36 1.2D + 1.0Di + 1.0Wi AZI 270	...	Y	DL	1.2	OL1	1	OL2		OL3	-1							
37 1.2D + 1.0Di + 1.0Wi AZI 300	...	Y	DL	1.2	OL1	1	OL2	.5	OL3	-.866							
38 1.2D + 1.0Di + 1.0Wi AZI 330	...	Y	DL	1.2	OL1	1	OL2	.866	OL3	-.5							
39 1.2DL + 1.5LV	...	Y	DL	1.2	LL	1.5											
40 1.2DL + 1.5LM (LM-1) + 1SWL (30 mph)	Y	DL	1.2	10	1.5	WLZ	.058	WLX								
41 1.2DL + 1.5LM (LM-1) + 1SWL (30 mph)	Y	DL	1.2	10	1.5	WLZ	.05	WLX	.029							
42 1.2DL + 1.5LM (LM-1) + 1SWL (30 mph)	Y	DL	1.2	10	1.5	WLZ	.029	WLX	.05							
43 1.2DL + 1.5LM (LM-1) + 1SWL (30 mph)	Y	DL	1.2	10	1.5	WLZ		WLX	.058							
44 1.2DL + 1.5LM (LM-1) + 1SWL (30 mph)	Y	DL	1.2	10	1.5	WLZ	-.029	WLX	.05							
45 1.2DL + 1.5LM (LM-1) + 1SWL (30 mph)	Y	DL	1.2	10	1.5	WLZ	-.05	WLX	.029							
46 1.2DL + 1.5LM (LM-1) + 1SWL (30 mph)	Y	DL	1.2	10	1.5	WLZ	-.058	WLX								
47 1.2DL + 1.5LM (LM-1) + 1SWL (30 mph)	Y	DL	1.2	10	1.5	WLZ	-.05	WLX	-.029							
48 1.2DL + 1.5LM (LM-1) + 1SWL (30 mph)	Y	DL	1.2	10	1.5	WLZ	-.029	WLX	-.05							
49 1.2DL + 1.5LM (LM-1) + 1SWL (30 mph)	Y	DL	1.2	10	1.5	WLZ		WLX	-.058							
50 1.2DL + 1.5LM (LM-1) + 1SWL (30 mph)	Y	DL	1.2	10	1.5	WLZ	.029	WLX	-.05							
51 1.2DL + 1.5LM (LM-1) + 1SWL (30 mph)	Y	DL	1.2	10	1.5	WLZ	.05	WLX	-.029							
52 1.2DL + 1.5LM (LM-2) + 1SWL (30 mph)	Y	DL	1.2	11	1.5	WLZ	.058	WLX								
53 1.2DL + 1.5LM (LM-2) + 1SWL (30 mph)	Y	DL	1.2	11	1.5	WLZ	.05	WLX	.029							
54 1.2DL + 1.5LM (LM-2) + 1SWL (30 mph)	Y	DL	1.2	11	1.5	WLZ	.029	WLX	.05							
55 1.2DL + 1.5LM (LM-2) + 1SWL (30 mph)	Y	DL	1.2	11	1.5	WLZ		WLX	.058							
56 1.2DL + 1.5LM (LM-2) + 1SWL (30 mph)	Y	DL	1.2	11	1.5	WLZ	-.029	WLX	.05							
57 1.2DL + 1.5LM (LM-2) + 1SWL (30 mph)	Y	DL	1.2	11	1.5	WLZ	-.05	WLX	.029							
58 1.2DL + 1.5LM (LM-2) + 1SWL (30 mph)	Y	DL	1.2	11	1.5	WLZ	-.058	WLX								
59 1.2DL + 1.5LM (LM-2) + 1SWL (30 mph)	Y	DL	1.2	11	1.5	WLZ	-.05	WLX	-.029							
60 1.2DL + 1.5LM (LM-2) + 1SWL (30 mph)	Y	DL	1.2	11	1.5	WLZ	-.029	WLX	-.05							
61 1.2DL + 1.5LM (LM-2) + 1SWL (30 mph)	Y	DL	1.2	11	1.5	WLZ		WLX	-.058							
62 1.2DL + 1.5LM (LM-2) + 1SWL (30 mph)	Y	DL	1.2	11	1.5	WLZ	.029	WLX	-.05							
63 1.2DL + 1.5LM (LM-2) + 1SWL (30 mph)	Y	DL	1.2	11	1.5	WLZ	.05	WLX	-.029							
64 1.2DL + 1.5LM (LM-3) + 1SWL (30 mph)	Y	DL	1.2	12	1.5	WLZ	.058	WLX								
65 1.2DL + 1.5LM (LM-3) + 1SWL (30 mph)	Y	DL	1.2	12	1.5	WLZ	.05	WLX	.029							
66 1.2DL + 1.5LM (LM-3) + 1SWL (30 mph)	Y	DL	1.2	12	1.5	WLZ	.029	WLX	.05							
67 1.2DL + 1.5LM (LM-3) + 1SWL (30 mph)	Y	DL	1.2	12	1.5	WLZ		WLX	.058							
68 1.2DL + 1.5LM (LM-3) + 1SWL (30 mph)	Y	DL	1.2	12	1.5	WLZ	-.029	WLX	.05							
69 1.2DL + 1.5LM (LM-3) + 1SWL (30 mph)	Y	DL	1.2	12	1.5	WLZ	-.05	WLX	.029							
70 1.2DL + 1.5LM (LM-3) + 1SWL (30 mph)	Y	DL	1.2	12	1.5	WLZ	-.058	WLX								
71 1.2DL + 1.5LM (LM-3) + 1SWL (30 mph)	Y	DL	1.2	12	1.5	WLZ	-.05	WLX	-.029							
72 1.2DL + 1.5LM (LM-3) + 1SWL (30 mph)	Y	DL	1.2	12	1.5	WLZ	-.029	WLX	-.05							
73 1.2DL + 1.5LM (LM-3) + 1SWL (30 mph)	Y	DL	1.2	12	1.5	WLZ		WLX	-.058							
74 1.2DL + 1.5LM (LM-3) + 1SWL (30 mph)	Y	DL	1.2	12	1.5	WLZ	.029	WLX	-.05							
75 1.2DL + 1.5LM (LM-3) + 1SWL (30 mph)	Y	DL	1.2	12	1.5	WLZ	.05	WLX	-.029							
76 1.2DL + 1.5LM (LM-4) + 1SWL (30 mph)	Y	DL	1.2	13	1.5	WLZ	.058	WLX								
77 1.2DL + 1.5LM (LM-4) + 1SWL (30 mph)	Y	DL	1.2	13	1.5	WLZ	.05	WLX	.029							
78 1.2DL + 1.5LM (LM-4) + 1SWL (30 mph)	Y	DL	1.2	13	1.5	WLZ	.029	WLX	.05							
79 1.2DL + 1.5LM (LM-4) + 1SWL (30 mph)	Y	DL	1.2	13	1.5	WLZ		WLX	.058							
80 1.2DL + 1.5LM (LM-4) + 1SWL (30 mph)	Y	DL	1.2	13	1.5	WLZ	-.029	WLX	.05							
81 1.2DL + 1.5LM (LM-4) + 1SWL (30 mph)	Y	DL	1.2	13	1.5	WLZ	-.05	WLX	.029							



Load Combinations (Continued)

	Description	BLC	Factor	BLC	Factor	BLC	Factor	BLC	Factor	F.....	F.....	F.....	F.....	F.....	F.....
82	1.2DL + 1.5LM (LM-4) + 1SWL (30 mph)	Y	DL	1.2	13	1.5	WLZ	-.058	WLX							
83	1.2DL + 1.5LM (LM-4) + 1SWL (30 mph)	Y	DL	1.2	13	1.5	WLZ	-.05	WLX	-.029						
84	1.2DL + 1.5LM (LM-4) + 1SWL (30 mph)	Y	DL	1.2	13	1.5	WLZ	-.029	WLX	-.05						
85	1.2DL + 1.5LM (LM-4) + 1SWL (30 mph)	Y	DL	1.2	13	1.5	WLZ		WLX	-.058						
86	1.2DL + 1.5LM (LM-4) + 1SWL (30 mph)	Y	DL	1.2	13	1.5	WLZ	.029	WLX	-.05						
87	1.2DL + 1.5LM (LM-4) + 1SWL (30 mph)	Y	DL	1.2	13	1.5	WLZ	.05	WLX	-.029						
88	1.2DL + 1.5LM (LM-5) + 1SWL (30 mph)	Y	DL	1.2	14	1.5	WLZ	.058	WLX							
89	1.2DL + 1.5LM (LM-5) + 1SWL (30 mph)	Y	DL	1.2	14	1.5	WLZ	.05	WLX	.029						
90	1.2DL + 1.5LM (LM-5) + 1SWL (30 mph)	Y	DL	1.2	14	1.5	WLZ	.029	WLX	.05						
91	1.2DL + 1.5LM (LM-5) + 1SWL (30 mph)	Y	DL	1.2	14	1.5	WLZ		WLX	.058						
92	1.2DL + 1.5LM (LM-5) + 1SWL (30 mph)	Y	DL	1.2	14	1.5	WLZ	-.029	WLX	.05						
93	1.2DL + 1.5LM (LM-5) + 1SWL (30 mph)	Y	DL	1.2	14	1.5	WLZ	-.05	WLX	.029						
94	1.2DL + 1.5LM (LM-5) + 1SWL (30 mph)	Y	DL	1.2	14	1.5	WLZ	-.058	WLX							
95	1.2DL + 1.5LM (LM-5) + 1SWL (30 mph)	Y	DL	1.2	14	1.5	WLZ	-.05	WLX	-.029						
96	1.2DL + 1.5LM (LM-5) + 1SWL (30 mph)	Y	DL	1.2	14	1.5	WLZ	-.029	WLX	-.05						
97	1.2DL + 1.5LM (LM-5) + 1SWL (30 mph)	Y	DL	1.2	14	1.5	WLZ		WLX	-.058						
98	1.2DL + 1.5LM (LM-5) + 1SWL (30 mph)	Y	DL	1.2	14	1.5	WLZ	.029	WLX	-.05						
99	1.2DL + 1.5LM (LM-5) + 1SWL (30 mph)	Y	DL	1.2	14	1.5	WLZ	.05	WLX	-.029						
100	1.2DL + 1.5LM (LM-6) + 1SWL (30 mph)	Y	DL	1.2	15	1.5	WLZ	.058	WLX							
101	1.2DL + 1.5LM (LM-6) + 1SWL (30 mph)	Y	DL	1.2	15	1.5	WLZ	.05	WLX	.029						
102	1.2DL + 1.5LM (LM-6) + 1SWL (30 mph)	Y	DL	1.2	15	1.5	WLZ	.029	WLX	.05						
103	1.2DL + 1.5LM (LM-6) + 1SWL (30 mph)	Y	DL	1.2	15	1.5	WLZ		WLX	.058						
104	1.2DL + 1.5LM (LM-6) + 1SWL (30 mph)	Y	DL	1.2	15	1.5	WLZ	-.029	WLX	.05						
105	1.2DL + 1.5LM (LM-6) + 1SWL (30 mph)	Y	DL	1.2	15	1.5	WLZ	-.05	WLX	.029						
106	1.2DL + 1.5LM (LM-6) + 1SWL (30 mph)	Y	DL	1.2	15	1.5	WLZ	-.058	WLX							
107	1.2DL + 1.5LM (LM-6) + 1SWL (30 mph)	Y	DL	1.2	15	1.5	WLZ	-.05	WLX	-.029						
108	1.2DL + 1.5LM (LM-6) + 1SWL (30 mph)	Y	DL	1.2	15	1.5	WLZ	-.029	WLX	-.05						
109	1.2DL + 1.5LM (LM-6) + 1SWL (30 mph)	Y	DL	1.2	15	1.5	WLZ		WLX	-.058						
110	1.2DL + 1.5LM (LM-6) + 1SWL (30 mph)	Y	DL	1.2	15	1.5	WLZ	.029	WLX	-.05						
111	1.2DL + 1.5LM (LM-6) + 1SWL (30 mph)	Y	DL	1.2	15	1.5	WLZ	.05	WLX	-.029						
112	1.2DL + 1.5LM (LM-7) + 1SWL (30 mph)	Y	DL	1.2	16	1.5	WLZ	.058	WLX							
113	1.2DL + 1.5LM (LM-7) + 1SWL (30 mph)	Y	DL	1.2	16	1.5	WLZ	.05	WLX	.029						
114	1.2DL + 1.5LM (LM-7) + 1SWL (30 mph)	Y	DL	1.2	16	1.5	WLZ	.029	WLX	.05						
115	1.2DL + 1.5LM (LM-7) + 1SWL (30 mph)	Y	DL	1.2	16	1.5	WLZ		WLX	.058						
116	1.2DL + 1.5LM (LM-7) + 1SWL (30 mph)	Y	DL	1.2	16	1.5	WLZ	-.029	WLX	.05						
117	1.2DL + 1.5LM (LM-7) + 1SWL (30 mph)	Y	DL	1.2	16	1.5	WLZ	-.05	WLX	.029						
118	1.2DL + 1.5LM (LM-7) + 1SWL (30 mph)	Y	DL	1.2	16	1.5	WLZ	-.058	WLX							
119	1.2DL + 1.5LM (LM-7) + 1SWL (30 mph)	Y	DL	1.2	16	1.5	WLZ	-.05	WLX	-.029						
120	1.2DL + 1.5LM (LM-7) + 1SWL (30 mph)	Y	DL	1.2	16	1.5	WLZ	-.029	WLX	-.05						
121	1.2DL + 1.5LM (LM-7) + 1SWL (30 mph)	Y	DL	1.2	16	1.5	WLZ		WLX	-.058						
122	1.2DL + 1.5LM (LM-7) + 1SWL (30 mph)	Y	DL	1.2	16	1.5	WLZ	.029	WLX	-.05						
123	1.2DL + 1.5LM (LM-7) + 1SWL (30 mph)	Y	DL	1.2	16	1.5	WLZ	.05	WLX	-.029						
124	1.2DL + 1.5LM (LM-8) + 1SWL (30 mph)	Y	DL	1.2	17	1.5	WLZ	.058	WLX							
125	1.2DL + 1.5LM (LM-8) + 1SWL (30 mph)	Y	DL	1.2	17	1.5	WLZ	.05	WLX	.029						
126	1.2DL + 1.5LM (LM-8) + 1SWL (30 mph)	Y	DL	1.2	17	1.5	WLZ	.029	WLX	.05						
127	1.2DL + 1.5LM (LM-8) + 1SWL (30 mph)	Y	DL	1.2	17	1.5	WLZ		WLX	.058						
128	1.2DL + 1.5LM (LM-8) + 1SWL (30 mph)	Y	DL	1.2	17	1.5	WLZ	-.029	WLX	.05						
129	1.2DL + 1.5LM (LM-8) + 1SWL (30 mph)	Y	DL	1.2	17	1.5	WLZ	-.05	WLX	.029						
130	1.2DL + 1.5LM (LM-8) + 1SWL (30 mph)	Y	DL	1.2	17	1.5	WLZ	-.058	WLX							
131	1.2DL + 1.5LM (LM-8) + 1SWL (30 mph)	Y	DL	1.2	17	1.5	WLZ	-.05	WLX	-.029						
132	1.2DL + 1.5LM (LM-8) + 1SWL (30 mph)	Y	DL	1.2	17	1.5	WLZ	-.029	WLX	-.05						
133	1.2DL + 1.5LM (LM-8) + 1SWL (30 mph)	Y	DL	1.2	17	1.5	WLZ		WLX	-.058						
134	1.2DL + 1.5LM (LM-8) + 1SWL (30 mph)	Y	DL	1.2	17	1.5	WLZ	.029	WLX	-.05						
135	1.2DL + 1.5LM (LM-8) + 1SWL (30 mph)	Y	DL	1.2	17	1.5	WLZ	.05	WLX	-.029						
136	1.2DL + 1.5LM (LM-9) + 1SWL (30 mph)	Y	DL	1.2	18	1.5	WLZ	.058	WLX							
137	1.2DL + 1.5LM (LM-9) + 1SWL (30 mph)	Y	DL	1.2	18	1.5	WLZ	.05	WLX	.029						
138	1.2DL + 1.5LM (LM-9) + 1SWL (30 mph)	Y	DL	1.2	18	1.5	WLZ	.029	WLX	.05						



Company : Infinigy Engineering, PLLC
 Designer : MP
 Job Number : 1039-B0002-B
 Model Name : 876399

July 22, 2019
 10:41 AM
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Load Combinations (Continued)

	Description	Y	DL	1.2	18	1.5	WLZ	WLX	Factor	F	F	F	F	F
139	1.2DL + 1.5LM (LM-9) + 1SWL (30 mph)	Y	DL	1.2	18	1.5	WLZ	WLX	.058					
140	1.2DL + 1.5LM (LM-9) + 1SWL (30 mph)	Y	DL	1.2	18	1.5	WLZ	WLX	-.029					
141	1.2DL + 1.5LM (LM-9) + 1SWL (30 mph)	Y	DL	1.2	18	1.5	WLZ	WLX	-.05					
142	1.2DL + 1.5LM (LM-9) + 1SWL (30 mph)	Y	DL	1.2	18	1.5	WLZ	WLX	-.058					
143	1.2DL + 1.5LM (LM-9) + 1SWL (30 mph)	Y	DL	1.2	18	1.5	WLZ	WLX	-.05					
144	1.2DL + 1.5LM (LM-9) + 1SWL (30 mph)	Y	DL	1.2	18	1.5	WLZ	WLX	-.029					
145	1.2DL + 1.5LM (LM-9) + 1SWL (30 mph)	Y	DL	1.2	18	1.5	WLZ	WLX	-.058					
146	1.2DL + 1.5LM (LM-9) + 1SWL (30 mph)	Y	DL	1.2	18	1.5	WLZ	WLX	.029					
147	1.2DL + 1.5LM (LM-9) + 1SWL (30 mph)	Y	DL	1.2	18	1.5	WLZ	WLX	-.029					

Envelope Joint Reactions

Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-ft]	LC	MY [lb-ft]	LC	MZ [lb-ft]	LC	
1	N94A	max	1633.616	5	3666.514	27	-132.926	14	6461.465	2	1750.426	11	1263.726	11
		min	-1632.497	11	-1032.756	20	-13719.512	33	-3813.797	20	-1751.74	5	-1266.97	5
3	N92A	max	11828.192	29	3657.055	35	6962.596	28	2088.767	15	717.909	8	5468.517	10
		min	348.502	22	-1000.805	16	-311.158	21	-3399.688	9	-720.635	2	-3181.712	16
5	N95B	max	-453.475	18	3653.523	31	7015.175	38	2148.578	25	1238.278	2	3160.035	24
		min	-11809.599	37	-1001.248	24	-620.195	19	-3467.401	7	-1234.522	8	-5442.481	6
7	Totals:	max	5835.825	17	9550.397	32	6090.464	2						
		min	-5835.826	11	2282.17	25	-6090.462	20						

Envelope AISC 15th(360-16): LRFD Steel Code Checks

Member	Shape	Code Check	Lo	LC	Shear	Lo	phi*P	phi*P	phi*	phi*	Cb	Eqn		
1	M48A	HSS3X3X5	.934	0	6	.247	0	y 8	8859...	92610	7612.5	7612.5	1.9...	H1-...
2	M49A	HSS3X3X5	.914	0	13	.258	0	z 5	8859...	92610	7612.5	7612.5	1.96	H1-...
3	M47A	HSS3X3X5	.900	0	10	.236	0	y 8	8859...	92610	7612.5	7612.5	1.9...	H1-...
4	M4	C5X9	.871	32...	13	.080	32...	y	3283...	85536	1909...	11853	1.3...	H1-...
5	M25	C5X9	.846	32...	7	.080	32...	y	3283...	85536	1909...	11853	1.3...	H1-...
6	M88	L2.5x2.5x3	.792	0	2	.115	15...	y 3	2738...	2919...	872.5...	1971...	2.0...	H2-1
7	M27	C5X9	.789	32...	9	.080	32...	y	3283...	85536	1909...	11853	1.3...	H1-...
8	M29	C5X9	.770	66.5	13	.296	15...	z 9	4994...	85536	1909...	8891...	1.3...	H1-...
9	M89	L2.5x2.5x3	.758	0	11	.109	15...	y	2738...	2919...	872.5...	1971...	2.16	H2-1
10	M28	C5X9	.728	10...	3	.299	15...	z 5	4994...	85536	1909...	9402...	1.4...	H1-...
11	M90	L2.5x2.5x3	.686	15...	6	.104	0	z 7	2738...	2919...	872.5...	1971...	2.0...	H2-1
12	MP5	PIPE 2.0	.613	60...	7	.273	61...	7	1214...	32130	1871...	1871...	2.0...	H1-...
13	M3	C5X9	.603	10...	11	.272	15...	z	4994...	85536	1909...	9570...	1.4...	H1-...
14	MP8	PIPE 2.0	.578	60...	3	.256	61...	3	1214...	32130	1871...	1871...	1.9...	H1-...
15	MP2	PIPE 2.0	.539	60...	11	.236	61...		1214...	32130	1871...	1871...	1.8...	H1-...
16	MP3	PIPE 2.0	.400	29...	2	.144	69...		1214...	32130	1871...	1871...	4.3...	H1-...
17	MP4	PIPE 2.0	.390	50...	2	.155	51...	2	1214...	32130	1871...	1871...	2.1...	H1-...
18	MP7	PIPE 2.0	.382	50...	10	.152	51...		1214...	32130	1871...	1871...	2.37	H1-...
19	MP6	PIPE 2.0	.360	68...	6	.150	69...	6	1214...	32130	1871...	1871...	2.1	H1-...
20	MP1	PIPE 2.0	.351	50...	6	.137	51...	6	1214...	32130	1871...	1871...	2.2...	H1-...
21	MP9	PIPE 2.0	.349	29...	11	.141	69...	2	1214...	32130	1871...	1871...	2.2...	H1-...
22	M74	PIPE 2.5	.302	11...	6	.225	11...		5066...	50715	3596...	3596...	1.0...	H1-...
23	M78	PIPE 2.5	.269	11...	2	.212	11...	6	5066...	50715	3596...	3596...	1.0...	H1-...
24	M70	PIPE 2.5	.262	11...	10	.257	11...	2	5066...	50715	3596...	3596...	1.0...	H1-...
25	M92	PIPE 2.0	.027	28.5	8	.097	57	5	2451...	32130	1871...	1871...	1.1...	H1-...
26	M93	PIPE 2.0	.024	28.5	4	.092	57		2451...	32130	1871...	1871...	1.1...	H1-...
27	M91	PIPE 2.0	.024	28.5	12	.095	0	9	2451...	32130	1871...	1871...	1.1...	H1-...



Company : Infinigy Engineering, PLLC
 Designer : MP
 Job Number : 1039-B0002-B
 Model Name : 876399

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Envelope Plate/Shell Principal Stresses

Plate	S...	Sigma	Sigma	Tau M	Angle	Von Mises [psi]	LC	
1	P9	m..T	5054...	36546.4...	213148...	37 .408 18	5764.446	37
2		m..	613.3...	17-1391...	27491.3...	20 -.437 23	1024.924	19
3		m..B	1554...	16-884.8...	252005...	29 2.356 45	3505.557	29
4		m..	-636.9...	12-2479...	29208.7...	24 -.783 55	819.609	24
5	P8	m..T	5030...	34496.6...	233135...	33 .422 21	5737.317	33
6		m..	536.3...	15-1381...	31496.7...	25 -.427 14	1002.324	25
7		m..B	1619...	16-862.5...	192000...	29 1.825 21	3489.809	29
8		m..	-730.3...	8 -2416...	29204.4...	20 .533 18	834.909	19
9	P12	m..T	5003...	30 558.11	193126...	29 1.744 17	5720.191	29
10		m..	609.6...	23-1393...	27436.8...	20 .896 22	947.794	21
11		m..B	1566...	37-981.3...	162023...	37 2.302 ...	3534.472	37
12		m..	-679.6...	4 -2480...	37200.0...	16 -.784 9	854.654	16
13	P21	m..T	4686...	23345.7...	252357...	5 2.355 1	4756.81	5
14		m..	-420.4...	7 -4796...	5 11 39	-.768 33	29.312	92
15		m..B	5029...	5 1994...	5 1517...	5 2.332 85	4386.722	5
16		m..	-1994...	11-4966...	11 4.24 90	-.78 34	31.949	92
17	P19	m..T	4718...	5 84.879	152378...	5 2.287 14	4737.52	5
18		m..	-447.2...	35-4671...	11 14.077	89 -.381 25	64.39	133
19		m..B	4978...	231902...	231544...	11 1.754 14	4447.119	5
20		m..	-2055...	5 -5103...	5 10.668 ...	-.225 15	43.015	133
21	P25	m..T	4520...	9 36.447	682322...	9 2.246 ...	4584.17	9
22		m..	-439.6...	27-4491...	3 22.529	57 -.629 ...	68.502	89
23		m..B	4791...	151885...	151459...	3 2.338 ...	4265.027	9
24		m..	-2034...	9 -4901...	9 11.982	90 -.755 31	44.52	89
25	P24	m..T	4493...	15330.6...	172283...	9 2.217 12	4577.577	9
26		m..	-400.4...	11-4588...	9 8.075 60	-.722 89	16.992	60
27		m..B	4837...	9 1952...	9 1442...	9 2.307 1	4215.243	9
28		m..	-1955...	3 -4793...	3 5.79 59	-.766 ...	16.992	60
29	P21A	m..T	5016...	5 2031...	5 1511...	11 2.333 26	4370.428	5
30		m..	-1889...	23-4901...	23 21.452	91 -.785 27	63.507	133
31		m..B	4698...	11385.5...	362341...	5 2.291 ...	4710.868	5
32		m..	-85.315	16-4737...	5 6.464 ...	-.581 85	45.25	133
33	P27A	m..T	4266...	19398.8...	212140...	13 2.262 ...	4323.51	13
34		m..	-468.3...	3 -4364...	13 8.277 ...	-.329 ...	16.678	124
35		m..B	4561.5	131803...	131379...	13 1.644 22	3979.129	13
36		m..	-1805...	7 -4513...	7 6.636 ...	-.475 23	16.678	124
37	P28	m..T	4277...	13 32.75 ...	2172...	13 2.262 65	4312.11	13
38		m..	-435.8...	31-4245...	7 12.804 ...	-.671 74	92.931	57
39		m..B	4536...	191739...	191405...	7 2.331 57	4050.366	13
40		m..	-1888...	13-4649...	13 6.386 57	-.737 89	45.805	57
41	P23	m..T	4851...	11 1960...	11 1477...	5 1.432 14	4294.104	5
42		m..	-1969...	5 -4925...	5 19.963	92 -.269 15	42.325	92
43		m..B	4852...	5 400.5...	7 2348...	5 1.745 14	4776.423	5
44		m..	-347.8...	25-4736...	23 3.675 ...	-.332 ...	49.418	39
45	P27	m..T	4834...	9 2031...	9 1427...	3 1.322 17	4204.869	9
46		m..	-1893...	15-4736...	15 25.47 39	-.447 18	67.492	89
47		m..B	4491...	3 376.3...	272280...	9 1.415 18	4537.417	9
48		m..	-62.883	69-4513...	9 8.047 ...	-.295 17	41.811	108
49	P26	m..T	4649...	3 1933...	3 1384...	9 2.148 18	4098.183	9
50		m..	-1939...	9 -4707...	9 19.509	39 .22 89	39.613	60
51		m..B	4666...	9 364.5...	11 2290...	9 1.956 41	4624.36	9
52		m..	-315.8...	17-4568...	15 8.443 57	.233 17	39.613	60
53	P30	m..T	4551...	13 1880...	13 1360...	7 2.074 22	3961.048	13
54		m..	-1741...	19-4449...	19 22.42 39	.207 23	68.009	57
55		m..B	4283...	7 379.8...	32 2150...	13 1.834 23	4305.433	13
56		m..	-65.212	...-4309...	13 6.973 75	.054 22	41.218	74



Company : Infinigy Engineering, PLLC
 Designer : MP
 Job Number : 1039-B0002-B
 Model Name : 876399

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Envelope Plate/Shell Principal Stresses (Continued)

	Plate	S...	Sigma.....	Sigma.....	Tau M.....	Angle	Von Mises [psi]	LC
57	P29A	m..T	4422....7	1785....7	1345....13	2.329 ...	3909.341	13
58		m..	-1792.713	-4484....13	19.465 ...	- .727 ...	35.315	124
59		m..B	4399....13	449.1....3	2131....13	2.352 96	4332.321	13
60		m..	-400.0.21	-4297....19	7.139 57	-.583 29	35.315	124
61	P4	m..T	1649....20	-807.8...18	2013....33	2.354 97	3512.347	33
62		m..	-878.312	-2457....34	185.0...24	-.784 ...	976.326	17
63		m..B	5038....38	538.6...15	3140....37	.462 18	5747.994	37
64		m..	485.3...19	-1391...35	414.9...17	-.414 25	875.12	17
65	P5	m..T	1597....20	-849.5...23	2008....33	1.829 14	3508.74	33
66		m..	-838.8.4	-2487....32	187.4...16	.53 18	986.616	23
67		m..B	5074....28	552.2...25	3158....29	.425 15	5783.273	29
68		m..	533.2...21	-1391...31	428.2...23	-.455 22	897.862	23
69	P1	m..T	1649....24	-922.5...14	1984....37	2.339 3	3464.628	37
70		m..	-815.6.8	-2420...36	191.3...20	-.761 15	927.416	21
71		m..B	5082....32	496.9...17	3162....33	1.724 19	5789.704	33
72		m..	496.1...25	-1383...35	479.4...15	.846 14	971.558	15
73	P7	m..T	3258....34	592.25 29	1519...34	2.311 2	3154.585	34
74		m..	385.7....	-687.4.22	49.84 6	-.617 25	336.837	101
75		m..B	2489....5	676.6...34	1457...4	2.329 9	2706.853	4
76		m..	179.1...24	-664.6 15	137.3...72	-.502 20	415.951	12
77	P10	m..T	3248....36	569.6...29	1497...36	2.326 3	3129.707	36
78		m..	390.5...79	-542.5.23	77.66 78	-.774 15	340.198	79
79		m..B	2524....3	655.7...36	1446...4	2.302 22	2700.22	4
80		m..	139.14 20	-653.0.21	138.6....	-.691 21	381.424	8
81	P11	m..T	3234....30	561.9...37	1493...30	2.1 14	3117.986	30
82		m..	382.6...20	-552.88 17	77.46127 25	338.045	145
83		m..B	2520....13	666.9...30	1448...12	2.157 21	2702.775	12
84		m..	108.9...20	-645.48 19	139.6....	-.376 8	309.319	8
85	P3	m..T	2523....9	675.5...38	1479...8	1.68 4	2749.162	8
86		m..	106.2...16	-661.5...19	136.5....	-.769 17	273.936	4
87		m..B	3267....38	592.5...32	1529...38	1.047 21	3167.899	38
88		m..	383.7...69	-739.6.25	72.405 70	-.777 22	335.573	69
89	P6	m..T	2519....7	663.8...28	1472...8	2.309 3	2739.904	8
90		m..	88.068 24	-657.9.21	137.4...76	-.551 16	247.3	12
91		m..B	3271....28	588.1...34	1523...28	2.278 10	3165.187	28
92		m..	386.8...47	-674.3.15	75.056 47	-.754 23	337.843	47
93	P2	m..T	2489....11	664.5...32	1457...12	2.29 7	2706.478	12
94		m..	159.0...16	-664.5.25	137.1...44	-.532 20	390.005	4
95		m..B	3277....32	601.1...37	1529...32	2.285 2	3173.838	32
96		m..	385.8....	-726.7...19	71.969 ...	-.685 15	337.194	123
97	P20	m..T	1471....5	-2.382 14	1487...5	2.273 20	2576.449	5
98		m..	-3.706 ...	-1503...5	5.71089 ...	13.268	89
99		m..B	615.5...11	-15.127 ...	626.21 11	2.192 33	1084.68	11
100		m..	-20.51 33	-636.8.11	5.71 ...	-.708 58	13.656	133
101	P23A	m..T	1431....9	-14.877 89	1452...9	1.7 ...	2515.53	9
102		m..	-5.498 ...	-1472...9	5.789 89	-.694 57	13.532	89
103		m..B	569.5...3	-14.877 89	576.1...3	1.656 89	997.957	3
104		m..	-6.63 ...	-582.7...3	5.789 89	-.778 18	13.532	89
105	P29	m..T	1341....13	-13.948 ...	1357.6 13	1.808 16	2351.486	13
106		m..	-8.028 ...	-1373...13	2.96 ...	-.447 75	12.125	125
107		m..B	562.4...8	-17.349 97	566.6...7	1.795 29	981.507	7
108		m..	-23.045 29	-576.6...7	2.859 97	-.637 97	15.313	97
109	P22	m..T	743.26 5	.303 58	728.17 11	1.849 15	1261.354	11
110		m..	8.72 ...	-746.04 11	9.647252 93	16.735	133
111		m..B	1416...5	19.625 20	1387...5	1.821 ...	2403.998	5
112		m..	-36.274 14	-1364...23	7.124 14	.253 17	12.855	89
113	P25A	m..T	690.03 9	37.198 12	677.61 3	2.249 24	1173.825	3



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Envelope Plate/Shell Principal Stresses (Continued)

Plate	S...	Sigma.....	Sigma.....	Tau M.....	Angle	Von Mises [psi]	LC
114	m..	-22.596	18-697.6...	3 9.301	89 -.554 ...	16.149	89
115	m..B	1384....	9 4.652 ...	1353....	9 2.174 89	2344.026	9
116	m..	8.18	89-1335...15	2.912 ...	-.74 61	9.092	127
117	P31 m..T	665.8...	13 8.389	90 657.7...	7 1.322 24	1139.355	7
118	m..	7.66	58-672.4...7	8.416	97 -.249 50	16.051	58
119	m..B	1294....	13 10.949	16 1268....	13 1.298 44	2197.565	13
120	m..	-30.344	22-1243...19	6.732 ...	-.269 14	11.977	125

APPENDIX D
ADDITIONAL CALCUATIONS

3" SQ. Tube to Tower Weld Check

Known/Assumptions:

$F_{exx} := 70 \text{ ksi}$	(assumed weld electrode str.)
$b := 3 \text{ in}$	(width of weld)
$d := 3 \text{ in}$	(depth of weld)
$a := 0.25 \text{ in}$	(assumed design weld size - <i>based off the tube thk.</i>)
$L_w := 2 \cdot b + 2 \cdot d = 12 \text{ in}$	(length of weld)
$\phi := 0.75$	(applicable reduction factor)

Joint Reactions per RISA 3D (Critical Joint Env. Solution):

$F_x := 10669.140 \text{ lbf}$	$M_x := -3083.498 \text{ lbf} \cdot \text{ft}$
$F_y := 3657.055 \text{ lbf}$	$M_y := 104.211 \text{ lbf} \cdot \text{ft}$
$F_z := 6066.388 \text{ lbf}$	$M_z := 5341.864 \text{ lbf} \cdot \text{ft}$

Weld Property Calculations per AISC 14th ED., Fig. 8-6:

$c_x := 0.5 \cdot b = 1.5 \text{ in}$	(radial distance from CG to point on weld remote from CG, x-x Direction)
$c_y := 0.5 \cdot d = 1.5 \text{ in}$	(radial distance from CG to point on weld remote from CG, y-y Direction)

$$S := b \cdot d + \frac{d^2}{3} = 12 \frac{1}{\text{in}} \text{ in}^3 \quad (\text{section modulus of weld})$$

$$I_p := \frac{(b+d)^3}{6} = 36 \frac{1}{\text{in}} \cdot \text{in}^4 \quad (\text{polar moment of inertia about CG of weld})$$

Weld Strength Calculations and Results (Eccentric Elastic Method per AISC 14th Ed., Part 8):

$$R_f := \frac{\sqrt{F_x^2 + F_y^2}}{L_w} = 939.875 \frac{\text{lbf}}{\text{in}}$$

$$R_{fz} := \frac{F_z}{L_w} = 505.532 \frac{\text{lbf}}{\text{in}}$$

$$R_{mx} := \frac{(M_z \cdot c_x)}{I_p} = (2.671 \cdot 10^3) \frac{\text{lbf}}{\text{in}}$$

$$R_{my} := \frac{(M_z \cdot c_y)}{I_p} = (2.671 \cdot 10^3) \frac{\text{lbf}}{\text{in}}$$

$$R_m := \sqrt{R_{mx}^2 + R_{my}^2}$$

$$R_{mz1} := \frac{M_x}{S} = -3.083 \cdot 10^3 \frac{\text{lbf}}{\text{in}}$$

$$R_{mz2} := \frac{M_y}{S} = 104.211 \frac{\text{lbf}}{\text{in}}$$

$$R_{mz} := \sqrt{R_{mz1}^2 + R_{mz2}^2} = (3.085 \cdot 10^3) \frac{\text{lbf}}{\text{in}}$$

(Demand)

$$R_u := \sqrt{(R_f + R_m)^2 + (R_{fz} + R_{mz})^2} = 5928.34 \frac{\text{lbf}}{\text{in}}$$

(Capacity of weld)

$$\phi R_{nw} := \phi \cdot 0.60 \cdot F_{exx} \cdot \frac{\sqrt{2}}{2} \cdot a = 5568.466 \frac{\text{lbf}}{\text{in}}$$

$$L_{stiffeners} := 6 \text{ in}$$

$$\therefore DCR := \frac{L_w \cdot R_u}{(L_{stiffeners} + L_w) \cdot \phi R_{nw}} = 71\%$$

APPENDIX E
MOUNT MODIFICATION DESIGN DRAWINGS (MDD)

GENERAL NOTES:

1. THESE DOCUMENTS WERE DESIGNED IN ACCORDANCE WITH THE LATEST VERSION OF APPLICABLE LOCAL/STATE/COUNTY/CITY BUILDING CODES, AS WELL AS ANSI/TIA-222 STANDARD, AWWA-D100 STANDARD, NDS, NEC, MSJC, AND/OR THE LATEST VERSION OF THE INTERNATIONAL BUILDING CODE, UNLESS NOTED OTHERWISE IN THE CORRESPONDING STRUCTURAL REPORT.
2. ALL CONSTRUCTION METHODS SHOULD FOLLOW STANDARDS OF GOOD CONSTRUCTION PRACTICE.
3. ALL WORK INDICATED ON THESE DRAWINGS SHALL BE PERFORMED BY QUALIFIED CONTRACTORS EXPERIENCED IN SIMILAR CONSTRUCTION.
4. ALL NEW WORK SHALL ACCOMMODATE EXISTING CONDITIONS. IF OBSTRUCTIONS ARE FOUND, CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD PRIOR TO CONTINUING WORK.
5. ANY CHANGES OR ADDITIONS MUST CONFORM TO THE REQUIREMENTS OF THESE NOTES AND SPECIFICATIONS, AND SHOULD BE SIMILAR TO THOSE SHOWN. ALL CHANGES OR ADDITIONS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO FABRICATION AND/OR CONSTRUCTION.
6. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND EXECUTION OF ALL MISCELLANEOUS SHORING, BRACING, TEMPORARY SUPPORTS, ETC. NECESSARY TO PROVIDE A COMPLETE AND STABLE STRUCTURE DURING CONSTRUCTION. TIA-1019-A-2011 IS AN APPROPRIATE REFERENCE FOR THOSE DESIGNS MEETING TIA STANDARDS. THE ENGINEER OF RECORD MAY PROVIDE FORMAL RIGGING PLANS AT THE REQUEST AND EXPENSE OF THE CONTRACTOR.
7. INSTALLATION SHALL NOT INTERFERE NOR DENY ADEQUATE ACCESS TO OR FROM ANY EXISTING OR PROPOSED OPERATIONAL AND SAFETY EQUIPMENT.
8. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS PRIOR TO ANY FABRICATION. CONTACT INFINIGY ENGINEERING IF ANY DISCREPANCIES EXIST.

STEEL CONSTRUCTION NOTES:

1. STRUCTURAL STEEL SHALL CONFORM TO THE AISC MANUAL OF STEEL CONSTRUCTION 14TH EDITION, FOR THE DESIGN AND FABRICATION OF STEEL COMPONENTS.
2. ALL FIELD CUT SURFACES, FIELD DRILLED HOLES, AND GROUND SURFACES WHERE EXISTING PAINT OR GALVANIZATION REMOVAL WAS REQUIRED SHALL BE REPAIRED WITH (2) BRUSHED COATS OF ZRC GALVALITE COLD GALVANIZING COMPOUND PER ASTM A780 AND MANUFACTURERS' RECOMMENDATIONS.
3. ALL FIELD DRILLED HOLES TO BE USED FOR FIELD BOLTING INSTALLATION SHALL BE STANDARD HOLES, AS DEFINED BY AISC, UNLESS NOTED OTHERWISE.
4. ALL EXTERIOR STEEL WORK SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123.
5. ALL STEEL MEMBERS AND CONNECTIONS SHALL MEET THE FOLLOWING GRADES:
 - ANGLES, CHANNELS, PLATES AND BARS TO BE A36. Fy=36 KSI, U.N.O.
 - W SHAPES TO BE A992. Fy=50 KSI, U.N.O.
 - RECTANGULAR HSS TO BE A500, GRADE B. Fy=46 KSI, U.N.O.
 - ROUND HSS TO BE A500, GRADE B. Fy=42 KSI, U.N.O.
 - STEEL PIPE TO BE A53, GRADE B. Fy=35 KSI, U.N.O.
 - BOLTS TO BE A325-X. Fu=120 KSI, U.N.O.
 - U-BOLTS AND LAG SCREWS TO BE A307 GR A. Fu=60 KSI, U.N.O.
6. ALL WELDING SHALL BE DONE USING E70XX ELECTRODES, U.N.O.
7. ALL WELDING SHALL CONFORM TO AISC AND AWS D1.1 LATEST EDITION.
8. ALL HILTI ANCHORS TO BE CARBON STEEL, U.N.O.
 - MECHANICAL ANCHORS: KWIK BOLT-TZ, U.N.O.
 - CMU BLOCK ANCHORS: ADHESIVE - HY120, U.N.O.
 - CONCRETE ANCHORS: ADHESIVE - HY150, U.N.O.
 - CONCRETE REBAR: ADHESIVE - RE500, U.N.O.
9. ALL STUDS TO BE NELSON CAPACITOR DISCHARGE 1/4"-20 LOW CARBON STEEL COPPER-FLASH AT 55 KSI ULT/50 KSI YIELD, U.N.O.
10. BOLTS SHALL BE TIGHTENED TO A "SNUG TIGHT" CONDITION AS DEFINED BY AISC.
11. MINIMUM EDGE DISTANCES SHALL CONFORM TO AISC TABLE J3.4.
12. REMOVAL/REPLACEMENT OF STRUCTURAL MEMBERS SHALL BE DONE ONE MEMBER AT A TIME. CONTRACTOR IS RESPONSIBLE FOR ENSURING THE STRUCTURAL INTEGRITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION.

CONCRETE CONSTRUCTION NOTES:

1. CONCRETE TO BE 4000 PSI @ 28 DAYS. REINFORCING BAR TO CONFORM TO ASTM A615 GRADE 60 SPECIFICATIONS. CONCRETE INSTALLATION TO CONFORM TO ACI-318 BUILDING REQUIREMENTS FOR REINFORCED CONCRETE. ALL CONCRETE TO BE PLACED AGAINST UNDISTURBED EARTH FREE OF WATER AND ALL FOREIGN OBJECTS AND MATERIALS. A MINIMUM OF THREE INCHES OF CONCRETE SHALL COVER ALL REINFORCEMENT. WELDING OF REBAR IS NOT PERMITTED.
2. EXISTING CONCRETE SURFACES THAT ARE TO BE IN CONTACT WITH NEW PROPOSED CONCRETE SHOULD BE WIRE BRUSHED CLEAN AND TREATED WITH APPROPRIATE MECHANICAL SCRATCH COAT AND REPAIR MATERIALS OR APPROPRIATE CHEMICAL METHODS SUCH AS THE APPLICATION OF A BONDING AGENT, EX. SAKRETE OR EQUIVALENT, TO ENSURE A QUALITY BOND BETWEEN EXISTING AND PROPOSED CONCRETE SURFACES.

FIBER REINFORCED POLYMER (FRP) NOTES:

1. FRP PLATES, SHAPES, BOLTS AND NUTS (STUD/NUT ASSEMBLIES) SHALL CONFORM TO ASTM D638, 695, 790. PLATES AND SHAPES TO BE FY = 5.35 KSI LW (SAFETY FACTOR OF 8), .945 KSI CW (SAFETY FACTOR OF 8) MIN.
2. IF FIELD FABRICATION IS REQUIRED, ALL CUT EDGES AND DRILLED HOLES TO BE SEALED USING VINYL ESTER SEALING KIT SUPPLIED BY THE MANUFACTURER.
3. ALL FASTENERS TO BE 1/2" DIA FRP THREADED ROD WITH FIBER REINFORCED THERMOPLASTIC NUT, SPACED AT 12 INCHES ON CENTER MAXIMUM, U.N.O., FOR PANELS AND AS DESIGNED FOR STRUCTURAL MEMBERS.
4. THE COLOR AND SURFACE PATTERN OF EXPOSED FRP PANELS SHALL MATCH THE EXTERIOR OF THE EXISTING BUILDING, U.N.O.
5. STUD/NUT ASSEMBLIES SHOULD BE LUBRICATED FOR INSTALLATION
6. ENSURE BEARING SURFACES OF THE NUTS ARE PARALLEL TO THE SURFACES BEING FASTENED.
7. TORQUE BOLTS ACCORDING TO THE FOLLOWING TABLE:

INSTALLATION TORQUE TABLE		
SIZE	ULTIMATE TORQUE STRENGTH	RECOMMENDED MAXIMUM INSTALLATION TORQUE
3/8-16 UNC	8 FT-LBS	4 FT-LBS
1/2-13 UNC	18 FT-LBS	8 FT-LBS
5/8-11 UNC	35 FT-LBS	16 FT-LBS
3/4-10 UNC	50 FT-LBS	24 FT-LBS
1-8 UNC	110 FT-LBS	50 FT-LBS

8. WHEN TIGHTENING FRP STUD/NUT ASSEMBLIES, WRENCHES MUST MAKE FULL CONTACT WITH ALL NUT EDGES. A STANDARD SIX POINT SOCKET IS RECOMMENDED.
9. STUD/NUT ASSEMBLIES SHOULD BE BONDED BY APPLYING BONDING AGENT TO ENTIRE NUT AND EXPOSED STUD.
10. ALL FRP MATERIALS TO BE PROVIDED BY FIBERGRATE COMPOSITE STRUCTURES, DALLAS TX, OR APPROVED EQUAL.
11. ALL FRP SHAPES TO BE DYNAFORM PULTRUDED STRUCTURAL SHAPES.
12. ALL FRP PLATES TO BE FIBERPLATE MOLDED FRP PLATE.
13. ALL FRP PANELS TO BE FIBERPLATE CLADDING PANEL.
14. EACH FRP PANEL TO BE IDENTIFIED WITH LARR#25536 AND FIBERGRATE COMPOSITE STRUCTURAL LABEL.
15. FRP MATERIAL TO BE CLASSIFIED AS CC1 OR BETTER, AND HAVE MAXIMUM FLAME SPREAD OF 50.
16. ALL DESIGN AND CONSTRUCTION TO BE COMPLETED IN ACCORDANCE WITH LOS ANGELES RESEARCH REPORT RR25536, DATED FEBRUARY 1, 2016.
17. SPECIAL INSPECTIONS MUST BE PROVIDED FOR ALL FRP INSTALLMENTS. SEE SPECIAL INSPECTION SECTION, THIS SHEET.

RATIO OF EDGE DISTANCE TO FRP FASTENER DIAMETER		
	RANGE	RECOMMENDED
EDGE DISTANCE - CL* BOLT TO END	2.0-4.0	3.0
EDGE DISTANCE - CL* BOLT TO SIDE	1.5-3.5	2.5
BOLT PITCH - CL* TO CL*	4.0-5.0	5.0

WOOD CONSTRUCTION NOTES:

1. ALL EXISTING WOOD SHAPES ARE ASSUMED TO BE DOUGLAS FIR-LARCH WITH A REFERENCE DESIGN BENDING VALUE OF 1000 PSI MIN.
2. ALL PROPOSED WOOD SHAPES ARE TO BE DOUGLAS FIR-LARCH WITH A REFERENCE DESIGN BENDING VALUE OF 1000 PSI MIN. U.N.O.
3. ALL EXISTING AND PROPOSED GLUED LAMINATED TIMBERS ARE TO BE 24F-1.8C DOUGLAS FIR BALANCED WITH A REFERENCE DESIGN BENDING VALUE OF 2400 PSI MIN. U.N.O.

MASONRY CONSTRUCTION NOTES:

1. ALL BRICK TO BE 1500 PSI MIN. REINFORCING BAR (IF APPLICABLE) TO CONFORM TO ASTM A615 GRADE 60 SPECIFICATIONS. ALL MORTAR TO BE 2000 PSI MIN.
 - FOR INTERIOR/ABOVE GRADE APPLICATIONS TYPE N MORTAR HAVING MINIMUM MODULUS OF RUPTURE OF 100 PSI SHALL BE USED. FOR EXTERIOR/BELOW GRADE APPLICATIONS TYPE M OR S MORTAR HAVING A MINIMUM MODULUS OF RUPTURE OF 133 PSI.
 - BRICK AND MORTAR INSTALLATION TO CONFORM TO MSJC BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES.
2. ALL CMU TO BE 1500 PSI MIN. REINFORCING BAR (IF APPLICABLE) TO CONFORM TO ASTM A615 GRADE 60 SPECIFICATIONS. ALL MORTAR TO BE 2000 PSI MIN.
 - FOR INTERIOR/ABOVE GRADE APPLICATIONS, TYPE N MORTAR HAVING MINIMUM MODULUS OF RUPTURE OF 64 PSI SHALL BE USED FOR UNGROUTED BLOCKS, AND 158 PSI FOR FULLY GROUTED BLOCKS.
 - FOR EXTERIOR/BELOW GRADE APPLICATIONS TYPE M OR S MORTAR HAVING A MINIMUM MODULUS OF RUPTURE OF 84 PSI SHALL BE USED FOR UNGROUTED BLOCKS, AND 163 PSI FOR FULLY GROUTED BLOCKS.
 - BRICK AND MORTAR INSTALLATION TO CONFORM TO MSJC BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES.

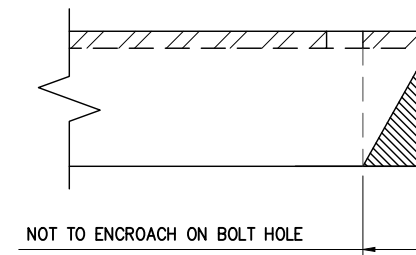
TOWER PLUMB & TENSION NOTES:

1. PLUMB AND TENSION TOWER UPON COMPLETION OF STRUCTURAL MODIFICATIONS DETAILED IN THESE DRAWINGS.
2. RETENSIONING OF EXISTING GUY WIRES SHALL BE PERFORMED AT A TIME WHEN THE WIND VELOCITY IS LESS THAN 10 MPH AT GROUND LEVEL AND WITH NO ICE ON THE STRUCTURE AND GUY WIRES.
3. PLUMB THE TOWER WHILE RETENSIONING THE EXISTING GUY WIRES. THE HORIZONTAL DISTANCE BETWEEN THE VERTICAL CENTERLINES AT ANY TWO ELEVATIONS SHALL NOT EXCEED 0.25% OF THE VERTICAL DISTANCE BETWEEN TWO ELEVATIONS FOR LATTICED STRUCTURES.
4. THE TWIST BETWEEN ANY TWO ELEVATIONS THROUGHOUT THE HEIGHT OF A LATTICE STRUCTURE SHALL NOT EXCEED 0.5 DEGREES IN 10 FEET. THE MAXIMUM TWIST OVER THE LATTICE STRUCTURE HEIGHT SHALL NOT EXCEED 5 DEGREES.

SPECIAL INSPECTIONS NOTES:

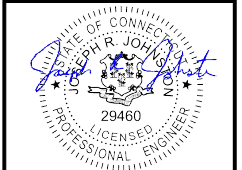
1. A QUALIFIED INDEPENDENT TESTING LABORATORY, EMPLOYED BY THE OWNER AND APPROVED BY THE JURISDICTION, SHALL PERFORM INSPECTION AND TESTING IN ACCORDANCE WITH THE THE GOVERNING BUILDING CODE, APPLICABLE SECTION(S) AS REQUIRED BY PROJECT SPECIFICATIONS FOR THE FOLLOWING CONSTRUCTION WORK:
 - a. STRUCTURAL WELDING (CONTINUOUS INSPECTION OF FIELD WELDS ONLY).
 - b. HIGH STRENGTH BOLTS (PERIODIC INSPECTION OF A325 AND/OR A490 BOLTS) TO BE TIGHTENED PER "TURN-OF-THE-NUT" METHOD.
 - c. MECHANICAL AND EPOXYED ANCHORAGES.
 - d. FIBER REINFORCED POLYMER.
 - THE SPECIAL INSPECTOR MUST VERIFY THAT THE FRP MATERIAL SPECIFIED ON THE APPROVED DESIGN DOCUMENTS IS BEING INSTALLED.
 - THE SPECIAL INSPECTOR MUST VERIFY THAT ALL CUT EDGES AND DRILLED HOLES ARE PROPERLY SEALED USING A VINYL ESTER SEALING KIT SUPPLIED BY THE MANUFACTURER.
 - THE SPECIAL INSPECTOR MUST VERIFY THAT THE STRUCTURE IS BUILT IN ACCORDANCE WITH THE APPROVED DESIGN DOCUMENTS.
2. THE INSPECTION AGENCY SHALL SUBMIT INSPECTION AND TEST REPORTS TO THE BUILDING DEPARTMENT, THE ENGINEER OF RECORD, AND THE OWNER UNLESS THE FABRICATOR IS APPROVED BY THE BUILDING OFFICIAL TO PERFORM WORK WITHOUT THE SPECIAL INSPECTIONS.

MAXIMUM ALLOWABLE ANGLE CLIP



INFINIGY ENGINEERING PLLC
 1033 WaterVliet Shaker Rd
 Albany, NY 12205
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 Fax # (518) 990-0793

CROWN CASTLE



07-23-2019

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0	FOR REVIEW	KD	07/23/19
No.	Submital / Revision	App'd	Date
Drawn:	BE	Date:	07/23/19
Designed:	MP	Date:	07/23/19
Checked:	KD	Date:	07/23/19

Project Number:
1039-B0002-B

Project Title:
876399
CT43XC804
(F) E. GRANBY
4Q2000 / GALASSO
60 SOUTH MAIN ST.
EAST GRANBY, CT 06026

Prepared For:

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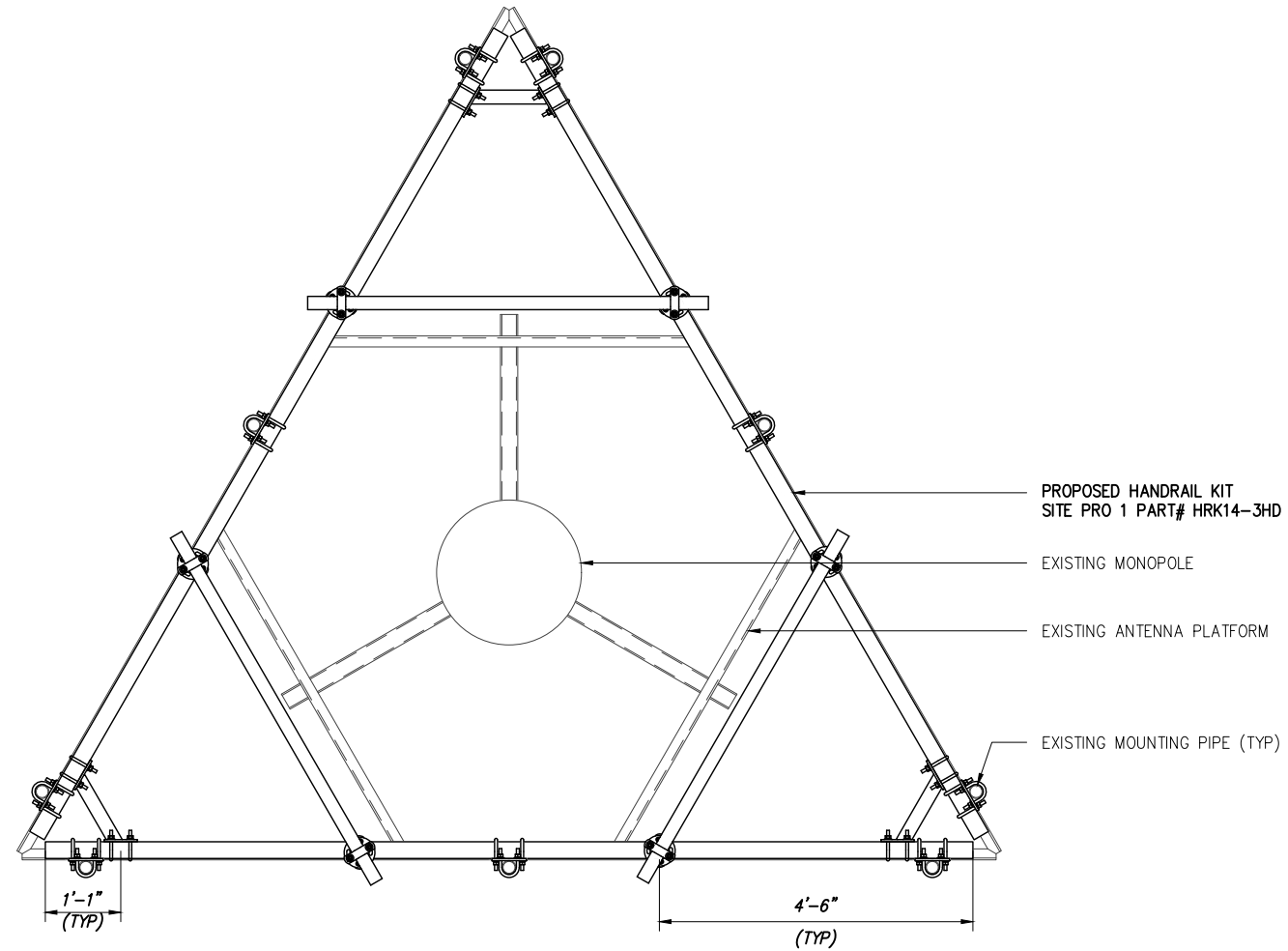
Drawing Scale:	AS NOTED
Date:	07/23/19

Drawing Title

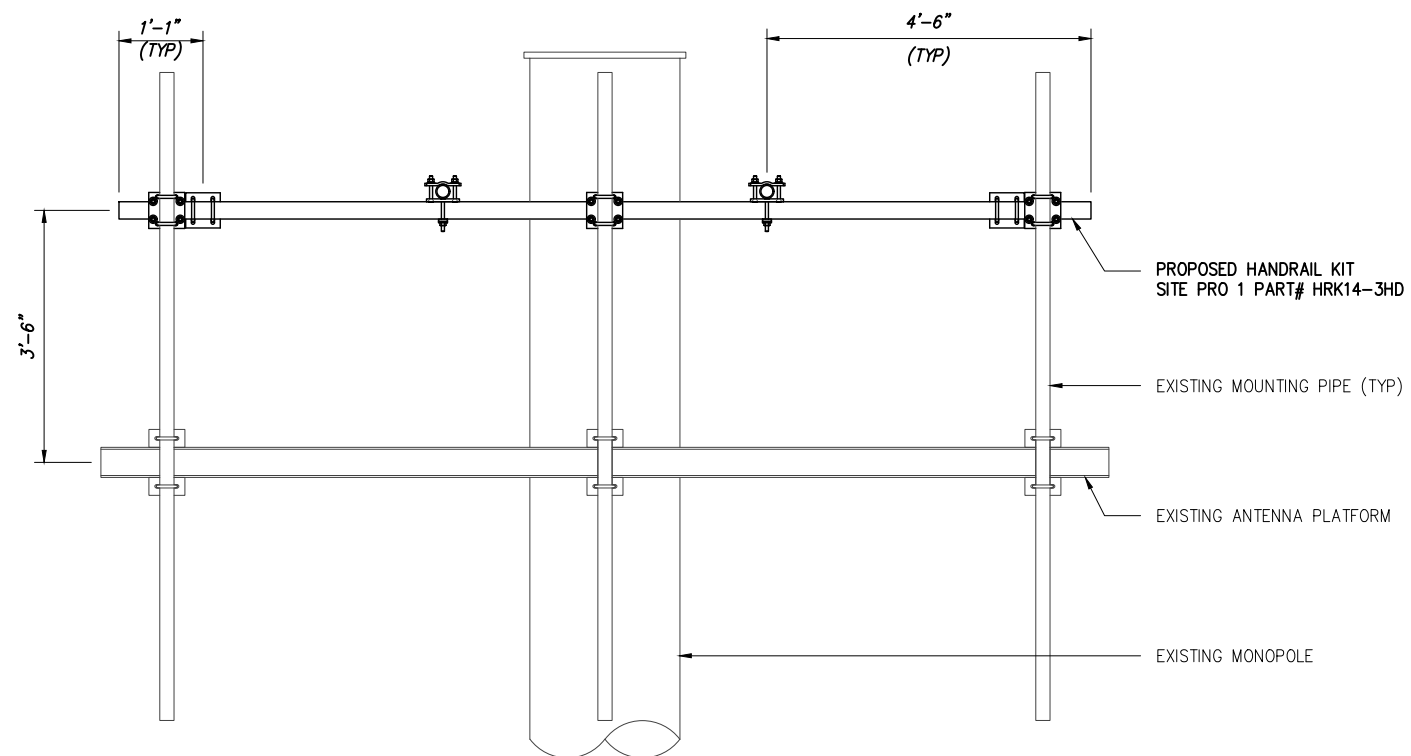
GENERAL NOTES

Drawing Number
S-1

NOTE: CONTRACTOR TO FIELD VERIFY AND CUT PROPOSED HRK14-3HD AS NECESSARY.



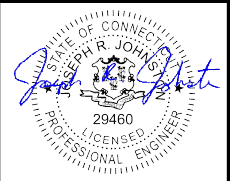
1 PLAN VIEW
SCALE: NOT TO SCALE



2 ELEVATION VIEW
SCALE: NOT TO SCALE

INFINIGY
ENGINEERING PLLC
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CROWN CASTLE



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Drawing Scale:
AS NOTED
Date:
07/23/19

Drawing Title
PLATFORM
MODIFICATION

Drawing Number
S-2

GENERAL NOTES:

1. THESE DOCUMENTS WERE DESIGNED IN ACCORDANCE WITH THE LATEST VERSION OF APPLICABLE LOCAL/STATE/COUNTY/CITY BUILDING CODES, AS WELL AS ANSI/TIA-222 STANDARD, AWWA-D100 STANDARD, NDS, NEC, MSJC, AND/OR THE LATEST VERSION OF THE INTERNATIONAL BUILDING CODE, UNLESS NOTED OTHERWISE IN THE CORRESPONDING STRUCTURAL REPORT.
2. ALL CONSTRUCTION METHODS SHOULD FOLLOW STANDARDS OF GOOD CONSTRUCTION PRACTICE.
3. ALL WORK INDICATED ON THESE DRAWINGS SHALL BE PERFORMED BY QUALIFIED CONTRACTORS EXPERIENCED IN SIMILAR CONSTRUCTION.
4. ALL NEW WORK SHALL ACCOMMODATE EXISTING CONDITIONS. IF OBSTRUCTIONS ARE FOUND, CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD PRIOR TO CONTINUING WORK.
5. ANY CHANGES OR ADDITIONS MUST CONFORM TO THE REQUIREMENTS OF THESE NOTES AND SPECIFICATIONS, AND SHOULD BE SIMILAR TO THOSE SHOWN. ALL CHANGES OR ADDITIONS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO FABRICATION AND/OR CONSTRUCTION.
6. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND EXECUTION OF ALL MISCELLANEOUS SHORING, BRACING, TEMPORARY SUPPORTS, ETC. NECESSARY TO PROVIDE A COMPLETE AND STABLE STRUCTURE DURING CONSTRUCTION. TIA-1019-A-2011 IS AN APPROPRIATE REFERENCE FOR THOSE DESIGNS MEETING TIA STANDARDS. THE ENGINEER OF RECORD MAY PROVIDE FORMAL RIGGING PLANS AT THE REQUEST AND EXPENSE OF THE CONTRACTOR.
7. INSTALLATION SHALL NOT INTERFERE NOR DENY ADEQUATE ACCESS TO OR FROM ANY EXISTING OR PROPOSED OPERATIONAL AND SAFETY EQUIPMENT.
8. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS PRIOR TO ANY FABRICATION. CONTACT INFINIGY ENGINEERING IF ANY DISCREPANCIES EXIST.

STEEL CONSTRUCTION NOTES:

1. STRUCTURAL STEEL SHALL CONFORM TO THE AISC MANUAL OF STEEL CONSTRUCTION 14TH EDITION, FOR THE DESIGN AND FABRICATION OF STEEL COMPONENTS.
2. ALL FIELD CUT SURFACES, FIELD DRILLED HOLES, AND GROUND SURFACES WHERE EXISTING PAINT OR GALVANIZATION REMOVAL WAS REQUIRED SHALL BE REPAIRED WITH (2) BRUSHED COATS OF ZRC GALVILITE COLD GALVANIZING COMPOUND PER ASTM A780 AND MANUFACTURERS' RECOMMENDATIONS.
3. ALL FIELD DRILLED HOLES TO BE USED FOR FIELD BOLTING INSTALLATION SHALL BE STANDARD HOLES, AS DEFINED BY AISC, UNLESS NOTED OTHERWISE.
4. ALL EXTERIOR STEEL WORK SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123.
5. ALL STEEL MEMBERS AND CONNECTIONS SHALL MEET THE FOLLOWING GRADES:
 - ANGLES, CHANNELS, PLATES AND BARS TO BE A36. Fy=36 KSI, U.N.O.
 - W SHAPES TO BE A992. Fy=50 KSI, U.N.O.
 - RECTANGULAR HSS TO BE A500, GRADE B. Fy=46 KSI, U.N.O.
 - ROUND HSS TO BE A500, GRADE B. Fy=42 KSI, U.N.O.
 - STEEL PIPE TO BE A53, GRADE B. Fy=35 KSI, U.N.O.
 - BOLTS TO BE A325-X. Fu=120 KSI, U.N.O.
 - U-BOLTS AND LAG SCREWS TO BE A307 GR A. Fu=60 KSI, U.N.O.
6. ALL WELDING SHALL BE DONE USING E70XX ELECTRODES, U.N.O.
7. ALL WELDING SHALL CONFORM TO AISC AND AWS D1.1 LATEST EDITION.
8. ALL HILTI ANCHORS TO BE CARBON STEEL, U.N.O.
 - MECHANICAL ANCHORS: KWIK BOLT-TZ, U.N.O.
 - CMU BLOCK ANCHORS: ADHESIVE - HY120, U.N.O.
 - CONCRETE ANCHORS: ADHESIVE - HY150, U.N.O.
 - CONCRETE REBAR: ADHESIVE - RE500, U.N.O.
9. ALL STUDS TO BE NELSON CAPACITOR DISCHARGE 1/4"-20 LOW CARBON STEEL COPPER-FLASH AT 55 KSI ULT/50 KSI YIELD, U.N.O.
10. BOLTS SHALL BE TIGHTENED TO A "SNUG TIGHT" CONDITION AS DEFINED BY AISC.
11. MINIMUM EDGE DISTANCES SHALL CONFORM TO AISC TABLE J3.4.
12. REMOVAL/REPLACEMENT OF STRUCTURAL MEMBERS SHALL BE DONE ONE MEMBER AT A TIME. CONTRACTOR IS RESPONSIBLE FOR ENSURING THE STRUCTURAL INTEGRITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION.

CONCRETE CONSTRUCTION NOTES:

1. CONCRETE TO BE 4000 PSI @ 28 DAYS. REINFORCING BAR TO CONFORM TO ASTM A615 GRADE 60 SPECIFICATIONS. CONCRETE INSTALLATION TO CONFORM TO ACI-318 BUILDING REQUIREMENTS FOR REINFORCED CONCRETE. ALL CONCRETE TO BE PLACED AGAINST UNDISTURBED EARTH FREE OF WATER AND ALL FOREIGN OBJECTS AND MATERIALS. A MINIMUM OF THREE INCHES OF CONCRETE SHALL COVER ALL REINFORCEMENT. WELDING OF REBAR IS NOT PERMITTED.
2. EXISTING CONCRETE SURFACES THAT ARE TO BE IN CONTACT WITH NEW PROPOSED CONCRETE SHOULD BE WIRE BRUSHED CLEAN AND TREATED WITH APPROPRIATE MECHANICAL SCRATCH COAT AND REPAIR MATERIALS OR APPROPRIATE CHEMICAL METHODS SUCH AS THE APPLICATION OF A BONDING AGENT, EX. SAKRETE OR EQUIVALENT, TO ENSURE A QUALITY BOND BETWEEN EXISTING AND PROPOSED CONCRETE SURFACES.

FIBER REINFORCED POLYMER (FRP) NOTES:

1. FRP PLATES, SHAPES, BOLTS AND NUTS (STUD/NUT ASSEMBLIES) SHALL CONFORM TO ASTM D638, 695, 790. PLATES AND SHAPES TO BE FY = 5.35 KSI LW (SAFETY FACTOR OF 8), .945 KSI CW (SAFETY FACTOR OF 8) MIN.
2. IF FIELD FABRICATION IS REQUIRED, ALL CUT EDGES AND DRILLED HOLES TO BE SEALED USING VINYL ESTER SEALING KIT SUPPLIED BY THE MANUFACTURER.
3. ALL FASTENERS TO BE 1/2" DIA FRP THREADED ROD WITH FIBER REINFORCED THERMOPLASTIC NUT, SPACED AT 12 INCHES ON CENTER MAXIMUM, U.N.O., FOR PANELS AND AS DESIGNED FOR STRUCTURAL MEMBERS.
4. THE COLOR AND SURFACE PATTERN OF EXPOSED FRP PANELS SHALL MATCH THE EXTERIOR OF THE EXISTING BUILDING, U.N.O.
5. STUD/NUT ASSEMBLIES SHOULD BE LUBRICATED FOR INSTALLATION
6. ENSURE BEARING SURFACES OF THE NUTS ARE PARALLEL TO THE SURFACES BEING FASTENED.
7. TORQUE BOLTS ACCORDING TO THE FOLLOWING TABLE:

INSTALLATION TORQUE TABLE		
SIZE	ULTIMATE TORQUE STRENGTH	RECOMMENDED MAXIMUM INSTALLATION TORQUE
3/8-16 UNC	8 FT-LBS	4 FT-LBS
1/2-13 UNC	18 FT-LBS	8 FT-LBS
5/8-11 UNC	35 FT-LBS	16 FT-LBS
3/4-10 UNC	50 FT-LBS	24 FT-LBS
1-8 UNC	110 FT-LBS	50 FT-LBS

8. WHEN TIGHTENING FRP STUD/NUT ASSEMBLIES, WRENCHES MUST MAKE FULL CONTACT WITH ALL NUT EDGES. A STANDARD SIX POINT SOCKET IS RECOMMENDED.
9. STUD/NUT ASSEMBLIES SHOULD BE BONDED BY APPLYING BONDING AGENT TO ENTIRE NUT AND EXPOSED STUD.
10. ALL FRP MATERIALS TO BE PROVIDED BY FIBERGRATE COMPOSITE STRUCTURES, DALLAS TX, OR APPROVED EQUAL.
11. ALL FRP SHAPES TO BE DYNAFORM PULTRUDED STRUCTURAL SHAPES.
12. ALL FRP PLATES TO BE FIBERPLATE MOLDED FRP PLATE.
13. ALL FRP PANELS TO BE FIBERPLATE CLADDING PANEL.
14. EACH FRP PANEL TO BE IDENTIFIED WITH LARR#25536 AND FIBERGRATE COMPOSITE STRUCTURAL LABEL.
15. FRP MATERIAL TO BE CLASSIFIED AS CC1 OR BETTER, AND HAVE MAXIMUM FLAME SPREAD OF 50.
16. ALL DESIGN AND CONSTRUCTION TO BE COMPLETED IN ACCORDANCE WITH LOS ANGELES RESEARCH REPORT RR25536, DATED FEBRUARY 1, 2016.
17. SPECIAL INSPECTIONS MUST BE PROVIDED FOR ALL FRP INSTALLMENTS. SEE SPECIAL INSPECTION SECTION, THIS SHEET.

RATIO OF EDGE DISTANCE TO FRP FASTENER DIAMETER		
	RANGE	RECOMMENDED
EDGE DISTANCE - CL* BOLT TO END	2.0-4.0	3.0
EDGE DISTANCE - CL* BOLT TO SIDE	1.5-3.5	2.5
BOLT PITCH - CL* TO CL*	4.0-5.0	5.0

WOOD CONSTRUCTION NOTES:

1. ALL EXISTING WOOD SHAPES ARE ASSUMED TO BE DOUGLAS FIR-LARCH WITH A REFERENCE DESIGN BENDING VALUE OF 1000 PSI MIN.
2. ALL PROPOSED WOOD SHAPES ARE TO BE DOUGLAS FIR-LARCH WITH A REFERENCE DESIGN BENDING VALUE OF 1000 PSI MIN. U.N.O.
3. ALL EXISTING AND PROPOSED GLUED LAMINATED TIMBERS ARE TO BE 24F-1.8C DOUGLAS FIR BALANCED WITH A REFERENCE DESIGN BENDING VALUE OF 2400 PSI MIN. U.N.O.

MASONRY CONSTRUCTION NOTES:

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 - FOR INTERIOR/ABOVE GRADE APPLICATIONS TYPE N MORTAR HAVING MINIMUM MODULUS OF RUPTURE OF 100 PSI SHALL BE USED. FOR EXTERIOR/BELOW GRADE APPLICATIONS TYPE M OR S MORTAR HAVING A MINIMUM MODULUS OF RUPTURE OF 133 PSI.
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 - BRICK AND MORTAR INSTALLATION TO CONFORM TO MSJC BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES.

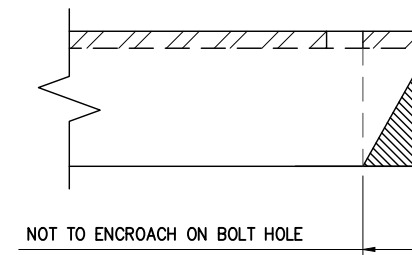
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3. PLUMB THE TOWER WHILE RETENSIONING THE EXISTING GUY WIRES. THE HORIZONTAL DISTANCE BETWEEN THE VERTICAL CENTERLINES AT ANY TWO ELEVATIONS SHALL NOT EXCEED 0.25% OF THE VERTICAL DISTANCE BETWEEN TWO ELEVATIONS FOR LATTICED STRUCTURES.
4. THE TWIST BETWEEN ANY TWO ELEVATIONS THROUGHOUT THE HEIGHT OF A LATTICE STRUCTURE SHALL NOT EXCEED 0.5 DEGREES IN 10 FEET. THE MAXIMUM TWIST OVER THE LATTICE STRUCTURE HEIGHT SHALL NOT EXCEED 5 DEGREES.

SPECIAL INSPECTIONS NOTES:

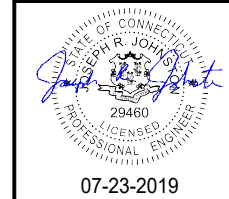
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 - a. STRUCTURAL WELDING (CONTINUOUS INSPECTION OF FIELD WELDS ONLY).
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MAXIMUM ALLOWABLE ANGLE CLIP



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CROWN CASTLE



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 60 SOUTH MAIN ST.
 EAST GRANBY, CT 06026

Prepared For:

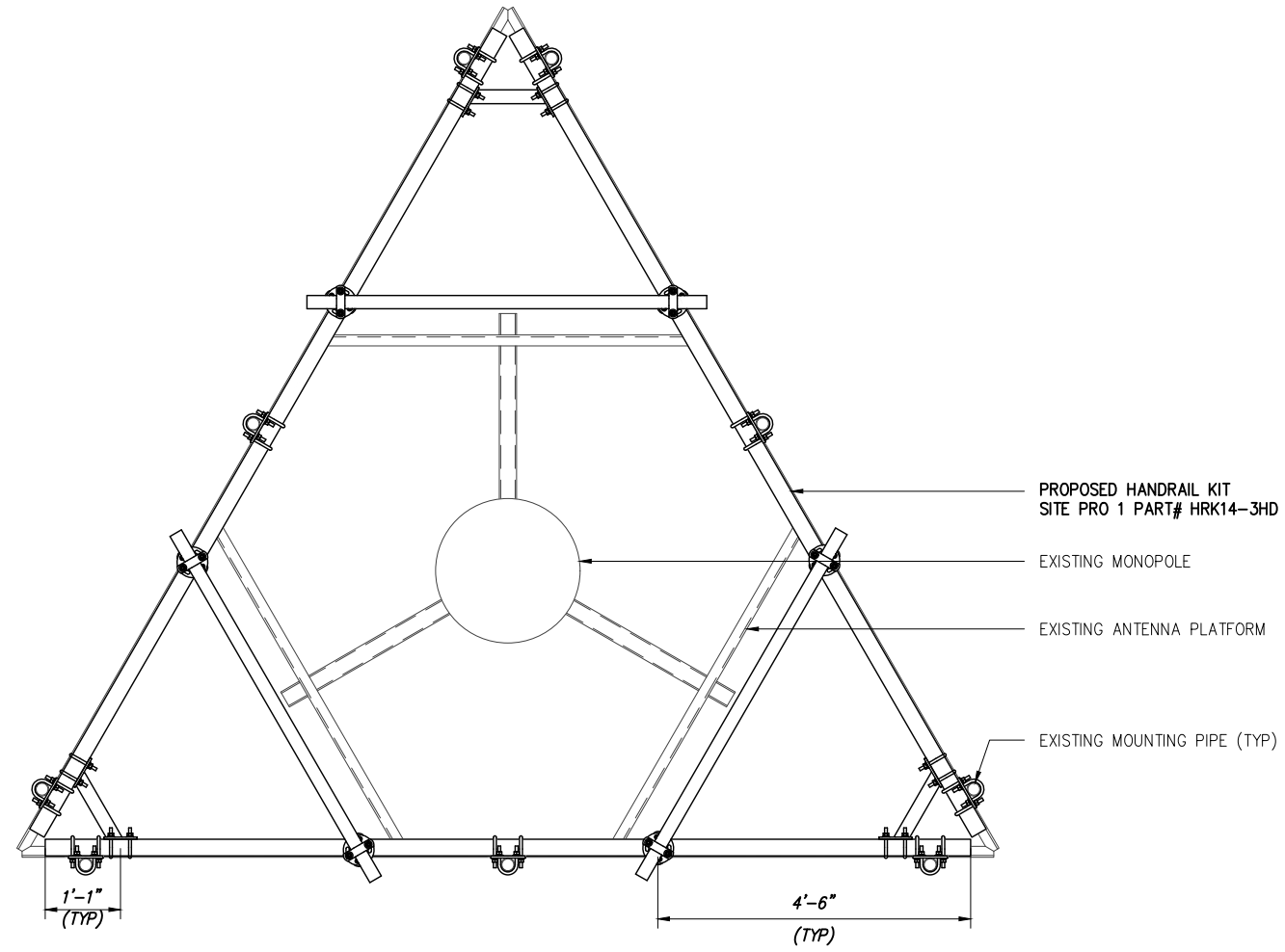
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Drawing Scale:
AS NOTED
 Date:
07/23/19

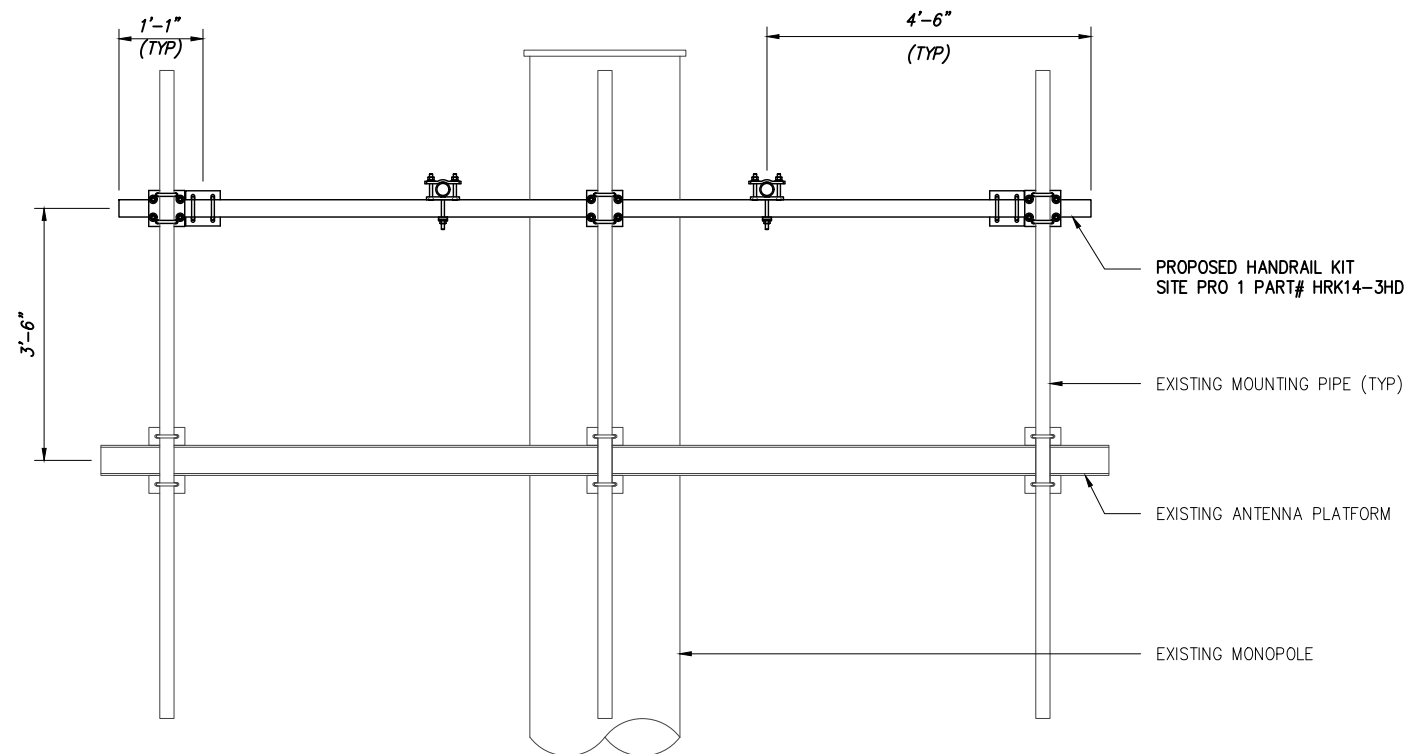
Drawing Title
GENERAL NOTES

Drawing Number
S-1

NOTE: CONTRACTOR TO FIELD VERIFY AND CUT PROPOSED HRK14-3HD AS NECESSARY.



1 PLAN VIEW
SCALE: NOT TO SCALE



2 ELEVATION VIEW
SCALE: NOT TO SCALE

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CROWN CASTLE

STATE OF CONNECTICUT
STEPHEN R. JOHNSON
29460
LICENSED PROFESSIONAL ENGINEER
07-23-2019

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sprint
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Drawing Scale:
AS NOTED
Date:
07/23/19

Drawing Title
PLATFORM
MODIFICATION

Drawing Number
S-2



Date: **July 29, 2019**

Charles Trask
Crown Castle
3530 Toringdon Way Suite 300
Charlotte, NC 28277

B+T Group
1717 S, Boulder, Suite 300
Tulsa, OK 74119
(918) 587-4630

Subject: **Structural Analysis Report**

Carrier Designation: **Sprint PCS Co-Locate**
Carrier Site Number: CT43XC804
Carrier Site Name: CT43XC804

Crown Castle Designation: **Crown Castle BU Number:** 876399
Crown Castle Site Name: (F) E. Granby 4Q2000 / Galasso
Crown Castle JDE Job Number: 447217
Crown Castle Work Order Number: 1773228
Crown Castle Order Number: 397084 Rev. 4

Engineering Firm Designation: **B+T Group Project Number:** 127643.006.01

Site Data: **60 South Main St., East Granby, Hartford County, CT**
Latitude 41° 56' 29.59", Longitude -72° 44' 19.248"
98 Foot - Monopole Tower

Dear Charles Trask,

B+T Group is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above-mentioned tower.

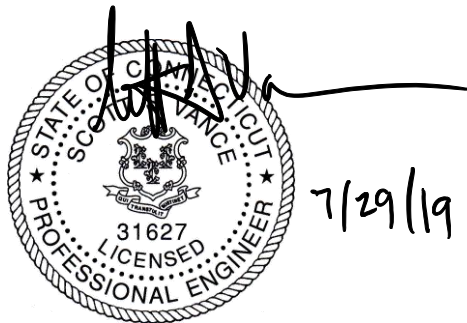
The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC4.7: Proposed Equipment Configuration with Proposed Modifications **Sufficient Capacity - 99.3%**

This analysis utilizes an ultimate 3-second gust wind speed of 120 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: Jacob Johnson

Respectfully submitted by: B+T Engineering, Inc.
COA: PEC.0001564; Expires: 02/10/2020



Scott S. Vance, P.E.

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1) INTRODUCTION

This is 98 ft. monopole designed by Engineered Endeavors, Inc. in September 2000. The monopole was originally designed for a wind speed of 85 mph per TIA/EIA-222-F. This tower has been modified multiple time and are incorporated in this analysis. Modifications designed by B+T Group, dated May 2019 were incorporated in this analysis and must be properly installed for the provided capacity to be effective.

2) ANALYSIS CRITERIA

TIA-222 Revision:	TIA-222-H
Risk Category:	II
Wind Speed:	120 mph
Exposure Category:	C
Topographic Factor:	1
Ice Thickness:	1 in
Wind Speed with Ice:	50 mph
Service Wind Speed:	60 mph

Table 1 - Proposed Equipment Configuration

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
94.0	97.0	3	Alcatel Lucent	PCS 1900MHz 4x45W-65MHz	3 1	1-1/4 7/8
		6	Alcatel Lucent	RRH2X50-800		
		3	Alcatel Lucent	TD-RRH8x20-25		
		3	Commscope	NNVV-65B-R4		
		3	RFS Celwave	APXVTM14-ALU-I20		
	94.0	1	Site Pro 1	HRK14-3HD Handrail Kit		
		1	--	Platform Mount [LP 714-1]		

Table 2 - Other Considered Equipment

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
89.0	90.0	3	Ericsson	RADIO 4449 B12/B71	1 11	1-3/8 7/8
		3	RFS Celwave	APXV18-209014-C		
		3	RFS Celwave	APXVAARR24_43-U-NA20		
		3	RFS Celwave	ATMPP1412D-1CWA		
	89.0	1	--	Platform Mount [LP 305-1]		
74.0	77.0	1	Andrew	SBNH-1D6565C	12 4 2	7/8 3/4 3/8
		3	CCI Antennas	TPA-65R-LCUUUU-H8		
		3	Ericsson	RRUS 11 B12		
		3	Ericsson	RRUS 32 B2		
		3	Ericsson	RRUS 32 B30		
		3	Kaelus	DBC0061F1V51-2		
		3	Powerwave Tech	7770.00		
		2	Powerwave Tech	P65-17-XLH-RR		
		3	Powerwave Tech	TT19-08BP111-001		
	74.0	2	Raycap	DC6-48-60-18-8F		
		1	--	Miscellaneous [NA 507-1]		
	74.0	1	--	Platform Mount [LP 303-1]		

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
67.0	67.0	3	Alcatel Lucent	B13 RRH 4X30	12 2	1-5/8 1-3/8
		3	Alcatel Lucent	B66A RRH4X45		
		6	Antel	LPA-80063/6CFX2		
		2	Commscope	RC2DC-3315-PF-48		
		6	Commscope	SBNHH-1D65B		
		1	--	Miscellaneous [NA 509-3]		
		1	--	Platform Mount [LP 303-1]		
52.0	54.0	1	Lucent	KS24019-L112A	1	7/8
	52.0	1	--	Side Arm Mount [SO 701-1]		

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Remarks	Reference	Source
Online Order Information	Sprint PCS Co-Locate, Rev. 4	397084	CCI Sites
Tower Manufacturer Drawing	EEI, Job No. 7832-E01	1613691	CCI Sites
Tower Modification Drawing	IETS, Project No. 2009-70644	2529017	CCI Sites
Post Modification Inspection	IETS, Project No. 2010-70158	2682749	CCI Sites
Tower Modification Drawing	PJF, Project No. 32912-0138 MO	3713021	CCI Sites
Post Modification Inspection		3713020	CCI Sites
Tower Modification Drawing	GPD, Project No. 2015777.876399.01	5803194	CCI Sites
Post Modification Inspection	ETS, Project No. 160019	6139057	CCI Sites
Tower Modification Drawing	B+T Group, Project No. 127643.001.01	8420875	CCI Sites
Foundation Drawings	EEI, Job No. 7832-E01	2066334	CCI Sites
Geotech Report	Delta Oaks, Project No. GEO18-03201-01	1531971	CCI Sites
Antenna Configuration	Crown CAD Package	Date: 12/19/2018	CCI Sites

3.1) Analysis Method

tnxTower (version 8.0.5.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) The tower and structures were built and have been maintained in accordance with the manufacturer's specification.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 3) Mount areas and weights are assumed based on photographs provided.
- 4) The existing base plate grout was not considered in this analysis.

This analysis may be affected if any assumptions are not valid or have been made in error. B+T Group should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	98 - 93	Pole	TP13.078x12x0.1875	1	-3.371	471.228	9.5	Pass
L2	93 - 88	Pole	TP14.156x13.078x0.1875	2	-5.746	510.643	25.2	Pass
L3	88 - 85.21	Pole	TP15.28x14.156x0.1875	3	-5.885	532.661	34.8	Pass
L4	85.21 - 80.21	Pole	TP15.445x14.384x0.25	4	-6.282	740.636	38.8	Pass
L5	80.21 - 75.21	Pole	TP16.507x15.445x0.25	5	-6.908	792.382	47.1	Pass
L6	75.21 - 70.21	Pole	TP17.569x16.507x0.25	6	-10.171	844.128	60.9	Pass
L7	70.21 - 65.21	Pole	TP18.63x17.569x0.25	7	-13.456	895.874	72.4	Pass
L8	65.21 - 60.21	Pole	TP19.692x18.63x0.25	8	-14.139	947.619	84.4	Pass
L9	60.21 - 59.17	Pole	TP19.912x19.692x0.25	9	-14.285	958.351	86.5	Pass
L10	59.17 - 58.9	Pole + Reinf.	TP19.97x19.912x0.5125	10	-14.354	1944.117	78.4	Pass
L11	58.9 - 58.75	Pole + Reinf.	TP20.001x19.97x0.5125	11	-14.382	1947.298	78.8	Pass
L12	58.75 - 54	Pole + Reinf.	TP21.01x20.001x0.5	12	-15.202	1999.347	88.7	Pass
L13	54 - 53.75	Pole + Reinf.	TP21.063x21.01x0.5125	13	-15.267	2053.380	79.6	Pass
L14	53.75 - 52.91	Pole + Reinf.	TP21.241x21.063x0.5	14	-15.432	2021.901	81.1	Pass
L15	52.91 - 52.66	Pole + Reinf.	TP21.294x21.241x0.675	15	-15.496	2713.525	78.3	Pass
L16	52.66 - 52.17	Pole + Reinf.	TP21.399x21.294x0.675	16	-15.601	2727.217	79.2	Pass
L17	52.17 - 51.92	Pole + Reinf.	TP21.452x21.399x0.525	17	-15.734	2141.958	84.7	Pass
L18	51.92 - 48.7	Pole + Reinf.	TP22.86x21.452x0.5125	18	-16.395	2160.438	90.1	Pass
L19	48.7 - 44.29	Pole + Reinf.	TP22.575x21.634x0.5625	19	-17.852	2413.981	87.4	Pass
L20	44.29 - 39.29	Pole + Reinf.	TP23.639x22.575x0.55	20	-19.017	2475.784	93.2	Pass
L21	39.29 - 34.29	Pole + Reinf.	TP24.703x23.639x0.5375	21	-20.215	2532.348	98.3	Pass
L22	34.29 - 33.5	Pole + Reinf.	TP24.87x24.703x0.525	22	-20.410	2491.881	99.1	Pass
L23	33.5 - 33.25	Pole + Reinf.	TP24.923x24.87x0.8375	23	-20.498	3932.796	67.1	Pass
L24	33.25 - 33	Pole + Reinf.	TP24.977x24.923x0.8375	24	-20.575	3941.490	67.3	Pass
L25	33 - 32.75	Pole + Reinf.	TP25.03x24.977x0.8125	25	-20.645	3836.217	73.0	Pass
L26	32.75 - 32	Pole + Reinf.	TP25.19x25.03x0.8	26	-20.852	3804.045	73.6	Pass
L27	32 - 31.75	Pole + Reinf.	TP25.243x25.19x0.5875	27	-20.916	2824.027	88.8	Pass
L28	31.75 - 28.5	Pole + Reinf.	TP25.934x25.243x0.575	28	-21.678	2842.885	91.4	Pass
L29	28.5 - 28.25	Pole + Reinf.	TP25.988x25.934x0.8625	29	-21.769	4224.927	63.7	Pass
L30	28.25 - 27.5	Pole + Reinf.	TP26.147x25.988x0.85	30	-21.992	4192.219	64.1	Pass
L31	27.5 - 27.25	Pole + Reinf.	TP26.2x26.147x0.575	31	-22.057	2872.705	92.4	Pass
L32	27.25 - 22.25	Pole + Reinf.	TP27.265x26.2x0.5625	32	-23.271	2928.334	95.8	Pass
L33	22.25 - 18	Pole + Reinf.	TP28.169x27.265x0.55	33	-24.333	2961.588	98.5	Pass
L34	18 - 17.75	Pole + Reinf.	TP28.222x28.169x0.5625	34	-24.416	3033.355	90.4	Pass
L35	17.75 - 15.45	Pole + Reinf.	TP28.712x28.222x0.425	35	-25.003	2343.820	91.2	Pass
L36	15.45 - 15.2	Pole + Reinf.	TP28.765x28.712x0.6875	36	-25.099	3763.431	89.7	Pass
L37	15.2 - 13.41	Pole + Reinf.	TP29.146x28.765x0.675	37	-25.616	3746.778	90.7	Pass
L38	13.41 - 13.16	Pole + Reinf.	TP29.199x29.146x0.5625	38	-25.706	3140.487	94.5	Pass
L39	13.16 - 8.16	Pole + Reinf.	TP30.263x29.199x0.55	39	-27.115	3186.151	97.0	Pass
L40	8.16 - 6.5	Pole + Reinf.	TP30.617x30.263x0.55	40	-27.583	3224.035	97.7	Pass
L41	6.5 - 6.25	Pole + Reinf.	TP30.67x30.617x0.6625	41	-27.683	3875.833	94.1	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L42	6.25 - 4.45	Pole + Reinf.	TP31.053x30.67x0.65	42	-28.220	3852.838	94.9	Pass
L43	4.45 - 4.2	Pole + Reinf.	TP31.106x31.053x0.5125	43	-28.308	3056.865	96.2	Pass
L44	4.2 - 0	Pole + Reinf.	TP32x31.106x0.5	44	-29.435	3070.662	97.8	Pass
							Summary	
						Pole (L9)	91.2	Pass
						Reinforcement	99.1	Pass
						Rating =	99.1	Pass

Table 5 - Tower Component Stresses vs. Capacity - LC4.7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	Base	64.8	Pass
1	Base Plate	Base	99.3	Pass
1	Base Foundation (Structure)	Base	27.6	Pass
1	Base Foundation (Soil Interaction)	Base	40.2	Pass
1	Base Foundation (Micropile Structure)	Base	49.0	Pass
1	Base Foundation (Micropile)	Base	86.9	Pass

Structure Rating (max from all components) =	99.3%
---	--------------

Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.
- 2) Rating per TIA-222-H Section 15.5.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration provided the modification drawings designed by B+T Group (Doc. ID # 8420875) are installed and the following changes are considered.

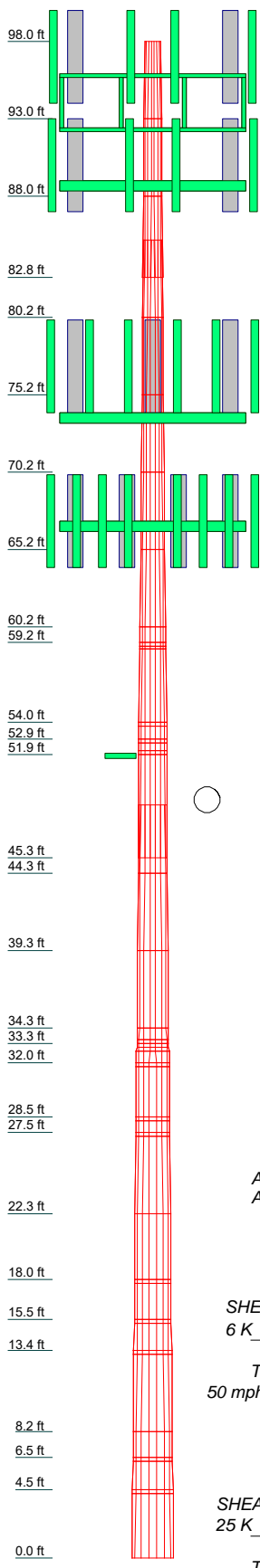
Loading Changes:

The proposed RRH2X50-800 and PCS 1900MHz 4x45W-65MHz TMEs must be installed behind the antenna such that front areas of these TME's are shielded.

APPENDIX A

TNXTOWER OUTPUT

Section	1	2	3	4	5	6	7	8	12	18	19	20	21	28	32	33	34	39	44	
Length (ft)	5.000	5.000	5.210	5.000	5.000	5.000	5.000	5.000	4.750	4.750	4.750	5.000	5.000	5.000	5.000	5.000	5.000	5.000	5.000	5.000
Number of Sides	18	18	18	18	18	18	18	18	18	18	18	18	18	18	18	18	18	18	18	18
Thickness (in)	0.188	0.188	0.188	0.250	0.250	0.250	0.250	0.250	0.512	0.512	0.563	0.550	0.537	0.576	0.563	0.550	0.550	0.550	0.550	0.550
Socket Length (ft)	2.417										3.417									
Top Dia (in)	12.000	13.078	14.156	14.384	15.445	16.507	17.569	18.630	19.692	20.754	21.816	22.878	23.940	25.002	26.064	27.126	28.188	29.250	30.312	31.374
Bot Dia (in)	13.078	14.156	15.234	16.312	17.390	18.468	19.546	20.624	21.702	22.780	23.858	24.936	26.014	27.092	28.170	29.248	30.326	31.404	32.482	33.560
Grade	A572-65																			
Weight (K)	0.1	0.1	0.2	0.2	0.2	0.2	0.2	0.3	0.5	0.8	1.1	1.4	1.7	2.1	2.5	2.9	3.3	3.7	4.1	4.5

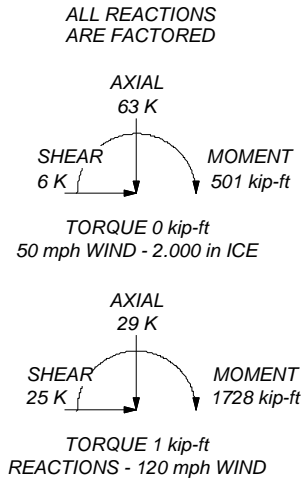



MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower is located in Hartford County, Connecticut.
2. Tower designed for Exposure C to the TIA-222-H Standard.
3. Tower designed for a 120 mph basic wind in accordance with the TIA-222-H Standard.
4. Tower is also designed for a 50 mph basic wind with 2.00 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Risk Category II.
7. Topographic Category 1 with Crest Height of 0.000 ft
8. TIA-222-H Annex S
9. TOWER RATING: 99.1%



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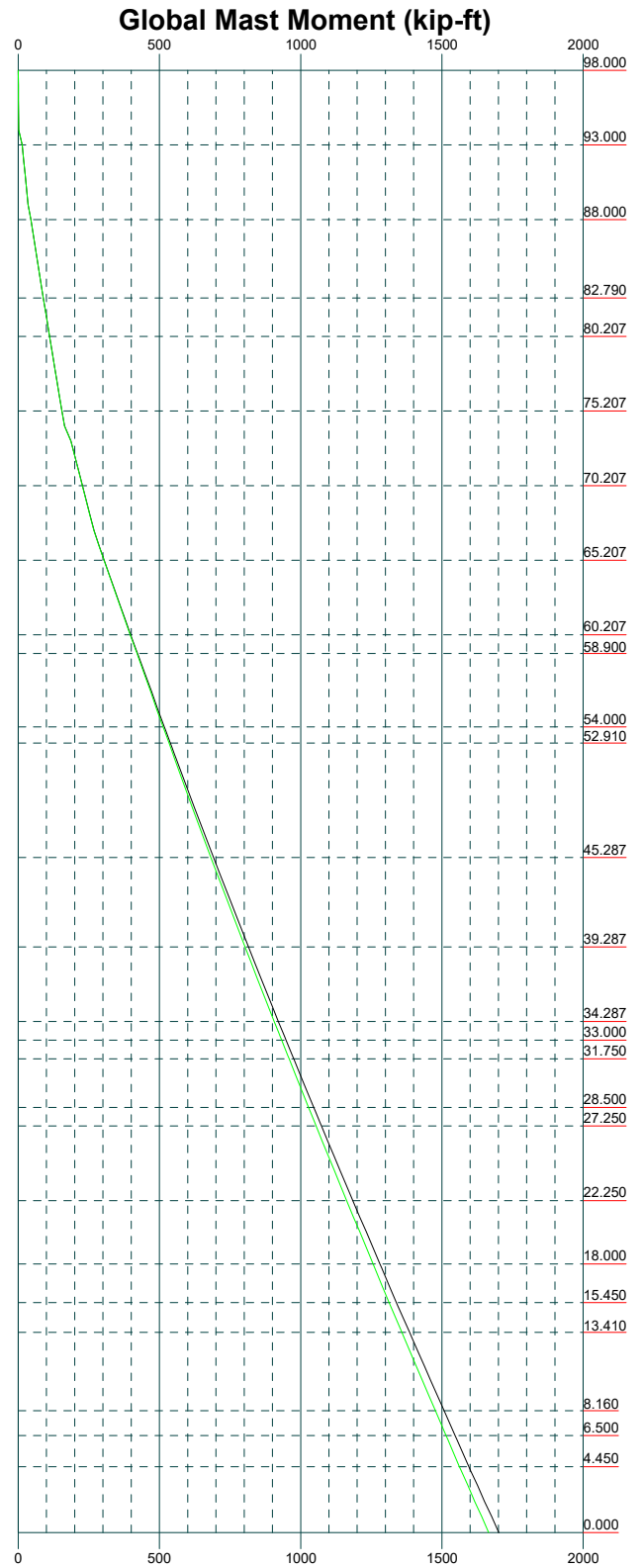
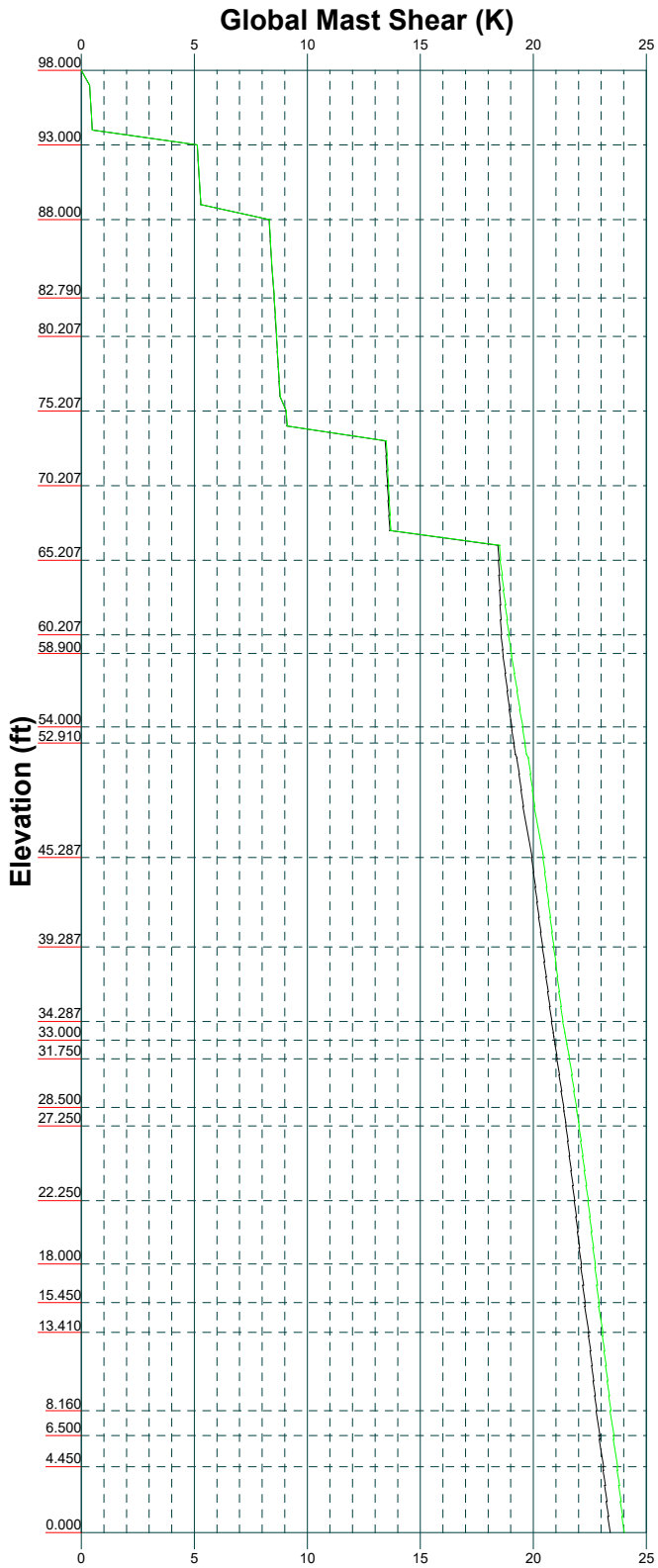
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Project:		
Client: Crown Castle	Drawn by: JD Prabhu	App'd:
Code: TIA-222-H	Date: 07/27/19	Scale: NTS
Path:	Dwg No. E-1	

Vx

Vz

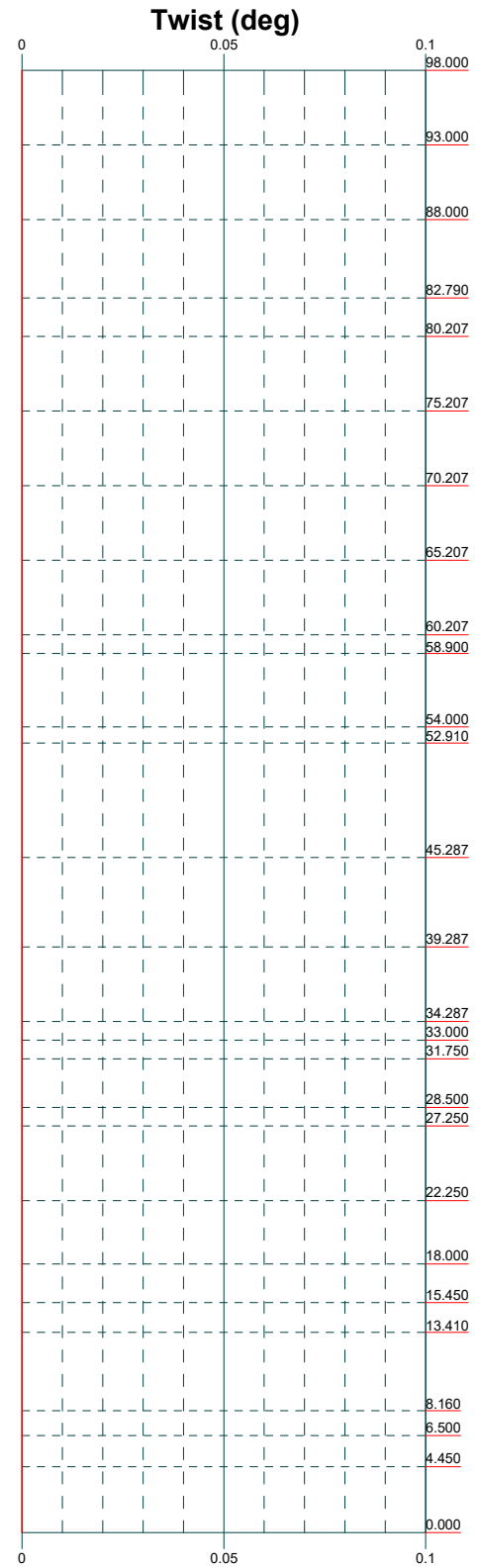
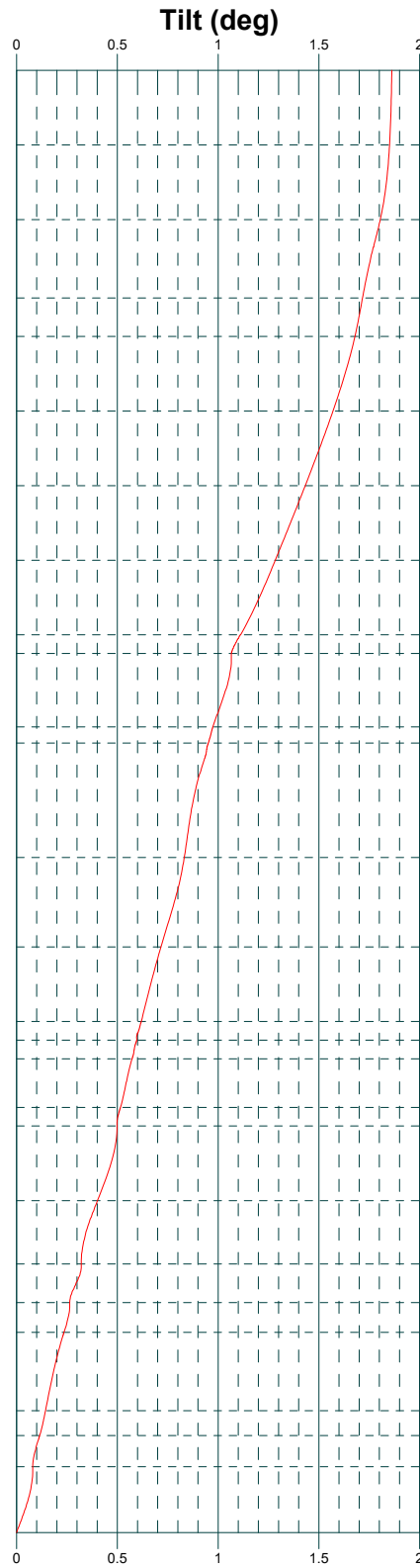
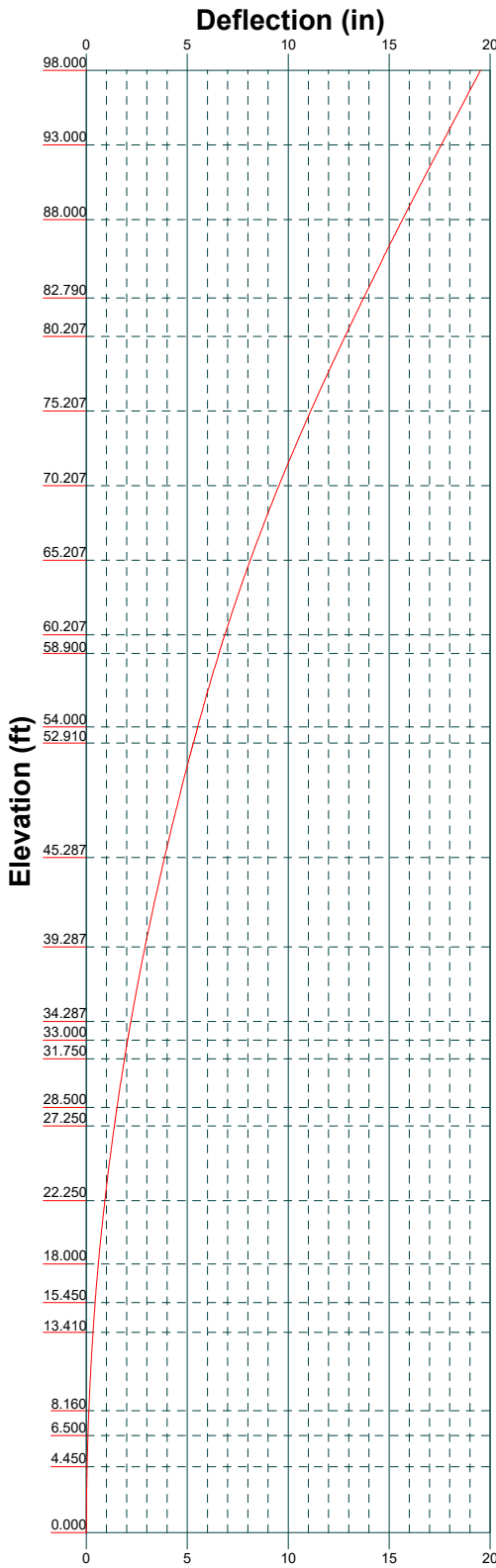
Mx

Mz



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Project:		
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Code: TIA-222-H	Date: 07/27/19	Scale: NTS
Path:	Dwg No. E-4	



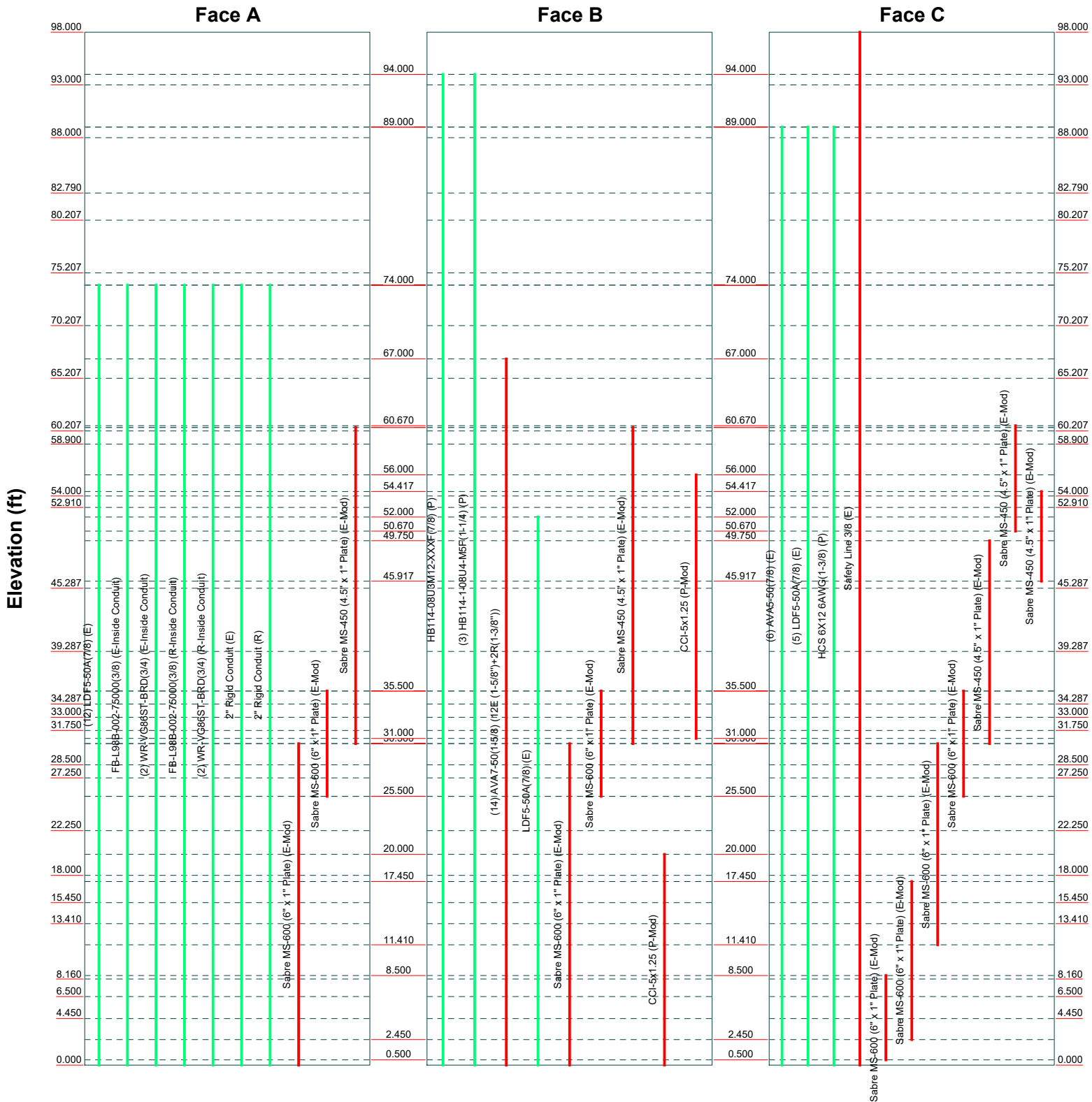
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Job: 127643.006.01 - (F) E. GRANBY 4Q2000 / GALASSO, CT (BU# 87639)		
Project:		
Client: Crown Castle	Drawn by: JD Prabhu	App'd:
Code: TIA-222-H	Date: 07/27/19	Scale: NTS
Path:	Dwg No. E-5	

Feed Line Distribution Chart

0' - 98'

— Round
 — Flat
 — App In Face
 — App Out Face
 — Truss Leg




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Job: 127643.006.01 - (F) E. GRANBY 4Q2000 / GALASSO, CT (BU# 87639)		
Project:		
Client: Crown Castle	Drawn by: JD Prabhu	App'd:
Code: TIA-222-H	Date: 07/27/19	Scale: NTS
Path:		Dwg No. E-7

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	<p>Project</p>	<p>Date 12:23:27 07/27/19</p>
	<p>Client Crown Castle</p>	<p>Designed by JD Prabhu</p>

Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

- Tower is located in Hartford County, Connecticut.
- Tower base elevation above sea level: 256.000 ft.
- Basic wind speed of 120 mph.
- Risk Category II.
- Exposure Category C.
- Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- Topographic Category: 1.
- Crest Height: 0.000 ft.
- Nominal ice thickness of 2.000 in.
- Ice thickness is considered to increase with height.
- Ice density of 56.000 pcf.
- A wind speed of 50 mph is used in combination with ice.
- Temperature drop of 50.000 °F.
- Deflections calculated using a wind speed of 60 mph.
- TIA-222-H Annex S.
- TOWER RATING: 99.1%.
- A non-linear (P-delta) analysis was used.
- Pressures are calculated at each section.
- Stress ratio used in pole design is 1.05.
- Tower analysis based on target reliabilities in accordance with Annex S.
- Load Modification Factors used: $K_{es}(F_w) = 0.95$, $K_{es}(t_i) = 0.85$.
- Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

- | | | |
|--|---|---|
| <ul style="list-style-type: none"> Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification Use Code Stress Ratios √ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric | <ul style="list-style-type: none"> Distribute Leg Loads As Uniform Assume Legs Pinned √ Assume Rigid Index Plate √ Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension √ Bypass Mast Stability Checks √ Use Azimuth Dish Coefficients √ Project Wind Area of Appurt. Autocalc Torque Arm Areas Add IBC .6D+W Combination Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs | <ul style="list-style-type: none"> Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation √ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice Exemption <li style="text-align: center;">Poles √ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known |
|--|---|---|

<p>tnxTower</p> <p>B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265</p>	<p>Job 127643.006.01 - (F) E. GRANBY 4Q2000 / GALASSO, CT (BU# 876399)</p>	<p>Page 2 of 45</p>
	<p>Project</p>	<p>Date 12:23:27 07/27/19</p>
	<p>Client Crown Castle</p>	<p>Designed by JD Prabhu</p>

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	98.000-93.000	5.000	0.000	18	12.000	13.078	0.188	0.750	A572-65 (65 ksi)
L2	93.000-88.000	5.000	0.000	18	13.078	14.156	0.188	0.750	A572-65 (65 ksi)
L3	88.000-82.790	5.210	2.417	18	14.156	15.280	0.188	0.750	A572-65 (65 ksi)
L4	82.790-80.207	5.000	0.000	18	14.384	15.445	0.250	1.000	A572-65 (65 ksi)
L5	80.207-75.207	5.000	0.000	18	15.445	16.507	0.250	1.000	A572-65 (65 ksi)
L6	75.207-70.207	5.000	0.000	18	16.507	17.569	0.250	1.000	A572-65 (65 ksi)
L7	70.207-65.207	5.000	0.000	18	17.569	18.630	0.250	1.000	A572-65 (65 ksi)
L8	65.207-60.207	5.000	0.000	18	18.630	19.692	0.250	1.000	A572-65 (65 ksi)
L9	60.207-59.170	1.037	0.000	18	19.692	19.912	0.250	1.000	A572-65 (65 ksi)
L10	59.170-58.900	0.270	0.000	18	19.912	19.970	0.512	2.050	A572-65 (65 ksi)
L11	58.900-58.750	0.150	0.000	18	19.970	20.001	0.512	2.050	A572-65 (65 ksi)
L12	58.750-54.000	4.750	0.000	18	20.001	21.010	0.500	2.000	A572-65 (65 ksi)
L13	54.000-53.750	0.250	0.000	18	21.010	21.063	0.512	2.050	A572-65 (65 ksi)
L14	53.750-52.910	0.840	0.000	18	21.063	21.241	0.500	2.000	A572-65 (65 ksi)
L15	52.910-52.660	0.250	0.000	18	21.241	21.294	0.675	2.700	A572-65 (65 ksi)
L16	52.660-52.170	0.490	0.000	18	21.294	21.399	0.675	2.700	A572-65 (65 ksi)
L17	52.170-51.920	0.250	0.000	18	21.399	21.452	0.525	2.100	A572-65 (65 ksi)
L18	51.920-45.287	6.633	3.417	18	21.452	22.860	0.512	2.050	A572-65 (65 ksi)
L19	45.287-44.287	4.417	0.000	18	21.634	22.575	0.563	2.250	A572-65 (65 ksi)
L20	44.287-39.287	5.000	0.000	18	22.575	23.639	0.550	2.200	A572-65 (65 ksi)
L21	39.287-34.287	5.000	0.000	18	23.639	24.703	0.537	2.150	A572-65 (65 ksi)
L22	34.287-33.500	0.787	0.000	18	24.703	24.870	0.525	2.100	A572-65 (65 ksi)
L23	33.500-33.250	0.250	0.000	18	24.870	24.923	0.838	3.350	A572-65 (65 ksi)
L24	33.250-33.000	0.250	0.000	18	24.923	24.977	0.838	3.350	A572-65 (65 ksi)
L25	33.000-32.750	0.250	0.000	18	24.977	25.030	0.813	3.250	A572-65 (65 ksi)
L26	32.750-32.000	0.750	0.000	18	25.030	25.190	0.800	3.200	A572-65 (65 ksi)
L27	32.000-31.750	0.250	0.000	18	25.190	25.243	0.588	2.350	A572-65 (65 ksi)
L28	31.750-28.500	3.250	0.000	18	25.243	25.934	0.575	2.300	A572-65 (65 ksi)
L29	28.500-28.250	0.250	0.000	18	25.934	25.988	0.863	3.450	A572-65 (65 ksi)

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L30	28.250-27.500	0.750	0.000	18	25.988	26.147	0.850	3.400	A572-65 (65 ksi)
L31	27.500-27.250	0.250	0.000	18	26.147	26.200	0.575	2.300	A572-65 (65 ksi)
L32	27.250-22.250	5.000	0.000	18	26.200	27.265	0.563	2.250	A572-65 (65 ksi)
L33	22.250-18.000	4.250	0.000	18	27.265	28.169	0.550	2.200	A572-65 (65 ksi)
L34	18.000-17.750	0.250	0.000	18	28.169	28.222	0.563	2.250	A572-65 (65 ksi)
L35	17.750-15.450	2.300	0.000	18	28.222	28.712	0.425	1.700	A572-65 (65 ksi)
L36	15.450-15.200	0.250	0.000	18	28.712	28.765	0.688	2.750	A572-65 (65 ksi)
L37	15.200-13.410	1.790	0.000	18	28.765	29.146	0.675	2.700	A572-65 (65 ksi)
L38	13.410-13.160	0.250	0.000	18	29.146	29.199	0.563	2.250	A572-65 (65 ksi)
L39	13.160-8.160	5.000	0.000	18	29.199	30.263	0.550	2.200	A572-65 (65 ksi)
L40	8.160-6.500	1.660	0.000	18	30.263	30.617	0.550	2.200	A572-65 (65 ksi)
L41	6.500-6.250	0.250	0.000	18	30.617	30.670	0.662	2.650	A572-65 (65 ksi)
L42	6.250-4.450	1.800	0.000	18	30.670	31.053	0.650	2.600	A572-65 (65 ksi)
L43	4.450-4.200	0.250	0.000	18	31.053	31.106	0.512	2.050	A572-65 (65 ksi)
L44	4.200-0.000	4.200		18	31.106	32.000	0.500	2.000	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	It/Q in ²	w in	w/t
L1	12.156	7.030	123.928	4.193	6.096	20.329	248.020	3.516	1.782	9.504
L2	13.251	7.672	161.057	4.576	6.644	24.242	322.325	3.837	1.972	10.516
L3	14.346	8.313	204.946	4.959	7.191	28.498	410.162	4.157	2.162	11.528
L4	15.487	8.982	258.481	5.358	7.762	33.300	517.303	4.492	2.359	12.583
L5	16.723	9.700	330.000	5.836	8.386	39.244	666.000	4.938	2.613	14.000
L6	18.059	10.467	423.000	6.414	9.114	46.000	866.000	5.493	2.938	16.000
L7	19.500	11.280	537.000	7.114	9.964	54.000	1137.000	6.200	3.366	18.500
L8	21.049	12.140	675.000	7.949	10.956	64.000	1497.000	7.080	3.914	22.500
L9	22.700	13.050	840.000	8.912	12.115	76.000	2000.000	8.140	4.613	28.000
L10	24.455	14.010	1037.000	10.000	13.464	92.000	2700.000	9.400	5.488	35.000
L11	26.310	15.020	1270.000	11.240	15.000	113.000	3700.000	10.870	6.613	45.000

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	I/Q in ²	w in	w/t
L12	20.231	31.702	1521.248	6.919	10.161	149.719	3044.499	15.854	2.618	5.109
	20.233	30.949	1487.002	6.923	10.161	146.348	2975.962	15.477	2.640	5.28
	21.257	32.549	1729.853	7.281	10.673	162.077	3461.983	16.278	2.818	5.636
L13	21.255	33.343	1769.860	7.277	10.673	165.825	3542.048	16.675	2.796	5.455
	21.309	33.429	1783.646	7.295	10.700	166.695	3569.638	16.718	2.805	5.473
L14	21.311	32.634	1743.319	7.300	10.700	162.927	3488.933	16.320	2.827	5.654
	21.492	32.917	1789.077	7.363	10.791	165.799	3580.508	16.461	2.858	5.717
L15	21.465	44.062	2354.634	7.301	10.791	218.211	4712.367	22.035	2.550	3.779
	21.519	44.176	2372.913	7.320	10.818	219.357	4748.949	22.092	2.560	3.792
L16	21.519	44.176	2372.913	7.320	10.818	219.357	4748.949	22.092	2.560	3.792
	21.625	44.399	2409.015	7.357	10.870	221.611	4821.199	22.204	2.578	3.819
L17	21.648	34.783	1914.659	7.410	10.870	176.134	3831.838	17.395	2.842	5.414
	21.702	34.871	1929.304	7.429	10.897	177.042	3861.146	17.439	2.851	5.431
L18	21.703	34.061	1886.745	7.433	10.897	173.137	3775.972	17.034	2.873	5.607
	23.134	36.352	2293.639	7.933	11.613	197.508	4590.296	18.180	3.121	6.09
L19	22.620	37.621	2110.485	7.481	10.990	192.031	4223.746	18.814	2.818	5.009
	22.836	39.300	2405.730	7.814	11.468	209.780	4814.626	19.654	2.983	5.303
L20	22.838	38.448	2356.279	7.819	11.468	205.468	4715.659	19.228	3.005	5.464
	23.918	40.306	2714.584	8.196	12.008	226.056	5432.740	20.157	3.192	5.804
L21	23.920	39.411	2657.200	8.201	12.008	221.278	5317.897	19.709	3.214	5.98
	25.001	41.227	3041.580	8.579	12.549	242.376	6087.162	20.617	3.402	6.329
L22	25.003	40.289	2975.458	8.583	12.549	237.107	5954.831	20.148	3.424	6.521
	25.173	40.568	3037.726	8.643	12.634	240.439	6079.449	20.288	3.453	6.577
L23	25.125	63.885	4661.673	8.532	12.634	368.975	9329.481	31.948	2.903	3.466
	25.179	64.026	4692.704	8.551	12.661	370.638	9391.582	32.019	2.913	3.478
L24	25.179	64.026	4692.704	8.551	12.661	370.638	9391.582	32.019	2.913	3.478
	25.233	64.167	4723.871	8.569	12.688	372.305	9453.959	32.090	2.922	3.489
L25	25.237	62.316	4597.114	8.578	12.688	362.315	9200.277	31.164	2.966	3.65
	25.291	62.454	4627.548	8.597	12.715	363.938	9261.185	31.233	2.975	3.662
L26	25.293	61.525	4563.414	8.602	12.715	358.895	9132.832	30.768	2.997	3.747
	25.455	61.930	4654.197	8.658	12.796	363.715	9314.518	30.971	3.025	3.782
L27	25.487	45.876	3508.045	8.734	12.796	274.146	7020.706	22.942	3.399	5.786
	25.542	45.975	3530.855	8.753	12.823	275.347	7066.356	22.992	3.409	5.802
L28	25.543	45.020	3460.989	8.757	12.823	269.898	6926.532	22.514	3.431	5.966
	26.246	46.282	3760.370	9.003	13.175	285.424	7525.687	23.146	3.552	6.178
L29	26.201	68.636	5450.880	8.901	13.175	413.739	10908.932	34.325	3.046	3.532
	26.255	68.782	5485.657	8.919	13.202	415.526	10978.532	34.398	3.056	3.543
L30	26.257	67.819	5414.227	8.924	13.202	410.115	10835.579	33.916	3.078	3.621
	26.419	68.249	5518.022	8.981	13.283	415.426	11043.305	34.131	3.106	3.654
L31	26.462	46.671	3855.842	9.078	13.283	290.288	7716.758	23.340	3.590	6.243
	26.516	46.768	3879.960	9.097	13.310	291.511	7765.026	23.388	3.599	6.26
L32	26.518	45.773	3801.170	9.101	13.310	285.591	7607.343	22.891	3.621	6.438
	27.598	47.673	4294.405	9.479	13.850	310.056	8594.460	23.841	3.809	6.771
L33	27.600	46.636	4204.873	9.484	13.850	303.592	8415.280	23.322	3.831	6.965
	28.519	48.215	4646.610	9.805	14.310	324.713	9299.333	24.112	3.990	7.254
L34	28.517	49.288	4745.765	9.800	14.310	331.642	9497.774	24.649	3.968	7.054
	28.571	49.383	4773.258	9.819	14.337	332.934	9552.796	24.696	3.977	7.07
L35	28.592	37.497	3660.514	9.868	14.337	255.320	7325.844	18.752	4.219	9.927
	29.089	38.158	3857.321	10.042	14.586	264.461	7719.717	19.082	4.305	10.13
L36	29.049	61.153	6067.676	9.949	14.586	416.004	12143.336	30.582	3.843	5.59
	29.103	61.269	6102.302	9.968	14.613	417.604	12212.633	30.640	3.853	5.604
L37	29.105	60.181	5999.356	9.972	14.613	410.560	12006.607	30.096	3.875	5.74
	29.491	60.998	6246.774	10.107	14.806	421.904	12501.767	30.505	3.942	5.84
L38	29.509	51.032	5267.598	10.147	14.806	355.771	10542.126	25.521	4.140	7.359
	29.563	51.127	5297.068	10.166	14.833	357.109	10601.107	25.568	4.149	7.376
L39	29.565	50.013	5186.141	10.170	14.833	349.631	10379.106	25.011	4.171	7.584
	30.645	51.871	5785.769	10.548	15.374	376.340	11579.151	25.940	4.358	7.924
L40	30.645	51.871	5785.769	10.548	15.374	376.340	11579.151	25.940	4.358	7.924
	31.004	52.487	5994.612	10.674	15.553	385.425	11997.111	26.249	4.421	8.037
L41	30.987	62.987	7140.031	10.634	15.553	459.070	14289.458	31.499	4.223	6.374
	31.041	63.099	7178.147	10.653	15.580	460.720	14365.739	31.555	4.232	6.388
L42	31.043	61.934	7051.515	10.657	15.580	452.592	14112.309	30.973	4.254	6.544

tnxTower B+T Group 1717 S, Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job 127643.006.01 - (F) E. GRANBY 4Q2000 / GALASSO, CT (BU# 876399)	Page 6 of 45
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	Client Crown Castle	Designed by JD Prabhu

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A_f	Adjust. Factor A_r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
ft	ft ²	in							
L26				1	1	0.907091			
32.750-32.000									
L27				1	1	0.929377			
32.000-31.750									
L28				1	1	0.938036			
31.750-28.500									
L29				1	1	0.893662			
28.500-28.250									
L30				1	1	0.902954			
28.250-27.500									
L31				1	1	0.93394			
27.500-27.250									
L32				1	1	0.938341			
27.250-22.250									
L33				1	1	0.946411			
22.250-18.000									
L34				1	1	1.05165			
18.000-17.750									
L35				1	1	1.21652			
17.750-15.450									
L36				1	1	0.954361			
15.450-15.200									
L37				1	1	0.964796			
15.200-13.410									
L38				1	1	1.03473			
13.410-13.160									
L39				1	1	1.04025			
13.160-8.160									
L40				1	1	1.0347			
8.160-6.500									
L41				1	1	0.956625			
6.500-6.250									
L42				1	1	0.968393			
6.250-4.450									
L43				1	1	0.980473			
4.450-4.200									
L44				1	1	0.993803			
4.200-0.000									

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Sector	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	Number Per Row	Start/End Position	Width or Diameter in	Perimeter in	Weight klf
*** AVA7-50(1-5/8) (12E (1-5/8")+2R(1-3/8")) ***	B	No	Surface Ar (CaAa)	67.000 - 0.000	14	7	-0.100 0.200	2.010		0.001
*** Safety Line 3/8 (E) ***	C	No	Surface Ar (CaAa)	98.000 - 0.000	1	1	0.100 0.100	0.375		0.000
***** Sabre MS-600 (6" x 1")	A	No	Surface Af	30.500 -	1	1	-0.500	6.000	14.000	0.000

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	<p>Project</p>	<p>Date 12:23:27 07/27/19</p>
	<p>Client Crown Castle</p>	<p>Designed by JD Prabhu</p>

Description	Sector	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	Number Per Row	Start/End Position	Width or Diameter in	Perimeter in	Weight klf
Plate) (E-Mod)			(CaAa)	0.000			-0.500			
Sabre MS-600 (6" x 1" Plate) (E-Mod)	B	No	Surface Af (CaAa)	30.500 - 0.000	1	1	-0.500 -0.500	6.000	14.000	0.000
Sabre MS-600 (6" x 1" Plate) (E-Mod)	C	No	Surface Af (CaAa)	8.500 - 0.500	1	1	-0.500 -0.500	6.000	14.000	0.000
Sabre MS-600 (6" x 1" Plate) (E-Mod)	C	No	Surface Af (CaAa)	17.450 - 2.450	1	1	-0.300 -0.300	6.000	14.000	0.000
Sabre MS-600 (6" x 1" Plate) (E-Mod)	C	No	Surface Af (CaAa)	30.500 - 11.410	1	1	-0.500 -0.500	6.000	14.000	0.000
Sabre MS-600 (6" x 1" Plate) (E-Mod)	A	No	Surface Af (CaAa)	35.500 - 25.500	1	1	-0.300 -0.300	6.000	14.000	0.000
Sabre MS-600 (6" x 1" Plate) (E-Mod)	B	No	Surface Af (CaAa)	35.500 - 25.500	1	1	-0.300 -0.300	6.000	14.000	0.000
Sabre MS-600 (6" x 1" Plate) (E-Mod)	C	No	Surface Af (CaAa)	35.500 - 25.500	1	1	-0.300 -0.300	6.000	14.000	0.000
Sabre MS-450 (4.5" x 1" Plate) (E-Mod)	C	No	Surface Af (CaAa)	49.750 - 30.500	1	1	-0.500 -0.500	4.500	11.000	0.000
Sabre MS-450 (4.5" x 1" Plate) (E-Mod)	A	No	Surface Af (CaAa)	60.500 - 30.500	1	1	-0.500 -0.500	4.500	11.000	0.000
Sabre MS-450 (4.5" x 1" Plate) (E-Mod)	B	No	Surface Af (CaAa)	60.500 - 30.500	1	1	-0.500 -0.500	4.500	11.000	0.000
Sabre MS-450 (4.5" x 1" Plate) (E-Mod)	C	No	Surface Af (CaAa)	60.670 - 50.670	1	1	-0.500 -0.500	4.500	11.000	0.000
Sabre MS-450 (4.5" x 1" Plate) (E-Mod)	C	No	Surface Af (CaAa)	54.417 - 45.917	1	1	-0.300 -0.300	4.500	11.000	0.000
CCI-5x1.25 (P-Mod) ***	B	No	Surface Af (CaAa)	20.000 - 0.000	1	1	0.000 0.000	5.000	12.500	0.000
CCI-5x1.25 (P-Mod) ***	B	No	Surface Af (CaAa)	56.000 - 31.000	1	1	0.000 0.000	5.000	12.500	0.000

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		C _A A _A ft ² /ft	Weight klf
HB114-08U3M12-X XXF(7/8) (P)	B	No	No	Inside Pole	94.000 - 0.000	1	No Ice	0.000	0.001
							1/2" Ice	0.000	0.001
							1" Ice	0.000	0.001

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Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A_R	A_F	C_{AA} In Face	C_{AA} Out Face	Weight K
			ft^2	ft^2	ft^2	ft^2	
L1	98.000-93.000	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.004
		C	0.000	0.000	0.188	0.000	0.001
L2	93.000-88.000	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.020
		C	0.000	0.000	0.188	0.000	0.006
L3	88.000-82.790	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.020
		C	0.000	0.000	0.195	0.000	0.028
L4	82.790-80.207	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.010
		C	0.000	0.000	0.097	0.000	0.014
L5	80.207-75.207	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.020
		C	0.000	0.000	0.188	0.000	0.027
L6	75.207-70.207	A	0.000	0.000	0.000	0.000	0.046
		B	0.000	0.000	0.000	0.000	0.020
		C	0.000	0.000	0.188	0.000	0.027
L7	70.207-65.207	A	0.000	0.000	0.000	0.000	0.060
		B	0.000	0.000	2.523	0.000	0.037
		C	0.000	0.000	0.188	0.000	0.027
L8	65.207-60.207	A	0.000	0.000	0.220	0.000	0.060
		B	0.000	0.000	7.255	0.000	0.069
		C	0.000	0.000	0.535	0.000	0.027
L9	60.207-59.170	A	0.000	0.000	0.778	0.000	0.012
		B	0.000	0.000	2.237	0.000	0.014
		C	0.000	0.000	0.817	0.000	0.006
L10	59.170-58.900	A	0.000	0.000	0.203	0.000	0.003
		B	0.000	0.000	0.582	0.000	0.004
		C	0.000	0.000	0.213	0.000	0.001
L11	58.900-58.750	A	0.000	0.000	0.113	0.000	0.002
		B	0.000	0.000	0.324	0.000	0.002
		C	0.000	0.000	0.118	0.000	0.001
L12	58.750-54.000	A	0.000	0.000	3.563	0.000	0.057
		B	0.000	0.000	11.912	0.000	0.065
		C	0.000	0.000	4.038	0.000	0.026
L13	54.000-53.750	A	0.000	0.000	0.188	0.000	0.003
		B	0.000	0.000	0.748	0.000	0.003
		C	0.000	0.000	0.375	0.000	0.001
L14	53.750-52.910	A	0.000	0.000	0.630	0.000	0.010
		B	0.000	0.000	2.512	0.000	0.012
		C	0.000	0.000	1.261	0.000	0.005
L15	52.910-52.660	A	0.000	0.000	0.188	0.000	0.003
		B	0.000	0.000	0.748	0.000	0.003
		C	0.000	0.000	0.375	0.000	0.001
L16	52.660-52.170	A	0.000	0.000	0.367	0.000	0.006
		B	0.000	0.000	1.465	0.000	0.007
		C	0.000	0.000	0.736	0.000	0.003
L17	52.170-51.920	A	0.000	0.000	0.188	0.000	0.003
		B	0.000	0.000	0.748	0.000	0.003
		C	0.000	0.000	0.375	0.000	0.001
L18	51.920-45.287	A	0.000	0.000	4.975	0.000	0.080
		B	0.000	0.000	19.835	0.000	0.093
		C	0.000	0.000	8.820	0.000	0.036
L19	45.287-44.287	A	0.000	0.000	0.750	0.000	0.012
		B	0.000	0.000	2.990	0.000	0.014
		C	0.000	0.000	0.787	0.000	0.005
L20	44.287-39.287	A	0.000	0.000	3.750	0.000	0.060

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
		B	0.000	0.000	14.952	0.000	0.070
		C	0.000	0.000	3.938	0.000	0.027
L21	39.287-34.287	A	0.000	0.000	4.856	0.000	0.060
		B	0.000	0.000	16.058	0.000	0.070
		C	0.000	0.000	5.044	0.000	0.027
L22	34.287-33.500	A	0.000	0.000	1.308	0.000	0.009
		B	0.000	0.000	3.071	0.000	0.011
		C	0.000	0.000	1.338	0.000	0.004
L23	33.500-33.250	A	0.000	0.000	0.416	0.000	0.003
		B	0.000	0.000	0.976	0.000	0.004
		C	0.000	0.000	0.425	0.000	0.001
L24	33.250-33.000	A	0.000	0.000	0.416	0.000	0.003
		B	0.000	0.000	0.976	0.000	0.004
		C	0.000	0.000	0.425	0.000	0.001
L25	33.000-32.750	A	0.000	0.000	0.416	0.000	0.003
		B	0.000	0.000	0.976	0.000	0.004
		C	0.000	0.000	0.425	0.000	0.001
L26	32.750-32.000	A	0.000	0.000	1.247	0.000	0.009
		B	0.000	0.000	2.927	0.000	0.011
		C	0.000	0.000	1.275	0.000	0.004
L27	32.000-31.750	A	0.000	0.000	0.416	0.000	0.003
		B	0.000	0.000	0.976	0.000	0.004
		C	0.000	0.000	0.425	0.000	0.001
L28	31.750-28.500	A	0.000	0.000	5.902	0.000	0.039
		B	0.000	0.000	11.100	0.000	0.046
		C	0.000	0.000	6.024	0.000	0.017
L29	28.500-28.250	A	0.000	0.000	0.478	0.000	0.003
		B	0.000	0.000	0.830	0.000	0.004
		C	0.000	0.000	0.487	0.000	0.001
L30	28.250-27.500	A	0.000	0.000	1.434	0.000	0.009
		B	0.000	0.000	2.489	0.000	0.011
		C	0.000	0.000	1.462	0.000	0.004
L31	27.500-27.250	A	0.000	0.000	0.478	0.000	0.003
		B	0.000	0.000	0.830	0.000	0.004
		C	0.000	0.000	0.487	0.000	0.001
L32	27.250-22.250	A	0.000	0.000	6.596	0.000	0.060
		B	0.000	0.000	13.631	0.000	0.070
		C	0.000	0.000	6.784	0.000	0.027
L33	22.250-18.000	A	0.000	0.000	4.250	0.000	0.051
		B	0.000	0.000	11.896	0.000	0.060
		C	0.000	0.000	4.409	0.000	0.023
L34	18.000-17.750	A	0.000	0.000	0.250	0.000	0.003
		B	0.000	0.000	0.810	0.000	0.004
		C	0.000	0.000	0.259	0.000	0.001
L35	17.750-15.450	A	0.000	0.000	2.300	0.000	0.028
		B	0.000	0.000	7.453	0.000	0.032
		C	0.000	0.000	4.386	0.000	0.012
L36	15.450-15.200	A	0.000	0.000	0.250	0.000	0.003
		B	0.000	0.000	0.810	0.000	0.004
		C	0.000	0.000	0.509	0.000	0.001
L37	15.200-13.410	A	0.000	0.000	1.790	0.000	0.022
		B	0.000	0.000	5.800	0.000	0.025
		C	0.000	0.000	3.647	0.000	0.010
L38	13.410-13.160	A	0.000	0.000	0.250	0.000	0.003
		B	0.000	0.000	0.810	0.000	0.004
		C	0.000	0.000	0.509	0.000	0.001
L39	13.160-8.160	A	0.000	0.000	5.000	0.000	0.060
		B	0.000	0.000	16.202	0.000	0.070
		C	0.000	0.000	7.225	0.000	0.027
L40	8.160-6.500	A	0.000	0.000	1.660	0.000	0.020
		B	0.000	0.000	5.379	0.000	0.023

<p>tnxTower</p> <p>B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265</p>	<p>Job 127643.006.01 - (F) E. GRANBY 4Q2000 / GALASSO, CT (BU# 876399)</p>	<p>Page 11 of 45</p>
	<p>Project</p>	<p>Date 12:23:27 07/27/19</p>
	<p>Client Crown Castle</p>	<p>Designed by JD Prabhu</p>

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
L41	6.500-6.250	C	0.000	0.000	3.127	0.000	0.009
		A	0.000	0.000	0.250	0.000	0.003
		B	0.000	0.000	0.810	0.000	0.004
L42	6.250-4.450	C	0.000	0.000	0.471	0.000	0.001
		A	0.000	0.000	1.800	0.000	0.022
		B	0.000	0.000	5.833	0.000	0.025
L43	4.450-4.200	C	0.000	0.000	3.391	0.000	0.010
		A	0.000	0.000	0.250	0.000	0.003
		B	0.000	0.000	0.810	0.000	0.004
L44	4.200-0.000	C	0.000	0.000	0.471	0.000	0.001
		A	0.000	0.000	4.200	0.000	0.050
		B	0.000	0.000	13.609	0.000	0.059
		C	0.000	0.000	5.039	0.000	0.023

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
L1	98.000-93.000	A	1.891	0.000	0.000	0.000	0.000	0.000
		B		0.000	0.000	0.000	0.000	0.004
		C		0.000	0.000	2.078	0.000	0.027
L2	93.000-88.000	A	1.880	0.000	0.000	0.000	0.000	0.000
		B		0.000	0.000	0.000	0.000	0.020
		C		0.000	0.000	2.068	0.000	0.032
L3	88.000-82.790	A	1.869	0.000	0.000	0.000	0.000	0.000
		B		0.000	0.000	0.000	0.000	0.020
		C		0.000	0.000	2.143	0.000	0.055
L4	82.790-80.207	A	1.861	0.000	0.000	0.000	0.000	0.000
		B		0.000	0.000	0.000	0.000	0.010
		C		0.000	0.000	1.063	0.000	0.027
L5	80.207-75.207	A	1.852	0.000	0.000	0.000	0.000	0.000
		B		0.000	0.000	0.000	0.000	0.020
		C		0.000	0.000	2.039	0.000	0.052
L6	75.207-70.207	A	1.840	0.000	0.000	0.000	0.000	0.046
		B		0.000	0.000	0.000	0.000	0.020
		C		0.000	0.000	2.027	0.000	0.052
L7	70.207-65.207	A	1.827	0.000	0.000	0.000	0.000	0.060
		B		0.000	0.000	3.972	0.000	0.099
		C		0.000	0.000	2.014	0.000	0.051
L8	65.207-60.207	A	1.813	0.000	0.000	0.326	0.000	0.064
		B		0.000	0.000	11.386	0.000	0.244
		C		0.000	0.000	2.440	0.000	0.057
L9	60.207-59.170	A	1.804	0.000	0.000	1.152	0.000	0.025
		B		0.000	0.000	3.443	0.000	0.063
		C		0.000	0.000	1.398	0.000	0.024
L10	59.170-58.900	A	1.802	0.000	0.000	0.300	0.000	0.007
		B		0.000	0.000	0.896	0.000	0.016
		C		0.000	0.000	0.364	0.000	0.006
L11	58.900-58.750	A	1.801	0.000	0.000	0.167	0.000	0.004
		B		0.000	0.000	0.498	0.000	0.009
		C		0.000	0.000	0.202	0.000	0.003
L12	58.750-54.000	A	1.793	0.000	0.000	5.266	0.000	0.116
		B		0.000	0.000	18.134	0.000	0.313
		C		0.000	0.000	6.765	0.000	0.112
L13	54.000-53.750	A	1.785	0.000	0.000	0.277	0.000	0.006
		B		0.000	0.000	1.126	0.000	0.018
		C		0.000	0.000	0.561	0.000	0.009

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
L14	53.750-52.910	A	1.784	0.000	0.000	0.930	0.000	0.020
		B		0.000	0.000	3.781	0.000	0.062
		C		0.000	0.000	1.884	0.000	0.029
L15	52.910-52.660	A	1.782	0.000	0.000	0.277	0.000	0.006
		B		0.000	0.000	1.125	0.000	0.018
		C		0.000	0.000	0.561	0.000	0.009
L16	52.660-52.170	A	1.781	0.000	0.000	0.542	0.000	0.012
		B		0.000	0.000	2.205	0.000	0.036
		C		0.000	0.000	1.099	0.000	0.017
L17	52.170-51.920	A	1.779	0.000	0.000	0.276	0.000	0.006
		B		0.000	0.000	1.125	0.000	0.018
		C		0.000	0.000	0.560	0.000	0.009
L18	51.920-45.287	A	1.767	0.000	0.000	7.319	0.000	0.160
		B		0.000	0.000	29.786	0.000	0.484
		C		0.000	0.000	14.099	0.000	0.209
L19	45.287-44.287	A	1.753	0.000	0.000	1.103	0.000	0.024
		B		0.000	0.000	4.491	0.000	0.073
		C		0.000	0.000	1.494	0.000	0.022
L20	44.287-39.287	A	1.741	0.000	0.000	5.491	0.000	0.120
		B		0.000	0.000	22.367	0.000	0.360
		C		0.000	0.000	7.419	0.000	0.109
L21	39.287-34.287	A	1.718	0.000	0.000	6.795	0.000	0.135
		B		0.000	0.000	23.622	0.000	0.372
		C		0.000	0.000	8.701	0.000	0.124
L22	34.287-33.500	A	1.705	0.000	0.000	1.718	0.000	0.029
		B		0.000	0.000	4.362	0.000	0.066
		C		0.000	0.000	2.016	0.000	0.027
L23	33.500-33.250	A	1.702	0.000	0.000	0.546	0.000	0.009
		B		0.000	0.000	1.385	0.000	0.021
		C		0.000	0.000	0.640	0.000	0.009
L24	33.250-33.000	A	1.701	0.000	0.000	0.546	0.000	0.009
		B		0.000	0.000	1.385	0.000	0.021
		C		0.000	0.000	0.640	0.000	0.009
L25	33.000-32.750	A	1.699	0.000	0.000	0.545	0.000	0.009
		B		0.000	0.000	1.385	0.000	0.021
		C		0.000	0.000	0.640	0.000	0.009
L26	32.750-32.000	A	1.697	0.000	0.000	1.636	0.000	0.028
		B		0.000	0.000	4.152	0.000	0.063
		C		0.000	0.000	1.918	0.000	0.026
L27	32.000-31.750	A	1.694	0.000	0.000	0.545	0.000	0.009
		B		0.000	0.000	1.384	0.000	0.021
		C		0.000	0.000	0.639	0.000	0.009
L28	31.750-28.500	A	1.685	0.000	0.000	7.576	0.000	0.123
		B		0.000	0.000	15.538	0.000	0.244
		C		0.000	0.000	8.788	0.000	0.116
L29	28.500-28.250	A	1.675	0.000	0.000	0.606	0.000	0.010
		B		0.000	0.000	1.150	0.000	0.018
		C		0.000	0.000	0.699	0.000	0.009
L30	28.250-27.500	A	1.672	0.000	0.000	1.817	0.000	0.029
		B		0.000	0.000	3.450	0.000	0.054
		C		0.000	0.000	2.095	0.000	0.027
L31	27.500-27.250	A	1.669	0.000	0.000	0.606	0.000	0.010
		B		0.000	0.000	1.150	0.000	0.018
		C		0.000	0.000	0.698	0.000	0.009
L32	27.250-22.250	A	1.652	0.000	0.000	8.554	0.000	0.148
		B		0.000	0.000	19.412	0.000	0.316
		C		0.000	0.000	10.386	0.000	0.135
L33	22.250-18.000	A	1.618	0.000	0.000	5.625	0.000	0.105
		B		0.000	0.000	17.133	0.000	0.268
		C		0.000	0.000	7.158	0.000	0.093
L34	18.000-17.750	A	1.599	0.000	0.000	0.330	0.000	0.006

<p style="text-align: center;">tnxTower</p> <p style="text-align: center;">B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265</p>	Job 127643.006.01 - (F) E. GRANBY 4Q2000 / GALASSO, CT (BU# 876399)	Page 13 of 45
	Project	Date 12:23:27 07/27/19
	Client Crown Castle	Designed by JD Prabhu

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
		B		0.000	0.000	1.158	0.000	0.017
		C		0.000	0.000	0.419	0.000	0.005
L35	17.750-15.450	A	1.587	0.000	0.000	3.030	0.000	0.056
		B		0.000	0.000	10.634	0.000	0.157
		C		0.000	0.000	6.327	0.000	0.074
L36	15.450-15.200	A	1.574	0.000	0.000	0.329	0.000	0.006
		B		0.000	0.000	1.154	0.000	0.017
		C		0.000	0.000	0.727	0.000	0.008
L37	15.200-13.410	A	1.564	0.000	0.000	2.350	0.000	0.043
		B		0.000	0.000	8.249	0.000	0.121
		C		0.000	0.000	5.193	0.000	0.060
L38	13.410-13.160	A	1.552	0.000	0.000	0.328	0.000	0.006
		B		0.000	0.000	1.150	0.000	0.017
		C		0.000	0.000	0.724	0.000	0.008
L39	13.160-8.160	A	1.518	0.000	0.000	6.518	0.000	0.118
		B		0.000	0.000	22.894	0.000	0.328
		C		0.000	0.000	10.497	0.000	0.127
L40	8.160-6.500	A	1.463	0.000	0.000	2.146	0.000	0.038
		B		0.000	0.000	7.541	0.000	0.106
		C		0.000	0.000	4.251	0.000	0.051
L41	6.500-6.250	A	1.442	0.000	0.000	0.322	0.000	0.006
		B		0.000	0.000	1.132	0.000	0.016
		C		0.000	0.000	0.638	0.000	0.008
L42	6.250-4.450	A	1.417	0.000	0.000	2.310	0.000	0.041
		B		0.000	0.000	8.124	0.000	0.112
		C		0.000	0.000	4.577	0.000	0.054
L43	4.450-4.200	A	1.387	0.000	0.000	0.319	0.000	0.006
		B		0.000	0.000	1.123	0.000	0.015
		C		0.000	0.000	0.633	0.000	0.007
L44	4.200-0.000	A	1.290	0.000	0.000	5.284	0.000	0.090
		B		0.000	0.000	18.609	0.000	0.242
		C		0.000	0.000	7.004	0.000	0.085

Feed Line Center of Pressure

Section	Elevation ft	CP _x in	CP _z in	CP _x Ice in	CP _z Ice in
L1	98.000-93.000	-0.062	0.292	-0.270	1.272
L2	93.000-88.000	-0.062	0.293	-0.277	1.305
L3	88.000-82.790	-0.062	0.293	-0.283	1.334
L4	82.790-80.207	-0.062	0.293	-0.286	1.347
L5	80.207-75.207	-0.062	0.293	-0.289	1.361
L6	75.207-70.207	-0.062	0.294	-0.294	1.381
L7	70.207-65.207	3.114	-1.201	2.046	-0.008
L8	65.207-60.207	4.327	-1.815	3.969	-1.164
L9	60.207-59.170	2.628	-1.113	2.349	-0.702
L10	59.170-58.900	2.639	-1.117	2.360	-0.706
L11	58.900-58.750	2.642	-1.119	2.363	-0.707
L12	58.750-54.000	3.138	-1.310	2.844	-0.917
L13	54.000-53.750	4.242	-1.015	3.885	-0.755
L14	53.750-52.910	4.257	-1.018	3.900	-0.758
L15	52.910-52.660	4.273	-1.022	3.917	-0.761
L16	52.660-52.170	4.283	-1.024	3.927	-0.763
L17	52.170-51.920	4.292	-1.026	3.937	-0.764
L18	51.920-45.287	4.179	-1.073	3.963	-0.828
L19	45.287-44.287	3.697	-1.754	3.608	-1.385

tnxTower B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job 127643.006.01 - (F) E. GRANBY 4Q2000 / GALASSO, CT (BU# 876399)	Page 14 of 45
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	Client Crown Castle	Designed by JD Prabhu

Section	Elevation	CP _x	CP _z	CP _x	CP _z
	ft	in	in	Ice in	Ice in
L20	44.287-39.287	3.756	-1.782	3.681	-1.416
L21	39.287-34.287	3.482	-1.652	3.501	-1.348
L22	34.287-33.500	2.723	-1.292	2.868	-1.105
L23	33.500-33.250	2.732	-1.297	2.878	-1.110
L24	33.250-33.000	2.736	-1.299	2.883	-1.111
L25	33.000-32.750	2.740	-1.300	2.887	-1.113
L26	32.750-32.000	2.747	-1.304	2.896	-1.117
L27	32.000-31.750	2.754	-1.307	2.905	-1.120
L28	31.750-28.500	2.083	-0.914	2.265	-0.732
L29	28.500-28.250	1.860	-0.786	2.056	-0.608
L30	28.250-27.500	1.865	-0.788	2.062	-0.610
L31	27.500-27.250	1.869	-0.790	2.068	-0.612
L32	27.250-22.250	2.347	-0.992	2.527	-0.750
L33	22.250-18.000	3.240	-1.467	3.388	-1.184
L34	18.000-17.750	3.768	-1.789	3.911	-1.521
L35	17.750-15.450	4.814	-0.961	4.853	-0.807
L36	15.450-15.200	4.980	-0.857	5.010	-0.717
L37	15.200-13.410	5.004	-0.861	5.039	-0.722
L38	13.410-13.160	5.027	-0.864	5.067	-0.727
L39	13.160-8.160	4.108	-0.597	4.190	-0.461
L40	8.160-6.500	4.907	-0.815	4.874	-0.655
L41	6.500-6.250	4.928	-0.818	4.901	-0.661
L42	6.250-4.450	4.950	-0.821	4.931	-0.667
L43	4.450-4.200	4.970	-0.824	4.961	-0.675
L44	4.200-0.000	4.086	-1.347	4.176	-1.163

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L1	27	Safety Line 3/8	93.00 - 98.00	1.0000	1.0000
L2	27	Safety Line 3/8	88.00 - 93.00	1.0000	1.0000
L3	27	Safety Line 3/8	82.79 - 88.00	1.0000	1.0000
L5	27	Safety Line 3/8	75.21 - 80.21	1.0000	1.0000
L6	27	Safety Line 3/8	70.21 - 75.21	1.0000	1.0000
L7	21	AVA7-50(1-5/8)	65.21 - 67.00	1.0000	1.0000
L7	27	Safety Line 3/8	65.21 - 70.21	1.0000	1.0000
L8	21	AVA7-50(1-5/8)	60.21 - 65.21	1.0000	1.0000
L8	27	Safety Line 3/8	60.21 - 65.21	1.0000	1.0000
L8	41	Sabre MS-450 (4.5" x 1" Plate)	60.21 - 60.50	1.0000	1.0000
L8	42	Sabre MS-450 (4.5" x 1" Plate)	60.21 - 60.50	1.0000	1.0000
L8	43	Sabre MS-450 (4.5" x 1" Plate)	60.21 - 60.67	1.0000	1.0000
L9	21	AVA7-50(1-5/8)	59.17 - 60.21	1.0000	1.0000
L9	27	Safety Line 3/8	59.17 - 60.21	1.0000	1.0000
L9	41	Sabre MS-450 (4.5" x 1" Plate)	59.17 - 60.21	1.0000	1.0000
L9	42	Sabre MS-450 (4.5" x 1" Plate)	59.17 - 60.21	1.0000	1.0000
L9	43	Sabre MS-450 (4.5" x 1" Plate)	59.17 - 60.21	1.0000	1.0000

tnxTower

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Client
Crown Castle
Designed by
JD Prabhu

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K_a No Ice	K_a Ice
L10	21	AVA7-50(1-5/8) Plate)	58.90 - 59.17	1.0000	1.0000
L10	27	Safety Line 3/8	58.90 - 59.17	1.0000	1.0000
L10	41	Sabre MS-450 (4.5" x 1" Plate)	58.90 - 59.17	1.0000	1.0000
L10	42	Sabre MS-450 (4.5" x 1" Plate)	58.90 - 59.17	1.0000	1.0000
L10	43	Sabre MS-450 (4.5" x 1" Plate)	58.90 - 59.17	1.0000	1.0000
L11	21	AVA7-50(1-5/8) Plate)	58.75 - 58.90	1.0000	1.0000
L11	27	Safety Line 3/8	58.75 - 58.90	1.0000	1.0000
L11	41	Sabre MS-450 (4.5" x 1" Plate)	58.75 - 58.90	1.0000	1.0000
L11	42	Sabre MS-450 (4.5" x 1" Plate)	58.75 - 58.90	1.0000	1.0000
L11	43	Sabre MS-450 (4.5" x 1" Plate)	58.75 - 58.90	1.0000	1.0000
L12	21	AVA7-50(1-5/8) Plate)	54.00 - 58.75	1.0000	1.0000
L12	27	Safety Line 3/8	54.00 - 58.75	1.0000	1.0000
L12	41	Sabre MS-450 (4.5" x 1" Plate)	54.00 - 58.75	1.0000	1.0000
L12	42	Sabre MS-450 (4.5" x 1" Plate)	54.00 - 58.75	1.0000	1.0000
L12	43	Sabre MS-450 (4.5" x 1" Plate)	54.00 - 58.75	1.0000	1.0000
L12	44	Sabre MS-450 (4.5" x 1" Plate)	54.00 - 54.42	1.0000	1.0000
L12	48	CCI-5x1.25	54.00 - 56.00	1.0000	1.0000
L13	21	AVA7-50(1-5/8) Plate)	53.75 - 54.00	1.0000	1.0000
L13	27	Safety Line 3/8	53.75 - 54.00	1.0000	1.0000
L13	41	Sabre MS-450 (4.5" x 1" Plate)	53.75 - 54.00	1.0000	1.0000
L13	42	Sabre MS-450 (4.5" x 1" Plate)	53.75 - 54.00	1.0000	1.0000
L13	43	Sabre MS-450 (4.5" x 1" Plate)	53.75 - 54.00	1.0000	1.0000
L13	44	Sabre MS-450 (4.5" x 1" Plate)	53.75 - 54.00	1.0000	1.0000
L13	48	CCI-5x1.25	53.75 - 54.00	1.0000	1.0000
L14	21	AVA7-50(1-5/8) Plate)	52.91 - 53.75	1.0000	1.0000
L14	27	Safety Line 3/8	52.91 - 53.75	1.0000	1.0000
L14	41	Sabre MS-450 (4.5" x 1" Plate)	52.91 - 53.75	1.0000	1.0000
L14	42	Sabre MS-450 (4.5" x 1" Plate)	52.91 - 53.75	1.0000	1.0000
L14	43	Sabre MS-450 (4.5" x 1" Plate)	52.91 - 53.75	1.0000	1.0000
L14	44	Sabre MS-450 (4.5" x 1" Plate)	52.91 - 53.75	1.0000	1.0000
L14	48	CCI-5x1.25	52.91 - 53.75	1.0000	1.0000
L15	21	AVA7-50(1-5/8) Plate)	52.66 - 52.91	1.0000	1.0000
L15	27	Safety Line 3/8	52.66 - 52.91	1.0000	1.0000
L15	41	Sabre MS-450 (4.5" x 1" Plate)	52.66 - 52.91	1.0000	1.0000
L15	42	Sabre MS-450 (4.5" x 1" Plate)	52.66 - 52.91	1.0000	1.0000
L15	43	Sabre MS-450 (4.5" x 1" Plate)	52.66 - 52.91	1.0000	1.0000
L15	44	Sabre MS-450 (4.5" x 1" Plate)	52.66 - 52.91	1.0000	1.0000
L15	48	CCI-5x1.25	52.66 - 52.91	1.0000	1.0000
L16	21	AVA7-50(1-5/8)	52.17 - 52.66	1.0000	1.0000

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L16	27	Safety Line 3/8	52.17 - 52.66	1.0000	1.0000
L16	41	Sabre MS-450 (4.5" x 1" Plate)	52.17 - 52.66	1.0000	1.0000
L16	42	Sabre MS-450 (4.5" x 1" Plate)	52.17 - 52.66	1.0000	1.0000
L16	43	Sabre MS-450 (4.5" x 1" Plate)	52.17 - 52.66	1.0000	1.0000
L16	44	Sabre MS-450 (4.5" x 1" Plate)	52.17 - 52.66	1.0000	1.0000
L16	48	CCI-5x1.25	52.17 - 52.66	1.0000	1.0000
L17	21	AVA7-50(1-5/8)	51.92 - 52.17	1.0000	1.0000
L17	27	Safety Line 3/8	51.92 - 52.17	1.0000	1.0000
L17	41	Sabre MS-450 (4.5" x 1" Plate)	51.92 - 52.17	1.0000	1.0000
L17	42	Sabre MS-450 (4.5" x 1" Plate)	51.92 - 52.17	1.0000	1.0000
L17	43	Sabre MS-450 (4.5" x 1" Plate)	51.92 - 52.17	1.0000	1.0000
L17	44	Sabre MS-450 (4.5" x 1" Plate)	51.92 - 52.17	1.0000	1.0000
L17	48	CCI-5x1.25	51.92 - 52.17	1.0000	1.0000
L18	21	AVA7-50(1-5/8)	45.29 - 51.92	1.0000	1.0000
L18	27	Safety Line 3/8	45.29 - 51.92	1.0000	1.0000
L18	40	Sabre MS-450 (4.5" x 1" Plate)	45.29 - 49.75	1.0000	1.0000
L18	41	Sabre MS-450 (4.5" x 1" Plate)	45.29 - 51.92	1.0000	1.0000
L18	42	Sabre MS-450 (4.5" x 1" Plate)	45.29 - 51.92	1.0000	1.0000
L18	43	Sabre MS-450 (4.5" x 1" Plate)	50.67 - 51.92	1.0000	1.0000
L18	44	Sabre MS-450 (4.5" x 1" Plate)	45.92 - 51.92	1.0000	1.0000
L18	48	CCI-5x1.25	45.29 - 51.92	1.0000	1.0000
L20	21	AVA7-50(1-5/8)	39.29 - 44.29	1.0000	1.0000
L20	27	Safety Line 3/8	39.29 - 44.29	1.0000	1.0000
L20	40	Sabre MS-450 (4.5" x 1" Plate)	39.29 - 44.29	1.0000	1.0000
L20	41	Sabre MS-450 (4.5" x 1" Plate)	39.29 - 44.29	1.0000	1.0000
L20	42	Sabre MS-450 (4.5" x 1" Plate)	39.29 - 44.29	1.0000	1.0000
L20	48	CCI-5x1.25	39.29 - 44.29	1.0000	1.0000
L21	21	AVA7-50(1-5/8)	34.29 - 39.29	1.0000	1.0000
L21	27	Safety Line 3/8	34.29 - 39.29	1.0000	1.0000
L21	36	Sabre MS-600 (6" x 1" Plate)	34.29 - 35.50	1.0000	1.0000
L21	37	Sabre MS-600 (6" x 1" Plate)	34.29 - 35.50	1.0000	1.0000
L21	38	Sabre MS-600 (6" x 1" Plate)	34.29 - 35.50	1.0000	1.0000
L21	40	Sabre MS-450 (4.5" x 1" Plate)	34.29 - 39.29	1.0000	1.0000
L21	41	Sabre MS-450 (4.5" x 1" Plate)	34.29 - 39.29	1.0000	1.0000
L21	42	Sabre MS-450 (4.5" x 1" Plate)	34.29 - 39.29	1.0000	1.0000
L21	48	CCI-5x1.25	34.29 - 39.29	1.0000	1.0000
L22	21	AVA7-50(1-5/8)	33.50 - 34.29	1.0000	1.0000
L22	27	Safety Line 3/8	33.50 - 34.29	1.0000	1.0000
L22	36	Sabre MS-600 (6" x 1" Plate)	33.50 - 34.29	1.0000	1.0000
L22	37	Sabre MS-600 (6" x 1" Plate)	33.50 - 34.29	1.0000	1.0000
L22	38	Sabre MS-600 (6" x 1" Plate)	33.50 - 34.29	1.0000	1.0000
L22	40	Sabre MS-450 (4.5" x 1" Plate)	33.50 - 34.29	1.0000	1.0000

tnxTower

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(BU# 876399)

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Client
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Designed by
JD Prabhu

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L22	41	Sabre MS-450 (4.5" x 1" Plate)	33.50 - 34.29	1.0000	1.0000
L22	42	Sabre MS-450 (4.5" x 1" Plate)	33.50 - 34.29	1.0000	1.0000
L22	48	CCI-5x1.25	33.50 - 34.29	1.0000	1.0000
L23	21	AVA7-50(1-5/8)	33.25 - 33.50	1.0000	1.0000
L23	27	Safety Line 3/8	33.25 - 33.50	1.0000	1.0000
L23	36	Sabre MS-600 (6" x 1" Plate)	33.25 - 33.50	1.0000	1.0000
L23	37	Sabre MS-600 (6" x 1" Plate)	33.25 - 33.50	1.0000	1.0000
L23	38	Sabre MS-600 (6" x 1" Plate)	33.25 - 33.50	1.0000	1.0000
L23	40	Sabre MS-450 (4.5" x 1" Plate)	33.25 - 33.50	1.0000	1.0000
L23	41	Sabre MS-450 (4.5" x 1" Plate)	33.25 - 33.50	1.0000	1.0000
L23	42	Sabre MS-450 (4.5" x 1" Plate)	33.25 - 33.50	1.0000	1.0000
L23	48	CCI-5x1.25	33.25 - 33.50	1.0000	1.0000
L24	21	AVA7-50(1-5/8)	33.00 - 33.25	1.0000	1.0000
L24	27	Safety Line 3/8	33.00 - 33.25	1.0000	1.0000
L24	36	Sabre MS-600 (6" x 1" Plate)	33.00 - 33.25	1.0000	1.0000
L24	37	Sabre MS-600 (6" x 1" Plate)	33.00 - 33.25	1.0000	1.0000
L24	38	Sabre MS-600 (6" x 1" Plate)	33.00 - 33.25	1.0000	1.0000
L24	40	Sabre MS-450 (4.5" x 1" Plate)	33.00 - 33.25	1.0000	1.0000
L24	41	Sabre MS-450 (4.5" x 1" Plate)	33.00 - 33.25	1.0000	1.0000
L24	42	Sabre MS-450 (4.5" x 1" Plate)	33.00 - 33.25	1.0000	1.0000
L24	48	CCI-5x1.25	33.00 - 33.25	1.0000	1.0000
L25	21	AVA7-50(1-5/8)	32.75 - 33.00	1.0000	1.0000
L25	27	Safety Line 3/8	32.75 - 33.00	1.0000	1.0000
L25	36	Sabre MS-600 (6" x 1" Plate)	32.75 - 33.00	1.0000	1.0000
L25	37	Sabre MS-600 (6" x 1" Plate)	32.75 - 33.00	1.0000	1.0000
L25	38	Sabre MS-600 (6" x 1" Plate)	32.75 - 33.00	1.0000	1.0000
L25	40	Sabre MS-450 (4.5" x 1" Plate)	32.75 - 33.00	1.0000	1.0000
L25	41	Sabre MS-450 (4.5" x 1" Plate)	32.75 - 33.00	1.0000	1.0000
L25	42	Sabre MS-450 (4.5" x 1" Plate)	32.75 - 33.00	1.0000	1.0000
L25	48	CCI-5x1.25	32.75 - 33.00	1.0000	1.0000
L26	21	AVA7-50(1-5/8)	32.00 - 32.75	1.0000	1.0000
L26	27	Safety Line 3/8	32.00 - 32.75	1.0000	1.0000
L26	36	Sabre MS-600 (6" x 1" Plate)	32.00 - 32.75	1.0000	1.0000
L26	37	Sabre MS-600 (6" x 1" Plate)	32.00 - 32.75	1.0000	1.0000
L26	38	Sabre MS-600 (6" x 1" Plate)	32.00 - 32.75	1.0000	1.0000
L26	40	Sabre MS-450 (4.5" x 1" Plate)	32.00 - 32.75	1.0000	1.0000
L26	41	Sabre MS-450 (4.5" x 1" Plate)	32.00 - 32.75	1.0000	1.0000
L26	42	Sabre MS-450 (4.5" x 1" Plate)	32.00 - 32.75	1.0000	1.0000
L26	48	CCI-5x1.25	32.00 - 32.75	1.0000	1.0000
L27	21	AVA7-50(1-5/8)	31.75 - 32.00	1.0000	1.0000
L27	27	Safety Line 3/8	31.75 - 32.00	1.0000	1.0000
L27	36	Sabre MS-600 (6" x 1" Plate)	31.75 - 32.00	1.0000	1.0000
L27	37	Sabre MS-600 (6" x 1" Plate)	31.75 - 32.00	1.0000	1.0000
L27	38	Sabre MS-600 (6" x 1" Plate)	31.75 - 32.00	1.0000	1.0000
L27	40	Sabre MS-450 (4.5" x 1" Plate)	31.75 - 32.00	1.0000	1.0000
L27	41	Sabre MS-450 (4.5" x 1" Plate)	31.75 - 32.00	1.0000	1.0000

tnxTower

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Client
Crown Castle
Designed by
JD Prabhu

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L27	42	Sabre MS-450 (4.5" x 1" Plate)	31.75 - 32.00	1.0000	1.0000
L27	48	CCI-5x1.25	31.75 - 32.00	1.0000	1.0000
L28	21	AVA7-50(1-5/8)	28.50 - 31.75	1.0000	1.0000
L28	27	Safety Line 3/8	28.50 - 31.75	1.0000	1.0000
L28	30	Sabre MS-600 (6" x 1" Plate)	28.50 - 30.50	1.0000	1.0000
L28	31	Sabre MS-600 (6" x 1" Plate)	28.50 - 30.50	1.0000	1.0000
L28	34	Sabre MS-600 (6" x 1" Plate)	28.50 - 30.50	1.0000	1.0000
L28	36	Sabre MS-600 (6" x 1" Plate)	28.50 - 31.75	1.0000	1.0000
L28	37	Sabre MS-600 (6" x 1" Plate)	28.50 - 31.75	1.0000	1.0000
L28	38	Sabre MS-600 (6" x 1" Plate)	28.50 - 31.75	1.0000	1.0000
L28	40	Sabre MS-450 (4.5" x 1" Plate)	30.50 - 31.75	1.0000	1.0000
L28	41	Sabre MS-450 (4.5" x 1" Plate)	30.50 - 31.75	1.0000	1.0000
L28	42	Sabre MS-450 (4.5" x 1" Plate)	30.50 - 31.75	1.0000	1.0000
L28	48	CCI-5x1.25	31.00 - 31.75	1.0000	1.0000
L29	21	AVA7-50(1-5/8)	28.25 - 28.50	1.0000	1.0000
L29	27	Safety Line 3/8	28.25 - 28.50	1.0000	1.0000
L29	30	Sabre MS-600 (6" x 1" Plate)	28.25 - 28.50	1.0000	1.0000
L29	31	Sabre MS-600 (6" x 1" Plate)	28.25 - 28.50	1.0000	1.0000
L29	34	Sabre MS-600 (6" x 1" Plate)	28.25 - 28.50	1.0000	1.0000
L29	36	Sabre MS-600 (6" x 1" Plate)	28.25 - 28.50	1.0000	1.0000
L29	37	Sabre MS-600 (6" x 1" Plate)	28.25 - 28.50	1.0000	1.0000
L29	38	Sabre MS-600 (6" x 1" Plate)	28.25 - 28.50	1.0000	1.0000
L30	21	AVA7-50(1-5/8)	27.50 - 28.25	1.0000	1.0000
L30	27	Safety Line 3/8	27.50 - 28.25	1.0000	1.0000
L30	30	Sabre MS-600 (6" x 1" Plate)	27.50 - 28.25	1.0000	1.0000
L30	31	Sabre MS-600 (6" x 1" Plate)	27.50 - 28.25	1.0000	1.0000
L30	34	Sabre MS-600 (6" x 1" Plate)	27.50 - 28.25	1.0000	1.0000
L30	36	Sabre MS-600 (6" x 1" Plate)	27.50 - 28.25	1.0000	1.0000
L30	37	Sabre MS-600 (6" x 1" Plate)	27.50 - 28.25	1.0000	1.0000
L30	38	Sabre MS-600 (6" x 1" Plate)	27.50 - 28.25	1.0000	1.0000
L31	21	AVA7-50(1-5/8)	27.25 - 27.50	1.0000	1.0000
L31	27	Safety Line 3/8	27.25 - 27.50	1.0000	1.0000
L31	30	Sabre MS-600 (6" x 1" Plate)	27.25 - 27.50	1.0000	1.0000
L31	31	Sabre MS-600 (6" x 1" Plate)	27.25 - 27.50	1.0000	1.0000
L31	34	Sabre MS-600 (6" x 1" Plate)	27.25 - 27.50	1.0000	1.0000
L31	36	Sabre MS-600 (6" x 1" Plate)	27.25 - 27.50	1.0000	1.0000
L31	37	Sabre MS-600 (6" x 1" Plate)	27.25 - 27.50	1.0000	1.0000
L31	38	Sabre MS-600 (6" x 1" Plate)	27.25 - 27.50	1.0000	1.0000
L32	21	AVA7-50(1-5/8)	22.25 - 27.25	1.0000	1.0000
L32	27	Safety Line 3/8	22.25 - 27.25	1.0000	1.0000
L32	30	Sabre MS-600 (6" x 1" Plate)	22.25 - 27.25	1.0000	1.0000
L32	31	Sabre MS-600 (6" x 1" Plate)	22.25 - 27.25	1.0000	1.0000
L32	34	Sabre MS-600 (6" x 1" Plate)	22.25 - 27.25	1.0000	1.0000
L32	36	Sabre MS-600 (6" x 1" Plate)	25.50 - 27.25	1.0000	1.0000
L32	37	Sabre MS-600 (6" x 1" Plate)	25.50 - 27.25	1.0000	1.0000
L32	38	Sabre MS-600 (6" x 1" Plate)	25.50 - 27.25	1.0000	1.0000
L33	21	AVA7-50(1-5/8)	18.00 - 22.25	1.0000	1.0000
L33	27	Safety Line 3/8	18.00 - 22.25	1.0000	1.0000
L33	30	Sabre MS-600 (6" x 1" Plate)	18.00 - 22.25	1.0000	1.0000
L33	31	Sabre MS-600 (6" x 1" Plate)	18.00 - 22.25	1.0000	1.0000
L33	34	Sabre MS-600 (6" x 1" Plate)	18.00 - 22.25	1.0000	1.0000
L33	46	CCI-5x1.25	18.00 - 20.00	1.0000	1.0000
L34	21	AVA7-50(1-5/8)	17.75 - 18.00	1.0000	1.0000
L34	27	Safety Line 3/8	17.75 - 18.00	1.0000	1.0000
L34	30	Sabre MS-600 (6" x 1" Plate)	17.75 - 18.00	1.0000	1.0000
L34	31	Sabre MS-600 (6" x 1" Plate)	17.75 - 18.00	1.0000	1.0000
L34	34	Sabre MS-600 (6" x 1" Plate)	17.75 - 18.00	1.0000	1.0000
L34	46	CCI-5x1.25	17.75 - 18.00	1.0000	1.0000

tnxTower

B+T Group
1717 S. Boulder, Suite 300
Tulsa, OK 74119
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Client
Crown Castle

Designed by
JD Prabhu

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L35	21	AVA7-50(1-5/8)	15.45 - 17.75	1.0000	1.0000
L35	27	Safety Line 3/8	15.45 - 17.75	1.0000	1.0000
L35	30	Sabre MS-600 (6" x 1" Plate)	15.45 - 17.75	1.0000	1.0000
L35	31	Sabre MS-600 (6" x 1" Plate)	15.45 - 17.75	1.0000	1.0000
L35	33	Sabre MS-600 (6" x 1" Plate)	15.45 - 17.45	1.0000	1.0000
L35	34	Sabre MS-600 (6" x 1" Plate)	15.45 - 17.75	1.0000	1.0000
L35	46	CCI-5x1.25	15.45 - 17.75	1.0000	1.0000
L36	21	AVA7-50(1-5/8)	15.20 - 15.45	1.0000	1.0000
L36	27	Safety Line 3/8	15.20 - 15.45	1.0000	1.0000
L36	30	Sabre MS-600 (6" x 1" Plate)	15.20 - 15.45	1.0000	1.0000
L36	31	Sabre MS-600 (6" x 1" Plate)	15.20 - 15.45	1.0000	1.0000
L36	33	Sabre MS-600 (6" x 1" Plate)	15.20 - 15.45	1.0000	1.0000
L36	34	Sabre MS-600 (6" x 1" Plate)	15.20 - 15.45	1.0000	1.0000
L36	46	CCI-5x1.25	15.20 - 15.45	1.0000	1.0000
L37	21	AVA7-50(1-5/8)	13.41 - 15.20	1.0000	1.0000
L37	27	Safety Line 3/8	13.41 - 15.20	1.0000	1.0000
L37	30	Sabre MS-600 (6" x 1" Plate)	13.41 - 15.20	1.0000	1.0000
L37	31	Sabre MS-600 (6" x 1" Plate)	13.41 - 15.20	1.0000	1.0000
L37	33	Sabre MS-600 (6" x 1" Plate)	13.41 - 15.20	1.0000	1.0000
L37	34	Sabre MS-600 (6" x 1" Plate)	13.41 - 15.20	1.0000	1.0000
L37	46	CCI-5x1.25	13.41 - 15.20	1.0000	1.0000
L38	21	AVA7-50(1-5/8)	13.16 - 13.41	1.0000	1.0000
L38	27	Safety Line 3/8	13.16 - 13.41	1.0000	1.0000
L38	30	Sabre MS-600 (6" x 1" Plate)	13.16 - 13.41	1.0000	1.0000
L38	31	Sabre MS-600 (6" x 1" Plate)	13.16 - 13.41	1.0000	1.0000
L38	33	Sabre MS-600 (6" x 1" Plate)	13.16 - 13.41	1.0000	1.0000
L38	34	Sabre MS-600 (6" x 1" Plate)	13.16 - 13.41	1.0000	1.0000
L38	46	CCI-5x1.25	13.16 - 13.41	1.0000	1.0000
L39	21	AVA7-50(1-5/8)	8.16 - 13.16	1.0000	1.0000
L39	27	Safety Line 3/8	8.16 - 13.16	1.0000	1.0000
L39	30	Sabre MS-600 (6" x 1" Plate)	8.16 - 13.16	1.0000	1.0000
L39	31	Sabre MS-600 (6" x 1" Plate)	8.16 - 13.16	1.0000	1.0000
L39	32	Sabre MS-600 (6" x 1" Plate)	8.16 - 8.50	1.0000	1.0000
L39	33	Sabre MS-600 (6" x 1" Plate)	8.16 - 13.16	1.0000	1.0000
L39	34	Sabre MS-600 (6" x 1" Plate)	11.41 - 13.16	1.0000	1.0000
L39	46	CCI-5x1.25	8.16 - 13.16	1.0000	1.0000
L40	21	AVA7-50(1-5/8)	6.50 - 8.16	1.0000	1.0000
L40	27	Safety Line 3/8	6.50 - 8.16	1.0000	1.0000
L40	30	Sabre MS-600 (6" x 1" Plate)	6.50 - 8.16	1.0000	1.0000
L40	31	Sabre MS-600 (6" x 1" Plate)	6.50 - 8.16	1.0000	1.0000
L40	32	Sabre MS-600 (6" x 1" Plate)	6.50 - 8.16	1.0000	1.0000
L40	33	Sabre MS-600 (6" x 1" Plate)	6.50 - 8.16	1.0000	1.0000
L40	46	CCI-5x1.25	6.50 - 8.16	1.0000	1.0000
L41	21	AVA7-50(1-5/8)	6.25 - 6.50	1.0000	1.0000
L41	27	Safety Line 3/8	6.25 - 6.50	1.0000	1.0000
L41	30	Sabre MS-600 (6" x 1" Plate)	6.25 - 6.50	1.0000	1.0000
L41	31	Sabre MS-600 (6" x 1" Plate)	6.25 - 6.50	1.0000	1.0000
L41	32	Sabre MS-600 (6" x 1" Plate)	6.25 - 6.50	1.0000	1.0000
L41	33	Sabre MS-600 (6" x 1" Plate)	6.25 - 6.50	1.0000	1.0000
L41	46	CCI-5x1.25	6.25 - 6.50	1.0000	1.0000
L42	21	AVA7-50(1-5/8)	4.45 - 6.25	1.0000	1.0000
L42	27	Safety Line 3/8	4.45 - 6.25	1.0000	1.0000
L42	30	Sabre MS-600 (6" x 1" Plate)	4.45 - 6.25	1.0000	1.0000
L42	31	Sabre MS-600 (6" x 1" Plate)	4.45 - 6.25	1.0000	1.0000
L42	32	Sabre MS-600 (6" x 1" Plate)	4.45 - 6.25	1.0000	1.0000
L42	33	Sabre MS-600 (6" x 1" Plate)	4.45 - 6.25	1.0000	1.0000
L42	46	CCI-5x1.25	4.45 - 6.25	1.0000	1.0000
L43	21	AVA7-50(1-5/8)	4.20 - 4.45	1.0000	1.0000
L43	27	Safety Line 3/8	4.20 - 4.45	1.0000	1.0000
L43	30	Sabre MS-600 (6" x 1" Plate)	4.20 - 4.45	1.0000	1.0000
L43	31	Sabre MS-600 (6" x 1" Plate)	4.20 - 4.45	1.0000	1.0000
L43	32	Sabre MS-600 (6" x 1" Plate)	4.20 - 4.45	1.0000	1.0000

tnxTower B+T Group 1717 S, Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job 127643.006.01 - (F) E. GRANBY 4Q2000 / GALASSO, CT (BU# 876399)	Page 20 of 45
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Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L43	33	Sabre MS-600 (6" x 1" Plate)	4.20 - 4.45	1.0000	1.0000
L43	46	CCI-5x1.25	4.20 - 4.45	1.0000	1.0000
L44	21	AVA7-50(1-5/8)	0.00 - 4.20	1.0000	1.0000
L44	27	Safety Line 3/8	0.00 - 4.20	1.0000	1.0000
L44	30	Sabre MS-600 (6" x 1" Plate)	0.00 - 4.20	1.0000	1.0000
L44	31	Sabre MS-600 (6" x 1" Plate)	0.00 - 4.20	1.0000	1.0000
L44	32	Sabre MS-600 (6" x 1" Plate)	0.50 - 4.20	1.0000	1.0000
L44	33	Sabre MS-600 (6" x 1" Plate)	2.45 - 4.20	1.0000	1.0000
L44	46	CCI-5x1.25	0.00 - 4.20	1.0000	1.0000

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K	
APXVTM14-ALU-I20 w/ Mount Pipe (P)	A	From Leg	4.000	0.000	94.000	No Ice	4.090	2.860	0.077
			0.000			1/2" Ice	4.480	3.230	0.127
			3.000			1" Ice	4.880	3.610	0.185
						2" Ice	5.710	4.400	0.331
APXVTM14-ALU-I20 w/ Mount Pipe (P)	B	From Leg	4.000	0.000	94.000	No Ice	4.090	2.860	0.077
			0.000			1/2" Ice	4.480	3.230	0.127
			3.000			1" Ice	4.880	3.610	0.185
						2" Ice	5.710	4.400	0.331
APXVTM14-ALU-I20 w/ Mount Pipe (P)	C	From Leg	4.000	0.000	94.000	No Ice	4.090	2.860	0.077
			0.000			1/2" Ice	4.480	3.230	0.127
			3.000			1" Ice	4.880	3.610	0.185
						2" Ice	5.710	4.400	0.331
NNVV-65B-R4 w/ Mount Pipe (P)	A	From Leg	4.000	0.000	94.000	No Ice	12.509	7.413	0.103
			0.000			1/2" Ice	13.108	8.598	0.194
			3.000			1" Ice	13.672	9.496	0.293
						2" Ice	14.822	11.328	0.520
NNVV-65B-R4 w/ Mount Pipe (P)	B	From Leg	4.000	0.000	94.000	No Ice	12.509	7.413	0.103
			0.000			1/2" Ice	13.108	8.598	0.194
			3.000			1" Ice	13.672	9.496	0.293
						2" Ice	14.822	11.328	0.520
NNVV-65B-R4 w/ Mount Pipe (P)	C	From Leg	4.000	0.000	94.000	No Ice	12.509	7.413	0.103
			0.000			1/2" Ice	13.108	8.598	0.194
			3.000			1" Ice	13.672	9.496	0.293
						2" Ice	14.822	11.328	0.520
(2) RRH2X50-800 (P)	A	From Leg	4.000	0.000	94.000	No Ice	0.000	1.282	0.053
			0.000			1/2" Ice	0.000	1.428	0.070
			3.000			1" Ice	0.000	1.580	0.090
						2" Ice	0.000	1.908	0.138
(2) RRH2X50-800 (P)	B	From Leg	4.000	0.000	94.000	No Ice	0.000	1.282	0.053
			0.000			1/2" Ice	0.000	1.428	0.070
			3.000			1" Ice	0.000	1.580	0.090
						2" Ice	0.000	1.908	0.138
(2) RRH2X50-800 (P)	C	From Leg	4.000	0.000	94.000	No Ice	0.000	1.282	0.053
			0.000			1/2" Ice	0.000	1.428	0.070
			3.000			1" Ice	0.000	1.580	0.090
						2" Ice	0.000	1.908	0.138

tnxTower B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job 127643.006.01 - (F) E. GRANBY 4Q2000 / GALASSO, CT (BU# 876399)	Page 21 of 45
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	Client Crown Castle	Designed by JD Prabhu

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K	
			Horz Lateral ft	Vert ft						
PCS 1900MHz 4x45W-65MHz (P)	A	From Leg	4.000	0.000	0.000	94.000	No Ice	0.000	2.238	0.060
			0.000				1/2" Ice	0.000	2.441	0.083
			3.000				1" Ice	0.000	2.651	0.110
							2" Ice	0.000	3.093	0.173
PCS 1900MHz 4x45W-65MHz (P)	B	From Leg	4.000	0.000	0.000	94.000	No Ice	0.000	2.238	0.060
			0.000				1/2" Ice	0.000	2.441	0.083
			3.000				1" Ice	0.000	2.651	0.110
							2" Ice	0.000	3.093	0.173
PCS 1900MHz 4x45W-65MHz (P)	C	From Leg	4.000	0.000	0.000	94.000	No Ice	0.000	2.238	0.060
			0.000				1/2" Ice	0.000	2.441	0.083
			3.000				1" Ice	0.000	2.651	0.110
							2" Ice	0.000	3.093	0.173
TD-RRH8x20-25 (P)	A	From Leg	4.000	0.000	0.000	94.000	No Ice	4.045	1.535	0.070
			0.000				1/2" Ice	4.298	1.714	0.097
			3.000				1" Ice	4.557	1.901	0.128
							2" Ice	5.098	2.295	0.201
TD-RRH8x20-25 (P)	B	From Leg	4.000	0.000	0.000	94.000	No Ice	4.045	1.535	0.070
			0.000				1/2" Ice	4.298	1.714	0.097
			3.000				1" Ice	4.557	1.901	0.128
							2" Ice	5.098	2.295	0.201
TD-RRH8x20-25 (P)	C	From Leg	4.000	0.000	0.000	94.000	No Ice	4.045	1.535	0.070
			0.000				1/2" Ice	4.298	1.714	0.097
			3.000				1" Ice	4.557	1.901	0.128
							2" Ice	5.098	2.295	0.201
6' x 2" Mount Pipe (E-per Photo)	A	From Leg	4.000	0.000	0.000	94.000	No Ice	1.425	1.425	0.022
			0.000				1/2" Ice	1.925	1.925	0.033
			0.000				1" Ice	2.294	2.294	0.048
							2" Ice	3.060	3.060	0.090
6' x 2" Mount Pipe (E-per Photo)	B	From Leg	4.000	0.000	0.000	94.000	No Ice	1.425	1.425	0.022
			0.000				1/2" Ice	1.925	1.925	0.033
			0.000				1" Ice	2.294	2.294	0.048
							2" Ice	3.060	3.060	0.090
6' x 2" Mount Pipe (E-per Photo)	C	From Leg	4.000	0.000	0.000	94.000	No Ice	1.425	1.425	0.022
			0.000				1/2" Ice	1.925	1.925	0.033
			0.000				1" Ice	2.294	2.294	0.048
							2" Ice	3.060	3.060	0.090
4'x2" Horizontal Mount Pipe (P-Handrail Support)	A	From Leg	2.000	0.000	0.000	94.000	No Ice	0.866	0.866	0.060
			0.000				1/2" Ice	1.111	1.111	0.068
			4.000				1" Ice	1.365	1.365	0.078
							2" Ice	1.901	1.901	0.107
4'x2" Horizontal Mount Pipe (P-Handrail Support)	B	From Leg	2.000	0.000	0.000	94.000	No Ice	0.866	0.866	0.060
			0.000				1/2" Ice	1.111	1.111	0.068
			4.000				1" Ice	1.365	1.365	0.078
							2" Ice	1.901	1.901	0.107
4'x2" Horizontal Mount Pipe (P-Handrail Support)	C	From Leg	2.000	0.000	0.000	94.000	No Ice	0.866	0.866	0.060
			0.000				1/2" Ice	1.111	1.111	0.068
			4.000				1" Ice	1.365	1.365	0.078
							2" Ice	1.901	1.901	0.107
Miscellaneous [NA 510-1] (P-Handrail Kit)	C	None			0.000	98.000	No Ice	6.360	6.360	0.256
							1/2" Ice	8.520	8.520	0.344
							1" Ice	10.620	10.620	0.459
							2" Ice	14.640	14.640	0.769
Platform Mount [LP 714-1] (E-Area changed to 14' Mount)	C	None			0.000	94.000	No Ice	34.972	34.972	1.493
							1/2" Ice	41.281	41.281	1.904
							1" Ice	47.590	47.590	2.315
							2" Ice	60.208	60.208	3.137

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tnxTower B+T Group 1717 S, Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job		127643.006.01 - (F) E. GRANBY 4Q2000 / GALASSO, CT (BU# 876399)		Page		23 of 45	
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	Client		Crown Castle		Designed by		JD Prabhu	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment °	Placement ft	C _{AA}		Weight K
			Horz Lateral ft	Vert ft			Front ft ²	Side ft ²	
(E)			0.000			1/2" Ice	1.028	1.028	0.035
			0.000			1" Ice	1.281	1.281	0.044
						2" Ice	1.814	1.814	0.072
Platform Mount [LP 305-1] (E)	C	None		0.000	89.000	No Ice	18.040	18.040	1.121
						1/2" Ice	22.040	22.040	1.470
						1" Ice	26.060	26.060	1.882
						2" Ice	34.160	34.160	2.896
_									
TPA-65R-LCUUUU-H8 w/ Mount Pipe (R)	A	From Leg	4.000 0.000 3.000	0.000	74.000	No Ice	11.850	8.990	0.115
						1/2" Ice	12.770	9.880	0.210
						1" Ice	13.710	10.790	0.319
						2" Ice	15.640	12.660	0.580
TPA-65R-LCUUUU-H8 w/ Mount Pipe (R)	B	From Leg	4.000 0.000 3.000	0.000	74.000	No Ice	11.850	8.990	0.115
						1/2" Ice	12.770	9.880	0.210
						1" Ice	13.710	10.790	0.319
						2" Ice	15.640	12.660	0.580
TPA-65R-LCUUUU-H8 w/ Mount Pipe (R)	C	From Leg	4.000 0.000 3.000	0.000	74.000	No Ice	11.850	8.990	0.115
						1/2" Ice	12.770	9.880	0.210
						1" Ice	13.710	10.790	0.319
						2" Ice	15.640	12.660	0.580
SBNH-1D6565C w/ Mount Pipe (R)	B	From Leg	4.000 0.000 3.000	0.000	74.000	No Ice	5.560	4.470	0.085
						1/2" Ice	6.070	4.970	0.167
						1" Ice	6.590	5.470	0.262
						2" Ice	7.650	6.520	0.495
RRUS 11 B12 (R)	A	From Leg	4.000 0.000 3.000	0.000	74.000	No Ice	2.833	1.182	0.051
						1/2" Ice	3.043	1.330	0.072
						1" Ice	3.259	1.485	0.095
						2" Ice	3.715	1.826	0.153
RRUS 11 B12 (R)	B	From Leg	4.000 0.000 3.000	0.000	74.000	No Ice	2.833	1.182	0.051
						1/2" Ice	3.043	1.330	0.072
						1" Ice	3.259	1.485	0.095
						2" Ice	3.715	1.826	0.153
RRUS 11 B12 (R)	C	From Leg	4.000 0.000 3.000	0.000	74.000	No Ice	2.833	1.182	0.051
						1/2" Ice	3.043	1.330	0.072
						1" Ice	3.259	1.485	0.095
						2" Ice	3.715	1.826	0.153
RRUS 32 B30 (R)	A	From Leg	4.000 0.000 3.000	0.000	74.000	No Ice	2.692	1.573	0.060
						1/2" Ice	2.912	1.756	0.080
						1" Ice	3.138	1.945	0.104
						2" Ice	3.614	2.346	0.161
RRUS 32 B30 (R)	B	From Leg	4.000 0.000 3.000	0.000	74.000	No Ice	2.692	1.573	0.060
						1/2" Ice	2.912	1.756	0.080
						1" Ice	3.138	1.945	0.104
						2" Ice	3.614	2.346	0.161
RRUS 32 B30 (R)	C	From Leg	4.000 0.000 3.000	0.000	74.000	No Ice	2.692	1.573	0.060
						1/2" Ice	2.912	1.756	0.080
						1" Ice	3.138	1.945	0.104
						2" Ice	3.614	2.346	0.161
RRUS 32 B2 (R)	A	From Leg	4.000 0.000 3.000	0.000	74.000	No Ice	2.731	1.668	0.053
						1/2" Ice	2.953	1.855	0.074
						1" Ice	3.182	2.049	0.098
						2" Ice	3.663	2.458	0.157
RRUS 32 B2 (R)	B	From Leg	4.000 0.000 3.000	0.000	74.000	No Ice	2.731	1.668	0.053
						1/2" Ice	2.953	1.855	0.074
						1" Ice	3.182	2.049	0.098
						2" Ice	3.663	2.458	0.157
RRUS 32 B2	C	From Leg	4.000	0.000	74.000	No Ice	2.731	1.668	0.053

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
			Horz Lateral ft	Vert ft					
(R)			0.000			1/2" Ice	2.953	1.855	0.074
			3.000			1" Ice	3.182	2.049	0.098
						2" Ice	3.663	2.458	0.157
DBC0061F1V51-2 (R)	A	From Leg	4.000	0.000	74.000	No Ice	0.433	0.413	0.025
			0.000			1/2" Ice	0.518	0.496	0.031
			3.000			1" Ice	0.609	0.586	0.038
						2" Ice	0.815	0.788	0.057
DBC0061F1V51-2 (R)	B	From Leg	4.000	0.000	74.000	No Ice	0.433	0.413	0.025
			0.000			1/2" Ice	0.518	0.496	0.031
			3.000			1" Ice	0.609	0.586	0.038
						2" Ice	0.815	0.788	0.057
DBC0061F1V51-2 (R)	C	From Leg	4.000	0.000	74.000	No Ice	0.433	0.413	0.025
			0.000			1/2" Ice	0.518	0.496	0.031
			3.000			1" Ice	0.609	0.586	0.038
						2" Ice	0.815	0.788	0.057
DC6-48-60-18-8F (R)	A	From Leg	4.000	0.000	74.000	No Ice	1.212	1.212	0.033
			0.000			1/2" Ice	1.892	1.892	0.055
			3.000			1" Ice	2.105	2.105	0.080
						2" Ice	2.570	2.570	0.138
7770.00 w/ Mount Pipe (E)	A	From Leg	4.000	0.000	74.000	No Ice	5.746	4.254	0.055
			0.000			1/2" Ice	6.179	5.014	0.103
			3.000			1" Ice	6.607	5.711	0.157
						2" Ice	7.488	7.155	0.287
7770.00 w/ Mount Pipe (E)	B	From Leg	4.000	0.000	74.000	No Ice	5.746	4.254	0.055
			0.000			1/2" Ice	6.179	5.014	0.103
			3.000			1" Ice	6.607	5.711	0.157
						2" Ice	7.488	7.155	0.287
7770.00 w/ Mount Pipe (E)	C	From Leg	4.000	0.000	74.000	No Ice	5.746	4.254	0.055
			0.000			1/2" Ice	6.179	5.014	0.103
			3.000			1" Ice	6.607	5.711	0.157
						2" Ice	7.488	7.155	0.287
P65-17-XLH-RR w/ Mount Pipe (E)	A	From Leg	4.000	0.000	74.000	No Ice	11.704	8.938	0.092
			0.000			1/2" Ice	12.424	10.450	0.178
			3.000			1" Ice	13.153	11.986	0.273
						2" Ice	14.517	14.313	0.498
P65-17-XLH-RR w/ Mount Pipe (E)	C	From Leg	4.000	0.000	74.000	No Ice	11.704	8.938	0.092
			0.000			1/2" Ice	12.424	10.450	0.178
			3.000			1" Ice	13.153	11.986	0.273
						2" Ice	14.517	14.313	0.498
TT19-08BP111-001 (E)	A	From Leg	4.000	0.000	74.000	No Ice	0.545	0.442	0.016
			0.000			1/2" Ice	0.641	0.530	0.022
			3.000			1" Ice	0.743	0.626	0.029
						2" Ice	0.971	0.840	0.049
TT19-08BP111-001 (E)	B	From Leg	4.000	0.000	74.000	No Ice	0.545	0.442	0.016
			0.000			1/2" Ice	0.641	0.530	0.022
			3.000			1" Ice	0.743	0.626	0.029
						2" Ice	0.971	0.840	0.049
TT19-08BP111-001 (E)	C	From Leg	4.000	0.000	74.000	No Ice	0.545	0.442	0.016
			0.000			1/2" Ice	0.641	0.530	0.022
			3.000			1" Ice	0.743	0.626	0.029
						2" Ice	0.971	0.840	0.049
DC6-48-60-18-8F (E)	B	From Leg	2.000	0.000	74.000	No Ice	1.212	1.212	0.033
			0.000			1/2" Ice	1.892	1.892	0.055
			3.000			1" Ice	2.105	2.105	0.080
						2" Ice	2.570	2.570	0.138
3' x 2" Pipe Mount (E-For DC6 Per Photo)	B	From Leg	1.000	0.000	74.000	No Ice	0.583	0.583	0.011
			0.000			1/2" Ice	0.770	0.770	0.017

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
			Horz Lateral ft	Vert ft					
				0.000					
						1" Ice	0.967	0.967	0.024
						2" Ice	1.388	1.388	0.047
Platform Mount [LP 303-1] (E)	C	None		0.000	74.000	No Ice	14.690	14.690	1.250
						1/2" Ice	18.010	18.010	1.569
						1" Ice	21.340	21.340	1.942
						2" Ice	28.080	28.080	2.852
Miscellaneous [NA 507-1] (R-Per e-mail)	C	None		0.000	76.000	No Ice	4.560	4.560	0.245
						1/2" Ice	6.390	6.390	0.311
						1" Ice	8.180	8.180	0.402
						2" Ice	11.660	11.660	0.657
_									
(2) SBNHH-1D65B w/ Mount Pipe (R)	A	From Leg	4.000 0.000 0.000	0.000	67.000	No Ice	4.090	3.300	0.066
						1/2" Ice	4.490	3.680	0.130
						1" Ice	4.890	4.070	0.204
						2" Ice	5.720	4.870	0.386
(2) SBNHH-1D65B w/ Mount Pipe (R)	B	From Leg	4.000 0.000 0.000	0.000	67.000	No Ice	4.090	3.300	0.066
						1/2" Ice	4.490	3.680	0.130
						1" Ice	4.890	4.070	0.204
						2" Ice	5.720	4.870	0.386
(2) SBNHH-1D65B w/ Mount Pipe (R)	C	From Leg	4.000 0.000 0.000	0.000	67.000	No Ice	4.090	3.300	0.066
						1/2" Ice	4.490	3.680	0.130
						1" Ice	4.890	4.070	0.204
						2" Ice	5.720	4.870	0.386
B13 RRH 4X30 (R)	A	From Leg	4.000 0.000 0.000	0.000	67.000	No Ice	2.055	1.320	0.056
						1/2" Ice	2.241	1.475	0.073
						1" Ice	2.433	1.638	0.093
						2" Ice	2.841	1.997	0.142
B13 RRH 4X30 (R)	B	From Leg	4.000 0.000 0.000	0.000	67.000	No Ice	2.055	1.320	0.056
						1/2" Ice	2.241	1.475	0.073
						1" Ice	2.433	1.638	0.093
						2" Ice	2.841	1.997	0.142
B13 RRH 4X30 (R)	C	From Leg	4.000 0.000 0.000	0.000	67.000	No Ice	2.055	1.320	0.056
						1/2" Ice	2.241	1.475	0.073
						1" Ice	2.433	1.638	0.093
						2" Ice	2.841	1.997	0.142
B66A RRH4X45 (R)	A	From Leg	4.000 0.000 0.000	0.000	67.000	No Ice	2.580	1.630	0.057
						1/2" Ice	2.794	1.811	0.077
						1" Ice	3.015	1.999	0.101
						2" Ice	3.479	2.396	0.158
B66A RRH4X45 (R)	B	From Leg	4.000 0.000 0.000	0.000	67.000	No Ice	2.580	1.630	0.057
						1/2" Ice	2.794	1.811	0.077
						1" Ice	3.015	1.999	0.101
						2" Ice	3.479	2.396	0.158
B66A RRH4X45 (R)	C	From Leg	4.000 0.000 0.000	0.000	67.000	No Ice	2.580	1.630	0.057
						1/2" Ice	2.794	1.811	0.077
						1" Ice	3.015	1.999	0.101
						2" Ice	3.479	2.396	0.158
RC2DC-3315-PF-48 (R)	A	From Leg	4.000 0.000 0.000	0.000	67.000	No Ice	3.792	2.512	0.032
						1/2" Ice	4.044	2.725	0.063
						1" Ice	4.303	2.945	0.099
						2" Ice	4.844	3.414	0.181
RC2DC-3315-PF-48 (R)	B	From Leg	4.000 0.000 0.000	0.000	67.000	No Ice	3.792	2.512	0.032
						1/2" Ice	4.044	2.725	0.063
						1" Ice	4.303	2.945	0.099
						2" Ice	4.844	3.414	0.181
(2) LPA-80063/6CFX2 w/ Mount Pipe	A	From Leg	4.000 0.000	0.000	67.000	No Ice	9.831	10.215	0.052
						1/2" Ice	10.400	11.384	0.145

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
			Horz ft	Lateral ft					
(E)			0.000						
(2) LPA-80063/6CFX2 w/ Mount Pipe (E)	B	From Leg	4.000	0.000	67.000	1" Ice	10.933	12.269	0.246
			0.000			2" Ice	12.026	14.086	0.476
			0.000			No Ice	9.831	10.215	0.052
			0.000			1/2" Ice	10.400	11.384	0.145
(2) LPA-80063/6CFX2 w/ Mount Pipe (E)	C	From Leg	4.000	0.000	67.000	1" Ice	10.933	12.269	0.246
			0.000			2" Ice	12.026	14.086	0.476
			0.000			No Ice	9.831	10.215	0.052
			0.000			1/2" Ice	10.400	11.384	0.145
Platform Mount [LP 303-1] (E)	C	None		0.000	67.000	1" Ice	10.933	12.269	0.246
						2" Ice	12.026	14.086	0.476
						No Ice	14.690	14.690	1.250
						1/2" Ice	18.010	18.010	1.569
Miscellaneous [NA 509-3] (E-Photo)	C	None		0.000	67.000	1" Ice	21.340	21.340	1.942
						2" Ice	28.080	28.080	2.852
						No Ice	11.840	11.840	0.275
						1/2" Ice	16.960	16.960	0.296
***_ KS24019-L112A (E)	C	From Leg	3.000	0.000	52.000	1" Ice	22.080	22.080	0.317
			0.000			2" Ice	32.320	32.320	0.360
			2.000			No Ice	0.141	0.141	0.005
						1/2" Ice	0.198	0.198	0.007
Side Arm Mount [SO 701-1] (E)	C	From Leg	1.500	0.000	52.000	1" Ice	0.262	0.262	0.009
			0.000			2" Ice	0.415	0.415	0.018
			0.000			No Ice	0.850	1.670	0.065
						1/2" Ice	1.140	2.340	0.079
***_ ***_						1" Ice	1.430	3.010	0.093
						2" Ice	2.010	4.350	0.121

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice

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Comb. No.	Description
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	98 - 93	Pole	Max Tension	26	0.000	0.000	-0.000
			Max. Compression	26	-9.655	-0.005	-0.012
			Max. Mx	20	-3.381	14.034	0.001
			Max. My	14	-3.375	0.001	-14.040
			Max. Vy	20	-5.127	14.034	0.001
			Max. Vx	2	-5.132	0.000	14.040
			Max. Torque	24			0.002
L2	93 - 88	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-16.920	-0.027	-0.016
			Max. Mx	20	-5.762	44.922	0.005
			Max. My	2	-5.750	-0.000	44.955
			Max. Vy	20	-8.312	44.922	0.005
			Max. Vx	2	-8.320	-0.000	44.955
			Max. Torque	24			0.002
L3	88 - 82.79	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-17.170	-0.046	-0.015
			Max. Mx	20	-5.900	68.277	0.008
			Max. My	2	-5.888	-0.001	68.333
			Max. Vy	20	-8.420	68.277	0.008
			Max. Vx	2	-8.429	-0.001	68.333
			Max. Torque	24			0.002

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L4	82.79 - 80.207	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-17.864	-0.079	-0.013
			Max. Mx	20	-6.299	110.924	0.014
			Max. My	2	-6.287	-0.002	111.023
			Max. Vy	20	-8.639	110.924	0.014
			Max. Vx	2	-8.648	-0.002	111.023
			Max. Torque	24			0.002
L5	80.207 - 75.207	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-19.070	-0.114	-0.012
			Max. Mx	20	-6.925	154.755	0.020
			Max. My	2	-6.912	-0.004	154.899
			Max. Vy	20	-9.064	154.755	0.020
			Max. Vx	2	-9.073	-0.004	154.899
			Max. Torque	24			0.002
L6	75.207 - 70.207	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-28.700	-0.524	0.428
			Max. Mx	8	-10.198	-226.811	0.301
			Max. My	2	-10.171	-0.312	227.332
			Max. Vy	20	-13.561	226.712	-0.057
			Max. Vx	2	-13.605	-0.312	227.332
			Max. Torque	17			-0.677
L7	70.207 - 65.207	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-38.940	-1.361	0.890
			Max. Mx	8	-13.493	-303.660	0.487
			Max. My	2	-13.456	-0.551	304.389
			Max. Vy	20	-18.485	303.232	-0.061
			Max. Vx	2	-18.553	-0.551	304.389
			Max. Torque	21			-0.685
L8	65.207 - 60.207	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-39.877	-1.616	1.011
			Max. Mx	8	-14.197	-396.309	0.557
			Max. My	2	-14.139	-0.630	398.046
			Max. Vy	20	-18.598	395.809	-0.060
			Max. Vx	2	-18.933	-0.630	398.046
			Max. Torque	21			-0.685
L9	60.207 - 59.17	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-40.111	-1.668	1.036
			Max. Mx	8	-14.344	-415.609	0.571
			Max. My	2	-14.285	-0.648	417.709
			Max. Vy	20	-18.654	415.093	-0.060
			Max. Vx	2	-19.017	-0.648	417.709
			Max. Torque	21			-0.684
L10	59.17 - 58.9	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-40.187	-1.682	1.043
			Max. Mx	8	-14.414	-420.644	0.575
			Max. My	2	-14.354	-0.652	422.844
			Max. Vy	20	-18.657	420.125	-0.060
			Max. Vx	2	-19.029	-0.652	422.844
			Max. Torque	21			-0.684
L11	58.9 - 58.75	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-40.229	-1.690	1.047
			Max. Mx	8	-14.441	-423.444	0.577
			Max. My	2	-14.382	-0.655	425.699
			Max. Vy	20	-18.668	422.922	-0.060
			Max. Vx	2	-19.045	-0.655	425.699
			Max. Torque	21			-0.684
L12	58.75 - 54	Pole	Max Tension	1	0.000	0.000	0.000

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L13	54 - 53.75	Pole	Max. Compression	26	-41.601	-1.954	1.170
			Max. Mx	8	-15.269	-512.970	0.643
			Max. My	2	-15.202	-0.736	517.318
			Max. Vy	20	-19.050	512.412	-0.059
			Max. Vx	2	-19.540	-0.736	517.318
			Max. Torque	21			-0.684
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-41.685	-1.971	1.175
			Max. Mx	8	-15.333	-517.729	0.647
			Max. My	2	-15.267	-0.740	522.204
L14	53.75 - 52.91	Pole	Max. Vy	20	-19.061	517.172	-0.059
			Max. Vx	2	-19.552	-0.740	522.204
			Max. Torque	21			-0.683
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-41.968	-2.028	1.191
			Max. Mx	8	-15.499	-533.754	0.658
			Max. My	2	-15.432	-0.755	538.661
			Max. Vy	20	-19.142	533.203	-0.058
			Max. Vx	2	-19.632	-0.755	538.661
			Max. Torque	21			-0.683
L15	52.91 - 52.66	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-42.058	-2.046	1.196
			Max. Mx	8	-15.563	-538.534	0.662
			Max. My	2	-15.496	-0.759	543.571
			Max. Vy	20	-19.159	537.986	-0.058
			Max. Vx	2	-19.649	-0.759	543.571
			Max. Torque	21			-0.683
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-42.232	-2.079	1.205
			Max. Mx	8	-15.668	-547.917	0.669
L16	52.66 - 52.17	Pole	Max. My	2	-15.601	-0.768	553.212
			Max. Vy	20	-19.207	547.379	-0.058
			Max. Vx	2	-19.697	-0.768	553.212
			Max. Torque	21			-0.683
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-42.462	-1.774	1.024
			Max. Mx	8	-15.802	-552.547	0.572
			Max. My	2	-15.734	-0.601	558.051
			Max. Vy	20	-19.282	552.377	-0.156
			Max. Vx	2	-19.788	-0.601	558.051
L17	52.17 - 51.92	Pole	Max. Torque	21			-0.683
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-43.543	-1.992	1.096
			Max. Mx	8	-16.462	-614.823	0.571
			Max. My	2	-16.396	-0.614	622.136
			Max. Vy	20	-19.567	614.783	-0.108
			Max. Vx	2	-20.071	-0.614	622.136
			Max. Torque	19			-0.668
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-45.804	-2.287	1.203
L18	51.92 - 45.287	Pole	Max. Mx	20	-17.931	702.161	-0.043
			Max. My	2	-17.879	-0.632	711.812
			Max. Vy	20	-20.023	702.161	-0.043
			Max. Vx	2	-20.525	-0.632	711.812
			Max. Torque	19			-0.667
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-47.546	-2.606	1.361
			Max. Mx	20	-19.095	803.141	0.031
			Max. My	2	-19.049	-0.655	815.381
			Max. Vy	20	-19.049	-0.655	815.381
L19	45.287 - 44.287	Pole	Max. Vy	20	-19.049	-0.655	815.381
			Max. Vx	2	-19.049	-0.655	815.381
			Max. Torque	19			-0.667
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-47.546	-2.606	1.361
			Max. Mx	20	-19.095	803.141	0.031
			Max. My	2	-19.049	-0.655	815.381
			Max. Vy	20	-19.049	-0.655	815.381
			Max. Vx	2	-19.049	-0.655	815.381
			Max. Torque	19			-0.667
L20	44.287 - 39.287	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-47.546	-2.606	1.361
			Max. Mx	20	-19.095	803.141	0.031
			Max. My	2	-19.049	-0.655	815.381
			Max. Vy	20	-19.049	-0.655	815.381
			Max. Vx	2	-19.049	-0.655	815.381
			Max. Torque	19			-0.667
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-47.546	-2.606	1.361
			Max. Mx	20	-19.095	803.141	0.031

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L21	39.287 - 34.287	Pole	Max. Vy	20	-20.420	803.141	0.031
			Max. Vx	2	-20.919	-0.655	815.381
			Max. Torque	19			-0.667
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-49.361	-2.929	1.521
			Max. Mx	20	-20.289	906.113	0.106
			Max. My	2	-20.249	-0.679	920.929
			Max. Vy	20	-20.824	906.113	0.106
			Max. Vx	2	-21.318	-0.679	920.929
			Max. Torque	19			-0.667
L22	34.287 - 33.5	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-49.674	-2.980	1.546
			Max. Mx	20	-20.484	922.507	0.118
			Max. My	2	-20.444	-0.683	937.734
			Max. Vy	20	-20.891	922.507	0.118
			Max. Vx	2	-21.403	-0.683	937.734
			Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-49.792	-2.996	1.555
			Max. Mx	20	-20.570	927.727	0.122
L23	33.5 - 33.25	Pole	Max. My	2	-20.531	-0.684	943.087
			Max. Vy	20	-20.907	927.727	0.122
			Max. Vx	2	-21.426	-0.684	943.087
			Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-49.910	-3.013	1.563
			Max. Mx	20	-20.647	932.953	0.126
			Max. My	2	-20.607	-0.686	948.448
			Max. Vy	20	-20.933	932.953	0.126
			Max. Vx	2	-21.457	-0.686	948.448
L24	33.25 - 33	Pole	Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-50.022	-3.029	1.571
			Max. Mx	20	-20.718	938.186	0.129
			Max. My	2	-20.678	-0.687	953.817
			Max. Vy	20	-20.958	938.186	0.129
			Max. Vx	2	-21.488	-0.687	953.817
			Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-50.357	-3.077	1.595
L25	33 - 32.75	Pole	Max. Mx	20	-20.924	953.921	0.141
			Max. My	2	-20.884	-0.690	969.970
			Max. Vy	20	-21.039	953.921	0.141
			Max. Vx	2	-21.585	-0.690	969.970
			Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-50.455	-3.094	1.603
			Max. Mx	20	-20.987	959.179	0.145
			Max. My	2	-20.947	-0.692	975.370
			Max. Vy	20	-21.057	959.179	0.145
L26	32.75 - 32	Pole	Max. Vx	2	-21.610	-0.692	975.370
			Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-51.710	-3.276	1.691
			Max. Mx	20	-21.746	1028.017	0.194
			Max. My	2	-21.709	-0.709	1046.086
			Max. Vy	20	-21.350	1028.017	0.194
			Max. Vx	2	-21.916	-0.709	1046.086
			Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
L27	32 - 31.75	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-51.710	-3.276	1.691
			Max. Mx	20	-21.746	1028.017	0.194
			Max. My	2	-21.709	-0.709	1046.086
			Max. Vy	20	-21.350	1028.017	0.194
			Max. Vx	2	-21.916	-0.709	1046.086
			Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-51.710	-3.276	1.691
			Max. Mx	20	-21.746	1028.017	0.194
L28	31.75 - 28.5	Pole	Max. My	2	-21.709	-0.709	1046.086
			Max. Vy	20	-21.350	1028.017	0.194
			Max. Vx	2	-21.916	-0.709	1046.086
			Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-51.710	-3.276	1.691
			Max. Mx	20	-21.746	1028.017	0.194
			Max. My	2	-21.709	-0.709	1046.086
			Max. Vy	20	-21.350	1028.017	0.194
			Max. Vx	2	-21.916	-0.709	1046.086
L29	28.5 - 28.25	Pole	Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000

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L30	28.25 - 27.5	Pole	Max. Compression	26	-51.826	-3.290	1.697
			Max. Mx	20	-21.835	1033.350	0.198
			Max. My	2	-21.799	-0.711	1051.566
			Max. Vy	20	-21.361	1033.350	0.198
			Max. Vx	2	-21.929	-0.711	1051.566
			Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-52.171	-3.330	1.716
			Max. Mx	20	-22.059	1049.386	0.209
			Max. My	2	-22.022	-0.715	1068.044
L31	27.5 - 27.25	Pole	Max. Vy	20	-21.438	1049.386	0.209
			Max. Vx	2	-22.011	-0.715	1068.044
			Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-52.269	-3.344	1.723
			Max. Mx	20	-22.123	1054.743	0.213
			Max. My	2	-22.087	-0.716	1073.550
			Max. Vy	20	-21.455	1054.743	0.213
			Max. Vx	2	-22.030	-0.716	1073.550
			Max. Torque	19			-0.666
L32	27.25 - 22.25	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-54.083	-3.612	1.851
			Max. Mx	20	-23.326	1162.853	0.290
			Max. My	2	-23.295	-0.746	1184.704
			Max. Vy	20	-21.836	1162.853	0.290
			Max. Vx	2	-22.440	-0.746	1184.704
			Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-55.605	-3.846	1.965
			Max. Mx	20	-24.380	1256.142	0.356
L33	22.25 - 18	Pole	Max. My	2	-24.354	-0.772	1280.679
			Max. Vy	20	-22.126	1256.142	0.356
			Max. Vx	2	-22.745	-0.772	1280.679
			Max. Torque	19			-0.666
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-55.702	-3.862	1.973
			Max. Mx	20	-24.462	1261.668	0.360
			Max. My	2	-24.437	-0.774	1286.365
			Max. Vy	20	-22.131	1261.668	0.360
			Max. Vx	2	-22.749	-0.774	1286.365
L34	18 - 17.75	Pole	Max. Torque	19			-0.665
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-56.572	-4.022	2.014
			Max. Mx	20	-25.044	1312.696	0.395
			Max. My	2	-25.022	-0.789	1338.864
			Max. Vy	20	-22.297	1312.696	0.395
			Max. Vx	2	-22.916	-0.789	1338.864
			Max. Torque	19			-0.665
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-56.679	-4.040	2.018
L35	17.75 - 15.45	Pole	Max. Mx	20	-25.138	1318.263	0.399
			Max. My	2	-25.117	-0.791	1344.591
			Max. Vy	20	-22.296	1318.263	0.399
			Max. Vx	2	-22.915	-0.791	1344.591
			Max. Torque	19			-0.665
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-57.446	-4.167	2.046
			Max. Mx	20	-25.653	1358.268	0.427
			Max. My	2	-25.634	-0.803	1385.744
			Max. Vy	20	-22.458	1358.268	0.427
L36	15.45 - 15.2	Pole	Max. Vx	2	-23.078	-0.803	1385.744
			Max. Torque	19			-0.665
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-57.446	-4.167	2.046
			Max. Mx	20	-25.653	1358.268	0.427
			Max. My	2	-25.634	-0.803	1385.744
L37	15.2 - 13.41	Pole	Max. Vy	20	-22.458	1358.268	0.427
			Max. Vx	2	-23.078	-0.803	1385.744
			Max. Torque	19			-0.665
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-57.446	-4.167	2.046
			Max. Mx	20	-25.653	1358.268	0.427

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L38	13.41 - 13.16	Pole	Max. Torque	12			0.692
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-57.548	-4.185	2.050
			Max. Mx	20	-25.741	1363.875	0.431
			Max. My	2	-25.723	-0.804	1391.511
			Max. Vy	20	-22.456	1363.875	0.431
			Max. Vx	2	-23.077	-0.804	1391.511
L39	13.16 - 8.16	Pole	Max. Torque	12			0.696
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-59.528	-4.497	2.176
			Max. Mx	20	-27.140	1476.928	0.510
			Max. My	2	-27.128	-0.839	1507.779
			Max. Vy	20	-22.816	1476.928	0.510
			Max. Vx	2	-23.438	-0.839	1507.779
L40	8.16 - 6.5	Pole	Max. Torque	12			0.759
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-60.195	-4.614	2.203
			Max. Mx	20	-27.604	1514.851	0.536
			Max. My	2	-27.594	-0.851	1546.773
			Max. Vy	20	-22.950	1514.851	0.536
			Max. Vx	2	-23.572	-0.851	1546.773
L41	6.5 - 6.25	Pole	Max. Torque	12			0.784
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-60.301	-4.631	2.207
			Max. Mx	20	-27.702	1520.580	0.540
			Max. My	2	-27.693	-0.853	1552.663
			Max. Vy	20	-22.945	1520.580	0.540
			Max. Vx	2	-23.567	-0.853	1552.663
L42	6.25 - 4.45	Pole	Max. Torque	12			0.788
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-61.064	-4.756	2.237
			Max. Mx	20	-28.237	1561.976	0.569
			Max. My	2	-28.229	-0.866	1595.221
			Max. Vy	20	-23.111	1561.976	0.569
			Max. Vx	2	-23.734	-0.866	1595.221
L43	4.45 - 4.2	Pole	Max. Torque	12			0.816
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-61.158	-4.773	2.242
			Max. Mx	20	-28.321	1567.746	0.573
			Max. My	2	-28.315	-0.868	1601.152
			Max. Vy	20	-23.107	1567.746	0.573
			Max. Vx	2	-23.730	-0.868	1601.152
L44	4.2 - 0	Pole	Max. Torque	12			0.820
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-62.661	-5.025	2.351
			Max. Mx	20	-29.436	1665.297	0.640
			Max. My	2	-29.436	-0.900	1701.420
			Max. Vy	20	-23.403	1665.297	0.640
			Max. Vx	2	-24.026	-0.900	1701.420
			Max. Torque	12			0.880

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	32	62.661	-3.208	-5.552

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Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Max. H _x	21	22.089	23.382	0.008
	Max. H _z	2	29.453	0.008	24.006
	Max. M _x	2	1701.420	0.008	24.006
	Max. M _z	8	1642.236	-22.260	-0.008
	Max. Torsion	12	0.880	-12.423	-21.568
	Min. Vert	7	22.089	-19.274	11.157
	Min. H _x	9	22.089	-22.260	-0.008
	Min. H _z	14	29.453	-0.008	-23.731
	Min. M _x	14	-1692.486	-0.008	-23.731
	Min. M _z	20	-1665.297	23.382	0.008
	Min. Torsion	24	-0.878	12.443	21.603

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead Only	24.544	0.000	0.000	-0.488	-0.739	0.000
1.2 Dead+1.0 Wind 0 deg - No Ice	29.453	-0.008	-24.006	-1701.420	-0.900	-0.148
0.9 Dead+1.0 Wind 0 deg - No Ice	22.089	-0.008	-24.006	-1679.586	-0.653	-0.149
1.2 Dead+1.0 Wind 30 deg - No Ice	29.453	11.469	-19.931	-1433.534	-825.845	-0.557
0.9 Dead+1.0 Wind 30 deg - No Ice	22.089	11.469	-19.931	-1414.932	-814.979	-0.558
1.2 Dead+1.0 Wind 60 deg - No Ice	29.453	19.274	-11.157	-823.601	-1422.326	-0.662
0.9 Dead+1.0 Wind 60 deg - No Ice	22.089	19.274	-11.157	-812.810	-1403.719	-0.663
1.2 Dead+1.0 Wind 90 deg - No Ice	29.453	22.260	0.008	-0.576	-1642.236	-0.591
0.9 Dead+1.0 Wind 90 deg - No Ice	22.089	22.260	0.008	-0.413	-1620.792	-0.591
1.2 Dead+1.0 Wind 120 deg - No Ice	29.453	20.293	11.754	836.706	-1447.067	-0.361
0.9 Dead+1.0 Wind 120 deg - No Ice	22.089	20.293	11.754	826.151	-1428.309	-0.361
1.2 Dead+1.0 Wind 150 deg - No Ice	29.453	12.423	21.568	1496.373	-862.858	-0.880
0.9 Dead+1.0 Wind 150 deg - No Ice	22.089	12.423	21.568	1477.609	-851.722	-0.879
1.2 Dead+1.0 Wind 180 deg - No Ice	29.453	0.008	23.731	1692.486	-0.964	0.147
0.9 Dead+1.0 Wind 180 deg - No Ice	22.089	0.008	23.731	1671.018	-0.727	0.148
1.2 Dead+1.0 Wind 210 deg - No Ice	29.453	-11.596	20.151	1442.177	829.670	0.559
0.9 Dead+1.0 Wind 210 deg - No Ice	22.089	-11.596	20.151	1423.822	819.248	0.560
1.2 Dead+1.0 Wind 240 deg - No Ice	29.453	-20.543	11.890	840.065	1451.081	0.664
0.9 Dead+1.0 Wind 240 deg - No Ice	22.089	-20.543	11.890	829.482	1432.770	0.665
1.2 Dead+1.0 Wind 270 deg - No Ice	29.453	-23.382	-0.008	-0.640	1665.297	0.591
0.9 Dead+1.0 Wind 270 deg - No Ice	22.089	-23.382	-0.008	-0.486	1644.184	0.591

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Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
No Ice						
1.2 Dead+1.0 Wind 300 deg - No Ice	29.453	-20.245	-11.726	-836.942	1443.502	0.359
0.9 Dead+1.0 Wind 300 deg - No Ice	22.089	-20.245	-11.726	-826.070	1425.230	0.359
1.2 Dead+1.0 Wind 330 deg - No Ice	29.453	-12.443	-21.603	-1498.269	861.396	0.878
0.9 Dead+1.0 Wind 330 deg - No Ice	22.089	-12.443	-21.603	-1479.178	850.735	0.877
1.2 Dead+1.0 Ice+1.0 Temp	62.661	0.000	-0.000	-2.351	-5.025	0.000
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	62.661	-0.012	-6.087	-492.370	-4.266	-0.088
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	62.661	2.949	-5.129	-419.720	-245.300	-0.188
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	62.661	5.087	-2.935	-242.304	-421.007	-0.201
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	62.661	5.881	0.012	-1.559	-485.821	-0.160
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	62.661	5.169	2.996	240.263	-424.068	-0.077
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	62.661	3.208	5.552	431.194	-255.609	-0.186
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	62.661	0.012	6.034	486.029	-5.899	0.088
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	62.661	-2.954	5.137	415.398	235.382	0.188
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	62.661	-5.217	3.010	239.948	414.991	0.201
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	62.661	-5.964	-0.012	-3.192	478.282	0.160
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	62.661	-5.159	-2.990	-244.820	413.566	0.076
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	62.661	-3.211	-5.558	-436.099	245.534	0.187
Dead+Wind 0 deg - Service	24.544	-0.002	-5.656	-398.745	-0.761	-0.036
Dead+Wind 30 deg - Service	24.544	2.702	-4.696	-335.992	-193.900	-0.134
Dead+Wind 60 deg - Service	24.544	4.541	-2.629	-193.187	-333.542	-0.159
Dead+Wind 90 deg - Service	24.544	5.245	0.002	-0.503	-385.027	-0.141
Dead+Wind 120 deg - Service	24.544	4.781	2.769	195.532	-339.356	-0.086
Dead+Wind 150 deg - Service	24.544	2.927	5.081	350.019	-202.597	-0.208
Dead+Wind 180 deg - Service	24.544	0.002	5.591	395.913	-0.777	0.036
Dead+Wind 210 deg - Service	24.544	-2.732	4.748	337.287	193.699	0.134
Dead+Wind 240 deg - Service	24.544	-4.840	2.801	196.320	339.198	0.159
Dead+Wind 270 deg - Service	24.544	-5.509	-0.002	-0.518	389.343	0.141
Dead+Wind 300 deg - Service	24.544	-4.770	-2.763	-196.321	337.415	0.086
Dead+Wind 330 deg - Service	24.544	-2.932	-5.090	-351.197	201.150	0.208

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.000	-24.544	0.000	0.000	24.544	0.000	0.000%
2	-0.008	-29.453	-24.006	0.008	29.453	24.006	0.000%
3	-0.008	-22.089	-24.006	0.008	22.089	24.006	0.000%
4	11.469	-29.453	-19.931	-11.469	29.453	19.931	0.000%
5	11.469	-22.089	-19.931	-11.469	22.089	19.931	0.000%
6	19.274	-29.453	-11.157	-19.274	29.453	11.157	0.000%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
7	19.274	-22.089	-11.157	-19.274	22.089	11.157	0.000%
8	22.260	-29.453	0.008	-22.260	29.453	-0.008	0.000%
9	22.260	-22.089	0.008	-22.260	22.089	-0.008	0.000%
10	20.293	-29.453	11.754	-20.293	29.453	-11.754	0.000%
11	20.293	-22.089	11.754	-20.293	22.089	-11.754	0.000%
12	12.423	-29.453	21.568	-12.423	29.453	-21.568	0.000%
13	12.423	-22.089	21.568	-12.423	22.089	-21.568	0.000%
14	0.008	-29.453	23.731	-0.008	29.453	-23.731	0.000%
15	0.008	-22.089	23.731	-0.008	22.089	-23.731	0.000%
16	-11.596	-29.453	20.151	11.596	29.453	-20.151	0.000%
17	-11.596	-22.089	20.151	11.596	22.089	-20.151	0.000%
18	-20.543	-29.453	11.890	20.543	29.453	-11.890	0.000%
19	-20.543	-22.089	11.890	20.543	22.089	-11.890	0.000%
20	-23.382	-29.453	-0.008	23.382	29.453	0.008	0.000%
21	-23.382	-22.089	-0.008	23.382	22.089	0.008	0.000%
22	-20.245	-29.453	-11.726	20.245	29.453	11.726	0.000%
23	-20.245	-22.089	-11.726	20.245	22.089	11.726	0.000%
24	-12.443	-29.453	-21.603	12.443	29.453	21.603	0.000%
25	-12.443	-22.089	-21.603	12.443	22.089	21.603	0.000%
26	0.000	-62.661	0.000	-0.000	62.661	0.000	0.000%
27	-0.012	-62.661	-6.087	0.012	62.661	6.087	0.000%
28	2.949	-62.661	-5.129	-2.949	62.661	5.129	0.000%
29	5.087	-62.661	-2.935	-5.087	62.661	2.935	0.000%
30	5.881	-62.661	0.012	-5.881	62.661	-0.012	0.000%
31	5.169	-62.661	2.996	-5.169	62.661	-2.996	0.000%
32	3.208	-62.661	5.552	-3.208	62.661	-5.552	0.000%
33	0.012	-62.661	6.034	-0.012	62.661	-6.034	0.000%
34	-2.954	-62.661	5.137	2.954	62.661	-5.137	0.000%
35	-5.217	-62.661	3.010	5.217	62.661	-3.010	0.000%
36	-5.964	-62.661	-0.012	5.964	62.661	0.012	0.000%
37	-5.159	-62.661	-2.990	5.159	62.661	2.990	0.000%
38	-3.211	-62.661	-5.558	3.211	62.661	5.558	0.000%
39	-0.002	-24.544	-5.656	0.002	24.544	5.656	0.000%
40	2.702	-24.544	-4.696	-2.702	24.544	4.696	0.000%
41	4.541	-24.544	-2.629	-4.541	24.544	2.629	0.000%
42	5.245	-24.544	0.002	-5.245	24.544	-0.002	0.000%
43	4.781	-24.544	2.769	-4.781	24.544	-2.769	0.000%
44	2.927	-24.544	5.081	-2.927	24.544	-5.081	0.000%
45	0.002	-24.544	5.591	-0.002	24.544	-5.591	0.000%
46	-2.732	-24.544	4.748	2.732	24.544	-4.748	0.000%
47	-4.840	-24.544	2.801	4.840	24.544	-2.801	0.000%
48	-5.509	-24.544	-0.002	5.509	24.544	0.002	0.000%
49	-4.770	-24.544	-2.763	4.770	24.544	2.763	0.000%
50	-2.932	-24.544	-5.090	2.932	24.544	5.090	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	5	0.00000001	0.00032558
3	Yes	5	0.00000001	0.00012455
4	Yes	7	0.00000001	0.00011738
5	Yes	6	0.00000001	0.00059399
6	Yes	7	0.00000001	0.00012217
7	Yes	6	0.00000001	0.00061996

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8	Yes	6	0.00000001	0.00006738
9	Yes	5	0.00000001	0.00047776
10	Yes	7	0.00000001	0.00011869
11	Yes	6	0.00000001	0.00059984
12	Yes	7	0.00000001	0.00012482
13	Yes	6	0.00000001	0.00062790
14	Yes	5	0.00000001	0.00033609
15	Yes	5	0.00000001	0.00013038
16	Yes	7	0.00000001	0.00012193
17	Yes	6	0.00000001	0.00061755
18	Yes	7	0.00000001	0.00011761
19	Yes	6	0.00000001	0.00059397
20	Yes	6	0.00000001	0.00006586
21	Yes	5	0.00000001	0.00046603
22	Yes	7	0.00000001	0.00012174
23	Yes	6	0.00000001	0.00061600
24	Yes	7	0.00000001	0.00012090
25	Yes	6	0.00000001	0.00060746
26	Yes	5	0.00000001	0.00026479
27	Yes	7	0.00000001	0.00057224
28	Yes	7	0.00000001	0.00078737
29	Yes	7	0.00000001	0.00079359
30	Yes	7	0.00000001	0.00056976
31	Yes	7	0.00000001	0.00078218
32	Yes	7	0.00000001	0.00080965
33	Yes	7	0.00000001	0.00056548
34	Yes	7	0.00000001	0.00076615
35	Yes	7	0.00000001	0.00076233
36	Yes	7	0.00000001	0.00055970
37	Yes	7	0.00000001	0.00077894
38	Yes	7	0.00000001	0.00079457
39	Yes	5	0.00000001	0.00006890
40	Yes	5	0.00000001	0.00056554
41	Yes	5	0.00000001	0.00062146
42	Yes	5	0.00000001	0.00009369
43	Yes	5	0.00000001	0.00057624
44	Yes	5	0.00000001	0.00065119
45	Yes	5	0.00000001	0.00006864
46	Yes	5	0.00000001	0.00061479
47	Yes	5	0.00000001	0.00056404
48	Yes	5	0.00000001	0.00009355
49	Yes	5	0.00000001	0.00061271
50	Yes	5	0.00000001	0.00060067

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	98 - 93	19.525	50	1.859	0.003
L2	93 - 88	17.580	50	1.853	0.003
L3	88 - 82.79	15.663	50	1.803	0.003
L4	85.207 - 80.207	14.621	50	1.755	0.003
L5	80.207 - 75.207	12.816	50	1.681	0.003
L6	75.207 - 70.207	11.114	50	1.568	0.003
L7	70.207 - 65.207	9.540	50	1.434	0.002
L8	65.207 - 60.207	8.117	50	1.281	0.002
L9	60.207 - 59.17	6.863	50	1.110	0.001
L10	59.17 - 58.9	6.626	50	1.073	0.001
L11	58.9 - 58.75	6.566	50	1.068	0.001

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L12	58.75 - 54	6.532	50	1.065	0.001
L13	54 - 53.75	5.518	50	0.973	0.001
L14	53.75 - 52.91	5.468	50	0.968	0.001
L15	52.91 - 52.66	5.299	50	0.951	0.001
L16	52.66 - 52.17	5.249	50	0.948	0.001
L17	52.17 - 51.92	5.152	50	0.940	0.001
L18	51.92 - 45.287	5.103	50	0.935	0.001
L19	48.704 - 44.287	4.494	50	0.872	0.001
L20	44.287 - 39.287	3.709	50	0.818	0.001
L21	39.287 - 34.287	2.905	50	0.718	0.001
L22	34.287 - 33.5	2.207	50	0.616	0.000
L23	33.5 - 33.25	2.106	50	0.600	0.000
L24	33.25 - 33	2.075	50	0.597	0.000
L25	33 - 32.75	2.044	50	0.593	0.000
L26	32.75 - 32	2.013	50	0.590	0.000
L27	32 - 31.75	1.921	50	0.579	0.000
L28	31.75 - 28.5	1.891	50	0.575	0.000
L29	28.5 - 28.25	1.521	50	0.514	0.000
L30	28.25 - 27.5	1.494	50	0.511	0.000
L31	27.5 - 27.25	1.414	50	0.501	0.000
L32	27.25 - 22.25	1.388	50	0.496	0.000
L33	22.25 - 18	0.918	50	0.403	0.000
L34	18 - 17.75	0.595	50	0.323	0.000
L35	17.75 - 15.45	0.578	50	0.319	0.000
L36	15.45 - 15.2	0.437	50	0.265	0.000
L37	15.2 - 13.41	0.424	50	0.261	0.000
L38	13.41 - 13.16	0.331	50	0.235	0.000
L39	13.16 - 8.16	0.319	50	0.230	0.000
L40	8.16 - 6.5	0.124	50	0.141	0.000
L41	6.5 - 6.25	0.080	50	0.112	0.000
L42	6.25 - 4.45	0.075	50	0.109	0.000
L43	4.45 - 4.2	0.038	50	0.083	0.000
L44	4.2 - 0	0.034	50	0.078	0.000

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
98.000	Miscellaneous [NA 510-1]	50	19.525	1.859	0.003	10789
94.000	APXVTM14-ALU-I20 w/ Mount Pipe	50	17.968	1.857	0.003	10789
89.000	APXVAARR24_43-U-NA20 w/ Mount Pipe	50	16.041	1.818	0.003	4732
76.000	Miscellaneous [NA 507-1]	50	11.376	1.588	0.003	2412
74.000	TPA-65R-LCUUUU-H8 w/ Mount Pipe	50	10.721	1.537	0.002	2251
67.000	(2) SBNHH-1D65B w/ Mount Pipe	50	8.608	1.337	0.002	1835
52.000	KS24019-L112A	50	5.119	0.937	0.001	3149

Maximum Tower Deflections - Design Wind

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	98 - 93	83.265	24	7.955	0.010
L2	93 - 88	74.986	24	7.928	0.010
L3	88 - 82.79	66.824	24	7.715	0.010
L4	85.207 - 80.207	62.391	24	7.510	0.010
L5	80.207 - 75.207	54.700	24	7.190	0.011
L6	75.207 - 70.207	47.449	24	6.707	0.011
L7	70.207 - 65.207	40.739	24	6.135	0.009
L8	65.207 - 60.207	34.668	24	5.481	0.007
L9	60.207 - 59.17	29.319	24	4.747	0.005
L10	59.17 - 58.9	28.307	24	4.589	0.004
L11	58.9 - 58.75	28.048	24	4.568	0.004
L12	58.75 - 54	27.905	24	4.556	0.004
L13	54 - 53.75	23.575	24	4.160	0.004
L14	53.75 - 52.91	23.358	24	4.140	0.004
L15	52.91 - 52.66	22.637	24	4.069	0.003
L16	52.66 - 52.17	22.425	24	4.053	0.003
L17	52.17 - 51.92	22.011	24	4.021	0.003
L18	51.92 - 45.287	21.802	24	4.000	0.003
L19	48.704 - 44.287	19.202	24	3.728	0.003
L20	44.287 - 39.287	15.847	24	3.499	0.003
L21	39.287 - 34.287	12.412	24	3.069	0.002
L22	34.287 - 33.5	9.428	24	2.633	0.002
L23	33.5 - 33.25	9.000	24	2.565	0.002
L24	33.25 - 33	8.866	24	2.550	0.002
L25	33 - 32.75	8.733	24	2.536	0.002
L26	32.75 - 32	8.601	24	2.521	0.002
L27	32 - 31.75	8.208	24	2.477	0.002
L28	31.75 - 28.5	8.079	24	2.457	0.002
L29	28.5 - 28.25	6.496	24	2.196	0.001
L30	28.25 - 27.5	6.382	24	2.183	0.001
L31	27.5 - 27.25	6.042	24	2.141	0.001
L32	27.25 - 22.25	5.931	24	2.122	0.001
L33	22.25 - 18	3.920	24	1.721	0.001
L34	18 - 17.75	2.540	24	1.381	0.001
L35	17.75 - 15.45	2.468	24	1.361	0.001
L36	15.45 - 15.2	1.868	24	1.130	0.001
L37	15.2 - 13.41	1.810	24	1.115	0.001
L38	13.41 - 13.16	1.413	24	1.002	0.001
L39	13.16 - 8.16	1.361	24	0.983	0.001
L40	8.16 - 6.5	0.532	24	0.602	0.000
L41	6.5 - 6.25	0.343	24	0.480	0.000
L42	6.25 - 4.45	0.318	24	0.465	0.000
L43	4.45 - 4.2	0.164	24	0.353	0.000
L44	4.2 - 0	0.146	24	0.334	0.000

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
98.000	Miscellaneous [NA 510-1]	24	83.265	7.955	0.011	2619
94.000	APXVTM14-ALU-I20 w/ Mount Pipe	24	76.638	7.943	0.011	2619
89.000	APXVAARR24_43-U-NA20 w/ Mount Pipe	24	68.437	7.777	0.011	1146
76.000	Miscellaneous [NA 507-1]	24	48.565	6.793	0.011	578
74.000	TPA-65R-LCUUUU-H8 w/ Mount	24	45.776	6.575	0.010	539

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Elevation	Appurtenance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
ft			in	°	°	ft
	Pipe					
67.000	(2) SBNHH-1D65B w/ Mount Pipe	24	36.766	5.720	0.007	437
52.000	KS24019-L112A	24	21.869	4.007	0.003	743

Compression Checks

Pole Design Data

Section No.	Elevation	Size	L	L _u	Kl/r	A	P _u	φP _n	Ratio P _u / φP _n
	ft		ft	ft		in ²	K	K	
L1	98 - 93 (1)	TP13.078x12x0.188	5.000	0.000	0.0	7.672	-3.371	448.789	0.008
L2	93 - 88 (2)	TP14.156x13.078x0.188	5.000	0.000	0.0	8.313	-5.746	486.327	0.012
L3	88 - 82.79 (3)	TP15.28x14.156x0.188	5.210	0.000	0.0	8.672	-5.885	507.296	0.012
L4	82.79 - 80.207 (4)	TP15.445x14.384x0.25	5.000	0.000	0.0	12.058	-6.282	705.368	0.009
L5	80.207 - 75.207 (5)	TP16.507x15.445x0.25	5.000	0.000	0.0	12.900	-6.908	754.650	0.009
L6	75.207 - 70.207 (6)	TP17.569x16.507x0.25	5.000	0.000	0.0	13.742	-10.171	803.931	0.013
L7	70.207 - 65.207 (7)	TP18.63x17.569x0.25	5.000	0.000	0.0	14.585	-13.456	853.213	0.016
L8	65.207 - 60.207 (8)	TP19.692x18.63x0.25	5.000	0.000	0.0	15.427	-14.139	902.494	0.016
L9	60.207 - 59.17 (9)	TP19.912x19.692x0.25	1.037	0.000	0.0	15.602	-14.285	912.715	0.016
L10	59.17 - 58.9 (10)	TP19.97x19.912x0.513	0.270	0.000	0.0	31.650	-14.354	1851.540	0.008
L11	58.9 - 58.75 (11)	TP20.001x19.97x0.513	0.150	0.000	0.0	31.702	-14.382	1854.570	0.008
L12	58.75 - 54 (12)	TP21.01x20.001x0.5	4.750	0.000	0.0	32.549	-15.202	1904.140	0.008
L13	54 - 53.75 (13)	TP21.063x21.01x0.513	0.250	0.000	0.0	33.429	-15.267	1955.600	0.008
L14	53.75 - 52.91 (14)	TP21.241x21.063x0.5	0.840	0.000	0.0	32.917	-15.432	1925.620	0.008
L15	52.91 - 52.66 (15)	TP21.294x21.241x0.675	0.250	0.000	0.0	44.176	-15.496	2584.310	0.006
L16	52.66 - 52.17 (16)	TP21.399x21.294x0.675	0.490	0.000	0.0	44.399	-15.601	2597.350	0.006
L17	52.17 - 51.92 (17)	TP21.452x21.399x0.525	0.250	0.000	0.0	34.871	-15.734	2039.960	0.008
L18	51.92 - 45.287 (18)	TP22.86x21.452x0.513	6.633	0.000	0.0	35.172	-16.395	2057.560	0.008
L19	45.287 - 44.287 (19)	TP22.575x21.634x0.563	4.417	0.000	0.0	39.300	-17.852	2299.030	0.008
L20	44.287 - 39.287 (20)	TP23.639x22.575x0.55	5.000	0.000	0.0	40.306	-19.017	2357.890	0.008
L21	39.287 - 34.287 (21)	TP24.703x23.639x0.538	5.000	0.000	0.0	41.227	-20.215	2411.760	0.008
L22	34.287 - 33.5 (22)	TP24.87x24.703x0.525	0.787	0.000	0.0	40.568	-20.410	2373.220	0.009
L23	33.5 - 33.25 (23)	TP24.923x24.87x0.838	0.250	0.000	0.0	64.026	-20.498	3745.520	0.005
L24	33.25 - 33 (24)	TP24.977x24.923x0.838	0.250	0.000	0.0	64.167	-20.575	3753.800	0.005
L25	33 - 32.75 (25)	TP25.03x24.977x0.813	0.250	0.000	0.0	62.454	-20.645	3653.540	0.006

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio P _u / φP _n
L26	32.75 - 32 (26)	TP25.19x25.03x0.8	0.750	0.000	0.0	61.930	-20.852	3622.900	0.006
L27	32 - 31.75 (27)	TP25.243x25.19x0.588	0.250	0.000	0.0	45.975	-20.916	2689.550	0.008
L28	31.75 - 28.5 (28)	TP25.934x25.243x0.575	3.250	0.000	0.0	46.282	-21.678	2707.510	0.008
L29	28.5 - 28.25 (29)	TP25.988x25.934x0.863	0.250	0.000	0.0	68.782	-21.769	4023.740	0.005
L30	28.25 - 27.5 (30)	TP26.147x25.988x0.85	0.750	0.000	0.0	68.249	-21.992	3992.590	0.006
L31	27.5 - 27.25 (31)	TP26.2x26.147x0.575	0.250	0.000	0.0	46.768	-22.057	2735.910	0.008
L32	27.25 - 22.25 (32)	TP27.265x26.2x0.563	5.000	0.000	0.0	47.673	-23.271	2788.890	0.008
L33	22.25 - 18 (33)	TP28.169x27.265x0.55	4.250	0.000	0.0	48.215	-24.333	2820.560	0.009
L34	18 - 17.75 (34)	TP28.222x28.169x0.563	0.250	0.000	0.0	49.383	-24.416	2888.910	0.008
L35	17.75 - 15.45 (35)	TP28.712x28.222x0.425	2.300	0.000	0.0	38.158	-25.003	2232.210	0.011
L36	15.45 - 15.2 (36)	TP28.765x28.712x0.688	0.250	0.000	0.0	61.269	-25.099	3584.220	0.007
L37	15.2 - 13.41 (37)	TP29.146x28.765x0.675	1.790	0.000	0.0	60.998	-25.616	3568.360	0.007
L38	13.41 - 13.16 (38)	TP29.199x29.146x0.563	0.250	0.000	0.0	51.127	-25.706	2990.940	0.009
L39	13.16 - 8.16 (39)	TP30.263x29.199x0.55	5.000	0.000	0.0	51.871	-27.115	3034.430	0.009
L40	8.16 - 6.5 (40)	TP30.617x30.263x0.55	1.660	0.000	0.0	52.487	-27.583	3070.510	0.009
L41	6.5 - 6.25 (41)	TP30.67x30.617x0.663	0.250	0.000	0.0	63.099	-27.683	3691.270	0.007
L42	6.25 - 4.45 (42)	TP31.053x30.67x0.65	1.800	0.000	0.0	62.724	-28.220	3669.370	0.008
L43	4.45 - 4.2 (43)	TP31.106x31.053x0.513	0.250	0.000	0.0	49.766	-28.308	2911.300	0.010
L44	4.2 - 0 (44)	TP32x31.106x0.5	4.200	0.000	0.0	49.991	-29.435	2924.440	0.010

Pole Bending Design Data

Section No.	Elevation ft	Size	M _{ux} kip-ft	φM _{ux} kip-ft	Ratio M _{ux} / φM _{ux}	M _{uy} kip-ft	φM _{uy} kip-ft	Ratio M _{uy} / φM _{uy}
L1	98 - 93 (1)	TP13.078x12x0.188	14.043	150.088	0.094	0.000	150.088	0.000
L2	93 - 88 (2)	TP14.156x13.078x0.188	44.968	176.441	0.255	0.000	176.441	0.000
L3	88 - 82.79 (3)	TP15.28x14.156x0.188	68.354	192.089	0.356	0.000	192.089	0.000
L4	82.79 - 80.207 (4)	TP15.445x14.384x0.25	111.058	277.548	0.400	0.000	277.548	0.000
L5	80.207 - 75.207 (5)	TP16.507x15.445x0.25	154.950	318.022	0.487	0.000	318.022	0.000
L6	75.207 - 70.207 (6)	TP17.569x16.507x0.25	227.333	361.249	0.629	0.000	361.249	0.000
L7	70.207 - 65.207 (7)	TP18.63x17.569x0.25	304.390	407.231	0.747	0.000	407.231	0.000
L8	65.207 - 60.207 (8)	TP19.692x18.63x0.25	398.047	455.967	0.873	0.000	455.967	0.000
L9	60.207 - 59.17 (9)	TP19.912x19.692x0.25	417.709	466.420	0.896	0.000	466.420	0.000
L10	59.17 - 58.9 (10)	TP19.97x19.912x0.513	422.844	923.883	0.458	0.000	923.883	0.000
L11	58.9 - 58.75 (11)	TP20.001x19.97x0.513	425.700	926.950	0.459	0.000	926.950	0.000
L12	58.75 - 54 (12)	TP21.01x20.001x0.5	517.319	1003.458	0.516	0.000	1003.458	0.000
L13	54 - 53.75 (13)	TP21.063x21.01x0.513	522.205	1032.050	0.506	0.000	1032.050	0.000

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Section No.	Elevation ft	Size	M_{ux} kip-ft	ϕM_{rx} kip-ft	Ratio $\frac{M_{ux}}{\phi M_{rx}}$	M_{uy} kip-ft	ϕM_{ry} kip-ft	Ratio $\frac{M_{uy}}{\phi M_{ry}}$
L14	53.75 - 52.91 (14)	TP21.241x21.063x0.5	538.662	1026.500	0.525	0.000	1026.500	0.000
L15	52.91 - 52.66 (15)	TP21.294x21.241x0.675	543.572	1358.092	0.400	0.000	1358.092	0.000
L16	52.66 - 52.17 (16)	TP21.399x21.294x0.675	553.212	1372.050	0.403	0.000	1372.050	0.000
L17	52.17 - 51.92 (17)	TP21.452x21.399x0.525	558.052	1096.117	0.509	0.000	1096.117	0.000
L18	51.92 - 45.287 (18)	TP22.86x21.452x0.513	622.136	1143.850	0.544	0.000	1143.850	0.000
L19	45.287 - 44.287 (19)	TP22.575x21.634x0.563	712.728	1298.800	0.549	0.000	1298.800	0.000
L20	44.287 - 39.287 (20)	TP23.639x22.575x0.55	817.953	1399.575	0.584	0.000	1399.575	0.000
L21	39.287 - 34.287 (21)	TP24.703x23.639x0.538	925.775	1500.608	0.617	0.000	1500.608	0.000
L22	34.287 - 33.5 (22)	TP24.87x24.703x0.525	942.983	1488.617	0.633	0.000	1488.617	0.000
L23	33.5 - 33.25 (23)	TP24.923x24.87x0.838	948.458	2294.717	0.413	0.000	2294.717	0.000
L24	33.25 - 33 (24)	TP24.977x24.923x0.838	953.950	2305.033	0.414	0.000	2305.033	0.000
L25	33 - 32.75 (25)	TP25.03x24.977x0.813	959.442	2253.233	0.426	0.000	2253.233	0.000
L26	32.75 - 32 (26)	TP25.19x25.03x0.8	975.975	2251.850	0.433	0.000	2251.850	0.000
L27	32 - 31.75 (27)	TP25.243x25.19x0.588	981.500	1704.742	0.576	0.000	1704.742	0.000
L28	31.75 - 28.5 (28)	TP25.934x25.243x0.575	1053.900	1767.133	0.596	0.000	1767.133	0.000
L29	28.5 - 28.25 (29)	TP25.988x25.934x0.863	1059.517	2572.625	0.412	0.000	2572.625	0.000
L30	28.25 - 27.5 (30)	TP26.147x25.988x0.85	1076.400	2572.008	0.419	0.000	2572.008	0.000
L31	27.5 - 27.25 (31)	TP26.2x26.147x0.575	1082.042	1804.817	0.600	0.000	1804.817	0.000
L32	27.25 - 22.25 (32)	TP27.265x26.2x0.563	1195.808	1919.633	0.623	0.000	1919.633	0.000
L33	22.25 - 18 (33)	TP28.169x27.265x0.55	1294.067	2010.375	0.644	0.000	2010.375	0.000
L34	18 - 17.75 (34)	TP28.222x28.169x0.563	1299.900	2061.283	0.631	0.000	2061.283	0.000
L35	17.75 - 15.45 (35)	TP28.712x28.222x0.425	1353.783	1637.342	0.827	0.000	1637.342	0.000
L36	15.45 - 15.2 (36)	TP28.765x28.712x0.688	1359.667	2585.492	0.526	0.000	2585.492	0.000
L37	15.2 - 13.41 (37)	TP29.146x28.765x0.675	1401.975	2612.108	0.537	0.000	2612.108	0.000
L38	13.41 - 13.16 (38)	TP29.199x29.146x0.563	1407.908	2210.950	0.637	0.000	2210.950	0.000
L39	13.16 - 8.16 (39)	TP30.263x29.199x0.55	1527.750	2330.017	0.656	0.000	2330.017	0.000
L40	8.16 - 6.5 (40)	TP30.617x30.263x0.55	1568.033	2386.267	0.657	0.000	2386.267	0.000
L41	6.5 - 6.25 (41)	TP30.67x30.617x0.663	1574.125	2852.433	0.552	0.000	2852.433	0.000
L42	6.25 - 4.45 (42)	TP31.053x30.67x0.65	1618.150	2874.850	0.563	0.000	2874.850	0.000
L43	4.45 - 4.2 (43)	TP31.106x31.053x0.513	1624.292	2305.683	0.704	0.000	2305.683	0.000
L44	4.2 - 0 (44)	TP32x31.106x0.5	1728.242	2386.767	0.724	0.000	2386.767	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V_u K	ϕV_n K	Ratio V_u ϕV_n	Actual T_u kip-ft	ϕT_n kip-ft	Ratio T_u ϕT_n
L1	98 - 93 (1)	TP13.078x12x0.188	5.133	134.637	0.038	0.000	151.992	0.000
L2	93 - 88 (2)	TP14.156x13.078x0.188	8.323	145.898	0.057	0.001	178.482	0.000
L3	88 - 82.79 (3)	TP15.28x14.156x0.188	8.431	152.189	0.055	0.001	194.205	0.000
L4	82.79 - 80.207 (4)	TP15.445x14.384x0.25	8.651	211.610	0.041	0.002	281.598	0.000
L5	80.207 - 75.207 (5)	TP16.507x15.445x0.25	9.077	226.395	0.040	0.002	322.322	0.000
L6	75.207 - 70.207 (6)	TP17.569x16.507x0.25	13.605	241.179	0.056	0.501	365.794	0.001
L7	70.207 - 65.207 (7)	TP18.63x17.569x0.25	18.553	255.964	0.072	0.152	412.015	0.000
L8	65.207 - 60.207 (8)	TP19.692x18.63x0.25	18.933	270.748	0.070	0.083	460.986	0.000
L9	60.207 - 59.17 (9)	TP19.912x19.692x0.25	19.017	273.815	0.069	0.066	471.487	0.000
L10	59.17 - 58.9 (10)	TP19.97x19.912x0.513	19.029	555.463	0.034	0.058	946.483	0.000
L11	58.9 - 58.75 (11)	TP20.001x19.97x0.513	19.045	556.372	0.034	0.056	949.583	0.000
L12	58.75 - 54 (12)	TP21.01x20.001x0.5	19.540	571.241	0.034	0.024	1026.042	0.000
L13	54 - 53.75 (13)	TP21.063x21.01x0.513	19.552	586.680	0.033	0.014	1055.858	0.000
L14	53.75 - 52.91 (14)	TP21.241x21.063x0.5	19.632	577.687	0.034	0.014	1049.325	0.000
L15	52.91 - 52.66 (15)	TP21.294x21.241x0.675	19.649	775.293	0.025	0.014	1399.983	0.000
L16	52.66 - 52.17 (16)	TP21.399x21.294x0.675	19.697	779.205	0.025	0.014	1414.150	0.000
L17	52.17 - 51.92 (17)	TP21.452x21.399x0.525	19.788	611.987	0.032	0.167	1121.558	0.000
L18	51.92 - 45.287 (18)	TP22.86x21.452x0.513	20.071	610.769	0.033	0.167	1168.825	0.000
L19	45.287 - 44.287 (19)	TP22.575x21.634x0.563	20.798	689.710	0.030	0.363	1329.558	0.000
L20	44.287 - 39.287 (20)	TP23.639x22.575x0.55	21.322	707.368	0.030	0.445	1430.292	0.000
L21	39.287 - 34.287 (21)	TP24.703x23.639x0.538	21.838	723.527	0.030	0.518	1531.183	0.000
L22	34.287 - 33.5 (22)	TP24.87x24.703x0.525	21.919	711.965	0.031	0.527	1517.933	0.000
L23	33.5 - 33.25 (23)	TP24.923x24.87x0.838	21.939	1123.660	0.020	0.529	2370.167	0.000
L24	33.25 - 33 (24)	TP24.977x24.923x0.838	21.969	1126.140	0.020	0.532	2380.650	0.000
L25	33 - 32.75 (25)	TP25.03x24.977x0.813	21.999	1096.060	0.020	0.534	2324.575	0.000
L26	32.75 - 32 (26)	TP25.19x25.03x0.8	22.092	1086.870	0.020	0.542	2321.458	0.000
L27	32 - 31.75 (27)	TP25.243x25.19x0.588	22.115	806.865	0.027	0.545	1742.167	0.000
L28	31.75 - 28.5 (28)	TP25.934x25.243x0.575	22.461	812.253	0.028	0.567	1803.892	0.000
L29	28.5 - 28.25 (29)	TP25.988x25.934x0.863	22.476	1207.120	0.019	0.568	2656.075	0.000
L30	28.25 - 27.5 (30)	TP26.147x25.988x0.85	22.566	1197.780	0.019	0.572	2653.567	0.000
L31	27.5 - 27.25 (31)	TP26.2x26.147x0.575	22.581	820.774	0.028	0.572	1841.942	0.000
L32	27.25 - 22.25 (32)	TP27.265x26.2x0.563	22.948	836.666	0.027	0.572	1956.492	0.000
L33	22.25 - 18 (33)	TP28.169x27.265x0.55	23.327	846.168	0.028	0.620	2046.658	0.000
L34	18 - 17.75 (34)	TP28.222x28.169x0.563	23.336	866.674	0.027	0.624	2099.350	0.000
L35	17.75 - 15.45 (35)	TP28.712x28.222x0.425	23.549	669.664	0.035	0.658	1658.900	0.000
L36	15.45 - 15.2 (36)	TP28.765x28.712x0.688	23.552	1075.270	0.022	0.662	2643.958	0.000
L37	15.2 - 13.41 (37)	TP29.146x28.765x0.675	23.750	1070.510	0.022	0.689	2669.150	0.000
L38	13.41 - 13.16 (38)	TP29.199x29.146x0.563	23.752	897.283	0.026	0.693	2250.258	0.000
L39	13.16 - 8.16 (39)	TP30.263x29.199x0.55	24.209	910.328	0.027	0.757	2368.808	0.000
L40	8.16 - 6.5 (40)	TP30.617x30.263x0.55	24.375	921.152	0.026	0.782	2425.475	0.000
L41	6.5 - 6.25 (41)	TP30.67x30.617x0.663	24.374	1107.380	0.022	0.786	2910.075	0.000
L42	6.25 - 4.45 (42)	TP31.053x30.67x0.65	24.576	1100.810	0.022	0.814	2930.950	0.000

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Section No.	Elevation ft	Size	Actual V_u K	ϕV_n K	Ratio $\frac{V_u}{\phi V_n}$	Actual T_u kip-ft	ϕT_n kip-ft	Ratio $\frac{T_u}{\phi T_n}$
L43	4.45 - 4.2 (43)	TP31.106x31.053x0.513	24.575	873.391	0.028	0.817	2340.025	0.000
L44	4.2 - 0 (44)	TP32x31.106x0.5	24.951	877.333	0.028	0.878	2420.225	0.000

Pole Interaction Design Data

Section No.	Elevation ft	Ratio P_u	Ratio M_{ux}	Ratio M_{uy}	Ratio V_u	Ratio T_u	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
		ϕP_n	ϕM_{ux}	ϕM_{uy}	ϕV_n	ϕT_n			
L1	98 - 93 (1)	0.008	0.094	0.000	0.038	0.000	0.103	1.050	4.8.2 ✓
L2	93 - 88 (2)	0.012	0.255	0.000	0.057	0.000	0.270	1.050	4.8.2 ✓
L3	88 - 82.79 (3)	0.012	0.356	0.000	0.055	0.000	0.371	1.050	4.8.2 ✓
L4	82.79 - 80.207 (4)	0.009	0.400	0.000	0.041	0.000	0.411	1.050	4.8.2 ✓
L5	80.207 - 75.207 (5)	0.009	0.487	0.000	0.040	0.000	0.498	1.050	4.8.2 ✓
L6	75.207 - 70.207 (6)	0.013	0.629	0.000	0.056	0.001	0.645	1.050	4.8.2 ✓
L7	70.207 - 65.207 (7)	0.016	0.747	0.000	0.072	0.000	0.769	1.050	4.8.2 ✓
L8	65.207 - 60.207 (8)	0.016	0.873	0.000	0.070	0.000	0.894	1.050	4.8.2 ✓
L9	60.207 - 59.17 (9)	0.016	0.896	0.000	0.069	0.000	0.916	1.050	4.8.2 ✓
L10	59.17 - 58.9 (10)	0.008	0.458	0.000	0.034	0.000	0.467	1.050	4.8.2 ✓
L11	58.9 - 58.75 (11)	0.008	0.459	0.000	0.034	0.000	0.468	1.050	4.8.2 ✓
L12	58.75 - 54 (12)	0.008	0.516	0.000	0.034	0.000	0.525	1.050	4.8.2 ✓
L13	54 - 53.75 (13)	0.008	0.506	0.000	0.033	0.000	0.515	1.050	4.8.2 ✓
L14	53.75 - 52.91 (14)	0.008	0.525	0.000	0.034	0.000	0.534	1.050	4.8.2 ✓
L15	52.91 - 52.66 (15)	0.006	0.400	0.000	0.025	0.000	0.407	1.050	4.8.2 ✓
L16	52.66 - 52.17 (16)	0.006	0.403	0.000	0.025	0.000	0.410	1.050	4.8.2 ✓
L17	52.17 - 51.92 (17)	0.008	0.509	0.000	0.032	0.000	0.518	1.050	4.8.2 ✓
L18	51.92 - 45.287 (18)	0.008	0.544	0.000	0.033	0.000	0.553	1.050	4.8.2 ✓
L19	45.287 - 44.287 (19)	0.008	0.549	0.000	0.030	0.000	0.557	1.050	4.8.2 ✓
L20	44.287 - 39.287 (20)	0.008	0.584	0.000	0.030	0.000	0.593	1.050	4.8.2 ✓

Section No.	Elevation ft	Ratio P_u ϕP_n	Ratio M_{ux} ϕM_{nx}	Ratio M_{uy} ϕM_{ny}	Ratio V_u ϕV_n	Ratio T_u ϕT_n	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L21	39.287 - 34.287 (21)	0.008	0.617	0.000	0.030	0.000	0.626	1.050	4.8.2 ✓
L22	34.287 - 33.5 (22)	0.009	0.633	0.000	0.031	0.000	0.643	1.050	4.8.2 ✓
L23	33.5 - 33.25 (23)	0.005	0.413	0.000	0.020	0.000	0.419	1.050	4.8.2 ✓
L24	33.25 - 33 (24)	0.005	0.414	0.000	0.020	0.000	0.420	1.050	4.8.2 ✓
L25	33 - 32.75 (25)	0.006	0.426	0.000	0.020	0.000	0.432	1.050	4.8.2 ✓
L26	32.75 - 32 (26)	0.006	0.433	0.000	0.020	0.000	0.440	1.050	4.8.2 ✓
L27	32 - 31.75 (27)	0.008	0.576	0.000	0.027	0.000	0.584	1.050	4.8.2 ✓
L28	31.75 - 28.5 (28)	0.008	0.596	0.000	0.028	0.000	0.605	1.050	4.8.2 ✓
L29	28.5 - 28.25 (29)	0.005	0.412	0.000	0.019	0.000	0.418	1.050	4.8.2 ✓
L30	28.25 - 27.5 (30)	0.006	0.419	0.000	0.019	0.000	0.424	1.050	4.8.2 ✓
L31	27.5 - 27.25 (31)	0.008	0.600	0.000	0.028	0.000	0.608	1.050	4.8.2 ✓
L32	27.25 - 22.25 (32)	0.008	0.623	0.000	0.027	0.000	0.632	1.050	4.8.2 ✓
L33	22.25 - 18 (33)	0.009	0.644	0.000	0.028	0.000	0.653	1.050	4.8.2 ✓
L34	18 - 17.75 (34)	0.008	0.631	0.000	0.027	0.000	0.640	1.050	4.8.2 ✓
L35	17.75 - 15.45 (35)	0.011	0.827	0.000	0.035	0.000	0.839	1.050	4.8.2 ✓
L36	15.45 - 15.2 (36)	0.007	0.526	0.000	0.022	0.000	0.533	1.050	4.8.2 ✓
L37	15.2 - 13.41 (37)	0.007	0.537	0.000	0.022	0.000	0.544	1.050	4.8.2 ✓
L38	13.41 - 13.16 (38)	0.009	0.637	0.000	0.026	0.000	0.646	1.050	4.8.2 ✓
L39	13.16 - 8.16 (39)	0.009	0.656	0.000	0.027	0.000	0.665	1.050	4.8.2 ✓
L40	8.16 - 6.5 (40)	0.009	0.657	0.000	0.026	0.000	0.667	1.050	4.8.2 ✓
L41	6.5 - 6.25 (41)	0.007	0.552	0.000	0.022	0.000	0.560	1.050	4.8.2 ✓
L42	6.25 - 4.45 (42)	0.008	0.563	0.000	0.022	0.000	0.571	1.050	4.8.2 ✓
L43	4.45 - 4.2 (43)	0.010	0.704	0.000	0.028	0.000	0.715	1.050	4.8.2 ✓
L44	4.2 - 0 (44)	0.010	0.724	0.000	0.028	0.000	0.735	1.050	4.8.2 ✓

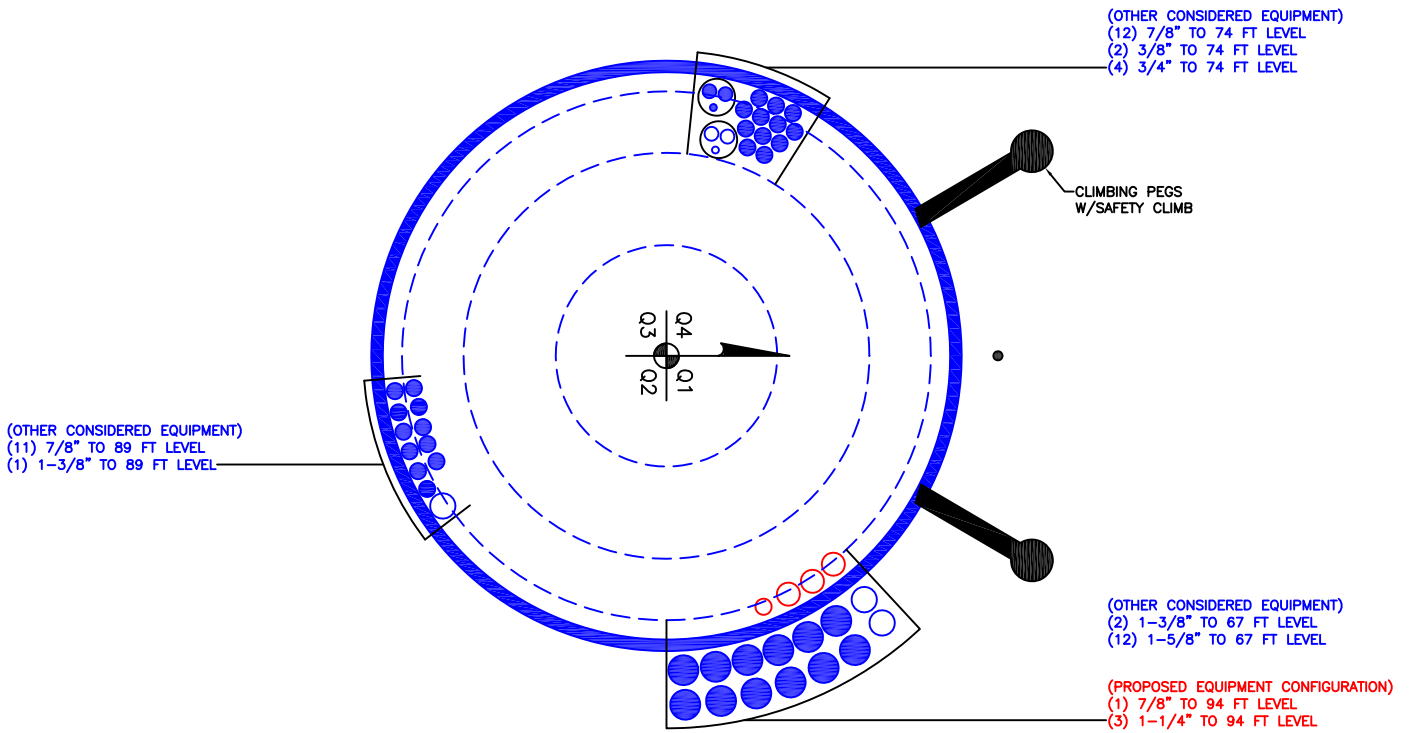
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Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail
L1	98 - 93	Pole	TP13.078x12x0.188	1	-3.371	471.228	**	**
L2	93 - 88	Pole	TP14.156x13.078x0.188	2	-5.746	510.643	**	**
L3	88 - 82.79	Pole	TP15.28x14.156x0.188	3	-5.885	532.661	**	**
L4	82.79 - 80.207	Pole	TP15.445x14.384x0.25	4	-6.282	740.636	**	**
L5	80.207 - 75.207	Pole	TP16.507x15.445x0.25	5	-6.908	792.382	**	**
L6	75.207 - 70.207	Pole	TP17.569x16.507x0.25	6	-10.171	844.128	**	**
L7	70.207 - 65.207	Pole	TP18.63x17.569x0.25	7	-13.456	895.874	**	**
L8	65.207 - 60.207	Pole	TP19.692x18.63x0.25	8	-14.139	947.619	**	**
L9	60.207 - 59.17	Pole	TP19.912x19.692x0.25	9	-14.285	958.351	**	**
L10	59.17 - 58.9	Pole	TP19.97x19.912x0.513	10	-14.354	1944.117	**	**
L11	58.9 - 58.75	Pole	TP20.001x19.97x0.513	11	-14.382	1947.298	**	**
L12	58.75 - 54	Pole	TP21.01x20.001x0.5	12	-15.202	1999.347	**	**
L13	54 - 53.75	Pole	TP21.063x21.01x0.513	13	-15.267	2053.380	**	**
L14	53.75 - 52.91	Pole	TP21.241x21.063x0.5	14	-15.432	2021.901	**	**
L15	52.91 - 52.66	Pole	TP21.294x21.241x0.675	15	-15.496	2713.525	**	**
L16	52.66 - 52.17	Pole	TP21.399x21.294x0.675	16	-15.601	2727.217	**	**
L17	52.17 - 51.92	Pole	TP21.452x21.399x0.525	17	-15.734	2141.958	**	**
L18	51.92 - 45.287	Pole	TP22.86x21.452x0.513	18	-16.395	2160.438	**	**
L19	45.287 - 44.287	Pole	TP22.575x21.634x0.563	19	-17.852	2413.981	**	**
L20	44.287 - 39.287	Pole	TP23.639x22.575x0.55	20	-19.017	2475.784	**	**
L21	39.287 - 34.287	Pole	TP24.703x23.639x0.538	21	-20.215	2532.348	**	**
L22	34.287 - 33.5	Pole	TP24.87x24.703x0.525	22	-20.410	2491.881	**	**
L23	33.5 - 33.25	Pole	TP24.923x24.87x0.838	23	-20.498	3932.796	**	**
L24	33.25 - 33	Pole	TP24.977x24.923x0.838	24	-20.575	3941.490	**	**
L25	33 - 32.75	Pole	TP25.03x24.977x0.813	25	-20.645	3836.217	**	**
L26	32.75 - 32	Pole	TP25.19x25.03x0.8	26	-20.852	3804.045	**	**
L27	32 - 31.75	Pole	TP25.243x25.19x0.588	27	-20.916	2824.027	**	**
L28	31.75 - 28.5	Pole	TP25.934x25.243x0.575	28	-21.678	2842.885	**	**
L29	28.5 - 28.25	Pole	TP25.988x25.934x0.863	29	-21.769	4224.927	**	**
L30	28.25 - 27.5	Pole	TP26.147x25.988x0.85	30	-21.992	4192.219	**	**
L31	27.5 - 27.25	Pole	TP26.2x26.147x0.575	31	-22.057	2872.705	**	**
L32	27.25 - 22.25	Pole	TP27.265x26.2x0.563	32	-23.271	2928.334	**	**
L33	22.25 - 18	Pole	TP28.169x27.265x0.55	33	-24.333	2961.588	**	**
L34	18 - 17.75	Pole	TP28.222x28.169x0.563	34	-24.416	3033.355	**	**
L35	17.75 - 15.45	Pole	TP28.712x28.222x0.425	35	-25.003	2343.820	**	**
L36	15.45 - 15.2	Pole	TP28.765x28.712x0.688	36	-25.099	3763.431	**	**
L37	15.2 - 13.41	Pole	TP29.146x28.765x0.675	37	-25.616	3746.778	**	**
L38	13.41 - 13.16	Pole	TP29.199x29.146x0.563	38	-25.706	3140.487	**	**
L39	13.16 - 8.16	Pole	TP30.263x29.199x0.55	39	-27.115	3186.151	**	**
L40	8.16 - 6.5	Pole	TP30.617x30.263x0.55	40	-27.583	3224.035	**	**
L41	6.5 - 6.25	Pole	TP30.67x30.617x0.663	41	-27.683	3875.833	**	**
L42	6.25 - 4.45	Pole	TP31.053x30.67x0.65	42	-28.220	3852.838	**	**
L43	4.45 - 4.2	Pole	TP31.106x31.053x0.513	43	-28.308	3056.865	**	**
L44	4.2 - 0	Pole	TP32x31.106x0.5	44	-29.435	3070.662	**	**
Summary								
Pole (L9)							**	**
RATING =							**	**

** Above stress ratios for reinforced sections are approximate. More exact calculations are presented in Appendix C

APPENDIX B
BASE LEVEL DRAWING



BUSINESS UNIT: 876399

APPENDIX C
ADDITIONAL CALCULATIONS

Site BU: 876399
Work Order: 1773228

Pole Geometry

	Pole Height Above Base (ft)	Section Length (ft)	Lap Splice Length (ft)	Number of Sides	Top Diameter (in)	Bottom Diameter (in)	Wall Thickness (in)	Bend Radius (in)	Pole Material
1	98	15.21	2.417	18	12	15.28	0.1875	Auto	A572-65
2	85.207	39.92	3.417	18	14.38	22.86	0.25	Auto	A572-65
3	48.704	48.704	0	18	21.63	32	0.3125	Auto	A572-65

Reinforcement Configuration

	Bottom Effective Elevation (ft)	Top Effective Elevation (ft)	Type	Model	Number	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
1	0	6.5	plate	MS-600 W(1.1875")	1												E2						
2	0	28.5	plate	MS-600 W(1.1875")	1																		E2
3	0	28.5	plate	MS-600 (1.1875")	1						E2												
4	4.45	15.45	plate	MS-600 (1.1875")	1											E2							
5	13.41	28.5	plate	MS-600 (1.1875")	1												E2						
6	27.5	33.5	plate	MS-600 (1.1875")	3					E2							E2					E2	
7	32	48.25	plate	MS-450 (1.1875")	1												E2						
8	32	59	plate	MS-450 (1.1875")	2						E2												E2
9	47.41	52.91	plate	MS-450 (1.1875")	1												E2						
10	52.17	59.17	plate	MS-450 (1.1875")	1												E2						
11	0	18	plate	CCI-WSFP-050125	1																P		
12	33	54	plate	CCI-SFP-050125	1																P		
13																							

Reinforcement Details

	B (in)	H (in)	Gross Area (in ²)	Pole Face to Centroid (in)	Bottom Termination Length (in)	Top Termination Length (in)	L _v (in)	Net Area (in ²)	Bolt Hole Size (in)	Reinforcement Material
1	6	1	6	0.5	n/a	24.000	16.375	4.750	1.1875	A572-65
2	6	1	6	0.5	n/a	24.000	16.375	4.750	1.1875	A572-65
3	6	1	6	0.5	24.000	24.000	16.375	4.750	1.1875	A572-65
4	6	1	6	0.5	24.000	24.000	16.375	4.750	1.1875	A572-65
5	6	1	6	0.5	24.000	24.000	16.375	4.750	1.1875	A572-65
6	6	1	6	0.5	24.000	24.000	16.375	4.750	1.1875	A572-65
7	4.5	1	4.5	0.5	18.000	18.000	20.625	3.250	1.1875	A572-65
8	4.5	1	4.5	0.5	18.000	18.000	20.625	3.250	1.1875	A572-65
9	4.5	1	4.5	0.5	18.000	18.000	20.625	3.250	1.1875	A572-65
10	4.5	1	4.5	0.5	18.000	18.000	20.625	3.250	1.1875	A572-65
11	5	1.25	6.25	0.625	n/a	24.000	23.000	4.688	1.1875	A572-65
12	5	1.25	6.25	0.625	24.000	24.000	23.000	4.688	1.1875	A572-65

TNX Geometry Input

Increment (ft): 5

	Section Height (ft)	Section Length (ft)	Lap Splice Length (ft)	Number of Sides	Top Diameter (in)	Bottom Diameter (in)	Wall Thickness (in)	Tapered Pole Grade	Weight Multiplier
1	98 - 93	5		18	12.000	13.078	0.1875	A572-65	1.000
2	93 - 88	5		18	13.078	14.156	0.1875	A572-65	1.000
3	88 - 85.207	5.21	2.417	18	14.156	15.280	0.1875	A572-65	1.000
4	85.207 - 80.207	5		18	14.384	15.445	0.25	A572-65	1.000
5	80.207 - 75.207	5		18	15.445	16.507	0.25	A572-65	1.000
6	75.207 - 70.207	5		18	16.507	17.569	0.25	A572-65	1.000
7	70.207 - 65.207	5		18	17.569	18.630	0.25	A572-65	1.000
8	65.207 - 60.207	5		18	18.630	19.692	0.25	A572-65	1.000
9	60.207 - 59.17	1.037		18	19.692	19.912	0.25	A572-65	1.000
10	59.17 - 58.9	0.27		18	19.912	19.970	0.5125	A572-65	0.921
11	58.9 - 58.75	0.15		18	19.970	20.001	0.5125	A572-65	0.920
12	58.75 - 54	4.75		18	20.001	21.010	0.5	A572-65	0.921
13	54 - 53.75	0.25		18	21.010	21.063	0.5125	A572-65	1.085
14	53.75 - 52.91	0.84		18	21.063	21.241	0.5	A572-65	1.106
15	52.91 - 52.66	0.25		18	21.241	21.294	0.675	A572-65	0.927
16	52.66 - 52.17	0.49		18	21.294	21.399	0.675	A572-65	0.924
17	52.17 - 51.92	0.25		18	21.399	21.452	0.525	A572-65	1.049
18	51.92 - 48.704	6.633	3.417	18	21.452	22.860	0.5125	A572-65	1.055
19	48.704 - 44.287	4.417		18	21.634	22.575	0.5625	A572-65	1.064
20	44.287 - 39.287	5		18	22.575	23.639	0.55	A572-65	1.064
21	39.287 - 34.287	5		18	23.639	24.703	0.5375	A572-65	1.066
22	34.287 - 33.5	0.787		18	24.703	24.870	0.525	A572-65	1.087
23	33.5 - 33.25	0.25		18	24.870	24.923	0.8375	A572-65	0.971
24	33.25 - 33	0.25		18	24.923	24.977	0.8375	A572-65	0.970
25	33 - 32.75	0.25		18	24.977	25.030	0.8125	A572-65	0.897
26	32.75 - 32	0.75		18	25.030	25.190	0.8	A572-65	0.907
27	32 - 31.75	0.25		18	25.190	25.243	0.5875	A572-65	0.929
28	31.75 - 28.5	3.25		18	25.243	25.934	0.575	A572-65	0.938
29	28.5 - 28.25	0.25		18	25.934	25.988	0.8625	A572-65	0.894
30	28.25 - 27.5	0.75		18	25.988	26.147	0.85	A572-65	0.903
31	27.5 - 27.25	0.25		18	26.147	26.200	0.575	A572-65	0.934
32	27.25 - 22.25	5		18	26.200	27.265	0.5625	A572-65	0.938
33	22.25 - 18	4.25		18	27.265	28.169	0.55	A572-65	0.946
34	18 - 17.75	0.25		18	28.169	28.222	0.5625	A572-65	1.052
35	17.75 - 15.45	2.3		18	28.222	28.712	0.425	A572-65	1.217
36	15.45 - 15.2	0.25		18	28.712	28.765	0.6875	A572-65	0.954
37	15.2 - 13.41	1.79		18	28.765	29.146	0.675	A572-65	0.965
38	13.41 - 13.16	0.25		18	29.146	29.199	0.5625	A572-65	1.035
39	13.16 - 8.16	5		18	29.199	30.263	0.55	A572-65	1.040
40	8.16 - 6.5	1.66		18	30.263	30.617	0.55	A572-65	1.035
41	6.5 - 6.25	0.25		18	30.617	30.670	0.6625	A572-65	0.957
42	6.25 - 4.45	1.8		18	30.670	31.053	0.65	A572-65	0.968
43	4.45 - 4.2	0.25		18	31.053	31.106	0.5125	A572-65	0.980
44	4.2 - 0	4.2		18	31.106	32.000	0.5	A572-65	0.994

TNX Section Forces

Increment (ft):		TNX Output		
	5	P _u	M _{ux} (kip-ft)	V _u (K)
	Section Height (ft)	(K)		
1	98 - 93	3.37	14.04	5.13
2	93 - 88	5.75	44.97	8.32
3	88 - 85.207	5.88	68.35	8.43
4	85.207 - 80.207	6.28	111.06	8.65
5	80.207 - 75.207	6.91	154.95	9.08
6	75.207 - 70.207	10.17	227.33	13.60
7	70.207 - 65.207	13.46	304.39	18.55
8	65.207 - 60.207	14.14	398.05	18.93
9	60.207 - 59.17	14.28	417.71	19.02
10	59.17 - 58.9	14.35	422.84	19.03
11	58.9 - 58.75	14.38	425.70	19.05
12	58.75 - 54	15.20	517.32	19.54
13	54 - 53.75	15.27	522.20	19.55
14	53.75 - 52.91	15.43	538.66	19.63
15	52.91 - 52.66	15.50	543.57	19.65
16	52.66 - 52.17	15.60	553.21	19.70
17	52.17 - 51.92	15.73	558.05	19.79
18	51.92 - 48.704	16.40	622.14	20.07
19	48.704 - 44.287	17.85	712.73	20.80
20	44.287 - 39.287	19.02	817.95	21.32
21	39.287 - 34.287	20.21	925.77	21.84
22	34.287 - 33.5	20.41	942.98	21.92
23	33.5 - 33.25	20.50	948.46	21.94
24	33.25 - 33	20.57	953.95	21.97
25	33 - 32.75	20.65	959.44	22.00
26	32.75 - 32	20.85	975.97	22.09
27	32 - 31.75	20.92	981.50	22.11
28	31.75 - 28.5	21.68	1053.90	22.46
29	28.5 - 28.25	21.77	1059.51	22.48
30	28.25 - 27.5	21.99	1076.40	22.57
31	27.5 - 27.25	22.06	1082.04	22.58
32	27.25 - 22.25	23.27	1195.81	22.95
33	22.25 - 18	24.33	1294.07	23.33
34	18 - 17.75	24.42	1299.90	23.34
35	17.75 - 15.45	25.00	1353.78	23.55
36	15.45 - 15.2	25.10	1359.67	23.55
37	15.2 - 13.41	25.62	1401.98	23.75
38	13.41 - 13.16	25.71	1407.91	23.75
39	13.16 - 8.16	27.12	1527.75	24.21
40	8.16 - 6.5	27.58	1568.04	24.38
41	6.5 - 6.25	27.68	1574.13	24.37
42	6.25 - 4.45	28.22	1618.15	24.58
43	4.45 - 4.2	28.31	1624.29	24.58
44	4.2 - 0	29.43	1728.24	24.95

Analysis Results

Elevation (ft)	Component Type	Size	Critical Element	% Capacity	Pass / Fail
98 - 93	Pole	TP13.078x12x0.1875	Pole	9.5%	Pass
93 - 88	Pole	TP14.156x13.078x0.1875	Pole	25.2%	Pass
88 - 85.21	Pole	TP15.28x14.156x0.1875	Pole	34.8%	Pass
85.21 - 80.21	Pole	TP15.445x14.384x0.25	Pole	38.8%	Pass
80.21 - 75.21	Pole	TP16.507x15.445x0.25	Pole	47.1%	Pass
75.21 - 70.21	Pole	TP17.569x16.507x0.25	Pole	60.9%	Pass
70.21 - 65.21	Pole	TP18.63x17.569x0.25	Pole	72.4%	Pass
65.21 - 60.21	Pole	TP19.692x18.63x0.25	Pole	84.4%	Pass
60.21 - 59.17	Pole	TP19.912x19.692x0.25	Pole	86.5%	Pass
59.17 - 58.9	Pole + Reinf.	TP19.97x19.912x0.5125	Reinf. 10 Compression	78.4%	Pass
58.9 - 58.75	Pole + Reinf.	TP20.001x19.97x0.5125	Reinf. 10 Compression	78.8%	Pass
58.75 - 54	Pole + Reinf.	TP21.01x20.001x0.5	Reinf. 10 Compression	88.7%	Pass
54 - 53.75	Pole + Reinf.	TP21.063x21.01x0.5125	Reinf. 10 Compression	79.6%	Pass
53.75 - 52.91	Pole + Reinf.	TP21.241x21.063x0.5	Reinf. 10 Compression	81.1%	Pass
52.91 - 52.66	Pole + Reinf.	TP21.294x21.241x0.675	Reinf. 8 Compression	78.3%	Pass
52.66 - 52.17	Pole + Reinf.	TP21.399x21.294x0.675	Reinf. 8 Compression	79.2%	Pass
52.17 - 51.92	Pole + Reinf.	TP21.452x21.399x0.525	Reinf. 9 Compression	84.7%	Pass
51.92 - 48.7	Pole + Reinf.	TP22.86x21.452x0.5125	Reinf. 9 Compression	90.1%	Pass
48.7 - 44.29	Pole + Reinf.	TP22.575x21.634x0.5625	Reinf. 7 Compression	87.4%	Pass
44.29 - 39.29	Pole + Reinf.	TP23.639x22.575x0.55	Reinf. 7 Compression	93.2%	Pass
39.29 - 34.29	Pole + Reinf.	TP24.703x23.639x0.5375	Reinf. 7 Compression	98.3%	Pass
34.29 - 33.5	Pole + Reinf.	TP24.87x24.703x0.525	Reinf. 7 Compression	99.1%	Pass
33.5 - 33.25	Pole + Reinf.	TP24.923x24.87x0.8375	Reinf. 7 Compression	67.1%	Pass
33.25 - 33	Pole + Reinf.	TP24.977x24.923x0.8375	Reinf. 7 Compression	67.3%	Pass
33 - 32.75	Pole + Reinf.	TP25.03x24.977x0.8125	Reinf. 7 Compression	73.0%	Pass
32.75 - 32	Pole + Reinf.	TP25.19x25.03x0.8	Reinf. 7 Compression	73.6%	Pass
32 - 31.75	Pole + Reinf.	TP25.243x25.19x0.5875	Reinf. 6 Tension Rupture	88.8%	Pass
31.75 - 28.5	Pole + Reinf.	TP25.934x25.243x0.575	Reinf. 6 Tension Rupture	91.4%	Pass
28.5 - 28.25	Pole + Reinf.	TP25.988x25.934x0.8625	Reinf. 6 Tension Rupture	63.7%	Pass
28.25 - 27.5	Pole + Reinf.	TP26.147x25.988x0.85	Reinf. 6 Tension Rupture	64.1%	Pass
27.5 - 27.25	Pole + Reinf.	TP26.2x26.147x0.575	Reinf. 5 Tension Rupture	92.4%	Pass
27.25 - 22.25	Pole + Reinf.	TP27.265x26.2x0.5625	Reinf. 5 Tension Rupture	95.8%	Pass
22.25 - 18	Pole + Reinf.	TP28.169x27.265x0.55	Reinf. 5 Tension Rupture	98.5%	Pass
18 - 17.75	Pole + Reinf.	TP28.222x28.169x0.5625	Reinf. 5 Tension Rupture	90.4%	Pass
17.75 - 15.45	Pole + Reinf.	TP28.712x28.222x0.425	Pole	91.2%	Pass
15.45 - 15.2	Pole + Reinf.	TP28.765x28.712x0.6875	Reinf. 3 Tension Rupture	89.7%	Pass
15.2 - 13.41	Pole + Reinf.	TP29.146x28.765x0.675	Reinf. 3 Tension Rupture	90.7%	Pass
13.41 - 13.16	Pole + Reinf.	TP29.199x29.146x0.5625	Reinf. 4 Tension Rupture	94.5%	Pass
13.16 - 8.16	Pole + Reinf.	TP30.263x29.199x0.55	Reinf. 4 Tension Rupture	97.0%	Pass
8.16 - 6.5	Pole + Reinf.	TP30.617x30.263x0.55	Reinf. 4 Tension Rupture	97.7%	Pass
6.5 - 6.25	Pole + Reinf.	TP30.67x30.617x0.6625	Reinf. 3 Tension Rupture	94.1%	Pass
6.25 - 4.45	Pole + Reinf.	TP31.053x30.67x0.65	Reinf. 3 Tension Rupture	94.9%	Pass
4.45 - 4.2	Pole + Reinf.	TP31.106x31.053x0.5125	Reinf. 1 Tension Rupture	96.2%	Pass
4.2 - 0	Pole + Reinf.	TP32x31.106x0.5	Reinf. 1 Tension Rupture	97.8%	Pass
				Summary	
			Pole	91.2%	Pass
			Reinforcement	99.1%	Pass
			Overall	99.1%	Pass

Additional Calculations

Section Elevation (ft)	Moment of Inertia (in ⁴)			Area (in ²)			% Capacity*												
	Pole	Reinf.	Total	Pole	Reinf.	Total	Pole	R1	R2	R3	R4	R5	R6	R7	R8	R9	R10	R11	R12
98 - 93	161	n/a	161	7.67	n/a	7.67	9.5%												
93 - 88	205	n/a	205	8.31	n/a	8.31	25.2%												
88 - 85.21	233	n/a	233	8.67	n/a	8.67	34.8%												
85.21 - 80.21	352	n/a	352	12.06	n/a	12.06	38.8%												
80.21 - 75.21	431	n/a	431	12.90	n/a	12.90	47.1%												
75.21 - 70.21	521	n/a	521	13.74	n/a	13.74	60.9%												
70.21 - 65.21	622	n/a	622	14.58	n/a	14.58	72.4%												
65.21 - 60.21	736	n/a	736	15.43	n/a	15.43	84.4%												
60.21 - 59.17	762	n/a	762	15.60	n/a	15.60	86.5%												
59.17 - 58.9	768	754	1522	15.65	13.50	29.15	43.5%								78.4%		78.4%		
58.9 - 58.75	772	756	1528	15.67	13.50	29.17	43.7%								78.8%		78.8%		
58.75 - 54	897	829	1726	16.47	13.50	29.97	49.3%								88.7%		88.7%		
54 - 53.75	905	864	1769	16.51	19.75	36.26	50.1%								79.6%		79.6%		52.1%
53.75 - 52.91	929	877	1806	16.66	19.75	36.41	51.0%								81.1%		81.1%		53.2%
52.91 - 52.66	1003	1447	2449	16.70	24.25	40.95	44.4%								78.4%	62.7%	58.5%		53.7%
52.66 - 52.17	1018	1460	2478	16.78	24.25	41.03	44.8%								79.2%	63.4%	59.1%		54.3%
52.17 - 51.92	958	956	1914	16.82	19.75	36.57	50.7%								80.1%	84.7%			59.1%
51.92 - 48.7	1054	1013	2067	17.36	19.75	37.11	54.0%								85.2%	90.1%			63.1%
48.7 - 44.29	1387	1008	2395	22.08	19.75	41.83	55.1%								87.4%	87.4%			60.1%
44.29 - 39.29	1595	1099	2694	23.14	19.75	42.89	58.7%								93.2%	93.2%			64.7%
39.29 - 34.29	1823	1194	3017	24.19	19.75	43.94	62.0%								98.3%	98.3%			68.8%
34.29 - 33.5	1861	1209	3070	24.36	19.75	44.11	62.4%								99.1%	99.1%			69.4%
33.5 - 33.25	1872	2805	4677	24.41	37.75	62.16	41.4%						64.4%	67.1%	67.1%				52.0%
33.25 - 33	1885	2816	4701	24.46	37.75	62.21	41.5%						64.6%	67.3%	67.3%				52.1%
33 - 32.75	1892	2708	4599	24.52	31.50	56.02	40.8%						65.9%	73.0%	73.0%				
32.75 - 32	1929	2740	4669	24.67	31.50	56.17	41.1%						66.5%	73.6%	73.6%				
32 - 31.75	1941	1577	3518	24.73	18.00	42.73	54.9%						88.8%						
31.75 - 28.5	2107	1660	3767	25.41	18.00	43.41	56.6%						91.4%						
28.5 - 28.25	2120	3333	5453	25.47	36.00	61.47	39.4%	63.7%	63.7%			63.7%	63.7%						
28.25 - 27.5	2160	3372	5532	25.62	36.00	61.62	39.7%	64.1%	64.1%			64.1%	64.1%						
27.5 - 27.25	2173	1692	3866	25.68	18.00	43.68	57.2%	92.4%	92.4%			92.4%							
27.25 - 22.25	2453	1825	4278	26.73	18.00	44.73	59.4%	95.8%	95.8%			95.8%							
22.25 - 18	2708	1942	4650	27.63	18.00	45.63	61.1%	98.5%	98.5%			98.5%							
18 - 17.75	2729	2035	4765	27.68	24.25	51.93	62.6%	90.4%	88.1%			90.4%							73.7%
17.75 - 15.45	3303	945	4247	28.17	18.25	46.42	91.2%	90.9%				90.9%							90.3%
15.45 - 15.2	2998	3180	6178	28.22	30.25	58.47	56.7%	84.1%	89.8%	71.5%		68.2%							74.9%
15.2 - 13.41	3118	3262	6380	28.60	30.25	58.85	57.3%	85.1%	90.7%	72.3%		69.0%							75.8%
13.41 - 13.16	3024	2280	5304	28.65	24.25	52.90	62.1%	87.6%	91.5%	94.5%									82.3%
13.16 - 8.16	3371	2441	5812	29.71	24.25	53.96	63.8%	89.9%	93.9%	97.0%									84.7%
8.16 - 6.5	3492	2496	5987	30.06	24.25	54.31	64.5%	90.7%	94.6%	97.7%									85.5%
6.5 - 6.25	3632	3601	7233	30.11	30.25	60.36	59.9%	72.1%	88.4%	94.1%	75.5%								79.1%
6.25 - 4.45	3769	3689	7458	30.49	30.25	60.74	60.6%	72.8%	89.2%	94.9%	76.2%								79.8%
4.45 - 4.2	3716	2201	5918	30.54	18.25	48.79	74.1%	96.2%	96.2%										95.4%
4.2 - 0	4047	2326	6373	31.43	18.25	49.68	75.7%	97.8%	97.8%										97.0%

Note: Section capacity checked in 5 degree increments.
Rating per TIA-222-H Section 15.5.

PROJECT **127643.006.01 - (F) E.GRANBY 4Q2000 GALASSO, CT**

SUBJECT **Anchor Rod Bracket Analysis**

DATE **07/27/19**

TIA-222 Rev.

H

V4.2.0

Apply TIA-222-H Section 15.5?

Yes



Analysis Criteria	
Design/Analysis	Analysis
Load Type	Current Load
Current load	208.3 kips
AR Capacity	415.0 kips

Tower Type	Monopole
------------	----------

Manufacturers Tower Prop.	
Pole Thickness	0.3125 in
Pole Grade	A572-65
Pole Sides #	18
Pole Base OD	32 in
Fy	65 ksi
Fu	80 ksi
Base Plate Gr.	A572-60
Fy	60 ksi
Fu	75 ksi
Anchor Rods	
Size	2.25 in
Quantity	8
Bolt Circle	40 in
Grade	A615-75
Fy	75 ksi
Fu	100 ksi

Post-Installed Adhesive AR Mod.	
ARB Type	Welded
Size	2.25 in
AR Layout	Symmetric
Quantity	3
Bolt Circle	44.0 in
Grade	'22-150 (William
Fy	127.7 ksi
Fu	150 ksi

Anchor Rod Bracket Analysis Checks		
Tube Bearing	40.5%	-
Tube Compression	N/A	
Gusset Shear	12.2%	-
Gusset Flexure	N/A	-
Welds	Gusset to Tower and BP	61.8% -
	Gusset to Tube	13.0% -
	Geometry	N/A -
Tower Punching	24.5%	-
Tube Punching	8.8%	-
Utilization		61.8%

Bracket Properties		
Gusset	Pipe/Tube	Weld - Gusset to Pipe/Tube
Thickness	1.25 in	FEXX
Width at Tube	3.5 in	70 ksi
Height at Pole	36 in	Weld Type
Height at Tube	36 in	Double Bevel+Fillet
Grade	A572-65	Fillet Size
Fy	65 ksi	5/8 in
Fu	80 ksi	Bevel Depth
		1/2
Weld - Gusset to Tower	Weld - Gusset to Base Plate	
FEXX	FEXX	
70 ksi	70 ksi	
Weld Type	Weld Type	
Double Fillet	Double Bevel+Fillet	
Fillet Size	Fillet Size	
3/8 in	5/8 in	
Length	Bevel Depth	
36 in	1/2 in	
Load Angle	Gap	
45 deg.	0 in	
	Notch (horiz)	
	0.75 in	
	Notch (vert)	
	0.75 in	
	Pipe/Tube Welded to Base/Footpad?	
	Yes	
	Fillet Size	
	3/8 in	

PROJECT **127643.006.01 - (F) E.GRANBY 4Q2000 GALASSO, CT**

SUBJECT **Anchor Rod Bracket Analysis**

DATE **07/27/19**

TIA-222 Rev.

H

V4.2.0

Apply TIA-222-H Section 15.5?

Yes



B+T GRP
1717 S. Boulder, Suite 300
Tulsa, OK 74119
(918) 587-4630

Analysis Criteria	
Design/Analysis	Analysis
Load Type	Current Load
Current load	101.24 kips
AR Capacity	199.5 kips

Tower Type	Monopole
------------	----------

Manufacturers Tower Prop.	
Pole Thickness	0.3125 in
Pole Grade	A572-65
Pole Sides #	18
Pole Base OD	32 in
Fy	65 ksi
Fu	80 ksi
Base Plate Gr.	A572-60
Fy	60 ksi
Fu	75 ksi
Anchor Rods	
Size	2.25 in
Quantity	8
Bolt Circle	40 in
Grade	A615-75
Fy	75 ksi
Fu	100 ksi

Post-Installed Adhesive AR Mod.	
ARB Type	Welded
Size	1.75 in
AR Layout	Asymmetric
Quantity	2
Bolt Circle	47.1 in
Grade	A193 Gr B7
Fy	105 ksi
Fu	125 ksi

Anchor Rod Bracket Analysis Checks		
Tube Bearing	23.7%	-
Tube Compression	N/A	-
Gusset Shear	7.1%	-
Gusset Flexure	N/A	-
Welds	Gusset to Tower and BP	18.9% -
	Gusset to Tube	22.4% -
	Geometry	N/A -
Tower Punching	14.9%	-
Tube Punching	4.6%	-
Utilization		23.7%

Bracket Properties			
Gusset	Pipe/Tube	Weld - Gusset to Pipe/Tube	
Thickness	1.25 in	FEXX	
Width at Tube	5.5 in	80 ksi	
Height at Pole	36 in	Weld Type	
Height at Tube	30 in	Double Fillet	
Grade	A572-65	Fillet Size	
Fy	65 ksi	5/16 in	
Fu	80 ksi		
	Size		
	HSS4x4x1/2		
	Total Length		
	27 in		
	Length above Gusset		
	0 in		
	Length below Gusset		
	0 in		
	Grade		
	A500 Grade C (Square)		
	Fy		
	50 ksi		
	Fu		
	62 ksi		
Weld - Gusset to Tower		Weld - Gusset to Base Plate	
FEXX	80 ksi	FEXX	80 ksi
Weld Type	Double Fillet	Weld Type	Double Bevel+Fillet
Fillet Size	5/16 in	Fillet Size	9/16 in
Length	36 in	Bevel Depth	9/16 in
Load Angle	45 deg.	Gap	0 in
		Notch (horiz)	0.75 in
		Notch (vert)	1.25 in
		Pipe/Tube Welded to Base/Footpad?	Yes
		Fillet Size	1/2 in

Monopole Base Plate Connection

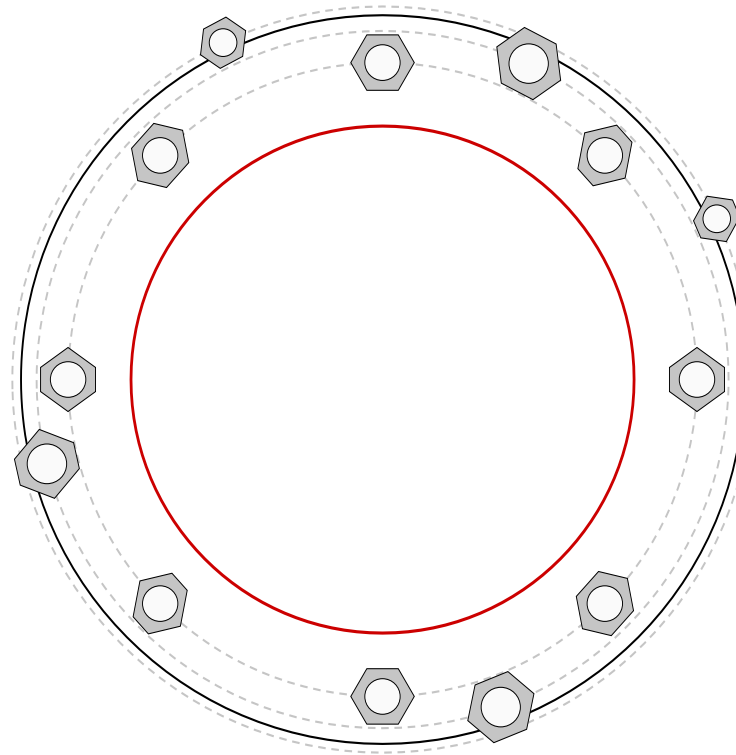


Site Info	
BU #	876399
Site Name	GRANBY 4Q2000 / GAL
Order #	397084, Rev. 4

Analysis Considerations	
TIA-222 Revision	H
Grout Considered:	No
l_{ar} (in)	0

Applied Loads	
Moment (kip-ft)	1728.24
Axial Force (kips)	29.43
Shear Force (kips)	24.95

*TIA-222-H Section 15.5 Applied



Connection Properties	Analysis Results
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Anchor Rod Data

GROUP 1: (8) 2-1/4" ϕ bolts (A615-75 N; $F_y=75$ ksi, $F_u=100$ ksi) on 40" BC
 GROUP 2: (3) 2-1/2" ϕ bolts (Williams N; $F_y=127.7$ ksi, $F_u=125$ ksi) on 44" BC
pos. (deg): 65, 194, 290

GROUP 3: (2) 1-3/4" ϕ bolts (A193 Gr. B7 N; $F_y=105$ ksi, $F_u=125$ ksi) on 47.1" BC

Base Plate Data

46" OD x 1.5" Plate (A572-60; $F_y=60$ ksi, $F_u=75$ ksi)

Stiffener Data

N/A

Pole Data

32" x 0.3125" 18-sided pole (A572-65; $F_y=65$ ksi, $F_u=80$ ksi)

Anchor Rod Summary (units of kips, kip-in)		
GROUP 1:		
$P_{u_c} = 165.44$	$\phi P_{n_c} = 243.75$	Stress Rating
$V_u = 3.12$	$\phi V_n = 73.13$	64.8%
$M_u = n/a$	$\phi M_n = n/a$	Pass
GROUP 2:		
$P_{u_c} = 208.3$	$\phi P_{n_c} = 510.8$	Stress Rating
$V_u = 0$	$\phi V_n = 153.24$	38.8%
$M_u = n/a$	$\phi M_n = n/a$	Pass
GROUP 3:		
$P_{u_c} = 101.24$	$\phi P_{n_c} = 199.5$	Stress Rating
$V_u = 0$	$\phi V_n = 59.85$	48.3%
$M_u = n/a$	$\phi M_n = n/a$	Pass

Base Plate Summary		
Max Stress (ksi):	56.32	(Flexural)
Allowable Stress (ksi):	54	
Stress Rating:	99.3%	Pass



PROJECT : GRANBY 4Q2000
 CLIENT : CROWN CASTLE
 JOB NO. : BU876399

DESIGN BY : KM
 REVIEW BY : US
 DATE : 7/27/2019

STRUCTURAL CAPACITY OF MICROPILE (UNCASED SECTION)

INPUT	
Design Load Comp.	190 kips
Production Test Load Comp.	203
Design Load Ten.	190 kips
Test Load Ten.	203
Nominal Bar Diameter,db	1.75
Yield (F _y) - 0.8 f Pu A	270.00 kips
Bar Diameter	1.75 in
Bit Diameter	5.00 in
Grout Strength (G _c)	5.00 ksi
Design Code (AASHTO or NYC)	TIA-222-G A2 (LRFD)
Unit Weight of Rock rd=	0.110 kcf
Micropile quantity, n	4.00

REV H ANALYSIS

Net Allow. capacity of each Pile=		190	kips
Base Moment	1728	kip-ft	Load per pile
			172
No. of micropile per leg		4	
Grade 150 All-Thread Rebar - Williams R71			
Soil Interaction	86.22%	Pass	
Micropile Steel	70.37%	Pass	

Safety Factor Steel Comp. Design (Yield):	0.64
Safety Factor Steel Comp. Test (Yield):	0.64
Safety Factor Steel Ten. Design (Yield):	0.80
Safety Factor Steel Ten. Test (Yield):	0.80
Safety Factor Grout Design:	0.54
Safety Factor Grout Test:	0.54

Checks:

Load Taken on Grout in Compression

$$F_g = A_g * (G_c * SF)$$

$$A_g = 17.23 \text{ in}^2$$

Design	F _g =	46.68 kips
Test	F _g =	46.68 kips

Load Taken on Steel in Compression

$$F_s = F_y * SF$$

*Steel Must Take 40% of Load

Design	F _s =	172.13 kips	78.7%	Pass
Test	F _s =	172.13 kips		

Total Load Taken on Pile in Compression

$$F_c = F_g + F_s$$

Design	F _c =	218.81 kips	>	190 kips	Pass
Test	F _c =	218.81 kips	>	202.67 kips	Pass

Total Load Taken on Pile in Tension

$$F_t = F_y * SF$$

Design	F _t =	216 kips	>	190 kips	Pass
Test	F _t =	216 kips	>	202.67 kips	Pass

Pull Test load Limited to 0.8*fy (PTI) for steel

GEOTECHNICAL BOND LENGTH OF MICROPILE

Geotechnical Design					
Geo-strata 1 ultimate bond stress, tu1 =				0	psi
Geo-strata 2 ultimate bond stress, tu2 =				17	psi
Geo-strata 3 ultimate bond stress, tu3 =				11	psi
Geo-strata 4 ultimate bond stress, tu4 =				21	psi
Geo-strata 5 ultimate bond stress, tu5 =				23	psi
Geo-strata 6 ultimate bond stress, tu6 =				450	psi
	<u>Ult kips/ft</u>	<u>FS (Design)-2</u>		<u>Depth</u>	<u>Load</u>
Soil-Grout Bond, α1 =	0.00	0.00	kips/ft	10	0.00 kips
Soil-Grout Bond, α2 =	3.11	2.33	kips/ft	5	11.66 kips
Soil-Grout Bond, α3 =	2.07	1.56	kips/ft	5	7.78 kips
Soil-Grout Bond, α4 =	3.86	2.90	kips/ft	5	14.49 kips
Soil-Grout Bond, α5 =	4.34	3.25	kips/ft	5	16.26 kips
Soil-Grout Bond, α5 =	84.82	63.62	kips/ft	2.5	159.04 kips
Bond Length				<u>22.5 FT</u>	
Total Length				<u>32.5 FT</u>	209 kips Pass

DELTA OAKS GROUP



SUBSURFACE STRENGTH PARAMETERS - MICROPILES

Boring	Depth (bgs)	Ultimate Grout to Ground Bond Strength - Type "A"		Ultimate Grout to Ground Bond Strength - Type "B"	
		Compression (psi)	Uplift (psi)	Compression (psi)	Uplift (psi)
B-1	0.0 – 3.3	-	-	-	-
	3.3 – 6.0	10.0	10.0	14.5	14.5
	6.0 – 8.0	6.5	6.5	8.0	8.0
	8.0 – 10.0	13.5	13.5	21.0	21.0
	10.0 – 15.0	22.0	16.5	34.5	25.5
	15.0 – 20.0	15.0	11.0	20.5	15.0
	20.0 – 25.0	27.5	20.5	45.0	33.5
	25.0 – 30.0	31.0	23.0	52.0	39.0
	30.0 – 50.0	450	450	-	-

- The micropiles should have an adequate design embedment length to resist the applied loads.
- Group effects can contribute to a reduction in resistance for the micropiles and should be taken into consideration during foundation analysis.
- Delta Oaks Group recommends an appropriate factor of safety be utilized and the appropriate manufacturer recommendations be followed for the analysis of the micropiles.

Pile Foundation

Checks the capacity of pile foundation configurations for monopoles or self-support towers with individual foundations in Rev. F, G, and H.



BU #: 876399
 Site Name: (F) E. GRANBY 4Q2000 / GAL
 Order: 397084, Rev. 4

Tower Type: Monopole
 TIA Revision: H

Top & Bot. Pad Rein. Different?:

Factored Design Reactions At Base		
Moment, M:	1728	ft-kips
Axial, Pu:	29	kips
Shear, Sc:	25	kips
Load Eccentricity, Ecc:	6	in
Bolt Circle / Bearing Plate Width, BC:	40	in

Pile Properties		
Pile Shape:	Round	
Pile Material:	Steel	
Length of Pile, Lpile:	32	ft
Pile Diameter:	6.0	in
Pile (Soil) Capacity Given?	Yes	
Steel Grade, Fy:	150	ksi

Pile Group		
Group Configuration:	Rectangular	
Number of Columns, Nx:	2	
Number of Rows, Ny:	2	
Column Spacing, Dx:	120	in
Row Spacing, Dy:	120	in
Orientation of Neutral Axis, θ:	0	deg
Group Efficiency Given in Geotech?	No	

Program Calculated Group Efficiency, Eg: 1.00

Pile Cap		
Cap Type:	Block	
Depth to Bottom of Block, D:	3.00	ft
Thickness of Block, T:	3.00	ft
Block Width, Wx:	14.00	ft
Block Length, Wy:	14.00	ft
Pad Rebar Size (Bot.), Spad:	8	
Pad Rebar Quantity (X-direction) (Bot.), Mpad:	15	
Pad Rebar Quantity (Y-direction) (Bot.), Mpad _y :	15	

Material Properties		
Rebar Grade, Fy:	60	ksi
Concrete Strength, Fc:	4	ksi
Clear Cover, cc:	3	in

Soil Properties		
Groundwater Depth, GW:	99.00	ft
Soil Unit Weight:	105	pcf
Cohesion, Co:	0	ksf
Friction Angle, φ:	0	deg
Neglected Depth, ND:	2	ft
Negative Friction Force (per pile), Sw:	0	kips
SPT Blow Count, N _{blows} :	10	

Design Checks				
	Capacity	Demand	Rating*	Check
PILE CHECKS				
Soil Compression (kips per pile):	190.00	173.41	86.9%	Pass
Soil Uplift (kips per pile):	190.00	111.04	55.7%	Pass
Pile Tensile Strength (kips):	216.00	111.04	49.0%	Pass
PAD CHECKS				
One-Way Shear (kips):	502.04	212.07	40.2%	Pass
Pad Shear - Comp Two-Way (ksi):	0.164	0.003	1.8%	Pass
Flexural Two-Way (Comp) (kip*ft):	3293.10	0.00	0.0%	Pass
Pad Flexure (kip*ft):	1646.55	477.26	27.6%	Pass

*Rating per TIA-222-H Section 15.5

Soil Rating:	86.9%
Structural Rating:	49.0%

Ultimate Pile Capacities		
Ultimate Compression, Cn:	253.3333333	kips
Ultimate Tension, Tn:	253.3333333	kips

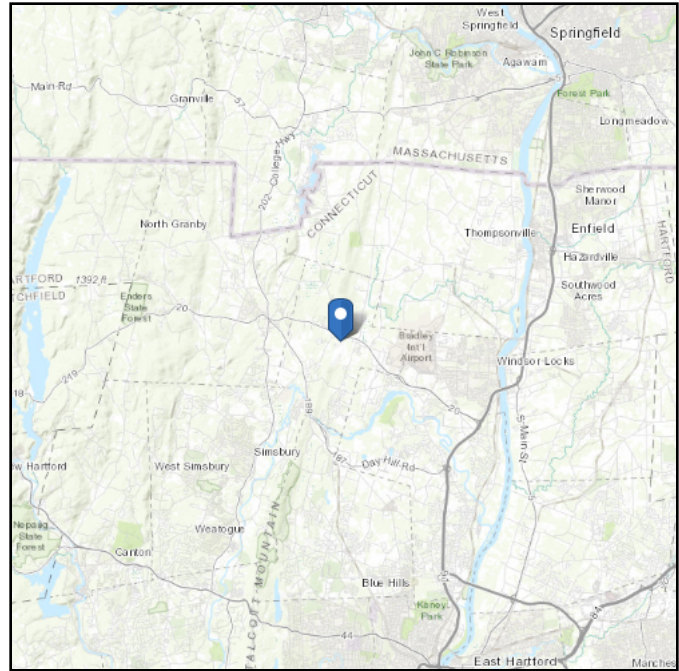
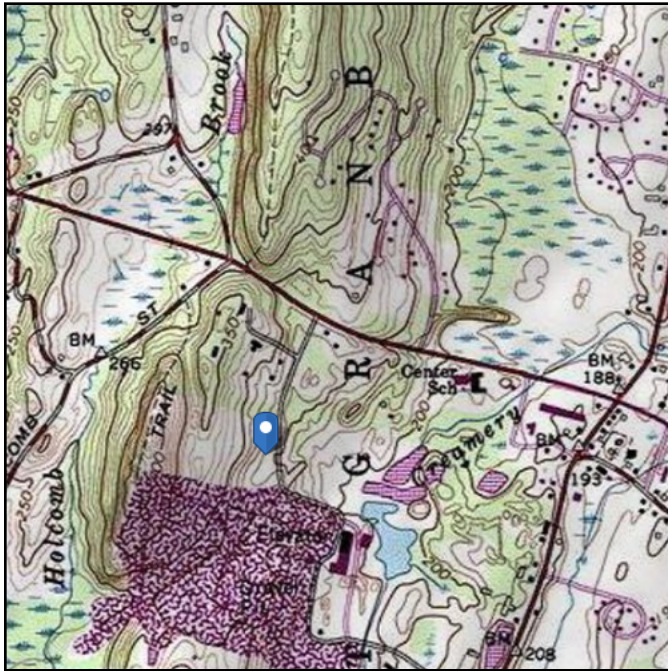
Per CCI sites Doc. # 8420875

ASCE 7 Hazards Report

Address:
No Address at This Location

Standard: ASCE/SEI 7-10
Risk Category: II
Soil Class: D - Stiff Soil

Elevation: 255.76 ft (NAVD 88)
Latitude: 41.941553
Longitude: -72.73868



Wind

Results:

Wind Speed:	119 Vmph
10-year MRI	76 Vmph
25-year MRI	86 Vmph
50-year MRI	91 Vmph
100-year MRI	98 Vmph

Data Source: ASCE/SEI 7-10, Fig. 26.5-1A and Figs. CC-1–CC-4, incorporating errata of March 12, 2014

Date Accessed: Thu Jul 25 2019

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.

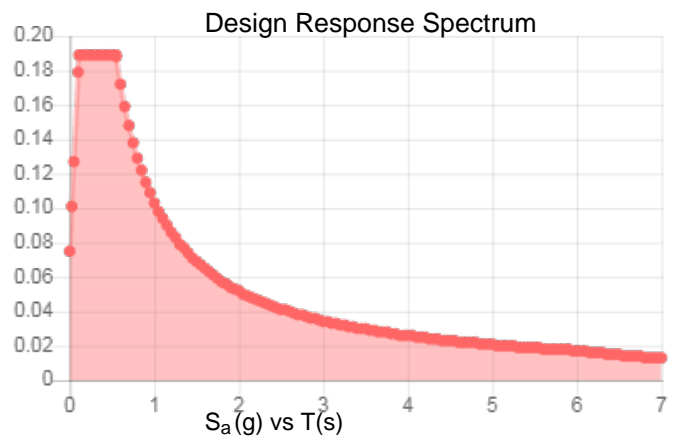
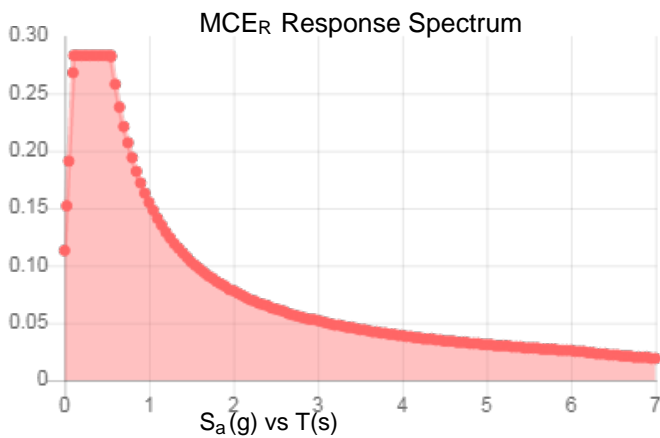
Mountainous terrain, gorges, ocean promontories, and special wind regions should be examined for unusual wind conditions.

Site Soil Class: D - Stiff Soil

Results:

S_S :	0.177	S_{DS} :	0.189
S_1 :	0.065	S_{D1} :	0.103
F_a :	1.6	T_L :	6
F_v :	2.4	PGA :	0.087
S_{MS} :	0.283	PGA _M :	0.14
S_{M1} :	0.155	F _{PGA} :	1.6
		I_e :	1

Seismic Design Category B



Data Accessed:

Thu Jul 25 2019

Date Source:

USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-10 Ch. 21 are available from USGS.

Ice

Results:

Ice Thickness: 1.00 in.

Concurrent Temperature: 5 F

Gust Speed: 50 mph

Data Source: Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Thu Jul 25 2019

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

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