

HPC Wireless Services

46 Mill Plain Rd.

Floor 2

Danbury, CT, 06811

P.: 203.797.1112



November 15, 2012

VIA OVERNIGHT COURIER

Connecticut Siting Council
 10 Franklin Square
 New Britain, Connecticut 06051
 Attn: Ms. Linda Roberts, Executive Director



Re: Sprint Spectrum, L.P. – exempt modification
303 Boxwood Lane, Danbury, Connecticut

Dear Ms. Roberts:

This letter and attachments are submitted on behalf of Sprint Spectrum, L.P. (“Sprint”). Sprint is undertaking modifications to certain existing sites in its Connecticut system in order to implement updated technology. Please accept this letter and attachments as notification, pursuant to R.C.S.A. Section 16-50j-73, of construction that constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2). In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter and attachments is being sent to the Mayor of the City of Danbury.

Sprint plans to modify the existing wireless communications facility owned by Western Connecticut State University and located at 303 Boxwood Lane in the City of Danbury (coordinates 41°23'41.93” N, 73°29'12.27” W). Attached are a compound plan and elevation depicting the planned changes, and documentation of the structural sufficiency of the structure to accommodate the revised antenna configuration. Also included is a power density report reflecting the modification to Sprint’s operations at the site.

The changes to the facility do not constitute a modification as defined in Connecticut General Statutes (“C.G.S.”) Section 16-50i(d) because the general physical characteristics of the facility will not be significantly changed. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in R.C.S.A. Section 16-50j-72(b)(2).

1. Sprint will replace three (3) existing CDMA antennas with three (3) dual-band panel antennas on existing mounts at a center line of approximately 89’. Six (6) RRHs

(remote radio heads) will be mounted to the existing frames adjacent to the antennas. Sprint will also install three (3) hybridflex cables along the existing coaxial cable run. The proposed modifications will not extend the height of the approximately 100' structure.

2. The proposed changes will not extend the site boundaries. Sprint will replace two (2) cabinets, add one (1) cabinet and install a fiber box on unistruts, all on the existing equipment pad. These changes will have no effect on the site boundaries.

3. The proposed changes will not increase the noise level at the existing facility by six decibels or more. The incremental effect of the proposed changes will be negligible.

4. The changes to the facility will not increase the calculated "worst case" power density for the combined operations at the site to a level at or above the applicable standard for uncontrolled environments as calculated for a mixed frequency site. As indicated on the attached report prepared by EBI Consulting, Sprint's operations at the site will result in a power density of approximately 64.739%; the combined site operations will result in a total power density of approximately 91.919%.

Please feel free to contact me by phone at (860) 798-7454 or by e-mail at jgaudet@hpcwireless.com with questions concerning this matter. Thank you for your consideration.

Respectfully yours,



Jennifer Young Gaudet

cc: Honorable Mark Boughton, Mayor, City of Danbury
Western Connecticut State University (underlying property owner)



1 INTERNATIONAL BLVD, SUITE 800
HARTFORD, CT 06183
P: 800-357-7441



1 CORNING ROAD
WESTFORD, MA 01886
P: (978) 932-1600



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2 East Palisade Avenue
Fairfield, NJ 07004
P: 201-507-0032 F: 201-507-8556

New England Office:
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Riverside, CT 06457
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NO	DATE	DESCRIPTION	BY
1	04/24/07	PRELIMINARY	AD
2	05/02/07	AS PER CLIENT COMMENT	AD
3	07/19/07	ISSUED AS FINAL	AD

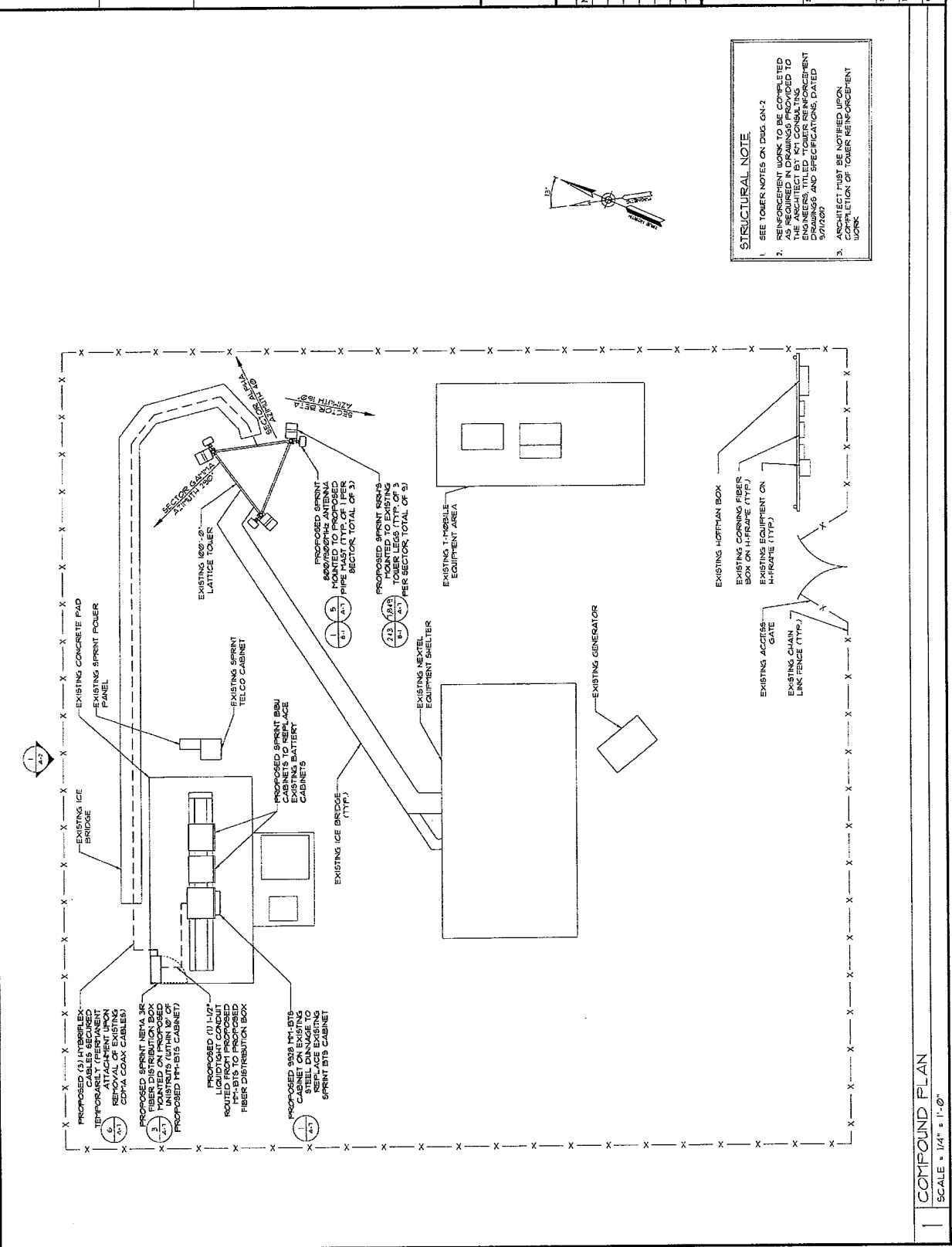
SITE NUMBER:
CT 43XC236

SITE NAME:
DANBURY-III CT UNIVERSITY

SITE ADDRESS:
303 BOXWOOD LANE
DANBURY, CT 06810

COMPOUND PLAN

SHEET FROM NO. CT 43XC236
SHEET NO. **A-1**
CHECKED BY: ADC



COMPOUND PLAN
SCALE = 1/4" = 1'-0"



1 INTERNATIONAL BLVD, SUITE 800
 WASHINGTON, DC 20004
 P: 800.337.7241

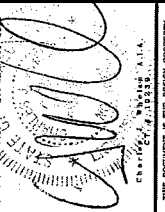


Alcatel-Lucent
 WESTBORO, MA 01581
 P: (978) 932-1600



New Jersey Office:
 8 East Palisade Avenue
 Suite 200
 Westfield, NJ 07090
 P: 908.427.0028 F: 908.427.0049

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SUBMITTALS			
NO.	DATE	DESCRIPTION	BY
1	06/01/10	PRELIMINARY	PAF
2	06/01/10	AS PER CLIENT COMMENT	PAF
3	06/02/10	ISSUED AS FINAL	AD

DATE NUMBER
 CT143XC236

SITE NAME:
 DANBURY-W. CT
 UNIVERSITY
 303 BOWDOEN LANE
 DANBURY, CT 06810

SHEET TITLE
 ELEVATION

SHEET NO.
 CT143XC236

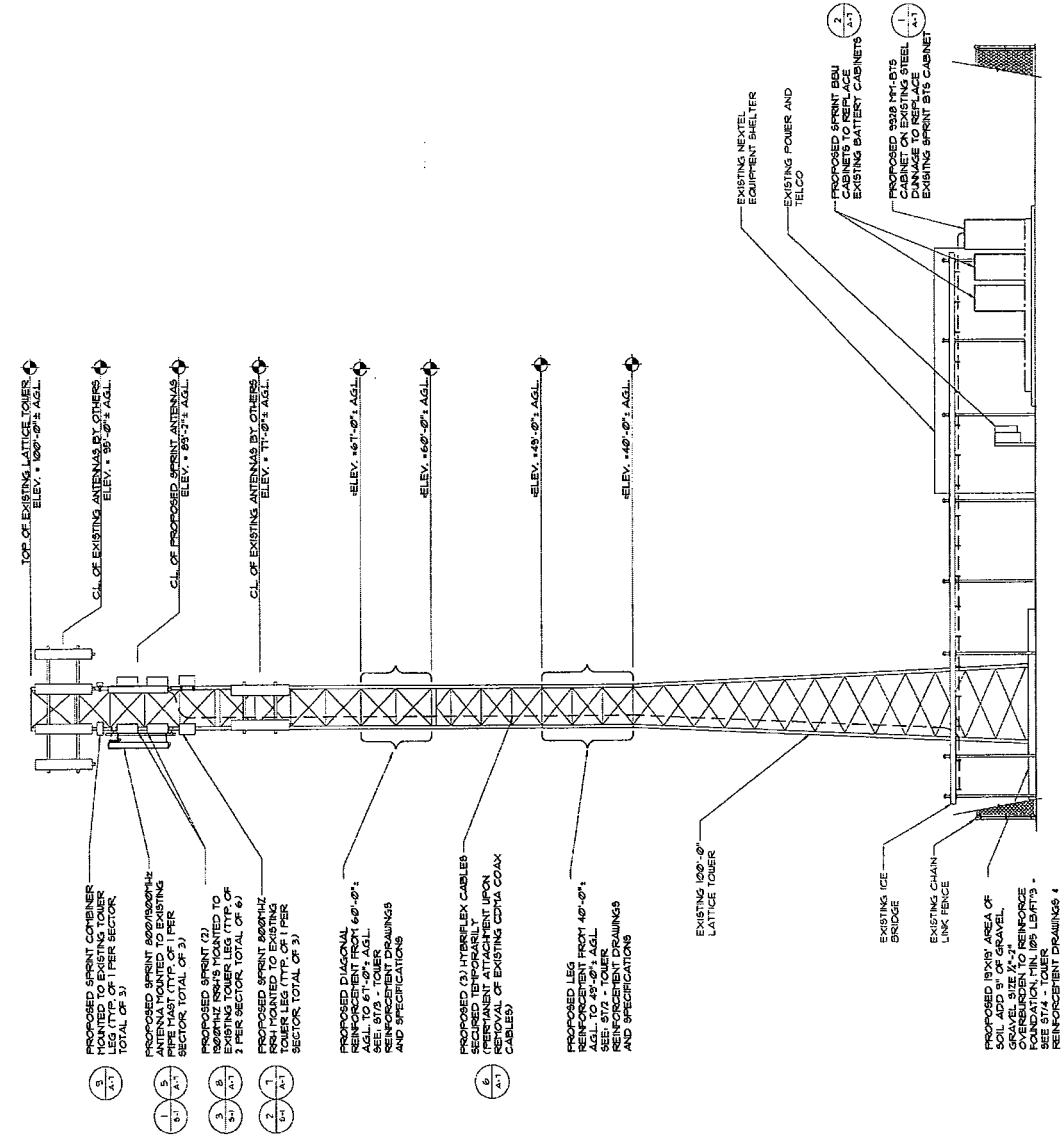
UNITS
 INCHES

DRAWN BY:
 ADC

A-2

ANTENNA CONFIGURATION NOTE
 ALL EXISTING CDMA ANTENNAS TO BE REMOVED FROM EXISTING TOWER ANTENNAS FOR FINAL CONFIGURATION. ANTENNA REPAIRS TO BE FIELD VERIFIED BY THE GENERAL CONTRACTOR.

STRUCTURAL NOTE
 1. SEE TOWER NOTES ON DWG. GN-2
 2. REINFORCEMENT WORK TO BE COMPLETED AS REQUIRED IN DRAWINGS PROVIDED TO THE GENERAL CONTRACTOR. ALL REINFORCEMENT WORKERS TO BE PROVIDED WITH BUSINESS TITLED TOWER REINFORCEMENT DRAWINGS AND SPECIFICATIONS, DATED 9/7/09.
 3. ARCHITECT MUST BE NOTIFIED UPON COMPLETION OF TOWER REINFORCEMENT WORK.



- 1. PROPOSED SPRINT 800MHz ANTENNA MOUNTED TO EXISTING TOWER LEG (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- 2. PROPOSED SPRINT 1900MHz ANTENNA MOUNTED TO EXISTING TOWER LEG (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- 3. PROPOSED SPRINT 800MHz REINFORCEMENT DRAWINGS AND SPECIFICATIONS.
- 4. PROPOSED SPRINT 1900MHz REINFORCEMENT DRAWINGS AND SPECIFICATIONS.

- 5. PROPOSED DIAGONAL REINFORCEMENT FROM 60'-0" AGL TO 45'-0" AGL. SEE 573 - TOWER REINFORCEMENT DRAWINGS AND SPECIFICATIONS.
- 6. PROPOSED (2) JIBS/REFLEX CABLES TO EXISTING TOWER (PERMANENT ATTACHMENT UPON REMOVAL OF EXISTING CDMA COAX CABLES).

- 7. PROPOSED LEG REINFORCEMENT FROM 40'-0" AGL TO 45'-0" AGL. SEE 574 - TOWER REINFORCEMENT DRAWINGS AND SPECIFICATIONS.

- 8. PROPOSED BRUSH AREA OF SOIL ADD 5" OF GRAVEL. GRAVEL SIZE 1/2"-3/4". SEE 574 - TOWER REINFORCEMENT DRAWINGS AND SPECIFICATIONS.

1 ELEVATION
 SCALE: 3/16" = 1'-0"



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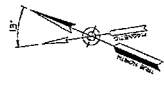
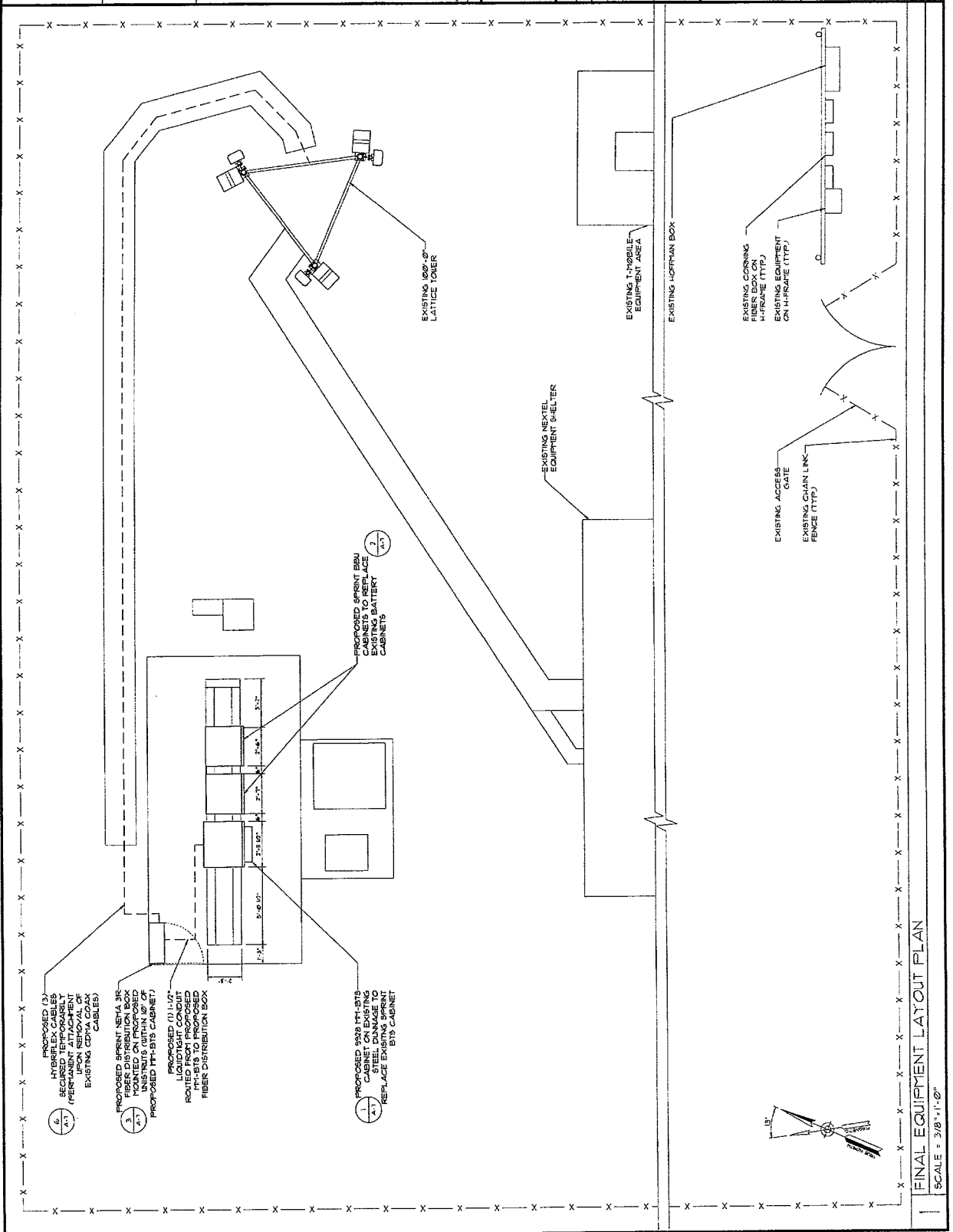
SUBMITTALS

NO.	DATE	DESCRIPTION	BY
1	08/14/10	PRELIMINARY	M.P.
2	08/14/10	AS PER CLIENT COMMENT	SA
3	08/14/10	BASED AS FINAL	AD

SITE NUMBER: CT43XC896
SITE NAME: DANBURY-WI, CT UNIVERSITY
SITE ADDRESS: 305 BOSWOOD LANE DANBURY, CT 06810

SHEET TITLE: ENLARGED EQUIPMENT CABINET LAYOUT

SHEET NO.: A-5
PROJECT NO.: CT43XC896
DATE: 08/14/10
DESIGNED BY: ADG



FINAL EQUIPMENT LAYOUT PLAN
SCALE = 3/8" = 1'-0"



1 INTERNATIONAL BLVD, SUITE 600
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Alcatel-Lucent
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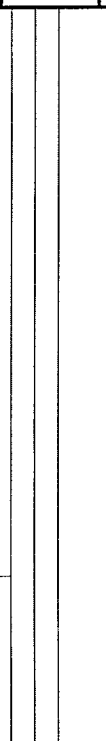
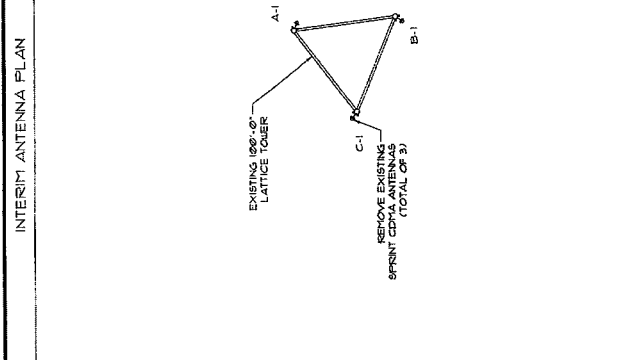
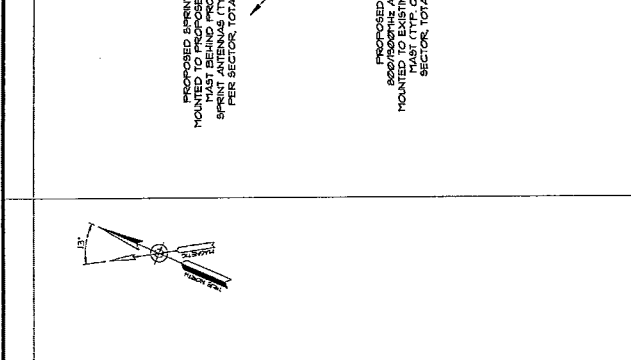
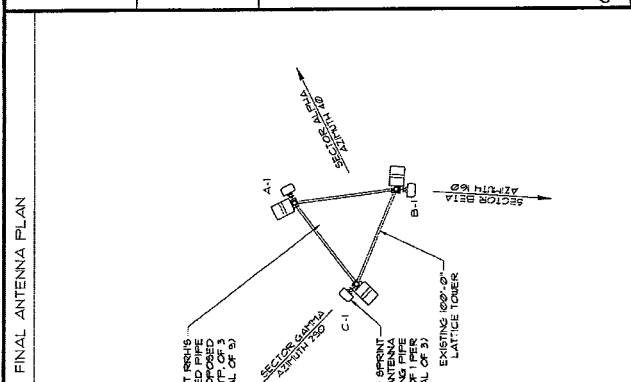
SUBMITTALS

NO	DATE	DESCRIPTION	BY
1	08/20/12	PRELIMINARY	SA
2	09/10/12	AS PER CLIENT COMMENTS	SA
3	09/10/12	ISSUED AS FINAL	AD

SITE NUMBER: **CT43XC836**
SITE NAME: **DANBURY III CT UNIVERSITY**
SITE ADDRESS: **303 BOXWOOD LANE DANBURY, CT 06810**

ANTENNA SCENARIO AND RF SYSTEM SCHEDULE

SAFETY PROJ. NO: **CT43XC836**
DATE: _____
CHECKED BY: **ADC**



4	POSITION	ANTENNA STATUS	FREQUENCY (MHz)	ANTENNA MAKE	ANTENNA MODEL	AZIMUTHS (EXISTING / PROPOSED)	MECHANICAL DOWN TILT	ELECTRICAL DOWN TILT	RAD CENTER (TAGL)	HYBRID CABLE LENGTH (FT)	RRH MODEL	TOP COAX JUMPER SIZE (IN)	TOP COAX JUMPER LENGTH (FT)	TOP COAX JUMPER MAKE	TOP COAX JUMPER MODEL	COMBINER	COMBINER LENGTH (FT)	ANTENNA CODING
1	A-1	PROPOSED	800/1900	RF6	APXV9FP18-C-A20	60°	0°	-6°	89'-3"	115	(1) 8007H4 (2) 18007H4	1/2	10	RF6	(2) LCFR-50J	BCP3004B-2	6	TBD
2	B-1	PROPOSED	800/1900	RF6	APXV9FP18-C-A20	160°	0°	-3°	69'-1"	115	(1) 8007H4 (2) 18007H4	1/2	10	RF6	(2) LCFR-50J	BCP3004B-2	6	TBD
3	C-1	PROPOSED	800/1900	RF6	APXV9FP18-C-A20	210°	0°	0°	69'-1"	115	(1) 8007H4 (2) 18007H4	1/2	10	RF6	(2) LCFR-50J	BCP3004B-2	6	TBD

* CONTRACTOR TO FIELD VERIFY ALL CABLE/JUMPER LENGTHS AGAINST CURRENT B.O.M.

RF SYSTEM SCHEDULE

SCALE = N.T.S.

2



KM Consulting Engineers, Inc.

Wireless Engineering and Project Management

October 3, 2012

Mr. Charles Whelan
Salient Associates, LLC
8 East Palisade Avenue
Englewood, NJ 07631

Re: Danbury
Salient Site No. CT43XC836
KM Project No. 120741.01

Dear Mr. Whelan:

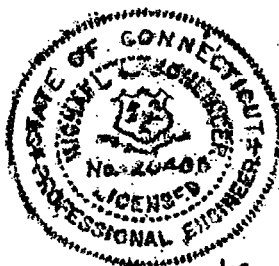
Further to your request, KM Consulting Engineers, Inc. (KMCE) has re-analyzed the CT43XC836 self support tower to include the proposed reinforcement as per KMCE drawings dated 9/21/12.

With the proposed modifications, KMCE finds the tower to have a stress rating of 99.6%, and therefore is deemed acceptable as per the TIA/EIA-222-F standards. See attached trnTower output pages for more details.

Should you have any questions or comments, please do not hesitate to contact our office.

Sincerely,
KM CONSULTING ENGINEERS, INC.

Michael Bohlinger, PE
Principal
CT License No. 20405

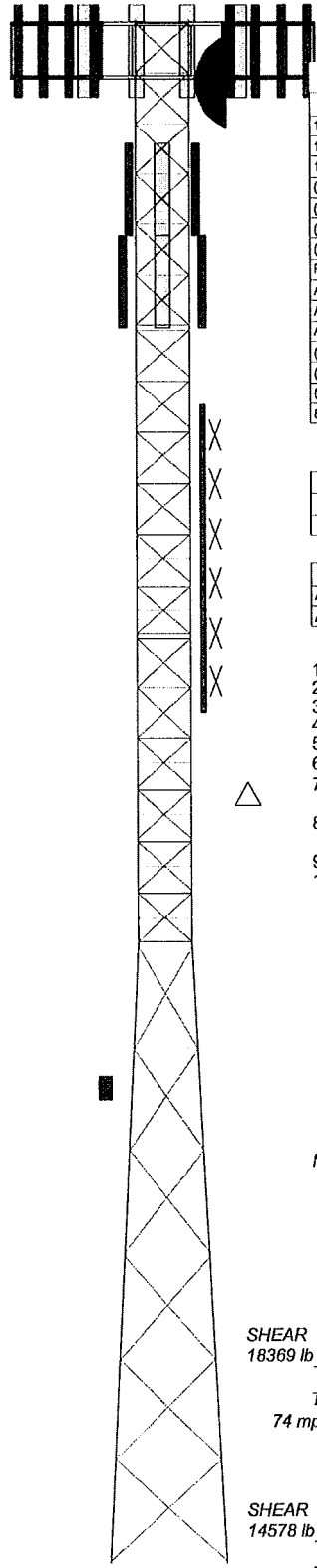
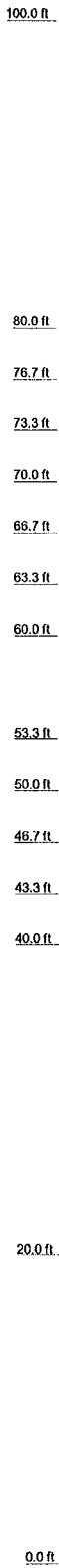


10/2/12
MB

K:\Salient Associates\Structural CT Sprint Jobs\CT43XC836\Admin\Reinforcement Letter.wpd

32 West Upper Ferry Road, Ewing, New Jersey 08628 Tel. (609)538-0400 Fax. (609)538-8858
Email: info@kmengr.com VISIT OUR WEB PAGE @ www.kmengr.com

Section	T14	T13	T12	T11	T10	T9	T8	T7	T6	T5	T4	T3	T2	T1
Legs	P5x.375 (GR)	A500M-61	A500M-61	A	P3x.3 (GR)	P3x.3 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)
Leg Grade	A500-50	A500M-61	A500M-61	A	P3x.3 (GR)	P3x.3 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)	P2.5x.276 (GR)
Diagonals	L2 1/2x2 1/2x3/16	L2x2x3/16	L2x2x3/16	L2x2x3/16	L2x2x3/16	L2x2x3/16	L2x2x3/16	L2x2x3/16	L2x2x3/16	L2x2x3/16	L2x2x3/16	L2x2x3/16	L2x2x3/16	L2x2x3/16
Diagonal Grade	A36	A36	A36	A36	A36	A36	A36	A36	A36	A36	A36	A36	A36	A36
Top Chords	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.
Bottom Chords	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.
Horizontals	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.
Sec. Horizontals	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.
Face Width (ft)	7.5	5.5	5.5	5.5	5.5	5.5	5.5	5.5	5.5	5.5	5.5	5.5	5.5	5.5
# Panels @ (ft)	6 @ 5.55556	6 @ 3.16667	4 @ 3.16667	4 @ 3.16667	4 @ 3.16667	2 @ 3.25	2 @ 3.25	2 @ 3.25	2 @ 3.25	2 @ 3.25	2 @ 3.25	2 @ 3.25	2 @ 3.25	2 @ 3.25
Weight (lb)	9374.2	2995.5	2757.1	310.7	275.0	302.5	286.6	591.9	287.0	249.3	187.4	187.4	187.4	772.2



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
12' Boom Starmount	98	800 MHz RRH (Sprint)	89
12' Boom Starmount	98	800 MHz RRH (Sprint)	89
12' Boom Starmount	98	APX16PV-16PVL-X w/mount pipe (T-Mobile)	83
(4) DBB44H90	98	APX16PV-16PVL-X w/mount pipe (T-Mobile)	83
(4) DBB44H90	98	APX16PV-16PVL-X w/mount pipe (T-Mobile)	83
(2) 60"x12"x4" Panel Antenna	98	APX16PV-16PVL-X w/mount pipe (T-Mobile)	83
(2) 71"x10"x5" Panel Antenna	98	APX16PV-16PVL-X w/mount pipe (T-Mobile)	83
Parabolic Grid	96	(2) ATMAA1412D-1A20 Twin TMA (T-Mobile)	80
APXVSP18-C-A20 (Sprint)	89	(2) ATMAA1412D-1A20 Twin TMA (T-Mobile)	80
APXVSP18-C-A20 (Sprint)	89	4-Bay 6810 w/Radome	65
APXVSP18-C-A20 (Sprint)	89	2.5" Tube x 2' Standoff (Sprint)	30
(2) 1900 MHz RRH (Sprint)	89	GPS (Sprint)	30
(2) 1900 MHz RRH (Sprint)	89		
(2) 1900 MHz RRH (Sprint)	89		
800 MHz RRH (Sprint)	89		

SYMBOL LIST

MARK	SIZE	MARK	SIZE
A	P3x.3 (GR) w/ 0.75" threaded rod	C	2L1 1/2x1 1/2x3/16
B	2L1 1/2x1 1/2x1/8		

MATERIAL STRENGTH

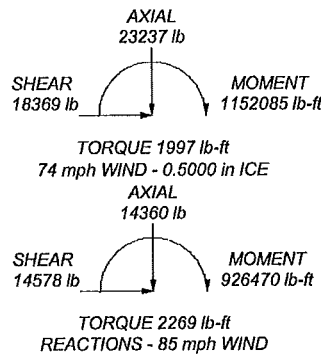
GRADE	Fy	Fu	GRADE	Fy	Fu
A500-50	50 ksi	62 ksi	A500M-61	61 ksi	75 ksi
A36	36 ksi	58 ksi			

TOWER DESIGN NOTES

1. Tower is located in Fairfield County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 74 mph basic wind with 0.50 in ice.
4. Deflections are based upon a 60 mph wind.
5. Grouted pipe f'c is 8 ksi
6. Weld together tower sections have flange connections.
7. Connections use galvanized A325 bolts, nuts and locking devices. Installation per TIA/EIA-222 and AISC Specifications.
8. Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
9. Welds are fabricated with ER-70S-6 electrodes.
10. TOWER RATING: 99.6%

MAX. CORNER REACTIONS AT BASE:

DOWN: 181005 lb
 UPLIFT: -165909 lb
 SHEAR: 12760 lb



KM Consulting Engineers Inc. 32 West Upper Ferry Road Ewing, NJ. 08628 Phone: (609) 538-0400 FAX: (609) 538-8858	Job: CT43XC836 LC1
	Project: 100 ft. Self Support Tower
	Client: Saliient Associates Code: TIA/EIA-222-F Path: K:\Saliient\Associates\Structural\CT Sprint Job\CT43XC836\Engineering\CT43XC836 LC1\kored.rvt
	Drawn by: Michael Bohlinger Date: 10/03/12
	App'd: Scale: NTS Dwg No. E-1

tnxTower KM Consulting Engineers Inc. 32 West Upper Ferry Road Ewing, NJ. 08628 Phone: (609) 538-0400 FAX: (609) 538-8858	Job CT43XC836 LC1	Page 36 of 37
	Project 100 ft. Self Support Tower	Date 09:51:03 10/03/12
	Client Salient Associates	Designed by Michael Bohlinger

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SF*P _{allow} lb	% Capacity	Pass Fail
T1	100 - 80	Leg	P2.5x.276	3	-28754.00	76501.53	37.6	Pass
		Diagonal	5/8	10	6519.77	8833.52	73.8	Pass
		Horizontal	L1 1/2x1 1/2x3/16	17	-4743.19	4905.28	96.7	Pass
		Top Girt	L1 1/2x1 1/2x3/16	5	-2486.63	4427.01	56.2	Pass
		Bottom Girt	L1 1/2x1 1/2x3/16	8	-1894.96	4427.01	42.8	Pass
T2	80 - 76.6667	Leg	P2.5x.276 (GR)	63	-36246.90	96899.10	37.4	Pass
		Diagonal	5/8	67	7108.00	8833.52	80.5	Pass
		Top Girt	2L1 1/2x1 1/2x3/16	65	-3529.75	15516.40	22.7	Pass
T3	76.6667 - 73.3333	Leg	P2.5x.276 (GR)	75	-44249.20	96899.10	45.7	Pass
		Diagonal	5/8	79	7489.52	8833.52	84.8	Pass
		Top Girt	2L1 1/2x1 1/2x3/16	77	-5457.26	15516.40	35.2	Pass
T4	73.3333 - 70	Leg	P2.5x.276 (GR)	87	-53226.30	96899.10	54.9	Pass
		Diagonal	5/8	91	8089.33	8833.52	91.6	Pass
		Top Girt	2L1 1/2x1 1/2x3/16	89	-5316.05	15516.40	34.3	Pass
T5	70 - 66.6667	Leg	P2.5x.276 (GR)	99	-62460.10	96899.10	64.5	Pass
		Diagonal	5/8	103	8250.62	8833.52	93.4	Pass
		Top Girt	2L1 1/2x1 1/2x3/16	101	-5251.07	15516.40	33.8	Pass
T6	66.6667 - 63.3333	Leg	P2.5x.276 (GR)	111	68302.20	90119.07	75.8	Pass
		Diagonal	(2) 5/8" SR	118	10893.00	17512.15	62.2	Pass
		Secondary Horizontal	L2x2x1/4	121	-1430.50	16216.74	8.8	Pass
T7	63.3333 - 60	Top Girt	2L1 1/2x1 1/2x3/16	113	-6423.47	15516.40	41.4	Pass
		Leg	P2.5x.276 (GR)	126	79292.20	90119.07	88.0	Pass
		Diagonal	(2) 5/8" SR	136	12936.80	17512.15	73.9	Pass
		Secondary Horizontal	L2x2x1/4	140	-1479.40	16216.74	9.1	Pass
		Top Girt	2L1 1/2x1 1/2x3/16	129	-8744.31	15516.40	56.4	Pass
T8	60 - 53.3333	Bottom Girt	2L1 1/2x1 1/2x3/16	130	-6634.89	20683.36	32.1	Pass
		Leg	P3x.3 (GR)	144	-108457.00	146901.93	73.8	Pass
		Diagonal	3/4	160	11037.70	12720.27	86.8	Pass
T9	53.3333 - 50	Horizontal	2L1 1/2x1 1/2x3/16	154	-10123.40	11995.48	84.4	Pass
		Top Girt	2L1 1/2x1 1/2x3/16	147	-4289.71	8998.86	47.7	Pass
		Leg	P3x.3 (GR)	165	114935.00	147140.53	78.1	Pass
T10	50 - 46.6667	Diagonal	3/4	169	11471.70	12720.27	90.2	Pass
		Secondary Horizontal	L2 1/2x2 1/2x5/16	176	-2099.32	30157.66	7.0	Pass
		Top Girt	2L1 1/2x1 1/2x1/8	166	-8898.26	14414.26	61.7	Pass
		Leg	P3x.3 (GR)	180	128024.00	147140.53	87.0	Pass
		Diagonal	3/4	184	12257.20	12720.27	96.4	Pass
T11	46.6667 - 43.3333	Secondary Horizontal	L2 1/2x2 1/2x5/16	191	-2333.63	30157.66	7.7	Pass
		Top Girt	2L1 1/2x1 1/2x3/16	181	-8760.27	11995.48	73.0	Pass
		Leg	P3x.3(GR) w/ 0.75" threaded rod	195	-148853.00	192039.97	77.5	Pass
T12	43.3333 - 40	Diagonal	3/4	199	12674.80	12720.27	99.6	Pass
		Secondary Horizontal	L2 1/2x2 1/2x5/16	206	-2578.20	30485.18	8.5	Pass
		Top Girt	2L1 1/2x1 1/2x3/16	196	-9141.64	11995.48	76.2	Pass
		Leg	P3x.3(GR) w/ 0.75" threaded rod	210	-162265.00	198854.27	81.6	Pass
		Diagonal	3/4	220	12294.90	12720.27	96.7	Pass
T13	40 - 20	Secondary Horizontal	L2 1/2x2 1/2x5/16	224	-2810.52	30485.18	9.2	Pass
		Top Girt	2L1 1/2x1 1/2x3/16	211	-10192.20	12060.40	84.5	Pass
		Bottom Girt	2L1 1/2x1 1/2x3/16	216	-3619.00	9047.56	40.0	Pass
T14	20 - 0	Leg	P5x.375 (GR)	228	157950.00	244417.54	64.6	Pass
		Diagonal	L2x2x3/16	241	-3335.07	10575.34	31.5	Pass
		Leg	P5x.375 (GR)	249	166380.00	244417.54	68.1	Pass
		Diagonal	L2 1/2x2 1/2x3/16	253	-2024.07	13140.91	15.4	Pass
						38.8 (b)		
						33.8 (b)		

tnxTower KM Consulting Engineers Inc. 32 West Upper Ferry Road Ewing, NJ, 08628 Phone: (609) 538-0400 FAX: (609) 538-8858	Job CT43XC836 LC1	Page 37 of 37
	Project 100 ft. Self Support Tower	Date 09:51:03 10/03/12
	Client Salient Associates	Designed by Michael Bohlinger

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SF*P _{allow} lb	% Capacity	Pass Fail
							Summary	
						Leg (T7)	88.0	Pass
						Diagonal (T11)	99.6	Pass
						Horizontal (T1)	96.7	Pass
						Secondary Horizontal (T12)	9.2	Pass
						Top Girt (T12)	84.5	Pass
						Bottom Girt (T1)	42.8	Pass
						Bolt Checks	56.1	Pass
						RATING =	99.6	Pass

TOWER REINFORCEMENT DRAWINGS & SPECIFICATIONS

T-1	TITLE
ST-1	TOWER ELEVATION
ST-2	LEG REINFORCEMENT: 40' TO 49' AGL
ST-3	DIAGONAL REINFORCEMENT: 60' TO 67' AGL
ST-4	FOUNDATION REINFORCEMENT
ST-5	SPECIAL INSPECTION NOTES
ST-6	SPECIAL INSPECTION NOTES



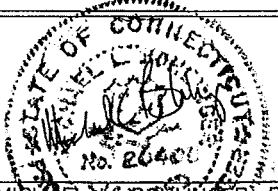
SITE LOCATION: 303 BOXWOOD LANE, DANBURY, CT 06810

SCOPE:

THE PURPOSE OF THESE REINFORCING DETAILS AND SPECIFICATIONS IS TO REINFORCE THE TOWER MEMBERS. A THREADED BAR WILL BE STRAPPED TO THE TOWER LEGS FROM 40' TO 49' AGL. A THREADED BAR WILL BE CLIPPED TO THE EXISTING PIPE LEGS AND RAN ALONGSIDE THE EXISTING DIAGONALS FROM 60' TO 67' AGL. GRAVEL WILL BE ADDED ABOVE THE EXISTING PAD AND PIER FOUNDATION.

REINFORCING INCLUDES: CLAMPING A 3/4"Ø DYWIDAG GRADE 150 THREADED BAR TO THE TOWER LEGS ON ALL THREE FACES FROM 40' TO 50' AGL USING CLAMPS SPACED APPROXIMATELY 1'-6" ON CENTER. SECURING A 5/8"Ø THREADED ROD TO THE EXISTING PIPE LEGS AND RAN ALONG THE EXISTING DIAGONAL MEMBERS USING ANGLE CLIPS. ADDING A 9" LAYER OF GRAVEL IN A 19'x19' AREA ABOVE THE EXISTING PAD AND PIER FOUNDATION.

THIS REINFORCEMENT IS REQUIRED AFTER ANALYZING THE TOWER. REFER TO STRUCTURAL ANALYSIS DATED AUGUST 23RD, 2012.

OWNER:	 KM Consulting Engineers, Inc. <i>Wireless Engineering & Project Management</i> 32 West Upper Ferry Road Ewing, NJ 08628 Phone: (609) 538-0400 Fax: (609) 538-8858	CLIENT:	 SALIENT ASSOCIATES 8 EAST PALISADES AVENUE ENLEWOOD, NJ 07631 (201) 567-0032 PH. (201) 567-9556 FAX	REVISIONS:
	APPROVALS & DATE:	PROJECT NAME:		1 11/8/12
	OWNER: _____ DATE: _____	CT43XC836		NO. DATE
	S.A.C.: _____ DATE: _____	DANBURY		DRAWING NO.:
R / F.: _____ DATE: _____	PROJECT ADDRESS:		T-1	
CONST.: _____ DATE: _____	303 BOXWOOD LANE			
DANBURY, CT 06810	PROJECT #:	SITE ID #:	DRAWING TITLE:	P.C.:
120741.01	TITLE SHEET	MLB	DJA	DATE:
11/8/12				9/21/12

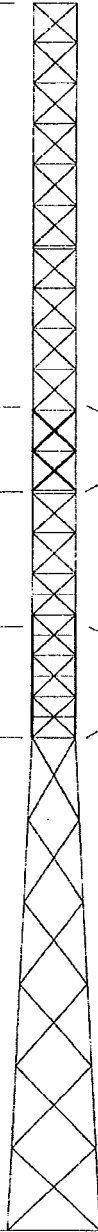
100' AGL

~67' AGL

60' AGL

49' AGL

40' AGL



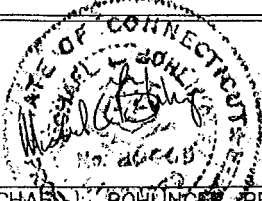


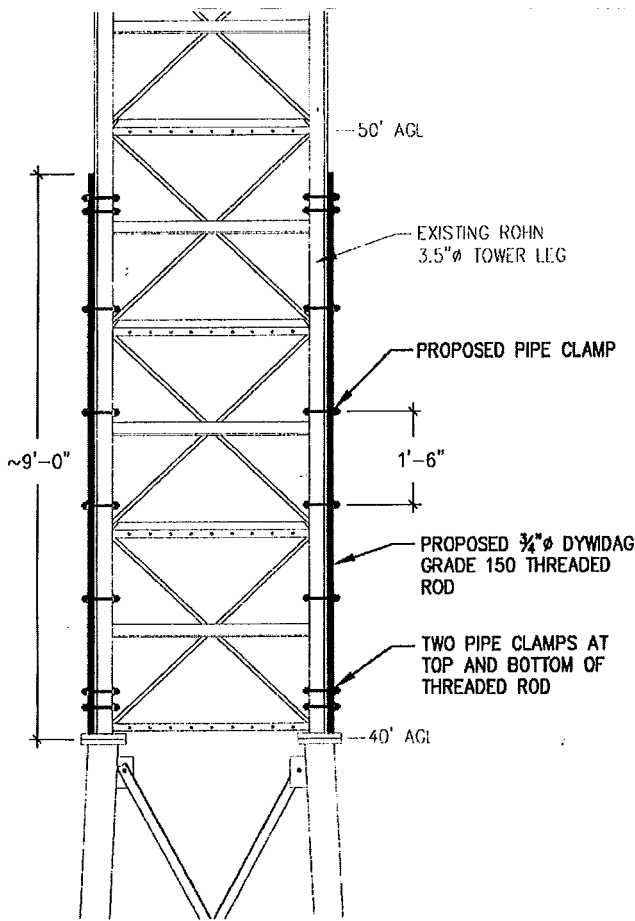
ST
3

ST
2

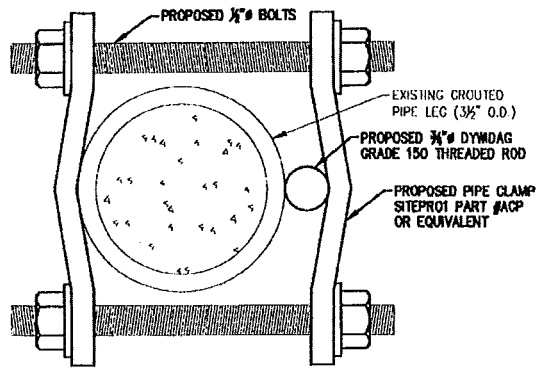
1 TOWER ELEVATION
ST-1

SCALE: NTS

OWNER:	 KM Consulting Engineers, Inc. <i>Wireless Engineering & Project Management</i> 32 West Upper Ferry Road Ewing, NJ 08628 Phone: (609) 638-0400 Fax: (609) 638-8858	CLIENT:  SALIENT ASSOCIATES 8 EAST PALISADES AVENUE ENLEWOOD, NJ 07631 (201) 567-0032 PH. (201) 567-9556 FAX	REVISIONS:
	APPROVALS & DATE: OWNER: _____ DATE: _____ S.A.C.: _____ DATE: _____ R / F.: _____ DATE: _____ CONST.: _____ DATE: _____	PROJECT NAME: CT43XC836 DANBURY PROJECT ADDRESS: 303 BOXWOOD LANE DANBURY, CT 06810	1 11/8/12 NO. DATE ST-1
MICHAEL J. BOHLINGER, P.E. CONNECTICUT PROFESSIONAL ENGINEER LICENSE # 20405	PROJECT #: 120741.01	DRAWING TITLE: TOWER ELEVATION	P.C.: _____ CHKD: MLB DRN: DJA DATE: 9/21/12



1 LEG REINFORCEMENT: 40'-50' AGL
 ST-2 SCALE: 3/8" = 1'-0"



2 SECTION A-A
 ST-2 SCALE: 6" = 1'

NOTES:

1. ALL MEMBERS, BOLTS HOLES, AND DIMENSIONS MUST BE FIELD VERIFIED PRIOR TO FABRICATION / PROCUREMENT OF REINFORCEMENT MATERIALS. ANY CHANGES TO THESE DRAWINGS AND SPECIFICATIONS OR CHANGES FOUND IN THE FIELD OF EXISTING TOWER MEMBERS MUST BE COMMUNICATED TO KM CONSULTING ENGINEERS INC. PRIOR TO INSTALLING REINFORCEMENT.

2. STEEL: ALL STEEL MEMBERS TO BE HOT-DIP GALVANIZED TO ASTM A-123.

3. IF STEEL IS FIELD CUT, ENDS OF STEEL MUST BE SPRAYED WITH COLD GALVANIZE ZRC.

SAFETY NOTICE!

INSTALLATION OF THESE TOWER MODIFICATIONS WILL REQUIRE TOWER CLIMBING AT HEIGHTS WHERE FALLING COULD HARM OR PROVE FATAL TO WORKERS. THESE DRAWINGS INDICATE ONLY THE REINFORCEMENT AND NOT THE MEANS, METHODS, AND REQUIRED CONTRACTOR SAFETY. THESE REINFORCEMENT MEMBERS SHOULD BE INSTALLED BY A QUALIFIED, PROFESSIONAL TOWER CLIMBING COMPANY. KM CONSULTING ENGINEERS INC. TAKES NO RESPONSIBILITY FOR THE CONTRACTORS SAFETY POLICIES, PRACTICES, AND METHODS.

CERTIFICATION OF INSTALLATION:

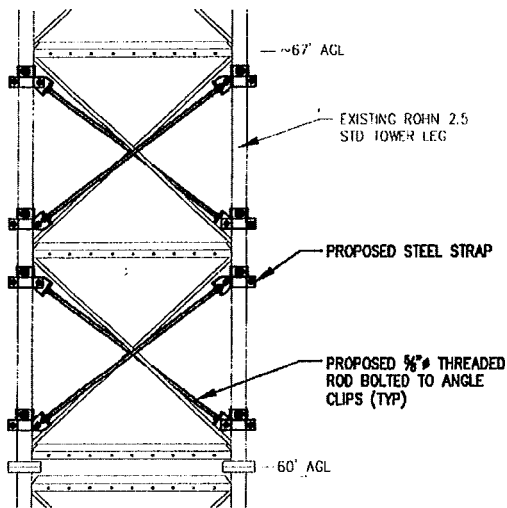
DURING OR UPON COMPLETION OF THESE MODIFICATIONS TO THE TOWER, A CERTIFICATION LETTER FROM A LICENSED PROFESSIONAL ENGINEER MUST BE SUBMITTED TO THE TOWER OWNER.

NOTES:

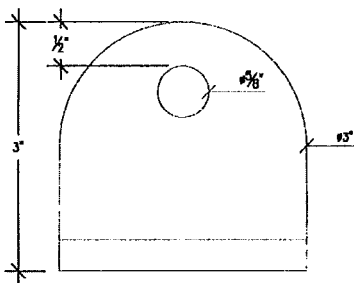
ANY INTERFERENCE OF EXISTING TOWER LEG STRUCTURE OR APPURTENANCES TO PROPOSED REINFORCEMENT, CONTRACTOR TO COORDINATE SHIFTING OF REINFORCEMENT ATTACHMENT WITH ENGINEER PRIOR TO INSTALL.

OWNER:	KM Consulting Engineers, Inc. <i>Wireless Engineering & Project Management</i> 32 West Upper Ferry Road Ewing, NJ 08828 Phone: (609) 538-0400 Fax: (609) 538-8858	CLIENT:	SALIENT ASSOCIATES 8 EAST PALISADES AVENUE ENLEWOOD, NJ 07631 (201) 567-0032 PH. (201) 567-9556 FAX	REVISIONS:
		APPROVALS & DATE:		PROJECT NAME:
	OWNER: _____ DATE: _____	CT43XC836		NO. DATE
	S.A.C.: _____ DATE: _____	DANBURY		DRAWING NO.:
	R / F.: _____ DATE: _____	PROJECT ADDRESS:		ST-2
	CONST.: _____ DATE: _____	303 BOXWOOD LANE		
		DANBURY, CT 06810		
MICHAEL J. BOHLANDER, PE CONNECTICUT PROFESSIONAL ENGINEER LICENSE # 20405 CT	PROJECT #: 120741.01	SITE ID #:	DRAWING TITLE: LEG REINFORCEMENT	P.C.: _____
				CHKD: MLB
				DRN: DJA
				DATE: 9/21/12

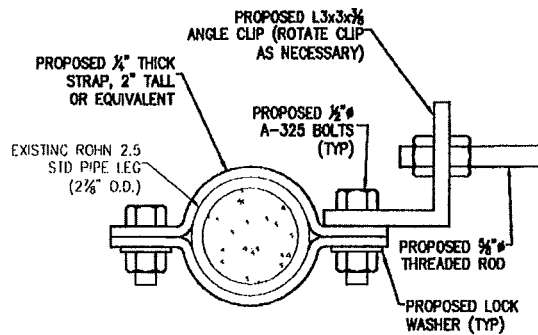
11/8/12



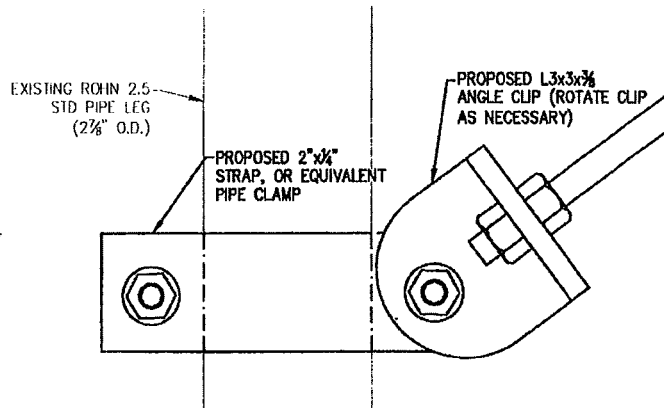
1 DIAGONAL REINFORCEMENT: 60'-67' AGL
ST-3 SCALE: 3/8" = 1'-0"



3 ANGLE CLIP DETAIL
ST-3 SCALE: 6" = 1'-0"



2 THREADED ROD CLIP PLAN VIEW
ST-3 SCALE: 3" = 1'



4 PIPE CLAMP DETAIL
ST-3 SCALE: NTS

- NOTES:
1. ALL MEMBERS, BOLTS HOLES, AND DIMENSIONS MUST BE FIELD VERIFIED PRIOR TO FABRICATION / PROCUREMENT OF REINFORCEMENT MATERIALS. ANY CHANGES TO THESE DRAWINGS AND SPECIFICATIONS OR CHANGES FOUND IN THE FIELD OF EXISTING TOWER MEMBERS MUST BE COMMUNICATED TO KM CONSULTING ENGINEERS INC. PRIOR TO INSTALLING REINFORCEMENT.
 2. STEEL: ALL STEEL MEMBERS TO BE HOT-DIP GALVANIZED TO ASTM A-123.
 3. IF STEEL IS FIELD CUT, ENDS OF STEEL MUST BE SPRAYED WITH COLD GALVANIZE ZRC.

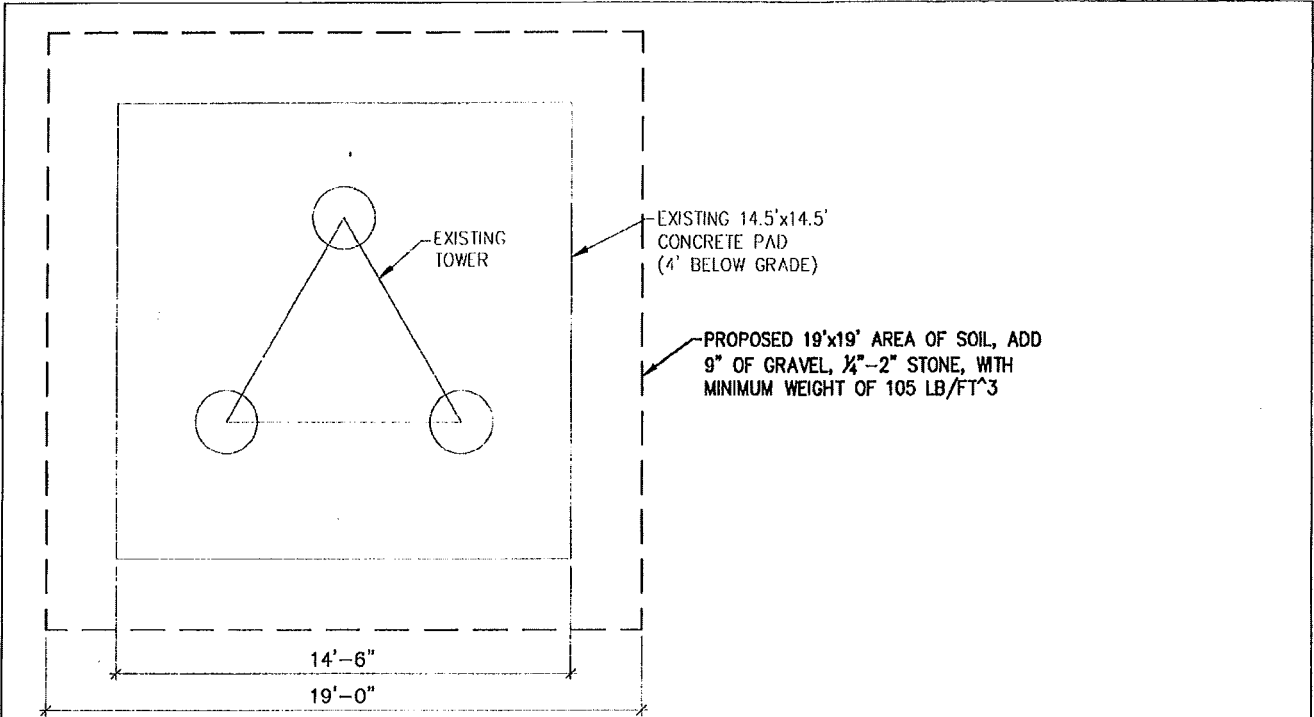
SAFETY NOTICE!
INSTALLATION OF THESE TOWER MODIFICATIONS WILL REQUIRE TOWER CLIMBING AT HEIGHTS WHERE FALLING COULD HARM OR PROVE FATAL TO WORKERS. THESE DRAWINGS INDICATE ONLY THE REINFORCEMENT AND NOT THE MEANS, METHODS, AND REQUIRED CONTRACTOR SAFETY. THESE REINFORCEMENT MEMBERS SHOULD BE INSTALLED BY A QUALIFIED, PROFESSIONAL TOWER CLIMBING COMPANY. KM CONSULTING ENGINEERS INC. TAKES NO RESPONSIBILITY FOR THE CONTRACTORS SAFETY POLICIES, PRACTICES, AND METHODS.

CERTIFICATION OF INSTALLATION:
DURING OR UPON COMPLETION OF THESE MODIFICATIONS TO THE TOWER, A CERTIFICATION LETTER FROM A LICENSED PROFESSIONAL ENGINEER MUST BE SUBMITTED TO THE TOWER OWNER.

NOTES:
ANY INTERFERENCE OF EXISTING TOWER LEG STRUCTURE OR APPURTENANCES TO PROPOSED REINFORCEMENT, CONTRACTOR TO COORDINATE SHIFTING OF REINFORCEMENT ATTACHMENT WITH ENGINEER PRIOR TO INSTALL.

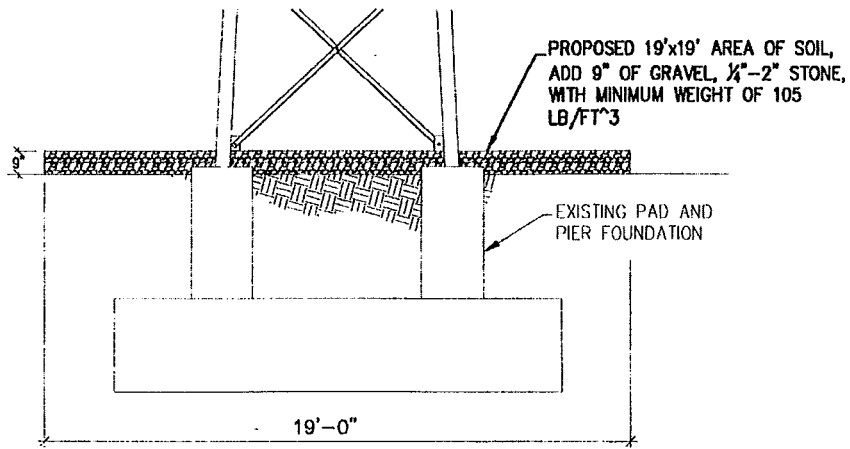
OWNER:	KM Consulting Engineers, Inc. <i>Wireless Engineering & Project Management</i> 32 West Upper Ferry Road Ewing, NJ 08628 Phone: (609) 538-0400 Fax: (609) 538-8858	CLIENT:	S&A SALIENT ASSOCIATES 8 EAST PALMBADES AVENUE ENLEWOOD, NJ 07831 (201) 587-0032 PH. (201) 587-8556 FAX	REVISIONS:
		APPROVALS & DATE: OWNER: _____ DATE: _____ S.A.C.: _____ DATE: _____ R / F.: _____ DATE: _____ CONST.: _____ DATE: _____		PROJECT NAME: CT43XC836 DANBURY PROJECT ADDRESS: 303 BOXWOOD LANE DANBURY, CT 06810
MICHAEL A. BOHLINGER, PE CONNECTICUT PROFESSIONAL ENGINEER LICENSE # 26606	PROJECT #: 120741.01 SITE ID #: _____ DRAWING TITLE: DIAGONAL REINFORCEMENT	P.C.: _____ CHKD: MLB DRN: DJA	DATE: 9/21/12	

11/8/12






1 FOUNDATION REINFORCEMENT
ST-4

SCALE: NTS



1 FOUNDATION REINFORCEMENT
ST-4

SCALE: NTS

OWNER:	 KM Consulting Engineers, Inc. <i>Wireless Engineering & Project Management</i> 32 West Upper Ferry Road Ewing, NJ 08828 Phone: (609) 538-0400 Fax: (609) 538-8858	CLIENT:  SALIENT ASSOCIATES 8 EAST PALMADEN AVENUE ENJEWOD, NJ 07831 (201) 567-0032 PH. (201) 567-9556 FAX	REVISIONS: <table border="1"> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> </table>								
	APPROVALS & DATE: OWNER: _____ DATE: _____ S.A.C.: _____ DATE: _____ R / F.: _____ DATE: _____ CONST.: _____ DATE: _____	PROJECT NAME: CT43XC836 DANBURY PROJECT ADDRESS: 303 BOXWOOD LANE DANBURY, CT 06810	1 11/8/12 NO. DATE DRAWING NO.: ST-4								
MICHAEL L. BOHLENDER, PE CONNECTICUT PROFESSIONAL ENGINEER LICENSE # 20400	PROJECT #: 120741.01 SITE ID #:	DRAWING TITLE: FOUNDATION REINFORCEMENT	P.C.: CHKD: MLB DRN: DJA DATE: 9/21/12								

**SECTION 1704
SPECIAL INSPECTIONS**

1704.1 General. Where application is made for construction as described in this section, the owner or the registered design professional in responsible charge acting as the owner's agent shall employ one or more approved agencies to perform inspections during construction on the types of work listed under Section 1704. These inspections are in addition to the inspections identified in Section 110.

The special inspector shall be a qualified person who shall demonstrate competence, to the satisfaction of the building official, for the inspection of the particular type of construction or operation requiring special inspection. The registered design professional in responsible charge and engineers of record involved in the design of the project are permitted to act as the approved agency and their personnel are permitted to act as the special inspector for the work designed by them, provided those personnel meet the qualification requirements of this section to the satisfaction of the building official. The special inspector shall provide written documentation to the building official demonstrating his or her competence and relevant experience or training. Experience or training shall be considered relevant when the documented experience or training is related in complexity to the same type of special inspection activities for projects of similar complexity and material qualities. These qualifications are in addition to qualifications specified in other sections of this code.

Exceptions:

1. Special inspections are not required for work of a minor nature or as warranted by conditions in the jurisdiction as approved by the building official.
2. Special inspections are not required for building components unless the design involves the practice of professional engineering or architecture as defined by applicable state statutes and regulations governing the professional registration and certification of engineers or architects.
3. Unless otherwise required by the building official, special inspections are not required for Group C occupancies that are accessory to a residential occupancy including, but not limited to, those listed in Section 312.1.

1704.1.1 Statement of special inspections. The applicant shall submit a statement of special inspections prepared by the registered design professional in responsible charge in accordance with Section 107.1 as a condition for issuance. This statement shall be in accordance with Section 1705.

Exceptions:

1. A statement of special inspections is not required for structures designed and constructed in accordance with the conventional construction provisions of Section 2308.
2. The statement of special inspections is permitted to be prepared by a qualified person approved by the building official for construction not designed by a registered design professional.

1704.1.2 Report requirement. Special inspectors shall keep records of inspections. The special inspector shall furnish inspection reports to the building official, and to the registered design professional in responsible charge. Reports shall indicate that work inspected was or was not completed in conformance to approved construction documents. Discrepancies shall be brought to the immediate attention of the contractor for correction. If they are not corrected, the discrepancies shall be brought to the attention of the building official and to the registered design professional in responsible charge prior to the completion of that phase of the work. A final report documenting required special inspections and correction of any discrepancies noted in the inspections shall be submitted at a point in time agreed upon prior to the start of work by the applicant and the building official.

1704.2 Inspection of fabricators. Where fabrication of structural load-bearing members and assemblies is being performed

1704.2.1 Fabrication and implementation procedures. The special inspector shall verify that the fabricator maintains detailed fabrication and quality control procedures that provide a basis for inspection control of the workmanship and the fabricator's ability to conform to approved construction documents and referenced standards. The special inspector shall review the procedures for completeness and adequacy relative to the code requirements for the fabricator's scope of work.

Exception: Special inspections as required by Section 1704.2 shall not be required where the fabricator is approved in accordance with Section 1704.2.2.

1704.2.2 Fabricator approval. Special inspections required by Section 1704 are not required where the work is done on the premises of a fabricator registered and approved to perform such work without special inspection. Approval shall be based upon review of the fabricator's written procedural and quality control manuals and periodic auditing of fabrication practices by an approved special inspection agency. At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building official stating that the work was performed in accordance with the approved construction documents.

1704.3 Steel construction. The special inspections for steel elements of buildings and structures shall be as required by Section 1704.3 and Table 1704.3.

Exceptions:

1. Special inspection of the steel fabrication process shall not be required where the fabricator does not perform any welding, thermal cutting or heating operation of any kind as part of the fabrication process. In such cases, the fabricator shall be required to submit a detailed procedure for material control that demonstrates the fabricator's ability to maintain suitable records and procedures such that, at any time during the fabrication process, the material specification, grade and mill test reports for the main stress-carrying elements are capable of being determined.
2. The special inspector need not be continuously present during welding of the following items, provided the materials, welding procedures and qualifications of welders are verified prior to the start of the work; periodic inspections are made of the work in progress and a visual inspection of all welds is made prior to completion or prior to shipment of shop welding.
 - 2.1. Single-pass fillet welds not exceeding $\frac{5}{16}$ inch (7.9 mm) in size.
 - 2.2. Floor and roof deck welding.
 - 2.3. Welded studs when used for structural diaphragm.
 - 2.4. Welded sheet steel for cold-formed steel members.
 - 2.5. Welding of stairs and railing systems.

1704.3.1 Welding. Welding inspection and welding inspector qualification shall be in accordance with this section.

1704.3.1.1 Structural steel. Welding inspection and welding inspector qualification for structural steel shall be in accordance with AWS D1.1.




1704.3.1.2 Cold-formed steel. Welding inspection and welding inspector qualification for cold-formed steel floor and roof decks shall be in accordance with AWS D1.3.

1704.3.1.3 Reinforcing steel. Welding inspection and welding inspector qualification for reinforcing steel shall be in accordance with AWS D1.4 and ACI 318.

1704.3.2 Details. The special inspector shall perform an inspection of the steel frame to verify compliance with the details shown on the approved construction documents, such as bracing, stiffening, member locations and proper application of joint details at each connection.

1704.3.3 High-strength bolts. Installation of high-strength bolts shall be inspected in accordance with AISC 360.

1704.3.3.1 General. While the work is in progress, the special inspector shall determine that the requirements for bolts, nuts, washers and paint; bolted parts and installation and tightening in such standards are met. For bolts requiring pretensioning, the special inspector shall observe the pretensioning testing and calibration procedures when such procedures are required by the installation method or by project plans or specifications; determine that all piles of connected materials have been drawn together and properly snugged and monitor the installation of bolts to verify that the selected procedure for installation is properly used to tighten bolts. For joints required to be tightened only to the snug-tight con-



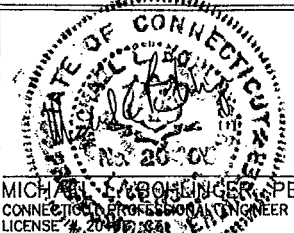
OWNER:	 KM Consulting Engineers, Inc. <i>Wireless Engineering & Project Management</i> 32 West Upper Ferry Road Ewing, NJ 08628 Phone: (609) 538-0400 Fax: (609) 538-8858	CLIENT:  SALIENT ASSOCIATES 8 EAST PALISADES AVENUE ENLEWOOD, NJ 07631 (201) 587-0032 PH. (201) 667-9666 FAX	REVISIONS:
	APPROVALS & DATE: OWNER: _____ DATE: _____ S.A.C.: _____ DATE: _____ R / F.: _____ DATE: _____ CONST.: _____ DATE: _____	PROJECT NAME: CT43XC836 DANBURY PROJECT ADDRESS: 303 BOXWOOD LANE DANBURY, CT 06810	1 11/8/12 NO. DATE DRAWING NO.: ST-5
MICHAEL A. BONLINGER, PE CONNECTICUT PROFESSIONAL ENGINEER LICENSE # 30450-01	PROJECT #: 120741.01	SITE ID #: _____ DRAWING TITLE: SPECIAL INSPECTION NOTES	P.C.: _____ CHKD: MLB DRN: DJA DATE: 9/21/12

11/8/12

TABLE 1704.3
REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
1. Material verification of high-strength bolts, nuts and washers:				
a. Identification markings to conform to ASTM standards specified in the approved construction documents.		X	AISC 360, Section A3.3 and applicable ASTM material standards	
b. Manufacturer's certificate of compliance required.		X		
2. Inspection of high-strength bolting:				
a. Snug-tight joints.		X		
b. Prestensioned and slip-critical joints using turn-of-nut with matchmarking, twist-off bolt or direct tension indicator methods of installation.		X	AISC 360, Section M2.5	1704.3.3
c. Prestensioned and slip-critical joints using turn-of-nut without matchmarking or calibrated wrench methods of installation.	X			
3. Material verification of structural steel and cold-formed steel deck:				
a. For structural steel, identification markings to conform to AISC 360.		X	AISC 360, Section M5.5	
b. For other steel, identification markings to conform to ASTM standards specified in the approved construction documents.		X	Applicable ASTM material standards	
c. Manufacturer's certified test reports.		X		
4. Material verification of weld filler materials:				
a. Identification markings to conform to AWS specification in the approved construction documents.		X	AISC 360, Section A3.5 and applicable AWS AS documents	
b. Manufacturer's certificate of compliance required.		X		
5. Inspection of welding:				
a. Structural steel and cold-formed steel deck:				
1) Complete and partial joint penetration groove welds.	X		AWS D1.1	1704.3.1
2) Multipass fillet welds.	X			
3) Single-pass fillet welds $\geq \frac{1}{16}$	X			
4) Plug and slot welds.	X			
5) Single-pass fillet welds $\leq \frac{1}{16}$		X		
6) Floor and roof deck welds.		X	AWS D1.3	
b. Reinforcing steel:				
1) Verification of weldability of reinforcing steel other than ASTM A 706.		X	AWS D1.4 ACI 318, Section 3.5.2	
2) Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special structural walls of concrete and shear reinforcement.	X			
3) Shear reinforcement.	X			
4) Other reinforcing steel.		X		
6. Inspection of steel frame joint details for compliance:				
a. Details such as bracing and stiffening.		X		1704.3.2
b. Member locations.		X		
c. Application of joint details at each connection.		X		

For SI: 1 inch = 25.4 mm

OWNER:	 KM Consulting Engineers, Inc. <i>Wireless Engineering & Project Management</i> 32 West Upper Ferry Road Ewing, NJ 08828 Phone: (609) 638-0400 Fax: (609) 638-8868	CLIENT:	 SALENT ASSOCIATES 8 EAST PALSADES AVENUE ENLEWOOD, NJ 07831 (201) 567-0032 PH (201) 567-9556 FAX	REVISIONS:		
		APPROVALS & DATE:		PROJECT NAME:	1 11/8/12 NO. DATE DRAWING NO.: ST-6	
OWNER: _____ DATE: _____	S.A.C.: _____ DATE: _____	R / F: _____ DATE: _____	CONST.: _____ DATE: _____	PROJECT ADDRESS:		
PROJECT #:	SIF ID #:	DRAWING TITLE:	P.C.:	CHKD:	DRN:	DATE:
120741.01		SPECIAL INSPECTION NOTES		MLB	DJA	9/21/12

11/8/12

RADIO FREQUENCY EMISSIONS ANALYSIS REPORT
EVALUATION OF HUMAN EXPOSURE POTENTIAL
TO NON-IONIZING EMISSIONS

Sprint Existing Facility

Site ID: CT43XC836

Danbury W. CT University

303 Boxwood Lane
Danbury, CT 06810

September 9, 2012

September 9, 2012

Sprint
Attn: RF Engineering Manager
1 International Boulevard, Suite 800
Mahwah, NJ 07495

Re: Emissions Values for Site **CT43XC836 - Danbury W. CT University**

EBI Consulting was directed to analyze the proposed upgrades to the existing Sprint facility located at 303 Boxwood Lane, Danbury, CT, for the purpose of determining whether the emissions from the proposed Sprint equipment upgrades on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limit for the cellular band is approximately $567 \mu\text{W}/\text{cm}^2$, and the general population exposure limit for the PCS band is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed upgrades to the existing Sprint Wireless antenna facility located at 303 Boxwood Lane, Danbury, CT, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. All calculations were performed assuming the main lobe of the antenna was focused at the base of the tower to present a worst case scenario. Actual values seen from this site will be dramatically less than those shown in this report. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all emissions were calculated using the following assumptions:

- 1) 5 CDMA Carriers (1900 MHz) were considered for each sector of the proposed installation.
- 2) 1 CDMA Carrier (850 MHz) was considered for each sector of the proposed installation
- 3) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 4) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The actual gain in this direction was used per the manufactures supplied specifications.
- 5) The antenna used in this modeling is the RFS APXVSPP18-C-A20. This is based on feedback from the carrier with regards to anticipated antenna selection. This antenna has a 15.9 dBd gain value at its main lobe at 1900 MHz and 13.4 dBd at its main lobe for 850 MHz. All calculations were performed assuming the main lobe of the antenna was focused at the base of the tower to present a worst case scenario.

- 6) The antenna mounting height centerline of the proposed antennas is **89.2 feet** above ground level (AGL)
- 7) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculation were done with respect to uncontrolled / general public and Controlled / Occupational threshold limits

Site ID	CT43XC856 - Danbury W. CT University
Site Address	303 Boxwood Lane, Danbury, CT 06810
Site Type	Self Support Tower

Sector 1																	
Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain in direction of sample point (dBi)	Antenna Height (ft)	Antenna analysis height	Cable Size	Cable Loss (dB)	Additional Loss	ERP	Power Density Value	Power Density Percentage
1a	RFS	APXVSP18-CA20	RRH	1900 MHz	CDMA / LTE	20	5	100	15.9	89.2	83.2	1/2"	0.5	0	3467.3685	180.0774	18.00774%
1a	RFS	APXVSP18-CA20	RRH	850 MHz	CDMA / LTE	20	1	20	13.4	89.2	83.2	1/2"	0.5	0	389.96892	20.253	3.57196%
													Sector total Power Density Value: 21.580%				

Sector 2																	
Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain in direction of sample point (dBi)	Antenna Height (ft)	Antenna analysis height	Cable Size	Cable Loss (dB)	Additional Loss	ERP	Power Density Value	Power Density Percentage
2a	RFS	APXVSP18-CA20	RRH	1900 MHz	CDMA / LTE	20	5	100	15.9	89.2	83.2	1/2"	0.5	0	3467.3685	180.0774	18.00774%
2a	RFS	APXVSP18-CA20	RRH	850 MHz	CDMA / LTE	20	1	20	13.4	89.2	83.2	1/2"	0.5	0	389.96892	20.253	3.57196%
													Sector total Power Density Value: 21.580%				

Sector 3																	
Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain in direction of sample point (dBi)	Antenna Height (ft)	Antenna analysis height	Cable Size	Cable Loss (dB)	Additional Loss	ERP	Power Density Value	Power Density Percentage
3a	RFS	APXVSP18-CA20	RRH	1900 MHz	CDMA / LTE	20	5	100	15.9	89.2	83.2	1/2"	0.5	0	3467.3685	180.0774	18.00774%
3a	RFS	APXVSP18-CA20	RRH	850 MHz	CDMA / LTE	20	1	20	13.4	89.2	83.2	1/2"	0.5	0	389.96892	20.253	3.57196%
													Sector total Power Density Value: 21.580%				

Site Composite MPE %	
Carrier	MPE %
Sprint	64.735%
T-Mobile	15.610%
WCI	1.280%
Nextel	12.790%
Total Site MPE %	
53.515%	

Summary

All calculations performed for this analysis yielded results that were well within the allowable limits for general public exposure to RF Emissions.

The anticipated Maximum Composite contributions from the Sprint facility are **64.739% (21.580% from each sector)** of the allowable FCC established general public limit considering all three sectors simultaneously sampled at the ground level.

The anticipated composite MPE value for this site assuming all carriers present is **93.919%** of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government



Scott Heffernan
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