November 14, 2014

Melanie A. Bachman Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

RE: Metro PCS-Exempt Modification - Crown Site BU: 876364

**Metro PCS Site ID: CTHA520A** 

Located at: 201 Main Street, Cromwell, CT 06416

Dear Ms. Bachman:

This letter and exhibits are submitted on behalf of Metro PCS. Metro PCS is making modifications to certain existing sites in its Connecticut system in order to implement their 700MHz technology. Please accept this letter and exhibits as notification, pursuant to § 16-50j-73 of the Regulations of Connecticut State Agencies ("R.C.S.A."), of construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In compliance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Mr. Jonathan Sistare, Town Manager for the Town of Cromwell and S & S Partners Inc., Property Owner.

Metro PCS plans to modify the existing wireless communications facility owned by Crown Castle and located at **201 Main Street, Cromwell, CT 06416**. Attached are a compound plan and elevation depicting the planned changes (Exhibit-1), and documentation of the structural sufficiency of the structure to accommodate the revised antenna configuration (Exhibit-2). Also included is a power density table report reflecting the modification to T-Mobile's operations at the site (Exhibit-3).

The changes to the facility do not constitute a modification as defined in Connecticut General Statutes ("C.G.S.") § 16-50i(d) because the general physical characteristics of the facility will not be significantly changed. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in the R.C.S.A. § 16-50j-72(b)(2).

- 1. The proposed modifications will not result in an increase in the height of the existing tower. T-Mobile's replacement antennas will be located at the same elevation on the existing tower.
- 2. There will be no proposed modifications to the ground and no extension of boundaries.
- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more.

- 4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative General Power Density table report for T-Mobile's modified facility is included as Exhibit-3.
- 5. A Structural Modification Report confirming that the tower and foundation can support T-Mobile's proposed modifications is included as Exhibit-2.

For the foregoing reasons, T-Mobile respectfully submits the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

Jerry Feathers Real Estate Specialist

#### Enclosure

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes

Tab 2: Exhibit-2: Structural Modification Report

Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: Mr. Jonathan Sistare, Town Manager
Office of the Town Manager
Nathaniel White Building
41 West Street
Cromwell, CT 06416

cc: S & S Partners Inc. P.O. Box 734

Old Lyme, CT 06371

# metro

SITE NAME: CROWN CROMWELL MONOPOLE

SITE ID NUMBER: CTHA520A

SITE ADDRESS: 201 MAIN ST.

CROMWELL, CT 06480

#### PROJECT SUMMARY

SITE ID NUMBER:

SITE NAME:

CROWN CROMWELL MONOPOLE

CROWN BU#:

876364

SITE ADDRESS:

201 MAIN ST.

CROMWELL, CT 06480

COUNTY:

MIDDLESEX

PROPERTY OWNER:

CROWN CASTLE USA

APPLICANT:

metroPCS WIRELESS. INC. 35 GRIFFIN ROAD SOUTH

BLOOMFIELD, CT 06002

**ENGINEER:** 

TECTONIC ENGINEERING AND SURVEYING CONSULTANTS, P.C.

1279 ROUTE 300

NEWBURGH, NY 12550

CONTACT: JAMES QUICKSELL PHONE: (845) 567-6656 EXT. 2835

SITE ACQUISITION:

CROWN CASTLE 1200 MACARTHUR BLVD

SUITE 200 MAHWAH, NJ 07430

CONTACT:

PHONE:

PETE TISI (201) 491-6009

PARCEL INFO:

51/47/36//

LATITUDE: (NAD 83)

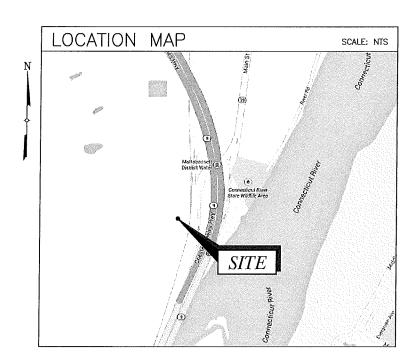
41.583364° N

LONGITUDE: (NAD 83)

72.649761° W

#### SITE DIRECTIONS

HEAD NORTHEAST ON GRIFFIN RD S TOWARD W NEWBERRY RD. TAKE THE SECOND RIGHT ONTO DAY HILL RD. MERGE ONTO -91 S VIA THE RAMP TO HARTFORD. MERGE ONTO I-91 S. TAKE THE EXIT 22S ON THE LEFT ONTO CT-9 TOWARD MIDDLETOWN/OLD SAYBROOK. TAKE EXIT 19 FOR CT-372/WEST ST TOWARD CROMWELL. TURN LEFT ONTO CT-372E/WEST ST. TURN RIGHT ONTO CT-99S. TAKE THE CONNECTICUT 9 S RAMP DESTINATION WILL BE ON THE RIGHT.



SHE	EET INDEX						
SHEET NO	DESCRIPTION	REV NO					
T-1	TITLE SHEET	1					
A-1	SITE PLAN	1_1_					
A-2 A-3	EQUIPMENT LAYOUT PLANS & DETAILS  ELEVATION & DETAIL	1 1					
A-3A A-4	ELEVATIONS & DETAILS ANTENNA LAYOUT PLANS & DETAILS	1					
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	THIS SET OF PLANS SHALL NOT BE UTILIZED AS CONSTRUCTION DOCUMENTS UNTIL ALL ITEMS HAVE BEEN ADDRESSED AND EACH						

OF THE DRAWINGS HAS BEEN REVISED AND ISSUED "FOR CONSTRUCTION".



CONFIGURATION REFER TO LATEST T-MOBILE RF DATA SHEET FOR FINAL RF DESIGN & BOM.



**TECTONIC** Engineering and Surveying Consultants, P.C.

Phone: (845) 567-6656 Fax: (845) 567-8703

### metroPCS



## LANDLORD CONSTRUCTION

SITE ACQ. PROJECT NUMBER

OPERATIONS

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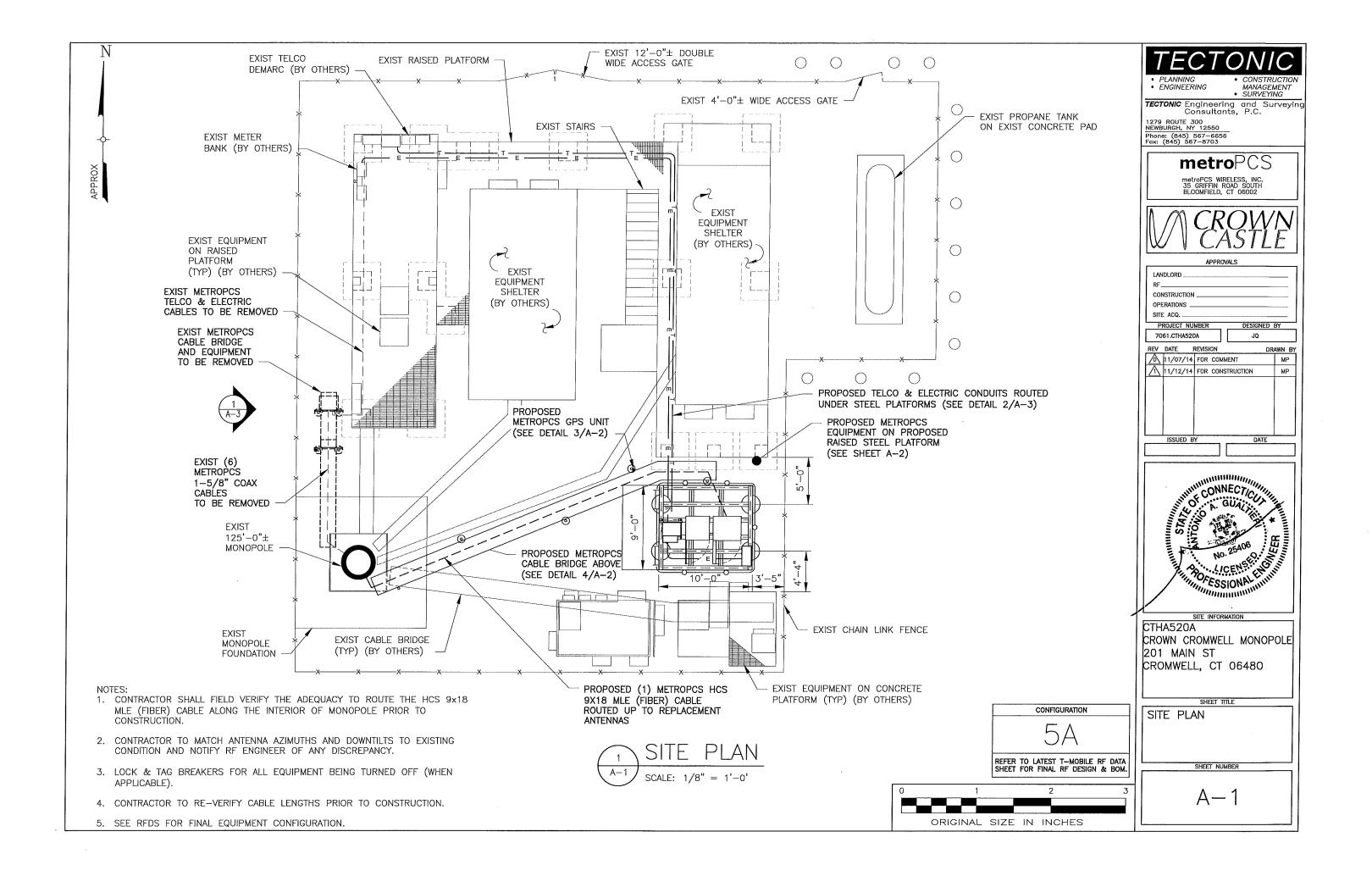


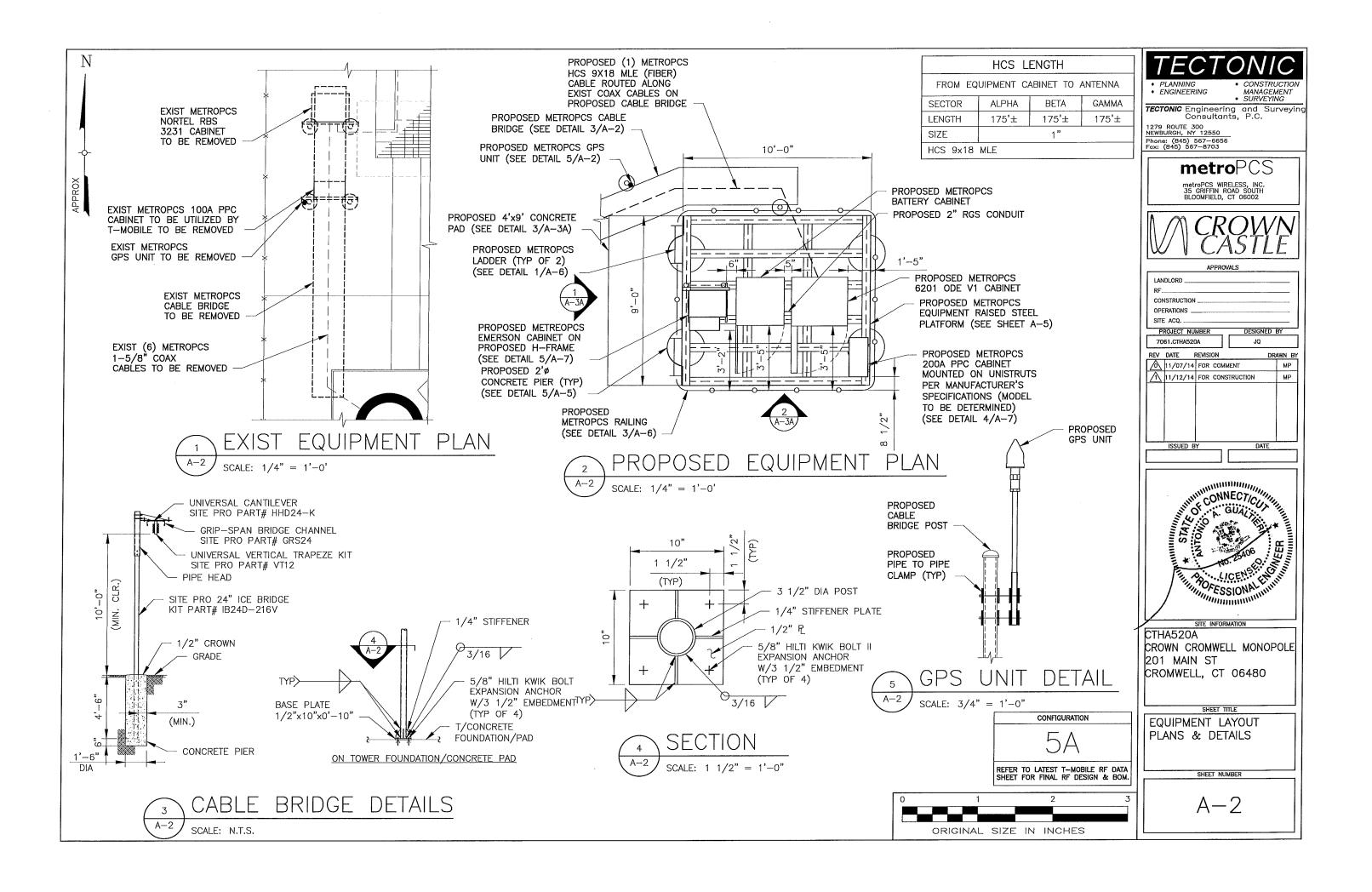
CTHA520A CROWN CROMWELL MONOPOLE 201 MAIN ST CROMWELL, CT 06480

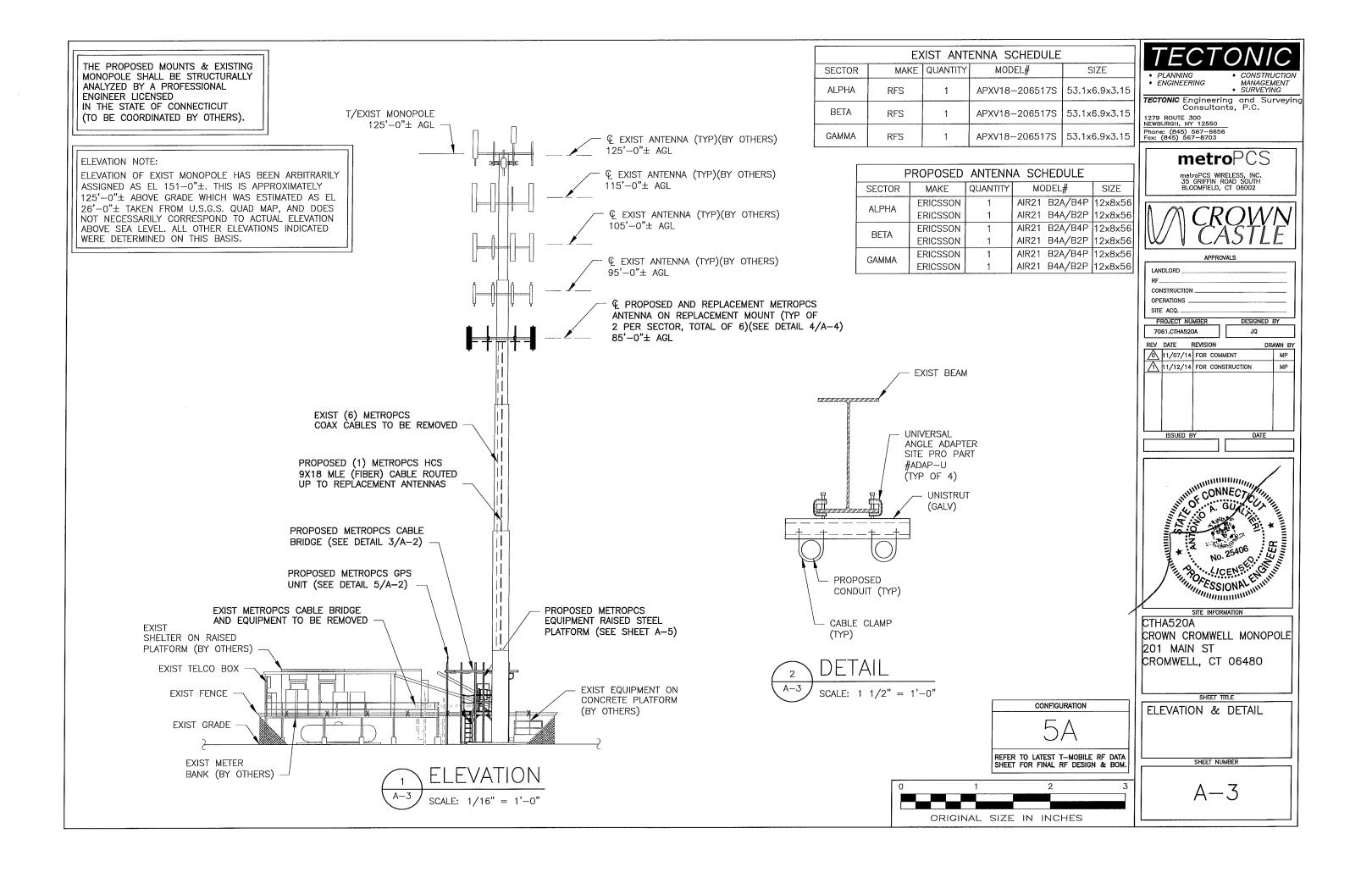
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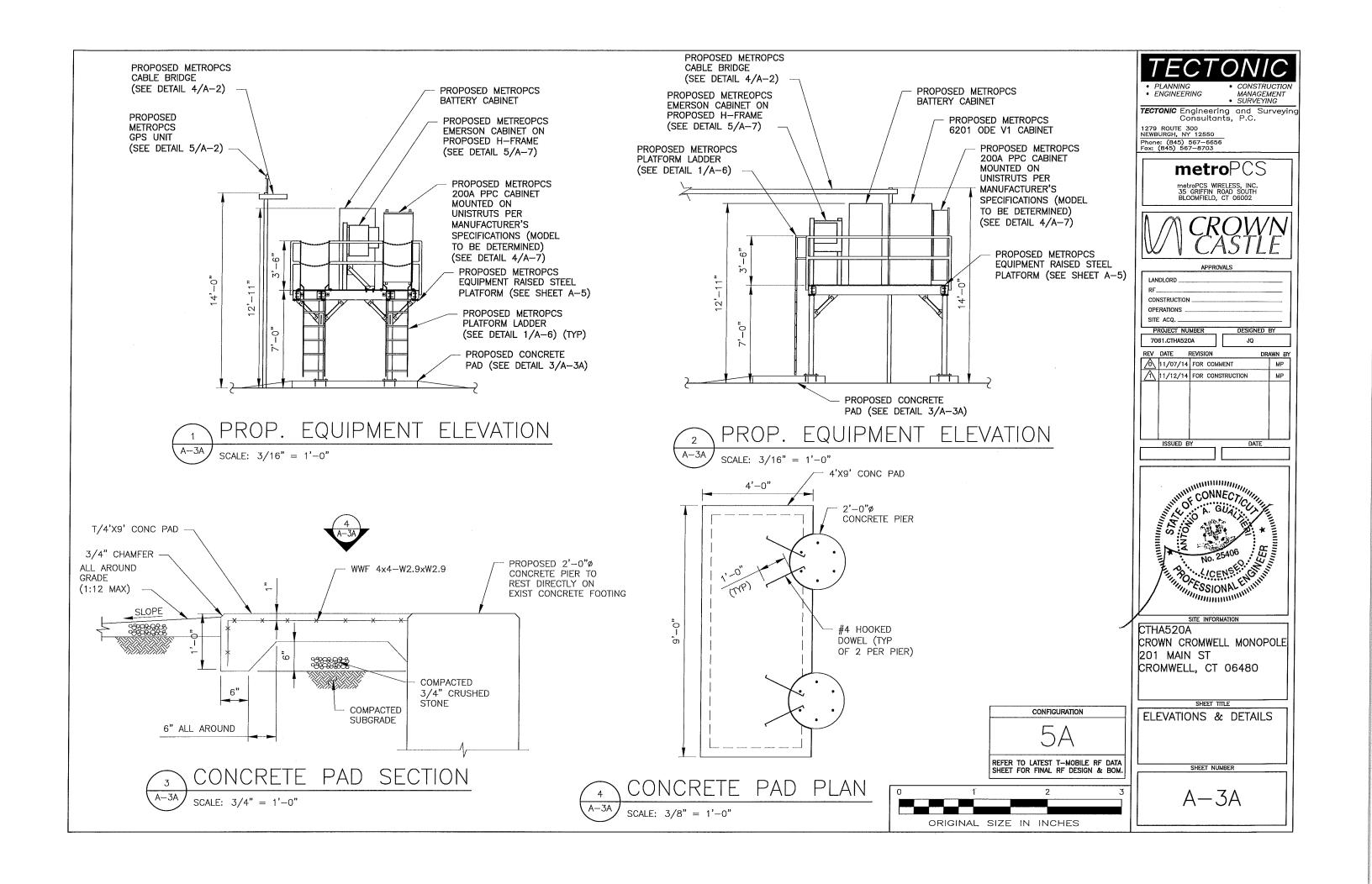
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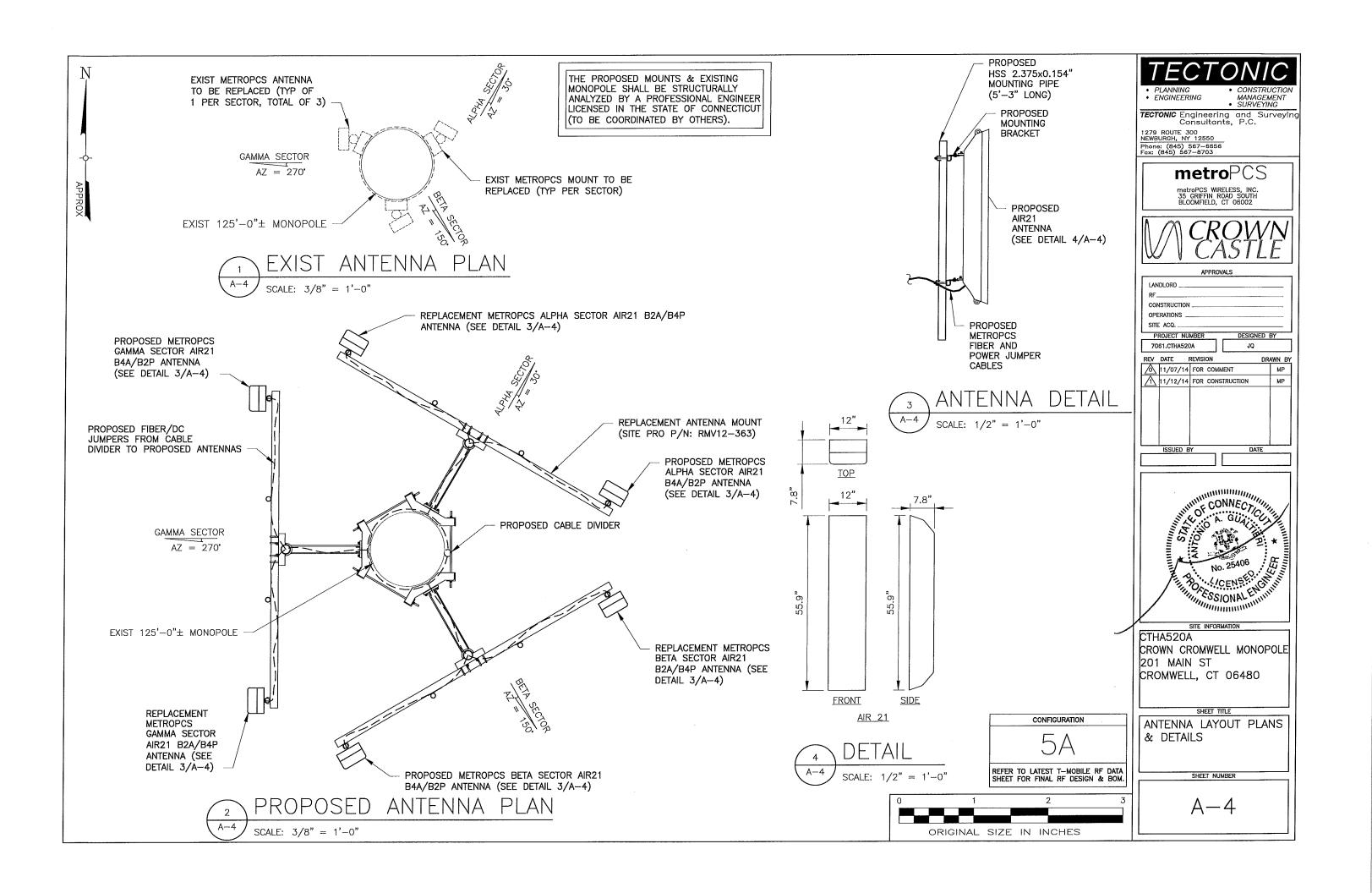
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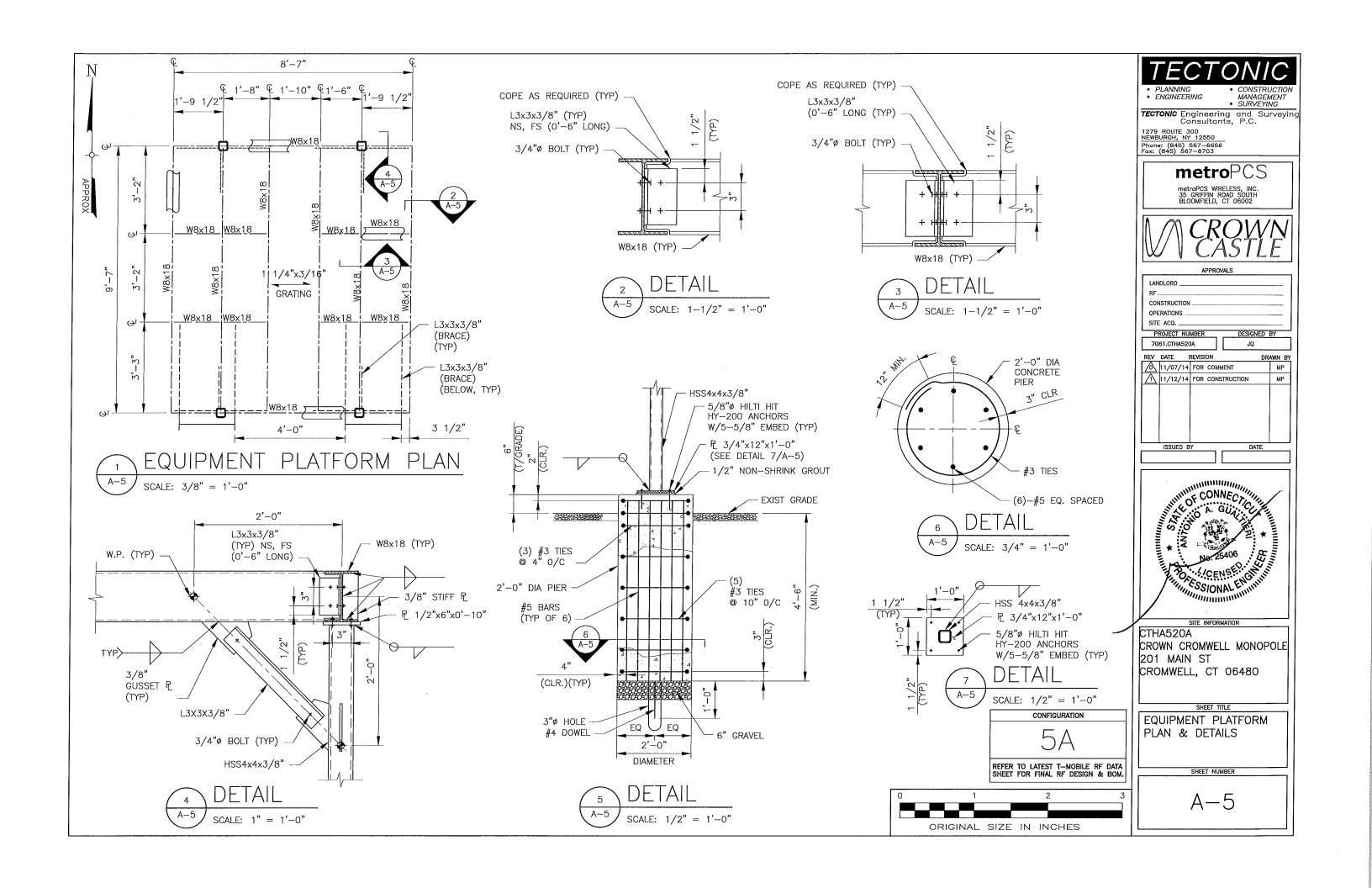


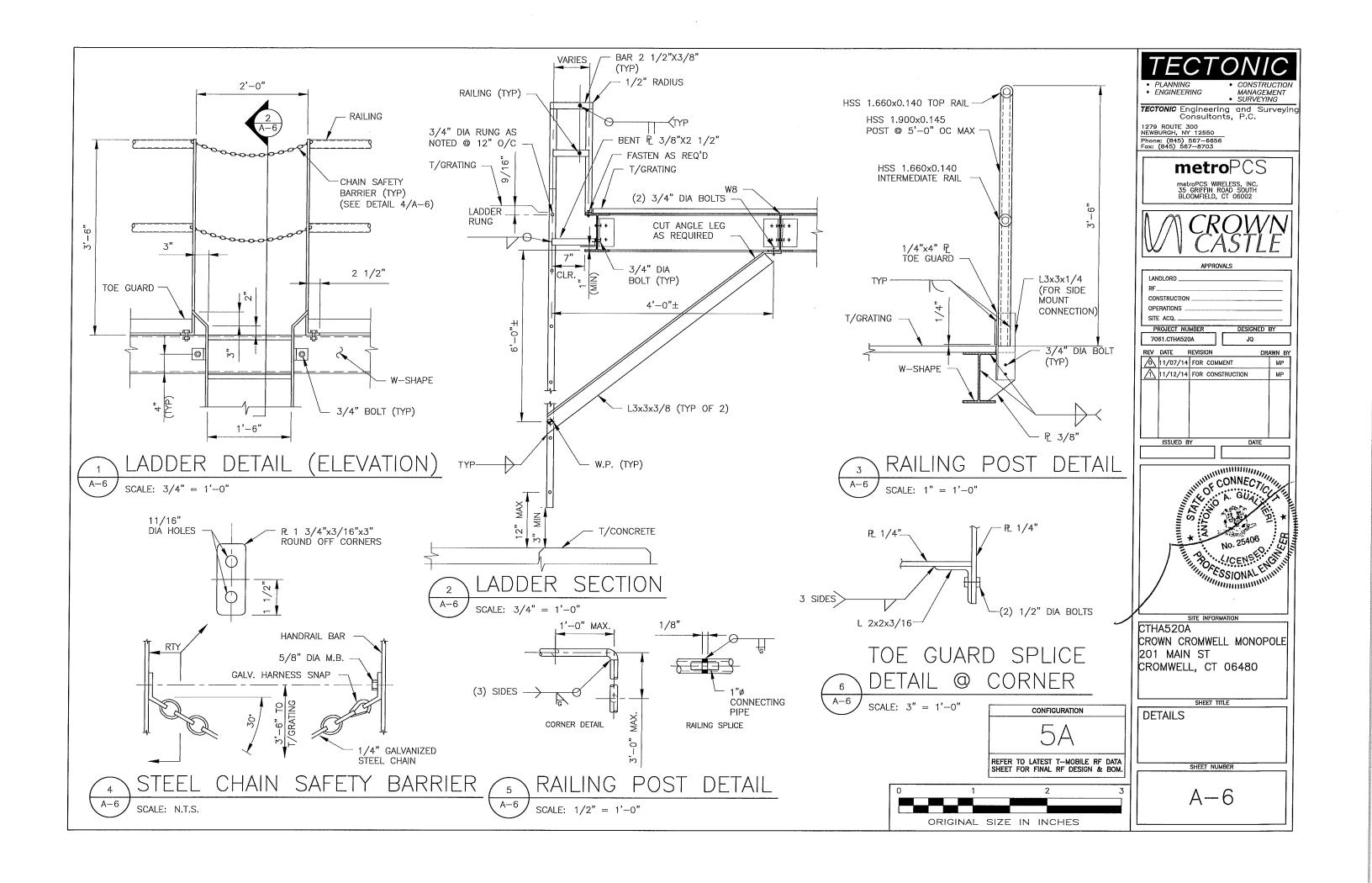


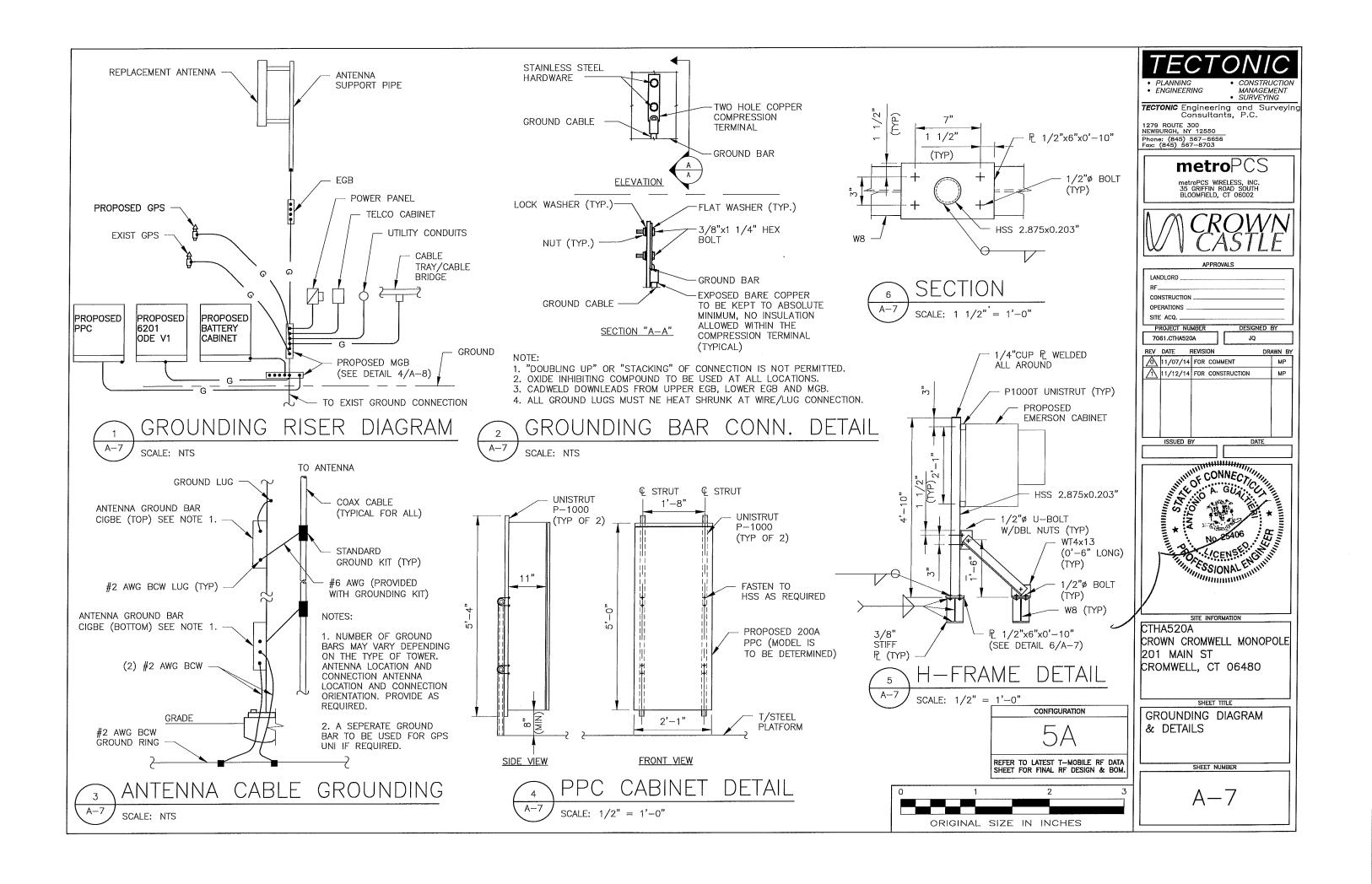


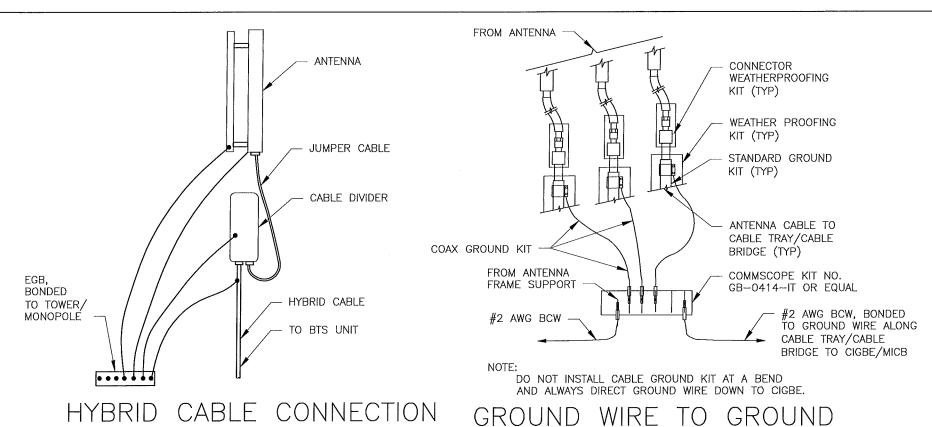












3 0 7/16" 2" 0 3/4" 3 1<sup>3</sup> 1<sup>3</sup> 1

#### LEGENI

- 1- COPPER TINNED GROUND BAR, 1/4"X 4"X 20", OR OTHER LENGTH AS REQUIRED
- 2- INSULATORS, NEWTON INSTRUMENT CAT. NO. 3061-4 OR EQUAL.
- 3- 5/8" LOCKWASHERS OR EQUAL
- 4- WALL MOUNTING BRACKET, NEWTON INSTRUMENT CO. CAT NO. A-6056 OR EQUAL
- 5- 5/8-11 X 1" H.H.C.S.BOLTS

#### NOTE:

ALL BOLTS, NUTS, WASHERS AND LOCK WASHERS SHALL BE 18-8 STAINLESS STEEL.

JND L /

GROUNDING BAR DETAIL

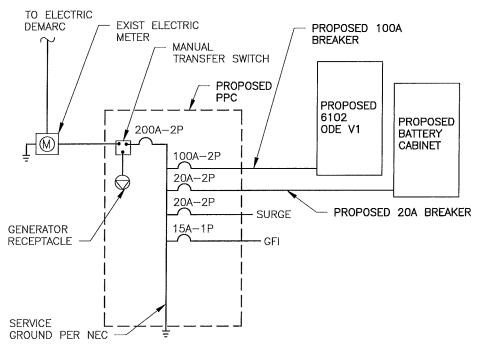
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BAR CONNECTION DETAIL

A-8 SCALE: NTS

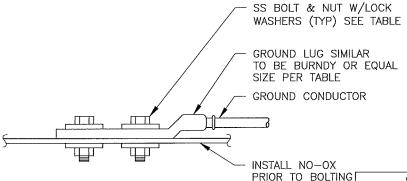


TABLE

WIRE SIZE LUG # BOLT DIA

#2 YA2CL-2TC14 1/4"

#2/0 YA25-2LN 1/2"



LUG GROUND CONNECTION

SCALE: N.T.S.

CONFIGURATION

SHEET FOR FINAL RF DESIGN & BOM.

O 1 2 3
ORIGINAL SIZE IN INCHES

SITE INFORMATION

CENSE ON ALEMAN

SURVEYING

DESIGNED BY

DATE

DRAWN BY

**TECTONIC** Engineering and Surveying Consultants, P.C.

metroPCS

1279 ROUTE 300 NEWBURGH, NY 12550 Phone: (845) 567-6656 Fax: (845) 567-8703

LANDLORD

CONSTRUCTION

PROJECT NUMBER

ISSUED BY

11/07/14 FOR COMMENT

11/12/14 FOR CONSTRUCTION

7061.CTHA520A REV DATE REVISION

OPERATIONS

SITE ACQ. ..

CTHA520A CROWN CROMWELL MONOPOLE 201 MAIN ST CROMWELL, CT 06480

SHEET TITLE

ONE-LINE POWER DIAGRAM & DETAILS

SHEET NUMBER

8-A

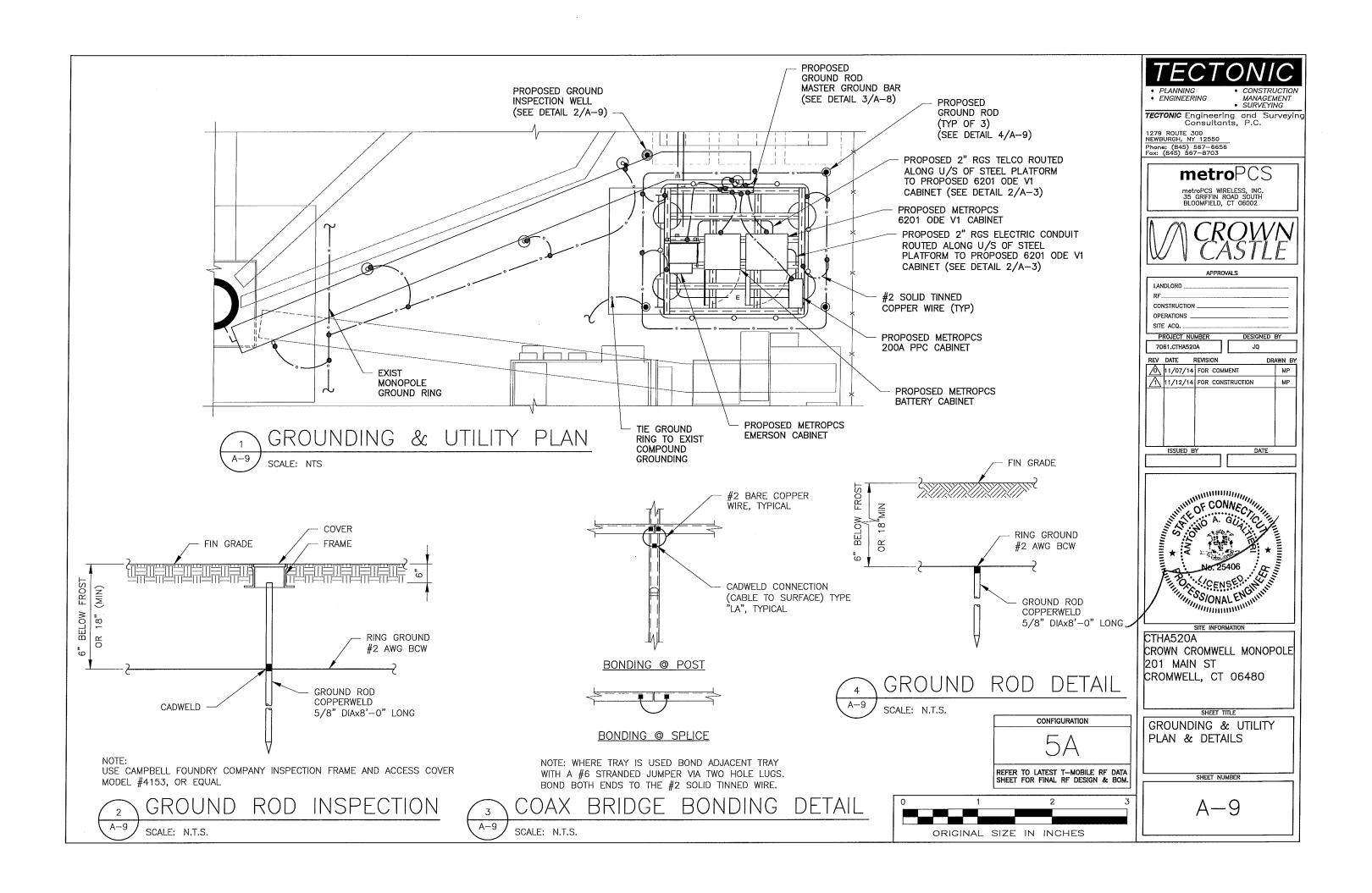
NOTE:

A-8

SCALE: NTS

1. ONE LINE DIAGRAM IS GENERIC. ELECTRICAL DESIGN TO BE APPROVED & PERFORMED BY OTHERS.
2. EXIST ELECTRIC SERVICE MAY NEED TO BE UPGRADED IF REQUIRED. TO BE DESIGNED BY OTHERS.



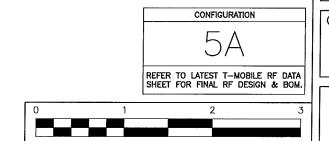


## GENERAL NOTES

- CONTRACTOR SHALL NOT COMMENCE ANY WORK UNTIL HE OBTAINS, AT HIS OWN EXPENSE, ALL INSURANCE REQUIRED BY METROPCS, THE PROPERTY OWNER AND/OR PROPERTY MANAGEMENT COMPANY
- THIS SET OF PLANS HAS BEEN PREPARED FOR THE PURPOSES OF MUNICIPAL AND AGENCY REVIEW AND APPROVAL. THIS SET OF PLANS SHALL NOT BE UTILIZED AS CONSTRUCTION DOCUMENTS UNTIL ALL CONDITIONS OF APPROVAL HAVE BEEN SATISFIED AND EACH OF THE DRAWINGS HAVE BEEN REVISED TO INDICATE "ISSUED FOR PERMIT"
- 3. THIS PLAN IS SUBJECT TO ALL EASEMENTS AND RESTRICTIONS OF RECORD.
- 4. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE CODES, ORDINANCES, LAWS AND REGULATIONS OF ALL MUNICIPALITIES, UTILITIES OR OTHER PUBLIC AUTHORITIES.
- 5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS THAT MAY BE REQUIRED BY ANY FEDERAL, STATE, COUNTY OR MUNICIPAL AUTHORITIES.
- 6. THE CONTRACTOR SHALL NOTIFY THE CONSTRUCTION MANAGER, IN WRITING, OF ANY CONFLICTS, ERRORS OR OMISSIONS PRIOR TO THE SUBMISSION OF BIDS OR PERFORMANCE OF WORK. MINOR OMISSIONS OR ERRORS IN THE BID DOCUMENTS SHALL NOT EXCUSE SAID CONTRACTOR FROM COMPLETING THIS PROJECT IN ACCORDANCE WITH THE OVERALL INTENT OF THESE DRAWINGS.
- 7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING ALL EXISTING SITE IMPROVEMENTS PRIOR TO COMMENCING CONSTRUCTION. THE CONTRACTOR SHALL REPAIR ANY DAMAGE CAUSED AS A RESULT OF CONSTRUCTION OF THIS FACILITY.
- 8. THE SCOPE OF WORK FOR THIS PROJECT SHALL INCLUDE PROVIDING ALL MATERIALS, EQUIPMENT AND LABOR REQUIRED TO COMPLETE THIS PROJECT. ALL EQUIPMENT SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- 9. THE CONTRACTOR SHALL VISIT THE PROJECT SITE PRIOR TO SUBMITTING A BID TO VERIFY THAT THE PROJECT CAN BE CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- 10. POWER TO THE FACILITY IS MONITORED BY AN EXISTING METER.
- 11. ALL STRUCTURAL ELEMENTS SHALL BE HOT DIP GALVANIZED STEEL.
- 12. CONTRACTOR SHALL MAKE A UTILITY "ONE CALL" TO LOCATE ALL UTILITIES PRIOR TO EXCAVATING.
- 13. IF ANY PIPING EXISTS BENEATH THE SITE AREA, CONTRACTOR MUST LOCATE IT AND CONTACT OWNER'S REPRESENTATIVE.
- 14. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR DETERMINING ALL CONSTRUCTION MEANS AND METHODS. THE CONTRACTOR IS ALSO RESPONSIBLE FOR ALL JOB SITE SAFETY.
- 15. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES AND EXISTING CONDITIONS AT THE SITE PRIOR TO FABRICATION AND/OR INSTALLATION OF ANY WORK IN THE CONTRACT AREA AND SUBMIT TO THE ENGINEER ANY DISCREPANCIES FROM THE DRAWINGS.
- 16. THE CONTRACTOR IS TO REVIEW ALL DRAWINGS AND SPECIFICATIONS IN THE CONTRACT DOCUMENT SET. THE CONTRACTOR SHALL COORDINATE ALL WORK SHOWN IN THE SET OF DRAWINGS. THE CONTRACTOR SHALL PROVIDE A COMPLETE SET OF DRAWINGS TO ALL SUB—CONTRACTORS AND RELATED PARTIES. THE SUB—CONTRACTOR SHALL EXAMINE ALL THE DRAWINGS AND SPECIFICATIONS FOR THE INFORMATION THAT AFFECTS THEIR WORK.
- 17. DETAILS ARE INTENDED TO SHOW END RESULT OF DESIGN. MINOR MODIFICATIONS MAY BE REQUIRED TO SUIT JOB DIMENSIONS OR CONDITIONS, AND SUCH MODIFICATIONS SHALL BE INCLUDED AS PART OF THE WORK.
- 18. ALL MATERIAL PROVIDED BY METROPCS IS TO BE REVIEWED BY THE CONTRACTOR AND ALL APPLICABLE SUB—CONTRACTORS PRIOR TO INSTALLATION. ANY DEFICIENCIES TO PROVIDE MATERIALS SHALL BE BROUGHT TO THE CONSTRUCTION MANAGER'S ATTENTION IMMEDIATELY.
- 19. THE MATERIALS INSTALLED SHALL MEET REQUIREMENTS OF CONTRACTORS DOCUMENTS, NO SUBSTITUTIONS ARE ALLOWED.
- 20. INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR CONFORMING MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE ENGINEER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER APPROVAL.

## GENERAL NOTES

- 21. THE CONTRACTOR SHALL RECEIVE CLARIFICATION AND AUTHORIZATION IN WRITING TO PROCEED BEFORE STARTING WORK ON ANY ITEMS NOT CLEARLY DEFINED OR IDENTIFIED BY THE CONSTRUCTION DOCUMENTS.
- 22. THE CONTRACTOR SHALL NOTIFY THE CONSTRUCTION MANAGER OF ALL PRODUCTS OR ITEMS NOTED AS "EXISTING" WHICH ARE NOT FOUND TO BE IN THE FIELD.
- 23. ERECTION SHALL BE DONE IN A WORKMANLIKE MANNER BY COMPETENT EXPERIENCED WORKMEN IN ACCORDANCE WITH APPLICABLE CODES AND THE BEST—ACCEPTED PRACTICE. ALL MEMBERS SHALL BE LAID PLUMB AND TRUE AS INDICATED ON THE DRAWINGS.
- 24. THE CONTRACTOR SHALL COORDINATE HIS WORK AND SCHEDULE HIS ACTIVITIES AND WORKING HOURS IN ACCORDANCE WITH THE REQUIREMENTS OF THE PROPERTY OWNER AND/OR PROPERTY MANAGEMENT COMPANY.
- 25. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING HIS WORK WITH THE WORK OF OTHERS AS IT MAY RELATE TO RADIO EQUIPMENT, ANTENNAS AND ANY OTHER PORTIONS OF THE WORK.
- 26. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY INDICATED OR WHERE LOCAL CODES OR REGULATIONS MAY TAKE PRECEDENCE.
- 27. THE CONTRACTOR SHALL REPAIR ALL EXISTING SURFACES DAMAGED DURING CONSTRUCTION SUCH THAT THEY MATCH AND BLEND WITH ADJACENT SURFACES.
- 28. THE CONTRACTOR SHALL KEEP CONTRACT AREA CLEAN, HAZARD FREE AND DISPOSE OF ALL DEBRIS AND RUBBISH. EQUIPMENT NOT SPECIFIED AS REMAINING ON THE PROPERTY OF THE OWNER SHALL BE REMOVED. LEAVE PREMISES IN CLEAN CONDITIONS AND FREE FROM PAINT SPOTS, DUST OR SMUDGES OF ANY NATURE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ALL ITEMS UNTIL COMPLETION OF CONSTRUCTION.
- 29. BEFORE FINAL ACCEPTANCE OF THE WORK, THE CONTRACTOR SHALL REMOVE ALL EQUIPMENT, TEMPORARY WORK, UNUSED AND USELESS MATERIALS, RUBBISH AND TEMPORARY STRUCTURES.
- 30. ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE 2005 STATE OF CONNECTICUT BUILDING CODE AND SUBSEQUENT AMENDMENTS.
- 31. CONTRACTOR SHALL VISIT THE JOB SITE AND SHALL FAMILIARIZE HIMSELF WITH ALL CONDITIONS AFFECTING THE PROPOSED WORK AND SHALL MAKE PROVISIONS AS TO THE COST THEREOF. CONTRACTOR SHALL BE RESPONSIBLE FOR FAMILIARIZING HIMSELF WITH ALL CONTRACT DOCUMENTS, FIELD CONDITIONS AND DIMENSIONS AND CONFIRMING THAT THE WORK MAY BE ACCOMPLISHED AS SHOWN PRIOR TO PROCEEDING WITH CONSTRUCTION. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO THE COMMENCEMENT OF WORK.
- 32. PLANS ARE NOT TO BE SCALED. THESE PLANS ARE INTENDED TO BE A DIAGRAMMATIC OUTLINE ONLY UNLESS OTHERWISE NOTED. THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT AND APPURTENANCES, AND LABOR NECESSARY TO EFFECT ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
- 33. CONTRACTOR SHALL BE RESPONSIBLE FOR THE SAFETY OF THE WORK AREA, ADJACENT AREAS AND BUILDING OCCUPANTS THAT ARE LIKELY TO BE AFFECTED BY THE WORK UNDER THIS CONTRACT. WORK SHALL CONFORM TO ALL OSHA REQUIREMENTS.
- 34. CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK USING THE BEST CONSTRUCTION SKILLS AND ATTENTION. CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES AND FOR COORDINATING ALL PORTIONS OF THE WORK UNDER CONTRACT, UNLESS OTHERWISE NOTED.



ORIGINAL SIZE IN INCHES



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• SURVEYING

TECTONIC Engineering and Surveying Consultants, P.C.

1279 ROUTE 300
NEWBURGH, NY 12550
Phone: (845) 567-6656
Fax: (845) 567-8703

## metroPCS

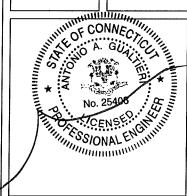
metroPCS WIRELESS, INC. 35 GRIFFIN ROAD SOUTH BLOOMFIELD CT 06002



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PROJECT NUMBER DESIGNED BY
7061.CTHA520A JQ
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SITE INFORMATION
CTHA520A
CROWN CROMWELL MONOPOLE
201 MAIN ST
CROMWELL, CT 06480

SHEET TITLE

| GENERAL NOTES

SHEET NUMBER

A - 10

## ELECTRICAL NOTES

- 1. CONTRACTOR SHALL PERFORM ALL VERIFICATION OBSERVATION TESTS, AND EXAMINATION WORK PRIOR TO THE ORDERING OF THE ELECTRICAL EQUIPMENT AND THE ACTUAL CONSTRUCTION. CONTRACTOR SHALL ISSUE A WRITTEN NOTICE OF ALL FINDINGS TO THE ENGINEER LISTING ALL MALFUNCTIONS, FAULTY EQUIPMENT AND DISCREPANCIES.
- CONTRACTOR SHALL PROVIDE ALL LABOR, MATERIALS, INSURANCE, EQUIPMENT, INSTALLATION, CONSTRUCTION TOOLS, TRANSPORTATION, ETC., FOR A COMPLETE AND PROPERLY OPERATIVE SYSTEM ENERGIZED THROUGHOUT AND AS INDICATED ON DRAWINGS, AS SPECIFIED HEREIN AND/OR AS OTHERWISE REQUIRED.
- 3. ALL MATERIALS AND EQUIPMENT SHALL BE NEW AND IN PERFECT CONDITION WHEN INSTALLED AND SHALL BE OF THE BEST GRADE AND OF THE SAME MANUFACTURER THROUGHOUT FOR EACH CLASS OR GROUP OF EQUIPMENT. MATERIALS SHALL BE LISTED AND APPROVED BY UNDERWRITER'S LABORATORIES (U.L.) AND SHALL BEAR THE INSPECTION LABEL "J" WHERE SUBJECT TO SUCH APPROVAL. MATERIALS SHALL MEET WITH APPROVAL OF ALL GOVERNING BODIES HAVING JURISDICTION. AND SHALL BE MANUFACTURED IN ACCORDANCE WITH APPLICABLE STANDARDS ESTABLISHED BY ANSI, NEMA AND NBFU.
- 4. CONTRACTOR TO COORDINATE WITH SITE OWNER FOR CONNECTION OF TEMPORARY AND PERMANENT POWER TO THE SITE. THE TEMPORARY POWER AND ALL HOOKUP COSTS TO BE PAID BY CONTRACTOR.
- 5. ALL ELECTRICAL EQUIPMENT SHALL BE LABELED WITH PERMANENT ENGRAVED PLASTIC LABELS.
- 6. ALL CIRCUIT BREAKERS, FUSES AND ELECTRICAL EQUIPMENT SHALL HAVE AN INTERRUPTING RATING NOT LESS THAN THE MAXIMUM SHORT CIRCUIT CURRENT TO WHICH THEY MAY BE SUBJECTED, AND A MINIMUM OF 10,000 A.I.C.
- 7. METER SOCKETS AMPERES, VOLTAGE AND NUMBER OF PHASES SHALL BE NOTED AND SHALL BE MANUFACTURED BY SQUARE "D" COMPANY, SANGAMO OR APPROVED EQUAL. METER SOCKET SHALL BE APPROVED BY UTILITY COMPANY PRIOR TO INSTALLATION.
- 8. WIRE AND CABLE CONDUCTORS SHALL BE COPPER #12 AWG MINIMUM WITH TYPE THHN INSULATION UNLESS SPECIFICALLY NOTED OTHERWISE.
- 9. ALL CONDUCTORS SHALL BE COPPER.
- 10. USE T-TAP CONNECTIONS ON ALL MULTI-CIRCUITS WITH COMMON NEUTRAL CONDUCTOR FOR LIGHTING FIXTURES.
- 11. EACH CONDUCTOR OF EVERY SYSTEM SHALL BE PERMANENTLY TAGGED IN EACH PANEL BOARD, PULLBOX, J-BOX, SWITCH BOX, ETC., IN COMPLIANCE WITH THE OCCUPATIONAL SAFETY AND HEALTH ACT (O.S.H.A.)
- 12. CONDUIT:
  - A. RIGID CONDUIT SHALL BE U.L. LABEL GALVANIZED ZINC COATED WITH ZINC INTERIOR AND SHALL BE USED WHEN INSTALLED IN OR UNDER CONCRETE SLABS, IN CONTACT WITH THE EARTH, UNDER PUBLIC ROADWAYS, IN MASONRY WALLS OR EXPOSED ON BUILDING EXTERIOR.
  - B. INTERMEDIATE METAL CONDUIT SHALL BE U.L. LABEL, FITTINGS SHALL BE THREADED ALUMINUM OR STEEL AND SHALL BE USED FOR ALL EXTERIOR RUNS. THREADLESS COUPLINGS AND CONNECTORS SHALL NOT BE USED.
  - C. ELECTRICAL METALLIC TUBING (EMT) SHALL HAVE U.L. LABEL, FITTINGS SHALL BE NO SET SCREW OR CRIMP TYPE FITTINGS SHALL BE USED. GLAND RING COMPRESSION TYPE. EMT SHALL BE USED ONLY FOR INTERIOR RUNS.
  - D. FLEXIBLE METALLIC CONDUIT SHALL HAVE U.L. LISTED LABEL AND MAY BE USED WHERE PERMITTED BY CODE. FITTINGS SHALL BE "JAKE" OR "SQUEEZE" TYPE, SEAL TIGHT FLEXIBLE CONDUIT. ALL CONDUIT IN EXCESS OF SIX FEET IN LENGTH SHALL HAVE FULL SIZE GROUND WIRE.
  - E. CONDUIT SHALL BE SIZED PER THE NEC AND AS SHOWN.
  - F. CONDUIT RUNS MAY BE SURFACE MOUNTED IN CEILINGS OR WALLS UNLESS INDICATED OTHERWISE. CONDUIT INDICATED SHALL RUN PARALLEL OR AT RIGHT ANGLES TO CEILING, FLOOR OR BEAMS. VERIFY EXACT ROUTING OF ALL EXPOSED CONDUIT WITH OWNER PRIOR TO INSTALLING.
  - G. ALL CONDUIT ONLY (C.O.) RUNS SHALL HAVE A PULL WIRE OR ROPE.

- 13. COVERPLATES SHALL BE BRUSHED STAINLESS STEEL FOR ALL SWITCHES, RECEPTACLES, TELEPHONE AND BLANKED OUTLETS, AND SHALL HAVE ENGRAVED LETTERING WHERE INDICATED WEATHERPROOF RECEPTACLES SHALL HAVE SIERRA #WPD—8 LIFT COVERPLATES.
- 14. REFER TO MANUFACTURERS MANUAL FOR RECOMMENDED FUSE AND WIRE SIZES.
- 15. ALL FINAL CONNECTIONS TO THE EQUIPMENT ARE TO BE OF FLEXIBLE WEATHERPROOF CONDUIT TO MEET APPLICABLE CODES.
- 16. THE ENTIRE ELECTRICAL INSTALLATION SHALL BE GROUNDED AS REQUIRED BY ALL APPLICABLE CODES.
- 17. GROUNDING CONDUCTORS SHALL BE SOLID TINNED COPPER AND ANNEALED #2, UNLESS OTHERWISE NOTED.
- 18. UPON COMPLETION OF WORK, CONDUCT CONTINUITY, SHORT CIRCUIT, AND FALL OF POTENTIAL GROUNDING TESTS FOR APPROVAL. SUBMIT TEST REPORTS TO THE CONSTRUCTION MANAGER. CLEAN PREMISES OF ALL DEBRIS RESULTING FROM WORK AND LEAVE WORK IN A COMPLETE AND UNDAMAGED CONDITION.
- 19. PROVIDE CONSTRUCTION MANAGER WITH ONE SET OF COMPLETE ELECTRICAL "AS INSTALLED" DRAWINGS AT THE COMPLETION OF THE JOB, SHOWING ACTUAL DIMENSIONS, ROUTINGS, AND CIRCUITS.
- 20. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING WITH GAINING APPROVALS AND PAYING ALL FEES ASSESSED BY UTILITY COMPANY FOR ELECTRICAL SERVICE.

## GROUNDING NOTES

- 1. THE ENTIRE ELECTRICAL INSTALLATION SHALL BE GROUNDED AS REQUIRED BY ALL APPLICABLE CODES.
- 2. ALL GROUNDING WORK SHALL BE IN ACCORDANCE WITH METROPCS STANDARD PRACTICE.
- 3. ALL BUS CONNECTORS SHALL BE TWO—HOLE, LONG—BARREL TYPE COMPRESSION LUGS, T&B OR EQUAL, UNLESS OTHERWISE NOTED ON DRAWINGS. ALL LUGS SHALL BE ATTACHED TO BUSSES USING BOLTS, NUTS, AND LOCK WASHERS. NO WASHERS ARE ALLOWED BETWEEN THE ITEMS BEING GROUNDED.
- 4. ALL CONNECTORS SHALL BE CRIMPED USING HYDRAULIC CRIMPING TOOLS, T&B #TBM 8 OR EQUIVALENT.
- I. ALL CONNECTIONS SHALL BE MADE TO BARE METAL. ALL PAINTED SURFACES SHALL BE FILED TO ENSURE PROPER CONTACT. NO WASHERS ARE ALLOWED BETWEEN THE ITEMS BEING GROUNDED. ALL CONNECTIONS ARE TO HAVE A NON-OXIDIZING AGENT APPLIED PRIOR TO INSTALLATION.
- 6. ALL COPPER BUSSES SHALL BE CLEANED, POLISHED, AND A NON-OXIDIZING AGENT APPLIED. NO FINGERPRINTS OR DISCOLORED COPPER WILL BE PERMITTED.
- 7. ALL BENDS SHALL BE AS SHALLOW AS POSSIBLE, WITH NO TURN SHORTER THAN AN 8-INCH NOMINAL RADIUS.
- 8. GROUNDING CONDUCTORS SHALL BE SOLID TINNED COPPER AND ANNEALED #2. ALL GROUNDING CONDUCTORS SHALL RUN THROUGH PVC SLEEVES WHEREVER CONDUCTORS RUN THROUGH WALLS, FLOORS, OR CEILINGS. IF CONDUCTORS MUST RUN THROUGH EMT, BOTH ENDS OF CONDUIT SHALL BE GROUNDED. SEAL BOTH ENDS OF CONDUIT WITH SILICONE CAULK.
- GROUNDING SYSTEM RESISTANCE SHALL NOT EXCEED 10 OHMS. IF THE RESISTANCE VALUE IS EXCEEDED, NOTIFY THE PROJECT MANAGER FOR FURTHER INSTRUCTION ON METHODS FOR REDUCING THE RESISTANCE VALUE.
- 10. ALL ROOF TOP ANTENNA MOUNTS SHALL BE GROUNDED WITH A #2 GROUND WIRE CONNECTED TO THE NEAREST GROUND BUS. ALL CONNECTIONS ARE TO BE CAD—WELDED IF POSSIBLE.
- 11. UPON COMPLETION OF WORK, CONDUCT CONTINUITY, SHORT CIRCUIT, AND FALL OF POTENTIAL GROUNDING TESTS FOR APPROVAL. SUBMIT TEST REPORTS TO THE PROJECT MANAGER.
- 12. GROUNDING CONNECTION TO TRAVEL IN A DOWNWARD DIRECTION.
- 13. ALL EXPOSED #2 WIRE MUST BE TINN NOT BTW.
- 14. TECTONIC TAKES NO RESPONSIBILITY OR LIABILITY FOR THE GROUNDING SYSTEM AS SHOWN ON THIS SITE. THIS IS A STANDARD GROUNDING SYSTEM.

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1279 ROUTE 300 NEWBURGH, NY 12550 Phone: (845) 567-6656 Fax: (845) 567-8703

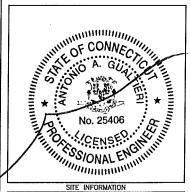
## metroPCS

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CTHA520A CROWN CROMWELL MONOPOLE 201 MAIN ST CROMWELL, CT 06480

SHEET TITLE

ELECTRICAL & GROUNDING NOTES

SHEET NUMBER

A-11

## STRUCTURAL NOTES

- DESIGN AND CONSTRUCTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN".
- 2. STRUCTURAL STEEL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, "STEEL FOR STRUCTURAL SHAPES FOR USE IN BUILDING FRAMING", GRADE 50, UNLESS OTHERWISE INDICATED. IF THE MEMBER SIZES INDICATED ARE NOT AVAILABLE IN THIS GRADE, ASTM A572 "HIGH-STRENGTH LOW-ALLOY COLUMBIUM-VANADIUM STRUCTURAL STEEL", GRADE 50, MAY BE SUBSTITUTED.
- 3. HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A500 "COLD—FORMED WELDED & SEAMLESS CARBON STEEL STRUCTURAL TUBING IN ROUNDS AND SHAPES", GRADE B.
- 4. MISCELLANEOUS STEEL, INCLUDING CHANNELS, ANGLES, PLATES, AND BARS SHALL CONFORM TO ASTM A36 "CARBON STRUCTURAL STEEL", UNLESS OTHERWISE INDICATED.
- ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 "ANCHOR BOLTS, STEEL, 36, 55, AND 105-KSI YIELD STRENGTH", GRADE 36.
- 6. STRUCTURAL CONNECTION BOLTS SHALL BE HIGH STRENGTH BOLTS CONFORMING TO ASTM A325 "STRUCTURAL BOLTS, STEEL, HEAT TREATED, 120/105 KSI MINIMUM TENSILE STRENGTH". BOLTS SHALL BE 3/4 INCH DIAMETER, TYPE X, UNLESS OTHERWISE NOTED.
- MATCHING NUTS SHALL BE HEAVY HEX TYPE, CONFORMING TO ASTM A563 "CARBON AND ALLOY STEEL NUTS".
   WASHERS, WHERE REQUIRED, SHALL CONFORM TO ASTM F436 "HARDENED STEEL WASHERS".
- 8. FIELD CONNECTIONS SHALL BE BOLTED UNLESS OTHERWISE INDICATED. ALL BOLTED CONNECTIONS SHALL BE MADE WITH NOT LESS THAN TWO (2) HIGH STRENGTH BOLTS, OR EQUIVALENT WELD.
- 9. STRUCTURAL CONNECTIONS SHALL BE SNUG TIGHT IN ACCORDANCE WITH THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS", UNLESS OTHERWISE NOTED.
- 10. BOLTS IN SLIP-CRITICAL CONNECTIONS SHALL BE FULLY PRETENSIONED BY THE TURN-OF-NUT METHOD IN ACCORDANCE WITH THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".
- 11. ANCHOR BOLTS SHALL BE TENSIONED BY THE TURN-OF-NUT METHOD AFTER GROUTING OF BASE PLATES.
- 12. CONTRACTOR SHALL COMPLY WITH AWS D1.1 "STRUCTURAL WELDING CODE STEEL' FOR PROCEDURES, APPEARANCE AND QUALITY OF WELDS, AND FOR METHODS USED IN CORRECTING WELDING. ALL WELDERS AND WELDING PROCESSES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURES".
- 13. GRATING SHALL BE TYPE "W/B" GALVANIZED WELDED STEEL BAR GRATING AS MANUFACTURED BY IKG BORDEN, OR APPROVED EQUAL. BEARING BARS SHALL BE AS FOLLOWS:

GRATING 1-1/4" x 3/16" SERRATED

BAND ALL EDGES, AND ATTACH TO SUPPORTING MEMBERS AT 18" ON CENTER WITH MODEL GG GALVANIZED G-CLIPS AS MANUFACTURED BY GRATING FASTENERS INC.

14. EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT III OR APPROVED EQUAL. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE 4-3/4" UNLESS OTHERWISE NOTED

## STRUCTURAL NOTES

15. EPOXY ANCHOR ASSEMBLIES SHALL BE AS MANUFACTURED BY HILTI OR ENGINEER APPROVED EQUAL, AS FOLLOWS:

BASE MATERIAL ANCHOR SYSTEM
CONCRETE OR GROUTED CMU HIT HY-200
HOLLOW CMU HIT HY-70

INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS.

- 16. ALL INTERIOR STRUCTURAL STEEL SHALL BE SHOP PRIME COATED WITH A RUST—INHIBITIVE PRIMER EXCEPT AREAS TO BE FIREPROOFED NEED NOT BE PAINTED. SURFACE PREPARATION SHALL BE IN ACCORDANCE WITH THE PAINT MANUFACTURER'S RECOMMENDATIONS. AREAS WHICH MAY BE INACCESSIBLE AFTER INSTALLATION SHALL RECEIVE TWO (2) COATS OF PRIMER. SEE ARCHITECTURAL DRAWINGS FOR FINISH PAINT.
- 17. FIELD CONNECTIONS AND DAMAGED OR ABRADED AREAS OF SHOP PRIME COAT SHALL BE TOUCH—UP PAINTED WITH COMPATIBLE FIELD PRIMER.
- 18. ALL EXTERIOR STEEL SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT-DIP GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS", UNLESS OTHERWISE NOTED.
- 19. ALL EXTERIOR BOLTS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC COATING (HOT-DIP) ON IRON AND STEEL HARDWARE", UNLESS OTHERWISE NOTED.
- 20. DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED BY COLD GALVANIZING IN ACCORDANCE WITH ASTM A780 "REPAIR OF DAMAGED AND UNCOATED AREAS OF HOT—DIP GALVANIZED COATINGS".
- 21. ALL STEEL WORK SHALL BE SUBJECT TO SPECIAL INSPECTIONS DURING CONSTRUCTION.
- 22. THE NOTES CONTAINED HEREIN ARE NOT PROJECT SPECIFIC. THE CONTRACTOR SHALL UTILIZE ALL NOTES WHICH SOLELY PERTAIN TO THE WORK DEPICTED ON THESE DRAWINGS.

## EARTHWORK NOTES

- ALL EXCAVATIONS SHALL BE DEWATERED BY SUMPING, PUMPING, ETC. IN A MANNER WHICH WILL NOT LOOSEN FOUNDATION SUBGRADE MATERIAL. SURFACE WATER SHALL BE DIVERTED AWAY FROM EXCAVATIONS BY MEANS OF BERMS, DIVERSION DITCHES, OR OTHER SUITABLE METHODS.
- CONFINED EXCAVATIONS FOR FOUNDATIONS, UTILITIES, ETC. SHALL BE LIMITED TO 4 FEET IN DEPTH UNLESS SHORING AND BRACING IS USED. TRENCH EXCAVATION GEOMETRY AND/OR BRACING SHALL CONFORM WITH OSHA REQUIREMENTS.
- 3. BACKFILL SHALL BE PLACED IN MAXIMUM LOOSE LIFT THICKNESSES OF 8 INCHES AND COMPACTED WITH SUITABLE COMPACTION EQUIPMENT. ALL FILL IN CONFINED AREAS WHERE ONLY HAND TAMPING IS FEASIBLE, FILL SHALL BE PLACED IN MAXIMUM 4 INCH LOOSE LIFTS AND COMPACTED TO THE AFOREMENTIONED CRITERIA. 95% OF MAXIMUM DRY DENSITY PER ASTM D1557.
- 4. ALL FILL SHALL BE CLEAN AND FREE OF LARGE ROCKS, NO ORGANIC MATTER SHALL BE PERMITTED.
- 5. TEMPORARY EROSION CONTROL STRUCTURES SHALL BE INSTALLED AS REQUIRED BEFORE SITE IS DISTURBED.
- 6. ALL AREAS DISTURBED BY ON-SITE GRADING THAT WILL NOT BE CONSTRUCTED UPON, SHALL BE ESTABLISHED WITH PERMANENT VEGETATION.

CONFIGURATION

5

REFER TO LATEST T-MOBILE RF DATA SHEET FOR FINAL RF DESIGN & BOM.

ORIGINAL SIZE IN INCHES



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### metroPCS

metroPCS WIRELESS, INC. 35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002



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SITE INFORMATION

CTHA520A

CROWN CROMWELL MONOPOLE

201 MAIN ST CROMWELL, CT 06480

SHEET TITLE

STRUCTURAL & EARTHWORK NOTES

SHEET NUMBER

A - 12

## CONCRETE NOTES

- DESIGN AND CONSTRUCTION OF ALL CONCRETE SHALL CONFORM TO THE AMERICAN CONCRETE INSTITUTE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" ACI 318.
- ULTIMATE COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE 3000 PSI. IF IT IS NOT SPECIFIED IN THE DRAWING.
- CEMENT SHALL BE PORTLAND CEMENT CONFORMING TO ASTM C150 TYPE I OR II.
- REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60, "DEFORMED AND PLAIN BILLET STEEL BARS FOR CONCRETE REINFORCEMENT".
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, "WELDED STEEL WIRE FABRIC PLAIN FOR CONCRETE REINFORCEMENT".
- CONCRETE WORK AND MATERIALS SHALL CONFORM TO THE AMERICAN CONCRETE INSTITUTE "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS", ACI 301.
- SUBMIT CONCRETE MIX DESIGN TO THE DESIGN ENGINEER FOR APPROVAL NOT LESS THAN 3 DAYS PRIOR TO CONSTRUCTION. MIX DESIGN SHALL BE APPROVED BY THE ENGINEER PRIOR TO PLACEMENT OF CONCRETE.
- READY MIX CONCRETE SHALL COMPLY WITH ACI 304 AND ASTM C94 WITH A MAXIMUM WATER-CEMENT RATIO OF 0.50. TIME BETWEEN INTRODUCTION OF WATER AND THE PLACEMENT OF CONCRETE SHALL NOT EXCEED 1-1/2 HOURS.
- CONCRETE AGGREGATES SHALL BE NORMAL WEIGHT, CONFORMING TO ASTM C33. MAXIMUM SIZE OF COARSE AGGREGATE SHALL BE 3/4".
- CHLORIDE-CONTAINING ADMIXTURES SHALL NOT BE USED
- CONCRETE SLUMP SHALL NOT EXCEED 5 INCHES UNLESS SPECIFICALLY AUTHORIZED BY THE ENGINEER. SLUMP SHALL BE DETERMINED IN ACCORDANCE WITH ASTM
- PROVIDE AIR ENTRAINMENT IN EXTERIOR EXPOSED CONCRETE TO OBTAIN TOTAL AIR CONTENT OF 5% ± 1% IN ACCORDANCE WITH ACI 301.
- FOR CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH, CONCRETE COVER FOR REINFORCING SHALL BE: 3" FOR ALL BARS FOR CONCRETE EXPOSED TO EARTH OR WEATHER, MINIMUM COVER SHALL BE: FOR #6 AND LARGER BARS
  - 1-1/2" FOR #5 AND SMALLER BARS OR WIRE FABRIC
- LAP SPLICES FOR REINFORCING SHALL BE IN ACCORDANCE WITH ACI 318.12 AND STANDARD HOOKS SHALL CONFORM TO ACI 318.7.
- WELDING OF REINFORCING STEEL OR THE APPLICATION OF HEAT TO FACILITATE BENDING IS SPECIFICALLY PROHIBITED.
- ALL REINFORCING, ANCHOR BOLTS, DOWELS, EMBEDDED STEEL, INSERTS AND ALL OTHER EMBEDDED ITEMS AND FORMED DETAILS SHALL BE IN PLACE BEFORE START OF CONCRETE PLACEMENT.
- PROVIDE A 3/4" CHAMFER AT ALL EXPOSED EDGES OF CONCRETE, UNLESS OTHERWISE NOTED.
- PROVIDE NOT LESS THAN 48 HOURS NOTICE TO THE FIELD REPRESENTATIVE PRIOR 18. TO PLACEMENT OF CONCRETE.
- WHEN AMBIENT TEMPERATURE IS BELOW 50 DEGREES F, CONCRETE MATERIALS AND PLACEMENT SHALL CONFORM TO THE RECOMMENDATIONS OF ACI 306R "COLD WEATHER CONCRETING".
- WHEN AMBIENT TEMPERATURE IS ABOVE 90 DEGREES F, CONCRETE MATERIALS AND 20. PLACEMENT SHALL CONFORM TO THE RECOMMENDATIONS OF ACI 305R "HOT WEATHER CONCRETING".
- REMOVE ALL LOOSE MATERIAL AND DEBRIS FROM COMPACTED SUBGRADE SURFACE PRIOR TO PLACING CONCRETE. CONCRETE SHALL NOT BE PLACED ON FROZEN
- 22. CONCRETE SHALL BE SUFFICIENTLY CONSOLIDATED BY VIBRATION TO REMOVE AIR VOIDS, VIBRATION SHALL BE IN ACCORDANCE WITH ACI 309 "STANDARD PRACTICE FOR CONSOLIDATION OF CONCRETE".

## CONCRETE NOTES

- THE TOP OF ALL CONCRETE SURFACES SHALL BE TRUE AND LEVEL WITH A SMOOTH FLOAT FINISH, UNLESS OTHERWISE NOTED. ALL DIMENSIONS SHALL BE WITHIN  $\pm$  1/8 INCH.
- TESTING OF CONCRETE SHALL BE PERFORMED UNDER THE DIRECTION OF THE 24. CONSTRUCTION MANAGER.
- 25. THROUGHOUT CONSTRUCTION, THE CONCRETE WORK SHALL BE ADEQUATELY PROTECTED AGAINST DAMAGE DUE TO EXCESSIVE LOADING, CONSTRUCTION EQUIPMENT, MATERIALS OR METHODS, ICE, RAIN, OR SNOW. PROTECT CONCRETE FROM EXCESSIVE HEAT AND FREEZING FOR NOT LESS THAN 14
- DRYING OUT OF CONCRETE, ESPECIALLY DURING THE FIRST 24 HOURS, SHALL BE CAREFULLY GUARDED AGAINST. ALL SURFACES SHALL BE MOIST CURED OR PROTECTED USING A MEMBRANE CURING AGENT CONFORMING TO ASTM C309 APPLIED AS SOON AS FORMS ARE REMOVED. IF MEMBRANE CURING AGENT IS USED, EXERCISE CARE NOT TO DAMAGE SURFACE.
- 27. CONTRACTOR SHALL BRING TO THE IMMEDIATE ATTENTION OF THE CONSTRUCTION MANAGER ANY DEFECTS OR ERRORS IN THE WORK, PRIOR TO MAKING REPAIRS. CONTRACTOR SHALL OBTAIN PERMISSION FROM THE CONSTRUCTION MANAGER TO PATCH OR OTHERWISE REPAIR DEFECTS OTHER THAN MINOR HONEYCOMBING,
- SET GROUT IN ACCORDANCE WITH MASTER BUILDERS TECHNOLOGIES OR APPROVED EQUAL, UNLESS SPECIFIED IN THE DRAWING.
- JOINT FILLER SHALL BE PREFORMED RESILIENT BITUMINOUS EXPANSION JOINT 29. FILLER CONFORMING TO ASTM D1751.
- 30. EXTERIOR WALKING SURFACES SHALL RECEIVE A BROOM FINISH.
- GROUT SHALL BE NON METALLIC, NON SHRINK PREPACKAGED GROUT WITH A 31. MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS. GROUT SHALL BE FIVE STAR GROUT AS MANUFACTURED BY FIVE STAR PRODUCTS, FAIRFIELD, CT OR APPROVED EQUAL
- CONCRETE ANCHORS SHALL BE HEADED STEEL STUDS MEETING THE REQUIREMENTS OF ASTM A108 "STEEL BARS, CARBON, COLD FINISHED, STANDARD QUALITY".

 ENGINEERING MANAGEMENT **TECTONIC** Engineering and Surveying Consultants, P.C. 1279 ROUTE 300 NEWBURGH, NY 12550 Phone: (845) 567-6656 Fax: (845) 567-8703 metroPCS metroPCS WIRELESS, INC. 35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002 LANDLORD CONSTRUCTION OPERATIONS SITE ACQ. DESIGNED BY PROJECT NUMBER 7061.CTHA520A REV DATE REVISION DRAWN BY 0 11/07/14 FOR COMMENT 11/12/14 FOR CONSTRUCTION MP

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CROWN CROMWELL MONOPOLE 201 MAIN ST CROMWELL, CT 06480

SHEET TITLE

CONCRETE NOTES

SHEET NUMBER

A - 13

REFER TO LATEST T-MOBILE RF DATA SHEET FOR FINAL RF DESIGN & BOM. ORIGINAL SIZE IN INCHES

CONFIGURATION

August 21, 2014

Andrew Bazinet Crown Castle 3 Corporate Park Drive Suite 101 Clifton Park, NY 12065 (585) 899-3442



B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 (918) 587-4630 btwo@btgrp.com

Subject:

Structural Analysis Report

Carrier Designation:

Metro PCS Co-Locate Carrier Site Number: Carrier Site Name:

HFC0210A

N/A

Crown Castle Designation:

Crown Castle BU Number:

876364

Crown Castle Site Name:

Cromwell / First Line Emergenc 295284

Crown Castle JDE Job Number: Crown Castle Work Order Number:

915224

**Crown Castle Application Number:** 

252176 Rev. 1

Engineering Firm Designation:

B+T Group Project Number:

84470.010.01

Site Data:

201 Main St., Cromwell, Middlesex County, CT Latitude 41° 35' 0.11", Longitude -72° 38' 59.14"

125 Foot - Monopole Tower

Dear Andrew Bazinet.

B+T Group is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 683239, in accordance with application 252176, revision 1.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment

Sufficient Capacity

Note: See Table 1 and Table 2 for the proposed and existing/reserved loading, respectively.

The analysis has been performed in accordance with the TIA/EIA-222-F standard and 2005 CT Building Code with 2009 amendment based upon a wind speed of 85 mph fastest mile.

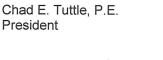
All equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

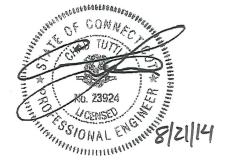
We at B+T Group appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted by: B+T Engineering, Inc.

tnxTower Report - version 6.1.4.1

Jennifer Tillson Project Engineer President





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tnxTower Output

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#### 1) INTRODUCTION

This tower is a 125 ft Monopole tower designed by Engineered Endeavors, Inc. in February of 2002. The tower was originally designed for a wind speed of 90 mph per TIA/EIA-222-F. This tower has modified multiple times and those modifications are incorporated in this analysis.

#### 2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 85 mph with no ice, 37.6 mph with 0.75 inch ice thickness and 50 mph under service loads.

**Table 1 - Proposed Antenna and Cable Information** 

•	Mounting Level (ft)	Flouretiens	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
,			3	Ericsson	ERICSSON AIR 21 B2A B4P			
	85.0	85.0	3	Ericsson	ERICSSON AIR 21 B4A B2P	1	1-3/16	
			1		T-Arm Mount [TA 601-3]			

Table 2 - Existing and Reserved Antenna and Cable Information

Mounting	Center	Number of Antennas	Antenna Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
	129.0	3	Argus Tech.	LLPX310R-V1			
	127.0	3	RFS Celwave	APXVSPP18-C-A20		5/16	
		2	Dragonwave	HORIZON COMPACT	3		
	125.0	3	Samsung Telecomm.	WIMAX DAP HEAD	3	1/4 1/2	1
125.0		1		Platform Mount [LP 712-1]	2 3	1-1/4	
	124.0	1	Andrew	VHLP2-11			
	124.0	1	Andrew	VHLP2-18			
	127.0	3	Alcatel Lucent	TD-RRH8x20-25	1	1-1/4	2
	127.0	3	RFS Celwave	APXVTM14-C-120			
	123.0	3	Alcatel Lucent	800MHZ 2X50W RRH W/FILTER			
123.0		3	Alcatel Lucent	PCS 1900MHz 4x45W- 65MHz			1
		1		Side Arm Mount [SO 102-3]			
		6	Comm. Components Inc.	DTMABP7819VG12A			
445.0	117.0	3	Ericsson	RRUS-11	12	1-1/4	
115.0		9	KMW Comm.	AM-X-CD-16-65-00T-RET	2	3/4 3/8	1
		1	Raycap	DC6-48-60-18-8F	'	3/0	
	115.0	1		Platform Mount [LP 303-1]			
		3	Alcatel Lucent	RRH2X40-AWS			
105.0	105.0	3	Andrew	HBX-6517DS-VTM	1	1 5/0	2
105.0	105.0	1	RFS Celwave	DB-T1-6Z-8AB-0Z	'	1-5/8	2
		3	Andrew	LNX-6514DS-VTM			

Mounting Level (ft)	Flevation	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note	
		3	Antel	BXA-171063-8BF-EDIN-2				
		3	Antel	BXA-70063-6CF-EDIN-0	12	1-5/8	1	
		6	RFS Celwave	FD9R6004/2C-3L				
		1		Platform Mount [LP 712-1]				
95.0	95.0	12	Decibel	DB844H65E-XY	12	1-5/8	3	
95.0	95.0	1		Platform Mount [LP 303-1]	12	1-5/8		
	85.0 85.0	3	3	Kathrein	742 213			3
85.0		1		Pipe Mount [PM 601-3]			<u> </u>	
					6	1-5/8	1	

Notes:

- **Existing Equipment** 1)
- 2) **3)**
- Reserved Equipment
  Equipment To Be Removed

**Table 3 - Design Antenna and Cable Information** 

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
		1	Generic	L.P.Platform		
125	125 125	6	Decibel	DB980H65	<b></b>	
		3	Decibel	DB980H90		
115	115	1	Generic	T-Arm		
110 110	6	Allgon	7250			
105 105	1	Generic	L.P.Platform			
100	105	12	Decibel	DB844		

#### 3) ANALYSIS PROCEDURE

**Table 4 - Documents Provided** 

Table 4 - Documents i Tovided					
Document	Remarks	Reference	Source		
Online Application	Metro PCS Co-Locate Rev # 1	252176	CCI Sites		
Tower Manufacturer Drawings	EEI, Job No. 10554	2068958	CCI Sites		
Tower Modification Drawings	Semaan Engineering Solutions, Job No. CT23XC558	2055765	CCI Sites		
Post-Modification Inspection	VSI, Job No. 2007-004-138	2182292	CCI Sites		
Tower Modification Drawings	B+T Group, Project No. 84890.001	3373019	CCI Sites		
Post Modification Inspection	B+T Group, Project No. 84890.002	3394680	CCI Sites		
Tower Modification Drawings	B+T Group, Project No. 84470.006.01	3669962	CCI Sites		
Post-Modification Inspection	TEP, Project No. 131001.876364	4009982	CCI Sites		
Foundation Drawings	EEI, Project No. 6464	1613909	CCI Sites		
Base Plate Details	Crown, Project No. 320820	2608627	CCI Sites		
Geotech Report	Dr. Clarence Welti, P.E., Date: 08/02/1999	1532312	CCI Sites		
Antenna Configuration	Crown CAD Package	Date: 06/10/2014	CCI Sites		

#### 3.1) Analysis Method

tnxTower (version 6.1.4.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

#### 3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by TIA/EIA-222-F.
- 5) Mount areas and weights are assumed based on photographs provided.
- 6) The existing base plate grout was not considered in this analysis.

This analysis may be affected if any assumptions are not valid or have been made in error. B+T Group should be notified to determine the effect on the structural integrity of the tower.

#### 4) ANALYSIS RESULTS

**Table 5 - Section Capacity (Summary)** 

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	125 - 94.45	Pole	TP25.067x18.5x0.188	1	-7.661	769.746	96.1	Pass
L2	94.45 - 85.04	Pole	TP27.09x25.067x0.439	2	-8.480	1445.985	61.2	Pass
L3	85.04 - 73.5	Pole	TP29.158x25.873x0.475	3	-12.889	1743.737	75.9	Pass
L4	73.5 - 73	Pole	TP29.264x29.158x0.609	4	-13.018	2235.441	60.3	Pass
L5	73 - 60.5	Pole	TP31.921x29.264x0.367	5	-15.482	1908.243	88.0	Pass
L6	60.5 - 40.457	Pole	TP36.18x31.921x0.436	6	-19.353	2492.137	81.8	Pass
L7	40.457 - 30.5	Pole	TP37.787x34.6x0.485	7	-24.485	2983.507	80.2	Pass
L8	30.5 - 0	Pole	TP44.25x37.787x0.456	8	-33.922	3297.309	90.1	Pass
							Summary	
						Pole (L1)	96.1	Pass
						Rating =	96.1	Pass

Table 6 - Tower Component Stresses vs. Capacity – LC7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	Base	84.0	Pass
1	Base Plate	Base	83.0	Pass
1	Base Foundation (Soil Interaction)	Base	80.9	Pass

Structure Rating (max from all components) =	96.1%
--	-------

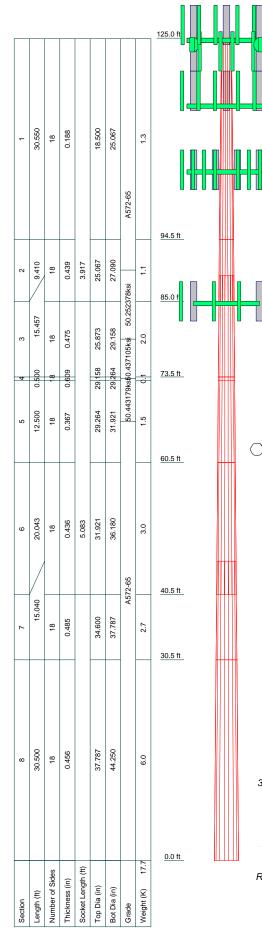
Notes:

<sup>1)</sup> See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

#### 4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the existing, reserved, and proposed loads. No modifications are required at this time.

## APPENDIX A TNXTOWER OUTPUT



#### **DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION
APXVTM14-C-120 w/ Mount Pipe	125	Platform Mount [LP 303-1] (E)	115
(R) APXVTM14-C-120 w/ Mount Pipe	125	(3) AM-X-CD-16-65-00T-RET w/ Mount Pipe (E)	115
(R) APXVTM14-C-120 w/ Mount Pipe	125	(3) AM-X-CD-16-65-00T-RET w/ Mount Pipe (E)	115
(R)		BXA-70063-6CF-EDIN-0 w/	105
TD-RRH8x20-25 (R)	125	Mount Pipe (E)	
TD-RRH8x20-25 (R) TD-RRH8x20-25 (R)	125	BXA-171063-8BF-EDIN-2 w/ Mount Pipe (E)	105
/ /		BXA-171063-8BF-EDIN-2 w/	105
LLPX310R-V1 w/ Mount Pipe (E)	125	Mount Pipe (E)	103
LLPX310R-V1 w/ Mount Pipe (E)	125	BXA-171063-8BF-EDIN-2 w/	105
LPX310R-V1 w/ Mount Pipe (E)	125	Mount Pipe (E)	
APXVSPP18-C-A20 w/ Mount Pipe (E)	125	(2) FD9R6004/2C-3L (E)	105
APXVSPP18-C-A20 w/ Mount	125	(2) FD9R6004/2C-3L (E)	105
Pipe (E)	-	(2) FD9R6004/2C-3L (E)	105
APXVSPP18-C-A20 w/ Mount Pipe (E)	125	LNX-6514DS-VTM w/ Mount Pipe (R)	105
WIMAX DAP HEAD (E)	125	LNX-6514DS-VTM w/ Mount Pipe	105
VIMAX DAP HEAD (E)	125	(R)	
VIMAX DAP HEAD (E)	125	LNX-6514DS-VTM w/ Mount Pipe (R)	105
HORIZON COMPACT (E)	125	. ,	405
HORIZON COMPACT (E)	125	HBX-6517DS-VTM w/ Mount Pipe (R)	105
(2) 6' x 2" Mount Pipe (E)	125	HBX-6517DS-VTM w/ Mount Pipe	105
(2) 6' x 2" Mount Pipe (E)	125	(R)	100
(2) 6' x 2" Mount Pipe (E)	125	HBX-6517DS-VTM w/ Mount Pipe	105
Platform Mount [LP 712-1] (E)	125	(R)	
VHLP2-18 (E)	125	RRH2X40-AWS (R)	105
VHLP2-11 (E)	125	RRH2X40-AWS (R)	105
PCS 1900MHz 4x45W-65MHz w /	123	RRH2X40-AWS (R)	105
Mount Pipe (E)		DB-T1-6Z-8AB-0Z (R)	105
800MHZ 2X50W RRH W/FILTER w / Mount Pipe (E)	123	Platform Mount [LP 712-1] (E)	105
BOOMHZ 2X50W RRH W/FILTER	123	BXA-70063-6CF-EDIN-0 w/ Mount Pipe (E)	105
w / Mount Pipe (E)	-	BXA-70063-6CF-EDIN-0 w/	105
800MHZ 2X50W RRH W/FILTER w / Mount Pipe (E)	123	Mount Pipe (E) ERICSSON AIR 21 B2A B4P w/	85
Side Arm Mount [SO 102-3] (E)	123	Mount Pipe (P)	
PCS 1900MHz 4x45W-65MHz w / Mount Pipe (E)	123	ERICSSON AIR 21 B4A B2P w/ Mount Pipe (P)	85
PCS 1900MHz 4x45W-65MHz w / Mount Pipe (E)	123	ERICSSON AIR 21 B4A B2P w/ Mount Pipe (P)	85
(3) AM-X-CD-16-65-00T-RET w/ Mount Pipe (E)	115	ERICSSON AIR 21 B4A B2P w/ Mount Pipe (P)	85
2) DTMABP7819VG12A (E)	115	T-Arm Mount [TA 601-3] (P)	85
(2) DTMABP7819VG12A (E)	115	ERICSSON AIR 21 B2A B4P w/	85
(2) DTMABP7819VG12A (E)	115	Mount Pipe (P)	
RRUS-11 (E)	115	ERICSSON AIR 21 B2A B4P w/	85
RRUS-11 (E)	115	Mount Pipe (P)	
RRUS-11 (E)	115	1	
DC6-48-60-18-8F (E)	115	1	

#### **MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi	50.437105ksi	50 ksi	65 ksi
50.252378ksi	50 ksi	65 ksi	50.443179ksi	50 ksi	65 ksi

#### **TOWER DESIGN NOTES**

AXIAL 51 K SHEAR 7K {

- 1. Tower is located in Middlesex County, Connecticut.
- 2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
- 3. Tower is also designed for a 38 mph basic wind with 0.75 in ice. Ice is considered voito increase in thickness with height.
- 4. Deflections are based upon a 50 mph wind.

TORQUE 0 kip-fi5. TOWER RATING: 96.1% 38 mph WIND - 0.750 in ICE

AXIAL 34 K MOMENT SHEAR<sup>4</sup> 28 K / 2629 kip-ft

TORQUE 0 kip-ft REACTIONS - 85 mph WIND

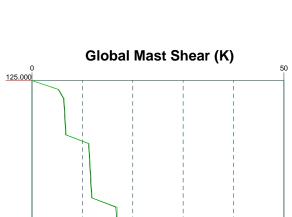
## GRP

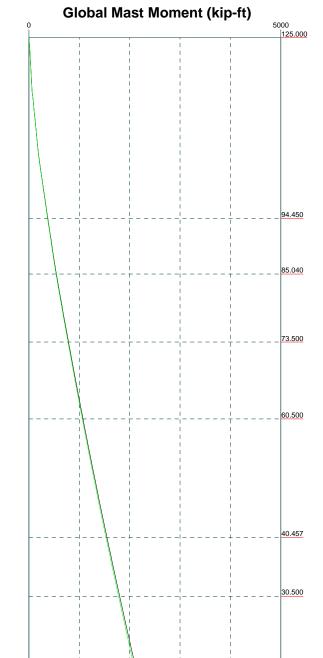
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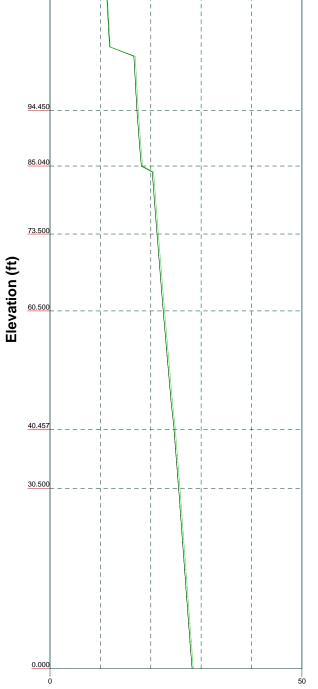
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#### 84470.010.01 - Cromwell/First Line Emergencl, CT (BU# 87636 Drawn by: JTillson App'd: Client: Crown Castle

Scale: NTS Date: 08/21/14 Code: TIA/EIA-222-F Dwg No. E-1







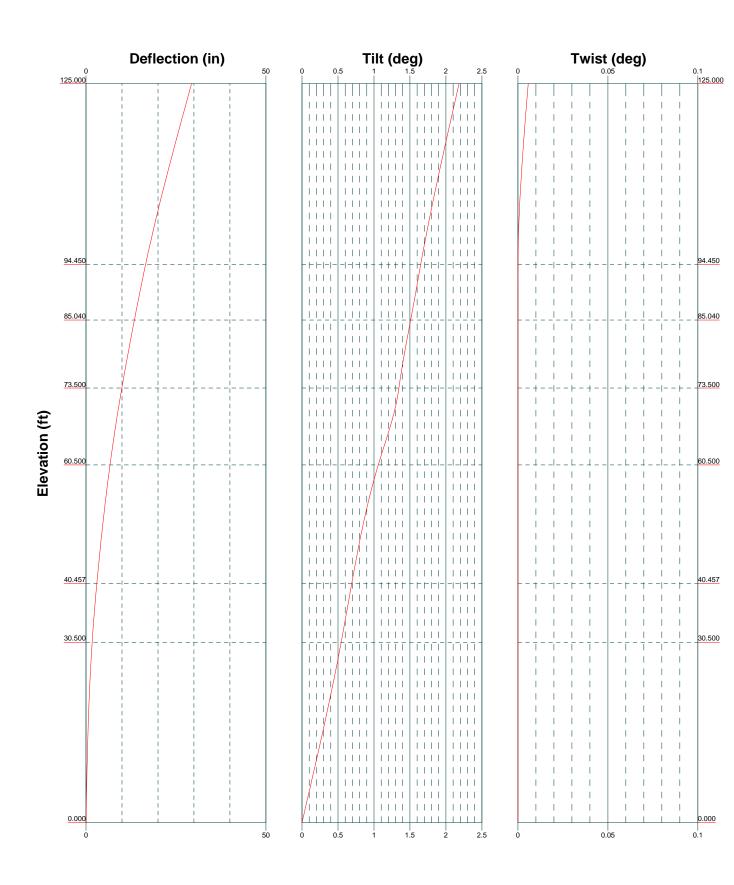


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lob: 84470.010.01 - Cromwell/First Line Emergencl, CT (BU# 87636					
Project:					
Client: Crown Castle	Drawn by: JTillson	App'd:			
Code: TIA/EIA-222-F	Date: 08/21/14	Scale: NTS			
Path:		Dwg No. ⊏ ⊿			

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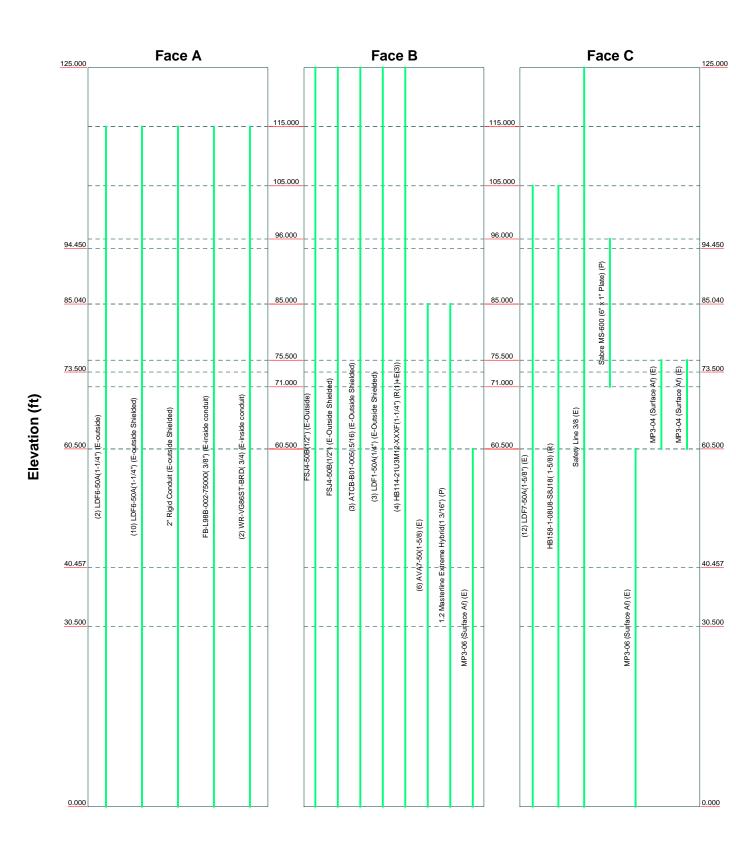


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<sup>Job:</sup> <b>84470.010.01 -</b>	Cromwell/First Line	Emergenci,	CT (BU# 87636
Project:			
Client: Crown Castle	Drawn by: ITillson	App'd:	

 Round \_\_\_\_\_\_ Flat \_\_\_\_\_ App In Face \_\_\_\_\_ App Out Face \_\_\_\_\_ Truss Leg



В+Т	P
в+т	GRP

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<sup>ob:</sup> 84470.010.01 - Cromwell/First Line Emergencl, CT (BU# 87636								
Project:		-						
Client: Crown Castle	Drawn by: JTillson	App'd:						
Code: TIA/EIA-222-F	Date: 08/21/14	Scale: NTS						
Path:		Dwg No. ⊏_7						

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Client	Designed by
Crown Castle	JTillson

### **Tower Input Data**

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in Middlesex County, Connecticut.

Basic wind speed of 85 mph.

Nominal ice thickness of 0.750 in.

Ice thickness is considered to increase with height.

Ice density of 56.000 pcf.

A wind speed of 38 mph is used in combination with ice.

Temperature drop of 50.000 °F.

Deflections calculated using a wind speed of 50 mph.

TOWER RATING: 96.1%.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

### **Options**

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

- √ Use Code Stress Ratios
- √ Use Code Safety Factors Guys
  - Escalate Ice
    Always Use Max Kz
    Use Special Wind Profile
    Include Bolts In Member Capacity
    Leg Bolts Are At Top Of Section
    Secondary Horizontal Braces Leg
    Use Diamond Inner Bracing (4 Sided)
    Add IBC .6D+W Combination

Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
- ✓ Use Clear Spans For Wind Area
   Use Clear Spans For KL/r
   Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- ✓ Project Wind Area of Appurt.
   Autocalc Torque Arm Areas
   SR Members Have Cut Ends
   Sort Capacity Reports By Component
   Triangulate Diamond Inner Bracing
   Use TIA-222-G Tension Splice Capacity
   Exemption

Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation

- √ Consider Feedline Torque Include Angle Block Shear Check Poles
- ✓ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets

## **Tapered Pole Section Geometry**

Section	Elevation	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft	Sides	in	in	in	in	
L1	125.000-94.450	30.550	0.000	18	18.500	25.067	0.188	0.750	A572-65 (65 ksi)
L2	94.450-85.040	9.410	3.917	18	25.067	27.090	0.439	1.757	50.252378ksi (50 ksi)
L3	85.040-73.500	15.457	0.000	18	25.873	29.158	0.475	1.899	50.437105ksi (50 ksi)
L4	73.500-73.000	0.500	0.000	18	29.158	29.264	0.609	2.437	50.443179ksi

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Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L5	73.000-60.500	12.500	0.000	18	29.264	31.921	0.367	1.466	(50 ksi) A572-65 (65 ksi)
L6	60.500-40.457	20.043	5.083	18	31.921	36.180	0.436	1.743	A572-65 (65 ksi)
L7	40.457-30.500	15.040	0.000	18	34.600	37.787	0.485	1.939	A572-65 (65 ksi)
L8	30.500-0.000	30.500		18	37.787	44.250	0.456	1.825	A572-65 (65 ksi)

Tapered Pole Properties											
Section	Tip Dia.	Area in <sup>2</sup>	$I$ $in^4$	r in	C in	I/C in³	$J$ $in^4$	It/Q in <sup>2</sup>	w in	w/t	
L1	18.785	10.898	461.730	6.501	9.398	49.131	924.069	5.450	2.926	15.605	
	25.454	14.807	1157.924	8.832	12.734	90.931	2317.371	7.405	4.082	21.77	
L2	25.454	34.331	2630.769	8.743	12.734	206.592	5265.000	17.169	3.639	8.285	
	27.508	37.151	3333.705	9.461	13.762	242.245	6671.796	18.579	3.995	9.096	
L3	27.117	38.276	3119.391	9.016	13.143	237.334	6242.888	19.142	3.718	7.83	
	29.608	43.226	4493.011	10.182	14.812	303.332	8991.935	21.617	4.296	9.048	
L4	29.608	55.204	5684.267	10.135	14.812	383.756	11376.014	27.607	4.060	6.664	
	29.716	55.409	5747.975	10.172	14.866	386.648	11503.512	27.710	4.078	6.694	
L5	29.716	33.616	3546.548	10.259	14.866	238.565	7097.762	16.811	4.505	12.293	
	32.413	36.706	4617.275	11.202	16.216	284.742	9240.626	18.357	4.973	13.569	
L6	32.413	43.541	5453.050	11.177	16.216	336.283	10913.275	21.775	4.851	11.134	
	36.738	49.432	7979.124	12.689	18.379	434.133	15968.747	24.720	5.601	12.855	
L7	36.227	52.486	7717.450	12.111	17.577	439.073	15445.054	26.248	5.236	10.803	
	38.370	57.389	10088.726	13.242	19.196	525.572	20190.728	28.700	5.797	11.96	
L8	38.370	54.065	9518.850	13.252	19.196	495.884	19050.225	27.038	5.847	12.815	
	44.933	63.426	15368.309	15.547	22.479	683.674	30756.839	31.719	6.985	15.308	

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade Adjust. Factor $A_f$	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	$ft^2$	in				in	in
L1			1	1	1		
125.000-94.45							
0							
L2			1	1	0.93228		
94.450-85.040							
L3			1	1	0.947822		
85.040-73.500							
L4			1	1	0.93408		
73.500-73.000							
L5			1	1	0.976519		
73.000-60.500					0.050000		
L6			1	1	0.959022		
60.500-40.457					0.066027		
L7			1	1	0.966927		
40.457-30.500					0.0761		
L8			1	1	0.9761		
30.500-0.000							

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Client	Designed by
Crown Castle	JTillson

## Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or S	Allow Shield	Component Type	Placement	Total Number	Number Per Row	Clear Spacing	Width or Diameter	Perimeter	Weight
	Leg			ft			in	in	in	klf
*\$\$\$*										

## Feed Line/Linear Appurtenances - Entered As Area

Description	Face	Allow	Component	Placement	Total		$C_A A_A$	Weight
	or	Shield	Type	C	Number		0210	110
EGIA 50D (1/01)	Leg	3.7	C 1 (0 + 05	ft		N. Y.	ft²/ft	klf
FSJ4-50B(1/2")	В	No	CaAa (Out Of	125.000 - 0.000	1	No Ice	0.052	0.000
(E-Outside)			Face)			1/2" Ice	0.152	0.001
						1" Ice	0.252	0.002
						2" Ice	0.452	0.006
	_					4" Ice	0.852	0.022
FSJ4-50B(1/2")	В	No	CaAa (Out Of	125.000 - 0.000	1	No Ice	0.000	0.000
(E-Outside Shielded)			Face)			1/2" Ice	0.000	0.001
						1" Ice	0.000	0.002
						2" Ice	0.000	0.006
						4" Ice	0.000	0.022
ATCB-B01-005( 5/16)	В	No	CaAa (Out Of	125.000 - 0.000	3	No Ice	0.000	0.000
(E-Outside Shielded)			Face)			1/2" Ice	0.000	0.001
						1" Ice	0.000	0.002
						2" Ice	0.000	0.006
						4" Ice	0.000	0.021
LDF1-50A(1/4")	В	No	CaAa (Out Of	125.000 - 0.000	3	No Ice	0.000	0.000
(E-Outside Shielded)			Face)			1/2" Ice	0.000	0.001
`			,			1" Ice	0.000	0.002
						2" Ice	0.000	0.006
						4" Ice	0.000	0.021
HB114-21U3M12-XXX	В	No	Inside Pole	125.000 - 0.000	4	No Ice	0.000	0.001
F(1-1/4")						1/2" Ice	0.000	0.001
(R(1)+E(3))						1" Ice	0.000	0.001
(11(1) 2(3))						2" Ice	0.000	0.001
						4" Ice	0.000	0.001
*\$\$\$*								*****
LDF6-50A(1-1/4")	Α	No	CaAa (Out Of	115.000 - 0.000	2	No Ice	0.155	0.001
(E-outside)			Face)		_	1/2" Ice	0.255	0.002
(L'outside)			1 400)			1" Ice	0.355	0.004
						2" Ice	0.555	0.009
						4" Ice	0.955	0.028
LDF6-50A(1-1/4")	Α	No	CaAa (Out Of	115.000 - 0.000	10	No Ice	0.000	0.001
(E-outside Shielded)	11	110	Face)	113.000 - 0.000	10	1/2" Ice	0.000	0.002
(E outside Sincided)			i acc)			1" Ice	0.000	0.002
						2" Ice	0.000	0.009
						4" Ice	0.000	0.009
2" Rigid Conduit	Α	No	CaAa (Out Of	115.000 - 0.000	1	No Ice	0.000	0.028
(E-outside Shielded)	A	INO	Face)	113.000 - 0.000	1	1/2" Ice	0.000	0.003
(E-outside Silielded)			race)			1" Ice	0.000	0.004
						2" Ice		0.008
							0.000	
ED 1 00D 000 75000/	A	NJ -	Cala (Out Of	115 000 0 000	1	4" Ice	0.000	0.032
FB-L98B-002-75000(	Α	No	CaAa (Out Of	115.000 - 0.000	1	No Ice	0.000	0.000
3/8")			Face)			1/2" Ice	0.000	0.001
(E-inside conduit)						1" Ice	0.000	0.002
						2" Ice	0.000	0.006
WID ALGO COTT DDT ( 2 ) !			0 1 (0 : 6 2	115000 0000	•	4" Ice	0.000	0.022
WR-VG86ST-BRD(3/4)	Α	No	CaAa (Out Of	115.000 - 0.000	2	No Ice	0.000	0.001

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Crown Castle	Designed by JTillson

Description	Face or	Allow Shield	Component Type	Placement	Total Number		$C_A A_A$	Weight
	Leg			ft			ft²/ft	klf
(E-inside conduit)			Face)			1/2" Ice	0.000	0.001
						1" Ice	0.000	0.003
						2" Ice	0.000	0.007
<del>ተ</del> ወወቅት						4" Ice	0.000	0.024
*\$\$\$* LDF7-50A(1-5/8")	С	No	Inside Pole	105.000 - 0.000	12	No Ice	0.000	0.001
, , ,	C	NO	mside i die	103.000 - 0.000	12	1/2" Ice	0.000	0.001
(E)						1/2 Ice 1" Ice	0.000	
						2" Ice		0.001
							0.000	0.001
HD150 1 00H0 00H0/	0		r :1 D 1	105 000 0 000		4" Ice	0.000	0.001
HB158-1-08U8-S8J18(	C	No	Inside Pole	105.000 - 0.000	1	No Ice	0.000	0.001
1-5/8)						1/2" Ice	0.000	0.001
(R)						1" Ice	0.000	0.001
						2" Ice	0.000	0.001
1.000						4" Ice	0.000	0.001
*\$\$\$* *\$\$\$*								
AVA7-50(1-5/8)	В	No	Inside Pole	85.000 - 0.000	6	No Ice	0.000	0.001
(E)	Ь	110	morae i ore	03.000 0.000	Ü	1/2" Ice	0.000	0.001
(L)						1" Ice	0.000	0.001
						2" Ice	0.000	0.001
						4" Ice	0.000	0.001
1.2 Mtlin- Ft	D	NI-	I: J. D.1.	85.000 - 0.000	1			
1.2 Masterline Extreme	В	No	Inside Pole	85.000 - 0.000	1	No Ice	0.000	0.001
Hybrid(1 3/16")						1/2" Ice	0.000	0.001
(P)						1" Ice	0.000	0.001
						2" Ice	0.000	0.001
*\$\$\$*						4" Ice	0.000	0.001
Safety Line 3/8	С	No	CaAa (Out Of	125.000 - 0.000	1	No Ice	0.037	0.000
(E)	C	110	Face)	123.000 - 0.000		1/2" Ice	0.137	0.001
(L)			i dec)			1" Ice	0.238	0.001
						2" Ice	0.437	0.001
						4" Ice	0.838	0.002
*\$\$\$*						1 100	0.050	0.001
Sabre MS-600 (6" x 1"	C	No	CaAa (Out Of	96.000 - 71.000	1	No Ice	0.167	0.000
Plate)			Face)			1/2" Ice	0.250	0.000
(P)			,			1" Ice	0.333	0.000
						2" Ice	0.500	0.000
						4" Ice	0.833	0.000
*\$\$\$*								
*\$\$\$*	C		0 1 (0 105	60.500 0.000		NY Y	0.424	0.020
MP3-06 (Surface Af)	C	No	CaAa (Out Of	60.500 - 0.000	1	No Ice	0.434	0.029
(E)			Face)			1/2" Ice	0.518	0.031
						1" Ice	0.601	0.034
						2" Ice	0.768	0.040
						4" Ice	1.101	0.057
MP3-06 (Surface Af)	В	No	CaAa (Out Of	60.500 - 0.000	1	No Ice	0.434	0.029
(E)			Face)			1/2" Ice	0.518	0.031
						1" Ice	0.601	0.034
						2" Ice	0.768	0.040
						4" Ice	1.101	0.057
*\$\$\$*	~	3.7	0 4 (0 : 00	75.500 (0.500	4	NT T	0.260	0.014
MP3-04 (Surface Af)	С	No	CaAa (Out Of	75.500 - 60.500	1	No Ice	0.268	0.014
(E)			Face)			1/2" Ice	0.352	0.016
						1" Ice	0.435	0.018
						2" Ice	0.602	0.022
						4" Ice	0.935	0.036
MP3-04 (Surface Af)	C	No	CaAa (Out Of	75.500 - 60.500	1	No Ice	0.268	0.014
(E)			Face)			1/2" Ice	0.352	0.016
						1" Ice	0.435	0.018
						2" Ice	0.602	0.022

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Tulsa, OK - 74119

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Description	Face	Allow	Component	Placement	Total		$C_A A_A$	Weight
	or	Shield	Type		Number			
	Leg			ft			ft²/ft	klf
						4" Ice	0.935	0.036
*\$\$\$*								

## Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	$A_R$	$A_F$	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation				In Face	Out Face	
	ft		$ft^2$	$ft^2$	ft <sup>2</sup>	$ft^2$	K
L1	125.000-94.450	A	0.000	0.000	0.000	6.371	0.246
		В	0.000	0.000	0.000	1.589	0.170
		C	0.000	0.000	0.000	1.404	0.124
L2	94.450-85.040	A	0.000	0.000	0.000	2.917	0.113
		В	0.000	0.000	0.000	0.489	0.052
		C	0.000	0.000	0.000	1.921	0.107
L3	85.040-73.500	Α	0.000	0.000	0.000	3.577	0.138
		В	0.000	0.000	0.000	0.600	0.123
		C	0.000	0.000	0.000	3.429	0.187
L4	73.500-73.000	A	0.000	0.000	0.000	0.155	0.006
		В	0.000	0.000	0.000	0.026	0.005
		C	0.000	0.000	0.000	0.370	0.020
L5	73.000-60.500	Α	0.000	0.000	0.000	3.875	0.149
		В	0.000	0.000	0.000	0.650	0.134
		C	0.000	0.000	0.000	7.510	0.494
L6	60.500-40.457	A	0.000	0.000	0.000	6.213	0.240
		В	0.000	0.000	0.000	9.748	0.792
		C	0.000	0.000	0.000	9.457	0.805
L7	40.457-30.500	Α	0.000	0.000	0.000	3.087	0.119
		В	0.000	0.000	0.000	4.842	0.394
		C	0.000	0.000	0.000	4.698	0.400
L8	30.500-0.000	A	0.000	0.000	0.000	9.455	0.365
		В	0.000	0.000	0.000	14.833	1.206
		C	0.000	0.000	0.000	14.391	1.225

## Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	$A_R$	$A_F$	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation	or	Thickness			In Face	Out Face	
	ft	Leg	in	$ft^2$	$ft^2$	$ft^2$	$ft^2$	K
L1	125.000-94.450	A	0.866	0.000	0.000	0.000	13.487	1.056
		В		0.000	0.000	0.000	6.878	0.506
		C		0.000	0.000	0.000	6.917	0.152
L2	94.450-85.040	A	0.846	0.000	0.000	0.000	6.100	0.473
		В		0.000	0.000	0.000	2.081	0.152
		C		0.000	0.000	0.000	4.839	0.115
L3	85.040-73.500	A	0.833	0.000	0.000	0.000	7.481	0.580
		В		0.000	0.000	0.000	2.552	0.246
		C		0.000	0.000	0.000	7.571	0.209
L4	73.500-73.000	A	0.825	0.000	0.000	0.000	0.320	0.025
		В		0.000	0.000	0.000	0.109	0.010
		C		0.000	0.000	0.000	0.659	0.023
L5	73.000-60.500	A	0.816	0.000	0.000	0.000	7.955	0.607
		В		0.000	0.000	0.000	2.690	0.260
		C		0.000	0.000	0.000	13.223	0.575
L6	60.500-40.457	A	0.789	0.000	0.000	0.000	12.538	0.943
		В		0.000	0.000	0.000	15.545	1.063

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Crown Castle	JTillson

Tower Section	Tower Elevation	Face	Ice Thickness	$A_R$	$A_F$	$C_AA_A$ In Face	C <sub>A</sub> A <sub>A</sub> Out Face	Weight
section	Elevation ft	or Leg	in in	$ft^2$	$ft^2$	in race ft <sup>2</sup>	oui race ft²	K
	J	C		0.000	0.000	0.000	15.254	0.900
L7	40.457-30.500	A	0.756	0.000	0.000	0.000	6.229	0.468
		В		0.000	0.000	0.000	7.722	0.528
		C		0.000	0.000	0.000	7.578	0.447
L8	30.500-0.000	A	0.750	0.000	0.000	0.000	18.605	1.367
		В		0.000	0.000	0.000	23.221	1.589
		C		0.000	0.000	0.000	22.778	1.362

## **Feed Line Center of Pressure**

Section	Elevation	$CP_X$	$CP_Z$	$CP_X$	$CP_Z$
				Ice	Ice
	ft	in	in	in	in
L1	125.000-94.450	0.005	-0.217	-0.003	-0.226
L2	94.450-85.040	-0.157	-0.216	-0.224	-0.247
L3	85.040-73.500	-0.252	-0.156	-0.333	-0.181
L4	73.500-73.000	-0.616	0.089	-0.734	0.098
L5	73.000-60.500	-0.517	0.017	-0.606	-0.001
L6	60.500-40.457	0.013	0.175	0.010	0.118
L7	40.457-30.500	0.013	0.179	0.011	0.122
L8	30.500-0.000	0.014	0.185	0.011	0.130

## **Discrete Tower Loads**

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ft ft ft	٥	ft		ft <sup>2</sup>	ft <sup>2</sup>	K
APXVTM14-C-120 w/ Mount Pipe (R)	A	From Leg	4.000 0.000 2.000	0.000	125.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	7.134 7.662 8.183 9.256 11.526	4.959 5.754 6.472 8.010 11.412	0.077 0.131 0.193 0.338 0.752
APXVTM14-C-120 w/ Mount Pipe (R)	В	From Leg	4.000 0.000 2.000	0.000	125.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	7.134 7.662 8.183 9.256 11.526	4.959 5.754 6.472 8.010 11.412	0.077 0.131 0.193 0.338 0.752
APXVTM14-C-120 w/ Mount Pipe (R)	С	From Leg	4.000 0.000 2.000	0.000	125.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	7.134 7.662 8.183 9.256 11.526	4.959 5.754 6.472 8.010 11.412	0.077 0.131 0.193 0.338 0.752
TD-RRH8x20-25 (R)	A	From Leg	4.000 0.000 2.000	0.000	125.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	4.720 5.014 5.316 5.948 7.314	1.703 1.920 2.145 2.622 3.680	0.070 0.097 0.128 0.201 0.397
TD-RRH8x20-25	В	From Leg	4.000	0.000	125.000	No Ice	4.720	1.703	0.070

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Project	,	Date 11:50:22 08/21/14				
Client	Crown Castle	Designed by JTillson				

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		$C_AA_A$ Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	0	ft		ft <sup>2</sup>	ft <sup>2</sup>	K
(R)			0.000			1/2" Ice	5.014	1.920	0.097
(11)			2.000			1" Ice	5.316	2.145	0.128
						2" Ice	5.948	2.622	0.201
						4" Ice	7.314	3.680	0.397
TD-RRH8x20-25	C	From Leg	4.000	0.000	125.000	No Ice	4.720	1.703	0.070
(R)		_	0.000			1/2" Ice	5.014	1.920	0.097
			2.000			1" Ice	5.316	2.145	0.128
						2" Ice	5.948	2.622	0.201
						4" Ice	7.314	3.680	0.397
LLPX310R-V1 w/ Mount	A	From Leg	4.000	0.000	125.000	No Ice	5.065	2.983	0.045
Pipe			0.000			1/2" Ice	5.480	3.526	0.083
(E)			4.000			1" Ice	5.905	4.086	0.126
						2" Ice	6.788	5.313	0.232
	_					4" Ice	8.704	8.131	0.544
LLPX310R-V1 w/ Mount	В	From Leg	4.000	0.000	125.000	No Ice	5.065	2.983	0.045
Pipe			0.000			1/2" Ice	5.480	3.526	0.083
(E)			4.000			1" Ice	5.905	4.086	0.126
						2" Ice	6.788	5.313	0.232
** ************************************			4.000	0.000	125.000	4" Ice	8.704	8.131	0.544
LLPX310R-V1 w/ Mount	C	From Leg	4.000	0.000	125.000	No Ice	5.065	2.983	0.045
Pipe			0.000			1/2" Ice	5.480	3.526	0.083
(E)			4.000			1" Ice	5.905	4.086	0.126
						2" Ice	6.788	5.313	0.232
A DAVISED DA O C. A 20 /		г т	4.000	0.000	125 000	4" Ice	8.704	8.131	0.544
APXVSPP18-C-A20 w/	Α	From Leg	4.000	0.000	125.000	No Ice	8.498	6.946	0.083
Mount Pipe			0.000			1/2" Ice	9.149	8.127	0.151
(E)			2.000			1" Ice 2" Ice	9.767 11.031	9.021 10.844	0.227 0.406
						4" Ice	13.679	14.851	0.400
APXVSPP18-C-A20 w/	В	From Leg	4.000	0.000	125.000	No Ice	8.498	6.946	0.909
Mount Pipe	Б	110III Leg	0.000	0.000	123.000	1/2" Ice	9.149	8.127	0.083
(E)			2.000			1" Ice	9.767	9.021	0.131
(L)			2.000			2" Ice	11.031	10.844	0.406
						4" Ice	13.679	14.851	0.909
APXVSPP18-C-A20 w/	C	From Leg	4.000	0.000	125.000	No Ice	8.498	6.946	0.083
Mount Pipe		110111 208	0.000	0.000	120.000	1/2" Ice	9.149	8.127	0.151
(E)			2.000			1" Ice	9.767	9.021	0.227
(-)			_,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			2" Ice	11.031	10.844	0.406
						4" Ice	13.679	14.851	0.909
WIMAX DAP HEAD	Α	From Leg	4.000	0.000	125.000	No Ice	1.804	0.778	0.033
(E)			0.000			1/2" Ice	1.988	0.918	0.045
,			0.000			1" Ice	2.180	1.067	0.058
						2" Ice	2.589	1.391	0.094
						4" Ice	3.512	2.143	0.201
WIMAX DAP HEAD	В	From Leg	4.000	0.000	125.000	No Ice	1.804	0.778	0.033
(E)		_	0.000			1/2" Ice	1.988	0.918	0.045
			0.000			1" Ice	2.180	1.067	0.058
						2" Ice	2.589	1.391	0.094
						4" Ice	3.512	2.143	0.201
WIMAX DAP HEAD	C	From Leg	4.000	0.000	125.000	No Ice	1.804	0.778	0.033
(E)			0.000			1/2" Ice	1.988	0.918	0.045
			0.000			1" Ice	2.180	1.067	0.058
						2" Ice	2.589	1.391	0.094
						4" Ice	3.512	2.143	0.201
HORIZON COMPACT	В	From Leg	4.000	0.000	125.000	No Ice	0.841	0.429	0.012
(E)			0.000			1/2" Ice	0.966	0.525	0.018
			0.000			1" Ice	1.099	0.629	0.026

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Project	Date
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Crown Castle	JTillson

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	$C_A A_A$ Side	Weight
			Vert ft ft ft	o	ft		ft <sup>2</sup>	ft²	K
			,			2" Ice	1.392	0.863	0.048
						4" Ice	2.082	1.435	0.122
HORIZON COMPACT	C	From Leg	4.000	0.000	125.000	No Ice	0.841	0.429	0.012
(E)			0.000			1/2" Ice	0.966	0.525	0.018
			0.000			1" Ice	1.099	0.629	0.026
						2" Ice	1.392	0.863	0.048
(a) (1 all ) (			4.000	0.000	127.000	4" Ice	2.082	1.435	0.122
(2) 6' x 2" Mount Pipe	Α	From Leg	4.000	0.000	125.000	No Ice	1.425	1.425	0.022
(E)			0.000			1/2" Ice	1.925	1.925	0.033
			0.000			1" Ice	2.294	2.294	0.048
						2" Ice	3.060	3.060	0.090
(2) 61 yr 2" Mount Ding	В	Erom Log	4.000	0.000	125.000	4" Ice No Ice	4.702 1.425	4.702 1.425	0.231 0.022
(2) 6' x 2" Mount Pipe	Ь	From Leg	0.000	0.000	123.000	1/2" Ice	1.423	1.423	0.022
(E)			0.000			1" Ice	2.294	2.294	0.033
			0.000			2" Ice	3.060	3.060	0.048
						4" Ice	4.702	4.702	0.090
(2) 6' x 2" Mount Pipe	C	From Leg	4.000	0.000	125.000	No Ice	1.425	1.425	0.022
(E)	C	110III Leg	0.000	0.000	123.000	1/2" Ice	1.925	1.925	0.022
(E)			0.000			1" Ice	2.294	2.294	0.033
			0.000			2" Ice	3.060	3.060	0.048
						4" Ice	4.702	4.702	0.030
Platform Mount [LP 712-1]	C	None		0.000	125.000	No Ice	24.530	24.530	1.335
(E)	C	Ttolle		0.000	125.000	1/2" Ice	29.940	29.940	1.646
(2)						1" Ice	35.350	35.350	1.956
						2" Ice	46.170	46.170	2.577
						4" Ice	67.810	67.810	3.820
*\$\$\$*									
PCS 1900MHz	Α	From Leg	1.000	0.000	123.000	No Ice	2.905	3.218	0.071
4x45W-65MHz w / Mount			0.000			1/2" Ice	3.206	3.647	0.101
Pipe			0.000			1" Ice	3.519	4.094	0.138
(E)						2" Ice	4.187	5.064	0.225
						4" Ice	5.703	7.343	0.480
PCS 1900MHz	В	From Leg	1.000	0.000	123.000	No Ice	2.905	3.218	0.071
4x45W-65MHz w / Mount			0.000			1/2" Ice	3.206	3.647	0.101
Pipe			0.000			1" Ice	3.519	4.094	0.138
(E)						2" Ice	4.187	5.064	0.225
DCG 1000MI	0	г т	1.000	0.000	122 000	4" Ice	5.703	7.343	0.480
PCS 1900MHz	C	From Leg	1.000	0.000	123.000	No Ice	2.905	3.218	0.071
4x45W-65MHz w / Mount			$0.000 \\ 0.000$			1/2" Ice 1" Ice	3.206 3.519	3.647 4.094	0.101 0.138
Pipe			0.000			2" Ice			0.138
(E)						4" Ice	4.187 5.703	5.064 7.343	0.223
800MHZ 2X50W RRH	Α	From Leg	1.000	0.000	123.000	No Ice	2.586	2.731	0.480
W/FILTER w / Mount Pipe	А	rioiii Leg	0.000	0.000	123.000	1/2" Ice	2.861	3.102	0.073
(E)			0.000			1" Ice	3.149	3.490	0.101
(E)			0.000			2" Ice	3.780	4.371	0.133
						4" Ice	5.207	6.396	0.453
800MHZ 2X50W RRH	В	From Leg	1.000	0.000	123.000	No Ice	2.586	2.731	0.433
W/FILTER w / Mount Pipe	_	205	0.000	0.000	125.000	1/2" Ice	2.861	3.102	0.101
(E)			0.000			1" Ice	3.149	3.490	0.135
(-)			0.500			2" Ice	3.780	4.371	0.216
						4" Ice	5.207	6.396	0.453
800MHZ 2X50W RRH	C	From Leg	1.000	0.000	123.000	No Ice	2.586	2.731	0.073
W/FILTER w / Mount Pipe	-	- 3	0.000			1/2" Ice	2.861	3.102	0.101
(E)			0.000			1" Ice	3.149	3.490	0.135

B+T Group

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84470.010.01 - Cromwell/First Line Emergencl, CT (BU# 876364)	9 of 20
Project	Date
	11:50:22 08/21/14
Client Crown Castle	Designed by JTillson

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		$C_AA_A$ Front	$C_AA_A$ Side	Weight
			ft ft ft	o	ft		ft <sup>2</sup>	ft <sup>2</sup>	K
	~					4" Ice	5.207	6.396	0.453
Side Arm Mount [SO 102-3]	C	None		0.000	123.000	No Ice	3.000	3.000	0.081
(E)						1/2" Ice	3.480	3.480	0.111
						1" Ice	3.960	3.960	0.141
						2" Ice 4" Ice	4.920 6.840	4.920 6.840	0.201 0.321
*\$\$\$*						7 100	0.040	0.040	0.521
(3)	Α	From Leg	4.000	0.000	115.000	No Ice	8.498	6.304	0.074
AM-X-CD-16-65-00T-RET		_	0.000			1/2" Ice	9.149	7.479	0.139
w/ Mount Pipe			2.000			1" Ice	9.767	8.368	0.212
(E)						2" Ice	11.031	10.179	0.385
						4" Ice	13.679	14.024	0.874
(3)	В	From Leg	4.000	0.000	115.000	No Ice	8.498	6.304	0.074
AM-X-CD-16-65-00T-RET			0.000			1/2" Ice	9.149	7.479	0.139
w/ Mount Pipe			2.000			1" Ice	9.767	8.368	0.212
(E)						2" Ice	11.031	10.179	0.385
						4" Ice	13.679	14.024	0.874
(3)	C	From Leg	4.000	0.000	115.000	No Ice	8.498	6.304	0.074
AM-X-CD-16-65-00T-RET			0.000			1/2" Ice	9.149	7.479	0.139
w/ Mount Pipe			2.000			1" Ice	9.767	8.368	0.212
(E)						2" Ice	11.031	10.179	0.385
						4" Ice	13.679	14.024	0.874
(2) DTMABP7819VG12A	Α	From Leg	4.000	0.000	115.000	No Ice	1.139	0.391	0.019
(E)			0.000			1/2" Ice	1.284	0.488	0.026
			2.000			1" Ice	1.437	0.595	0.036
						2" Ice	1.769	0.833	0.060
(2) DTMA DD7910VC12A	В	From Log	4.000	0.000	115.000	4" Ice No Ice	2.538 1.139	1.414 0.391	0.140 0.019
(2) DTMABP7819VG12A (E)	Ь	From Leg	0.000	0.000	113.000	1/2" Ice	1.139	0.391	0.019
(E)			2.000			1" Ice	1.437	0.488	0.026
			2.000			2" Ice	1.769	0.833	0.060
						4" Ice	2.538	1.414	0.140
(2) DTMABP7819VG12A	C	From Leg	4.000	0.000	115.000	No Ice	1.139	0.391	0.019
(E)	C	1 Tom Leg	0.000	0.000	113.000	1/2" Ice	1.284	0.488	0.026
(2)			2.000			1" Ice	1.437	0.595	0.036
			2.000			2" Ice	1.769	0.833	0.060
						4" Ice	2.538	1.414	0.140
RRUS-11	Α	From Leg	4.000	0.000	115.000	No Ice	3.249	1.373	0.048
(E)			0.000			1/2" Ice	3.491	1.551	0.068
· /			2.000			1" Ice	3.741	1.738	0.092
						2" Ice	4.268	2.138	0.150
						4" Ice	5.426	3.042	0.310
RRUS-11	В	From Leg	4.000	0.000	115.000	No Ice	3.249	1.373	0.048
(E)		_	0.000			1/2" Ice	3.491	1.551	0.068
			2.000			1" Ice	3.741	1.738	0.092
						2" Ice	4.268	2.138	0.150
						4" Ice	5.426	3.042	0.310
RRUS-11	C	From Leg	4.000	0.000	115.000	No Ice	3.249	1.373	0.048
(E)			0.000			1/2" Ice	3.491	1.551	0.068
			2.000			1" Ice	3.741	1.738	0.092
						2" Ice	4.268	2.138	0.150
			,			4" Ice	5.426	3.042	0.310
DC6-48-60-18-8F	Α	From Leg	4.000	0.000	115.000	No Ice	1.467	1.467	0.019
(E)			0.000			1/2" Ice	1.667	1.667	0.037
			2.000			1" Ice	1.878	1.878	0.057
						2" Ice 4" Ice	2.333 3.378	2.333 3.378	0.105 0.239

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Client	Designed by
Crown Castle	JTillson

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	$C_AA_A$ Side	Weight
			Vert ft ft ft	0	ft		$ft^2$	ft <sup>2</sup>	K
Platform Mount [LP 303-1] (E)	С	None	<u> </u>	0.000	115.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	14.660 18.870 23.080 31.500 48.340	14.660 18.870 23.080 31.500 48.340	1.250 1.481 1.713 2.175 3.101
*\$\$\$* BXA-70063-6CF-EDIN-0 w/ Mount Pipe (E)	A	From Leg	4.000 0.000 0.000	0.000	105.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	7.969 8.609 9.216 10.459 13.066	5.801 6.953 7.819 9.601 13.366	0.042 0.103 0.171 0.335 0.804
BXA-70063-6CF-EDIN-0 w/ Mount Pipe (E)	В	From Leg	4.000 0.000 0.000	0.000	105.000	No Ice 1/2" Ice 1" Ice 2" Ice	7.969 8.609 9.216 10.459	5.801 6.953 7.819 9.601	0.804 0.042 0.103 0.171 0.335
BXA-70063-6CF-EDIN-0 w/ Mount Pipe (E)	С	From Leg	4.000 0.000 0.000	0.000	105.000	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice	13.066 7.969 8.609 9.216 10.459	13.366 5.801 6.953 7.819 9.601	0.804 0.042 0.103 0.171 0.335
BXA-171063-8BF-EDIN-2 w/ Mount Pipe (E)	A	From Leg	4.000 0.000 0.000	0.000	105.000	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice	13.066 3.179 3.555 3.964 4.853	13.366 3.353 3.971 4.595 5.893	0.804 0.029 0.061 0.099 0.193
BXA-171063-8BF-EDIN-2 w/ Mount Pipe (E)	В	From Leg	4.000 0.000 0.000	0.000	105.000	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice	6.767 3.179 3.555 3.964 4.853	8.885 3.353 3.971 4.595 5.893	0.488 0.029 0.061 0.099 0.193
BXA-171063-8BF-EDIN-2 w/ Mount Pipe (E)	С	From Leg	4.000 0.000 0.000	0.000	105.000	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice	6.767 3.179 3.555 3.964 4.853	8.885 3.353 3.971 4.595 5.893	0.488 0.029 0.061 0.099 0.193
(2) FD9R6004/2C-3L (E)	A	From Leg	4.000 0.000 0.000	0.000	105.000	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice	6.767 0.367 0.451 0.543 0.755	8.885 0.085 0.136 0.196 0.343	0.488 0.003 0.005 0.009 0.020
(2) FD9R6004/2C-3L (E)	В	From Leg	4.000 0.000 0.000	0.000	105.000	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice	1.281 0.367 0.451 0.543 0.755	0.740 0.085 0.136 0.196 0.343	0.063 0.003 0.005 0.009 0.020
(2) FD9R6004/2C-3L (E)	C	From Leg	4.000 0.000 0.000	0.000	105.000	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice	1.281 0.367 0.451 0.543 0.755	0.740 0.085 0.136 0.196 0.343	0.063 0.003 0.005 0.009 0.020
LNX-6514DS-VTM w/ Mount Pipe (R)	A	From Leg	4.000 0.000 0.000	0.000	105.000	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice	1.281 8.568 9.220 9.838 11.104	0.740 7.004 8.185 9.081 10.904	0.063 0.059 0.127 0.204 0.384
LNX-6514DS-VTM w/	В	From Leg	4.000	0.000	105.000	4" Ice No Ice	13.754 8.568	14.926 7.004	0.890 0.059

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Client	Designed by
Crown Castle	JTillson

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	0	ft		$ft^2$	ft <sup>2</sup>	K
Mount Pipe			0.000			1/2" Ice	9.220	8.185	0.127
(R)			0.000			1" Ice	9.838	9.081	0.204
						2" Ice	11.104	10.904	0.384
						4" Ice	13.754	14.926	0.890
LNX-6514DS-VTM w/	C	From Leg	4.000	0.000	105.000	No Ice	8.568	7.004	0.059
Mount Pipe			0.000			1/2" Ice	9.220	8.185	0.127
(R)			0.000			1" Ice	9.838	9.081	0.204
						2" Ice	11.104	10.904	0.384
						4" Ice	13.754	14.926	0.890
HBX-6517DS-VTM w/	Α	From Leg	4.000	0.000	105.000	No Ice	5.541	5.021	0.045
Mount Pipe			0.000			1/2" Ice	6.112	6.223	0.092
(R)			0.000			1" Ice	6.654	7.167	0.146
						2" Ice	7.750	9.011	0.281
	_					4" Ice	10.109	12.898	0.692
HBX-6517DS-VTM w/	В	From Leg	4.000	0.000	105.000	No Ice	5.541	5.021	0.045
Mount Pipe			0.000			1/2" Ice	6.112	6.223	0.092
(R)			0.000			1" Ice	6.654	7.167	0.146
						2" Ice	7.750	9.011	0.281
HDW (517DC NEW /		ъ т	4.000	0.000	105.000	4" Ice	10.109	12.898	0.692
HBX-6517DS-VTM w/	C	From Leg	4.000	0.000	105.000	No Ice	5.541	5.021	0.045
Mount Pipe			0.000			1/2" Ice	6.112	6.223	0.092
(R)			0.000			1" Ice	6.654	7.167	0.146
						2" Ice	7.750 10.109	9.011	0.281
RRH2X40-AWS	Α	Erom Log	4.000	0.000	105.000	4" Ice No Ice	2.522	12.898 1.589	0.692 0.044
(R)	Α	From Leg	0.000	0.000	103.000	1/2" Ice	2.753	1.795	0.044
(K)			0.000			1" Ice	2.733	2.010	0.081
			0.000			2" Ice	3.499	2.465	0.082
						4" Ice	4.615	3.479	0.132
RRH2X40-AWS	В	From Leg	4.000	0.000	105.000	No Ice	2.522	1.589	0.273
(R)	Ь	1 Tom Leg	0.000	0.000	105.000	1/2" Ice	2.753	1.795	0.061
(11)			0.000			1" Ice	2.993	2.010	0.082
			0.000			2" Ice	3.499	2.465	0.132
						4" Ice	4.615	3.479	0.275
RRH2X40-AWS	С	From Leg	4.000	0.000	105.000	No Ice	2.522	1.589	0.044
(R)			0.000			1/2" Ice	2.753	1.795	0.061
()			0.000			1" Ice	2.993	2.010	0.082
						2" Ice	3.499	2.465	0.132
						4" Ice	4.615	3.479	0.275
DB-T1-6Z-8AB-0Z	Α	From Leg	4.000	0.000	105.000	No Ice	5.600	2.333	0.044
(R)		C	0.000			1/2" Ice	5.915	2.558	0.080
			0.000			1" Ice	6.240	2.791	0.120
						2" Ice	6.914	3.284	0.213
						4" Ice	8.365	4.373	0.455
Platform Mount [LP 712-1]	C	None		0.000	105.000	No Ice	24.530	24.530	1.335
(E)						1/2" Ice	29.940	29.940	1.646
						1" Ice	35.350	35.350	1.956
						2" Ice	46.170	46.170	2.577
*\$\$\$*						4" Ice	67.810	67.810	3.820
*\$\$\$* EDICCON AID 21 D2A		Enoug I	4.000	0.000	95 000	No I	6 025	5 6 40	0.112
ERICSSON AIR 21 B2A	A	From Leg	4.000	0.000	85.000	No Ice	6.825	5.642	0.112
B4P w/ Mount Pipe			0.000			1/2" Ice 1" Ice	7.347	6.480	0.169
(P)			0.000				7.863	7.257	0.233
						2" Ice 4" Ice	8.926	8.864 12.293	0.383 0.807
ERICSSON AIR 21 B2A	В	From Leg	4.000	0.000	85.000	No Ice	11.175 6.825	5.642	0.807
ENICSSON AIR 21 D2A	D	riom Leg	4.000	0.000	05.000	INO ICE	0.623	3.042	0.112

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Client	Designed by
Crown Castle	JTillson

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		$C_A A_A$ Front	$C_AA_A$ Side	Weigh
			Vert ft ft	0	ft		ft <sup>2</sup>	ft²	K
			ft						
B4P w/ Mount Pipe			0.000			1/2" Ice	7.347	6.480	0.169
(P)			0.000			1" Ice	7.863	7.257	0.233
						2" Ice	8.926	8.864	0.383
	~					4" Ice	11.175	12.293	0.807
ERICSSON AIR 21 B2A	C	From Leg	4.000	0.000	85.000	No Ice	6.825	5.642	0.112
B4P w/ Mount Pipe			0.000			1/2" Ice	7.347	6.480	0.169
(P)			0.000			1" Ice	7.863	7.257	0.233
						2" Ice	8.926	8.864	0.383
						4" Ice	11.175	12.293	0.807
ERICSSON AIR 21 B4A	Α	From Leg	4.000	0.000	85.000	No Ice	6.825	5.642	0.112
B2P w/ Mount Pipe			0.000			1/2" Ice	7.347	6.480	0.169
(P)			0.000			1" Ice	7.863	7.257	0.233
						2" Ice	8.926	8.864	0.383
						4" Ice	11.175	12.293	0.807
ERICSSON AIR 21 B4A	В	From Leg	4.000	0.000	85.000	No Ice	6.825	5.642	0.112
B2P w/ Mount Pipe			0.000			1/2" Ice	7.347	6.480	0.169
(P)			0.000			1" Ice	7.863	7.257	0.233
						2" Ice	8.926	8.864	0.383
						4" Ice	11.175	12.293	0.80
ERICSSON AIR 21 B4A	C	From Leg	4.000	0.000	85.000	No Ice	6.825	5.642	0.112
B2P w/ Mount Pipe			0.000			1/2" Ice	7.347	6.480	0.169
(P)			0.000			1" Ice	7.863	7.257	0.233
						2" Ice	8.926	8.864	0.383
						4" Ice	11.175	12.293	0.807
T-Arm Mount [TA 601-3]	C	None		0.000	85.000	No Ice	10.900	10.900	$0.72\epsilon$
(P)						1/2" Ice	14.650	14.650	0.926
						1" Ice	18.400	18.400	1.125
						2" Ice	25.900	25.900	1.524
*\$\$\$*						4" Ice	40.900	40.900	2.322

Description	Face	Dish	Offset	Offsets:	Azimuth	3 dB	Elevation	Outside		Aperture	Weight
	or Leg	Туре	Type	Horz Lateral	Adjustment	Beam Width		Diameter		Area	
	Leg			Vert		wiam					
				ft	0	0	ft	ft		$ft^2$	K
VHLP2-18	В	Paraboloid	From	4.000	62.000		125.000	2.175	No Ice	3.720	0.031
(E)		w/Shroud (HP)	Leg	0.000					1/2" Ice	4.010	0.050
				-1.000					1" Ice	4.300	0.070
									2" Ice	4.880	0.110
									4" Ice	6.040	0.200
VHLP2-11	C	Paraboloid	From	4.000	90.000		125.000	2.175	No Ice	3.720	0.027
(E)		w/Shroud (HP)	Leg	0.000					1/2" Ice	4.010	0.050
				-1.000					1" Ice	4.300	0.070
									2" Ice	4.880	0.110
									4" Ice	6.040	0.190
*\$\$\$*											

B+T Group

1717 South Boulder Ave, Suite 300 Tulsa, OK - 74119 Phone: (918) 587-4630 FAX: (918) 295-0265

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Client	Designed by
Crown Castle	JTillson

#### **Load Combinations**

Comb. No.	Description	
1	Dead Only	
2	Dead+Wind 0 deg - No Ice	
3	Dead+Wind 30 deg - No Ice	
4	Dead+Wind 60 deg - No Ice	
5	Dead+Wind 90 deg - No Ice	
6	Dead+Wind 120 deg - No Ice	
7	Dead+Wind 150 deg - No Ice	
8	Dead+Wind 180 deg - No Ice	
9	Dead+Wind 210 deg - No Ice	
10	Dead+Wind 240 deg - No Ice	
11	Dead+Wind 270 deg - No Ice	
12	Dead+Wind 300 deg - No Ice	
13	Dead+Wind 330 deg - No Ice	
14	Dead+Ice+Temp	
15	Dead+Wind 0 deg+Ice+Temp	
16	Dead+Wind 30 deg+Ice+Temp	
17	Dead+Wind 60 deg+Ice+Temp	
18	Dead+Wind 90 deg+Ice+Temp	
19	Dead+Wind 120 deg+Ice+Temp	
20	Dead+Wind 150 deg+Ice+Temp	
21	Dead+Wind 180 deg+Ice+Temp	
22	Dead+Wind 210 deg+Ice+Temp	
23	Dead+Wind 240 deg+Ice+Temp	
24	Dead+Wind 270 deg+Ice+Temp	
25	Dead+Wind 300 deg+Ice+Temp	
26	Dead+Wind 330 deg+Ice+Temp	
27	Dead+Wind 0 deg - Service	
28	Dead+Wind 30 deg - Service	
29	Dead+Wind 60 deg - Service	
30	Dead+Wind 90 deg - Service	
31 32	Dead+Wind 120 deg - Service	
32	Dead+Wind 150 deg - Service	
33 34	Dead+Wind 180 deg - Service Dead+Wind 210 deg - Service	
34 35	Dead+Wind 240 deg - Service Dead+Wind 240 deg - Service	
35 36	Dead+Wind 270 deg - Service Dead+Wind 270 deg - Service	
37	Dead+Wind 300 deg - Service	
38	Dead+Wind 330 deg - Service	

#### **Maximum Member Forces**

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	125 - 94.45	Pole	Max Tension Max. Compression	1 14	0.000 -16.869	0.000	0.000 1.272
			Max. Mx	5	-7.702	-369.492	-3.225
			Max. My	2	-7.661	1.548	374.765
			Max. Vy	5	17.259	-369.492	-3.225
			Max. Vx	2	-17.522	1.548	374.765
			Max. Torque	5			-1.098
L2	94.45 - 85.04	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	14	-18.069	-0.321	1.552
			Max. Mx	5	-8.519	-465.541	-3.828
			Max. My	2	-8.480	1.839	472.320

**B+T Group** 1717 South Boulder Ave, Suite 300 Tulsa, OK - 74119 Phone: (918) 587-4630 FAX: (918) 295-0265

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Client Crown Castle	Designed by JTillson

Section	Elevation	Component	Condition	Gov.	Force	Major Axis	Minor Axis
No.	ft	Туре		Load		Moment	Moment
				Comb.	K	kip-ft	kip-ft
			Max. Vy	5	17.726	-465.541	-3.828
			Max. Vx	2	-17.989	1.839	472.320
			Max. Torque	12			-0.402
L3	85.04 - 73.5	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	14	-24.801	-0.412	2.344
			Max. Mx	5	-12.925	-774.610	-5.562
			Max. My	2	-12.889	2.723	785.695
			Max. Vy	5	21.238	-774.610	-5.562
			Max. Vx	2	-21.504	2.723	785.695
			Max. Torque	9			-0.414
L4	73.5 - 73	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	14	-24.962	-0.400	2.362
			Max. Mx	5	-13.053	-785.226	-5.626
			Max. My	2	-13.018	2.765	796.456
			Max. Vy	5	21.291	-785.226	-5.626
			Max. Vx	2	-21.557	2.765	796.456
			Max. Torque	9			-0.416
L5	73 - 60.5	Pole	Max Tension	1	0.000	0.000	0.000
20	75 00.5	1010	Max. Compression	14	-28.260	-0.072	2.811
			Max. Mx	11	-15.511	1058.632	3.487
			Max. My	2	-15.482	3.819	1073.171
			Max. Vy	5	22.480	-1058.291	-7.205
			Max. Vx	8	22.744	-3.921	-1072.092
			Max. Torque	9	22.744	-3.921	-0.461
L6	60.5 - 40.457	Pole	Max Tension	1	0.000	0.000	0.000
LU	00.3 - 40.437	roic	Max. Compression	14	-33.169	-0.241	2.995
			Max. Mx	14			
					-19.375	1406.005	3.813
			Max. My	2	-19.353	4.613	1424.249
			Max. Vy	5	24.006	-1405.798	-9.402
			Max. Vx	8	24.270	-4.950	-1423.906
T 7	40 457 20 5	D 1	Max. Torque	9	0.000	0.000	-0.461
L7	40.457 - 30.5	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	14	-39.574	-0.421	3.189
			Max. Mx	11	-24.501	1779.019	4.118
			Max. My	8	-24.485	-5.981	-1801.394
			Max. Vy	5	25.540	-1778.947	-11.623
			Max. Vx	8	25.803	-5.981	-1801.394
			Max. Torque	9			-0.442
L8	30.5 - 0	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	14	-51.022	-0.806	3.556
			Max. Mx	5	-33.923	-2597.765	-16.165
			Max. My	8	-33.922	-8.041	-2628.987
			Max. Vy	5	28.206	-2597.765	-16.165
			Max. Vx	2	-28.461	6.980	2626.739
			Max. Torque	9			-0.433

### **Maximum Reactions**

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	15	51.022	0.011	7.208
	Max. H <sub>x</sub>	11	33.941	28.177	0.045
	Max. H <sub>z</sub>	2	33.941	0.051	28.439
	$Max. M_x$	2	2626.739	0.051	28.439
	Max. M <sub>z</sub>	5	2597.765	-28.184	-0.117
	Max. Torsion	11	0.225	28.177	0.045

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Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, Z
		Load	K	K	K
		Comb.			
	Min. Vert	1	33.941	0.000	0.000
	Min. H <sub>x</sub>	5	33.941	-28.184	-0.117
	Min. H <sub>z</sub>	8	33.941	-0.066	-28.439
	Min. M <sub>x</sub>	8	-2628.987	-0.066	-28.439
	Min. M <sub>z</sub>	11	-2597.557	28.177	0.045
	Min. Torsion	9	-0.404	13.981	-24.641

### **Tower Mast Reaction Summary**

Load Combination	Vertical	$Shear_x$	$Shear_z$	Overturning	Overturning	Torque
Combination	K	K	K	$Moment, M_x$ $kip-ft$	$Moment, M_z$ $kip-ft$	kip-ft
Dead Only	33.941	0.000	0.000	1.128	0.380	0.000
Dead+Wind 0 deg - No Ice	33.941	-0.051	-28.439	-2626.739	6.980	0.000
Dead+Wind 30 deg - No Ice	33.941	14.022	-24.656	-2020.739	-1289.611	-0.019
Dead+Wind 60 deg - No Ice	33.941	24.350	-14.267	-1318.930	-2242.200	0.061
Dead+Wind 90 deg - No Ice	33.941	28.184	0.117	16.165	-2597.765	0.314
Dead+Wind 120 deg - No Ice	33.941	24.412	14.310	1326.751	-2250.149	0.163
Dead+Wind 150 deg - No Ice	33.941	14.106	24.657	2280.533	-1300.415	0.103
Dead+Wind 180 deg - No Ice	33.941	0.066	28.439	2628.987	-8.041	0.134
Dead+Wind 210 deg - No Ice	33.941	-13.981	24.641	2278.493	1285.187	0.404
Dead+Wind 240 deg - No Ice	33.941	-24.377	14.195	1311.938	2246.468	-0.131
Dead+Wind 270 deg - No Ice	33.941	-28.177	-0.045	-4.597	2597.557	-0.131
Dead+Wind 300 deg - No Ice	33.941	-24.370	-14.314	-1324.956	2245.451	0.292
Dead+Wind 330 deg - No Ice	33.941	-14.093	-24.663	-2279.064	1299.579	0.234
Dead+Ice+Temp	51.022	0.000	-0.000	-3.556	-0.806	0.000
Dead+Wind 0 deg+Ice+Temp	51.022	-0.011	-7.208	-684.658	0.688	-0.007
Dead+Wind 30 deg+Ice+Temp	51.022	3.561	-6.248	-594.213	-335.977	-0.039
Dead+Wind 60 deg+Ice+Temp	51.022	6.182	-3.614	-345.550	-583.206	-0.007
Dead+Wind 90 deg+Ice+Temp	51.022	7.154	0.026	-0.204	-675.279	0.054
Dead+Wind 120 deg+Ice+Temp	51.022	6.196	3.624	339.525	-585.037	0.027
Dead+Wind 150 deg+Ice+Temp	51.022	3.580	6.248	586.941	-338.458	0.036
Dead+Wind 180 deg+Ice+Temp	51.022	0.015	7.208	677.353	-2.757	0.084
Dead+Wind 210 deg+Ice+Temp	51.022	-3.552	6.245	586.466	333.136	0.109
Dead+Wind 240 deg+Ice+Temp	51.022	-6.188	3.598	336.124	582.359	-0.009
Dead+Wind 270 deg+Ice+Temp	51.022	-7.152	-0.010	-4.965	673.406	-0.033
Dead+Wind 300 deg+Ice+Temp	51.022	-6.187	-3.625	-346.934	582.130	0.076
Dead+Wind 330 deg+Ice+Temp	51.022	-3.577	-6.250	-594.426	336.439	0.053
Dead+Wind 0 deg - Service	33.941	-0.018	-9.840	-909.472	2.679	0.007
Dead+Wind 30 deg - Service	33.941	4.852	-8.531	-788.674	-446.605	-0.032
Dead+Wind 60 deg - Service	33.941	8.426	-4.937	-456.281	-776.674	0.021
Dead+Wind 90 deg - Service	33.941	9.752	0.040	6.341	-899.874	0.110
Dead+Wind 120 deg - Service	33.941	8.447	4.952	460.472	-779.439	0.058
Dead+Wind 150 deg - Service	33.941	4.881	8.532	790.978	-450.353	0.054
Dead+Wind 180 deg - Service	33.941	0.023	9.840	911.725	-2.527	0.113
Dead+Wind 210 deg - Service	33.941	-4.838	8.526	790.261	445.589	0.141
Dead+Wind 240 deg - Service	33.941	-8.435	4.912	455.331	778.671	-0.046
Dead+Wind 270 deg - Service	33.941	-9.750	-0.015	-0.856	900.322	-0.079
Dead+Wind 300 deg - Service	33.941	-8.432	-4.953	-458.374	778.326	0.103
Dead+Wind 330 deg - Service	33.941	-4.877	-8.534	-788.995	450.583	0.083

### **Solution Summary**

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	Sur	n of Applied Force	s		Sum of Reaction	Sum of Reactions	
Load	PX	PY	PZ	PX	PY	PZ	% Erro
Comb.	K	K	K	K	K	K	
1	0.000	-33.941	0.000	0.000	33.941	0.000	0.000%
2	-0.051	-33.941	-28.439	0.051	33.941	28.439	0.000%
3	14.022	-33.941	-24.656	-14.022	33.941	24.656	0.000%
4	24.350	-33.941	-14.267	-24.350	33.941	14.267	0.000%
5	28.184	-33.941	0.117	-28.184	33.941	-0.117	0.000%
6	24.412	-33.941	14.310	-24.412	33.941	-14.310	0.000%
7	14.106	-33.941	24.657	-14.106	33.941	-24.657	0.000%
8	0.066	-33.941	28.439	-0.066	33.941	-28.439	0.000%
9	-13.981	-33.941	24.641	13.981	33.941	-24.641	0.000%
10	-24.377	-33.941	14.195	24.377	33.941	-14.195	0.000%
11	-28.177	-33.941	-0.045	28.177	33.941	0.045	0.000%
12	-24.370	-33.941	-14.314	24.370	33.941	14.314	0.000%
13	-14.093	-33.941	-24.663	14.093	33.941	24.663	0.000%
14	0.000	-51.022	0.000	-0.000	51.022	0.000	0.000%
15	-0.011	-51.022	-7.208	0.011	51.022	7.208	0.000%
16	3.561	-51.022	-6.248	-3.561	51.022	6.248	0.000%
17	6.182	-51.022	-3.614	-6.182	51.022	3.614	0.000%
18	7.154	-51.022	0.026	-7.154	51.022	-0.026	0.000%
19	6.196	-51.022	3.624	-6.196	51.022	-3.624	0.000%
20	3.580	-51.022	6.248	-3.580	51.022	-6.248	0.000%
21	0.015	-51.022	7.208	-0.015	51.022	-7.208	0.000%
22	-3.552	-51.022	6.245	3.552	51.022	-6.245	0.000%
23	-6.188	-51.022	3.598	6.188	51.022	-3.598	0.000%
24	-7.152	-51.022	-0.010	7.152	51.022	0.010	0.000%
25	-6.187	-51.022	-3.625	6.187	51.022	3.625	0.000%
26	-3.577	-51.022	-6.250	3.577	51.022	6.250	0.000%
27	-0.018	-33.941	-9.840	0.018	33.941	9.840	0.000%
28	4.852	-33.941	-8.531	-4.852	33.941	8.531	0.000%
29	8.426	-33.941	-4.937	-8.426	33.941	4.937	0.000%
30	9.752	-33.941	0.040	-9.752	33.941	-0.040	0.000%
31	8.447	-33.941	4.952	-8.447	33.941	-4.952	0.000%
32	4.881	-33.941	8.532	-4.881	33.941	-8.532	0.000%
33	0.023	-33.941	9.840	-0.023	33.941	-9.840	0.000%
34	-4.838	-33.941	8.526	4.838	33.941	-8.526	0.000%
35	-8.435	-33.941	4.912	8.435	33.941	-4.912	0.000%
36	-9.750	-33.941	-0.015	9.750	33.941	0.015	0.000%
37	-8.432	-33.941	-4.953	8.432	33.941	4.953	0.000%
38	-4.877	-33.941	-8.534	4.877	33.941	8.534	0.000%

## Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	4	0.00000001	0.00041128
3	Yes	6	0.00000001	0.00005417
4	Yes	6	0.00000001	0.00005446
5	Yes	5	0.00000001	0.00004173
6	Yes	6	0.00000001	0.00005492
7	Yes	6	0.00000001	0.00005446
8	Yes	4	0.00000001	0.00030634
9	Yes	6	0.00000001	0.00005473
10	Yes	6	0.00000001	0.00005446
11	Yes	4	0.00000001	0.00029675
12	Yes	6	0.00000001	0.00005514
13	Yes	6	0.00000001	0.00005427

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14	Yes	4	0.00000001	0.00004200
15	Yes	5	0.0000001	0.00052954
16	Yes	5	0.0000001	0.00068787
17	Yes	5	0.00000001	0.00068844
18	Yes	5	0.0000001	0.00052059
19	Yes	5	0.0000001	0.00067976
20	Yes	5	0.0000001	0.00067988
21	Yes	5	0.00000001	0.00052110
22	Yes	5	0.00000001	0.00067441
23	Yes	5	0.0000001	0.00067221
24	Yes	5	0.00000001	0.00051929
25	Yes	5	0.00000001	0.00069030
26	Yes	5	0.00000001	0.00068908
27	Yes	4	0.0000001	0.00015835
28	Yes	5	0.00000001	0.00013393
29	Yes	5	0.0000001	0.00013471
30	Yes	4	0.00000001	0.00020042
31	Yes	5	0.00000001	0.00013702
32	Yes	5	0.00000001	0.00013525
33	Yes	4	0.00000001	0.00017353
34	Yes	5	0.0000001	0.00013664
35	Yes	5	0.00000001	0.00013468
36	Yes	4	0.0000001	0.00016010
37	Yes	5	0.00000001	0.00013828
38	Yes	5	0.00000001	0.00013479

#### **Maximum Tower Deflections - Service Wind**

Elevation	Horz.	Gov.	Tilt	Twist
	Deflection	Load		
ft	in	Comb.	0	0
125 - 94.45	29.347	27	2.180	0.004
94.45 - 85.04	16.576	33	1.651	0.001
88.957 - 73.5	14.724	33	1.570	0.001
73.5 - 73	9.959	33	1.348	0.001
73 - 60.5	9.818	33	1.341	0.001
60.5 - 40.457	6.675	33	1.058	0.000
45.54 - 30.5	3.815	33	0.768	0.000
30.5 - 0	1.705	33	0.543	0.000
	ft  125 - 94.45  94.45 - 85.04  88.957 - 73.5  73.5 - 73  73 - 60.5  60.5 - 40.457  45.54 - 30.5	ft         Deflection in           125 - 94.45         29.347           94.45 - 85.04         16.576           88.957 - 73.5         14.724           73.5 - 73         9.959           73 - 60.5         9.818           60.5 - 40.457         6.675           45.54 - 30.5         3.815	ft         Deflection in         Load Comb.           125 - 94.45         29.347         27           94.45 - 85.04         16.576         33           88.957 - 73.5         14.724         33           73.5 - 73         9.959         33           73 - 60.5         9.818         33           60.5 - 40.457         6.675         33           45.54 - 30.5         3.815         33	ft         Deflection in         Load Comb.         °           125 - 94.45         29.347         27         2.180           94.45 - 85.04         16.576         33         1.651           88.957 - 73.5         14.724         33         1.570           73.5 - 73         9.959         33         1.348           73 - 60.5         9.818         33         1.341           60.5 - 40.457         6.675         33         1.058           45.54 - 30.5         3.815         33         0.768

#### **Critical Deflections and Radius of Curvature - Service Wind**

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	٥	ft
125.000	APXVTM14-C-120 w/ Mount Pipe	27	29.347	2.180	0.004	11161
124.000	VHLP2-18	27	28.895	2.162	0.004	11161
123.000	PCS 1900MHz 4x45W-65MHz w /	27	28.444	2.144	0.004	11161
	Mount Pipe					
115.000	(3) AM-X-CD-16-65-00T-RET w/	27	24.866	1.998	0.003	5580
	Mount Pipe					
105.000	BXA-70063-6CF-EDIN-0 w/ Mount	27	20.601	1.822	0.001	2789
	Pipe					
85.000	ERICSSON AIR 21 B2A B4P w/	33	13.442	1.512	0.001	5067
	Mount Pipe					

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#### **Maximum Tower Deflections - Design Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	125 - 94.45	84.506	2	6.280	0.012
L2	94.45 - 85.04	47.772	8	4.759	0.003
L3	88.957 - 73.5	42.438	8	4.525	0.002
L4	73.5 - 73	28.707	8	3.887	0.002
L5	73 - 60.5	28.302	8	3.867	0.002
L6	60.5 - 40.457	19.245	8	3.052	0.001
L7	45.54 - 30.5	10.999	8	2.215	0.001
L8	30.5 - 0	4.917	8	1.566	0.000

### Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	0	ft
125.000	APXVTM14-C-120 w/ Mount Pipe	2	84.506	6.280	0.012	3958
124.000	VHLP2-18	2	83.207	6.227	0.011	3958
123.000	PCS 1900MHz 4x45W-65MHz w /	2	81.909	6.174	0.011	3958
	Mount Pipe					
115.000	(3) AM-X-CD-16-65-00T-RET w/	2	71.624	5.754	0.007	1978
	Mount Pipe					
105.000	BXA-70063-6CF-EDIN-0 w/ Mount	2	59.359	5.249	0.003	987
	Pipe					
85.000	ERICSSON AIR 21 B2A B4P w/	8	38.744	4.357	0.002	1780
	Mount Pipe					

#### Compression Checks

#### Pole Design Data

Section No.	Elevation	Size	L	$L_u$	Kl/r	$F_a$	A	Actual P	Allow. $P_a$	Ratio P
	ft		ft	ft		ksi	$in^2$	K	K	$P_a$
L1	125 - 94.45 (1)	TP25.067x18.5x0.188	30.550	0.000	0.0	39.000	14.807	-7.661	577.454	0.013
L2	94.45 - 85.04 (2)	TP27.09x25.067x0.439	9.410	0.000	0.0	30.151	35.977	-8.480	1084.760	0.008
L3	85.04 - 73.5 (3)	TP29.158x25.873x0.475	15.457	0.000	0.0	30.262	43.227	-12.889	1308.130	0.010
L4	73.5 - 73 (4)	TP29.264x29.158x0.609	0.500	0.000	0.0	30.266	55.409	-13.018	1677.000	0.008
L5	73 - 60.5 (5)	TP31.921x29.264x0.367	12.500	0.000	0.0	39.000	36.706	-15.482	1431.540	0.011
L6	60.5 - 40.457 (6)	TP36.18x31.921x0.436	20.043	0.000	0.0	39.000	47.938	-19.353	1869.570	0.010
L7	40.457 - 30.5 (7)	TP37.787x34.6x0.485	15.040	0.000	0.0	39.000	57.389	-24.485	2238.190	0.011
L8	30.5 - 0 (8)	TP44.25x37.787x0.456	30.500	0.000	0.0	39.000	63.426	-33.922	2473.600	0.014

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	Pole Bending Design Data											
Section No.	Elevation	Size	Actual M <sub>x</sub>	$Actual f_{bx}$	Allow. $F_{bx}$	Ratio $f_{bx}$	Actual M <sub>y</sub>	$Actual f_{by}$	Allow. $F_{by}$	Ratio $f_{by}$		
	ft		kip-ft	ksi	ksi	$F_{bx}$	kip-ft	ksi	ksi	$F_{bv}$		
L1	125 - 94.45 (1)	TP25.067x18.5x0.188	374.768	49.458	39.000	1.268	0.000	0.000	39.000	0.000		
L2	94.45 - 85.04 (2)	TP27.09x25.067x0.439	472.323	24.962	30.151	0.828	0.000	0.000	30.151	0.000		
L3	85.04 - 73.5 (3)	TP29.158x25.873x0.475	785.700	31.083	30.262	1.027	0.000	0.000	30.262	0.000		
L4	73.5 - 73 (4)	TP29.264x29.158x0.609	796.461	24.719	30.266	0.817	0.000	0.000	30.266	0.000		
L5	73 - 60.5 (5)	TP31.921x29.264x0.367	1073.17 5	45.227	39.000	1.160	0.000	0.000	39.000	0.000		
L6	60.5 - 40.457 (6)	TP36.18x31.921x0.436	1424.25 8	41.876	39.000	1.074	0.000	0.000	39.000	0.000		
L7	40.457 - 30.5 (7)	TP37.787x34.6x0.485	1801.40 0	41.130	39.000	1.055	0.000	0.000	39.000	0.000		
L8	30.5 - 0 (8)	TP44.25x37.787x0.456	2629.00 0	46.145	39.000	1.183	0.000	0.000	39.000	0.000		

Section	Elevation	Size	Actual	Actual	Allow.	Ratio	Actual	Actual	Allow.	Ratio		
No.			V	$f_{v}$	$F_{\nu}$	$f_{v}$	T	$f_{vt}$	$F_{vt}$	$f_{vt}$		
	ft		K	ksi	ksi	$F_{v}$	kip-ft	ksi	ksi	$F_{vt}$		
L1	125 - 94.45 (1)	TP25.067x18.5x0.188	17.522	1.183	26.000	0.091	0.099	0.006	26.000	0.000		
L2	94.45 - 85.04	TP27.09x25.067x0.439	17.989	0.500	20.101	0.050	0.095	0.002	20.101	0.000		
	(2)											
L3	85.04 - 73.5 (3)	TP29.158x25.873x0.475	21.504	0.497	20.175	0.049	0.070	0.001	20.175	0.000		
L4	73.5 - 73 (4)	TP29.264x29.158x0.609	21.557	0.389	20.177	0.039	0.068	0.001	20.177	0.000		
L5	73 - 60.5 (5)	TP31.921x29.264x0.367	22.744	0.620	26.000	0.048	0.016	0.000	26.000	0.000		
L6	60.5 - 40.457 (6)	TP36.18x31.921x0.436	24.270	0.506	26.000	0.039	0.013	0.000	26.000	0.000		
L7	40.457 - 30.5 (7)	TP37.787x34.6x0.485	25.803	0.450	26.000	0.035	0.328	0.004	26.000	0.000		
L8	30.5 - 0 (8)	TP44.25x37.787x0.456	28.461	0.449	26.000	0.035	0.324	0.003	26.000	0.000		

			Pol	e Intera	action	Desigr	Data		
Section No.	Elevation ft	Ratio P	Ratio $\frac{f_{bx}}{F_{bx}}$	$F_{by}$	Ratio $f_v$ $F_v$	Ratio $\frac{f_{vt}}{F_{vt}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	125 - 94.45 (1)	0.013	1.268	0.000	0.091	0.000	1.284	1.333	H1-3+VT 🗸
L2	94.45 - 85.04 (2)	0.008	0.828	0.000	0.050	0.000	0.836	1.333	H1-3+VT 🖊
L3	85.04 - 73.5 (3)	0.010	1.027	0.000	0.049	0.000	1.038	1.333	H1-3+VT 🗸
L4	73.5 - 73 (4)	0.008	0.817	0.000	0.039	0.000	0.825	1.333	H1-3+VT 🖊
L5	73 - 60.5 (5)	0.011	1.160	0.000	0.048	0.000	1.171	1.333	H1-3+VT 🗸

B+T Group

1717 South Boulder Ave, Suite 300 Tulsa, OK - 74119 Phone: (918) 587-4630 FAX: (918) 295-0265

Job	Page
84470.010.01 - Cromwell/First Line Emergencl, CT (BL 876364)	J# 20 of 20
Project	Date
	11:50:22 08/21/14
Client	Designed by
Crown Castle	JTillson

Section No.	Elevation	Ratio P	$Ratio \ f_{bx}$	$egin{aligned} Ratio \ f_{by} \end{aligned}$	Ratio $f_v$	$Ratio \ f_{vt}$	Comb. Stress	Allow. Stress	Criteria
	ft	$P_a$	$F_{bx}$	$F_{by}$	$\overline{F_{\nu}}$	$F_{vt}$	Ratio	Ratio	
							~		
L6	60.5 - 40.457 (6)	0.010	1.074	0.000	0.039	0.000	1.084	1.333	H1-3+VT 🗸
L7	40.457 - 30.5 (7)	0.011	1.055	0.000	0.035	0.000	1.066	1.333	H1-3+VT 🗸
L8	30.5 - 0 (8)	0.014	1.183	0.000	0.035	0.000	1.197	1.333	H1-3+VT 🗸

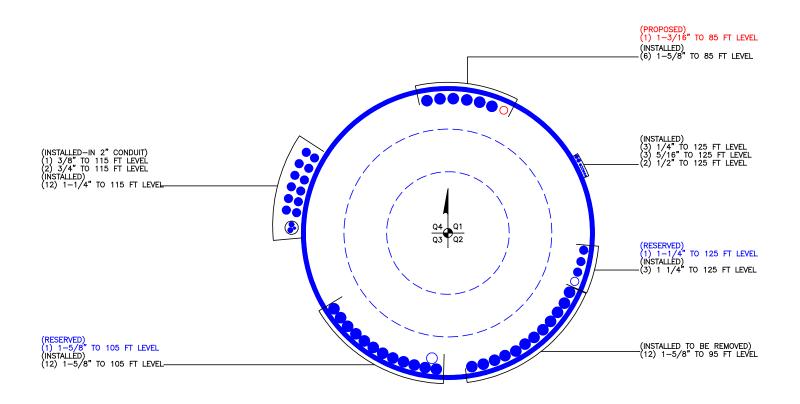
#### **Section Capacity Table**

Section	Elevation	Component	Size	Critical	P	$SF*P_{allow}$	%	Pass
No.	ft	Type		Element	K	K	Capacity	Fail
L1	125 - 94.45	Pole	TP25.067x18.5x0.188	1	-7.661	769.746	**	**
L2	94.45 - 85.04	Pole	TP27.09x25.067x0.439	2	-8.480	1445.985	**	**
L3	85.04 - 73.5	Pole	TP29.158x25.873x0.475	3	-12.889	1743.737	**	**
L4	73.5 - 73	Pole	TP29.264x29.158x0.609	4	-13.018	2235.441	**	**
L5	73 - 60.5	Pole	TP31.921x29.264x0.367	5	-15.482	1908.243	**	**
L6	60.5 - 40.457	Pole	TP36.18x31.921x0.436	6	-19.353	2492.137	**	**
L7	40.457 - 30.5	Pole	TP37.787x34.6x0.485	7	-24.485	2983.507	**	**
L8	30.5 - 0	Pole	TP44.25x37.787x0.456	8	-33.922	3297.309	**	**
							Summary	
						Pole (L1)	96.1	Pass
						RATING =	96.1	Pass

#### \*\*See additional calculations

 $Program\ Version\ 6.1.4.1-12/17/2013\ File: S:/Projects/Crown\ Castle/84470\_876364\_Cromwell/Engineering/tnxTower/84470\_010\_01\_Cromwell\\ First\ Line\ Emergenc\_CT\_AS.eri$ 

## APPENDIX B BASE LEVEL DRAWING



BUSINESS UNIT: 876364 TOWER ID: C\_BASELEVEL

# APPENDIX C ADDITIONAL CALCULATIONS



5500 Flatirons Parkway, Suite 100 Boulder, CO 80301 720-304-6882

<b>Dimensions and Properties</b>														Compression	1			Axial				
																			ASD-9		LR	RFD
					Centroid	Centroid													Allowable			
					from	from Bolt	Web			Flange	Hole			Slender.		Slender.			Axial w/		Design Axial	
	Weight		Moment of	Moment of	Mating Edge	Hole Center	Thickness		Flange	Thickness	Diameter	Yield Stress	Ultimate	Ratio	Unbraced	Ratio	Unbraced	Allowable	increase	Governing	Strength	Governing
Model	(lb/ft)	Area (in²)	Inertia (in <sup>4</sup> )	Inertia (in <sup>4</sup> )	(in)	(in)	(in)	Width (in)	Width (in)	(in)	(in)	(ksi)	Stress (ksi)	Coefficient	Length (in)	Coefficient	Length (in)	Axial (kip)	(kip)	Axial	(kip)	Axial
Model	Wt	Α	lx	ly	Υ	X	Tw	W	Wf	Tf	Dh	Fy	Fu	Kx	Lx	Ку	Ly	PAII	Pall.inc	Ptype.ASD	phiPn	Ptype.LRFD
MP404	12.1	3.56	0.17	6.70	0.375	0	0.75	4.75	0	0	1.21875	100	110	0.80	14	1.00	14	143.1	190.8	Rupture	214.6	Rupture
MP406	20.7	6.09	0.79	12.07	0.625	0	1.25	4.875	0	0	1.21875	100	110	0.80	23	1.00	23	247.1	329.4	Rupture	370.6	Rupture
MS-600	20.4	6.00	0.50	18.00	0.5	0	1	6	0	0	1.21875	65	80	0.80	16.375	1.00	16.375	187.8	250.4	Compress.	283.1	Rupture

Rein1												Flats	(Used fo	or relati	ve orie	ntation	only. A	ctual fla	at num	bers ma	ay vary.)				
Bottom	Тор	Qty	1	Model	Positio	n T	or T&C	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
0	30	).5	3	MP40	6	F	T&C			1						1						1			
30.5	60	).5	3	MP40	6	F	T&C				1						1						1		
60.5	73	3.5	3	MP404	4	F	T&C			1						1						1			
						F	T&C																		
						F	T&C																		
						F	T&C																		
						F	T&C																		
						F	T&C																		
						F	T&C																		
Rein2	_	0.	_			_	<b>T</b> 0.0																		
	Top	Qty		Model			or T&C																		
73	87.		3			F	T&C	1						1						1					
87.45	94.	45	3	MS-600	U	F	T&C		1						1						1				
						-	T&C																		
						F -	T&C																		
						-	T&C																		
						F	T&C																		
						r -	T&C																		
						r	T&C T&C																		
						r	TAC	<u> </u>																	
Rein3																									
	Тор	Qty	r	Model	Positio	n T	or T&C																		
0		Qty		viouci	1 031110	F .	T&C																		
						F	T&C																		
						F	T&C																		
						F	T&C																		
						F	T&C																		
						F	T&C																		
						F	T&C																		
							T&C																		
						г С	T&C																		
							TAC																		



Section		Loads				Pole																				Unrei	inforced F	ole - Rev	. #			Reinfo	rced Pole					Rev. F	Reinforce	ment 1									Reinforce	ment 2								Comp	osite							
														Percent o Composit	of it	Dis	itance				Allov	Alli wable Be	owable ending A	llowable	Allowab	de				Torsion		Mom in Po	ent le				Torsion						Betv		nsion nly								Gap	Tension only												
									Yield			M	foment		Ang	de	to 5	Section	Torsion	Polygos	nal Ben	nding M	ament	Axial	Shear	Beno	fing A	lodal	Shear	Shear		whe	n Ben	ding A	ocial	Shear	Shear	Reinforced				Positis		and c			Axial					Position	Pole and	or	Total	Axia			/ /						Derated	
	Elevation	Momen	nt Compress	4	Torsion	Number		Thicknes	s Strengt	th Flat	t	of	finertia	Moment	rt Offset	t to Ext	treme N	Modulus	Constan	Compa	ict Str	ress Str	nength	Stress	Stress	Stn	ess S	tress	Stress	Stress	Stress	Reinfo	rce Str	ress S	ress	Stress	Stress	Pole Stress				(F-Fla	at, Bac	ck of Tensi	ion & Mo	ment of	Force	Stress				(F-Flat,	Back of	Tension &	Moment	of Force	Stre	is Cent	oid	Mo	ment of c	ontrolling	Thickness	Weight	Yield Stress	a Derated
Section	(ft)	(ft-kip)	a) on (kip)	Shear (kip)	(kip-ft)	of Sides	00 (in	(in)	(ksi)	Width	(in) Are	ra (in²)	(in <sup>4</sup> )	of Inertia	a Pole I	flat fib	er (in)	(in <sup>3</sup> )	(in*)	Criterio	on (k	csi) (f	t-kip)	(ksi)	(ksi)	(ka	(0)	(ksi)	(ksi)	(ksi)	Ratio	d	(k:	si) (	ksi)	(ksi)	(ksi)	Ratio	Qty		Model	C-Corn	ner) Bein	. (in) Con	mp. Ine	rtia (in <sup>4</sup> )	(kip)	Ratio	Qty	Mod	ed C	-Corner)	tein. (in)	Comp.	Inertia (in	) (kipi	Rat	Office	(in) Area	(in <sup>2</sup> ) Ine	rtia (in <sup>4</sup> ) St	tress Ratio	(in)	Multiplier	(ksi)	Stre
1	125	7.0	2.4	4.6	0.0	18	18,500	0.1875	65	2.9	3 1	10.9	461	100%	TRU	UE 1	9.38	49	914	126	53	2.0	13.0	52.0	34.7	1	7	0.22	0.00	0.00	0.037	7.0	1	.7 0	1.22	0.00	0.00	0.037																		$\neg$		0.0	00 1	.9	461	0.037	0.1875	1.00	65.0	26.0
2	94.45	374.8	8 7.7	17.5	0.1	18	25.067	2 0.1875	65	4.0	9 1	14.8	1157	100%	TRU	UE 1	2.71	91	2293	176	53	2.0	194.2	52.0	34.7	45	1.4	0.52	0.00	0.01	0.963	374	8 41	9.4 0	0.52	0.00	0.00	0.961																		1		0.0	00 1	.8	1157	0.961	0.1875	1.00	65.0	0.31
3	88.957	472.4	4 8.5	18.0	0.1	18	26.248	0.1875	65	4.3	0 1	15.5	1329	44%		) 1	3.12	101	2635	185	53	2.0	38.8	52.0	34.7	56	.0	0.55	0.00	0.01	1.087	207	4 24	4.6 0	0.55	0.00	0.00	0.483											3	MS-6	00	,	0	T&C	1698	153.	0.6	2 0.0	00 3	.5	3028	0.612	0.4392	0.93	50.3	2.51
4	87.45	498.9	9 84	173	0.1	18	26 571	0.4375	65	3.0	1 9	86.2	3126	64%		1	3.29	235	6203	72		20 1	019.2	52.0	34.7	25	4	0.23	0.00	0.00	0.494	320	6 1	64 4	128	0.00	0.00	0.319											*	MS.6	nn n		0	T&C	1738	95.0	0.8	3 00	00 5	2	4864	0.383	0.7004	0.94	50.3	85
- 5	85.04	543.6	6 96	18.4	0.1	18	27.090	0.4375	65	4.0	1 9	37.0	3315	65%		1	3.55	245	6579	74	- 5	20 1	060.4	52.0	34.7	26	7	0.26	0.00	0.00	0.518	352	1 1	7.8 .	126	0.00	0.00	0.337											3	MS.6	00			T8.C		107			100 55						50.3	
6	73.5	785.7	7 12.9	21.5	0.1	18	29 157	8 0.2500	65	4.7	n 3	22.9	2419	54%		1	4 58	166	4795	152		20 3	1188	52.0	34.7	56	8	0.56	0.00	0.00	1 10	423	0 31	0.6 4	156	0.00	0.00	0.599											3	MS.6	00	-	0	T8.C	2074	189	0.7	9 00	100 40	9	4493	0.759	0.4748	0.95	50.4	
7	73	796.5	5 13.0	21.6	0.1	18	29 264	0.2500	65	4.7	2 2	23.0	2446	43%		1	4.63	167	4848	152		20 3	724.1	52.0	34.7	53	2 .	0.57	0.00	0.00	1 111	338	9 24	43 4	157	0.00	0.00	0.479	- 1		MD404			о т	2.0	1214	816	0.439	- 1	MS.6	00	-	0	T8.C	2082	151	0.6	rs 0.0	00 5	7	5748	0.603	0.6092	0.93	50.4	2 31
	60.5	1072	2 15.5	22.2	0.0	19	21.030	0.2500	66	6.1	0 1	26.1	3191	60W	1/	0 1	6 19	107	6306	167		20 1	61.4	63.0	24.7	- 61	.5	0.63	0.00	0.00	1.32	739	4 4	51 6	162	0.00	0.00	0.880	-		MP404	-	_	0 7		1436	150.0	0.010			_	_	_				-	0.0	-00 2	*	4617	0.890	0.3665	0.00	65.0	0.31
	45.54	1474	2 10.4	24.2	0.0	19	25 000	0.2500	65	5.2		27.6	4230	5.9W	1 10	0 1		238	8401	195	1 6	20 1	031.6	53.0	34.7			0.70	0.00	0.00	1000	879			2.20	0.00	0.00	0.818	-		MD406	-		0 7				0.779			_	-		_		_		0.0					0.4357		65.0	0.21
10	40.457	1540	1 21.6	24.0	0.0	19	36 180	0.5635	65	5.7	0 6		10173	769	1 10	0 1	8 33				- 6	20 2	40E O	52.0				0.24	0.00	0.00	0.651				1.24	0.00	0.00	0.496	-		MD406			0 7							_			_		_		0.0	100 81						65.0	
11	20.5	1801	4 24.6	25.9	0.0	19	37.786	0.3135	65	6.1	1 1		6587	65%			9.16					20 2	480.4	52.0	24.7			0.66	0.00	0.00	0.03		2 4		1.66	0.00	0.00	0.400	-		MD406			0 7				0.767	_		-	-	-			_		0.0	100 55						65.0	
12	a).5	26291		25.8	0.3	18	41.780	0.3125				43.6			1 10		2.44	493	21016	187			040.7	52.0			7		0.00		-	1814				0.00			-		MPNUG	-		0 1		4750			-		-	_	-	_	_	-	_								65.0	
12	0	20293	D 33.9	28.5	0.3	18	44.230	0.3125	- 65	7.2	5 4	43.0	10019	69%	- 10	0 2	2.44	4/3	21044	10/		2.0 2	049.7	52.0	34.7	00	<i>u</i>	0./6	0.00	0.00	1.29	1010	.4	0.1	1.76	0.00	0.00	0.961	,		MP4U0	_	_	0 1	ar.	4/30	280.4	0.851			_	_				_		u.c	AU 67	.0		0.961	0.4303	0.76	65.0	4 0.51

#### Stiffened or Unstiffened, Ungrouted, Circular Base Plate - Any Rod Material

#### TIA Rev F

Site Data

BU#: 876364

Site Name: CROMWELL - FIRST LINE

App #: 252176 Revision # 1

Pole Manufacturer: Other

And	chor Rod Da	ata
Qty:	12	
Diam:	2.25	in
Rod Material:	A615-J	
Strength (Fu):	100	ksi
Yield (Fy):	75	ksi
Bolt Circle:	53	in

	Plate Data	
Diam:	59	in
Thick:	1.75	in
Grade:	60	ksi
Single-Rod B-eff:	11.70	in

Stiffener Da	ata (Welding	at both sides)
Config:	1	*
Weld Type:	Fillet	
Groove Depth:		< Disregard
Groove Angle:		< Disregard
Fillet H. Weld:	0.625	in
Fillet V. Weld:	0.375	in
Width:	7	in
Height:	22	in
Thick:	0.75	in
Notch:	0.75	in
Grade:	50	ksi
Weld str.:	70	ksi

	Pole Data	
Diam:	44.25	in
Thick:	0.3125	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None

Stress	Increase F	actor
ASIF:	1.333	

Reactions		
Moment:	2205.1483	ft-kips
Axial:	33.9222	kips
Shear:	28.460976	kips

If No stiffeners, Criteria:

AISC ASD <-Only Applicable to Unstiffened Cases

**Anchor Rod Results** 

Maximum Rod Tension: 163.6 Kips Allowable Tension: 195.0 Kips 83.9% Pass

Anchor Rod Stress Ratio:

**Base Plate Results** Flexural Check Base Plate Stress: 49.8 ksi Allowable Plate Stress: 60.0 ksi Base Plate Stress Ratio: 83.0% Pass

Stiffened Service, ASD 0.75\*Fy\*ASIF Y.L. Length: N/A, Roark

Stiffened

Service, ASD

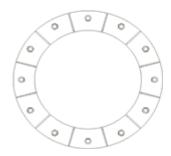
Fty\*ASIF

Stiffener Results

Horizontal Weld: 73.6% Pass Vertical Weld: 39.2% Pass Plate Flex+Shear, fb/Fb+(fv/Fv)^2: 15.5% Pass Plate Tension+Shear, ft/Ft+(fv/Fv)^2: 64.1% Pass Plate Comp. (AISC Bracket): 64.7% Pass

**Pole Results** 

12.2% Pass Pole Punching Shear Check:



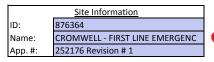


Analysis Date: 8/21/2014

<sup>\* 0 =</sup> none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

<sup>\*\*</sup> Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

#### Anchor Rod Information for TIA/EIA-222-F and TIA-222-G-2





Base	Reactions	
Moment:	2629	ft-kip
Axial:	34	kip
Shear:	28	kip
Base Plate Type:	Circular	

Design I	Information	
TIA Code:	F	
ASIF:	1.333	
Failure:	100%	
eta Factor:	0.50	

Original A	Anchor Rod Dat	<u>ta</u>
Quantity:	12	
Diameter:	2.25	in
Material:	#18J	
Bolt Circle:	53.0	in
Bolt Spacing:		in
Bolt Group Area:	47.71	in²
Bolt Group MOIx:	16753	in <sup>4</sup>
Moment: Axial:	2205.1 33.9	kip-ft kip
Shear:	28.5	kip
	NR Capacity Che	
Tension Load:	163.6	kip
Allowable load:	194.8	kip
AR Capacity:	84.0%	Pass

First Added Anchor Rod Data					
Quantity:	3				
Diameter:	1.75	in			
Material:	A193 B7				
Bolt Circle:	59.8	in			
Bolt Group Area:	7.22	in²			
Bolt Group MOIx:	3220	in <sup>4</sup>			
Reactions Seen b Moment:	<u>y First Added A</u> 423.9	AR Group kip-ft			
Axial:	0.0	kip			
Shear:	0.0	kip			
First Added AR Capacity Check					
Tension Load:	113.5	kip			
		1.1			
Allowable load:	132.3	kip			

Second Adde	d Anchor Rod	Data
Quantity:		
Diameter:		in
Material:		
Bolt Circle:		in
Bolt Group Area:	0.00	in²
Bolt Group MOIx:	0	in <sup>4</sup>
Reactions Seen by	Second Added	d AR Grou
Moment:	0.0	kip-ft
Axial:	0.0	kip
Shear:	0.0	kip
Second Added	I AR Capacity	<u>Check</u>
Tension Load:	0.0	kip
Allowable load:	0.0	kip
AR Capacity:	0.0%	

Third Added Anchor Rod Data					
Quantity:					
Diameter:		in			
Material:					
Bolt Circle:		in			
Bolt Group Area:	0.00	in²			
Bolt Group MOIx:	0	in <sup>4</sup>			
Reactions Seen by Second Added AR Group					
Moment:	0.0	kip-ft			
Axial:	0.0	kip			
Shear:	0.0	kip			
Second Added AR Capacity Check					
Tension Load:	0.0	kip			
Allowable load:	0.0	kip			
AR Capacity:	0.0%				

PROJECT	876364 - CROMWELL / FIRST LINE EMERGENC, CT					
SUBJECT	T Foundation Analysis					
DATE	08/21/14	PAGE	1	OF	1	



Rev. Type:

### Monopole Pad & Pier Foundation Analysis

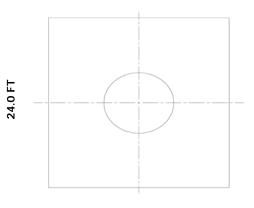
#### Design Loads:

Input unfactored loads

**28.0** kips Shear: Moment: 2,629.0 ft-kips Tower Height: **125.0** ft Tower Weight: 34.0 kips

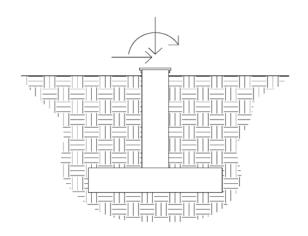
#### Pad & Pier Dimensions / Properties:

Pole Diameter at Base:	
Bearing Depth:	<b>5.0</b> ft
Pad Width:	<b>24.0</b> ft
Neglected Depth:	<b>3.3</b> ft
Thickness:	<b>3.0</b> ft
Pier Diameter:	<b>14.0</b> ft
Pier Height Above Grade:	<b>1.0</b> ft
BP Dist. Above Pier:	<b>0.0</b> in
Clear Cover:	3.0 in
Pier Rebar Size:	8
Pier Rebar Quanity:	24
Pad Rebar Size:	8
Pad Rebar Quanity:	30
Pier Tie Size:	4
Tie Quanity:	7
Rebar Yield Strength:	<b>60000</b> psi
Concrete Strength:	<b>3000</b> psi
Concrete Unit Weight:	<b>0.15</b> kcf



24.0 FT

#### **Elevation Overview**



#### Soil Data:

Allowable Values			
0.125	kcf		
8.000	ksf		
30.000	deg		
0.000	ksf		
0.000	ksf		
0.300	_		
	0.125 8.000 30.000 0.000		

#### **Summary of Results**

Req'd Pier Diam.	OK
Overturning	80.9%
Shear Capacity	38.7%
Bearing	43.8%
Pad Shear - 1-way	18.9%
Pad Shear - 2-way	6.7%
Pad Moment Capacity	13.8%
Pier Moment Capacity	50.1%



#### RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

MetroPCS Existing Facility

Site ID: CTHA520A

Crown Cromwell Monopole 201 Main Street Cromwell, CT 06416

**September 24, 2014** 

Site Compliance Summary			
Compliance Status:	COMPLIANT		
Site total MPE% of	04.04.0/		
FCC general public allowable limit:	81.81 %		



September 24, 2014

MetroPCS Attn: Jason Overbey, RF Manager 35 Griffin Road South Bloomfield, CT 06002

Emissions Analysis for Site: CTHA520A – Crown Cromwell Monopole

EBI Consulting was directed to analyze the proposed MetroPCS facility located at **201 Main Street**, **Cromwell**, **CT**, for the purpose of determining whether the emissions from the Proposed MetroPCS Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu$ W/cm<sup>2</sup>). The number of  $\mu$ W/cm<sup>2</sup> calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu$ W/cm<sup>2</sup>). The general population exposure limit for both the PCS and AWS bands is 1000  $\mu$ W/cm<sup>2</sup>. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

#### **CALCULATIONS**

Calculations were done for the proposed MetroPCS Wireless antenna facility located at **201 Main Street**, **Cromwell**, **CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since MetroPCS is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 GSM channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel
- 2) 2 UMTS channels (AWS Band 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 3) 2 LTE channels (AWS Band 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 4) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.



- 5) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 6) The antennas used in this modeling are the **Ericsson AIR21 B4A/B2P** for 1900 MHz (PCS) and 2100 MHz (AWS) channels. This is based on feedback from the carrier with regards to anticipated antenna selection. The **Ericsson AIR21 B4A/B2P** has a maximum gain of **15.9 dBd** at its main lobe. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antenna mounting height centerline of the proposed antennas is **85 feet** above ground level (AGL).
- 8) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general public threshold limits.



#### **MetroPCS Site Inventory and Power Data**

Sector:	A	Sector:	В	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	Ericsson AIR21	Make / Model:	Ericsson AIR21	Make / Model:	Ericsson AIR21
Wake / Woder.	B4A/B2P	wiake / wiodei.	B4A/B2P	wake / wiodei.	B4A/B2P
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	85	Height (AGL):	85	Height (AGL):	85
Engavanay Danda	1900 MHz(PCS) /	Emagyamay, Danda	1900 MHz(PCS) /	Emaguan ay Danda	1900 MHz(PCS) /
Frequency Bands	2100 MHz (AWS)	Frequency Bands	2100 MHz (AWS)	Frequency Bands	2100 MHz (AWS)
Channel Count	2	Channel Count	2	# PCS Channels:	2
Total TX Power:	120	Total TX Power:	120	# AWS Channels:	120
ERP (W):	1,906.06	ERP (W):	1,906.06	ERP (W):	1,906.06
Antenna A1 MPE%	2.69	Antenna B1 MPE%	2.69	Antenna C1 MPE%	2.69
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	Ericsson AIR21	Make / Model:	Ericsson AIR21	Make / Model:	Ericsson AIR21
Wiake / Wiodel.	B4A/B2P		B4A/B2P	Wake / Wiodel.	B4A/B2P
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	85	Height (AGL):	85	Height (AGL):	85
Frequency Bands	1900 MHz(PCS) /	Frequency Bands	1900 MHz(PCS) /	Frequency Bands	1900 MHz(PCS) /
rrequeitcy Bands	2100 MHz (AWS)	rrequency bands	2100 MHz (AWS)	riequency Bands	2100 MHz (AWS)
Channel Count	4	Channel Count	4	Channel Count	4
Total TX Power:	120	Total TX Power:	120	Total TX Power:	120
ERP (W):	1,906.06	ERP (W):	1,906.06	ERP (W):	1,906.06
Antenna A2 MPE%	2.69	Antenna B2 MPE%	2.69	Antenna C2 MPE%	2.69

Site Composite MPE%			
Carrier	MPE%		
MetroPCS	16.14		
Sprint	9.26 %		
Clearwire	1.22 %		
Verizon Wireless	13.06 %		
Nextel	8.43 %		
AT&T	33.70 %		
Site Total MPE %:	81.81 %		

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MetroPCS Sector 1 Total:	5.38 %
MetroPCS Sector 2 Total:	5.38 %
MetroPCS Sector 3 Total:	5.38 %
Site Total:	81.81 %

21 B Street Burlington, MA 01803 Tel: (781) 273.2500 Fax: (781) 273.3311



#### **Summary**

All calculations performed for this analysis yielded results that were **within** the allowable limits for general public exposure to RF Emissions.

The anticipated maximum composite contributions from the MetroPCS facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general public exposure to RF Emissions are shown here:

MetroPCS Sector	Power Density Value (%)
Sector 1:	5.38 %
Sector 2:	5.38 %
Sector 3:	5.38 %
MetroPCS Total:	16.14 %
Site Total:	81.81 %
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **81.81**% of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

Scott Heffernan

RF Engineering Director

**EBI Consulting** 

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