

4 Davis Road West, Suite 5 - Old Lyme, CT 06371

Ms. Melanie Bachman Executive Director CT Siting Council 10 Franklin Square New Britain, CT 06051

Re: Notice of Exempt Modification Application 600 Old Hartford Road, Colchester, CT 06415

May 11, 2018

Dear Ms. Bachman:

Sprint Spectrum Realty Company, L.P. ("Sprint"), is submitting to the Connecticut Siting Council for a Notice of Exempt Modification for Proposed Modifications to an Existing Telecommunications Facility located at the above-referenced site. Sprint currently maintains 3 panel antennas and 6 Remote Radio at the 180' level of the Tower. Sprint proposes to add 3 new panel antennas (1 per sector) and 6 new Remote Radio Heads (2 per sector) and further proposes to add 1 new hybrid cable.

The earliest CT Siting Council submission I could find was issued to Sprint on May 30, 2014. The original Building permit for the actual tower construction issued by the Town was unavailable but there is a Building Permit from June 09, 2014. The attached construction and structural documents enclosed reflect the current reality of all the installations on the Tower.

If you have any questions, please feel free to contact me.

Thank you,

By: Paul F. Sagrístano

Paul F. Sagristano Cherundolo Consulting 917.841.0247 psagristano@lrivassoc.com



4 Davis Road West, Suite 5 – Old Lyme, CT 06371

Ms. Melanie Bachman Executive Director CT Siting Council 10 Franklin Square New Britain, CT 06051

Re: Notice of Exempt Modification Application 600 Old Hartford Road, Colchester, CT 06415

Lat: N 41.5867 Long: W72.3782

May 11, 2018

Dear Ms. Bachman:

Sprint currently maintains 3 panel antennas and 6 Remote Radio Heads at the 150' level of the above noted wireless tower. Sprint proposes to add 3 panel antennas (1 per sector) and add 6 remote radio heads (2 per sector) at the 180' tower level as well as 1 new hybrid cable. Sprint is performing a new high-performance upgrade for cellular mobile communications. It is designed to increase the capacity and speed of mobile telephone networks.

The earliest CT Siting Council approval available was from May 30, 2014. The earliest building permit for the Tower construction was not available but a recent one from June 9, 2014 is included.

Attached is a summary of the planned modifications, including power density calculations reflecting the change in Sprint's operations at the site. Also included is documentation of the structural sufficiency of the tower with proposed modifications to accommodate the revised antenna configuration.

Existing Facility

The Colchester facility is located at 600 Old Hartford Road. The Site coordinates are: N41.5867, W72.3782. The existing facility consists of a 180' Guyed Tower. Sprint currently operates wireless communications equipment on a platform on a concrete slab at the facility and has 3 antennas and 6 remote radio heads at a centerline of 180' feet on the tower. Please accept this letter as notification to the Council, pursuant to R.C.S.A. Section 16-50j-73, for construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2).

In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter is being sent to:

Art Shiloski, 1st Selectman of Colchester – Via Fed Ex Randall Benson, Town Planner – Via Fed Ex Cordless Data Transfer, Inc., the tower owner and Land owner– Via Fed Ex

Statutory Considerations

The planned modifications to the facility fall within the activities explicitly provided for in R.C.S.A. 16-50j-72(b)(2)

1. The height of the overall structure will be unaffected.

2. The proposed changes will not require an extension of the property boundaries.

3. The proposed additions will not increase the noise level at the existing facility by

six decibels or more, or to levels that exceed state and/or local criteria

4. The changes will not increase the calculated "worst case" power density for the combined operations at the site to a level at or above the Federal Communications Commission safety standard.

5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.

6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, Sprint respectfully submits that the proposed changes at the referenced site constitute exempt modifications under R.C.S.A Section §16-50j-72(b)(2).

Respectfully submitted,

Paul F. Sagrístano

Paul F. Sagristano Charles Cherundolo Consulting 917-841-0247 <u>psagristano@lrivassoc.com</u>

PFS/mtf

Additional Recipients: Art Shiloski, 1st Selectman of Colchester – Via Fed Ex Randall Benson, Town Planner – Via Fed Ex Cordless Data Transfer, Inc., the tower owner and Land owner– Via Fed Ex



May 18,2018

Dear Customer:

The following is the proof-of-delivery for tracking number 772212311312.

Delivery Information:			
Status:	Delivered	Delivered to:	Receptionist/Front Desk
Signed for by:	K.DEAN	Delivery location:	127 NORWICH AVE COLCHESTER, CT 0641
Service type: Special Handling:	FedEx Express Saver Deliver Weekday	Delivery date:	May 17, 2018 14:45
	Direct Signature Required	l	



Shipping Information:				
Tracking number:	772212311312	Ship date:	May 14, 2018	
		Weight:	0.5 lbs/0.2 kg	
Recipient:		Shipper:		
Art Shilosky, 1st Selectman		Paul Sagristano		
Town of Colchester		CCC		
127 Norwich Ave		4 Davis Road West		
COLCHESTER, CT 06415 US		Suite 5		
		OLD LYME, CT 06371 US		
Reference		CT33XC576 CSC submission		

Thank you for choosing FedEx.



May 18,2018

Dear Customer:

The following is the proof-of-delivery for tracking number 772212325400.

Delivery Information	:		
Status:	Delivered	Delivered to:	Receptionist/Front Desk
Signed for by:	K.AVERN	Delivery location:	127 NORWICH AVE COLCHESTER, CT 06415
Service type: Special Handling:	FedEx Express Saver Deliver Weekday	Delivery date:	May 17, 2018 14:44
	Direct Signature Requirec		



Shipping Information:				
Tracking number:	772212325400	Ship date:	May 14, 2018	
		Weight:	0.5 lbs/0.2 kg	
Recipient:		Shipper:		
Randall Benson, Town Planner		Paul Sagristano		
Town of Colchester		CCC		
127 Norwich Ave		4 Davis Road West		
COLCHESTER, CT 06415 US		Suite 5		
		OLD LYME, CT 06371 US		
Reference		CT33XC576 CSC submission		

Thank you for choosing FedEx.

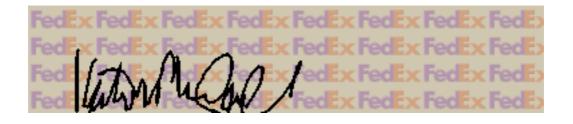


May 18,2018

Dear Customer:

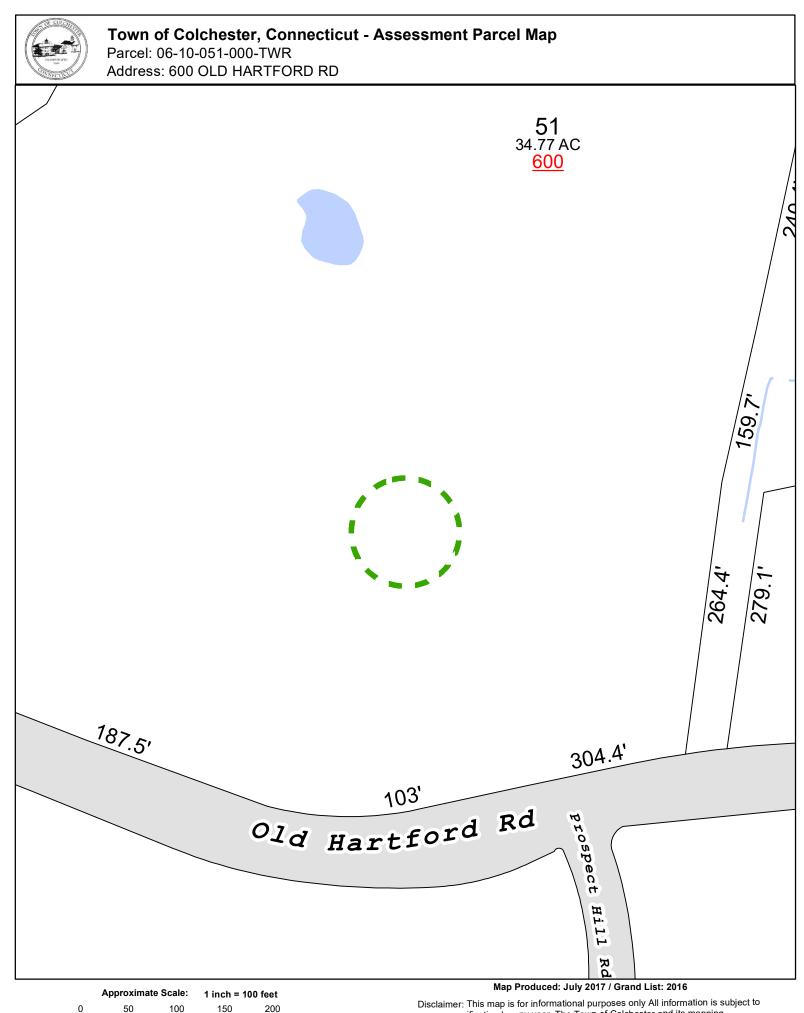
The following is the proof-of-delivery for tracking number 772212271109.

Delivery Information:			
Status:	Delivered	Delivered to:	Residence
Signed for by:	K.LEGALT	Delivery location:	600 OLD HARTFORD ROAD COLCHESTER, CT 06415
Service type: Special Handling:	FedEx Express Saver Deliver Weekday	Delivery date:	May 17, 2018 15:02
	Residential Delivery		
	Direct Signature Required	1	



Shipping Information: Tracking number: 772212271109 Ship date: May 14, 2018 Weight: 0.5 lbs/0.2 kg **Recipient:** Shipper: Mark Legault Paul Sagristano CDT, Inc CCC 600 Old Hartford Road 4 Davis Road West COLCHESTER, CT 06415 US Suite 5 OLD LYME, CT 06371 US CT33XC576 CSC Sub Reference

Thank you for choosing FedEx.



verification by any user. The Town of Colchester and its mapping contractors assume no legal responsibility for the information contained herein.

Feet



Property Listing Report

Map Block Lot 06-10

06-10/051-000/TWR Account

11AT0006

0006 PID

105116

Property Information

Property Location	600 OLD HARTFORD RD			
Owner	AT&T MO	AT&T MOBILITY		
Co-Owner	ATTN TAX MANAGER			
Mailing Address	909 CHES	TNUT ST		
Maining Address	ST LOUIS	M	10	63101
Land Use	4310	Tel Rel Tw		
Land Class	1			
Zoning Code		······································		
Census Tract				
Sub Lot				
Neighborhood				
Acreage	0			
Utilities				
Lot Setting/Desc				
Survey Map				
Additional Info				

No Phote	o Avail	lable		

Primary Construction Details

Year Built	
Stories	
Building Style	
Building Use	
Building Condition	
Floors	
Total Rooms	

Bedrooms	
Full Bathrooms	
Half Bathrooms	
Bath Style	
Kitchen Style	
Roof Style	
Roof Cover	

Exterior Walls	
Interior Walls	
Heating Type	
Heating Fuel	
АС Туре	
Gross Bldg Area	
Total Living Area	



Town of Colchester, CT

Property Listing Report

Map Block Lot 06-1

ot 06-10/051-000/TWR

Valuation Summary (Assessed value = 70% of Appraised Value)

Item	Appraised	Assessed
Buildings	0	0
Extras	0	0
Outbuildings	493400	345300
Land	0	0
Total	493400	345300

Sub Areas

Subarea Type	Gross Area (sq ft)	Living Area (sq ft)
		
······································		
		- m
Total Arca		0

Outbuilding and Extra Items

Account

Туре	Description
Fence 8' Chain	360.00 L.F.
Cell Shed	312.00 S.F.
Cell Tower	2.00 SITES
·······	
	<u></u>

Sales History

Owner of Record	Book/ Page	Sale Date	Sale Price	
Owner of Record				

000/ 000

10/1/2011

AT&T MOBILITY

Report Created On 5/11/2018



STATE OF CONNECTICUT CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051 Phone: (860) 827-2935 Fax: (860) 827-2950 E-Mail: siting.council@ct.gov www.ct.gov/csc

May 30, 2014

Jennifer Young Gaudet HPC Wireless Services 22 Shelter Rock Lane, Building C Danbury, CT 06811

RE: EM-SPRINT-028-140512 – Sprint Spectrum L.P. notice of intent to modify an existing telecommunications facility located at 600 Old Hartford Road, Colchester, Connecticut.

Dear Ms. Gaudet:

The Connecticut Siting Council (Council) hereby acknowledges your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies with the following conditions:

- Any deviation from the proposed modification as specified in this notice and supporting materials with the Council shall render this acknowledgement invalid;
- Any material changes to this modification as proposed shall require the filing of a new notice with the Council;
- Within 45 days after completion of construction, the Council shall be notified in writing that construction has been completed;
- The validity of this action shall expire one year from the date of this letter; and
- The applicant may file a request for an extension of time beyond the one year deadline provided that such request is submitted to the Council not less than 60 days prior to the expiration.

The proposed modifications including the placement of all necessary equipment and shelters within the tower compound are to be implemented as specified here and in your notice dated May 9, 2014. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site by any dimension, increase noise levels at the tower site boundary by six decibels or more, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standards adopted by the Federal Communications Commission pursuant to Section 704 of the Telecommunications Act of 1996 and by the state Department of Energy and Environmental Protection pursuant to Connecticut General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such



NV - CT33XC576

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May 30, 2014 Page 2

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notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Thank you for your attention and cooperation.

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Very truly yours,

Mélanie A. Bachman Acting Executive Director

MAB/RM/cm

c: The Honorable Gregg B. Schuster, First Selectman, Town of Colchester Adam Turner, Town Planner, Town of Colchester Cordless Data Transfer

		OLCHESTER		Street 58 Map Date 69	OFFICE USE ONI 9 0/0/1	HART Ford Rd
		G PERMIT	tenniu niterat	PERMIT	18526	4
FEES PAID	Structural Septic Electrical	Plumbing Heating Well	q ni si shevr orli o seolq boverqe Mo <u>selleting</u> off	Misc. (Misc. (Total Fee P	aid	304
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owned by Said: erection	alteration	, enlargement	_ , repairs	, removal	, demolit	ion , to b
as described in A application, all p subject. If no w provided by law.		necticut Building Code iin six months from th	and to comply	form with plans with all other la nce, this permit	ws and rule	es relating to the
11	2000	tisq ni to eloriw ni bi		Timo	My-	elle
Receipt No.	rged, extended or ail			noth	Bi I	Voil
	WHITE: Applicar	ease refer to notice on nt CANARY: Assessor	PINK: Gen. File (GOLDEN ROD: Stre	et File	
		-				

4	PPLICATION FOR BUIL			
DATE OF APPLICATION6/	S/14_ASSESSOR'S	TAX MAP & LOT #	06-10,	OSI TWR
The undersigned hereby applies for a pe DEMOLISH (), a building or structure he	erein described and in accordance	e with plans and spec	ncations submitted.	
	OLD MARTFORD P	ROPERTY OWNER	MARKLE	GAULT
UNINER SAUDRESS 600	0.00 001-11	1.1.011		
BUILDER CENTERUN	IE COMMUNICAT	PHON	(781)71	3-4725
BUILDER'S ADDRESS 95 R	IAN OR. , RAYNMAN	MA LICEN	SE# MCO. 0	902831
USE GROUP TYPE O	F CONSTRUCTION	02767 SIZE		X
GARAGE SIZE A				
NUMBER OF BATHS	NUMBER OF BEDROOMS _	JACUZ	ZI/HOT-TUBS	GAL
HEATING TYPE SIDING	SEPTIC	WELL	CITY WATER	
CITY SEWERGARB	AGE DISPOSAL	ACCESSORY	BUILDING SIZE _	15 000
IS PROPERTY WITHIN 100 YEAR	FLOOD PLAIN?	EST.CONSTRU	ICTION VALUE \$_	50,000
The applicant agrees to comply with a	Il the provisions of the building	code and with the	provisions of all othe	r laws and rules
governing building construction. Signed (Owner or Agent)	201	Print Name	ERIC DAU	1
APPROVED (Building Official)	Climathy ?	E Joh		OIL COW
DESCRIPTION OF PROPOSED WO				
ANTENNAS WITH		,		and and
RADIO EQJIPME		,		HEST HEST
SUE	CONTRACTORS		OFFICIAL US	EONLY D
Electrician			Electrical	
Name Signature	Address Lic.#		Plumbing	
Plumber			Heating	
Name Signature	Address Lic.#		Sed/Erosion Septic	
Heating Contractor			Well	
Name Signature	Address Lic.#		Driveway	
Remodeler			Building	300
Name Signature	Address Lic.#		Education Fee	300 - <u>4</u> 304
Sprinkler Contractor			State Fee	304
Name Signature	Address Lic.#		Total Fee	
			- •	

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RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

SPRINT Existing Facility

Site ID: CT33XC576

North Colchester 600 Old Hartford Road Colchester, CT 06415

February 12, 2018

EBI Project Number: 6218000957

Site Compliance Summary				
Compliance Status:	COMPLIANT			
Site total MPE% of				
FCC general				
population 2.14 %				
allowable limit:				



February 12, 2018

SPRINT Attn: RF Engineering Manager 1 International Boulevard, Suite 800 Mahwah, NJ 07495

Emissions Analysis for Site: CT33XC576 - North Colchester

EBI Consulting was directed to analyze the proposed SPRINT facility located at **600 Old Hartford Road, Colchester, CT**, for the purpose of determining whether the emissions from the Proposed SPRINT Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter (μ W/cm2). The number of μ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

<u>General population/uncontrolled exposure</u> limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

General population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter (μ W/cm²). The general population exposure limits for the 850 MHz Band is approximately 567 μ W/cm². The general population exposure limit for the 1900 MHz (PCS) and 2500 MHz (BRS) bands is 1000 μ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



<u>Occupational/controlled exposure</u> limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and can exercise control over the potential for exposure and

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed SPRINT Wireless antenna facility located at **600 Old Hartford Road, Colchester, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since SPRINT is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 1 CDMA channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.
- 2) 2 LTE channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.
- 3) 5 CDMA channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 16 Watts per Channel.
- 4) 2 LTE channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
- 5) 8 LTE channels (2500 MHz (BRS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.



- 6) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 7) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antennas used in this modeling are the RFS APXV9ERR18-C-A20 and the Commscope DT465B-2XR for transmission in the 850 MHz, 1900 MHz (PCS) and 2500 MHz (BRS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 9) The antenna mounting height centerlines of the proposed antennas are **180 feet** above ground level (AGL) for **Sector A**, **180 feet** above ground level (AGL) for **Sector B** and **180 feet** above ground level (AGL) for **Sector C**.
- 10) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general population threshold limits.



SPRINT Site Inventory and Power Data by Antenna

~		~	-	ň	~
Sector:	А	Sector:	В	Sector:	С
Antenna #:	1	Antenna #:	1	Antenna #:	1
	RFS		RFS		RFS
Make / Model:	APXV9ERR18-C-	Make / Model:	APXV9ERR18-C-	Make / Model:	APXV9ERR18-C-
	A20		A20		A20
Gain:	11.9 / 14.9 dBd	Gain:	11.9 / 14.9 dBd	Gain:	11.9 / 14.9 dBd
Height (AGL):	180 feet	Height (AGL):	180 feet	Height (AGL):	180 feet
En and Day 1	850 MHz /	En and a Dan da	850 MHz /	En and Dan da	850 MHz /
Frequency Bands	1900 MHz (PCS)	Frequency Bands	1900 MHz (PCS)	Frequency Bands	1900 MHz (PCS)
Channel Count	10	Channel Count	10	Channel Count	10
Total TX	220 114	Total TX	220 114	Total TX	220 114
Power(W):	220 Watts	Power(W):	220 Watts	Power(W):	220 Watts
ERP (W):	5,873.76	ERP (W):	5,873.76	ERP (W):	5,873.76
Antenna A1	0 50 0/	Antenna B1	0.70.0/	Antenna C1	0.79.9/
MPE%	0.78 %	MPE%	0.78 %	MPE%	0.78 %
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	Commscope	Make / Model:	Commscope	Make / Model:	Commscope
Make / Model:	DT465B-2XR	Make / Model:	DT465B-2XR	Make / Model:	DT465B-2XR
Gain:	15.05 dBd	Gain:	15.05 dBd	Gain:	15.05 dBd
Height (AGL):	180 feet	Height (AGL):	180 feet	Height (AGL):	180 feet
Frequency Bands	2500 MHz (BRS)	Frequency Bands	2500 MHz (BRS)	Frequency Bands	2500 MHz (BRS)
Channel Count	8	Channel Count	8	Channel Count	8
Total TX	160 Watts	Total TX	160 Watts	Total TX	160 Watts
Power(W):	100 watts	Power(W):	100 watts	Power(W):	100 watts
ERP (W):	5,118.23	ERP (W):	5,118.23	ERP (W):	5,118.23
Antenna A2	0.61 %	Antenna B2	0.61 %	Antenna C2	0.61 %
MPE%	0.01 /0	MPE%	0.01 /0	MPE%	0.01 /0

Site Composite MPE%				
Carrier	MPE%			
SPRINT – Max per sector	1.39 %			
CEC	0.31 %			
CSP	0.16 %			
NEC	0.00 %			
OEM	0.12 %			
Omnipoint / T-Mobile	0.16 %			
Site Total MPE %:	2.14 %			

SPRINT Sector A Total:	1.39 %
SPRINT Sector B Total:	1.39 %
SPRINT Sector C Total:	1.39 %
Site Total:	2.14 %

SPRINT _ Frequency Band / Technology Max Power Values (Per Sector)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm²)	Frequency (MHz)	Allowable MPE (µW/cm ²)	Calculated % MPE
Sprint 850 MHz CDMA	1	309.76	180	0.37	850 MHz	567	0.07%
Sprint 850 MHz LTE	2	309.76	180	0.74	850 MHz	567	0.13%
Sprint 1900 MHz (PCS) CDMA	5	494.45	180	2.94	1900 MHz (PCS)	1000	0.29%
Sprint 1900 MHz (PCS) LTE	2	1,236.12	180	2.94	1900 MHz (PCS)	1000	0.29%
Sprint 2500 MHz (BRS) LTE	8	639.78	180	6.08	2500 MHz (BRS)	1000	0.61%
						Total:	1.39%



Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the SPRINT facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

SPRINT Sector	Power Density Value (%)
Sector A:	1.39 %
Sector B:	1.39 %
Sector C:	1.39 %
SPRINT Maximum Total (per sector):	1.39 %
Site Total:	2.14 %
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **2.14 %** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.



FRED A. NUDD CORPORATION

1743 ROUTE 104, BOX 577 ONTARIO, NY 14519 (315) 524-2531 FAX (315) 524-4249

www.nuddtowers.com

Mark LeGault Cordless Data Transfer, Inc. 600 Old Hartford Road Colchester, CT 06415 January 14, 2018

Nudd Job Number: 117-23243.2

Site Location: 600 Old Hartford Road, Colchester, CT 06415, New London County (Latitude and Longitude: 41-35-12, -72-22-40)

Subject: Structural Analysis of an existing 180 ft Guyed Tower

Fred A. Nudd Corporation has completed a three-dimensional, finite element model structural analysis of the above noted guyed tower. This tower was analyzed considered appurtenance loads noted in the appurtenance loading table on the following page. The design loading criteria and strength design are per the TIA/EIA-222-G standard, which is the recommended design standard per the 2012 International Building Code and is the basis of the 2016 Connecticut State Building Code. Tower and foundation dimensions have been taken from original design drawings by Fred A. Nudd Corporation (Drawing Number 00-7265-1 & 00-7265-2, March 10, 2000). Onsite soil conditions were taken from a geotechnical report by Coneco Engineers (dated March 15, 2000). The tower is assumed to be in good, undamaged and equivalent to as new condition and has been maintained / inspected per criteria by TIA-222.

The purpose of this analysis is to determine the structure's ability to support new Sprint equipment installed at a rad center of 180 ft above ground level (AGL). The new equipment to be installed, which includes antennas, and associated hardware are listed on the following page in the appurtenance loading table.

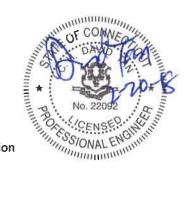
Results of the analysis indicate the tower will be able to the support the design loads noted in the appurtenance loading table on the following page. Specific section design loads, capacities and stress ratios are provided on the following pages. Maximum member usage was found to be 77%.

The tower base foundation and anchors were analyzed considering onsite soil information from the aforementioned geotechnical report. Based on this analysis, the foundation and anchors will be able support the proposed appurtenance loading, in addition to the existing wireless equipment and tower superstructure. Specific design loads, capacities and stress ratios are provided on the following pages.

In conclusion, the tower superstructure and substructure can support the listed existing and proposed appurtenance loading.

We trust this report satisfies your needs. Please contact us with any questions or concerns regarding this report.

Best Regards,



Fred A. Nudd Corporation

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THE R. N.

Code Design Criteria

TIA/EIA-222-G Windspeed = 99 mph, V_{asd} / 128 mph, V_{ult}, 3-Second Gust Radial Ice = 0.75 inch Ice Windspeed = 50 mph, V_{asd}, 3-Second Gust Exposure = B Topographic Category = 1 Structure Class = II Seismic Accelerations are less than 1.0g, thus seismic loading can be ignored

Appurtenance Loading – Existing / Remaining

Height (ft)	Carrier	Appurtenance	Mount	Coax (in)
172	AT&T Mobility	 (6) Powerwave 7770.00 (6) Powerwave LGP21401 (6) Diplexors (1) Powerwave P65-17-XLH-RR (1) KMW AM-X-CD-16-65-00T-RET (1) Andrew SBNH-1D6565C (1) Raycap DC6-48-60-18-8F (6) Ericsson RRUs-11 	(3) 12 ft Boom / Frame	(12) 1-1/4 (1) 1.34 Fiber (2) 0.65 DC
150	T-Mobile	(3) RFS APXV18-206516S-C-A20 (3) Commscope LNX-6515DS-A1M (3) TMA	(3) 12 ft Boom / Frame	(12) 1-5/8

• Height measurement taken as distance from top of base foundation to center of appurtenance.

Appurtenance Loading - Final Configuration for Sprint

Height (ft)	Carrier	Appurtenance	Mount	Coax (in)
180	Sprint	 (3) RFS APXV9ERR18-C-A20 (3) Alcatel Lucent 4x45W, 1900 MHz (3) Alcatel Lucent TD-RRH8x200-25 (6) Alcatel Lucent 2x50, 800 MHz (3) Commscope DT465B-2XR 	(3) 12 ft Boom / Frame	(4) 1-1/4 Hybrid

• Height measurement taken as distance from top of base foundation to center of appurtenance.

• Sprint's proposed coax may be installed on any of tower faces.

Maximum Member Usage

Member	Percentage	
Leg	72	
Diagonal	70	
Horizontal	73	
Bolts	33	
Guys	52	
Anchor Rod	56	

• Percentage less than 100% denote member stress levels are satisfactory for loading

• Percentage greater than 100% indicates member strengthening is required

Foundation Usage

Design Load	Design Load Capacity (kips)		Design Load Capacity (kips) Analysis (kips)		Percentage
Base Axial	216.0	161.6	77		
Anchor Uplift	80.3	32.7	41		
Anchor Shear	78.1	38.3	49		

• Percentage less than 100% denote foundation is satisfactory for loading

• Percentage greater than 100% indicates foundation analysis is required

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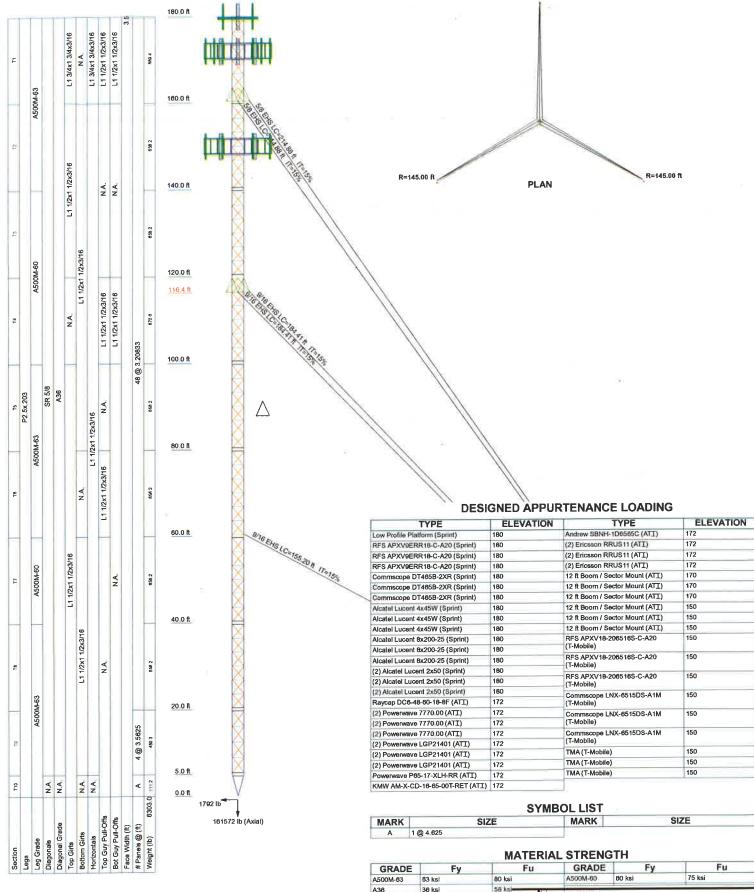
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Fy Fu		GRADE	Fy	Fu	
63 ksi	80 ksi	A500M-60	60 ksi	75 ksi	
36 ksi	58 ksi	light and a second	-		
		Project. Colcheste	r, CT		
		Client: CDT	Drawn by	FAN App'd:	
	1000	Code mus and a	Date: 01/		
		Path:		Dwg N	- E-1
	63 ksi	83 ksi 80 ksi 36 ksi 56 ksi	B3 ksi B0 ksi A500M-60 36 ksi 56 ksi Ideb 117-23243. Project: Colcheste Client: CDT Code: TIA-222-G	B3 ksi B0 ksi A500M-60 B0 ksi 36 ksi 56 ksi 1000 Project Colchester, CT Client: CDT Drawn by Phone: Code: TIA-222-G Date: 01/	B3 ksi B0 ksi A500M-60 B0 ksi 75 ksi 36 ksi 58 ksi 58 ksi 100° T17-23243.2 Project: Colchester, CT Client: CDT Drawn by FAN App'd. Phone: Code: TIA-222-G Date: 01/14/18 Scale:

Ib

R=145.00 ft

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Tower	Input	Data

The main tower is a 3x guyed tower with an overall height of 180.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 3.50 ft at the top and tapered at the base.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in New London County, Connecticut.

Basic wind speed of 99 mph.

Structure Class II.

Exposure Category B.

Topographic Category 1. Crest Height 0.00 ft.

Nominal ice thickness of 0.7500 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

Weld together tower sections have flange connections..

Tension only take-up is 0.0313 in.

Pressures are calculated at each section.

Safety factor used in guy design is 1.

Stress ratio used in tower member design is 1.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

- ✓ Use Code Stress Ratios
 ✓ Use Code Safety Factors Guys Escalate Ice Always Use Max Kz
- Use Special Wind Profile
- ✓ Include Bolts In Member Capacity
- √ Leg Bolts Are At Top Of Section √ Secondary Horizontal Braces Leg
- ✓ Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) Add IBC .6D+W Combination

Distribute Leg Loads As Uniform

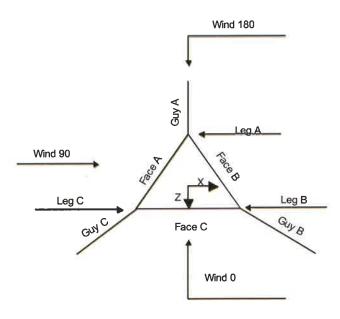
- Assume Legs Pinned
- √ Assume Rigid Index Plate
- Use Clear Spans For Wind Area
- √ Use Clear Spans For KL/r
 √ Retension Guys To Initial Tension
- Bypass Mast Stability Checks
- Use Azimuth Dish Coefficients
- Project Wind Area of Appurt.
 Autocalc Torque Arm Areas
 SR Members Have Cut Ends
- SK Members Have Cut Ends Sort Capacity Reports By Component √ Triangulate Diamond Inner Bracing

Treat Feedline Bundles As Cylinder

- Use ASCE 10 X-Brace Ly Rules √ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression
- ✓ All Leg Panels Have Same Allowable Offset Girt At Foundation
- ✓ Consider Feedline Torque Include Angle Block Shear Check Poles

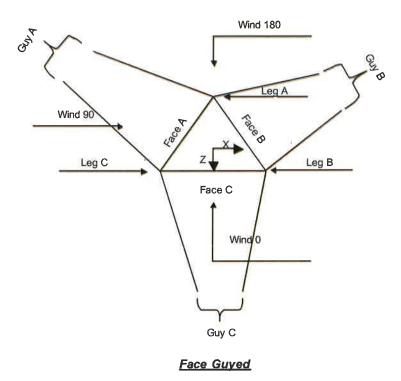
Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets

RISATower	Job 117-23243.2	Page 2 of 45
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Corner & Starmount Guyed Tower

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Tower Section Geometry						
Tower	Tower	Assembly	Description	Section	Number	Section
Section	Elevation	Database	-	Width	of	Length
	Ĥ			Ĥ	Sections	ft
T1	180.00-160.00			3,50	1	20.00
T2	160,00-140,00			3,50	1	20.00
Т3	140.00-120.00			3.50	1	20.00
T4	120.00-100.00			3.50	1	20.00
T5	100.00-80.00			3.50	1	20.00
T6	80.00-60.00			3.50	1	20,00
T7	60.00-40.00			3.50	1	20.00
T8	40.00-20.00			3,50	1	20.00
Т9	20.00-5.00			3,50	1	15.00
T10	5.00-0.00			3.50	1	5.00

RISATower	Job 117-23243.2		Page 4 of 45
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Tower	Tower	Diagonal	Bracing	Has	Has	Top Girt	Bottom Gir
Section	Elevation	Spacing	Туре	K Brace	Horizontals	Offset	Offset
				End			
	ft	ß		Panels		in	in
TI	180.00-160.00	3.21	TX Brace	No	Yes	4.5000	4.5000
T2	160.00-140.00	3.21	TX Brace	No	Yes	4.5000	4,5000
T3	140.00-120.00	3.21	TX Brace	No	Yes	4.5000	4,5000
T4	120.00-100.00	3.21	TX Brace	No	Yes	4.5000	4.5000
T5	100.00-80.00	3.21	TX Brace	No	Yea	4.5000	4,5000
T6	80.00-60.00	3.21	TX Brace	No	Yes	4,5000	4.5000
T7	60.00-40.00	3.21	TX Brace	No	Yes	4,5000	4.5000
T8	40,00-20,00	3.21	TX Brace	No	Yes	4.5000	4.5000
T9	20.00-5,00	3.56	TX Brace	No	Yes	4.5000	4.5000
T10	5.00-0.00	4.63	TX Brace	No	Yes	4,5000	0,0000

		Tower	Section G	eometry (conťd)		
Tower Elevation	Leg	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade	
ft	Туре	5/20	0/442	Type	Dize		
T1 180.00-160.00	Pipe	P2.5x.203	A500M-63 (63 ksi)	Solid Round	5/8	A36 (36 ksi)	
T2 160.00-140.00	Pipe	P2.5x.203	A500M-63 (63 ksi)	Solid Round	5/8	A36 (36 ksi)	
T3 140.00-120.00	Pipe	P2.5x.203	A500M-60 (60 ksi)	Solid Round	5/8	A36 (36 ksi)	
τ4 120 00-100 00	Pipe	P2.5x.203	A500M-60 (60 ksi)	Solid Round	5/8	A36 (36 ksi)	
T5 100.00-80.00	Pipe	P2.5x.203	A500M-63 (63 ksi)	Solid Round	5/8	A36 (36 ksi)	
T6 80.00-60.00	Pipe	P2.5x.203	A500M-63 (63 ksi)	Solid Round	5/8	A36 (36 ksi)	
T7 60.00-40.00	Pipe	P2.5x.203	A500M-60 (60 ksi)	Solid Round	5/8	A36 (36 ksi)	
T8 40.00-20.00	Pipe	P2.5x 203	A500M-63 (63 ksi)	Solid Round	5/8	A36 (36 ksi)	
T9 20.00-5.00	Pipe	P2.5x.203	A500M-63 (63 ksi)	Solid Round	5/8	A36 (36 ksi)	
T10 5.00-0.00	Pipe	P2.5x.203	A500M-63 (63 ksi)	Solid Round	5/8	A36 (36 ksi)	

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 180.00-160.00	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T2 160.00-140.00	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T3 140 00-120 00	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T4 120.00-100.00	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T5 100.00-80.00	Equal Angle	L1 1/2x1 1/2x3/16	A36	Equal Angle	L1 1/2x1 1/2x3/16	A36

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Tower	Top Girt	Top Girt	Top Girt	Bottom Girt	Bottom Girt	Bottom Giri
Elevation ft	Туре	Size	Grade	Туре	Size	Grade
			(36 ksi)			(36 kai)
T6 80.00-60.00	Equal Angle	L1 1/2x1 1/2x3/16	A36	Equal Angle	L1 1/2x1 1/2x3/16	A36
			(36 kai)			(36 kai)
T7 60 00-40 00	Equal Angle	L1 1/2x1 1/2x3/16	A36	Equal Angle	L1 1/2x1 1/2x3/16	A36
			(36 kai)			(36 kai)
T8 40.00-20.00	Equal Angle	L1 1/2x1 1/2x3/16	A36	Equal Angle	L1 1/2x1 1/2x3/16	A36
			(36 ksi)			(36 ksi)
T9 20 00-5 00	Equal Angle	L1 1/2x1 1/2x3/16	A36	Equal Angle	L1 1/2x1 1/2x3/16	A36
			(36 ksi)			(36 ksi)
T10 5.00-0.00	Equal Angle	L1 1/2x1 1/2x3/16	A36	Equal Angle	L1 1/2x1 1/2x3/16	A36
			(36 ksi)			(36 ksi)

Tower	No.	Mid Girt	Mid Girt	Mid Girt	Horizontal	Horizontal	Horizonta
Elevation	of	Type	Size	Grade	Туре	Size	Grade
	Mid						
fi	Girts						
F1 180.00-160.00	None	Flat Bar		A36	Single Angle	L1 3/4x1 3/4x3/16	A36
				(36 ksi)			(36 ksi)
F2 160.00-140.00	None	Flat Bar		A36	Single Angle	L1 1/2x1 1/2x3/16	A36
				(36 ksi)			(36 ksi)
F3 140.00-120.00	None	Flat Bar		A36	Single Angle	L1 1/2x1 1/2x3/16	A36
				(36 ksi)			(36 ksi)
T4 120.00-100.00	None	Flat Bar		A36	Single Angle	L1 1/2x1 1/2x3/16	A36
				(36 ksi)			(36 ksi)
T5 100.00-80.00	None	Flat Bar		A36	Single Angle	L1 1/2x1 1/2x3/16	A36
				(36 ksi)			(36 ksi)
T6 80.00-60.00	None	Flat Bar		A36	Single Angle	L1 1/2x1 1/2x3/16	A36
				(36 ksi)			(36 ksi)
T7 60.00-40.00	None	Flat Bar		A36	Single Angle	L1 1/2x1 1/2x3/16	A36
				(36 ksi)			(36 ksi)
T8 40 00-20.00	None	Flat Bar		A36	Single Angle	L1 1/2x1 1/2x3/16	A36
				(36 ksi)			(36 ksi)
T9 20.00-5.00	None	Flat Bar		A36	Single Angle	L1 1/2x1 1/2x3/16	A36
				(36 ksi)			(36 ksi)
T10 5.00-0.00	None	Flat Bar		A36	Single Angle	L1 1/2x1 1/2x3/16	A36
				(36 ksi)			(36 ksi)

Tower	Gusset	Gusset	Gusset Grade	Adjust. Factor	Adjust.	Weight Mult.	Double Angle	0
Elevation	Area	Thickness		A_f	Factor		Stitch Bolt	Stitch Bolt
	(per face)				A_r		Spacing	Spacing
							Diagonals	Horizontals
ft	ft ²	in					in	in
T1	0.00	0.0000	A36	1	1	1	36,0000	36.0000
180.00-160.00			(36 ksi)					
T2	0.00	0.0000	A36	1	1	1	36,0000	36.0000
160.00-140.00			(36 ksi)					
Т3	0.00	0.0000	A36	1	1	1	36,0000	36.0000

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Tower	Gusset	Gusset	Gusset Grade	Adjust. Factor	Adjust.	Weight Mult.	Double Angle	Double Angle
Elevation	Area	Thickness		A_f	Factor		Stitch Bolt	Stitch Bolt
	(per face)				Ar		Spacing	Spacing
							Diagonals	Horizontals
ft	ft ²	in					în	in
140.00-120.00			(36 ksi)					
T4	0,00	0.0000	A36	1	ı	1	36.0000	36,0000
120.00-100.00			(36 ksi)					
T5	0.00	0.0000	A36	1	1	1	36.0000	36.0000
100.00-80.00			(36 ksi)					
T6 80 00-60 00	0.00	0.0000	A36	1	1	1	36.0000	36,0000
			(36 ksi)					
F7 60 00-40.00	0.00	0.0000	A36	1	1	1	36,0000	36.0000
			(36 ksi)					
18 40.00-20.00	0.00	0.0000	A36	1	1	1	36.0000	36.0000
			(36 ksi)					
T9 20 00-5 00	0.00	0.0000	A36	1	1	1	36.0000	36,0000
			(36 ksi)					
T10 5.00-0.00	0.00	0.0000	A36	1	1	1	36.0000	36.0000
			(36 ksi)					

						K Fa	ctors'			
Tower Elevation	Calc K Single	Calc K Solid	Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz	Inner Brace
	Angles	Rounds		X	x	Х	Х	Х	Х	Х
fi	0			Y	Y	Y	Y	Y	Y	Υ
TI	Yes	Yes	1	1	1	1	1	1	1	1
80,00-160.00				1	1	1	1	1	1	1
T2	Yes	Yes	1	1	1	1	1	1	1	1
60.00-140.00				1	1	1	1	1	1	1
T3	Yes	Yes	1	1	1	1	1	1	1	1
40.00-120.00				1	1	1	1	1	1	1
T4	Yes	Yes	1	1	1	1	1	1	1	1
20.00-100.00				1	1	1	1	1	1	1
T5	Yes	Yes	1	1	1	1	1	1	1	1
100.00-80.00				1	1	1	1	1	1	1
T6	Yes	Yes	1	1	1	1	1	1	1	1
80.00-60.00				1	1	1	1	1	1	1
T7	Yes	Yes	1	1	T	3	1	1	1	1
60.00-40.00				1	1	1	1	1	1	1
T8	Yes	Yes	1	1	1	1	1	1	1	1
40.00-20.00				1	1	1	1	1	1	1
r9 20.00-5.00	Yes	Yes	1	1	1	1	1	1	1	1
				1	1	1	1	1	1	1
r10 5.00-0.00	Yes	Yes	0.33	1	1	1	1	1	1	1

¹Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

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Tower Elevation fl	Leg		Diagon	al	Top G	irt	Bottom	Girt	Mid	Girt	Long Ho	rizontal	Short Ho	orizontal
,	Net Width Deduct In	U	Net Width Deduct IN	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct In	U	Net Width Deduct in	U
TI	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
180.00-160.00														
T2	0.0000	1	0.0000	1	0,0000	1	0,0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
160.00-140.00						620							0.0000	0.54
Т3	0,0000	1	0,0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
140.00-120.00				_					0.0000		0.0000		0.0000	0.74
T4	0,0000	1	0,0000	1	0.0000	3 1	0.0000	L.	0.0000	0.75	0,0000	1	0.0000	0.75
120.00-100.00					0.0000	042	0.0000		0.0000	0.54	0.0000		0.0000	0.96
T5	0.0000	ł	0,0000	1	0,0000	1	0.0000	L.	0,0000	0.75	0.0000	L.	0.0000	0.75
100,00-80.00	0.0000		0.0000		0.0000		0.0000		0.0000	0.74	0.0000		0.0000	0.75
T6 80.00-60.00		1	0.0000		0.0000		0.0000		0.0000	0.75	0.0000			
T7 60.00-40.00	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0,0000	1	0,0000	0.75
T8 40.00-20.00	0.0000	1	0.0000	1	0,0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
T9 20.00-5.00	0.0000	1	0,0000	1	0,0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
T10 5.00-0.00	0.0000	1	0,0000	1	0,0000	1	0,0000	1	0.0000	0.75	0.0000	1	0.0000	0.75

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top G	irt	Bottom	Girt	Mid G	irt	Long Horizontal		Short Horizontal	
2	~ 1	<i>Bolt Size</i> in	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.
TI	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
180.00-160.00	1 tange	A325N	· ·	A325N	v	A325N	Ŷ	A325N	Ŭ	A325N		A325N	°	A325N	100.
T2	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
160.00-140.00	1 101.00	A325N	·	A325N		A325N		A325N		A325N		A325N		A325N	
T3	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
140.00-120.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T4	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
120.00-100.00	U	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T5	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
100.00-80.00	Ũ	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T6 80.00-60.00	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
	-	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
Г7 60.00-40.00	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
	-	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T8 40.00-20.00	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T9 20 00-5.00	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0,6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T10 5.00-0.00	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	

Curv Dete	
Guy Data	

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Guy	Guy	_	Guy	Initial	%	Guy	Guy	L _H	Anchor	Anchor	Anchor	End
Elevation	Grade		Size	Tension		Modulus	Weight		Radius	Azimuth	Elevation	Fitting
							•			Adj.		Efficienc
ft				lb		ksi	plf	ft	ft	0	ft	%
160.375	EHS	A	3/8	6360.00	13%	21000	0.813	214.61	145 00	0.0000	0.00	100%
		B	3/8	6360.00	13%	21000	0.813	214.61	145 00	0.0000	0.00	100%
		C	\$/8	6360.00	15%	21000	0.813	214.61	145.00	0.0000	0.00	100%
116.417	EHS	A	9/16	\$250.00	15%	21000	0.671	184.18	145.00	0.0000	0,00	100%
		B	9/16	5250.00	15%	21000	0.671	184.18	145.00	0,0000	0.00	100%
		C	9/16	\$2\$0.00	15%	21000	0.671	184.18	145.00	0.0000	0.00	100%
60.375	EHS	A	9/16	\$250.00	15%	21000	0.671	155.01	145.00	0.0000	0.00	100%
		B	9/16	\$250,00	15%	21000	0.671	155.01	145.00	0.0000	0.00	100%
		C	9/16	\$250,00	15%	21000	0,671	155.01	145.00	0.0000	0.00	100%

	Guy Data(cont'd)												
Guy Elevation ft	Mount Type	Torque-Arm Spread	Torque-Arm Leg Angle	Torque-Arm Style	Torque-Arm Grade	Torque-Arm Type	Torque-Arm Size						
0.375	Torque Arm	<i>fi</i> 7.00	30,0000	Dog Ear	A36	Single Angle	L2x2x5/16						
6.417	Torque Arm	7.00	30,0000	Dog Ear	(36 ksi) A36 (36 ksi)	Single Angle	L3x3x1/4 L2x2x5/16 L3x3x1/4						
0.375	Corner												

Guy Data	(cont'd))
----------	----------	---

Guy	Diagonal	Diagonal	Upper Diagonal	Lower Diagonal	Is	Pull-Off	Pull-Off Type	Pull-Off Size
Elevation	Grade	Туре	Size	Size	Strap.	Grade		
ft								
160.38	A572-50	Solid Round			No	A36	Equal Angle	L1 1/2x1 1/2x3/16
	(50 ksi)					(36 ksi)		
116.42	A572-50	Solid Round			No	A36	Equal Angle	L1 1/2x1 1/2x3/16
	(50 ksi)					(36 ksi)		
60.38	À572-50	Solid Round			No	A36	Equal Angle	L1 1/2x1 1/2x3/16
	(50 ksi)					(36 ksi)		

	Guy Data (cont'd)										
Guy	Cable	Cable	Cable	Cable	Tower	Tower	Tower	Tower			
Elevation	Weight	Weight	Weight	Weight	Intercept	Intercept	Intercept	Intercept			
	A	В	C	D	A	B	C	D			
	lb	lb	lb	lb			ft				
160.375	174.48	174.48	174.48		2.92	2.92	2.92				
					2.9 sec/pulse	2.9 sec/pulse	2.9 sec/pulse				
116.417	123.58	123,58	123.58		2.15	2.15	2.15				
	0				2.5 sec/pulse	2.5 sec/pulse	2.5 sec/pulse				
60:375	104.01	104.01	104.01		1.53	1.53	1.53				
					2.1 sec/pulse	2.1 sec/pulse	2.1 sec/pulse				

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			_	G	uy Da	ta (col	nťd)	
			Torqu	e Arm	Pul	l Off	Diag	onal
Guy	Calc	Calc	Kx	K _v	K _x	K _v	Kx	K _v
Elevation	K	K						
ft	Single	Solid						
	Angles	Rounds						
160.375	No	No	1	1	0.65	0.65	t	1
116.417	No	No	1	1	0.65	0.65	1	1
60.375	No	No			0.65	0.65	1	1

		Torq	ie-Arm			Pu	ll Off		Diagonal			
Guy Elevation ft	Bolt Size in	Number	Net Width Deduct in	U	<i>Bolt Size</i> ìn	Number	Net Width Deduct in	U	Bolt Size In	Number	Net Width Deduct in	U
160.375	0.7500 A325N	2	0.0000	1	0.6250 A325N	0	0.0000	1	0.6250 A325N	0	0,0000	1
116.417	0.7500 A325N	2	0.0000	1	0.6250 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	1
60. 375	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0,0000	1	0.6250 A325N	0	0,0000	1

Guy Pressures

Guy	Guy	2	q_z	<i>q</i> =	Ice
Elevation	Location			Ice	Thicknes
ft		fi	psf	psf	īn
160,375	Α	80.19	20	5	1,6393
	В	80.19	20	5	1.6393
	С	80.19	20	5	1.6393
116.417	Α	58.21	18	5	1.5876
	В	58.21	18	5	1.5876
	С	58.21	18	5	1.5876
60.375	Α	30.19	15	4	1.4867
	В	30.19	15	4	1:4867
	С	30,19	15	4	1.4867

	Guy-Mast Forces (Excluding Wind) - No Ice									
Guy Elevation	Guy Location	Chord Angle	Guy Tension Top Bottom Ib	F _x	F _y	Fz	M _x	M_y	M _z	
ſi		0		lb	lb	lb	lb-ft	lb-ft	lb-ft	
160.375	А	48 2735	6490.22 6360.00	-104.64	4882.39	-4274.84	-9865.97	15173.38	-17088.36	
	Α	48.2735	6490.22	104,64	4882.39	-4274.84	-9865.97	-15173.38	17088.36	

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Guy	Guy	Chord	Guy Tension	F_{x}	F_{y}	F_{z}	M_{x}	M_{r}	M_{z}
Elevation	Location	Angle	Тор						
			Bottom						
			lb						
ft		0		lb	lb	lb	lb-fi	lb-fi	lb-ft
			6360.00						
	B	48.2735	6490.22	3754.44	4882.39	2046.79	19731.94	15173.38	0.00
			6360.00						
	B	48.2735	6490.22	3649.79	4882,39	2228.04	-9865.97	-15173.38	-17088.3
			6360.00						
	C	48 2735	6490.22	-3649.79	4882.39	2228.04	-9865.97	15173.38	17088.36
			6360.00						
	C	48.2735	6490.22	-3754.44	4882.39	2046 79	19731.94	-15173.38	0.00
			6360.00						
			Sum:	0,00	29294.33	0,00	-0.00	0.00	0,00
116.417	A	39.1448	5328.01	-100.37	3400.60	-4100.44	-6871.68	14554.35	-11902.1
			5250.00						
	Α	39.1448	5328.01	100.37	3400.60	-4100.44	-6871.68	-14554.35	11902 11
			5250.00						
	В	39.1448	5328.01	3601.27	3400.60	1963.29	13743,37	14554.35	0.00
			5250.00						
	B	39.1448	5328.01	3500.89	3400.60	2137.14	-6871.68	-14554.35	-11902.1
			5250.00						
	С	39.1448	5328.01	-3500.89	3400.60	2137.14	-6871.68	14554.35	11902.11
			5250.00						
	С	39,1448	5328.01	-3601,27	3400.60	1963.29	13743.37	-14554.35	0.00
			5250.00						
			Sum:	0.00	20403.61	0.00	-0.00	0.00	0.00
60.375	Α	22,8926	5290.46	0,00	2102.12	-4854.90	-4247.81	0.00	0,00
			5250.00						
	В	22.8926	5290.46	4204_47	2102 12	2427.45	2123.90	0.00	-3678.71
			5250.00						
	С	22,8926	5290,46	-4204.47	2102.12	2427.45	2123,90	-0,00	3678,71
			5250.00						
			Sum:	0.00	6306.36	0.00	0.00	0.00	0.00

		(Guy-Mas	st Forc	es (Exc	luding	Wind) -	Ice	
Guy Elevation	Guy Location	Chord Angle	Guy Tension Top Bottom Ib	F _x	F_y	F.	M _x	$M_{ u}$	<i>M</i> _
ft		0		lb	lb	<i>lb</i>	lb-ft	lb-ft	lb-fi
160.375	Α	48.2735	9780.14 8923.84	-152.09	7551.53	-6213.09	-15259.56	22053,13	-26430 34
	Α	48,2735	9780.14 8923.84	152.09	7551.53	-6213.09	-15259.56	-22053,13	26430,34
	В	48.2735	9780.14 8923.84	5456.74	7551.53	2974.83	30519.13	22053.13	0.00
	В	48 2735	9780,14 8923.84	5304,65	7551.53	3238 26	-15259.56	-22053,13	- 26430.34
	С	48.2735	9780.14 8923.84	-5304.65	7551,53	3238.26	-15259.56	22053.13	26430,34
	С	48.2735	9780 14 8923 84	-5456.74	7551.53	2974.83	30519.13	-22053,13	0,00
			Sum:	0.00	45309.15	0.00	-0.00	0.00	0.00
116.417	А	39,1448	8161.99 7599.28	-149.36	5419.13	-6101.54	-10950.57	21657.20	-18966.95
	Α	39,1448	8161.99	149.36	5419.13	-6101.54	-10950.57	-21657.20	18966.95

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Guy Elevation	Guy Location	Chord Angle	Guy Tension Top	F_{π}	F_y	$\overline{F_z}$	M _x	M_{y}	<i>M</i> -
ft		o	Bottom Ib	lb	lb	lb	lb-ft	lb-ft	lb-ft
			7599 28						
	В	39,1448	8161.99 7599.28	3338.77	3419.13	2921.42	21901.14	21657.20	0.00
	В	39,1448	8161.99 7599.28	5209.41	\$419,13	3180.12	-10950.37	-21657.20	-18966.95
	C	39,1448	8161.99 7599.28	-5209.41	\$419.13	3180.12	-10950.57	21657.20	18966.95
	ç	39.1448	8161.99 7599.28	-5358.77	\$419.13	2921.42	21901.14	-21657.20	0.00
			Sum:	0.00	32514.76	0.00	-0.00	0.00	0,00
60.375	A	22.8926	7815.32	0,00	3328.32	-7071.17	-6725.63	0.00	0.00
	₿	22 8926	7815.32 7550,50	6123.81	3328.32	3535.58	3362.82	0.00	-5824.57
	С	22.8926	7815.32 7550.50	-6123,81	3328,32	3535.58	3362.82	-0.00	5824.57
			Sum:	0.00	9984.97	0.00	0.00	0.00	0,00

_		Gu	y-Mast	orces	(Exclue	ding Wi	ind) - S	ervice	
Guy Nevation	Guy Location	Chord Angle	Guy Tension Top Bottom Ib	F _x	F_y	F.	M _x	$M_{ m y}$	M _z
ſt		0		lb	lb	lb	lb-ft	lb-ft	lb-ft
60.375	А	48.2735	6490.22 6360.00	-104_64	4882.39	-4274.84	-9865.97	15173.38	-17088.36
	А	48.2735	6490.22 6360,00	104.64	4882.39	-4274.84	-9865.97	-15173.38	17088.36
	В	48.2735	6490,22 6360.00	3754,44	4882.39	2046.79	19731.94	15173.38	0,00
	В	48.2735	6490.22 6360.00	3649.79	4882.39	2228.04	-9865.97	-15173.38	-17088.36
	С	48,2735	6490.22 6360.00	-3649.79	4882.39	2228.04	-9865.97	15173,38	17088.36
	С	48.2735	6490 22 6360 00	-3754.44	4882.39	2046.79	19731.94	-15173.38	0.00
			Sum:	0.00	29294.33	0.00	-0.00	0.00	0,00
16.417	A	39.1448	5328.01 5250.00	-100.37	3400.60	-4100.44	-6871.68	14554.35	-11902.11
	A	39 1448	5328.01 5250.00	100.37	3400.60	-4100.44	-6871.68	-14554.35	11902.11
	В	39.1448	5328.01 5250.00	3601.27	3400.60	1963.29	13743.37	14554,35	0.00
	В	39 1448	5328.01 5250.00	3500.89	3400.60	2137.14	-6871.68	-14554.35	-11902.11
	С	39.1448	5328 01 5250 00	-3500.89	3400.60	2137.14	-6871.68	14554.35	11902.11
	С	39 1448	5328.01 5250.00	-3601 27	3400.60	1963.29	13743 37	-14554.35	0.00
			Sum:	0.00	20403.61	0.00	-0.00	0.00	0.00
50.375	Α	22.8926	5290.46 5250.00	0.00	2102.12	-4854.90	-4247.81	0.00	0.00
	В	22,8926	5290.46	4204.47	2102.12	2427.45	2123.90	0.00	-3678.71

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			Sum:	0.00	6306.36	0.00	0,00	0.00	0.00
	C	22,8926	5250.00 5290.46 5250.00	-4204.47	2102.12	2427.45	2123.90	-0,00	3678.7
ft		്റ	lb	lb	lb	lb	lb-fi	lb-ft	lb-ft
Guy Elevation	Guy Location	Chord Angle	Guy Tension Top Bottom	F_x	F_{y}	F_{z}	M _x	M_r	M_z

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Face	Allow Shield	Component Type	Placement	Face Offset	Lateral Offset	#	# Per	Clear Spacing	Width or Diameter	Perimeter	Weight
1000		-)]**	ſt	in	(Frac FW)		Row	in	in	in	plf
A	No	Ar (CaAa)	180.00 - 0.00	0.0000	0.25	4	4	0.5000	1.5500		0,66
B	No	Ar (CaAa)	180.00 - 0.00	0.0000	0.25	1	1	0.5000	0,3750		0.22
Α	No	Ar (CaAa)	172.00 - 0.00	0,0000	-0.25	12	6	0,5000	1.5500		0.66
Α	No	Ar (CaAa)	172.00 - 0.00	0.0000	-0.25	1	1	0,5000	3.0000		0.22
Α	No	Ar (CaAa)	172.00 - 0.00	0.0000	-0.25	1	1	0.5000	0.0000		0.15
Α	No	Ar (CaAa)	172.00 - 0.00	0.0000	-0.25	2	2	0.5000	0,0000		0.10
В	No	Ar (CaAa)	150.00 - 0.00	0.0000	0	12	6	0.5000	1.9800		0.82
	or Leg A B A A A A	orShieldLegNoBNoANoANoANoANoANoANo	orShieldTypeLegANoAr (CaAa)BNoAr (CaAa)ANoAr (CaAa)ANoAr (CaAa)ANoAr (CaAa)ANoAr (CaAa)ANoAr (CaAa)ANoAr (CaAa)	or Shield Type ft A No Ar (CaAa) 180.00 - 0.00 B No Ar (CaAa) 180.00 - 0.00 A No Ar (CaAa) 180.00 - 0.00 A No Ar (CaAa) 172.00 - 0.00	or Shield Type Offset ft Offset in A No Ar (CaAa) 180.00 - 0.00 0.0000 B No Ar (CaAa) 180.00 - 0.00 0.0000 A No Ar (CaAa) 180.00 - 0.00 0.0000 A No Ar (CaAa) 172.00 - 0.00 0.0000	or Shield Type Offset Offset Offset (Frac FW) A No Ar (CaAa) 180.00 - 0.00 0.0000 0.25 B No Ar (CaAa) 180.00 - 0.00 0.0000 0.25 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25	A No Ar (CaAa) 180.00 - 0.00 0.0000 0.25 4 B No Ar (CaAa) 180.00 - 0.00 0.0000 0.25 1 A No Ar (CaAa) 180.00 - 0.00 0.0000 0.25 1 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 12 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1	A No Ar (CaAa) 180.00 - 0.00 0.0000 0.25 4 4 B No Ar (CaAa) 180.00 - 0.00 0.0000 0.25 1 1 A No Ar (CaAa) 180.00 - 0.00 0.0000 0.25 1 1 A No Ar (CaAa) 180.00 - 0.00 0.0000 -0.25 1 1 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 1 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 1 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 1 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 1 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 2 2 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 2 2	Index and or or Eeg Shield TypeTypeIndexiant ft Offset in (Frac FW)Per RowSpacing in inANoAr (CaAa)180.00 - 0.000.00000.25440.5000BNoAr (CaAa)180.00 - 0.000.00000.25110.5000ANoAr (CaAa)172.00 - 0.000.0000-0.251260.5000ANoAr (CaAa)172.00 - 0.000.0000-0.25110.5000ANoAr (CaAa)172.00 - 0.000.0000-0.25110.5000ANoAr (CaAa)172.00 - 0.000.0000-0.25110.5000ANoAr (CaAa)172.00 - 0.000.0000-0.25110.5000ANoAr (CaAa)172.00 - 0.000.0000-0.25220.5000	A No Ar (CaAa) 180.00 - 0.00 0.0000 0.25 4 4 0.5000 1.5500 B No Ar (CaAa) 180.00 - 0.00 0.0000 0.25 1 1 0.5000 1.5500 B No Ar (CaAa) 180.00 - 0.00 0.0000 -0.25 1 1 0.5000 0.3750 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 1 0.5000 0.3750 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 1 0.5000 3.0000 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 1 0.5000 3.0000 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 1 0.5000 3.0000 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 1 1 0.5000 0.0000 A No Ar (CaAa) 172.00 - 0.00 0.0000 -0.25 2 2 0.5000 <	Index <th< td=""></th<>

Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	A_R	A_F	C _A A _A In Face	$C_A A_A$ Out Face	Weight
Section	Elevation ft		ft ²	ft²	ft ²	f	lb
T1	180.00-160.00	Α	0.000	0.000	38.320	0.000	154.68
		в	0.000	0.000	0.750	0.000	4.40
		С	0.000	0.000	0.000	0.000	0.00
T2	160.00-140.00	Α	0.000	0.000	55.600	0.000	222.60
		в	0.000	0.000	24.510	0.000	102.80
		С	0.000	0.000	0.000	0.000	0.00
T3	140.00-120.00	Α	0.000	0.000	55.600	0.000	222.60
		в	0.000	0.000	48.270	0.000	201.20
		С	0.000	0.000	0.000	0.000	0.00
T4	120.00-100.00	Α	0.000	0.000	55.600	0.000	222.60
		В	0.000	0.000	48.270	0.000	201.20
		С	0.000	0.000	0.000	0.000	0.00
T5	100.00-80.00	Α	0.000	0.000	55,600	0.000	222.60
		В	0.000	0.000	48.270	0.000	201.20
		С	0.000	0.000	0.000	0.000	0.00
Т6	80.00-60.00	Ā	0.000	0.000	55.600	0.000	222.60
		В	0.000	0.000	48.270	0.000	201.20
		c	0.000	0.000	0.000	0.000	0,00

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Tower Section	Tower Elevation	Face	A_R	A_F	C _A A _A In Face	C _A A _A Out Face	Weight
	ft		ft²	fr²	ft ²	ft²	lb
T7	60.00-40.00	A	0.000	0.000	33 600	0.000	222.60
		B	0.000	0.000	48.270	0.000	201.20
		C	0.000	0.000	0.000	0.000	0.00
T8	40.00-20.00	A	0.000	0.000	\$5,600	0.000	222.60
		B	0.000	0.000	48.270	0.000	201.20
		C	0.000	0.000	0.000	0.000	0.00
T9	20.00-5.00	A	0.000	0.000	41.700	0.000	166.95
		B	0.000	0.000	36.203	0.000	150.90
		C	0.000	0.000	0.000	0.000	0.00
T10	5.00-0.00	A	0,000	0.000	13.900	0,000	55.65
		B	0.000	0,000	12.068	0.000	50.30
		C	0,000	0.000	0,000	0.000	0.00

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation	or	Thickness	-	.1	In Face	Out Face	
	ſI	Leg	in	ft²	ft²	ft²	ft²	lb
T1	180.00-160.00	Α	1.767	0,000	0.000	76.326	0.000	1099 41
		В		0.000	0.000	7.819	0.000	96.90
		С		0.000	0.000	0.000	0.000	0,00
T2	160.00-140.00	Α	1.745	0.000	0.000	106.035	0.000	1549.68
		В		0.000	0.000	32.180	0.000	577.07
		С		0.000	0.000	0.000	0.000	0.00
T3	140.00-120.00	Α	1.720	0.000	0.000	105.333	0.000	1528,18
		В		0.000	0.000	56.366	0.000	1047.97
		С		0.000	0.000	0.000	0.000	0.00
T4	120.00-100.00	Α	1.692	0.000	0.000	104.527	0.000	1503.64
		В		0.000	0.000	56.064	0.000	1034.98
		С		0.000	0.000	0.000	0.000	0,00
T5	100.00-80.00	Α	1,658	0.000	0.000	103.576	0.000	1474.97
		В		0.000	0.000	55.708	0.000	1019.76
		С		0.000	0.000	0.000	0.000	0.00
T6	80.00-60.00	Α	1.617	0.000	0.000	102,413	0.000	1440.23
		В		0.000	0.000	55.272	0.000	1001.28
		С		0.000	0.000	0.000	0.000	0.00
T7	60.00-40.00	Α	1.564	0.000	0.000	100.901	0.000	1395.69
		В		0.000	0.000	54.706	0.000	977.49
		С		0.000	0.000	0.000	0.000	0.00
T8	40.00-20.00	Α	1.486	0.000	0.000	98.703	0.000	1332.12
		В		0.000	0.000	53.883	0.000	943.38
		С		0.000	0.000	0.000	0.000	0.00
Т9	20.00-5.00	A	1.361	0.000	0.000	71.396	0.000	925.14
		В		0.000	0.000	39.426	0.000	667.55
		С		0.000	0.000	0.000	0.000	0.00
T10	5.00-0.00	Α	1.159	0.000	0.000	22.377	0.000	270.35
		В		0.000	0.000	12.609	0.000	201.66
		С		0.000	0.000	0.000	0.000	0.00

		Feed Line Center of Pressure							
Section	Elevation	CP _X	CPz	CP_X	CPz				
	Ĥ	in	in	Ice in	Ice in				

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Section	Elevation	CP_X	CP _Z	CP_X	CP _Z
				Ice	Ice
	ft	in	in	in	in
TI	180.00-160.00	-2.0049	-0.6642	-0 6459	-0.2145
T2	160 00-140 00	-1.3186	-0.7060	-0 6322	-0 2490
T3	140 00-120 00	-0.5458	-0.8960	-0.3923	-0.3476
T4	120.00-100.00	-0.5458	-0.8960	-0.3957	-0.3535
T5	100.00-80.00	-0.5458	-0.8960	-0,3998	-0.3605
T6	80.00-60.00	-0.5458	-0.8960	-0.4047	-0.3691
T7	60.00-40.00	-0.5458	-0.8960	-0.4111	-0.3806
T8	40.00-20.00	-0.5458	-0.8960	-0.4202	-0.3978
T9	20.00-5.00	-0.5479	-0.8994	-0.4441	-0.4356
T10	5.00-0.00	-0.4194	-1.2089	-0.4254	-0,7576

Shielding Factor Ka

Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.		Segment Elev.	No Ice	Ice
T1	1	LDF6-50A (1-1/4 FOAM)	160.00 -	0.6000	0.3843
			180.00	0.0000	0.004
T۱	2	Safety Line 3/8	160.00 -	0.6000	0.3843
	3	LDF6-50A (1-1/4 FOAM)	180.00 160.00 -	0.6000	0.3843
T1	3	LDF6-30A (1-1/4 FOAM)	172.00	0.0000	0.364.
TI	4	3 in Conduit	160.00 -	0.6000	0.3843
	· · · · ·		172.00		
TI	5	1,34 in Fiber	160.00 -	0.6000	0,3843
			172.00		
TI	6	0.65 DC	160.00 -	0,6000	0.384
			172.00		
T2	1	LDF6-50A (1-1/4 FOAM)		0.6000	0,393
			160.00	0.0000	0.202
T2	2	Safety Line 3/8	140.00 -	0.6000	0.393
T2	3	LDF6-50A (1-1/4 FOAM)	160.00 140.00 -	0.6000	0.393
12	2	LDF6-30A (1-1/4 FOAM)	160.00	0.0000	0,393
T2	4	3 in Conduit	140.00 -	0,6000	0.393
1.2	· · ·	5 III Conduit	160.00	0.0000	0.050
T2	5	1.34 in Fiber	140.00 -	0.6000	0.393
	-		160.00		
T2	6	0.65 DC	140.00 -	0.6000	0.393
1			160.00		
T2	7	LDF7-50A (1-5/8 FOAM)	140.00 -	0.6000	0.393
			150.00	0.0000	0.000
T3	1	LDF6-50A (1-1/4 FOAM)	120.00 -	0.6000	0.398
T 2		C-C-t-Line 2/0	140.00 120.00 -	0.6000	0.398
T3	2	Safety Line 3/8	140.00	0.0000	0.396
Т3	3	LDF6-50A (1-1/4 FOAM)	120.00 -	0.6000	0.398
13	5	EDI 0-307 (1-114 07M)	140.00	0.0000	0.570
Т3	4	3 in Conduit		0.6000	0.398
			140.00		
Т3	5	1.34 in Fiber	120.00 -	0.6000	0.398
			140.00	100000000000000000000000000000000000000	
Т3	6	0.65 DC	120.00 -	0.6000	0.398
			140.00	0 (000	0.000
T3	7	LDF7-50A (1-5/8 FOAM)	120.00 -	0.6000	0,398
1		ļ	140.00		

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Tower	Feed Line	Description	Feed Line Ka		Ka
Section	Record No.	Description	Segment Elev.	No Ice	Ice
T4		1 DE4 204 /1 1/1 E(1416)	100.00 -	0.6000	0.4047
14	1	LDF6-50A (1-1/4 FOAM)	1 1 200 201 11 1000	0.0000	0.4047
		D-C-t- Line 2/0	120.00	0.6000	0.4047
T4	2	Safety Line 3/8	100.00 -	0.6000	0.4047
			120.00		0.10.15
T4	3	LDF6-50A (1-1/4 FOAM)	100.00 -	0,6000	0.4047
			120.00		
T4	4	3 in Conduit	100.00 -	0.6000	0,4047
			120.00		
T4	5	1.34 in Fiber	100.00 -	0,6000	0,4047
			120.00		
T4	6	0.65 DC	100.00 -	0,6000	0.4047
			120.00		_
T4	7	LDF7-50A (1-5/8 FOAM)	100,00 -	0,6000	0,4047
			120.00		
T5	1	LDF6-50A (1-1/4 FOAM)	80.00 - 100.00	0.6000	0.4119
T5	2	Safety Line 3/8	80.00 - 100.00	0.6000	0.4119
T5	3	LDF6-50A (1-1/4 FOAM)		0.6000	0.4119
T5	4		80.00 - 100.00	0.6000	0.4119
T5	5		80.00 - 100.00	0.6000	0.4119
TS	6	0.65 DC		0.6000	0,4119
TS	7	LDF7-50A (1-5/8 FOAM)		0.6000	0.4119
Т6	1	LDF6-50A (1-1/4 FOAM)	60.00 - 80.00	0.6000	0.4208
T6	2	Safety Line 3/8	60.00 - 80.00	0.6000	0.4208
T6	3	LDF6-50A (1-1/4 FOAM)	60.00 - 80.00	0.6000	0.4208
T6	4	3 in Conduit	60.00 - 80.00	0.6000	0.4208
T6	5	1.34 in Fiber		0.6000	0.4208
T6	6	0.65 DC	60.00 - 80.00	0.6000	0.4208
T6	7	LDF7-50A (1-5/8 FOAM)	60.00 - 80.00	0.6000	0.4208
T7	1	LDF6-50A (1-1/4 FOAM)	40.00 - 60.00	0.6000	0.4208
T7		Safety Line 3/8	40.00 - 60.00	0.6000	0.4325
T7	23	LDF6-50A (1-1/4 FOAM)	40.00 - 60.00	0.6000	0.4325
T7	3		40.00 - 60.00	0.6000	0.4325
	45	3 in Conduit		0.6000	0.4325
T7		1.34 in Fiber	40.00 - 60.00		Para ta Secondaria
T7	6	0.65 DC	40.00 - 60.00	0.6000	0.4325
T7	7	LDF7-50A (1-5/8 FOAM)	40.00 - 60.00	0.6000	0.4325
T8	1	LDF6-50A (1-1/4 FOAM)	20.00 - 40.00	0.6000	0.4495
T8	2	Safety Line 3/8	20.00 - 40.00	0.6000	0.4495
T8	3	LDF6-50A (1-1/4 FOAM)	20.00 - 40.00	0.6000	0.4495
T8	4	3 in Conduit	20.00 - 40.00	0.6000	0.4495
T8	5	1.34 in Fiber	20.00 - 40.00	0.6000	0.4495
T8	6	0.65 DC	20.00 - 40.00	0.6000	0.4495
T8	7	LDF7-50A (1-5/8 FOAM)	20.00 - 40.00	0.6000	0.4495
T9	1	LDF6-50A (1-1/4 FOAM)	5.00 - 20.00	0.6000	0.4939
Т9	2	Safety Line 3/8	5.00 - 20.00	0.6000	0.4939
Т9	3	LDF6-50A (1-1/4 FOAM)	5.00 - 20.00	0.6000	0.4939
Т9	4	3 in Conduit	5.00 - 20.00	0.6000	0.4939
Т9	5	1.34 in Fiber	5.00 - 20.00	0.6000	0.4939
Т9	6	0.65 DC	5.00 - 20.00	0.6000	0.4939
Т9	7	LDF7-50A (1-5/8 FOAM)	5.00 - 20.00	0.6000	0.4939
T10	1	LDF6-50A (1-1/4 FOAM)	0.00 - 5.00	0.6000	0.4910
T10	2	Safety Line 3/8	0.00 - 5.00	0.6000	0.4910
T10	3	LDF6-50A (1-1/4 FOAM)	0.00 - 5.00	0,6000	0.4910
T10	4	3 in Conduit	0.00 - 5.00	0.6000	0.4910
T10	5	1,34 in Fiber	0.00 - 5.00	0.6000	0.4910
T10	6	0.65 DC	0.00 - 5.00	0.6000	0.4910
T10	7	LDF7-50A (1-5/8 FOAM)	0.00 - 5.00	0.6000	0.4910

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Discrete Tower Loads									
	E	08	Official	Animuth	Placement		$C_A A_A$	$C_A A_A$	Weight
Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		Front	Side	weigni
			ft ft ft	o	ft		ft²	ft²	lb
Low Profile Platform (Sprint)	A	None		0.0000	180.00	No Ice 1/2" Ice 1" Ice	26.30 35.60 44.90	26.30 35.60 44.90	1950.00 2340.00 2730.00
ft Boom / Sector Mount (AT&T)	A	From Leg	0,00 0,00 0,00	0,0000	170.00	No Ice 1/2" Ice 1" Ice	17,50 22,50 28,00	8.50 11.00 14.00	450.00 700.00 900.00
tl Boom / Sector Mount (AT&T)	B	From Leg	0,00 0,00 0,00	0,0000	170.00	No Ice 1/2" Ice 1" Ice	17,50 22,50 28,00	8.50 11.00 14.00	450.00 700.00 900.00
ft Boom / Sector Mount (AT&T)	C	From Leg	0,00 0,00 0,00	0.0000	170.00	No Ice 1/2" Ice 1" Ice	17.50 22.50 28,00	8.50 11.00 14.00	450.00 700.00 900.00
) Powerwave 7770.00 (AT&T)	A	From Leg	3.00 0.00 0.00	0.0000	172.00	No Ice 1/2" Ice 1" Ice	5,88 6.25 6.64	2,93 3,29 3,64	35.00 67.60 105.10
) Powerwave 7770.00 (AT&T)	В	From Leg	3.00 0.00 0.00	0.0000	172.00	No Ice 1/2" Ice 1" Ice	5,88 6,25 6,64	2.93 3.29 3.64	35.00 67.60 105.10
) Powerwave 7770.00 (AT&T)	С	From Leg	3.00 0.00 0.00	0.0000	172,00	No Ice 1/2" Ice 1" Ice	5.88 6.25 6.64	2,93 3,29 3,64	35.00 67.60 105.10
Powerwave LGP21401 (AT&T)	A	From Leg	3.00 0.00 0.00	0.0000	172.00	No Ice 1/2" Ice 1" Ice	1.95 2.11 2.28	0.53 0.63 0.75	31.00 42.00 55,30
Powerwave LGP21401 (AT&T)	В	From Leg	3.00 0.00 0.00	0.0000	172.00	No Ice 1/2" Ice 1" Ice	1.95 2.11 2.28	0.53 0.63 0.75	31.00 42.00 55,30
Powerwave LGP21401 (AT&T)	С	From Leg	3.00 0.00 0.00	0.0000	172.00	No Ice 1/2" Ice 1" Ice	1.95 2.11 2.28	0.53 0.63 0.75	31,00 42.00 55,30
erwave P65-17-XLH-RR (AT&T)	A	From Leg	3.00 0.00 0.00	0.0000	172.00	No Ice 1/2" Ice 1" Ice	11.47 12.08 12.69	4,00 4,68 5,32	62.00 124.10 193.70
KMW -X-CD-16-65-00T-RET (AT&T)	В	From Leg	3.00 0.00 0.00	0.0000	172.00	No Ice 1/2" Ice 1" Ice	8.26 8.73 9.21	4,64 5.12 5.59	48.50 95.00 147.50
drew SBNH-1D6565C (AT&T)	С	From Leg	3.00 0.00 0.00	0.0000	1 72 .00	No Ice 1/2" Ice 1" Ice	11.41 12.03 12.64	7.70 8.36 9.00	60.90 126.60 199.90
2) Ericsson RRUS11 (AT&T)	A	From Leg	3.00 0.00 0.00	0.0000	172.00	No Ice 1/2" Ice 1" Ice	2.99 3.19 3.41	1.25 1.39 1.55	55.00 74.60 97.10
2) Ericsson RRUS11 (AT&T)	В	From Leg	3 00 0 00 0.00	0.0000	172.00	No lce 1/2" lce 1" lce	2.99 3.19 3.41	1.25 1.39 1.55	55.00 74.60 97.10
2) Ericsson RRUS11 (AT&T)	С	From Leg	3.00 0.00 0.00	0.0000	172.00	No Ice 1/2" Ice 1" Ice	2.99 3.19 3.41	1.25 1.39 1.55	55,00 74.60 97,10
ycap DC6-48-60-18-8F (AT&T)	А	From Leg	3.00 0.00	0.0000	172.00	No Ice 1/2" Ice 1" Ice	1 47 1 67 1.88	1.47 1.67 1.88	31.80 54.40 80.10
ft Boom / Sector Mount	А	From Leg	0.00 0.00	0.0000	150.00	No Ice	1.88	8.50	450.00

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weight
			Vert fl fl fl	o	ft		ft²	ft²	lb
(AT&T)			0.00			1/2" Ice 1" Ice	22.50 28.00	11.00 14.00	700.00 900.00
12 ft Boom / Sector Mount (AT&T)	В	From Leg	0.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	17.50 22.50 28.00	8.50 11.00 14.00	450.00 700.00 900.00
12 ft Boom / Sector Mount (AT&T)	C	From Leg	0.00 0.00 0.00	0,0000	150,00	No Ice 1/2" Ice 1" Ice	17,50 22,50 28,00	8.50 11,00 14.00	450,00 700.00 900.00
RFS APXV18-206516S-C-A20 (T-Mabile)	A	From Log	3,00 0,00 0,00	0,0000	150.00	No Ice 1/2" Ice 1" Ice	3.62 4.29 4.97	2.01 2.72 3.38	18.70 63.10 125.50
RFS APXV18-206516S-C-A20 (T-Mobile)	В	From Leg	3.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	3.62 4.29 4.97	2.01 2.72 3.38	18.70 63.10 125.50
RFS APXV18-206516S-C-A20 (T-Mobile)	С	From Leg	3.00 0.00 0.00	0.0000	150,00	No Ice 1/2" Ice 1" Ice	3.62 4.29 4.97	2.01 2.72 3.38	18.70 63.10 125.50
Commscope LNX-6515DS-A1M (T-Mobile)	A	From Leg	3,00 0,00 0,00	0,0000	150.00	No Ice 1/2" Ice 1" Ice	11.45 12.67 13,89	7.70 8.99 10.22	50.30 189.70 360.60
Commscope LNX-6515DS-A1M	В	From Leg	3.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	11.45 12.67 13.89	7.70 8.99 10.22	50.30 189.70 360.60
(T-Mobile) Commscope LNX-6515DS-A1M	С	From Leg	3.00 0,00 0,00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	11.45 12.67 13.89	7.70 8,99 10,22	50.00 50.30 189.70 360.60
(T-Mobile) TMA (T-Mobile)	А	From Leg	3.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	2.06 2.39 2.75	0.50 0.72 0.97	22.00 49.80 88,20
TMA (T-Mobile)	В	From Leg	3.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	2.75 2.06 2.39 2.75	0.50 0.72 0.97	22.00 49.80 88.20
TMA (T-Mobile)	С	From Leg	3.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	2.06 2.39 2.75	0.50 0.72 0.97	22.00 49.80 88.20
RFS APXV9ERR18-C-A20 (Sprint)	Α	From Leg	3.00 0.00 0.00	0.0000	180.00	No Ice 1/2" Ice 1" Ice	8.02 8.48 8.93	5.81 6.27 6.73	62.00 114.00 172.10
RFS APXV9ERR18-C-A20 (Sprint)	В	From Leg	3.00 0.00 0.00	0.0000	180.00	No Ice 1/2" Ice 1" Ice	8.02 8.48 8.93	5.81 6.27 6.73	62.00 114.00 172.10
RFS APXV9ERR18-C-A20 (Sprint)	С	From Leg	3.00 0.00	0.0000	180.00	No Ice 1/2" Ice 1" Ice	8.02 8.48 8.93	5.81 6.27 6.73	62.00 114.00 172.10
Commscope DT465B-2XR (Sprint)	Α	From Leg	0.00 3.00 0.00 0.00	0.0000	180.00	No Ice 1/2" Ice 1" Ice	9.22 9.68 10.14	5.87 6.33 6.79	50.00 108.00 172.40
Commscope DT465B-2XR (Sprint)	Α	From Leg	3.00 0.00 0.00	0.0000	180.00	No Ice 1/2" Ice 1" Ice	9.22 9.68 10.14	5.87 6.33 6.79	50.00 108.00 172.40
Commscope DT465B-2XR (Sprint)	В	From Leg	3.00 0.00 0.00	0,0000	180.00	No Ice 1/2" Ice 1" Ice	9 22 9 68 10 14	5.87 6.33 6.79	50.00 108.00 172.40
Alcatel Lucent 4x45W (Sprint)	А	From Leg	3.00 0.00 0.00	0.0000	180,00	No Ice 1/2" Ice 1" Ice	2.54 2.72 2.92	1.61 1.78 1.96	51.00 71.10 94.30
Alcatel Lucent 4x45W	В	From Leg	3.00	0.0000	180.00	No Ice	2.54	1.61	51.00

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C _A A _A Front	C ₄ A ₄ Side	Weight
			Vert ft ft	o	ft		ft²	ft²	lb
(Sprint)			ft 0.00			1/2" Ice	2.72	1.78	71.10
(coparing)			0.00			1" lce	2,92	1.96	94.30
Alcatel Lucent 4x45W	С	From Leg	3.00	0.0000	180.00	No Ice	2.54	1.61	\$1.00
(Sprint)			0.00			1/2" Ice	2.72	1.78	71.10
			0.00			1" Ice	2.92	1.96	94.30
Alcatel Lucent 8x200-25	А	From Log	3.00	0.0000	180.00	No Ice	4.05	1,53	70.00
(Sprint)			0.00			1/2" Ice	4.27	1.70	97.10
(0.00			1" Ice	4,50	1,88	127.80
Alcatel Lucent 8x200-25	B	From Leg	3.00	0,0000	180.00	No lce	4,05	1.53	70,00
(Sprint)			0.00			1/2" Ice	4.27	1.70	97.10
			0.00			1" Ice	4.50	1.88	127.80
Alcatel Lucent 8x200-25	С	From Leg	3.00	0.0000	180.00	No Ice	4.05	1.53	70,00
(Sprint)			0.00			1/2" Ice	4.27	1.70	97,10
			0.00			1" Ice	4.50	1.88	127.80
(2) Alcatel Lucent 2x50	Α	From Leg	3.00	0,0000	180.00	No Ice	2.27	1.35	42.00
(Sprint)			0.00			1/2" Ice	2.45	1.51	59.30
(0.00			1" lce	2.64	1.68	79.60
(2) Alcatel Lucent 2x50	В	From Leg	3.00	0.0000	180.00	No lce	2.27	1.35	42.00
(Sprint)		•	0,00			1/2" Ice	2.45	1.51	59.30
			0.00			1" Ice	2.64	1.68	79.60
(2) Alcatel Lucent 2x50	С	From Leg	3.00	0.0000	180.00	No Ice	2.27	1.35	42.00
(Sprint)			0.00			1/2" Ice	2.45	1.51	59.30
			0.00			I" Ice	2.64	1.68	79.60

Tower Pressures - No Ice

 $G_H = 0.850$

Section	Z	Kz	q_z	A_G	F	A_F	A_R	Aleg	Leg	$C_A A_A$	C_{AA}
Elevation	1	- C			а				%	In	Out
					с					Face	Face
ft	ſl		psf	ft ²	е	ft ²	ft^2	ft^2		_ft ²	ft ²
T1	170.00	1.15	25	74.792	Α	3.192	12.348	9.583	61.67	38.320	0.000
180.00-160.00	100000000	17			B	3.192	12.348		61.67	0.750	0.000
					C	3.192	12.348		61.67	0.000	0.000
T2	150.00	1:11	24	74,792	A	2.853	12.348	9,583	63.05	55.600	0.000
160.00-140.00	(5				B	2.853	12.348		63.05	24.510	0.000
100,00 11000					C	2.853	12.348		63.05	0.000	0.000
Т3	130.00	1.065	23	74,792	A	2.853	12,348	9.583	63.05	55.600	0.000
140.00-120.00					B	2.853	12,348		63.05	48.270	0.000
1,0.00 120.00					C	2.853	12.348		63.05	0.000	0.000
T4	110.00	1.016	22	74,792	A	2.853	12.348	9.583	63.05	55,600	0.000
120.00-100.00					В	2.853	12.348		63.05	48.270	0.000
120.00 100.00					С	2.853	12.348		63,05	0.000	0,000
T5	90.00	0.959	20	74,792	Α	2.853	12.348	9.583	63.05	55.600	0.000
100,00-80.00					В	2 853	12.348		63.05	48.270	0.000
100,00 00,00					C	2.853	12.348		63.05	0.000	0.000
T6 80,00-60,00	70.00	0.892	19	74,792	A	2,853	12.348	9.583	63.05	55.600	0,000
10 00 00-00 00	10.00				В	2 853	12.348		63,05	48.270	0.000
					C	2.853	12.348		63.05	0.000	0.000

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Section Elevation	Ζ	Kz	qz	A _G	F a	Â _F	A_R	A _{leg}	Leg %	$C_A A_A$ In	C _A A _A Out
Licraiion				a i	с	22				Face	Face
ſĨ	ft		psf	ft ²	е	ft ²	f^2	ft^2		ft ²	ſŕ
T7 60.00-40.00	\$0.00	0.811	17	74.792	A	2.853	12.348	9.583	63.03	55.600	0.000
	= 0 =	9.5		N-680-55	B	2.853	12.348		63.03	48.270	0.000
					C	2 853	12,348		63,03	0.000	0.000
T8 40.00-20.00	30.00	0.701	13	74.792	A	2.853	12,348	9,583	63.05	\$\$,600	0,000
					B	2.853	12,348		63.05	48.270	0.000
					C	2.853	12.348		63.05	0.000	0,000
T9 20.00-5.00	12.50	0.7	15	\$6.094	A	2.038	9,126	7.188	64.38	41.700	0.000
					B	2.038	9.126		64.38	36.203	0.000
					C	2,038	9,126		64.38	0,000	0,000
T10 5.00-0.00	2,50	0.7	15	10,019	A	0.375	2.584	2,584	87.33	13,900	0,000
					B	0.375	2,584		87,33	12.068	0.000
					C	0.375	2.584		87.33	0.000	0.000

Tower Pressure - With Ice

$G_H = 0.850$

Section	Z	Kz	q_z	1 _Z	AG	F	AF	A_R	Aleg	Leg	$C_A A_A$	$C_A A_A$
Elevation						a				%	In	Out
						С					Face	Face
ft	ft		psf	in	fr ²	е	ft ²	ft ²	ft ²		ft ²	fî ²
T1	170.00	1.15	6	1.7672	80.682	Α	3.192	46.484	21.365	43.01	76.326	0.000
180.00-160.00				Ū.		В	3.192	46.484		43.01	7.819	0.000
						С	3.192	46.484		43.01	0.000	0.000
T2	150.00	1.11	6	1.7452	80.609	Α	2.853	46.059	21.218	43.38	106.035	0,000
160.00-140.00						В	2.853	46.059		43.38	32,180	0.000
						С	2.853	46.059		43.38	0.000	0.000
T3	130.00	1.065	6	1.7204	80.526	Α	2.853	45.580	21.053	43.47	105.333	0.000
140.00-120.00						В	2.853	45.580		43.47	56.366	0.000
						C	2.853	45.580		43.47	0.000	0.000
T4	110.00	1.016	6	1.6919	80.431	Α	2.853	45,030	20.863	43.57	104.527	0.000
120.00-100.00		N3				В	2.853	45,030		43.57	56.064	0.000
						С	2.853	45,030		43.57	0.000	0.000
T5 100.00-80.00	90,00	0.959	5	1.6583	80.319	A	2.853	44.380	20.639	43.70	103.576	0.000
						В	2.853	44,380		43.70	55.708	0.000
						С	2.853	44,380		43.70	0.000	0.000
T6 80.00-60.00	70.00	0.892	5	1.6171	80.182	Α	2.853	43.585	20.364	43.85	102,413	0.000
						В	2.853	43.585		43.85	55.272	0.000
		· · · · ·				С	2.853	43.585		43.85	0.000	0.000
T7 60.00-40.00	50.00	0.811	4	1.5636	80.004	Α	2.853	42.552	20.008	44.07	100.901	0.000
						В	2.853	42.552		44.07	54.706	0.000
						С	2.853	42.552		44.07	0.000	0.000
T8 40.00-20.00	30.00	0,701	4	1.4858	79.744	Α	2.853	41.048	19.488	44.39	98.703	0.000
						в	2.853	41.048		44.39	53.883	0.000
						С	2.853	41.048		44.39	0.000	0.000
T9 20.00-5.00	12.50	0.7	4	1.3612	59,497	A	2.038	28.074	13.994	46.47	71.396	0.000
				12	3	В	2.038	28,074		46.47	39.426	0.000
						C	2.038	28.074		46.47	0.000	0.000
T10 5.00-0.00	2,50	0.7	4	1.1589	11.042	Ă	0.375	5.246	4.667	83.03	22,377	0.000
		• • •				В	0.375	5,246		83.03	12,609	0.000
						Ĉ	0.375	5.246		83.03	0.000	0.000
						-						_

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Т	ower	Press	ure -	Servi	се

Section	Z	Kz	q_z	AG	F	A_F	A_R	Aleg	Leg	CAAA	$C_A A_A$
Elevation	-		74	0	a				%	In	Out
Dicvation					с					Face	Face
A	ft	2	psf	ft ²	е	ft ²	ft ²	ft ²		ft ²	ft ²
T1	170.00	1.13	9	74,792	A	3.192	12,348	9.583	61.67	38.320	0,000
180 00-160 00					B	3.192	12.348		61.67	0.750	0.000
					C	3 192	12.348		61.67	0.000	0.000
T2	150.00	-1.11	9	74, 792	A	2 853	12.348	9,583	63.05	\$\$,600	0.000
160.00-140.00					B	2.853	12,348		63.05	24,510	0.000
					C	2.853	12.348		63.05	0,000	0.000
T3	130.00	1.065	8	74,792		2.853	12.348	9.583	63.05	\$\$,600	0,000
140.00-120.00					B	2 853	12.348		63.05	48.270	0.000
					C	2.853	12.348		63.05	0.000	0.000
T4	110.00	1.016	8	74,792	A	2.853	12.348	9.583	63.05	55,600	0.000
120.00-100.00					B	2.853	12.348		63.05	48.270	0.000
		-			С	2.853	12.348	1	63.05	0,000	0,000
T5	90.00	0.959	8	74.792	A	2.853	12.348	9,583	63.05	55,600	0,000
100.00-80.00	,				B	2.853	12.348		63.05	48.270	0,000
					С	2,853	12.348		63.05	0,000	0,000
T6 80.00-60.00	70.00	0.892	7	74,792	A	2.853	12.348	9.583	63.05	55,600	0.000
					B	2.853	12.348		63.05	48.270	0.000
					C	2,853	12,348		63.05	0.000	0,000
T7 60.00-40.00	50.00	0.811	6	74,792	Ā	2.853	12.348	9.583	63.05	55,600	0.000
11 00:00 10:00		_			B	2.853	12.348		63.05	48.270	0.000
					c	2.853	12.348		63.05	0.000	0.000
T8 40.00-20.00	30.00	0.701	5	74,792	Ā	2.853	12.348	9.583	63.05	55.600	0.000
10 40.00-20.00	50.00	0.701	Ĩ		B	2.853	12.348		63.05	48,270	0.000
					č	2.853	12.348		63.05	0.000	0,000
T9 20.00-5.00	12.50	0.7	5	56.094	Ă	2.038	9.126	7,188	64.38	41.700	0,000
1 7 20.00-0.00	18.20	v. 1	Ĭ		B	2,038	9,126		64.38	36.203	0,000
					č	2.038	9.126	0	64.38	0.000	0.000
T10 5.00-0.00	2.50	0.7	5	10.019	Ă	0.375	2.584	2.584	87.33	13.900	0.000
110 9.00-0.00	2.50	0.7			B	0.375	2.584		87.33	12.068	0.000
					č	0.375	2.584		87.33	0.000	0.000

 $G_H = 0.850$

Tower Forces - No Ice - Wind Normal To Face

Section	Add	Self	F	е	C_F	<i>q</i> =	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	lb	lb	е						ft ²	lb	plf	
T1	159.08	674.99	Α	0.208	2.57	25	1	1	10.303	982.99	49.15	A
180.00-160.00		TA 214.38	В	0.208	2.57		1	1	10.303			
			С	0.208	2.57		1	1	10.303			
T2	325.40	658.24	Α	0.203	2.585	24	1	1	9.953	1209.45	60.47	A
160.00-140.00			В	0.203	2.585		1	1	9.953			
			С	0.203	2.585		1	1	9.953			
Т3	423.80	658.24	Α	0.203	2.585	23	1	1	9.953	1261.50	63.07	A
140.00-120.00			В	0.203	2.585		1	1	9,953			
			C	0.203	2.585		1	1	9.953			
T4	423.80	658.24	Α	0.203	2.585	22	1	1	9.953	1202.70	60.14	A
120.00-100.00		TA 214.38	B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9,953			
T5	423.80	658.24	A	0.203	2.585	20	I.	1	9.953	1135.69	56.78	A
100.00-80.00			B	0.203	2.585		Î.	1	9.953	1		

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Section	Add	Self	F	е	C_F	q_{z}	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a					- 1				Face
			С			psf						
ft	lb	lb	е						ft²	lb	plf	
			C	0.203	2.585	_	1	1	9.953			
TG	423.80	638.24	A	0.203	2.385	19	1	1	9,953	1037.00	\$2.85	A
80.00-60.00			B	0.203	2.385		1	1	9.933			
			C	0.203	2.585		1	1	9.953			
T7	423,80	658.24	A	0.203	2.585	17	1	1	9.953	960.12	48.01	A
60 00-40 00			B	0.203	2 585		1	1	9.953			
			Ç	0.203	2.585		1	1	9,953			
T8	423.80	658.24	A	0.203	2.585	15	1	1	9,953	829.73	41.49	A
40.00-20.00			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T9 20.00-5.00	317.85	480.27	A	0.199	2.599	15	1	1	7.279	616,96	41.13	A
			B	0.199	2.599		1	1	7.279			
			С	0,199	2,599		1	1	7.279			
T10 5.00-0.00	105.95	111.24	A	0 295	2.309	15	1	1	1.919	181,83	36.37	A
			B	0.295	2.309		1	1	1.919			
			С	0.295	2,309		1	1	1.919			
Sum Weight:	3451.08	6302.97								9437.97		

			0	wer Fo	orces	s - N	o Ice	- VV	ind 60	To Face	-	
Section	Add	Self	F	е	C _F	q_z	D_F	D_R	A _E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			С			psf						
ft	lb	lb	е						ft ²	lb	plf	
T1	159.08	674.99	A	0.208	2.57	25	0.8	1	9.665	948.78	47.44	C
80.00-160.00		TA 214.38	В	0.208	2.57		0.8	1	9.665		111100	
			C	0.208	2.57		0.8	1	9.665			
T2	325.40	658,24	Α	0.203	2.585	24	0.8	1	9.383	1179.78	58.99	C
160.00-140.00			B	0.203	2.585		0.8	1	9.383			
~ ~ I			С	0.203	2.585		0.8	-1	9.383			
T3	423.80	658.24	Α	0,203	2,585	23	0.8	1	9.383	1233.02	61.65	С
40.00-120.00			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
T4	423.80	658.24	Α	0.203	2.585	22	0.8	1	9.383	1175.55	58.78	С
20.00-100.00		TA 214.38	В	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9,383			
T5	423.80	658.24	Α	0.203	2.585	20	0.8	1	9.383	1110.04	55.50	С
100.00-80.00			В	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
T6	423.80	658.24	Α	0.203	2.585	19	0.8	1	9.383	1033.13	51.66	С
80.00-60.00			В	0.203	2,585		0.8	1	9.383			
			C	0.203	2 585		0.8	1	9.383			
T7	423.80	658.24	Α	0.203	2.585	17	0.8	1	9.383	938.44	46.92	С
60.00-40.00			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2,585		0.8	1	9.383			
T8	423.80	658.24	A	0.203	2.585	15	0.8	1	9.383	811.00	40.55	С
40.00-20.00			B	0.203	2.585		0,8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
T9 20.00-5.00	317.85	480.27	A	0.199	2,599	15	0.8	1	6.871	603.52	40.23	С
		(В	0.199	2,599		0.8	1	6.871			
			C	0.199	2.599		0.8	1	6,871			
T10 5.00-0.00	105.95	111.24	A	0.295	2.309	15	0.8	1	1.844	179.64	35.93	С
			В	0.295	2.309		0,8	1	1.844			
			С	0.295	2.309		0.8	1	1.844			

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Section	Add	Self	F	0	C	a	D_F	D_R	Ar	F	w	Ctrl.
Elevation		Weight	a	c	€ŗ	q_{z}	Dr	DA	**E			Face
Elevation	Weight	weight	u			naf						
			С			psf			n ²	lb	plf	
ft	16	ID	e						μ		pŋ	
Sum Weight:	3431.08	6302.97	1							9212.89		

			Γον	ver Fo	orces	5 - N	o Ice	- W	ind 90	To Face		
Section	Add	Self	F	е	C_F	<i>q</i> :	D_F	D_R	A _E	F	w	Ctrl.
Elevation	Weight	Weight	a			- 1-						Face
Literation			с			psf						
A	lb	lb	e						ft ²	lb	plf	
T1	159.08	674.99	A	0.208	2.57	25	0.85	1	9.824	918.87	45.94	C
80.00-160.00		TA 214.38	B	0.208	2.57		0.85	1	9,824			
			Ç	0.208	2.57		0.85	1	9,824			
T2	325.40	658.24	Ā	0.203	2.585	24	0.85	1	9,526	1200.17	60.01	С
160.00-140.00			B	0.203	2.585		0.85	1	9,526			
			Ç	0.203	2.585		0.85	1	9,526			
тз	423.80	658.24	Ā	0.203	2.585	23	0.85	1	9.526	1324.42	66.22	С
140.00-120.00			B	0.203	2.585		0.85	1	9,526			
			С	0.203	2.585		0.85	1	9.526			
T4	423.80	658.24	Ā	0.203	2.585	22	0.85	1	9.526	1262.69	63.13	С
120.00-100.00		TA 214.38	B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
T5	423.80	658.24	A	0.203	2.585	20	0.85	1	9.526	1192.33	59.62	С
100.00-80.00			В	0.203	2.585		0.85	1	9.526			
100.00 00.00			C	0.203	2.585		0.85	1	9.526			
т6	423.80	658.24	A	0.203	2.585	19	0.85	1	9,526	1109.72	55.49	С
80.00-60.00			В	0.203	2.585		0.85	ĩ	9.526			
			c	0.203	2.585		0.85	1	9.526			
т7	423.80	658.24	Ă	0.203	2.585	17	0.85	1	9,526	1008.01	50.40	C
60.00-40.00			B	0 203	2.585		0.85	1	9.526			
			ĉ	0.203	2.585		0.85	1	9,526			
тв	423.80	658.24	Ă	0.203	2.585	15	0.85	Ť	9.526	871.12	43.56	С
40.00-20.00			B	0.203	2.585		0.85	1	9.526			
			č	0.203	2.585		0.85	1	9.526			
T9 20.00-5.00	317.85	480.27	Ă	0.199	2.599	15	0.85	1	6.973	648.42	43.23	С
17 20 00-5.00	517.05		B	0.199	2.599		0.85	1	6.973			
			č	0.199	2.599		0.85	1	6.973			
T10 5.00-0.00	105,95	111.24	Ă	0.295	2,309	15	0.85	1	1.862	194.03	38.81	С
10 5.00-0.00	105.75		в	0.295	2.309		0.85	1	1.862			
			č	0.295	2.309		0.85	1	1.862			
Sum Weight:	3451.08	6302.97	Ĩ.					8		9729.79		

<u> </u>		Tow	/er	Force	es - V	Vith	lce -	Wind	d Norma	al To Fa	ace	_
Section Elevation	Add Weight	Self Weight	F a	е	C_F	<i>q</i> =	D_F	D_R	A _E	F	w	Ctrl. Face
ft	lb	lb	c e			psf			ft ²	lb	plf	
T1	1196.31	2816.31	A	0.616	1.795	6	1	1	38.232	526.46	26.32	Α
80.00-160.00		TA 769.52	B	0.616	1.795		1	1	38.232			
			I C I	0.616	1.795		1	1	38.232			

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	Section	Add	Self	F	е	C_F	<i>q</i> :	D_F	D_R	A_E	F	w	Ctrl.
	Elevation	Weight	Weight	а									Face
				С	I 1		psf						
	fi	lb	lb	е						ft ²	lb	plf	
	T2	2126.75	2719.08	A	0.607	1.8	6	1	1	37.308	\$87.03	29.33	A
	160.00-140.00			B	0.607	1.8		1	1	37.308			
				C	0.607	1.8		1	1	37.308			
	ТЭ	2576.14	2673.93	A	0.601	1.803	6	1	1	36,793	\$86.63	29.33	A
	140.00-120.00			B	0,601	1.803		1	1	36.793			
				C	0.601	1.803		1	1	36.795			
	T4	2538.62	2622.61	A	0.395	1.807	6	1	1	36.211	\$\$6.97	27.85	A
	120.00-100.00		TA 738.18	B	0.395	1.807		1	1.	36.211			
				C	0.595	1.807		1.	1	36.211			
	T5	2494.72	2562.85	A	0.588	1.812	5	1	1	35.529	523.37	26.17	A
	100.00-80.00			B	0.588	1.812		1	1	35.529			
				C	0.588	1.812		1	1	35,529			
	T6	2441.51	2490.82	A	0.579	1.818	5	1	1	34.703	484.23	24.21	A
2	80.00-60.00			B	0.579	1.818		1	1	34.703			
				C	0.579	1.818		1	1	34,703			
	T7	2373.18	2399.07	A	0.568	1.828	4	1	1	33.646	436.48	21.82	A
	60.00-40.00			B	0.568	1.828		1	1	33.646			
				Ç	0.568	1.828		1	1	33,646			
	T8	2275.50	2269.35	A	0.551	1.843	4	1	1	32.141	373.03	18.65	A
	40.00-20.00			B	0.551	1.843		1	1	32.141			
				C	0.551	1.843		1	1	32.141			
	T9 20.00-5.00	1592,69	1497,28	Α	0.506	1.892	4	1	1	21.362	274.89	18.33	A
				В	0.506	1.892		1	1	21.362			
				С	0.506	1.892		1	1	21.362			
	T10 5.00-0.00	472.02	248.75	Α	0.509	1.889	4	1	1	3.994	69.41	13.88	A
				В	0.509	1.889		1	1	3.994			
				С	0.509	1.889		1	1	3.994			
	Sum Weight:	20087.44	23807.75								4418.49		

		Т	ow	er Fo	rces	- Wi	th Ic	e - W	'ind 60	Го Гасе	•	
Section	Add	Self	F	е	C_F	q_z	D_F	D_R	A _E	F	w	Ctrl.
Elevation	Weight	Weight	a			125						Face
	U	J. J.	с			psf						
ft	lb	lb	е						ft^2	lb	plf	
T1	1196.31	2816.31	A	0.616	1.795	6	0.8	1	37.593	520.37	26.02	C
180.00-160.00		TA 769.52	В	0.616	1.795		0.8	1	37.593			
			C	0.616	1.795		0.8	1	37.593			
T2	2126.75	2719.08	Α	0.607	1.8	6	0.8	1	36.738	581.76	29.09	С
160.00-140.00			В	0.607	1.8		0.8	1	36.738			
			C	0.607	1.8		0.8	1	36.738	- 1		
T3	2576.14	2673.93	A	0.601	1.803	6	0.8	1	36.225	581.56	29.08	С
140.00-120.00			B	0.601	1.803		0.8	1	36.225			
· · ·			C	0.601	1.803		0.8	1	36.225			
T4	2538.62	2622.61	A	0.595	1.807	6	0.8	1	35.641	552.13	27.61	С
120.00-100.00		TA 738.18	B	0.595	1.807		0.8	1	35.641			
			C	0.595	1.807		0.8	1	35.641			
T5	2494.72	2562.85	A	0.588	1.812	5	0.8	1	34.958	518.79	25.94	С
100.00-80.00			B	0.588	1.812		0.8	1	34.958			
			C	0.588	1.812		0.8	1	34.958			
T6	2441.51	2490.82	A	0.579	1.818	5	0.8	1	34,133	479.94	24.00	С
80.00-60.00			B	0.579	1,818		0.8	1	34.133			
			C	0.579	1.818		0.8		34.133			
T7	2373.18	2399.07	A	0.568	1.828	4	0.8	1	33.076	432.57	21.63	С

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Section	Add	Self	F	е	C_F	<i>q</i> :	D_F	D_R	AE	F	W	Ctr
Elevation	Weight	Weight	а									Fa
			с			psf						
ft	lb	Ib	е	_					ft ²	lb	plf	
60 00-40.00			B	0.568	1.828		0.8	1	33.076			
			Ç	0.368	1.828		8.0	1	33.076	I		
T8	2275.50	2269.35	A	0.551	1.843	4	0.8	1	31.571	369.62	18.48	C
40.00-20.00			B	0.\$\$1	1.843		0.8	1	31.571			
			C	0.551	1.843		0.8	1	31.571			
9 20.00-5.00	1592.69	1497.28	A	0.506	1.892	4	0.8	1	20.934	272.39	18.16	C
			B	0.\$06	1.892		0.8	1	20.954			
			C	0.\$06	1.892		0.8	1	20.954			
10 5.00-0.00	472.02	248.75	A	0.509	1.889	4	8.0	1	3.919	68.95	13.79	C
		24	B	0.\$09	1.889		0,8	1	3,919			
			C	0.\$09	1,889		0.8	1	3.919			
Sum Weight:	20087.44	23807.75								4378.09		

		Т	ow	er Fo	rces	- Wi	th Ic	e - W	ind 90	To Face	9	_
Section Elevation	Add Weight	Self Weight	F a	е	C _F	<i>q</i> :	D_F	D_R	A_E	F	w	Ctrl. Face
fi	lb	lb	с е	*		psf			ft ²	lb	plf	
	1196.31	2816.31	A	0.616	1.795	6	0.85	1	37.753	507.75	25.39	С
80.00-160.00	(1)0.31	TA 769.52	B	0.616	1.795		0.85	- i	37,753			
00.00 100.00			c	0.616	1.795		0.85	1	37,753			
T2	2126.75	2719.08	Ă	0.607	1.8	6	0.85	1	36.880	577.50	28.87	С
60.00-140.00			B	0.607	1.8		0.85	1	36.880			
			c	0,607	1.8		0.85	1	36.880			
Т3	2576.14	2673.93	Ā	0,601	1.803	6	0.85	1	36.368	589.57	29.48	С
40.00-120.00			B	0.601	1.803		0.85	1	36.368			
	11		С	0,601	1.803		0.85	1	36.368			
T4	2538.62	2622.61	A	0.595	1.807	6	0.85	1	35.783	559.86	27.99	С
20.00-100.00		TA 738.18	В	0.595	1.807		0.85	1	35.783			
			C	0.595	1.807		0.85	1	35.783			
T5	2494.72	2562.85	A	0.588	1.812	5	0.85	1	35.101	526.20	26.31	C
100.00-80.00			B	0.588	1.812		0.85	1	35.101			
120			C	0.588	1.812		0.85	1	35.101			
T6	2441.51	2490.82	A	0.579	1.818	5	0.85	1	34.275	486.97	24.35	С
80.00-60.00			B	0.579	1.818		0.85	1	34.275			
	1. A.		C	0.579	1.818		0.85	1	34.275			
T7	2373.18	2399.07	A	0.568	1.828	4	0.85	1	33.218	439.11	21.96	С
60.00-40.00			B	0.568	1.828		0.85	1	33.218			
~	F1		C	0.568	1.828		0.85	1	33.218			
T8	2275,50	2269.35	A	0.551	1.843	4	0.85	1	31.713	375.47	18.77	C
40.00-20.00			B	0.551	1.843		0.85	1	31,713			
			C	0.551	1.843		0.85	1	31.713			
Г9 20.00-5.00	1592.69	1497.28	A	0.506	1.892	4	0.85	1	21.056	277.13	18.48	C
			B	0.506	1.892		0.85	1	21.056			
			C	0.506	1.892		0.85	1	21.056			
Г10 5.00-0.00	472.02	248.75	A	0.509	1.889	4	0.85	1	3.938	70.43	14.09	C
			B	0.509	1.889		0.85	1	3.938			
			C	0.509	1.889		0.85	1	3.938			
Sum Weight:	20087.44	23807.75						_		4409.98		

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		Τον	ver	Forc	es - 🖁	Serv	ice -	Win	d Norn	nal To F	ace		
		-	_									0.1	
Section	Add	Self	F	е	C_F	<i>q</i> :	D_F	D_R	A _E	F	w	Ctrl.	1
Elevation	Weight	Weight	а									Face	1
			С			psf			-2				1
ft	lb	lb	е					-	ft ²	lb	plf		1
TI	139.08	674.99	A	0.208	2.57	9	1	1	10.303	361.06	18.05	A	1
180.00-160.00		TA 214.38	B	0.208	2.37		1	1	10,303				1
			C	0.208	2.37		1	1	10.303				1
T2	325.40	658.24	A	0.203	2.585	9	1	1	9,953	444.24	22.21	A	1
160.00-140.00			B	0,203	2.585		1. U	1	9.953				1
			¢	0,203	2.585		1	1	9,953				1
T3	423.80	658.24	A	0,203	2.585	8	1	1	9,953	463.36	23.17	A	1
140,00-120,00			B	0.203	2.585		1	1	9.953				
			C	0.203	2.585		1	1	9,953				
T4	423.80	658.24	A	0,203	2.585	8	1	1	9,953	441.76	22.09	A	1
120.00-100.00		TA 214.38	B	0,203	2.585		1	1	9.953				1
			C	0.203	2.585		1	1	9.953				1
T5	423.80	658.24	A	0.203	2.585	8	1	1	9,953	417.15	20,86	A	1
100.00-80.00			B	0.203	2.585		1	1	9.953				1
			C	0.203	2.585		1	1	9,953				1
T 6	423.80	658.24	A	0,203	2.585	7	1	1	9.953	388.25	19,41	A	1
80,00-60,00			B	0.203	2.585		1	1	9.953				
			С	0.203	2.585		1	1	9.953				1
T7	423.80	658.24	A	0.203	2.585	6	1	1	9.953	352.66	17.63	A	1
60.00-40.00			B	0.203	2.585		1	1	9,953				1
	()		С	0.203	2.585		T	1	9.953				
T8	423.80	658.24	A	0.203	2.585	5	Ť	1	9,953	304.77	15.24	A	
40.00-20.00			в	0.203	2.585		1	1	9.953				
			C	0.203	2.585		1	1	9,953				
T9 20.00-5.00	317.85	480.27	Ă	0.199	2.599	5	1	1	7.279	226.62	15.11	A	1
17 20:00 0:00			B	0.199	2 599	-	1	1	7.279				1
			č	0,199	2,599		i.	î.	7,279				
T10 5.00-0.00	105.95	111.24	Ă	0.295	2,309	5	î.	1	1.919	66.79	13.36	A	
110 3.30-0 00			B	0.295	2.309		î	î î	1.919				
			č	0.295	2.309		î.	î	1,919				
Sum Weight:	3451.08	6302.97	-				78	^		3466.66			

		. T	ow	ver Fo	rces	- Se	rvice	e - W	ind 60 1	To Face	•	_
Section Elevation	Add Weight	Self Weight	F a	е	C _F	q:	D_F	D_R	A _E	F	w	Ctrl. Face
fi	lb	lb	с е			psf			ft ²	lb	plf	
T1	159.08	674.99	A	0.208	2.57	9	0.8	1	9.665	348.49	17.42	С
180.00-160.00		TA 214.38	в	0.208	2.57		0.8	1	9.665			
· · · · ·			C	0.208	2.57		0.8	1	9.665			
T2	325.40	658.24	A	0.203	2.585	9	0.8	1	9.383	433.34	21.67	С
160.00-140.00			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
Т3	423.80	658.24	A	0.203	2.585	8	0.8	1	9.383	452.90	22.64	С
140.00-120.00			B	0.203	2.585		0.8	1	9.383			
			c	0.203	2.585		0.8	1	9.383			
T4	423,80	658.24	A	0.203	2.585	8	0.8	1	9,383	431.79	21.59	С
120.00-100.00		TA 214.38	B	0.203	2.585		0,8	1	9.383			
1121			C	0.203	2.585		0.8	1.	9.383			

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Section	Add	Self	F	е	CF	<i>q</i> :	D_F	D_R	AE	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
	U	Ū	с			psf						
ft	lb	lb	е						ft ²	Ib	plf	
TS	423.80	658.24	A	0.203	2.585	8	0.8	1	9,383	407.73	20.39	¢
100.00-80.00			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.385		0.8	1	9,383			
T6	423.80	658.24	A	0.203	2,585	7	0.8	- E	9.383	379.48	18.97	¢
80.00-60.00			B	0.203	2.383		0.8	1	9,383			
			C	0.203	2,383		0.8	1	9,383			
T7	423.80	658.24	A	0.203	2,585	6	0.8	- 1	9,383	344.70	17.23	¢
60.00-40.00			B	0.203	2,585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9,383			_
T 8	423.80	658.24	A	0.203	2.585	- 5	0.8	1	9.383	297.89	14.89	¢
40.00-20.00			B	0.203	2.585		0.8	1	9.383		100	
			C	0.203	2.585		0.8	1	9,383			
T9 20.00-5.00	317.85	480,27	A	0,199	2.599	\$	0.8	1	6.871	221.68	14.78	С
			B	0.199	2.599		0.8	1	6.871			
			C	0,199	2,599		0.8	1	6,871			
T10 5.00-0.00	105.95	111.24	A	0.295	2.309	5	0.8	1	1.844	65.98	13.20	С
			B	0.295	2.309		0.8	1	1.844			
			C	0,295	2.309		Q.8	1	1.844			
Sum Weight:	3451.08	6302.97								3383.98		

	_												
'trl. ace		w	F	A _E	D_R	D_F	<i>q</i> =	C _F	е	F	Self	Add	Section
ILE	11									a	Weight	Weight	Elevation
		plf	1b	ft ²			psf			c e	lb	lb	ſt
С	T	16.88	337.51	9.824	1	0.85	9	2.57	0,208	A	674.99	159.08	TI
		ov		9.824	1	0.85	-	2.57	0.208	B	TA 214.38		180.00-160.00
				9,824	1	0.85		2.57	0.208	c			
С		22.04	440.83	9.526	1	0.85	9	2.585	0.203	A	658.24	325.40	T2
				9.526	1	0.85		2.585	0,203	B			160.00-140.00
				9.526	1	0.85		2.585	0,203	c			
С		24.32	486.47	9.526	1	0.85	8	2.585	0.203		658.24	423.80	T3
				9.526	1	0.85		2.585	0.203	в		_	140.00-120.00
	1			9.526	1	0.85		2.585	0.203	С			
С		23.19	463.80	9.526	1	0.85	8	2.585	0.203	A	658.24	423.80	T4
				9.526	1	0.85		2.585	0.203	в	TA 214.38		120.00-100.00
				9.526	1	0.85		2.585	0.203	C			
С		21.90	437.96	9.526	1	0.85	8	2.585	0.203	A	658.24	423.80	T5
				9.526	1	0.85		2.585	0.203	в			100.00-80.00
				9.526	1	0.85		2.585	0.203	C			
С	1	20.38	407.61	9.526	1	0.85	7	2.585	0.203	A	658.24	423.80	T6
				9.526	1	0.85		2.585	0.203	B			80.00-60.00
				9.526	1	0.85		2.585	0.203	C			
С		18.51	370.25	9.526	1	0.85	6	2.585	0.203	A	658.24	423.80	T7
				9.526	1	0.85		2.585	0.203	B			60.00-40.00
				9.526	1	0,85		2.585	0.203	C			
С	4	16.00	319.97	9.526	1	0.85	5	2:585	0.203	A	658.24	423.80	T8
	1			9.526	1	0.85		2.585	0,203	B			40.00-20.00
	1			9.526	1	0.85		2,585	0.203	C			
С	1	15.88	238,17	6.973	1	0.85	5	2.599	0.199	A	480.27	317.85	T9 20 00-5.00
	1	1		6.973	1	0.85		2.599	0.199	B			
				6 973	1	0.85		2.599	0,199	C			
С	1	14.25	71.27	1.862	1	0.85	5	2,309	0.295		111.24	105.95	T10 5.00-0.00

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Section Elevation	Add Weight	Self Weight	F a	е	C_F	<i>q</i> :	D_F	D_R	A_E	F	w	Ctrl. Face
Lievation	Ū	, ,	с			psf			<i>c</i> 2	"	16	Tuce
п	lb	lb	e B	0.293	2.309		0.83	1	1.862	lb	plf	
			ç	0.295	2.309		0.85	i	1.862			
Sum Weight:	3451.08	6302.97			1		· · · · · ·			3573.84		

Force Totals (Does not include forces on guys)

Load	Vertical	Sum of	Sum of	Sum of Torques
Case	Forces	Forces	Forces	
		X	Z	1940 (Mar)
	lb	lb	lb	lb-fi
Leg Weight	3138.04	COLUMN TO A		A DECK
Bracing Weight	3164.93	1.1.1		1. C. Y. 1994
Total Member Self-Weight	6302.97	A Share U.S.		
Guy Weight	2100.38	and the second		
Total Weight	18657.64	S. M. A.		
Wind 0 deg - No Ice	and the second second	-19.88	-13073.32	-314.12
Wind 30 deg - No Ice	13 T 13 A	6417.62	-11260.37	-868.19
Wind 60 deg - No Ice		12360.58	-7196.99	-1341.86
Wind 90 deg - No Ice	S	14977.13	19.88	-1376.64
Wind 120 deg - No Ice	L. J. H. King	12727.79	7431.95	-956.67
Wind 150 deg - No Ice	13 11 2,111	6828.23	11931.82	-320.65
Wind 180 deg - No Ice	State and the second	19.88	13073.32	314.12
Wind 210 deg - No Ice	I DALESSI T	-6417.62	11260.37	868.19
Wind 240 deg - No Ice		-12360.58	7196.99	1341.86
Wind 270 deg - No Ice	Silve Addition	-14977.13	-19.88	1376.64
Wind 300 deg - No Ice		-12727.79	-7431.95	956.67
Wind 330 deg - No Ice	100x 0	-6828.23	-11931.82	320.65
Member Ice	17504.78	1.00 1.00 1.00	Last and St.	- 100 1 10
Guy Ice	12178.48	12010 1210	1 31 36 35	Sec. Same
Total Weight Ice	94511.93	1 1 2 1 2	13 M 1 1 1 1	
Wind 0 deg - Ice		-6.29	-7429.74	-29.55
Wind 30 deg - Ice		3690.39	-6423.83	-276.60
Wind 60 deg - Ice	1.1.1.1.1.2.1	6616.76	-3831.33	-461.66
Wind 90 deg - Ice		7813.94	6.29	-515.86
Wind 120 deg - Ice		6777.59	3931.46	-423.73
Wind 150 deg - Ice	Company of the local division of the local d	3790.51	6584.66	-222.66
Wind 180 deg - Ice	ALC: NOT THE OWNER.	6.29	7429.74	29.55
Wind 210 deg - Ice	A ALL A REAL POINT	-3690.39	6423.83	276.60
Wind 240 deg - Ice	Strain 1 Miles	-6616.76	3831.33	461.66
Wind 270 deg - Ice	THE MOUNT	-7813.94	-6.29	515.86
Wind 300 deg - Ice		-6777.59	-3931.46	423.73
Wind 330 deg - Ice	1.	-3790.51	-6584.66	222.66
Total Weight	18657.64	A ROLL	THE OWNER WATCHING	
Wind 0 deg - Service	and the second second	-7.30	-4801.95	-115.38
Wind 30 deg - Service		2357.25	-4136.04	-318.89
Wind 60 deg - Service	1. 1. A.	4540.16	-2643.52	-492.88
Wind 90 deg - Service		5501.24	7.30	-505.65
Wind 120 deg - Service	1 - 90.16 - 2.	4675.04	2729.83	-351.39
Wind 150 deg - Service		2508.07	4382.67	-117.78
Wind 180 deg - Service		7.30	4801.95	115.38
Wind 210 deg - Service		-2357.25	4136.04	318,89
Wind 240 deg - Service		-4540.16	2643.52	492.88
Wind 270 deg - Service	10 12 0 11	-5501.24	-7.30	505.65
Wind 300 deg - Service		-4675.04	-2729.83	351.39

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Load	Vertical	Sum of	Sum of	Sum of Torques
Case	Forces	Forces	Forces	
	1 1	X	Ζ	
	lb	lb	lb	lb-ft
Wind 330 deg - Service	entransister a	-2508.07	-4382.67	117.78

Load Combinations

Comb.	Desc	ription		
No.				
1	Dead Only			
2	1.2 Dead+1.6 Wind 0 deg - No Ice+1.0 Guy			
3	1.2 Dead+1.6 Wind 30 deg - No Ice+1.0 Guy			
4	1.2 Dead+1.6 Wind 60 deg - No Ice+1.0 Guy			
5	1.2 Dead+1.6 Wind 90 deg - No Ice+1.0 Guy			
6	1.2 Dead+1.6 Wind 120 deg - No Ice+1.0 Guy			
7	1.2 Dead+1.6 Wind 150 deg - No Ice+1.0 Guy			
8	1.2 Dead+1.6 Wind 180 deg - No Ice+1.0 Guy			
9	1.2 Dead+1.6 Wind 210 deg - No Ice+1.0 Guy			
10	1.2 Dead+1.6 Wind 240 deg - No Ice+1.0 Guy			
11	1.2 Dead+1.6 Wind 270 deg - No Ice+1.0 Guy			
12	1 2 Dead+1 6 Wind 300 deg - No Ice+1.0 Guy			
13	1.2 Dead+1.6 Wind 330 deg - No Ice+1.0 Guy			
14	1.2 Dead+1.0 Ice+1.0 Temp+Guy			
15	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp+1.0 Guy			
16	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp+1.0 Guy			
17	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp+1.0 Guy			
18	1 2 Dead+1 0 Wind 90 deg+1 0 Ice+1 0 Temp+1 0 Guy			
19	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp+1.0 Guy			
20	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp+1.0 Guy			
21	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp+1.0 Guy			
22	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp+1.0 Guy			
23	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp+1.0 Guy			
24	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp+1.0 Guy			
25	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp+1.0 Guy			
26	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp+1.0 Guy			
27	Dead+Wind 0 deg - Service+Guy			
28	Dead+Wind 30 deg - Service+Guy			
29	Dead+Wind 60 deg - Service+Guy			
30	Dead+Wind 90 deg - Service+Guy			
31	Dead+Wind 120 deg - Service+Guy			
32	Dead+Wind 150 deg - Service+Guy			
33	Dead+Wind 180 deg - Service+Guy			
34	Dead+Wind 210 deg - Service+Guy			
35	Dead+Wind 240 deg - Service+Guy			
36	Dead+Wind 270 deg - Service+Guy			
37	Dead+Wind 300 deg - Service+Guy			
38	Dead+Wind 330 deg - Service+Guy			

			Maxim	um Reactio	ns
Location	Condition	Gov. Load	Vertical Ib	Horizontal, X lb	Horizontal, Z lb
Mast	Max, Vert	Comb. 15	161572.04	-27.33	390.98

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Location	Condition	Gov. Load	Vertical Ib	Horizontal, X lb	Horizontal, Z lb
		Loaa Comb.	10	10	10
	Max H	11	78952 47	1775.64	-17.86
	Max. Hz	2	78846.19	-2 30	1704.34
	Max. M.	1	0.00	-2.77	-7.34
	Max. M.	1	0.00	-2.77	-7.34
	Max. Torsion	1	0.00	-2.77	-7.34
	Min. Vert	1	72557.05	-2.77	-7.34
	Min. Hx	5	78967.80	-1782.33	-19.31
	Min. H	8	79178.65	-3.93	-1653.09
	Min. M.	1	0.00	-2.77	-7.34
	Min. Mz	i	0.00	-2.77	-7.34
	Min. Torsion	1	0.00	-2.77	-7.34
Guy C @ 145 ft Elev 0 ft	Max. Vert	10	-5181,49	-5946.29	3429.28
Azimuth 240 dog					
-	Max H _x	10	-5181.49	-5946.29	3429.28
	Max. H _z	4	-32539.44	-32999.27	19059.36
	Min. Vert	4	-32539.44	-32999.27	19059.36
	Min. H _x	4	-32539.44	-32999 27	19059.36
	Min. Ha	10	-5181.49	-5946.29	3429.28
Guy B @ 145 ft Elev 0 ft	Max. Vert	6	-5162.52	5934.35	3423.82
Azimuth 120 deg					
	Max. H _x	12	-32597.75	33039.46	19080.60
	Max, H,	12	-32597.75	33039.46	19080.60
	Min. Vert	12	-32597.75	33039.46	19080.60
	Min. H _x	6	-5162.52	5934.35	3423.82
	Min H _z	6	-5162.52	5934.35	3423.82
Guy A @ 145 ft Elev 0 ft	Max. Vert	2	-5096.72	-1.01	-6806.09
Azimuth 0 deg			10030 54	5 20.40	005/0 5/
	Max. H _x	11	-18933.56	738.48	-22562.56
	Max. Hz	2	-5096.72	-1.01	-6806.09
	Min. Vert	8	-32708.57	1.39	-38248.02
	Min, H _x Min, Hz	5 8	-18991.65 -32708.57	-738.59 1.39	-22608.30 -38248.02

Tower Mast Reaction Summary

Load	Vertical	Shear _x	Shear:	Overturning	Overturning	Torque
Combination				Moment, M _x	Moment, M _z	
	lb	lb	lb	lb-ft	lb-fl	lb-ft
Dead Only	72557.05	2.77	7.34	0.00	0.00	0.00
1.2 Dead+1.6 Wind 0 deg - No	78846.19	2.50	-1704.34	0.00	0.00	0.00
Ice+1.0 Guy						
1.2 Dead+1.6 Wind 30 deg - No	79121.54	882.87	-1540.17	0.00	0.00	0.00
Ice+1.0 Guy						
1.2 Dead+1.6 Wind 60 deg - No	79123.26	1423,34	-813.43	0.00	0.00	0.00
Ice+1.0 Guy						
1.2 Dead+1.6 Wind 90 deg - No	78967.80	1782.33	19.31	0.00	0.00	0.00
Ice+1.0 Guy						
1.2 Dead+1.6 Wind 120 deg -	78622.73	1484.70	865.76	0.00	0.00	0.00
No Ice+1.0 Guy						
1.2 Dead+1.6 Wind 150 deg -	79071-87	902.57	1547.79	0.00	0.00	0.00
No Ice+1 0 Guy						
1.2 Dead+1.6 Wind 180 deg -	79178.65	3,93	1653.09	0.00	0.00	0.00
No Ice+1.0 Guy						
1.2 Dead+1.6 Wind 210 deg -	79013.13	-895.04	1546.80	0.00	0.00	0.00

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Load Combination	Vertical	Shear _x	Shear _z	Overturning Moment, M _x	Overturning Moment, M ₌	Torque
	lb	lb	lb	lb-fi	lb-ft	lb-fi
la lco+1.0 Guy .2 Dead+1.6 Wind 240 deg -	78362.89	-1477.43	864.42	0.00	0,00	0.0
la lco+1.0 Guy 2 Dead+1.6 Wind 270 deg -	78952.47	-1775.64	17.86	0.00	0.00	0.0
la Ice+1.0 Guy 2 Dead+1.6 Wind 300 deg -	79143.99	-1417.48	-814.54	0.00	0.00	0,0
lo lee+1.0 Guy 2 Dead+1.6 Wind 330 deg -	79154.94	-877.64	-1540.66	0,00	0,00	0.0
o Ico+1.0 Guy 2 Dead+1.0 Ico+1.0	160292.45	27.33	33.79	0,00	0.00	0.0
emp+Guy 2 Dead+1.0 Wind 0 deg+1.0	161572.04	27.33	-390,98	0.00	0,00	0.0
e+1.0 Temp+1.0 Guy 2 Dead+1.0 Wind 30 deg+1.0	161183,04	223.91	-336.54	0,00	0.00	0,0
ce+1.0 Temp+1.0 Guy 2 Dead+1.0 Wind 60 deg+1.0	160840.89	372.71	-168.08	0,00	0,00	0.0
2 Dead+1.0 Wind 90 deg+1.0	161166.48	443.99	45.57	0.00	0.00	0.0
e+1.0 Temp+1.0 Guy 2 Dead+1.0 Wind 120	161539.23	392.93	243,49	0.00	0.00	0.0
eg+1.0 Ice+1.0 Temp+1.0 Guy 2 Dead+1.0 Wind 150	161159.90	247.86	386.82	0.00	0,00	0.0
2 Dead+1.0 Ice+1.0 Temp+1.0 Guy 2 Dead+1.0 Wind 180	160826.96	27,74	431.86	0.00	0,00	0.0
g+1.0 Ice+1.0 Temp+1.0 Guy 2 Dead+1.0 Wind 210	161162.31	-192.43	386.63	0.00	0.00	0.0
eg+1.0 Ice+1.0 Temp+1.0 Guy 2 Dead+1.0 Wind 240	161546.34	-337.64	242.96	0.00	0.00	0.0
eg+1.0 Ice+1.0 Temp+1.0 Guy 2 Dead+1.0 Wind 270 eg+1.0 Ice+1.0 Temp+1.0 Guy	161176.87	-388.93	45.10	0.00	0.00	0.0
2 Dead+1.0 Wind 300 eg+1.0 Ice+1.0 Temp+1.0 Guy	160852.37	-317,89	-168.45	0.00	0,00	0.0
2 Dead+1.0 Wind 330 eg+1.0 Ice+1.0 Temp+1.0 Guy	161191.02	-169.03	-336.74	0_00	0.00	0.0
ead+Wind 0 deg - ervice+Guy	72678.50	2.64	-384.29	0.00	0.00	0.0
ead+Wind 30 deg - ervice+Guy	72641.88	207.51	-349.57	0.00	0.00	0.0
ead+Wind 60 deg - ervice+Guy	72607.77	333.20	-183,80	0.00	0.00	0.0
ead+Wind 90 deg - ervice+Guy	72642.48	413.72	8.18	0.00	0.00	0.0
ead+Wind 120 deg - ervice+Guy	72679.21	341.27	203.24	0.00	0.00	0.0
ead+Wind 150 deg - ervice+Guy	72642.33	209.19	364.01	0.00	0.00	0.0
ead+Wind 180 deg - ervice+Guy	72607.79	2.95	389.80	0.00	0.00	0.
ead+Wind 210 deg - ervice+Guy	72642.42	-203.15	363.50	0.00	0.00	0.
ead+Wind 240 deg - ervice+Guy	72679.31	-335.52	202.96	0.00	0.00	0.
ead+Wind 270 deg - ervice+Guy	72642.54	-408.14	7.86	0.00	0.00	0.
ead+Wind 300 deg - ervice+Guy	72607.76	-327.78	-184.08	0.00	0.00	0.
ervice+Guy ead+Wind 330 deg - ervice+Guy	72641,85	-202.44	-350.02	0.00	0.00	0.

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			So	lution Su	mmary	_	_
	Sui	n of Applied Force.	5		Sum of Reaction		
Load	PX	PY	PZ	PX	ΡY	PZ	% Error
Comb.	lb	lb	Ib	lb	lb	lb	0.01.101
1	0.00	-18657.18	0,00	-0.52	18657.18	-2.53	0.014%
2	-31.80	-22142.81	-26421.04	31.84	22140.14	26361.62	0.173%
3	13297.72	-21968.63	-23263.89	-13287.71	21966.24	23186.92	0.224%
4	22353.76	-21794.45	-13002.92	-22232 32	21792.36	12930.22	0.419%
5	26650.52	-21968 63	31.80	-26565 88	21966.12	0.29	0.262%
6	22697.43	-22142.81	13238.06	-22640 34	22140.06	-13205.09	0.192%
7	13352.80	-21968.63	23295.69	-13293.68	21966.42	-23250.08	0.215%
8	31,80	-21794.45	26060.92	-30.72	21792.28	-25918.35	0.420%
9	-13297.72	-21968.63	23263.89	13226.12	21966.02	-23205.70	0.266%
10	-22665.63	-22142.81	13182,98	22610.52	22140.19	-13151.10	0.186%
11	-26650.52	-21968.63	-31,80	26566.83	21966.18	63,35	0,259%
12	-22385.56	-21794.45	-13058.00	22264.37	21792.32	12984.14	0.419%
13	-13352.80	-21968.63	-23295.69	13342.96	21966.18	23217.49	0.227%
14	0.00	-97820.27	0.00	-6.78	97820.24	-13.35	0.015%
15	-6.29	-97998.49	-10594.81	6.39	97998.09	10537.87	0.058%
16	5269.47	-97820.27	-9158.87	-5238.51	97819.82	9102.43	0.065%
17	9111.64	-97642.05	-5271.75	-9045.20	97641.48	5232.77	0.078%
18	10549.83	-97820.27	6.29	-10486.34	97819.88	-5.16	0.065%
19	9152.92	-97998.49	5302.86	-9092.39	97998.09	-5267.22	0.071%
20	5280.37	-97820.27	9165.17	-5247.90	97819.89	-9109.75	0.065%
21	6.29	-97642.04	10554,40	-6.54	97641.51	-10476.45	0.079%
22	-5269.47	-97820.27	9158.87	5236.66	97819.87	-9103.30	0.066%
23	-9146.63	-97998.49	5291.96	9085.80	97998.07	-5256.05	0.072%
24	-10549.83	-97820.27	-6.29	10486.21	97819.86	7.69	0.065%
25	-9117.93	-97642.05	-5282.65	9051.45	97641.46	5243.91	0.078%
26	-5280.37	-97820.27	-9165.17	5249.55	97819.81	9108.81	0.065%
27	-7.30	-18697,16	-6065,44	7.28	18697,14	6013,82	0.263%
28	3052.74	-18657,18	-5340.65	-3031.40	18657.14	5305,06	0.211%
29	5131.72	-18617.19	-2985.06	-5086.84	18617 14	2959.08	0.265%
30	6118.12	-18657.18	7.30	-6076.69	18657.14	-8,00	0.211%
31	5210.61	-18697.16	3039.04	-5166.06	18697.14	-3013.19	0.262%
32	3065.38	-18657.18	5347.95	-3045.28	18657.14	-5311.55	0.212%
33	7.30	-18617.19	5982.76	-7.27	18617.13	-5930.74	0.266%
34	-3052.74	-18657.18	5340.65	3032.67	18657.14	-5304.29	0.211%
35	-5203 31	-18697.16	3026.40	5158.78	18697.14	-3000.59	0.262%
36	-6118.12	-18657.18	-7.30	6076.69	18657.14	6.57	0.211%
37	-5139.02	-18617.19	-2997.71	5094.12	18617.13	2971.68	0.266%
38	-3065.38	-18657.18	-5347.95	3044.02	18657.14	5312.31	0.211%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	50	0.00000001	0.00002257
2	Yes	77	0.00136161	0.00049277
3	Yes	74	0.00134426	0.00044085
4	Yes	69	0.00138824	0.00039552
5	Yes	73	0.00145508	0.00045909
6	Yes	76	0.00144335	0.00050606
7	Yes	74	0.00127464	0.00041527
8	Yes	69	0.00139456	0.00040255
9	Yes	73	0.00148494	0.00047317

Conversion of the second seco	RISATower	Job	117	-23243.2	Page 32 of 45
Phome: FAX: Client CDT Designed by FAN 10 Yes 76 0.00139290 0.00048533 11 Yes 73 0.00143017 0.000485023 12 Yes 69 0.00139267 0.000398991 13 Yes 74 0.00136337 0.00044991 14 Yes 50 0.00039807 0.00011318 15 Yes 72 0.0013465 0.0001102308 17 Yes 71 0.0013455 0.0009990 18 Yes 72 0.001439272 0.00013080 18 Yes 72 0.001439272 0.000130997 20 Yes 72 0.00123953 0.000019977 21 Yes 72 0.00149347 0.00011007 22 Yes 72 0.00125743 0.00011375 22 Yes 72 0.00127039 0.00011327 23 Yes 72 0.0012373		Project	Colc	hester. CT	Date 22:10:56 01/14/18
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		Client			
11Yes73 0.00143017 0.00043023 12Yes69 0.00139267 0.00039891 13Yes74 0.00136337 0.00044991 14Yes30 0.00079507 0.00012308 15Yes73 0.00130660 0.00011318 16Yes72 0.00134653 0.00011623 17Yes71 0.0012305 0.00012000 18Yes72 0.00123955 0.00012000 19Yes72 0.00123955 0.00009990 19Yes72 0.00125014 0.00009977 21Yes71 0.00140035 0.00011375 22Yes72 0.00127039 0.00010259 23Yes72 0.00127039 0.00010259 23Yes72 0.0013285 0.0001027 24Yes72 0.0013285 0.00010327 25Yes72 0.0013285 0.00011375 26Yes72 0.0013285 0.00011466 27Yes66 0.00147855 0.00014273 28Yes67 0.00119668 0.00011865 29Yes66 0.00149525 0.00014262 30Yes67 0.0011975 0.00011865 31Yes66 0.00149527 0.00014274 34Yes67 0.00119833 0.00011868 35Yes66 0.00147567 0.00014274 <th>FAX:</th> <th></th> <th></th> <th></th> <th></th>	FAX:				
12Yes69 0.00139267 0.00039891 13Yes74 0.0013337 0.00044991 14Yes50 0.00079507 0.00012308 15Yes73 0.0013060 0.00011318 16Yes72 0.00133465 0.00011623 17Yes71 0.0012395 0.0009990 18Yes72 0.00123955 0.0009990 19Yes72 0.00143477 0.00011285 20Yes72 0.00125914 0.00009977 21Yes72 0.00127039 0.00010259 23Yes72 0.00127039 0.00010259 23Yes72 0.00125743 0.00010259 24Yes72 0.00125743 0.00010259 25Yes72 0.0013285 0.0001327 26Yes72 0.0013285 0.0001327 27Yes66 0.00147855 0.00014273 28Yes67 0.00119705 0.00011865 29Yes66 0.00147855 0.00011865 30Yes67 0.00119705 0.00011853 31Yes66 0.00147627 0.00011906 33Yes66 0.0014787 0.00011906 33Yes66 0.0014787 0.00011868 35Yes66 0.0014757 0.00011868					
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14Yes30 0.00079507 0.00012308 15Yes73 0.00130660 0.00011623 16Yes72 0.0013262 0.00011623 17Yes71 0.00142072 0.00012090 18Yes72 0.00123953 0.00009990 19Yes72 0.00143072 0.00011285 20Yes72 0.00123943 0.00009977 21Yes71 0.00140005 0.00011375 22Yes72 0.00127039 0.00010259 23Yes72 0.0012739 0.00010259 24Yes72 0.0012738 0.00013277 25Yes71 0.00142838 0.00012334 26Yes72 0.00133285 0.00011865 29Yes66 0.00149525 0.00014262 30Yes67 0.0011975 0.00014262 31Yes66 0.00147627 0.00014263 33Yes66 0.00149837 0.00014274 34Yes67 0.00119834 0.00014274 34Yes66 0.00149837 0.00014274					
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35 Yes 66 0.00147567 0.00014182					
46 2.90 67 1100119094 1110011633					
37 Yes 66 0.00149622 0.00014293 38 Yes 67 0.00119716 0.00011891					

Maximum Tower Deflections - Service Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
TI	180 - 160	1.629	27	0.1472	0.0342
T2	160 - 140	1.041	33	0.1118	0.0203
T3	140 - 120	0.691	32	0.0720	0.0188
T4	120 - 100	0.467	34	0.0357	0.0153
T5	100 - 80	0.417	30	0.0062	0.0228
T6	80 - 60	0.407	30	0.0057	0.0306
T7	60 - 40	0.373	30	0.0060	0.0365
T8	40 - 20	0.347	30	0.0167	0.0406
Т9	20 - 5	0.224	30	0.0424	0.0430
T10	5-0	0.061	30	0.0555	0.0439

Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
fi		Comb.	ĭn	•	0	ft
180.00	Low Profile Platform	27	1.629	0.1472	0.0342	57225
172.00	(2) Powerwave 7770.00	27	1.374	0.1335	0.0274	35766
170.00	12 ft Boom / Sector Mount	27	1.313	0.1300	0.0259	28612
160.38	Guy	33	1.050	0.1125	0.0205	15564
150.00	12 ft Boom / Sector Mount	33	0.843	0.0920	0.0191	25584

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Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	īn	0	0	ſt
116.42	Guy	34	0.446	0.0292	0.0138	22638
60.38	Guy	30	0.374	0.0060	0.0364	103383

Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
TI	180 - 160	8.334	2	0.7307	0.2019
T2	160 - 140	5.371	2	0.5817	0.1405
Т3	140 - 120	3.449	13	0.4016	0.1193
T4	120 - 100	2 204	3	0.2106	0.0891
T5	100 - 80	1.861	\$	0.0442	0,1202
T6	80 - 60	1.773	5	0.0339	0,1540
T7	60 - 40	1.620	5	0.0286	0.1789
T8	40 - 20	1.510	5	0.0720	0,1968
T9	20 - 5	0,978	5	0.1846	0.2075
T10	5-0	0.266	5	0.2419	0.2115

Critical Deflections and Radius of Curvature - Design Wind									
Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature			
ſt		Comb.	in	0	o	ft			
180.00	Low Profile Platform	2	8.334	0.7307	0.2019	13520			
172.00	(2) Powerwave 7770.00	2	7.069	0.6737	0.1731	8450			
170.00	12 ft Boom / Sector Mount	2	6.764	0.6591	0.1665	6760			
160.38	Guy	2	5.418	0.5848	0.1412	3669			
150.00	12 ft Boom / Sector Mount	2	4.279	0.4945	0.1290	5748			
116.42	Guy	5	2.081	0.1750	0.0903	4357			
60.38	Guy	5	1.622	0.0285	0.1785	23645			

Bolt Design Data

Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of	Maximum Load per	Allowable Load	Ratio Load	Allowable Ratio	Criteria
	ft			in	Bolts	Bolt Ib	lb	Allowable	-	
T 1	180	Leg	A325N	0.7500	4	264.86	29820.60	0.009	1	Bolt Tension
		Torque Arm Top@160.375	A325N	0,7500	2	5945.80	17892.40	0.332 🖌	1	Bolt Shear
		Torque Arm Bottom@160.37 5	A325N	0.7500	2	4830.53	17892.40	0.270	1	Bolt Shear
T2	160	Leg	A325N	0,7500	4	3068.00	29820,60	0.103	1	Bolt Tension
T3	140	Leg	A325N	0.7500	4	2817.32	29820 60	0.094 🖌	1	Bolt Tension

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Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of	Maximum Load per	Allowable Load	Ratio Load		wable atio	Criteria
110.	ft	, ypc	-	în	Bolts	Bolt Ib	lb	Allowal	ble		
T4	120	Leg	A325N	0.7300	4	3269.70	29820.60	0.110	/	1	Balt Tension
		Torque Arm Top@116.417	A323N	0.7300	2	3981.98	17892.40		~	1	Bolt Shear
		Torque Arm Bottom@116.41 7	A325N	0.7500	2	2521.73	17892 40	0.141		1	Bolt Shear
TS.	100	Leg	A325N	0.7500	4	3823.24	29820.60	0.128		1	Bolt Tension
T6	80	Leg	A325N	0.7500	4	3726.73	29820.60		/	1	Bolt Tensior
Т7	60	Leg	A325N	0.7500	4	4132.79	29820.60		/	1	Bolt Tension
T 8	40	Leg	A325N	0.7500	4	4457.73	29820.60			1	Bolt Tensior
Т9	20	Leg	A325N	0.7500	4	4599.51	29820.60		/	1	Bolt Tensior
T 10	5	Leg	A325N	0.7500	4	4558.31	29820.60		1	1	Bolt Tension

Guv	Design	Data

ection No.	Elevation ft	Size	Initial Tension Ib	Breaking Load Ib	Actual T _u Ib	Allowable	Required S.F.	Actual S.F.
TI	160.38 (A) (541)	5/8 EHS	6360.00	42399.99	13252.80	25440.00	1.000	1.920 🗸
	160.38 (A) (542)	5/8 EHS	6360.00	42399.99	13282.20	25440.00	1.000	1.915 🖌
	160.38 (B) (535)	5/8 EHS	6360.00	42399.99	13138.40	25440.00	1.000	1 936 🖌
	160.38 (B) (536)	5/8 EHS	6360.00	42399.99	13246.20	25440.00	1.000	1.921 🗸
	160.38 (C) (529)	5/8 EHS	6360.00	42399.99	13217.70	25440.00	1.000	1.925 🖌
	160.38 (C) (530)	5/8 EHS	6360.00	42399.99	13074.60	25440.00	1.000	1.946 🖌
T4	116.42 (A) (559)	9/16 EHS	5250.00	35000.04	8333.53	21000.00	1.000	2.520
	116.42 (A) (560)	9/16 EHS	5250.00	35000.04	8337.35	21000.00	1.000	2.519 🖌
	116.42 (B) (553)	9/16 EHS	5250.00	35000.04	8286.39	21000.00	1.000	2.534 🖌
	116.42 (B) (554)	9/16 EHS	5250.00	35000.04	8302.66	21000.00	1.000	2,529
	116.42 (C) (547)	9/16 EHS	5250.00	35000.04	8335.75	21000.00	1.000	2,519
	116.42 (C) (548)	9/16 EHS	5250.00	35000.04	8285.13	21000.00	1.000	2.535 🖌
Т6	60.38 (A) (567)	9/16 EHS	5250.00	35000.04	8336.18	21000.00	1,000	2.519 🖌
	60.38 (B) (566)	9/16 EHS	5250.00	35000.04	8346.56	21000.00	1.000	2.516
	60.38 (C) (565)	9/16 EHS	5250.00	35000.04	8354.57	21000.00	1.000	2.514 🖌

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Compression Checks

Leg Design Data (Compression)

Section No	Elevation	Size	L	Lu	Kl/r	A	Mast Stability	Ри	¢P _n	Ratio P _u
	fi		fi	ſt		in ²	Index	lb	lb	₽"
Tl	180 - 160	P2.5x 203	20.00	3.21	40.6 K≖1.00	1 7040	1.00	-40829.80	82983.90	0.492
T2	160 - 140	P2 5x 203	20.00	3.21	40.6 K≖1,00	1.7040	1.00	-38947 50	82983.90	0.469
Т3	140 - 120	P2.5x.203	20.00	3.21	40,6 K=1,00	1.7040	1,00	-41608.10	79606,90	0.523
Т4	120 - 100	P2.5x.203	20,00	3.21	40.6 K≖1.00	1,7040	1.00	-51064,30	79606.90	0.641
T5	100 - 80	P2,5x.203	20.00	3.21	40.6 K=1.00	1.7040	1.00	-47818.20	82983.90	0.576
T 6	80 - 60	P2.5x.203	20,00	3,21	40.6 K≖1.00	1.7040	1.00	-49586.80	82983.90	0.598
T7	60 - 40	P2.5x.203	20.00	3.21	40.6 K=1.00	1.7040	0.98	-54713.80	78143.80	0.700 ¹
Т8	40 - 20	P2.5x.203	20.00	3.21	40,6 K=1,00	1.7040	0,98	-56581.80	81392.10	0.695 '
T 9	20 - 5	P2.5x.203	15.00	3.56	45.1 K=1.00	1.7040	1.00	-56216.00	80094.30	0.702 '
T 10	5-0	P2.5x 203	5.39	4,99	20.9 K=0.33	1.7040	0.88	-58957.20	81517.60	0.723 '

 $^{1}P_{\mu}$ / ϕP_{n} controls

		Horizor	ntal De	sign	Data (Comp	pression	1)	
Section No	Elevation	Size	L	Lu	Kl/r	A	Pu	φP _n	Ratio P _u
	ft		ft	ft		in ²	lb	lb	φ <i>P</i> ,,
TI	180 - 160	L1 3/4x1 3/4x3/16	3.50	3.26	117.0 K=1.03	0.6211	-6117.78	9793.71	0.625
T2	160 - 140	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0.5273	-5261.67	7190,10	0.732
Т3	140 - 120	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0.5273	-5217,59	7190.10	0.726 '
T 4	120 - 100	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0.5273	-4350.83	7190.10	0.605
T5	100 - 80	L1 1/2x1 1/2x3/16	3.50	3 26	128.2 K=0.96	0.5273	-4000.40	7190.10	0.556 '
T6	80 - 60	LI 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0 5273	-4224 07	7190.10	0.587 1
Т7	60 - 40	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0,5273	-3952.32	7190.10	0.550 '

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Section No.	Elevation	Size	L	L_{u}	Kl/r	А	Pu	\mathbf{P}_n	Ratio P _u
ſl		ft	ft		in ²	lb	Ib	hP_n	
T &	40 - 20	L1 1/2x1 1/2x3/16	3,50	3.26	128 2 K=0.96	0.5273	-4069 23	7190.10	0.366
T9	20 - 3	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0.5273	-3547 70	7190.10	0.493

 $P_{\mu} \neq P_{\pi}$ controls

		Тор G	irt Des	sign D	Data (C	Compr	ession)		
Section No.	Elevation	Size	L	Ĺ	Kl/r	A	P _u	$\mathbf{A}P_n$	Ratio P _u
	ft		ft	ft		in ²	lb	lb	ϕP_n
TI	180 - 160	L1 3/4x1 3/4x3/16	3.50	3.26	117.0 K=1.03	0.6211	-3613.37	9793.71	0,369
T2	160 - 140	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K≖0.96	0.5273	-3623.65	7190.10	0.504 1
тэ	140 - 120	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0.5273	-2618.32	7190_10	0.364
T5	100 - 80	L1 1/2x1 1/2x3/16	3,50	3.26	128.2 K=0.96	0.5273	-2460.04	7190.10	0.342
T 6	80 - 60	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0.5273	-2098.75	7190.10	0.292
T7	60 - 40	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0,5273	-1992,38	7190.10	0.277 '
T8	40 - 20	L1 1/2x1 1/2x3/16	3,50	3.26	128.2 K=0.96	0,5273	-2003.74	7190.10	0 279 '
T9	20 - 5	L1 1/2x1 1/2x3/16	3.50	3 26	128.2 K=0.96	0,5273	-1904.19	7190,10	0.265 1

¹ P_u / ϕP_n controls

Bottom	Girt Design	Data (Com	pression)

Section No.	Elevation	Size	L	L_n	Kl/r	A	P _u	ϕP_n	Ratio P _u
	ft		ft	ft		in ²	lb	lb	ϕP_n
T2	160 - 140	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0.5273	-2723.28	7190.10	0.379
Т3	140 - 120	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0.5273	-3250.87	7190.10	0.452
T 4	120 - 100	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0.5273	-2220.54	7190.10	0.309 1
T5	100 - 80	L1 1/2x1 1/2x3/16	3.50	3 26	128.2 K=0.96	0.5273	-2095 82	7190.10	0,291
T7	60 - 40	L1 1/2x1 1/2x3/16	3.50	3.26	128.2 K=0.96	0.5273	-2230.42	7190.10	0.310
T8	40 - 20	L1 1/2x1 1/2x3/16	3.50	3.26	128.2	0.5273	-2009.97	7190.10	0.280

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Section No.	Elevation	Size	L	L_{μ}	Kl/r	Α	P_u	¢₽"	Ratio P _n
1101	ft		ft	ft		in ²	lb	lb	♦P _n
			K=0.96						V

 P_{μ} / P_{π} controls

		Top Guy P	ull-Off	Desi	gn Da	ta (Co	ompres	sion)	
Section No.	Elevation	Size	L	Lu	Kl/r	Å	Pu	¢P,	Ratio P _u
	ft		ft	ft		in ²	lb	lb	ϕP_n
TI	180 - 160	L1 1/2x1 1/2x3/16	3,50	3.26	86.7 K≖0.65	0.5273	-8971.52	11503.00	0.780
T4	120 - 100	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-3242.26	11503.00	0.282
T 6	80 - 60	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-601.02	11503.00	0.052 1

 $P_{\mu} / \phi P_{\mu}$ controls

	Top Guy Pull-Off Bending Design Data										
Section	Elevation	Size	Mux	♠M _{nx}	Ratio	Muy	ϕM_{ny}	Ratio			
No.					Mux		• •	Muv			
	ft		lb-fi	lb-ft	♦M _{ns}	lb-ft	lb-ft	ϕM_{ny}			
T1	180 - 160	L1 1/2x1 1/2x3/16	0.00	711.05	0.000	0.00	368.03	0.000			
T4	120 - 100	L1 1/2x1 1/2x3/16	0.00	711.05	0.000	0.00	368.03	0.000			
T6	80 - 60	L1 1/2x1 1/2x3/16	0.00	711.05	0.000	0.00	368.03	0.000			

Top Guy Pull-Off Interaction Design Data

Section	Elevation	Size	Ratio	Ratio	Ratio	Comb.	Allow.	Criteria
No			Pu	M _{ux}	$M_{\mu\nu}$	Stress	Stress	
ft		♦ <i>P</i> _{<i>n</i>}	¢M _{nx}	ϕM_{ny}	Ratio	Ratio		
TI	180 - 160	L1 1/2x1 1/2x3/16	0.780	0.000	0.000	0.780 1	1.000	4.9-3 🖌
T4	120 - 100	L1 1/2x1 1/2x3/16	0.282	0,000	0.000	0.282 '	1.000	4.9-3
Т6	80 - 60	L1 1/2x1 1/2x3/16	0.052	0.000	0.000	0.052 '	1.000	4.9-4 🖌

 $^{1}P_{u}$ / ϕP_{a} controls

Bottom Guy Pull-Off Design Data (Compression)

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Section No.	Elevation	Size	L	Lu	Kl/r	A	P_{u}	¢P _n	Ratio P _u
110	ft		ft	ft		in ²	lb	lb	P _n
TI	180 - 160	L1 1/2x1 1/2x3/16	3,50	3.26	86.7 K=0.63	0 5273	-4345 41	11503.00	0.378 '
T4	120 - 100	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.63	0 3273	-6497 29	11503.00	0.565

 $1_{P_u} / \phi P_n$ controls

Bottom Guy Pull-Off Bending Design Data									
Section No.	Elevation	Size	Mux	♦ <i>M_{nx}</i>	Ratio M _{ux}	Muy	♠ <i>M</i> _{ny}	Ratio M _{uv}	
10.	fi		lb-ft	lb-ft	¢M _µ ,	lb-fi	lb-ft		
T1	180 - 160	L1 1/2x1 1/2x3/16	0,00	711.05	0.000	0.00	368.03	0.000	
T4	120 - 100	L1 1/2x1 1/2x3/16	0.00	711.05	0.000	0.00	368.03	0.000	

Bottom Guy Pull-Off Interaction Design Data										
Section No.	Elevation	Size	Ratio Pu	Ratio M _{ux}	Ratio M _{uv}	Comb. Stress	Allow. Stress	Criteria		
	ft		ϕP_n	$\phi M_{n\pi}$	$\phi M_{\mu\nu}$	Ratio	Ratio			
TI	180 - 160	L1 1/2x1 1/2x3/16	0.378	0.000	0.000	0.378	1.000	4.9-3		
T4	120 - 100	L1 1/2x1 1/2x3/16	0.565	0.000	0.000	0.565	1.000	4.9-3 🖌		

 $^{1}P_{u}$ / ϕP_{u} controls

Torque-Arm Bottom Design Data

Section No.	Elevation	Size	L	L_u	Kl/r	A	P_{u}	ϕP_n	Ratio Pu
	ft		ſl	ft		in ²	lb	lb	φ <i>P</i> ,,
T 1	180 - 160 (533)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-9570.23	36439.50	0.263
T1	180 - 160 (534)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-9661.05	36439.50	0 265 1
T1	180 - 160 (539)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-9501 25	36439.50	0.261 '
T 1	180 - 160 (540)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1,4400	-9572,14	36439.50	0.263
T1	180 - 160 (545)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-9597 19	36439.50	0.263
Tl	180 - 160 (546)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-9623-14	36439.50	0.264

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Section No.	Elevation	Size	L	L_{u}	Kl/r	А	P_u	$\mathbf{A}P_n$	Ratio P ₁
	ft		ft	ft		in ²	lb	lb	P_n
T4	120 - 100 (551)	L3x3x1/4	3.30	3.38	68.5 K=1.00	1.4400	-5043.46	36439.50	0.138
T4	120 - 100 (352)	L3x3x1/4	3.50	3.38	68.3 K=1.00	1.4400	-3029.72	36439.50	0.138
T4	120 - 100 (557)	L3x3x1/4	3,30	3.38	68.3 K=1.00	1,4400	-4915.74	36439.50	0.135
T4	120 - 100 (558)	L3x3x1/4	3,50	3.38	68.5 K=1.00	1,4400	-4915.11	36439.50	0.135
T4	120 - 100 (563)	L3x3x1/4	3,50	3,38	68.5 K=1.00	1.4400	-5001,15	36439.50	0.137
T 4	120 - 100 (564)	L3x3x1/4	3,50	3.38	68.5 K=1,00	1.4400	-4989.68	36439.50	0.1371

 $P_{\mu} / \phi P_{\mu}$ controls

Tension Checks

			Leg Des	sign E	Data (Tensio	on)		
Section No.	Elevation	Size	L	L _u	Kl/r	A	P _u	♦ <i>P</i> _n	Ratio P _u
	ft		ft	ft		in ²	lb	lb	♦P _n
Tl	180 - 160	P2.5x 203	20.00	3.21	40.6	1.7040	15020.10	96619.60	0.155
T2	160 - 140	P2.5x 203	20.00	3.21	40.6	1.7040	10202.00	96619.60	0,106 1
Т3	140 - 120	P2.5x.203	20.00	3.21	40.6	1.7040	4996.16	92018.70	0.054 '
T 4	120 - 100	P2.5x.203	20.00	3.21	40_6	1.7040	4992.29	92018.70	0.054 '

 $^{1}P_{u}$ / ϕP_{n} controls

Diagonal Design Data (Tension)

Section No.	Elevation	Size	L	Lu	Kl/r	A	Ри	¢₽ _n	Ratio P _u
110.	ft		ft	ft		in^2	lb	lb	φP _n
TI	180 - 160	5/8	4.75	4.42	339.7	0.3068	7000.12	9940.20	0.704 1
T2	160 - 140	5/8	4.75	4.42	339.7	0.3068	5166.59	9940.20	0.520
Т3	140 - 120	5/8	4.75	4.42	339.7	0,3068	4388.11	9940,20	0.441
T4	120 - 100	5/8	4.75	4.42	339,7	0.3068	4134,64	9940,20	0.416

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Section	Elevation	Size	L	$L_{w_{i}}$	Kl/r	A	P_{u}	$\bullet P_n$	Ratio P _n
No.	fi		ft	ft		in ²	lb	lb	
T5	100 - 80	3/8	4.75	4.42	339.7	0.3068	3791.07	9940.20	0.381
T6	80 - 60	5/8	4.75	4.42	339.7	0.3068	3686.41	9940.20	0.371
T7	60 - 40	5/8	4.75	4,42	339.7	0.3068	3950.27	9940.20	0.397
Т8	40 - 20	5/8	4.75	4.42	339.7	0.3068	3111.34	9940.20	0.313
T9	20 - 5	5/8	4,99	4.65	357.3	0.3068	3442.55	9940.20	0.346

¹ P_u / ϕP_n controls

Ratio P _u	ϕP_n	Pu	A	Kl/r	L _H	Ĺ	Size	Elevation	Section No.
ϕP_n	lb	lb	in ²		ft	ft		ſl	100.
0.035 '	20123.40	707.19	0.6211	72,9	3.26	3.50	L1 3/4x1 3/4x3/16	180 - 160	TI
0.039	17085,90	674.59	0.5273	85.7	3.26	3.50	L1 1/2x1 1/2x3/16	160 - 140	T2
0.042 ¹	17085.90	720.67	0.5273	85:7	3.26	3.50	L1 1/2x1 1/2x3/16	140 - 120	Т3
0.052 ¹	17085.90	884.46	0.5273	85. 7	3.26	3.50	L1 1/2x1 1/2x3/16	120 - 100	T4
0.048 1	17085.90	828.24	0.5273	85.7	3.26	3,50	L1 1/2x1 1/2x3/16	100 - 80	T5
0.050 '	17085.90	858.87	0.5273	85.7	3.26	3.50	L1 1/2x1 1/2x3/16	80 - 60	T 6
0.055 1	17085.90	947.67	0.5273	85.7	3.26	3.50	L1 1/2x1 1/2x3/16	60 - 40	T7
0.057 1	17085.90	980.03	0.5273	85.7	3.26	3.50	L1 1/2x1 1/2x3/16	40 - 20	T8
0.057	17085.90	973.69	0.5273	85.7	3.26	3.50	L1 1/2x1 1/2x3/16	20 - 5	т9

 $^{1}P_{u}$ / ϕP_{a} controls

	Top Girt Design Data (Tension)									
Section No.	Elevation	Size	L	Lu	Kl/r	A	P _u	♦ <i>P</i> _n	Ratio P _u	
140.	fi		ſt	ft		in ²	lb	lb	ϕP_{μ}	
T10	5 - 0	L1 1/2x1 1/2x3/16	3.24	3.00	78.8	0.5273	6384.59	17085.90	0.374	

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Section No.	Elevation	Size	L	L_{u}	KI/r	A	P _u	ϕP_n	Ratio P.
	ft		ft	ft		in ²	lb	lb	►P _n
									V

$^{1}P_{\mu}$ / ϕP_{μ} controls

		Botto	m Girl	Desi	gn Da	ata (Te	nsion)		
Section No.	Elevation	Size	L	Lu	Kl/r	А	Pu	♦P _n	Ratio P _u
	fi		ft	ft		in ²	lb	lb	ϕP_n
T9	20 - 5	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	5712.70	17085.90	0.334

$^{1}P_{\mu}$ / ϕP_{n} controls

		Top Guy	y Pull-	Off De	esign	Data (Tensio	n)	
Section No.	Elevation	Size	L	L_u	Kl/r	A	P _u	¢P _n	Ratio P _n
	fi		ft	ft		in ²	lb	lb	ϕP_n
Т6	80 - 60	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	2553.50	17085.90	0.1491

 $^{1}P_{u}$ / ϕP_{u} controls

	Top Guy Pull-Off Bending Design Data									
Section No.	Elevation	Size	M _{ux}	¢M _{nx}	Ratio M _{ux}	Muy	ϕM_{ny}	Ratio M _{uv}		
	ſt		lb-fi	lb-fl	♦M _{nx}	lb-ft	lb-ft	♦ <i>M</i> _{<i>ny</i>}		
T6	80 - 60	L1 1/2x1 1/2x3/16	-10.33	711.05	0.015	-10.33	368.03	0.028		

Top Guy Pull-Off Interaction Design Data

Section No.	Elevation	Size	Ratio P _u	Ratio M _{ux}	Ratio M _{uy}	Comb. Stress	Allow. Stress	Criteria
	ft		¢P.	ϕM_{nx}	ϕM_{av}	Ratio	Ratio	
T6	80 - 60	L1 1/2x1 1/2x3/16	0.149	0.015	0 028	0.149 '	1.000	4.9-4 🚩

 $^{1}P_{u}$ / ϕP_{u} controls

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		То	orque-A	rm To	op De	esign [Data		
Section No.	Elevation	Size	Ĺ	L_u	Kl/r	Å	P _u	♦ <i>P</i> ,	Ratio Pu
	fi		ft	ft		in ²	lb	lb	ϕP_{μ}
Tl	180 - 160 (\$31)	L2x2x5/16	4.75	4.59	91.6	1.1500	11591.70	37260.00	0.311
TI	180 - 160 (532)	L2x2x5/16	4.75	4,59	91.6	1.1500	11885.90	37260 00	0.319
TI	180 - 160 (537)	L2x2x5/16	4.75	4.59	91.6	1.1500	11891 60	37260.00	0.3191
T 1	180 - 160 (\$38)	L2x2x5/16	4.75	4.59	91.6	1,1500	11856.70	37260.00	0.318
TI	180 - 160 (543)	L2x2x5/16	4.75	4.59	91.6	1.1500	11568,50	37260,00	0.310
τı	180 - 160 (544)	L2x2x5/16	4.75	4,59	91,6	1.1500	11830.20	37260.00	0.318
T4	120 - 100 (549)	L2x2x5/16	4.75	4.59	91.6	1.1500	7938.95	37260.00	0.213
T4	120 - 100 (550)	L2x2x5/16	4.75	4.59	91.6	1.1500	7888.89	37260.00	0,212
T4	120 - 100 (555)	L2x2x5/16	4.75	4,59	91.6	1.1500	7877.34	37260.00	0.211
T4	120 - 100 (556)	L2x2x5/16	4,75	4,59	91.6	1.1500	7923.77	37260.00	0.213 1
T4	120 - 100 (561)	L2x2x5/16	4.75	4,59	91.6	1.1500	7963.95	37260.00	0.214 1
T4	120 - 100 (562)	L2x2x5/16	4.75	4.59	91.6	1.1500	7867.41	37260.00	0.211

 $^{1}P_{u}$ / ϕP_{n} controls

		Tor	que-Arı	m Bot	tom I	Desigr	n Data		
Section No.	Elevation	Size	L	L_{u}	Kl/r	A	Pu	♦ <i>P</i> _n	Ratio P _u
110	ſt		ſl	ft		in ²	lb	lb	φ <i>P</i> ,,
TI	180 - 160 (533)	L3x3x1/4	3.50	3.38	43,6	1.4400	3033.30	46656.00	0.065
Tl	180 - 160 (534)	L3x3x1/4	3.50	3,38	43.6	1.4400	3144.18	46656.00	0.067 1
T 1	180 - 160 (539)	L3x3x1/4	3.50	3.38	43.6	1.4400	3026.46	46656.00	0.065 1
T 1	180 - 160 (540)	L3x3x1/4	3.50	3,38	43.6	1.4400	3065.45	46656.00	0.066 '
Tl	180 - 160 (545)	L3x3x1/4	3.50	3.38	43.6	1.4400	3040.45	46656.00	0.065 '
Τl	180 - 160 (546)	L3x3x1/4	3.50	3.38	43.6	1,4400	3123.34	46656.00	0.067
T 4	120 - 100 (551)	L3x3x1/4	3,50	3.38	43.6	1,4400	2067.02	46656.00	0.044 '

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Section No.	Elevation	Size	L	L_u	Kl/r	Α	P_{u}	ΦP_n	Ratio P _u
	ft		fi	ſl		in ²	lb	lb	¢P _n
44	120 - 100 (552)	1.3x3x1/4	3.50	3.38	43.6	1.4400	2084.22	46656.00	0.045
T4	120 - 100 (557)	L3x3x1/4	3.50	3.38	43.6	1.4400	1964.41	46636.00	0.042
T4	120 - 100 (558)	L3x3x1/4	3.50	3,38	43.6	1,4400	1965.03	46636.00	0.042
T4	120 - 100 (563)	L3x3x1/4	3.50	3.38	43,6	1,4400	2031.95	46656.00	0.044
T4	120 - 100 (564)	L3x3x1/4	3,50	3,38	43.6	1.4400	2046.50	46656.00	0.044

 $P_{\mu} / \phi P_n$ controls

Section Capacity Table

Section	Elevation	Component	Size	Critical	Р	@Pallow	%	Pass
No.	ft	Туре		Element	lb	lb	Capacity	Fail
TI	180 - 160	Leg	P2.5x.203	3	-40829.80	82983.90	49.2	Pass
		Diagonal	5/8	31	7000.12	9940.20	70.4	Pass
		Horizontal	L1 3/4x1 3/4x3/16	52	-6117.78	9793.71	62.5	Pass
		Top Girt	L1 3/4x1 3/4x3/16	4	-3613.37	9793.71	36.9	Pass
		Guy A@160.375	5/8	542	13282.20	25440.00	52.2	Pass
		Guy B@160.375	5/8	536	13246.20	25440.00	52.1	Pass
		Guy C@160.375	5/8	529	13217.70	25440.00	52.0	Pass
		Top Guy Pull-Off@160.375	L1 1/2x1 1/2x3/16	16	-8971.52	11503.00	78.0	Pass
		Bottom Guy Pull-Off@160.375	L1 1/2x1 1/2x3/16	7	-4345.41	11503,00	37.8	Pass
		Torque Arm Top@160.375	L2x2x5/16	537	11891.60	37260,00	31.9 33.2 (b)	Pass
		Torque Arm Bottom@160.375	L3x3x1/4	534	-9661.05	36439.50	26:5 27.0 (b)	Pass
T2	160 - 140	Leg	P2.5x.203	63	-38947.50	82983.90	46.9	Pass
		Diagonal	5/8	115	5166.59	9940.20	52.0	Pass
		Horizontal	L1 1/2x1 1/2x3/16	112	-5261.67	7190.10	73.2	Pass
		Top Girt	L1 1/2x1 1/2x3/16	64	-3623.65	7190.10	50.4	Pass
		Bottom Girt	L1 1/2x1 1/2x3/16	67	-2723.28	7190.10	37.9	Pass
Т3	140 - 120	Leg	P2.5x.203	123	-41608.10	79606.90	52.3	Pass
		Diagonal	5/8	135	4388.11	9940.20	44.1	Pass
		Horizontal	L1 1/2x1 1/2x3/16	136	-5217.59	7190.10	72.6	Pass
		Top Girt	L1 1/2x1 1/2x3/16	124	-2618.32	7190.10	36.4	Pass
		Bottom Girt	L1 1/2x1 1/2x3/16	127	-3250.87	7190.10	45.2	Pass
T4	120 - 100	Leg	P2.5x.203	183	-51064.30	79606.90	64.I	Pass
		Diagonal	5/8	231	4134.64	9940.20	41.6	Pass
		Horizontal	L1 1/2x1 1/2x3/16	214	-4350.83	7190.10	60.5	Pass
		Bottom Girt	L1 1/2x1 1/2x3/16	189	-2220.54	7190.10	30.9	Pass
		Guy A@116.417	9/16	560	8337.35	21000.00	39.7	Pass
		Guy B@116.417	9/16	554	8302.66	21000.00	39.5	Pass
		Guy C@116.417	9/16	547	8335,75	21000.00	39.7	Pass
		Top Guy Pull-Off@116.417	L1 1/2x1 1/2x3/16	185	-3242.26	11503.00	28.2	Pass
		Bottom Guy Pull-Off@116.417	L1 1/2x1 1/2x3/16	232	-6497.29	11503.00	56.5	Pass
		Torque Arm	L2x2x5/16	561	7963.95	37260.00	21.4	Pass

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Section	Elevation	Component	Size	Critical	Р	oP_{allow}	%	Pass
No.	ft	Туре		Element	lb	lb	Capacity	Fail
		Top@116.417					22 3 (b)	
		Torque Arm	L3x3x1/4	551	-5043.46	36439.30	13.8	Pass
		Bottom@116.417					14.1 (b)	
T3	100 - 80	Leg	P2.3x 203	243	-47818.20	\$2983.90	\$7.6	Pass
		Diagonal	5/8	300	3791.07	9940.20	38.1	Pass
		Horizontal	L1 1/2x1 1/2x3/16	292	-4000.40	7190.10	33 6	Pass
		Top Girt	L1 1/2x1 1/2x3/16	244	-2460.04	7190.10	34.2	Pass
		Bottom Girt	L1 1/2x1 1/2x3/16	249	-2095.82	7190.10	29.1	Pass
T6	80 - 60	Leg	P2 5x 203	301	-49586.80	82983.90	\$9.8	Pass
		Diagonal	5/8	310	3686.41	9940.20	37.1	Pass
		Horizontal	L1 1/2x1 1/2x3/16	317	-4224.07	7190.10	58.7	Pass
		Top Girt	L1 1/2x1 1/2x3/16	305	-2098.75	7190.10	29.2	Pass
		Guy A@60.375	9/16	567	8336.18	21000.00	39.7	Pass
		Guy B@60.375	9/16	566	8346.56	21000.00	39.7	Pass
		Guy C@60.375	9/16	565	8354.57	21000.00	39.8	Pass
		Top Guy	L1 1/2x1 1/2x3/16	307	2553.50	17085.90	14.9	Pass
		Pull-Off@60.375						
T7	60 - 40	Leg	P2.5x.203	361	-54713.80	78143.80	70.0	Pass
		Diagonal	5/8	419	3950.27	9940.20	39.7	Pass
		Horizontal	L1 1/2x1 1/2x3/16	377	-3952.32	7190.10	55.0	Pass
		Top Girt	L1 1/2x1 1/2x3/16	365	-1992.38	7190.10	27.7	Pass
		Bottom Girt	L1 1/2x1 1/2x3/16	369	-2230.42	7190.10	31.0	Pass
T8	40 - 20	Leg	P2.5x.203	421	-56581.80	81392.10	69.5	Pass
		Diagonal	5/8	480	3111.34	9940.20	31.3	Pass
		Horizontal	L1 1/2x1 1/2x3/16	474	-4069.23	7190.10	56.6	Pass
		Top Girt	L1 1/2x1 1/2x3/16	424	-2003.74	7190.10	27.9	Pass
		Bottom Girt	L1 1/2x1 1/2x3/16	429	-2009.97	7190.10	28.0	Pass
T9	20 - 5	Leg	P2 5x.203	481	-56216.00	80094.30	70.2	Pass
		Diagonal	5/8	490	3442.55	9940.20	34.6	Pass
		Horizontal	L1 1/2x1 1/2x3/16	497	-3547.70	7190.10	49.3	Pass
		Top Girt	L1 1/2x1 1/2x3/16	484	-1904.19	7190 .10	26.5	Pass
		Bottom Girt	L1 1/2x1 1/2x3/16	487	5712.70	17085.90	33.4	Pass
T10	5-0	Leg	P2_5x.203	525	-58957.20	81517.60	72.3	Pass
		Top Girt	L1 1/2x1 1/2x3/16	528	6384.59	17085.90	37.4	Pass
							Summary	
						Leg (T10)	72.3	Pass
						Diagonal (T1)	70.4	Pass
						Horizontal (T2)	73.2	Pass
						Top Girt	50.4	Pass

	Sminna	У
Leg (T	10) 72.3	Pass
Diago (T1		Pass
Horizo (T2		Pass
Top C (T2		Pass
Bottom (T3		Pass
Guy A	(T1) 52.2	Pass
Guy B	(T1) 52.1	Pass
Guy C	(T1) 52.0	Pass
Top G Pull-C (T1	ffC	Pass
Bottom Pull-C (T4	Off	Pass
Torque Top (1		Pass
Torque Bottom		Pass
Bolt Ch	ecks 33.2	Pass
RATIN	IG = 78.0	Pass

RISATower	Job	117-23243.2	Page 45 of 45
	Project	Colchester, CT	Date 22:10:56 01/14/18
Phone: FAX:	Client	CDT	Designed by FAN

Site Name:	
Client:	
Job Number:	
Date:	

Colchester CDT 117-23243.2 1/14/2018

Design Base Loads (Factored) per TIA-222-G

Moment (M _u);	0.0	k-ft
Shear/Leg (Vu):	2.5	k
Compression/Leg (Pu):	161.6	k
Uplift/Leg (Tu):	0.0	k
Diameter of Prismatic Portion of Pier (d):	1.0	ft
Depth to Base of Foundation:	2.0	ft
Pier Height Above Ground (h):	2.0	ft
Length / Width of Pad (w):	6.0	ft.
Thickness of Pad (t):	4.0	ft
Depth Below Ground Surface to Water Table (w):	20.0	ft
Unit Weight of Concrete:	150.0	pcf
Unit Weight of Water:	62.4	pcf
Unit Weight of Soil Above Water Table:	120.0	pcf
Unit Weight of Soil Below Water Table:	65.0	pcf
Friction Angle of Uplift from Top of Pad:	30	Degrees
Friction Angle of Uplift from Base of Pad:	30	Degrees
Uplift Angle Started at Top or Base of Pad (T/B):	т	
Ultimate Skin Friction:		psf
Ultimate Compressive Bearing Pressure:	10000	psf
Capacity Increase (Due to Transient Loads):	1.00	
Bearing Strength Reduction Factor (ϕ_s):	0.60	
Uplift Strength Reduction Factor (ϕ_s):	0.75	

Concrete Compressive Strength (f'c):	3000.0	psi
Bending/Tension Reduction Factor (\$e):	0.90	
Shear Reduction Factor (\$v):	0.75	
Compression Reduction Factor (ϕ_V) :	0.65	
Steel Elastic Modulus:	29000	ksi
Pad Steel Rebar Size #:	4	1000
Pad Steel Rebar Area:	0.20	in²
Pad Steel Rebar Yield Strength (Fy):	60	ksi
# of Rebar in Top of Pad:		
# of Rebar in Base of Pad:	2	
Pad Clear Cover:	3	in

Axial Capacities

Nominal Uplift Capacity per Leg ($\phi_s T_n$):
Nominal Compressive Capacity per Leg ($\phi_s P_n$):
P _u :
T _u /φ _s T _n :
P _u /φ _s P _n :

12.0	k	
216.0	k	
167.0	k	
0.00	Result:	ОК
0.77	Result:	ОК

Site Name:	Colchester
Client:	CDT
Job Number:	117-23046
Date:	1/14/2018

Design Standard per TIA-222-G

Anchor Radius:	145.0	ft
Uplift (Factored - Pu):	32.7	k
Shear (Factored - V.):	38.3	k
Anchor Base Depth (d):	7.5	ft
Width of Anchor (W):	5.5	ft
Length of Anchor (L):	11.5	ft
Thickness of Anchor (t):	2.0	ft
Depth Below Ground Surface to Water Table (w):	20.0	ft
Soil Uplift at Base / Top of Anchor (B/T):	т	
Unit Weight of Concrete:	150.0	pcf
Unit Weight of Soil Above Water Table:	120.0	pcf
Unit Weight of Water:	62.4	pcf
Submerged Soil Unit Weight:	65.0	pcf
Internal Angle of Friction:	30	Degrees
Cohesion:	500	psf
Ultimate Skin Friction of Pad Sides to Soil:	0	psf
Ultimate Coefficient of Shear Friction:	0.30	
Maximum Top Conical Failure Angle:	30	Degrees
Maximum Base Conical Failure Angle:	30	Degrees
Allowable Capacity Increase:	1.00	(Due to Transient Loads)
Uplift Strength Reduction Factor (ϕ_u):	0.75	
Shear Strength Reduction Factor (ϕ_v):	0.75	
Concrete Uplift Strength Reduction Factor (ϕ_u):	0.90	
1 1		

<u>Uplift</u>

Weight of Concrete (Buoyancy Effect Considered):	19.0 k
Weight of Soil (Buoyancy Effect Considered):	84.3 k
Ultimate Uplift Resistance from Skin Friction:	0.0 k
Nominal Factored Uplift Resistance ($\phi_u P_n$)::	80.3 k
Բս / փս Բո:	0.41 Result: OK

<u>Shear</u>

Ultimate Shear Friction Resistance Due to Normal Force - Uplift:	10.9 k
Passive Pressure:	4072 psf
Ultimate Passure Pressure Resistance:	93.7 k
Nominal Shear Resistance ($\phi_v V_n$):	78.4 k
$V_u / \phi_v V_n$:	0.49 Result: OK

Anchor Rod Capacity

# of Anchor Rods:	1	Rod F _y :	47 ks	si
Anchor Rod Gross Area:	2.41 in ²	Rod F _u :	62 ks	si
Anchor Rod Net Area:	2.41 in ²	φ _v :	0.80	
Resultant Tensile Load (T _u):	50.4 k	φ _t :	0.65	
Anchor Rod Tensile Resistance (ϕT_n)::	90.4 k			
Τ _u / φΤ _n :	0.56 Result:	OK		

Strength Analysis of Reinforced Concrete

Concrete Compressive Srength (f ^e e):	3000	
Longitudinal Rebar Yield Strength:	60000	psi
# Longitudinal Rebar (Top):	9	
# Longitudinal Rebar (1 Side):	3	
Rebar Size:	4	
Strength Reduction Factor for Shear (,):	0.75	
Strength Reduction Factor for Flexure (\$+):	0,9	
Compression Zone Factor (B ₁):	0.85	
Area of Single Rebar:	0.20	in ^a
One Way Shear due to Shear Load (V _u):	10.5	k
Nominal One Way Shear Capacity for Shear Load ($\phi_e V_n$):	122.3	k
Vu/ 6 vVn:	0.09	Result: OK
One Way Shear due to Uplift (V _u):	14.0	k
Nominal One Way Shear Capacity for Uplift (\$\vert_vV_n):	108.4	k
Vu/¢vVn:	0.13	Result: OK
Pad Flexure due to Shear Load (M _u):	55.1	k-ft
Nominal Flexural Capacity for Shear Load (\$\$M_n):	167.4	k-ft
Pad Flexure due to Uplift (M _u):	47.0	k-ft
Nominal Flexural Capacity for Uplift ($\phi_b M_n$):	161.9	k-ft
M _u /φ _b M _n (Max.):	0.33	Result: OK

SITE ID: CT33XC576 SITE NAME: NORTH COLCHESTER

SITE INI	
ADDRESS:	589 OLD HARTFORD ROAD COLCHESTER, CT 06415
JURISDICTION:	TOWN OF COLCHESTER
COUNTY :	NEW LONDON
PROPERTY OWNER:	MARK LEGAULT OLD HARTFORD ROAD COLCHESTER, CT 06415
APPLICANT:	SPRINT 201 STATE ROUTE 17 NORTH RUTHERFORD, NJ 07070
LATITUDE (NAD 83):	N 41.58667222°
LONGITUDE (NAD 83):	W 72.37823888°
CURRENT USE:	UNMANNED TELECOMMUNICATIONS FACILITY
PROPOSED USE: UTILITY COMPANY:	NO CHANGE CONNECTICUT LIGHT AND POWER
	PHONE: 800-266-2000
	FIGURATION
START OF CONSTRUCTION.	NT CONSTRUCTION MANAGER PRIOR TO
PROJEC	
NAME:	COMPANY: PHONE #:
NAME:	COMPANY: PHONE #: MASER 973.398.3110
NAME: ENGINEER: JEREMY MCKEON	COMPANY: PHONE #: N MASER 973.398.3110 CONSULTING P.A.
NAME: NGINEER: JEREMY MCKEON	COMPANY: PHONE #: MASER 973.398.3110
NAME: ENGINEER: JEREMY MCKEON CONSTRUCTION: TOM JUPIN	COMPANY: PHONE #: MASER 973.398.3110 CONSULTING P.A. CHERUNDOLO 973.819.9033
NAME: ENGINEER: JEREMY MCKEON CONSTRUCTION: TOM JUPIN STRUCTION: TOM JUPIN THE PROPOSED ANTENNA AND EQUI INCLUDING THE NEW LOAD CONDIT EXISTING STRUCTURE. THESE PLANS F TELECOMMUNICATION FACILITY TO I WITH THE SCOPE OF WORK PROVIDE INCORPORATED THE SCOPE OF WOR STRUCTURE AFFECTED BY THE SCOPE STRUCTURE AFFECTED BY THE SCOPE COVER. MASER ASSUMES NO RESPONS	COMPANY: PHONE #: MASER 973.398.3110 CONSULTING P.A. CHERUNDOLO 973.819.9033 CONSULTING
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600 OLD HARTFORD ROAD COLCHESTER, CT 06415

DO MACRO PROJECT



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CONSULTING, P.A

- INSTALL (30) JUMPER CABLES
- INSTALL (I) HYBRID CABLE

ROM SPRINT OFFICES, RUTHERFORD, NJ: TAKE NJ-I7 N TO I-80 W. TAKE THE I-80 W EXIT FROM NJ-I7 N. HEAD SOUTH. SLIGHT LEFT TOWARD VETERANS BLVD. TURN RIGHT TOWARD VETERANS BLVD. TURN LEFT ONTO ROUGH ST. TURN RIGHT TOWARD MERGED ONTO L80 W. STATE REGOTTO LARE EXTI ST. OWARD MILAND AVESADDLE BROOK. SETINATION IN COLCHESTER TAKE EXTI 16 FROM CT-2 E MERGE ONTO GANDEN STATE PKWY. KERTERINS NEW YORK. USE THE RIGHT 2 LANES TO AKE EXTI 17 TO MERGE ONTO 1287 E1673. SKEP LEFT AT THE FORK FOLLOW SIGNS FOR WHET PLANISRYE TAKE EXTI 691. TO SAFE STO AKE EXTI 17 TO MERGE TONTO DIAST E1673. SKEP LEFT AT THE FORK FOLLOW SIGNS FOR WESTCHESTER AVE AND MERGE ONTO THE STCHESTER AVE AND MERGE ONTO CT-15 N. KEEP RIGHT TAKE EXTI 690. TO THE STATE FORM TO STAY ON CT-15 N. TAKE EXTI 690. TO STAY ON CT-15 N. TAKE EXTI 690. TO STAY ON CT-16 FOR THE STCHESTER AVE AND MERGE TOWN. TAKE EXTI 25-26 TO MERGE ONTO CT-3 N. TOWARD GLASTONBURY. TAKE THE EXTI ONTO CT-2 E OWARD MERGE ONTO CT-3 N. TOWARD GLASTONBURY. TAKE THE EXTI ONTO CT-2 MERGE ONTO CT-3 N. TOWARD GLASTONBURY. TAKE THE EXTI ONTO CT-2 MESTCHESTER RD. TURN RIGHT ONTO ARATCHEST AVE AVESTCHESTER RD. TURN RIGHT ONTO ARATCHEST AVESTCHESTER RD. TURN RIGHT ONTO ARATCHEST AVESTCHESTER RD. TURN RIGHT ONTO ARATCHEST AVESTCHESTER RD

DRIVING DIRECTIONS

DRAWING INDEX					
NYC DOB NUMBER	REV.				
T-001.00	TITLE SHEET	0			
ANT-001.00	GENERAL NOTES - I	0			
ANT-002.00	GENERAL NOTES - 2	0			
ANT-003.00	GENERAL NOTES - 3	0			
ANT-004.00	SITE PLAN	0			
ANT-005.00	EQUIPMENT PLAN AND ELEVATION	0			
ANT-006.00	ANTENNA ORIENTATION PLAN	0			
ANT-007.00	DETAILS	0			
ANT-008.00	ANTENNA SCHEDULE, WIRING DIAGRAM, BILL OF MATERIALS AND NOTES	0			
ANT-009.00	FIBER PLUMBING DIAGRAMS - I	0			
ANT-010.00	FIBER PLUMBING DIAGRAMS - 2	0			
ANT-012.00	CABLE COLOR CODING, DC POWER DETAILS & PANEL SCHEDULES	0			
ANT-012.00	ELECTRICAL AND GROUNDING NOTES	0			
ANT-013.00	GROUNDING SCHEMATIC AND DETAILS	0			

APPLICABLE BUILDING CODES & STANDARDS

ALL WORK AND MATERIALS SHALL BE PERFORMED AND INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THE LATEST EDITIONS OF THE FOLLOWING CODES.

- 2016 CONNECTICUT STATE BUILDING CODE, INCORPORATING THE 2012 INTERNATIONAL BUILDING CODE
- TIA/EIA-222-G OR LATEST EDITION NFPA 780-LIGHTNING PROTECTION CODE 201
- 2014 NATIONAL ELECTRIC CODE OR LATEST EDITION ANY OTHER NATIONAL OR LOCAL APPLICABLE CODES MOST RECENT EDITIONS
- CT BUILDING CODE
- LOCAL BUILDING CODE CITY/COUNTY ORDINANCES

SPRINT PROPOSED TO MODIFY AN EXISTING UNMANNED TELECOMMUNICATIONS FACILITY.

- INSTALL (3) NEW PANEL ANTENNAS INSTALL (6) NEW RRH'S

SCOPE OF WORK



GENERAL NOTES

- I. CONTRACTOR SHALL NOT COMMENCE ANY WORK UNTIL HE OBTAINS, AT HIS OWN EXPENSE, ALL INSURANCE REQUIRED BY SPRINT, THE PROPERTY OWNER AND/OR PROPERTY MANAGEMENT COMPANY.
- THIS SET OF PLANS HAS BEEN PREPARED FOR THE PURPOSES OF MUNICIPAL AND AGENCY REVIEW AND APPROVAL THIS SET OF PLANS SHALL NOT BE UTILIZED AS CONSTRUCTION DOCUMENTS UNTIL ALL CONDITIONS OF APPROVAL HAVE BEEN SATISFIED AND EACH OF THE DRAWINGS HAVE BEEN REVISED TO INDICATE "ISSUED FOR CONSTRUCTION.
- 3. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE CODES, ORDINANCES, LAWS AND REGULATIONS OF ALL MUNICIPALITIES, UTILITIES OR OTHER PUBLIC AUTHORITIES.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS THAT MAY BE REQUIRED BY ANY FEDERAL, STATE, COUNTY OR MUNICIPAL AUTHORITIES.
- 5. THE CONTRACTOR SHALL NOTIFY THE CONSTRUCTION MANAGER, IN WRITING, OF ANY CONFLICTS, ERRORS OR OMISSIONS PRIOR TO THE SUBMISSION OF BIDS FOR PERFORMANCE OF WORK. MINOR OMISSIONS OR ERRORS IN THE BID DOCUMENTS SHALL NOT EXCUSE SAID CONTRACTOR FROM COMPLETING THIS PROJECT IN ACCORDANCE WITH THE OVERALL INTENT OF THESE DRAWINGS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING ALL EXISTING SITE IMPROVEMENTS PRIOR TO COMMENCING DEMOLITION. THE CONTRACTOR SHALL REPAIR ANY DAMAGE CAUSED AS A RESULT OF REMOVAL OF THIS FACILITY.
- THE SCOPE OF WORK FOR THIS PROJECT SHALL INCLUDE PROVIDING ALL MATERIALS, EQUIPMENT AND LABOR AS REQUIRED TO COMPLETE THIS PROJECT. ALL EQUIPMENT SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- 8. THE CONTRACTOR SHALL VISIT THE PROJECT SITE PRIOR TO SUBMITTING A BID TO VERIFY THAT THE PROJECT CAN BE REMOVED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- 9. THE DEMOLITION CONTRACTOR IS SOLELY RESPONSIBLE FOR DETERMINING ALL REMOVAL MEANS AND METHODS. THE DEMOLITION CONTRACTOR IS ALSO RESPONSIBLE FOR ALL JOB SITE SAFETY.
- 10. THE CONTRACTOR IS TO REVIEW ALL DRAWINGS AND SPECIFICATIONS IN THE CONTRACT DOCUMENT SET. THE CONTRACTOR SHALL COORDINATE ALL WORK SHOWN IN THE SET OF DRAWINGS. THE CONTRACTOR SHALL PROVIDE A COMPLETE SET OF DRAWINGS TO ALL SUBCONTRACTORS AND RELATED PARTIES. THE SUBCONTRACTOR SHALL EXAMINE ALL THE DRAWINGS AND SPECIFICATIONS FOR THE INFORMATION THAT EFFECTS THEIR WORK.
- 11. THE CONTRACTOR SHALL MAINTAIN A CURRENT SET OF DRAWINGS AND SPECIFICATIONS ON THE SITE AT ALL TIMES AND INSURE THE DISTRIBUTION OF NEW DRAWINGS TO SUBCONTRACTORS AND OTHER RELEVANT PARTIES AS SOON AS THEY ARE MADE AVAILABLE. OLD DRAWINGS SHALL BE MARKED VOID AND REMOVED FROM THE CONTRACT AREA CONTRACTOR FURNISH 3 SETS OF REDLINE "AS-REMOVED" DRAWINGS TO SPRINT UPON COMPLETION OF THE WORK.
- 12. REPAIR MATERIALS INSTALLED SHALL MEET REQUIREMENTS OF CONTRACTORS DOCUMENTS. NO SUBSTITUTIONS ARE ALLOWED.
- THE CONTRACTOR SHALL RECEIVE CLARIFICATION IN WRITING AND SHALL RECEIVE IN WRITING AUTHORIZATION TO PROCEED BEFORE STARTING WORK ON ANY ITEMS NOT CLEARLY DEFINED OR IDENTIFIED BY THE CONTRACT DOCUMENTS.
- 14. THE CONTRACTOR SHALL NOTIFY THE CONSTRUCTION MANAGER OF ALL PRODUCTS OR ITEMS NOTED AS EXISTING WHICH ARE NOT FOUND TO BE IN THE FIELD.
- 15. DEMOLITION SHALL BE DONE IN A WORKMANLIKE MANNER BY COMPETENT EXPERIENCED WORKMEN IN ACCORDANCE WITH APPLICABLE CODES AND THE BEST-ACCEPTED PRACTICE. ALL SURFACES SHALL BE REPAIRED TO MATCH THEIR SURROUNDINGS AND PROVIDE WEATHER TIGHT SEAL ON SAME DAY AS REMOVAL.
- THE CONTRACTOR SHALL COORDINATE HIS WORK AND SCHEDULE HIS ACTIVITIES AND WORKING HOURS IN ACCORDANCE WITH THE REQUIREMENTS OF THE PROPERTY OWNER AND/OR PROPERTY MANAGEMENT COMPANY.
- 17. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING HIS WORK WITH THE WORK OF OTHERS AS IT MAY RELATE TO RADIO EQUIPMENT, ANTENNAS AND ANY OTHER PORTIONS OF THE WORK.
- THE CONTRACTOR SHALL REPAIR ALL EXISTING SURFACES DAMAGED DURING REMOVAL SUCH THAT THEY MATCH AND BLEND WITH ADJACENT SURFACES.
- 19. THE CONTRACTOR SHALL KEEP CONTRACT AREA CLEAN, HAZARD FREE AND DISPOSE OF ALL DEBRIS AND RUBBISH. LEAVE PREMISES IN CLEAN CONDITION AND FREE FROM PAINT SPOTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ALL ITEMS UNTIL COMPLETION OF CONSTRUCTION.
- 20. BEFORE FINAL ACCEPTANCE OF THE WORK, THE CONTRACTOR SHALL REMOVE ALL EQUIPMENT, TEMPORARY WORKS, UNUSED AND USELESS MATERIALS, RUBBISH AND TEMPORARY STRUCTURES.



SECTION 01 100 - SCOPE OF WORK

THE WORK: THESE STANDARD CONSTRUCTION SPECIFICATIONS IN CONJUNCTION WITH THE CONSTRUCTION DRAWINGS AND ASSOCIATED OUTLINE SPECIFICATIONS AND THE SITE SPECIFIC WORK ORDER, DESCRIBE THE WORK TO BE PERFORMED BY THIS CONSTRUCTION CONTRACTOR (SUPPLIER).

- RELATED DOCUMENTS: A. THE REQUIREMENTS OF EACH SECTION OF THIS SPECIFICATION APPLY TO ALL SECTIONS, INDIVIDUALLY AND COLLECTIVELY. B. RELATED DOCUMENTS: THE CONTRACTOR SHALL COMPLY WITH THE MOST CURRENT VERSION OF THE
- COLLOWING SUPPLEMENTAL REQUIREMENTS FOR INSTALLATION AND TESTING. 1.EN-2012-001: (FIBER OPTIC, DC CABLE, AND DC CIRCUIT BREAKER TAGGING STANDARDS) 2.TS-0200 (TRANSMISSION ANTENNA LINE ACCEPTANCE STANDARDS)
- 3 EL-0568 (FIBER TESTING POLICY)
- 4.NP-312-2011: (EXTERIOR GROUNDING SYSTEM TESTING) 5.NP-760-500: ETHERNET, MICROWAVE, TESTING AND ACCEPTANCE

PRECEDENCE: SHOULD CONFLICTS OCCUR BETWEEN THE STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES AND THE CONSTRUCTION DRAWINGS, INFORMATION ON THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE. NOTIFY SPRINT CONSTRUCTION MANAGER IF THIS OCCURS.

NATIONALLY RECOGNIZED CODES AND STANDARDS:

THE WORK SHALL COMPLY WITH APPLICABLE NATIONAL AND LOCAL CODES AND STANDARDS LATEST THE WORK SHALL CONFET WITH APPLICABLE INATIONAL AND LOCAL CODES AND STANDAR EDITION, AND PORTIONS THEREOF, INCLUDED BUT NOT LIMITED TO THE FOLLOWING: A. GR-63-CORE NEBS REQUIREMENTS: PHYSICAL PROTECTION B. GR-78-CORE GENERIC REQUIREMENTS FOR THE PHYSICAL DESIGN AND MANUFACTURE OF

- B. GR.7/8-CORE GENERIC REQUIREMENTS FOR THE PHYSICAL DESIGN AND MANUFACTURE OF TELECOMMUNICATIONS EQUIPMENT.
 C. GR-1089 CORE, ELECTROMAGNETIC COMPATIBILITY AND ELECTRICAL SAFETY -GENERIC CRITERIA FOR NETWORK TELECOMMUNICATIONS EQUIPMENT.
 D. NATIONAL FIRE PROTECTION ASSOCIATION CODES AND STANDARDS (NFPA) INCLUDING NFPA 70 (NATIONAL ELECTRICAL CODE "NEC") AND NFPA 101 (LIFE SAFETY CODE).
 E. AMERICAN SOCIETY FOR TESTING OF MATERIALS (ASTM)
 E. INCTUTIE OF ELECTRONIC AND ELECTRICAL ENCLUSIERE
- . INSTITUTE OF ELECTRONIC AND ELECTRICAL ENGINEERS (IEEE)
- G. AMERICAN CONCRETE INSTITUTE (ACI) H. AMERICAN WIRE PRODUCERS ASSOCIATION (AWPA)
- I. CONCRETE REINFORCING STEEL INSTITUTE (CRSI)
- J. CONCRETE REINFORCING STEEL INSTITUTE (CKSI) J. AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) K. PORTLAND CEMENT ASSOCIATION (PCA) L. NATIONAL CONCRETE MASONRY ASSOCIATION (NCMA)
- M. BRICK INDUSTRY ASSOCIATION (BIA)
- N. AMERICAN WELDING SOCIETY (MS) O. NATIONAL ROOFING CONTRACTORS ASSOCIATION (NRCA) P. SHEET METAL AND AIR CONDITIONING CONTRACTORS' NATIONAL ASSOCIATION (SMACNA)
- O DOOR AND HARDWARE INSTITUTE (DHI)
- B. OCCUPATIONAL SAFETY AND HEALTH ACT (OSHA)
 APPLICABLE BUILDING CODES INCLUDING UNIFORM BUILDING CODE, SOUTHERN BUILDING CODE, BOCA, AND THE INTERNATIONAL BUILDING CODE.

- DEFINITIONS: A. WORK: THE SUM OF TASKS AND RESPONSIBILITIES IDENTIFIED IN THE CONTRACT DOCUMENTS. DEFINITION: AND ITS OPERATING ENTITIES.
- B. COMPANY: "SPRINT"; SPRINT NEXTEL CORPORATION AND IT'S OPERATING ENTITIES. C. ENGINEER: SYNONYMOUS WITH ARCHITECT & ENGINEER AND "A&E". THE DESIGN PROFESSIONAL HAVING PROFESSIONAL RESPONSIBILITY FOR DESIGN OF THE PROJECT. D. CONTRACTOR: CONSTRUCTION CONTRACTOR, SUPPLIER, CONSTRUCTION VENDOR; INDIVIDUAL OR
- ENTITY WHO AFTER EXECUTION OF A CONTRACT OR, SIGHTLER, CONSTRUCTION OF INDOK, INDUCAL OK ENTITY WHO AFTER EXECUTION OF A CONTRACT IS BOUND TO ACCOMPLISH THE WORK. E. THIRD PARTY VENDOR OR AGENCY: A VENDOR OR AGENCY ENGAGED SEPARATELY BY THE COMPANY, A&E, OR CONTRACTOR TO PROVIDE MATERIALS OR TO ACCOMPLISH SPECIFIC TASKS RELATED TO BUT NOT
- INCLUDED IN THE WORK.
- I: CONSTRUCTION MANAGER ALL PROJECTS RELATED COMMUNICATION TO FLOW THROUGH SPRINT REPRESENTATIVE IN CHARGE OF PROJECT.

<u>SITE FAMILIARITY:</u> CONTRACTOR SHALL BE RESPONSIBLE FOR FAMILIARIZING HIMSELF WITH ALL CONTRACT DOCUMENTS, FIELD CONDITIONS AND DIMENSIONS PRIOR TO PROCEEDING WITH CONSTRUCTION. ANY DISCREPANCIES SHALL RE BROUGHT TO THE ATTENTION OF THE SPRINT CONSTRUCTION MANAGER PRIOR TO THE COMMENCEMENT OF WORK. NO COMPENSATION WILL BE AWARDED BASED ON CLAIM OF LACK OF KNOWLEDGE OR FIELD CONDITIONS.

POINT OF CONTACT: COMMUNICATION BETWEEN SPRINT AND THE CONTRACTOR SHALL FLOW THROUGH THE SINGLE SPRINT CONSTRUCTION MANAGER APPOINTED TO MANAGE THE PROJECT FOR SPRINT.

ON-SITE SUPERVISION: THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL EMPLOY A COMPETENT SUPERINTENDENT WHO SHALL BE IN ATTENDANCE AT THE SITE AT ALL TIMES DURING PERFORMANCE OF THE WORK

DRAWINGS REQUIRED AT JOBSITE: THE CONSTRUCTION CONTRACTOR SHALL MAINTAIN A FULL SET OF THE CONSTRUCTION DRAWINGS FOR WIRELESS SITES AND THE STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES AND THE STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES AT THE JOBSITE FROM MOBILIZATION THROUGH CONSTRUCTION COMPLETION.

- A THE JOBSITE DRAWINGS SHALL BE CLEARLY MARKED DAILY IN RED PENCIL WITH ANY CHANGES IN CONSTRUCTION OVER WHAT IS DEPICTED IN THE DOCUMENTS. AT CONSTRUCTION COMPLETION, THIS JOBSITE MARKUP SET SHALL BE DELIVERED TO THE COMPANY OR COMPANY'S DESIGNATED REPRESENTATIVE TO BE FORWARDED TO THE COMPANY'S A&E VENDOR FOR PRODUCTION OF "AS-BUILT" DRAWINGS
- B. DIMENSIONS SHOWN ARE TO FINISH SURFACES UNLESS NOTED OTHERWISE. SPACING BETWEEN EQUIPMENT IS THE REQUIRED CLEARANCE. SHOULD THERE BE ANY QUESTIONS REGARDING THE CONTRACT DOCUMENTS, EXISTING CONDITIONS AND/OR DESIGN INTENT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING A CLARIFICATION FROM THE SPRINT CONSTRUCTION MANAGER PRIOR TO PROCEEDING WITH THE WORK.

<u>USE OF JOB SITE:</u> THE CONTRACTOR SHALL CONFINE ALL CONSTRUCTION AND RELATED OPERATIONS INCLUDING STAGING AND STORAGE OF MATERIALS AND EQUIPMENT, PARKING, TEMPORARY FACILITIES, AND WASTE STORAGE TO THE LEASE PARCEL UNLESS OTHERWISE PERMITTED BY THE CONTRACT DOCUMENTS.

UTILITY SERVICES: WHERE NECESSARY TO CUT EXISTING PIPES, ELECTRICAL WIRES, CONDUITS, CABLES, ETC., OF UTILITY SERVICES, OR OF FIRE PROTECTION OR COMMUNICATIONS SYSTEMS, THEY SHALL BE CUT AND CAPPED AT SUITABLE PLACES OR WHERE SHOWN. ALL SUCH ACTIONS SHALL BE COORDINATED WITH THE UTILITY COMPANY INVOLVED:

PERMITS/FEES: WHEN REQUIRED THAT A PERMIT OR CONNECTION FEE BE PAID TO A PUBLIC UTILITY PROVIDER FOR NEW SERVICE TO THE CONSTRUCTION PROJECT, PAYMENT OF SUCH FEE SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR

<u>CONTRACTOR:</u> CONTRACTOR SHALL TAKE ALL MEASURES AND PROVIDE ALL MATERIAL NECESSARY FOR PROTECTING EXISTING EQUIPMENT AND PROPERTY.

USE OF ELECTRONIC PROJECT MANAGEMENT SYSTEMS: CONTRACTOR WILL UTILIZE ITS BEST EFFORTS TO WORK WITH SPRINT ELECTRONIC PROJECT MANAGEMENT SYSTEMS. CONTRACTOR UNDERSTANDS THAT SUFFICIENT INTERNET ACCESS, EQUIVALENT TO "BROADBAND"

OR BETTER, IS REQUIRED TO TIMELY AND EFFECTIVELY UTILIZE SPRINT DATA AND DOCUMENT MANAGEMENT SYSTEMS AND AGREES TO MAINTAIN APPROPRIATE CONNECTIONS FOR CONTRACTOR'S STAFF AND OFFICES THAT ARE COMPATIBLE WITH SPRINT DATA AND DOCUMENT MANAGEMENT SYSTEMS

<u>"EMPORARY UTILITIES AND FACILITIES:</u> "HE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TEMPORARY UTILITIES AND FACILITIES NECESSARY EXCEPT AS OTHERWISE INDICATED IN THE CONSTRUCTION DOCUMENTS. TEMPORARY UTILITIES AND FACILITIES INCLUDE POTABLE WATER, HEAT, HVAC, ELECTRICITY, SANITARY FACILITIES, WASTE DISPOSAL FACILITIES, AND TELEPHONE/COMMUNICATION SERVICES, PROVIDE TEMPORARY UTILITIES AND FACILITIES IN ACCORDANCE WITH OSHA AND THE AUTHORITY HAVING JURISDICTION. CONTRACTOR MAY UTILIZE THE COMPANY ELECTRICAL SERVICE IN THE COMPLETION OF THE WORK WHEN IT BECOMES AVAILABLE. USE OF THE LESSOR'S OR SITE OWNER'S UTILITIES OR FACILITIES IS EXPRESSLY FORBIDDEN EXCEPT AS OTHERWISE ALLOWED IN THE CONTRACT DOCUMENTS.

ACCESS TO WORK: THE CONTRACTOR SHALL PROVIDE ACCESS TO THE JOB SITE FOR AUTHORIZED COMPANY PERSONNEL AND AUTHORIZED REPRESENTATIVES OF THE ARCHITECT/ENGINEER DURING ALL PHASES OF THE WORK

DIMENSIONS: VERIFY DIMENSIONS INDICATED ON DRAWINGS WITH FIELD DIMENSIONS BEFORE FABRICATION OR ORDERING OF MATERIALS. DO NOT SCALE DRAWINGS.

EXISTING CONDITIONS: NOTIFY THE SPRINT CONSTRUCTION MANAGER OF EXISTING CONDITIONS DIFFERING FROM THOSE INDICATED APPROVAL FROM THE ARCHITECT AND ENGINEER.

SECTION 01 200 - COMPANY FURNISHED MATERIAL AND EQUIPMENT

FURNISHED MATERIALS: COMPANY FURNISHED MATERIALS AND EQUIPMENT TO BE INSTALLED BY THE CONTRACTOR (OFIC) IS IDENTIFIED ON THE RF DATA SHEET IN THE CONSTRUCTION DOCUMENTS.

RECEIPT OF MATERIAL AND EQUIPMENT: A. THE CONTRACTOR IS RESPONSIBLE FOR SPRINT PROVIDED MATERIAL AND EQUIPMENT AND UPON RECEIPT

- SHALL
- 1. ACCEPT DELIVERIES AS SHIPPED AND TAKE RECEIPT. 2. VERIFY COMPLETENESS AND CONDITION OF ALL DELIVERIES.
- 3. TAKE RESPONSIBILITY FOR EQUIPMENT AND PROVIDE INSURANCE PROTECTION AS REQUIRED IN AGREEMENT. B. RECORD ANY DEFECTS OR DAMAGES AND WITHIN TWENTY-FOUR HOURS AFTER RECEIPT, REPORT TO SPRINT OR ITS DESIGNATED PROJECT REPRESENTATIVE OF SUCH.

- C. PROVIDE SECURE AND NECESSARY WEATHER PROTECTED WAREHOUSING
- D. COORDINATE SAFE AND SECURE TRANSPORTATION OF MATERIAL AND EQUIPMENT, DELIVERING AND OFF-LOADING FROM CONTRACTOR'S WAREHOUSE TO SITE.

DELVERABLES: A. COMPLETE SHIPPING AND RECEIPT DOCUMENTATION IN ACCORDANCE WITH COMPANY PRACTICE. B. IF APPLICABLE, COMPLETE LOST/STOLEN/DAMAGED DOCUMENTATION REPORT AS NECESSARY IN ACCORDANCE WITH COMPANY PRACTICE, AND AS DIRECTED BY COMPANY

SECTION 01 300 - CELL SITE CONSTRUCTION

NOTICE TO PROCEED

A. NO WORK SHALL COMMENCE PRIOR TO COMPANY'S ISSUANCE OF THE WORK ORDER. B. UPON RECEIVING NOTICE TO PROCEED, CONTRACTOR SHALL FULLY PERFORM ALL WORK NECESSARY TO PROVIDE SPRINT WITH AN OPERATIONAL WIRELESS FACILITY

- GENERAL REQUIREMENTS FOR CONSTRUCTION: A. CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH. AT THE COMPLETION OF THE WORK, CONTRACTOR SHALL REMOVE FROM THE SITE ALL REMAINING RUBBISH, IMPLEMENTS, TEMPORARY FACILITIES, AND SURPLUS MATERIALS. B. EQUIPMENT ROOMS SHALL AT ALL TIMES BE MAINTAINED "BROOM CLEAN" AND CLEAR OF DEBRIS.
- C. CONTRACTOR SHALL TAKE ALL REASONABLE PRECAUTIONS TO DISCOVER AND LOCATE ANY HAZARDOUS CONDITION.
 - IN THE EVENT CONTRACTOR ENCOUNTERS ANY HAZARDOUS CONDITION WHICH HAS NOT BEEN ABATED OR OTHERWISE MITIGATED, CONTRACTOR AND ALL OTHER PERSONS SHALL IMMEDIATELY
- STOP WORK IN THE AFFECTED AREA AND NOTICY COMPANY IN WRITING THE WORK IN THE AFFECTED AREA SHALL NOT BE RESUMED EXCEPT BY WRITTEN NOTIFICATION BY COMPANY. 2. CONTRACTOR AGREES TO USE CARE WHILE ON THE SITE AND SHALL NOT TAKE ANY ACTION THAT WILL
- OR MAY RESULT IN OR CAUSE THE HAZARDOUS CONDITION TO BE FURTHER RELEASED IN THE ENVIRONMENT, OR TO FURTHER EXPOSE INDIVIDUALS TO THE HAZARD. D. CONTRACTOR'S ACTIVITIES SHALL BE RESTRICTED TO THE PROJECT LIMITS. SHOULD AREAS OUTSIDE THE PROJECT LIMITS BE AFFECTED BY CONTRACTOR'S ACTIVITIES, CONTRACTOR SHALL IMMEDIATELY RETURN
- THEM TO ORIGINAL CONDITION

REQUIRED.

- FUNCTIONAL REQUIREMENTS: A. THE ACTIVITIES DESCRIBED IN THIS PARAGRAPH REPRESENT MINIMUM ACTIONS AND PROCESSES
- REQUIRED TO SUCCESSFULLY COMPLETE THE WORK. CONTRACTOR SHALL ACTIONS AND INFORMATION AS IN RECEISARY TO SUCCESSFULLY COMPLETE THE CONSTRUCTION OF A FULLY FUNCTIONING WIRELESS FACILITY AT THE SITE IN ACCORDANCE WITH COMPANY PROCESSES.
- B. SUBMIT SPECIFIC DOCUMENTATION AS INDICATED HEREIN, AND OBTAIN REQUIRED APPROVALS WHILE THE WORK IS BEING PERFORMED.
- C. MANAGE AND CONDUCT ALL FIELD CONSTRUCTION SERVICE RELATED ACTIVITIES D. PROVIDE CONSTRUCTION ACTIVITIES TO THE EXTENT REQUIRED BY THE CONTRACT DOCUMENTS,

L PROVIDE SLABS AND FOUIPMENT PLATFORMS

- I. PERFORM ANY REQUIRED STITE ENVIRONMENTAL MITIGATION. 2. PREPARE GROUND SITES; PROVIDE DE-GRUBBING; AND ROUGH AND FINAL GRADING, AND COMPOUND
- SURFACE TREATMENTS
- SURFACE TREATMENTS. 3. MANAGE AND CONDUCT ALL ACTIVITIES FOR INSTALLATION OF UTILITIES INCLUDING ELECTRICAL AND BACKHAUL (FIBER, COPPER, OR MICROWAVE). 4. INSTALL UNDERGROUND FACILITIES INCLUDING UNDERGROUND POWER AND COMMUNICATIONS

12. INSTALL COMPOUND FENCING, SIGHT SHIELDING, LANDSCAPING AND ACCESS BARRIERS. 13. PERFORM INSPECTION AND MATERIAL TESTING AS REQUIRED HEREINAFTER.

16. INSTALL FIXED GENERATOR SETS AND OTHER STANDBY POWER SOLUTIONS. 16. INSTALL TOWERS, ANTENNA SUPPORT STRUCTURES AND PLATFORMS ON EXISTING TOWERS AS

9 PERFORM DOCUMENT AND CLOSE OUT ALL URISDICTIONAL PERMITTING REQUIREMENTS AND

ANY CONSTRUCTION CONTROL DOCUMENTS THAT MAY BE REQUIRED BY GOVERNMENT AGENCIES AND LANDLORDS.

17. INSTALL CELL SITE RADIOS, MICROWAVE, GPS, COAXIAL MAINLINE, ANTENNAS, CROSS BAND

COUPLERS, TOWER TOP AMPLIFIERS, LOW NOISE AMPLIFIERS AND RELATED EQUIPMENT 18. CONDUCT ALL REQUIRED TESTS AND INSPECTIONS

AND LANDLORDS. 20. PERFORM ALL ADDITIONAL WORK AS IDENTIFIED IN SCOPE OF SERVICES ATTACHED TO THE SUPPLIER AGREEMENT FOR THIS PROJECT. THIS WORK MAY INCLUDE COMMISSIONING, INTEGRATION, SPECIAL WAREHOUSING, REVERSE LOGISTICS ACTIVITIES, ETC. PERFORM COMMISSIONING AND INTEGRATION ACTIVITIES PER APPLICABLE MOPS.

CONDUITS, AND UNDERGROUND GROUNDING SYSTEM. 5. INSTALL ABOVE GROUND GROUNDING SYSTEM. 6. PROVIDE NEW HVAC INSTALLATIONS AND MODIFICATIONS. 7. INSTALL "H-FRAMES", CABINETS AND PADS AND PLATFORMS AS INDICATED.

4. CONDUCT SITE RESISTANCE TO EARTH TESTING AS REQUIRED HEREINAFTER

INSTALL ROADS, ACCESS WAYS, CURBS AND DRAINS AS INDICATED.
 ACCOMPLISH REQUIRED MODIFICATION OF EXISTING FACILITIES.
 PROVIDE ANTENNA SUPPORT STRUCTURE FOUNDATIONS.

DELIVERABLES: A. THE CONTRACTOR SHALL PROVIDE ALL REQUIRED TEST REPORTS AND DOCUMENTATION INCLUDED BUT

I THE CONTRACT ON STALL FROM THE ALL REQUIRED TEST FROM IS AND DOCUMENTATION INCLUDED BUT NOT LIMITED TO THE FOLLOWING: I. PRODUCT SPECIFICATIONS FOR MATERIALS OR SPECIAL CONSTRUCTION IF REQUESTED BY SPRINT 2. ACTUALIZE ALL CONSTRUCTION RELATED MILESTONES IN SITERRA AND COMPLETE ALL ON-LINE FORMS AND COMPLETE DOCUMENT UP-LOADS. UPLOAD ALL REQUIRED CLOSEOUT DOCUMENTS AND FINAL

SITE PHOTOS 3. SCANABLE BARCODE PHOTOGRAPHS OF TOWER TOP AND INACCESSIBLE SERIALIZED EQUIPMENT LEFT ON SITE INSIDE BASE OF MAIN RF CABINET IN A PROTECTIVE POUCH.

A. ALL REQUIRED TEST REPORTS.
 REQUIRED CLOSEOUT DOCUMENTATION INCLUDING BUT NOT LIMITED TO:
 a. ALL JURISDICTIONAL PERMITTING AND OCCUPANCY INFORMATION

LIED TRONIC AS-BUILT DRAWINGS IN AUTOCAD AND PDF FORMATS
 LIEN WAIVERS

b. PDF SCAN OF REDLINES PRODUCED IN THE FIELD

2. PROIECT PROGRESS REPORTS.

4. SITE RESISTANCE TO EARTH TEST

3. CHEMICAL GROUNDING SYSTEM . 4. REINFORCEMENT CERTIFICATIONS 5. STRUCTURAL BACKFILL TEST RESULTS

COST REDUCTION PROPOSED FOR USE OF ALTERNATE PRODUCT.

6. SWEEP AND FIBER TESTS

B. REOUIRED THIRD PARTY TESTS:

4. REBAR PLACEMENT VERIFICATION WITH REPORT 5. TESTING TENSION STUDY FOR ROCK ANCHORS

C. REQUIRED TESTS BY CONTRACTOR
 COAL SWEEP TESTS PER SPRINT STANDARD TS-0200
 FIBER TESTS PER SPRINT STANDARD EL-0568

MICROWAVE LINK TESTS PER NP-760-500

6. ALL THIRD PARTY TESTS AS REQUIRED BY LOCAL JURISDICTION

STANDARDS

PARTY AGENCY.

CONSTRUCTION IS COMPLETE.

DOCUMENTATION.

STANDARDS

3. PRE-CONSTRUCTION MEETING NOTES.

d. LEN WAIVERS
 e. FINAL PAYMENT APPLICATION
 f. REQUIRED FINAL CONSTRUCTION PHOTOS
 g. CONSTRUCTION AND COMMISSIONING CHECKLIST COMPLETE WITH NO DEFICIENT ITEMS
 h. LISTS OF SUBCONTRACTORS
 B. PROVIDE ADDITIONAL DOCUMENTATION INCLUDING, BUT NOT LIMITED TO, THE FOLLOWING.

DOCUMENTATION SHALL BE FORWARDED IN ORIGINAL FORMAT AND/OR UPLOADED INTO SMS. 1. ALL CORRESPONDENCE AND PRELIMINARY CONSTRUCTION REPORTS.

SECTION 01 400 - TESTS, INSPECTIONS, SUBMITTALS, AND PROJECT CLOSEOUT

TESTS AND INSPECTIONS: A. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROJECT

B. CONTRACTOR SHALL ACCOMPLISH TESTING INCLUDING BUT NOT LIMITED TO THE FOLLOWING I. COAX SWEEPS AND FIBER TESTS PER TS-0200 (CURRENT VERSION) ANTENNA LINE ACCEPTANCE

2. POST CONSTRUCTION HEIGHT VERIFICATION, AZIMUTH AND DOWNTILT USING ELECTRONIC COMMERCIAL MADE-FOR-THE-PURPOSE ANTENNA ALIGNMENT TOOL. 3. CONCRETE BREAK TESTS

5. STRUCTURAL BACKFILL COMPACTION TESTS 6. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL CORRECTIONS TO ANY WORK IDENTIFIED AS UNACCEPTABLE IN SITE INSPECTION ACTIVITIES AND/OR AS A RESULT OF TESTING. 7. ADDITIONAL TESTING AS REQUIRED ELSEWHERE IN THIS SPECIFICATION.

<u>SUBMITTALS:</u> A. THE WORK IN ALL ASPECTS SHALL COMPLY WITH THE CONSTRUCTION DRAWINGS AND THESE

CONCRETE BREAK TESTS AS SPECIFIED HEREIN.

7. ANTENNA AZIMUTH AND DOWN-TILT VERIFICATION 8. POST CONSTRUCTION HEIGHT VERIFICATION 9. ADDITIONAL SUBMITTALS MAY BE REQUIRED FOR SPECIAL CONSTRUCTION OR MINOR MATERIALS C. ALTERNATES: AT THE COMPANYS REQUEST, ANY ALTERNATIVES TO THE MATERIALS OF MICHAEL AND SECOND SPECIFIED SHALL BE SUBMITTED TO SPRINT'S CONSTRUCTION MANAGER FOR APPROVAL PRIOR TO BEING SHIPPED TO SITE. SPRINT WILL REVIEW AND APPROVE ONLY THOSE REQUESTS MADE IN WRITING. NO VERBAL APPROVALS WILL BE CONSIDERED. SUBMITTAL FOR APPROVAL SHALL INCLUDE A STATEMENT OF

TESTING BY THIRD PARTY AGENCY: A. EMPLOY AN AGENCY OF ENGINEERS AND SCIENTISTS WHO IS REGULARLY ENGAGED IN FIELD AND LABORATORY TESTING AND ANALYSIS. AGENCY SHALL HAVE BEEN IN BUSINESS A MINIMUM OF FIVE YEARS, AND BE LICENSED AS PROFESSIONAL ENGINEERS IN THE STATE WHERE THE PROJECT IS LOCATED.

AGENCY IS SUBJECT TO APPROVAL BY COMPANY.
 AGENCY IS SUBJECT TO APPROVAL BY COMPANY.
 AGENCY MUST HAVE A THOROUGH UNDERSTANDING OF LOCAL AVAILABLE MATERIALS, INCLUDING THE SOIL, ROCK, AND GROUNDWATER CONDITIONS.
 AGENCY IS TO BE FAMILIAR WITH THE APPLICABLE REQUIREMENTS FOR THE TESTS TO BE DONE,

EQUIPMENT TO BE USED, AND ASSOCIATED HEALTH AND SAFETY ISSUES. 3. EXPERIENCE IN SOILS, CONCRETE, MASONRY, AGGREGATE, AND ASPHALT TESTING USING ASTM, AASJTO, AND OTHER METHODS IS NEEDED.

2. CONCRETE CYLINDER BREAK TESTS FOR TOWER PIER AND ANCHORS PER NATIONALLY RECOGNIZED

3. STRUCTURAL SOILS COMPACTION TESTS PER NATIONALLY RECOGNIZED STANDARDS

ANTENNA AZIMUTHS AND DOWN TILT USING ELECTRONIC ALIGNMENT TOOL PER ANTENNA INSTALLATION SPECIFICATION HEREIN.

5. POST CONSTRUCTION HEIGHT VERIFICATION AS REQUIRED HEREWITH IN THE TOWER INSTALLATION

POST CONSTRUCTION REIGHT VERHICATION AS REQUIRED REREWITH IN THE TOWER INSTALLATION SPECIFICATIONS.
 ASPHALT ROADWAY COMPACTED THICKNESS, SURFACE SMOOTHNESS, AND COMPACTED DENSITY TESTING AS SPECIFIED HEREWITH IN THE ASPHALT PAVING SPECIFICATIONS.

5. FIELD QUALITY CONTROL TESTING AS SPECIFIED HEREWITH IN THE CONCRETE PAVING SPECIFICATIONS. 8. TESTING REQUIRED HEREWITH UNDER SPECIFICATIONS FOR AGGREGATE BASE FOR ROADWAYS

9. ALL OTHER TESTS REQUIRED BY LOCAL JURISDICTION
 D. INSPECTIONS BY COMPANY: THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL CORRECTIONS TO ANY WORK IDENTIFIED AS UNACCEPTABLE IN INSPECTION ACTIVITIES, FINAL ACCEPTANCE / PUNCH WALK

REVIEW, AND/OR AS A REDUIT OF TESTING E. SPRINT RESERVES THE RIGHT TO INSPECT THE CONSTRUCTION SITE AT ANY TIME VIA SITE WALKS AND/OR PHOTO REVIEWS. CONTRACTOR SHALL GIVE SPRINT 24 HOURS NOTICE PRIOR TO THE COMMENCEMENT OF THE FOLLOWING CONSTRUCTION ACTIVITIES AND PHOTOGRAPHS OF THE IN-PROGRESS WORK. I. GROUNDING SYSTEM AND BURIED UTILITIES INSTALLATION PRIOR TO EARTH CONCEALMENT DOCUMENTED WITH DIGITAL PHOTOGRAPHS BY CONTRACTOR, APPROVED BY A&E OR SPRINT

2. FORMING FOR CONCRETE AND REBAR PLACEMENT PRIOR TO POUR DOCUMENTED WITH DIGITAL COMPACTION OF BACKFILL MATERIALS, AGGREGATE BASE FOR ROADS, PADS, AND ANCHORS, ASSHALT PAVING, AND SHAFT BACKFILL FOR CONCRETE AND WOOD POLES, BY INDEPENDENT THIRD

4. RE AND POST CONSTRUCTION ROOFTOP AND STRUCTURAL INSPECTIONS ON EXISTING FACILITIES. PRIOR TO CONSTRUCTION ACTIVITIES AND AFTER CONSTRUCTION IS COMPLETE, PROVIDE PHOTOGRAPHIC DOCUMENTATION OF ROOF, FLASHINGS, AND PARAPETS, BOTH BEFORE AND AFTER

5. TOWER ERECTION SECTION STACKING AND PLATFORM ATTACHMENT DOCUMENTED BY DIGITAL PHOTOGRAPHS BY THIRD PARTY AGENCY. 6. TOWER TOP AND INACCESSIBLE EQUIPMENT (RRUS, ANTENNAS, AND CABLING): PROVIDE PHOTOS OF THE BACKS OF ALL ANTENNAS, RAUGE, COMBINERS, FILTERS, FIBER AND DC CABLING, CABLE COLOR CODING, EQUIPMENT GROUNDING AND CONNECTOR WATER PROOFING INCLUDING NAME PLATE AND SERIAL NUMBER FOR ALL SERIALIZED EQUIPMENT.



- PROJECT CLOSEOUT
- A. FINAL ACCEPTANCE PUNCH WALK AND INSPECTION: AS IDENTIFIED IN THE SCOPE OF SERVICES, SPRINT HINCL ACCEPTIONE FORCE FORCE WALK AND INSPECTION: AS IDENTIFIED IN THE SCOPE OF SERVICES, STAND WILL CONDUCT A FINAL PUNCH WALK OR FINAL DESK TOP PHOTO REVIEW (SITE MODIFICATIONS), PUNCH WALKS MUST BE SCHEDULED IN ADVANCE AS REQUIRED. AT THE PUNCH WALK / REVIEW, SPRINT MAY IDENTIFY CRITICAL DEFICIENCIES WHICH MUST BE CORRECTED PRIOR TO PUTTING SITE ON AIR. MINOR DEFICIENCIES MUST BE CORRECTED WITHIN 30 DAYS EXCEPT AS OTHERWISE REQUIRED. VERIFICATIONS AT COMPANY'S SOLE DISCRETION.
- B. CLOSEOUT DOCUMENTATION: ALL CLOSEOUT DOCUMENTATION AND PHOTOGRAPHS SHALL BE UPLOADED
- PRIOR TO FINAL ACCEPTANCE. SPRINT WILL REVIEW CLOSEOUT DOCUMENTATION FOR PRESENCE AND CONTENT. CLOSEOUT DOCUMENTATION SHALL INCLUDE BUT IS NOT LIMITED TO THE FOLLOWING AS APPLICABLE:
- COAX SWEEP TESTS:
- COA SWEET 1515: FIBER TESTS: JURISDICTION FINAL INSPECTION DOCUMENTATION REINFORCEMENT CERTIFICATION (MILL CERTIFICATION)
- CONCRETEINZ DESIGN AND PRODUCT DATA (TOWER FOUNDATION) LIEN WAIVERS AND RELEASES. POST -CONSTRUCTION HEIGHT VERIFICATION
- URISDICTION CERTIFICATE OF OCCUPANCY
- 9. ELECTRONIC ANTENNA AZIMUTH AND DOWN TILT VERIFICATION 10. STRUCTURAL BACKFILL TEST RESULTS (IF APPLICABLE)
- 11. CELL SITE UTILITY SETUP
- 12 AS-BUILT REDUNE CONSTRUCTION DRAWINGS (PDE SCAN OF FIELD MARKS)
- 13. AS-BUILT CONSTRUCTION DRAWINGS IN DWG AND PDF FORMATS
- 14. LIST OF SUB CONTRACTORS 15. APPROVED PERMITTING DOCUMENTS
- AFROVED PENTIONING DOCUMENTS
 FINAL SITE PHOTOS UP-LOADED TO SITERRA. INCLUDE THE FOLLOWING AS APPLICABLE:
 TOWER, ANTENNAS, RRUS, AND MAINLINE: INSPECTION AND PHOTOGRAPHS OF SECTION STACKING; INSPECTION AND PHOTOGRAPHS OF PLATFORM COMPONENT ATTACHMENT POINTS; PHOTOGRAPHS OF TOWER TOP GROUNDING; PHOTOS OF TOWER COAX/CABLE LINE COLOR CODING AT THE TOP AND AT GROUND LEVEL; INSPECTION AND PHOTOGRAPHS OF OPERATIONAL OF TOWER LIGHTING, AND PLACEMENT OF FAA REGISTRATION SIGN; PHOTOGRAPHS SHOWING ADDITIONAL GROUNDING POINTS FOR TOWERS GREATER THAN 200 FEET .: PHOTOS OF ANTENNA ADDINING ALL SUBJECTS OF ANTENNAS; ONE PHOTOS OF ANTENNAS; ONE PHOTOS OF FACH SECTOR OF ANTENNAS; ONE PHOTOGRAPH LOOKING AT THE SECTOR AND ONE FROM BEHIND SHOWING THE PROJECTED COVERAGE AREA: PHOTOS OF COAX WEATHERPROOFING - TOP AND BOTTOM, PHOTOS OF COAX GROUNDING-TOP AND BOTTOM, PHOTOS OF ANTENNA AND MAST GROUNDING; PHOTOS OF COAX CABLE ENTRY INTO SHELTER PHOTOS OF PLATFORM MECHANICAL CONNECTIONS TO TOWER/MONOPOLE.
- b. ROOF TOPS: PRE-CONSTRUCTION AND POST-CONSTRUCTION VISUAL INSPECTION AND TOP CONSTRUCTION INSPECTION AND INTERIOR TO DETERMINE AND DOCUMENT CONDITIONS; ROOF TOP CONSTRUCTION INSPECTIONS AS REQUIRED BY THE JURISDICTION; PHOTOGRAPHS OF CABLE TRAY AND/OR ICE BRIDGE; PHOTOGRAPHS OF DOGHOUSE/CABLE EXIT FROM ROOF;
- c. SITE LAYOUT PHOTOGRAPHS OF THE OVERALL COMPOUND, INCLUDING EQUIPMENT PLATFORM c. SITE DATOOT - PHOTOGRAPHS OF THE OVERALL COMPOUND, INCLUDING EQUIPHENT PLATFOR FROM ALL FOUR CORNERS.
 d. FINISHED UTILITIES: CLOSE-UP PHOTOGRAPHS OF THE PPC BREAKER PANEL; CLOSE-UP PHOTOGRAPH OF THE INSIDE OF THE TELCO PANEL AND NIU; CLOSE-UP PHOTOGRAPH OF THE
- POWER METER AND DISCONNECT; PHOTOS OF POWER AND TELCO ENTRANCE TO COMPANY ENCLOSURE; PHOTOGRAPHS AT METER BOX AND/OR FACILITY DISTRIBUTION PANEL.

- PROJECT PHOTOGRAPHS: A. PROVIDE PROJECT CLOSEOUT GENERAL ARRANGEMENT PHOTOS OF ALL NEW WORK. THE FOLLOWING LIST REPRESENTS MINIMUM REQUIREMENTS AND MINIMUM QUANTITY. ADDITIONAL PHOTOS MAY BE REQUIRED TO ADEQUATELY DOCUMENT THE WORK.

- ASR AND REVERSION OF IN PLACE, SUPPLIER NOTIFIES EMS FIELD REPRESENTATIVE)
 BACK OF ANTENNAS AND RRUS (I EACH SECTOR)
 BACK OF ANTENNAS AND RRUS (I EACH SECTOR) CLOSE UP SHOWING WEATHERPROOFING AND GROUNDING (AS REQUIRED). CLOSE-UP OF BACK SIDE OF EACH PERMANENT RRU SHOWING SERIAL
- NUMBER/BAR CODE. 4. VIEW (I EACH SECTOR) ALONG THE AZIMUTH AND TILT OF THE ANTENNAS
- 5. TOP OF TOWER FROM GROUND, I EACH SECTOR 6. MAINLINE HYBRID CABLE ROUTE DOWN TOWER SHOWING FASTENERS AND SUPPORT 7. MAINLINE/HYBRID CABLE ROUTE ALONG ICE BRIDGE OR IN CABLE TRAY SHOWING FASTENERS AND
- SUPPORT
- 8 GROUND MOUNTED RELEBACKS (FRONT AND BACK)
- 9. FRONT, SIDE AND BACK ELEVATIONS OF ALL GROUND CABINETS 10. VIEW OF COMPOUND FROM A DISTANCE 11. VIEW OF EACH GROUND CABINET (POWER, RF, FIBER SPOOL, PPC POWER, PPC TELCO WITH DOOR
- 12. BACKHAUL FIBER MEET-ME-POINT AND CONDUIT ROUTE (MICROWAVE INSTALLATION IF NOT FIBER) 13. AAV NETWORK INTERFACE DEVICE OR MICROWAVE RADIO INSTALLATION

DEFICIENCY CORRECTIONS: CONTRACTOR IS RESPONSIBLE FOR ALL CORRECTIONS TO DEFICIENCIES IDENTIFIED THROUGH TESTING, REVIEW OF SUBMITTALS, INSPECTIONS AND CLOSEOUT REVIEWS.

SECTION 01 500 - PROJECT REPORTING

- WEEKLY REPORTS: A. CONTRACTOR SHALL REPORT TO SPRINT AT MINIMUM ON A WEEKLY BASIS VIA SITERRA BY UPDATING ALL APPLICABLE POST END KEEPING MILESTONES WITH ACTUAL AND FORECASTED COMPLETION DATES. B. ADDITIONAL REQUIREMENTS FOR REPORTING MAY BE IDENTIFIED ELSEWHERE OR REQUIRED BY THE SCOPE
- OF SERVICES OR SPRINTS LOCAL MARKET CONSTRUCTION MANAGER. THIS INFORMATION WILL PROVIDE A BASIS FOR PROGRESS MONITORING AND PAYMENT. PROJECT CONFERENCE CALLS:
- SPRINT MAY HOLD PERIODIC PROJECT CONFERENCE CALLS. CONTRACTOR WILL BE REQUIRED TO COMMUNICATE SITE STATUS, MILESTONE COMPLETIONS AND UPCOMING MILESTONE PROJECTIONS, AND ANSWER ANY OTHER SITE STATUS OUESTIONS AS NECESSARY.
- FINAL PROJECT ACCEPTANCE: PRIOR TO SPRINTS FINAL PROJECT ACCEPTANCE. ALL REQUIRED MILESTONE ACTUALS MUST BE UPDATED IN SITERRA AND ALL REQUIRED REPORTING TASKS MUST BE COMPLETE.

SECTION 11 700 - ANTENNA ASSEMBLY, REMOTE RADIO UNITS AND CABLE INSTALLATION

SUMMARY: THIS SECTION SPECIFIES INSTALLATION OF ANTENNAS, RRU'S, AND CABLE EQUIPMENT, INSTALLATION, AND TESTING OF COAXIAL FIBER CABLE.

ANTENNAS AND RRU'S: THE NUMBER AND TYPE OF ANTENNAS AND RRU'S TO BE INSTALLED IS DETAILED ON THE CONSTRUCTION DRAWINGS.

HYBRID CABLE HYBRID CABLE WILL BE DC/FIBER AND FURNISHED FOR INSTALLATION AT EACH SITE. CABLE SHALL BE HYBRID CABLE WILL BE DC/FIBER AND FURNISHED FOR INSTALLATION AT EACH SITE. CABLE SHALL BE INSTALLED PER THE CONSTRUCTION DRAWINGS AND THE APPLICABLE MANUFACTURER'S REQUIREMENTS

JUMPERS AND CONNECTORS: FURNISH AND INSTALL 1/2" COAX JUMPER CABLES BETWEEN THE RRU'S AND ANTENNAS. JUMPERS SHALL BE TYPE LDF 4, FLC 12-50, CR 540, OR FXL 540. SUPER-FLEX CABLES ARE NOT ACCEPTABLE. JUMPERS BETWEEN THE RRU'S AND ANTENNAS OR TOWER TOP AMPLIFIERS SHALL CONSIST OF 1/2 INCH FOAM DIELECTRIC, OUTDOOR RATED COAXIAL CABLE, MIN. LENGTH FOR JUMPER SHALL BE 10"-0".

REMOTE ELECTRICAL TILT (RET) CABLES:

MISCELLANEOUS: INSTALL SPLITTERS, COMBINERS, FILTERS PER RF DATA SHEET, FURNISHED BY SPRINT.

- - <u>ANTENNA INSTALLATION:</u> THE CONTRACTOR SHALL ASSEMBLE ALL ANTENNAS ONSITE IN ACCORDANCE WITH THE INSTRUCTIONS SUPPLIED BY THE MANUFACTURER, ANTENNA HEIGHT, AZIMUTH, AND FEED OBJENTATION INFORMATION

 - SUPLIED BY THE PANOPACTORE. ANTENNA REIGHT, ALTIOUTH, AND FEED ONERTATION INFORMATION SHALL BE A DESIGNATED ON THE CONSTRUCTION DRAWINGS. A. THE CONTRACTOR SHALL POSITION THE ANTENNA ON TOWER PIPE MOUNTS SO THAT THE BOTTOM STRUT IS LEVEL. THE PIPE MOUNTS SHALL BE PLUMB TO WITHIN I DEGREE.
 - B. ANTENNA MOUNTING REQUIREMENTS: PROVIDE ANTENNA MOUNTING HARDWARE AS INDICATED ON THE
 - DRAWINGS.

- <u>HYBRID CABLE INSTALLATION:</u> A. THE CONTRACTOR SHALL ROUTE, TEST, AND INSTALL ALL CABLES AS INDICATED ON THE CONSTRUCTION DRAWINGS AND IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. B. THE INSTALLED RADIUS OF THE CABLES SHALL NOT BE LESS THAN THE MANUFACTURER'S SPECIFICATIONS
- FOR BENDING RADII
- C. EXTREME CARE SHALL BE TAKEN TO AVOID DAMAGE TO THE CABLES DURING HANDLING AND INSTALLATION. I. FASTENING MAIN HYBRID CABLES: ALL CABLES SHALL BE INSTALLED INSIDE MONOPOLE WITH CABLE SUPPORT GRIPS AS REQUIRED BY THE MANUFACTURER.
- SUFFORT GRIEF AS REQUIRED BT THE PRAVORACTORER. 2. FASTENING INDIVIDUAL FIBER AND DC CABLES ABOVE BREAKOUT ENCLOSURE (MEDUSA), WITHIN THE MMBS CABINET AND ANY INTERMEDIATE DISTRIBUTION BOXES: a. FIBER: SUPPORT FIBER BUNDLES USING 1/2 " VELCRO STRAPS OF THE REQUIRED LENGTH AT 18" O.C. STRAPS SHALL BE UV, OIL AND WATER RESISTANT AND SUITABLE FOR
- INDUSTRIAL INSTALLATIONS AS MANUFACTURED BY TEXTOL OR APPROVED EQUAL b. DC: SUPPORT DC BUNDLES WITH ZIP TIES OF THE ADEQUATE LENGTH. ZIP TIES TO BE UV STABILIZED, BLACK NYLON, WITH TENSILE STRENGTH AT 12,000 PSI AS MANUFACTURED BY NELCO PRODUCTS OR FOLIAL
- 5. FASTENING JUMPERS SECURE JUMPERS TO THE SIDE ARMS OR HEAD FRAMES USING STAINLESS STEEL TIE WRAPS OR STAINLESS STEEL BUTTERFLY CLIPS.
 - 4. CABLE INSTALLATION:
 - a. INSPECT CABLE PRIOR TO USE FOR SHIPPING DAMAGE, NOTIFY THE CONSTRUCTION
 MANAGER.
 b. CABLE ROUTING: CABLE INSTALLATION SHALL BE PLANNED TO ENSURE THAT THE LINES
 - WILL BE PROPERLY ROUTED IN THE CABLE ENVELOP AS INDICATED ON THE DRAWINGS. AVOID TWISTING AND CROSSOVERS. c. HOIST CABLE USING PROPER HOISTING GRIPS. DO NOT EXCEED MANUFACTURER'S
- RECOMMENDED MAXIMUM BEND RADIUS.
- 5. GROUNDING OF TRANSMISSION LINES: ALL TRANSMISSION LINES SHALL BE GROUNDED AS INDICATED ON DRAWINGS.
- 6. HYBRID CABLE COLOR CODING: ALL COLOR CODING SHALL BE AS REQUIRED IN TS 0200 (CURRENT VERSION)
- ALPHA-NUMERICALLY ACCORDING TO SPRINT CELL SITE ENGINEERING NOTICE EN 2012-001, REV 1
- WEATHERPROOFING EXTERIOR CONNECTORS AND HYBRID CABLE GROUND KITS: A. ALL FIBER & COAX CONNECTORS AND GROUND KITS SHALL BE WEATHERPROOFED. B. WEATHERPROOFED USING ONE OF THE FOLLOWING METHODS. ALL INSTALLATIONS MUST BE DONE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND INDUSTRY BEST PRACTICES. 1. COLD SHRINK: ENCOMPASS CONNECTOR IN COLD SHRINK TUBING AND PROVIDE A DOUBLE WRAP OF 2" ELECTRICAL TAPE EXTENDING 2" BEYOND TUBING, PROVIDE 3M COLD SHRINK CXS SERIES OR
- EOUAL.
- 2. SELF-AMALGAMATING TAPE: CLEAN SURFACES, APPLY A DOUBLE WRAP OF SELF-AMALGAMATING TAPE 2. SEITAL DECONNECTOR, APPLY A SECOND WRAP OF SELF-AMALGAMATING TAPE IN OPPOSITE DIRECTION. APPLY DOUBLE WRAP OF 2 " WIDE ELECTRICAL TAPE EXTENDING 2 " BEYOND THE
- SELF-AMALGAMATING TAPE
- 3M SLIM LOCK CLOSURE 716: SUBSTITUTIONS WILL NOT BE ALLOWED.
 OPEN FLAME ON JOB SITE IS NOT ACCEPTABLE

SECTION 11 800 - INSTALLATION OF MULTIMODAL BASE STATIONS (MMBS) AND RELATED EQUIPMENT

- SUMMARY: A. THIS SECTION SPECIFIES MMBS CABINETS, POWER CABINETS, AND INTERNAL EQUIPMENT INCLUDING BY NOT LIMITED TO RECTIFIERS. POWER DISTRIBUTION UNITS, BASE BAND UNITS, SURGE ARRESTORS BATTERIES, AND SIMILAR EQUIPMENT FURNISHED BY THE COMPANY FOR INSTALLATION BY THE
- CONTRACTOR (OFCI). B. CONTRACTOR SHALL PROVIDE AND INSTALL ALL MISCELLANEOUS MATERIALS AND PROVIDE ALL LABOR
- REQUIRED FOR INSTALLATION EQUIPMENT IN EXISTING CABINET OR NEW CABINET AS SHOWN ON DRAWINGS AND AS REQUIRED BY THE APPLICABLE INSTALLATION MOPS. C. COMPLY WITH MANUFACTURER'S INSTALLATION AND START-UP REQUIREMENTS
- DC CIRCUIT BREAKER LABELING A. NEW DC CIRCUIT IS REQUIRED IN MMBS CABINET SHALL BE CLEARLY IDENTIFIED AS TO RRU BEING SERVICED

SECTION 26 100 - BASIC ELECTRICAL REQUIREMENTS

SUMMARY: THIS SECTION SPECIFIES BASIC ELECTRICAL REQUIREMENTS FOR SYSTEMS AND COMPONENTS

- <u>QUALITY ASSURANCE:</u> A. ALL EQUIPMENT FURNISHED UNDER DIVISION 26 SHALL CARRY UL LABELS AND LISTINGS WHERE SUCH LABELS AND LISTINGS ARE AVAILABLE IN THE INDUSTRY. B. MANUFACTURERS OF EQUIPMENT SHALL HAVE A MINIMUM OF THREE YEARS EXPERIENCE WITH THEIR
- EQUIPMENT INSTALLED AND OPERATING IN THE FIELD IN A USE SIMILAR TO THE PROPOSED USE FOR THIS
- C. MATERIALS AND EQUIPMENT: ALL MATERIALS AND EQUIPMENT SPECIFIED IN DIVISION 26 OF THE SAME TYPE SHALL BE OF THE SAME MANUFACTURER AND SHALL BE NEW, OF THE BEST QUALITY AND DESIGN. AND FREE FROM DEFECTS.

SERVICE.

STRUCTURES.

SUPPORTING DEVICES: A. MANUFACTURED STRUCTURAL SUPPORT MATERIALS: SUBJECT TO COMPLIANCE WITH REQUIREMENTS, PROVIDE PRODUCTS BY THE FOLLOWING: 1. ALLIED TUBE AND CONDUIT.

L EXPANSION ANCHORS: CARBON STEEL WEDGE OR SLEEVE TYPE.
 POWER-DRIVEN THREADED STUDS: HEAT-TREATED STEEL, DESIGNED SPECIFICALLY FOR THE INTENDED

4. TOGGLE BOLTS ON HOLLOW MASONRY UNITS. 5. CONCRETE INSERTS OR EXPANSION BOLTS ON CONCRETE OR SOLID MASONRY

9. IN PARTITIONS OF LIGHT STEEL CONSTRUCTION, USE SHEET METAL SCREWS.

MACHINE SCREWS, WELDED THREADED STUDS, OR SPRING-TENSION CLAMPS ON STEEL.
 EXPLOSIVE DEVICES FOR ATTACHING HANGERS TO STRUCTURE SHALL NOT BE PERMITTED.
 DO NOT WELD CONDUIT, PIPE STRAPS, OR ITEMS OTHER THAN THREADED STUDS TO STEEL

- 2. B-LINE SYSTEM.
- B. UNISTRUT DIVERSIFIED PRODUCTS.
- . THOMAS & BETTS. B. FASTENERS: TYPES, MATERIALS, AND CONSTRUCTION FEATURES AS FOLLOWS:

3. FASTEN BY MEANS OF WOOD SCREWS ON WOOD.

SUPPORTING DEVICES: A. INSTALL SUPPORTING DEVICES TO FASTEN ELECTRICAL COMPONENTS SECURELY AND PERMANENTLY IN

B COORDINATE WITH THE BUILDING STRUCTURAL SYSTEM AND WITH OTHER TRADES C. UNLESS OTHERVISE INDICATED ON THE DRAWINGS, FASTEN ELECTRICAL TEMS AND THEIR SUPPORTING HARDWARE SECURELY TO THE STRUCTURE IN ACCORDANCE WITH THE FOLLOWING: I. ENSURE THAT THE LOAD APPLIED BY ANY FASTENER DOES NOT EXCEED 25 PERCENT OF

2. USE VIBRATION AND SHOCK-RESISTANT FASTENERS FOR ATTACHMENTS TO CONCRETE

ACCORDANCE WITH NEC

SLABS.

EOUAL.

RADIUS ELBOWS.

THE PROOF TEST LOAD

OF AC PANEL BOARDS WITH ANY CHANGES MADE TO THE AC SYSTEM

BE MANUFACTURED BY ALLIED, REPUBLIC OR WHEATLAND.

OR LINIVERSAL METAL HOSE OR APPROVED FOLIAL

F. MINIMUM SIZE CONDUIT SHALL BE 3/4 INCH (21MM)

B. CABLE TERMINATION FITTINGS FOR CONDUIT

PROTECT CABLE INSULATION.

CROUSE-HINDS FORM 8 OR EQUAL

CONDUIT AND CONDUCTOR INSTALLATION

OUTSIDE AND INSIDE.

REPLACEMENT INSTRUCTION USING THREADED ROD KITS.

ROXTEC.

ELECTRICAL IDENTIFICATION: A. UPDATE AND PROVIDE TYPED CIRCUIT BREAKER SCHEDULES IN THE MOUNTING BRACKET, INSIDE DOORS

AS SUCH AT THE BRANCH CIRCUIT PANELBOARD.

SECTION 26 200 - ELECTRICAL MATERIALS AND EQUIPMENT

A. RIGID GALVANIZED STEEL (RGS) CONDUIT SHALL BE USED FOR EXTERIOR LOCATIONS ABOVE GROUND AND IN UNFINISHED INTERIOR LOCATIONS AND FOR UNDERGROUND RUNS. RIGID CONDUIT AND FITTINGS SHALL BE STEEL, COATED WITH ZINC EXTERIOR AND INTERIOR BY THE HOT DIP GALVANIZING PROCESS CONDUIT SHALL BE PRODUCED TO ANSI SPECIFICATIONS CRU L FEDERAL SPECIFICATION THREADED - SET SCREW OR COMPRESSION FITTINGS WILL NOT BE ACCEPTABLE. RGS CONDUITS SHALL

B. UNDERGROUND CONDUIT IN CONCRETE SHALL BE POLYVINYLCHLORIDE (PVC) SUITABLE FOR DIRECT BURIAL AS APPLICABLE, JOINTS SHALL BE BELLED, AND FLUSH SOLVENT WELDED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS. CONDUIT SHALL BE CARLON ELECTRICAL PRODUCTS OR APPROVED

C. TRANSITIONS BETWEEN PVC AND RIGID (RGS) SHALL BE MADE WITH PVC COATED METALLIC LONG SWEEP

D EMT OR RIGID GALVANIZED STEEL CONDUIT MAY BE USED IN FINISHED SPACES CONCEALED IN WALLS AND CEILINGS. EMT SHALL BE MILD STEEL, ELECTRODUCTION DE DIN FINISHED STACES CONCELED IN WALLS AND CEILINGS. EMT SHALL BE MILD STEEL, ELECTROCALLY WELDED, ELECTROCALVANIZED OR HOT-DIPPED GALVANIZED AND PRODUCED TO ANSI SPECIFICATION C80.3, FEDERAL SPECIFICATION WW-C-563, AND SHALL BE UL LISTED. EMT SHALL BE MANUFACTURED BY ALLIED, REPUBLIC OR WHEATLAND, OR APPROVED EQUAL. FITTINGS SHALL BE METALLIC COMPRESSION. SET SCREW CONNECTIONS SHALL NOT

BE ACCEPTABLE. E. LIQUID TIGHT FLEXIBLE METALLIC CONDUIT SHALL BE USED FOR FINAL CONNECTION TO EQUIPMENT FITTINGS SHALL BE METALLIC GLAND TYPE COMPRESSION FITTINGS, MAINTAINING THE INTEGRITY OF CONDUIT SYSTEM, SET SCREW CONNECTIONS SHALL NOT BE ACCEPTABLE. MAXIMUM LENGTH OF FLEXIBLE CONDUIT SHALL NOT EXCEED 6-FEET. LFMC SHALL BE PROTECTED AND SUPPORTED AS REQUIRED BY NEC. MANUFACTURERS OF FLEXIBLE CONDUITS SHALL BE CAROL, ANACONDA METAL HOSE

HUBS AND BOXES: A. AT ENTRANCES TO CABINETS OR OTHER EQUIPMENT NOT HAVING INTEGRAL THREADED HUBS PROVIDE METALLIC THREADED HUBS OF THE SIZE AND CONFIGURATION REQUIRED. HUB SHALL INCLUDE LOCKNUT AND NEOPRENE O-RING SEAL. PROVIDE IMPACT RESISTANT 105 DEGREE C PLASTIC BUSHINGS TO

I. CABLE TERMINATORS FOR RGS CONDUITS SHALL BE TYPE CRC BY O-Z/GEDNEY OR EQUAL BY

2. CABLE TERMINATORS FOR LFMC SHALL BE ETCO - CL2075; OR MADE FOR THE PURPOSE

PRODUCTS BY ROXTEC. C. EXTERIOR PULL BOXES AND PULL BOXES IN INTERIOR INDUSTRIAL AREAS SHALL BE PLATED CAST ALLOY, HEAVY DUTY, WEATHERPROOF, DUST PROOF, WITH GASKET, PLATED IRON ALLOY COVER AND STAINLESS

STEEL COVER SCREWS, CROUSE-HINDS WAB SERIES OR EQUAL. D. CONDUIT OUTLET BODIES SHALL BE PLATED CAST ALLOY WITH SIMILAR GASKET COVERS. OUTLET BODIES SHALL BE OF THE CONFIGURATION AND SIZE SUITABLE FOR THE APPLICATION. PROVIDE

E MANUFACTURER FOR BOXES AND COVERS SHALL BE HOFFMAN, SQUARE "D", CROUSE-HINDS, COOPER, ADALET, APPLETON, O-Z GEDNEY, RACO, OR APPROVED EQUAL.

SUPPLEMENTAL GROUNDING SYSTEM: A. FURNISH AND INSTALL A SUPPLEMENTAL GROUNDING SYSTEM TO THE EXTENT INDICATED ON THE DRAWINGS. SUPPORT SYSTEM WITH NON-MAGNETIC STAINLESS STEEL CLIPS WITH RUBBER GROMMETS. GROUNDING CONNECTORS SHALL BE TINNED COPPER WIRE, SIZES AS INDICATED ON THE DRAWINGS. PROVIDE STRANDED OR SOLID BARE OR INSULATED CONDUCTORS EXCEPT AS OTHERWISE NOTED. B. SUPPLEMENTAL GROUNDING SYSTEM: ALL CONNECTIONS TO BE MADE WITH CAD WELDS, EXCEPT AT EQUIPMENT USE LUGS OR OTHER AVAILABLE GROUNDING MEANS AS REQUIRED BY MANUFACTURER: AT

GROUND BARS USE TWO HOLE SPADES WITH NO-OX. C. STOLEN GROUND-BARS: IN THE EVENT OF STOLEN GROUND BARS, CONTACT SPRINT CM FOR

EXISTING STRUCTURE: A. EXISTING EXPOSED WIRING AND ALL EXPOSED OUTLETS, RECEPTACLES, SWITCHES, DEVICES, BOXES, A. EXISTING EXPOSED WIRING AND ALL EXPOSED OUTLETS, RECEPTACLES, SWITCHES, DEVICES, BOXES, AND OTHER EQUIPMENT THAT ARE NOT TO BE UTILIZED IN THE COMPLETED PROJECT SHALL BE REMOVED OR DE-ENERGIZED AND CAPPED IN THE WALL CEILING OR FLOOR SO THAT THEY ARE CONCEALED AND SAFE. WALL, CEILING, OR FLOOR SHALL BE PATCHED TO MATCH THE ADJACENT CONSTRUCTION

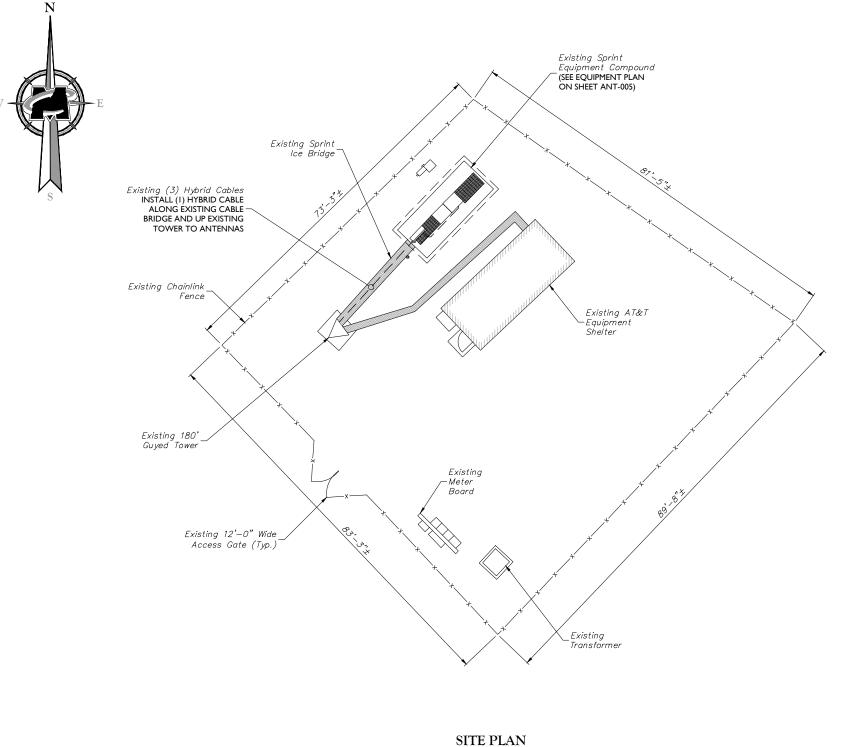
A. CONDUITS HALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-PERFORATED STRAPS AND HANGERS, EXPLOSIVE DEVICES FOR ATTACHING HANGERS TO STRUCTURE WILL NOT BE PERMITTED. CLOSELY FOLLOW THE LINES OF THE STRUCTURE, MAINTAIN CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES, CHANGES IN DIRECTION TO ROUTE AROUND OBSTACLES SHALL BE MADE WITH CONDUIT OUTLET BODIES, CONDUIT SHALL BE INSTALLED IN A NEAT AND WORKMANLIKE MANNER, PARALLEL AND PERPENDICULAR TO STRUCTURE WALL AND CEILING LINES, ALL CONDUIT SHALL BE FISHED TO CLEAR OBSTRUCTIONS, ENDS OF CONDUITS SHALL BE TEMPORARILY CAPPED TO PREVENT CONCRETE, PLASTER OR DIRT FROM ENTERING, CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED MALLEABLE IRON BUSHING ON INSIDE AND GALVANIZED MALLEABLE IRON LOCKNUT ON

B. CONDUCTORS SHALL BE PULLED IN ACCORDANCE WITH ACCEPTED GOOD PRACTICE.



GENERAL NOTES:

- I. SITE INFORMATION OBTAINED FROM THE FOLLOWING:
 - A. DRAWINGS ENTITLED "NORTH COLCHESTER", PREPARED BY NATCOMM, LLC OF BRANFORD, CONNECTICUT DATED 12/21/99.
- CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ARCHITECT/ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

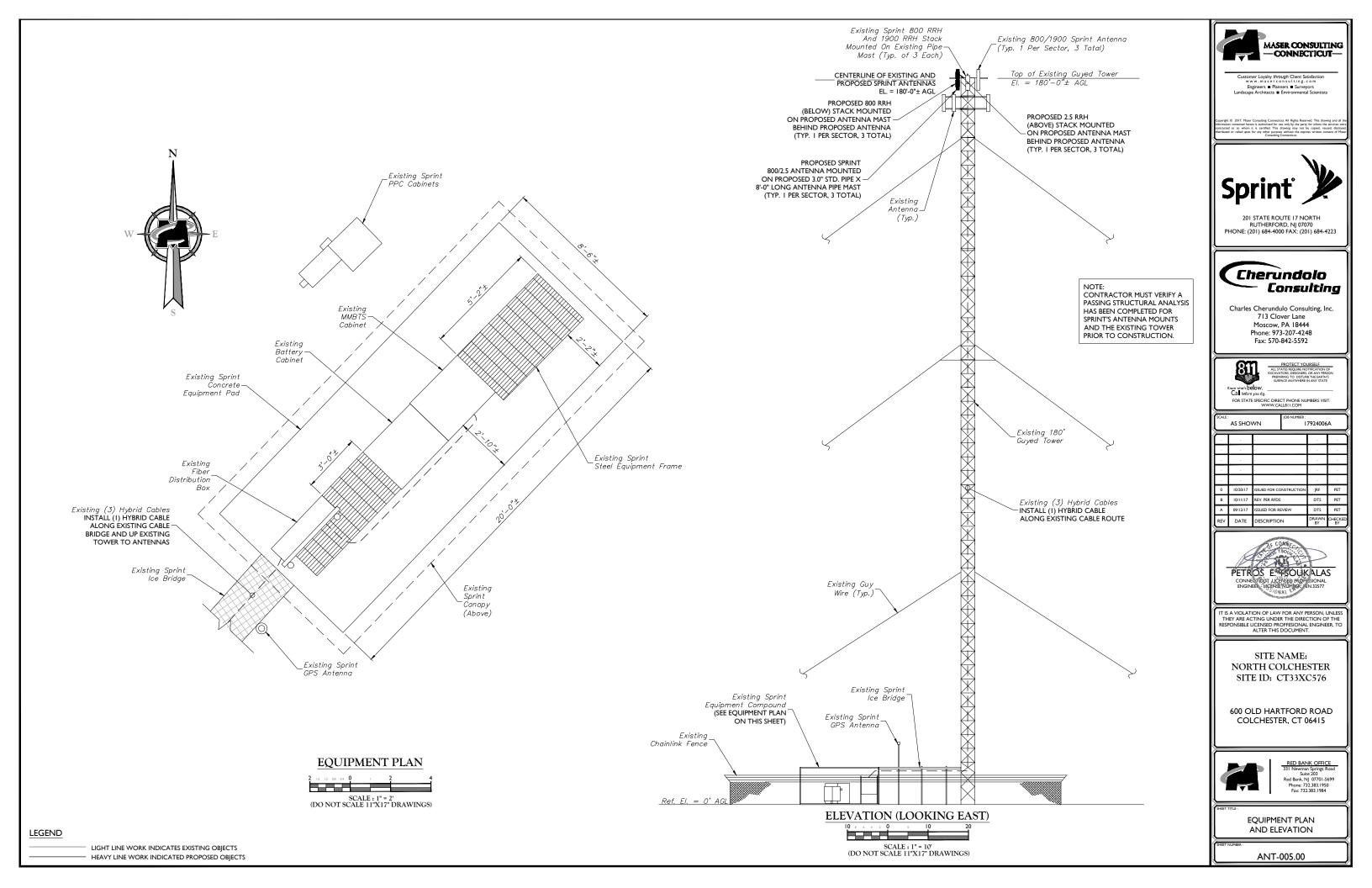


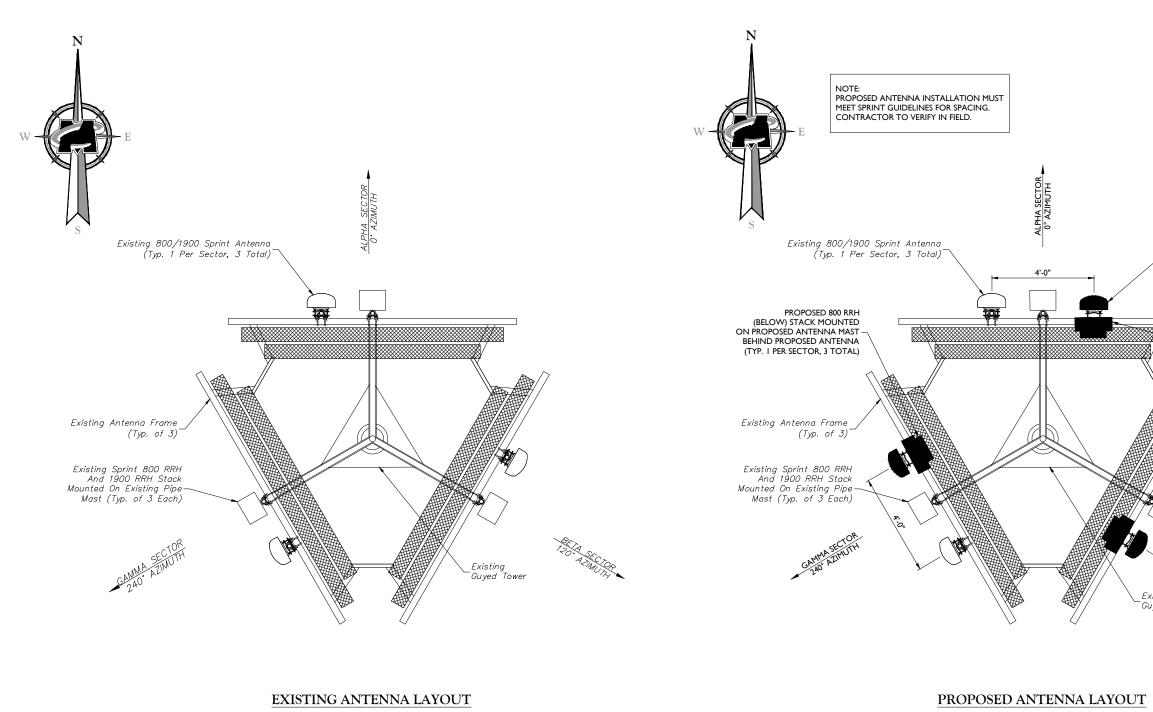


LEGEND

LIGHT LINE WORK INDICATES EXISTING OBJECTS
 HEAVY LINE WORK INDICATED PROPOSED OBJECTS

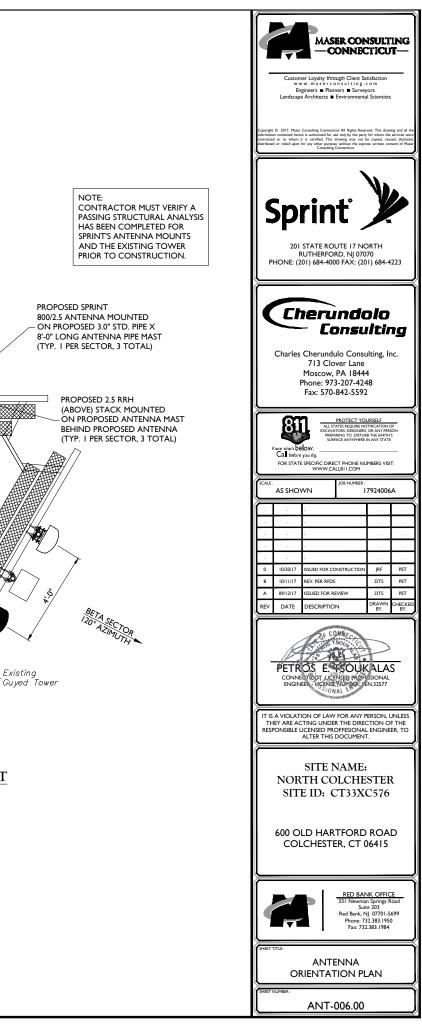


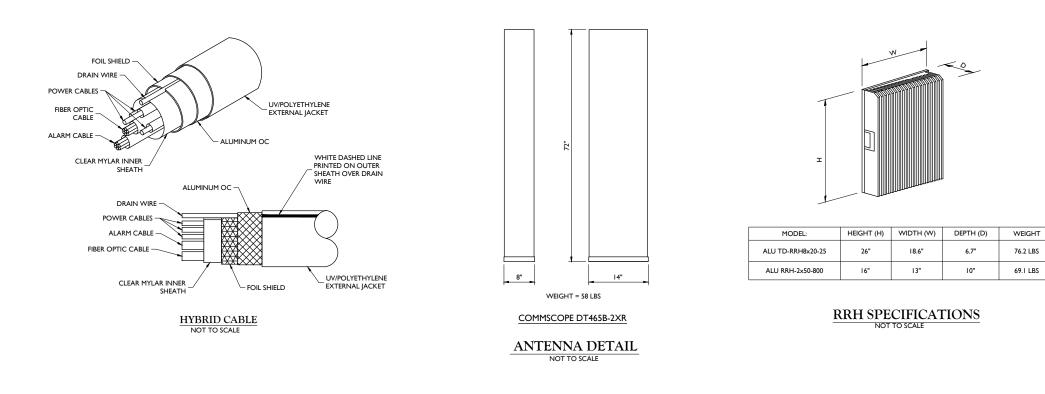


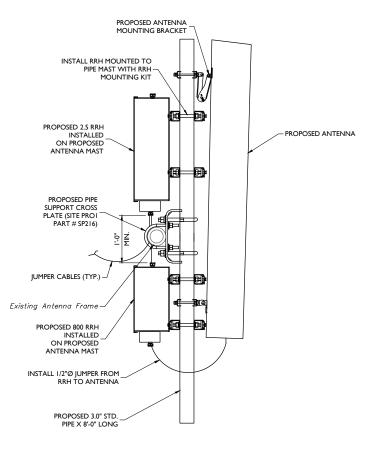


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SCALE : 1" = 2' (DO NOT SCALE 11"X17" DRAWINGS)







ANTENNA AND RRH MOUNTING DETAIL NOT TO SCALE

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RF NOTES

- I. ACTUAL CABLE LENGTHS SHALL BE DETERMINED PER SITE CONDITION BY SUBCONTRACTOR.
- 2. THE DESIGN IS BASED ON RF DATA SHEETS, SIGNED AND APPROVED.
- 3. RADIO SIGNAL CABLE AND RACEWAY SHALL COMPLY WITH THE REQUIREMENTS OF THE NATIONAL ELECTRICAL CODE (NEC, NFPA 70), CHAPTER 8.
- ALL SPECIFIED MATERIAL FOR EACH LOCATION (E.G., OUTDOORS, INDOORS-OCCUPIED, INDOORS-UNOCCUPIED, PLENUMS, RISER SHAFTS, ETC.) SHALL BE APPROVED, LISTED, OR LABELED AS REQUIRED BY THE NEC.
- 5. HARDLINE AND JUMPER CABLES SHALL BE SUPPORTED WITH HANGERS AND AT INTERVALS AS REQUIRED BY THE MANUFACTURER FOR 125 mph WIND SPEED AND EXPECTED ICE CONDITIONS. FOR SITES WITH TOWER HEIGHT OVER 300 OR ARE LOCATED IN THE EXTREME WEATHER/OPERATION AREAS, THE WORST CASE SCENARIO FOR 150 mph WIND SPEED AND I" ICE CONDITION SHOULD BE APPLIED, ALL CABLES SHOULD BE SUPPORTED AT HALF THE DISTANCE OF THE MAXIMUM HANGER SPACING FROM THE CABLE SONNECTOR LOCATION TO THE IST HANGER. MANUFACTURER RECOMMENDED CABLE SUPPORT ACCESSORIES SHALL BE USED. PLASTIC CABLE TIES ARE NOT ACCEPTABLE. HANGER STACKING LIMIT SHOULD ALSO REFER TO VENDOR'S RECOMMENDATION.
- 6. THE OUTDOOR CABLE SUPPORT SYSTEM SHALL BE PROVIDED WITH AN ICE SHIELD TO SUPPORT AND PROTECT ANTENNA CABLE RUNS.
- 7. DRIP LOOPS SHALL BE REQUIRED ON ALL OUTSIDE CABLES. CABLES SHALL BE SLOPED AWAY FROM THE BUILDING OR OUTDOOR BTS CABINETS TO PREVENT WATER FROM ENTERING THROUGH THE COAXIAL CABLE PORT.
- ALL FEEDER LINE AND JUMPER CONNECTORS SHALL BE 7/16 DIN CABLE CONNECTORS THAT MEET IP68 STANDARDS.
- 9. CONNECTORS IN INDOOR APPLICATIONS REQUIRE NO WEATHERPROOFING. OUTDOOR APPLICATIONS REQUIRE WEATHERPROOFING AND THE FOLLOWING PROCEDURES SHOULD BE FOLLOWED:

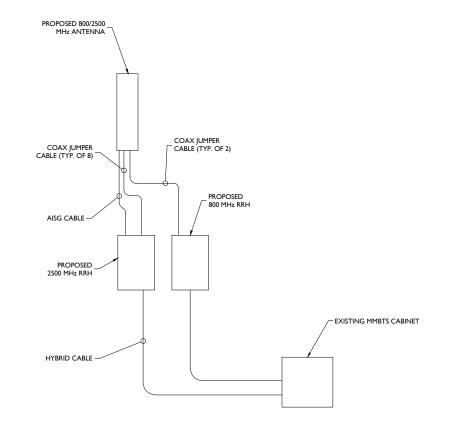
RE-ENTERABLE AND RE-SEALABLE PLASTIC ENCLOSURE APPROVED BY CABLE MANUFACTURER AND CONTRACTOR IS RECOMMENDED METHOD TO WEATHERPROOF CONNECTORS.

ALSO ACCEPTABLE IS THE USE OF BUTYL RUBBER WEATHERPROOFING KIT APPROVED BY CABLE MANUFACTURE AND CONTRACTOR. START BUTYL RUBBER TAPE APPROXIMATELY SINCHES FROM THE CONNECTOR AND WRAP 2 INCHES TOWARD THE CONNECTOR, THEN REVERSE THE TAPE SO THAT THE STICKY SIDE IS UP. TAPE OVER THE CONNECTOR OR SURGE ARRESTOR UNTIL THREE (3) TO FOUR (4) INCHES BEYOND THE CONNECTOR AND REVERSE AGAIN WITH THE STICKY SIDE DOWN FOR ANOTHER TWO INCHES. FINISH WITH TWO LAYERS OF VINYL TAPE. COLD SHRINK IS STRICTLY PROHIBITED. SELF-BONDING, AMALGAMATING TAPE MAYBE USED AS AN ALTERNATIVE TO BUTYL RUBBER TAPE.

- ANTENNAS SHALL BE PAINTED, WHEN REQUIRED, BY THE LANDLORD OR AUTHORITY HAVING JURISDICTION IN ACCORDANCE WITH ANTENNA MANUFACTURERS' SURFACE PREPARATION AND PAINTING REQUIREMENTS.
- 11. CABLE SHIELDS, AND TOWER CONDUITS SHALL BE GROUNDED AT THE TOP OF THE TOWER, WITHIN 10 FEET OF THEIR CONNECTORS, AND AT THE BOTTOM OF THE TOWER ABOUT 6 INCHES BEFORE THEY TURN TOWARD THE FACILITY. THEY SHALL BE GROUNDED AT THE MIDPOINT OF TOWERS THAT ARE BETWEEN 100 FEET AND 200 FEET HIGH, AND AT INTERVALS OF 100 FEET OR LESS ON TOWERS THAT ARE HIGHER THAN 200 FEET.
- 12. APPROVED GROUNDING KITS, WHICH INCLUDE GROUNDING STRAPS, SHALL BE USED TO GROUND THE COAXIAL CABLE SHIELDS, AND CONDUITS. THE GROUND CONDUCTORS FOR THE KITS AT THE TOP OF THE TOWER, AND IN THE MIDDLE SECTION OF THE TOWER, ARE BONDED DIRECTLY TO TOWER STEEL USING BOLTED, OR APPROVED CLAMP CONNECTIONS. EXOTHERMIC WELDS SHALL BE PERMITTED ON TOWERS ONLY WITH THE EXPRESS APPROVAL OF THE TOWER MANUFACTURER OR THE CONTRACTORS STRUCTURAL ENGINEER.
- 13. ALL RADIO SIGNAL CABLE SHALL BE LABELED AND COLOR CODED PER MARKET REQUIREMENTS.
- 14. ANTENNA FEED LINE SYSTEM SWEEP TESTING SHALL BE PERFORMED AND REPORTED IN ACCORDANCE WITH THE REQUIREMENTS OF PROJECT SPECIFICATIONS. CONTRACTOR WILL NOT ACCEPT A RADIO SIGNAL CABLE INSTALLATION WITH UNSATISFACTORY SWEEP TEST RESULTS.
- 15. PIM TESTS SHALL BE PERFORMED ON NEW AND MOVED OR MODIFIED COAXIAL CABLE INSTALLATIONS. TEST SHALL BE PERFORMED AND REPORTED IN ACCORDANCE WITH PROJECT SPECIFICATIONS.
- DC CONNECTORS AT OUTDOOR BIAS-Ts OR DIPLEXER/TRIPLEXER PORTS SHALL BE WEATHERPROOFED PER MANUFACTURER RECOMMENDATIONS.
- 17. AISG CONNECTIONS DO NOT REQUIRE ADDITIONAL WEATHERPROOFING UNLESS RECOMMENDED BY MANUFACTURER OR BY MARKET REQUIREMENTS.
- 18. INSTALL ONLY STANDARD RF JUMPER CABLES (e.g. LDF4 OR LCF12) AT TOWER-TOP APPLICATIONS. FLEXIBLE RF CABLES (e.g. FS)4 OR SCF12) SHALL NOT BE USED.
- 19. CABLES AND CONNECTORS MUST BE PREPARED AND INSTALLED USING THE TOOLS RECOMMENDED BY THE COAXIAL CABLE MANUFACTURER. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT THE CORRECT TOOLS ARE USED FOR THE SIZE AND TYPE OF COAX AND CONNECTOR. ALL ASPECTS OF INSTALLATION OF ALL COAXIAL CABLE SHALL FOLLOW THE CABLE MANUFACTURER'S RECOMMENDATIONS, INCLUDING THOSE FOR PULLING, MOUNTING AND GROUNDING.

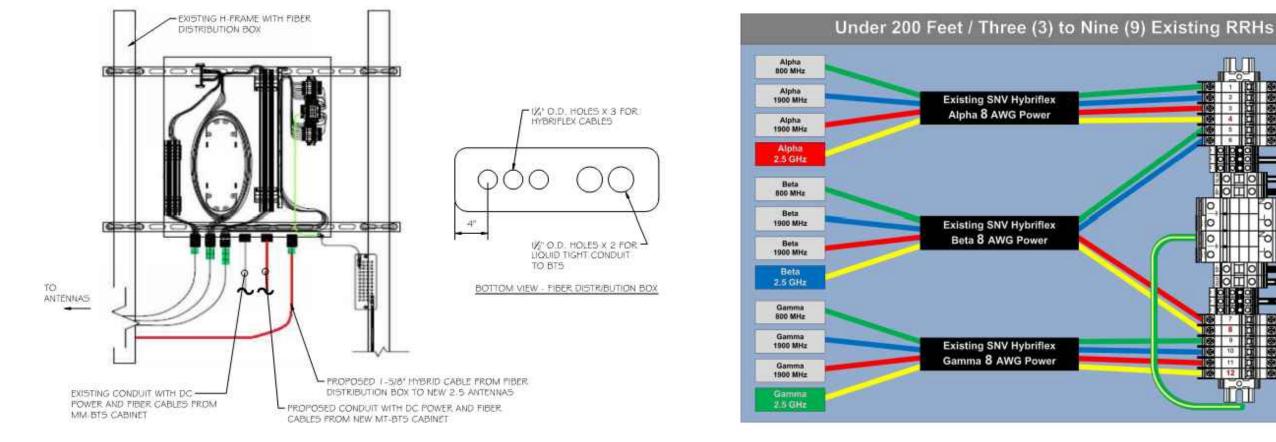
	PROPOSED ANTENNA CONFIGURATION																	
Γ	SECTOR		PROPOSED ANTENNA	TECH.	ANTENNA	HEIGHT	WIDTH	DEPTH	WEIGHT		ANTENNA AZIMUTH	ANTENNA		ANTENNA	ANTENNA	ANT. CL.	ELECTRICAL	MECHANICAL
	SECTOR		PROPOSED ANTENNA	TECH.	STATUS	(in)	(in)	(in)	(lbs)			ELEV (ft.)	DOWNTILT	DOWNTILT				
Γ	ALPHA	A1	COMMSCOPE DT465B-2XR	800/2500	NEW	72	14	8	58	0°	180'	5°/2°	0°					
	BETA	B1	COMMSCOPE DT465B-2XR	800/2500	NEW	72	14	8	58	120°	180'	5°/2°	0°					
	GAMMA	C1	COMMSCOPE DT465B-2XR	800/2500	NEW	72	14	8	58	240°	180'	5°/2°	0°					

	BILL OF MATERIALS							
NUMBER	QUANTITY	DESCRIPTION	MANUFACTURER	MODEL NUMBER				
I	3	PANEL ANTENNA	COMMSCOPE	DT465B-2XR				
2	3	2500MHZ RRH	ALU	TD-RRH8X20-25				
3	3	800MHZ RRH	ALU	RRH-2X50-800				
4	240 LF	I-I/4"Ø HYBRID FIBER RISER	ALU	TBD				
5	30	1/2"Ø JUMPER CABLE (8' LONG)	TBD					
6	3	0.315"Ø AISG CABLE (8' LONG)	COMMSCOPE	ATCB-B01-006				
7	3	PIPE SUPPORT CROSS PLATE	SITE PROI	SP216				

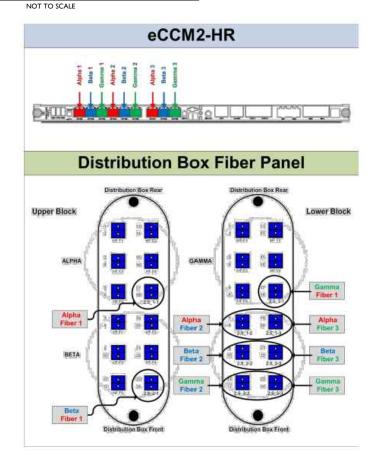


ANTENNA WIRING DIAGRAM



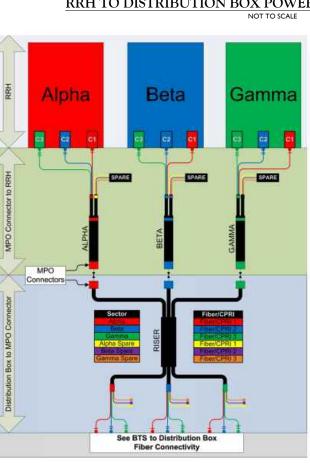


TYPICAL FIBER DISTRIBUTION BOX DETAIL

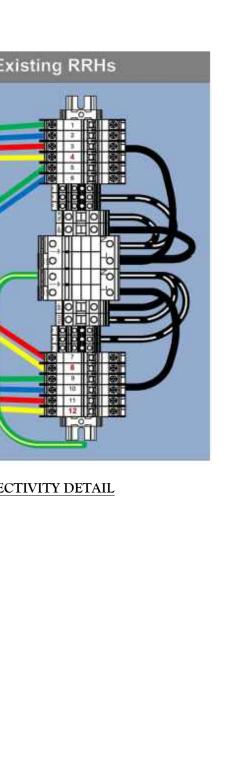


BTS TO DISTRIBUTION BOX FIBER CONNECTIVITY DETAIL

RRH TO DISTRIBUTION BOX POWER CONNECTIVITY DETAIL



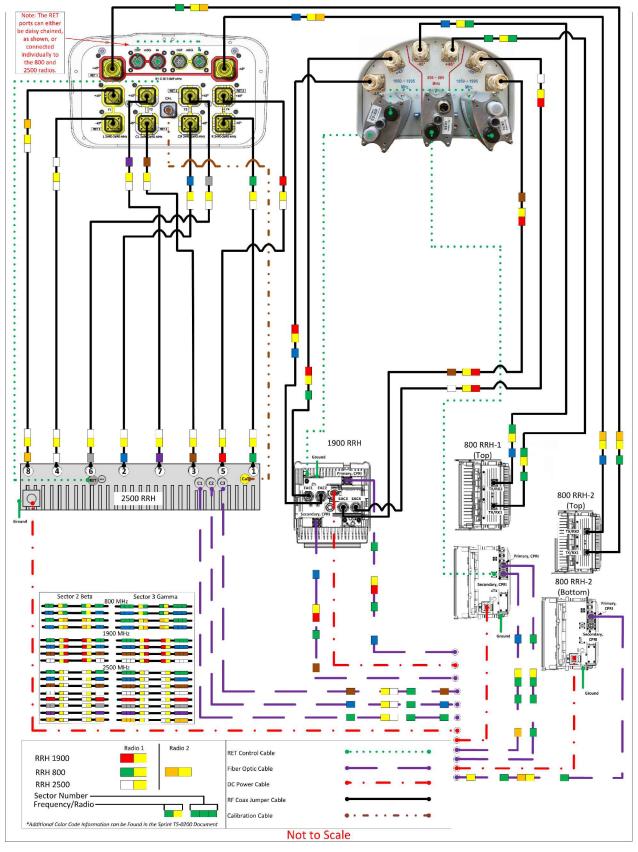
RRH TO DISTRIBUTION BOX FIBER CONNECTIVITY DETAIL





Prepared By Mark Elliott	Revision Date Revision Number August 23, 2017	R4	Sprint 🕨
Approved By RAN Hardware & Antenna Teams	Approval Date DRAFT-Macro Generated		Sprine /

ALU 211 DT465B-2XR & APXVSPP18-C-A20 wo Filters



ANTENNA WIRING DIAGRAM

NOT TO SCALE



CABLE MARKING NOTES

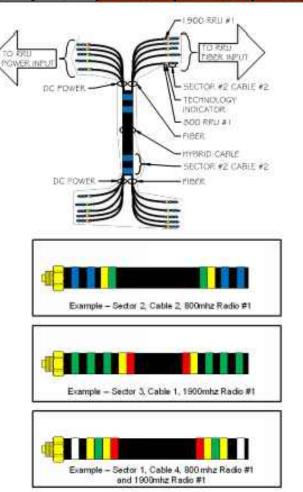
- ALL CABLES SHALL BE MARKED WITH 2" WIDE, UV STABILIZED, UL APPROVED TAPE.
- 2. THE FIRST RING SHALL BE CLOSEST TO THE END OF THE CABLE AND SPACED APPROXIMATELY 2" FROM THE END CONNECTOR, WEATHERPROOFING, OR BREAKOUT UNIT. THERE SHALL BE I'' SPACE BETWEEN EACH RING.
- 3. A 2" GAP SHALL SEPARATE THE CABLE COLOR CODE FROM THE FREQUENCY COLOR CODE. THE 2" COLOR RINGS FOR THE FREQUENCY CODE SHALL BE PLACED NEXT TO EACH OTHER WITH NO SPACES.
- 4. THE 2" COLORED TAPE(S) SHALL BE WRAPPED A MINIMUM OF 3 TIMES AROUND THE INDIVIDUAL CABLES, AND THE TAPE SHALL BE KEPT IN THE SAME LOCATION AS MUCH AS POSSIBLE.
- 5. SITES WITH MORE THAN FOUR (4) SECTORS WILL REQUIRE ADDITIONAL RINGS FOR EACH SECTOR, FOLLOWING THE PATTERN. HIGH CAPACITY SITES WILL USE THE SECOND CABLE IDENTIFIED BY BLUE BANDS OF TAPE
- 6. HYBRID FIBER CABLE SHALL BE SECTOR IDENTIFIED INSIDE THE CABILET ON FREQUENCY BUNDLES, ON THE SEALTITE, ON THE MAIN LINE UPON EXIT OF SEALTITE, AND BEFORE AND AFTER THE BREAKOUT UNIT (MEDUSA), AS WELL AS BEFORE AND AFTER ANY ENTRANCE OR EXIT
- 7. HFC "MAIN TRUNK" WILL NOT BE MARKED WITH THE FREQUENCY CODES, AS IT CONTAINS ALL FREQUENCIES.

8. INDIVIDUAL POWER PAIRS AND FIBER BUNDLES SHALL BE LABEL

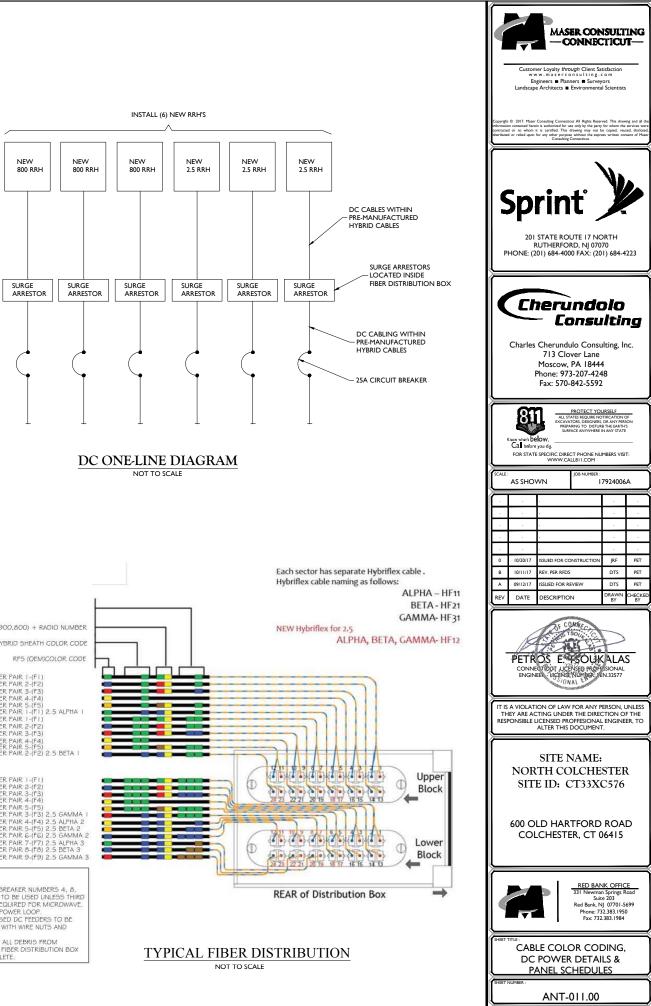
2.5 FREQUENCY	IN	DICATOR	ID
2500 -1	YEL	WHT	GRN
2500 -2	YEL	WHT	RED
2500 -3	YEL	WHT	BRN
2500 -4	YEL	WHT	BLU
2500 -5	YEL	WHT	SLT
2500 -6	YEL	WHT	ORG
2500 -7	YEL	WHT	WHT
2500 -8	YEL	WHT	ppt

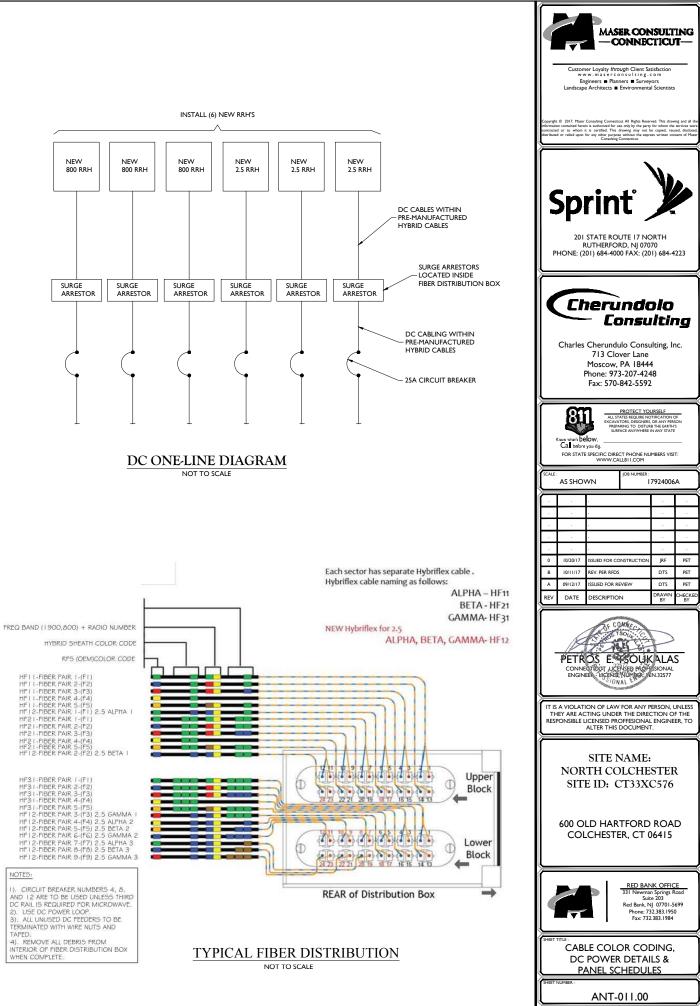
NV FREQUENCY	INDICATOR	ID
800-1	YEL	GRN
1900-1	YEL	HED:
1900-2	YEL	BRN
1900-3	YEL	BLU
1900-4	YEL	SLT
800-1	YEL.	ORG
RESERVED	YEL	WHT
RESERVED	YEL	pp

Sector	Cable	First Ring	Second Ring	Third Ring
1 Alpha	1	Green	No Tape	No Tape
1	2	Hann	No Tape	No Tape
1	3	Brown	No Tape	No Tape
1	4	White	No Tape	No Tape
1	5	Red	No Tape	No Tape
1	6	Grey	No Tape	No Tape
1	7	Purple	No Tape	No Tape
1	8	Orange	No Tape	No Tape
2 Beta	1	Green	Green	No Tape
2	2	- Shian	HILL W	No Tape
2	3	Brown	Brown	No Tape
2	4	White	White	No Tape
2	5	Red	Red	No Tape
2	6	Grey	Grey	No Tape
2	7	Pumple	Purple	No Tape
2	8	Orange	Orange	No Tape
3 Gamma	1	Green	Green	Green
3	2		and the	
3	3	Brown	Brown	Browth
3	4	White	White	White
3	5	Red	Red	Red
3	6	Grey	Grey	Grey
3	7	Purple	Purple	Purple
3	8	Orange	Orange	Orange









GENERAL REQUIREMENTS

- 1. THE WORK TO BE DONE UNDER THIS PROJECT INCLUDES PROVIDING ALL EQUIPMENT, MATERIALS, LABOR AND SERVICES, AND PERFORMING ALL OPERATIONS FOR COMPLETE AND OPERATING SYSTEMS. ANY WORK NOT SPECIFICALLY COVERED BY INECESSARY TO COMPLETE THIS INSTALLATION, SHALL BE PROVIDED, ALL EQUIPMENT AND WIRING TO BE NEW AND PROVIDED UNDER THIS CONTRACT UNLESS OTHERWISE NOTED.
- 2. ENTIRE INSTALLATION, INCLUDING MATERIALS, EQUIPMENT AND WORKMANSHIP, SHALL CONFORM TO THE 2014 EDITION OF THE NATIONAL ELECTRIC CODE (NEC) AS WELL AS ALL APPLICABLE LAWS AND REGULATIONS AND REGULATORY BODIES HAVING JURISDICTION OVER THIS WORK.
- 3. THE TERM "FURNISH" SHALL MEAN TO OBTAIN AND SUPPLY THE JOB SITE. THE TERM "INSTALL" SHALL MEAN TO FIX IN POSITION AND CONNECT FOR USE. THE TERM "PROVIDE" SHALL MEAN TO FURNISH AND INSTALL. THE TERM "CONTRACTOR" SHALL MEAN ELECTRICAL CONTRACTOR.
- 4. ONLY WRITTEN CHANGES AND/OR MODIFICATIONS APPROVED BY THE ENGINEER, CONSULTING ENGINEER OR OWNER'S REPRESENTATIVE WILL BE RECOGNIZED.
- 5. THE ELECTRICAL CONTRACTOR SHALL SUBMIT, FOR THE ENGINEER'S APPROVAL, DETAILED SHOP DRAWINGS OF ALL EQUIPMENT SPECIFIED.
- 6. CONTRACTOR SHALL COORDINATE WITH SPECIFICATIONS BY OTHER TRADES.
- 7. PROVIDE OPERATING AND MAINTENANCE MANUALS, PER SPECIFICATIONS, AND GIVE INSTRUCTIONS TO USER FOR ALL EQUIPMENT AND SYSTEMS PROVIDED UNDER THIS CONTRACT AFTER ALL ARE CLEANED AND OPERATING.
- 8. KEEP PREMISES FREE FROM RUBBISH. REMOVE ALL ELECTRICAL RUBBISH FROM SITE.
- 9. ALL WORK SHALL BE INSTALLED CONCEALED UNLESS OTHERWISE NOTED.
- 10. THE WORK SHALL INCLUDE ALL PANELS, DEVICES, FEEDERS AND BRANCH CIRCUIT WIRING AS REQUIRED FOR THE DISTRIBUTION SYSTEM INDICATED AND CALLED FOR ON THE DRAWINGS. REQUIRED BY SPECIFICATIONS AND AS NECESSARY FOR COMPLETE FUNCTIONAL SYSTEMS PRESENTED AND INTENDED.
- 11. THE CONTRACTOR SHALL FURNISH ALL MATERIAL, LABOR, TOOLS, EQUIPMENT, CONSUMABLES AND SERVICES REQUIRED FOR OBTAINING, DELIVERY, INSTALLATION, CONNECTION, DISCONNECTION, REMOVAL, RELOCATION, REPAR, REPLACEMENT, TESTING AND COMMISSIONING OF ALL EQUIPMENT AND DEVICES INCLUDED IN OR INCESSARY FOR THE WORK, AS APPLICABLE THIS INCLUDES SCAFFOLDING, LADDERS, REGGING, HOISTING, ETC.
- 12. ELECTRICAL WORK SHALL INCLUDE ALL REQUIRED CUTTING, PATCHING AND THE FULL RESTORATION OF WALL AND FLOOR STRUCTURE AND SURFACES. ALL EQUIPMENT, WALLS, FLOORS, ETC., DISTURBED OR DAMAGED DURING CONSTRUCTION SHALL BE REPAIRED TO THE SATISFACTION OF THE OWNER, AT THE CONTRACTORS EXPENSE.
- 13. BEFORE SUBMITTING HIS BID, THE CONTRACTOR SHALL FULLY ACQUAINT HIMSELF/HERSELF WITH THE JOB CONDITIONS AND DIFFICULTIES THAT WILL PERTAIN TO THE EXECUTION OF THIS WORK, SUBMISSION OF A PROPOSAL WILL BE CONSTRUED AS EVIDENCE THAT SUCH AN EXAMINATION HAS BEEN MADE. LATER CLAIMS WILL NOT BE RECOGNIZED FOR EXTRA LABOR. EQUIPMENT OR MATERIALS REQUIRED BECAUSE OF DIFFICULTIES ENCOUNTERED, WHICH COULD NOT HAVE BEEN FORESEEN HAD SUCH AN EXAMINATION BEEN MADE.
- 14. THE CONTRACTOR SHALL CONFIRM THE LOCATION OF ALL UTILITIES. THE CONTRACTOR IS RESPONSIBLE FOR REPAIRING ANY DAMAGE TO EXISTING UTILITIES.
- 15. UPON COMPLETION OF THE ELECTRICAL WORK, THE CONTRACTOR SHALL TEST THE COMPLETE ELECTRICAL SYSTEM FOR SHORTS, GROUNDS, AND PROPER OPERATION, IN THE PRESENCE OF THE OWNER'S REPRESENTATIVE.
- 16. UPON COMPLETION OF WORK, THE CONTRACTOR SHALL CLEAN AND ADJUST ALL EQUIPMENT AND LIGHTING AND TEST SYSTEMS TO THE SATISFACTION OF OWNER AND ENGINEER. RESULTS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- 17. THE CONTRACTOR SHALL FIELD VERIFY DIMENSIONS OF FINISHED CONSTRUCTION PRI/OR TO FABRICATION AND INSTALLATION OF FIXTURES AND EQUIPMENT.
- 18. EXACT ROUTING OF CONDUITS AND "MC" CABLES SHALL BE DETERMINED IN THE FIELD.
- 19. IF THE OWNER AND/OR HIS REPRESENTATIVE CONSIDERS ANY WORK TO BE INFERIOR, THE RESPECTIVE CONTRACTOR SHALL REPLACE SAME WITH CONTRACT STANDARD WORK WITHOUT ADDITIONAL CHARGE. ALL WORK SHALL BE DONE IN A NEAT, WORKMANLIKE MANNER. LEFT CLEAN AND FREE FROM DEFECTS, AND COMPLETELY OPERABLE.
- 20. THE CONTRACTOR SHALL PROVIDE ALL MATERIALS AS SHOWN ON THE DRAWINGS AND/OR AS SPECIFIED. ALL MATERIALS SHALL BE NEW, AND BEAR THE UL LABEL ALL WORK SHALL BE GUARANTEED BY THE CONTRACTOR FOR A PERIOD OF ONE (I) YEAR FROM THE DATE OF ACCEPTANCE BY THE OWNER.
- 21. DRAWINGS ARE TO BE CONSIDERED DIAGRAMMATIC, AND SHALL BE FOLLOWED AS CLOSELY AS CONDITIONS ALLOW TO COMPLETE THE INTENT OF THE CONTRACT. THE DRAWINGS AND SPECIFICATIONS COMPLIMENT AND VICE VERSA, IS TO BE INCLUDED IN THE SCOPE OF WORK.
- 22. ALL EQUIPMENT CONNECTIONS SHALL BE INSTALLED PER APPLICABLE SEISMIC REQUIREMENTS.
- 23. ENGINEER WILL MAKE A FINAL INSPECTION WITH THE OWNER AND CONTRACTOR AND WILL NOTIFY THE CONTRACTOR IN WRITING OF ALL PARTICULARS IN WHICH THIS INSPECTION REVEALS THAT THE WORK IS INCOMPLETE OR DEPECTIVE. THE CONTRACTOR SHALL IMMEDIATELY TAKE SUCH MEASURES AS ARE NECESSARY TO COMPLETE SUCH WORK OR REMEDY SUCH DEFICIENCIES.
- 24. THE CONTRACTOR SHALL PERFORM ALL EXCAVATION, TRENCHING, AND BACKFILL AS REQUIRED FOR ELECTRICAL WORK. BACKFILL SHALL BE SUITABLE MATERIAL PROPERLY COMPACTED TO 95% DENSITY IN EACH LAYER OF SIX (6) INCH DEPTH. CONDUIT SHALL BE MINIMUM 36" BELOW FINISHED GRADE.

PROJECT COORDINATION:

- I. THE CONTRACTOR SHALL VERIFY FIELD CONDITIONS AT THE SITE AND NOTIFY THE OWNER OF ANY DISCREPANCIES, PRIOR TO COMMENCING WITH THE WORK.
- 2. THE CONTRACTOR SHALL REVIEW AND COORDINATE WITH THE DOCUMENTS OF ALL TRADES.
- 3. THE CONTRACTOR SHALL FURNISH A SCHEDULE INDICATING HIS PORTION OF TIME, WITHIN THE OVERALL SCHEDULE, REQUIRED TO COMPLETE THE WORK, IN CONJUNCTION WITH ALL TRADES. ALL WORK THAT MAY AFFECT OPERATION OF BUILDING SYSTEMS SHALL BE COORDINATED WITH THE OWNER'S REPRESENTATIVE.
- 4. SHUT DOWN OF POWER SHALL BE COORDINATED WITH THE OWNER, ARCHITECT AND PROJECT MANAGER AT LEAST 14 WORKING DAYS PRIOR TO SHUT DOWN. SHUT DOWNS LONGER THAN 2 DAYS SHALL BE COORDINATED WITH THE ABOVE PERSONNEL AT LEAST ONCE A MONTH IN ADVANCE. TEMPORARY POWER FOR CONSTRUCTION SHALL BE PROVIDED BY THE ELECTRICAL CONTRACTOR FOR SHUT DOWNS OVER 2 DAYS.
- 5. ALL CONDUITS AND DEVICE BOXES SHALL BE PROVIDED BY THE ELECTRICAL CONTRACTOR, INCLUDING ALL TECHNOLOGY CONDUITS AND BOXES.
- 6. INSTALL NEW WORK AND CONNECT TO EXISTING WORK WITH MINIMUM INTERFERENCE TO EXISTING FACILITIES. ALARM AND EMERGENCY SYSTEMS SHALL NOT BE INTERRUPTED. TEMPORARY SHUT DOWNS OF ANY SYSTEMS SHALL BE COORDINATED WITH AND APPROVED BY THE OWNER AND ARCHITECT.

PROTECTION OF WORK:

I. EFFECTIVELY PROTECT ALL MATERIALS AND EQUIPMENT FROM ENVIRONMENTAL AND PHYSICAL DAMAGE UNTIL FINAL ACCEPTANCE. CLOSE AND PROTECT ALL OPENINGS DURING CONSTRUCTION. PROVIDE NEW MATERIALS AND EQUIPMENT TO REPLACE ITEMS DAMAGED.

WARRANTIES AND BONDS:

- I. ALL MATERIALS, EQUIPMENT AND WORKMANSHIP SHALL BE GUARANTEED IN WRITING FOR A MINIMUM OF ONE YEAR AFTER FINAL ACCEPTANCE BY OWNER.
- 2. OBTAIN AND DELIVER TO THE OWNER'S REPRESENTATIVE ALL GUARANTEES AND CERTIFICATES OF COMPLIANCE.

PERMITS

I. CONTRACTOR SHALL OBTAIN AND PAY FOR ALL REQUIRED PERMITS AND INSPECTION FEES FOR ELECTRICAL WORK.

RACEWAYS:

- 1. ALL CONDUIT SHALL BE MINIMUM SIZE OF 3/4" FOR POWER CIRCUITS AND CONTROL CIRCUITS EXCEPT WHERE FLEXIBLE CONDUIT IS CALLED FOR ON PROJECT DOCUMENTS. ALL EXTERIOR EXPOSED CONDUIT SHALL BE GRC (GALVANIZED RIGID METAL CONDUIT). ALL UNDERGROUND, IN SLAB OR UNDER SLAB SHALL BE RNC (RIGID NONMETALLIC CONDUIT). CHANGE RIGID METALLIC CONDUIT FOR INTERMEDIATE METALLIC CONDUIT BEFORE EXITING OUT OF CONCRETE OR PENETRATING A WALL, FLOOR OR ROOF. EMT IS ALLOWED IN INTERIOR DRY LOCATIONS WHERE NOT SUBJECT TO DAMAGE.
- 2. ALL FLEXIBLE CONDUIT IN WET OR DRY AREAS SHALL BE LIQUID TIGHT CONDUIT. NONMETALLIC FLEXIBLE CONDUIT IS SPECIFICALLY PROHIBITED.
- 3. CONDUIT SHALL BE RUN AT RIGHT ANGLES AND PARALLEL TO BUILDING LINES, SHALL BE NEATLY RACKED AND SECURELY FASTENED. JUNCTION BOXES SHALL BE PROVIDED WHERE REQUIRED TO FACILITATE INSTALLATION OF WIRES.
- 4. ALL CONDUIT AND ELECTRICAL EQUIPMENT SHALL BE SUPPORTED FROM THE BUILDING STRUCTURE IN AN APPROVED MANNER
- 5. ALL EMPTY RACEWAYS SHALL BE FURNISHED WITH A 200 LB. TEST NYLON DRAG LINE.
- 6. ARRANGEMENT OF CONDUIT AND EQUIPMENT SHALL BE AS INDICATED, UNLESS MODIFICATION IS REQUIRED TO AVOID INTERFERENCES.
- 7. FOR CONDUITS CROSSING EXPANSION JOINTS, PROVIDE EXPANSION FITTINGS FOR SIZE 1 1/4" AND LARGER. PROVIDE SECTIONS OF FLEXIBLE CONDUIT WITH GROUNDING JUMPERS FOR SIZES 1" AND SMALLER.
- 8. THE CONTRACTOR SHALL INSTALL DETECTABLE UNDERGROUND TAPES FOR THE PROTECTION, LOCATION AND IDENTIFICATION OF UNDERGROUND CONDUIT INSTALLATION.
- 9. EXACT ROUTING OF CONDUITS AND CABLES SHALL BE DETERMINED IN FIELD.

WIRING

- ALL WIRE SHALL BE COPPER WITH TYPE THNN/THWN 600 VOLT INSULATION, MINIMUM #12 AWG FOR POWER AND LIGHTING CIRCUITS AND #16 AWG FOR CONTROL CIRCUITS.
- 2. UNDER NO CIRCUMSTANCES SHALL FEEDERS BE SPLICED.
- 3. ALL COMPUTER CIRCUITS SHALL HAVE SEPARATE NEUTRAL CONDUCTORS. ALL OTHER CIRCUITS MAY SHARE GROUND AND NEUTRAL CONDUCTORS.
- 4. WHERE EQUIPMENT, LIGHTING FIXTURES AND WIRING DEVICES ARE SHOWN WITH CIRCUIT NUMBERS ONLY, THE MINIMUM BRANCH CIRCUITING REQUIREMENTS SHALL BE AS FOLLOWS.
- 5. CONTRACTOR SHALL INCREASE SIZE OF CIRCUIT WIRING/CONDUCTORS TO COMPENSATE FOR VOLTAGE DROP
- 6. WIRE SIZES SHALL BE INCREASED TO COMPENSATE FOR VOLTAGE DROP AS FOLLOWS:

GROUNDING:

- 1. PROVIDE A COMPLETE EQUIPMENT GROUND SYSTEM FOR THE ELECTRICAL SYSTEM AS REQUIRED BY ARTICLE 250, OF THE NEC, AND AS SPECIFIED HEREIN.
- 2. ALL BRANCH CIRCUITS FOR POWER WIRING SHALL CONTAIN A COPPER GROUND WIRE. NO FLEXIBLE METAL CONDUIT OF ANY KIND OR LENGTH SHALL BE USED AS THE EQUIPMENT GROUNDING CONDUCTOR.
- THE EQUIPMENT BONDING JUMPER SHALL BE PERMITTED TO BE INSTALLED INSIDE OR OUTSIDE OF A RACEWAY OR ENCLOSURE. WHERE INSTALLED ON OUTSIDE, THE LENGTH OF THE EQUIPMENT BONDING JUMPER SHALL NOT EXCEED 6 FEET AND SHALL BE ROUTED WITH THE RACEWAY OR ENCLOSURE. REFER TO NEC 2011 - 250.102 (E)
- 4. ALL GROUNDING DEVICES SHALL BE U.L. APPROVED OR LISTED FOR THEIR INTENDED USE
- 5. ALL WIRES SHALL BE AWG THHN/THWN COPPER UNLESS NOTED OTHERWISE.
- 6. GROUNDING CONNECTIONS TO GROUND RODS, GROUND RING WIRE, TOWER BASE AND FENCE POSTS SHALL BE EXOTHERMIC ("CADWELDS") UNLESS NOTED OTHERWISE. CLEAN SURFACES TO SHINY METAL, WHERE GROUND WIRES ARE CADWELDED TO GALVANIZED SURFACES, SPRAY CADWELD WITH GALVANIZING PAINT.
- GROUNDING CONNECTIONS TO GROUND BARS ARE TO BE TWO-HOLE BRASS MECHANICAL CONNECTORS WITH STAINLESS STEEL HARDWARE (INCLUDE SCREW SET). CLEAN GROUND BAR TO SHINY METAL. AFTER MECHANICAL CONNECTION, TREAT WITH PROTECTIVE ANTIOXIDANT COATING.
- 8. GROUND COAXIAL CABLE SHIELDS AT BOTH ENDS WITH MANUFACTURERS' GROUNDING KITS.
- 9. ROUTE GROUNDING CONDUCTORS THE SHORTEST AND STRAIGHTEST PATH POSSIBLE. BEND GROUNDING LEADS WITH A MINIMUM 12" RADIUS.
- 10. INSTALL #2 AWG GREEN-INSULATED STRANDED WIRE FOR ABOVE GRADE GROUNDING AND #2 BARE TINNED COPPER WIRE FOR BELOW GRADE GROUNDING UNLESS OTHERWISE NOTED.
- 11. GROUNDING CONNECTIONS SHALL BE EXOTHERMIC TYPE ("CADWELDS") TO GROUND RING, REMAINING GROUNDING CONNECTIONS SHALL BE COMPRESSION FITTINGS, CONNECTIONS TO GROUND BARS SHALL BE MADE WITH TWO-HOLE LUGS.
- 12. EXOTHERMIC WELDS SHALL BE MADE IN ACCORDANCE WITH ERICO PRODUCTS BULLETIN A-AT
- 13. CONSTRUCTION OF GROUND RING AND CONNECTIONS TO EXISTING GROUND RING SYSTEM SHALL BE DOCUMENTED WITH PHOTOGRAPHS PRIOR TO BACKFILLING SITE, PROVIDE PHOTOS TO CARRIER'S CONSTRUCTION MANAGER.
- 14. ALL GROUND LEADS EXCEPT THOSE TO THE EQUIPMENT ARE TO BE #2/0 TINNED. ALL EXTERIOR GROUND BARS TINNED COPPER.
- 15. PRIOR TO INSTALLING LUGS ON GROUND WIRES, APPLY THOMAS & BETTS KOPR-SHIELD (TM OF JET LUBE INC.) PRIOR TO BOLTING GROUND WIRE LUGS TO GROUND BARS, APPLY KOPR-SHIELD OR EQUAL.
- 16. ENGAGE IN INDEPENDENTLY ELECTRICAL TESTING FIRM TO TEST AND VERIFY THAT IMPEDANCE DOES NOT EXCEED FIVE OHMS TO GROUND BY MEANS OF "FALL OF POTENTIAL TEST". TEST SHALL BE WITNESSED BY CARRIER REPRESENTATIVE, AND RECORDED ON CARRIER'S "GROUND RESISTANCE TEST" FORM.
- 17. WHERE BARE COPPER GROUND WIRES ARE ROUTED FROM ANY CONNECTION ABOVE GRADE TO GROUND RING, INSTALL WIRE IN 3/4" PVC SLEEVE, FROM 1" BELOW GRADE AND SEAL TOP WITH SILICONE MATERIAL.
- PREPARE ALL BONDING SURFACES FOR GROUNDING CONNECTIONS BY REMOVING ALL PAINT AND CORROSION DOWN TO SHINY METAL. FOLLOWING CONNECTION, APPLY APPROPRIATE ANTI-OXIDIZATION PAINT.
- 19. ANY SITE WHERE THE EQUIPMENT (BTS, CABLE BRIDGE, PPC, GENERATOR, ETC.) IS LOCATED WITHIN 6 FEET OF METAL FENCING THE BGR SHALL BE BONDED TO THE NEAREST FENCE POST USING (2) RUNS OF #2 BARE TINNED COPPER WIRE.



