

ORIGINAL

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**DETAILED STRUCTURAL ANALYSIS AND  
EVALUATION WITH PROPOSED  
REINFORCEMENT OF 320' SELF SUPPORTING  
LATTICE TOWER FOR NEW ANTENNA  
ARRANGEMENT**

Connecticut State Police  
112 Munn Road  
Colchester, Connecticut

EM-CING-028-060314

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*prepared for*



Cingular Wireless  
500 Enterprise Drive, Suite 3A  
Rocky Hill, CT 06067

RECEIVED  
MAR 14 2006  
CONNECTICUT  
SITING COUNCIL

*prepared by*



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March 14, 2006

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1. EXECUTIVE SUMMARY

This report summarizes the structural analysis of the existing 320' self-supporting lattice tower structure located at 112 Munn Road in Colchester, Connecticut. The analysis was conducted in accordance with the TIA/EIA-222-F standard for wind velocity of 90 mph concurrent with 1/2" ice. The antenna loading considered in the analysis consists of all existing and proposed antennas, transmission lines, and ancillary items as outlined in the Introduction Section of this report. The proposed Cingular Wireless modification is as follows:

Proposed Antenna and Mount	Carrier	Antenna Center Elevation
Install (12) Powerwave 7770.00 antennas , (12) Powerwave LPG21401 TMA's and (12) Powerwave LPG13519 Diplexers on (3) new T-Arms with (24) 1 5/8" coax cables stacked (12) on (12).	Cingular Wireless (Proposed)	@ 200'

The results of the analysis indicate that the existing tower structure is not in compliance with the proposed loading conditions. **The tower is not considered structurally adequate under the TIA/EIA-222-F wind load specified above and the existing and proposed antenna loadings. Reinforcement of the tower is considered feasible to meet the TIA/EIA-222-F wind load specified above with all existing and proposed antenna loads.** Recommended modifications are outlined in greater detail in section 4 and 6 of this report. With reinforcement, the tower sway is 0.75 degrees, and the tower twist is 0.42 degrees. These are within the Connecticut State Police specification of 0.75 degrees for twist and sway.

This analysis is based on:

- 1) The tower structure's theoretical capacity, not including any assessment of the condition of the tower.
- 2) Tower geometry and structural member sizes taken from original construction drawings (Rohn File #: 43233AE) prepared by Rohn Industries, Inc., approved May 10, 2001.
- 3) Antenna and mount configuration as specified on the following page of this report.
- 4) Coax cable orientation as specified in section 6 of this report.

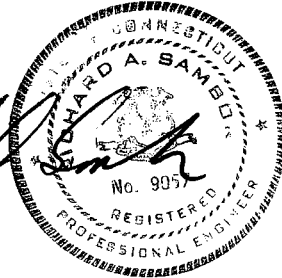
This report is only valid as per the assumptions and data utilized in this report for antenna inventory, mounts and associated cables. The user of this report shall field verify the assumption of the antenna and mount configuration. Notify the engineer in writing immediately if any of the information in this report is found to be other than specified.

If you should have any questions, please call.

Sincerely,

URS Corporation

Richard A. Sambor, P.E.  
Manager Facilities Design



RAS/jek

cc: AA, DR, IA – URS  
CF/Book

## 2. INTRODUCTION

The subject tower is located at 112 Munn Road in Colchester, Connecticut. The structure is a 320' self-supporting lattice tower structure designed by Rohn Industries, Inc.

The tower geometry and structure member sizes were taken from the original construction drawings (Rohn File #: 43233AE) prepared by Rohn Industries, Inc., approved May 10, 2001.

The inventory is summarized in the table below:

<i>Antenna Type</i>	<i>Carrier</i>	<i>Mount</i>	<i>Antenna Centerline Elevation</i>	<i>Cable</i>
(1) PD128 antenna	(existing)	Side Arm Mount	320'	(1) 7/8" coax cable
(1) PD128 antenna	(existing)	Side Arm Mount	318'	(1) 7/8" coax cable
(1) 8 FT dish	(existing)	Dish Mount	315'	(1) 7/8" coax cable
(3) 6 FT dishes	(existing)	(3) Dish Mount	308'	(3) EW63 coax cables
(1) DB224 antenna	(existing)	Side Arm Mount	294'	(1) 7/8" coax cable
(1) PD320 antenna	(existing)	Side Arm Mount	292'	(1) 7/8" coax cable
(1) DB809 antenna	(existing)	Side Arm Mount	285'	(1) 1 5/8" coax cable
(1) OGT9 antenna	(existing)	Side Arm Mount	275'	(1) 1 5/8" coax cable
(1) PD440 antenna	(existing)	Side Arm Mount	257'	(1) 7/8" coax cable
(1) PD128 antenna	(existing)	Side Arm Mount	250'	(1) 7/8" coax cable
(1) PD320 antenna	(existing)	Side Arm Mount	243'	(1) 7/8" coax cable
(6) DB844 antennas and (6) DB948F85T2E-M antennas	Verizon (existing)	(3) T-Arms	220'	(12) 1 5/8" coax cables
<b>(12) Powerwave 7770.00 antennas , (12) LPG21401 TMA's and (12) LPG13519 Diplexers</b>	<b>Cingular (proposed)</b>	<b>(3) T-Arms</b>	<b>200'</b>	<b>(24) 1 5/8" coax cables</b>
(1) BA1012 antenna	(existing)	Side Arm Mount	140'	(1) 7/8" coax cable
(1) PD688S antenna	(existing)	Side Arm Mount	140'	(1) 7/8" coax cable
(1) 6 FT dish	(reserved)	Dish Mount	115'	(1) EW63 coax cable
(1) PD156S antenna	(existing)	Flush Mount	138'	(1) 7/8" coax cable
(1) 6 FT dish	(reserved)	Dish Mount	115'	(1) EW63 coax cable
(1) 2 FT dish	(existing)	Dish Mount	112'	(1) EW108 coax cable
(1) 6 FT dish	(existing)	Dish Mount	105'	(1) EW65 coax cable
(1) PD458 antenna	(existing)	Side Arm Mount	105'	(1) 7/8" coax cable
(1) DB437 antenna	(existing)	Side Arm Mount	100'	(1) 7/8" coax cable
(1) 6 FT dish	(existing)	Dish Mount	97'	(1) 7/8" coax cable
(1) 4 FT dish	(existing)	Dish Mount	90'	(1) 7/8" coax cable

This structural analysis of the communications tower was performed by URS Corporation (URS) for Cingular Wireless. The purpose of this analysis was to investigate the structural integrity of the existing tower with its existing and proposed antenna loads. This analysis was conducted to evaluate stress on the tower and the effect of forces to the foundation of the tower resulting from existing and proposed antenna arrangements.

### 3. ANALYSIS METHODOLOGY AND LOADING CONDITIONS

The structural analysis was done in accordance with TIA/EIA-222-F, Structural Standard for Steel Antenna Towers and Antenna Supporting Structures; 2003 IBC with the 2005 Connecticut State Building Code Supplement; and the American Institute of Steel Construction (AISC) Manual of Steel Construction, Allowable Stress Design (ASD).

The analysis was conducted using ERI Tower 3.0. One load condition was evaluated as shown below which was compared to allowable stresses according to AISC and TIA/EIA.

Load Condition 1 = 90 mph (fastest mile) Wind Load (with ice) + Ice Load + Tower Dead Load

The TIA/EIA standard permits a one-third increase in allowable stresses for towers and monopoles less than 700 feet tall. For the purposes of this analysis, in computing the load capacity the allowable stresses of the tower members were increased by one-third.

### 4. FINDINGS AND EVALUATION

The calculated stresses on the tower structure were evaluated to compare with the allowable stress in accordance with AISC. The results of the analysis indicate that the existing tower structure is not in compliance with the proposed loading conditions. **The tower is not considered structurally adequate under the TIA/EIA-222-F wind load specified above and the existing and proposed antenna loadings. Reinforcement of the tower is considered feasible to meet the TIA/EIA-222-F wind load specified above with all existing and proposed antenna loads.** Several Redundant Diagonals and Redundant Horizontals are overstressed. We recommend replacing the overstressed members.

Section	Overstressed Members		
	Redundant Diagonal 1	Redundant Diagonal 2	Redundant Horizontal 2
0'—30'	---	---	Rohn 2 EH
30'—60'	Rohn 1.5 STD	Rohn 2 STD	---
100'—120'	Rohn 1.5 STD	---	---

For recommended member sizes see drawing SK-1 in section 6 of this report.

With reinforcement, the tower sway is 0.75 degrees, and the tower twist is 0.42 degrees. These are within the Connecticut State Police specification of 0.75 degrees for twist and sway. No further analysis was performed on the anchor bolts and foundation since the calculated reactions at the top of the foundation were below the original design.

## 5. CONCLUSIONS

The results of the analysis indicate that the existing tower structure is not in compliance with the proposed loading conditions. **The tower is not considered structurally adequate under the TIA/EIA-222-F wind load specified above and the existing and proposed antenna loadings. Reinforcement of the tower is considered feasible to meet the TIA/EIA-222-F wind load specified above with all existing and proposed antenna loads.** Recommended modifications are outlined in greater detail in section 4 and 6 of this report. With reinforcement, the tower sway is 0.75 degrees, and the tower twist is 0.42 degrees. These are within the Connecticut State Police specification of 0.75 degrees for twist and sway.

### **Limitations/Assumptions:**

This report is based on the following:

1. Tower inventory as listed in this report.
2. Tower is properly installed and maintained.
3. All members are as specified in the original design documents and are in good condition.
4. All required members are in place.
5. All bolts are in place and are properly tightened.
6. Tower is in plumb condition.
7. All member protective coatings are in good condition.
8. All tower members were properly designed, detailed, fabricated, and installed and have been properly maintained since erection.
9. Foundations were properly constructed to support original design loads as specified in the original design documents.

URS is not responsible for any modifications completed prior to or hereafter in which URS is not or was not directly involved. Modifications include but are not limited to:

- A. Adding antennas
- B. Removing/replacing antennas
- C. Adding coaxial cables

URS hereby states that this document represents the entire report and that it assumes no liability for any factual changes that may occur after the date of this report. All representations, recommendations, and conclusions are based upon information contained and set forth herein. If you are aware of any information which conflicts with that which is contained herein, or you are aware of any defects arising from original design, material, fabrication, or erection deficiencies, you should disregard this report and immediately contact URS. URS disclaims all liability for any representation, recommendation, or conclusion not expressly stated herein.

### **Ongoing and Periodic Inspection and Maintenance:**

After the Contractor has successfully completed the installation and the work has been accepted, the owner will be responsible for the ongoing and periodic inspection and maintenance of the tower.

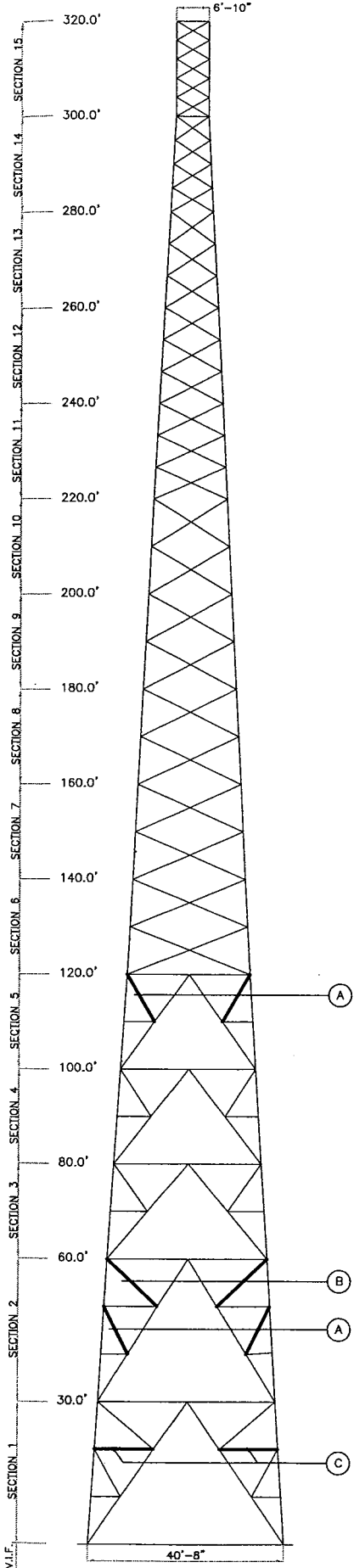
The owner shall refer to TIA/EIA-222-F for recommendations for maintenance and inspection. The frequency of the inspection and maintenance intervals is to be determined by the owner based upon actual site and environmental conditions. It is recommended that a complete and thorough inspection of the entire tower structural system be performed at least yearly and more frequently as conditions warrant. According to TIA/EIA-222-F section 14.1, Note 1: It is recommended that the structure be inspected after severe wind and/or ice storms or other extreme loading conditions.

## 6. DRAWINGS AND DATA

**SK-1 EXISTING TOWER WITH REINFORCEMENT**



# REINFORCED TOWER



## LEGEND

(A)	ROHN 2 STD GRADE 50 TYP. AT ALL THREE FACES
(B)	ROHN 2.5 STD GRADE 50 TYP. AT ALL THREE FACES
(C)	ROHN 2.5 EH GRADE 50 TYP. AT ALL THREE FACES

**1** EXISTING TOWER & PROPOSED REINFORCEMENT  
 SK-1 SCALE: N.T.S.

I.D. No.:  
 Designed by:  
 Drawn by: JEK  
 Checked by:

**URS**  
 URS CORPORATION AES  
 500 ENTERPRISE DRIVE  
 ROCKY HILL, CT. 06067  
 (860)-529-8882

CINGULAR WIRELESS

SITE ADDRESS: **CSP**  
 112 MUNN ROAD  
 COLCHESTER, CONNECTICUT

REV.	DATE:	DESCRIPTION

Scale: AS SHOWN Date: 3/14/06

Job No. 36921843 File No. SK-1

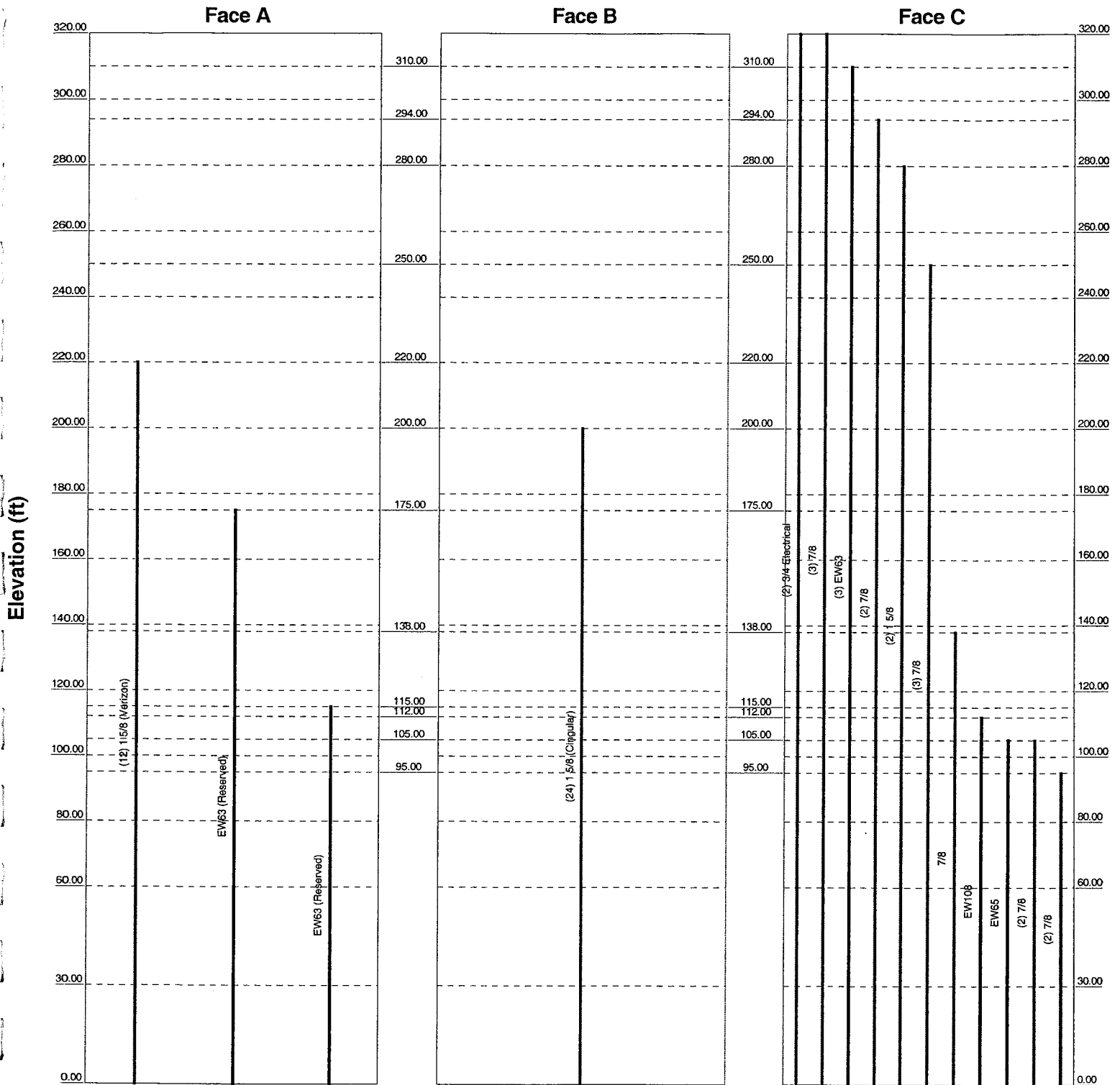
Dwg. No.  
**SK-1**  
 Dwg. 1 of 1

# ERI TOWER FEEDLINE DISTRIBUTION CHART

**Feedline Distribution Chart**

**0' - 320'**

Round      Flat      App In Face      App Out Face      Truss Leg

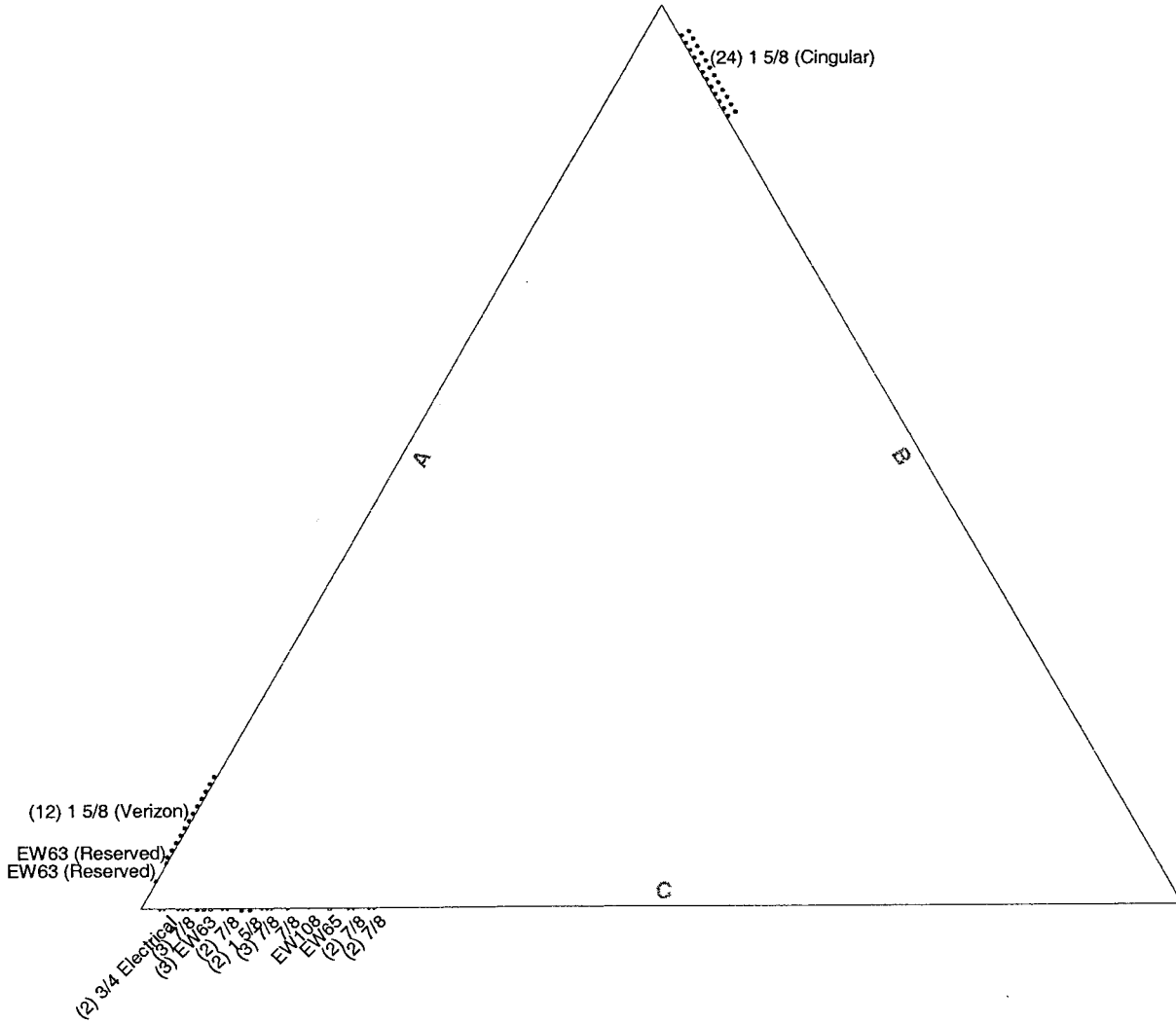


<p align="center"><b>URS Corporation</b>                  500 Enterprise Drive, Suite 3B                  Rocky Hill, CT 06067                  Phone: (860) 529-8882                  FAX: (860) 529-3991</p>	Job: <b>320' Rohn SSMW</b>		
	Project: <b>CSP Tower - Colchester, CT</b>		
	Client: Cingular Wireless	Drawn by: Jed Kiernan	App'd:
	Code: TIA/EIA-222-F	Date: 03/14/06	Scale: NTS
	Path: P:\08\ERI Files\Reinforced 320' Rohn SSMW.eri		Dwg No. E-7

# ERI TOWER FEEDLINE PLAN

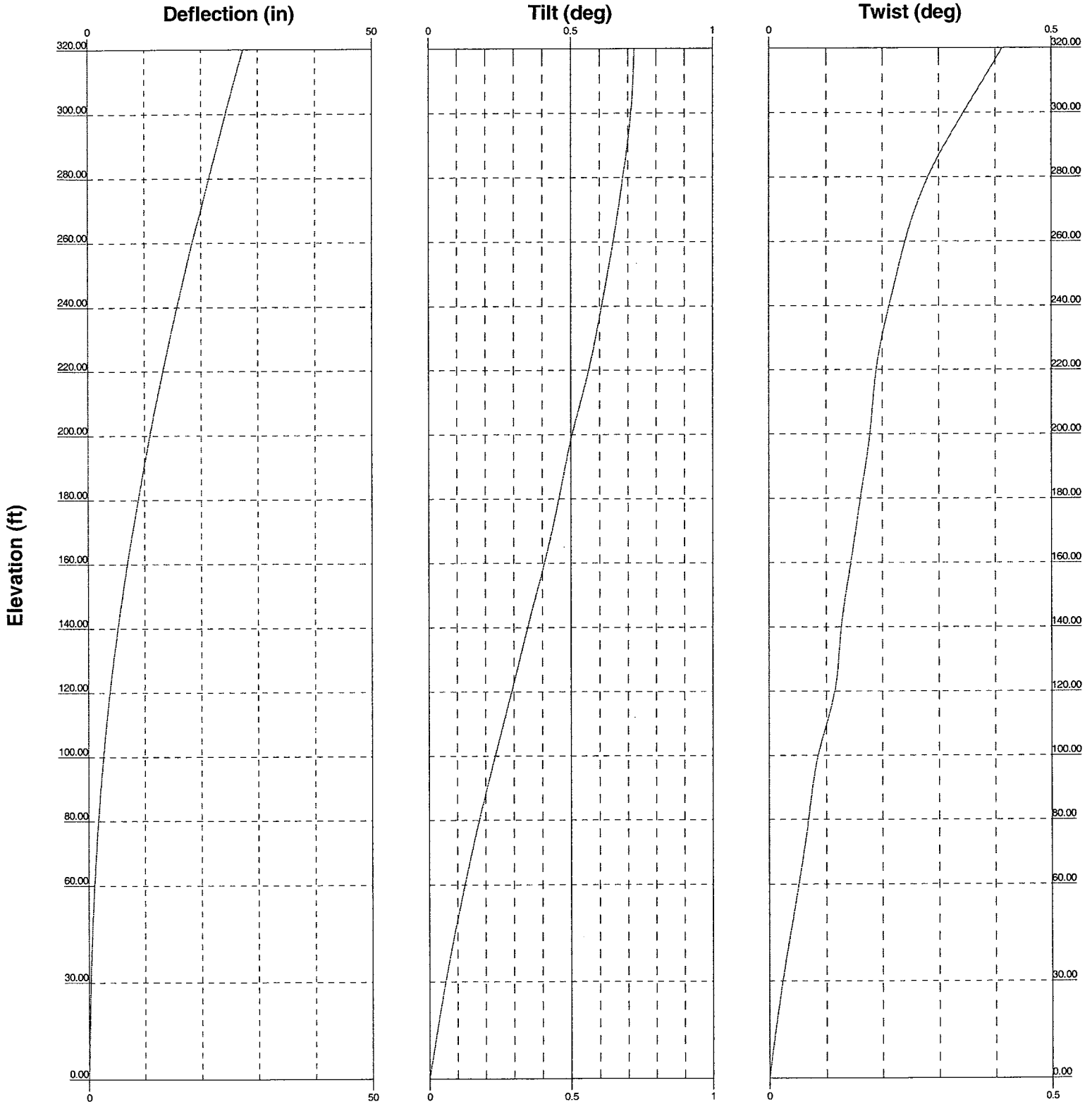
# Reedline Plan

\_\_\_\_\_ Round \_\_\_\_\_ Flat \_\_\_\_\_ App In Face \_\_\_\_\_ App Out Face



<b>URS Corporation</b>		<b>Job: 320' Rohn SSMW</b>	
500 Enterprise Drive, Suite 3B		Project: <b>CSP Tower - Colchester, CT</b>	
Rocky Hill, CT 06067		Client: Cingular Wireless	Drawn by: Jed Kiernan
Phone: (860) 529-8882		Code: TIA/EIA-222-F	Date: 03/14/06
FAX: (860) 529-3991		Path: P:\08\ERI Files\Reinforced 320' Rohn SSMW.eri	Scale: NTS
			Dwg No. E-7

**ERI TOWER DEFLECTION, TILT, TWIST**



<b>URS Corporation</b>		<b>Job: 320' Rohn SSMW</b>	
500 Enterprise Drive, Suite 3B		<b>Project: CSP Tower - Colchester, CT</b>	
Rocky Hill, CT 06067		Client: Cingular Wireless	Drawn by: Jed Kiernan
Phone: (860) 529-8882		Code: TIA/EIA-222-F	Date: 03/14/06
FAX: (860) 529-3991		Path: P:\OB\ERI Files\Reinforced 320' Rohn SSMW.eri	App'd: _____
			Scale: NTS
			Dwg No. E-5

## **EXISTING TOWER**



## **ERI TOWER INPUT/OUTPUT SUMMARY**

**APPURTENANCES**

TYPE	ELEVATION	TYPE	ELEVATION
Dual Lights	320	Mounting Frame (Verizon)	220
PD128	320	DB948F8T2E-M (Verizon)	220
6 Side Mount Standoff	320	DB948F8T2E-M (Verizon)	220
PD128	318	DB844 (Verizon)	220
6 Side Mount Standoff	318	DB844 (Verizon)	220
6"x4" Pipe Mount	315	PIROD 12' Lightweight T-Frame (Cingular)	200
8 FT DISH	315	PIROD 12' Lightweight T-Frame (Cingular)	200
5"3/4" Pipe Mount	308	PIROD 12' Lightweight T-Frame (Cingular)	200
5"3/4" Pipe Mount	308	(4) 7770.00 (Cingular)	200
5"3/4" Pipe Mount	308	(4) 7770.00 (Cingular)	200
6 FT DISH	308	(4) 7770.00 (Cingular)	200
6 FT DISH	308	(4) LP321401 TMA (Cingular)	200
6 FT DISH	294	(4) LP321401 TMA (Cingular)	200
DB224	294	(4) LP321401 TMA (Cingular)	200
6 Side Mount Standoff	294	(4) LP321401 TMA (Cingular)	200
PD320	292	(4) LP321401 TMA (Cingular)	200
6 Side Mount Standoff	292	(4) LP321401 TMA (Cingular)	200
DB809	285	(4) LP321401 TMA (Cingular)	200
6 Side Mount Standoff	285	5"3/4" Pipe Mount	175
OG19	275	6 FT DISH (Reserved)	175
6 Side Mount Standoff	275	BA1012-0	140
PD440	257	PD888S-4	140
6 Side Mount Standoff	257	6 Side Mount Standoff	140
PD128	250	PD156S	138
6 Side Mount Standoff	250	3/4"x4" Pipe Mount	115
PD320	243	5"3/4" Pipe Mount	115
6 Side Mount Standoff	243	6 FT DISH (Reserved)	112
DB844 (Verizon)	220	3/4"x4" Pipe Mount	112
DB844 (Verizon)	220	2 FT DISH	105
DB844 (Verizon)	220	5"3/4" Pipe Mount	105
DB948F8T2E-M (Verizon)	220	6 FT DISH	100
DB948F8T2E-M (Verizon)	220	6 Side Mount Standoff	100
DB844 (Verizon)	220	DB437	100
DB844 (Verizon)	220	PD458	97
DB948F8T2E-M (Verizon)	220	6 FT DISH	97
DB948F8T2E-M (Verizon)	220	5"3/4" Pipe Mount	90
Mounting Frame (Verizon)	220	4 FT DISH	90
Mounting Frame (Verizon)	220	3/4"x4" Pipe Mount	90

**SYMBOL LIST**

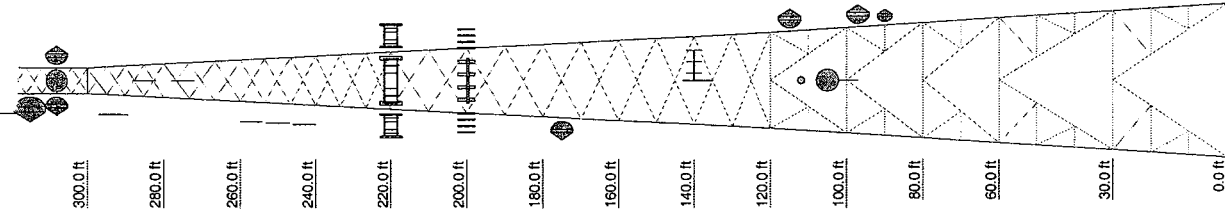
MARK	SIZE	MARK	SIZE
A	ROHN 5 EH	E	ROHN 3 EH
B	ROHN 6 EH	F	ROHN 2 STD
C	L1 3/4x1 3/4x3/16	G	ROHN 1.5 STD
D	L2 1/2x2 1/2x1/4		

**MATERIAL STRENGTH**

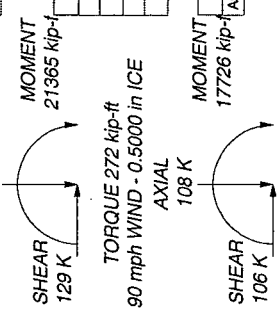
GRADE	Fu	Fy	GRADE	Fu	Fy
A572-50	50 ksi	50 ksi	A36	58 ksi	36 ksi

**TOWER DESIGN NOTES**

1. Tower designed for a 90 mph basic wind in accordance with the TIA/EIA-222-F Standard.
2. Tower is also designed for a 90 mph basic wind with 0.50 in ice.
3. Deflections are based upon a 90 mph wind.
4. TOWER RATING: 188.8%



MAX PIER FORCES:  
 DOWN: 656 K  
 UPLIFT: -524 K  
 SHEAR: 79 K



Section	Legs	Leg Grade	Diagonals	Diagonal Grade	Top Girts	Horizontal	Vertical	Inner Bracing	Face Width (ft)	# Panels @ (ft)	Weight (K)
T1	A	A572-50	ROHN 3 EH	A572-50	A36	N.A.	N.A.	ROHN 3 STD	40.69	2 @ 30	90.4
T2	B	ROHN 12 EH	ROHN 3 EH	ROHN 3.5 EH	ROHN 3.5 EH	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 2 STD	36.8	2 @ 30	129
T3									33.14		11.1
T4									30.47		7.3
T5									27.97		6.2
T6									25.39		6.0
T7									23.21		7.2
T8									21.3		6.9
T9									19.22		5.8
T10									17.08		5.7
T11									15.09		4.8
T12									13.08		4.8
T13									11.04		3.8
T14									8.85		3.5
T15									6.81		2.5

**URS Corporation**  
 500 Enterprise Drive, Suite 3B  
 Rocky Hill, CT 06067  
 Phone: (860) 529-8882  
 FAX: (860) 529-3991

Job: **320' Rohm SSVMW**  
 Project: **CSP Tower - Colchester, CT**  
 Client: **Cingular Wireless**  
 Code: **TIA/EIA-222-F**  
 Date: **03/14/06**  
 Drawn by: **Jed Kiernan**  
 App'd: \_\_\_\_\_  
 Scale: **NTS**  
 Path: **P:\08\ERI Files\320' Rohm SSVMW.er**  
 Dwg No: **E-**

## ERI TOWER DETAILED OUTPUT

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 1 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 15:34:40 03/10/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

## Tower Input Data

The main tower is a 3x free standing tower with an overall height of 320.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 6.81 ft at the top and 40.69 ft at the base.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Basic wind speed of 90 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 90 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 90 mph.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

## Options

- |  |  |  |
|--|--|--|
| <ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>Include Bolts In Member Capacity</li> <li>Leg Bolts Are At Top Of Section</li> <li>Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>Add IBC .6D+W Combination</li> </ul> | <ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>√ Use Clear Spans For Wind Area</li> <li>√ Use Clear Spans For KL/r</li> <li>Retension Guys To Initial Tension</li> <li>Bypass Mast Stability Checks</li> <li>Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>Autocalc Torque Arm Areas</li> <li>SR Members Have Cut Ends</li> <li>√ Sort Capacity Reports By Component</li> <li>Triangulate Diamond Inner Bracing</li> </ul> | <ul style="list-style-type: none"> <li>Treat Feedline Bundles As Cylinder</li> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>√ Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>SR Leg Bolts Resist Compression</li> <li>√ All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>√ Consider Feedline Torque</li> <li>Include Angle Block Shear Check</li> <li style="background-color: #cccccc;">Poles</li> <li>Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> </ul> |
|--|--|--|



<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 3 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 15:34:40 03/10/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Tower Section	Tower Elevation ft	Diagonal Spacing ft	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset in	Bottom Girt Offset in
T1	320.00-300.00	4.00	X Brace	No	No	0.0000	0.0000
T2	300.00-280.00	5.00	X Brace	No	No	0.0000	0.0000
T3	280.00-260.00	6.67	X Brace	No	No	0.0000	0.0000
T4	260.00-240.00	6.67	X Brace	No	No	0.0000	0.0000
T5	240.00-220.00	6.67	X Brace	No	No	0.0000	0.0000
T6	220.00-200.00	10.00	X Brace	No	No	0.0000	0.0000
T7	200.00-180.00	10.00	X Brace	No	No	0.0000	0.0000
T8	180.00-160.00	10.00	X Brace	No	No	0.0000	0.0000
T9	160.00-140.00	10.00	X Brace	No	No	0.0000	0.0000
T10	140.00-120.00	10.00	X Brace	No	No	0.0000	0.0000
T11	120.00-100.00	20.00	K1 Down	No	Yes	0.0000	0.0000
T12	100.00-80.00	20.00	K1 Down	No	Yes	0.0000	0.0000
T13	80.00-60.00	20.00	K1 Down	No	Yes	0.0000	0.0000
T14	60.00-30.00	30.00	K2 Down	No	Yes	0.0000	0.0000
T15	30.00-0.00	30.00	K2 Down	No	Yes	0.0000	0.0000

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 320.00-300.00	Pipe	ROHN 5 EH	A572-50 (50 ksi)	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T2 300.00-280.00	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Equal Angle	L2x2x1/4	A36 (36 ksi)
T3 280.00-260.00	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T4 260.00-240.00	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Equal Angle	L3x3x1/4	A572-50 (50 ksi)
T5 240.00-220.00	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Equal Angle	L4x4x5/16	A572-50 (50 ksi)
T6 220.00-200.00	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Equal Angle	L4x4x3/8	A572-50 (50 ksi)
T7 200.00-180.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Equal Angle	L4x4x3/8	A572-50 (50 ksi)
T8 180.00-160.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Equal Angle	L4x4x3/8	A572-50 (50 ksi)
T9 160.00-140.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Equal Angle	L5x5x3/8	A572-50 (50 ksi)
T10 140.00-120.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Equal Angle	L5x5x3/8	A572-50 (50 ksi)
T11 120.00-100.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Pipe	ROHN 3 EH	A572-50 (50 ksi)
T12 100.00-80.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Pipe	ROHN 3 EH	A572-50 (50 ksi)
T13 80.00-60.00	Pipe	ROHN 12 EH	A572-50 (50 ksi)	Pipe	ROHN 3 EH	A572-50 (50 ksi)
T14 60.00-30.00	Pipe	ROHN 12 EH	A572-50 (50 ksi)	Pipe	ROHN 3.5 EH	A572-50 (50 ksi)
T15 30.00-0.00	Pipe	ROHN 12 EHS	A572-50 (50 ksi)	Pipe	ROHN 3.5 EH	A572-50 (50 ksi)

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 4 of 51
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	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

**Tower Section Geometry (cont'd)**

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 320.00-300.00	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T2 300.00-280.00	Equal Angle	L2x2x1/4	A36 (36 ksi)	Solid Round		A36 (36 ksi)

**Tower Section Geometry (cont'd)**

Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T11 120.00-100.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T12 100.00-80.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T13 80.00-60.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 3 EH	A572-50 (50 ksi)
T14 60.00-30.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 3.5 EH	A572-50 (50 ksi)
T15 30.00-0.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 4 STD	A572-50 (50 ksi)

**Tower Section Geometry (cont'd)**

Tower Elevation ft	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
T11 120.00-100.00	Pipe		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T12 100.00-80.00	Pipe		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T13 80.00-60.00	Pipe		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T14 60.00-30.00	Pipe		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T15 30.00-0.00	Pipe		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)

**Tower Section Geometry (cont'd)**

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Tower Elevation	Redundant Bracing Grade	Redundant Type	Redundant Size	K Factor	
<i>ft</i>					
T11 120.00-100.00	A572-50 (50 ksi)	Horizontal (1) Diagonal (1)	Pipe Pipe	ROHN 1.5 STD ROHN 1.5 STD	1 1
T12 100.00-80.00	A572-50 (50 ksi)	Horizontal (1) Diagonal (1)	Pipe Pipe	ROHN 1.5 STD ROHN 2 STD	1 1
T13 80.00-60.00	A572-50 (50 ksi)	Horizontal (1) Diagonal (1)	Pipe Pipe	ROHN 2 STD ROHN 2 STD	1 1
T14 60.00-30.00	A572-50 (50 ksi)	Horizontal (1) Horizontal (2) Diagonal (1) Diagonal (2)	Pipe  Pipe	ROHN 1.5 STD ROHN 2 EH ROHN 1.5 STD ROHN 2 STD	1  1 1
T15 30.00-0.00	A572-50 (50 ksi)	Horizontal (1) Horizontal (2) Diagonal (1) Diagonal (2)	Pipe  Pipe	ROHN 1.5 STD ROHN 2 EH ROHN 2 STD ROHN 2.5 STD	1  1 1

### Tower Section Geometry (cont'd)

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor $A_f$	Adjust. Factor $A_r$	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
<i>ft</i>	<i>ft<sup>2</sup></i>	<i>in</i>					<i>in</i>	<i>in</i>
T1 320.00-300.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T2 300.00-280.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T3 280.00-260.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T4 260.00-240.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T5 240.00-220.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T6 220.00-200.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T7 200.00-180.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T8 180.00-160.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T9 160.00-140.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T10 140.00-120.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T11 120.00-100.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T12 100.00-80.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T13 80.00-60.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T14 60.00-30.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T15 30.00-0.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000

### Tower Section Geometry (cont'd)





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Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T6 220.00-200.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 200.00-180.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 180.00-160.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T9 160.00-140.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T10 140.00-120.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T11 120.00-100.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T12 100.00-80.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T13 80.00-60.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T14 60.00-30.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T15 30.00-0.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 320.00-300.00	Flange	1.0000	6	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T2 300.00-280.00	Flange	1.0000	8	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T3 280.00-260.00	Flange	1.0000	8	0.7500	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T4 260.00-240.00	Flange	1.0000	8	0.7500	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T5 240.00-220.00	Flange	1.0000	8	0.8750	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T6 220.00-200.00	Flange	1.0000	12	0.7500	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T7 200.00-180.00	Flange	1.0000	12	0.8750	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T8 180.00-160.00	Flange	1.0000	12	0.8750	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T9 160.00-140.00	Flange	1.0000	12	0.8750	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T10 140.00-120.00	Flange	1.0000	12	0.8750	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
T11 120.00-100.00	Flange	1.0000	12	0.7500	0	0.6250	0	0.6250	0	0.6250	0	0.7500	2	0.6250	0
T12 100.00-80.00	Flange	1.0000	16	0.7500	0	0.6250	0	0.6250	0	0.6250	0	0.7500	2	0.6250	0

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Tower Elevation ft	Leg Connection Type	Leg Bolt Size in	Leg No.	Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
				Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T13 80.00-60.00	Flange	1.0000	16	0.7500	0	0.6250	0	0.6250	0	0.6250	0	0.7500	2	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T14 60.00-30.00	Flange	1.0000	16	0.8750	0	0.6250	0	0.6250	0	0.6250	0	0.8750	2	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T15 30.00-0.00	Flange	1.0000	24	0.8750	0	0.6250	0	0.6250	0	0.6250	0	0.8750	2	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	

**Feed Line/Linear Appurtenances - Entered As Round Or Flat**

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
1 5/8 (Verizon)	A	Yes	Ar (CfAe)	220.00 - 0.00	0.0000	-0.4	12	12	1.9800	1.9800		1.04
3/4 Electrical	C	Yes	Ar (CfAe)	320.00 - 0.00	0.0000	0.48	2	2	1.1100	1.1100		0.54
7/8	C	Yes	Ar (CfAe)	320.00 - 0.00	0.0000	0.46	3	3	1.1100	1.1100		0.54
EW63	C	Yes	Af (CfAe)	310.00 - 0.00	0.0000	0.44	3	3	1.5742	1.5742	5.0668	0.51
7/8	C	Yes	Ar (CfAe)	294.00 - 0.00	0.0000	0.42	2	2	1.1100	1.1100		0.54
1 5/8	C	Yes	Ar (CfAe)	280.00 - 0.00	0.0000	0.4	2	2	1.9800	1.9800		1.04
7/8	C	Yes	Ar (CfAe)	250.00 - 0.00	0.0000	0.38	3	3	1.1100	1.1100		0.54
7/8	C	Yes	Ar (CfAe)	138.00 - 0.00	0.0000	0.36	1	1	1.1100	1.1100		0.54
EW108	C	Yes	Af (CfAe)	112.00 - 0.00	0.0000	0.34	1	1	0.5899	0.5899	2.0063	0.15
EW65	C	Yes	Af (CfAe)	105.00 - 0.00	0.0000	0.32	1	1	1.5742	1.5742	5.0668	0.51
7/8	C	Yes	Ar (CfAe)	105.00 - 0.00	0.0000	0.3	2	2	1.1100	1.1100		0.54
7/8	C	Yes	Ar (CfAe)	95.00 - 0.00	0.0000	0.28	2	2	1.1100	1.1100		0.54
EW63 (Reserved)	A	Yes	Af (CfAe)	175.00 - 0.00	0.0000	-0.45	1	1	1.5742	1.5742	5.0668	0.51
EW63 (Reserved)	A	Yes	Af (CfAe)	115.00 - 0.00	0.0000	-0.47	1	1	1.5742	1.5742	5.0668	0.51
1 5/8 (Cingular)	B	Yes	Ar (CfAe)	200.00 - 0.00	0.0000	-0.42	24	12	1.9800	1.9800		1.04

**Feed Line/Linear Appurtenances Section Areas**

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
T1	320.00-300.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	9.250	3.936	0.000	0.000	0.07
T2	300.00-280.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	11.840	7.871	0.000	0.000	0.10
T3	280.00-260.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	19.550	7.871	0.000	0.000	0.15
T4	260.00-240.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	22.325	7.871	0.000	0.000	0.16
T5	240.00-220.00	A	0.000	0.000	0.000	0.000	0.00

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Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
		B	0.000	0.000	0.000	0.000	0.00
		C	25.100	7.871	0.000	0.000	0.18
T6	220.00-200.00	A	39.600	0.000	0.000	0.000	0.25
		B	0.000	0.000	0.000	0.000	0.00
		C	25.100	7.871	0.000	0.000	0.18
T7	200.00-180.00	A	39.600	0.000	0.000	0.000	0.25
		B	39.600	0.000	0.000	0.000	0.50
		C	25.100	7.871	0.000	0.000	0.18
T8	180.00-160.00	A	39.600	1.968	0.000	0.000	0.26
		B	39.600	0.000	0.000	0.000	0.50
		C	25.100	7.871	0.000	0.000	0.18
T9	160.00-140.00	A	39.600	2.624	0.000	0.000	0.26
		B	39.600	0.000	0.000	0.000	0.50
		C	25.100	7.871	0.000	0.000	0.18
T10	140.00-120.00	A	39.600	2.624	0.000	0.000	0.26
		B	39.600	0.000	0.000	0.000	0.50
		C	26.765	7.871	0.000	0.000	0.19
T11	120.00-100.00	A	39.600	4.591	0.000	0.000	0.27
		B	39.600	0.000	0.000	0.000	0.50
		C	27.875	9.117	0.000	0.000	0.20
T12	100.00-80.00	A	39.600	5.247	0.000	0.000	0.27
		B	39.600	0.000	0.000	0.000	0.50
		C	33.425	11.478	0.000	0.000	0.24
T13	80.00-60.00	A	39.600	5.247	0.000	0.000	0.27
		B	39.600	0.000	0.000	0.000	0.50
		C	34.350	11.478	0.000	0.000	0.25
T14	60.00-30.00	A	59.400	7.871	0.000	0.000	0.41
		B	59.400	0.000	0.000	0.000	0.75
		C	51.525	17.217	0.000	0.000	0.37
T15	30.00-0.00	A	59.400	7.871	0.000	0.000	0.41
		B	59.400	0.000	0.000	0.000	0.75
		C	51.525	17.217	0.000	0.000	0.37

**Feed Line/Linear Appurtenances Section Areas - With Ice**

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
T1	320.00-300.00	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		17.583	5.602	0.000	0.000	0.21
T2	300.00-280.00	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		22.507	11.204	0.000	0.000	0.31
T3	280.00-260.00	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		34.550	11.204	0.000	0.000	0.43
T4	260.00-240.00	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		39.825	11.204	0.000	0.000	0.47
T5	240.00-220.00	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		45.100	11.204	0.000	0.000	0.52
T6	220.00-200.00	A	0.500	59.600	0.000	0.000	0.000	0.61
		B		0.000	0.000	0.000	0.000	0.00
		C		45.100	11.204	0.000	0.000	0.52
T7	200.00-180.00	A	0.500	59.600	0.000	0.000	0.000	0.61
		B		59.600	0.000	0.000	0.000	1.23

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 10 of 51
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	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
T8	180.00-160.00	C	0.500	45.100	11.204	0.000	0.000	0.52
		A		59.600	2.801	0.000	0.000	0.64
		B		59.600	0.000	0.000	0.000	1.23
T9	160.00-140.00	C	0.500	45.100	11.204	0.000	0.000	0.52
		A		59.600	3.735	0.000	0.000	0.65
		B		59.600	0.000	0.000	0.000	1.23
T10	140.00-120.00	C	0.500	45.100	11.204	0.000	0.000	0.52
		A		59.600	3.735	0.000	0.000	0.65
		B		59.600	0.000	0.000	0.000	1.23
T11	120.00-100.00	C	0.500	48.265	11.204	0.000	0.000	0.55
		A		59.600	6.536	0.000	0.000	0.68
		B		59.600	0.000	0.000	0.000	1.23
T12	100.00-80.00	C	0.500	50.375	13.395	0.000	0.000	0.58
		A		59.600	7.470	0.000	0.000	0.69
		B		59.600	0.000	0.000	0.000	1.23
T13	80.00-60.00	C	0.500	60.925	17.033	0.000	0.000	0.71
		A		59.600	7.470	0.000	0.000	0.69
		B		59.600	0.000	0.000	0.000	1.23
T14	60.00-30.00	C	0.500	62.683	17.033	0.000	0.000	0.73
		A		89.400	11.204	0.000	0.000	1.03
		B		89.400	0.000	0.000	0.000	1.84
T15	30.00-0.00	C	0.500	94.025	25.550	0.000	0.000	1.09
		A		89.400	11.204	0.000	0.000	1.03
		B		89.400	0.000	0.000	0.000	1.84
		C		94.025	25.550	0.000	0.000	1.09

### Feed Line Shielding

Section	Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>R</sub> Ice ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	A <sub>F</sub> Ice ft <sup>2</sup>
T1	320.00-300.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	1.211	3.047
T2	300.00-280.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	1.727	4.132
T3	280.00-260.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	2.066	4.526
T4	260.00-240.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	2.589	5.523
T5	240.00-220.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	3.649	7.485
T6	220.00-200.00	A	0.000	0.000	3.109	5.460
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	2.589	5.310
T7	200.00-180.00	A	0.000	0.000	3.015	5.294
		B	0.000	0.000	3.015	5.294
		C	0.000	0.000	2.510	5.149
T8	180.00-160.00	A	0.000	0.000	3.091	5.449
		B	0.000	0.000	2.945	5.170
		C	0.000	0.000	2.452	5.029
T9	160.00-140.00	A	0.000	0.000	3.858	6.616
		B	0.000	0.000	3.618	6.172

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Section	Elevation ft	Face	$A_R$	$A_R$	$A_F$	$A_F$
			$ft^2$	Ice $ft^2$	$ft^2$	Ice $ft^2$
T10	140.00-120.00	C	0.000	0.000	3.013	6.003
		A	0.000	0.000	3.805	6.526
		B	0.000	0.000	3.569	6.087
		C	0.000	0.000	3.121	6.244
T11	120.00-100.00	A	2.823	5.897	0.000	0.000
		B	2.530	5.237	0.000	0.000
		C	2.363	5.792	0.000	0.000
T12	100.00-80.00	A	2.913	5.993	0.000	0.000
		B	2.572	5.239	0.000	0.000
		C	2.917	7.097	0.000	0.000
T13	80.00-60.00	A	2.899	5.918	0.000	0.000
		B	2.560	5.173	0.000	0.000
		C	2.963	7.160	0.000	0.000
T14	60.00-30.00	A	4.393	9.110	0.000	0.000
		B	3.879	7.963	0.000	0.000
		C	4.489	11.022	0.000	0.000
T15	30.00-0.00	A	4.601	9.317	0.000	0.000
		B	4.063	8.144	0.000	0.000
		C	4.701	11.273	0.000	0.000

### Feed Line Center of Pressure

Section	Elevation ft	$CP_x$	$CP_z$	$CP_x$	$CP_z$
		in	in	Ice in	Ice in
T1	320.00-300.00	-4.3846	2.8159	-5.6631	3.6242
T2	300.00-280.00	-6.2629	4.0917	-8.0736	5.2614
T3	280.00-260.00	-8.6123	5.7929	-11.3313	7.5878
T4	260.00-240.00	-10.0462	6.8193	-13.4219	9.0887
T5	240.00-220.00	-10.3624	7.0868	-14.1930	9.6963
T6	220.00-200.00	-25.3004	13.8166	-31.3392	17.3390
T7	200.00-180.00	-20.8086	-1.9859	-25.7687	-1.2958
T8	180.00-160.00	-23.2764	-1.7646	-28.7614	-0.9837
T9	160.00-140.00	-23.1016	-1.6036	-29.0278	-0.8473
T10	140.00-120.00	-24.9157	-1.3025	-31.4341	-0.3099
T11	120.00-100.00	-33.7534	-0.4304	-39.7629	1.1311
T12	100.00-80.00	-37.9500	2.1221	-44.7665	4.4919
T13	80.00-60.00	-38.3178	2.4115	-45.9571	4.9716
T14	60.00-30.00	-41.6737	2.6357	-49.9910	5.4185
T15	30.00-0.00	-44.3764	2.8203	-53.5102	5.8110

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	$C_{AA}$	$C_{AA}$	Weight	
			Horz Lateral	Vert			Front	Side		
			ft	ft	°	ft	$ft^2$	$ft^2$	K	
Dual Lights	C	None			0.0000	320.00	No Ice	4.00	4.00	0.25
							1/2" Ice	4.80	4.80	0.40

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	<b>Client</b>		Cingular Wireless		<b>Designed by</b>		Jed Kiernan	

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>AA</sub>		Weight K	
						Front ft <sup>2</sup>	Side ft <sup>2</sup>		
PD128	C	From Leg	6.00 0.00 0.00	0.0000	320.00	No Ice 1/2" Ice	1.00 1.80	1.00 1.80	0.01 0.02
6' Side Mount Standoff	C	None		0.0000	320.00	No Ice 1/2" Ice	6.50 8.50	6.50 8.50	0.10 0.17
PD128	C	From Leg	6.00 0.00 0.00	0.0000	318.00	No Ice 1/2" Ice	1.00 1.80	1.00 1.80	0.01 0.02
6' Side Mount Standoff	C	None		0.0000	318.00	No Ice 1/2" Ice	6.50 8.50	6.50 8.50	0.10 0.17
6'8"x4" Pipe Mount	C	From Leg	0.50 0.00 0.00	0.0000	315.00	No Ice 1/2" Ice	2.60 3.01	2.60 3.01	0.07 0.09
5'3"x4" Pipe Mount	A	From Leg	0.50 0.00 0.00	0.0000	308.00	No Ice 1/2" Ice	1.88 2.21	1.88 2.21	0.06 0.07
5'3"x4" Pipe Mount	B	From Leg	0.50 0.00 0.00	0.0000	308.00	No Ice 1/2" Ice	1.88 2.21	1.88 2.21	0.06 0.07
5'3"x4" Pipe Mount	C	From Leg	0.50 0.00 0.00	0.0000	308.00	No Ice 1/2" Ice	1.88 2.21	1.88 2.21	0.06 0.07
DB224	C	From Leg	6.00 0.00 0.00	0.0000	294.00	No Ice 1/2" Ice	3.15 5.67	3.15 5.67	0.03 0.04
6' Side Mount Standoff	C	None		0.0000	294.00	No Ice 1/2" Ice	6.50 8.50	6.50 8.50	0.10 0.17
PD320	C	From Leg	6.00 0.00 0.00	0.0000	292.00	No Ice 1/2" Ice	2.25 4.05	2.25 4.05	0.03 0.04
6' Side Mount Standoff	C	None		0.0000	292.00	No Ice 1/2" Ice	6.50 8.50	6.50 8.50	0.10 0.17
DB809	A	From Leg	6.00 0.00 0.00	0.0000	285.00	No Ice 1/2" Ice	3.39 4.55	3.39 4.55	0.03 0.06
6' Side Mount Standoff	A	None		0.0000	285.00	No Ice 1/2" Ice	6.50 8.50	6.50 8.50	0.10 0.17
OGT9	A	From Leg	6.00 0.00 0.00	0.0000	275.00	No Ice 1/2" Ice	3.15 5.67	3.15 5.67	0.03 0.04
6' Side Mount Standoff	A	None		0.0000	275.00	No Ice 1/2" Ice	6.50 8.50	6.50 8.50	0.10 0.17
PD440	C	From Leg	6.00 0.00 0.00	0.0000	257.00	No Ice 1/2" Ice	1.38 2.48	1.38 2.48	0.02 0.02
6' Side Mount Standoff	C	None		0.0000	257.00	No Ice 1/2" Ice	6.50 8.50	6.50 8.50	0.10 0.17
PD128	C	From Leg	6.00 0.00 0.00	0.0000	250.00	No Ice 1/2" Ice	1.00 1.80	1.00 1.80	0.01 0.02
6' Side Mount Standoff	C	None		0.0000	250.00	No Ice 1/2" Ice	6.50 8.50	6.50 8.50	0.10 0.17
PD320	C	From Leg	6.00 0.00 0.00	0.0000	243.00	No Ice 1/2" Ice	2.25 4.05	2.25 4.05	0.03 0.04
6' Side Mount Standoff	C	None		0.0000	243.00	No Ice 1/2" Ice	6.50 8.50	6.50 8.50	0.10 0.17

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Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz	Lateral	Vert					
DB844 (Verizon)	A	From Leg	5.00	0.0000	220.00	No Ice	3.06	3.73	0.01	
			6.00			1/2" Ice	3.39	4.10	0.04	
			0.00							
DB844 (Verizon)	A	From Leg	5.00	0.0000	220.00	No Ice	3.06	3.73	0.01	
			-6.00			1/2" Ice	3.39	4.10	0.04	
			0.00							
DB948F85T2E-M (Verizon)	A	From Leg	5.00	0.0000	220.00	No Ice	1.92	3.26	0.01	
			4.00			1/2" Ice	2.22	3.62	0.03	
			0.00							
DB948F85T2E-M (Verizon)	A	From Leg	5.00	0.0000	220.00	No Ice	1.92	3.26	0.01	
			-4.00			1/2" Ice	2.22	3.62	0.03	
			0.00							
DB844 (Verizon)	B	From Leg	5.00	0.0000	220.00	No Ice	3.06	3.73	0.01	
			6.00			1/2" Ice	3.39	4.10	0.04	
			0.00							
DB844 (Verizon)	B	From Leg	5.00	0.0000	220.00	No Ice	3.06	3.73	0.01	
			-6.00			1/2" Ice	3.39	4.10	0.04	
			0.00							
DB948F85T2E-M (Verizon)	B	From Leg	5.00	0.0000	220.00	No Ice	1.92	3.26	0.01	
			4.00			1/2" Ice	2.22	3.62	0.03	
			0.00							
DB948F85T2E-M (Verizon)	B	From Leg	5.00	0.0000	220.00	No Ice	1.92	3.26	0.01	
			-4.00			1/2" Ice	2.22	3.62	0.03	
			0.00							
DB844 (Verizon)	C	From Leg	5.00	0.0000	220.00	No Ice	3.06	3.73	0.01	
			6.00			1/2" Ice	3.39	4.10	0.04	
			0.00							
DB844 (Verizon)	C	From Leg	5.00	0.0000	220.00	No Ice	3.06	3.73	0.01	
			-6.00			1/2" Ice	3.39	4.10	0.04	
			0.00							
DB948F85T2E-M (Verizon)	C	From Leg	5.00	0.0000	220.00	No Ice	1.92	3.26	0.01	
			4.00			1/2" Ice	2.22	3.62	0.03	
			0.00							
DB948F85T2E-M (Verizon)	C	From Leg	5.00	0.0000	220.00	No Ice	1.92	3.26	0.01	
			-4.00			1/2" Ice	2.22	3.62	0.03	
			0.00							
Mounting Frame (Verizon)	A	From Leg	5.00	0.0000	220.00	No Ice	17.00	17.00	0.56	
			0.00			1/2" Ice	20.00	20.00	0.70	
			0.00							
Mounting Frame (Verizon)	B	From Leg	5.00	0.0000	220.00	No Ice	17.00	17.00	0.56	
			0.00			1/2" Ice	20.00	20.00	0.70	
			0.00							
Mounting Frame (Verizon)	C	From Leg	5.00	0.0000	220.00	No Ice	17.00	17.00	0.56	
			0.00			1/2" Ice	20.00	20.00	0.70	
			0.00							
PD156S	A	From Leg	1.00	0.0000	138.00	No Ice	0.44	0.44	0.01	
			0.00			1/2" Ice	0.79	0.79	0.01	
			0.00							
3'4"x4" Pipe Mount	A	From Leg	0.50	0.0000	138.00	No Ice	1.05	1.05	0.04	
			0.00			1/2" Ice	1.27	1.27	0.05	
			0.00							
3'4"x4" Pipe Mount	A	From Leg	0.50	0.0000	112.00	No Ice	1.05	1.05	0.04	
			0.00			1/2" Ice	1.27	1.27	0.05	
			0.00							
5'3"x4" Pipe Mount	A	From Leg	0.50	0.0000	105.00	No Ice	1.88	1.88	0.06	
			0.00			1/2" Ice	2.21	2.21	0.07	
			0.00							



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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Horz	Lateral					
PD458	A	From Leg	6.00	0.0000	100.00	No Ice	2.88	2.88	0.02
			0.00			1/2" Ice	4.34	4.34	0.05
			0.00						
DB437	A	From Leg	6.00	0.0000	100.00	No Ice	0.45	0.45	0.01
			0.00			1/2" Ice	0.81	0.81	0.01
			0.00						
6' Side Mount Standoff	A	None		0.0000	100.00	No Ice	6.50	6.50	0.10
						1/2" Ice	8.50	8.50	0.17
5'3"x4" Pipe Mount	B	From Leg	0.50	0.0000	97.00	No Ice	1.88	1.88	0.06
			0.00			1/2" Ice	2.21	2.21	0.07
			0.00						
3'4"x4" Pipe Mount	B	From Leg	0.50	0.0000	90.00	No Ice	1.05	1.05	0.04
			0.00			1/2" Ice	1.27	1.27	0.05
			0.00						
5'3"x4" Pipe Mount	C	From Leg	0.50	0.0000	175.00	No Ice	1.88	1.88	0.06
			0.00			1/2" Ice	2.21	2.21	0.07
			0.00						
5'3"x4" Pipe Mount	B	From Leg	0.50	0.0000	115.00	No Ice	1.88	1.88	0.06
			0.00			1/2" Ice	2.21	2.21	0.07
			0.00						
BA1012-0	A	From Leg	6.00	0.0000	140.00	No Ice	0.47	0.47	0.00
			0.00			1/2" Ice	0.96	0.96	0.01
			0.00						
PD688S-4	A	From Leg	6.00	0.0000	140.00	No Ice	0.35	0.35	0.00
			0.00			1/2" Ice	0.63	0.63	0.00
			0.00						
6' Side Mount Standoff	A	None		0.0000	140.00	No Ice	6.50	6.50	0.10
						1/2" Ice	8.50	8.50	0.17
PiROD 12' Lightweight T-Frame (Cingular)	A	None		0.0000	200.00	No Ice	10.20	10.20	0.25
						1/2" Ice	16.20	16.20	0.35
PiROD 12' Lightweight T-Frame (Cingular)	B	None		0.0000	200.00	No Ice	10.20	10.20	0.25
						1/2" Ice	16.20	16.20	0.35
PiROD 12' Lightweight T-Frame (Cingular)	C	None		0.0000	200.00	No Ice	10.20	10.20	0.25
						1/2" Ice	16.20	16.20	0.35
(4) 7770.00 (Cingular)	A	From Leg	3.00	0.0000	200.00	No Ice	5.88	2.93	0.04
			0.00			1/2" Ice	6.31	3.27	0.07
			0.00						
(4) 7770.00 (Cingular)	B	From Leg	3.00	0.0000	200.00	No Ice	5.88	2.93	0.04
			0.00			1/2" Ice	6.31	3.27	0.07
			0.00						
(4) 7770.00 (Cingular)	C	From Leg	3.00	0.0000	200.00	No Ice	5.88	2.93	0.04
			0.00			1/2" Ice	6.31	3.27	0.07
			0.00						
(4) LPG21401 TMA (Cingular)	A	From Leg	3.00	0.0000	200.00	No Ice	0.95	0.37	0.02
			0.00			1/2" Ice	1.09	0.48	0.02
			0.00						
(4) LPG21401 TMA (Cingular)	B	From Leg	3.00	0.0000	200.00	No Ice	0.95	0.37	0.02
			0.00			1/2" Ice	1.09	0.48	0.02
			0.00						
(4) LPG21401 TMA (Cingular)	C	From Leg	3.00	0.0000	200.00	No Ice	0.95	0.37	0.02
			0.00			1/2" Ice	1.09	0.48	0.02
			0.00						
(4) LPG13519 Diplexer (Cingular)	A	From Leg	3.00	0.0000	200.00	No Ice	0.27	0.18	0.01
			0.00			1/2" Ice	0.34	0.25	0.01

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b>	320' Rohn SSVMW	<b>Page</b>	15 of 51
	<b>Project</b>	CSP Tower - Colchester, CT	<b>Date</b>	15:34:40 03/10/06
	<b>Client</b>	Cingular Wireless	<b>Designed by</b>	Jed Kiernan

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz	Lateral						
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
(4) LPG13519 Diplexer (Cingular)	A	From Leg	0.00	3.00	0.0000	200.00	No Ice	0.27	0.18	0.01
			0.00	0.00			1/2" Ice	0.34	0.25	0.01
			0.00	0.00						
(4) LPG13519 Diplexer (Cingular)	A	From Leg	3.00	0.00	0.0000	200.00	No Ice	0.27	0.18	0.01
			0.00	0.00			1/2" Ice	0.34	0.25	0.01
			0.00	0.00						

### Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets:		Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weight	
				Horz	Lateral							
				ft	ft	°	°	ft	ft	ft <sup>2</sup>	K	
8 FT DISH	C	Paraboloid w/Radome	From Leg	1.00	0.00	Worst		315.00	8.00	No Ice	50.30	0.25
				0.00	0.00					1/2" Ice	51.29	0.51
				0.00	0.00							
6 FT DISH	A	Paraboloid w/Radome	From Leg	1.00	0.00	Worst		308.00	6.00	No Ice	28.27	0.14
				0.00	0.00					1/2" Ice	29.05	0.29
				0.00	0.00							
6 FT DISH	B	Paraboloid w/Radome	From Leg	1.00	0.00	Worst		308.00	6.00	No Ice	28.27	0.14
				0.00	0.00					1/2" Ice	29.05	0.29
				0.00	0.00							
6 FT DISH	C	Paraboloid w/Radome	From Leg	1.00	0.00	Worst		308.00	6.00	No Ice	28.27	0.14
				0.00	0.00					1/2" Ice	29.05	0.29
				0.00	0.00							
2 FT DISH	A	Paraboloid w/Radome	From Leg	1.00	0.00	Worst		112.00	2.00	No Ice	3.14	0.03
				0.00	0.00					1/2" Ice	3.41	0.04
				0.00	0.00							
6 FT DISH	A	Paraboloid w/Radome	From Leg	1.00	0.00	Worst		105.00	6.00	No Ice	28.27	0.14
				0.00	0.00					1/2" Ice	29.05	0.29
				0.00	0.00							
6 FT DISH	B	Paraboloid w/Radome	From Leg	1.00	0.00	Worst		97.00	6.00	No Ice	28.27	0.14
				0.00	0.00					1/2" Ice	29.05	0.29
				0.00	0.00							
4 FT DISH	B	Paraboloid w/Radome	From Leg	1.00	0.00	Worst		90.00	4.00	No Ice	12.56	0.17
				0.00	0.00					1/2" Ice	13.09	0.24
				0.00	0.00							
6 FT DISH (Reserved)	C	Paraboloid w/Radome	From Leg	1.00	0.00	Worst		175.00	6.00	No Ice	28.27	0.14
				0.00	0.00					1/2" Ice	29.05	0.29
				0.00	0.00							
6 FT DISH (Reserved)	B	Paraboloid w/Radome	From Leg	1.00	0.00	Worst		115.00	6.00	No Ice	28.27	0.14
				0.00	0.00					1/2" Ice	29.05	0.29
				0.00	0.00							

### Tower Pressures - No Ice

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 16 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 15:34:40 03/10/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

$$G_H = 1.084$$

Section Elevation ft	z ft	K <sub>Z</sub>	q <sub>z</sub> psf	A <sub>G</sub> ft <sup>2</sup>	F a c e	A <sub>F</sub> ft <sup>2</sup>	A <sub>R</sub> ft <sup>2</sup>	A <sub>leg</sub> ft <sup>2</sup>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>
T1 320.00-300.00	310.00	1.897	39	145.472	A	11.659	18.543	18.543	61.40	0.000	0.000
					B	11.659	18.543		61.40		
					C	14.384	27.793		43.97		
T2 300.00-280.00	290.00	1.861	39	167.656	A	12.596	22.122	22.122	63.72	0.000	0.000
					B	12.596	22.122		63.72		
					C	18.740	33.962		41.97		
T3 280.00-260.00	270.00	1.823	38	213.297	A	13.934	28.807	28.807	67.40	0.000	0.000
					B	13.934	28.807		67.40		
					C	19.739	48.357		42.30		
T4 260.00-240.00	250.00	1.783	37	255.594	A	19.443	28.800	28.800	59.70	0.000	0.000
					B	19.443	28.800		59.70		
					C	24.725	51.125		37.97		
T5 240.00-220.00	230.00	1.741	36	296.093	A	29.581	28.798	28.798	49.33	0.000	0.000
					B	29.581	28.798		49.33		
					C	33.803	53.898		32.84		
T6 220.00-200.00	210.00	1.697	35	336.193	A	21.026	68.398	28.798	32.20	0.000	0.000
					B	24.136	28.798		54.40		
					C	29.418	53.898		34.56		
T7 200.00-180.00	190.00	1.649	34	381.042	A	23.330	75.501	35.901	36.33	0.000	0.000
					B	23.330	75.501		36.33		
					C	31.706	61.001		38.73		
T8 180.00-160.00	170.00	1.597	33	423.141	A	27.674	75.498	35.898	34.79	0.000	0.000
					B	25.852	75.498		35.42		
					C	34.216	60.998		37.70		
T9 160.00-140.00	150.00	1.541	32	463.037	A	37.797	75.488	35.888	31.68	0.000	0.000
					B	35.413	75.488		32.36		
					C	43.890	60.988		34.22		
T10 140.00-120.00	130.00	1.48	31	503.943	A	41.004	75.504	35.904	30.82	0.000	0.000
					B	38.616	75.504		31.46		
					C	46.935	62.669		32.76		
T11 120.00-100.00	110.00	1.411	29	551.554	A	4.591	98.870	35.933	34.73	0.000	0.000
					B	0.000	99.164		36.24		
					C	9.117	87.605		37.15		
T12 100.00-80.00	90.00	1.332	28	602.352	A	5.247	101.189	35.927	33.75	0.000	0.000
					B	0.000	101.530		35.39		
					C	11.478	95.010		33.74		
T13 80.00-60.00	70.00	1.24	26	657.397	A	5.247	109.942	42.626	37.01	0.000	0.000
					B	0.000	110.281		38.65		
					C	11.478	104.629		36.71		
T14 60.00-30.00	45.00	1.093	23	1081.034	A	7.871	166.737	63.908	36.60	0.000	0.000
					B	0.000	167.251		38.21		
					C	17.217	158.766		36.31		
T15 30.00-0.00	15.00	1	21	1194.292	A	7.871	173.197	63.928	35.31	0.000	0.000
					B	0.000	173.735		36.80		
					C	17.217	165.221		35.04		

### Tower Pressure - With Ice

$$G_H = 1.084$$

Section Elevation ft	z ft	K <sub>Z</sub>	q <sub>z</sub> psf	t <sub>z</sub> in	A <sub>G</sub> ft <sup>2</sup>	F a c e	A <sub>F</sub> ft <sup>2</sup>	A <sub>R</sub> ft <sup>2</sup>	A <sub>leg</sub> ft <sup>2</sup>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>
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<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b>	320' Rohn SSMW	<b>Page</b>	17 of 51
	<b>Project</b>	CSP Tower - Colchester, CT	<b>Date</b>	15:34:40 03/10/06
	<b>Client</b>	Cingular Wireless	<b>Designed by</b>	Jed Kiernan

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	t <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face
ft	ft		psf	in	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
T1 320.00-300.00	310.00	1.897	39	0.5000	147.138	A	16.101	21.877	21.877	57.60	0.000	0.000
						B	16.101	21.877		57.60		
						C	18.656	39.460		37.64		
T2 300.00-280.00	290.00	1.861	39	0.5000	169.325	A	16.795	25.461	25.461	60.25	0.000	0.000
						B	16.795	25.461		60.25		
						C	23.867	47.967		35.44		
T3 280.00-260.00	270.00	1.823	38	0.5000	214.966	A	17.649	32.147	32.147	64.56	0.000	0.000
						B	17.649	32.147		64.56		
						C	24.328	66.697		35.32		
T4 260.00-240.00	250.00	1.783	37	0.5000	257.263	A	23.764	32.139	32.139	57.49	0.000	0.000
						B	23.764	32.139		57.49		
						C	29.445	71.964		31.69		
T5 240.00-220.00	230.00	1.741	36	0.5000	297.762	A	34.511	32.137	32.137	48.22	0.000	0.000
						B	34.511	32.137		48.22		
						C	38.230	77.237		27.83		
T6 220.00-200.00	210.00	1.697	35	0.5000	337.862	A	22.699	91.737	32.137	28.08	0.000	0.000
						B	28.158	32.137		53.30		
						C	34.052	77.237		28.88		
T7 200.00-180.00	190.00	1.649	34	0.5000	382.711	A	25.442	98.841	39.241	31.57	0.000	0.000
						B	25.442	98.841		31.57		
						C	36.791	84.341		32.39		
T8 180.00-160.00	170.00	1.597	33	0.5000	424.810	A	30.948	98.837	39.237	30.23	0.000	0.000
						B	28.426	98.837		30.83		
						C	39.772	84.337		31.62		
T9 160.00-140.00	150.00	1.541	32	0.5000	464.706	A	41.354	98.826	39.226	27.98	0.000	0.000
						B	38.064	98.826		28.66		
						C	49.437	84.326		29.33		
T10 140.00-120.00	130.00	1.48	31	0.5000	505.612	A	45.019	98.844	39.244	27.28	0.000	0.000
						B	41.722	98.844		27.92		
						C	52.770	87.509		27.98		
T11 120.00-100.00	110.00	1.411	29	0.5000	553.224	A	6.536	127.872	39.275	29.22	0.000	0.000
						B	0.000	128.531		30.56		
						C	13.395	118.752		29.72		
T12 100.00-80.00	90.00	1.332	28	0.5000	604.022	A	7.470	130.704	39.269	28.42	0.000	0.000
						B	0.000	131.458		29.87		
						C	17.033	130.925		26.54		
T13 80.00-60.00	70.00	1.24	26	0.5000	659.068	A	7.470	140.004	45.969	31.17	0.000	0.000
						B	0.000	140.749		32.66		
						C	17.033	141.845		28.93		
T14 60.00-30.00	45.00	1.093	23	0.5000	1083.539	A	11.204	211.934	68.920	30.89	0.000	0.000
						B	0.000	213.080		32.34		
						C	25.550	214.646		28.69		
T15 30.00-0.00	15.00	1	21	0.5000	1196.797	A	11.204	219.406	68.942	29.90	0.000	0.000
						B	0.000	220.579		31.26		
						C	25.550	222.075		27.84		

**Tower Pressure - Service**

$G_H = 1.084$

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face
ft	ft		psf	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
T1 320.00-	310.00	1.897	39	145.472	A	11.659	18.543	18.543	61.40	0.000	0.000

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 18 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 15:34:40 03/10/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face
ft	ft		psf	ft <sup>2</sup>	e	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
300.00					B	11.659	18.543		61.40		
					C	14.384	27.793		43.97		
T2 300.00-280.00	290.00	1.861	39	167.656	A	12.596	22.122	22.122	63.72	0.000	0.000
					B	12.596	22.122		63.72		
					C	18.740	33.962		41.97		
T3 280.00-260.00	270.00	1.823	38	213.297	A	13.934	28.807	28.807	67.40	0.000	0.000
					B	13.934	28.807		67.40		
					C	19.739	48.357		42.30		
T4 260.00-240.00	250.00	1.783	37	255.594	A	19.443	28.800	28.800	59.70	0.000	0.000
					B	19.443	28.800		59.70		
					C	24.725	51.125		37.97		
T5 240.00-220.00	230.00	1.741	36	296.093	A	29.581	28.798	28.798	49.33	0.000	0.000
					B	29.581	28.798		49.33		
					C	33.803	53.898		32.84		
T6 220.00-200.00	210.00	1.697	35	336.193	A	21.026	68.398	28.798	32.20	0.000	0.000
					B	24.136	28.798		54.40		
					C	29.418	53.898		34.56		
T7 200.00-180.00	190.00	1.649	34	381.042	A	23.330	75.501	35.901	36.33	0.000	0.000
					B	23.330	75.501		36.33		
					C	31.706	61.001		38.73		
T8 180.00-160.00	170.00	1.597	33	423.141	A	27.674	75.498	35.898	34.79	0.000	0.000
					B	25.852	75.498		35.42		
					C	34.216	60.998		37.70		
T9 160.00-140.00	150.00	1.541	32	463.037	A	37.797	75.488	35.888	31.68	0.000	0.000
					B	35.413	75.488		32.36		
					C	43.890	60.988		34.22		
T10 140.00-120.00	130.00	1.48	31	503.943	A	41.004	75.504	35.904	30.82	0.000	0.000
					B	38.616	75.504		31.46		
					C	46.935	62.669		32.76		
T11 120.00-100.00	110.00	1.411	29	551.554	A	4.591	98.870	35.933	34.73	0.000	0.000
					B	0.000	99.164		36.24		
					C	9.117	87.605		37.15		
T12 100.00-80.00	90.00	1.332	28	602.352	A	5.247	101.189	35.927	33.75	0.000	0.000
					B	0.000	101.530		35.39		
					C	11.478	95.010		33.74		
T13 80.00-60.00	70.00	1.24	26	657.397	A	5.247	109.942	42.626	37.01	0.000	0.000
					B	0.000	110.281		38.65		
					C	11.478	104.629		36.71		
T14 60.00-30.00	45.00	1.093	23	1081.03	A	7.871	166.737	63.908	36.60	0.000	0.000
				4	B	0.000	167.251		38.21		
					C	17.217	158.766		36.31		
T15 30.00-0.00	15.00	1	21	1194.29	A	7.871	173.197	63.928	35.31	0.000	0.000
				2	B	0.000	173.735		36.80		
					C	17.217	165.221		35.04		

### Tower Forces - No Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	1	1	22.637	3.11	155.53	C
			B	0.208	2.571	0.592	1	1	22.637			
			C	0.29	2.323	0.613	1	1	31.417			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	1	1	25.689	3.76	187.98	C
			B	0.207	2.573	0.592	1	1	25.689			
			C	0.314	2.259	0.62	1	1	39.810			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	19 of 51
	Project	CSP Tower - Colchester, CT	Date	15:34:40 03/10/06
	Client	Cingular Wireless	Designed by	Jed Kiernan

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	1	1	30.944	4.58	229.20	C
			B	0.2	2.595	0.59	1	1	30.944			
			C	0.319	2.246	0.622	1	1	49.816			
T4 260.00-240.00	0.16	3.83	A	0.189	2.634	0.588	1	1	36.382	5.19	259.37	C
			B	0.189	2.634	0.588	1	1	36.382			
			C	0.297	2.305	0.615	1	1	56.162			
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	1	1	46.567	6.04	302.05	C
			B	0.197	2.605	0.59	1	1	46.567			
			C	0.296	2.306	0.615	1	1	66.937			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	1	1	62.481	5.76	288.09	C
			B	0.157	2.744	0.583	1	1	40.915			
			C	0.248	2.444	0.601	1	1	61.828			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	1	1	68.956	6.22	311.10	C
			B	0.259	2.41	0.604	1	1	68.956			
			C	0.243	2.458	0.6	1	1	68.318			
T8 180.00-160.00	0.94	5.93	A	0.244	2.456	0.6	1	1	72.996	6.44	321.79	A
			B	0.24	2.469	0.599	1	1	71.095			
			C	0.225	2.515	0.596	1	1	70.560			
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	1	1	83.129	7.06	353.23	A
			B	0.24	2.469	0.599	1	1	80.649			
			C	0.226	2.51	0.596	1	1	80.248			
T10 140.00-120.00	0.95	7.17	A	0.231	2.495	0.597	1	1	86.099	7.14	357.13	A
			B	0.226	2.51	0.596	1	1	83.628			
			C	0.217	2.539	0.594	1	1	84.168			
T11 120.00-100.00	0.97	5.96	A	0.188	2.638	0.588	1	1	62.722	5.24	262.24	A
			B	0.18	2.665	0.586	1	1	58.158			
			C	0.175	2.68	0.586	1	1	60.426			
T12 100.00-80.00	1.01	6.17	A	0.177	2.676	0.586	1	1	64.536	5.38	268.86	C
			B	0.169	2.704	0.584	1	1	59.343			
			C	0.177	2.675	0.586	1	1	67.148			
T13 80.00-60.00	1.02	7.26	A	0.175	2.681	0.586	1	1	69.636	5.43	271.28	C
			B	0.168	2.707	0.584	1	1	64.443			
			C	0.177	2.676	0.586	1	1	72.781			
T14 60.00-30.00	1.52	11.10	A	0.162	2.73	0.583	1	1	105.129	7.35	245.02	C
			B	0.155	2.754	0.582	1	1	97.375			
			C	0.163	2.725	0.584	1	1	109.859			
T15 30.00-0.00	1.52	12.90	A	0.152	2.766	0.582	1	1	108.624	7.03	234.48	C
			B	0.145	2.788	0.581	1	1	100.904			
			C	0.153	2.761	0.582	1	1	113.359			
Sum Weight:	10.89	90.41						OTM	13329.95 kip-ft	85.74		

### Tower Forces - No Ice - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	0.825	1	20.596	2.86	143.07	C
			B	0.208	2.571	0.592	0.825	1	20.596			
			C	0.29	2.323	0.613	0.825	1	28.900			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	0.825	1	23.485	3.45	172.49	C
			B	0.207	2.573	0.592	0.825	1	23.485			
			C	0.314	2.259	0.62	0.825	1	36.530			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	0.825	1	28.505	4.27	213.31	C
			B	0.2	2.595	0.59	0.825	1	28.505			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	20 of 51
	Project	CSP Tower - Colchester, CT	Date	15:34:40 03/10/06
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Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T4 260.00-240.00	0.16	3.83	C	0.319	2.246	0.622	0.825	1	46.362			
			A	0.189	2.634	0.588	0.825	1	32.980	4.79	239.39	C
			B	0.189	2.634	0.588	0.825	1	32.980			
T5 240.00-220.00	0.18	4.90	C	0.297	2.305	0.615	0.825	1	51.835			
			A	0.197	2.605	0.59	0.825	1	41.390	5.51	275.36	C
			B	0.197	2.605	0.59	0.825	1	41.390			
T6 220.00-200.00	0.43	4.82	C	0.296	2.306	0.615	0.825	1	61.021			
			A	0.266	2.39	0.606	0.825	1	58.802	5.36	267.98	A
			B	0.157	2.744	0.583	0.825	1	36.691			
T7 200.00-180.00	0.93	5.71	C	0.248	2.444	0.601	0.825	1	56.680			
			A	0.259	2.41	0.604	0.825	1	64.873	5.79	289.63	B
			B	0.259	2.41	0.604	0.825	1	64.873			
T8 180.00-160.00	0.94	5.93	C	0.243	2.458	0.6	0.825	1	62.769			
			A	0.244	2.456	0.6	0.825	1	68.154	6.01	300.44	A
			B	0.24	2.469	0.599	0.825	1	66.571			
T9 160.00-140.00	0.94	6.89	C	0.225	2.515	0.596	0.825	1	64.572			
			A	0.245	2.454	0.601	0.825	1	76.515	6.50	325.12	A
			B	0.24	2.469	0.599	0.825	1	74.452			
T10 140.00-120.00	0.95	7.17	C	0.226	2.51	0.596	0.825	1	72.568			
			A	0.231	2.495	0.597	0.825	1	78.924	6.55	327.36	A
			B	0.226	2.51	0.596	0.825	1	76.871			
T11 120.00-100.00	0.97	5.96	C	0.217	2.539	0.594	0.825	1	75.954			
			A	0.188	2.638	0.588	0.825	1	61.918	5.18	258.88	A
			B	0.18	2.665	0.586	0.825	1	58.158			
T12 100.00-80.00	1.01	6.17	C	0.175	2.68	0.586	0.825	1	58.830			
			A	0.177	2.676	0.586	0.825	1	63.618	5.22	260.81	C
			B	0.169	2.704	0.584	0.825	1	59.343			
T13 80.00-60.00	1.02	7.26	C	0.177	2.675	0.586	0.825	1	65.140			
			A	0.175	2.681	0.586	0.825	1	68.717	5.28	263.79	C
			B	0.168	2.707	0.584	0.825	1	64.443			
T14 60.00-30.00	1.52	11.10	C	0.177	2.676	0.586	0.825	1	70.772			
			A	0.162	2.73	0.583	0.825	1	103.752	7.15	238.30	C
			B	0.155	2.754	0.582	0.825	1	97.375			
T15 30.00-0.00	1.52	12.90	C	0.163	2.725	0.584	0.825	1	106.846			
			A	0.152	2.766	0.582	0.825	1	107.246	6.85	228.25	C
			B	0.145	2.788	0.581	0.825	1	100.904			
Sum Weight:	10.89	90.41	C	0.153	2.761	0.582	0.825	1	110.346			
								OTM	12409.79	80.75		
									kip-ft			

### Tower Forces - No Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	0.8	1	20.305	2.83	141.29	C
			B	0.208	2.571	0.592	0.8	1	20.305			
			C	0.29	2.323	0.613	0.8	1	28.541			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	0.8	1	23.170	3.41	170.28	C
			B	0.207	2.573	0.592	0.8	1	23.170			
			C	0.314	2.259	0.62	0.8	1	36.062			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	0.8	1	28.157	4.22	211.03	C
			B	0.2	2.595	0.59	0.8	1	28.157			
			C	0.319	2.246	0.622	0.8	1	45.868			
T4 260.00-	0.16	3.83	A	0.189	2.634	0.588	0.8	1	32.494	4.73	236.54	C

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	21 of 51
	Project	CSP Tower - Colchester, CT	Date	15:34:40 03/10/06
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Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
240.00			B	0.189	2.634	0.588	0.8	1	32.494			
			C	0.297	2.305	0.615	0.8	1	51.217			
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	0.8	1	40.651	5.43	271.55	C
			B	0.197	2.605	0.59	0.8	1	40.651			
			C	0.296	2.306	0.615	0.8	1	60.176			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	0.8	1	58.276	5.31	265.58	A
			B	0.157	2.744	0.583	0.8	1	36.088			
			C	0.248	2.444	0.601	0.8	1	55.944			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	0.8	1	64.290	5.74	287.03	B
			B	0.259	2.41	0.604	0.8	1	64.290			
			C	0.243	2.458	0.6	0.8	1	61.977			
T8 180.00-160.00	0.94	5.93	A	0.244	2.456	0.6	0.8	1	67.462	5.95	297.40	A
			B	0.24	2.469	0.599	0.8	1	65.924			
			C	0.225	2.515	0.596	0.8	1	63.717			
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	0.8	1	75.570	6.42	321.10	A
			B	0.24	2.469	0.599	0.8	1	73.567			
			C	0.226	2.51	0.596	0.8	1	71.470			
T10 140.00-120.00	0.95	7.17	A	0.231	2.495	0.597	0.8	1	77.898	6.46	323.11	A
			B	0.226	2.51	0.596	0.8	1	75.905			
			C	0.217	2.539	0.594	0.8	1	74.781			
T11 120.00-100.00	0.97	5.96	A	0.188	2.638	0.588	0.8	1	61.804	5.17	258.40	A
			B	0.18	2.665	0.586	0.8	1	58.158			
			C	0.175	2.68	0.586	0.8	1	58.602			
T12 100.00-80.00	1.01	6.17	A	0.177	2.676	0.586	0.8	1	63.487	5.19	259.66	C
			B	0.169	2.704	0.584	0.8	1	59.343			
			C	0.177	2.675	0.586	0.8	1	64.853			
T13 80.00-60.00	1.02	7.26	A	0.175	2.681	0.586	0.8	1	68.586	5.25	262.72	C
			B	0.168	2.707	0.584	0.8	1	64.443			
			C	0.177	2.676	0.586	0.8	1	70.485			
T14 60.00-30.00	1.52	11.10	A	0.162	2.73	0.583	0.8	1	103.555	7.12	237.34	C
			B	0.155	2.754	0.582	0.8	1	97.375			
			C	0.163	2.725	0.584	0.8	1	106.416			
T15 30.00-0.00	1.52	12.90	A	0.152	2.766	0.582	0.8	1	107.049	6.82	227.35	C
			B	0.145	2.788	0.581	0.8	1	100.904			
			C	0.153	2.761	0.582	0.8	1	109.916			
Sum Weight:	10.89	90.41						OTM	12282.10 kip-ft	80.05		

### Tower Forces - No Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	0.85	1	20.888	2.90	144.85	C
			B	0.208	2.571	0.592	0.85	1	20.888			
			C	0.29	2.323	0.613	0.85	1	29.260			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	0.85	1	23.800	3.49	174.71	C
			B	0.207	2.573	0.592	0.85	1	23.800			
			C	0.314	2.259	0.62	0.85	1	36.999			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	0.85	1	28.854	4.31	215.58	C
			B	0.2	2.595	0.59	0.85	1	28.854			
			C	0.319	2.246	0.622	0.85	1	46.855			
T4 260.00-240.00	0.16	3.83	A	0.189	2.634	0.588	0.85	1	33.466	4.84	242.25	C
			B	0.189	2.634	0.588	0.85	1	33.466			
			C	0.297	2.305	0.615	0.85	1	52.454			



<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	22 of 51
	Project	CSP Tower - Colchester, CT	Date	15:34:40 03/10/06
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Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	0.85	1	42.130	5.58	279.17	C
			B	0.197	2.605	0.59	0.85	1	42.130			
			C	0.296	2.306	0.615	0.85	1	61.866			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	0.85	1	59.327	5.41	270.37	A
			B	0.157	2.744	0.583	0.85	1	37.294			
			C	0.248	2.444	0.601	0.85	1	57.415			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	0.85	1	65.457	5.84	292.24	B
			B	0.259	2.41	0.604	0.85	1	65.457			
			C	0.243	2.458	0.6	0.85	1	63.562			
T8 180.00-160.00	0.94	5.93	A	0.244	2.456	0.6	0.85	1	68.845	6.07	303.49	A
			B	0.24	2.469	0.599	0.85	1	67.217			
			C	0.225	2.515	0.596	0.85	1	65.428			
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	0.85	1	77.460	6.58	329.13	A
			B	0.24	2.469	0.599	0.85	1	75.337			
			C	0.226	2.51	0.596	0.85	1	73.665			
T10 140.00-120.00	0.95	7.17	A	0.231	2.495	0.597	0.85	1	79.949	6.63	331.62	A
			B	0.226	2.51	0.596	0.85	1	77.836			
			C	0.217	2.539	0.594	0.85	1	77.128			
T11 120.00-100.00	0.97	5.96	A	0.188	2.638	0.588	0.85	1	62.033	5.19	259.36	A
			B	0.18	2.665	0.586	0.85	1	58.158			
			C	0.175	2.68	0.586	0.85	1	59.058			
T12 100.00-80.00	1.01	6.17	A	0.177	2.676	0.586	0.85	1	63.749	5.24	261.96	C
			B	0.169	2.704	0.584	0.85	1	59.343			
			C	0.177	2.675	0.586	0.85	1	65.426			
T13 80.00-60.00	1.02	7.26	A	0.175	2.681	0.586	0.85	1	68.849	5.30	264.86	C
			B	0.168	2.707	0.584	0.85	1	64.443			
			C	0.177	2.676	0.586	0.85	1	71.059			
T14 60.00-30.00	1.52	11.10	A	0.162	2.73	0.583	0.85	1	103.949	7.18	239.26	C
			B	0.155	2.754	0.582	0.85	1	97.375			
			C	0.163	2.725	0.584	0.85	1	107.276			
T15 30.00-0.00	1.52	12.90	A	0.152	2.766	0.582	0.85	1	107.443	6.87	229.14	C
			B	0.145	2.788	0.581	0.85	1	100.904			
			C	0.153	2.761	0.582	0.85	1	110.777			
Sum Weight:	10.89	90.41						OTM	12537.47 kip-ft	81.44		

### Tower Forces - With Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.21	2.46	A	0.258	2.413	0.604	1	1	29.314	3.91	195.73	C
			B	0.258	2.413	0.604	1	1	29.314			
			C	0.395	2.074	0.65	1	1	44.288			
T2 300.00-280.00	0.31	3.23	A	0.25	2.439	0.602	1	1	32.116	4.69	234.63	C
			B	0.25	2.439	0.602	1	1	32.116			
			C	0.424	2.018	0.662	1	1	55.611			
T3 280.00-260.00	0.43	4.32	A	0.232	2.494	0.597	1	1	36.853	5.66	283.14	C
			B	0.232	2.494	0.597	1	1	36.853			
			C	0.423	2.019	0.661	1	1	68.444			
T4 260.00-240.00	0.47	4.83	A	0.217	2.539	0.594	1	1	42.857	6.34	316.81	C
			B	0.217	2.539	0.594	1	1	42.857			
			C	0.394	2.076	0.649	1	1	76.168			
T5 240.00-220.00	0.52	6.22	A	0.224	2.518	0.596	1	1	53.651	7.21	360.39	C
			B	0.224	2.518	0.596	1	1	53.651			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 23 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 15:34:40 03/10/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T6 220.00-200.00	1.13	5.95	C	0.388	2.089	0.647	1	1	88.179	6.97	348.67	C
			A	0.339	2.198	0.629	1	1	80.356			
			B	0.178	2.67	0.586	1	1	46.998			
T7 200.00-180.00	2.36	7.00	C	0.329	2.221	0.625	1	1	82.351	7.44	372.24	C
			A	0.325	2.232	0.624	1	1	87.097			
			B	0.325	2.232	0.624	1	1	87.097			
T8 180.00-160.00	2.39	7.29	C	0.317	2.253	0.621	1	1	89.174	7.61	380.59	C
			A	0.306	2.281	0.618	1	1	91.990			
			B	0.3	2.297	0.616	1	1	89.287			
T9 160.00-140.00	2.40	8.55	C	0.292	2.317	0.614	1	1	91.515	8.15	407.60	C
			A	0.302	2.292	0.616	1	1	102.271			
			B	0.295	2.311	0.614	1	1	98.768			
T10 140.00-120.00	2.42	8.93	C	0.288	2.329	0.612	1	1	101.066	8.32	415.80	C
			A	0.285	2.338	0.611	1	1	105.441			
			B	0.278	2.356	0.609	1	1	101.960			
T11 120.00-100.00	2.49	7.17	C	0.277	2.358	0.609	1	1	106.085	6.62	331.12	C
			A	0.243	2.459	0.6	1	1	83.272			
			B	0.232	2.492	0.598	1	1	76.801			
T12 100.00-80.00	2.63	7.45	C	0.239	2.471	0.599	1	1	84.539	7.02	351.17	C
			A	0.229	2.503	0.597	1	1	85.459			
			B	0.218	2.538	0.594	1	1	78.107			
T13 80.00-60.00	2.64	8.67	C	0.245	2.453	0.601	1	1	95.668	7.01	350.46	C
			A	0.224	2.519	0.596	1	1	90.847			
			B	0.214	2.551	0.593	1	1	83.500			
T14 60.00-30.00	3.96	13.22	C	0.241	2.465	0.6	1	1	102.089	9.50	316.79	C
			A	0.206	2.576	0.592	1	1	136.590			
			B	0.197	2.607	0.59	1	1	125.658			
T15 30.00-0.00	3.96	15.19	C	0.222	2.525	0.595	1	1	153.278	9.08	302.57	C
			A	0.193	2.621	0.589	1	1	140.421			
			B	0.184	2.649	0.587	1	1	129.551			
Sum Weight:	28.31	110.48	C	0.207	2.573	0.592		OTM	16236.95 kip-ft	105.55		

### Tower Forces - With Ice - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.21	2.46	A	0.258	2.413	0.604	0.825	1	26.496	3.63	181.31	C
			B	0.258	2.413	0.604	0.825	1	26.496			
			C	0.395	2.074	0.65	0.825	1	41.023			
T2 300.00-280.00	0.31	3.23	A	0.25	2.439	0.602	0.825	1	29.177	4.34	217.00	C
			B	0.25	2.439	0.602	0.825	1	29.177			
			C	0.424	2.018	0.662	0.825	1	51.435			
T3 280.00-260.00	0.43	4.32	A	0.232	2.494	0.597	0.825	1	33.765	5.31	265.52	C
			B	0.232	2.494	0.597	0.825	1	33.765			
			C	0.423	2.019	0.661	0.825	1	64.187			
T4 260.00-240.00	0.47	4.83	A	0.217	2.539	0.594	0.825	1	38.698	5.91	295.37	C
			B	0.217	2.539	0.594	0.825	1	38.698			
			C	0.394	2.076	0.649	0.825	1	71.015			
T5 240.00-220.00	0.52	6.22	A	0.224	2.518	0.596	0.825	1	47.611	6.66	333.04	C
			B	0.224	2.518	0.596	0.825	1	47.611			
			C	0.388	2.089	0.647	0.825	1	81.489			
T6 220.00-	1.13	5.95	A	0.339	2.198	0.629	0.825	1	76.384	6.47	323.44	C

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 24 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 15:34:40 03/10/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
200.00			B	0.178	2.67	0.586	0.825	1	42.071			
			C	0.329	2.221	0.625	0.825	1	76.392			
T7 200.00-180.00	2.36	7.00	A	0.325	2.232	0.624	0.825	1	82.645	6.91	345.37	C
			B	0.325	2.232	0.624	0.825	1	82.645			
			C	0.317	2.253	0.621	0.825	1	82.736			
T8 180.00-160.00	2.39	7.29	A	0.306	2.281	0.618	0.825	1	86.574	7.09	354.50	A
			B	0.3	2.297	0.616	0.825	1	84.312			
			C	0.292	2.317	0.614	0.825	1	84.555			
T9 160.00-140.00	2.40	8.55	A	0.302	2.292	0.616	0.825	1	95.034	7.54	377.15	A
			B	0.295	2.311	0.614	0.825	1	92.107			
			C	0.288	2.329	0.612	0.825	1	92.414			
T10 140.00-120.00	2.42	8.93	A	0.285	2.338	0.611	0.825	1	97.563	7.59	379.60	C
			B	0.278	2.356	0.609	0.825	1	94.658			
			C	0.277	2.358	0.609	0.825	1	96.851			
T11 120.00-100.00	2.49	7.17	A	0.243	2.459	0.6	0.825	1	82.128	6.44	321.94	C
			B	0.232	2.492	0.598	0.825	1	76.801			
			C	0.239	2.471	0.599	0.825	1	82.195			
T12 100.00-80.00	2.63	7.45	A	0.229	2.503	0.597	0.825	1	84.152	6.80	340.23	C
			B	0.218	2.538	0.594	0.825	1	78.107			
			C	0.245	2.453	0.601	0.825	1	92.687			
T13 80.00-60.00	2.64	8.67	A	0.224	2.519	0.596	0.825	1	89.540	6.80	340.23	C
			B	0.214	2.551	0.593	0.825	1	83.500			
			C	0.241	2.465	0.6	0.825	1	99.108			
T14 60.00-30.00	3.96	13.22	A	0.206	2.576	0.592	0.825	1	134.630	9.23	307.55	C
			B	0.197	2.607	0.59	0.825	1	125.658			
			C	0.222	2.525	0.595	0.825	1	148.807			
T15 30.00-0.00	3.96	15.19	A	0.193	2.621	0.589	0.825	1	138.460	8.82	293.95	C
			B	0.184	2.649	0.587	0.825	1	129.551			
			C	0.207	2.573	0.592	0.825	1	152.510			
Sum Weight:	28.31	110.48						OTM	15164.41 kip-ft	99.54		

### Tower Forces - With Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.21	2.46	A	0.258	2.413	0.604	0.8	1	26.094	3.58	179.24	C
			B	0.258	2.413	0.604	0.8	1	26.094			
			C	0.395	2.074	0.65	0.8	1	40.557			
T2 300.00-280.00	0.31	3.23	A	0.25	2.439	0.602	0.8	1	28.757	4.29	214.49	C
			B	0.25	2.439	0.602	0.8	1	28.757			
			C	0.424	2.018	0.662	0.8	1	50.838			
T3 280.00-260.00	0.43	4.32	A	0.232	2.494	0.597	0.8	1	33.323	5.26	263.01	C
			B	0.232	2.494	0.597	0.8	1	33.323			
			C	0.423	2.019	0.661	0.8	1	63.579			
T4 260.00-240.00	0.47	4.83	A	0.217	2.539	0.594	0.8	1	38.104	5.85	292.31	C
			B	0.217	2.539	0.594	0.8	1	38.104			
			C	0.394	2.076	0.649	0.8	1	70.279			
T5 240.00-220.00	0.52	6.22	A	0.224	2.518	0.596	0.8	1	46.748	6.58	329.14	C
			B	0.224	2.518	0.596	0.8	1	46.748			
			C	0.388	2.089	0.647	0.8	1	80.533			
T6 220.00-200.00	1.13	5.95	A	0.339	2.198	0.629	0.8	1	75.816	6.40	319.83	C
			B	0.178	2.67	0.586	0.8	1	41.367			
			C	0.329	2.221	0.625	0.8	1	75.541			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	25 of 51
	Project	CSP Tower - Colchester, CT	Date	15:34:40 03/10/06
	Client	Cingular Wireless	Designed by	Jed Kiernan

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T7 200.00-180.00	2.36	7.00	A	0.325	2.232	0.624	0.8	1	82.009	6.83	341.53	C
			B	0.325	2.232	0.624	0.8	1	82.009			
			C	0.317	2.253	0.621	0.8	1	81.816			
T8 180.00-160.00	2.39	7.29	A	0.306	2.281	0.618	0.8	1	85.800	7.03	351.33	A
			B	0.3	2.297	0.616	0.8	1	83.602			
			C	0.292	2.317	0.614	0.8	1	83.561			
T9 160.00-140.00	2.40	8.55	A	0.302	2.292	0.616	0.8	1	94.000	7.46	373.04	A
			B	0.295	2.311	0.614	0.8	1	91.155			
			C	0.288	2.329	0.612	0.8	1	91.178			
T10 140.00-120.00	2.42	8.93	A	0.285	2.338	0.611	0.8	1	96.437	7.50	374.81	A
			B	0.278	2.356	0.609	0.8	1	93.615			
			C	0.277	2.358	0.609	0.8	1	95.531			
T11 120.00-100.00	2.49	7.17	A	0.243	2.459	0.6	0.8	1	81.965	6.41	320.63	C
			B	0.232	2.492	0.598	0.8	1	76.801			
			C	0.239	2.471	0.599	0.8	1	81.860			
T12 100.00-80.00	2.63	7.45	A	0.229	2.503	0.597	0.8	1	83.965	6.77	338.66	C
			B	0.218	2.538	0.594	0.8	1	78.107			
			C	0.245	2.453	0.601	0.8	1	92.261			
T13 80.00-60.00	2.64	8.67	A	0.224	2.519	0.596	0.8	1	89.353	6.78	338.77	C
			B	0.214	2.551	0.593	0.8	1	83.500			
			C	0.241	2.465	0.6	0.8	1	98.682			
T14 60.00-30.00	3.96	13.22	A	0.206	2.576	0.592	0.8	1	134.350	9.19	306.23	C
			B	0.197	2.607	0.59	0.8	1	125.658			
			C	0.222	2.525	0.595	0.8	1	148.168			
T15 30.00-0.00	3.96	15.19	A	0.193	2.621	0.589	0.8	1	138.180	8.78	292.72	C
			B	0.184	2.649	0.587	0.8	1	129.551			
			C	0.207	2.573	0.592	0.8	1	151.872			
Sum Weight:	28.31	110.48						OTM	15014.82 kip-ft	98.70		

### Tower Forces - With Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.21	2.46	A	0.258	2.413	0.604	0.85	1	26.899	3.67	183.37	C
			B	0.258	2.413	0.604	0.85	1	26.899			
			C	0.395	2.074	0.65	0.85	1	41.490			
T2 300.00-280.00	0.31	3.23	A	0.25	2.439	0.602	0.85	1	29.597	4.39	219.52	C
			B	0.25	2.439	0.602	0.85	1	29.597			
			C	0.424	2.018	0.662	0.85	1	52.031			
T3 280.00-260.00	0.43	4.32	A	0.232	2.494	0.597	0.85	1	34.206	5.36	268.04	C
			B	0.232	2.494	0.597	0.85	1	34.206			
			C	0.423	2.019	0.661	0.85	1	64.795			
T4 260.00-240.00	0.47	4.83	A	0.217	2.539	0.594	0.85	1	39.293	5.97	298.44	C
			B	0.217	2.539	0.594	0.85	1	39.293			
			C	0.394	2.076	0.649	0.85	1	71.751			
T5 240.00-220.00	0.52	6.22	A	0.224	2.518	0.596	0.85	1	48.474	6.74	336.95	C
			B	0.224	2.518	0.596	0.85	1	48.474			
			C	0.388	2.089	0.647	0.85	1	82.444			
T6 220.00-200.00	1.13	5.95	A	0.339	2.198	0.629	0.85	1	76.951	6.54	327.04	C
			B	0.178	2.67	0.586	0.85	1	42.775			
			C	0.329	2.221	0.625	0.85	1	77.243			
T7 200.00-180.00	2.36	7.00	A	0.325	2.232	0.624	0.85	1	83.281	6.98	349.20	C
			B	0.325	2.232	0.624	0.85	1	83.281			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 26 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 15:34:40 03/10/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T8 180.00-160.00	2.39	7.29	C	0.317	2.253	0.621	0.85	1	83.656	7.15	357.67	A
			A	0.306	2.281	0.618	0.85	1	87.348			
			B	0.3	2.297	0.616	0.85	1	85.023			
T9 160.00-140.00	2.40	8.55	C	0.292	2.317	0.614	0.85	1	85.549	7.62	381.25	A
			A	0.302	2.292	0.616	0.85	1	96.068			
			B	0.295	2.311	0.614	0.85	1	93.058			
T10 140.00-120.00	2.42	8.93	C	0.288	2.329	0.612	0.85	1	93.650	7.70	384.77	C
			A	0.285	2.338	0.611	0.85	1	98.688			
			B	0.278	2.356	0.609	0.85	1	95.701			
T11 120.00-100.00	2.49	7.17	C	0.277	2.358	0.609	0.85	1	98.170	6.47	323.25	C
			A	0.243	2.459	0.6	0.85	1	82.292			
			B	0.232	2.492	0.598	0.85	1	76.801			
T12 100.00-80.00	2.63	7.45	C	0.239	2.471	0.599	0.85	1	82.530	6.84	341.79	C
			A	0.229	2.503	0.597	0.85	1	84.339			
			B	0.218	2.538	0.594	0.85	1	78.107			
T13 80.00-60.00	2.64	8.67	C	0.245	2.453	0.601	0.85	1	93.112	6.83	341.69	C
			A	0.224	2.519	0.596	0.85	1	89.726			
			B	0.214	2.551	0.593	0.85	1	83.500			
T14 60.00-30.00	3.96	13.22	C	0.241	2.465	0.6	0.85	1	99.534	9.27	308.87	C
			A	0.206	2.576	0.592	0.85	1	134.910			
			B	0.197	2.607	0.59	0.85	1	125.658			
T15 30.00-0.00	3.96	15.19	C	0.222	2.525	0.595	0.85	1	149.445	8.86	295.18	C
			A	0.193	2.621	0.589	0.85	1	138.740			
			B	0.184	2.649	0.587	0.85	1	129.551			
Sum Weight:	28.31	110.48	C	0.207	2.573	0.592	0.85	OTM	15314.99 kip-ft	100.38		

### Tower Forces - Service - Wind Normal To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	1	1	22.637	3.11	155.53	C
			B	0.208	2.571	0.592	1	1	22.637			
			C	0.29	2.323	0.613	1	1	31.417			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	1	1	25.689	3.76	187.98	C
			B	0.207	2.573	0.592	1	1	25.689			
			C	0.314	2.259	0.62	1	1	39.810			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	1	1	30.944	4.58	229.20	C
			B	0.2	2.595	0.59	1	1	30.944			
			C	0.319	2.246	0.622	1	1	49.816			
T4 260.00-240.00	0.16	3.83	A	0.189	2.634	0.588	1	1	36.382	5.19	259.37	C
			B	0.189	2.634	0.588	1	1	36.382			
			C	0.297	2.305	0.615	1	1	56.162			
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	1	1	46.567	6.04	302.05	C
			B	0.197	2.605	0.59	1	1	46.567			
			C	0.296	2.306	0.615	1	1	66.937			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	1	1	62.481	5.76	288.09	C
			B	0.157	2.744	0.583	1	1	40.915			
			C	0.248	2.444	0.601	1	1	61.828			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	1	1	68.956	6.22	311.10	C
			B	0.259	2.41	0.604	1	1	68.956			
			C	0.243	2.458	0.6	1	1	68.318			
T8 180.00-	0.94	5.93	A	0.244	2.456	0.6	1	1	72.996	6.44	321.79	A

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 27 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 15:34:40 03/10/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
160.00			B	0.24	2.469	0.599	1	1	71.095			
			C	0.225	2.515	0.596	1	1	70.560			
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	1	1	83.129	7.06	353.23	A
			B	0.24	2.469	0.599	1	1	80.649			
			C	0.226	2.51	0.596	1	1	80.248			
T10 140.00-120.00	0.95	7.17	A	0.231	2.495	0.597	1	1	86.099	7.14	357.13	A
			B	0.226	2.51	0.596	1	1	83.628			
			C	0.217	2.539	0.594	1	1	84.168			
T11 120.00-100.00	0.97	5.96	A	0.188	2.638	0.588	1	1	62.722	5.24	262.24	A
			B	0.18	2.665	0.586	1	1	58.158			
			C	0.175	2.68	0.586	1	1	60.426			
T12 100.00-80.00	1.01	6.17	A	0.177	2.676	0.586	1	1	64.536	5.38	268.86	C
			B	0.169	2.704	0.584	1	1	59.343			
			C	0.177	2.675	0.586	1	1	67.148			
T13 80.00-60.00	1.02	7.26	A	0.175	2.681	0.586	1	1	69.636	5.43	271.28	C
			B	0.168	2.707	0.584	1	1	64.443			
			C	0.177	2.676	0.586	1	1	72.781			
T14 60.00-30.00	1.52	11.10	A	0.162	2.73	0.583	1	1	105.129	7.35	245.02	C
			B	0.155	2.754	0.582	1	1	97.375			
			C	0.163	2.725	0.584	1	1	109.859			
T15 30.00-0.00	1.52	12.90	A	0.152	2.766	0.582	1	1	108.624	7.03	234.48	C
			B	0.145	2.788	0.581	1	1	100.904			
			C	0.153	2.761	0.582	1	1	113.359			
Sum Weight:	10.89	90.41						OTM	13329.95 kip-ft	85.74		

**Tower Forces - Service - Wind 45 To Face**

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	0.825	1	20.596	2.86	143.07	C
			B	0.208	2.571	0.592	0.825	1	20.596			
			C	0.29	2.323	0.613	0.825	1	28.900			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	0.825	1	23.485	3.45	172.49	C
			B	0.207	2.573	0.592	0.825	1	23.485			
			C	0.314	2.259	0.62	0.825	1	36.530			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	0.825	1	28.505	4.27	213.31	C
			B	0.2	2.595	0.59	0.825	1	28.505			
			C	0.319	2.246	0.622	0.825	1	46.362			
T4 260.00-240.00	0.16	3.83	A	0.189	2.634	0.588	0.825	1	32.980	4.79	239.39	C
			B	0.189	2.634	0.588	0.825	1	32.980			
			C	0.297	2.305	0.615	0.825	1	51.835			
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	0.825	1	41.390	5.51	275.36	C
			B	0.197	2.605	0.59	0.825	1	41.390			
			C	0.296	2.306	0.615	0.825	1	61.021			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	0.825	1	58.802	5.36	267.98	A
			B	0.157	2.744	0.583	0.825	1	36.691			
			C	0.248	2.444	0.601	0.825	1	56.680			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	0.825	1	64.873	5.79	289.63	B
			B	0.259	2.41	0.604	0.825	1	64.873			
			C	0.243	2.458	0.6	0.825	1	62.769			
T8 180.00-160.00	0.94	5.93	A	0.244	2.456	0.6	0.825	1	68.154	6.01	300.44	A
			B	0.24	2.469	0.599	0.825	1	66.571			
			C	0.225	2.515	0.596	0.825	1	64.572			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b>	320' Rohn SSVMW	<b>Page</b>	28 of 51
	<b>Project</b>	CSP Tower - Colchester, CT	<b>Date</b>	15:34:40 03/10/06
	<b>Client</b>	Cingular Wireless	<b>Designed by</b>	Jed Kiernan

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	0.825	1	76.515	6.50	325.12	A
			B	0.24	2.469	0.599	0.825	1	74.452			
			C	0.226	2.51	0.596	0.825	1	72.568			
T10 140.00-120.00	0.95	7.17	A	0.231	2.495	0.597	0.825	1	78.924	6.55	327.36	A
			B	0.226	2.51	0.596	0.825	1	76.871			
			C	0.217	2.539	0.594	0.825	1	75.954			
T11 120.00-100.00	0.97	5.96	A	0.188	2.638	0.588	0.825	1	61.918	5.18	258.88	A
			B	0.18	2.665	0.586	0.825	1	58.158			
			C	0.175	2.68	0.586	0.825	1	58.830			
T12 100.00-80.00	1.01	6.17	A	0.177	2.676	0.586	0.825	1	63.618	5.22	260.81	C
			B	0.169	2.704	0.584	0.825	1	59.343			
			C	0.177	2.675	0.586	0.825	1	65.140			
T13 80.00-60.00	1.02	7.26	A	0.175	2.681	0.586	0.825	1	68.717	5.28	263.79	C
			B	0.168	2.707	0.584	0.825	1	64.443			
			C	0.177	2.676	0.586	0.825	1	70.772			
T14 60.00-30.00	1.52	11.10	A	0.162	2.73	0.583	0.825	1	103.752	7.15	238.30	C
			B	0.155	2.754	0.582	0.825	1	97.375			
			C	0.163	2.725	0.584	0.825	1	106.846			
T15 30.00-0.00	1.52	12.90	A	0.152	2.766	0.582	0.825	1	107.246	6.85	228.25	C
			B	0.145	2.788	0.581	0.825	1	100.904			
			C	0.153	2.761	0.582	0.825	1	110.346			
Sum Weight:	10.89	90.41						OTM	12409.79 kip-ft	80.75		

### Tower Forces - Service - Wind 60 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	0.8	1	20.305	2.83	141.29	C
			B	0.208	2.571	0.592	0.8	1	20.305			
			C	0.29	2.323	0.613	0.8	1	28.541			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	0.8	1	23.170	3.41	170.28	C
			B	0.207	2.573	0.592	0.8	1	23.170			
			C	0.314	2.259	0.62	0.8	1	36.062			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	0.8	1	28.157	4.22	211.03	C
			B	0.2	2.595	0.59	0.8	1	28.157			
			C	0.319	2.246	0.622	0.8	1	45.868			
T4 260.00-240.00	0.16	3.83	A	0.189	2.634	0.588	0.8	1	32.494	4.73	236.54	C
			B	0.189	2.634	0.588	0.8	1	32.494			
			C	0.297	2.305	0.615	0.8	1	51.217			
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	0.8	1	40.651	5.43	271.55	C
			B	0.197	2.605	0.59	0.8	1	40.651			
			C	0.296	2.306	0.615	0.8	1	60.176			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	0.8	1	58.276	5.31	265.58	A
			B	0.157	2.744	0.583	0.8	1	36.088			
			C	0.248	2.444	0.601	0.8	1	55.944			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	0.8	1	64.290	5.74	287.03	B
			B	0.259	2.41	0.604	0.8	1	64.290			
			C	0.243	2.458	0.6	0.8	1	61.977			
T8 180.00-160.00	0.94	5.93	A	0.244	2.456	0.6	0.8	1	67.462	5.95	297.40	A
			B	0.24	2.469	0.599	0.8	1	65.924			
			C	0.225	2.515	0.596	0.8	1	63.717			
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	0.8	1	75.570	6.42	321.10	A
			B	0.24	2.469	0.599	0.8	1	73.567			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 29 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 15:34:40 03/10/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T10 140.00-120.00	0.95	7.17	C	0.226	2.51	0.596	0.8	1	71.470	6.46	323.11	A
			A	0.231	2.495	0.597	0.8	1	77.898			
			B	0.226	2.51	0.596	0.8	1	75.905			
T11 120.00-100.00	0.97	5.96	C	0.217	2.539	0.594	0.8	1	74.781	5.17	258.40	A
			A	0.188	2.638	0.588	0.8	1	61.804			
			B	0.18	2.665	0.586	0.8	1	58.158			
T12 100.00-80.00	1.01	6.17	C	0.175	2.68	0.586	0.8	1	58.602	5.19	259.66	C
			A	0.177	2.676	0.586	0.8	1	63.487			
			B	0.169	2.704	0.584	0.8	1	59.343			
T13 80.00-60.00	1.02	7.26	C	0.177	2.675	0.586	0.8	1	64.853	5.25	262.72	C
			A	0.175	2.681	0.586	0.8	1	68.586			
			B	0.168	2.707	0.584	0.8	1	64.443			
T14 60.00-30.00	1.52	11.10	C	0.177	2.676	0.586	0.8	1	70.485	7.12	237.34	C
			A	0.162	2.73	0.583	0.8	1	103.555			
			B	0.155	2.754	0.582	0.8	1	97.375			
T15 30.00-0.00	1.52	12.90	C	0.163	2.725	0.584	0.8	1	106.416	6.82	227.35	C
			A	0.152	2.766	0.582	0.8	1	107.049			
			B	0.145	2.788	0.581	0.8	1	100.904			
Sum Weight:	10.89	90.41	C	0.153	2.761	0.582	0.8	1	109.916	80.05		
								OTM	12282.10			
									kip-ft			

### Tower Forces - Service - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	0.85	1	20.888	2.90	144.85	C
			B	0.208	2.571	0.592	0.85	1	20.888			
			C	0.29	2.323	0.613	0.85	1	29.260			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	0.85	1	23.800	3.49	174.71	C
			B	0.207	2.573	0.592	0.85	1	23.800			
			C	0.314	2.259	0.62	0.85	1	36.999			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	0.85	1	28.854	4.31	215.58	C
			B	0.2	2.595	0.59	0.85	1	28.854			
			C	0.319	2.246	0.622	0.85	1	46.855			
T4 260.00-240.00	0.16	3.83	A	0.189	2.634	0.588	0.85	1	33.466	4.84	242.25	C
			B	0.189	2.634	0.588	0.85	1	33.466			
			C	0.297	2.305	0.615	0.85	1	52.454			
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	0.85	1	42.130	5.58	279.17	C
			B	0.197	2.605	0.59	0.85	1	42.130			
			C	0.296	2.306	0.615	0.85	1	61.866			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	0.85	1	59.327	5.41	270.37	A
			B	0.157	2.744	0.583	0.85	1	37.294			
			C	0.248	2.444	0.601	0.85	1	57.415			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	0.85	1	65.457	5.84	292.24	B
			B	0.259	2.41	0.604	0.85	1	65.457			
			C	0.243	2.458	0.6	0.85	1	63.562			
T8 180.00-160.00	0.94	5.93	A	0.244	2.456	0.6	0.85	1	68.845	6.07	303.49	A
			B	0.24	2.469	0.599	0.85	1	67.217			
			C	0.225	2.515	0.596	0.85	1	65.428			
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	0.85	1	77.460	6.58	329.13	A
			B	0.24	2.469	0.599	0.85	1	75.337			
			C	0.226	2.51	0.596	0.85	1	73.665			
T10 140.00-	0.95	7.17	A	0.231	2.495	0.597	0.85	1	79.949	6.63	331.62	A



<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 30 of 51
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	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
120.00			B	0.226	2.51	0.596	0.85	1	77.836			
			C	0.217	2.539	0.594	0.85	1	77.128			
T11 120.00-100.00	0.97	5.96	A	0.188	2.638	0.588	0.85	1	62.033	5.19	259.36	A
			B	0.18	2.665	0.586	0.85	1	58.158			
			C	0.175	2.68	0.586	0.85	1	59.058			
T12 100.00-80.00	1.01	6.17	A	0.177	2.676	0.586	0.85	1	63.749	5.24	261.96	C
			B	0.169	2.704	0.584	0.85	1	59.343			
			C	0.177	2.675	0.586	0.85	1	65.426			
T13 80.00-60.00	1.02	7.26	A	0.175	2.681	0.586	0.85	1	68.849	5.30	264.86	C
			B	0.168	2.707	0.584	0.85	1	64.443			
			C	0.177	2.676	0.586	0.85	1	71.059			
T14 60.00-30.00	1.52	11.10	A	0.162	2.73	0.583	0.85	1	103.949	7.18	239.26	C
			B	0.155	2.754	0.582	0.85	1	97.375			
			C	0.163	2.725	0.584	0.85	1	107.276			
T15 30.00-0.00	1.52	12.90	A	0.152	2.766	0.582	0.85	1	107.443	6.87	229.14	C
			B	0.145	2.788	0.581	0.85	1	100.904			
			C	0.153	2.761	0.582	0.85	1	110.777			
Sum Weight:	10.89	90.41						OTM	12537.47 kip-ft	81.44		

### Force Totals

Load Case	Vertical Forces	Sum of Forces X	Sum of Forces Z	Sum of Overturning Moments, M <sub>x</sub>	Sum of Overturning Moments, M <sub>z</sub>	Sum of Torques
	K	K	K	kip-ft	kip-ft	kip-ft
Leg Weight	50.33					
Bracing Weight	40.08					
Total Member Self-Weight	90.41					
Total Weight	108.28			-34.22	55.23	
Wind 0 deg - No Ice		0.00	-105.85	-17968.68	55.23	-174.58
Wind 30 deg - No Ice		50.75	-87.94	-14879.62	-8511.89	-132.36
Wind 45 deg - No Ice		71.29	-71.31	-12065.15	-11970.22	-99.91
Wind 60 deg - No Ice		86.71	-50.08	-8477.53	-14562.30	-61.15
Wind 90 deg - No Ice		101.51	0.00	-34.22	-17079.01	24.87
Wind 120 deg - No Ice		91.63	52.92	8933.01	-15469.77	109.72
Wind 135 deg - No Ice		71.29	71.31	11996.70	-11970.22	134.85
Wind 150 deg - No Ice		50.75	87.94	14811.17	-8511.89	157.23
Wind 180 deg - No Ice		0.00	100.16	16852.38	55.23	164.81
Wind 210 deg - No Ice		-50.75	87.94	14811.17	8622.35	132.36
Wind 225 deg - No Ice		-71.29	71.31	11996.70	12080.68	99.91
Wind 240 deg - No Ice		-91.63	52.92	8933.01	15580.23	64.86
Wind 270 deg - No Ice		-101.51	0.00	-34.22	17189.47	-24.87
Wind 300 deg - No Ice		-86.71	-50.08	-8477.53	14672.76	-103.66
Wind 315 deg - No Ice		-71.29	-71.31	-12065.15	12080.68	-134.85
Wind 330 deg - No Ice		-50.75	-87.94	-14879.62	8622.35	-157.23
Member Ice	20.07					
Total Weight Ice	149.73			-73.21	155.49	
Wind 0 deg - Ice		0.00	-129.01	-21693.63	155.49	-272.41
Wind 30 deg - Ice		61.90	-107.25	-17998.60	-10189.46	-204.02
Wind 45 deg - Ice		86.94	-86.97	-14602.75	-14368.00	-152.40
Wind 60 deg - Ice		105.76	-61.08	-10272.36	-17502.53	-91.21
Wind 90 deg - Ice		123.80	0.00	-73.21	-20534.40	43.76
Wind 120 deg - Ice		111.69	64.50	10736.99	-18560.92	175.76
Wind 135 deg - Ice		86.94	86.97	14456.33	-14368.00	213.84
Wind 150 deg - Ice		61.90	107.25	17852.17	-10189.46	247.79

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 31 of 51
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	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, $M_x$ kip-ft	Sum of Overturning Moments, $M_z$ kip-ft	Sum of Torques kip-ft
Wind 180 deg - Ice		0.00	122.16	20325.07	155.49	257.11
Wind 210 deg - Ice		-61.90	107.25	17852.17	10500.44	204.02
Wind 225 deg - Ice		-86.94	86.97	14456.33	14678.98	152.40
Wind 240 deg - Ice		-111.69	64.50	10736.99	18871.91	96.65
Wind 270 deg - Ice		-123.80	0.00	-73.21	20845.38	-43.76
Wind 300 deg - Ice		-105.76	-61.08	-10272.36	17813.51	-165.90
Wind 315 deg - Ice		-86.94	-86.97	-14602.75	14678.98	-213.84
Wind 330 deg - Ice		-61.90	-107.25	-17998.60	10500.44	-247.79
Total Weight	108.28			-34.22	55.23	
Wind 0 deg - Service		0.00	-105.85	-17933.71	-3.99	-174.58
Wind 30 deg - Service		50.75	-87.94	-14844.64	-8571.11	-132.36
Wind 45 deg - Service		71.29	-71.31	-12030.17	-12029.44	-99.91
Wind 60 deg - Service		86.71	-50.08	-8442.55	-14621.52	-61.15
Wind 90 deg - Service		101.51	0.00	0.75	-17138.23	24.87
Wind 120 deg - Service		91.63	52.92	8967.98	-15528.98	109.72
Wind 135 deg - Service		71.29	71.31	12031.67	-12029.44	134.85
Wind 150 deg - Service		50.75	87.94	14846.14	-8571.11	157.23
Wind 180 deg - Service		0.00	100.16	16887.36	-3.99	164.81
Wind 210 deg - Service		-50.75	87.94	14846.14	8563.13	132.36
Wind 225 deg - Service		-71.29	71.31	12031.67	12021.46	99.91
Wind 240 deg - Service		-91.63	52.92	8967.98	15521.01	64.86
Wind 270 deg - Service		-101.51	0.00	0.75	17130.26	-24.87
Wind 300 deg - Service		-86.71	-50.08	-8442.55	14613.54	-103.66
Wind 315 deg - Service		-71.29	-71.31	-12030.17	12021.46	-134.85
Wind 330 deg - Service		-50.75	-87.94	-14844.64	8563.13	-157.23

### Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 45 deg - No Ice
5	Dead+Wind 60 deg - No Ice
6	Dead+Wind 90 deg - No Ice
7	Dead+Wind 120 deg - No Ice
8	Dead+Wind 135 deg - No Ice
9	Dead+Wind 150 deg - No Ice
10	Dead+Wind 180 deg - No Ice
11	Dead+Wind 210 deg - No Ice
12	Dead+Wind 225 deg - No Ice
13	Dead+Wind 240 deg - No Ice
14	Dead+Wind 270 deg - No Ice
15	Dead+Wind 300 deg - No Ice
16	Dead+Wind 315 deg - No Ice
17	Dead+Wind 330 deg - No Ice
18	Dead+Ice+Temp
19	Dead+Wind 0 deg+Ice+Temp
20	Dead+Wind 30 deg+Ice+Temp
21	Dead+Wind 45 deg+Ice+Temp
22	Dead+Wind 60 deg+Ice+Temp
23	Dead+Wind 90 deg+Ice+Temp
24	Dead+Wind 120 deg+Ice+Temp
25	Dead+Wind 135 deg+Ice+Temp
26	Dead+Wind 150 deg+Ice+Temp

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Comb. No.	Description
27	Dead+Wind 180 deg+Ice+Temp
28	Dead+Wind 210 deg+Ice+Temp
29	Dead+Wind 225 deg+Ice+Temp
30	Dead+Wind 240 deg+Ice+Temp
31	Dead+Wind 270 deg+Ice+Temp
32	Dead+Wind 300 deg+Ice+Temp
33	Dead+Wind 315 deg+Ice+Temp
34	Dead+Wind 330 deg+Ice+Temp
35	Dead+Wind 0 deg - Service
36	Dead+Wind 30 deg - Service
37	Dead+Wind 45 deg - Service
38	Dead+Wind 60 deg - Service
39	Dead+Wind 90 deg - Service
40	Dead+Wind 120 deg - Service
41	Dead+Wind 135 deg - Service
42	Dead+Wind 150 deg - Service
43	Dead+Wind 180 deg - Service
44	Dead+Wind 210 deg - Service
45	Dead+Wind 225 deg - Service
46	Dead+Wind 240 deg - Service
47	Dead+Wind 270 deg - Service
48	Dead+Wind 300 deg - Service
49	Dead+Wind 315 deg - Service
50	Dead+Wind 330 deg - Service

### Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	320 - 300	Leg	Max Tension	27	14.72	-0.07	0.06
			Max. Compression	30	-18.74	0.55	-0.43
			Max. Mx	24	-17.72	-0.87	0.17
			Max. My	19	5.86	0.27	1.04
			Max. Vy	31	1.17	-0.19	-0.17
			Max. Vx	19	1.21	0.00	-0.10
		Diagonal	Max Tension	23	4.28	0.00	0.00
			Max. Compression	24	-4.35	0.00	0.00
			Max. Mx	30	3.16	0.01	0.00
			Max. My	34	-2.77	0.01	0.00
			Max. Vy	30	-0.01	0.01	0.00
			Max. Vx	34	0.00	0.00	0.00
		Top Girt	Max Tension	30	0.07	0.00	0.00
			Max. Compression	32	-0.10	0.00	0.00
			Max. Mx	18	-0.01	-0.02	0.00
			Max. Vy	18	-0.01	0.00	0.00
			Max. Vx	24	-0.27	0.41	-0.04
			Max. Vy	24	-0.27	0.41	-0.04
T2	300 - 280	Leg	Max Tension	27	42.97	-0.12	0.03
			Max. Compression	30	-50.84	0.80	-0.05
			Max. Mx	30	-50.84	0.80	-0.05
			Max. My	34	-2.43	0.04	1.10
			Max. Vy	24	-0.27	0.41	-0.04
			Max. Vx	26	-0.51	-0.03	-0.24
		Diagonal	Max Tension	20	5.25	0.00	0.00
			Max. Compression	19	-5.43	0.00	0.00
			Max. Mx	32	2.58	0.02	0.01
			Max. My	34	-3.73	0.01	0.01
			Max. Vy	32	0.01	0.02	0.01
			Max. Vx	34	-0.00	0.00	0.00
Top Girt	Max Tension	30	0.09	0.00	0.00		

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T3	280 - 260	Leg	Max. Compression	32	-0.11	0.00	0.00	
			Max. Mx	18	-0.01	-0.03	0.00	
			Max. My	18	-0.01	0.00	0.00	
			Max. Vy	18	0.02	0.00	0.00	
			Max. Vx	18	-0.00	0.00	0.00	
			Max Tension	32	70.68	-0.59	0.00	
			Max. Compression	30	-83.19	1.00	0.00	
		Diagonal	Max. Mx	30	-83.19	1.00	0.00	
			Max. My	34	-5.50	-0.02	1.07	
			Max. Vy	27	0.24	-0.64	0.01	
			Max. Vx	31	0.34	-0.02	-0.64	
			Max Tension	20	6.53	0.00	0.00	
			Max. Compression	19	-6.60	0.00	0.00	
			Max. Mx	27	3.37	0.04	-0.01	
T4	260 - 240	Leg	Max. My	26	-3.85	0.01	-0.01	
			Max. Vy	27	0.02	0.04	-0.01	
			Max. Vx	26	0.00	0.00	0.00	
			Max Tension	32	100.97	-0.71	0.00	
			Max. Compression	30	-119.28	1.91	-0.02	
			Max. Mx	19	-117.70	1.91	0.37	
			Max. My	34	-7.66	0.03	1.91	
		Diagonal	Max. Vy	19	-0.38	1.91	0.37	
			Max. Vx	26	0.48	0.03	-1.91	
			Max Tension	20	7.97	0.00	0.00	
			Max. Compression	19	-8.13	0.00	0.00	
			Max. Mx	27	4.43	0.06	-0.01	
			Max. My	34	-5.27	0.03	0.01	
			Max. Vy	27	0.03	0.06	-0.01	
T5	240 - 220	Leg	Max. Vx	34	-0.00	0.00	0.00	
			Max Tension	32	133.35	-0.84	0.00	
			Max. Compression	30	-158.39	1.68	-0.01	
			Max. Mx	19	-130.04	1.91	0.37	
			Max. My	34	-7.95	0.03	1.91	
			Max. Vy	19	0.29	1.91	0.37	
			Max. Vx	34	0.32	0.03	1.91	
		Diagonal	Max Tension	20	9.52	0.00	0.00	
			Max. Compression	19	-9.68	0.00	0.00	
			Max. Mx	30	7.35	0.13	-0.01	
			Max. My	34	-5.99	0.07	0.03	
			Max. Vy	27	0.06	0.13	-0.02	
			Max. Vx	34	-0.00	0.00	0.00	
			Max Tension	32	166.90	-0.85	0.20	
T6	220 - 200	Leg	Max. Compression	30	-200.48	1.88	-0.03	
			Max. Mx	24	-197.86	1.88	-0.21	
			Max. My	34	-12.08	-0.11	2.63	
			Max. Vy	22	-1.58	-1.58	0.01	
			Max. Vx	26	-1.44	0.01	-0.78	
			Max Tension	20	13.07	0.00	0.00	
			Max. Compression	19	-13.32	0.00	0.00	
		Diagonal	Max. Mx	30	10.15	0.22	-0.03	
			Max. My	34	8.94	0.20	0.04	
			Max. Vy	33	0.08	0.21	0.04	
			Max. Vx	19	-0.01	0.00	0.00	
			Max Tension	32	207.01	-1.90	-0.00	
			Max. Compression	30	-250.20	2.40	-0.06	
			Max. Mx	24	-247.03	2.41	-0.22	
T7	200 - 180	Leg	Max. My	31	-13.42	-0.11	-2.77	
			Max. Vy	27	-1.73	-1.68	-0.24	
			Max. Vx	26	-1.53	0.06	-0.44	
			Max Tension	20	15.38	0.00	0.00	
			Max. Compression	20	-15.43	0.00	0.00	
			Diagonal	Max. Mx	30	10.15	0.22	-0.03
				Max. My	34	8.94	0.20	0.04
		Max. Vy		33	0.08	0.21	0.04	
		Max. Vx		19	-0.01	0.00	0.00	
		Max Tension		32	207.01	-1.90	-0.00	
		Max. Compression		30	-250.20	2.40	-0.06	
		Max. Mx		24	-247.03	2.41	-0.22	

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T8	180 - 160	Leg	Max. Mx	32	9.43	0.23	0.03
			Max. My	19	-14.74	0.11	0.04
			Max. Vy	32	0.08	0.23	0.03
			Max. Vx	19	-0.01	0.00	0.00
			Max Tension	32	248.31	-2.30	0.21
			Max. Compression	30	-300.55	3.01	-0.10
			Max. Mx	30	-300.55	3.01	-0.10
		Diagonal	Max. My	34	-20.85	0.10	2.71
			Max. Vy	22	0.38	-2.31	0.07
			Max. Vx	34	-0.63	-0.10	2.42
			Max Tension	20	17.16	0.00	0.00
			Max. Compression	20	-17.31	0.00	0.00
			Max. Mx	32	10.22	0.26	0.04
			Max. My	19	-16.78	0.16	0.04
T9	160 - 140	Leg	Max. Vy	32	0.09	0.26	0.04
			Max. Vx	19	-0.01	0.00	0.00
			Max Tension	32	290.55	-1.92	0.03
			Max. Compression	30	-352.34	3.67	-0.11
			Max. Mx	30	-352.34	3.67	-0.11
			Max. My	34	-23.64	-0.22	3.31
			Max. Vy	24	-0.40	3.65	-0.14
		Diagonal	Max. Vx	34	0.56	-0.22	3.31
			Max Tension	20	19.33	0.00	0.00
			Max. Compression	19	-19.90	0.00	0.00
			Max. Mx	30	13.85	0.43	-0.05
			Max. My	19	10.84	0.37	0.07
			Max. Vy	33	0.13	0.41	0.06
			Max. Vx	19	-0.01	0.00	0.00
T10	140 - 120	Leg	Max Tension	32	330.58	-2.49	0.05
			Max. Compression	30	-402.87	-3.29	0.05
			Max. Mx	30	-378.05	3.67	-0.11
			Max. My	34	-28.34	-0.85	5.62
			Max. Vy	24	0.74	2.39	-0.03
			Max. Vx	34	-0.70	-0.12	4.71
			Max Tension	20	19.72	0.00	0.00
		Diagonal	Max. Compression	19	-20.18	0.00	0.00
			Max. Mx	29	9.20	0.50	-0.07
			Max. My	34	13.69	0.44	0.09
			Max. Vy	29	-0.14	0.50	-0.07
			Max. Vx	34	-0.01	0.00	0.00
			Max Tension	32	336.67	1.50	0.03
			Max. Compression	30	-412.37	-13.83	-0.63
T11	120 - 100	Leg	Max. Mx	30	-411.56	17.30	0.55
			Max. My	34	-30.42	-2.09	11.00
			Max. Vy	30	3.38	17.30	0.55
			Max. Vx	26	1.94	-2.09	-10.97
			Max Tension	20	30.02	-0.20	0.06
			Max. Compression	19	-32.00	0.00	0.00
			Max. Mx	27	15.80	-0.26	0.06
		Diagonal	Max. My	27	15.80	-0.26	0.06
			Max. Vy	27	-0.07	-0.26	0.06
			Max. Vx	27	-0.00	0.00	0.00
			Max Tension	20	16.81	-0.21	0.00
			Max. Compression	19	-16.75	-0.26	-0.02
			Max. Mx	32	-2.78	-0.30	-0.03
			Max. My	24	-1.54	-0.13	0.04
Horizontal	Max. Vy	32	0.09	-0.30	-0.03		
	Max. Vx	24	-0.00	0.00	0.00		
	Max Tension	30	6.20	0.00	0.00		
	Redund Horz 1 Bracing	Max. Compression	30	-6.20	0.00	0.00	

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T12	100 - 80	Redund Diag 1 Bracing	Max. Mx	18	0.38	0.02	0.00	
			Max. Vy	18	0.01	0.00	0.00	
			Max Tension	30	5.63	0.00	0.00	
			Max. Compression	30	-5.63	0.00	0.00	
			Max. Mx	18	0.41	0.03	0.00	
			Max. Vy	18	-0.01	0.00	0.00	
			Inner Bracing	Max Tension	19	0.28	0.00	0.00
				Max. Compression	19	-0.30	0.00	0.00
				Max. Mx	18	-0.01	0.20	0.00
				Max. Vy	18	-0.06	0.00	0.00
				Max Tension	32	368.00	9.36	1.02
				Max. Compression	30	-452.69	-15.15	-0.60
		Leg	Max. Mx	30	-451.86	19.37	0.45	
			Max. My	34	-33.78	-2.31	11.63	
			Max. Vy	30	3.69	19.37	0.45	
			Max. Vx	26	-2.15	-2.09	-10.97	
			Diagonal	Max Tension	20	31.34	-0.21	0.06
				Max. Compression	19	-33.45	0.00	0.00
				Max. Mx	28	13.25	-0.27	0.06
				Max. My	28	13.25	-0.27	0.06
				Max. Vy	28	0.07	-0.27	0.06
				Max. Vx	28	0.01	0.00	0.00
			Horizontal	Max Tension	20	18.64	-0.25	0.00
				Max. Compression	19	-18.88	-0.30	-0.02
		Max. Mx		32	0.35	-0.33	-0.04	
		Max. My		24	3.79	-0.18	0.04	
		Max. Vy		32	-0.09	-0.33	-0.04	
		Max. Vx		24	-0.00	0.00	0.00	
		Redund Horz 1 Bracing	Max Tension	30	6.81	0.00	0.00	
			Max. Compression	30	-6.81	0.00	0.00	
Max. Mx	18		0.43	0.03	0.00			
Max. Vy	18		-0.01	0.00	0.00			
Redund Diag 1 Bracing	Max Tension		30	5.78	0.00	0.00		
	Max. Compression		30	-5.78	0.00	0.00		
	Max. Mx	18	0.42	0.05	0.00			
	Max. Vy	18	-0.02	0.00	0.00			
	Inner Bracing	Max Tension	19	0.33	0.00	0.00		
		Max. Compression	19	-0.34	0.00	0.00		
Max. Mx		18	0.00	0.25	0.00			
Max. Vy		18	-0.07	0.00	0.00			
Leg		Max Tension	32	401.20	10.19	1.34		
		Max. Compression	30	-495.64	-15.03	-0.90		
	Max. Mx	30	-494.68	23.14	0.73			
	Max. My	34	-37.58	-3.08	18.43			
	Max. Vy	30	-3.95	23.14	0.73			
	Max. Vx	34	-2.76	-3.08	18.43			
	Diagonal	Max Tension	20	29.24	-0.21	0.07		
		Max. Compression	19	-32.00	0.00	0.00		
		Max. Mx	27	19.64	-0.26	0.07		
		Max. My	27	16.38	-0.26	0.07		
		Max. Vy	27	-0.07	-0.26	0.07		
		Max. Vx	27	0.01	0.00	0.00		
Horizontal	Max Tension	20	18.44	-0.38	0.00			
	Max. Compression	19	-18.93	-0.42	-0.02			
	Max. Mx	32	-3.97	-0.45	-0.03			
	Max. My	24	3.99	-0.30	0.03			
	Max. Vy	32	0.13	-0.45	-0.03			
	Max. Vx	24	-0.00	0.00	0.00			

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T14	60 - 30	Redund Horz 1 Bracing	Max Tension	30	7.47	0.00	0.00	
			Max. Compression	30	-7.45	0.00	0.00	
			Max. Mx	18	0.57	0.04	0.00	
		Redund Horz 1 Bracing	Max. Vy	18	-0.02	0.00	0.00	
			Redund Diag 1 Bracing	Max Tension	30	5.96	0.00	0.00
				Max. Compression	30	-5.97	0.00	0.00
		Max. Mx		18	0.44	0.06	0.00	
		Inner Bracing	Max. Vy	18	-0.02	0.00	0.00	
			Leg	Max Tension	19	0.33	0.00	0.00
				Max. Compression	19	-0.34	0.00	0.00
		Max. Mx		18	0.00	0.29	0.00	
		Diagonal	Max. Vy	18	0.08	0.00	0.00	
			Max Tension	32	433.05	8.62	1.68	
			Max. Compression	30	-537.57	4.60	0.55	
			Max. Mx	30	-530.55	28.62	0.76	
			Max. My	34	-42.76	-1.66	22.50	
			Max. Vy	30	5.33	28.62	0.76	
			Max. Vx	26	3.10	-1.67	-22.48	
			Horizontal	Max Tension	20	42.80	-0.31	0.12
				Max. Compression	19	-46.21	0.00	0.00
				Max. Mx	32	31.64	-0.36	-0.12
				Max. My	32	31.64	-0.36	-0.12
				Max. Vy	19	-0.08	-0.15	-0.11
		Max. Vx		32	-0.01	0.00	0.00	
		Redund Horz 1 Bracing	Max Tension	20	21.79	-0.55	0.00	
			Max. Compression	20	-21.65	-0.55	0.00	
			Max. Mx	32	-2.74	-0.71	-0.04	
			Max. My	24	3.00	-0.38	0.04	
			Max. Vy	32	0.17	-0.71	-0.04	
			Max. Vx	24	-0.00	0.00	0.00	
		Redund Horz 2 Bracing	Max Tension	30	8.10	0.00	0.00	
			Max. Compression	30	-8.28	0.00	0.00	
			Max. Mx	18	0.54	0.02	0.00	
Redund Diag 1 Bracing	Max. Vy	18	-0.01	0.00	0.00			
	Max Tension	30	8.10	0.00	0.00			
	Max. Compression	30	-8.17	0.00	0.00			
Redund Diag 2 Bracing	Max. Mx	18	0.54	0.10	0.00			
	Max. Vy	18	0.04	0.00	0.00			
	Max Tension	30	8.35	0.00	0.00			
Redund Horz 1 Bracing	Max. Compression	30	-8.18	0.00	0.00			
	Max. Mx	18	0.82	0.03	0.00			
	Max. Vy	18	-0.01	0.00	0.00			
Redund Horz 2 Bracing	Max Tension	30	5.36	0.00	0.00			
	Max. Compression	30	-5.30	0.00	0.00			
	Max. Mx	18	0.47	0.10	0.00			
Inner Bracing	Max. Vy	18	-0.03	0.00	0.00			
	Max Tension	28	0.36	0.00	0.00			
	Max. Compression	28	-0.40	0.00	0.00			
Redund Horz 1 Bracing	Max. Mx	18	-0.02	0.34	0.00			
	Max. Vy	18	-0.08	0.00	0.00			
	Leg	Max Tension	32	481.28	19.72	2.64		
Max. Compression		30	-600.80	5.45	0.64			
Max. Mx		30	-597.32	25.52	1.05			
Redund Horz 2 Bracing	Max. My	34	-46.61	-1.66	22.50			
	Max. Vy	30	-3.00	5.45	0.64			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	37 of 51
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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
		Diagonal	Max. Vx	26	-3.03	-1.67	-22.48
			Max Tension	20	40.13	-0.29	0.13
		Horizontal	Max. Compression	20	-42.05	0.00	0.00
			Max. Mx	32	27.01	-0.36	-0.14
			Max. My	32	24.71	-0.36	-0.14
			Max. Vy	19	-0.08	-0.20	-0.13
			Max. Vx	32	-0.01	0.00	0.00
			Max Tension	20	21.71	-0.56	0.00
			Max. Compression	19	-23.05	-0.63	-0.03
			Max. Mx	32	-0.99	-0.69	-0.06
			Max. My	24	0.58	-0.41	0.06
			Max. Vy	32	-0.17	-0.69	-0.06
		Redund Horz 1 Bracing	Max. Vx	24	0.00	-0.41	0.06
			Max Tension	30	9.10	0.00	0.00
			Max. Compression	30	-9.07	0.00	0.00
		Redund Horz 2 Bracing	Max. Mx	18	0.66	0.02	0.00
			Max. Vy	18	-0.01	0.00	0.00
			Max Tension	30	9.07	0.00	0.00
		Redund Diag 1 Bracing	Max. Compression	30	-9.25	0.00	0.00
			Max. Mx	18	0.64	0.13	0.00
			Max. Vy	18	-0.04	0.00	0.00
		Redund Diag 2 Bracing	Max Tension	30	8.43	0.00	0.00
			Max. Compression	30	-8.46	0.00	0.00
			Max. Mx	18	0.69	0.04	0.00
		Inner Bracing	Max. Vy	18	-0.01	0.00	0.00
			Max Tension	30	5.82	0.00	0.00
			Max. Compression	30	-5.67	0.00	0.00
		Inner Bracing	Max. Mx	18	0.62	0.18	0.00
			Max. Vy	18	0.05	0.00	0.00
			Max Tension	19	0.40	0.00	0.00
		Inner Bracing	Max. Compression	19	-0.42	0.00	0.00
			Max. Mx	18	0.01	0.42	0.00
			Max. Vy	18	0.09	0.00	0.00

### Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg C	Max. Vert	30	656.18	68.02	-40.94
	Max. H <sub>x</sub>	30	656.18	68.02	-40.94
	Max. H <sub>z</sub>	21	-501.14	-53.84	36.60
	Min. Vert	22	-516.44	-57.30	34.49
	Min. H <sub>x</sub>	22	-516.44	-57.30	34.49
Leg B	Min. H <sub>z</sub>	29	606.53	61.08	-40.95
	Max. Vert	24	648.54	-69.87	-37.46
	Max. H <sub>x</sub>	32	-524.09	59.20	31.47
	Max. H <sub>z</sub>	33	-508.78	56.51	32.24
	Min. Vert	32	-524.09	59.20	31.47
Leg A	Min. H <sub>x</sub>	24	648.54	-69.87	-37.46
	Min. H <sub>z</sub>	24	648.54	-69.87	-37.46
	Max. Vert	19	655.66	-3.94	79.38
	Max. H <sub>x</sub>	30	-249.85	10.68	-33.37



<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 38 of 51
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Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Max. H <sub>z</sub>	19	655.66	-3.94	79.38
	Min. Vert	27	-517.33	3.57	-66.90
	Min. H <sub>x</sub>	22	336.65	-10.32	39.77
	Min. H <sub>z</sub>	27	-517.33	3.57	-66.90

### Tower Mast Reaction Summary

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Dead Only	108.28	-0.00	-0.00	-34.22	55.23	0.00
Dead+Wind 0 deg - No Ice	108.28	-0.00	-105.85	-17725.73	55.23	-174.59
Dead+Wind 30 deg - No Ice	108.28	50.75	-87.94	-14673.82	-8393.07	-132.36
Dead+Wind 45 deg - No Ice	108.28	71.29	-71.31	-11897.74	-11802.81	-99.91
Dead+Wind 60 deg - No Ice	108.28	86.71	-50.08	-8359.59	-14358.03	-61.15
Dead+Wind 90 deg - No Ice	108.28	101.51	-0.00	-34.22	-16841.38	24.87
Dead+Wind 120 deg - No Ice	108.28	91.63	52.92	8811.53	-15259.36	109.72
Dead+Wind 135 deg - No Ice	108.28	71.29	71.31	11829.29	-11802.81	134.85
Dead+Wind 150 deg - No Ice	108.28	50.75	87.94	14605.37	-8393.07	157.23
Dead+Wind 180 deg - No Ice	108.28	-0.00	100.16	16616.52	55.23	164.81
Dead+Wind 210 deg - No Ice	108.28	-50.75	87.94	14605.37	8503.53	132.36
Dead+Wind 225 deg - No Ice	108.28	-71.29	71.31	11829.29	11913.27	99.91
Dead+Wind 240 deg - No Ice	108.28	-91.63	52.92	8811.53	15369.82	64.86
Dead+Wind 270 deg - No Ice	108.28	-101.51	-0.00	-34.22	16951.83	-24.87
Dead+Wind 300 deg - No Ice	108.28	-86.71	-50.08	-8359.59	14468.49	-103.66
Dead+Wind 315 deg - No Ice	108.28	-71.29	-71.31	-11897.74	11913.27	-134.85
Dead+Wind 330 deg - No Ice	108.28	-50.75	-87.94	-14673.82	8503.53	-157.23
Dead+Ice+Temp	149.73	-0.00	-0.00	-73.21	155.49	0.00
Dead+Wind 0 deg+Ice+Temp	149.73	-0.00	-129.01	-21345.79	155.49	-272.42
Dead+Wind 30 deg+Ice+Temp	149.73	61.90	-107.25	-17704.87	-10019.87	-204.03
Dead+Wind 45 deg+Ice+Temp	149.73	86.94	-86.97	-14363.95	-14129.19	-152.40
Dead+Wind 60 deg+Ice+Temp	149.73	105.76	-61.08	-10104.22	-17211.30	-91.21
Dead+Wind 90 deg+Ice+Temp	149.73	123.80	-0.00	-73.21	-20195.23	43.76
Dead+Wind 120 deg+Ice+Temp	149.73	111.69	64.50	10563.07	-18259.68	175.76
Dead+Wind 135 deg+Ice+Temp	149.73	86.94	86.97	14217.52	-14129.19	213.84
Dead+Wind 150 deg+Ice+Temp	149.73	61.90	107.25	17558.44	-10019.87	247.79
Dead+Wind 180 deg+Ice+Temp	149.73	-0.00	122.16	19988.79	155.49	257.11
Dead+Wind 210 deg+Ice+Temp	149.73	-61.90	107.25	17558.44	10330.85	204.03
Dead+Wind 225 deg+Ice+Temp	149.73	-86.94	86.97	14217.52	14440.17	152.40
Dead+Wind 240 deg+Ice+Temp	149.73	-111.69	64.50	10563.07	18570.67	96.65
Dead+Wind 270 deg+Ice+Temp	149.73	-123.80	-0.00	-73.21	20506.22	-43.76
Dead+Wind 300 deg+Ice+Temp	149.73	-105.76	-61.08	-10104.22	17522.29	-165.90
Dead+Wind 315 deg+Ice+Temp	149.73	-86.94	-86.97	-14363.95	14440.17	-213.84
Dead+Wind 330 deg+Ice+Temp	149.73	-61.90	-107.25	-17704.87	10330.85	-247.79
Dead+Wind 0 deg - Service	108.28	-0.00	-105.85	-17725.73	55.23	-174.59
Dead+Wind 30 deg - Service	108.28	50.75	-87.94	-14673.82	-8393.07	-132.36
Dead+Wind 45 deg - Service	108.28	71.29	-71.31	-11897.74	-11802.81	-99.91
Dead+Wind 60 deg - Service	108.28	86.71	-50.08	-8359.59	-14358.03	-61.15
Dead+Wind 90 deg - Service	108.28	101.51	-0.00	-34.22	-16841.38	24.87
Dead+Wind 120 deg - Service	108.28	91.63	52.92	8811.53	-15259.36	109.72
Dead+Wind 135 deg - Service	108.28	71.29	71.31	11829.29	-11802.81	134.85
Dead+Wind 150 deg - Service	108.28	50.75	87.94	14605.37	-8393.07	157.23
Dead+Wind 180 deg - Service	108.28	-0.00	100.16	16616.52	55.23	164.81
Dead+Wind 210 deg - Service	108.28	-50.75	87.94	14605.37	8503.53	132.36
Dead+Wind 225 deg - Service	108.28	-71.29	71.31	11829.29	11913.27	99.91
Dead+Wind 240 deg - Service	108.28	-91.63	52.92	8811.53	15369.82	64.86
Dead+Wind 270 deg - Service	108.28	-101.51	-0.00	-34.22	16951.83	-24.87

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Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>y</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>y</sub> kip-ft	Torque kip-ft
Dead+Wind 300 deg - Service	108.28	-86.71	-50.08	-8359.59	14468.49	-103.66
Dead+Wind 315 deg - Service	108.28	-71.29	-71.31	-11897.74	11913.27	-134.85
Dead+Wind 330 deg - Service	108.28	-50.75	-87.94	-14673.82	8503.53	-157.23

### Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-108.28	0.00	0.00	108.28	0.00	0.000%
2	-0.00	-108.28	-105.85	0.00	108.28	105.85	0.000%
3	50.75	-108.28	-87.94	-50.75	108.28	87.94	0.000%
4	71.29	-108.28	-71.31	-71.29	108.28	71.31	0.000%
5	86.71	-108.28	-50.08	-86.71	108.28	50.08	0.000%
6	101.51	-108.28	0.00	-101.51	108.28	0.00	0.000%
7	91.63	-108.28	52.92	-91.63	108.28	-52.92	0.000%
8	71.29	-108.28	71.31	-71.29	108.28	-71.31	0.000%
9	50.75	-108.28	87.94	-50.75	108.28	-87.94	0.000%
10	0.00	-108.28	100.16	0.00	108.28	-100.16	0.000%
11	-50.75	-108.28	87.94	50.75	108.28	-87.94	0.000%
12	-71.29	-108.28	71.31	71.29	108.28	-71.31	0.000%
13	-91.63	-108.28	52.92	91.63	108.28	-52.92	0.000%
14	-101.51	-108.28	0.00	101.51	108.28	0.00	0.000%
15	-86.71	-108.28	-50.08	86.71	108.28	50.08	0.000%
16	-71.29	-108.28	-71.31	71.29	108.28	71.31	0.000%
17	-50.75	-108.28	-87.94	50.75	108.28	87.94	0.000%
18	0.00	-149.73	0.00	0.00	149.73	0.00	0.000%
19	-0.00	-149.73	-129.01	0.00	149.73	129.01	0.000%
20	61.90	-149.73	-107.25	-61.90	149.73	107.25	0.000%
21	86.94	-149.73	-86.97	-86.94	149.73	86.97	0.000%
22	105.76	-149.73	-61.08	-105.76	149.73	61.08	0.000%
23	123.80	-149.73	0.00	-123.80	149.73	0.00	0.000%
24	111.69	-149.73	64.50	-111.69	149.73	-64.50	0.000%
25	86.94	-149.73	86.97	-86.94	149.73	-86.97	0.000%
26	61.90	-149.73	107.25	-61.90	149.73	-107.25	0.000%
27	0.00	-149.73	122.16	0.00	149.73	-122.16	0.000%
28	-61.90	-149.73	107.25	61.90	149.73	-107.25	0.000%
29	-86.94	-149.73	86.97	86.94	149.73	-86.97	0.000%
30	-111.69	-149.73	64.50	111.69	149.73	-64.50	0.000%
31	-123.80	-149.73	0.00	123.80	149.73	0.00	0.000%
32	-105.76	-149.73	-61.08	105.76	149.73	61.08	0.000%
33	-86.94	-149.73	-86.97	86.94	149.73	86.97	0.000%
34	-61.90	-149.73	-107.25	61.90	149.73	107.25	0.000%
35	-0.00	-108.28	-105.85	0.00	108.28	105.85	0.000%
36	50.75	-108.28	-87.94	-50.75	108.28	87.94	0.000%
37	71.29	-108.28	-71.31	-71.29	108.28	71.31	0.000%
38	86.71	-108.28	-50.08	-86.71	108.28	50.08	0.000%
39	101.51	-108.28	0.00	-101.51	108.28	0.00	0.000%
40	91.63	-108.28	52.92	-91.63	108.28	-52.92	0.000%
41	71.29	-108.28	71.31	-71.29	108.28	-71.31	0.000%
42	50.75	-108.28	87.94	-50.75	108.28	-87.94	0.000%
43	0.00	-108.28	100.16	0.00	108.28	-100.16	0.000%
44	-50.75	-108.28	87.94	50.75	108.28	-87.94	0.000%
45	-71.29	-108.28	71.31	71.29	108.28	-71.31	0.000%
46	-91.63	-108.28	52.92	91.63	108.28	-52.92	0.000%
47	-101.51	-108.28	0.00	101.51	108.28	0.00	0.000%
48	-86.71	-108.28	-50.08	86.71	108.28	50.08	0.000%
49	-71.29	-108.28	-71.31	71.29	108.28	71.31	0.000%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
50	-50.75	-108.28	-87.94	50.75	108.28	87.94	0.000%

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	320 - 300	23.783	46	0.6262	0.3052
T2	300 - 280	21.111	46	0.6163	0.2443
T3	280 - 260	18.511	46	0.5887	0.1952
T4	260 - 240	16.031	46	0.5613	0.1674
T5	240 - 220	13.675	46	0.5267	0.1468
T6	220 - 200	11.493	46	0.4852	0.1342
T7	200 - 180	9.496	46	0.4378	0.1244
T8	180 - 160	7.669	35	0.3966	0.1139
T9	160 - 140	6.009	35	0.3519	0.1015
T10	140 - 120	4.550	35	0.3034	0.0909
T11	120 - 100	3.296	35	0.2523	0.0800
T12	100 - 80	2.283	35	0.2028	0.0621
T13	80 - 60	1.464	35	0.1531	0.0479
T14	60 - 30	0.841	40	0.1114	0.0358
T15	30 - 0	0.257	40	0.0499	0.0176

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
320.00	Dual Lights	46	23.783	0.6262	0.3052	258302
318.00	PD128	46	23.515	0.6257	0.2990	258302
315.00	8 FT DISH	46	23.112	0.6249	0.2898	258302
308.00	6 FT DISH	46	22.175	0.6222	0.2683	107626
294.00	DB224	46	20.320	0.6093	0.2269	51672
292.00	PD320	46	20.058	0.6066	0.2214	48446
285.00	DB809	46	19.150	0.5962	0.2052	39756
275.00	OGT9	46	17.879	0.5818	0.1868	37488
257.00	PD440	46	15.669	0.5567	0.1640	41129
250.00	PD128	46	14.835	0.5450	0.1563	32802
243.00	PD320	46	14.018	0.5323	0.1494	27266
220.00	DB844	46	11.493	0.4852	0.1342	25732
200.00	PiROD 12' Lightweight T-Frame	46	9.496	0.4378	0.1244	28349
175.00	6 FT DISH	35	7.237	0.3860	0.1108	28314
140.00	BA1012-0	35	4.550	0.3034	0.0909	25282
138.00	PD156S	35	4.415	0.2983	0.0900	24650
115.00	6 FT DISH	35	3.021	0.2399	0.0759	19347
112.00	2 FT DISH	35	2.863	0.2325	0.0732	20595
105.00	6 FT DISH	35	2.516	0.2153	0.0666	24226
100.00	PD458	35	2.283	0.2028	0.0621	26872
97.00	6 FT DISH	35	2.148	0.1952	0.0596	26975
90.00	4 FT DISH	35	1.850	0.1774	0.0544	25575

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**Maximum Tower Deflections - Design Wind**

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	320 - 300	28.639	30	0.7527	0.4156
T2	300 - 280	25.431	30	0.7408	0.3464
T3	280 - 260	22.307	30	0.7088	0.2866
T4	260 - 240	19.320	30	0.6766	0.2494
T5	240 - 220	16.479	30	0.6353	0.2204
T6	220 - 200	13.846	30	0.5855	0.2019
T7	200 - 180	11.437	30	0.5284	0.1873
T8	180 - 160	9.232	30	0.4786	0.1716
T9	160 - 140	7.229	30	0.4245	0.1538
T10	140 - 120	5.471	30	0.3658	0.1388
T11	120 - 100	3.961	30	0.3041	0.1232
T12	100 - 80	2.746	30	0.2441	0.0968
T13	80 - 60	1.765	19	0.1843	0.0748
T14	60 - 30	1.017	24	0.1340	0.0559
T15	30 - 0	0.316	24	0.0601	0.0275

**Critical Deflections and Radius of Curvature - Design Wind**

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
320.00	Dual Lights	30	28.639	0.7527	0.4156	223133
318.00	PD128	30	28.317	0.7521	0.4087	223133
315.00	8 FT DISH	30	27.834	0.7510	0.3984	223133
308.00	6 FT DISH	30	26.709	0.7477	0.3743	92972
294.00	DB224	30	24.482	0.7327	0.3255	45005
292.00	PD320	30	24.167	0.7295	0.3194	42281
285.00	DB809	30	23.076	0.7175	0.2994	34892
275.00	OGT9	30	21.547	0.7007	0.2756	32751
257.00	PD440	30	18.884	0.6711	0.2447	34777
250.00	PD128	30	17.878	0.6572	0.2339	27563
243.00	PD320	30	16.893	0.6421	0.2241	22818
220.00	DB844	30	13.846	0.5855	0.2019	21205
200.00	PiROD 12' Lightweight T-Frame	30	11.437	0.5284	0.1873	23649
175.00	6 FT DISH	30	8.711	0.4657	0.1672	23278
140.00	BA1012-0	30	5.471	0.3658	0.1388	20952
138.00	PD156S	30	5.308	0.3597	0.1375	20399
115.00	6 FT DISH	30	3.630	0.2890	0.1172	15835
112.00	2 FT DISH	30	3.442	0.2801	0.1133	16903
105.00	6 FT DISH	30	3.025	0.2592	0.1036	20061
100.00	PD458	30	2.746	0.2441	0.0968	22407
97.00	6 FT DISH	30	2.584	0.2349	0.0930	22548
90.00	4 FT DISH	30	2.227	0.2135	0.0850	21462

**Bolt Design Data**

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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T1	320	Leg	A325N	1.0000	6	2.45	34.56	0.071 ✓	1.333	Bolt Tension
T2	300	Leg	A325N	1.0000	8	5.37	34.56	0.155 ✓	1.333	Bolt Tension
T3	280	Leg	A325N	1.0000	8	8.84	34.56	0.256 ✓	1.333	Bolt Tension
T4	260	Leg	A325N	1.0000	8	12.62	34.56	0.365 ✓	1.333	Bolt Tension
T5	240	Leg	A325N	1.0000	8	16.67	34.56	0.482 ✓	1.333	Bolt Tension
T6	220	Leg	A325N	1.0000	12	13.91	34.56	0.402 ✓	1.333	Bolt Tension
T7	200	Leg	A325N	1.0000	12	17.25	34.56	0.499 ✓	1.333	Bolt Tension
T8	180	Leg	A325N	1.0000	12	20.69	34.56	0.599 ✓	1.333	Bolt Tension
T9	160	Leg	A325N	1.0000	12	24.21	34.56	0.701 ✓	1.333	Bolt Tension
T10	140	Leg	A325N	1.0000	12	27.55	34.56	0.797 ✓	1.333	Bolt Tension
T11	120	Leg	A325N	1.0000	12	27.98	34.55	0.810 ✓	1.333	Bolt Tension
		Horizontal	A325N	0.7500	2	8.40	9.28	0.906 ✓	1.333	Bolt Shear
T12	100	Leg	A325N	1.0000	16	22.92	34.56	0.663 ✓	1.333	Bolt Tension
		Horizontal	A325N	0.7500	2	9.44	9.28	1.018 ✓	1.333	Bolt Shear
T13	80	Leg	A325N	1.0000	16	25.02	34.56	0.724 ✓	1.333	Bolt Tension
		Horizontal	A325N	0.7500	2	9.47	9.28	1.020 ✓	1.333	Bolt Shear
T14	60	Leg	A325N	1.0000	16	26.54	34.55	0.768 ✓	1.333	Bolt Tension
		Horizontal	A325N	0.8750	2	10.90	12.63	0.863 ✓	1.333	Bolt Shear
T15	30	Leg	A325N	1.0000	24	19.83	34.56	0.574 ✓	1.333	Bolt Tension
		Horizontal	A325N	0.8750	2	11.53	12.63	0.913 ✓	1.333	Bolt Shear

**Compression Checks**

**Leg Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	320 - 300	ROHN 5 EH	20.00	4.00	26.1 K=1.00	27.622	6.1120	-18.74	168.82	0.111 ✓
T2	300 - 280	ROHN 6 EH	20.03	5.01	27.4 K=1.00	27.470	8.4049	-50.84	230.89	0.220 ✓
T3	280 - 260	ROHN 8 EH	20.04	6.68	27.9 K=1.00	27.414	12.7627	-83.19	349.88	0.238 ✓
T4	260 - 240	ROHN 8 EH	20.03	6.68	27.8 K=1.00	27.415	12.7627	-119.28	349.89	0.341 ✓
T5	240 - 220	ROHN 8 EH	20.03	6.68	27.8 K=1.00	27.415	12.7627	-158.39	349.89	0.453 ✓
T6	220 - 200	ROHN 8 EH	20.03	10.02	41.8 K=1.00	25.582	12.7627	-200.48	326.50	0.614 ✓

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Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T7	200 - 180	ROHN 10 EH	20.04	10.02	33.1 K=1.00	26.757	16.1007	-250.20	430.80	0.581
T8	180 - 160	ROHN 10 EH	20.04	10.02	33.1 K=1.00	26.757	16.1007	-300.55	430.81	0.698
T9	160 - 140	ROHN 10 EH	20.03	10.02	33.1 K=1.00	26.758	16.1007	-352.34	430.82	0.818
T10	140 - 120	ROHN 10 EH	20.04	10.02	33.1 K=1.00	26.756	16.1007	-402.87	430.79	0.935
T11	120 - 100	ROHN 10 EH	20.06	10.03	33.2 K=1.00	26.753	16.1007	-412.37	430.74	0.957
T12	100 - 80	ROHN 10 EH	20.05	10.03	33.2 K=1.00	26.753	16.1007	-452.69	430.75	1.051
T13	80 - 60	ROHN 12 EH	20.06	10.03	27.8 K=1.00	27.425	19.2423	-495.63	527.71	0.939
T14	60 - 30	ROHN 12 EH	30.07	10.02	27.8 K=1.00	27.426	19.2423	-537.57	527.74	1.019
T15	30 - 0	ROHN 12 EHS	30.08	10.03	28.0 K=1.00	27.392	23.8074	-600.80	652.14	0.921

### Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	320 - 300	L1 3/4x1 3/4x3/16	7.90	3.68	126.6 K=0.98	9.320	0.6211	-4.35	5.79	0.752
T2	300 - 280	L2x2x1/4	9.94	4.80	140.9 K=0.96	7.526	0.9380	-5.43	7.06	0.770
T3	280 - 260	L2 1/2x2 1/2x1/4	12.59	6.09	141.9 K=0.95	7.413	1.1900	-6.60	8.82	0.749
T4	260 - 240	L3x3x1/4	14.38	6.98	136.4 K=0.96	8.031	1.4400	-8.13	11.57	0.703
T5	240 - 220	L4x4x5/16	16.19	7.89	119.7 K=1.00	10.418	2.4000	-9.68	25.00	0.387
T6	220 - 200	L4x4x3/8	19.37	9.56	139.5 K=0.96	7.671	2.8600	-13.32	21.94	0.607
T7	200 - 180	L4x4x3/8	21.20	10.39	149.2 K=0.94	6.709	2.8600	-15.43	19.19	0.804
T8	180 - 160	L4x4x3/8	23.06	11.32	160.0 K=0.93	5.834	2.8600	-17.31	16.68	1.038
T9	160 - 140	L5x5x3/8	24.84	12.19	141.2 K=0.96	7.491	3.6100	-19.90	27.04	0.736
T10	140 - 120	L5x5x3/8	26.78	13.20	150.5 K=0.94	6.589	3.6100	-20.18	23.79	0.849
T11	120 - 100	ROHN 3 EH	24.42	23.63	124.8 K=0.50	9.588	3.0159	-32.00	28.92	1.107
T12	100 - 80	ROHN 3 EH	25.15	24.41	128.9 K=0.50	8.987	3.0159	-33.45	27.10	1.234
T13	80 - 60	ROHN 3 EH	25.98	25.15	132.8 K=0.50	8.467	3.0159	-32.00	25.54	1.253

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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T14	60 - 30	ROHN 3.5 EH	35.21	34.19	103.6 K=0.33	13.854	3.6784	-46.21	50.96	0.907
T15	30 - 0	ROHN 3.5 EH	36.27	35.32	107.0 K=0.33	13.033	3.6784	-42.05	47.94	0.877

### Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T11	120 - 100	ROHN 3 STD	25.39	12.25	126.3 K=1.00	9.361	2.2285	-16.75	20.86	0.803
T12	100 - 80	ROHN 3 STD	27.97	13.54	139.6 K=1.00	7.662	2.2285	-18.88	17.07	1.106
T13	80 - 60	ROHN 3 EH	30.47	14.79	156.2 K=1.00	6.124	3.0159	-18.93	18.47	1.025
T14	60 - 30	ROHN 3.5 EH	33.14	16.04	147.3 K=1.00	6.883	3.6784	-21.65	25.32	0.855
T15	30 - 0	ROHN 4 STD	36.80	17.87	142.0 K=1.00	7.401	3.1741	-23.05	23.49	0.981

### Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	320 - 300	L1 3/4x1 3/4x3/16	6.81	6.35	182.6 K=0.82	4.480	0.6211	-0.10	2.78	0.034
T2	300 - 280	L2x2x1/4	6.81	6.35	166.0 K=0.85	5.420	0.9380	-0.11	5.08	0.021

### Redundant Horizontal (1) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T11	120 - 100	ROHN 1.5 STD	6.35	5.90	113.7 K=1.00	11.550	0.7995	-6.20	9.23	0.672
T12	100 - 80	ROHN 1.5 STD	6.99	6.54	126.1 K=1.00	9.385	0.7995	-6.81	7.50	0.908
T13	80 - 60	ROHN 2 STD	7.62	7.09	108.0 K=1.00	12.795	1.0745	-7.45	13.75	0.542
T14	60 - 30	ROHN 1.5 STD	5.52	4.99	96.2 K=1.00	15.570	0.7995	-8.28	12.45	0.665

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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T15	30 - 0	ROHN 1.5 STD	6.13	5.60	108.0 K=1.00	12.809	0.7995	-9.07	10.24	0.886 ✓

### Redundant Horizontal (2) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T14	60 - 30	ROHN 2 EH	11.05	10.52	164.2 K=1.00	5.535	1.4807	-8.17	8.20	0.997 ✓
T15	30 - 0	ROHN 2 EH	12.27	11.74	183.3 K=1.00	4.444	1.4807	-9.25	6.58	1.406 X
		H1-3 (1.41 CR) - 331								

### Redundant Diagonal (1) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T11	120 - 100	ROHN 1.5 STD	11.52	10.61	204.6 K=1.00	3.568	0.7995	-5.63	2.85	1.973 X
T12	100 - 80	H1-3 (1.97 CR) - 211 ROHN 2 STD	11.86	11.03	168.1 K=1.00	5.283	1.0745	-5.78	5.68	1.017 ✓
T13	80 - 60	ROHN 2 STD	12.18	11.40	173.8 K=1.00	4.944	1.0745	-5.97	5.31	1.124 ✓
T14	60 - 30	ROHN 1.5 STD	11.15	9.95	191.7 K=1.00	4.064	0.7995	-8.18	3.25	2.516 X
T15	30 - 0	H1-3 (2.52 CR) - 293 ROHN 2 STD	11.41	10.31	157.2 K=1.00	6.046	1.0745	-8.46	6.50	1.303 ✓

### Redundant Diagonal (2) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T14	60 - 30	ROHN 2 STD	14.46	13.72	209.2 K=1.00	3.412	1.0745	-5.30	3.67	1.446 X
T15	30 - 0	H1-3 (1.45 CR) - 294 ROHN 2.5 STD	15.33	14.63	185.3 K=1.00	4.347	1.7040	-5.67	7.41	0.765 ✓



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**Inner Bracing Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T11	120 - 100	ROHN 3 STD	12.69	12.69	130.9 K=1.00	8.712	2.2285	-0.30	19.41	0.016
T12	100 - 80	ROHN 3 STD	13.99	13.99	144.2 K=1.00	7.179	2.2285	-0.34	16.00	0.021
T13	80 - 60	ROHN 3 STD	15.24	15.24	157.1 K=1.00	6.049	2.2285	-0.34	13.48	0.025
T14	60 - 30	ROHN 3 STD	16.57	16.57	170.9 K=1.00	5.114	2.2285	-0.40	11.40	0.035
T15	30 - 0	ROHN 3 STD	18.40	18.40	189.8 K=1.00	4.147	2.2285	-0.42	9.24	0.046

**Tension Checks**

**Leg Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	320 - 300	ROHN 5 EH	20.00	4.00	26.1	30.000	6.1120	14.72	183.36	0.080
T2	300 - 280	ROHN 6 EH	20.03	5.01	27.4	30.000	8.4049	42.92	252.15	0.170
T3	280 - 260	ROHN 8 EH	20.04	6.68	27.9	30.000	12.7627	70.68	382.88	0.185
T4	260 - 240	ROHN 8 EH	20.03	6.68	27.8	30.000	12.7627	100.97	382.88	0.264
T5	240 - 220	ROHN 8 EH	20.03	6.68	27.8	30.000	12.7627	133.35	382.88	0.348
T6	220 - 200	ROHN 8 EH	20.03	10.02	41.8	30.000	12.7627	166.90	382.88	0.436
T7	200 - 180	ROHN 10 EH	20.04	10.02	33.1	30.000	16.1007	207.01	483.02	0.429
T8	180 - 160	ROHN 10 EH	20.04	10.02	33.1	30.000	16.1007	248.31	483.02	0.514
T9	160 - 140	ROHN 10 EH	20.03	10.02	33.1	30.000	16.1007	290.55	483.02	0.602
T10	140 - 120	ROHN 10 EH	20.04	10.02	33.1	30.000	16.1007	330.58	483.02	0.684
T11	120 - 100	ROHN 10 EH	20.06	10.03	33.2	30.000	16.1007	336.67	483.02	0.697
T12	100 - 80	ROHN 10 EH	20.05	10.03	33.2	30.000	16.1007	368.00	483.02	0.762
T13	80 - 60	ROHN 12 EH	20.06	10.03	27.8	30.000	19.2423	401.20	577.27	0.695
T14	60 - 30	ROHN 12 EH	30.07	10.02	27.8	30.000	19.2423	433.05	577.27	0.750

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Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T15	30 - 0	ROHN 12 EHS	30.08	10.03	28.0	30.000	23.8074	481.28	714.22	0.674

### Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T1	320 - 300	L1 3/4x1 3/4x3/16	7.90	3.68	82.2	21.600	0.6211	4.28	13.42	0.319
T2	300 - 280	L2x2x1/4	9.94	4.80	94.6	21.600	0.9380	5.25	20.26	0.259
T3	280 - 260	L2 1/2x2 1/2x1/4	12.59	6.09	95.0	21.600	1.1900	6.53	25.70	0.254
T4	260 - 240	L3x3x1/4	14.38	6.98	90.0	32.500	1.0800	7.97	35.10	0.227
T5	240 - 220	L4x4x5/16	16.19	7.89	76.3	32.500	1.8000	9.52	58.50	0.163
T6	220 - 200	L4x4x3/8	19.37	9.56	93.3	32.500	2.1450	13.07	69.71	0.187
T7	200 - 180	L4x4x3/8	21.20	10.39	101.4	32.500	2.1450	15.38	69.71	0.221
T8	180 - 160	L4x4x3/8	23.06	11.32	110.5	32.500	2.1450	17.16	69.71	0.246
T9	160 - 140	L5x5x3/8	24.84	12.19	93.8	32.500	2.7075	19.33	87.99	0.220
T10	140 - 120	L5x5x3/8	26.78	13.20	101.6	32.500	2.7075	19.72	87.99	0.224
T11	120 - 100	ROHN 3 EH	24.42	23.63	249.6	30.000	3.0159	30.02	90.48	0.332
T12	100 - 80	ROHN 3 EH	25.15	24.41	257.8	30.000	3.0159	31.34	90.48	0.346
T13	80 - 60	ROHN 3 EH	25.98	25.15	265.6	30.000	3.0159	29.24	90.48	0.323
T14	60 - 30	ROHN 3.5 EH	35.21	34.19	314.0	30.000	3.6784	42.80	110.35	0.388
T15	30 - 0	ROHN 3.5 EH	36.27	35.32	324.4	30.000	3.6784	40.13	110.35	0.364

### Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T11	120 - 100	ROHN 3 STD	25.39	12.25	126.3	30.000	2.2285	16.81	66.85	0.251

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b>	320' Rohn SSVMW	<b>Page</b>	48 of 51
	<b>Project</b>	CSP Tower - Colchester, CT	<b>Date</b>	15:34:40 03/10/06
	<b>Client</b>	Cingular Wireless	<b>Designed by</b>	Jed Kiernan

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T12	100 - 80	ROHN 3 STD	27.97	13.54	139.6	30.000	2.2285	18.64	66.85	0.279
T13	80 - 60	ROHN 3 EH	30.47	14.79	156.2	30.000	3.0159	18.44	90.48	0.204
T14	60 - 30	ROHN 3.5 EH	33.14	16.04	147.3	30.000	3.6784	21.79	110.35	0.197
T15	30 - 0	ROHN 4 STD	36.80	17.87	142.0	30.000	3.1741	21.71	95.22	0.228

### Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	320 - 300	L1 3/4x1 3/4x3/16	6.81	6.35	141.8	21.600	0.6211	0.07	13.42	0.005
T2	300 - 280	L2x2x1/4	6.81	6.35	125.1	21.600	0.9380	0.09	20.26	0.004

### Redundant Horizontal (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T11	120 - 100	ROHN 1.5 STD	6.35	5.90	113.7	30.000	0.7995	6.20	23.98	0.259
T12	100 - 80	ROHN 1.5 STD	6.99	6.54	126.1	30.000	0.7995	6.81	23.98	0.284
T13	80 - 60	ROHN 2 STD	7.62	7.09	108.0	30.000	1.0745	7.47	32.24	0.232
T14	60 - 30	ROHN 1.5 STD	5.52	4.99	96.2	30.000	0.7995	8.10	23.98	0.338
T15	30 - 0	ROHN 1.5 STD	6.13	5.60	108.0	30.000	0.7995	9.10	23.98	0.379

### Redundant Horizontal (2) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T14	60 - 30	ROHN 2 EH	11.05	10.52	164.2	30.000	1.4807	8.10	44.42	0.182
T15	30 - 0	ROHN 2 EH	12.27	11.74	183.3	30.000	1.4807	9.07	44.42	0.204

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 49 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 15:34:40 03/10/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
H1-3 (1.41 CR) - 331										

### Redundant Diagonal (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T11	120 - 100	ROHN 1.5 STD	11.52	10.61	204.6	30.000	0.7995	5.63	23.98	0.235 ✓
T12	100 - 80	H1-3 (1.97 CR) - 228 ROHN 2 STD	11.86	11.03	168.1	30.000	1.0745	5.78	32.24	0.179 ✓
T13	80 - 60	ROHN 2 STD	12.18	11.40	173.8	30.000	1.0745	5.96	32.24	0.185 ✓
T14	60 - 30	ROHN 1.5 STD	11.15	9.95	191.7	30.000	0.7995	8.35	23.98	0.348 ✓
T15	30 - 0	H1-3 (2.52 CR) - 293 ROHN 2 STD	11.41	10.31	157.2	30.000	1.0745	8.43	32.24	0.262 ✓

### Redundant Diagonal (2) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T14	60 - 30	ROHN 2 STD	14.46	13.72	209.2	30.000	1.0745	5.36	32.24	0.166 ✓
T15	30 - 0	H1-3 (1.45 CR) - 294 ROHN 2.5 STD	15.33	14.63	185.3	30.000	1.7040	5.82	51.12	0.114 ✓

### Inner Bracing Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T11	120 - 100	ROHN 3 STD	12.69	12.69	130.9	30.000	2.2285	0.28	66.85	0.004 ✓
T12	100 - 80	ROHN 3 STD	13.99	13.99	144.2	30.000	2.2285	0.33	66.85	0.005 ✓
T13	80 - 60	ROHN 3 STD	15.24	15.24	157.1	30.000	2.2285	0.33	66.85	0.005 ✓
T14	60 - 30	ROHN 3 STD	16.57	16.57	170.9	30.000	2.2285	0.36	66.85	0.005 ✓
T15	30 - 0	ROHN 3 STD	18.40	18.40	189.8	30.000	2.2285	0.40	66.85	0.006 ✓

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 50 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 15:34:40 03/10/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section No.	Elevation ft	Size	L ft	L <sub>a</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
										✓

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P <sub>allow</sub> K	% Capacity	Pass Fail
T1	320 - 300	Leg	ROHN 5 EH	1	-18.74	225.04	8.3	Pass
T2	300 - 280	Leg	ROHN 6 EH	37	-50.84	307.77	16.5	Pass
T3	280 - 260	Leg	ROHN 8 EH	67	-83.19	466.39	17.8	Pass
T4	260 - 240	Leg	ROHN 8 EH	88	-119.28	466.41	25.6	Pass
T5	240 - 220	Leg	ROHN 8 EH	109	-158.39	466.41	34.0	Pass
T6	220 - 200	Leg	ROHN 8 EH	130	-200.48	435.22	46.1	Pass
T7	200 - 180	Leg	ROHN 10 EH	145	-250.20	574.26	43.6	Pass
T8	180 - 160	Leg	ROHN 10 EH	160	-300.55	574.26	52.3	Pass
T9	160 - 140	Leg	ROHN 10 EH	175	-352.34	574.29	61.4	Pass
T10	140 - 120	Leg	ROHN 10 EH	190	-402.87	574.25	70.2	Pass
T11	120 - 100	Leg	ROHN 10 EH	205	-412.37	574.17	71.8	Pass
T12	100 - 80	Leg	ROHN 10 EH	232	-452.69	574.19	78.8	Pass
T13	80 - 60	Leg	ROHN 12 EH	259	-495.63	703.44	70.5	Pass
T14	60 - 30	Leg	ROHN 12 EH	286	-537.57	703.48	76.4	Pass
T15	30 - 0	Leg	ROHN 12 EHS	325	-600.80	869.31	69.1	Pass
T1	320 - 300	Diagonal	L1 3/4x1 3/4x3/16	8	-4.35	7.72	56.4	Pass
T2	300 - 280	Diagonal	L2x2x1/4	47	-5.43	9.41	57.8	Pass
T3	280 - 260	Diagonal	L2 1/2x2 1/2x1/4	74	-6.60	11.76	56.2	Pass
T4	260 - 240	Diagonal	L3x3x1/4	95	-8.13	15.42	52.7	Pass
T5	240 - 220	Diagonal	L4x4x5/16	116	-9.68	33.33	29.0	Pass
T6	220 - 200	Diagonal	L4x4x3/8	137	-13.32	29.25	45.5	Pass
T7	200 - 180	Diagonal	L4x4x3/8	152	-15.43	25.58	60.3	Pass
T8	180 - 160	Diagonal	L4x4x3/8	167	-17.31	22.24	77.8	Pass
T9	160 - 140	Diagonal	L5x5x3/8	182	-19.90	36.05	55.2	Pass
T10	140 - 120	Diagonal	L5x5x3/8	197	-20.18	31.71	63.7	Pass
T11	120 - 100	Diagonal	ROHN 3 EH	223	-32.00	38.55	83.0	Pass
T12	100 - 80	Diagonal	ROHN 3 EH	250	-33.45	36.13	92.6	Pass
T13	80 - 60	Diagonal	ROHN 3 EH	277	-32.00	34.04	94.0	Pass
T14	60 - 30	Diagonal	ROHN 3.5 EH	312	-46.21	67.93	68.0	Pass
T15	30 - 0	Diagonal	ROHN 3.5 EH	351	-42.05	63.90	65.8	Pass
T11	120 - 100	Horizontal	ROHN 3 STD	222	-16.75	27.81	60.2	Pass
T12	100 - 80	Horizontal	ROHN 3 STD	249	-18.88	22.76	83.0	Pass
T13	80 - 60	Horizontal	ROHN 3 EH	276	-18.93	24.62	76.9	Pass
T14	60 - 30	Horizontal	ROHN 3.5 EH	311	-21.65	33.75	64.2	Pass
T15	30 - 0	Horizontal	ROHN 4 STD	350	-23.05	31.31	73.6	Pass
T1	320 - 300	Top Girt	L1 3/4x1 3/4x3/16	6	-0.10	3.71	2.6	Pass
T2	300 - 280	Top Girt	L2x2x1/4	42	-0.11	6.78	1.6	Pass
T11	120 - 100	Redund Horz 1 Bracing	ROHN 1.5 STD	210	-6.20	12.31	50.4	Pass
T12	100 - 80	Redund Horz 1 Bracing	ROHN 1.5 STD	237	-6.81	10.00	68.1	Pass
T13	80 - 60	Redund Horz 1 Bracing	ROHN 2 STD	264	-7.45	18.33	40.7	Pass
T14	60 - 30	Redund Horz 1 Bracing	ROHN 1.5 STD	291	-8.28	16.59	49.9	Pass
T15	30 - 0	Redund Horz 1 Bracing	ROHN 1.5 STD	330	-9.07	13.65	66.4	Pass
T14	60 - 30	Redund Horz 2 Bracing	ROHN 2 EH	319	-8.17	10.93	74.8	Pass
T15	30 - 0	Redund Horz 2 Bracing	ROHN 2 EH	331	-9.25	8.77	105.5	Fail ✗

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 51 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 15:34:40 03/10/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P <sub>allow</sub> K	% Capacity	Pass Fail	
T11	120 - 100	Bracing Redund Diag 1	ROHN 1.5 STD	211	-5.63	3.80	148.0	Fail X	
T12	100 - 80	Bracing Redund Diag 1	ROHN 2 STD	238	-5.78	7.57	76.3	Pass	
T13	80 - 60	Bracing Redund Diag 1	ROHN 2 STD	265	-5.97	7.08	84.3	Pass	
T14	60 - 30	Bracing Redund Diag 1	ROHN 1.5 STD	320	-8.18	4.33	188.8	Fail X	
T15	30 - 0	Bracing Redund Diag 1	ROHN 2 STD	332	-8.46	8.66	97.7	Pass	
T14	60 - 30	Bracing Redund Diag 2	ROHN 2 STD	294	-5.30	4.89	108.5	Fail X	
T15	30 - 0	Bracing Redund Diag 2	ROHN 2.5 STD	333	-5.67	9.87	57.4	Pass	
T11	120 - 100	Inner Bracing	ROHN 3 STD	231	-0.30	25.88	1.2	Pass	
T12	100 - 80	Inner Bracing	ROHN 3 STD	258	-0.34	21.33	1.6	Pass	
T13	80 - 60	Inner Bracing	ROHN 3 STD	285	-0.34	17.97	1.9	Pass	
T14	60 - 30	Inner Bracing	ROHN 3 STD	324	-0.40	15.19	2.6	Pass	
T15	30 - 0	Inner Bracing	ROHN 3 STD	363	-0.42	12.32	3.4	Pass	
							Summary		
							Leg (T12)	78.8	Pass
							Diagonal (T13)	94.0	Pass
							Horizontal (T12)	83.0	Pass
							Top Girt (T1)	2.6	Pass
							Redund Horz 1 Bracing (T12)	68.1	Pass
							Redund Horz 2 Bracing (T15)	105.5	Fail X
							Redund Diag 1 Bracing (T14)	188.8	Fail X
							Redund Diag 2 Bracing (T14)	108.5	Fail X
							Inner Bracing (T15)	3.4	Pass
							Bolt Checks	76.5	Pass
							<b>RATING =</b>	<b>188.8</b>	<b>Fail X</b>

# REINFORCED TOWER

## **ERI TOWER INPUT/OUTPUT SUMMARY**



**APPURTENANCES**

TYPE	ELEVATION	TYPE	ELEVATION
Dual Lights	320	Mounting Frame (Verizon)	220
PD128	320	DB948F8572E-M (Verizon)	220
6" Side Mount Standoff	320	DB948F8572E-M (Verizon)	220
PD128	318	DB844 (Verizon)	220
6" Side Mount Standoff	318	DB844 (Verizon)	220
6"8"x4" Pipe Mount	315	PIFOD 12' Lightweight T-Frame (Circular)	200
8 FT DISH	308	PIFOD 12' Lightweight T-Frame (Circular)	200
5"3"x4" Pipe Mount	308	PIFOD 12' Lightweight T-Frame (Circular)	200
5"3"x4" Pipe Mount	308	(4) 770.00 (Circular)	200
5"3"x4" Pipe Mount	308	(4) 770.00 (Circular)	200
6 FT DISH	308	(4) 770.00 (Circular)	200
6 FT DISH	308	(4) LFG21401 TMA (Circular)	200
6 FT DISH	308	(4) LFG21401 TMA (Circular)	200
DB824	294	(4) LFG21401 TMA (Circular)	200
6" Side Mount Standoff	292	(4) LFG13519 Diplexer (Circular)	200
PD320	292	(4) LFG13519 Diplexer (Circular)	200
6" Side Mount Standoff	285	5"3"x4" Pipe Mount	175
DB809	285	6 FT DISH (Reserved)	175
6" Side Mount Standoff	275	BA1015-0	140
6" Side Mount Standoff	275	PD6885-4	140
PD440	257	6" Side Mount Standoff	140
6" Side Mount Standoff	257	PD1565	138
PD128	250	3"4"x4" Pipe Mount	138
6" Side Mount Standoff	250	5"3"x4" Pipe Mount	115
PD320	243	6 FT DISH (Reserved)	115
6" Side Mount Standoff	243	3"4"x4" Pipe Mount	112
DB844 (Verizon)	220	2 FT DISH	112
DB844 (Verizon)	220	5"3"x4" Pipe Mount	105
DB948F8572E-M (Verizon)	220	6 FT DISH	105
DB948F8572E-M (Verizon)	220	6" Side Mount Standoff	100
DB844 (Verizon)	220	DB437	100
DB948F8572E-M (Verizon)	220	PD468	100
DB948F8572E-M (Verizon)	220	6 FT DISH	97
Mounting Frame (Verizon)	220	5"3"x4" Pipe Mount	97
Mounting Frame (Verizon)	220	4 FT DISH	90
	220	3"4"x4" Pipe Mount	90

**SYMBOL LIST**

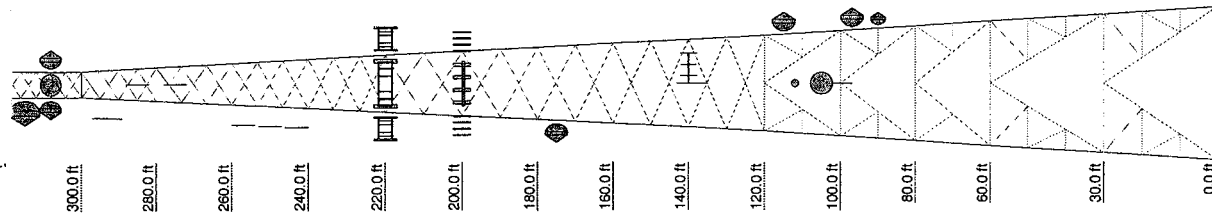
MARK	SIZE	MARK	SIZE
A	ROHN 5 EH	D	L2 1/2x2 1/2x1/4
B	ROHN 6 EH	E	ROHN 3 EH
C	L1 3/4x1 3/4x3/16	F	ROHN 2 STD

**MATERIAL STRENGTH**

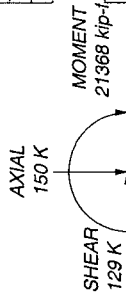
GRADE	FY	GRADE	FY
A572-50	50 ksi	A36	36 ksi
	50 ksi		58 ksi

**TOWER DESIGN NOTES**

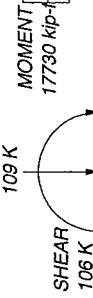
1. Tower designed for a 90 mph basic wind in accordance with the TIA/EIA-222-F Standard.
2. Tower is also designed for a 90 mph basic wind with 0.50 in ice.
3. Deflections are based upon a 90 mph wind.
4. TOWER RATING: 97.8%



MAX PIER FORCES:  
DOWN: 656 K  
UPLIFT: -524 K  
SHEAR: 80 K



TORQUE 272 kip-ft  
90 mph WIND - 0.5000 in ICE



TORQUE 174 kip-ft  
REACTIONS - 90 mph WIND

Section	Legs	Leg Grade	Diagonals	Diagonal Grade	Top Girts	Horizontal	Red. Horizontals	Red. Diagonals	Inner Bracing	Face Width (ft)	# Panels @ (ft)	Weight (K)
T1	ROHN 12 EH	A572-50	ROHN 3.5 EH	ROHN 3.5 EH	N.A.	ROHN 3 STD	ROHN 1.5 STD	ROHN 2 STD	ROHN 3 STD	40.69	2 @ 30	90.9
T2	ROHN 10 EH	A572-50	ROHN 3 EH	ROHN 3 EH	N.A.	ROHN 3 STD	ROHN 1.5 STD	ROHN 2 STD	ROHN 3 STD	30.47	3 @ 20	73
T3	ROHN 8 EH	A572-50	ROHN 3 EH	ROHN 3 EH	N.A.	ROHN 3 STD	ROHN 1.5 STD	ROHN 2 STD	ROHN 3 STD	33.14	2 @ 30	119
T4										36.8	2 @ 30	131
T5										36.8	2 @ 30	131
T6										36.8	2 @ 30	131
T7										36.8	2 @ 30	131
T8										36.8	2 @ 30	131
T9										36.8	2 @ 30	131
T10										36.8	2 @ 30	131
T11										36.8	2 @ 30	131
T12										36.8	2 @ 30	131
T13										36.8	2 @ 30	131
T14										36.8	2 @ 30	131
T15										36.8	2 @ 30	131

**URS Corporation**

500 Enterprise Drive, Suite 3B  
Rocky Hill, CT 06067  
Phone: (860) 529-8882  
FAX: (860) 529-3991

Job: 320' Rohm SSVMMW

Project: CSP Tower - Colchester, CT  
Client: Circular Wireless  
Code: TIA/EIA-222-F  
Date: 03/14/06  
Scale: NTS  
Dwg No.: E.

App'd: Jed Kiernan  
Drawn by: Jed Kiernan

## ERI TOWER DETAILED OUTPUT

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 1 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 08:00:06 03/14/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

## Tower Input Data

The main tower is a 3x free standing tower with an overall height of 320.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 6.81 ft at the top and 40.69 ft at the base.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Basic wind speed of 90 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 90 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 90 mph.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

## Options

- |  |  |  |
|--|--|--|
| <ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>Include Bolts In Member Capacity</li> <li>Leg Bolts Are At Top Of Section</li> <li>Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>Add IBC .6D+W Combination</li> </ul> | <ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>√ Use Clear Spans For Wind Area</li> <li>√ Use Clear Spans For KL/r</li> <li>Retension Guys To Initial Tension</li> <li>Bypass Mast Stability Checks</li> <li>Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>Autocalc Torque Arm Areas</li> <li>SR Members Have Cut Ends</li> <li>√ Sort Capacity Reports By Component</li> <li>Triangulate Diamond Inner Bracing</li> </ul> | <ul style="list-style-type: none"> <li>Treat Feedline Bundles As Cylinder</li> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>√ Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>SR Leg Bolts Resist Compression</li> <li>√ All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>√ Consider Feedline Torque</li> <li>Include Angle Block Shear Check</li> <li style="background-color: #cccccc;">Poles</li> <li>Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> </ul> |
|--|--|--|



<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 3 of 51
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	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Tower Section	Tower Elevation ft	Diagonal Spacing ft	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset in	Bottom Girt Offset in
T1	320.00-300.00	4.00	X Brace	No	No	0.0000	0.0000
T2	300.00-280.00	5.00	X Brace	No	No	0.0000	0.0000
T3	280.00-260.00	6.67	X Brace	No	No	0.0000	0.0000
T4	260.00-240.00	6.67	X Brace	No	No	0.0000	0.0000
T5	240.00-220.00	6.67	X Brace	No	No	0.0000	0.0000
T6	220.00-200.00	10.00	X Brace	No	No	0.0000	0.0000
T7	200.00-180.00	10.00	X Brace	No	No	0.0000	0.0000
T8	180.00-160.00	10.00	X Brace	No	No	0.0000	0.0000
T9	160.00-140.00	10.00	X Brace	No	No	0.0000	0.0000
T10	140.00-120.00	10.00	X Brace	No	No	0.0000	0.0000
T11	120.00-100.00	20.00	K1 Down	No	Yes	0.0000	0.0000
T12	100.00-80.00	20.00	K1 Down	No	Yes	0.0000	0.0000
T13	80.00-60.00	20.00	K1 Down	No	Yes	0.0000	0.0000
T14	60.00-30.00	30.00	K2 Down	No	Yes	0.0000	0.0000
T15	30.00-0.00	30.00	K2 Down	No	Yes	0.0000	0.0000

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 320.00-300.00	Pipe	ROHN 5 EH	A572-50 (50 ksi)	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T2 300.00-280.00	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Equal Angle	L2x2x1/4	A36 (36 ksi)
T3 280.00-260.00	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T4 260.00-240.00	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Equal Angle	L3x3x1/4	A572-50 (50 ksi)
T5 240.00-220.00	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Equal Angle	L4x4x5/16	A572-50 (50 ksi)
T6 220.00-200.00	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Equal Angle	L4x4x3/8	A572-50 (50 ksi)
T7 200.00-180.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Equal Angle	L4x4x3/8	A572-50 (50 ksi)
T8 180.00-160.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Equal Angle	L4x4x3/8	A572-50 (50 ksi)
T9 160.00-140.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Equal Angle	L5x5x3/8	A572-50 (50 ksi)
T10 140.00-120.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Equal Angle	L5x5x3/8	A572-50 (50 ksi)
T11 120.00-100.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Pipe	ROHN 3 EH	A572-50 (50 ksi)
T12 100.00-80.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Pipe	ROHN 3 EH	A572-50 (50 ksi)
T13 80.00-60.00	Pipe	ROHN 12 EH	A572-50 (50 ksi)	Pipe	ROHN 3 EH	A572-50 (50 ksi)
T14 60.00-30.00	Pipe	ROHN 12 EH	A572-50 (50 ksi)	Pipe	ROHN 3.5 EH	A572-50 (50 ksi)
T15 30.00-0.00	Pipe	ROHN 12 EHS	A572-50 (50 ksi)	Pipe	ROHN 3.5 EH	A572-50 (50 ksi)

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	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

**Tower Section Geometry (cont'd)**

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 320.00-300.00	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T2 300.00-280.00	Equal Angle	L2x2x1/4	A36 (36 ksi)	Solid Round		A36 (36 ksi)

**Tower Section Geometry (cont'd)**

Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T11 120.00-100.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T12 100.00-80.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T13 80.00-60.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 3 EH	A572-50 (50 ksi)
T14 60.00-30.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 3.5 EH	A572-50 (50 ksi)
T15 30.00-0.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 4 STD	A572-50 (50 ksi)

**Tower Section Geometry (cont'd)**

Tower Elevation ft	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
T11 120.00-100.00	Pipe		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T12 100.00-80.00	Pipe		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T13 80.00-60.00	Pipe		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T14 60.00-30.00	Pipe		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T15 30.00-0.00	Pipe		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)

**Tower Section Geometry (cont'd)**

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 5 of 51
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Tower Elevation	Redundant Bracing Grade	Redundant Type	Redundant Size	K Factor	
ft					
T11 120.00-100.00	A572-50 (50 ksi)	Horizontal (1) Diagonal (1)	Pipe Pipe	ROHN 1.5 STD ROHN 2 STD	1 1
T12 100.00-80.00	A572-50 (50 ksi)	Horizontal (1) Diagonal (1)	Pipe Pipe	ROHN 1.5 STD ROHN 2 STD	1 1
T13 80.00-60.00	A572-50 (50 ksi)	Horizontal (1) Diagonal (1)	Pipe Pipe	ROHN 2 STD ROHN 2 STD	1 1
T14 60.00-30.00	A572-50 (50 ksi)	Horizontal (1) Horizontal (2) Diagonal (1) Diagonal (2)	Pipe Pipe Pipe Pipe	ROHN 1.5 STD ROHN 2 EH ROHN 2 STD ROHN 2.5 STD	1 1 1 1
T15 30.00-0.00	A572-50 (50 ksi)	Horizontal (1) Horizontal (2) Diagonal (1) Diagonal (2)	Pipe Pipe Pipe Pipe	ROHN 1.5 STD ROHN 2.5 EH ROHN 2 STD ROHN 2.5 STD	1 1 1 1

### Tower Section Geometry (cont'd)

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor $A_f$	Adjust. Factor $A_r$	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft <sup>2</sup>	in					in	in
T1 320.00-300.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T2 300.00-280.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T3 280.00-260.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T4 260.00-240.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T5 240.00-220.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T6 220.00-200.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T7 200.00-180.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T8 180.00-160.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T9 160.00-140.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T10 140.00-120.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T11 120.00-100.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T12 100.00-80.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T13 80.00-60.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T14 60.00-30.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T15 30.00-0.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000

### Tower Section Geometry (cont'd)





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Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T6 220.00-200.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 200.00-180.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 180.00-160.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T9 160.00-140.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T10 140.00-120.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T11 120.00-100.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T12 100.00-80.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T13 80.00-60.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T14 60.00-30.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T15 30.00-0.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 320.00-300.00	Flange	1.0000 A325N	6	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T2 300.00-280.00	Flange	1.0000 A325N	8	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T3 280.00-260.00	Flange	1.0000 A325N	8	0.7500 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T4 260.00-240.00	Flange	1.0000 A325N	8	0.7500 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T5 240.00-220.00	Flange	1.0000 A325N	8	0.8750 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T6 220.00-200.00	Flange	1.0000 A325N	12	0.7500 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T7 200.00-180.00	Flange	1.0000 A325N	12	0.8750 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T8 180.00-160.00	Flange	1.0000 A325N	12	0.8750 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T9 160.00-140.00	Flange	1.0000 A325N	12	0.8750 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T10 140.00-120.00	Flange	1.0000 A325N	12	0.8750 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T11 120.00-100.00	Flange	1.0000 A325N	12	0.7500 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.7500 A325N	2	0.6250 A325N	0
T12 100.00-80.00	Flange	1.0000 A325N	16	0.7500 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.7500 A325N	2	0.6250 A325N	0

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Tower Elevation ft	Leg Connection Type	Leg Bolt Size in	Leg No.	Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
				Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T13 80.00-60.00	Flange	1.0000	16	0.7500	0	0.6250	0	0.6250	0	0.6250	0	0.7500	2	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T14 60.00-30.00	Flange	1.0000	16	0.8750	0	0.6250	0	0.6250	0	0.6250	0	0.8750	2	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T15 30.00-0.00	Flange	1.0000	24	0.8750	0	0.6250	0	0.6250	0	0.6250	0	0.8750	2	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	

### Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
1 5/8 (Verizon)	A	Yes	Ar (CfAe)	220.00 - 0.00	0.0000	-0.4	12	12	1.9800	1.9800		1.04
3/4 Electrical	C	Yes	Ar (CfAe)	320.00 - 0.00	0.0000	0.48	2	2	1.1100	1.1100		0.54
7/8	C	Yes	Ar (CfAe)	320.00 - 0.00	0.0000	0.46	3	3	1.1100	1.1100		0.54
EW63	C	Yes	Af (CfAe)	310.00 - 0.00	0.0000	0.44	3	3	1.5742	1.5742	5.0668	0.51
7/8	C	Yes	Ar (CfAe)	294.00 - 0.00	0.0000	0.42	2	2	1.1100	1.1100		0.54
1 5/8	C	Yes	Ar (CfAe)	280.00 - 0.00	0.0000	0.4	2	2	1.9800	1.9800		1.04
7/8	C	Yes	Ar (CfAe)	250.00 - 0.00	0.0000	0.38	3	3	1.1100	1.1100		0.54
7/8	C	Yes	Ar (CfAe)	138.00 - 0.00	0.0000	0.36	1	1	1.1100	1.1100		0.54
EW108	C	Yes	Af (CfAe)	112.00 - 0.00	0.0000	0.34	1	1	0.5899	0.5899	2.0063	0.15
EW65	C	Yes	Af (CfAe)	105.00 - 0.00	0.0000	0.32	1	1	1.5742	1.5742	5.0668	0.51
7/8	C	Yes	Ar (CfAe)	105.00 - 0.00	0.0000	0.3	2	2	1.1100	1.1100		0.54
7/8	C	Yes	Ar (CfAe)	95.00 - 0.00	0.0000	0.28	2	2	1.1100	1.1100		0.54
EW63 (Reserved)	A	Yes	Af (CfAe)	175.00 - 0.00	0.0000	-0.45	1	1	1.5742	1.5742	5.0668	0.51
EW63 (Reserved)	A	Yes	Af (CfAe)	115.00 - 0.00	0.0000	-0.47	1	1	1.5742	1.5742	5.0668	0.51
1 5/8 (Cingular)	B	Yes	Ar (CfAe)	200.00 - 0.00	0.0000	-0.42	24	12	1.9800	1.9800		1.04

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
T1	320.00-300.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	9.250	3.936	0.000	0.000	0.07
T2	300.00-280.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	11.840	7.871	0.000	0.000	0.10
T3	280.00-260.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	19.550	7.871	0.000	0.000	0.15
T4	260.00-240.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	22.325	7.871	0.000	0.000	0.16
T5	240.00-220.00	A	0.000	0.000	0.000	0.000	0.00

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	Project	CSP Tower - Colchester, CT	Date	08:00:06 03/14/06
	Client	Cingular Wireless	Designed by	Jed Kiernan

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
T6	220.00-200.00	B	0.000	0.000	0.000	0.000	0.00
		C	25.100	7.871	0.000	0.000	0.18
		A	39.600	0.000	0.000	0.000	0.25
T7	200.00-180.00	B	0.000	0.000	0.000	0.000	0.00
		C	25.100	7.871	0.000	0.000	0.18
		A	39.600	0.000	0.000	0.000	0.25
T8	180.00-160.00	B	39.600	0.000	0.000	0.000	0.50
		C	25.100	7.871	0.000	0.000	0.18
		A	39.600	1.968	0.000	0.000	0.26
T9	160.00-140.00	B	39.600	0.000	0.000	0.000	0.50
		C	25.100	7.871	0.000	0.000	0.18
		A	39.600	2.624	0.000	0.000	0.26
T10	140.00-120.00	B	39.600	0.000	0.000	0.000	0.50
		C	25.100	7.871	0.000	0.000	0.18
		A	39.600	2.624	0.000	0.000	0.26
T11	120.00-100.00	B	39.600	0.000	0.000	0.000	0.50
		C	26.765	7.871	0.000	0.000	0.19
		A	39.600	4.591	0.000	0.000	0.27
T12	100.00-80.00	B	39.600	0.000	0.000	0.000	0.50
		C	27.875	9.117	0.000	0.000	0.20
		A	39.600	5.247	0.000	0.000	0.27
T13	80.00-60.00	B	39.600	0.000	0.000	0.000	0.50
		C	33.425	11.478	0.000	0.000	0.24
		A	39.600	5.247	0.000	0.000	0.27
T14	60.00-30.00	B	39.600	0.000	0.000	0.000	0.50
		C	34.350	11.478	0.000	0.000	0.25
		A	59.400	7.871	0.000	0.000	0.41
T15	30.00-0.00	B	59.400	0.000	0.000	0.000	0.75
		C	51.525	17.217	0.000	0.000	0.37
		A	59.400	7.871	0.000	0.000	0.41

**Feed Line/Linear Appurtenances Section Areas - With Ice**

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
T1	320.00-300.00	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		17.583	5.602	0.000	0.000	0.21
T2	300.00-280.00	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		22.507	11.204	0.000	0.000	0.31
T3	280.00-260.00	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		34.550	11.204	0.000	0.000	0.43
T4	260.00-240.00	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		39.825	11.204	0.000	0.000	0.47
T5	240.00-220.00	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		45.100	11.204	0.000	0.000	0.52
T6	220.00-200.00	A	0.500	59.600	0.000	0.000	0.000	0.61
		B		0.000	0.000	0.000	0.000	0.00
		C		45.100	11.204	0.000	0.000	0.52
T7	200.00-180.00	A	0.500	59.600	0.000	0.000	0.000	0.61
		B		59.600	0.000	0.000	0.000	1.23

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 10 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 08:00:06 03/14/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
T8	180.00-160.00	C	0.500	45.100	11.204	0.000	0.000	0.52
		A		59.600	2.801	0.000	0.000	0.64
		B		59.600	0.000	0.000	0.000	1.23
T9	160.00-140.00	C	0.500	45.100	11.204	0.000	0.000	0.52
		A		59.600	3.735	0.000	0.000	0.65
		B		59.600	0.000	0.000	0.000	1.23
T10	140.00-120.00	C	0.500	45.100	11.204	0.000	0.000	0.52
		A		59.600	3.735	0.000	0.000	0.65
		B		59.600	0.000	0.000	0.000	1.23
T11	120.00-100.00	C	0.500	48.265	11.204	0.000	0.000	0.55
		A		59.600	6.536	0.000	0.000	0.68
		B		59.600	0.000	0.000	0.000	1.23
T12	100.00-80.00	C	0.500	50.375	13.395	0.000	0.000	0.58
		A		59.600	7.470	0.000	0.000	0.69
		B		59.600	0.000	0.000	0.000	1.23
T13	80.00-60.00	C	0.500	60.925	17.033	0.000	0.000	0.71
		A		59.600	7.470	0.000	0.000	0.69
		B		59.600	0.000	0.000	0.000	1.23
T14	60.00-30.00	C	0.500	62.683	17.033	0.000	0.000	0.73
		A		89.400	11.204	0.000	0.000	1.03
		B		89.400	0.000	0.000	0.000	1.84
T15	30.00-0.00	C	0.500	94.025	25.550	0.000	0.000	1.09
		A		89.400	11.204	0.000	0.000	1.03
		B		89.400	0.000	0.000	0.000	1.84
		C		94.025	25.550	0.000	0.000	1.09

### Feed Line Shielding

Section	Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>R</sub> Ice ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	A <sub>F</sub> Ice ft <sup>2</sup>
T1	320.00-300.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	1.211	3.047
T2	300.00-280.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	1.727	4.132
T3	280.00-260.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	2.066	4.526
T4	260.00-240.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	2.589	5.523
T5	240.00-220.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	3.649	7.485
T6	220.00-200.00	A	0.000	0.000	3.109	5.460
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	2.589	5.310
T7	200.00-180.00	A	0.000	0.000	3.015	5.294
		B	0.000	0.000	3.015	5.294
		C	0.000	0.000	2.510	5.149
T8	180.00-160.00	A	0.000	0.000	3.091	5.449
		B	0.000	0.000	2.945	5.170
		C	0.000	0.000	2.452	5.029
T9	160.00-140.00	A	0.000	0.000	3.858	6.616
		B	0.000	0.000	3.618	6.172

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	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 08:00:06 03/14/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section	Elevation	Face	$A_R$	$A_R$	$A_F$	$A_F$
	ft		ft <sup>2</sup>	Ice ft <sup>2</sup>	ft <sup>2</sup>	Ice ft <sup>2</sup>
T10	140.00-120.00	C	0.000	0.000	3.013	6.003
		A	0.000	0.000	3.805	6.526
		B	0.000	0.000	3.569	6.087
		C	0.000	0.000	3.121	6.244
T11	120.00-100.00	A	2.999	6.165	0.000	0.000
		B	2.688	5.475	0.000	0.000
		C	2.511	6.055	0.000	0.000
T12	100.00-80.00	A	2.913	5.993	0.000	0.000
		B	2.572	5.239	0.000	0.000
		C	2.917	7.097	0.000	0.000
T13	80.00-60.00	A	2.899	5.918	0.000	0.000
		B	2.560	5.173	0.000	0.000
		C	2.963	7.160	0.000	0.000
T14	60.00-30.00	A	4.723	9.612	0.000	0.000
		B	4.171	8.402	0.000	0.000
		C	4.827	11.630	0.000	0.000
T15	30.00-0.00	A	4.693	9.458	0.000	0.000
		B	4.144	8.267	0.000	0.000
		C	4.796	11.443	0.000	0.000

### Feed Line Center of Pressure

Section	Elevation	$CP_x$	$CP_z$	$CP_x$	$CP_z$
	ft	in	in	Ice in	Ice in
T1	320.00-300.00	-4.3846	2.8159	-5.6631	3.6242
T2	300.00-280.00	-6.2629	4.0917	-8.0736	5.2614
T3	280.00-260.00	-8.6123	5.7929	-11.3313	7.5878
T4	260.00-240.00	-10.0462	6.8193	-13.4219	9.0887
T5	240.00-220.00	-10.3624	7.0868	-14.1930	9.6963
T6	220.00-200.00	-25.3004	13.8166	-31.3392	17.3390
T7	200.00-180.00	-20.8086	-1.9859	-25.7687	-1.2958
T8	180.00-160.00	-23.2764	-1.7646	-28.7614	-0.9837
T9	160.00-140.00	-23.1016	-1.6036	-29.0278	-0.8473
T10	140.00-120.00	-24.9157	-1.3025	-31.4341	-0.3099
T11	120.00-100.00	-33.3821	-0.4257	-39.4094	1.1181
T12	100.00-80.00	-37.9500	2.1221	-44.7665	4.4919
T13	80.00-60.00	-38.3178	2.4115	-45.9571	4.9716
T14	60.00-30.00	-41.0715	2.5977	-49.4034	5.3502
T15	30.00-0.00	-44.0946	2.8023	-53.2386	5.7802

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	$C_{AA}$ Front ft <sup>2</sup>	$C_{AA}$ Side ft <sup>2</sup>	Weight K	
Dual Lights	C	None		0.0000	320.00	No Ice 1/2" Ice	4.00 4.80	4.00 4.80	0.25 0.40

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	<b>Project</b>	CSP Tower - Colchester, CT	<b>Date</b>	08:00:06 03/14/06
	<b>Client</b>	Cingular Wireless	<b>Designed by</b>	Jed Kiernan

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>A</sub>		Weight
			Horz	Lateral			Front	Side	
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
PD128	C	From Leg	6.00	0.00	0.0000	320.00	No Ice	1.00	0.01
			0.00	0.00			1/2" Ice	1.80	0.02
6' Side Mount Standoff	C	None			0.0000	320.00	No Ice	6.50	0.10
							1/2" Ice	8.50	0.17
PD128	C	From Leg	6.00	0.00	0.0000	318.00	No Ice	1.00	0.01
			0.00	0.00			1/2" Ice	1.80	0.02
6' Side Mount Standoff	C	None			0.0000	318.00	No Ice	6.50	0.10
							1/2" Ice	8.50	0.17
6'8"x4" Pipe Mount	C	From Leg	0.50	0.00	0.0000	315.00	No Ice	2.60	0.07
			0.00	0.00			1/2" Ice	3.01	0.09
5'3"x4" Pipe Mount	A	From Leg	0.50	0.00	0.0000	308.00	No Ice	1.88	0.06
			0.00	0.00			1/2" Ice	2.21	0.07
5'3"x4" Pipe Mount	B	From Leg	0.50	0.00	0.0000	308.00	No Ice	1.88	0.06
			0.00	0.00			1/2" Ice	2.21	0.07
5'3"x4" Pipe Mount	C	From Leg	0.50	0.00	0.0000	308.00	No Ice	1.88	0.06
			0.00	0.00			1/2" Ice	2.21	0.07
DB224	C	From Leg	6.00	0.00	0.0000	294.00	No Ice	3.15	0.03
			0.00	0.00			1/2" Ice	5.67	0.04
6' Side Mount Standoff	C	None			0.0000	294.00	No Ice	6.50	0.10
							1/2" Ice	8.50	0.17
PD320	C	From Leg	6.00	0.00	0.0000	292.00	No Ice	2.25	0.03
			0.00	0.00			1/2" Ice	4.05	0.04
6' Side Mount Standoff	C	None			0.0000	292.00	No Ice	6.50	0.10
							1/2" Ice	8.50	0.17
DB809	A	From Leg	6.00	0.00	0.0000	285.00	No Ice	3.39	0.03
			0.00	0.00			1/2" Ice	4.55	0.06
6' Side Mount Standoff	A	None			0.0000	285.00	No Ice	6.50	0.10
							1/2" Ice	8.50	0.17
OGT9	A	From Leg	6.00	0.00	0.0000	275.00	No Ice	3.15	0.03
			0.00	0.00			1/2" Ice	5.67	0.04
6' Side Mount Standoff	A	None			0.0000	275.00	No Ice	6.50	0.10
							1/2" Ice	8.50	0.17
PD440	C	From Leg	6.00	0.00	0.0000	257.00	No Ice	1.38	0.02
			0.00	0.00			1/2" Ice	2.48	0.02
6' Side Mount Standoff	C	None			0.0000	257.00	No Ice	6.50	0.10
							1/2" Ice	8.50	0.17
PD128	C	From Leg	6.00	0.00	0.0000	250.00	No Ice	1.00	0.01
			0.00	0.00			1/2" Ice	1.80	0.02
6' Side Mount Standoff	C	None			0.0000	250.00	No Ice	6.50	0.10
							1/2" Ice	8.50	0.17
PD320	C	From Leg	6.00	0.00	0.0000	243.00	No Ice	2.25	0.03
			0.00	0.00			1/2" Ice	4.05	0.04
6' Side Mount Standoff	C	None			0.0000	243.00	No Ice	6.50	0.10
							1/2" Ice	8.50	0.17

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	<b>Client</b>	Cingular Wireless	<b>Designed by</b>	Jed Kiernan

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz	Lateral					
DB844 (Verizon)	A	From Leg	5.00	0.0000	220.00	No Ice	3.06	3.73	0.01
			6.00			1/2" Ice	3.39	4.10	0.04
			0.00						
DB844 (Verizon)	A	From Leg	5.00	0.0000	220.00	No Ice	3.06	3.73	0.01
			-6.00			1/2" Ice	3.39	4.10	0.04
			0.00						
DB948F85T2E-M (Verizon)	A	From Leg	5.00	0.0000	220.00	No Ice	1.92	3.26	0.01
			4.00			1/2" Ice	2.22	3.62	0.03
			0.00						
DB948F85T2E-M (Verizon)	A	From Leg	5.00	0.0000	220.00	No Ice	1.92	3.26	0.01
			-4.00			1/2" Ice	2.22	3.62	0.03
			0.00						
DB844 (Verizon)	B	From Leg	5.00	0.0000	220.00	No Ice	3.06	3.73	0.01
			6.00			1/2" Ice	3.39	4.10	0.04
			0.00						
DB844 (Verizon)	B	From Leg	5.00	0.0000	220.00	No Ice	3.06	3.73	0.01
			-6.00			1/2" Ice	3.39	4.10	0.04
			0.00						
DB948F85T2E-M (Verizon)	B	From Leg	5.00	0.0000	220.00	No Ice	1.92	3.26	0.01
			4.00			1/2" Ice	2.22	3.62	0.03
			0.00						
DB948F85T2E-M (Verizon)	B	From Leg	5.00	0.0000	220.00	No Ice	1.92	3.26	0.01
			-4.00			1/2" Ice	2.22	3.62	0.03
			0.00						
DB844 (Verizon)	C	From Leg	5.00	0.0000	220.00	No Ice	3.06	3.73	0.01
			6.00			1/2" Ice	3.39	4.10	0.04
			0.00						
DB844 (Verizon)	C	From Leg	5.00	0.0000	220.00	No Ice	3.06	3.73	0.01
			-6.00			1/2" Ice	3.39	4.10	0.04
			0.00						
DB948F85T2E-M (Verizon)	C	From Leg	5.00	0.0000	220.00	No Ice	1.92	3.26	0.01
			4.00			1/2" Ice	2.22	3.62	0.03
			0.00						
DB948F85T2E-M (Verizon)	C	From Leg	5.00	0.0000	220.00	No Ice	1.92	3.26	0.01
			-4.00			1/2" Ice	2.22	3.62	0.03
			0.00						
Mounting Frame (Verizon)	A	From Leg	5.00	0.0000	220.00	No Ice	17.00	17.00	0.56
			0.00			1/2" Ice	20.00	20.00	0.70
			0.00						
Mounting Frame (Verizon)	B	From Leg	5.00	0.0000	220.00	No Ice	17.00	17.00	0.56
			0.00			1/2" Ice	20.00	20.00	0.70
			0.00						
Mounting Frame (Verizon)	C	From Leg	5.00	0.0000	220.00	No Ice	17.00	17.00	0.56
			0.00			1/2" Ice	20.00	20.00	0.70
			0.00						
PD156S	A	From Leg	1.00	0.0000	138.00	No Ice	0.44	0.44	0.01
			0.00			1/2" Ice	0.79	0.79	0.01
			0.00						
3'4"x4" Pipe Mount	A	From Leg	0.50	0.0000	138.00	No Ice	1.05	1.05	0.04
			0.00			1/2" Ice	1.27	1.27	0.05
			0.00						
3'4"x4" Pipe Mount	A	From Leg	0.50	0.0000	112.00	No Ice	1.05	1.05	0.04
			0.00			1/2" Ice	1.27	1.27	0.05
			0.00						
5'3"x4" Pipe Mount	A	From Leg	0.50	0.0000	105.00	No Ice	1.88	1.88	0.06
			0.00			1/2" Ice	2.21	2.21	0.07
			0.00						

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	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 08:00:06 03/14/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz	Vert						
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
PD458	A	From Leg	6.00	0.00	0.0000	100.00	No Ice 1/2" Ice	2.88 4.34	2.88 4.34	0.02 0.05
DB437	A	From Leg	6.00	0.00	0.0000	100.00	No Ice 1/2" Ice	0.45 0.81	0.45 0.81	0.01 0.01
6' Side Mount Standoff	A	None			0.0000	100.00	No Ice 1/2" Ice	6.50 8.50	6.50 8.50	0.10 0.17
5'3"x4" Pipe Mount	B	From Leg	0.50	0.00	0.0000	97.00	No Ice 1/2" Ice	1.88 2.21	1.88 2.21	0.06 0.07
3'4"x4" Pipe Mount	B	From Leg	0.50	0.00	0.0000	90.00	No Ice 1/2" Ice	1.05 1.27	1.05 1.27	0.04 0.05
5'3"x4" Pipe Mount	C	From Leg	0.50	0.00	0.0000	175.00	No Ice 1/2" Ice	1.88 2.21	1.88 2.21	0.06 0.07
5'3"x4" Pipe Mount	B	From Leg	0.50	0.00	0.0000	115.00	No Ice 1/2" Ice	1.88 2.21	1.88 2.21	0.06 0.07
BA1012-0	A	From Leg	6.00	0.00	0.0000	140.00	No Ice 1/2" Ice	0.47 0.96	0.47 0.96	0.00 0.01
PD688S-4	A	From Leg	6.00	0.00	0.0000	140.00	No Ice 1/2" Ice	0.35 0.63	0.35 0.63	0.00 0.00
6' Side Mount Standoff	A	None			0.0000	140.00	No Ice 1/2" Ice	6.50 8.50	6.50 8.50	0.10 0.17
PiROD 12' Lightweight T-Frame (Cingular)	A	None			0.0000	200.00	No Ice 1/2" Ice	10.20 16.20	10.20 16.20	0.25 0.35
PiROD 12' Lightweight T-Frame (Cingular)	B	None			0.0000	200.00	No Ice 1/2" Ice	10.20 16.20	10.20 16.20	0.25 0.35
PiROD 12' Lightweight T-Frame (Cingular)	C	None			0.0000	200.00	No Ice 1/2" Ice	10.20 16.20	10.20 16.20	0.25 0.35
(4) 7770.00 (Cingular)	A	From Leg	3.00	0.00	0.0000	200.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
(4) 7770.00 (Cingular)	B	From Leg	3.00	0.00	0.0000	200.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
(4) 7770.00 (Cingular)	C	From Leg	3.00	0.00	0.0000	200.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
(4) LPG21401 TMA (Cingular)	A	From Leg	3.00	0.00	0.0000	200.00	No Ice 1/2" Ice	0.95 1.09	0.37 0.48	0.02 0.02
(4) LPG21401 TMA (Cingular)	B	From Leg	3.00	0.00	0.0000	200.00	No Ice 1/2" Ice	0.95 1.09	0.37 0.48	0.02 0.02
(4) LPG21401 TMA (Cingular)	C	From Leg	3.00	0.00	0.0000	200.00	No Ice 1/2" Ice	0.95 1.09	0.37 0.48	0.02 0.02
(4) LPG13519 Diplexer (Cingular)	A	From Leg	3.00	0.00	0.0000	200.00	No Ice 1/2" Ice	0.27 0.34	0.18 0.25	0.01 0.01



<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b>	320' Rohn SSVMW	<b>Page</b>	15 of 51
	<b>Project</b>	CSP Tower - Colchester, CT	<b>Date</b>	08:00:06 03/14/06
	<b>Client</b>	Cingular Wireless	<b>Designed by</b>	Jed Kiernan

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz	Lateral						
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
(4) LPG13519 Diplexer (Cingular)	A	From Leg	0.00		0.0000	200.00	No Ice	0.27	0.18	0.01
			3.00				1/2" Ice	0.34	0.25	0.01
			0.00							
(4) LPG13519 Diplexer (Cingular)	A	From Leg	0.00		0.0000	200.00	No Ice	0.27	0.18	0.01
			3.00				1/2" Ice	0.34	0.25	0.01
			0.00							

### Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets:		Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weight	
				Horz	Lateral							
				ft	ft	°	°	ft	ft	ft <sup>2</sup>	K	
8 FT DISH	C	Paraboloid w/Radome	From Leg	1.00		Worst		315.00	8.00	No Ice	50.30	0.25
				0.00						1/2" Ice	51.29	0.51
				0.00								
6 FT DISH	A	Paraboloid w/Radome	From Leg	1.00		Worst		308.00	6.00	No Ice	28.27	0.14
				0.00						1/2" Ice	29.05	0.29
				0.00								
6 FT DISH	B	Paraboloid w/Radome	From Leg	1.00		Worst		308.00	6.00	No Ice	28.27	0.14
				0.00						1/2" Ice	29.05	0.29
				0.00								
6 FT DISH	C	Paraboloid w/Radome	From Leg	1.00		Worst		308.00	6.00	No Ice	28.27	0.14
				0.00						1/2" Ice	29.05	0.29
				0.00								
2 FT DISH	A	Paraboloid w/Radome	From Leg	1.00		Worst		112.00	2.00	No Ice	3.14	0.03
				0.00						1/2" Ice	3.41	0.04
				0.00								
6 FT DISH	A	Paraboloid w/Radome	From Leg	1.00		Worst		105.00	6.00	No Ice	28.27	0.14
				0.00						1/2" Ice	29.05	0.29
				0.00								
6 FT DISH	B	Paraboloid w/Radome	From Leg	1.00		Worst		97.00	6.00	No Ice	28.27	0.14
				0.00						1/2" Ice	29.05	0.29
				0.00								
4 FT DISH	B	Paraboloid w/Radome	From Leg	1.00		Worst		90.00	4.00	No Ice	12.56	0.17
				0.00						1/2" Ice	13.09	0.24
				0.00								
6 FT DISH (Reserved)	C	Paraboloid w/Radome	From Leg	1.00		Worst		175.00	6.00	No Ice	28.27	0.14
				0.00						1/2" Ice	29.05	0.29
				0.00								
6 FT DISH (Reserved)	B	Paraboloid w/Radome	From Leg	1.00		Worst		115.00	6.00	No Ice	28.27	0.14
				0.00						1/2" Ice	29.05	0.29
				0.00								

### Tower Pressures - No Ice

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 16 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 08:00:06 03/14/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

$$G_H = 1.084$$

Section Elevation ft	z ft	K <sub>Z</sub>	q <sub>z</sub> psf	A <sub>G</sub> ft <sup>2</sup>	F a c e	A <sub>F</sub> ft <sup>2</sup>	A <sub>R</sub> ft <sup>2</sup>	A <sub>leg</sub> ft <sup>2</sup>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>
T1 320.00-300.00	310.00	1.897	39	145.472	A	11.659	18.543	18.543	61.40	0.000	0.000
					B	11.659	18.543	61.40			
					C	14.384	27.793	43.97			
T2 300.00-280.00	290.00	1.861	39	167.656	A	12.596	22.122	22.122	63.72	0.000	0.000
					B	12.596	22.122	63.72			
					C	18.740	33.962	41.97			
T3 280.00-260.00	270.00	1.823	38	213.297	A	13.934	28.807	28.807	67.40	0.000	0.000
					B	13.934	28.807	67.40			
					C	19.739	48.357	42.30			
T4 260.00-240.00	250.00	1.783	37	255.594	A	19.443	28.800	28.800	59.70	0.000	0.000
					B	19.443	28.800	59.70			
					C	24.725	51.125	37.97			
T5 240.00-220.00	230.00	1.741	36	296.093	A	29.581	28.798	28.798	49.33	0.000	0.000
					B	29.581	28.798	49.33			
					C	33.803	53.898	32.84			
T6 220.00-200.00	210.00	1.697	35	336.193	A	21.026	68.398	28.798	32.20	0.000	0.000
					B	24.136	28.798	54.40			
					C	29.418	53.898	34.56			
T7 200.00-180.00	190.00	1.649	34	381.042	A	23.330	75.501	35.901	36.33	0.000	0.000
					B	23.330	75.501	36.33			
					C	31.706	61.001	38.73			
T8 180.00-160.00	170.00	1.597	33	423.141	A	27.674	75.498	35.898	34.79	0.000	0.000
					B	25.852	75.498	35.42			
					C	34.216	60.998	37.70			
T9 160.00-140.00	150.00	1.541	32	463.037	A	37.797	75.488	35.888	31.68	0.000	0.000
					B	35.413	75.488	32.36			
					C	43.890	60.988	34.22			
T10 140.00-120.00	130.00	1.48	31	503.943	A	41.004	75.504	35.904	30.82	0.000	0.000
					B	38.616	75.504	31.46			
					C	46.935	62.669	32.76			
T11 120.00-100.00	110.00	1.411	29	551.554	A	4.591	99.534	35.933	34.51	0.000	0.000
					B	0.000	99.846	35.99			
					C	9.117	88.298	36.89			
T12 100.00-80.00	90.00	1.332	28	602.352	A	5.247	101.189	35.927	33.75	0.000	0.000
					B	0.000	101.530	35.39			
					C	11.478	95.010	33.74			
T13 80.00-60.00	70.00	1.24	26	657.397	A	5.247	109.942	42.626	37.01	0.000	0.000
					B	0.000	110.281	38.65			
					C	11.478	104.629	36.71			
T14 60.00-30.00	45.00	1.093	23	1081.034	A	7.871	168.337	63.908	36.27	0.000	0.000
					B	0.000	168.890	37.84			
					C	17.217	160.359	35.99			
T15 30.00-0.00	15.00	1	21	1194.292	A	7.871	174.073	63.928	35.14	0.000	0.000
					B	0.000	174.622	36.61			
					C	17.217	166.095	34.87			

### Tower Pressure - With Ice

$$G_H = 1.084$$

Section Elevation ft	z ft	K <sub>Z</sub>	q <sub>z</sub> psf	t <sub>z</sub> in	A <sub>G</sub> ft <sup>2</sup>	F a c e	A <sub>F</sub> ft <sup>2</sup>	A <sub>R</sub> ft <sup>2</sup>	A <sub>leg</sub> ft <sup>2</sup>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>
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<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 17 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 08:00:06 03/14/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	t <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>AA</sub> In Face	C <sub>AA</sub> Out Face
ft	ft		psf	in	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
T1 320.00-300.00	310.00	1.897	39	0.5000	147.138	A	16.101	21.877	21.877	57.60	0.000	0.000
						B	16.101	21.877		57.60		
						C	18.656	39.460		37.64		
T2 300.00-280.00	290.00	1.861	39	0.5000	169.325	A	16.795	25.461	25.461	60.25	0.000	0.000
						B	16.795	25.461		60.25		
						C	23.867	47.967		35.44		
T3 280.00-260.00	270.00	1.823	38	0.5000	214.966	A	17.649	32.147	32.147	64.56	0.000	0.000
						B	17.649	32.147		64.56		
						C	24.328	66.697		35.32		
T4 260.00-240.00	250.00	1.783	37	0.5000	257.263	A	23.764	32.139	32.139	57.49	0.000	0.000
						B	23.764	32.139		57.49		
						C	29.445	71.964		31.69		
T5 240.00-220.00	230.00	1.741	36	0.5000	297.762	A	34.511	32.137	32.137	48.22	0.000	0.000
						B	34.511	32.137		48.22		
						C	38.230	77.237		27.83		
T6 220.00-200.00	210.00	1.697	35	0.5000	337.862	A	22.699	91.737	32.137	28.08	0.000	0.000
						B	28.158	32.137		53.30		
						C	34.052	77.237		28.88		
T7 200.00-180.00	190.00	1.649	34	0.5000	382.711	A	25.442	98.841	39.241	31.57	0.000	0.000
						B	25.442	98.841		31.57		
						C	36.791	84.341		32.39		
T8 180.00-160.00	170.00	1.597	33	0.5000	424.810	A	30.948	98.837	39.237	30.23	0.000	0.000
						B	28.426	98.837		30.83		
						C	39.772	84.337		31.62		
T9 160.00-140.00	150.00	1.541	32	0.5000	464.706	A	41.354	98.826	39.226	27.98	0.000	0.000
						B	38.064	98.826		28.66		
						C	49.437	84.326		29.33		
T10 140.00-120.00	130.00	1.48	31	0.5000	505.612	A	45.019	98.844	39.244	27.28	0.000	0.000
						B	41.722	98.844		27.92		
						C	52.770	87.509		27.98		
T11 120.00-100.00	110.00	1.411	29	0.5000	553.224	A	6.536	128.444	39.275	29.10	0.000	0.000
						B	0.000	129.134		30.41		
						C	13.395	119.329		29.59		
T12 100.00-80.00	90.00	1.332	28	0.5000	604.022	A	7.470	130.704	39.269	28.42	0.000	0.000
						B	0.000	131.458		29.87		
						C	17.033	130.925		26.54		
T13 80.00-60.00	70.00	1.24	26	0.5000	659.068	A	7.470	140.004	45.969	31.17	0.000	0.000
						B	0.000	140.749		32.66		
						C	17.033	141.845		28.93		
T14 60.00-30.00	45.00	1.093	23	0.5000	1083.539	A	11.204	213.363	68.920	30.69	0.000	0.000
						B	0.000	214.572		32.12		
						C	25.550	215.970		28.54		
T15 30.00-0.00	15.00	1	21	0.5000	1196.797	A	11.204	220.234	68.942	29.79	0.000	0.000
						B	0.000	221.424		31.14		
						C	25.550	222.873		27.75		

**Tower Pressure - Service**

$G_H = 1.084$

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>AA</sub> In Face	C <sub>AA</sub> Out Face
ft	ft		psf	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
T1 320.00-	310.00	1.897	39	145.472	A	11.659	18.543	18.543	61.40	0.000	0.000

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 18 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 08:00:06 03/14/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>AA</sub> In Face	C <sub>AA</sub> Out Face
ft	ft		psf	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
300.00					B	11.659	18.543		61.40		
					C	14.384	27.793		43.97		
T2 300.00-280.00	290.00	1.861	39	167.656	A	12.596	22.122	22.122	63.72	0.000	0.000
					B	12.596	22.122		63.72		
					C	18.740	33.962		41.97		
T3 280.00-260.00	270.00	1.823	38	213.297	A	13.934	28.807	28.807	67.40	0.000	0.000
					B	13.934	28.807		67.40		
					C	19.739	48.357		42.30		
T4 260.00-240.00	250.00	1.783	37	255.594	A	19.443	28.800	28.800	59.70	0.000	0.000
					B	19.443	28.800		59.70		
					C	24.725	51.125		37.97		
T5 240.00-220.00	230.00	1.741	36	296.093	A	29.581	28.798	28.798	49.33	0.000	0.000
					B	29.581	28.798		49.33		
					C	33.803	53.898		32.84		
T6 220.00-200.00	210.00	1.697	35	336.193	A	21.026	68.398	28.798	32.20	0.000	0.000
					B	24.136	28.798		54.40		
					C	29.418	53.898		34.56		
T7 200.00-180.00	190.00	1.649	34	381.042	A	23.330	75.501	35.901	36.33	0.000	0.000
					B	23.330	75.501		36.33		
					C	31.706	61.001		38.73		
T8 180.00-160.00	170.00	1.597	33	423.141	A	27.674	75.498	35.898	34.79	0.000	0.000
					B	25.852	75.498		35.42		
					C	34.216	60.998		37.70		
T9 160.00-140.00	150.00	1.541	32	463.037	A	37.797	75.488	35.888	31.68	0.000	0.000
					B	35.413	75.488		32.36		
					C	43.890	60.988		34.22		
T10 140.00-120.00	130.00	1.48	31	503.943	A	41.004	75.504	35.904	30.82	0.000	0.000
					B	38.616	75.504		31.46		
					C	46.935	62.669		32.76		
T11 120.00-100.00	110.00	1.411	29	551.554	A	4.591	99.534	35.933	34.51	0.000	0.000
					B	0.000	99.846		35.99		
					C	9.117	88.298		36.89		
T12 100.00-80.00	90.00	1.332	28	602.352	A	5.247	101.189	35.927	33.75	0.000	0.000
					B	0.000	101.530		35.39		
					C	11.478	95.010		33.74		
T13 80.00-60.00	70.00	1.24	26	657.397	A	5.247	109.942	42.626	37.01	0.000	0.000
					B	0.000	110.281		38.65		
					C	11.478	104.629		36.71		
T14 60.00-30.00	45.00	1.093	23	1081.03	A	7.871	168.337	63.908	36.27	0.000	0.000
				4	B	0.000	168.890		37.84		
					C	17.217	160.359		35.99		
T15 30.00-0.00	15.00	1	21	1194.29	A	7.871	174.073	63.928	35.14	0.000	0.000
				2	B	0.000	174.622		36.61		
					C	17.217	166.095		34.87		

**Tower Forces - No Ice - Wind Normal To Face**

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	1	1	22.637	3.11	155.53	C
			B	0.208	2.571	0.592	1	1	22.637			
			C	0.29	2.323	0.613	1	1	31.417			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	1	1	25.689	3.76	187.98	C
			B	0.207	2.573	0.592	1	1	25.689			
			C	0.314	2.259	0.62	1	1	39.810			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSMW	Page	19 of 51
	Project	CSP Tower - Colchester, CT	Date	08:00:06 03/14/06
	Client	Cingular Wireless	Designed by	Jed Kiernan

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	1	1	30.944	4.58	229.20	C
			B	0.2	2.595	0.59	1	1	30.944			
			C	0.319	2.246	0.622	1	1	49.816			
T4 260.00-240.00	0.16	3.83	A	0.189	2.634	0.588	1	1	36.382	5.19	259.37	C
			B	0.189	2.634	0.588	1	1	36.382			
			C	0.297	2.305	0.615	1	1	56.162			
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	1	1	46.567	6.04	302.05	C
			B	0.197	2.605	0.59	1	1	46.567			
			C	0.296	2.306	0.615	1	1	66.937			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	1	1	62.481	5.76	288.09	C
			B	0.157	2.744	0.583	1	1	40.915			
			C	0.248	2.444	0.601	1	1	61.828			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	1	1	68.956	6.22	311.10	C
			B	0.259	2.41	0.604	1	1	68.956			
			C	0.243	2.458	0.6	1	1	68.318			
T8 180.00-160.00	0.94	5.93	A	0.244	2.456	0.6	1	1	72.996	6.44	321.79	A
			B	0.24	2.469	0.599	1	1	71.095			
			C	0.225	2.515	0.596	1	1	70.560			
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	1	1	83.129	7.06	353.23	A
			B	0.24	2.469	0.599	1	1	80.649			
			C	0.226	2.51	0.596	1	1	80.248			
T10 140.00-120.00	0.95	7.17	A	0.231	2.495	0.597	1	1	86.099	7.14	357.13	A
			B	0.226	2.51	0.596	1	1	83.628			
			C	0.217	2.539	0.594	1	1	84.168			
T11 120.00-100.00	0.97	6.02	A	0.189	2.634	0.588	1	1	63.135	5.27	263.56	A
			B	0.181	2.661	0.587	1	1	58.581			
			C	0.177	2.676	0.586	1	1	60.852			
T12 100.00-80.00	1.01	6.17	A	0.177	2.676	0.586	1	1	64.536	5.38	268.86	C
			B	0.169	2.704	0.584	1	1	59.343			
			C	0.177	2.675	0.586	1	1	67.148			
T13 80.00-60.00	1.02	7.26	A	0.175	2.681	0.586	1	1	69.636	5.43	271.28	C
			B	0.168	2.707	0.584	1	1	64.443			
			C	0.177	2.676	0.586	1	1	72.781			
T14 60.00-30.00	1.52	11.34	A	0.163	2.724	0.584	1	1	106.104	7.40	246.70	C
			B	0.156	2.749	0.582	1	1	98.370			
			C	0.164	2.72	0.584	1	1	110.828			
T15 30.00-0.00	1.52	13.09	A	0.152	2.763	0.582	1	1	109.153	7.06	235.34	C
			B	0.146	2.785	0.581	1	1	101.438			
			C	0.153	2.759	0.582	1	1	113.887			
Sum Weight:	10.89	90.92						OTM	13335.51 kip-ft	85.84		

**Tower Forces - No Ice - Wind 45 To Face**

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	0.825	1	20.596	2.86	143.07	C
			B	0.208	2.571	0.592	0.825	1	20.596			
			C	0.29	2.323	0.613	0.825	1	28.900			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	0.825	1	23.485	3.45	172.49	C
			B	0.207	2.573	0.592	0.825	1	23.485			
			C	0.314	2.259	0.62	0.825	1	36.530			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	0.825	1	28.505	4.27	213.31	C
			B	0.2	2.595	0.59	0.825	1	28.505			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	20 of 51
	Project	CSP Tower - Colchester, CT	Date	08:00:06 03/14/06
	Client	Cingular Wireless	Designed by	Jed Kiernan

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T4 260.00-240.00	0.16	3.83	C	0.319	2.246	0.622	0.825	1	46.362	4.79	239.39	C
			A	0.189	2.634	0.588	0.825	1	32.980			
			B	0.189	2.634	0.588	0.825	1	32.980			
T5 240.00-220.00	0.18	4.90	C	0.297	2.305	0.615	0.825	1	51.835	5.51	275.36	C
			A	0.197	2.605	0.59	0.825	1	41.390			
			B	0.197	2.605	0.59	0.825	1	41.390			
T6 220.00-200.00	0.43	4.82	C	0.296	2.306	0.615	0.825	1	61.021	5.36	267.98	A
			A	0.266	2.39	0.606	0.825	1	58.802			
			B	0.157	2.744	0.583	0.825	1	36.691			
T7 200.00-180.00	0.93	5.71	C	0.248	2.444	0.601	0.825	1	56.680	5.79	289.63	B
			A	0.259	2.41	0.604	0.825	1	64.873			
			B	0.259	2.41	0.604	0.825	1	64.873			
T8 180.00-160.00	0.94	5.93	C	0.243	2.458	0.6	0.825	1	62.769	6.01	300.44	A
			A	0.244	2.456	0.6	0.825	1	68.154			
			B	0.24	2.469	0.599	0.825	1	66.571			
T9 160.00-140.00	0.94	6.89	C	0.225	2.515	0.596	0.825	1	64.572	6.50	325.12	A
			A	0.245	2.454	0.601	0.825	1	76.515			
			B	0.24	2.469	0.599	0.825	1	74.452			
T10 140.00-120.00	0.95	7.17	C	0.226	2.51	0.596	0.825	1	72.568	6.55	327.36	A
			A	0.231	2.495	0.597	0.825	1	78.924			
			B	0.226	2.51	0.596	0.825	1	76.871			
T11 120.00-100.00	0.97	6.02	C	0.217	2.539	0.594	0.825	1	75.954	5.20	260.20	A
			A	0.189	2.634	0.588	0.825	1	62.332			
			B	0.181	2.661	0.587	0.825	1	58.581			
T12 100.00-80.00	1.01	6.17	C	0.177	2.676	0.586	0.825	1	59.256	5.22	260.81	C
			A	0.177	2.676	0.586	0.825	1	63.618			
			B	0.169	2.704	0.584	0.825	1	59.343			
T13 80.00-60.00	1.02	7.26	C	0.177	2.675	0.586	0.825	1	65.140	5.28	263.79	C
			A	0.175	2.681	0.586	0.825	1	68.717			
			B	0.168	2.707	0.584	0.825	1	64.443			
T14 60.00-30.00	1.52	11.34	C	0.177	2.676	0.586	0.825	1	70.772	7.20	240.00	C
			A	0.163	2.724	0.584	0.825	1	104.727			
			B	0.156	2.749	0.582	0.825	1	98.370			
T15 30.00-0.00	1.52	13.09	C	0.164	2.72	0.584	0.825	1	107.815	6.87	229.11	C
			A	0.152	2.763	0.582	0.825	1	107.775			
			B	0.146	2.785	0.581	0.825	1	101.438			
Sum Weight:	10.89	90.92						OTM	12415.37 kip-ft	80.85		

### Tower Forces - No Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	0.8	1	20.305	2.83	141.29	C
			B	0.208	2.571	0.592	0.8	1	20.305			
			C	0.29	2.323	0.613	0.8	1	28.541			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	0.8	1	23.170	3.41	170.28	C
			B	0.207	2.573	0.592	0.8	1	23.170			
			C	0.314	2.259	0.62	0.8	1	36.062			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	0.8	1	28.157	4.22	211.03	C
			B	0.2	2.595	0.59	0.8	1	28.157			
			C	0.319	2.246	0.622	0.8	1	45.868			
T4 260.00-	0.16	3.83	A	0.189	2.634	0.588	0.8	1	32.494	4.73	236.54	C

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	<b>Job</b> 320' Rohn SSVMW	<b>Page</b> 21 of 51
	<b>Project</b> CSP Tower - Colchester, CT	<b>Date</b> 08:00:06 03/14/06
	<b>Client</b> Cingular Wireless	<b>Designed by</b> Jed Kiernan

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
240.00			B	0.189	2.634	0.588	0.8	1	32.494			
			C	0.297	2.305	0.615	0.8	1	51.217			
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	0.8	1	40.651	5.43	271.55	C
			B	0.197	2.605	0.59	0.8	1	40.651			
			C	0.296	2.306	0.615	0.8	1	60.176			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	0.8	1	58.276	5.31	265.58	A
			B	0.157	2.744	0.583	0.8	1	36.088			
			C	0.248	2.444	0.601	0.8	1	55.944			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	0.8	1	64.290	5.74	287.03	B
			B	0.259	2.41	0.604	0.8	1	64.290			
			C	0.243	2.458	0.6	0.8	1	61.977			
T8 180.00-160.00	0.94	5.93	A	0.244	2.456	0.6	0.8	1	67.462	5.95	297.40	A
			B	0.24	2.469	0.599	0.8	1	65.924			
			C	0.225	2.515	0.596	0.8	1	63.717			
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	0.8	1	75.570	6.42	321.10	A
			B	0.24	2.469	0.599	0.8	1	73.567			
			C	0.226	2.51	0.596	0.8	1	71.470			
T10 140.00-120.00	0.95	7.17	A	0.231	2.495	0.597	0.8	1	77.898	6.46	323.11	A
			B	0.226	2.51	0.596	0.8	1	75.905			
			C	0.217	2.539	0.594	0.8	1	74.781			
T11 120.00-100.00	0.97	6.02	A	0.189	2.634	0.588	0.8	1	62.217	5.19	259.72	A
			B	0.181	2.661	0.587	0.8	1	58.581			
			C	0.177	2.676	0.586	0.8	1	59.028			
T12 100.00-80.00	1.01	6.17	A	0.177	2.676	0.586	0.8	1	63.487	5.19	259.66	C
			B	0.169	2.704	0.584	0.8	1	59.343			
			C	0.177	2.675	0.586	0.8	1	64.853			
T13 80.00-60.00	1.02	7.26	A	0.175	2.681	0.586	0.8	1	68.586	5.25	262.72	C
			B	0.168	2.707	0.584	0.8	1	64.443			
			C	0.177	2.676	0.586	0.8	1	70.485			
T14 60.00-30.00	1.52	11.34	A	0.163	2.724	0.584	0.8	1	104.530	7.17	239.04	C
			B	0.156	2.749	0.582	0.8	1	98.370			
			C	0.164	2.72	0.584	0.8	1	107.385			
T15 30.00-0.00	1.52	13.09	A	0.152	2.763	0.582	0.8	1	107.579	6.85	228.22	C
			B	0.146	2.785	0.581	0.8	1	101.438			
			C	0.153	2.759	0.582	0.8	1	110.443			
Sum Weight:	10.89	90.92						OTM	12287.69 kip-ft	80.16		

**Tower Forces - No Ice - Wind 90 To Face**

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	0.85	1	20.888	2.90	144.85	C
			B	0.208	2.571	0.592	0.85	1	20.888			
			C	0.29	2.323	0.613	0.85	1	29.260			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	0.85	1	23.800	3.49	174.71	C
			B	0.207	2.573	0.592	0.85	1	23.800			
			C	0.314	2.259	0.62	0.85	1	36.999			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	0.85	1	28.854	4.31	215.58	C
			B	0.2	2.595	0.59	0.85	1	28.854			
			C	0.319	2.246	0.622	0.85	1	46.855			
T4 260.00-240.00	0.16	3.83	A	0.189	2.634	0.588	0.85	1	33.466	4.84	242.25	C
			B	0.189	2.634	0.588	0.85	1	33.466			
			C	0.297	2.305	0.615	0.85	1	52.454			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	22 of 51
	Project	CSP Tower - Colchester, CT	Date	08:00:06 03/14/06
	Client	Cingular Wireless	Designed by	Jed Kiernan

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	0.85	1	42.130	5.58	279.17	C
			B	0.197	2.605	0.59	0.85	1	42.130			
			C	0.296	2.306	0.615	0.85	1	61.866			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	0.85	1	59.327	5.41	270.37	A
			B	0.157	2.744	0.583	0.85	1	37.294			
			C	0.248	2.444	0.601	0.85	1	57.415			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	0.85	1	65.457	5.84	292.24	B
			B	0.259	2.41	0.604	0.85	1	65.457			
			C	0.243	2.458	0.6	0.85	1	63.562			
T8 180.00-160.00	0.94	5.93	A	0.244	2.456	0.6	0.85	1	68.845	6.07	303.49	A
			B	0.24	2.469	0.599	0.85	1	67.217			
			C	0.225	2.515	0.596	0.85	1	65.428			
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	0.85	1	77.460	6.58	329.13	A
			B	0.24	2.469	0.599	0.85	1	75.337			
			C	0.226	2.51	0.596	0.85	1	73.665			
T10 140.00-120.00	0.95	7.17	A	0.231	2.495	0.597	0.85	1	79.949	6.63	331.62	A
			B	0.226	2.51	0.596	0.85	1	77.836			
			C	0.217	2.539	0.594	0.85	1	77.128			
T11 120.00-100.00	0.97	6.02	A	0.189	2.634	0.588	0.85	1	62.447	5.21	260.68	A
			B	0.181	2.661	0.587	0.85	1	58.581			
			C	0.177	2.676	0.586	0.85	1	59.484			
T12 100.00-80.00	1.01	6.17	A	0.177	2.676	0.586	0.85	1	63.749	5.24	261.96	C
			B	0.169	2.704	0.584	0.85	1	59.343			
			C	0.177	2.675	0.586	0.85	1	65.426			
T13 80.00-60.00	1.02	7.26	A	0.175	2.681	0.586	0.85	1	68.849	5.30	264.86	C
			B	0.168	2.707	0.584	0.85	1	64.443			
			C	0.177	2.676	0.586	0.85	1	71.059			
T14 60.00-30.00	1.52	11.34	A	0.163	2.724	0.584	0.85	1	104.924	7.23	240.95	C
			B	0.156	2.749	0.582	0.85	1	98.370			
			C	0.164	2.72	0.584	0.85	1	108.246			
T15 30.00-0.00	1.52	13.09	A	0.152	2.763	0.582	0.85	1	107.972	6.90	230.00	C
			B	0.146	2.785	0.581	0.85	1	101.438			
			C	0.153	2.759	0.582	0.85	1	111.304			
Sum Weight:	10.89	90.92						OTM	12543.06 kip-ft	81.55		

### Tower Forces - With Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.21	2.46	A	0.258	2.413	0.604	1	1	29.314	3.91	195.73	C
			B	0.258	2.413	0.604	1	1	29.314			
			C	0.395	2.074	0.65	1	1	44.288			
T2 300.00-280.00	0.31	3.23	A	0.25	2.439	0.602	1	1	32.116	4.69	234.63	C
			B	0.25	2.439	0.602	1	1	32.116			
			C	0.424	2.018	0.662	1	1	55.611			
T3 280.00-260.00	0.43	4.32	A	0.232	2.494	0.597	1	1	36.853	5.66	283.14	C
			B	0.232	2.494	0.597	1	1	36.853			
			C	0.423	2.019	0.661	1	1	68.444			
T4 260.00-240.00	0.47	4.83	A	0.217	2.539	0.594	1	1	42.857	6.34	316.81	C
			B	0.217	2.539	0.594	1	1	42.857			
			C	0.394	2.076	0.649	1	1	76.168			
T5 240.00-220.00	0.52	6.22	A	0.224	2.518	0.596	1	1	53.651	7.21	360.39	C
			B	0.224	2.518	0.596	1	1	53.651			



<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	23 of 51
	Project	CSP Tower - Colchester, CT	Date	08:00:06 03/14/06
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Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T6 220.00-200.00	1.13	5.95	C	0.388	2.089	0.647	1	1	88.179	6.97	348.67	C
			A	0.339	2.198	0.629	1	1	80.356			
			B	0.178	2.67	0.586	1	1	46.998			
T7 200.00-180.00	2.36	7.00	C	0.329	2.221	0.625	1	1	82.351	7.44	372.24	C
			A	0.325	2.232	0.624	1	1	87.097			
			B	0.325	2.232	0.624	1	1	87.097			
T8 180.00-160.00	2.39	7.29	C	0.317	2.253	0.621	1	1	89.174	7.61	380.59	C
			A	0.306	2.281	0.618	1	1	91.990			
			B	0.3	2.297	0.616	1	1	89.287			
T9 160.00-140.00	2.40	8.55	C	0.292	2.317	0.614	1	1	91.515	8.15	407.60	C
			A	0.302	2.292	0.616	1	1	102.271			
			B	0.295	2.311	0.614	1	1	98.768			
T10 140.00-120.00	2.42	8.93	C	0.288	2.338	0.611	1	1	101.066	8.32	415.80	C
			A	0.285	2.338	0.611	1	1	105.441			
			B	0.278	2.356	0.609	1	1	101.960			
T11 120.00-100.00	2.49	7.25	C	0.277	2.358	0.609	1	1	106.085	6.64	332.17	C
			A	0.244	2.456	0.6	1	1	83.649			
			B	0.233	2.488	0.598	1	1	77.195			
T12 100.00-80.00	2.63	7.45	C	0.24	2.468	0.599	1	1	84.915	7.02	351.17	C
			A	0.229	2.503	0.597	1	1	85.459			
			B	0.218	2.538	0.594	1	1	78.107			
T13 80.00-60.00	2.64	8.67	C	0.245	2.453	0.601	1	1	95.668	7.01	350.46	C
			A	0.224	2.519	0.596	1	1	90.847			
			B	0.214	2.551	0.593	1	1	83.500			
T14 60.00-30.00	3.96	13.51	C	0.241	2.465	0.6	1	1	102.089	9.54	318.05	C
			A	0.207	2.572	0.592	1	1	137.495			
			B	0.198	2.603	0.59	1	1	126.598			
T15 30.00-0.00	3.96	15.41	C	0.223	2.521	0.595	1	1	154.125	9.10	303.28	C
			A	0.193	2.618	0.589	1	1	140.938			
			B	0.185	2.647	0.587	1	1	130.077			
Sum Weight:	28.31	111.08	C	0.208	2.571	0.592		OTM	16241.27 kip-ft	105.63		

### Tower Forces - With Ice - Wind 45 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T1 320.00-300.00	0.21	2.46	A	0.258	2.413	0.604	0.825	1	26.496	3.63	181.31	C
			B	0.258	2.413	0.604	0.825	1	26.496			
			C	0.395	2.074	0.65	0.825	1	41.023			
T2 300.00-280.00	0.31	3.23	A	0.25	2.439	0.602	0.825	1	29.177	4.34	217.00	C
			B	0.25	2.439	0.602	0.825	1	29.177			
			C	0.424	2.018	0.662	0.825	1	51.435			
T3 280.00-260.00	0.43	4.32	A	0.232	2.494	0.597	0.825	1	33.765	5.31	265.52	C
			B	0.232	2.494	0.597	0.825	1	33.765			
			C	0.423	2.019	0.661	0.825	1	64.187			
T4 260.00-240.00	0.47	4.83	A	0.217	2.539	0.594	0.825	1	38.698	5.91	295.37	C
			B	0.217	2.539	0.594	0.825	1	38.698			
			C	0.394	2.076	0.649	0.825	1	71.015			
T5 240.00-220.00	0.52	6.22	A	0.224	2.518	0.596	0.825	1	47.611	6.66	333.04	C
			B	0.224	2.518	0.596	0.825	1	47.611			
			C	0.388	2.089	0.647	0.825	1	81.489			
T6 220.00-	1.13	5.95	A	0.339	2.198	0.629	0.825	1	76.384	6.47	323.44	C

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	24 of 51
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Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
200.00			B	0.178	2.67	0.586	0.825	1	42.071			
			C	0.329	2.221	0.625	0.825	1	76.392			
T7 200.00-180.00	2.36	7.00	A	0.325	2.232	0.624	0.825	1	82.645	6.91	345.37	C
			B	0.325	2.232	0.624	0.825	1	82.645			
			C	0.317	2.253	0.621	0.825	1	82.736			
T8 180.00-160.00	2.39	7.29	A	0.306	2.281	0.618	0.825	1	86.574	7.09	354.50	A
			B	0.3	2.297	0.616	0.825	1	84.312			
			C	0.292	2.317	0.614	0.825	1	84.555			
T9 160.00-140.00	2.40	8.55	A	0.302	2.292	0.616	0.825	1	95.034	7.54	377.15	A
			B	0.295	2.311	0.614	0.825	1	92.107			
			C	0.288	2.329	0.612	0.825	1	92.414			
T10 140.00-120.00	2.42	8.93	A	0.285	2.338	0.611	0.825	1	97.563	7.59	379.60	C
			B	0.278	2.356	0.609	0.825	1	94.658			
			C	0.277	2.358	0.609	0.825	1	96.851			
T11 120.00-100.00	2.49	7.25	A	0.244	2.456	0.6	0.825	1	82.505	6.46	323.00	C
			B	0.233	2.488	0.598	0.825	1	77.195			
			C	0.24	2.468	0.599	0.825	1	82.571			
T12 100.00-80.00	2.63	7.45	A	0.229	2.503	0.597	0.825	1	84.152	6.80	340.23	C
			B	0.218	2.538	0.594	0.825	1	78.107			
			C	0.245	2.453	0.601	0.825	1	92.687			
T13 80.00-60.00	2.64	8.67	A	0.224	2.519	0.596	0.825	1	89.540	6.80	340.23	C
			B	0.214	2.551	0.593	0.825	1	83.500			
			C	0.241	2.465	0.6	0.825	1	99.108			
T14 60.00-30.00	3.96	13.51	A	0.207	2.572	0.592	0.825	1	135.534	9.26	308.83	C
			B	0.198	2.603	0.59	0.825	1	126.598			
			C	0.223	2.521	0.595	0.825	1	149.654			
T15 30.00-0.00	3.96	15.41	A	0.193	2.618	0.589	0.825	1	138.977	8.84	294.67	C
			B	0.185	2.647	0.587	0.825	1	130.077			
			C	0.208	2.571	0.592	0.825	1	153.014			
Sum Weight:	28.31	111.08						OTM	15168.77 kip-ft	99.62		

### Tower Forces - With Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.21	2.46	A	0.258	2.413	0.604	0.8	1	26.094	3.58	179.24	C
			B	0.258	2.413	0.604	0.8	1	26.094			
			C	0.395	2.074	0.65	0.8	1	40.557			
T2 300.00-280.00	0.31	3.23	A	0.25	2.439	0.602	0.8	1	28.757	4.29	214.49	C
			B	0.25	2.439	0.602	0.8	1	28.757			
			C	0.424	2.018	0.662	0.8	1	50.838			
T3 280.00-260.00	0.43	4.32	A	0.232	2.494	0.597	0.8	1	33.323	5.26	263.01	C
			B	0.232	2.494	0.597	0.8	1	33.323			
			C	0.423	2.019	0.661	0.8	1	63.579			
T4 260.00-240.00	0.47	4.83	A	0.217	2.539	0.594	0.8	1	38.104	5.85	292.31	C
			B	0.217	2.539	0.594	0.8	1	38.104			
			C	0.394	2.076	0.649	0.8	1	70.279			
T5 240.00-220.00	0.52	6.22	A	0.224	2.518	0.596	0.8	1	46.748	6.58	329.14	C
			B	0.224	2.518	0.596	0.8	1	46.748			
			C	0.388	2.089	0.647	0.8	1	80.533			
T6 220.00-200.00	1.13	5.95	A	0.339	2.198	0.629	0.8	1	75.816	6.40	319.83	C
			B	0.178	2.67	0.586	0.8	1	41.367			
			C	0.329	2.221	0.625	0.8	1	75.541			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSMW	Page	25 of 51
	Project	CSP Tower - Colchester, CT	Date	08:00:06 03/14/06
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Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T7 200.00-180.00	2.36	7.00	A	0.325	2.232	0.624	0.8	1	82.009	6.83	341.53	C
			B	0.325	2.232	0.624	0.8	1	82.009			
			C	0.317	2.253	0.621	0.8	1	81.816			
T8 180.00-160.00	2.39	7.29	A	0.306	2.281	0.618	0.8	1	85.800	7.03	351.33	A
			B	0.3	2.297	0.616	0.8	1	83.602			
			C	0.292	2.317	0.614	0.8	1	83.561			
T9 160.00-140.00	2.40	8.55	A	0.302	2.292	0.616	0.8	1	94.000	7.46	373.04	A
			B	0.295	2.311	0.614	0.8	1	91.155			
			C	0.288	2.329	0.612	0.8	1	91.178			
T10 140.00-120.00	2.42	8.93	A	0.285	2.338	0.611	0.8	1	96.437	7.50	374.81	A
			B	0.278	2.356	0.609	0.8	1	93.615			
			C	0.277	2.358	0.609	0.8	1	95.531			
T11 120.00-100.00	2.49	7.25	A	0.244	2.456	0.6	0.8	1	82.342	6.43	321.69	C
			B	0.233	2.488	0.598	0.8	1	77.195			
			C	0.24	2.468	0.599	0.8	1	82.236			
T12 100.00-80.00	2.63	7.45	A	0.229	2.503	0.597	0.8	1	83.965	6.77	338.66	C
			B	0.218	2.538	0.594	0.8	1	78.107			
			C	0.245	2.453	0.601	0.8	1	92.261			
T13 80.00-60.00	2.64	8.67	A	0.224	2.519	0.596	0.8	1	89.353	6.78	338.77	C
			B	0.214	2.551	0.593	0.8	1	83.500			
			C	0.241	2.465	0.6	0.8	1	98.682			
T14 60.00-30.00	3.96	13.51	A	0.207	2.572	0.592	0.8	1	135.254	9.23	307.51	C
			B	0.198	2.603	0.59	0.8	1	126.598			
			C	0.223	2.521	0.595	0.8	1	149.015			
T15 30.00-0.00	3.96	15.41	A	0.193	2.618	0.589	0.8	1	138.697	8.80	293.44	C
			B	0.185	2.647	0.587	0.8	1	130.077			
			C	0.208	2.571	0.592	0.8	1	152.375			
Sum Weight:	28.31	111.08						OTM	15019.19 kip-ft	98.79		

### Tower Forces - With Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.21	2.46	A	0.258	2.413	0.604	0.85	1	26.899	3.67	183.37	C
			B	0.258	2.413	0.604	0.85	1	26.899			
			C	0.395	2.074	0.65	0.85	1	41.490			
T2 300.00-280.00	0.31	3.23	A	0.25	2.439	0.602	0.85	1	29.597	4.39	219.52	C
			B	0.25	2.439	0.602	0.85	1	29.597			
			C	0.424	2.018	0.662	0.85	1	52.031			
T3 280.00-260.00	0.43	4.32	A	0.232	2.494	0.597	0.85	1	34.206	5.36	268.04	C
			B	0.232	2.494	0.597	0.85	1	34.206			
			C	0.423	2.019	0.661	0.85	1	64.795			
T4 260.00-240.00	0.47	4.83	A	0.217	2.539	0.594	0.85	1	39.293	5.97	298.44	C
			B	0.217	2.539	0.594	0.85	1	39.293			
			C	0.394	2.076	0.649	0.85	1	71.751			
T5 240.00-220.00	0.52	6.22	A	0.224	2.518	0.596	0.85	1	48.474	6.74	336.95	C
			B	0.224	2.518	0.596	0.85	1	48.474			
			C	0.388	2.089	0.647	0.85	1	82.444			
T6 220.00-200.00	1.13	5.95	A	0.339	2.198	0.629	0.85	1	76.951	6.54	327.04	C
			B	0.178	2.67	0.586	0.85	1	42.775			
			C	0.329	2.221	0.625	0.85	1	77.243			
T7 200.00-180.00	2.36	7.00	A	0.325	2.232	0.624	0.85	1	83.281	6.98	349.20	C
			B	0.325	2.232	0.624	0.85	1	83.281			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSMW	Page	26 of 51
	Project	CSP Tower - Colchester, CT	Date	08:00:06 03/14/06
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Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T8 180.00-160.00	2.39	7.29	C	0.317	2.253	0.621	0.85	1	83.656	7.15	357.67	A
			A	0.306	2.281	0.618	0.85	1	87.348			
			B	0.3	2.297	0.616	0.85	1	85.023			
T9 160.00-140.00	2.40	8.55	C	0.292	2.317	0.614	0.85	1	85.549	7.62	381.25	A
			A	0.302	2.292	0.616	0.85	1	96.068			
			B	0.295	2.311	0.614	0.85	1	93.058			
T10 140.00-120.00	2.42	8.93	C	0.288	2.329	0.612	0.85	1	93.650	7.70	384.77	C
			A	0.285	2.338	0.611	0.85	1	98.688			
			B	0.278	2.356	0.609	0.85	1	95.701			
T11 120.00-100.00	2.49	7.25	C	0.277	2.358	0.609	0.85	1	98.170	6.49	324.31	C
			A	0.244	2.456	0.6	0.85	1	82.668			
			B	0.233	2.488	0.598	0.85	1	77.195			
T12 100.00-80.00	2.63	7.45	C	0.24	2.468	0.599	0.85	1	82.906	6.84	341.79	C
			A	0.229	2.503	0.597	0.85	1	84.339			
			B	0.218	2.538	0.594	0.85	1	78.107			
T13 80.00-60.00	2.64	8.67	C	0.245	2.453	0.601	0.85	1	93.112	6.83	341.69	C
			A	0.224	2.519	0.596	0.85	1	89.726			
			B	0.214	2.551	0.593	0.85	1	83.500			
T14 60.00-30.00	3.96	13.51	C	0.241	2.465	0.6	0.85	1	99.534	9.30	310.15	C
			A	0.207	2.572	0.592	0.85	1	135.814			
			B	0.198	2.603	0.59	0.85	1	126.598			
T15 30.00-0.00	3.96	15.41	C	0.223	2.521	0.595	0.85	1	150.293	8.88	295.90	C
			A	0.193	2.618	0.589	0.85	1	139.257			
			B	0.185	2.647	0.587	0.85	1	130.077			
Sum Weight:	28.31	111.08	C	0.208	2.571	0.592	0.85	1	153.653	100.46		
								OTM	15319.35 kip-ft			

### Tower Forces - Service - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	1	1	22.637	3.11	155.53	C
			B	0.208	2.571	0.592	1	1	22.637			
			C	0.29	2.323	0.613	1	1	31.417			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	1	1	25.689	3.76	187.98	C
			B	0.207	2.573	0.592	1	1	25.689			
			C	0.314	2.259	0.62	1	1	39.810			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	1	1	30.944	4.58	229.20	C
			B	0.2	2.595	0.59	1	1	30.944			
			C	0.319	2.246	0.622	1	1	49.816			
T4 260.00-240.00	0.16	3.83	A	0.189	2.634	0.588	1	1	36.382	5.19	259.37	C
			B	0.189	2.634	0.588	1	1	36.382			
			C	0.297	2.305	0.615	1	1	56.162			
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	1	1	46.567	6.04	302.05	C
			B	0.197	2.605	0.59	1	1	46.567			
			C	0.296	2.306	0.615	1	1	66.937			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	1	1	62.481	5.76	288.09	C
			B	0.157	2.744	0.583	1	1	40.915			
			C	0.248	2.444	0.601	1	1	61.828			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	1	1	68.956	6.22	311.10	C
			B	0.259	2.41	0.604	1	1	68.956			
			C	0.243	2.458	0.6	1	1	68.318			
T8 180.00-	0.94	5.93	A	0.244	2.456	0.6	1	1	72.996	6.44	321.79	A

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	27 of 51
	Project	CSP Tower - Colchester, CT	Date	08:00:06 03/14/06
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Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
160.00			B	0.24	2.469	0.599	1	1	71.095			
			C	0.225	2.515	0.596	1	1	70.560			
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	1	1	83.129	7.06	353.23	A
			B	0.24	2.469	0.599	1	1	80.649			
			C	0.226	2.51	0.596	1	1	80.248			
T10 140.00-120.00	0.95	7.17	A	0.231	2.495	0.597	1	1	86.099	7.14	357.13	A
			B	0.226	2.51	0.596	1	1	83.628			
			C	0.217	2.539	0.594	1	1	84.168			
T11 120.00-100.00	0.97	6.02	A	0.189	2.634	0.588	1	1	63.135	5.27	263.56	A
			B	0.181	2.661	0.587	1	1	58.581			
			C	0.177	2.676	0.586	1	1	60.852			
T12 100.00-80.00	1.01	6.17	A	0.177	2.676	0.586	1	1	64.536	5.38	268.86	C
			B	0.169	2.704	0.584	1	1	59.343			
			C	0.177	2.675	0.586	1	1	67.148			
T13 80.00-60.00	1.02	7.26	A	0.175	2.681	0.586	1	1	69.636	5.43	271.28	C
			B	0.168	2.707	0.584	1	1	64.443			
			C	0.177	2.676	0.586	1	1	72.781			
T14 60.00-30.00	1.52	11.34	A	0.163	2.724	0.584	1	1	106.104	7.40	246.70	C
			B	0.156	2.749	0.582	1	1	98.370			
			C	0.164	2.72	0.584	1	1	110.828			
T15 30.00-0.00	1.52	13.09	A	0.152	2.763	0.582	1	1	109.153	7.06	235.34	C
			B	0.146	2.785	0.581	1	1	101.438			
			C	0.153	2.759	0.582	1	1	113.887			
Sum Weight:	10.89	90.92						OTM	13335.51 kip-ft	85.84		

### Tower Forces - Service - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	0.825	1	20.596	2.86	143.07	C
			B	0.208	2.571	0.592	0.825	1	20.596			
			C	0.29	2.323	0.613	0.825	1	28.900			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	0.825	1	23.485	3.45	172.49	C
			B	0.207	2.573	0.592	0.825	1	23.485			
			C	0.314	2.259	0.62	0.825	1	36.530			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	0.825	1	28.505	4.27	213.31	C
			B	0.2	2.595	0.59	0.825	1	28.505			
			C	0.319	2.246	0.622	0.825	1	46.362			
T4 260.00-240.00	0.16	3.83	A	0.189	2.634	0.588	0.825	1	32.980	4.79	239.39	C
			B	0.189	2.634	0.588	0.825	1	32.980			
			C	0.297	2.305	0.615	0.825	1	51.835			
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	0.825	1	41.390	5.51	275.36	C
			B	0.197	2.605	0.59	0.825	1	41.390			
			C	0.296	2.306	0.615	0.825	1	61.021			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	0.825	1	58.802	5.36	267.98	A
			B	0.157	2.744	0.583	0.825	1	36.691			
			C	0.248	2.444	0.601	0.825	1	56.680			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	0.825	1	64.873	5.79	289.63	B
			B	0.259	2.41	0.604	0.825	1	64.873			
			C	0.243	2.458	0.6	0.825	1	62.769			
T8 180.00-160.00	0.94	5.93	A	0.244	2.456	0.6	0.825	1	68.154	6.01	300.44	A
			B	0.24	2.469	0.599	0.825	1	66.571			
			C	0.225	2.515	0.596	0.825	1	64.572			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	28 of 51
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Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	0.825	1	76.515	6.50	325.12	A
			B	0.24	2.469	0.599	0.825	1	74.452			
			C	0.226	2.51	0.596	0.825	1	72.568			
T10 140.00-120.00	0.95	7.17	A	0.231	2.495	0.597	0.825	1	78.924	6.55	327.36	A
			B	0.226	2.51	0.596	0.825	1	76.871			
			C	0.217	2.539	0.594	0.825	1	75.954			
T11 120.00-100.00	0.97	6.02	A	0.189	2.634	0.588	0.825	1	62.332	5.20	260.20	A
			B	0.181	2.661	0.587	0.825	1	58.581			
			C	0.177	2.676	0.586	0.825	1	59.256			
T12 100.00-80.00	1.01	6.17	A	0.177	2.676	0.586	0.825	1	63.618	5.22	260.81	C
			B	0.169	2.704	0.584	0.825	1	59.343			
			C	0.177	2.675	0.586	0.825	1	65.140			
T13 80.00-60.00	1.02	7.26	A	0.175	2.681	0.586	0.825	1	68.717	5.28	263.79	C
			B	0.168	2.707	0.584	0.825	1	64.443			
			C	0.177	2.676	0.586	0.825	1	70.772			
T14 60.00-30.00	1.52	11.34	A	0.163	2.724	0.584	0.825	1	104.727	7.20	240.00	C
			B	0.156	2.749	0.582	0.825	1	98.370			
			C	0.164	2.72	0.584	0.825	1	107.815			
T15 30.00-0.00	1.52	13.09	A	0.152	2.763	0.582	0.825	1	107.775	6.87	229.11	C
			B	0.146	2.785	0.581	0.825	1	101.438			
			C	0.153	2.759	0.582	0.825	1	110.874			
Sum Weight:	10.89	90.92						OTM	12415.37 kip-ft	80.85		

### Tower Forces - Service - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	0.8	1	20.305	2.83	141.29	C
			B	0.208	2.571	0.592	0.8	1	20.305			
			C	0.29	2.323	0.613	0.8	1	28.541			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	0.8	1	23.170	3.41	170.28	C
			B	0.207	2.573	0.592	0.8	1	23.170			
			C	0.314	2.259	0.62	0.8	1	36.062			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	0.8	1	28.157	4.22	211.03	C
			B	0.2	2.595	0.59	0.8	1	28.157			
			C	0.319	2.246	0.622	0.8	1	45.868			
T4 260.00-240.00	0.16	3.83	A	0.189	2.634	0.588	0.8	1	32.494	4.73	236.54	C
			B	0.189	2.634	0.588	0.8	1	32.494			
			C	0.297	2.305	0.615	0.8	1	51.217			
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	0.8	1	40.651	5.43	271.55	C
			B	0.197	2.605	0.59	0.8	1	40.651			
			C	0.296	2.306	0.615	0.8	1	60.176			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	0.8	1	58.276	5.31	265.58	A
			B	0.157	2.744	0.583	0.8	1	36.088			
			C	0.248	2.444	0.601	0.8	1	55.944			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	0.8	1	64.290	5.74	287.03	B
			B	0.259	2.41	0.604	0.8	1	64.290			
			C	0.243	2.458	0.6	0.8	1	61.977			
T8 180.00-160.00	0.94	5.93	A	0.244	2.456	0.6	0.8	1	67.462	5.95	297.40	A
			B	0.24	2.469	0.599	0.8	1	65.924			
			C	0.225	2.515	0.596	0.8	1	63.717			
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	0.8	1	75.570	6.42	321.10	A
			B	0.24	2.469	0.599	0.8	1	73.567			

<b>ERITower</b>  <b>URS Corporation</b> 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: (860) 529-8882 FAX: (860) 529-3991	Job	320' Rohn SSVMW	Page	29 of 51
	Project	CSP Tower - Colchester, CT	Date	08:00:06 03/14/06
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Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T10 140.00-120.00	0.95	7.17	A	0.226	2.51	0.596	0.8	1	71.470	6.46	323.11	A
			A	0.231	2.495	0.597	0.8	1	77.898			
			B	0.226	2.51	0.596	0.8	1	75.905			
T11 120.00-100.00	0.97	6.02	C	0.217	2.539	0.594	0.8	1	74.781	5.19	259.72	A
			A	0.189	2.634	0.588	0.8	1	62.217			
			B	0.181	2.661	0.587	0.8	1	58.581			
T12 100.00-80.00	1.01	6.17	C	0.177	2.676	0.586	0.8	1	59.028	5.19	259.66	C
			A	0.177	2.676	0.586	0.8	1	63.487			
			B	0.169	2.704	0.584	0.8	1	59.343			
T13 80.00-60.00	1.02	7.26	C	0.177	2.675	0.586	0.8	1	64.853	5.25	262.72	C
			A	0.175	2.681	0.586	0.8	1	68.586			
			B	0.168	2.707	0.584	0.8	1	64.443			
T14 60.00-30.00	1.52	11.34	C	0.177	2.676	0.586	0.8	1	70.485	7.17	239.04	C
			A	0.163	2.724	0.584	0.8	1	104.530			
			B	0.156	2.749	0.582	0.8	1	98.370			
T15 30.00-0.00	1.52	13.09	C	0.164	2.72	0.584	0.8	1	107.385	6.85	228.22	C
			A	0.152	2.763	0.582	0.8	1	107.579			
			B	0.146	2.785	0.581	0.8	1	101.438			
Sum Weight:	10.89	90.92	C	0.153	2.759	0.582	0.8	1	110.443	80.16		
								OTM	12287.69 kip-ft			

**Tower Forces - Service - Wind 90 To Face**

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 320.00-300.00	0.07	1.79	A	0.208	2.571	0.592	0.85	1	20.888	2.90	144.85	C
			B	0.208	2.571	0.592	0.85	1	20.888			
			C	0.29	2.323	0.613	0.85	1	29.260			
T2 300.00-280.00	0.10	2.50	A	0.207	2.573	0.592	0.85	1	23.800	3.49	174.71	C
			B	0.207	2.573	0.592	0.85	1	23.800			
			C	0.314	2.259	0.62	0.85	1	36.999			
T3 280.00-260.00	0.15	3.48	A	0.2	2.595	0.59	0.85	1	28.854	4.31	215.58	C
			B	0.2	2.595	0.59	0.85	1	28.854			
			C	0.319	2.246	0.622	0.85	1	46.855			
T4 260.00-240.00	0.16	3.83	A	0.189	2.634	0.588	0.85	1	33.466	4.84	242.25	C
			B	0.189	2.634	0.588	0.85	1	33.466			
			C	0.297	2.305	0.615	0.85	1	52.454			
T5 240.00-220.00	0.18	4.90	A	0.197	2.605	0.59	0.85	1	42.130	5.58	279.17	C
			B	0.197	2.605	0.59	0.85	1	42.130			
			C	0.296	2.306	0.615	0.85	1	61.866			
T6 220.00-200.00	0.43	4.82	A	0.266	2.39	0.606	0.85	1	59.327	5.41	270.37	A
			B	0.157	2.744	0.583	0.85	1	37.294			
			C	0.248	2.444	0.601	0.85	1	57.415			
T7 200.00-180.00	0.93	5.71	A	0.259	2.41	0.604	0.85	1	65.457	5.84	292.24	B
			B	0.259	2.41	0.604	0.85	1	65.457			
			C	0.243	2.458	0.6	0.85	1	63.562			
T8 180.00-160.00	0.94	5.93	A	0.244	2.456	0.6	0.85	1	68.845	6.07	303.49	A
			B	0.24	2.469	0.599	0.85	1	67.217			
			C	0.225	2.515	0.596	0.85	1	65.428			
T9 160.00-140.00	0.94	6.89	A	0.245	2.454	0.601	0.85	1	77.460	6.58	329.13	A
			B	0.24	2.469	0.599	0.85	1	75.337			
			C	0.226	2.51	0.596	0.85	1	73.665			
T10 140.00-	0.95	7.17	A	0.231	2.495	0.597	0.85	1	79.949	6.63	331.62	A

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Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
120.00			B	0.226	2.51	0.596	0.85	1	77.836			
			C	0.217	2.539	0.594	0.85	1	77.128			
T11 120.00-100.00	0.97	6.02	A	0.189	2.634	0.588	0.85	1	62.447	5.21	260.68	A
			B	0.181	2.661	0.587	0.85	1	58.581			
			C	0.177	2.676	0.586	0.85	1	59.484			
T12 100.00-80.00	1.01	6.17	A	0.177	2.676	0.586	0.85	1	63.749	5.24	261.96	C
			B	0.169	2.704	0.584	0.85	1	59.343			
			C	0.177	2.675	0.586	0.85	1	65.426			
T13 80.00-60.00	1.02	7.26	A	0.175	2.681	0.586	0.85	1	68.849	5.30	264.86	C
			B	0.168	2.707	0.584	0.85	1	64.443			
			C	0.177	2.676	0.586	0.85	1	71.059			
T14 60.00-30.00	1.52	11.34	A	0.163	2.724	0.584	0.85	1	104.924	7.23	240.95	C
			B	0.156	2.749	0.582	0.85	1	98.370			
			C	0.164	2.72	0.584	0.85	1	108.246			
T15 30.00-0.00	1.52	13.09	A	0.152	2.763	0.582	0.85	1	107.972	6.90	230.00	C
			B	0.146	2.785	0.581	0.85	1	101.438			
			C	0.153	2.759	0.582	0.85	1	111.304			
Sum Weight:	10.89	90.92						OTM	12543.06 kip-ft	81.55		

### Force Totals

Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, M <sub>x</sub> kip-ft	Sum of Overturning Moments, M <sub>z</sub> kip-ft	Sum of Torques kip-ft
Leg Weight	50.33					
Bracing Weight	40.59					
Total Member Self-Weight	90.92			-34.22	55.23	
Total Weight	108.78			-34.22	55.23	
Wind 0 deg - No Ice		0.00	-105.95	-17974.24	55.23	-174.23
Wind 30 deg - No Ice		50.81	-88.03	-14884.45	-8514.68	-132.07
Wind 45 deg - No Ice		71.36	-71.39	-12069.10	-11974.17	-99.68
Wind 60 deg - No Ice		86.80	-50.13	-8480.32	-14567.14	-60.99
Wind 90 deg - No Ice		101.61	0.00	-34.22	-17084.60	24.85
Wind 120 deg - No Ice		91.72	52.97	8935.78	-15474.58	109.53
Wind 135 deg - No Ice		71.36	71.39	12000.65	-11974.17	134.60
Wind 150 deg - No Ice		50.81	88.03	14816.00	-8514.68	156.92
Wind 180 deg - No Ice		0.00	100.26	16857.97	55.23	164.48
Wind 210 deg - No Ice		-50.81	88.03	14816.00	8625.14	132.07
Wind 225 deg - No Ice		-71.36	71.39	12000.65	12084.63	99.68
Wind 240 deg - No Ice		-91.72	52.97	8935.78	15585.04	64.70
Wind 270 deg - No Ice		-101.61	0.00	-34.22	17195.05	-24.85
Wind 300 deg - No Ice		-86.80	-50.13	-8480.32	14677.60	-103.48
Wind 315 deg - No Ice		-71.36	-71.39	-12069.10	12084.63	-134.60
Wind 330 deg - No Ice		-50.81	-88.03	-14884.45	8625.14	-156.92
Member Ice	20.16					
Total Weight Ice	150.33			-73.21	155.49	
Wind 0 deg - Ice		0.00	-129.09	-21697.94	155.49	-271.87
Wind 30 deg - Ice		61.94	-107.32	-18002.37	-10191.63	-203.60
Wind 45 deg - Ice		87.00	-87.03	-14605.84	-14371.08	-152.07
Wind 60 deg - Ice		105.83	-61.12	-10274.54	-17506.31	-91.00
Wind 90 deg - Ice		123.88	0.00	-73.21	-20538.76	43.71
Wind 120 deg - Ice		111.76	64.54	10739.15	-18564.66	175.44
Wind 135 deg - Ice		87.00	87.03	14459.41	-14371.08	213.44
Wind 150 deg - Ice		61.94	107.32	17855.94	-10191.63	247.31



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Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, $M_x$ kip-ft	Sum of Overturning Moments, $M_z$ kip-ft	Sum of Torques kip-ft
Wind 180 deg - Ice		0.00	122.25	20329.44	155.49	256.60
Wind 210 deg - Ice		-61.94	107.32	17855.94	10502.62	203.60
Wind 225 deg - Ice		-87.00	87.03	14459.41	14682.06	152.07
Wind 240 deg - Ice		-111.76	64.54	10739.15	18875.64	96.42
Wind 270 deg - Ice		-123.88	0.00	-73.21	20849.74	-43.71
Wind 300 deg - Ice		-105.83	-61.12	-10274.54	17817.30	-165.60
Wind 315 deg - Ice		-87.00	-87.03	-14605.84	14682.06	-213.44
Wind 330 deg - Ice		-61.94	-107.32	-18002.37	10502.62	-247.31
Total Weight	108.78			-34.22	55.23	
Wind 0 deg - Service		0.00	-105.95	-17939.26	-3.99	-174.23
Wind 30 deg - Service		50.81	-88.03	-14849.48	-8573.90	-132.07
Wind 45 deg - Service		71.36	-71.39	-12034.12	-12033.39	-99.68
Wind 60 deg - Service		86.80	-50.13	-8445.35	-14626.36	-60.99
Wind 90 deg - Service		101.61	0.00	0.75	-17143.81	24.85
Wind 120 deg - Service		91.72	52.97	8970.76	-15533.80	109.53
Wind 135 deg - Service		71.36	71.39	12035.63	-12033.39	134.60
Wind 150 deg - Service		50.81	88.03	14850.98	-8573.90	156.92
Wind 180 deg - Service		0.00	100.26	16892.95	-3.99	164.48
Wind 210 deg - Service		-50.81	88.03	14850.98	8565.93	132.07
Wind 225 deg - Service		-71.36	71.39	12035.63	12025.41	99.68
Wind 240 deg - Service		-91.72	52.97	8970.76	15525.82	64.70
Wind 270 deg - Service		-101.61	0.00	0.75	17135.84	-24.85
Wind 300 deg - Service		-86.80	-50.13	-8445.35	14618.38	-103.48
Wind 315 deg - Service		-71.36	-71.39	-12034.12	12025.41	-134.60
Wind 330 deg - Service		-50.81	-88.03	-14849.48	8565.93	-156.92

### Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 45 deg - No Ice
5	Dead+Wind 60 deg - No Ice
6	Dead+Wind 90 deg - No Ice
7	Dead+Wind 120 deg - No Ice
8	Dead+Wind 135 deg - No Ice
9	Dead+Wind 150 deg - No Ice
10	Dead+Wind 180 deg - No Ice
11	Dead+Wind 210 deg - No Ice
12	Dead+Wind 225 deg - No Ice
13	Dead+Wind 240 deg - No Ice
14	Dead+Wind 270 deg - No Ice
15	Dead+Wind 300 deg - No Ice
16	Dead+Wind 315 deg - No Ice
17	Dead+Wind 330 deg - No Ice
18	Dead+Ice+Temp
19	Dead+Wind 0 deg+Ice+Temp
20	Dead+Wind 30 deg+Ice+Temp
21	Dead+Wind 45 deg+Ice+Temp
22	Dead+Wind 60 deg+Ice+Temp
23	Dead+Wind 90 deg+Ice+Temp
24	Dead+Wind 120 deg+Ice+Temp
25	Dead+Wind 135 deg+Ice+Temp
26	Dead+Wind 150 deg+Ice+Temp

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Comb. No.	Description
27	Dead+Wind 180 deg+Ice+Temp
28	Dead+Wind 210 deg+Ice+Temp
29	Dead+Wind 225 deg+Ice+Temp
30	Dead+Wind 240 deg+Ice+Temp
31	Dead+Wind 270 deg+Ice+Temp
32	Dead+Wind 300 deg+Ice+Temp
33	Dead+Wind 315 deg+Ice+Temp
34	Dead+Wind 330 deg+Ice+Temp
35	Dead+Wind 0 deg - Service
36	Dead+Wind 30 deg - Service
37	Dead+Wind 45 deg - Service
38	Dead+Wind 60 deg - Service
39	Dead+Wind 90 deg - Service
40	Dead+Wind 120 deg - Service
41	Dead+Wind 135 deg - Service
42	Dead+Wind 150 deg - Service
43	Dead+Wind 180 deg - Service
44	Dead+Wind 210 deg - Service
45	Dead+Wind 225 deg - Service
46	Dead+Wind 240 deg - Service
47	Dead+Wind 270 deg - Service
48	Dead+Wind 300 deg - Service
49	Dead+Wind 315 deg - Service
50	Dead+Wind 330 deg - Service

### Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	320 - 300	Leg	Max Tension	27	14.72	-0.07	0.06
			Max. Compression	30	-18.74	0.55	-0.43
			Max. Mx	24	-17.72	-0.87	0.17
			Max. My	19	5.86	0.27	1.04
			Max. Vy	31	1.17	-0.19	-0.17
			Max. Vx	19	1.21	0.00	-0.10
		Diagonal	Max Tension	23	4.28	0.00	0.00
			Max. Compression	24	-4.35	0.00	0.00
			Max. Mx	30	3.16	0.01	0.00
			Max. My	34	-2.80	0.01	0.00
			Max. Vy	30	-0.01	0.01	0.00
			Max. Vx	34	0.00	0.00	0.00
		Top Girt	Max Tension	30	0.07	0.00	0.00
			Max. Compression	32	-0.10	0.00	0.00
			Max. Mx	18	-0.01	-0.02	0.00
T2	300 - 280	Leg	Max Tension	27	42.97	-0.12	0.03
			Max. Compression	30	-50.84	0.80	-0.05
			Max. Mx	30	-50.84	0.80	-0.05
			Max. My	34	-2.43	0.04	1.10
			Max. Vy	24	-0.27	0.41	-0.04
			Max. Vx	26	-0.51	-0.03	-0.24
		Diagonal	Max Tension	20	5.25	0.00	0.00
			Max. Compression	19	-5.43	0.00	0.00
			Max. Mx	32	2.58	0.02	0.01
			Max. My	34	-3.73	0.01	0.01
			Max. Vy	32	0.01	0.02	0.01
			Max. Vx	34	-0.00	0.00	0.00
		Top Girt	Max Tension	30	0.09	0.00	0.00

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T3	280 - 260	Leg	Max. Compression	32	-0.11	0.00	0.00
			Max. Mx	18	-0.01	-0.03	0.00
			Max. My	18	-0.01	0.00	0.00
			Max. Vy	18	0.02	0.00	0.00
			Max. Vx	18	0.00	0.00	0.00
			Max Tension	32	70.68	-0.59	0.00
			Max. Compression	30	-83.19	1.00	0.00
		Diagonal	Max. Mx	30	-83.19	1.00	0.00
			Max. My	34	-5.50	-0.02	1.07
			Max. Vy	27	0.24	-0.64	0.01
			Max. Vx	31	0.34	-0.02	-0.64
			Max Tension	20	6.53	0.00	0.00
			Max. Compression	19	-6.60	0.00	0.00
			Max. Mx	27	3.37	0.04	-0.01
T4	260 - 240	Leg	Max. My	26	-3.90	0.02	-0.01
			Max. Vy	27	0.02	0.04	-0.01
			Max. Vx	26	0.00	0.00	0.00
			Max Tension	32	100.97	-0.71	0.00
			Max. Compression	30	-119.28	1.91	-0.02
			Max. Mx	19	-117.70	1.91	0.37
			Max. My	34	-7.66	0.03	1.91
		Diagonal	Max. Vy	19	-0.38	1.91	0.37
			Max. Vx	26	0.48	0.03	-1.91
			Max Tension	20	7.97	0.00	0.00
			Max. Compression	19	-8.13	0.00	0.00
			Max. Mx	27	4.43	0.06	-0.01
			Max. My	34	-5.17	0.04	0.01
			Max. Vy	27	0.03	0.06	-0.01
T5	240 - 220	Leg	Max. Vx	27	0.02	0.04	-0.01
			Max. Vx	26	0.00	0.00	0.00
			Max Tension	32	133.35	-0.84	0.00
			Max. Compression	30	-158.39	1.68	-0.01
			Max. Mx	19	-130.04	1.91	0.37
			Max. My	34	-7.95	0.03	1.91
			Max. Vy	19	0.29	1.91	0.37
		Diagonal	Max. Vx	34	0.32	0.03	1.91
			Max Tension	20	9.52	0.00	0.00
			Max. Compression	19	-9.68	0.00	0.00
			Max. Mx	30	7.35	0.13	-0.01
			Max. My	34	-5.99	0.07	0.03
			Max. Vy	27	0.06	0.13	-0.02
			Max. Vx	34	-0.00	0.00	0.00
T6	220 - 200	Leg	Max Tension	32	166.90	-0.85	0.20
			Max. Compression	30	-200.48	1.88	-0.03
			Max. Mx	24	-197.86	1.88	-0.21
			Max. My	34	-12.08	-0.11	2.63
			Max. Vy	22	-1.58	-1.58	0.01
			Max. Vx	26	-1.44	0.01	-0.78
			Max Tension	20	13.07	0.00	0.00
		Diagonal	Max. Compression	19	-13.32	0.00	0.00
			Max. Mx	30	10.15	0.22	-0.03
			Max. My	34	8.94	0.20	0.04
			Max. Vy	33	0.08	0.21	0.04
			Max. Vx	19	-0.01	0.00	0.00
			Max Tension	32	207.01	-1.90	-0.00
			Max. Compression	30	-250.20	2.40	-0.06
T7	200 - 180	Leg	Max. Mx	24	-247.03	2.41	-0.22
			Max. My	31	-13.42	-0.11	-2.77
			Max. Vy	27	-1.73	-1.68	-0.24
			Max. Vx	26	-1.53	0.06	-0.44
			Max Tension	20	15.38	0.00	0.00
			Max. Compression	20	-15.43	0.00	0.00
			Diagonal	Max. Mx	24	-247.03	2.41
		Max. My		31	-13.42	-0.11	-2.77
		Max. Vy		27	-1.73	-1.68	-0.24
		Max. Vx		26	-1.53	0.06	-0.44
		Max Tension		20	15.38	0.00	0.00
		Max. Compression		20	-15.43	0.00	0.00
		Max. Mx		24	-247.03	2.41	-0.22

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft		
T8	180 - 160	Leg	Max. Mx	32	9.43	0.23	0.03		
			Max. My	19	-15.01	0.09	0.04		
			Max. Vy	32	0.08	0.23	0.03		
			Max. Vx	19	-0.01	0.00	0.00		
			Max Tension	32	248.31	-2.30	0.21		
			Max. Compression	30	-300.55	3.02	-0.10		
			Max. Mx	30	-300.55	3.02	-0.10		
		Diagonal	Max. My	34	-20.85	0.10	2.70		
			Max. Vy	22	0.38	-2.31	0.07		
			Max. Vx	34	-0.63	-0.10	2.42		
			Max Tension	20	17.16	0.00	0.00		
			Max. Compression	20	-17.31	0.00	0.00		
			Max. Mx	32	10.23	0.26	0.04		
			Max. My	19	-16.78	0.16	0.04		
T9	160 - 140	Leg	Max. Vy	32	0.09	0.26	0.04		
			Max. Vx	19	-0.01	0.00	0.00		
			Max Tension	32	290.55	-1.92	0.02		
			Max. Compression	30	-352.33	3.66	-0.11		
			Max. Mx	30	-352.33	3.66	-0.11		
			Max. My	34	-22.67	-0.23	3.33		
			Max. Vy	24	-0.40	3.64	-0.14		
		Diagonal	Max. Vx	34	0.56	-0.23	3.33		
			Max Tension	20	19.33	0.00	0.00		
			Max. Compression	19	-19.91	0.00	0.00		
			Max. Mx	30	13.84	0.43	-0.05		
			Max. My	19	11.37	0.43	0.07		
			Max. Vy	33	0.13	0.41	0.06		
			Max. Vx	19	-0.01	0.00	0.00		
T10	140 - 120	Leg	Max Tension	32	330.59	-2.55	0.04		
			Max. Compression	30	-402.87	-3.63	0.05		
			Max. Mx	30	-378.06	3.66	-0.11		
			Max. My	34	-28.34	-0.87	5.86		
			Max. Vy	24	0.78	2.46	-0.02		
			Max. Vx	34	-0.70	-0.12	4.70		
			Diagonal	Max Tension	20	19.72	0.00	0.00	
		Max. Compression		19	-20.18	0.00	0.00		
		Max. Mx		29	9.22	0.50	-0.07		
		Max. My		34	13.08	0.46	0.09		
		Max. Vy		29	-0.14	0.50	-0.07		
		Max. Vx		34	-0.01	0.00	0.00		
		T11		120 - 100	Leg	Max Tension	32	336.53	1.79
			Max. Compression			30	-412.23	-14.16	-0.63
Max. Mx	30		-411.84			17.85	0.55		
Max. My	34		-30.43			-2.11	11.23		
Max. Vy	30		3.47			17.85	0.55		
Max. Vx	26		2.00			-2.12	-11.20		
Diagonal	Max Tension		20			30.02	-0.20	0.06	
	Max. Compression		19		-32.07	0.00	0.00		
	Max. Mx		27		18.72	-0.27	0.06		
	Max. My		27		15.80	-0.27	0.06		
	Max. Vy		27		-0.07	-0.27	0.06		
	Max. Vx		27		-0.01	0.00	0.00		
	Horizontal		Max Tension		20	16.80	-0.21	0.00	
Max. Compression			19		-16.77	-0.26	-0.02		
Max. Mx		32	-2.82	-0.30	-0.03				
Max. My		24	-1.51	-0.13	0.04				
Max. Vy		32	0.09	-0.30	-0.03				
Max. Vx		24	-0.00	0.00	0.00				
Redund Horz 1 Bracing		Max Tension	30	6.20	0.00	0.00			
		Max. Compression	30	-6.20	0.00	0.00			

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T12	100 - 80	Redund Diag 1 Bracing	Max. Mx	18	0.45	0.02	0.00
			Max. Vy	18	-0.01	0.00	0.00
			Max Tension	30	5.63	0.00	0.00
			Max. Compression	30	-5.63	0.00	0.00
			Max. Mx	18	0.41	0.04	0.00
			Max. Vy	18	-0.02	0.00	0.00
			Max Tension	19	0.28	0.00	0.00
			Max. Compression	19	-0.30	0.00	0.00
			Max. Mx	18	-0.01	0.20	0.00
			Max. Vy	18	-0.06	0.00	0.00
			Max Tension	32	367.95	9.64	1.06
			Max. Compression	30	-452.69	-15.14	-0.60
		Inner Bracing	Max. Mx	30	-451.86	19.42	0.45
			Max. My	34	-33.80	-2.32	11.58
			Max. Vy	30	3.70	19.42	0.45
			Max. Vx	26	-2.18	-2.11	-11.20
			Max Tension	20	31.36	-0.21	0.06
			Max. Compression	19	-33.48	0.00	0.00
			Max. Mx	28	9.37	-0.27	0.06
			Max. My	28	13.28	-0.27	0.06
			Max. Vy	28	0.07	-0.27	0.06
			Max. Vx	28	0.01	0.00	0.00
			Max Tension	20	18.65	-0.25	0.00
			Max. Compression	19	-18.90	-0.30	-0.02
		Diagonal	Max. Mx	32	0.34	-0.33	-0.04
			Max. My	24	-0.44	-0.18	0.04
			Max. Vy	32	-0.09	-0.33	-0.04
			Max. Vx	24	-0.00	0.00	0.00
			Max Tension	30	6.81	0.00	0.00
			Max. Compression	30	-6.81	0.00	0.00
			Max. Mx	18	0.43	0.03	0.00
			Max. Vy	18	-0.01	0.00	0.00
			Max Tension	30	5.78	0.00	0.00
			Max. Compression	30	-5.78	0.00	0.00
			Max. Mx	18	0.42	0.05	0.00
			Max. Vy	18	0.02	0.00	0.00
		Horizontal	Max Tension	19	0.33	0.00	0.00
			Max. Compression	19	-0.34	0.00	0.00
			Max. Mx	18	0.00	0.25	0.00
			Max. Vy	18	-0.07	0.00	0.00
Max Tension	32		401.17	10.18	1.33		
Max. Compression	30		-495.66	-15.22	-0.93		
Max. Mx	30		-494.71	23.17	0.73		
Max. My	34		-37.61	-3.08	19.05		
Max. Vy	30		3.96	23.17	0.73		
Max. Vx	34		-2.82	-3.08	19.05		
Max Tension	20		29.21	-0.21	0.07		
Max. Compression	19		-31.99	0.00	0.00		
Redund Horiz 1 Bracing	Max. Mx	27	16.39	-0.26	0.07		
	Max. My	27	19.66	-0.26	0.07		
	Max. Vy	27	-0.07	-0.26	0.07		
	Max. Vx	27	0.01	0.00	0.00		
	Max Tension	20	18.43	-0.38	0.00		
	Max. Compression	19	-18.90	-0.42	-0.02		
	Max. Mx	32	-3.92	-0.45	-0.03		
	Max. My	24	-0.54	-0.30	0.03		
	Max. Vy	32	0.13	-0.45	-0.03		
	Max. Vx	24	-0.00	0.00	0.00		

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T14	60 - 30	Redund Horz 1 Bracing	Max Tension	30	7.47	0.00	0.00
			Max. Compression	30	-7.45	0.00	0.00
			Max. Mx	18	0.57	0.04	0.00
		Redund Diag 1 Bracing	Max. Vy	18	-0.02	0.00	0.00
			Max Tension	30	5.96	0.00	0.00
			Max. Compression	30	-5.97	0.00	0.00
		Inner Bracing	Max. Mx	18	0.44	0.06	0.00
			Max. Vy	18	-0.02	0.00	0.00
			Max Tension	19	0.33	0.00	0.00
		Leg	Max. Compression	19	-0.34	0.00	0.00
			Max. Mx	18	0.00	0.29	0.00
			Max. Vy	18	0.08	0.00	0.00
		Diagonal	Max Tension	32	432.96	8.81	1.73
			Max. Compression	30	-537.63	4.25	0.56
			Max. Mx	30	-530.19	29.94	0.80
			Max. My	34	-42.84	-1.71	23.74
			Max. Vy	30	5.59	29.94	0.80
			Max. Vx	26	3.29	-1.72	-23.72
			Max Tension	20	42.81	-0.32	0.12
			Max. Compression	19	-46.45	0.00	0.00
			Max. Mx	32	27.36	-0.38	-0.12
			Max. My	32	31.95	-0.38	-0.12
			Max. Vy	19	-0.08	-0.16	-0.12
			Max. Vx	32	-0.01	0.00	0.00
		Horizontal	Max Tension	20	21.79	-0.55	0.00
			Max. Compression	20	-21.67	-0.55	0.00
			Max. Mx	32	-2.75	-0.71	-0.04
			Max. My	24	-1.77	-0.38	0.04
			Max. Vy	32	0.17	-0.71	-0.04
		Redund Horz 1 Bracing	Max. Vx	24	-0.00	0.00	0.00
			Max Tension	30	8.10	0.00	0.00
			Max. Compression	30	-8.29	0.00	0.00
		Redund Horz 2 Bracing	Max. Mx	18	0.54	0.02	0.00
			Max. Vy	18	-0.01	0.00	0.00
			Max Tension	30	8.10	0.00	0.00
		Redund Diag 1 Bracing	Max. Compression	30	-8.17	0.00	0.00
			Max. Mx	18	0.54	0.10	0.00
			Max. Vy	18	-0.04	0.00	0.00
		Redund Diag 2 Bracing	Max Tension	30	8.37	0.00	0.00
			Max. Compression	30	-8.18	0.00	0.00
			Max. Mx	18	0.83	0.04	0.00
		Inner Bracing	Max. Vy	18	-0.01	0.00	0.00
Max Tension	30		5.36	0.00	0.00		
Max. Compression	30		-5.30	0.00	0.00		
Leg	Max. Mx	18	0.48	0.15	0.00		
	Max. Vy	18	-0.04	0.00	0.00		
	Max Tension	28	0.36	0.00	0.00		
T15	30 - 0	Leg	Max. Compression	28	-0.40	0.00	0.00
			Max. Mx	18	-0.02	0.34	0.00
			Max. Vy	18	-0.08	0.00	0.00
		Max Tension	32	481.08	20.88	2.76	
		Max. Compression	30	-600.87	6.07	0.67	
		Max. Mx	30	-597.80	25.43	1.05	
Max. My	34	-46.75	-1.71	23.74			
Max. Vy	30	-3.19	6.07	0.67			

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
		Diagonal	Max. Vx	26	-3.23	-1.72	-23.71
			Max Tension	20	40.13	-0.29	0.13
			Max. Compression	19	-42.08	0.00	0.00
			Max. Mx	32	27.07	-0.36	-0.14
			Max. My	27	-36.11	-0.21	0.14
			Max. Vy	19	-0.08	-0.20	-0.14
		Horizontal	Max. Vx	27	-0.01	-0.21	0.14
			Max Tension	20	21.72	-0.56	0.00
			Max. Compression	19	-23.11	-0.63	-0.03
			Max. Mx	32	-1.07	-0.69	-0.06
			Max. My	24	0.66	-0.41	0.06
			Max. Vy	32	-0.17	-0.69	-0.06
		Redund Horz 1 Bracing	Max. Vx	24	0.00	-0.41	0.06
			Max Tension	30	9.10	0.00	0.00
			Max. Compression	30	-9.07	0.00	0.00
		Redund Horz 2 Bracing	Max. Mx	18	0.67	0.02	0.00
			Max. Vy	18	-0.01	0.00	0.00
			Max Tension	30	9.07	0.00	0.00
		Redund Diag 1 Bracing	Max. Compression	30	-9.26	0.00	0.00
			Max. Mx	18	0.64	0.18	0.00
			Max. Vy	18	-0.06	0.00	0.00
		Redund Diag 2 Bracing	Max Tension	30	8.43	0.00	0.00
			Max. Compression	30	-8.47	0.00	0.00
			Max. Mx	18	0.69	0.04	0.00
		Inner Bracing	Max. Vy	18	-0.01	0.00	0.00
			Max Tension	30	5.83	0.00	0.00
			Max. Compression	30	-5.67	0.00	0.00
		Inner Bracing	Max. Mx	18	0.63	0.18	0.00
			Max. Vy	18	-0.05	0.00	0.00
			Max Tension	19	0.40	0.00	0.00
		Inner Bracing	Max. Compression	19	-0.42	0.00	0.00
			Max. Mx	18	0.01	0.42	0.00
			Max. Vy	18	-0.09	0.00	0.00

### Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg C	Max. Vert	30	656.47	68.11	-40.99
	Max. H <sub>x</sub>	30	656.47	68.11	-40.99
	Max. H <sub>z</sub>	21	-501.03	-53.88	36.62
	Min. Vert	22	-516.34	-57.34	34.51
	Min. H <sub>x</sub>	22	-516.34	-57.34	34.51
Leg B	Min. H <sub>z</sub>	29	606.81	61.17	-40.99
	Max. Vert	24	648.83	-69.96	-37.51
	Max. H <sub>x</sub>	32	-523.98	59.23	31.50
	Max. H <sub>z</sub>	33	-508.67	56.55	32.26
	Min. Vert	32	-523.98	59.23	31.50
Leg A	Min. H <sub>x</sub>	24	648.83	-69.96	-37.51
	Min. H <sub>z</sub>	24	648.83	-69.96	-37.51
	Max. Vert	19	655.95	-3.93	79.48
	Max. H <sub>x</sub>	30	-249.69	10.65	-33.38

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Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Max. H <sub>z</sub>	19	655.95	-3.93	79.48
	Min. Vert	27	-517.22	3.56	-66.95
	Min. H <sub>x</sub>	22	336.89	-10.30	39.83
	Min. H <sub>z</sub>	27	-517.22	3.56	-66.95

### Tower Mast Reaction Summary

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Dead Only	108.78	-0.00	-0.00	-34.22	55.23	0.00
Dead+Wind 0 deg - No Ice	108.78	-0.00	-105.95	-17729.73	55.23	-174.23
Dead+Wind 30 deg - No Ice	108.78	50.81	-88.03	-14677.30	-8395.09	-132.07
Dead+Wind 45 deg - No Ice	108.78	71.36	-71.39	-11900.59	-11805.67	-99.68
Dead+Wind 60 deg - No Ice	108.78	86.80	-50.13	-8361.62	-14361.53	-61.00
Dead+Wind 90 deg - No Ice	108.78	101.61	-0.00	-34.22	-16845.40	24.85
Dead+Wind 120 deg - No Ice	108.78	91.72	52.97	8813.53	-15262.82	109.53
Dead+Wind 135 deg - No Ice	108.78	71.36	71.39	11832.14	-11805.67	134.60
Dead+Wind 150 deg - No Ice	108.78	50.81	88.03	14608.86	-8395.09	156.93
Dead+Wind 180 deg - No Ice	108.78	-0.00	100.26	16620.56	55.23	164.48
Dead+Wind 210 deg - No Ice	108.78	-50.81	88.03	14608.86	8505.55	132.07
Dead+Wind 225 deg - No Ice	108.78	-71.36	71.39	11832.14	11916.12	99.68
Dead+Wind 240 deg - No Ice	108.78	-91.72	52.97	8813.53	15373.28	64.70
Dead+Wind 270 deg - No Ice	108.78	-101.61	-0.00	-34.22	16955.86	-24.85
Dead+Wind 300 deg - No Ice	108.78	-86.80	-50.13	-8361.62	14471.99	-103.48
Dead+Wind 315 deg - No Ice	108.78	-71.36	-71.39	-11900.59	11916.12	-134.60
Dead+Wind 330 deg - No Ice	108.78	-50.81	-88.03	-14677.30	8505.55	-156.93
Dead+Ice+Temp	150.33	-0.00	-0.00	-73.21	155.49	0.00
Dead+Wind 0 deg+Ice+Temp	150.33	-0.00	-129.09	-21348.89	155.49	-271.87
Dead+Wind 30 deg+Ice+Temp	150.33	61.94	-107.32	-17707.59	-10021.44	-203.60
Dead+Wind 45 deg+Ice+Temp	150.33	87.00	-87.03	-14366.18	-14131.42	-152.07
Dead+Wind 60 deg+Ice+Temp	150.33	105.83	-61.12	-10105.80	-17214.04	-91.00
Dead+Wind 90 deg+Ice+Temp	150.33	123.88	-0.00	-73.21	-20198.38	43.71
Dead+Wind 120 deg+Ice+Temp	150.33	111.76	64.54	10564.62	-18262.37	175.44
Dead+Wind 135 deg+Ice+Temp	150.33	87.00	87.03	14219.75	-14131.42	213.44
Dead+Wind 150 deg+Ice+Temp	150.33	61.94	107.32	17561.16	-10021.44	247.31
Dead+Wind 180 deg+Ice+Temp	150.33	-0.00	122.25	19991.95	155.49	256.60
Dead+Wind 210 deg+Ice+Temp	150.33	-61.94	107.32	17561.16	10332.43	203.60
Dead+Wind 225 deg+Ice+Temp	150.33	-87.00	87.03	14219.75	14442.40	152.07
Dead+Wind 240 deg+Ice+Temp	150.33	-111.76	64.54	10564.62	18573.35	96.43
Dead+Wind 270 deg+Ice+Temp	150.33	-123.88	-0.00	-73.21	20509.36	-43.71
Dead+Wind 300 deg+Ice+Temp	150.33	-105.83	-61.12	-10105.80	17525.02	-165.60
Dead+Wind 315 deg+Ice+Temp	150.33	-87.00	-87.03	-14366.18	14442.40	-213.44
Dead+Wind 330 deg+Ice+Temp	150.33	-61.94	-107.32	-17707.59	10332.43	-247.31
Dead+Wind 0 deg - Service	108.78	-0.00	-105.95	-17729.73	55.23	-174.23
Dead+Wind 30 deg - Service	108.78	50.81	-88.03	-14677.30	-8395.09	-132.07
Dead+Wind 45 deg - Service	108.78	71.36	-71.39	-11900.59	-11805.67	-99.68
Dead+Wind 60 deg - Service	108.78	86.80	-50.13	-8361.62	-14361.53	-61.00
Dead+Wind 90 deg - Service	108.78	101.61	-0.00	-34.22	-16845.40	24.85
Dead+Wind 120 deg - Service	108.78	91.72	52.97	8813.53	-15262.82	109.53
Dead+Wind 135 deg - Service	108.78	71.36	71.39	11832.14	-11805.67	134.60
Dead+Wind 150 deg - Service	108.78	50.81	88.03	14608.86	-8395.09	156.93
Dead+Wind 180 deg - Service	108.78	-0.00	100.26	16620.56	55.23	164.48
Dead+Wind 210 deg - Service	108.78	-50.81	88.03	14608.86	8505.55	132.07
Dead+Wind 225 deg - Service	108.78	-71.36	71.39	11832.14	11916.12	99.68
Dead+Wind 240 deg - Service	108.78	-91.72	52.97	8813.53	15373.28	64.70
Dead+Wind 270 deg - Service	108.78	-101.61	-0.00	-34.22	16955.86	-24.85



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Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturing Moment, M <sub>x</sub> kip-ft	Overturing Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Dead+Wind 300 deg - Service	108.78	-86.80	-50.13	-8361.62	14471.99	-103.48
Dead+Wind 315 deg - Service	108.78	-71.36	-71.39	-11900.59	11916.12	-134.60
Dead+Wind 330 deg - Service	108.78	-50.81	-88.03	-14677.30	8505.55	-156.93

## Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-108.78	0.00	0.00	108.78	0.00	0.000%
2	-0.00	-108.78	-105.95	0.00	108.78	105.95	0.000%
3	50.81	-108.78	-88.03	-50.81	108.78	88.03	0.000%
4	71.36	-108.78	-71.39	-71.36	108.78	71.39	0.000%
5	86.80	-108.78	-50.13	-86.80	108.78	50.13	0.000%
6	101.61	-108.78	0.00	-101.61	108.78	0.00	0.000%
7	91.72	-108.78	52.97	-91.72	108.78	-52.97	0.000%
8	71.36	-108.78	71.39	-71.36	108.78	-71.39	0.000%
9	50.81	-108.78	88.03	-50.81	108.78	-88.03	0.000%
10	0.00	-108.78	100.26	0.00	108.78	-100.26	0.000%
11	-50.81	-108.78	88.03	50.81	108.78	-88.03	0.000%
12	-71.36	-108.78	71.39	71.36	108.78	-71.39	0.000%
13	-91.72	-108.78	52.97	91.72	108.78	-52.97	0.000%
14	-101.61	-108.78	0.00	101.61	108.78	0.00	0.000%
15	-86.80	-108.78	-50.13	86.80	108.78	50.13	0.000%
16	-71.36	-108.78	-71.39	71.36	108.78	71.39	0.000%
17	-50.81	-108.78	-88.03	50.81	108.78	88.03	0.000%
18	0.00	-150.33	0.00	0.00	150.33	0.00	0.000%
19	-0.00	-150.33	-129.09	0.00	150.33	129.09	0.000%
20	61.94	-150.33	-107.32	-61.94	150.33	107.32	0.000%
21	87.00	-150.33	-87.03	-87.00	150.33	87.03	0.000%
22	105.83	-150.33	-61.12	-105.83	150.33	61.12	0.000%
23	123.88	-150.33	0.00	-123.88	150.33	0.00	0.000%
24	111.76	-150.33	64.54	-111.76	150.33	-64.54	0.000%
25	87.00	-150.33	87.03	-87.00	150.33	-87.03	0.000%
26	61.94	-150.33	107.32	-61.94	150.33	-107.32	0.000%
27	0.00	-150.33	122.25	0.00	150.33	-122.25	0.000%
28	-61.94	-150.33	107.32	61.94	150.33	-107.32	0.000%
29	-87.00	-150.33	87.03	87.00	150.33	-87.03	0.000%
30	-111.76	-150.33	64.54	111.76	150.33	-64.54	0.000%
31	-123.88	-150.33	0.00	123.88	150.33	0.00	0.000%
32	-105.83	-150.33	-61.12	105.83	150.33	61.12	0.000%
33	-87.00	-150.33	-87.03	87.00	150.33	87.03	0.000%
34	-61.94	-150.33	-107.32	61.94	150.33	107.32	0.000%
35	-0.00	-108.78	-105.95	0.00	108.78	105.95	0.000%
36	50.81	-108.78	-88.03	-50.81	108.78	88.03	0.000%
37	71.36	-108.78	-71.39	-71.36	108.78	71.39	0.000%
38	86.80	-108.78	-50.13	-86.80	108.78	50.13	0.000%
39	101.61	-108.78	0.00	-101.61	108.78	0.00	0.000%
40	91.72	-108.78	52.97	-91.72	108.78	-52.97	0.000%
41	71.36	-108.78	71.39	-71.36	108.78	-71.39	0.000%
42	50.81	-108.78	88.03	-50.81	108.78	-88.03	0.000%
43	0.00	-108.78	100.26	0.00	108.78	-100.26	0.000%
44	-50.81	-108.78	88.03	50.81	108.78	-88.03	0.000%
45	-71.36	-108.78	71.39	71.36	108.78	-71.39	0.000%
46	-91.72	-108.78	52.97	91.72	108.78	-52.97	0.000%
47	-101.61	-108.78	0.00	101.61	108.78	0.00	0.000%
48	-86.80	-108.78	-50.13	86.80	108.78	50.13	0.000%
49	-71.36	-108.78	-71.39	71.36	108.78	71.39	0.000%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
50	-50.81	-108.78	-88.03	50.81	108.78	88.03	0.000%

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	320 - 300	23.780	46	0.6261	0.3050
T2	300 - 280	21.108	46	0.6163	0.2441
T3	280 - 260	18.508	46	0.5887	0.1950
T4	260 - 240	16.029	46	0.5612	0.1672
T5	240 - 220	13.673	46	0.5266	0.1466
T6	220 - 200	11.492	46	0.4851	0.1340
T7	200 - 180	9.495	46	0.4378	0.1242
T8	180 - 160	7.668	35	0.3966	0.1137
T9	160 - 140	6.009	35	0.3518	0.1012
T10	140 - 120	4.550	35	0.3033	0.0907
T11	120 - 100	3.296	35	0.2522	0.0797
T12	100 - 80	2.283	35	0.2027	0.0619
T13	80 - 60	1.465	35	0.1531	0.0477
T14	60 - 30	0.842	40	0.1113	0.0356
T15	30 - 0	0.258	40	0.0499	0.0176

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
320.00	Dual Lights	46	23.780	0.6261	0.3050	258298
318.00	PD128	46	23.512	0.6256	0.2988	258298
315.00	8 FT DISH	46	23.109	0.6248	0.2896	258298
308.00	6 FT DISH	46	22.172	0.6221	0.2681	107624
294.00	DB224	46	20.317	0.6092	0.2267	51672
292.00	PD320	46	20.056	0.6065	0.2212	48445
285.00	DB809	46	19.148	0.5961	0.2049	39757
275.00	OGT9	46	17.877	0.5817	0.1866	37488
257.00	PD440	46	15.667	0.5566	0.1637	41129
250.00	PD128	46	14.833	0.5450	0.1561	32802
243.00	PD320	46	14.016	0.5323	0.1492	27266
220.00	DB844	46	11.492	0.4851	0.1340	25732
200.00	PiROD 12' Lightweight T-Frame	46	9.495	0.4378	0.1242	28349
175.00	6 FT DISH	35	7.237	0.3859	0.1106	28313
140.00	BA1012-0	35	4.550	0.3033	0.0907	25284
138.00	PD156S	35	4.415	0.2982	0.0897	24653
115.00	6 FT DISH	35	3.021	0.2398	0.0757	19362
112.00	2 FT DISH	35	2.863	0.2324	0.0730	20614
105.00	6 FT DISH	35	2.516	0.2152	0.0664	24259
100.00	PD458	35	2.283	0.2027	0.0619	26912
97.00	6 FT DISH	35	2.148	0.1951	0.0594	27000
90.00	4 FT DISH	35	1.850	0.1773	0.0542	25554

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### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	320 - 300	28.635	30	0.7526	0.4153
T2	300 - 280	25.428	30	0.7408	0.3461
T3	280 - 260	22.304	30	0.7087	0.2863
T4	260 - 240	19.317	30	0.6765	0.2491
T5	240 - 220	16.476	30	0.6352	0.2200
T6	220 - 200	13.844	30	0.5854	0.2015
T7	200 - 180	11.435	30	0.5283	0.1870
T8	180 - 160	9.230	30	0.4785	0.1713
T9	160 - 140	7.228	30	0.4244	0.1535
T10	140 - 120	5.470	30	0.3657	0.1385
T11	120 - 100	3.960	30	0.3040	0.1228
T12	100 - 80	2.746	30	0.2441	0.0965
T13	80 - 60	1.765	19	0.1842	0.0745
T14	60 - 30	1.018	24	0.1340	0.0557
T15	30 - 0	0.316	24	0.0601	0.0275

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
320.00	Dual Lights	30	28.635	0.7526	0.4153	223130
318.00	PD128	30	28.313	0.7520	0.4084	223130
315.00	8 FT DISH	30	27.830	0.7509	0.3981	223130
308.00	6 FT DISH	30	26.705	0.7476	0.3740	92971
294.00	DB224	30	24.478	0.7326	0.3251	45004
292.00	PD320	30	24.164	0.7294	0.3191	42281
285.00	DB809	30	23.073	0.7174	0.2990	34892
275.00	OGT9	30	21.544	0.7006	0.2753	32751
257.00	PD440	30	18.881	0.6710	0.2443	34777
250.00	PD128	30	17.875	0.6571	0.2336	27563
243.00	PD320	30	16.891	0.6420	0.2238	22818
220.00	DB844	30	13.844	0.5854	0.2015	21205
200.00	PiROD 12' Lightweight T-Frame	30	11.435	0.5283	0.1870	23648
175.00	6 FT DISH	30	8.709	0.4656	0.1669	23277
140.00	BA1012-0	30	5.470	0.3657	0.1385	20953
138.00	PD156S	30	5.307	0.3596	0.1371	20401
115.00	6 FT DISH	30	3.630	0.2889	0.1169	15847
112.00	2 FT DISH	30	3.441	0.2800	0.1130	16918
105.00	6 FT DISH	30	3.025	0.2591	0.1033	20084
100.00	PD458	30	2.746	0.2441	0.0965	22432
97.00	6 FT DISH	30	2.584	0.2349	0.0928	22563
90.00	4 FT DISH	30	2.227	0.2134	0.0847	21441

### Bolt Design Data

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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T1	320	Leg	A325N	1.0000	6	2.45	34.56	0.071 ✓	1.333	Bolt Tension
T2	300	Leg	A325N	1.0000	8	5.37	34.56	0.155 ✓	1.333	Bolt Tension
T3	280	Leg	A325N	1.0000	8	8.84	34.56	0.256 ✓	1.333	Bolt Tension
T4	260	Leg	A325N	1.0000	8	12.62	34.56	0.365 ✓	1.333	Bolt Tension
T5	240	Leg	A325N	1.0000	8	16.67	34.56	0.482 ✓	1.333	Bolt Tension
T6	220	Leg	A325N	1.0000	12	13.91	34.56	0.402 ✓	1.333	Bolt Tension
T7	200	Leg	A325N	1.0000	12	17.25	34.56	0.499 ✓	1.333	Bolt Tension
T8	180	Leg	A325N	1.0000	12	20.69	34.56	0.599 ✓	1.333	Bolt Tension
T9	160	Leg	A325N	1.0000	12	24.21	34.56	0.701 ✓	1.333	Bolt Tension
T10	140	Leg	A325N	1.0000	12	27.55	34.56	0.797 ✓	1.333	Bolt Tension
T11	120	Leg	A325N	1.0000	12	27.96	34.55	0.809 ✓	1.333	Bolt Tension
		Horizontal	A325N	0.7500	2	8.40	9.28	0.906 ✓	1.333	Bolt Shear
T12	100	Leg	A325N	1.0000	16	22.91	34.56	0.663 ✓	1.333	Bolt Tension
		Horizontal	A325N	0.7500	2	9.45	9.28	1.018 ✓	1.333	Bolt Shear
T13	80	Leg	A325N	1.0000	16	25.02	34.56	0.724 ✓	1.333	Bolt Tension
		Horizontal	A325N	0.7500	2	9.45	9.28	1.019 ✓	1.333	Bolt Shear
T14	60	Leg	A325N	1.0000	16	26.51	34.55	0.767 ✓	1.333	Bolt Tension
		Horizontal	A325N	0.8750	2	10.89	12.63	0.863 ✓	1.333	Bolt Shear
T15	30	Leg	A325N	1.0000	24	19.82	34.56	0.573 ✓	1.333	Bolt Tension
		Horizontal	A325N	0.8750	2	11.55	12.63	0.915 ✓	1.333	Bolt Shear

### Compression Checks

### Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P/P <sub>a</sub>
T1	320 - 300	ROHN 5 EH	20.00	4.00	26.1 K=1.00	27.622	6.1120	-18.74	168.82	0.111 ✓
T2	300 - 280	ROHN 6 EH	20.03	5.01	27.4 K=1.00	27.470	8.4049	-50.84	230.89	0.220 ✓
T3	280 - 260	ROHN 8 EH	20.04	6.68	27.9 K=1.00	27.414	12.7627	-83.19	349.88	0.238 ✓
T4	260 - 240	ROHN 8 EH	20.03	6.68	27.8 K=1.00	27.415	12.7627	-119.28	349.89	0.341 ✓
T5	240 - 220	ROHN 8 EH	20.03	6.68	27.8 K=1.00	27.415	12.7627	-158.39	349.89	0.453 ✓
T6	220 - 200	ROHN 8 EH	20.03	10.02	41.8 K=1.00	25.582	12.7627	-200.48	326.50	0.614 ✓

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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T7	200 - 180	ROHN 10 EH	20.04	10.02	33.1 K=1.00	26.757	16.1007	-250.20	430.80	0.581
T8	180 - 160	ROHN 10 EH	20.04	10.02	33.1 K=1.00	26.757	16.1007	-300.55	430.81	0.698
T9	160 - 140	ROHN 10 EH	20.03	10.02	33.1 K=1.00	26.758	16.1007	-352.33	430.82	0.818
T10	140 - 120	ROHN 10 EH	20.04	10.02	33.1 K=1.00	26.756	16.1007	-402.87	430.79	0.935
T11	120 - 100	ROHN 10 EH	20.06	10.03	33.2 K=1.00	26.753	16.1007	-412.23	430.74	0.957
T12	100 - 80	ROHN 10 EH	20.05	10.03	33.2 K=1.00	26.753	16.1007	-452.69	430.75	1.051
T13	80 - 60	ROHN 12 EH	20.06	10.03	27.8 K=1.00	27.425	19.2423	-495.66	527.71	0.939
T14	60 - 30	ROHN 12 EH	30.07	10.02	27.8 K=1.00	27.426	19.2423	-537.63	527.74	1.019
T15	30 - 0	ROHN 12 EHS	30.08	10.03	28.0 K=1.00	27.392	23.8074	-600.87	652.14	0.921

### Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	320 - 300	L1 3/4x1 3/4x3/16	7.90	3.68	126.6 K=0.98	9.320	0.6211	-4.35	5.79	0.752
T2	300 - 280	L2x2x1/4	9.94	4.80	140.9 K=0.96	7.526	0.9380	-5.43	7.06	0.770
T3	280 - 260	L2 1/2x2 1/2x1/4	12.59	6.09	141.9 K=0.95	7.413	1.1900	-6.60	8.82	0.749
T4	260 - 240	L3x3x1/4	14.38	6.98	136.4 K=0.96	8.031	1.4400	-8.13	11.57	0.703
T5	240 - 220	L4x4x5/16	16.19	7.89	119.7 K=1.00	10.418	2.4000	-9.68	25.00	0.387
T6	220 - 200	L4x4x3/8	19.37	9.56	139.5 K=0.96	7.671	2.8600	-13.32	21.94	0.607
T7	200 - 180	L4x4x3/8	21.20	10.39	149.2 K=0.94	6.709	2.8600	-15.43	19.19	0.804
T8	180 - 160	L4x4x3/8	23.06	11.32	160.0 K=0.93	5.834	2.8600	-17.31	16.68	1.038
T9	160 - 140	L5x5x3/8	24.84	12.19	141.2 K=0.96	7.491	3.6100	-19.91	27.04	0.736
T10	140 - 120	L5x5x3/8	26.78	13.20	150.5 K=0.94	6.589	3.6100	-20.18	23.79	0.848
T11	120 - 100	ROHN 3 EH	24.42	23.63	124.8 K=0.50	9.588	3.0159	-32.07	28.92	1.109
T12	100 - 80	ROHN 3 EH	25.15	24.41	128.9 K=0.50	8.987	3.0159	-33.48	27.10	1.235
T13	80 - 60	ROHN 3 EH	25.98	25.15	132.8 K=0.50	8.467	3.0159	-31.99	25.54	1.253

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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T14	60 - 30	ROHN 3.5 EH	35.21	34.19	103.6 K=0.33	13.854	3.6784	-46.45	50.96	0.911 ✓
T15	30 - 0	ROHN 3.5 EH	36.27	35.32	107.0 K=0.33	13.033	3.6784	-42.08	47.94	0.878 ✓

### Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T11	120 - 100	ROHN 3 STD	25.39	12.25	126.3 K=1.00	9.361	2.2285	-16.77	20.86	0.804 ✓
T12	100 - 80	ROHN 3 STD	27.97	13.54	139.6 K=1.00	7.662	2.2285	-18.90	17.07	1.107 ✓
T13	80 - 60	ROHN 3 EH	30.47	14.79	156.2 K=1.00	6.124	3.0159	-18.90	18.47	1.023 ✓
T14	60 - 30	ROHN 3.5 EH	33.14	16.04	147.3 K=1.00	6.883	3.6784	-21.67	25.32	0.856 ✓
T15	30 - 0	ROHN 4 STD	36.80	17.87	142.0 K=1.00	7.401	3.1741	-23.11	23.49	0.984 ✓

### Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	320 - 300	L1 3/4x1 3/4x3/16	6.81	6.35	182.6 K=0.82	4.480	0.6211	-0.10	2.78	0.034 ✓
T2	300 - 280	L2x2x1/4	6.81	6.35	166.0 K=0.85	5.420	0.9380	-0.11	5.08	0.021 ✓

### Redundant Horizontal (1) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T11	120 - 100	ROHN 1.5 STD	6.35	5.90	113.7 K=1.00	11.550	0.7995	-6.20	9.23	0.672 ✓
T12	100 - 80	ROHN 1.5 STD	6.99	6.54	126.1 K=1.00	9.385	0.7995	-6.81	7.50	0.908 ✓
T13	80 - 60	ROHN 2 STD	7.62	7.09	108.0 K=1.00	12.795	1.0745	-7.45	13.75	0.542 ✓
T14	60 - 30	ROHN 1.5 STD	5.52	4.99	96.2 K=1.00	15.570	0.7995	-8.29	12.45	0.666 ✓

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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T15	30 - 0	ROHN 1.5 STD	6.13	5.60	108.0 K=1.00	12.809	0.7995	-9.07	10.24	0.886 ✓

**Redundant Horizontal (2) Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T14	60 - 30	ROHN 2 EH	11.05	10.52	164.2 K=1.00	5.535	1.4807	-8.17	8.20	0.997 ✓
T15	30 - 0	ROHN 2.5 EH	12.27	11.74	152.4 K=1.00	6.430	2.2535	-9.26	14.49	0.639 ✓

**Redundant Diagonal (1) Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T11	120 - 100	ROHN 2 STD	11.52	10.61	161.8 K=1.00	5.702	1.0745	-5.63	6.13	0.918 ✓
T12	100 - 80	ROHN 2 STD	11.86	11.03	168.1 K=1.00	5.283	1.0745	-5.78	5.68	1.017 ✓
T13	80 - 60	ROHN 2 STD	12.18	11.40	173.8 K=1.00	4.944	1.0745	-5.97	5.31	1.124 ✓
T14	60 - 30	ROHN 2 STD	11.15	9.95	151.6 K=1.00	6.496	1.0745	-8.18	6.98	1.171 ✓
T15	30 - 0	ROHN 2 STD	11.41	10.31	157.2 K=1.00	6.046	1.0745	-8.47	6.50	1.303 ✓

**Redundant Diagonal (2) Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T14	60 - 30	ROHN 2.5 STD	14.46	13.72	173.8 K=1.00	4.943	1.7040	-5.30	8.42	0.630 ✓
T15	30 - 0	ROHN 2.5 STD	15.33	14.63	185.3 K=1.00	4.347	1.7040	-5.67	7.41	0.765 ✓

**Inner Bracing Design Data (Compression)**

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Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T11	120 - 100	ROHN 3 STD	12.69	12.69	130.9 K=1.00	8.712	2.2285	-0.30	19.41	0.016
T12	100 - 80	ROHN 3 STD	13.99	13.99	144.2 K=1.00	7.179	2.2285	-0.34	16.00	0.021
T13	80 - 60	ROHN 3 STD	15.24	15.24	157.1 K=1.00	6.049	2.2285	-0.34	13.48	0.025
T14	60 - 30	ROHN 3 STD	16.57	16.57	170.9 K=1.00	5.114	2.2285	-0.40	11.40	0.035
T15	30 - 0	ROHN 3 STD	18.40	18.40	189.8 K=1.00	4.147	2.2285	-0.42	9.24	0.046

**Tension Checks**

**Leg Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	320 - 300	ROHN 5 EH	20.00	4.00	26.1	30.000	6.1120	14.72	183.36	0.080
T2	300 - 280	ROHN 6 EH	20.03	5.01	27.4	30.000	8.4049	42.92	252.15	0.170
T3	280 - 260	ROHN 8 EH	20.04	6.68	27.9	30.000	12.7627	70.68	382.88	0.185
T4	260 - 240	ROHN 8 EH	20.03	6.68	27.8	30.000	12.7627	100.97	382.88	0.264
T5	240 - 220	ROHN 8 EH	20.03	6.68	27.8	30.000	12.7627	133.35	382.88	0.348
T6	220 - 200	ROHN 8 EH	20.03	10.02	41.8	30.000	12.7627	166.90	382.88	0.436
T7	200 - 180	ROHN 10 EH	20.04	10.02	33.1	30.000	16.1007	207.01	483.02	0.429
T8	180 - 160	ROHN 10 EH	20.04	10.02	33.1	30.000	16.1007	248.31	483.02	0.514
T9	160 - 140	ROHN 10 EH	20.03	10.02	33.1	30.000	16.1007	290.55	483.02	0.602
T10	140 - 120	ROHN 10 EH	20.04	10.02	33.1	30.000	16.1007	330.58	483.02	0.684
T11	120 - 100	ROHN 10 EH	20.06	10.03	33.2	30.000	16.1007	336.53	483.02	0.697
T12	100 - 80	ROHN 10 EH	20.05	10.03	33.2	30.000	16.1007	367.95	483.02	0.762
T13	80 - 60	ROHN 12 EH	20.06	10.03	27.8	30.000	19.2423	401.17	577.27	0.695
T14	60 - 30	ROHN 12 EH	30.07	10.02	27.8	30.000	19.2423	432.96	577.27	0.750
T15	30 - 0	ROHN 12 EHS	30.08	10.03	28.0	30.000	23.8074	481.08	714.22	0.674



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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
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### Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	320 - 300	L1 3/4x1 3/4x3/16	7.90	3.68	82.2	21.600	0.6211	4.28	13.42	0.319
T2	300 - 280	L2x2x1/4	9.94	4.80	94.6	21.600	0.9380	5.25	20.26	0.259
T3	280 - 260	L2 1/2x2 1/2x1/4	12.59	6.09	95.0	21.600	1.1900	6.53	25.70	0.254
T4	260 - 240	L3x3x1/4	14.38	6.98	90.0	32.500	1.0800	7.97	35.10	0.227
T5	240 - 220	L4x4x5/16	16.19	7.89	76.3	32.500	1.8000	9.52	58.50	0.163
T6	220 - 200	L4x4x3/8	19.37	9.56	93.3	32.500	2.1450	13.07	69.71	0.187
T7	200 - 180	L4x4x3/8	21.20	10.39	101.4	32.500	2.1450	15.38	69.71	0.221
T8	180 - 160	L4x4x3/8	23.06	11.32	110.5	32.500	2.1450	17.16	69.71	0.246
T9	160 - 140	L5x5x3/8	24.84	12.19	93.8	32.500	2.7075	19.33	87.99	0.220
T10	140 - 120	L5x5x3/8	26.78	13.20	101.6	32.500	2.7075	19.72	87.99	0.224
T11	120 - 100	ROHN 3 EH	24.42	23.63	249.6	30.000	3.0159	30.02	90.48	0.332
T12	100 - 80	ROHN 3 EH	25.15	24.41	257.8	30.000	3.0159	31.36	90.48	0.347
T13	80 - 60	ROHN 3 EH	25.98	25.15	265.6	30.000	3.0159	29.21	90.48	0.323
T14	60 - 30	ROHN 3.5 EH	35.21	34.19	314.0	30.000	3.6784	42.81	110.35	0.388
T15	30 - 0	ROHN 3.5 EH	36.27	35.32	324.4	30.000	3.6784	40.13	110.35	0.364

### Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T11	120 - 100	ROHN 3 STD	25.39	12.25	126.3	30.000	2.2285	16.80	66.85	0.251
T12	100 - 80	ROHN 3 STD	27.97	13.54	139.6	30.000	2.2285	18.65	66.85	0.279
T13	80 - 60	ROHN 3 EH	30.47	14.79	156.2	30.000	3.0159	18.43	90.48	0.204

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Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T14	60 - 30	ROHN 3.5 EH	33.14	16.04	147.3	30.000	3.6784	21.79	110.35	0.197 ✓
T15	30 - 0	ROHN 4 STD	36.80	17.87	142.0	30.000	3.1741	21.72	95.22	0.228 ✓

### Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	320 - 300	L1 3/4x1 3/4x3/16	6.81	6.35	141.8	21.600	0.6211	0.07	13.42	0.005 ✓
T2	300 - 280	L2x2x1/4	6.81	6.35	125.1	21.600	0.9380	0.09	20.26	0.004 ✓

### Redundant Horizontal (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T11	120 - 100	ROHN 1.5 STD	6.35	5.90	113.7	30.000	0.7995	6.20	23.98	0.259 ✓
T12	100 - 80	ROHN 1.5 STD	6.99	6.54	126.1	30.000	0.7995	6.81	23.98	0.284 ✓
T13	80 - 60	ROHN 2 STD	7.62	7.09	108.0	30.000	1.0745	7.47	32.24	0.232 ✓
T14	60 - 30	ROHN 1.5 STD	5.52	4.99	96.2	30.000	0.7995	8.10	23.98	0.338 ✓
T15	30 - 0	ROHN 1.5 STD	6.13	5.60	108.0	30.000	0.7995	9.10	23.98	0.379 ✓

### Redundant Horizontal (2) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T14	60 - 30	ROHN 2 EH	11.05	10.52	164.2	30.000	1.4807	8.10	44.42	0.182 ✓
T15	30 - 0	ROHN 2.5 EH	12.27	11.74	152.4	30.000	2.2535	9.07	67.61	0.134 ✓

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**Redundant Diagonal (1) Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T11	120 - 100	ROHN 2 STD	11.52	10.61	161.8	30.000	1.0745	5.63	32.24	0.175
T12	100 - 80	ROHN 2 STD	11.86	11.03	168.1	30.000	1.0745	5.78	32.24	0.179
T13	80 - 60	ROHN 2 STD	12.18	11.40	173.8	30.000	1.0745	5.96	32.24	0.185
T14	60 - 30	ROHN 2 STD	11.15	9.95	151.6	30.000	1.0745	8.37	32.24	0.260
T15	30 - 0	ROHN 2 STD	11.41	10.31	157.2	30.000	1.0745	8.43	32.24	0.262

**Redundant Diagonal (2) Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T14	60 - 30	ROHN 2.5 STD	14.46	13.72	173.8	30.000	1.7040	5.36	51.12	0.105
T15	30 - 0	ROHN 2.5 STD	15.33	14.63	185.3	30.000	1.7040	5.83	51.12	0.114

**Inner Bracing Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T11	120 - 100	ROHN 3 STD	12.69	12.69	130.9	30.000	2.2285	0.28	66.85	0.004
T12	100 - 80	ROHN 3 STD	13.99	13.99	144.2	30.000	2.2285	0.33	66.85	0.005
T13	80 - 60	ROHN 3 STD	15.24	15.24	157.1	30.000	2.2285	0.33	66.85	0.005
T14	60 - 30	ROHN 3 STD	16.57	16.57	170.9	30.000	2.2285	0.36	66.85	0.005
T15	30 - 0	ROHN 3 STD	18.40	18.40	189.8	30.000	2.2285	0.40	66.85	0.006

**Section Capacity Table**

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P <sub>allow</sub> K	% Capacity	Pass Fail
T1	320 - 300	Leg	ROHN 5 EH	1	-18.74	225.04	8.3	Pass

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Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P <sub>allow</sub> K	% Capacity	Pass Fail
T2	300 - 280	Leg	ROHN 6 EH	37	-50.84	307.77	16.5	Pass
T3	280 - 260	Leg	ROHN 8 EH	67	-83.19	466.39	17.8	Pass
T4	260 - 240	Leg	ROHN 8 EH	88	-119.28	466.41	25.6	Pass
T5	240 - 220	Leg	ROHN 8 EH	109	-158.39	466.41	34.0	Pass
T6	220 - 200	Leg	ROHN 8 EH	130	-200.48	435.22	46.1	Pass
T7	200 - 180	Leg	ROHN 10 EH	145	-250.20	574.26	43.6	Pass
T8	180 - 160	Leg	ROHN 10 EH	160	-300.55	574.26	52.3	Pass
T9	160 - 140	Leg	ROHN 10 EH	175	-352.33	574.29	61.4	Pass
T10	140 - 120	Leg	ROHN 10 EH	190	-402.87	574.25	70.2	Pass
T11	120 - 100	Leg	ROHN 10 EH	205	-412.23	574.17	71.8	Pass
T12	100 - 80	Leg	ROHN 10 EH	232	-452.69	574.19	78.8	Pass
T13	80 - 60	Leg	ROHN 12 EH	259	-495.66	703.44	70.5	Pass
T14	60 - 30	Leg	ROHN 12 EH	286	-537.63	703.48	76.4	Pass
T15	30 - 0	Leg	ROHN 12 EHS	325	-600.87	869.31	69.1	Pass
T1	320 - 300	Diagonal	L1 3/4x1 3/4x3/16	8	-4.35	7.72	56.4	Pass
T2	300 - 280	Diagonal	L2x2x1/4	47	-5.43	9.41	57.8	Pass
T3	280 - 260	Diagonal	L2 1/2x2 1/2x1/4	74	-6.60	11.76	56.2	Pass
T4	260 - 240	Diagonal	L3x3x1/4	95	-8.13	15.42	52.7	Pass
T5	240 - 220	Diagonal	L4x4x5/16	116	-9.68	33.33	29.0	Pass
T6	220 - 200	Diagonal	L4x4x3/8	137	-13.32	29.25	45.5	Pass
T7	200 - 180	Diagonal	L4x4x3/8	152	-15.43	25.58	60.3	Pass
T8	180 - 160	Diagonal	L4x4x3/8	167	-17.31	22.24	77.8	Pass
T9	160 - 140	Diagonal	L5x5x3/8	182	-19.91	36.05	55.2	Pass
T10	140 - 120	Diagonal	L5x5x3/8	197	-20.18	31.71	63.6	Pass
T11	120 - 100	Diagonal	ROHN 3 EH	223	-32.07	38.55	83.2	Pass
T12	100 - 80	Diagonal	ROHN 3 EH	250	-33.48	36.13	92.7	Pass
T13	80 - 60	Diagonal	ROHN 3 EH	277	-31.99	34.04	94.0	Pass
T14	60 - 30	Diagonal	ROHN 3.5 EH	312	-46.45	67.93	68.4	Pass
T15	30 - 0	Diagonal	ROHN 3.5 EH	351	-42.08	63.90	65.9	Pass
T11	120 - 100	Horizontal	ROHN 3 STD	222	-16.77	27.81	60.3	Pass
T12	100 - 80	Horizontal	ROHN 3 STD	249	-18.90	22.76	83.0	Pass
T13	80 - 60	Horizontal	ROHN 3 EH	276	-18.90	24.62	76.8	Pass
T14	60 - 30	Horizontal	ROHN 3.5 EH	311	-21.67	33.75	64.2	Pass
T15	30 - 0	Horizontal	ROHN 4 STD	350	-23.11	31.31	73.8	Pass
T1	320 - 300	Top Girt	L1 3/4x1 3/4x3/16	6	-0.10	3.71	2.6	Pass
T2	300 - 280	Top Girt	L2x2x1/4	42	-0.11	6.78	1.6	Pass
T11	120 - 100	Redund Horz 1 Bracing	ROHN 1.5 STD	210	-6.20	12.31	50.4	Pass
T12	100 - 80	Redund Horz 1 Bracing	ROHN 1.5 STD	237	-6.81	10.00	68.1	Pass
T13	80 - 60	Redund Horz 1 Bracing	ROHN 2 STD	281	-7.45	18.33	40.7	Pass
T14	60 - 30	Redund Horz 1 Bracing	ROHN 1.5 STD	318	-8.29	16.59	50.0	Pass
T15	30 - 0	Redund Horz 1 Bracing	ROHN 1.5 STD	330	-9.07	13.65	66.4	Pass
T14	60 - 30	Redund Horz 2 Bracing	ROHN 2 EH	292	-8.17	10.93	74.8	Pass
T15	30 - 0	Redund Horz 2 Bracing	ROHN 2.5 EH	331	-9.26	19.31	48.0	Pass
T11	120 - 100	Redund Diag 1 Bracing	ROHN 2 STD	211	-5.63	8.17	68.9	Pass
T12	100 - 80	Redund Diag 1 Bracing	ROHN 2 STD	238	-5.78	7.57	76.3	Pass
T13	80 - 60	Redund Diag 1 Bracing	ROHN 2 STD	282	-5.97	7.08	84.3	Pass
T14	60 - 30	Redund Diag 1 Bracing	ROHN 2 STD	293	-8.18	9.30	87.9	Pass
T15	30 - 0	Redund Diag 1 Bracing	ROHN 2 STD	332	-8.47	8.66	97.8	Pass
T14	60 - 30	Redund Diag 2	ROHN 2.5 STD	294	-5.30	11.23	47.2	Pass

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Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P <sub>allow</sub> K	% Capacity	Pass Fail	
T15	30 - 0	Bracing Redund Diag 2	ROHN 2.5 STD	333	-5.67	9.87	57.4	Pass	
T11	120 - 100	Bracing Inner Bracing	ROHN 3 STD	231	-0.30	25.88	1.2	Pass	
T12	100 - 80	Inner Bracing	ROHN 3 STD	258	-0.34	21.33	1.6	Pass	
T13	80 - 60	Inner Bracing	ROHN 3 STD	285	-0.34	17.97	1.9	Pass	
T14	60 - 30	Inner Bracing	ROHN 3 STD	324	-0.40	15.19	2.6	Pass	
T15	30 - 0	Inner Bracing	ROHN 3 STD	363	-0.42	12.32	3.4	Pass	
							<b>Summary</b>		
							Leg (T12)	78.8	Pass
							Diagonal (T13)	94.0	Pass
							Horizontal (T12)	83.0	Pass
							Top Girt (T1)	2.6	Pass
							Redund Horz 1 Bracing (T12)	68.1	Pass
							Redund Horz 2 Bracing (T14)	74.8	Pass
							Redund Diag 1 Bracing (T15)	97.8	Pass
							Redund Diag 2 Bracing (T15)	57.4	Pass
							Inner Bracing (T15)	3.4	Pass
							Bolt Checks	76.4	Pass
							<b>RATING =</b>	<b>97.8</b>	<b>Pass</b>