

CONNECTICUT  
SITING COUNCIL

280 Trumbull Street  
Hartford, CT 06103-3597  
Main (860) 275-8200  
Fax (860) 275-8299  
kbaldwin@rc.com  
Direct (860) 275-8345

EM-VER-027-130603  
48 Cow Hill Road, Clinton

Also admitted in Massachusetts

July 29, 2014

Melanie A. Bachman  
Acting Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

Re: **Completion of Construction Activity**

Dear Ms. Bachman:

The purpose of this letter is to notify the Siting Council that construction activity associated with the Cellco Partnership d/b/a Verizon Wireless telecommunications facility modifications listed below has been completed.

- EM-VER-083-130308 – 213 Court Street, Middletown, Connecticut
- EM-VER-089-130308 – 200 Stanley Street, New Britain, Connecticut
- EM-VER-137-130314 – 7 Broadway Avenue Ext., Stonington, Connecticut
- EM-VER-148-130312 – 20 Alexander Drive, Wallingford, Connecticut
- EM-VER-089-130322 – Lester Street, New Britain, Connecticut
- EM-VER-110-130325 – 21-35 East Main Street (a/k/a 1 Central Square), Plainville, Connecticut
- EM-VER-155-130322 – 1358 New Britain Avenue, West Hartford, Connecticut
- EM-VER-084-130411 – 26185 Research Drive, Milford, Connecticut
- EM-VER-104-130401 – 2 Hinkley Hill Road, Norwich, Connecticut
- EM-VER-148-130408 – 90 North Plains Industrial Road, Wallingford, Connecticut
- EM-VER-159-130411 – 250 Silas Deane Highway, Wethersfield, Connecticut
- EM-VER-146-130416 – 197 South Street, Vernon, Connecticut
- EM-VER-076-130425 – 252 Ridge Road, Madison, Connecticut
- EM-VER-077-130425 – 53 Slater Street, Manchester, Connecticut
- EM-VER-129-130425 – 400 Main Street, Somers, Connecticut
- EM-VER-052-130430 – Town Farm Road, Farmington, Connecticut
- EM-VER-080-130430 – 38 Elm Street, Meriden, Connecticut

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Melanie A. Bachman  
July 29, 2014  
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**EM-VER-014-130509 – 850 West Main Street, Branford, Connecticut**  
**EM-VER-025-130506 – 705 West Johnson Avenue, Cheshire, Connecticut**  
**EM-VER-041-130524 – 135 Henry Hill Road, East Haddam, Connecticut**  
**EM-VER-115-130524 – 54 Waterbury Road, Prospect, Connecticut**  
**EM-VER-156-130524 – 668 Jones Hill Road, West Haven, Connecticut**  
**EM-VER-027-130603 – 48 Cow Hill Road, Clinton, Connecticut**  
**EM-VER-148-130603 – 945 East Center Street, Wallingford, Connecticut**

If you have any questions or need any additional information regarding this facility please do not hesitate to contact me.

Sincerely,



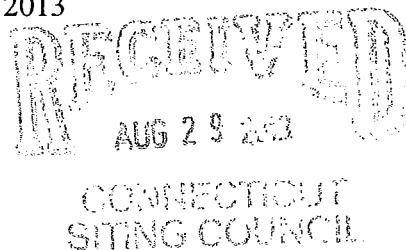
Kenneth C. Baldwin

Copy to:  
Sandy M. Carter

280 Trumbull Street  
Hartford, CT 06103-3597  
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Direct (860) 275-8345

Also admitted in Massachusetts

August 28, 2013



Melanie A. Bachman  
Acting Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

Re: **EM-VER-100-120416 – 38 Lower Road, North Canaan, Connecticut**  
**EM-VER-051-121114 – 3965 Congress Street, Fairfield, Connecticut**  
**EM-VER-135-130603 – 1590 Newfield Avenue, Stamford, Connecticut**  
**EM-VER-014-130607 – 180 North Main Street, Branford, Connecticut**  
**EM-VER-033-130618 – 179 Shunpike Road, Cromwell, Connecticut**  
**EM-VER-041-130524 – 135 Honey Hill Road, East Haddam, Connecticut**  
**EM-VER-027-130603 – 48 Cow Hill Road, Clinton, Connecticut**  
**EM-VER-076-130425 – 252 Ridge Road, Madison, Connecticut**

**Completion of Construction Activity**

Dear Ms. Bachman:

The purpose of this letter is to notify the Siting Council that construction activity associated with the above-referenced Cellco Partnership d/b/a Verizon Wireless telecommunications facilities has been completed.

If you have any questions or need any additional information regarding this facility please do not hesitate to contact me.

Sincerely,

  
Kenneth C. Baldwin

Copy to:  
Sandy M. Carter



Law Offices

BOSTON

PROVIDENCE

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NEW LONDON

STAMFORD

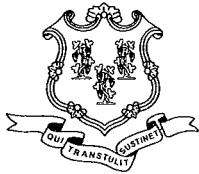
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STATE OF CONNECTICUT  
CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: [siting.council@ct.gov](mailto:siting.council@ct.gov)

[www.ct.gov/csc](http://www.ct.gov/csc)

June 27, 2013

Kenneth C. Baldwin, Esq.  
Robinson & Cole  
280 Trumbull Street  
Hartford, CT 06103-3597

RE: **EM-VER -027-130603** – Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at 48 Cow Hill Road, Clinton, Connecticut.

Dear Attorney Baldwin:

The Connecticut Siting Council (Council) hereby acknowledges your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies with the following conditions:

- Any deviation from the proposed modification as specified in this notice and supporting materials with the Council shall render this acknowledgement invalid;
- Any material changes to this modification as proposed shall require the filing of a new notice with the Council;
- Within 45 days after completion of construction, the Council shall be notified in writing that construction has been completed;
- The validity of this action shall expire one year from the date of this letter; and
- The applicant may file a request for an extension of time beyond the one year deadline provided that such request is submitted to the Council not less than 60 days prior to the expiration;

The proposed modifications including the placement of all necessary equipment and shelters within the tower compound are to be implemented as specified here and in your notice dated May 29, 2013. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Thank you for your attention and cooperation.

Very truly yours,

Melanie A. Bachman  
Acting Executive Director

MAB/CDM/jb

c: The Honorable William W. Fritz, Jr., First Selectman, Town of Clinton  
Thomas Lane, Zoning Enforcement Officer, Town of Clinton  
Crown Castle

EM-VER-027-130603

280 Trumbull Street  
Hartford, CT 06103-3597  
Main (860) 275-8200  
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Direct (860) 275-8345

Also admitted in Massachusetts

**ORIGINAL**

May 29, 2013



Melanie A. Bachman  
Acting Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

Re: **Notice of Exempt Modification – Facility Modification  
48 Cow Hill Road, Clinton, Connecticut**

Dear Ms. Bachman:

Cellco Partnership d/b/a Verizon Wireless (“Cellco”) currently maintains twelve (12) antennas at the 209-foot level of the existing 212-foot tower at 48 Cow Hill Road in Clinton. The tower is owned by Crown Castle. The Council approved Cellco’s use of this tower in 1992 (Docket No. 148). Cellco now intends to replace two (2) of its existing antennas with two (2) model BXA-70063-6CF LTE antennas at the same height on the tower. Attached behind Tab 1 are the specifications for the replacement antennas.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to William W. Fritz, Clinton’s First Selectman. A copy of this letter is also being sent to Raymond E. Heser, the owner of the property on which the tower is located.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing tower. Cellco’s replacement antennas will be located at the 209-foot level of the existing 212-foot tower.



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Melanie A. Bachman  
May 29, 2013  
Page 2

2. The proposed modifications will not involve any change to ground-mounted equipment and, therefore, will not require the extension of the site boundary.

3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.

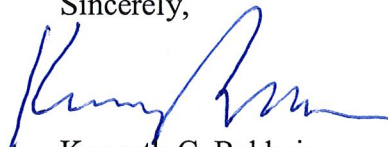
4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) safety standard. A cumulative General Power Density table for Cellco's modified facility are included behind Tab 2.

5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.

6. The tower and its foundation can support Cellco's proposed antenna modifications. (*See Structural Analysis Report attached behind Tab 3*).

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,



Kenneth C. Baldwin

Enclosures

Copy to:

William W. Fritz, Clinton First Selectman

Raymond E. Heser

Sandy M. Carter



## BXA-70063-6CF-EDIN-X

X-Pol | FET Panel | 63° | 14.5 dBd

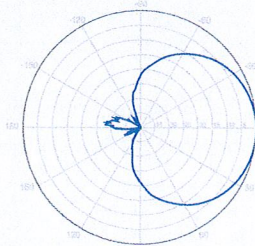
Replace "X" with desired electrical downtilt.

Antenna is also available with NE connector(s). Replace "EDIN" with "NE" in the model number when ordering.



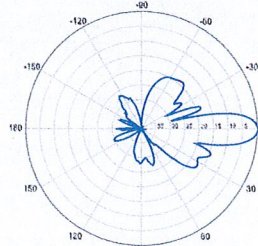
Electrical Characteristics	696-900 MHz			
Frequency bands	696-806 MHz		806-900 MHz	
Polarization	±45°			
Horizontal beamwidth	65°		63°	
Vertical beamwidth	13°		11°	
Gain	14.0 dBd (16.1 dBi)		14.5 dBd (16.6 dBi)	
Electrical downtilt (X)	0, 2, 3, 4, 5, 6, 8, 10			
Impedance	50Ω			
VSWR	≤1.35:1			
Upper sidelobe suppression (0°)	-18.3 dB		-18.2 dB	
Front-to-back ratio (+/-30°)	-33.4 dB		-36.3 dB	
Null fill	5% (-26.02 dB)			
Isolation between ports	< -25 dB			
Input power with EDIN connectors	500 W			
Input power with NE connectors	300 W			
Lightning protection	Direct Ground			
Connector(s)	2 Ports / EDIN or NE / Female / Center (Back)			
Mechanical Characteristics				
Dimensions Length x Width x Depth	1804 x 285 x 132 mm		71.0 x 11.2 x 5.2 in	
Depth with z-brackets	172 mm		6.8 in	
Weight without mounting brackets	7.9 kg		17 lbs	
Survival wind speed	> 201 km/hr		> 125 mph	
Wind area	Front: 0.51 m <sup>2</sup>	Side: 0.24 m <sup>2</sup>	Front: 5.5 ft <sup>2</sup>	Side: 2.6 ft <sup>2</sup>
Wind load @ 161 km/hr (100 mph)	Front: 759 N	Side: 391 N	Front: 169 lbf	Side: 89 lbf
Mounting Options	Part Number	Fits Pipe Diameter		Weight
3-Point Mounting & Downtilt Bracket Kit	36210008	40-115 mm	1.57-4.5 in	6.9 kg 15.2 lbs
Concealment Configurations	For concealment configurations, order BXA-70063-6CF-EDIN-X-FP			

BXA-70063-6CF-EDIN-X



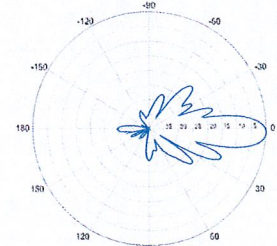
Horizontal | 750 MHz

BXA-70063-6CF-EDIN-0

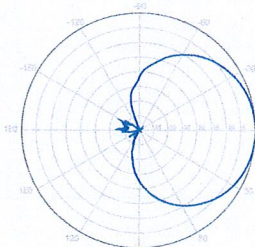


0° | Vertical | 750 MHz

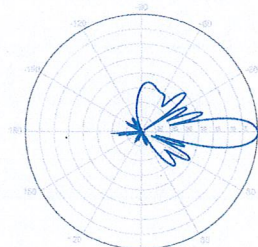
BXA-70063-6CF-EDIN-2



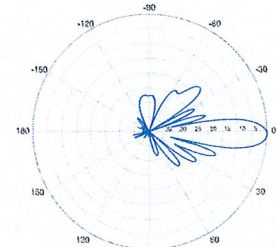
2° | Vertical | 750 MHz



Horizontal | 850 MHz



0° | Vertical | 850 MHz



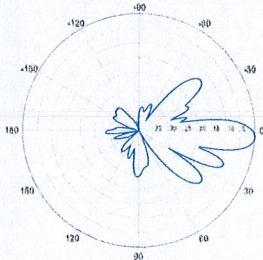
2° | Vertical | 850 MHz

Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.

**BXA-70063-6CF-EDIN-X**

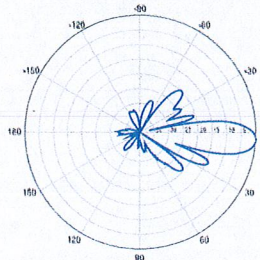
X-Pol | FET Panel | 63° | 14.5 dBd

**BXA-70063-6CF-EDIN-3**



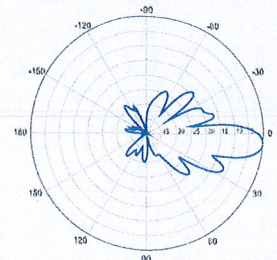
3° | Vertical | 750 MHz

**BXA-70063-6CF-EDIN-4**

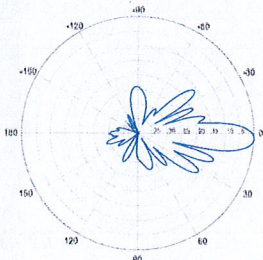


4° | Vertical | 750 MHz

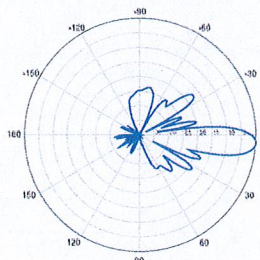
**BXA-70063-6CF-EDIN-5**



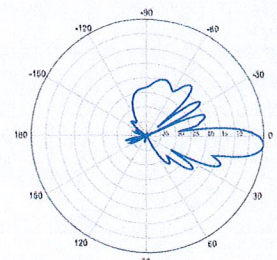
5° | Vertical | 750 MHz



3° | Vertical | 850 MHz

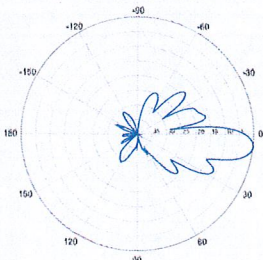


4° | Vertical | 850 MHz



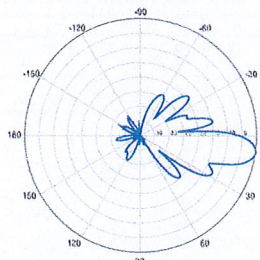
5° | Vertical | 850 MHz

**BXA-70063-6CF-EDIN-6**



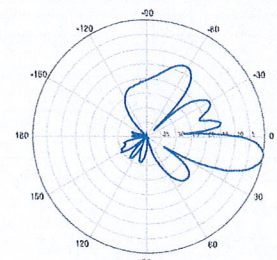
6° | Vertical | 750 MHz

**BXA-70063-6CF-EDIN-8**

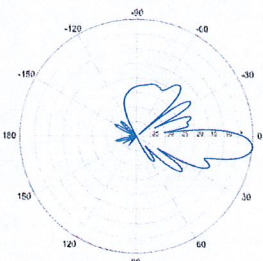


8° | Vertical | 750 MHz

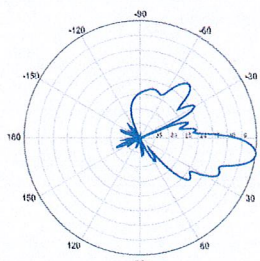
**BXA-70063-6CF-EDIN-10**



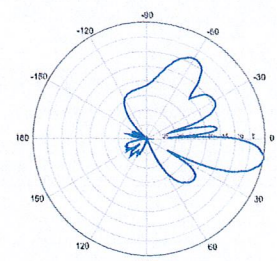
10° | Vertical | 750 MHz



6° | Vertical | 850 MHz



8° | Vertical | 850 MHz



10° | Vertical | 850 MHz

Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.





Date: April 23, 2013



Marianne Dunst  
Crown Castle  
3530 Toringdon Way, Suite 300  
Charlotte, NC 28277  
(704) 405-6580

Vertical Structures, Inc.  
309 Spangler Drive, Suite E  
Richmond, KY 40475  
(859) 624-8360  
csandlin@verticalstructures.com

**Subject: Structural Analysis Report**

**Carrier Designation:** Verizon Wireless Change-Out  
**Carrier Site Number:** 119685  
**Carrier Site Name:** Clinton CT

**Crown Castle Designation:** Crown Castle BU Number: 806363  
Crown Castle Site Name: HRT 105  
Crown Castle JDE Job Number: 232704  
Crown Castle Work Order Number: 601844  
Crown Castle Application Number: 186869 Rev. 1

**Engineering Firm Designation:** Vertical Structures, Inc. Project Number: 2013-004-035

**Site Data:** 48 Cow Hill Road, Clinton, CT, Middlesex County  
Latitude 41° 17' 20.2", Longitude -72° 32' 18.5"  
212.625 Foot - Self Support Tower

Dear Marianne Dunst,

Vertical Structures, Inc. is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 539691.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment

**Sufficient Capacity**

Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

The analysis has been performed in accordance with the TIA/EIA-222-F standard and the 2005 Connecticut State Building Code based upon a wind speed of 85 mph fastest mile.

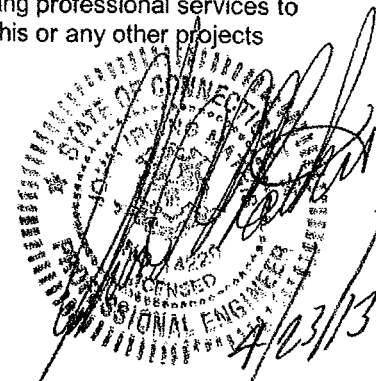
All modifications and equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

We at Vertical Structures, Inc. appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted by:

A handwritten signature in black ink, appearing to read "Chris Sandlin", written over a horizontal line.

Chris Sandlin, P.E.  
Project Engineer



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tnxTower Output

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Additional Calculations

## 1) INTRODUCTION

This tower is a 212.625 ft Self Support tower designed by Rohn in 1992. The tower was originally designed for a wind speed of 90 mph per TIA/EIA-222-E. The tower was reworked in 2007 to accommodate additional loading.

## 2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 85 mph with no ice and 50 mph under service loads. Also, per Crown Castle's direction and in accordance with ASCE-7-05 we have considered a fastest mile wind speed of 38 mph with an escalating 0.75 inch ice thickness.

**Table 1 - Proposed Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
208.0	209.0	2	antel	BXA-70063/6CF-EDIN w/ Mount Pipe			

**Table 2 - Existing and Reserved Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
208.0	209.0	2	antel	BXA-70063/4CF w/ Mount Pipe			3
		1	antel	BXA-70063/6CF-EDIN w/Mount Pipe			
		6	antel	LPA-171080/8CFx2 w/ Mount Pipe	18	1 5/8	1
		6	antel	LPA-80080/6CF w/ mount pipe			
	208.0	1		Sector Mount [SM 510-3]			
	199.0	1		Sector Mount [SM 505-3]			1
199.0	198.0	3	alcatel lucent	1900MHz RRH (65MHz) TMA			
		3	alcatel lucent	800MHz 2x50W RRH w/ Filter	3	1 1/4	2
		3	celwave	APXVSP18-C-A20 w/ Mount Pipe			

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
189.0	190.0	6	adc	Cleargain Dual Band 800/1900 TMA	12 2 1	1 5/8 7/8 1/4	1
		6	ericsson	RRUS-11 BTS			
		3	kmw communications	AM-X-CD-14-65-00T-RET w/ Mount Pipe			
		6	powerwave technologies	7770.00 w/ mount pipe			
		6	powerwave technologies	LGP13519 Diplexer			
	1	raycap	DC6-48-60-18-8F				
	189.0	1		14' Angle Sector Frame (3)			
183.0	183.0	3	celwave	APXV18-206517LS w/Mount Pipe	6	1 5/8	1
175.0	179.0	2	radiowaves	HPD2-23	12 5	1 1/4 3/8	1
	176.0	12	decibel	DB844H90E-XY w/Mount Pipe			
	175.0	1		Sector Mount [SM 510-3]			
167.0	173.0	1	celwave	1151-3	1	7/8	1
	167.0	1		Side Arm Mount [SO 308-1]			
164.0	173.0	1	celwave	1151-3	1 1	7/8 1/4	1
	164.0	1		Side Arm Mount [SO 308-1]			
	162.0	1	sinclair	SD310-HL			
147.0	153.0	1	celwave	1151-3	1	7/8	1
	147.0	1		Side Arm Mount [SO 308-1]			
145.0	148.0	1		SD310-HL	1	7/8	1
	145.0	1		Side Arm Mount [SO 308-1]			
139.0	140.0	3	celwave	APX16DWV-16DWV-S-E-A20	6 6	1 5/8 1 1/4	1
		3	celwave	ATMAA1412D-1A20 TMA			
		3	celwave	ATMPP1412D-1CWA TMA			
		3	ems wireless	RR90-17-02DP w/Mount Pipe			
	139.0	1		Side Arm Mount [SO 201-3]			
128.0	132.0	1	celwave	1142-2C	1	7/8	1
	128.0	1		Side Arm Mount [SO 308-1]			
51.0	51.0	1		GPS	1	1/2	1
		1		Side Arm Mount [SO 702-1]			

- Notes:  
 1) Existing Equipment  
 2) Reserved Equipment  
 3) Equipment to be Removed

**Table 3 - Design Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
212	212	4	celwave	PD10017		
		6	rohn	6' Sidearm		
		12	sinclair	SRL410C		
200	200	2		6' Grid Dish		
190	190	3	rohn	Mounting Frame		
		9	swedcom	ALP9212N		
100	100	1	decibel	DB222		
		1	rohn	Mount		
90	90	1	decibel	DB225		
		1	rohn	Mount		
80	80	2	decibel	DB225-2		
		2	rohn	Mount		
60	60	1	decibel	DB212-2		
		1	decibel	DB225		
		1	decibel	DB225-2		
		3	rohn	Mount		
50	50	1	decibel	DB212-2		
		1	rohn	Mount		
40	40	1	decibel	DB212		
		1	rohn	Mount		

### 3) ANALYSIS PROCEDURE

**Table 4 - Documents Provided**

Document	Remarks	Reference	Source
Online Application	Verizon Wireless Change-Out Revision #1	186869	CCIsites
Tower Drawing	Rohn Drawing No. C921279	262274	CCIsites
Foundation Drawing	Rohn Drawing No. A921278-1	262273	CCIsites
Geotechnical Report	Dr. Clarence Welti Report Dated July 6, 1992	262276	CCIsites
Rework Drawings	Vertical Structures Job No. 2007-004-075	2169576	CCIsites

#### 3.1) Analysis Method

tnxTower (version 6.0.4.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

#### 3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by TIA/EIA-222-F.

This analysis may be affected if any assumptions are not valid or have been made in error. Vertical Structures, Inc. should be notified to determine the effect on the structural integrity of the tower.

### 4) ANALYSIS RESULTS

**Table 5 - Section Capacity (Summary)**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (lb)	SF*P_allow (lb)	% Capacity	Pass / Fail
T1	212.625 - 202.458	Leg	ROHN 2.5 STD	2	-5564.11	51333.83	12.7	Pass
		Diagonal	ROHN 2 STD	8	-2936.54	24117.17	12.2	Pass
		Horizontal	ROHN 1.5 STD	7	-2172.98	20300.52	10.7 12.9 (b)	Pass
		Top Girt	ROHN 1.5 STD	4	-228.36	20340.78	1.1	Pass
T2	202.458 - 182.292	Inner Bracing	L2x2x1/8	17	-4.35	5828.76	0.3	Pass
		Leg	ROHN 3 EH	28	-37114.70	84964.22	43.7	Pass
		Diagonal	ROHN 2 STD	39	-9539.35	17252.62	55.3	Pass
		Horizontal	ROHN 1.5 STD	37	-5303.13	20251.47	26.2 31.1 (b)	Pass
T3	182.292 - 162.104	Inner Bracing	L2x2x1/8	40	-7.52	5726.17	0.3	Pass
		Leg	ROHN 4 EH	67	-77173.20	140142.28	55.1	Pass
		Diagonal	ROHN 2 STD	72	-10460.40	14724.98	71.0	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (lb)	SF*P_allow (lb)	% Capacity	Pass / Fail
		Horizontal	ROHN 1.5 STD	70	-6710.40	17358.06	38.7 39.1 (b)	Pass
		Inner Bracing	L2x2x1/8	79	-7.19	4212.61	0.3	Pass
T4	162.104 - 141.896	Leg	ROHN 5 EH	106	-	207361.47	55.2	Pass
		Diagonal	ROHN 2 STD	111	-10512.60	12580.63	83.6	Pass
		Horizontal	ROHN 2 STD	109	-7418.12	24641.04	30.1 43.2 (b)	Pass
		Inner Bracing	L2x2x1/8	119	-7.86	2887.25	0.4	Pass
T5	141.896 - 121.688	Leg	ROHN 6 EHS	145	-	213793.20	69.9	Pass
		Diagonal	ROHN 2.5 STD	150	-12824.10	15376.69	83.4	Pass
		Horizontal	ROHN 2 STD	148	-7817.58	20379.30	38.4 45.5 (b)	Pass
		Inner Bracing	L2x2x1/8	157	-7.91	2178.27	0.4	Pass
T6	121.688 - 101.479	Leg	ROHN 6 EH	172	-	266297.40	67.5	Pass
		Diagonal	ROHN 2.5 STD	177	-11778.10	13473.43	87.4	Pass
		Horizontal	ROHN 2 STD	175	-7860.94	14810.83	53.1	Pass
		Inner Bracing	L2 1/2x2 1/2x3/16	186	-7.53	4589.77	0.3	Pass
T7	101.479 - 81.2708	Leg	ROHN 6 EH	199	-	266297.40	78.2	Pass
		Diagonal	ROHN 3 STD	204	-11769.50	23342.43	50.4	Pass
		Horizontal	ROHN 2.5 STD	202	-8370.37	25350.59	33.0 48.7 (b)	Pass
		Inner Bracing	L3x3x3/16	213	-7.46	4535.72	0.5	Pass
T8	81.2708 - 61	Leg	ROHN 8 EHS	226	-	334029.79	70.4	Pass
		Diagonal	ROHN 3 STD	231	-11851.00	20458.62	57.9	Pass
		Horizontal	ROHN 2.5 STD	229	-8821.49	19807.45	44.5 51.4 (b)	Pass
		Inner Bracing	L3 1/2x3 1/2x1/4	238	-9.06	7384.85	0.3	Pass
T9	61 - 40.6667	Leg	ROHN 8 EHS	253	-	334051.12	78.6	Pass
		Diagonal	ROHN 3 STD	258	-12823.50	17935.11	71.5	Pass
		Horizontal	ROHN 2.5 STD	256	-9899.75	15667.41	63.2	Pass
		Inner Bracing	L3 1/2x3 1/2x1/4	267	-9.27	5884.99	0.4	Pass
T10	40.6667 - 20.3333	Leg	ROHN 8 EH	280	-	437213.32	65.8 67.4 (b)	Pass
		Diagonal	ROHN 3 STD	287	-17711.60	30741.38	57.6	Pass
		Horizontal	ROHN 3 STD	283	-9970.33	27715.33	36.0 40.3 (b)	Pass
		Redund Horz 1 Bracing	ROHN 1.5 STD	304	-4989.94	11745.65	42.5	Pass
		Redund Diag 1 Bracing	ROHN 2 STD	305	-4474.65	8179.69	54.7	Pass
		Redund Hip 1 Bracing	ROHN 1.5 STD	308	-51.73	10699.36	0.5	Pass
		Redund Hip Diagonal Bracing	ROHN 2.5 STD	307	-48.19	7078.91	0.7	Pass
		Inner Bracing	ROHN 3 STD	310	-11.28	19625.00	1.6	Pass
T11	20.3333 - 0	Leg	ROHN 8 EH	313	-	437299.96	72.0	Pass
		Diagonal	ROHN 3 STD	320	-19880.20	29095.39	68.3	Pass
		Horizontal	ROHN 3 STD	316	-11825.00	22586.22	52.4	Pass



Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (lb)	SF*P_allow (lb)	% Capacity	Pass / Fail
		Redund Horiz 1 Bracing	ROHN 1.5 STD	318	-5458.84	9790.38	55.8	Pass
		Redund Diag 1 Bracing	ROHN 2 STD	319	-4588.72	7507.55	61.1	Pass
		Redund Hip 1 Bracing	ROHN 1.5 STD	341	-52.14	8807.40	0.6	Pass
		Redund Hip Diagonal Bracing	ROHN 2.5 STD	340	-47.55	6355.22	0.7	Pass
		Inner Bracing	ROHN 3 STD	343	-10.11	16082.00	1.4	Pass
							Summary	
							Leg (T9)	78.6 Pass
							Diagonal (T6)	87.4 Pass
							Horizontal (T9)	63.2 Pass
							Top Girt (T1)	1.1 Pass
							Redund Horiz 1 Bracing (T11)	55.8 Pass
							Redund Diag 1 Bracing (T11)	61.1 Pass
							Redund Hip 1 Bracing (T11)	0.6 Pass
							Redund Hip Diagonal Bracing (T11)	0.7 Pass
							Inner Bracing (T10)	1.6 Pass
							Bolt Checks	67.4 Pass
							Rating =	87.4 Pass

**Table 6 - Tower Component Stresses vs. Capacity – LC7**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	62.1	Pass
1	Base Foundation Soil Interaction	0	56.0	Pass

<b>Structure Rating (max from all components) =</b>	<b>87.4%</b>
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Notes:

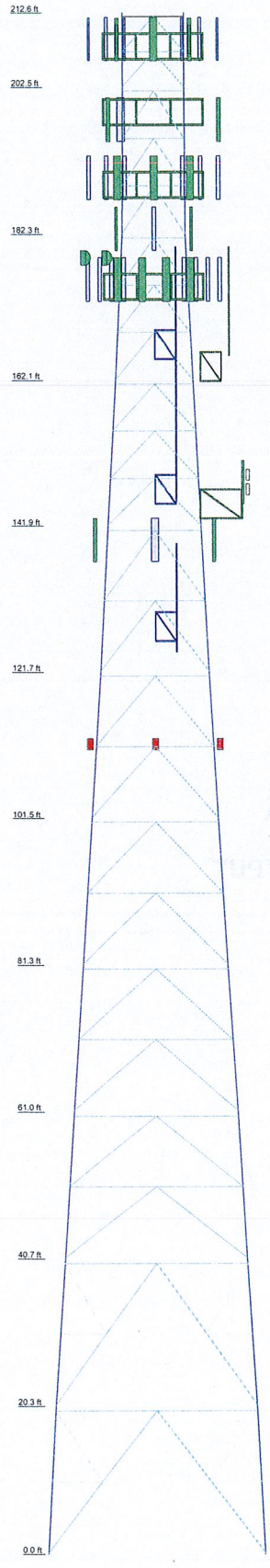
- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity.

**4.1) Recommendations**

N/A

**APPENDIX A**  
**TNXTOWER OUTPUT**

Section	11	10	9	8	7	6	5	4	3	2	1
Legs	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD
Left Grade	AS72-50	AS72-50	AS72-50	AS72-50	AS72-50	AS72-50	AS72-50	AS72-50	AS72-50	AS72-50	AS72-50
Diagonal Grade	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.
Top Chords	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD
Horizontals	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD
Wind Diagonals	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD
Wind Hoops	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD	ROHN 1.5 STD
Inner Bracing	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD	ROHN 3 STD
Face Wash (R)	30.0417	27.8334	25.1774	22.6774	20.0417	17.5417	15.0417	12.7917	10.7035	8.6254	6.5472
# Panels @ (R)	2 @ 19.5833	2 @ 19.5833	2 @ 19.5833	2 @ 19.5833	2 @ 19.5833	2 @ 19.5833	2 @ 19.5833	2 @ 19.5833	2 @ 19.5833	2 @ 19.5833	2 @ 19.5833
Weight (R)	35733.6	34817.2	33900.8	32984.4	32068.0	31151.6	30235.2	29318.8	28402.4	27486.0	26569.6



**DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION
Flash Beacon Lighting	212	AM-X-CD-14-65-00T-RET w/ Mount Pipe	189
Sector Mount (SM 510-3) (Verizon Wireless)	208	(2) RRUS-11 BTS (19.69 x 16.97 x 7.17)	189
(2) LPA-800806CF w/ mount pipe (Verizon Wireless)	208	(2) RRUS-11 BTS (19.69 x 16.97 x 7.17)	189
(2) LPA-800806CF w/ mount pipe (Verizon Wireless)	208	OCB-48-60-18-8F	189
(2) LPA-800806CF w/ mount pipe (Verizon Wireless)	208	APXV18-206517LS w/ Mount Pipe	183
(2) LPA-800806CF w/ mount pipe (Verizon Wireless)	208	APXV18-206517LS w/ Mount Pipe	183
BXA-700636CF-EDIN w/ Mount Pipe (Verizon Wireless)	208	APXV18-206517LS w/ Mount Pipe	183
(2) LPA-1710806CFx2 w/ Mount Pipe (Verizon Wireless)	208	Sector Mount (SM 510-3)	175
(2) LPA-1710806CFx2 w/ Mount Pipe (Verizon Wireless)	208	(4) DB844H90E-XY w/ Mount Pipe	175
(2) LPA-1710806CFx2 w/ Mount Pipe (Verizon Wireless)	208	(4) DB844H90E-XY w/ Mount Pipe	175
(2) LPA-1710806CFx2 w/ Mount Pipe (Verizon Wireless)	208	(4) DB844H90E-XY w/ Mount Pipe	175
BXA-700636CF-EDIN w/ Mount Pipe (Verizon Wireless)	208	8x2 Antenna Mount Pipe	175
BXA-700636CF-EDIN w/ Mount Pipe (Verizon Wireless)	208	8x2 Antenna Mount Pipe	175
Sector Mount (SM 505-3)	199	HPD2-23	175
APXVSP18-C-A20 w/ Mount Pipe	199	HPD2-23	175
APXVSP18-C-A20 w/ Mount Pipe	199	Side Arm Mount (SO 308-1)	167
APXVSP18-C-A20 w/ Mount Pipe	199	1151-3	164
1500MHz RRH (65MHz) TMA	199	SD310-4L	164
1500MHz RRH (65MHz) TMA	199	Side Arm Mount (SO 308-1)	147
800MHz 2x50W RRH w/ Filter	199	1151-3	147
800MHz 2x50W RRH w/ Filter	199	Side Arm Mount (SO 308-1)	145
800MHz 2x50W RRH w/ Filter	199	SD310-4L	145
800MHz 2x50W RRH w/ Filter	199	Side Arm Mount (SO 201-3)	139
(4) 6 Empty Mount Pipe	199	RR90-17-02DP w/ Mount Pipe	139
(4) 6 Empty Mount Pipe	199	RR90-17-02DP w/ Mount Pipe	139
(4) 6 Empty Mount Pipe	199	RR90-17-02DP w/ Mount Pipe	139
(4) 6 Empty Mount Pipe	199	APX16DWW-16DWW-S-E-A20	139
(4) 6 Empty Mount Pipe	199	APX16DWW-16DWW-S-E-A20	139
(4) 6 Empty Mount Pipe	199	APX16DWW-16DWW-S-E-A20	139
14 Angle Sector Frame (3)	189	ATMPP14120-1CWA TMA	139
(2) 7770.00 w/ mount pipe	189	ATMPP14120-1CWA TMA	139
(2) 7770.00 w/ mount pipe	189	ATMPP14120-1CWA TMA	139
(2) 7770.00 w/ mount pipe	189	ATMPP14120-1CWA TMA	139
(2) LGP13519 Diplexer	189	ATMAA14120-1A20 TMA	139
(2) LGP13519 Diplexer	189	ATMAA14120-1A20 TMA	139
(2) LGP13519 Diplexer	189	ATMAA14120-1A20 TMA	139
(2) Cleargain Dual Band 800/1600 TMA (VSI)	189	Side Arm Mount (SO 308-1)	128
(2) Cleargain Dual Band 800/1600 TMA (VSI)	189	118-2-C	126
(2) Cleargain Dual Band 800/1600 TMA (VSI)	189	Intermediate Side Lights (VSI)	111
(2) Cleargain Dual Band 800/1600 TMA (VSI)	189	Intermediate Side Lights (VSI)	111
AM-X-CD-14-65-00T-RET w/ Mount Pipe	189	Intermediate Side Lights (VSI)	111
AM-X-CD-14-65-00T-RET w/ Mount Pipe	189	Side Arm Mount (SO 702-1)	51
		Generic GPS (VSI)	51

**MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi			

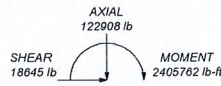
**TOWER DESIGN NOTES**

1. Tower is located in Middlesex County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 38 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 87.4%

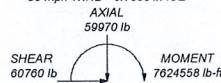
**MAX. CORNER REACTIONS AT BASE:**

DOWN: 313052 lb  
SHEAR: 36065 lb

UPLIFT: -268255 lb  
SHEAR: 32713 lb



TORQUE 7056 lb-ft  
38 mph WIND - 0.7500 in ICE



TORQUE 44328 lb-ft  
REACTIONS - 85 mph WIND

**Vertical Structures, Inc.**  
309 Spangler Drive, Suite E  
Richmond, KY 40475  
Phone: (859) 624-8360  
FAX: (859) 624-8369

Job: **HRT 105, CT BU#806363**  
Project: **Vertical Structures Job No. 2013-004-035**  
Client: **Crown Castle** Drawn by: **Casey Hymore** App'd:  
Code: **TIA/EIA-222-F** Date: **04/23/13** Scale: **NTS**  
Path: **\\fs1\proj\proj\2013\04\035\101.ctb** Dwg No: **E-1**

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 1 of 29
	<b>Project</b> Vertical Structures Job No. 2013-004-035	<b>Date</b> 16:45:29 04/23/13
	<b>Client</b> Crown Castle	<b>Designed by</b> Casey Hymore

## Tower Input Data

The main tower is a 3x free standing tower with an overall height of 212.63 ft above the ground line.  
The base of the tower is set at an elevation of 0.00 ft above the ground line.  
The face width of the tower is 8.50 ft at the top and 30.04 ft at the base.  
This tower is designed using the TIA/EIA-222-F standard.

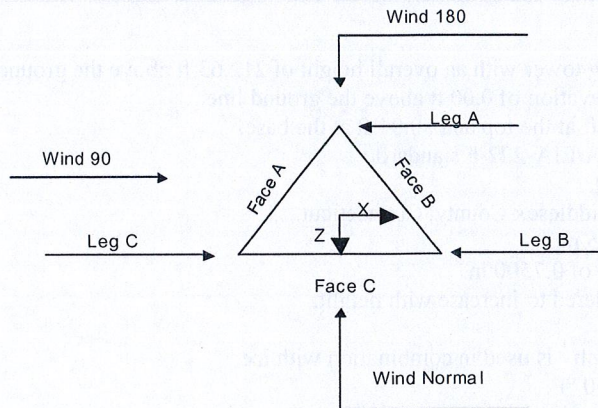
The following design criteria apply:

- Tower is located in Middlesex County, Connecticut.
- Basic wind speed of 85 mph.
- Nominal ice thickness of 0.7500 in.
- Ice thickness is considered to increase with height.
- Ice density of 56 pcf.
- A wind speed of 38 mph is used in combination with ice.
- Temperature drop of 50 °F.
- Deflections calculated using a wind speed of 50 mph.
- A non-linear (P-delta) analysis was used.
- Pressures are calculated at each section.
- Stress ratio used in tower member design is 1.333.
- Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

## Options

- |  |  |  |
|--|--|--|
| <ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>√ Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>√ Include Bolts In Member Capacity</li> <li>Leg Bolts Are At Top Of Section</li> <li>√ Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>Add IBC .6D+W Combination</li> </ul> | <ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>√ Use Clear Spans For Wind Area</li> <li>√ Use Clear Spans For KL/r</li> <li>√ Retension Guys To Initial Tension</li> <li>√ Bypass Mast Stability Checks</li> <li>√ Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>√ Autocalc Torque Arm Areas</li> <li>√ SR Members Have Cut Ends</li> <li>Sort Capacity Reports By Component</li> <li>√ Triangulate Diamond Inner Bracing</li> </ul> | <ul style="list-style-type: none"> <li>Treat Feedline Bundles As Cylinder</li> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>√ Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>SR Leg Bolts Resist Compression</li> <li>√ All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>√ Consider Feedline Torque</li> <li>Include Angle Block Shear Check Poles</li> <li>Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> </ul> |
|--|--|--|

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 2 of 29
	<b>Project</b> Vertical Structures Job No. 2013-004-035	<b>Date</b> 16:45:29 04/23/13
	<b>Client</b> Crown Castle	<b>Designed by</b> Casey Hymore



Triangular Tower

### Tower Section Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	ft			ft		ft
T1	212.63-202.46			8.50	1	10.17
T2	202.46-182.29			8.54	1	20.17
T3	182.29-162.10			8.63	1	20.19
T4	162.10-141.90			10.71	1	20.21
T5	141.90-121.69			12.79	1	20.21
T6	121.69-101.48			15.04	1	20.21
T7	101.48-81.27			17.54	1	20.21
T8	81.27-61.00			20.04	1	20.27
T9	61.00-40.67			22.68	1	20.33
T10	40.67-20.33			25.18	1	20.33
T11	20.33-0.00			27.83	1	20.33

### Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T1	212.63-202.46	4.79	K Brace Down	No	Yes	6.0000	1.0000
T2	202.46-182.29	6.53	K Brace Down	No	Yes	6.0000	1.0000
T3	182.29-162.10	6.53	K Brace Down	No	Yes	6.0000	1.2500
T4	162.10-141.90	6.53	K Brace Down	No	Yes	6.2500	1.2500

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 3 of 29
	<b>Project</b> Vertical Structures Job No. 2013-004-035	<b>Date</b> 16:45:29 04/23/13
	<b>Client</b> Crown Castle	<b>Designed by</b> Casey Hymore

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T5	141.90-121.69	9.79	K Brace Down	No	Yes	6.2500	1.2500
T6	121.69-101.48	9.79	K Brace Down	No	Yes	6.2500	1.2500
T7	101.48-81.27	9.79	K Brace Down	No	Yes	6.2500	1.2500
T8	81.27-61.00	9.79	K Brace Down	No	Yes	6.2500	2.0000
T9	61.00-40.67	9.79	K Brace Down	No	Yes	7.0000	2.0000
T10	40.67-20.33	19.58	K1 Down	No	Yes	7.0000	2.0000
T11	20.33-0.00	19.58	K1 Down	No	Yes	7.0000	2.0000

### Tower Section Geometry (cont'd)

Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
ft						
T1 212.63-202.46	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T2 202.46-182.29	Pipe	ROHN 3 EH	A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T3 182.29-162.10	Pipe	ROHN 4 EH	A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T4 162.10-141.90	Pipe	ROHN 5 EH	A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T5 141.90-121.69	Pipe	ROHN 6 EHS	A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)
T6 121.69-101.48	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)
T7 101.48-81.27	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T8 81.27-61.00	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T9 61.00-40.67	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T10 40.67-20.33	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T11 20.33-0.00	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
ft							
T1 212.63-202.46	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 1.5 STD	A572-50 (50 ksi)
T2 202.46-182.29	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 1.5 STD	A572-50 (50 ksi)
T3 182.29-162.10	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 1.5 STD	A572-50 (50 ksi)
T4 162.10-141.90	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T5 141.90-121.69	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b>	HRT 105, CT BU#806363	<b>Page</b>	4 of 29
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Tower Elevation <i>ft</i>	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T6 121.69-101.48	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T7 101.48-81.27	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)
T8 81.27-61.00	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)
T9 61.00-40.67	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)
T10 40.67-20.33	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T11 20.33-0.00	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation <i>ft</i>	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
T1 212.63-202.46	Single Angle		A572-50 (50 ksi)	Single Angle	L2x2x1/8	A36 (36 ksi)
T2 202.46-182.29	Single Angle		A572-50 (50 ksi)	Single Angle	L2x2x1/8	A36 (36 ksi)
T3 182.29-162.10	Single Angle		A572-50 (50 ksi)	Single Angle	L2x2x1/8	A36 (36 ksi)
T4 162.10-141.90	Single Angle		A572-50 (50 ksi)	Single Angle	L2x2x1/8	A36 (36 ksi)
T5 141.90-121.69	Single Angle		A572-50 (50 ksi)	Single Angle	L2x2x1/8	A36 (36 ksi)
T6 121.69-101.48	Single Angle		A572-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T7 101.48-81.27	Single Angle		A572-50 (50 ksi)	Single Angle	L3x3x3/16	A36 (36 ksi)
T8 81.27-61.00	Single Angle		A572-50 (50 ksi)	Single Angle	L3 1/2x3 1/2x1/4	A572-50 (50 ksi)
T9 61.00-40.67	Single Angle		A572-50 (50 ksi)	Single Angle	L3 1/2x3 1/2x1/4	A572-50 (50 ksi)
T10 40.67-20.33	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T11 20.33-0.00	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation <i>ft</i>	Redundant Bracing Grade	Redundant Type	Redundant Type	Redundant Size	K Factor
T10 40.67-20.33	A36 (36 ksi)	Horizontal (1) Diagonal (1) Hip (1)	Pipe Pipe Pipe	ROHN 1.5 STD ROHN 2 STD ROHN 1.5 STD	1 1 1





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Tower Elevation ft	Calc K Single Angles	Calc K Solid Rounds	K Factors <sup>1</sup>								
			Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace	
				X Y	X Y	X Y	X Y	X Y	X Y	X Y	
141.90-121.69				1	1	1	1	1	1	1	1
T6	No	No	1	1	1	1	1	1	1	1	1
121.69-101.48				1	1	1	1	1	1	1	1
T7	No	No	1	1	1	1	1	1	1	1	1
101.48-81.27				1	1	1	1	1	1	1	1
T8	No	No	1	1	1	1	1	1	1	1	1
81.27-61.00				1	1	1	1	1	1	1	1
T9	No	No	1	1	1	1	1	1	1	1	1
61.00-40.67				1	1	1	1	1	1	1	1
T10	No	No	1	1	1	1	1	1	1	1	1
40.67-20.33				1	1	1	1	1	1	1	1
T11	No	No	1	1	1	1	1	1	1	1	1
20.33-0.00				1	1	1	1	1	1	1	1

<sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	0.75
212.63-202.46														
T2	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	0.75
202.46-182.29														
T3	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	0.75
182.29-162.10														
T4	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	0.75
162.10-141.90														
T5	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	0.75
141.90-121.69														
T6	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	0.75
121.69-101.48														
T7	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	0.75
101.48-81.27														
T8	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	0.75
81.27-61.00														
T9	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	0.75
61.00-40.67														
T10	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	0.75
40.67-20.33														
T11	0.0000	1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	0.75
20.33-0.00														

### Tower Section Geometry (cont'd)

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 7 of 29
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Tower Elevation ft	Connection Offsets							
	Diagonal				K-Bracing			
	Vert. Top	Horiz. Top	Vert. Bot.	Horiz. Bot.	Vert. Top	Horiz. Top	Vert. Bot.	Horiz. Bot.
	in	in	in	in	in	in	in	in
T1 212.63-202.46	0.0000	0.0000	0.0000	0.0000	0.9500	0.9500	4.7500	1.4375
T2 202.46-182.29	0.0000	0.0000	0.0000	0.0000	0.9500	0.9500	4.7500	1.7500
T3 182.29-162.10	0.0000	0.0000	0.0000	0.0000	0.9500	0.9500	4.7500	2.2500
T4 162.10-141.90	0.0000	0.0000	0.0000	0.0000	1.1875	1.1875	4.7500	2.7815
T5 141.90-121.69	0.0000	0.0000	0.0000	0.0000	1.1875	1.1875	4.7500	3.3125
T6 121.69-101.48	0.0000	0.0000	0.0000	0.0000	1.1875	1.1875	4.7500	3.3125
T7 101.48-81.27	0.0000	0.0000	0.0000	0.0000	1.4375	1.4375	4.7500	3.3125
T8 81.27-61.00	0.0000	0.0000	0.0000	0.0000	1.4375	1.4375	4.7500	4.3125
T9 61.00-40.67	0.0000	0.0000	0.0000	0.0000	1.4375	1.4375	4.7500	4.3125
T10 40.67-20.33	0.0000	0.0000	0.0000	0.0000	1.7500	1.7500	4.7500	4.3125
T11 20.33-0.00	0.0000	0.0000	0.0000	0.0000	1.7500	1.7500	4.7500	4.3125

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 212.63-202.46	Flange	0.7500	4	0.6250	3	0.6250	0	0.6250	0	0.6250	0	0.6250	2	0.6250	0
T2 202.46-182.29	Flange	0.8750	4	0.6250	3	0.6250	0	0.6250	0	0.6250	0	0.6250	2	0.6250	0
T3 182.29-162.10	Flange	1.0000	4	0.6250	3	0.6250	0	0.6250	0	0.6250	0	0.6250	2	0.6250	0
T4 162.10-141.90	Flange	1.0000	6	0.6250	3	0.6250	0	0.6250	0	0.6250	0	0.6250	2	0.6250	0
T5 141.90-121.69	Flange	1.0000	6	0.6250	3	0.6250	0	0.6250	0	0.6250	0	0.6250	2	0.6250	0
T6 121.69-101.48	Flange	1.0000	6	0.6250	3	0.6250	0	0.6250	0	0.6250	0	0.6250	2	0.6250	0
T7 101.48-81.27	Flange	1.0000	8	0.6250	3	0.6250	0	0.6250	0	0.6250	0	0.6250	2	0.6250	0
T8 81.27-61.00	Flange	1.0000	8	0.6250	3	0.6250	0	0.6250	0	0.6250	0	0.6250	2	0.6250	0
T9 61.00-40.67	Flange	1.0000	8	0.6250	3	0.6250	0	0.6250	0	0.6250	0	0.6250	2	0.6250	0
T10 40.67-20.33	Flange	1.0000	8	0.7500	3	0.6250	0	0.6250	0	0.6250	0	0.7500	2	0.6250	0
T11 20.33-0.00	Flange	1.0000	0	0.7500	3	0.6250	0	0.6250	0	0.6250	0	0.7500	2	0.6250	0
		A354-BC		A325N		A325N		A325X		A325X		A325N		A325X	

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**Feed Line/Linear Appurtenances - Entered As Round Or Flat**

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
LDF7-50A (1-5/8 FOAM) (Verizon Wireless)	C	Yes	Ar (CfAe)	209.00 - 8.00	1.0000	0.42	9	6	0.2700 1.0000	1.9800		0.82
LDF7-50A (1-5/8 FOAM) (Verizon Wireless)	C	Yes	Ar (CfAe)	209.00 - 8.00	1.0000	0.38	9	6	0.2700 1.0000	1.9800		0.82
Feedline Ladder (1-1/2" Rails) (Af) (Verizon Wireless) ***	C	Yes	Af (CfAe)	203.00 - 8.00	0.5000	0.4	1	1	1.5000	1.5000	12.0000	3.66
9114 (11/32)	A	Yes	Ar (CfAe)	179.00 - 8.00	1.0000	-0.45	5	3	0.0000	0.3800		0.03
LDF5-50A (7/8 FOAM)	A	Yes	Ar (CfAe)	128.00 - 8.00	1.0000	-0.35	5	5	1.1600	1.0900		0.33
LDF5-50A (7/8 FOAM)	A	Yes	Ar (CfAe)	145.00 - 128.00	1.0000	-0.35	4	4	1.1600	1.0900		0.33
LDF5-50A (7/8 FOAM)	A	Yes	Ar (CfAe)	147.00 - 145.00	1.0000	-0.35	3	3	1.1600	1.0900		0.33
LDF5-50A (7/8 FOAM)	A	Yes	Ar (CfAe)	164.00 - 147.00	1.0000	-0.35	2	2	1.1600	1.0900		0.33
LDF5-50A (7/8 FOAM)	A	Yes	Ar (CfAe)	167.00 - 164.00	1.0000	-0.35	1	1	1.1600	1.0900		0.33
CR 50 1873 (1-5/8 FOAM)	A	Yes	Ar (CfAe)	190.00 - 8.00	1.0000	-0.42	12	6	0.2700 1.0000	1.9800		0.83
LDF5-50A (7/8 FOAM)	A	Yes	Ar (CfAe)	190.00 - 8.00	1.0000	-0.25	2	2	1.0000	1.0900		0.33
LDF1-50A (1/4 FOAM)	A	Yes	Ar (CfAe)	190.00 - 8.00	1.0000	-0.25	1	1	0.3500	0.3500		0.06
Feedline Ladder (1-1/2" Rails) (Af)	A	Yes	Af (CfAe)	190.00 - 8.00	0.5000	-0.4	1	1	3.0000	3.0000	12.0000	3.66
Feedline Ladder (1-1/2" Rails) (Af)	A	Yes	Af (CfAe)	198.00 - 8.00	0.5000	0.4	1	1	3.0000	3.0000	12.0000	3.66
HB114-1-08U 4-M5J (1-1/4")	A	Yes	Ar (CfAe)	198.00 - 8.00	1.0000	0.4	3	3	1.0000	1.5400		0.66
LDF4-50A (1/2 FOAM) ***	A	Yes	Ar (CfAe)	51.00 - 8.00	1.0000	0.45	1	1	0.6300	0.6300		0.15
LDF7-50A (1-5/8 FOAM)	B	Yes	Ar (CfAe)	140.00 - 8.00	1.0000	-0.4	12	6	1.0000	1.9800		0.82
Feedline Ladder (1-1/2" Rails) (Af)	B	Yes	Af (CfAe)	140.00 - 8.00	0.5000	-0.4	1	1	3.0000	3.0000	12.0000	3.66
LDF6-50A (1-1/4 FOAM)	B	Yes	Ar (CfAe)	176.00 - 8.00	1.0000	0.4	12	12	0.7000	1.5500		0.66
Feedline Ladder (1-1/2" Rails) (Af) **	B	Yes	Af (CfAe)	176.00 - 8.00	0.5000	0.4	1	1	3.0000	3.0000	12.0000	3.66
Feedline Ladder (1-1/2" Rails) (Af)	C	Yes	Af (CfAe)	183.00 - 8.00	0.5000	-0.35	1	1	3.0000	3.0000	12.0000	3.66

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Description	Face or Leg	Allow or Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
AVA7-50 (1-5/8 LOW DENS. FOAM)	C	Yes	Ar (CfAe)	183.00 - 8.00	1.0000	-0.35	6	6	1.0000	1.9800		0.72

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight lb
T1	212.63-202.46	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	12.953	0.068	0.000	0.000	98.54
T2	202.46-182.29	A	15.304	5.854	0.000	0.000	199.13
		B	0.000	0.000	0.000	0.000	0.00
		C	40.631	2.698	0.000	0.000	377.12
T3	182.29-162.10	A	34.236	10.094	0.000	0.000	408.12
		B	21.539	3.474	0.000	0.000	160.91
		C	59.957	7.570	0.000	0.000	532.95
T4	162.10-141.90	A	38.384	10.104	0.000	0.000	422.84
		B	31.323	5.052	0.000	0.000	234.01
		C	60.019	7.578	0.000	0.000	533.50
T5	141.90-121.69	A	41.883	10.104	0.000	0.000	435.55
		B	49.452	9.630	0.000	0.000	481.23
		C	60.019	7.578	0.000	0.000	533.50
T6	121.69-101.48	A	43.145	10.104	0.000	0.000	440.14
		B	51.329	10.104	0.000	0.000	506.82
		C	60.019	7.578	0.000	0.000	533.50
T7	101.48-81.27	A	43.145	10.104	0.000	0.000	440.14
		B	51.329	10.104	0.000	0.000	506.82
		C	60.019	7.578	0.000	0.000	533.50
T8	81.27-61.00	A	43.278	10.135	0.000	0.000	441.50
		B	51.488	10.135	0.000	0.000	508.39
		C	60.204	7.602	0.000	0.000	535.15
T9	61.00-40.67	A	43.954	10.167	0.000	0.000	444.41
		B	51.647	10.167	0.000	0.000	509.96
		C	60.390	7.625	0.000	0.000	536.80
T10	40.67-20.33	A	44.479	10.167	0.000	0.000	445.91
		B	51.647	10.167	0.000	0.000	509.96
		C	60.390	7.625	0.000	0.000	536.80
T11	20.33-0.00	A	26.979	6.167	0.000	0.000	270.47
		B	31.327	6.167	0.000	0.000	309.32
		C	36.630	4.625	0.000	0.000	325.60

### Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight lb
T1	212.63-202.46	A	0.935	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		4.198	12.390	0.000	0.000	384.94
T2	202.46-182.29	A	0.927	10.211	23.484	0.000	0.000	688.66
		B		0.000	0.000	0.000	0.000	0.00

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 10 of 29
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	<b>Client</b> Crown Castle	<b>Designed by</b> Casey Hymore

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	$A_R$ ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$C_A A_A$ In Face ft <sup>2</sup>	$C_A A_A$ Out Face ft <sup>2</sup>	Weight lb
T3	182.29-162.10	C		13.111	43.539	0.000	0.000	1375.81
		A	0.914	24.952	46.609	0.000	0.000	1445.73
		B		3.913	33.546	0.000	0.000	615.34
T4	162.10-141.90	C		19.223	74.590	0.000	0.000	2006.39
		A	0.901	29.033	51.778	0.000	0.000	1545.94
		B		5.644	48.755	0.000	0.000	886.44
T5	141.90-121.69	C		19.106	74.606	0.000	0.000	1991.52
		A	0.886	28.724	58.931	0.000	0.000	1623.98
		B		11.317	77.838	0.000	0.000	1830.44
T6	121.69-101.48	C		18.951	74.538	0.000	0.000	1972.52
		A	0.868	28.370	61.458	0.000	0.000	1637.21
		B		11.792	80.774	0.000	0.000	1909.24
T7	101.48-81.27	C		18.774	74.459	0.000	0.000	1950.83
		A	0.847	27.955	61.366	0.000	0.000	1612.59
		B		11.653	80.682	0.000	0.000	1885.95
T8	81.27-61.00	C		18.566	74.367	0.000	0.000	1925.47
		A	0.822	27.533	61.443	0.000	0.000	1587.69
		B		11.520	80.818	0.000	0.000	1863.41
T9	61.00-40.67	C		18.370	74.484	0.000	0.000	1900.53
		A	0.790	28.860	61.485	0.000	0.000	1569.84
		B		11.335	80.921	0.000	0.000	1832.48
T10	40.67-20.33	C		18.096	74.566	0.000	0.000	1866.49
		A	0.750	29.754	61.305	0.000	0.000	1536.28
		B		11.065	80.740	0.000	0.000	1787.77
T11	20.33-0.00	C		17.690	74.386	0.000	0.000	1817.88
		A	0.750	18.048	37.185	0.000	0.000	931.84
		B		6.711	48.974	0.000	0.000	1084.38
		C		10.730	45.119	0.000	0.000	1102.65

### Feed Line Shielding

Section	Elevation ft	Face	$A_R$ ft <sup>2</sup>	$A_R$ Ice ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$A_F$ Ice ft <sup>2</sup>
T1	212.63-202.46	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	1.168	2.766	0.000	0.000
T2	202.46-182.29	A	1.631	4.950	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	3.340	8.187	0.000	0.000
T3	182.29-162.10	A	3.188	9.703	0.000	0.000
		B	1.799	5.031	0.000	0.000
		C	4.856	12.637	0.000	0.000
T4	162.10-141.90	A	3.522	10.581	0.000	0.000
		B	2.642	7.078	0.000	0.000
		C	4.910	12.229	0.000	0.000
T5	141.90-121.69	A	3.092	8.845	0.000	0.000
		B	3.514	8.983	0.000	0.000
		C	4.021	9.420	0.000	0.000
T6	121.69-101.48	A	2.975	8.452	0.000	0.000
		B	3.432	8.704	0.000	0.000
		C	3.776	8.766	0.000	0.000
T7	101.48-81.27	A	3.446	8.990	0.000	0.000
		B	3.976	9.287	0.000	0.000
		C	4.374	9.346	0.000	0.000
T8	81.27-61.00	A	3.320	8.518	0.000	0.000
		B	3.830	8.834	0.000	0.000

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b>	HRT 105, CT BU#806363	<b>Page</b>	11 of 29
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	<b>Client</b>	Crown Castle	<b>Designed by</b>	Casey Hymore

Section	Elevation	Face	$A_R$	$A_R$	$A_F$	$A_F$
			$ft^2$	Ice $ft^2$	$ft^2$	Ice $ft^2$
T9	61.00-40.67	C	4.215	8.882	0.000	0.000
		A	3.262	8.272	0.000	0.000
		B	3.725	8.443	0.000	0.000
T10	40.67-20.33	C	4.099	8.480	0.000	0.000
		A	3.621	9.406	0.000	0.000
		B	4.096	9.482	0.000	0.000
T11	20.33-0.00	C	4.507	9.510	0.000	0.000
		A	2.101	5.455	0.000	0.000
		B	2.377	5.499	0.000	0.000
		C	2.615	5.515	0.000	0.000

### Feed Line Center of Pressure

Section	Elevation	$CP_x$	$CP_z$	$CP_x$	$CP_z$
		Ice $in$	$in$	Ice $in$	Ice $in$
T1	212.63-202.46	-8.6747	6.9971	-4.4795	3.6128
T2	202.46-182.29	-14.9350	7.2546	-9.0584	3.9750
T3	182.29-162.10	-5.7052	12.3280	-1.7545	8.4458
T4	162.10-141.90	-4.4233	14.2829	-0.2988	10.0574
T5	141.90-121.69	-4.6772	9.2666	-0.8754	6.3116
T6	121.69-101.48	-5.5766	9.9750	-1.4141	6.8013
T7	101.48-81.27	-6.0500	10.7815	-1.5309	7.4592
T8	81.27-61.00	-6.3597	11.3003	-1.6712	8.0187
T9	61.00-40.67	-7.0001	12.1600	-1.8918	8.2922
T10	40.67-20.33	-7.4872	12.7381	-1.9662	8.2910
T11	20.33-0.00	-5.9049	10.0341	-1.5859	6.5926

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	$C_{AA}$ Front	$C_{AA}$ Side	Weight
			Horz Lateral $ft$	Vert $ft$					
Flash Beacon Lighting	B	From Leg	0.00	0.0000	212.00	No Ice	2.70	2.70	50.00
						1/2" Ice	3.10	3.10	70.00
						1" Ice	3.50	3.50	90.00
						2" Ice	4.30	4.30	130.00
						4" Ice	5.90	5.90	210.00
Intermediate Side Lights (VSI)	A	From Leg	1.00	0.0000	111.00	No Ice	0.79	0.79	29.13
						1/2" Ice	1.04	1.04	39.42
						1" Ice	1.32	1.32	53.41
						2" Ice	1.98	1.98	89.69
						4" Ice	3.59	3.59	212.79
Intermediate Side Lights (VSI)	B	From Leg	1.00	0.0000	111.00	No Ice	0.79	0.79	29.13
						1/2" Ice	1.04	1.04	39.42
						1" Ice	1.32	1.32	53.41
						2" Ice	1.98	1.98	89.69
						4" Ice	3.59	3.59	212.79

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 12 of 29
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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight	
			Horz	Lateral			Front	Side		
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	lb	
Intermediate Side Lights (VSI)	C	From Leg	1.00		0.0000	111.00	No Ice	0.79	0.79	29.13
			0.00				1/2" Ice	1.04	1.04	39.42
			0.00				1" Ice	1.32	1.32	53.41
							2" Ice	1.98	1.98	89.69
							4" Ice	3.59	3.59	212.79
**										
Sector Mount [SM 510-3] (Verizon Wireless)	A	None			0.0000	208.00	No Ice	40.10	40.10	2396.40
							1/2" Ice	57.33	57.33	3089.00
							1" Ice	74.56	74.56	3781.60
							2" Ice	109.02	109.02	5166.80
							4" Ice	177.94	177.94	7937.20
(2) LPA-80080/6CF w/ mount pipe (Verizon Wireless)	A	From Face	5.00		-6.0000	208.00	No Ice	4.35	10.51	42.90
			0.00				1/2" Ice	4.79	11.56	104.60
			1.00				1" Ice	5.25	12.49	177.42
							2" Ice	6.17	14.40	348.65
							4" Ice	8.11	18.43	824.28
(2) LPA-80080/6CF w/ mount pipe (Verizon Wireless)	B	From Face	5.00		-6.0000	208.00	No Ice	4.35	10.51	42.90
			0.00				1/2" Ice	4.79	11.56	104.60
			1.00				1" Ice	5.25	12.49	177.42
							2" Ice	6.17	14.40	348.65
							4" Ice	8.11	18.43	824.28
(2) LPA-80080/6CF w/ mount pipe (Verizon Wireless)	C	From Face	5.00		-6.0000	208.00	No Ice	4.35	10.51	42.90
			0.00				1/2" Ice	4.79	11.56	104.60
			1.00				1" Ice	5.25	12.49	177.42
							2" Ice	6.17	14.40	348.65
							4" Ice	8.11	18.43	824.28
BXA-70063/6CF-EDIN w/ Mount Pipe (Verizon Wireless)	B	From Face	5.00		-6.0000	208.00	No Ice	8.23	5.66	46.20
			0.00				1/2" Ice	8.98	6.92	104.67
			1.00				1" Ice	9.70	8.03	175.63
							2" Ice	11.08	9.93	344.24
							4" Ice	13.96	13.93	826.15
(2) LPA-171080/8CFx2 w/ Mount Pipe (Verizon Wireless)	A	From Face	5.00		-6.0000	208.00	No Ice	2.50	4.42	26.75
			0.00				1/2" Ice	2.86	5.09	58.67
			1.00				1" Ice	3.24	5.76	98.96
							2" Ice	4.01	7.17	198.35
							4" Ice	5.87	10.25	499.64
(2) LPA-171080/8CFx2 w/ Mount Pipe (Verizon Wireless)	B	From Face	5.00		-6.0000	208.00	No Ice	2.50	4.42	26.75
			0.00				1/2" Ice	2.86	5.09	58.67
			1.00				1" Ice	3.24	5.76	98.96
							2" Ice	4.01	7.17	198.35
							4" Ice	5.87	10.25	499.64
(2) LPA-171080/8CFx2 w/ Mount Pipe (Verizon Wireless)	C	From Face	5.00		-6.0000	208.00	No Ice	2.50	4.42	26.75
			0.00				1/2" Ice	2.86	5.09	58.67
			1.00				1" Ice	3.24	5.76	98.96
							2" Ice	4.01	7.17	198.35
							4" Ice	5.87	10.25	499.64
BXA-70063/6CF-EDIN w/ Mount Pipe (Verizon Wireless)	A	From Face	5.00		-6.0000	208.00	No Ice	8.23	5.66	46.20
			0.00				1/2" Ice	8.98	6.92	104.67
			1.00				1" Ice	9.70	8.03	175.63
							2" Ice	11.08	9.93	344.24
							4" Ice	13.96	13.93	826.15
BXA-70063/6CF-EDIN w/ Mount Pipe (Verizon Wireless)	C	From Face	5.00		-6.0000	208.00	No Ice	8.23	5.66	46.20
			0.00				1/2" Ice	8.98	6.92	104.67
			1.00				1" Ice	9.70	8.03	175.63
							2" Ice	11.08	9.93	344.24
							4" Ice	13.96	13.93	826.15

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<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b>	HRT 105, CT BU#806363	<b>Page</b>	13 of 29
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	<b>Client</b>	Crown Castle	<b>Designed by</b>	Casey Hymore

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz	Lateral					
Sector Mount [SM 505-3]	A	None			0.0000	199.00	No Ice 34.86	34.86	1725.30
							1/2" Ice 49.79	49.79	2316.90
							1" Ice 64.72	64.72	2908.50
							2" Ice 94.58	94.58	4091.70
							4" Ice 154.30	154.30	6458.10
APXVSP18-C-A20 w/ Mount Pipe	C	From Leg	4.00	-36.0000	199.00		No Ice 8.50	6.95	82.55
			-3.00				1/2" Ice 9.15	8.13	147.74
			-1.00				1" Ice 9.77	9.02	224.90
							2" Ice 11.03	10.84	405.88
							4" Ice 13.68	14.85	908.85
APXVSP18-C-A20 w/ Mount Pipe	B	From Leg	3.50	-46.0000	199.00		No Ice 8.50	6.95	82.55
			-3.50				1/2" Ice 9.15	8.13	147.74
			-1.00				1" Ice 9.77	9.02	224.90
							2" Ice 11.03	10.84	405.88
							4" Ice 13.68	14.85	908.85
APXVSP18-C-A20 w/ Mount Pipe	A	From Leg	2.00	-66.0000	199.00		No Ice 8.50	6.95	82.55
			-4.50				1/2" Ice 9.15	8.13	147.74
			-1.00				1" Ice 9.77	9.02	224.90
							2" Ice 11.03	10.84	405.88
							4" Ice 13.68	14.85	908.85
1900MHz RRH (65MHz) TMA	C	From Leg	4.00	-36.0000	199.00		No Ice 2.77	2.70	60.00
			-3.00				1/2" Ice 3.01	2.94	83.90
			-1.00				1" Ice 3.26	3.18	111.08
							2" Ice 3.78	3.70	176.02
							4" Ice 4.93	4.85	353.75
1900MHz RRH (65MHz) TMA	B	From Leg	3.50	-46.0000	199.00		No Ice 2.77	2.70	60.00
			-3.50				1/2" Ice 3.01	2.94	83.90
			-1.00				1" Ice 3.26	3.18	111.08
							2" Ice 3.78	3.70	176.02
							4" Ice 4.93	4.85	353.75
1900MHz RRH (65MHz) TMA	A	From Leg	2.00	-66.0000	199.00		No Ice 2.77	2.70	60.00
			-4.50				1/2" Ice 3.01	2.94	83.90
			-1.00				1" Ice 3.26	3.18	111.08
							2" Ice 3.78	3.70	176.02
							4" Ice 4.93	4.85	353.75
800MHz 2x50W RRH w/ Filter	C	From Leg	4.00	-36.0000	199.00		No Ice 2.40	2.25	64.00
			-3.00				1/2" Ice 2.61	2.46	86.12
			-1.00				1" Ice 2.83	2.68	111.30
							2" Ice 3.30	3.13	171.62
							4" Ice 4.34	4.15	337.52
800MHz 2x50W RRH w/ Filter	B	From Leg	3.50	-46.0000	199.00		No Ice 2.40	2.25	64.00
			-3.50				1/2" Ice 2.61	2.46	86.12
			-1.00				1" Ice 2.83	2.68	111.30
							2" Ice 3.30	3.13	171.62
							4" Ice 4.34	4.15	337.52
800MHz 2x50W RRH w/ Filter	A	From Leg	2.00	-66.0000	199.00		No Ice 2.40	2.25	64.00
			-4.50				1/2" Ice 2.61	2.46	86.12
			-1.00				1" Ice 2.83	2.68	111.30
							2" Ice 3.30	3.13	171.62
							4" Ice 4.34	4.15	337.52
(4) 6' Empty Mount Pipe	C	From Leg	4.00	0.0000	199.00		No Ice 1.43	1.43	22.90
			-3.00				1/2" Ice 1.95	1.95	36.54
			-1.00				1" Ice 2.34	2.34	55.61
							2" Ice 3.20	3.20	102.08
							4" Ice 5.15	5.15	258.02
(4) 6' Empty Mount Pipe	B	From Leg	3.50	0.0000	199.00		No Ice 1.43	1.43	22.90
			-3.50				1/2" Ice 1.95	1.95	36.54



<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b>	HRT 105, CT BU#806363	<b>Page</b>	14 of 29
	<b>Project</b>	Vertical Structures Job No. 2013-004-035	<b>Date</b>	16:45:29 04/23/13
	<b>Client</b>	Crown Castle	<b>Designed by</b>	Casey Hymore

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight lb
			-1.00			1" Ice 2.34	2.34	55.61
						2" Ice 3.20	3.20	102.08
						4" Ice 5.15	5.15	258.02
(4) 6' Empty Mount Pipe	A	From Leg	2.00	0.0000	199.00	No Ice 1.43	1.43	22.90
			-4.50			1/2" Ice 1.95	1.95	36.54
			-1.00			1" Ice 2.34	2.34	55.61
						2" Ice 3.20	3.20	102.08
						4" Ice 5.15	5.15	258.02
**								
14' Angle Sector Frame (3)	A	None		0.0000	189.00	No Ice 24.75	24.75	750.00
						1/2" Ice 33.19	33.19	1125.00
						1" Ice 41.63	41.63	1500.00
						2" Ice 58.50	58.50	2250.00
						4" Ice 92.25	92.25	3750.00
(2) 7770.00 w/ mount pipe	A	From Face	5.00	-7.0000	189.00	No Ice 6.22	4.35	56.90
			0.00			1/2" Ice 6.77	5.20	102.99
			1.00			1" Ice 7.30	5.92	159.01
						2" Ice 8.38	7.41	293.01
						4" Ice 10.69	10.76	679.74
(2) 7770.00 w/ mount pipe	B	From Face	5.00	-9.0000	189.00	No Ice 6.22	4.35	56.90
			0.00			1/2" Ice 6.77	5.20	102.99
			1.00			1" Ice 7.30	5.92	159.01
						2" Ice 8.38	7.41	293.01
						4" Ice 10.69	10.76	679.74
(2) 7770.00 w/ mount pipe	C	From Face	5.00	-8.0000	189.00	No Ice 6.22	4.35	56.90
			0.00			1/2" Ice 6.77	5.20	102.99
			1.00			1" Ice 7.30	5.92	159.01
						2" Ice 8.38	7.41	293.01
						4" Ice 10.69	10.76	679.74
(2) LGP13519 Diplexer	A	From Face	5.00	-7.0000	189.00	No Ice 0.27	0.18	5.50
			0.00			1/2" Ice 0.34	0.25	7.92
			1.00			1" Ice 0.43	0.32	11.41
						2" Ice 0.62	0.49	22.43
						4" Ice 1.10	0.94	66.02
(2) LGP13519 Diplexer	B	From Face	5.00	-9.0000	189.00	No Ice 0.27	0.18	5.50
			0.00			1/2" Ice 0.34	0.25	7.92
			1.00			1" Ice 0.43	0.32	11.41
						2" Ice 0.62	0.49	22.43
						4" Ice 1.10	0.94	66.02
(2) LGP13519 Diplexer	C	From Face	5.00	-8.0000	189.00	No Ice 0.27	0.18	5.50
			0.00			1/2" Ice 0.34	0.25	7.92
			1.00			1" Ice 0.43	0.32	11.41
						2" Ice 0.62	0.49	22.43
						4" Ice 1.10	0.94	66.02
(2) Cleargain Dual Band 800/1900 TMA (VSI)	A	From Face	5.00	-7.0000	189.00	No Ice 1.54	0.80	22.50
			0.00			1/2" Ice 1.71	0.94	33.51
			1.00			1" Ice 1.89	1.08	46.74
						2" Ice 2.27	1.39	80.63
						4" Ice 3.14	2.11	183.58
(2) Cleargain Dual Band 800/1900 TMA (VSI)	B	From Face	5.00	-9.0000	189.00	No Ice 1.54	0.80	22.50
			0.00			1/2" Ice 1.71	0.94	33.51
			1.00			1" Ice 1.89	1.08	46.74
						2" Ice 2.27	1.39	80.63
						4" Ice 3.14	2.11	183.58
(2) Cleargain Dual Band 800/1900 TMA (VSI)	C	From Face	5.00	-8.0000	189.00	No Ice 1.54	0.80	22.50
			0.00			1/2" Ice 1.71	0.94	33.51
			1.00			1" Ice 1.89	1.08	46.74

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b>	HRT 105, CT BU#806363	<b>Page</b>	15 of 29
	<b>Project</b>	Vertical Structures Job No. 2013-004-035	<b>Date</b>	16:45:29 04/23/13
	<b>Client</b>	Crown Castle	<b>Designed by</b>	Casey Hymore

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>1</sub> Front	C <sub>A</sub> A <sub>2</sub> Side	Weight
			Horz	Lateral					
			ft	ft					
						2" Ice	2.27	1.39	80.63
						4" Ice	3.14	2.11	183.58
AM-X-CD-14-65-00T-RET w/ Mount Pipe	A	From Face	5.00	-7.0000	189.00	No Ice	5.74	4.02	34.75
			0.00			1/2" Ice	6.20	4.63	77.95
			1.00			1" Ice	6.66	5.28	129.96
						2" Ice	7.62	6.68	254.09
						4" Ice	9.67	9.74	610.07
AM-X-CD-14-65-00T-RET w/ Mount Pipe	B	From Face	5.00	-9.0000	189.00	No Ice	5.74	4.02	34.75
			0.00			1/2" Ice	6.20	4.63	77.95
			1.00			1" Ice	6.66	5.28	129.96
						2" Ice	7.62	6.68	254.09
						4" Ice	9.67	9.74	610.07
AM-X-CD-14-65-00T-RET w/ Mount Pipe	C	From Face	5.00	-8.0000	189.00	No Ice	5.74	4.02	34.75
			0.00			1/2" Ice	6.20	4.63	77.95
			1.00			1" Ice	6.66	5.28	129.96
						2" Ice	7.62	6.68	254.09
						4" Ice	9.67	9.74	610.07
(2) RRUS-11 BTS (19.69 x 16.97 x 7.17)	A	From Face	5.00	-7.0000	189.00	No Ice	3.25	1.37	47.62
			0.00			1/2" Ice	3.49	1.55	68.42
			1.00			1" Ice	3.74	1.74	92.25
						2" Ice	4.27	2.14	149.81
						4" Ice	5.43	3.04	309.89
(2) RRUS-11 BTS (19.69 x 16.97 x 7.17)	B	From Face	5.00	-9.0000	189.00	No Ice	3.25	1.37	47.62
			0.00			1/2" Ice	3.49	1.55	68.42
			1.00			1" Ice	3.74	1.74	92.25
						2" Ice	4.27	2.14	149.81
						4" Ice	5.43	3.04	309.89
(2) RRUS-11 BTS (19.69 x 16.97 x 7.17)	C	From Face	5.00	-8.0000	189.00	No Ice	3.25	1.37	47.62
			0.00			1/2" Ice	3.49	1.55	68.42
			1.00			1" Ice	3.74	1.74	92.25
						2" Ice	4.27	2.14	149.81
						4" Ice	5.43	3.04	309.89
DC6-48-60-18-8F	A	From Face	5.00	-7.0000	189.00	No Ice	2.57	4.32	18.90
			0.00			1/2" Ice	2.80	4.60	50.21
			1.00			1" Ice	3.04	4.88	85.17
						2" Ice	3.54	5.49	166.87
						4" Ice	4.66	6.80	382.77
**									
APXV18-206517LS w/Mount Pipe	A	From Leg	1.00	54.0000	183.00	No Ice	5.06	4.44	49.40
			0.00			1/2" Ice	5.51	5.37	88.94
			0.00			1" Ice	5.96	6.17	139.08
						2" Ice	6.90	7.83	263.36
						4" Ice	8.99	11.36	638.76
APXV18-206517LS w/Mount Pipe	B	From Leg	1.00	54.0000	183.00	No Ice	5.06	4.44	49.40
			0.00			1/2" Ice	5.51	5.37	88.94
			0.00			1" Ice	5.96	6.17	139.08
						2" Ice	6.90	7.83	263.36
						4" Ice	8.99	11.36	638.76
APXV18-206517LS w/Mount Pipe	C	From Leg	1.00	54.0000	183.00	No Ice	5.06	4.44	49.40
			0.00			1/2" Ice	5.51	5.37	88.94
			0.00			1" Ice	5.96	6.17	139.08
						2" Ice	6.90	7.83	263.36
						4" Ice	8.99	11.36	638.76
**									
Sector Mount [SM 510-3]	A	None		0.0000	175.00	No Ice	40.10	40.10	2396.40
						1/2" Ice	57.33	57.33	3089.00
						1" Ice	74.56	74.56	3781.60

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 16 of 29
	<b>Project</b> Vertical Structures Job No. 2013-004-035	<b>Date</b> 16:45:29 04/23/13
	<b>Client</b> Crown Castle	<b>Designed by</b> Casey Hymore

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Horz	Lateral					
(4) DB844H90E-XY w/Mount Pipe	A	From Face	5.00	-6.0000	175.00	2" Ice	109.02	109.02	5166.80
			0.00			4" Ice	177.94	177.94	7937.20
			1.00			No Ice	3.58	5.40	35.55
						1/2" Ice	4.20	6.49	76.59
						1" Ice	4.73	7.30	127.74
(4) DB844H90E-XY w/Mount Pipe	B	From Face	5.00	-6.0000	175.00	2" Ice	5.86	8.96	251.11
			0.00			4" Ice	8.27	12.49	616.43
			1.00			No Ice	3.58	5.40	35.55
						1/2" Ice	4.20	6.49	76.59
						1" Ice	4.73	7.30	127.74
(4) DB844H90E-XY w/Mount Pipe	C	From Face	5.00	-6.0000	175.00	2" Ice	5.86	8.96	251.11
			0.00			4" Ice	8.27	12.49	616.43
			1.00			No Ice	3.58	5.40	35.55
						1/2" Ice	4.20	6.49	76.59
						1" Ice	4.73	7.30	127.74
8'x2" Antenna Mount Pipe	C	From Leg	3.00	0.0000	175.00	2" Ice	5.86	8.96	251.11
			3.00			4" Ice	8.27	12.49	616.43
			0.00			No Ice	1.90	1.90	26.00
						1/2" Ice	2.73	2.73	40.34
						1" Ice	3.40	3.40	59.96
8'x2" Antenna Mount Pipe	C	From Leg	3.00	0.0000	175.00	2" Ice	4.40	4.40	115.66
			-3.00			4" Ice	6.50	6.50	297.15
			0.00			No Ice	1.90	1.90	26.00
						1/2" Ice	2.73	2.73	40.34
						1" Ice	3.40	3.40	59.96
** Side Arm Mount [SO 308-1]	B	From Leg	3.00	0.0000	164.00	2" Ice	4.40	4.40	115.66
			0.00			4" Ice	6.50	6.50	297.15
			0.00			No Ice	0.98	3.03	53.00
						1/2" Ice	1.70	5.22	78.75
						1" Ice	2.42	7.41	104.50
Side Arm Mount [SO 308-1]	A	Stand-Off Right	3.00	0.0000	167.00	2" Ice	3.86	11.79	156.01
			0.00			4" Ice	6.74	20.55	259.02
			0.00			No Ice	0.98	3.03	53.00
						1/2" Ice	1.70	5.22	78.75
						1" Ice	2.42	7.41	104.50
1151-3	B	From Leg	6.00	0.0000	164.00	2" Ice	3.86	11.79	156.01
			0.00			4" Ice	6.74	20.55	259.02
			9.00			No Ice	4.18	4.18	16.00
						1/2" Ice	5.73	5.73	46.53
						1" Ice	7.30	7.30	86.79
1151-3	A	Stand-Off Right	6.00	0.0000	167.00	2" Ice	10.48	10.48	197.06
			0.00			4" Ice	14.75	14.75	540.93
			6.00			No Ice	4.18	4.18	16.00
						1/2" Ice	5.73	5.73	46.53
						1" Ice	7.30	7.30	86.79
** SD310-HL	A	Stand-Off Right	6.00	0.0000	164.00	2" Ice	10.48	10.48	197.06
			0.00			4" Ice	14.75	14.75	540.93
			-2.00			No Ice	0.76	0.76	7.10
						1/2" Ice	1.01	1.01	13.19
						1" Ice	1.26	1.26	22.08
** Side Arm Mount [SO 308-1]	A	Stand-Off Right	3.00	0.0000	147.00	2" Ice	1.80	1.80	48.87
			0.00			4" Ice	3.09	3.09	142.70
						No Ice	0.98	3.03	53.00
					1/2" Ice	1.70	5.22	78.75	

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 17 of 29
	<b>Project</b> Vertical Structures Job No. 2013-004-035	<b>Date</b> 16:45:29 04/23/13
	<b>Client</b> Crown Castle	<b>Designed by</b> Casey Hymore

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz Lateral	Vert					
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	lb
			0.00				1" Ice 2.42	7.41	104.50
							2" Ice 3.86	11.79	156.01
							4" Ice 6.74	20.55	259.02
Side Arm Mount [SO 308-1]	C	Stand-Off Left	3.00		0.0000	145.00	No Ice 0.98	3.03	53.00
			0.00				1/2" Ice 1.70	5.22	78.75
			0.00				1" Ice 2.42	7.41	104.50
							2" Ice 3.86	11.79	156.01
1151-3	A	Stand-Off Right	6.00		0.0000	147.00	4" Ice 6.74	20.55	259.02
			0.00				No Ice 4.18	4.18	16.00
			6.00				1/2" Ice 5.73	5.73	46.53
							1" Ice 7.30	7.30	86.79
							2" Ice 10.48	10.48	197.06
SD310-HL	C	Stand-Off Left	6.00		0.0000	145.00	4" Ice 14.75	14.75	540.93
			0.00				No Ice 1.50	1.50	15.00
			3.00				1/2" Ice 2.70	2.70	19.50
							1" Ice 3.90	3.90	24.00
							2" Ice 6.30	6.30	33.00
							4" Ice 11.10	11.10	51.00
**									
Side Arm Mount [SO 201-3]	A	None			0.0000	139.00	No Ice 5.71	5.71	288.00
							1/2" Ice 7.91	7.91	351.14
							1" Ice 10.11	10.11	414.28
							2" Ice 14.51	14.51	540.56
							4" Ice 23.31	23.31	793.12
RR90-17-02DP w/Mount Pipe	A	From Leg	2.00		30.0000	139.00	No Ice 4.91	3.64	43.55
			0.00				1/2" Ice 5.57	4.70	81.64
			1.00				1" Ice 6.14	5.48	130.14
							2" Ice 7.32	7.08	249.13
							4" Ice 9.81	10.47	609.39
RR90-17-02DP w/Mount Pipe	B	From Leg	2.00		19.0000	139.00	No Ice 4.91	3.64	43.55
			0.00				1/2" Ice 5.57	4.70	81.64
			1.00				1" Ice 6.14	5.48	130.14
							2" Ice 7.32	7.08	249.13
							4" Ice 9.81	10.47	609.39
RR90-17-02DP w/Mount Pipe	C	From Leg	2.00		24.0000	139.00	No Ice 4.91	3.64	43.55
			0.00				1/2" Ice 5.57	4.70	81.64
			1.00				1" Ice 6.14	5.48	130.14
							2" Ice 7.32	7.08	249.13
							4" Ice 9.81	10.47	609.39
APX16DWV-16DWV-S-E-A 20	A	From Leg	2.00		84.0000	139.00	No Ice 7.23	2.15	40.70
			0.00				1/2" Ice 7.68	2.49	74.24
			1.00				1" Ice 8.14	2.84	112.65
							2" Ice 9.09	3.55	204.92
							4" Ice 11.09	5.08	456.62
APX16DWV-16DWV-S-E-A 20	B	From Leg	2.00		90.0000	139.00	No Ice 7.23	2.15	40.70
			0.00				1/2" Ice 7.68	2.49	74.24
			1.00				1" Ice 8.14	2.84	112.65
							2" Ice 9.09	3.55	204.92
							4" Ice 11.09	5.08	456.62
APX16DWV-16DWV-S-E-A 20	C	From Leg	2.00		54.0000	139.00	No Ice 7.23	2.15	40.70
			0.00				1/2" Ice 7.68	2.49	74.24
			1.00				1" Ice 8.14	2.84	112.65
							2" Ice 9.09	3.55	204.92
							4" Ice 11.09	5.08	456.62
ATMPP1412D-1CWA TMA	A	From Leg	2.00		30.0000	139.00	No Ice 1.17	0.42	12.50
			0.00				1/2" Ice 1.32	0.53	19.51
			1.00				1" Ice 1.48	0.65	28.38

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b>	HRT 105, CT BU#806363	<b>Page</b>	18 of 29
	<b>Project</b>	Vertical Structures Job No. 2013-004-035	<b>Date</b>	16:45:29 04/23/13
	<b>Client</b>	Crown Castle	<b>Designed by</b>	Casey Hymore

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz	Lateral					
							ft <sup>2</sup>	ft <sup>2</sup>	lb
ATMPP1412D-1CWA TMA	B	From Leg	2.00	19.0000	139.00	2" Ice	1.82	0.92	52.45
						4" Ice	2.61	1.57	131.40
						No Ice	1.17	0.42	12.50
						1/2" Ice	1.32	0.53	19.51
						1" Ice	1.48	0.65	28.38
ATMPP1412D-1CWA TMA	C	From Leg	2.00	24.0000	139.00	2" Ice	1.82	0.92	52.45
						4" Ice	2.61	1.57	131.40
						No Ice	1.17	0.42	12.50
						1/2" Ice	1.32	0.53	19.51
						1" Ice	1.48	0.65	28.38
ATMAA1412D-1A20 TMA	A	From Leg	2.00	84.0000	139.00	2" Ice	1.82	0.92	52.45
						4" Ice	2.61	1.57	131.40
						No Ice	1.17	0.47	13.00
						1/2" Ice	1.31	0.57	20.62
						1" Ice	1.47	0.69	30.11
ATMAA1412D-1A20 TMA	B	From Leg	2.00	90.0000	139.00	2" Ice	1.81	0.95	55.52
						4" Ice	2.58	1.57	137.44
						No Ice	1.17	0.47	13.00
						1/2" Ice	1.31	0.57	20.62
						1" Ice	1.47	0.69	30.11
ATMAA1412D-1A20 TMA	C	From Leg	2.00	54.0000	139.00	2" Ice	1.81	0.95	55.52
						4" Ice	2.58	1.57	137.44
						No Ice	1.17	0.47	13.00
						1/2" Ice	1.31	0.57	20.62
						1" Ice	1.47	0.69	30.11
**						4" Ice	2.58	1.57	137.44
**									
Side Arm Mount [SO 308-1]	A	Stand-Off Right	3.00	0.0000	128.00	No Ice	0.98	3.03	53.00
			0.00			1/2" Ice	1.70	5.22	78.75
			0.00			1" Ice	2.42	7.41	104.50
						2" Ice	3.86	11.79	156.01
						4" Ice	6.74	20.55	259.02
1142-2C	A	Stand-Off Right	6.00	0.0000	128.00	No Ice	2.09	2.09	24.00
			0.00			1/2" Ice	3.37	3.37	40.81
			4.00			1" Ice	4.67	4.67	65.63
						2" Ice	7.32	7.32	139.94
						4" Ice	10.79	10.79	391.52
**									
Side Arm Mount [SO 702-1]	C	Stand-Off Right	1.50	0.0000	51.00	No Ice	1.00	1.43	27.00
			0.00			1/2" Ice	1.00	2.05	38.00
			0.00			1" Ice	1.00	2.67	49.00
						2" Ice	1.00	3.91	71.00
						4" Ice	1.00	6.39	115.00
Generic GPS (VSI)	C	Stand-Off Right	3.00	0.0000	51.00	No Ice	1.40	1.40	25.00
			0.00			1/2" Ice	1.70	1.70	30.00
			0.00			1" Ice	1.90	1.90	35.00
						2" Ice	2.20	2.20	40.00
						4" Ice	2.50	2.50	45.00

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### Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weight	
				ft	°	°	ft	ft	ft <sup>2</sup>	lb	
HPD2-23	C	Paraboloid w/Shroud (HP)	From Leg	3.00	-90.0000		175.00	2.04	No Ice	3.27	27.00
				-3.00					1/2" Ice	3.55	45.21
				4.00					1" Ice	3.82	63.42
									2" Ice	4.36	99.84
									4" Ice	5.46	172.68
HPD2-23	C	Paraboloid w/Shroud (HP)	From Leg	3.00	74.0000		175.00	2.04	No Ice	3.27	27.00
				3.00					1/2" Ice	3.55	45.21
				4.00					1" Ice	3.82	63.42
									2" Ice	4.36	99.84
									4" Ice	5.46	172.68

### Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt lb	Allowable Load lb	Ratio Load Allowable	Allowable Ratio	Criteria	
T1	212.625	Leg	A325N	0.7500	4	782.24	19383.70	0.040	✓	1.333	Bolt Tension
		Diagonal	A325N	0.6250	3	978.85	6442.72	0.152	✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	1104.48	6442.72	0.171	✓	1.333	Bolt Shear
T2	202.458	Leg	A325N	0.8750	4	7507.43	26160.00	0.287	✓	1.333	Bolt Tension
		Diagonal	A325N	0.6250	3	3179.78	6442.72	0.494	✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	2668.92	6442.72	0.414	✓	1.333	Bolt Shear
T3	182.292	Leg	A325N	1.0000	4	16465.70	34271.30	0.480	✓	1.333	Bolt Tension
		Diagonal	A325N	0.6250	3	3486.80	6442.72	0.541	✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	3355.20	6442.72	0.521	✓	1.333	Bolt Shear
T4	162.104	Leg	A325N	1.0000	6	16746.20	34407.50	0.487	✓	1.333	Bolt Tension
		Diagonal	A325N	0.6250	3	3603.57	6442.72	0.559	✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	3709.06	6442.72	0.576	✓	1.333	Bolt Shear
T5	141.896	Leg	A325N	1.0000	6	21913.60	34380.00	0.637	✓	1.333	Bolt Tension
		Diagonal	A325N	0.6250	3	4322.41	6442.72	0.671	✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	3908.79	6442.72	0.607	✓	1.333	Bolt Shear
T6	121.688	Leg	A325N	1.0000	6	26348.60	34378.80	0.766	✓	1.333	Bolt Tension
		Diagonal	A325N	0.6250	3	3964.21	6442.72	0.615	✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	3930.47	6442.72	0.610	✓	1.333	Bolt Shear
T7	101.479	Leg	A325N	1.0000	8	22821.40	34440.80	0.663	✓	1.333	Bolt Tension
		Diagonal	A325N	0.6250	3	3980.08	6442.72	0.618	✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	4185.18	6442.72	0.650	✓	1.333	Bolt Shear
T8	81.2708	Leg	A325N	1.0000	8	25617.60	34434.30	0.744	✓	1.333	Bolt Tension
		Diagonal	A325N	0.6250	3	3950.33	6442.72	0.613	✓	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	4417.61	6442.72	0.686	✓	1.333	Bolt Shear

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 20 of 29
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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt lb	Allowable Load lb	Ratio Load Allowable	Allowable Ratio	Criteria
T9	61	Leg	A325N	1.0000	8	28437.40	34405.70	0.827	✓	1.333 Bolt Tension
		Diagonal	A325N	0.6250	3	4274.49	6442.72	0.663	✓	1.333 Bolt Shear
		Horizontal	A325N	0.6250	2	5013.78	6442.72	0.778	✓	1.333 Bolt Shear
T10	40.6667	Leg	A325N	1.0000	8	30920.80	34405.60	0.899	✓	1.333 Bolt Tension
		Diagonal	A325N	0.7500	3	5903.86	9277.52	0.636	✓	1.333 Bolt Shear
		Horizontal	A325N	0.7500	2	4985.17	9277.52	0.537	✓	1.333 Bolt Shear
T11	20.3333	Diagonal	A325N	0.7500	3	6626.74	9277.52	0.714	✓	1.333 Bolt Shear
		Horizontal	A325N	0.7500	2	5912.48	9277.52	0.637	✓	1.333 Bolt Shear

### Compression Checks

### Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P P <sub>a</sub>
T1	212.625 - 202.458	ROHN 2.5 STD	10.17	4.79	60.7 K=1.00	22.599	1.7040	-5592.10	38510.00	0.145 ✓
T2	202.458 - 182.292	ROHN 3 EH	20.17	6.53	68.9 K=1.00	21.134	3.0159	-37114.70	63739.10	0.582 ✓
T3	182.292 - 162.104	ROHN 4 EH	20.22	6.54	53.1 K=1.00	23.854	4.4074	-77173.20	105133.00	0.734 ✓
T4	162.104 - 141.896	ROHN 5 EH	20.24	6.54	42.7 K=1.00	25.452	6.1120	-114552.00	155560.00	0.736 ✓
T5	141.896 - 121.688	ROHN 6 EHS	20.25	9.81	52.9 K=1.00	23.891	6.7133	-149489.00	160385.00	0.932 ✓
T6	121.688 - 101.479	ROHN 6 EH	20.26	9.82	53.7 K=1.00	23.769	8.4049	-179798.00	199773.00	0.900 ✓
T7	101.479 - 81.2708	ROHN 6 EH	20.26	9.82	53.7 K=1.00	23.769	8.4049	-208369.00	199773.00	1.043 ✓
T8	81.2708 - 61	ROHN 8 EHS	20.33	9.82	40.4 K=1.00	25.782	9.7193	-235237.00	250585.00	0.939 ✓
T9	61 - 40.6667	ROHN 8 EHS	20.38	9.82	40.3 K=1.00	25.784	9.7193	-262503.00	250601.00	1.047 ✓
T10	40.6667 - 20.3333	ROHN 8 EH	20.39	9.82	40.9 K=1.00	25.699	12.7627	-287482.00	327992.00	0.876 ✓
T11	20.3333 - 0	ROHN 8 EH	20.37	9.81	40.9 K=1.00	25.704	12.7627	-314703.00	328057.00	0.959 ✓

### Diagonal Design Data (Compression)

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 21 of 29
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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P P <sub>a</sub>
T1	212.625 - 202.458	ROHN 2 STD	5.93	5.93	90.5 K=1.00	16.837	1.0745	-2936.54	18092.40	0.162
T2	202.458 - 182.292	ROHN 2 STD	7.30	7.30	111.3 K=1.00	12.045	1.0745	-9539.35	12942.70	0.737
T3	182.292 - 162.104	ROHN 2 STD	7.91	7.91	120.5 K=1.00	10.280	1.0745	-10460.40	11046.50	0.947
T4	162.104 - 141.896	ROHN 2 STD	8.55	8.55	130.4 K=1.00	8.783	1.0745	-10512.60	9437.83	1.114
T5	141.896 - 121.688	ROHN 2.5 STD	11.73	11.73	148.5 K=1.00	6.769	1.7040	-12824.10	11535.40	1.112
T6	121.688 - 101.479	ROHN 2.5 STD	12.53	12.53	158.7 K=1.00	5.932	1.7040	-11778.10	10107.60	1.165
T7	101.479 - 81.2708	ROHN 3 STD	13.37	13.37	137.9 K=1.00	7.858	2.2285	-11769.50	17511.20	0.672
T8	81.2708 - 61	ROHN 3 STD	14.28	14.28	147.3 K=1.00	6.887	2.2285	-11851.00	15347.80	0.772
T9	61 - 40.6667	ROHN 3 STD	15.25	15.25	157.3 K=1.00	6.038	2.2285	-12823.50	13454.70	0.953
T10	40.6667 - 20.3333	ROHN 3 STD	23.30	11.65	120.1 K=1.00	10.349	2.2285	-17711.60	23061.80	0.768
T11	20.3333 - 0	ROHN 3 STD	23.95	11.97	123.5 K=1.00	9.795	2.2285	-19880.20	21827.00	0.911

### Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P P <sub>a</sub>
T1	212.625 - 202.458	ROHN 1.5 STD	8.52	4.14	79.8 K=1.00	19.049	0.7995	-2172.98	15229.20	0.143
T2	202.458 - 182.292	ROHN 1.5 STD	8.60	4.15	80.0 K=1.00	19.003	0.7995	-5303.13	15192.40	0.349
T3	182.292 - 162.104	ROHN 1.5 STD	10.02	4.82	93.0 K=1.00	16.288	0.7995	-6710.40	13021.80	0.515
T4	162.104 - 141.896	ROHN 2 STD	12.11	5.82	88.8 K=1.00	17.203	1.0745	-7418.12	18485.40	0.401
T5	141.896 - 121.688	ROHN 2 STD	13.94	6.69	102.1 K=1.00	14.228	1.0745	-7817.58	15288.30	0.511
T6	121.688 - 101.479	ROHN 2 STD	16.32	7.88	120.2 K=1.00	10.340	1.0745	-7860.94	11110.90	0.707
T7	101.479 - 81.2708	ROHN 2.5 STD	18.82	9.13	115.7 K=1.00	11.160	1.7040	-8370.37	19017.70	0.440
T8	81.2708 - 61	ROHN 2.5 STD	21.38	10.33	130.9 K=1.00	8.720	1.7040	-8821.49	14859.30	0.594
T9	61 - 40.6667	ROHN 2.5 STD	23.95	11.62	147.1 K=1.00	6.897	1.7040	-9899.75	11753.50	0.842
T10	40.6667 - 20.3333	ROHN 3 STD	25.25	12.27	126.5 K=1.00	9.330	2.2285	-9970.33	20791.70	0.480
T11	20.3333 - 0	ROHN 3 STD	27.90	13.59	140.1 K=1.00	7.603	2.2285	-11825.00	16943.90	0.698



<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b>	HRT 105, CT BU#806363	<b>Page</b>	22 of 29
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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P P <sub>a</sub>
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**Top Girt Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P P <sub>a</sub>
T1	212.625 - 202.458	ROHN 1.5 STD	8.50	4.13	79.6 K=1.00	19.087	0.7995	-228.36	15259.40	0.015 ✓

**Redundant Horizontal (1) Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P P <sub>a</sub>
T10	40.6667 - 20.3333	ROHN 1.5 STD	6.31	5.95	114.8 K=1.00	11.022	0.7995	-4989.94	8811.44	0.566 ✓
T11	20.3333 - 0	ROHN 1.5 STD	6.97	6.61	127.5 K=1.00	9.187	0.7995	-5458.84	7344.62	0.743 ✓

**Redundant Diagonal (1) Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P P <sub>a</sub>
T10	40.6667 - 20.3333	ROHN 2 STD	11.32	10.61	161.7 K=1.00	5.711	1.0745	-4474.65	6136.30	0.729 ✓
T11	20.3333 - 0	ROHN 2 STD	11.72	11.07	168.8 K=1.00	5.241	1.0745	-4588.72	5632.07	0.815 ✓

**Redundant Hip (1) Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P P <sub>a</sub>
T10	40.6667 - 20.3333	ROHN 1.5 STD	6.31	6.31	121.7 K=1.00	10.040	0.7995	-51.73	8026.53	0.006 ✓
T11	20.3333 - 0	ROHN 1.5 STD	6.97	6.97	134.4 K=1.00	8.265	0.7995	-52.14	6607.20	0.008 ✓

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 23 of 29
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**Redundant Hip Diagonal Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P/P <sub>a</sub>
T10	40.6667 - 20.3333	ROHN 2.5 STD	14.97	14.97	189.6 K=1.00	4.154	1.7040	-48.19	7078.91	0.007*
T11	20.3333 - 0	ROHN 2.5 STD	15.80	15.80	200.1 K=1.00	3.729	1.7040	-47.55	6355.22	0.007*

\* DL controls

**Inner Bracing Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P/P <sub>a</sub>
T1	212.625 - 202.458	L2x2x1/8	4.26	4.26	128.6 K=1.00	9.027	0.4844	-4.35	4372.66	0.001
T2	202.458 - 182.292	L2x2x1/8	4.30	4.30	129.8 K=1.00	8.869	0.4844	-7.58	4295.70	0.002
T3	182.292 - 162.104	L2x2x1/8	5.01	5.01	151.3 K=1.00	6.524	0.4844	-7.19	3160.25	0.002
T4	162.104 - 141.896	L2x2x1/8	6.05	6.05	182.7 K=1.00	4.472	0.4844	-7.89	2165.98	0.004
T5	141.896 - 121.688	L2x2x1/8	6.97	6.97	210.4 K=1.00	3.374	0.4844	-7.91	1634.11	0.005
T6	121.688 - 101.479	L2 1/2x2 1/2x3/16	8.16	8.16	197.8 K=1.00	3.817	0.9020	-7.53	3443.19	0.002
T7	101.479 - 81.2708	L3x3x3/16	9.41	9.41	189.4 K=1.00	4.161	1.0900	-7.46	4535.72	0.002*
T8	81.2708 - 61	L3 1/2x3 1/2x1/4	10.69	10.69	184.9 K=1.00	4.370	1.6900	-9.12	7384.85	0.001*
T9	61 - 40.6667	L3 1/2x3 1/2x1/4	11.98	11.98	207.1 K=1.00	3.482	1.6900	-9.27	5884.99	0.002*
T10	40.6667 - 20.3333	ROHN 3 STD	12.63	12.63	130.2 K=1.00	8.807	2.2285	-16.28	19625.00	0.001
T11	20.3333 - 0	ROHN 3 STD	13.95	13.95	143.9 K=1.00	7.217	2.2285	-15.52	16082.00	0.001

\* DL controls

**Tension Checks**

**Leg Design Data (Tension)**

**tnxTower**

**Vertical Structures, Inc.**  
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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P/P <sub>a</sub>
T1	212.625 - 202.458	ROHN 2.5 STD	10.17	4.79	60.7	30.000	1.7040	3128.98	51121.50	0.061
T2	202.458 - 182.292	ROHN 3 EH	20.17	6.53	68.9	30.000	3.0159	30029.70	90477.90	0.332
T3	182.292 - 162.104	ROHN 4 EH	20.22	6.54	53.1	30.000	4.4074	65863.00	132223.00	0.498
T4	162.104 - 141.896	ROHN 5 EH	20.24	6.54	42.7	30.000	6.1120	100477.00	183359.00	0.548
T5	141.896 - 121.688	ROHN 6 EHS	20.25	9.81	52.9	30.000	6.7133	131482.00	201398.00	0.653
T6	121.688 - 101.479	ROHN 6 EH	20.26	9.82	53.7	30.000	8.4049	158091.00	252148.00	0.627
T7	101.479 - 81.2708	ROHN 6 EH	20.26	9.82	53.7	30.000	8.4049	182571.00	252148.00	0.724
T8	81.2708 - 61	ROHN 8 EHS	20.33	9.82	40.4	30.000	9.7193	204941.00	291579.00	0.703
T9	61 - 40.6667	ROHN 8 EHS	20.38	9.82	40.3	30.000	9.7193	227499.00	291579.00	0.780
T10	40.6667 - 20.3333	ROHN 8 EH	20.39	9.82	40.9	30.000	12.7627	247366.00	382882.00	0.646
T11	20.3333 - 0	ROHN 8 EH	20.37	9.81	40.9	30.000	12.7627	269780.00	382882.00	0.705

**Diagonal Design Data (Tension)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P/P <sub>a</sub>
T1	212.625 - 202.458	ROHN 2 STD	5.93	5.93	90.5	30.000	1.0745	2879.47	32235.90	0.089
T2	202.458 - 182.292	ROHN 2 STD	7.30	7.30	111.3	30.000	1.0745	9488.38	32235.90	0.294
T3	182.292 - 162.104	ROHN 2 STD	7.91	7.91	120.5	30.000	1.0745	10395.60	32235.90	0.322
T4	162.104 - 141.896	ROHN 2 STD	8.09	8.09	123.3	30.000	1.0745	10735.20	32235.90	0.333
T5	141.896 - 121.688	ROHN 2.5 STD	11.40	11.40	144.4	30.000	1.7040	12837.50	51121.50	0.251
T6	121.688 - 101.479	ROHN 2.5 STD	12.13	12.13	153.6	30.000	1.7040	11723.90	51121.50	0.229
T7	101.479 - 81.2708	ROHN 3 STD	12.94	12.94	133.4	30.000	2.2285	11661.40	66854.10	0.174
T8	81.2708 - 61	ROHN 3 STD	14.28	14.28	147.3	30.000	2.2285	11461.80	66854.10	0.171
T9	61 - 40.6667	ROHN 3 STD	15.25	15.25	157.3	30.000	2.2285	12365.50	66854.10	0.185
T10	40.6667 - 20.3333	ROHN 3 STD	23.30	11.65	120.1	30.000	2.2285	17108.30	66854.10	0.256
T11	20.3333 - 0	ROHN 3 STD	23.95	11.97	123.5	30.000	2.2285	19277.60	66854.10	0.288

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 25 of 29
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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio $\frac{P}{P_a}$
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### Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio $\frac{P}{P_a}$
T1	212.625 - 202.458	ROHN 1.5 STD	8.52	4.14	79.8	30.000	0.7995	2208.96	23983.70	0.092
T2	202.458 - 182.292	ROHN 1.5 STD	8.60	4.15	80.0	30.000	0.7995	5337.83	23983.70	0.223
T3	182.292 - 162.104	ROHN 1.5 STD	10.02	4.82	93.0	30.000	0.7995	6681.13	23983.70	0.279
T4	162.104 - 141.896	ROHN 2 STD	12.11	5.82	88.8	30.000	1.0745	7394.48	32235.90	0.229
T5	141.896 - 121.688	ROHN 2 STD	13.94	6.69	102.1	30.000	1.0745	7674.30	32235.90	0.238
T6	121.688 - 101.479	ROHN 2 STD	16.32	7.88	120.2	30.000	1.0745	7751.02	32235.90	0.240
T7	101.479 - 81.2708	ROHN 2.5 STD	18.82	9.13	115.7	30.000	1.7040	8295.39	51121.50	0.162
T8	81.2708 - 61	ROHN 2.5 STD	21.38	10.33	130.9	30.000	1.7040	8835.22	51121.50	0.173
T9	61 - 40.6667	ROHN 2.5 STD	23.95	11.62	147.1	30.000	1.7040	10027.60	51121.50	0.196
T10	40.6667 - 20.3333	ROHN 3 STD	25.25	12.27	126.5	30.000	2.2285	9630.63	66854.10	0.144
T11	20.3333 - 0	ROHN 3 STD	27.90	13.59	140.1	30.000	2.2285	11677.00	66854.10	0.175

### Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio $\frac{P}{P_a}$
T1	212.625 - 202.458	ROHN 1.5 STD	8.50	4.13	79.6	30.000	0.7995	229.07	23983.70	0.010

### Redundant Horizontal (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio $\frac{P}{P_a}$
T10	40.6667 - 20.3333	ROHN 1.5 STD	6.31	5.95	114.8	21.600	0.7995	4989.94	17268.30	0.289

<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 26 of 29
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	<b>Client</b> Crown Castle	<b>Designed by</b> Casey Hymore

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P P <sub>a</sub>
T11	20.3333 - 0	ROHN 1.5 STD	6.97	6.61	127.5	21.600	0.7995	5458.84	17268.30	0.316

### Redundant Diagonal (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P P <sub>a</sub>
T10	40.6667 - 20.3333	ROHN 2 STD	11.32	10.61	161.7	21.600	1.0745	4474.65	23209.90	0.193
T11	20.3333 - 0	ROHN 2 STD	11.72	11.07	168.8	21.600	1.0745	4588.72	23209.90	0.198

### Redundant Hip Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P P <sub>a</sub>
T10	40.6667 - 20.3333	ROHN 2.5 STD	14.97	14.97	189.6	21.600	1.7040	87.07	36807.50	0.002
T11	20.3333 - 0	ROHN 2.5 STD	15.80	15.80	200.1	21.600	1.7040	89.02	36807.50	0.002

### Inner Bracing Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P P <sub>a</sub>
T1	212.625 - 202.458	L2x2x1/8	4.25	4.25	81.5	21.600	0.4844	3.96	10462.50	0.000
T2	202.458 - 182.292	L2x2x1/8	4.30	4.30	82.4	21.600	0.4844	6.00	10462.50	0.001
T3	182.292 - 162.104	L2x2x1/8	4.34	4.34	83.1	21.600	0.4844	5.74	10462.50	0.001
T4	162.104 - 141.896	L2x2x1/8	5.38	5.38	103.1	21.600	0.4844	6.73	10462.50	0.001
T5	141.896 - 121.688	L2x2x1/8	6.42	6.42	123.1	21.600	0.4844	4.19	10462.50	0.000
T6	121.688 - 101.479	L2 1/2x2 1/2x3/16	7.55	7.55	116.5	21.600	0.9020	2.05	19483.20	0.000
T7	101.479 - 81.2708	L3x3x3/16	8.80	8.80	112.5	21.600	1.0900	1.14	23544.00	0.000
T10	40.6667 - 20.3333	ROHN 3 STD	12.63	12.63	130.2	30.000	2.2285	0.96	66854.10	0.000

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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio $\frac{P}{P_a}$
T11	20.3333 - 0	ROHN 3 STD	13.95	13.95	143.9	30.000	2.2285	2.79	66854.10	0.000

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SF*P <sub>allow</sub> lb	% Capacity	Pass Fail	
T1	212.625 - 202.458	Leg	ROHN 2.5 STD	2	-5564.11	51333.83	12.7	Pass	
		Diagonal	ROHN 2 STD	8	-2936.54	24117.17	12.2	Pass	
		Horizontal	ROHN 1.5 STD	7	-2172.98	20300.52	10.7	Pass	
T2	202.458 - 182.292	Top Girt	ROHN 1.5 STD	4	-228.36	20340.78	1.1	Pass	
			L2x2x1/8	17	-4.35	5828.76	0.3	Pass	
		Inner Bracing	Leg	ROHN 3 EH	28	-37114.70	84964.22	43.7	Pass
			Diagonal	ROHN 2 STD	39	-9539.35	17252.62	55.3	Pass
			Horizontal	ROHN 1.5 STD	37	-5303.13	20251.47	26.2	Pass
T3	182.292 - 162.104	Inner Bracing	L2x2x1/8	40	-7.52	5726.17	0.3	Pass	
			Leg	ROHN 4 EH	67	-77173.20	140142.28	55.1	Pass
		Diagonal	ROHN 2 STD	72	-10460.40	14724.98	71.0	Pass	
			Horizontal	ROHN 1.5 STD	70	-6710.40	17358.06	38.7	Pass
T4	162.104 - 141.896	Inner Bracing	L2x2x1/8	79	-7.19	4212.61	0.3	Pass	
			Leg	ROHN 5 EH	106	-114552.00	207361.47	55.2	Pass
		Diagonal	ROHN 2 STD	111	-10512.60	12580.63	83.6	Pass	
T5	141.896 - 121.688	Horizontal	ROHN 2 STD	109	-7418.12	24641.04	30.1	Pass	
			Inner Bracing	L2x2x1/8	119	-7.86	2887.25	0.4	Pass
		Leg	ROHN 6 EHS	145	-149489.00	213793.20	69.9	Pass	
			Diagonal	ROHN 2.5 STD	150	-12824.10	15376.69	83.4	Pass
			Horizontal	ROHN 2 STD	148	-7817.58	20379.30	38.4	Pass
T6	121.688 - 101.479	Inner Bracing	L2x2x1/8	157	-7.91	2178.27	0.4	Pass	
			Leg	ROHN 6 EH	172	-179798.00	266297.40	67.5	Pass
		Diagonal	ROHN 2.5 STD	177	-11778.10	13473.43	87.4	Pass	
			Horizontal	ROHN 2 STD	175	-7860.94	14810.83	53.1	Pass
T7	101.479 - 81.2708	Inner Bracing	L2 1/2x2 1/2x3/16	186	-7.53	4589.77	0.3	Pass	
			Leg	ROHN 6 EH	199	-208369.00	266297.40	78.2	Pass
		Diagonal	ROHN 3 STD	204	-11769.50	23342.43	50.4	Pass	
T8	81.2708 - 61	Horizontal	ROHN 2.5 STD	202	-8370.37	25350.59	33.0	Pass	
			Inner Bracing	L3x3x3/16	213	-7.46	4535.72	0.5	Pass
		Leg	ROHN 8 EHS	226	-235237.00	334029.79	70.4	Pass	
			Diagonal	ROHN 3 STD	231	-11851.00	20458.62	57.9	Pass
		Horizontal	ROHN 2.5 STD	229	-8821.49	19807.45	44.5	Pass	
			Inner Bracing	L3 1/2x3 1/2x1/4	238	-9.06	7384.85	0.3	Pass
T9	61 - 40.6667	Leg	ROHN 8 EHS	253	-262503.00	334051.12	78.6	Pass	
			Diagonal	ROHN 3 STD	258	-12823.50	17935.11	71.5	Pass

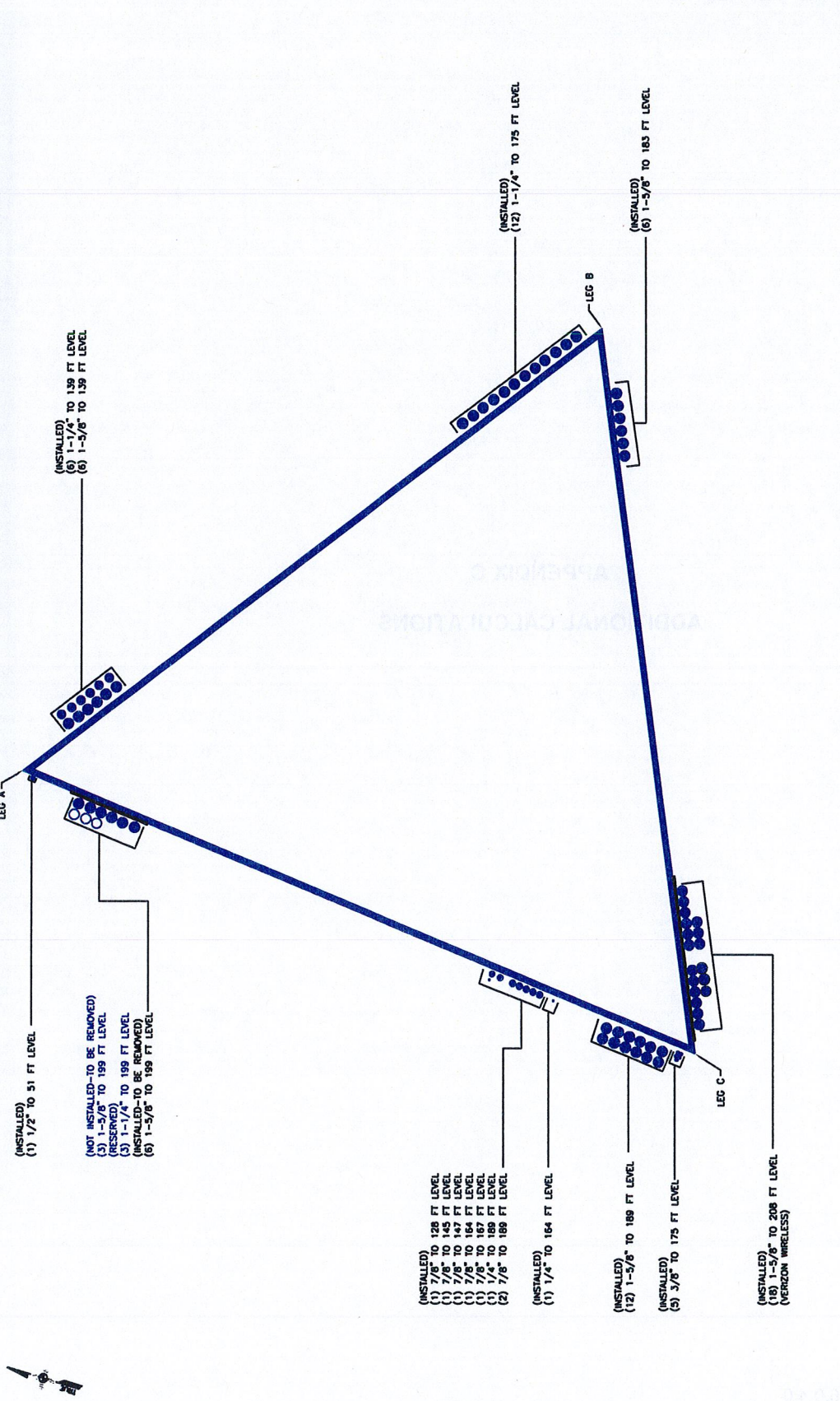
<b>tnxTower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> HRT 105, CT BU#806363	<b>Page</b> 28 of 29
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Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SF*P <sub>allow</sub> lb	% Capacity	Pass Fail		
T10	40.6667 - 20.3333	Horizontal	ROHN 2.5 STD	256	-9899.75	15667.41	63.2	Pass		
		Inner Bracing	L3 1/2x3 1/2x1/4	267	-9.27	5884.99	0.4	Pass		
		Leg	ROHN 8 EH	280	-287482.00	437213.32	65.8	Pass		
								67.4 (b)		
		Diagonal	ROHN 3 STD	287	-17711.60	30741.38	57.6	Pass		
		Horizontal	ROHN 3 STD	283	-9970.33	27715.33	36.0	Pass		
								40.3 (b)		
		Redund Horz 1 Bracing	ROHN 1.5 STD	304	-4989.94	11745.65	42.5	Pass		
		Redund Diag 1 Bracing	ROHN 2 STD	305	-4474.65	8179.69	54.7	Pass		
		Redund Hip 1 Bracing	ROHN 1.5 STD	308	-51.73	10699.36	0.5	Pass		
		Redund Hip Diagonal Bracing	ROHN 2.5 STD	307	-48.19	7078.91	0.7	Pass		
		T11	20.3333 - 0	Inner Bracing	ROHN 3 STD	310	-11.28	19625.00	1.6	Pass
Leg	ROHN 8 EH			313	-314703.00	437299.96	72.0	Pass		
Diagonal	ROHN 3 STD			320	-19880.20	29095.39	68.3	Pass		
Horizontal	ROHN 3 STD			316	-11825.00	22586.22	52.4	Pass		
Redund Horz 1 Bracing	ROHN 1.5 STD			318	-5458.84	9790.38	55.8	Pass		
Redund Diag 1 Bracing	ROHN 2 STD			319	-4588.72	7507.55	61.1	Pass		
Redund Hip 1 Bracing	ROHN 1.5 STD			341	-52.14	8807.40	0.6	Pass		
Redund Hip Diagonal Bracing	ROHN 2.5 STD			340	-47.55	6355.22	0.7	Pass		
Inner Bracing	ROHN 3 STD			343	-10.11	16082.00	1.4	Pass		
								Summary		
								Leg (T9)	78.6	Pass
								Diagonal (T6)	87.4	Pass
						Horizontal (T9)	63.2	Pass		
						Top Girt (T1)	1.1	Pass		
						Redund Horz 1 Bracing (T11)	55.8	Pass		
						Redund Diag 1 Bracing (T11)	61.1	Pass		
						Redund Hip 1 Bracing (T11)	0.6	Pass		
						Redund Hip Diagonal Bracing (T11)	0.7	Pass		
						Inner Bracing (T10)	1.6	Pass		
						Bolt Checks	67.4	Pass		
						<b>RATING =</b>	<b>87.4</b>	<b>Pass</b>		

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	<b>Client</b> Casey Hymore/Open Jobs/2013-004-035 HRT 105, CT/tnx/806363.eri Crown Castle	<b>Designed by</b> Casey Hymore



**APPENDIX B**  
**BASE LEVEL DRAWING**



(INSTALLED)  
(1) 1-1/2 TO 51 FT LEVEL

(NOT INSTALLED--TO BE REMOVED)  
(3) 1-5/8 TO 99 FT LEVEL (RESERVED)  
(3) 1-1/4 TO 99 FT LEVEL (RESERVED)  
(INSTALLED--TO BE REMOVED)  
(6) 1-5/8 TO 199 FT LEVEL

(INSTALLED)  
(1) 7/8 TO 128 FT LEVEL  
(1) 7/8 TO 145 FT LEVEL  
(1) 7/8 TO 147 FT LEVEL  
(1) 7/8 TO 164 FT LEVEL  
(1) 7/8 TO 167 FT LEVEL  
(1) 1/8 TO 187 FT LEVEL  
(2) 7/8 TO 188 FT LEVEL

(INSTALLED)  
(1) 1/4 TO 164 FT LEVEL

(INSTALLED)  
(12) 1-5/8 TO 189 FT LEVEL

(INSTALLED)  
(3) 3/8 TO 175 FT LEVEL

(INSTALLED)  
(18) 1-5/8 TO 208 FT LEVEL (VERIZON WIRELESS)

(INSTALLED)  
(6) 1-5/8 TO 183 FT LEVEL

(INSTALLED)  
(12) 1-1/4 TO 175 FT LEVEL

LEG A

LEG B

LEG C

BUSINESS UNIT: 600383 TOWER ID: C\_BASELEVEL

BASE LEVEL DRAWING

SCALE: N.T.S.

1

A1-0

CROWN REGION ADDRESS  
USA

DRAWN BY: SJK  
CHECKED BY:  
DRAWING DATE: 150607

SITE NUMBER:  
SITE NAME:  
SITE ADDRESS:  
BUSINESS UNIT NUMBER:  
SHEET TITLE:  
BASE LEVEL  
SHEET NUMBER

HRT 106 943201  
806583  
61 COW HILL ROAD  
CLINTON, CT 06413  
MIDDLESEX COUNTY  
USA

15/07/11  
18/07/11  
21/07/11  
04/08/11  
11/08/11  
11/08/11  
14/08/11  
15/08/11  
18/08/11  
21/08/11  
24/08/11  
28/08/11  
31/08/11

**APPENDIX C**  
**ADDITIONAL CALCULATIONS**



## ANCHOR BOLT CALCULATIONS

**Customer:** Crown Castle  
**Site Name:** HRT 105, CT BU#806363  
**Job Number:** 2013-004-035  
**Tower Model:** 212' Rohn SSMW Self-Supporting Tower  
**Date:** 4/23/2013

### Input Information:

# Bolts	10	
Bolt Diameter	1	in
Allowable Tension, $F_u$	125	ksi
Steel Grade	A354-BC	
Applied Shear	32.713	kips
Uplift per Leg	268.255	kips

---

Bolt Cross-Sectional Area, $A$	0.785	in <sup>2</sup>
Applied Shear, $f_v$	4.17	ksi
Maximum Allowable Tensile Stress, $F_t$	41.25	ksi
Allowable Tension Force	323.98	kips
Maximum Allowable	431.86	kips
<b>% Capacity</b>	<b>62.1%</b>	

**The Bolt Group is sufficient for the applied Uplift Force**

Maximum Allowable Tensile Stress,  $F_t$

$$0.43F_u - 1.8f_v \leq 0.33F_u$$

This equation is for threaded parts, A449 bolts over 1 1/2" dia. (threads included in shear plane) Manual of Steel Construction ASD, 9th Edition, pg. 5-74, Table J3.3



## Overturning Calculation for Square Mat Foundations

**Customer:** Crown Castle  
**Site Name:** HRT 105, CT BU#806363  
**Job Number:** 2013-004-035  
**Tower Model:** 212' Rohn SSMW Self-Supporting Tower  
**Date:** 4/23/2013

Soil Ultimate Bearing	8	ksf
Unit wt soil	0.1	kcf
Unit wt concrete	0.15	kcf

Mat Width	40.25	ft
Mat Thickness	4.5	ft
Depth of Soil Over Mat	0	ft
Has Pedestals? (Y or N)	N	
Pedestal Round or Square? (R or S)	R	
Number of Pedestals	0	
Pedestal Height	0	ft
Pedestal Diameter or Width	0	ft

Applied Shear	60.76	kip
Applied Axial Force	59.97	kip
Applied Moment	7624.558	k-ft

wt. Concrete =	1093.542	kip
wt. Soil =	0.000	kip
x =	3.582	ft
Shear Moment =	273.42	k-ft

Allowable Bearing =	4	ksf
Mat Width / 6 =	6.71	ft
e =	6.85	ft
L =	39.83	ft
Bearing =	0.99	ksf
Resisting Moment =	21148.2987	k-ft
SF =	2.678	

BEARING ADEQUATE

OVERTURNING ADEQUATE