



QC Development

PO Box 916

Storrs, CT 06268

860-670-9068

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March 2, 2018

Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Notice of Exempt Modification – New Cingular Wireless PCS, LLC (AT&T) – CT2024
48 Cow Hill Road, Clinton, CT 06413
N 41-17-19
W 72-32-21

Dear Ms. Bachman:

AT&T currently maintains nine (9) antennas at the 190-foot level of the existing 212-foot Self-Support Tower at 48 Cow Hill Road, Clinton, CT. The tower and property are owned by Crown Castle. AT&T now intends to add three (3) Andrew antennas and three (3) Ericsson Remote Radio Units (RRUS-32 B66). These antennas and RRUs would be installed at the 190-foot level of the tower.

This facility was approved by the Connecticut Siting Council, Docket No. 148 on May 5, 1992. There were no conditions that could feasibly be violated by this modification, including total facility height or mounting restrictions. This modification therefore complies with the aforementioned approval.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Christine Goupil, First Selectman of the Town of Clinton, as elected official, to Clinton's Zoning Enforcement Officer and to the property and tower owner.

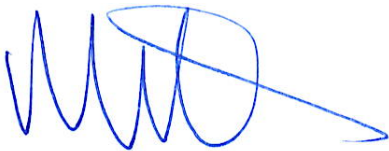
The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing structure.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, AT&T respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitute an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Please feel free to call me at (860) 670-9068 with any questions regarding this matter. Thank you for your consideration.

Sincerely,

A handwritten signature in blue ink, appearing to read 'MR', with a long horizontal stroke extending to the right.

Mark Roberts
QC Development
Consultant for AT&T

Attachments

cc: Christine Goupil - as Elected Official
Eric Knapp- ZEO
Crown Castle- Tower and Property Owner

Power Density

Existing Loading on Tower

Carrier	# of Channels	ERP/Ch (W)	Antenna Centerline Height (ft)	Power Density (mW/cm ²)	Freq. Band (MHz ^{**})	Limit S (mW/cm ²)	%MPE
Other Carriers*							9.06%
AT&T GSM	1	141	190	0.0015	880	0.5867	0.03%
AT&T UMTS	1	259	190	0.0028	880	0.5867	0.05%
AT&T UMTS	2	344	190	0.0037	1900	1.0000	0.04%
AT&T LTE	2	793	190	0.0168	740	0.4933	0.34%
AT&T LTE	2	1734	190	0.0368	1900	1.0000	0.37%
AT&T LTE	2	1094	190	0.0232	2300	1.0000	0.23%
Site Total							10.11%

*Per CSC Records (available upon request, includes calculation formulas)

** If a range of frequencies are used, such as 880-894, enter the lowest value, i.e. 880

Proposed Loading on Tower

Carrier	# of Channels	ERP/Ch (W)	Antenna Centerline Height (ft)	Power Density (mW/cm ²)	Freq. Band (MHz ^{**})	Limit S (mW/cm ²)	%MPE
Other Carriers*							9.06%
AT&T UMTS	1	344	190	0.0037	880	0.5867	0.06%
AT&T LTE	1	793	190	0.0084	740	0.4933	0.17%
AT&T LTE	2	1734	190	0.0368	1900	1.0000	0.37%
AT&T LTE	1	3837	190	0.0408	2100	1.0000	0.41%
AT&T LTE	1	1094	190	0.0116	2300	1.0000	0.12%
Site Total							10.18%

*Per CSC Records (available upon request, includes calculation formulas)

** If a range of frequencies are used, such as 880-894, enter the lowest value, i.e. 880

Note: Proposed Loading may also include corrections to certain Existing Loading values

PROJECT INFORMATION

SCOPE OF WORK: ITEMS TO BE MOUNTED ON LATTICE TOWER:
 • NEW AT&T RRUS: RRUS-32 B66 (TYP. OF 1 PER SECTOR, TOTAL OF 3)
 • NEW AT&T ANTENNA: SBNHH-1D65A (TYP. OF 1 PER SECTOR, TOTAL OF 3)
 • COAX JUMPERS (1) PER SECTOR, FROM EACH RRU (TOTAL OF 3).
 • FIBER JUMPERS (2) PER SECTOR, FROM THE SQUID TO EACH RRU (TOTAL OF 6).

ITEMS TO REMAIN:
 • (9) ANTENNAS, (6) RRU'S, (2) SURGE ARRESTORS, (4) DC POWER CABLES, & (2) FIBER RUN.

SITE ADDRESS: 49 COW HILL ROAD
CLINTON, CT 06413

LATITUDE: 41.2889361° N 41° 17' 20.16" N
 LONGITUDE: 72.5384711° W 72° 32' 18.49" W

TYPE OF SITE: LATTICE / INDOOR EQUIPMENT

TOWER HEIGHT: 212'-0"±

RAD CENTER: 190'-0"±

JURISDICTION: NATIONAL, STATE & LOCAL CODES OR ORDINANCES

CURRENT USE: TELECOMMUNICATIONS FACILITY

PROPOSED USE: TELECOMMUNICATIONS FACILITY



SITE NUMBER: CT2024

SITE NAME: CLINTON-COW HILL ROAD

PROJECT: LTE 4C 2018 UPGRADE

VICINITY MAP

DIRECTIONS TO SITE:
 I-95 TO EXIT 63 TO RT 81 NORTH. JUST PAST HIGH SCHOOL, TURN LEFT ONTO WALNUT HILL ROAD. CONTINUE TO TOP OF HILL. APROX. .4 MILES TURN LEFT (ALMOST 180) ONTO COW HILL ROAD AND CONTINUE .3 MILES TO I-95 INDUSTRIAL PARK ON RIGHT NEXT TO HWY. SITE LOCATED IN REAR.

GENERAL NOTES

1. THIS DOCUMENT IS THE CREATION, DESIGN, PROPERTY AND COPYRIGHTED WORK OF AT&T. ANY DUPLICATION OR USE WITHOUT EXPRESS WRITTEN CONSENT IS STRICTLY PROHIBITED. DUPLICATION AND USE BY GOVERNMENT AGENCIES FOR THE PURPOSES OF CONDUCTING THEIR LAWFULLY AUTHORIZED REGULATORY AND ADMINISTRATIVE FUNCTIONS IS SPECIFICALLY ALLOWED.
2. THE FACILITY IS AN UNMANNED PRIVATE AND SECURED EQUIPMENT INSTALLATION. IT IS ONLY ACCESSED BY TRAINED TECHNICIANS FOR PERIODIC ROUTINE MAINTENANCE AND THEREFORE DOES NOT REQUIRE ANY WATER OR SANITARY SEWER SERVICE. THE FACILITY IS NOT GOVERNED BY REGULATIONS REQUIRING PUBLIC ACCESS PER ADA REQUIREMENTS.
3. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE AT&T MOBILITY REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.
4. CONSTRUCTION DRAWINGS ARE VALID FOR SIX MONTHS AFTER ENGINEER OF RECORD'S STAMPED AND SIGNED SUBMITTAL DATE LISTED HEREIN.

DRAWING INDEX

SHEET NO.	DESCRIPTION	REV.
T-1	TITLE SHEET	B
GN-1	GENERAL NOTES	B
A-1	COMPOUND & EQUIPMENT PLAN	B
A-2	ELEVATION & ANTENNA LAYOUTS	B
A-3	DETAILS	B
RF-1	RF-PLUMBING DIAGRAM	B
G-1	GROUNDING DETAILS	B



72 HOURS



CALL BEFORE YOU DIG
 CALL TOLL FREE 1-800-922-4455
 OR CALL 811

UNDERGROUND SERVICE ALERT

CCI SITE #: 806363
CCI SITE NAME: HRT105

HGD HUDSON Design Group LLC
 45 BEECHWOOD DRIVE NORTH ANDOVER, MA 01845
 TEL: (978) 557-5553 FAX: (978) 336-5586

SAI
 12 INDUSTRIAL WAY
 SALEM, NH 03079

SITE NUMBER: CT2024
SITE NAME: CLINTON-COW HILL ROAD
CCI SITE NUMBER: 806363
 49 COW HILL ROAD
 CLINTON, CT 06413
 MIDDLESEX COUNTY

at&t
 500 ENTERPRISE DRIVE, SUITE 3A
 ROCKY HILL, CT 06067

B 02/28/18 ISSUED FOR PERMITTING		MR	AT	DPH		AT&T		
A 02/05/18 ISSUED FOR REVIEW		TB	AT	DPH		TITLE SHEET LTE 4C 2018 UPGRADE		
NO.	DATE	REVISIONS	BY	CHK	APP'D	SITE NUMBER	DRAWING NUMBER	REV
SCALE: AS SHOWN		DESIGNED BY: AT	DRAWN BY: LL			CT2024	T-1	B

GROUNDING NOTES

1. THE SUBCONTRACTOR SHALL REVIEW AND INSPECT THE EXISTING FACILITY GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRICT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ), THE SITE-SPECIFIC (UL, LPI, OR NFPA) LIGHTING PROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND TIA GROUNDING STANDARDS. THE SUBCONTRACTOR SHALL REPORT ANY VIOLATIONS OR ADVERSE FINDINGS TO THE CONTRACTOR FOR RESOLUTION.
2. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GES'S) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
3. THE SUBCONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR NEW GROUND ELECTRODE SYSTEMS. THE SUBCONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
4. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
5. EACH BTS CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, 6 AWG STRANDED COPPER OR LARGER FOR INDOOR BTS 2 AWG STRANDED COPPER FOR OUTDOOR BTS.
6. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
7. APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
8. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO GROUND BAR.
9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
10. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
11. METAL CONDUIT SHALL BE MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH 6 AWS COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
12. ALL NEW STRUCTURES WITH A FOUNDATION AND/OR FOOTING HAVING 20 FT. OR MORE OF 1/2 IN. OR GREATER ELECTRICALLY CONDUCTIVE REINFORCING STEEL MUST HAVE IT BONDED TO THE GROUND RING USING AN EXOTHERMIC WELD CONNECTION USING #2 AWG SOLID BARE TINNED COPPER GROUND WIRE, PER NEC 250.50

GENERAL NOTES

1. FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:
 CONTRACTOR – SAI
 SUBCONTRACTOR – GENERAL CONTRACTOR (CONSTRUCTION)
 OWNER – AT&T MOBILITY
2. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING SUBCONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CONTRACTOR.
3. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES. SUBCONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
4. DRAWINGS PROVIDED HERE ARE NOT TO BE SCALED AND ARE INTENDED TO SHOW OUTLINE ONLY.
5. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
6. "KITTING LIST" SUPPLIED WITH THE BID PACKAGE IDENTIFIES ITEMS THAT WILL BE SUPPLIED BY CONTRACTOR. ITEMS NOT INCLUDED IN THE BILL OF MATERIALS AND KITTING LIST SHALL BE SUPPLIED BY THE SUBCONTRACTOR.
7. THE SUBCONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
8. IF THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE SUBCONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION SPACE FOR APPROVAL BY THE CONTRACTOR.
9. SUBCONTRACTOR SHALL DETERMINE ACTUAL ROUTING OF CONDUIT, POWER AND T1 CABLES, GROUNDING CABLES AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING. SUBCONTRACTOR SHALL UTILIZE EXISTING TRAYS AND/OR SHALL ADD NEW TRAYS AS NECESSARY. SUBCONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH THE CONTRACTOR.
10. THE SUBCONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT SUBCONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.
11. SUBCONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
12. SUBCONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION.
13. ALL CONCRETE REPAIR WORK SHALL BE DONE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301.

14. ANY NEW CONCRETE NEEDED FOR THE CONSTRUCTION SHALL BE AIR-ENTRAINED AND SHALL HAVE 4000 PSI STRENGTH AT 28 DAYS. ALL CONCRETE WORK SHALL BE DONE IN ACCORDANCE WITH ACI 318 CODE REQUIREMENTS.
15. ALL STRUCTURAL STEEL WORK SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH AISC SPECIFICATIONS. ALL STRUCTURAL STEEL SHALL BE ASTM A36 (Fy = 36 ksi) UNLESS OTHERWISE NOTED. PIPES SHALL BE ASTM A53 TYPE E (Fy = 36 ksi). ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED. TOUCHUP ALL SCRATCHES AND OTHER MARKS IN THE FIELD AFTER STEEL IS ERECTED USING A COMPATIBLE ZINC RICH PAINT.
16. CONSTRUCTION SHALL COMPLY WITH LTE SPECIFICATIONS AND "GENERAL CONSTRUCTION SERVICES FOR CONSTRUCTION OF AT&T SITES."
17. SUBCONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS MUST BE VERIFIED. SUBCONTRACTOR SHALL NOTIFY THE CONTRACTOR OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH CONSTRUCTION.
18. THE EXISTING CELL SITE IS IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION WORK BY SUBCONTRACTOR SHALL NOT DISRUPT THE EXISTING NORMAL OPERATION. ANY WORK ON EXISTING EQUIPMENT MUST BE COORDINATED WITH CONTRACTOR. ALSO, WORK SHOULD BE SCHEDULED FOR AN APPROPRIATE MAINTENANCE WINDOW USUALLY IN LOW TRAFFIC PERIODS AFTER MIDNIGHT.
19. SINCE THE CELL SITE IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE ADVISED TO BE WORN TO ALERT OF ANY DANGEROUS EXPOSURE LEVELS.
20. APPLICABLE BUILDING CODES:
 SUBCONTRACTOR'S WORK SHALL COMPLY WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AS ADOPTED BY THE LOCAL AUTHORITY HAVING JURISDICTION (AHJ) FOR THE LOCATION. THE EDITION OF THE AHJ ADOPTED CODES AND STANDARDS IN EFFECT ON THE DATE OF CONTRACT AWARD SHALL GOVERN THE DESIGN.
 BUILDING CODE: IBC 2012 WITH 2016 CT BUILDING CODE AMENDMENTS
 ELECTRICAL CODE: REFER TO ELECTRICAL DRAWINGS
 LIGHTENING CODE: REFER TO ELECTRICAL DRAWINGS

 SUBCONTRACTOR'S WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING STANDARDS:

 AMERICAN CONCRETE INSTITUTE (ACI) 318; BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE;

 AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC)
 MANUAL OF STEEL CONSTRUCTION, ASD, FOURTEENTH EDITION;

 TELECOMMUNICATIONS INDUSTRY ASSOCIATION (TIA) 222-G,
 STRUCTURAL STANDARDS FOR STEEL

 EQUIPMENT AND ANTENNA SUPPORTING STRUCTURES; REFER TO ELECTRICAL DRAWINGS FOR SPECIFIC ELECTRICAL STANDARDS.

 FOR ANY CONFLICTS BETWEEN SECTIONS OF LISTED CODES AND STANDARDS REGARDING MATERIAL, METHODS OF CONSTRUCTION, OR OTHER REQUIREMENTS, THE MOST RESTRICTIVE REQUIREMENT SHALL GOVERN. WHERE THERE IS CONFLICT BETWEEN A GENERAL REQUIREMENT AND A SPECIFIC REQUIREMENT, THE SPECIFIC REQUIREMENT SHALL GOVERN.

ABBREVIATIONS

AGL	ABOVE GRADE LEVEL	EQ	EQUAL	REQ	REQUIRED
AWG	AMERICAN WIRE GAUGE	GC	GENERAL CONTRACTOR	RF	RADIO FREQUENCY
BBU	BATTERY BACKUP UNIT	GRC	GALVANIZED RIGID CONDUIT	TBD	TO BE DETERMINED
BTCW	BARE TINNED SOLID COPPER WIRE	MGB	MASTER GROUND BAR	TBR	TO BE REMOVED
BGR	BURIED GROUND RING	MIN	MINIMUM	TBRR	TO BE REMOVED AND REPLACED
BTS	BASE TRANSCEIVER STATION	P	PROPOSED	TYP	TYPICAL
E	EXISTING	NTS	NOT TO SCALE	UG	UNDER GROUND
EGB	EQUIPMENT GROUND BAR	RAD	RADIATION CENTER LINE (ANTENNA)	VIF	VERIFY IN FIELD
EGR	EQUIPMENT GROUND RING	REF	REFERENCE		

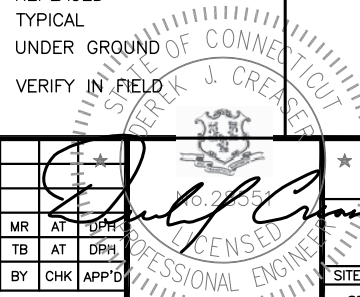
HGD HUDSON Design Group LLC
 45 BEECHWOOD DRIVE NORTH ANDOVER, MA 01845
 TEL: (978) 557-5553 FAX: (978) 336-5586

SAI
 12 INDUSTRIAL WAY SALEM, NH 03079

SITE NUMBER: CT2024
SITE NAME: CLINTON-COW HILL ROAD
CCI SITE NUMBER: 806363
49 COW HILL ROAD
CLINTON, CT 06413
MIDDLESEX COUNTY

at&t
 500 ENTERPRISE DRIVE, SUITE 3A
 ROCKY HILL, CT 06067

NO.	DATE	REVISIONS	BY	CHK	APP'D
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A	02/05/18	ISSUED FOR REVIEW	TB	AT	DPH
SCALE: AS SHOWN		DESIGNED BY: AT	DRAWN BY: LL		

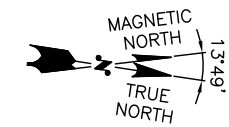
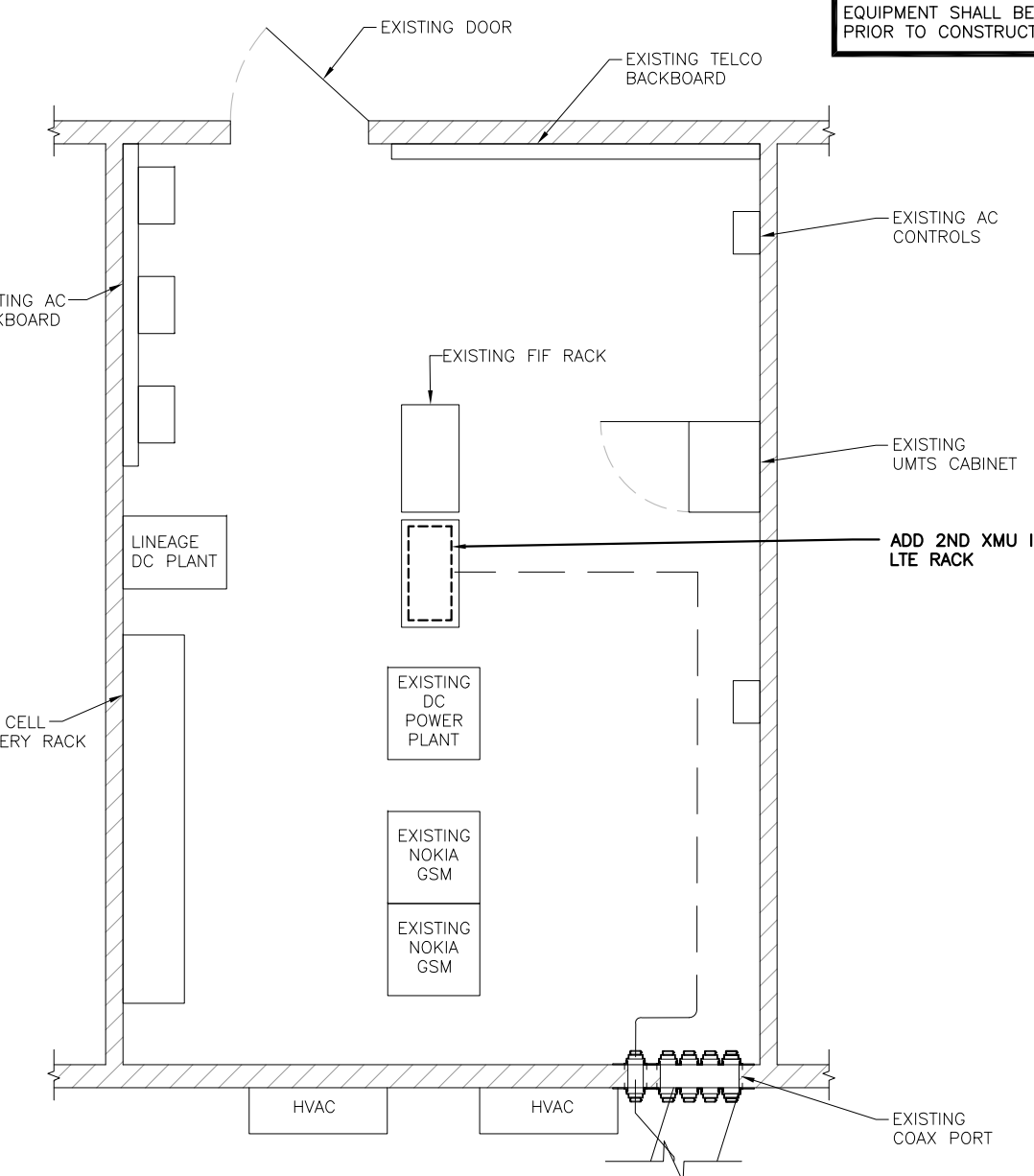
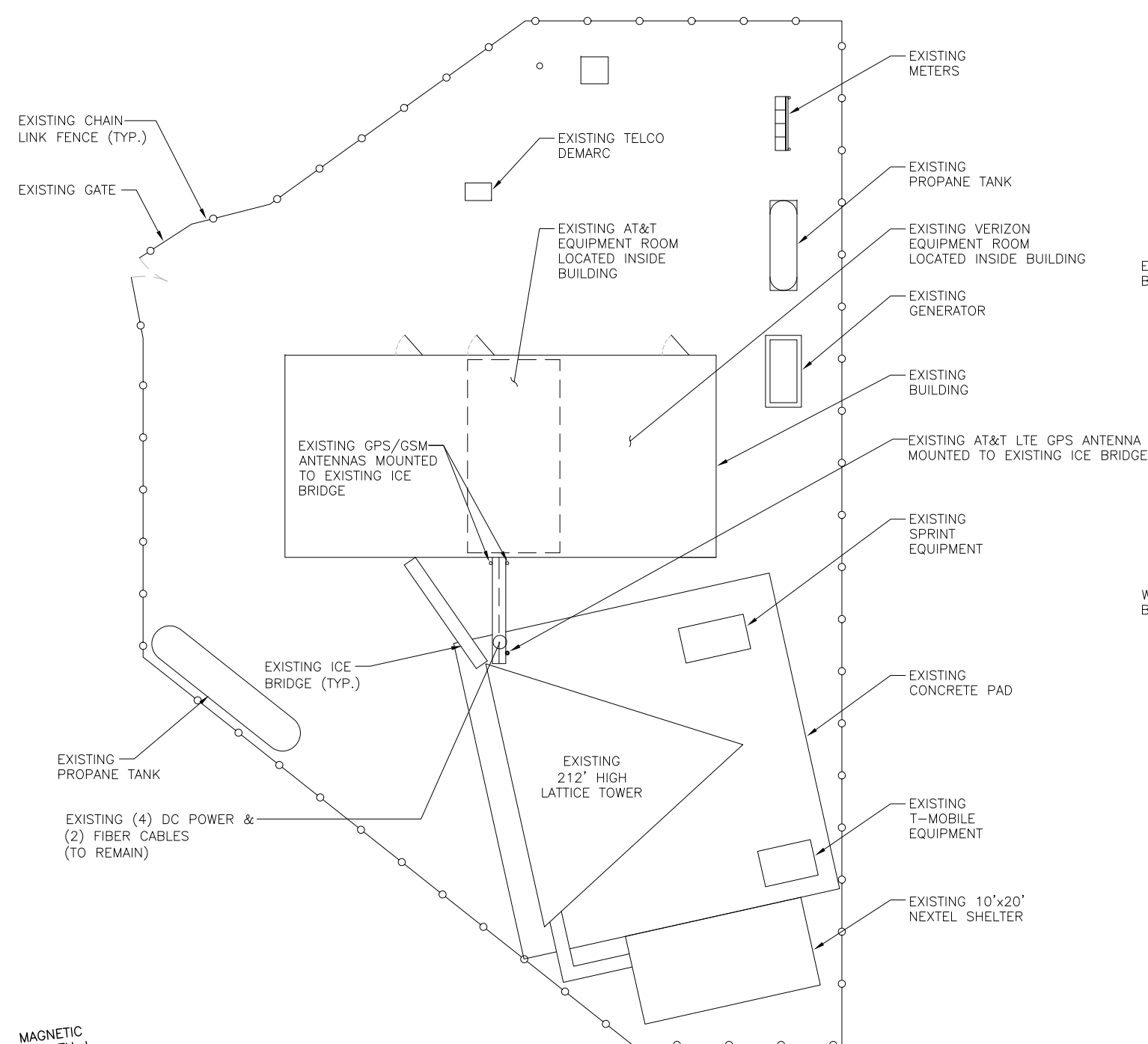


AT&T
GENERAL NOTES
LTE 4C 2018 UPGRADE
 SITE NUMBER: **CT2024**
 DRAWING NUMBER: **GN-1**
 REV: **B**

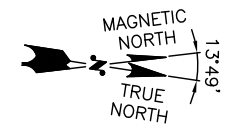
NOTE:
REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.

NOTE:
ALL ANTENNAS AND LINES TO BE INSTALLED IN ACCORDANCE WITH STRUCTURAL ANALYSIS PROVIDED BY CROWN CASTLE AND FINAL AT&T RF DATA SHEET.

NOTE:
AN ANALYSIS FOR THE CAPACITY OF THE EXISTING MOUNT TO SUPPORT THE PROPOSED EQUIPMENT SHALL BE DETERMINED PRIOR TO CONSTRUCTION.



COMPOUND PLAN
22x34 SCALE: 1/8"=1'-0"
11x17 SCALE: 1/16"=1'-0"
1
A-1



EQUIPMENT PLAN
22x34 SCALE: 1/2"=1'-0"
11x17 SCALE: 1/4"=1'-0"
2
A-1

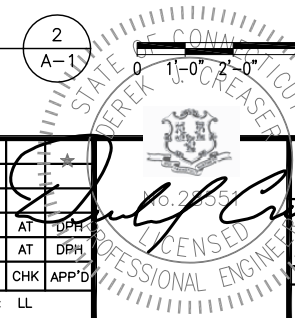
HGD HUDSON
Design Group LLC
45 BEECHWOOD DRIVE
NORTH ANDOVER, MA 01845
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SAI
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SALEM, NH 03079

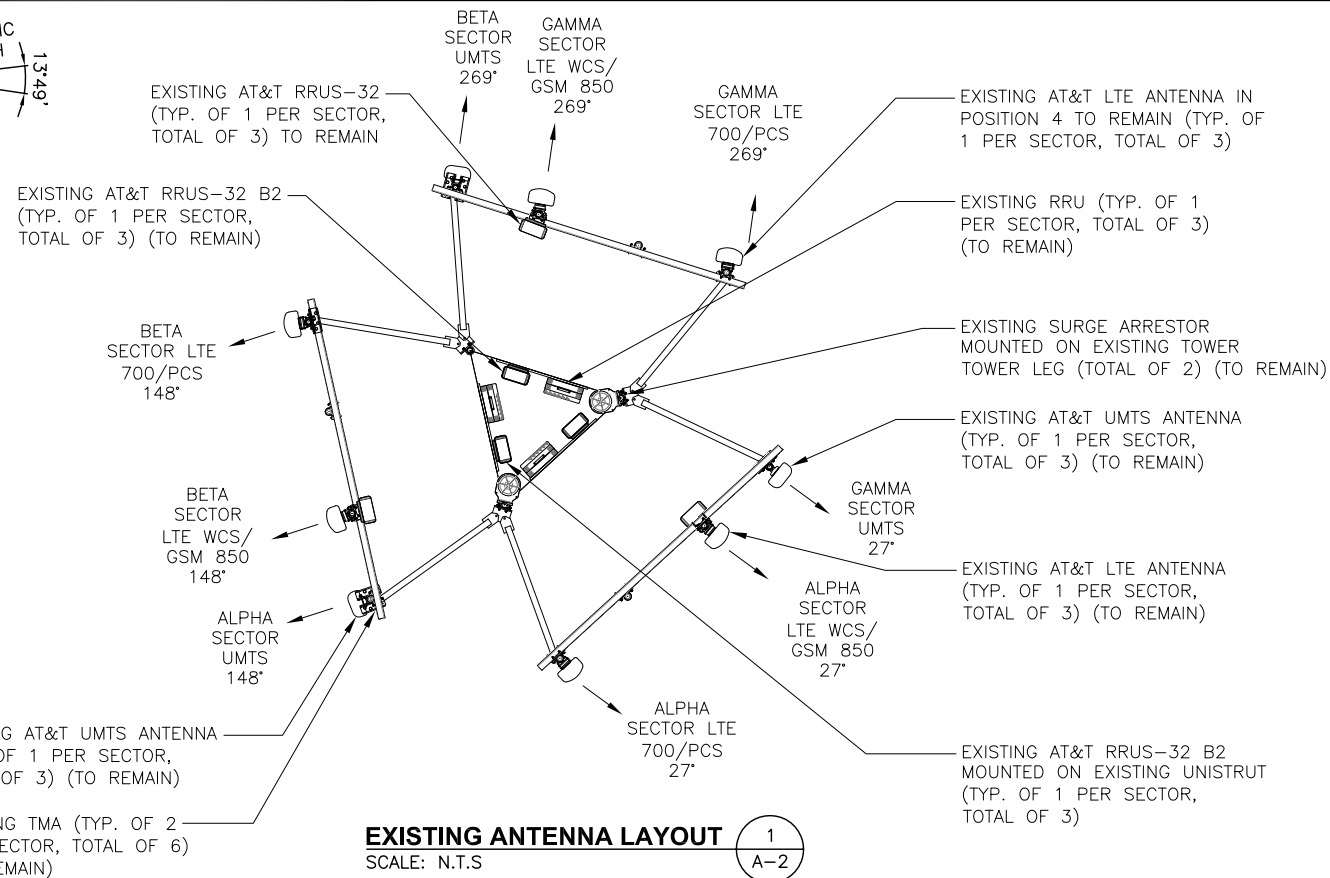
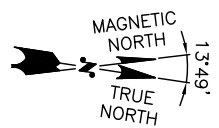
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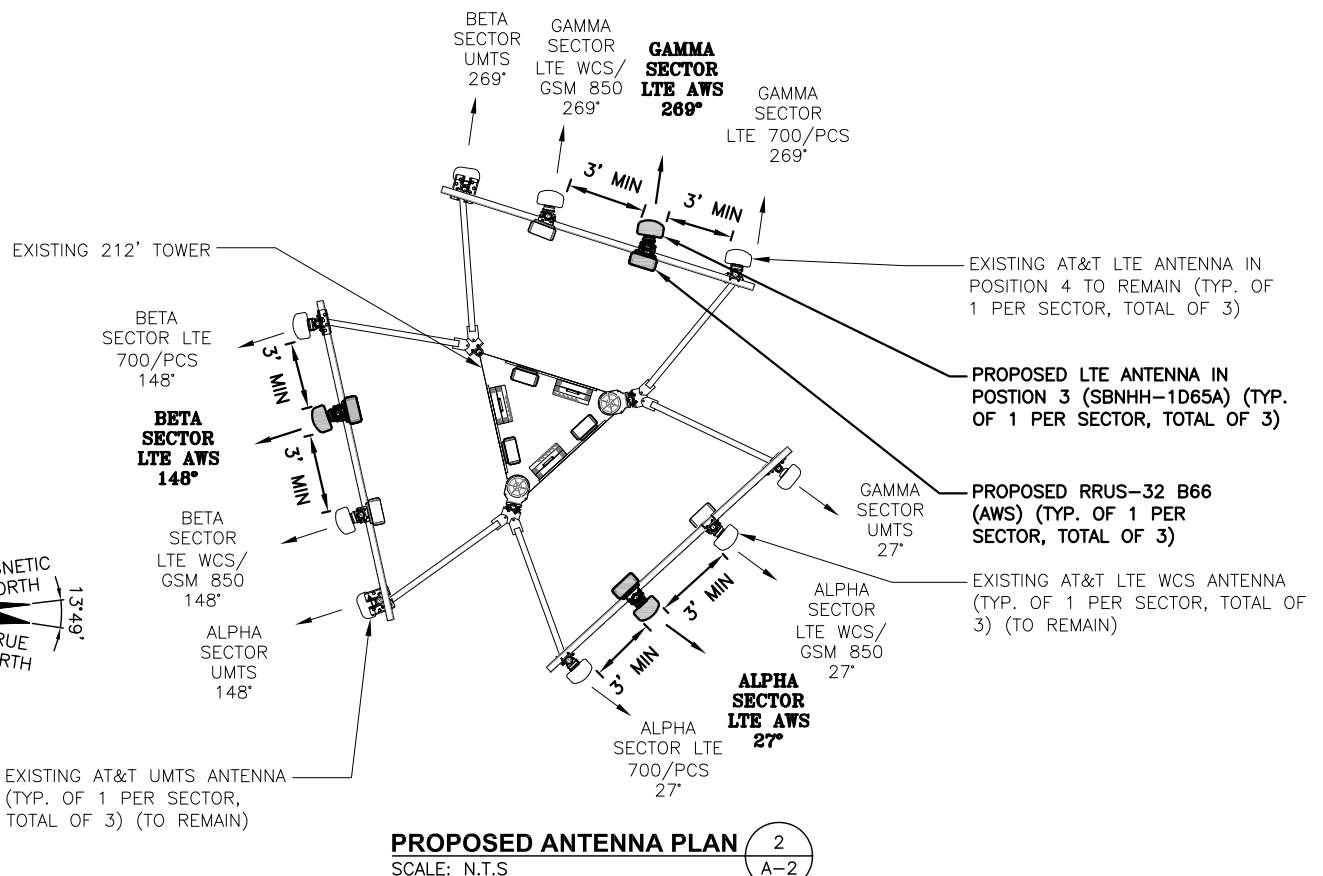
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AT&T
COMPOUND & EQUIPMENT PLAN
LTE 4C 2018 UPGRADE
SITE NUMBER: CT2024
DRAWING NUMBER: A-1
REV: B



EXISTING ANTENNA LAYOUT 1
SCALE: N.T.S. A-2

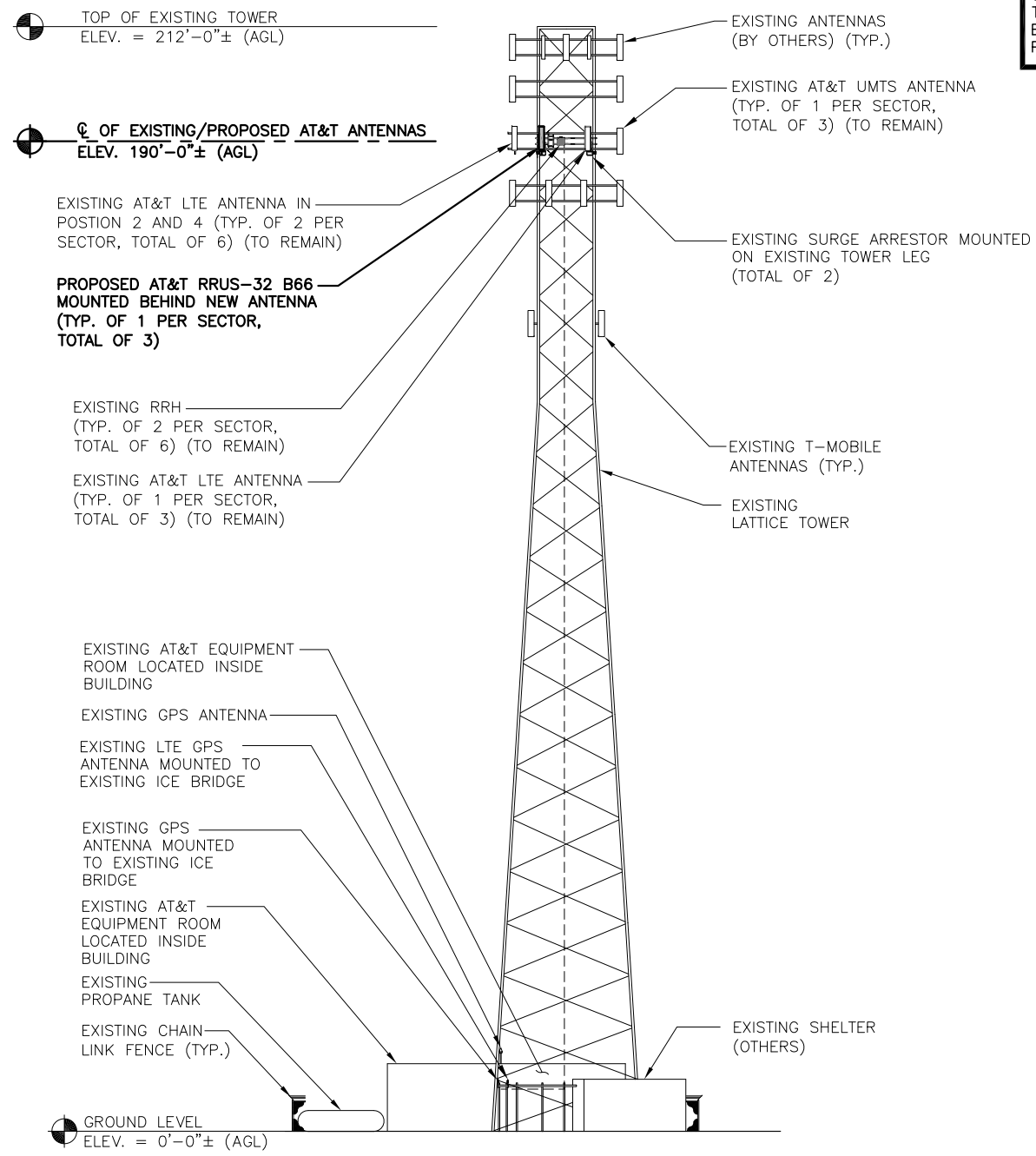


PROPOSED ANTENNA PLAN 2
SCALE: N.T.S. A-2

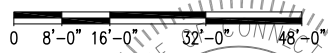
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AN ANALYSIS FOR THE CAPACITY OF THE EXISTING MOUNT TO SUPPORT THE PROPOSED EQUIPMENT SHALL BE DETERMINED PRIOR TO CONSTRUCTION.



ELEVATION 3
22x34 SCALE: 1/16"=1'-0"
11x17 SCALE: 1/32"=1'-0" A-2



HDG HUDSON Design Group LLC
45 BEECHWOOD DRIVE
NORTH ANDOVER, MA 01845
TEL: (978) 557-5553
FAX: (978) 336-5586

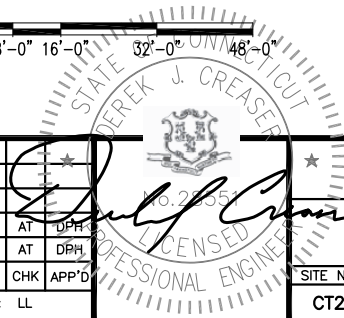
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ROCKY HILL, CT 06067

NO.	DATE	REVISIONS	BY	CHK	APP'D
B	02/28/18	ISSUED FOR PERMITTING	MR	AT	DPH
A	02/05/18	ISSUED FOR REVIEW	TB	AT	DPH

SCALE: AS SHOWN DESIGNED BY: AT DRAWN BY: LL



AT&T
ANTENNA LAYOUTS & ELEVATION
LTE 4C 2018 UPGRADE
SITE NUMBER: CT2024 DRAWING NUMBER: A-2 REV: B

NOTE:
REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.

NOTE:
ALL ANTENNAS AND LINES TO BE INSTALLED IN ACCORDANCE WITH STRUCTURAL ANALYSIS PROVIDED BY CROWN CASTLE AND FINAL AT&T RF DATA SHEET.

NOTE:
AN ANALYSIS FOR THE CAPACITY OF THE EXISTING MOUNT TO SUPPORT THE PROPOSED EQUIPMENT SHALL BE DETERMINED PRIOR TO CONSTRUCTION.

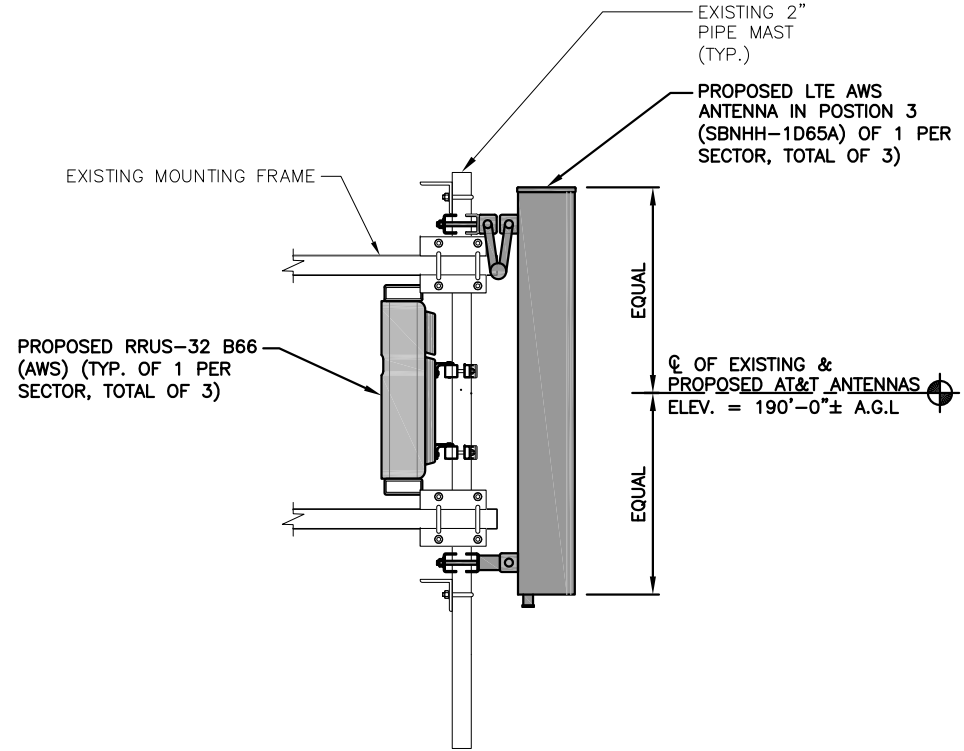
FINAL ANTENNA SCHEDULE													
SECTOR	BAND	ANTENNA	SIZE (INCHES) (L X W X D)	RAD CENTER	AZIMUTH	TMA'S	RRU'S	SIZE (INCHES) (L X W X D)	COAX JUMPERS	FIBER JUMPERS	COAX		
ALPHA	UMTS 850	EXISTING	7770	55X11X5	190'-0"±	148'	EXISTING (2) 1900W800	-	-	-	-	-	(2) 1-5/8"
	LTE WCS	EXISTING	SBNHH-1D65A	55X11.9X7.1	190'-0"±	27'	-	EXISTING	RRUS-32 (WCS)	-	-	-	(2) 1-5/8"
	LTE AWS	PROPOSED	SBNHH-1D65A	55X11.9X7.1	190'-0"±	27'	-	PROPOSED	RRUS-32 B66	27.2X12.1X7.0	1*	2*	-
	LTE 700/PCS	EXISTING	SBNHH-1D65A	55X11.9X7.1	190'-0"±	27'	-	EXISTING EXISTING	RRUS-32 B2 (PCS) RRUS-11 (700)	-	-	-	-
BETA	UMTS 850	EXISTING	7770	55X11X5	190'-0"±	269'	EXISTING (2) 1900W800	-	-	-	-	-	(2) 1-5/8"
	LTE WCS	EXISTING	SBNHH-1D65A	55X11.9X7.1	190'-0"±	148'	-	EXISTING	RRUS-32 (WCS)	-	-	-	(2) 1-5/8"
	LTE AWS	PROPOSED	SBNHH-1D65A	55X11.9X7.1	190'-0"±	148'	-	PROPOSED	RRUS-32 B66	27.2X12.1X7.0	1*	2*	-
	LTE 700/PCS	EXISTING	SBNHH-1D65A	55X11.9X7.1	190'-0"±	148'	-	EXISTING EXISTING	RRUS-32 B2 (PCS) RRUS-11 (700)	-	-	-	-
GAMMA	UMTS 850	EXISTING	7770	55X11X5	190'-0"±	27'	EXISTING (2) 1900W800	-	-	-	-	-	(2) 1-5/8"
	LTE WCS	EXISTING	SBNHH-1D65A	55X11.9X7.1	190'-0"±	269'	-	EXISTING	RRUS-32 (WCS)	-	-	-	(2) 1-5/8"
	LTE AWS	PROPOSED	SBNHH-1D65A	55X11.9X7.1	190'-0"±	269'	-	PROPOSED	RRUS-32 B66	27.2X12.1X7.0	1*	2*	-
	LTE 700/PCS	EXISTING	SBNHH-1D65A	55X11.9X7.1	190'-0"±	269'	-	EXISTING EXISTING	RRUS-32 B2 (PCS) RRUS-11 (700)	-	-	-	-

FINAL ANTENNA CONFIGURATION
SCALE: N.T.S

3
A-3

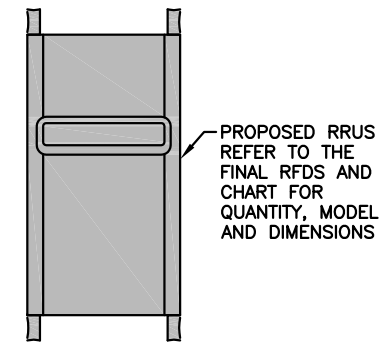
***COAX JUMPER NOTE:**
COAX JUMPERS (1) PER SECTOR, FROM EACH RRU (TOTAL OF 3).

****FIBER JUMPER NOTE:**
FIBER JUMPERS (2) PER SECTOR, FROM THE SQUID TO EACH RRU (TOTAL OF 6).

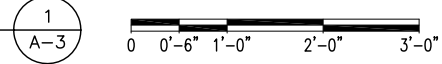


RRU CHART				
QUANTITY	MODEL	L	W	D
3(E)	RRUS-11	19.7"	17.0"	7.2"
6(E)	RRUS-32	27.2"	12.1"	7.0"
3(P)	RRUS-32	27.2"	12.1"	7.0"
-	RRUS-E2	20.4"	18.5"	7.5"
-	LTE-A2	16.4"	15.2"	3.4"

NOTE:
MOUNT PER MANUFACTURER'S SPECIFICATIONS

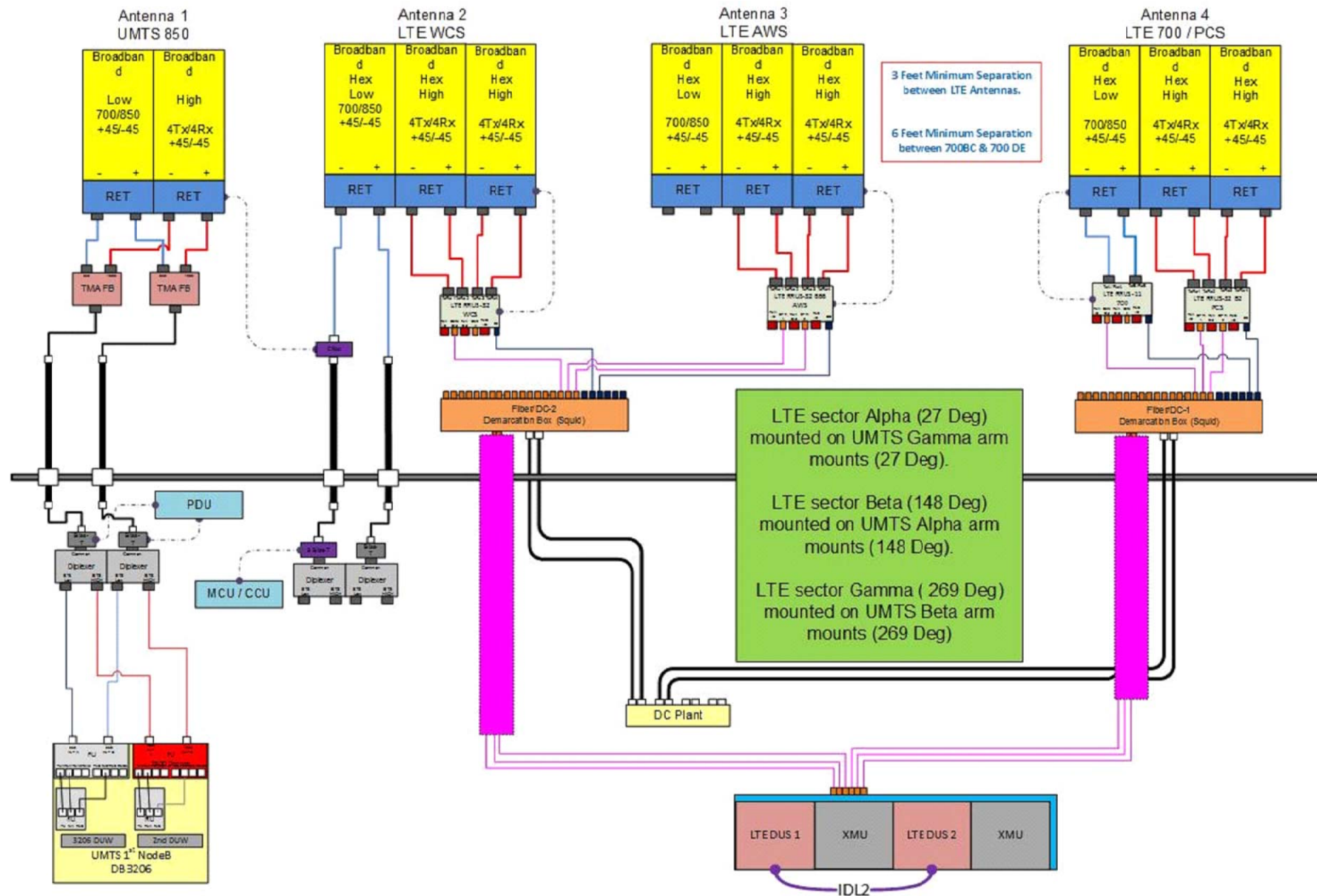


PROPOSED ANTENNA & RRU MOUNTING DETAIL
22x34 SCALE: 1"=1'-0"
11x17 SCALE: 1/2"=1'-0"



NOTE:
MOUNT PER MANUFACTURER'S SPECIFICATIONS.

RRUS DETAIL
SCALE: N.T.S



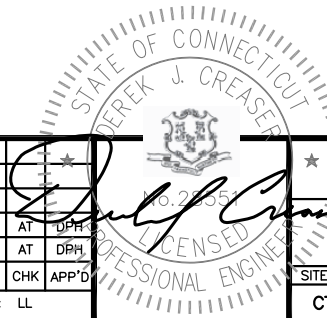
3 Feet Minimum Separation
between LTE Antennas.
6 Feet Minimum Separation
between 700BC & 700 DE

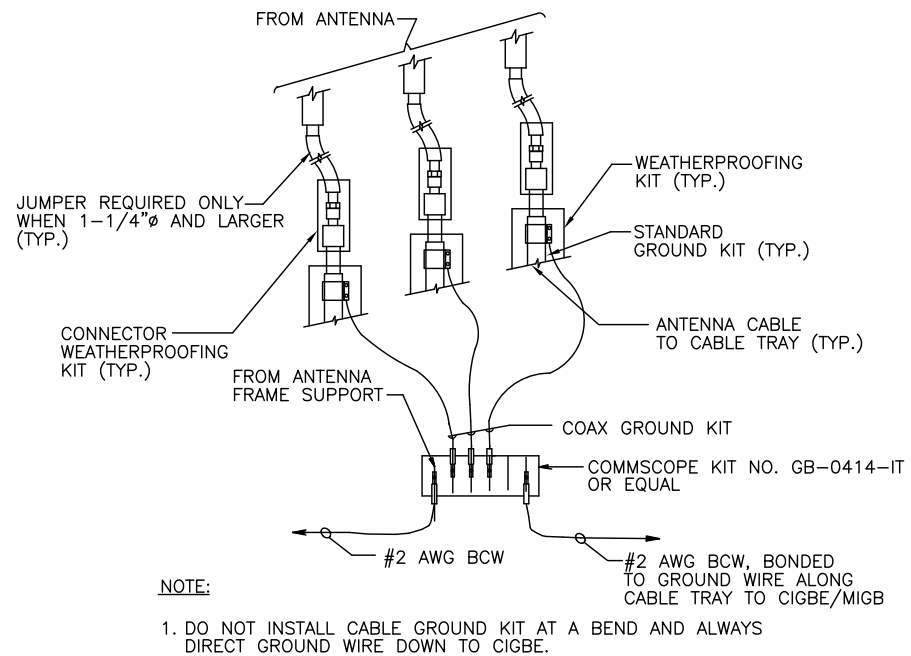
RF PLUMBING DIAGRAM
SCALE: N.T.S.

1
RF-1

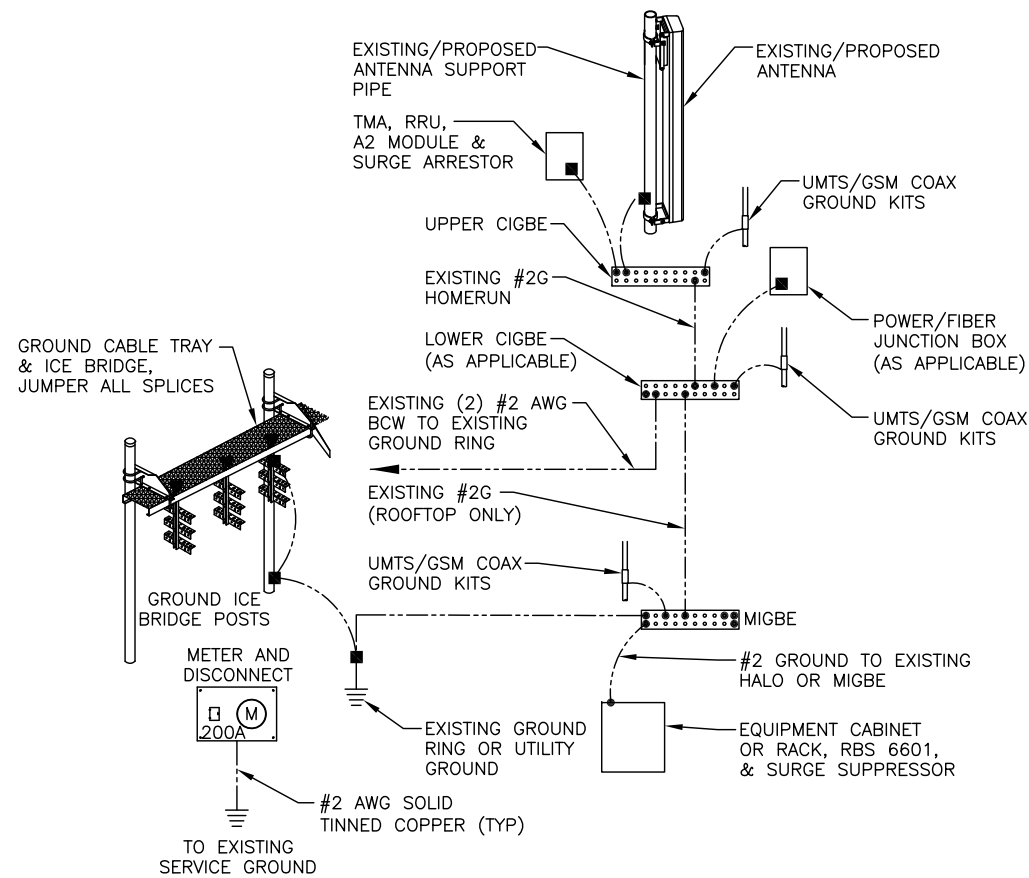
NOTE:
REFER TO THE FINAL RF DATA SHEET
FOR FINAL ANTENNA SETTINGS.

NOTE:
1. CONTRACTOR TO CONFIRM ALL PARTS.
2. INSTALL ALL EQUIPMENT TO
MANUFACTURER'S RECOMMENDATIONS

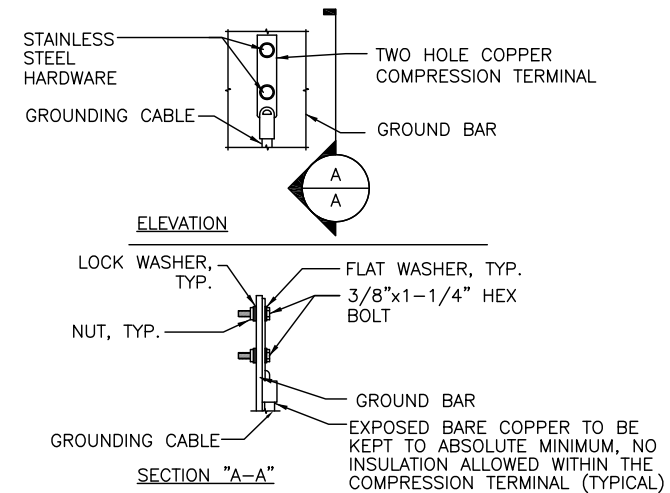




GROUND WIRE TO GROUND BAR CONNECTION DETAIL 1
SCALE: N.T.S. G-1



GROUNDING RISER DIAGRAM 2
SCALE: N.T.S. G-1



TYPICAL GROUND BAR CONNECTION DETAIL 3
SCALE: N.T.S. G-1

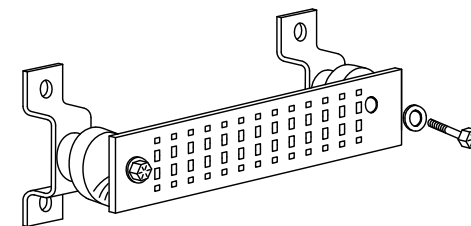
EACH GROUND CONDUCTOR TERMINATING ON ANY GROUND BAR SHALL HAVE AN IDENTIFICATION TAG ATTACHED AT EACH END THAT WILL IDENTIFY ITS ORIGIN AND DESTINATION.

SECTION "P" - SURGE PRODUCERS

- CABLE ENTRY PORTS (HATCH PLATES) (#2)
- GENERATOR FRAMEWORK (IF AVAILABLE) (#2)
- TELCO GROUND BAR
- COMMERCIAL POWER COMMON NEUTRAL/GROUND BOND (#2)
- +24V POWER SUPPLY RETURN BAR (#2)
- 48V POWER SUPPLY RETURN BAR (#2)
- RECTIFIER FRAMES.

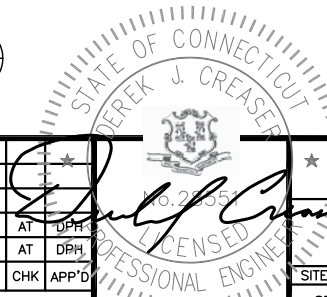
SECTION "A" - SURGE ABSORBERS

- INTERIOR GROUND RING (#2)
- EXTERNAL EARTH GROUND FIELD (BURIED GROUND RING) (#2)
- METALLIC COLD WATER PIPE (IF AVAILABLE) (#2)
- BUILDING STEEL (IF AVAILABLE) (#2)



GROUND BAR - DETAIL 4
SCALE: N.T.S. G-1

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A	02/05/18	ISSUED FOR REVIEW	TB	AT	DPH
SCALE:	AS SHOWN	DESIGNED BY:	AT	DRAWN BY:	LL





January 25, 2018

Rebecca Klein
Crown Castle
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Charlotte, NC 28277
(704) 405-6525

B+T Group
1717 S. Boulder, Suite 300
Tulsa, OK 74119
(918) 587-4630
btwo@btgrp.com

Subject: **Structural Analysis Report**

Carrier Designation: **AT&T Mobility Co-Locate**
Carrier Site Number: CT2024
Carrier Site Name: Clinton - Cow Hill RD

Crown Castle Designation: **Crown Castle BU Number:** 806363
Crown Castle Site Name: HRT 105 943201
Crown Castle JDE Job Number: 478064
Crown Castle Work Order Number: 1516360
Crown Castle Application Number: 421190 Rev. 3

Engineering Firm Designation: **B+T Group Project Number:** 100083.002.01

Site Data: **48 Cow Hill Road, Clinton, Middlesex County, CT**
Latitude 41° 17' 20.2", Longitude -72° 32' 18.5"
212.625 Foot - Self Support Tower

Dear Rebecca Klein,

B+T Group is pleased to submit this “**Structural Analysis Report**” to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural ‘Statement of Work’ and the terms of Crown Castle Purchase Order Number 1132878, in accordance with application 421190, revision 3.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment **Sufficient Capacity**
Note: See Table 1 and Table 2 for the proposed and existing/reserved loading, respectively.

This analysis has been performed in accordance with the 2016 Connecticut State Building Code based upon an ultimate 3-second gust wind speed of 130 mph converted to a nominal 3-second gust wind speed of 101 mph per Section 1609.3 and Appendix N as required for use in the TIA-222-G Standard per Exception #5 of Section 1609.1.1. Exposure Category B and Risk Category II were used in this analysis.

All equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

We at B+T Group appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Structural analysis prepared by: Jason Brock, E.I.

Respectfully submitted by: B&T Engineering, Inc.
COA: PEC.0001564 Expires: 02/10/2018

Scott S. Vance, P.E.

tnxTower Report - version 7.0.5.1

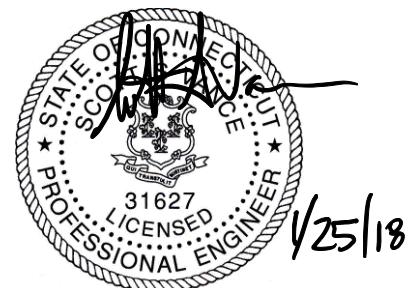


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1) INTRODUCTION

This tower is a 212.625 ft Self Support tower designed by ROHN in June of 1992. The tower was originally designed for a wind speed of 90 mph per TIA/EIA-222-E. The tower has been modified by VSI in June of 2007 and those modifications were incorporated in this analysis.

2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA-222-G Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a 3-second gust wind speed of 101 mph with no ice, 50 mph with 0.75 inch ice thickness and 60 mph under service loads, exposure category B with topographic category 1 and crest height of 0 feet.

Table 1 - Proposed Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
189.0	190.0	3	Andrew	SBNHH-1D65A	4	3/4	--
		3	Ericsson	RRUS 32			
		3	Ericsson	RRUS 32 B2			
		3	Ericsson	RRUS 32 B66			

Table 2 - Existing and Reserved Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
208.0	209.0	3	Alcatel Lucent	RRH2X60-AWS	2	1-5/8	2
		3	Alcatel Lucent	RRH2X60-PCS			
		3	Alcatel Lucent	RRH2x60-700			
		9	Andrew	SBNHH-1D65B			
		1	Rfs Celwave	DB-B1-6C-12AB-0Z			
	6	Antel	LPA-80080/6CF	18 1	1-5/8 1-1/4	1	
	1	Rfs Celwave	DB-T1-6Z-8AB-0Z				
	208.0	1	--	Sector Mount [SM 510-3]			
199.0	199.0	1	--	Sector Mount [SM 504-3]			
	198.0	3	Alcatel Lucent	1900MHz RRH (65MHz)	4	1-1/4	1
		3	Alcatel Lucent	800MHz 2X50W RRH W/FILTER			
		3	Alcatel Lucent	TD-RRH8x20-25			
		3	Rfs Celwave	APXVSP18-C-A20			
		3	Rfs Celwave	APXVTM14-C-120			
189.0	190.0	3	Ericsson	RRUS 11	2	7/8	4
		3	Ericsson	WCS RRUS-32-B30			
		6	ADC	Dual Band 800/1900 Full Band Masthead	12 2 2	1-5/8 13/16 3/8	1
		6	Andrew	SBNHH-1D65A			
		3	Ericsson	RRUS 11			
		3	Powerwave Tech.	7020.00			

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
		3	Powerwave Tech.	7770.00			
		2	Raycap	DC6-48-60-18-8F			
	189.0	1	--	Sector Mount [SM 602-3]			
183.0	183.0	3	Rfs Celwave	APXV18-206517LS	--	--	1
	179.0	2	Radio waves	HPD2-23	4	1/4	1
175.0	176.0	12	Decibel	DB844H90E-XY	--	--	3
	175.0	1	--	Sector Mount [SM 510-3]			
167.0	173.0	1	Rfs Celwave	1151-3	1	7/8	1
	167.0	1	--	Side Arm Mount [SO 308-1]			
164.0	173.0	1	Rfs Celwave	1151-3	1	7/8	1
	164.0	1	--	Side Arm Mount [SO 308-1]			
	162.0	1	Andrew	DB224-JJ			
162.0	162.0	1	--	Side Arm Mount [SO 308-1]	1	7/8	1
	160.0	1	Sinclair	SD310-HL			
147.0	153.0	1	Rfs Celwave	1151-3	1	7/8	1
	147.0	1	--	Side Arm Mount [SO 308-1]			
145.0	148.0	1	Sinclair	SD310-HL	1	7/8	1
	145.0	1	--	Side Arm Mount [SO 308-1]			
139.0	140.0	3	Ericsson	ERICSSON AIR 21 B2A B4P	7	1-5/8	1
		3	Ericsson	ERICSSON AIR 21 B4A B2P	5	1-1/4	
		3	Ericsson	KRY 112 144/1	3	7/8	
	139.0	1	--	Side Arm Mount [SO 104-3]			
128.0	132.0	1	Rfs Celwave	1142-2C	--	--	1
	128.0	1	--	Side Arm Mount [SO 308-1]			
51.0	51.0	1	GPS	GPS_A	1	1/2	1
		1	--	Side Arm Mount [SO 701-1]			

Notes:

- 1) Existing Equipment
- 2) Reserved Equipment
- 3) Abandoned Equipment; Considered in This Analysis
- 4) **Equipment To Be Removed; Not Considered in This Analysis**

Table 3 - Design Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
212	212	12	Sinclair	SRL410C4 w/ 6' Side Arms	--	--
200	200	2	Generic	6' Grid Dish	--	--
190	190	9	Swedcom	ALP9212N w/ Mounting Frame	--	--
100	100	1	Decibel	DB222 w/ Mount	--	--
90	90	1	Decibel	DB225 w/ Mount	--	--
80	80	2	Decibel	DB225-2 w/ Mounts	--	--
60	60	1	Decibel	DB212-2 w/ Mount	--	--
		1	Decibel	DB225 w/ Mount		
		1	Decibel	DB225-2 w/ Mount		
50	50	1	Decibel	DB212-2 w/ Mount	--	--
40	40	1	Decibel	DB212 w/ Mount	--	--

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

Document	Remarks	Reference	Source
Online Application	AT&T Mobility Co-Locate, Revision # 3	421190	CCI Sites
Tower Manufacturer Drawing	ROHN, Eng. File No: 28529JC	262274	CCI Sites
Tower Modification Drawing	VSI, Date: 06/29/2007	2169576	CCI Sites
Post Modification Inspection	VSI, Date: 10/30/2007	2146143	CCI Sites
Foundation Drawing	ROHN, Eng. File No: 28529JC	262273	CCI Sites
Geotech Report	Clarence Welti Assoc., Inc., Date: 07/06/1992	262276	CCI Sites
Antenna Configuration	Crown CAD Package	Date: 01/22/2018	CCI Sites

3.1) Analysis Method

tnxTower (version 7.0.5.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) Mount areas and weights are assumed based on photographs provided.
- 5) The existing base plate grout was considered in this analysis. Grout must be maintained and inspected periodically, and must be replaced if damaged or cracked. Refer to crown document ENG-BUL-10122, Tower Base Plate Grout Inspection and Classification.

This analysis may be affected if any assumptions are not valid or have been made in error. B+T Group should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	212.625 - 202.458	Leg	ROHN 2.5 STD	1	-5.717	56.631	10.1	Pass
T2	202.458 - 182.292	Leg	ROHN 3 EH	28	-30.268	93.888	32.2	Pass
T3	182.292 - 162.104	Leg	ROHN 4 EH	67	-76.131	159.259	47.8	Pass
T4	162.104 - 141.896	Leg	ROHN 5 EH	108	-128.440	238.686	53.8	Pass
T5	141.896 - 121.688	Leg	ROHN 6 EHS	146	-160.525	242.933	66.1	Pass
T6	121.688 - 101.479	Leg	ROHN 6 EH	173	-196.727	302.237	65.1	Pass
T7	101.479 - 81.2708	Leg	ROHN 6 EH	200	-229.984	302.237	76.1	Pass
T8	81.2708 - 61	Leg	ROHN 8 EHS	227	-261.735	384.981	68.0	Pass
T9	61 - 40.6667	Leg	ROHN 8 EHS	254	-292.818	384.707	76.1	Pass
T10	40.6667 - 20.3333	Leg	ROHN 8 EH	281	-306.548	503.236	60.9	Pass
T11	20.3333 - 0	Leg	ROHN 8 EH	314	-335.559	503.352	66.7	Pass
T1	212.625 - 202.458	Diagonal	ROHN 2 STD	14	-3.494	23.829	14.7	Pass
T2	202.458 - 182.292	Diagonal	ROHN 2 STD	39	-10.586	17.541	60.4	Pass
T3	182.292 - 162.104	Diagonal	ROHN 2 STD	77	-10.843	15.159	71.5	Pass
T4	162.104 - 141.896	Diagonal	ROHN 2 STD	117	-10.858	13.026	83.4	Pass
T5	141.896 - 121.688	Diagonal	ROHN 2.5 STD	156	-14.129	16.287	86.8	Pass
T6	121.688 - 101.479	Diagonal	ROHN 2.5 STD	183	-13.171	14.278	92.2	Pass
T7	101.479 - 81.2708	Diagonal	ROHN 3 STD	210	-13.495	24.700	54.6	Pass
T8	81.2708 - 61	Diagonal	ROHN 3 STD	237	-13.437	21.813	61.6	Pass
T9	61 - 40.6667	Diagonal	ROHN 3 STD	264	-14.582	19.146	76.2	Pass
T10	40.6667 - 20.3333	Diagonal	ROHN 3 STD	303	-20.582	31.156	66.1	Pass
T11	20.3333 - 0	Diagonal	ROHN 3 STD	336	-23.698	29.608	80.0	Pass
T1	212.625 - 202.458	Horizontal	ROHN 1.5 STD	13	-2.534	22.582	11.2	Pass
T2	202.458 - 182.292	Horizontal	ROHN 1.5 STD	37	-5.794	22.520	25.7	Pass
T3	182.292 - 162.104	Horizontal	ROHN 1.5 STD	76	-6.833	19.143	35.7	Pass
T4	162.104 - 141.896	Horizontal	ROHN 2 STD	115	-7.578	27.209	27.9 30.5 (b)	Pass
T5	141.896 - 121.688	Horizontal	ROHN 2 STD	154	-8.521	22.640	37.6	Pass
T6	121.688 - 101.479	Horizontal	ROHN 2 STD	181	-8.739	16.864	51.8	Pass
T7	101.479 - 81.2708	Horizontal	ROHN 2.5 STD	208	-9.608	28.852	33.3 38.7 (b)	Pass
T8	81.2708 - 61	Horizontal	ROHN 2.5 STD	235	-10.137	22.530	45.0	Pass
T9	61 - 40.6667	Horizontal	ROHN 2.5 STD	262	-11.495	17.820	64.5	Pass
T10	40.6667 - 20.3333	Horizontal	ROHN 3 STD	299	-11.375	31.651	35.9	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail	
T11	20.3333 - 0	Horizontal	ROHN 3 STD	332	-13.885	25.753	53.9	Pass	
T1	212.625 - 202.458	Top Girt	ROHN 1.5 STD	5	-0.256	22.635	1.1	Pass	
T10	40.6667 - 20.3333	Redund Horz 1 Bracing	ROHN 1.5 STD	288	-5.324	13.007	40.9	Pass	
T11	20.3333 - 0	Redund Horz 1 Bracing	ROHN 1.5 STD	321	-5.823	11.053	52.7	Pass	
T10	40.6667 - 20.3333	Redund Diag 1 Bracing	ROHN 2 STD	293	-4.918	8.811	55.8	Pass	
T11	20.3333 - 0	Redund Diag 1 Bracing	ROHN 2 STD	322	-5.030	8.111	62.0	Pass	
T10	40.6667 - 20.3333	Redund Hip 1 Bracing	ROHN 1.5 STD	306	-0.029	11.936	0.2	Pass	
T11	20.3333 - 0	Redund Hip 1 Bracing	ROHN 1.5 STD	339	-0.027	10.041	0.3	Pass	
T10	40.6667 - 20.3333	Redund Hip Diagonal 1 Bracing	ROHN 2.5 STD	307	-0.094	10.381	0.9	Pass	
T11	20.3333 - 0	Redund Hip Diagonal 1 Bracing	ROHN 2.5 STD	340	-0.086	9.348	0.9	Pass	
T1	212.625 - 202.458	Inner Bracing	L2x2x1/8	17	-0.004	6.538	0.5	Pass	
T2	202.458 - 182.292	Inner Bracing	L2x2x1/8	41	-0.007	6.440	0.5	Pass	
T3	182.292 - 162.104	Inner Bracing	L2x2x1/8	79	-0.006	4.790	0.6	Pass	
T4	162.104 - 141.896	Inner Bracing	L2x2x1/8	120	-0.007	3.283	0.7	Pass	
T5	141.896 - 121.688	Inner Bracing	L2x2x1/8	159	-0.009	2.480	0.8	Pass	
T6	121.688 - 101.479	Inner Bracing	L2 1/2x2 1/2x3/16	186	-0.011	5.225	0.7	Pass	
T7	101.479 - 81.2708	Inner Bracing	L3x3x3/16	212	-0.014	6.881	0.7	Pass	
T8	81.2708 - 61	Inner Bracing	L3 1/2x3 1/2x1/4	239	-0.017	11.196	0.5	Pass	
T9	61 - 40.6667	Inner Bracing	L3 1/2x3 1/2x1/4	266	-0.017	8.922	0.5	Pass	
T10	40.6667 - 20.3333	Inner Bracing	ROHN 3 STD	311	-0.020	29.869	0.3	Pass	
T11	20.3333 - 0	Inner Bracing	ROHN 3 STD	344	-0.017	24.440	0.2	Pass	
							Summary		
							Leg (T9)	76.1	Pass
							Diagonal (T6)	92.2	Pass
							Horizontal (T9)	64.5	Pass
							Top Girt (T1)	1.1	Pass
							Redund Horz 1 Bracing (T11)	52.7	Pass
							Redund Diag 1 Bracing (T11)	62.0	Pass
							Redund Hip 1 Bracing (T11)	0.3	Pass
							Redund Hip	0.9	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
						Diagonal 1 Bracing (T11)		
						Inner Bracing (T5)	0.8	Pass
						Bolt Checks	60.9	Pass
						Rating =	92.2	Pass

Table 6 - Tower Component Stresses vs. Capacity – LC7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	Base	63.0	Pass
1	Base Foundation (Steel)	Base	25.8	Pass
1	Base Foundation (Soil Interaction)	Base	51.7	Pass

Structure Rating (max from all components) =	92.2%
---	--------------

Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

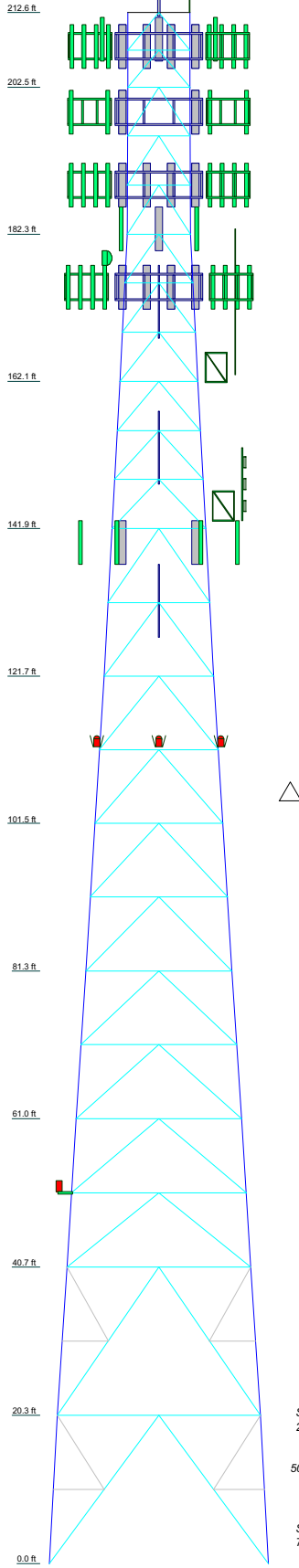
4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the final load configuration. No modifications are required at this time.

APPENDIX A

TNXTOWER OUTPUT

Section	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37					
Legs	ROHN 2.5 STD	ROHN 3.5 EH	ROHN 4.4 EH	ROHN 5.5 EH	ROHN 6.8 EH	ROHN 8.5 EH	ROHN 10.5 STD	ROHN 12.5 STD	ROHN 14.5 STD	ROHN 16.5 STD	ROHN 18.5 STD	ROHN 20.5 STD	ROHN 22.5 STD	ROHN 24.5 STD	ROHN 26.5 STD	ROHN 28.5 STD	ROHN 30.5 STD	ROHN 32.5 STD	ROHN 34.5 STD	ROHN 36.5 STD	ROHN 38.5 STD	ROHN 40.5 STD	ROHN 42.5 STD	ROHN 44.5 STD	ROHN 46.5 STD	ROHN 48.5 STD	ROHN 50.5 STD	ROHN 52.5 STD	ROHN 54.5 STD	ROHN 56.5 STD	ROHN 58.5 STD	ROHN 60.5 STD
Leg Grade	A572-50																															
Diagonal	ROHN 2.5 STD																															
Diagonal Grade	A572-50																															
Top Grids	ROHN 1.5 STD																															
Horizontals	ROHN 3 STD																															
Red. Horizontals	ROHN 1.5 STD																															
Red. Diagonals	ROHN 2 STD																															
Red. Hpps	ROHN 1.5 STD																															
Inner Bracing	ROHN 3 STD																															
Face Width (ft)	30.0417																															
# Panels @ (ft)	2 @ 30.3533																															
Weight (K)	37.5	25.1771	27.8333	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	22.8774	



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
Flash Beacon Lighting (E)	216.625	DC6-48-60-18-8F (Hor.Off./Photo)	189
Lighting Rod 5/8" x 6" (E)	213.625	SBNH-1D65A w/ Mount Pipe (P)	189
Climb Leg Extension (E)	212.625	SBNH-1D65A w/ Mount Pipe (P)	189
4' x 2" Pipe Mount (E-For Beacon)	212.625	SBNH-1D65A w/ Mount Pipe (P)	189
(2) LPA-80086CF w/ Mount Pipe (E)	208	RRUS 32 (P)	189
(2) LPA-80086CF w/ Mount Pipe (E)	208	RRUS 32 (P)	189
(2) LPA-80086CF w/ Mount Pipe (E)	208	RRUS 32 (P)	189
DB-T1-6Z-8AB-0Z (E)	208	RRUS 32 B66 (P)	189
(3) SBNH-1D65B w/ Mount Pipe (R)	208	RRUS 32 B66 (P)	189
(3) SBNH-1D65B w/ Mount Pipe (R)	208	RRUS 32 B66 (P)	189
(3) SBNH-1D65B w/ Mount Pipe (R)	208	RRUS 32 B2 (P)	189
RRH2X60-PCS (R)	208	RRUS 32 B2 (P)	189
RRH2X60-PCS (R)	208	RRUS 32 B2 (P)	189
RRH2X60-PCS (R)	208	Sector Mount [SM 602-3] (E)	189
RRH2x60-700 (R)	208	APXV18-206517LS w/ Mount Pipe (Leg Mounted/Photo)	183
RRH2x60-700 (R)	208	APXV18-206517LS w/ Mount Pipe (Leg Mounted/Photo)	183
RRH2x60-700 (R)	208	APXV18-206517LS w/ Mount Pipe (Leg Mounted/Photo)	183
DB-B1-6C-12AB-0Z (R)	208	APXV18-206517LS w/ Mount Pipe (Leg Mounted/Photo)	183
RRH2X60-AWS (R)	208	RRUS 32 B2 (P)	189
RRH2X60-AWS (R)	208	RRUS 32 B2 (P)	189
RRH2X60-AWS (R)	208	RRUS 32 B2 (P)	189
RRH2X60-AWS (R)	208	(4) DB844HQE-XY w/ Mount Pipe (AB-Inactive/TIA)	175
RRH2X60-AWS (R)	208	(4) DB844HQE-XY w/ Mount Pipe (AB-Inactive/TIA)	175
Sector Mount [SM 510-3] (E)	208	(4) DB844HQE-XY w/ Mount Pipe (AB-Inactive/TIA)	175
APXVSP18-C-A20 w/ Mount Pipe (E)	199	(4) DB844HQE-XY w/ Mount Pipe (AB-Inactive/TIA)	175
APXVSP18-C-A20 w/ Mount Pipe (E)	199	(4) DB844HQE-XY w/ Mount Pipe (AB-Inactive/TIA)	175
APXVSP18-C-A20 w/ Mount Pipe (E)	199	(4) DB844HQE-XY w/ Mount Pipe (AB-Inactive/TIA)	175
APXVSP18-C-A20 w/ Mount Pipe (E)	199	(4) DB844HQE-XY w/ Mount Pipe (AB-Inactive/TIA)	175
APXVTM14-C-120 w/ Mount Pipe (E)	199	6' x 2' Mount Pipe (Dish Mount)	175
APXVTM14-C-120 w/ Mount Pipe (E)	199	6' x 2' Mount Pipe (Dish Mount)	175
APXVTM14-C-120 w/ Mount Pipe (E)	199	Sector Mount [SM 510-3] (E)	175
APXVTM14-C-120 w/ Mount Pipe (E)	199	HDP2-23 (Az. Face/Prev.SA)	175
800MHz 2X50W RRH W/FILTER (E)	199	HDP2-23 (Az. Face/Prev.SA)	175
800MHz 2X50W RRH W/FILTER (E)	199	1151-3 (E)	167
800MHz 2X50W RRH W/FILTER (E)	199	1151-3 (E)	167
1900MHz RRH (65MHz) (E)	199	Side Arm Mount [SO 308-1] (E)	167
1900MHz RRH (65MHz) (E)	199	1151-3 (E)	167
1900MHz RRH (65MHz) (E)	199	1151-3 (E)	167
1900MHz RRH (65MHz) (E)	199	Andrew DB224-JJ (Per Photo)	164
TD-RRHx20-25 (E)	199	Side Arm Mount [SO 308-1] (E)	164
TD-RRHx20-25 (E)	199	SD310-HL (E)	162
TD-RRHx20-25 (E)	199	Side Arm Mount [SO 308-1] (E)	162
TD-RRHx20-25 (E)	199	1151-3 (E)	147
(3) 4' x 2" Pipe Mount (E)	199	Side Arm Mount [SO 308-1] (E)	147
(3) 4' x 2" Pipe Mount (E)	199	SD310-HL (E)	145
(3) 4' x 2" Pipe Mount (E)	199	SD310-HL (E)	145
Sector Mount [SM 504-3] (14/TIA)	199	Side Arm Mount [SO 308-1] (E)	145
770.00 w/ Mount Pipe (E)	189	ERICSSON AIR 21 B2A B4P w/ Mount Pipe (E)	139
770.00 w/ Mount Pipe (E)	189	ERICSSON AIR 21 B2A B4P w/ Mount Pipe (E)	139
770.00 w/ Mount Pipe (E)	189	ERICSSON AIR 21 B2A B4P w/ Mount Pipe (E)	139
770.00 w/ Mount Pipe (E)	189	ERICSSON AIR 21 B4A B2P w/ Mount Pipe (E)	139
(2) SBNH-1D65A w/ Mount Pipe (E)	189	ERICSSON AIR 21 B4A B2P w/ Mount Pipe (E)	139
(2) SBNH-1D65A w/ Mount Pipe (E)	189	ERICSSON AIR 21 B4A B2P w/ Mount Pipe (E)	139
(2) SBNH-1D65A w/ Mount Pipe (E)	189	ERICSSON AIR 21 B4A B2P w/ Mount Pipe (E)	139
(2) DUAL BAND 800/1900 FULL BAND MASTHEAD (E)	189	KRY 112 144/1 (E)	139
(2) DUAL BAND 800/1900 FULL BAND MASTHEAD (E)	189	KRY 112 144/1 (E)	139
(2) DUAL BAND 800/1900 FULL BAND MASTHEAD (E)	189	KRY 112 144/1 (E)	139
(2) DUAL BAND 800/1900 FULL BAND MASTHEAD (E)	189	Side Arm Mount [SO 104-3] (E)	139
(2) DUAL BAND 800/1900 FULL BAND MASTHEAD (E)	189	1142-2C (Inactive/TIA)	128
7020.00 (E)	189	Side Arm Mount [SO 308-1] (E)	128
7020.00 (E)	189	Side Lighting (E)	112
7020.00 (E)	189	Side Lighting (E)	112
7020.00 (E)	189	Side Lighting (E)	112
RRUS 11 (E)	189	GPS. A (E)	51
RRUS 11 (E)	189	Side Arm Mount [SO 701-1] (E)	51
RRUS 11 (E)	189	Side Arm Mount [SO 701-1] (E)	51
DC6-48-60-18-8F (Hor.Off./Photo)	189		

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi			

TOWER DESIGN NOTES

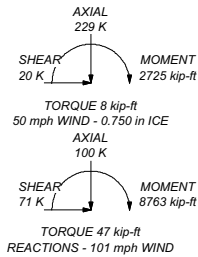
1. Tower is located in Middlesex County, Connecticut.
2. Tower designed for Exposure B to the TIA-222-G Standard.
3. Tower designed for a 101 mph basic wind in accordance with the TIA-222-G Standard.
4. Tower is also designed for a 50 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Structure Class II.
7. Topographic Category 1 with Crest Height of 0.000 ft
8. TOWER RATING: 92.2%

ALL REACTIONS ARE FACTORED

MAX. CORNER REACTIONS AT BASE:

DOWN: 388 K
SHEAR: 43 K

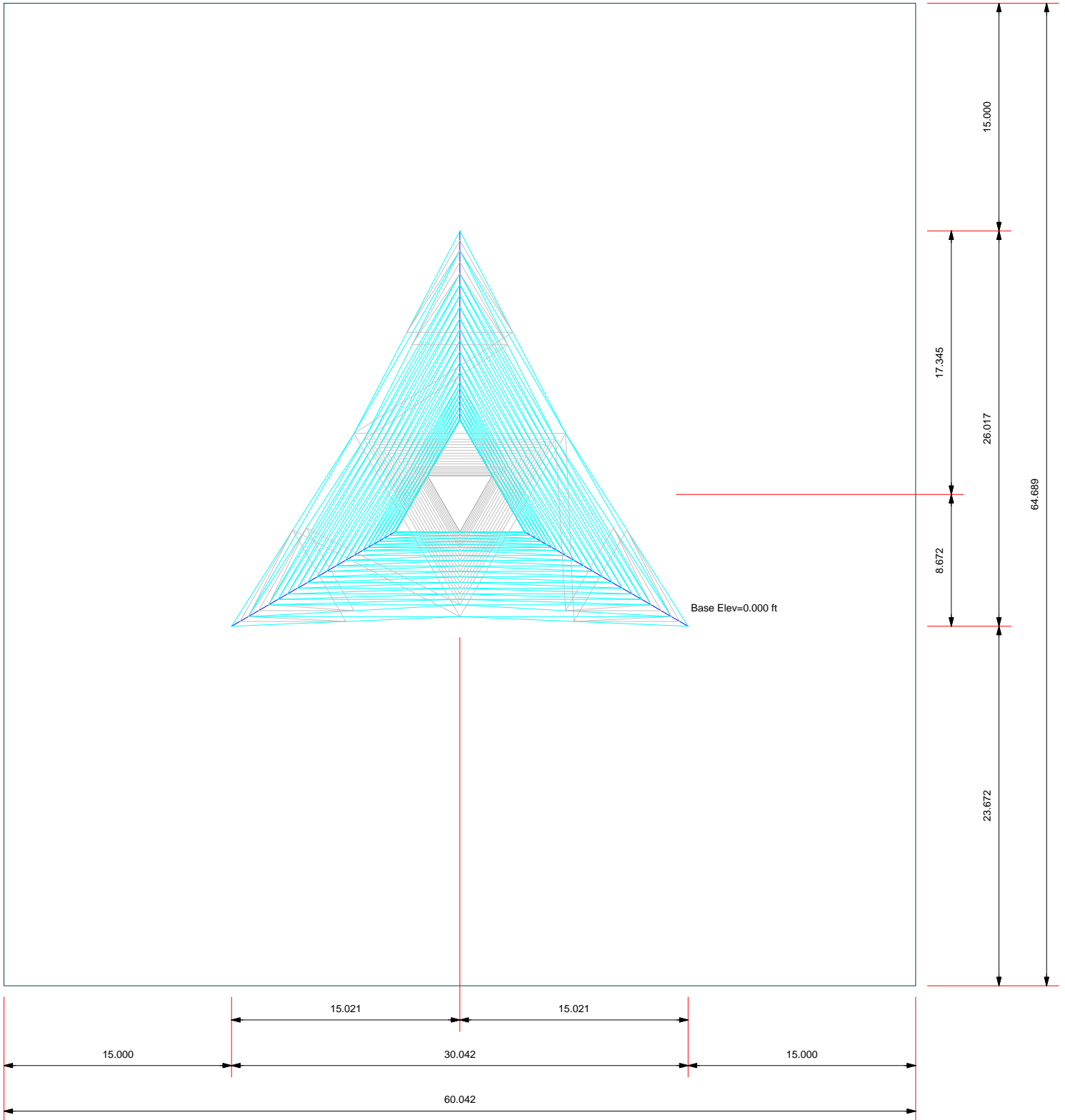
UPLIFT: -311 K
SHEAR: 39 K



TORQUE 47 kip-ft
REACTIONS - 101 mph WIND

B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265		Doc: 100083.002.01 - HRT 105 943201, CT (BU# 80636)
		Project: Crown Castle
Client: Crown Castle Code: TIA-222-G Path:		Drawn by: Yathish Date: 01/25/18 Scale: NTS Dwg No. E-1

Plot Plan
Total Area - 0.09 Acres

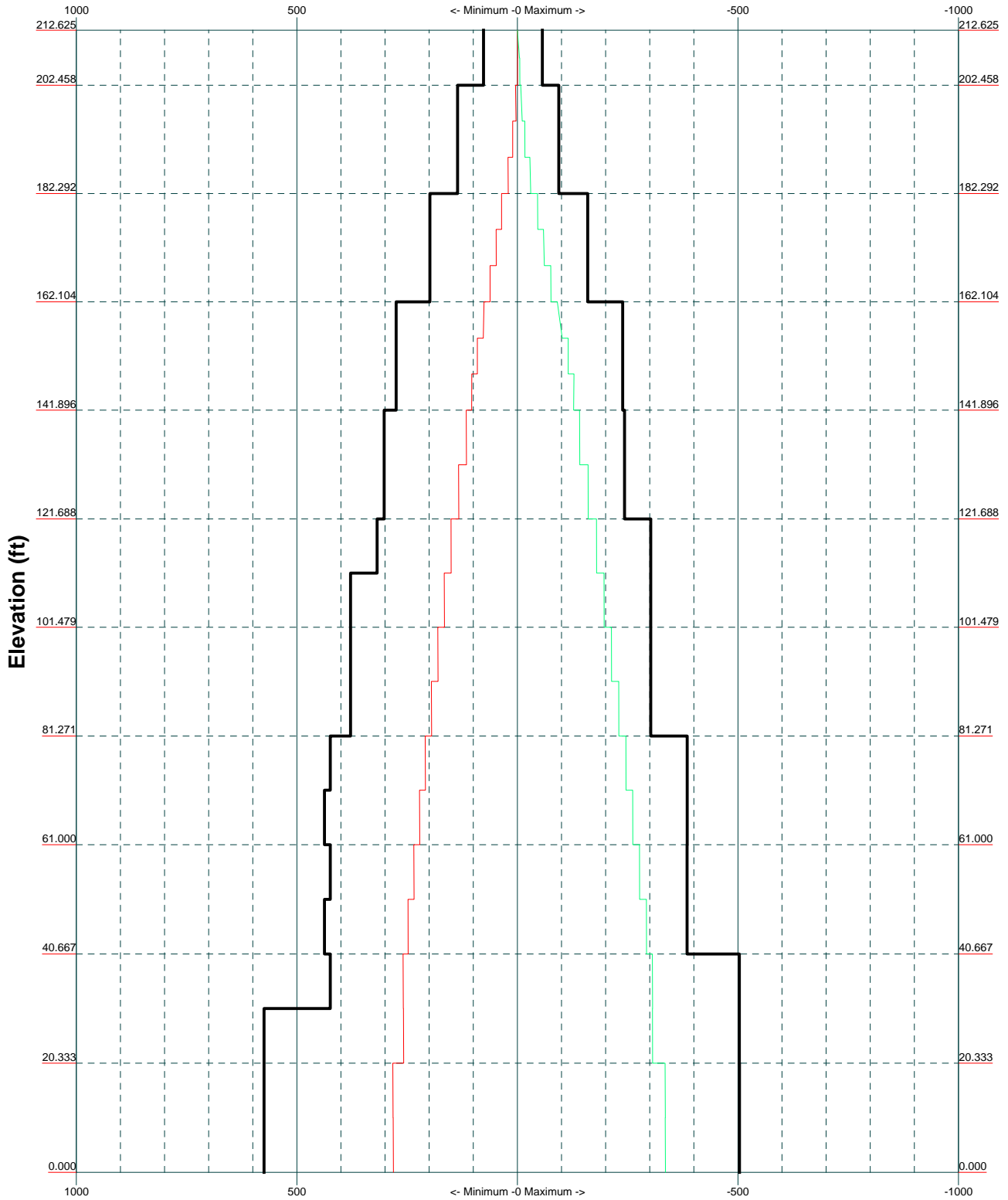


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Job: 100083.002.01 - HRT 105 943201, CT (BU# 80636)		
Project:		
Client: Crown Castle	Drawn by: Yathish	App'd:
Code: TIA-222-G	Date: 01/25/18	Scale: NTS
Path:		Dwg No: E-2

TIA-222-G - 101 mph/50 mph 0.750 in Ice Exposure B

Leg Capacity ———
Leg Compression (K)



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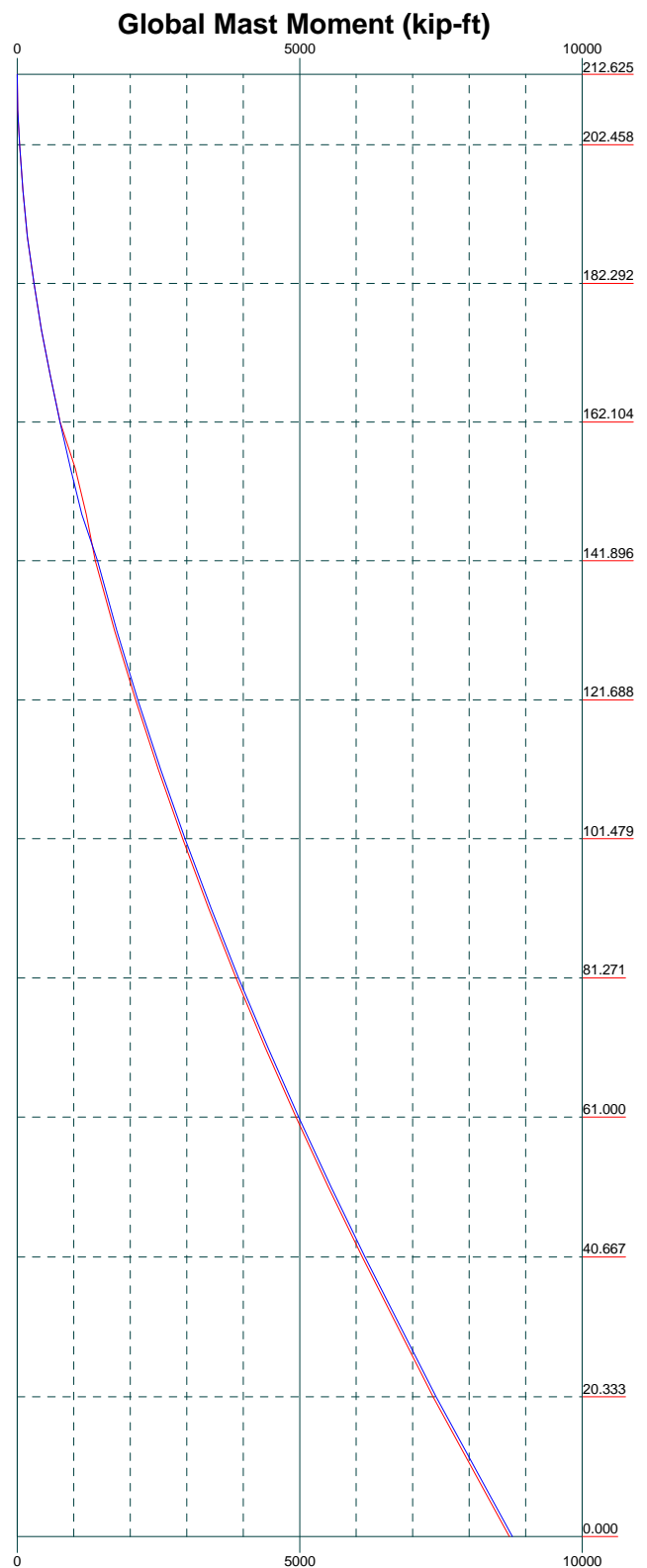
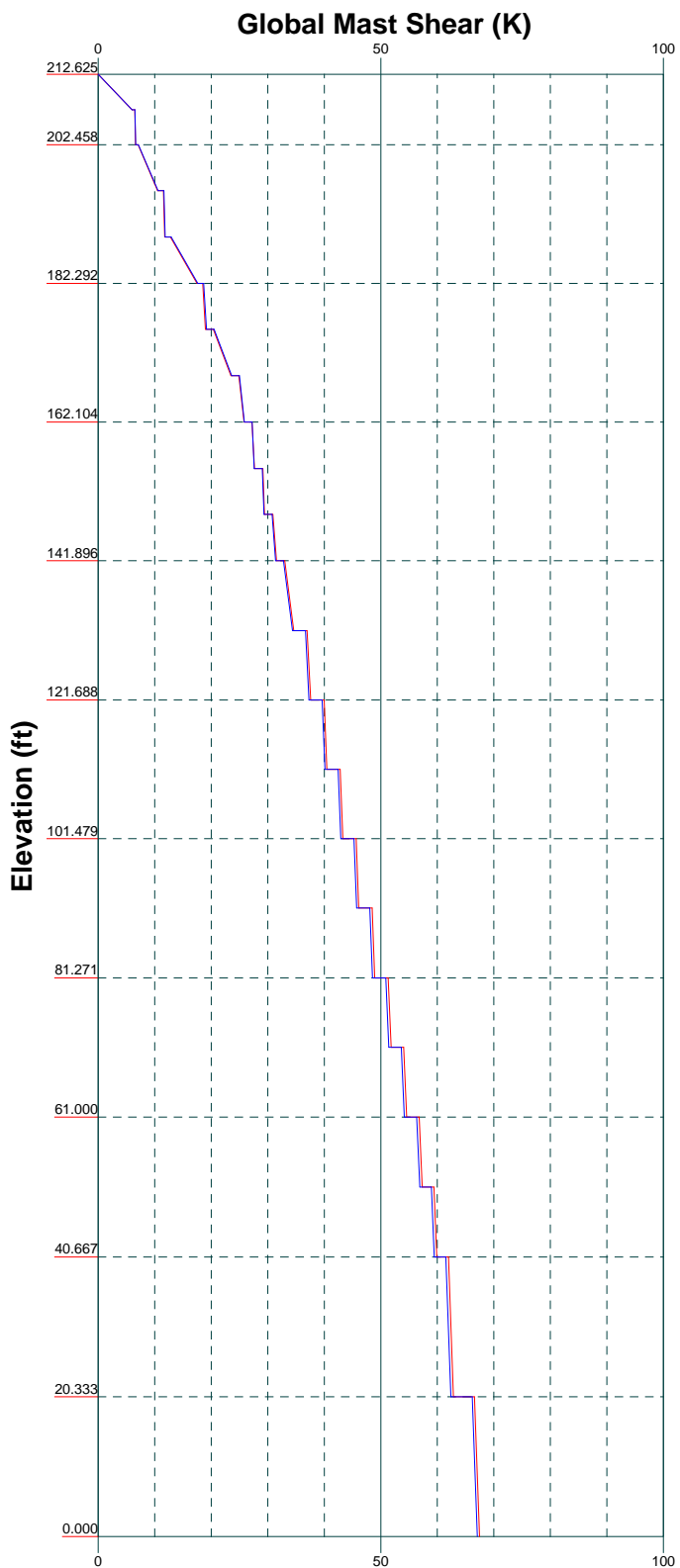
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Project:		
Client: Crown Castle	Drawn by: Yathish	App'd:
Code: TIA-222-G	Date: 01/25/18	Scale: NTS
Path:		Dwg No: E-3

Vx

Vz

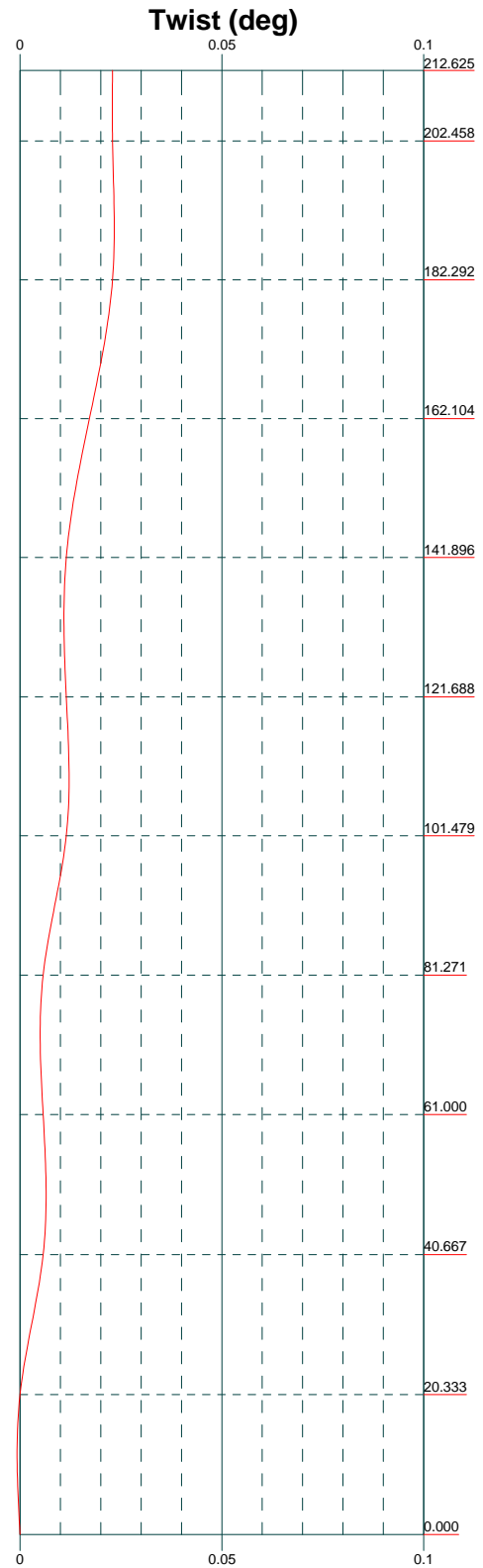
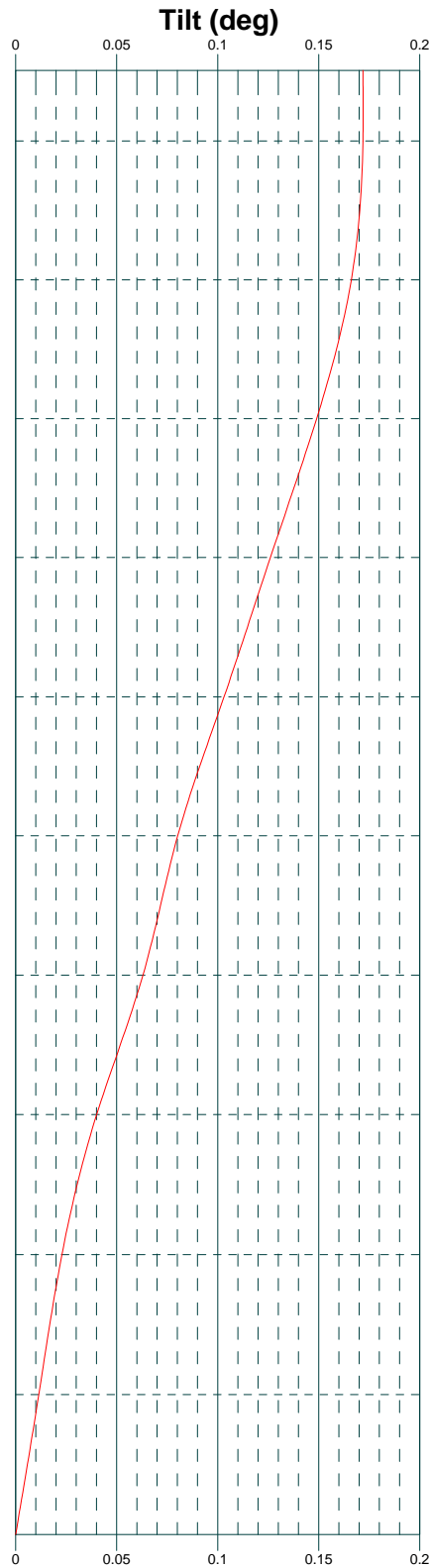
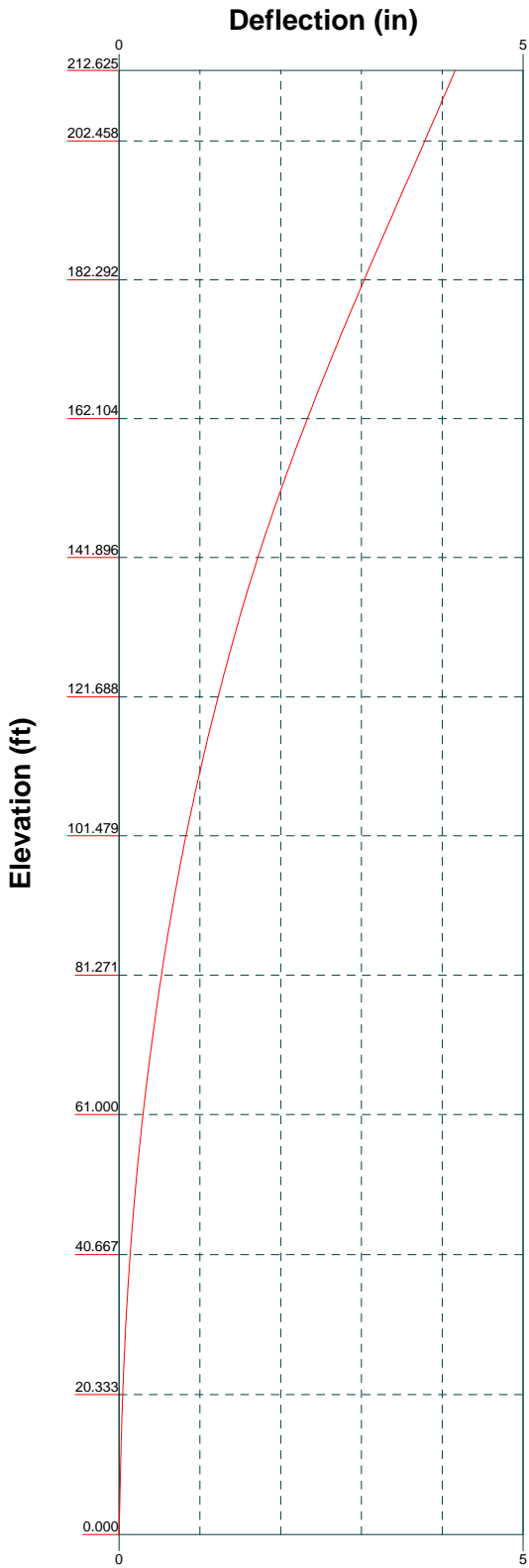
Mx

Mz



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Project:		
Client: Crown Castle	Drawn by: Yathish	App'd:
Code: TIA-222-G	Date: 01/25/18	Scale: NTS
Path:	Dwg No: E-4	



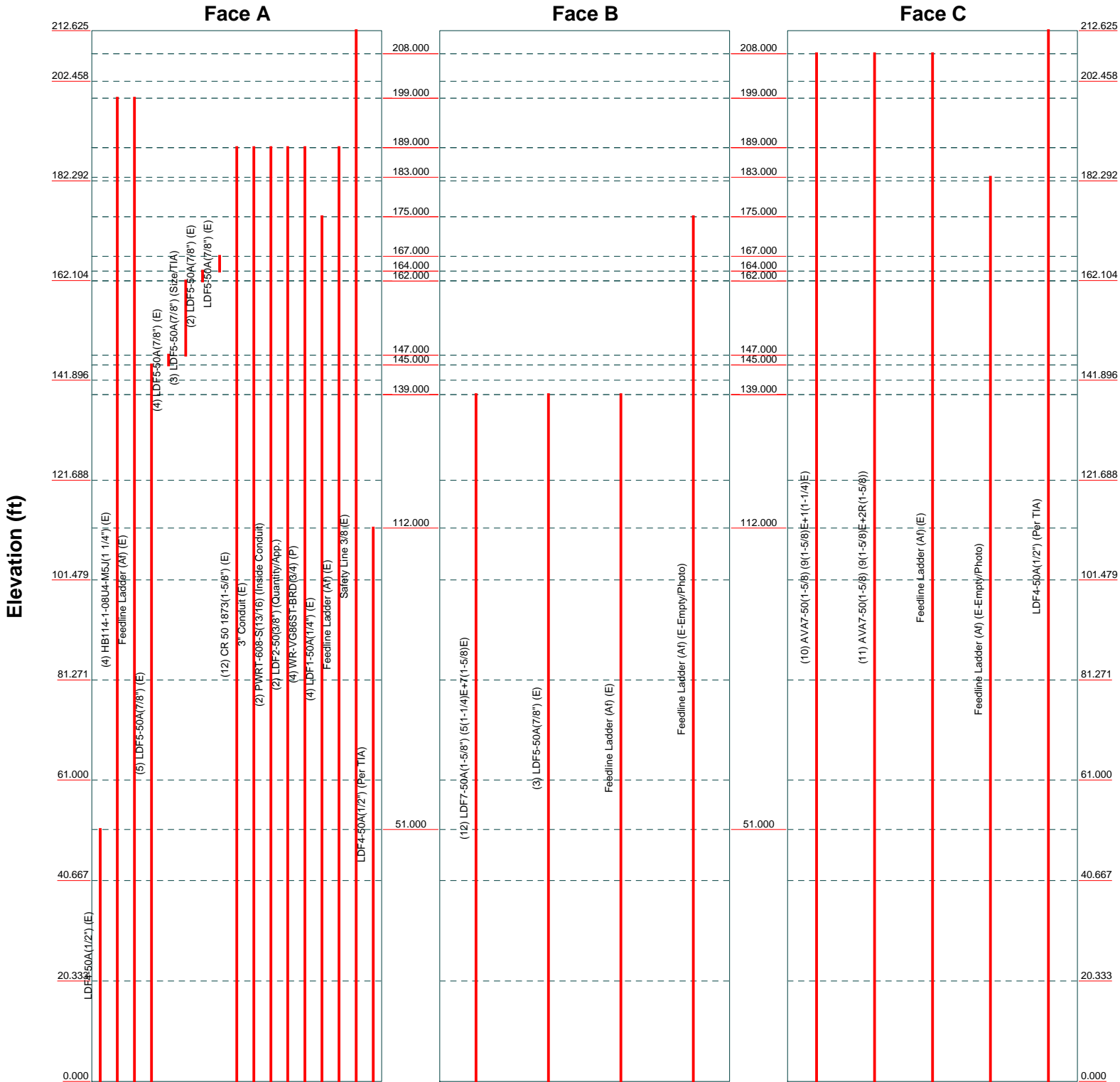
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 FAX: (918) 295-0265


Job: 100083.002.01 - HRT 105 943201, CT (BU# 80636)		
Project:		
Client: Crown Castle	Drawn by: Yathish	App'd:
Code: TIA-222-G	Date: 01/25/18	Scale: NTS
Path:	Dwg No: E-5	

Feed Line Distribution Chart

0' - 212'7-1/2"

— Round
 — Flat
 — App In Face
 — App Out Face
 — Truss Leg




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Job: 100083.002.01 - HRT 105 943201, CT (BU# 80636)		
Project:		
Client: Crown Castle	Drawn by: Yathish	App'd:
Code: TIA-222-G	Date: 01/25/18	Scale: NTS
Path:	Dwg No. E-7	

tnxTower B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job 100083.002.01 - HRT 105 943201, CT (BU# 806363)	Page 1 of 44
	Project	Date 16:57:31 01/25/18
	Client Crown Castle	Designed by Yathish

Tower Input Data

The main tower is a 3x free standing tower with an overall height of 212.625 ft above the ground line.

The base of the tower is set at an elevation of 0.000 ft above the ground line.

The face width of the tower is 8.500 ft at the top and 30.042 ft at the base.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Middlesex County, Connecticut.

Basic wind speed of 101 mph.

Structure Class II.

Exposure Category B.

Topographic Category 1.

Crest Height 0.000 ft.

Nominal ice thickness of 0.750 in.

Ice thickness is considered to increase with height.

Ice density of 56.000 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50.000 °F.

Deflections calculated using a wind speed of 60 mph.

Pressures are calculated at each section.

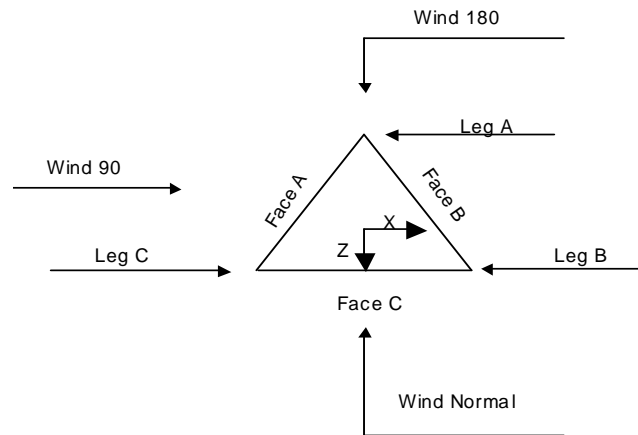
Stress ratio used in tower member design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

<ul style="list-style-type: none"> Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification √ Use Code Stress Ratios √ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile √ Include Bolts In Member Capacity Leg Bolts Are At Top Of Section √ Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric 	<ul style="list-style-type: none"> Distribute Leg Loads As Uniform Assume Legs Pinned √ Assume Rigid Index Plate √ Use Clear Spans For Wind Area √ Use Clear Spans For KL/r Retention Guys To Initial Tension √ Bypass Mast Stability Checks √ Use Azimuth Dish Coefficients √ Project Wind Area of Appurt. Autocalc Torque Arm Areas Add IBC .6D+W Combination √ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder 	<ul style="list-style-type: none"> Use ASCE 10 X-Brace Ly Rules √ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA √ SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation √ Consider Feed Line Torque √ Include Angle Block Shear Check Use TIA-222-G Bracing Resist. Exemption Use TIA-222-G Tension Splice Exemption <li style="text-align: center;">Poles Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets
--	--	---

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	Project	Date 16:57:31 01/25/18
	Client Crown Castle	Designed by Yathish



Triangular Tower

Tower Section Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	<i>ft</i>			<i>ft</i>		<i>ft</i>
T1	212.625-202.458			8.500	1	10.167
T2	202.458-182.292			8.542	1	20.167
T3	182.292-162.104			8.625	1	20.188
T4	162.104-141.896			10.708	1	20.208
T5	141.896-121.688			12.792	1	20.208
T6	121.688-101.479			15.042	1	20.208
T7	101.479-81.271			17.542	1	20.208
T8	81.271-61.000			20.042	1	20.271
T9	61.000-40.667			22.677	1	20.333
T10	40.667-20.333			25.177	1	20.333
T11	20.333-0.000			27.833	1	20.333

Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	<i>ft</i>	<i>ft</i>				<i>in</i>	<i>in</i>
T1	212.625-202.458	5.083	K Brace Down	No	Yes	0.000	0.000
T2	202.458-182.292	6.722	K Brace Down	No	Yes	0.000	0.000
T3	182.292-162.104	6.729	K Brace Down	No	Yes	0.000	0.000
T4	162.104-141.896	6.736	K Brace Down	No	Yes	0.000	0.000

tnxTower B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job 100083.002.01 - HRT 105 943201, CT (BU# 806363)	Page 3 of 44
	Project	Date 16:57:31 01/25/18
	Client Crown Castle	Designed by Yathish

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T5	141.896-121.688	10.104	K Brace Down	No	Yes	0.000	0.000
T6	121.688-101.479	10.104	K Brace Down	No	Yes	0.000	0.000
T7	101.479-81.271	10.104	K Brace Down	No	Yes	0.000	0.000
T8	81.271-61.000	10.135	K Brace Down	No	Yes	0.000	0.000
T9	61.000-40.667	10.167	K Brace Down	No	Yes	0.000	0.000
T10	40.667-20.333	20.333	K1 Down	No	Yes	0.000	0.000
T11	20.333-0.000	20.333	K1 Down	No	Yes	0.000	0.000

Tower Section Geometry (cont'd)

Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
ft						
T1 212.625-202.458	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T2 202.458-182.292	Pipe	ROHN 3 EH	A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T3 182.292-162.104	Pipe	ROHN 4 EH	A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T4 162.104-141.896	Pipe	ROHN 5 EH	A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T5 141.896-121.688	Pipe	ROHN 6 EHS	A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)
T6 121.688-101.479	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)
T7 101.479-81.271	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T8 81.271-61.000	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T9 61.000-40.667	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T10 40.667-20.333	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T11 20.333-0.000	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)

Tower Section Geometry (cont'd)

Tower Elevation	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
ft							
T1 212.625-202.458	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 1.5 STD	A572-50 (50 ksi)
T2 202.458-182.292	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 1.5 STD	A572-50 (50 ksi)
T3 182.292-162.104	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 1.5 STD	A572-50 (50 ksi)
T4 162.104-141.896	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T5 141.896-121.688	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)

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Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T6 121.688-101.479	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T7 101.479-81.271	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)
T8 81.271-61.000	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)
T9 61.000-40.667	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)
T10 40.667-20.333	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T11 20.333-0.000	None	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
T1 212.625-202.458	Single Angle		A572-50 (50 ksi)	Equal Angle	L2x2x1/8	A36 (36 ksi)
T2 202.458-182.292	Single Angle		A572-50 (50 ksi)	Equal Angle	L2x2x1/8	A36 (36 ksi)
T3 182.292-162.104	Single Angle		A572-50 (50 ksi)	Equal Angle	L2x2x1/8	A36 (36 ksi)
T4 162.104-141.896	Single Angle		A572-50 (50 ksi)	Equal Angle	L2x2x1/8	A36 (36 ksi)
T5 141.896-121.688	Single Angle		A572-50 (50 ksi)	Equal Angle	L2x2x1/8	A36 (36 ksi)
T6 121.688-101.479	Single Angle		A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T7 101.479-81.271	Single Angle		A572-50 (50 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)
T8 81.271-61.000	Single Angle		A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A572-50 (50 ksi)
T9 61.000-40.667	Single Angle		A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A572-50 (50 ksi)
T10 40.667-20.333	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)
T11 20.333-0.000	Single Angle		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Redundant Bracing Grade	Redundant Type	Redundant Type	Redundant Size	K Factor
T10 40.667-20.333	A36 (36 ksi)	Horizontal (1)	Pipe	ROHN 1.5 STD	1
		Diagonal (1)	Pipe	ROHN 2 STD	1
		Hip (1)	Pipe	ROHN 1.5 STD	1

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T6 121.688-101.479	0.000	1	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	1	0.000	0.75
T7 101.479-81.271	0.000	1	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	1	0.000	0.75
T8 81.271-61.000	0.000	1	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	1	0.000	0.75
T9 61.000-40.667	0.000	1	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	1	0.000	0.75
T10 40.667-20.333	0.000	1	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	1	0.000	0.75
T11 20.333-0.000	0.000	1	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	1	0.000	0.75

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 212.625-202.458	Flange	0.750 A325N	4	0.625 A325N	3	0.625 A325N	0	0.625 A325X	0	0.625 A325X	0	0.625 A325N	2	0.625 A325X	0
T2 202.458-182.292	Flange	0.875 A325N	4	0.625 A325N	3	0.625 A325N	0	0.625 A325X	0	0.625 A325X	0	0.625 A325N	2	0.625 A325X	0
T3 182.292-162.104	Flange	1.000 A325N	4	0.625 A325N	3	0.625 A325N	0	0.625 A325X	0	0.625 A325X	0	0.625 A325N	2	0.625 A325X	0
T4 162.104-141.896	Flange	1.000 A325N	6	0.625 A325N	3	0.625 A325N	0	0.625 A325X	0	0.625 A325X	0	0.625 A325N	2	0.625 A325X	0
T5 141.896-121.688	Flange	1.000 A325N	6	0.625 A325N	3	0.625 A325N	0	0.625 A325X	0	0.625 A325X	0	0.625 A325N	2	0.625 A325X	0
T6 121.688-101.479	Flange	1.000 A325N	6	0.625 A325N	3	0.625 A325N	0	0.625 A325X	0	0.625 A325X	0	0.625 A325N	2	0.625 A325X	0
T7 101.479-81.271	Flange	1.000 A325N	8	0.625 A325N	3	0.625 A325N	0	0.625 A325X	0	0.625 A325X	0	0.625 A325N	2	0.625 A325X	0
T8 81.271-61.000	Flange	1.000 A325N	8	0.625 A325N	3	0.625 A325N	0	0.625 A325X	0	0.625 A325X	0	0.625 A325N	2	0.625 A325X	0
T9 61.000-40.667	Flange	1.000 A325N	8	0.625 A325N	3	0.625 A325N	0	0.625 A325X	0	0.625 A325X	0	0.625 A325N	2	0.625 A325X	0
T10 40.667-20.333	Flange	1.000 A325N	8	0.750 A325N	3	0.625 A325N	0	0.625 A325X	0	0.625 A325X	0	0.750 A325N	2	0.625 A325X	0
T11 20.333-0.000	Flange	1.000 A354-BC	0	0.750 A325N	3	0.625 A325N	0	0.625 A325X	0	0.625 A325X	0	0.750 A325N	2	0.625 A325X	0

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Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight klf
LDF4-50A(1/2") (E)	A	No	Ar (CaAa)	51.000 - 0.000	0.000	0.46	1	1	0.630	0.630		0.000
HB114-1-08U 4-M5J(1 1/4") (E)	A	No	Ar (CaAa)	199.000 - 0.000	0.000	0.42	4	4	0.850 0.750	1.540		0.001
Feedline Ladder (Af) (E) *_*_*_*	A	No	Af (CaAa)	199.000 - 0.000	0.000	-0.43	1	1	3.000	3.000		0.008
LDF5-50A(7/8") (E)	A	No	Ar (CaAa)	145.000 - 0.000	0.000	-0.4	5	5	1.000	1.090		0.000
LDF5-50A(7/8") (E)	A	No	Ar (CaAa)	147.000 - 145.000	0.000	-0.4	4	4	1.000	1.090		0.000
LDF5-50A(7/8") (Size/TIA)	A	No	Ar (CaAa)	162.000 - 147.000	0.000	-0.4	3	3	1.000	1.090		0.000
LDF5-50A(7/8") (E)	A	No	Ar (CaAa)	164.000 - 162.000	0.000	-0.4	2	2	1.000	1.090		0.000
LDF5-50A(7/8") (E)	A	No	Ar (CaAa)	167.000 - 164.000	0.000	-0.4	1	1	1.000	1.090		0.000
***** CR 50 1873(1-5/8") (E)	A	No	Ar (CaAa)	189.000 - 0.000	0.000	-0.44	12	6	0.850 0.750	1.980		0.001
3" Conduit (E)	A	No	Ar (CaAa)	189.000 - 0.000	0.000	-0.4	1	1	3.000	3.000		0.003
PWRT-608-S(13/16) (Inside Conduit)	A	No	Ar (CaAa)	189.000 - 0.000	0.000	-0.36	2	1	0.850 0.750	0.820		0.001
LDF2-50(3/8") (Quantity/App)	A	No	Ar (CaAa)	189.000 - 0.000	0.000	-0.36	2	2	0.440	0.440		0.000
WR-VG86ST- BRD(3/4) (P) *_*_*_*	A	No	Ar (CaAa)	189.000 - 0.000	0.000	-0.35	4	2	0.850 0.750	0.795		0.001
LDF1-50A(1/4") (E)	A	No	Ar (CaAa)	175.000 - 0.000	0.000	-0.47	4	2	0.345	0.345		0.000
Feedline Ladder (Af) (E) *_*_*_*	A	No	Af (CaAa)	189.000 - 0.000	0.000	-0.4	1	1	3.000	3.000		0.008
Safety Line 3/8 (E) *_*_*_*	A	No	Ar (CaAa)	212.625 - 0.000	0.000	0	1	1	0.375	0.375		0.000
LDF7-50A(1-5/8")	B	No	Ar (CaAa)	139.000 - 0.000	0.000	-0.45	12	6	0.850 0.750	1.980		0.001

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Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight klf
(5(1-1/4)E+7(1-5/8)E)												
LDF5-50A(7/8") (E)	B	No	Ar (CaAa)	139.000 - 0.000	0.000	-0.41	3	2	0.500	1.090		0.000
Feedline Ladder (Af) (E)	B	No	Af (CaAa)	139.000 - 0.000	0.000	-0.45	1	1	3.000	3.000		0.008
*_*_*_*_*												
Feedline Ladder (Af) (E-Empty/Photo)	B	No	Af (CaAa)	175.000 - 0.000	0.000	0.45	1	1	3.000	3.000		0.008
*_*_*_*_*												
AVA7-50(1-5/8)	C	No	Ar (CaAa)	208.000 - 0.000	0.000	0.45	10	6	0.500	2.010		0.001
(9(1-5/8)E+1(1-1/4)E)												
AVA7-50(1-5/8)	C	No	Ar (CaAa)	208.000 - 0.000	0.000	0.38	11	6	0.500	2.010		0.001
(9(1-5/8)E+2R(1-5/8))												
Feedline Ladder (Af) (E)	C	No	Af (CaAa)	208.000 - 0.000	0.000	0.43	1	1	3.000	3.000		0.008
Feedline Ladder (Af) (E-Empty/Photo)	C	No	Af (CaAa)	183.000 - 0.000	0.000	-0.45	1	1	3.000	3.000		0.008
*_*_*_*_*												
LDF4-50A(1/2") (Per TIA)	A	No	Ar (CaAa)	112.000 - 0.000	0.000	-0.49	1	1	0.300	0.630		0.000
LDF4-50A(1/2") (Per TIA)	C	No	Ar (CaAa)	212.625 - 0.000	0.000	0.49	1	1	0.300	0.630		0.000
*_*_*_*_*												

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	C _A A _A ft ² /ft	Weight klf
*_*_*_*_*							

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
T1	212.625-202.458	A	0.000	0.000	0.381	0.000	0.002
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	26.803	0.000	0.130
T2	202.458-182.292	A	0.000	0.000	44.532	0.000	0.384

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Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	96.832	0.000	0.475
T3	182.292-162.104	A	0.000	0.000	101.406	0.000	0.769
		B	0.000	0.000	6.448	0.000	0.108
		C	0.000	0.000	106.671	0.000	0.639
T4	162.104-141.896	A	0.000	0.000	109.242	0.000	0.792
		B	0.000	0.000	10.104	0.000	0.170
		C	0.000	0.000	106.781	0.000	0.640
T5	141.896-121.688	A	0.000	0.000	112.813	0.000	0.803
		B	0.000	0.000	65.556	0.000	0.503
		C	0.000	0.000	106.781	0.000	0.640
T6	121.688-101.479	A	0.000	0.000	113.476	0.000	0.804
		B	0.000	0.000	74.831	0.000	0.558
		C	0.000	0.000	106.781	0.000	0.640
T7	101.479-81.271	A	0.000	0.000	114.086	0.000	0.806
		B	0.000	0.000	74.831	0.000	0.558
		C	0.000	0.000	106.781	0.000	0.640
T8	81.271-61.000	A	0.000	0.000	114.439	0.000	0.808
		B	0.000	0.000	75.063	0.000	0.560
		C	0.000	0.000	107.111	0.000	0.642
T9	61.000-40.667	A	0.000	0.000	115.443	0.000	0.812
		B	0.000	0.000	75.294	0.000	0.562
		C	0.000	0.000	107.441	0.000	0.644
T10	40.667-20.333	A	0.000	0.000	116.073	0.000	0.814
		B	0.000	0.000	75.294	0.000	0.562
		C	0.000	0.000	107.441	0.000	0.644
T11	20.333-0.000	A	0.000	0.000	116.073	0.000	0.814
		B	0.000	0.000	75.294	0.000	0.562
		C	0.000	0.000	107.441	0.000	0.644

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
T1	212.625-202.458	A	1.803	0.000	0.000	4.047	0.000	0.051
		B		0.000	0.000	0.000	0.000	0.000
		C		0.000	0.000	36.645	0.000	0.673
T2	202.458-182.292	A	1.789	0.000	0.000	96.811	0.000	1.687
		B		0.000	0.000	0.000	0.000	0.000
		C		0.000	0.000	126.543	0.000	2.356
T3	182.292-162.104	A	1.769	0.000	0.000	211.763	0.000	3.610
		B		0.000	0.000	11.012	0.000	0.274
		C		0.000	0.000	142.881	0.000	2.752
T4	162.104-141.896	A	1.748	0.000	0.000	240.640	0.000	3.893
		B		0.000	0.000	17.167	0.000	0.425
		C		0.000	0.000	142.470	0.000	2.728
T5	141.896-121.688	A	1.723	0.000	0.000	246.812	0.000	3.959
		B		0.000	0.000	96.327	0.000	1.932
		C		0.000	0.000	141.840	0.000	2.698
T6	121.688-101.479	A	1.694	0.000	0.000	249.213	0.000	3.952
		B		0.000	0.000	108.964	0.000	2.157
		C		0.000	0.000	141.116	0.000	2.663
T7	101.479-81.271	A	1.661	0.000	0.000	250.818	0.000	3.926
		B		0.000	0.000	108.233	0.000	2.126
		C		0.000	0.000	140.263	0.000	2.623
T8	81.271-61.000	A	1.620	0.000	0.000	248.782	0.000	3.849
		B		0.000	0.000	107.669	0.000	2.094

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		Yathish	

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
T9	61.000-40.667	C		0.000	0.000	139.649	0.000	2.582
		A	1.566	0.000	0.000	249.764	0.000	3.792
		B		0.000	0.000	106.826	0.000	2.051
T10	40.667-20.333	C		0.000	0.000	138.711	0.000	2.526
		A	1.488	0.000	0.000	247.860	0.000	3.664
		B		0.000	0.000	105.114	0.000	1.980
T11	20.333-0.000	C		0.000	0.000	136.718	0.000	2.434
		A	1.333	0.000	0.000	236.629	0.000	3.338
		B		0.000	0.000	101.720	0.000	1.843
		C		0.000	0.000	132.766	0.000	2.257

Feed Line Center of Pressure

Section	Elevation ft	CP _x in	CP _z in	CP _x Ice in	CP _z Ice in
T1	212.625-202.458	-6.387	4.639	-4.498	2.779
T2	202.458-182.292	-8.208	4.481	-6.527	3.135
T3	182.292-162.104	-8.289	5.175	-5.715	4.210
T4	162.104-141.896	-9.514	6.106	-6.318	4.978
T5	141.896-121.688	-9.107	2.315	-6.462	2.512
T6	121.688-101.479	-10.294	2.004	-7.454	2.479
T7	101.479-81.271	-11.614	2.274	-8.583	2.883
T8	81.271-61.000	-12.756	2.493	-9.547	3.186
T9	61.000-40.667	-14.143	2.684	-10.577	3.224
T10	40.667-20.333	-15.461	2.855	-11.532	3.211
T11	20.333-0.000	-16.766	3.092	-12.713	3.479

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
T1	22	Safety Line 3/8	202.46 - 212.63	0.6000	0.6000
T1	33	AVA7-50(1-5/8)	202.46 - 208.00	0.6000	0.6000
T1	34	AVA7-50(1-5/8)	202.46 - 208.00	0.6000	0.6000
T1	36	Feedline Ladder (Af)	202.46 - 208.00	0.6000	0.6000
T1	41	LDF4-50A(1/2")	202.46 - 212.63	0.6000	0.6000
T2	2	HB114-1-08U4-M5J(1 1/4")	182.29 - 199.00	0.6000	0.6000
T2	3	Feedline Ladder (Af)	182.29 - 199.00	0.6000	0.6000
T2	13	CR 50 1873(1-5/8")	182.29 - 189.00	0.6000	0.6000
T2	14	3" Conduit	182.29 - 189.00	0.6000	0.6000
T2	15	PWRT-608-S(13/16)	182.29 -	0.0000	0.0000

tnxTower

B+T Group
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Client
Crown Castle
Designed by
Yathish

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K_a No Ice	K_a Ice
T2	16	LDF2-50(3/8")	189.00 182.29 - 189.00	0.6000	0.6000
T2	17	WR-VG86ST-BRD(3/4)	182.29 - 189.00	0.6000	0.6000
T2	20	Feedline Ladder (Af)	182.29 - 189.00	0.6000	0.6000
T2	22	Safety Line 3/8	182.29 - 202.46	0.6000	0.6000
T2	33	AVA7-50(1-5/8)	182.29 - 202.46	0.6000	0.6000
T2	34	AVA7-50(1-5/8)	182.29 - 202.46	0.6000	0.6000
T2	36	Feedline Ladder (Af)	182.29 - 202.46	0.6000	0.6000
T2	38	Feedline Ladder (Af)	182.29 - 183.00	0.6000	0.6000
T2	41	LDF4-50A(1/2")	182.29 - 202.46	0.6000	0.6000
T3	2	HB114-1-08U4-M5J(1 1/4")	162.10 - 182.29	0.6000	0.6000
T3	3	Feedline Ladder (Af)	162.10 - 182.29	0.6000	0.6000
T3	9	LDF5-50A(7/8")	162.10 - 164.00	0.6000	0.6000
T3	10	LDF5-50A(7/8")	164.00 - 167.00	0.6000	0.6000
T3	13	CR 50 1873(1-5/8")	162.10 - 182.29	0.6000	0.6000
T3	14	3" Conduit	162.10 - 182.29	0.6000	0.6000
T3	15	PWRT-608-S(13/16)	162.10 - 182.29	0.0000	0.0000
T3	16	LDF2-50(3/8")	162.10 - 182.29	0.6000	0.6000
T3	17	WR-VG86ST-BRD(3/4)	162.10 - 182.29	0.6000	0.6000
T3	19	LDF1-50A(1/4")	162.10 - 175.00	0.6000	0.6000
T3	20	Feedline Ladder (Af)	162.10 - 182.29	0.6000	0.6000
T3	22	Safety Line 3/8	162.10 - 182.29	0.6000	0.6000
T3	30	Feedline Ladder (Af)	162.10 - 175.00	0.6000	0.6000
T3	33	AVA7-50(1-5/8)	162.10 - 182.29	0.6000	0.6000
T3	34	AVA7-50(1-5/8)	162.10 - 182.29	0.6000	0.6000
T3	36	Feedline Ladder (Af)	162.10 - 182.29	0.6000	0.6000
T3	38	Feedline Ladder (Af)	162.10 - 182.29	0.6000	0.6000
T3	41	LDF4-50A(1/2")	162.10 - 182.29	0.6000	0.6000
T4	2	HB114-1-08U4-M5J(1 1/4")	141.90 - 162.10	0.6000	0.6000
T4	3	Feedline Ladder (Af)	141.90 - 162.10	0.6000	0.6000
T4	6	LDF5-50A(7/8")	141.90 - 145.00	0.6000	0.6000
T4	7	LDF5-50A(7/8")	145.00 -	0.6000	0.6000

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Client Crown Castle	Designed by Yathish

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K_a No Ice	K_a Ice
T4	8	LDF5-50A(7/8")	147.00 147.00 - 162.00	0.6000	0.6000
T4	9	LDF5-50A(7/8")	162.00 - 162.10	0.6000	0.6000
T4	13	CR 50 1873(1-5/8")	141.90 - 162.10	0.6000	0.6000
T4	14	3" Conduit	141.90 - 162.10	0.6000	0.6000
T4	15	PWRT-608-S(13/16)	141.90 - 162.10	0.0000	0.0000
T4	16	LDF2-50(3/8")	141.90 - 162.10	0.6000	0.6000
T4	17	WR-VG86ST-BRD(3/4)	141.90 - 162.10	0.6000	0.6000
T4	19	LDF1-50A(1/4")	141.90 - 162.10	0.6000	0.6000
T4	20	Feedline Ladder (Af)	141.90 - 162.10	0.6000	0.6000
T4	22	Safety Line 3/8	141.90 - 162.10	0.6000	0.6000
T4	30	Feedline Ladder (Af)	141.90 - 162.10	0.6000	0.6000
T4	33	AVA7-50(1-5/8)	141.90 - 162.10	0.6000	0.6000
T4	34	AVA7-50(1-5/8)	141.90 - 162.10	0.6000	0.6000
T4	36	Feedline Ladder (Af)	141.90 - 162.10	0.6000	0.6000
T4	38	Feedline Ladder (Af)	141.90 - 162.10	0.6000	0.6000
T4	41	LDF4-50A(1/2")	141.90 - 162.10	0.6000	0.6000
T5	2	HB114-1-08U4-M5J(1 1/4")	121.69 - 141.90	0.6000	0.6000
T5	3	Feedline Ladder (Af)	121.69 - 141.90	0.6000	0.6000
T5	6	LDF5-50A(7/8")	121.69 - 141.90	0.6000	0.6000
T5	13	CR 50 1873(1-5/8")	121.69 - 141.90	0.6000	0.6000
T5	14	3" Conduit	121.69 - 141.90	0.6000	0.6000
T5	15	PWRT-608-S(13/16)	121.69 - 141.90	0.0000	0.0000
T5	16	LDF2-50(3/8")	121.69 - 141.90	0.6000	0.6000
T5	17	WR-VG86ST-BRD(3/4)	121.69 - 141.90	0.6000	0.6000
T5	19	LDF1-50A(1/4")	121.69 - 141.90	0.6000	0.6000
T5	20	Feedline Ladder (Af)	121.69 - 141.90	0.6000	0.6000
T5	22	Safety Line 3/8	121.69 - 141.90	0.6000	0.6000
T5	25	LDF7-50A(1-5/8")	121.69 - 139.00	0.6000	0.6000
T5	26	LDF5-50A(7/8")	121.69 - 139.00	0.6000	0.6000
T5	27	Feedline Ladder (Af)	121.69 - 139.00	0.6000	0.6000
T5	30	Feedline Ladder (Af)	121.69 -	0.6000	0.6000

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Client Crown Castle	Designed by Yathish

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
T5	33	AVA7-50(1-5/8)	141.90 121.69 - 141.90	0.6000	0.6000
T5	34	AVA7-50(1-5/8)	121.69 - 141.90	0.6000	0.6000
T5	36	Feedline Ladder (Af)	121.69 - 141.90	0.6000	0.6000
T5	38	Feedline Ladder (Af)	121.69 - 141.90	0.6000	0.6000
T5	41	LDF4-50A(1/2")	121.69 - 141.90	0.6000	0.6000
T6	2	HB114-1-08U4-M5J(1 1/4")	101.48 - 121.69	0.6000	0.6000
T6	3	Feedline Ladder (Af)	101.48 - 121.69	0.6000	0.6000
T6	6	LDF5-50A(7/8")	101.48 - 121.69	0.6000	0.6000
T6	13	CR 50 1873(1-5/8")	101.48 - 121.69	0.6000	0.6000
T6	14	3" Conduit	101.48 - 121.69	0.6000	0.6000
T6	15	PWRT-608-S(13/16)	101.48 - 121.69	0.0000	0.0000
T6	16	LDF2-50(3/8")	101.48 - 121.69	0.6000	0.6000
T6	17	WR-VG86ST-BRD(3/4)	101.48 - 121.69	0.6000	0.6000
T6	19	LDF1-50A(1/4")	101.48 - 121.69	0.6000	0.6000
T6	20	Feedline Ladder (Af)	101.48 - 121.69	0.6000	0.6000
T6	22	Safety Line 3/8	101.48 - 121.69	0.6000	0.6000
T6	25	LDF7-50A(1-5/8")	101.48 - 121.69	0.6000	0.6000
T6	26	LDF5-50A(7/8")	101.48 - 121.69	0.6000	0.6000
T6	27	Feedline Ladder (Af)	101.48 - 121.69	0.6000	0.6000
T6	30	Feedline Ladder (Af)	101.48 - 121.69	0.6000	0.6000
T6	33	AVA7-50(1-5/8)	101.48 - 121.69	0.6000	0.6000
T6	34	AVA7-50(1-5/8)	101.48 - 121.69	0.6000	0.6000
T6	36	Feedline Ladder (Af)	101.48 - 121.69	0.6000	0.6000
T6	38	Feedline Ladder (Af)	101.48 - 121.69	0.6000	0.6000
T6	40	LDF4-50A(1/2")	101.48 - 112.00	0.6000	0.6000
T6	41	LDF4-50A(1/2")	101.48 - 121.69	0.6000	0.6000
T7	2	HB114-1-08U4-M5J(1 1/4")	81.27 - 101.48	0.6000	0.6000
T7	3	Feedline Ladder (Af)	81.27 - 101.48	0.6000	0.6000
T7	6	LDF5-50A(7/8")	81.27 - 101.48	0.6000	0.6000
T7	13	CR 50 1873(1-5/8")	81.27 - 101.48	0.6000	0.6000
T7	14	3" Conduit	81.27 - 101.48	0.6000	0.6000
T7	15	PWRT-608-S(13/16)	81.27 - 101.48	0.0000	0.0000
T7	16	LDF2-50(3/8")	81.27 - 101.48	0.6000	0.6000
T7	17	WR-VG86ST-BRD(3/4)	81.27 - 101.48	0.6000	0.6000
T7	19	LDF1-50A(1/4")	81.27 - 101.48	0.6000	0.6000

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Job	100083.002.01 - HRT 105 943201, CT (BU# 806363)	Page	15 of 44
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Client	Crown Castle	Designed by	Yathish

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K_a No Ice	K_a Ice
T7	20	Feedline Ladder (Af)	81.27 - 101.48	0.6000	0.6000
T7	22	Safety Line 3/8	81.27 - 101.48	0.6000	0.6000
T7	25	LDF7-50A(1-5/8")	81.27 - 101.48	0.6000	0.6000
T7	26	LDF5-50A(7/8")	81.27 - 101.48	0.6000	0.6000
T7	27	Feedline Ladder (Af)	81.27 - 101.48	0.6000	0.6000
T7	30	Feedline Ladder (Af)	81.27 - 101.48	0.6000	0.6000
T7	33	AVA7-50(1-5/8)	81.27 - 101.48	0.6000	0.6000
T7	34	AVA7-50(1-5/8)	81.27 - 101.48	0.6000	0.6000
T7	36	Feedline Ladder (Af)	81.27 - 101.48	0.6000	0.6000
T7	38	Feedline Ladder (Af)	81.27 - 101.48	0.6000	0.6000
T7	40	LDF4-50A(1/2")	81.27 - 101.48	0.6000	0.6000
T7	41	LDF4-50A(1/2")	81.27 - 101.48	0.6000	0.6000
T8	2	HB114-1-08U4-M5J(1 1/4")	61.00 - 81.27	0.6000	0.6000
T8	3	Feedline Ladder (Af)	61.00 - 81.27	0.6000	0.6000
T8	6	LDF5-50A(7/8")	61.00 - 81.27	0.6000	0.6000
T8	13	CR 50 1873(1-5/8")	61.00 - 81.27	0.6000	0.6000
T8	14	3" Conduit	61.00 - 81.27	0.6000	0.6000
T8	15	PWRT-608-S(13/16)	61.00 - 81.27	0.0000	0.0000
T8	16	LDF2-50(3/8")	61.00 - 81.27	0.6000	0.6000
T8	17	WR-VG86ST-BRD(3/4)	61.00 - 81.27	0.6000	0.6000
T8	19	LDF1-50A(1/4")	61.00 - 81.27	0.6000	0.6000
T8	20	Feedline Ladder (Af)	61.00 - 81.27	0.6000	0.6000
T8	22	Safety Line 3/8	61.00 - 81.27	0.6000	0.6000
T8	25	LDF7-50A(1-5/8")	61.00 - 81.27	0.6000	0.6000
T8	26	LDF5-50A(7/8")	61.00 - 81.27	0.6000	0.6000
T8	27	Feedline Ladder (Af)	61.00 - 81.27	0.6000	0.6000
T8	30	Feedline Ladder (Af)	61.00 - 81.27	0.6000	0.6000
T8	33	AVA7-50(1-5/8)	61.00 - 81.27	0.6000	0.6000
T8	34	AVA7-50(1-5/8)	61.00 - 81.27	0.6000	0.6000
T8	36	Feedline Ladder (Af)	61.00 - 81.27	0.6000	0.6000
T8	38	Feedline Ladder (Af)	61.00 - 81.27	0.6000	0.6000
T8	40	LDF4-50A(1/2")	61.00 - 81.27	0.6000	0.6000
T8	41	LDF4-50A(1/2")	61.00 - 81.27	0.6000	0.6000
T9	1	LDF4-50A(1/2")	40.67 - 51.00	0.6000	0.6000
T9	2	HB114-1-08U4-M5J(1 1/4")	40.67 - 61.00	0.6000	0.6000
T9	3	Feedline Ladder (Af)	40.67 - 61.00	0.6000	0.6000
T9	6	LDF5-50A(7/8")	40.67 - 61.00	0.6000	0.6000
T9	13	CR 50 1873(1-5/8")	40.67 - 61.00	0.6000	0.6000
T9	14	3" Conduit	40.67 - 61.00	0.6000	0.6000
T9	15	PWRT-608-S(13/16)	40.67 - 61.00	0.0000	0.0000
T9	16	LDF2-50(3/8")	40.67 - 61.00	0.6000	0.6000
T9	17	WR-VG86ST-BRD(3/4)	40.67 - 61.00	0.6000	0.6000
T9	19	LDF1-50A(1/4")	40.67 - 61.00	0.6000	0.6000
T9	20	Feedline Ladder (Af)	40.67 - 61.00	0.6000	0.6000
T9	22	Safety Line 3/8	40.67 - 61.00	0.6000	0.6000
T9	25	LDF7-50A(1-5/8")	40.67 - 61.00	0.6000	0.6000
T9	26	LDF5-50A(7/8")	40.67 - 61.00	0.6000	0.6000
T9	27	Feedline Ladder (Af)	40.67 - 61.00	0.6000	0.6000
T9	30	Feedline Ladder (Af)	40.67 - 61.00	0.6000	0.6000
T9	33	AVA7-50(1-5/8)	40.67 - 61.00	0.6000	0.6000
T9	34	AVA7-50(1-5/8)	40.67 - 61.00	0.6000	0.6000
T9	36	Feedline Ladder (Af)	40.67 - 61.00	0.6000	0.6000
T9	38	Feedline Ladder (Af)	40.67 - 61.00	0.6000	0.6000
T9	40	LDF4-50A(1/2")	40.67 - 61.00	0.6000	0.6000
T9	41	LDF4-50A(1/2")	40.67 - 61.00	0.6000	0.6000
T10	1	LDF4-50A(1/2")	20.33 - 40.67	0.6000	0.6000
T10	2	HB114-1-08U4-M5J(1 1/4")	20.33 - 40.67	0.6000	0.6000
T10	3	Feedline Ladder (Af)	20.33 - 40.67	0.6000	0.6000
T10	6	LDF5-50A(7/8")	20.33 - 40.67	0.6000	0.6000
T10	13	CR 50 1873(1-5/8")	20.33 - 40.67	0.6000	0.6000
T10	14	3" Conduit	20.33 - 40.67	0.6000	0.6000
T10	15	PWRT-608-S(13/16)	20.33 - 40.67	0.0000	0.0000

<p>tnxTower</p> <p>B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265</p>	<p>Job</p> <p>100083.002.01 - HRT 105 943201, CT (BU# 806363)</p>	<p>Page</p> <p>16 of 44</p>
	<p>Project</p>	<p>Date</p> <p>16:57:31 01/25/18</p>
	<p>Client</p> <p>Crown Castle</p>	<p>Designed by</p> <p>Yathish</p>

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
T10	16	LDF2-50(3/8")	20.33 - 40.67	0.6000	0.6000
T10	17	WR-VG86ST-BRD(3/4)	20.33 - 40.67	0.6000	0.6000
T10	19	LDF1-50A(1/4")	20.33 - 40.67	0.6000	0.6000
T10	20	Feedline Ladder (Af)	20.33 - 40.67	0.6000	0.6000
T10	22	Safety Line 3/8	20.33 - 40.67	0.6000	0.6000
T10	25	LDF7-50A(1-5/8")	20.33 - 40.67	0.6000	0.6000
T10	26	LDF5-50A(7/8")	20.33 - 40.67	0.6000	0.6000
T10	27	Feedline Ladder (Af)	20.33 - 40.67	0.6000	0.6000
T10	30	Feedline Ladder (Af)	20.33 - 40.67	0.6000	0.6000
T10	33	AVA7-50(1-5/8)	20.33 - 40.67	0.6000	0.6000
T10	34	AVA7-50(1-5/8)	20.33 - 40.67	0.6000	0.6000
T10	36	Feedline Ladder (Af)	20.33 - 40.67	0.6000	0.6000
T10	38	Feedline Ladder (Af)	20.33 - 40.67	0.6000	0.6000
T10	40	LDF4-50A(1/2")	20.33 - 40.67	0.6000	0.6000
T10	41	LDF4-50A(1/2")	20.33 - 40.67	0.6000	0.6000
T11	1	LDF4-50A(1/2")	0.00 - 20.33	0.6000	0.6000
T11	2	HB114-1-08U4-M5J(1 1/4")	0.00 - 20.33	0.6000	0.6000
T11	3	Feedline Ladder (Af)	0.00 - 20.33	0.6000	0.6000
T11	6	LDF5-50A(7/8")	0.00 - 20.33	0.6000	0.6000
T11	13	CR 50 1873(1-5/8")	0.00 - 20.33	0.6000	0.6000
T11	14	3" Conduit	0.00 - 20.33	0.6000	0.6000
T11	15	PWRT-608-S(13/16)	0.00 - 20.33	0.0000	0.0000
T11	16	LDF2-50(3/8")	0.00 - 20.33	0.6000	0.6000
T11	17	WR-VG86ST-BRD(3/4)	0.00 - 20.33	0.6000	0.6000
T11	19	LDF1-50A(1/4")	0.00 - 20.33	0.6000	0.6000
T11	20	Feedline Ladder (Af)	0.00 - 20.33	0.6000	0.6000
T11	22	Safety Line 3/8	0.00 - 20.33	0.6000	0.6000
T11	25	LDF7-50A(1-5/8")	0.00 - 20.33	0.6000	0.6000
T11	26	LDF5-50A(7/8")	0.00 - 20.33	0.6000	0.6000
T11	27	Feedline Ladder (Af)	0.00 - 20.33	0.6000	0.6000
T11	30	Feedline Ladder (Af)	0.00 - 20.33	0.6000	0.6000
T11	33	AVA7-50(1-5/8)	0.00 - 20.33	0.6000	0.6000
T11	34	AVA7-50(1-5/8)	0.00 - 20.33	0.6000	0.6000
T11	36	Feedline Ladder (Af)	0.00 - 20.33	0.6000	0.6000
T11	38	Feedline Ladder (Af)	0.00 - 20.33	0.6000	0.6000
T11	40	LDF4-50A(1/2")	0.00 - 20.33	0.6000	0.6000
T11	41	LDF4-50A(1/2")	0.00 - 20.33	0.6000	0.6000

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment °	Placement ft	C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K
Lightning Rod 5/8" x 6' (E)	A	From Leg	0.000	0.000	213.625	No Ice	0.375	0.033
			0.000			1/2" Ice	0.989	0.037
			3.000			1" Ice	1.619	0.045
Climb Leg Extension (E)	A	From Leg	0.000	0.000	212.625	No Ice	1.473	0.025
			0.000			1/2" Ice	1.803	0.038
			2.000			1" Ice	2.119	0.054
Flash Beacon Lighting (E)	B	From Leg	0.000	0.000	216.625	No Ice	2.700	0.050
			0.000			1/2" Ice	3.100	0.070

tnxTower

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Client
Crown Castle
Designed by
Yathish

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight	
			Horz	Lateral						
			ft	ft	°	ft	ft ²	ft ²	K	
4' x 2" Pipe Mount (E-For Beacon)	B	From Leg	0.500		0.000	212.625	1" Ice	3.500	3.500	0.090
			0.000				No Ice	0.785	0.785	0.029
			0.000				1/2" Ice	1.028	1.028	0.035
			2.000				1" Ice	1.281	1.281	0.044
Side Lighting (E)	A	From Leg	0.500		0.000	112.000	No Ice	0.113	0.113	0.005
			0.000				1/2" Ice	0.170	0.170	0.007
			0.000				1" Ice	0.233	0.233	0.010
			0.500				No Ice	0.113	0.113	0.005
Side Lighting (E)	B	From Leg	0.000		0.000	112.000	1/2" Ice	0.170	0.170	0.007
			0.000				1" Ice	0.233	0.233	0.010
			0.500				No Ice	0.113	0.113	0.005
			0.000				1/2" Ice	0.170	0.170	0.007
Side Lighting (E)	C	From Leg	0.500		0.000	112.000	No Ice	0.113	0.113	0.005
			0.000				1/2" Ice	0.170	0.170	0.007
			0.000				1" Ice	0.233	0.233	0.010
			0.000				1" Ice	0.233	0.233	0.010
*_*_*_*										
(2) LPA-80080/6CF w/ Mount Pipe (E)	A	From Leg	4.000		0.000	208.000	No Ice	4.564	10.259	0.046
			0.000				1/2" Ice	5.105	11.427	0.113
			1.000				1" Ice	5.612	12.312	0.187
			4.000				No Ice	4.564	10.259	0.046
(2) LPA-80080/6CF w/ Mount Pipe (E)	B	From Leg	0.000		0.000	208.000	1/2" Ice	5.105	11.427	0.113
			1.000				1" Ice	5.612	12.312	0.187
			4.000				No Ice	4.564	10.259	0.046
			0.000				1/2" Ice	5.105	11.427	0.113
(2) LPA-80080/6CF w/ Mount Pipe (E)	C	From Leg	4.000		0.000	208.000	No Ice	4.564	10.259	0.046
			0.000				1/2" Ice	5.105	11.427	0.113
			1.000				1" Ice	5.612	12.312	0.187
			4.000				No Ice	4.800	2.000	0.044
DB-T1-6Z-8AB-0Z (E)	C	From Leg	0.000		0.000	208.000	1/2" Ice	5.070	2.193	0.080
			1.000				1" Ice	5.348	2.393	0.120
			4.000				No Ice	8.397	7.071	0.066
			0.000				1/2" Ice	8.960	8.260	0.135
(3) SBNHH-1D65B w/ Mount Pipe (R)	A	From Leg	1.000		0.000	208.000	1" Ice	9.490	9.170	0.212
			4.000				No Ice	8.397	7.071	0.066
			0.000				1/2" Ice	8.960	8.260	0.135
			1.000				1" Ice	9.490	9.170	0.212
(3) SBNHH-1D65B w/ Mount Pipe (R)	B	From Leg	4.000		0.000	208.000	No Ice	8.397	7.071	0.066
			0.000				1/2" Ice	8.960	8.260	0.135
			1.000				1" Ice	9.490	9.170	0.212
			4.000				No Ice	8.397	7.071	0.066
(3) SBNHH-1D65B w/ Mount Pipe (R)	C	From Leg	0.000		0.000	208.000	1/2" Ice	8.960	8.260	0.135
			1.000				1" Ice	9.490	9.170	0.212
			4.000				No Ice	8.397	7.071	0.066
			0.000				1/2" Ice	8.960	8.260	0.135
RRH2X60-PCS (R)	A	From Leg	4.000		0.000	208.000	No Ice	2.200	1.723	0.055
			0.000				1/2" Ice	2.393	1.901	0.075
			1.000				1" Ice	2.593	2.087	0.099
			4.000				No Ice	2.200	1.723	0.055
RRH2X60-PCS (R)	B	From Leg	0.000		0.000	208.000	1/2" Ice	2.393	1.901	0.075
			1.000				1" Ice	2.593	2.087	0.099
			4.000				No Ice	2.200	1.723	0.055
			0.000				1/2" Ice	2.393	1.901	0.075
RRH2X60-PCS (R)	C	From Leg	1.000		0.000	208.000	1" Ice	2.593	2.087	0.099
			4.000				No Ice	2.200	1.723	0.055
			0.000				1/2" Ice	2.393	1.901	0.075
			1.000				1" Ice	2.593	2.087	0.099
RRH2x60-700 (R)	A	From Leg	4.000		0.000	208.000	No Ice	3.500	1.816	0.060
			0.000				1/2" Ice	3.761	2.052	0.083
			1.000				1" Ice	4.029	2.289	0.109
			4.000				No Ice	3.500	1.816	0.060
RRH2x60-700 (R)	B	From Leg	0.000		0.000	208.000	1/2" Ice	3.761	2.052	0.083
			1.000				1" Ice	4.029	2.289	0.109
			4.000				No Ice	3.500	1.816	0.060
			0.000				1/2" Ice	3.761	2.052	0.083
RRH2x60-700 (R)	C	From Leg	1.000		0.000	208.000	1" Ice	4.029	2.289	0.109
			4.000				No Ice	3.500	1.816	0.060
			0.000				1/2" Ice	3.761	2.052	0.083
			1.000				1" Ice	4.029	2.289	0.109
DB-B1-6C-12AB-0Z (R)	A	From Leg	4.000		0.000	208.000	No Ice	3.364	2.192	0.021
			0.000				1/2" Ice	3.597	2.395	0.050
			1.000				1" Ice	3.838	2.606	0.082
			4.000				No Ice	3.500	1.816	0.060
RRH2X60-AWS	A	From Leg	4.000	0.000	208.000	No Ice	3.500	1.816	0.060	

tnxTower

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Job	100083.002.01 - HRT 105 943201, CT (BU# 806363)	Page	18 of 44
Project		Date	16:57:31 01/25/18
Client	Crown Castle	Designed by	Yathish

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight
			Horz	Lateral					
(R)			0.000						
			1.000			1/2" Ice	3.761	2.052	0.083
RRH2X60-AWS	B	From Leg	4.000	0.000	208.000	1" Ice	4.029	2.289	0.109
(R)			0.000			No Ice	3.500	1.816	0.060
			1.000			1/2" Ice	3.761	2.052	0.083
RRH2X60-AWS	C	From Leg	4.000	0.000	208.000	1" Ice	4.029	2.289	0.109
(R)			0.000			No Ice	3.500	1.816	0.060
			1.000			1/2" Ice	3.761	2.052	0.083
Sector Mount [SM 510-3]	C	None		0.000	208.000	1" Ice	4.029	2.289	0.109
(E)						No Ice	40.100	40.100	2.396
						1/2" Ice	57.330	57.330	3.089
						1" Ice	74.560	74.560	3.782
*_*_*_*									
APXVSPP18-C-A20 w/	A	From Leg	4.000	0.000	199.000	No Ice	8.262	6.946	0.083
Mount Pipe			0.000			1/2" Ice	8.822	8.127	0.151
(E)			-1.000			1" Ice	9.346	9.021	0.227
APXVSPP18-C-A20 w/	B	From Leg	4.000	0.000	199.000	No Ice	8.262	6.946	0.083
Mount Pipe			0.000			1/2" Ice	8.822	8.127	0.151
(E)			-1.000			1" Ice	9.346	9.021	0.227
APXVSPP18-C-A20 w/	C	From Leg	4.000	0.000	199.000	No Ice	8.262	6.946	0.083
Mount Pipe			0.000			1/2" Ice	8.822	8.127	0.151
(E)			-1.000			1" Ice	9.346	9.021	0.227
APXVTM14-C-120 w/	A	From Leg	4.000	0.000	199.000	No Ice	6.580	4.959	0.077
Mount Pipe			0.000			1/2" Ice	7.031	5.754	0.131
(E)			-1.000			1" Ice	7.473	6.472	0.193
APXVTM14-C-120 w/	B	From Leg	4.000	0.000	199.000	No Ice	6.580	4.959	0.077
Mount Pipe			0.000			1/2" Ice	7.031	5.754	0.131
(E)			-1.000			1" Ice	7.473	6.472	0.193
APXVTM14-C-120 w/	C	From Leg	4.000	0.000	199.000	No Ice	6.580	4.959	0.077
Mount Pipe			0.000			1/2" Ice	7.031	5.754	0.131
(E)			-1.000			1" Ice	7.473	6.472	0.193
800MHz 2X50W RRH	A	From Leg	4.000	0.000	199.000	No Ice	2.058	1.932	0.064
W/FILTER			0.000			1/2" Ice	2.240	2.109	0.086
(E)			-1.000			1" Ice	2.429	2.293	0.111
800MHz 2X50W RRH	B	From Leg	4.000	0.000	199.000	No Ice	2.058	1.932	0.064
W/FILTER			0.000			1/2" Ice	2.240	2.109	0.086
(E)			-1.000			1" Ice	2.429	2.293	0.111
800MHz 2X50W RRH	C	From Leg	4.000	0.000	199.000	No Ice	2.058	1.932	0.064
W/FILTER			0.000			1/2" Ice	2.240	2.109	0.086
(E)			-1.000			1" Ice	2.429	2.293	0.111
1900MHz RRH (65MHz)	A	From Leg	4.000	0.000	199.000	No Ice	2.313	2.375	0.060
(E)			0.000			1/2" Ice	2.517	2.581	0.084
			-1.000			1" Ice	2.728	2.794	0.111
1900MHz RRH (65MHz)	B	From Leg	4.000	0.000	199.000	No Ice	2.313	2.375	0.060
(E)			0.000			1/2" Ice	2.517	2.581	0.084
			-1.000			1" Ice	2.728	2.794	0.111
1900MHz RRH (65MHz)	C	From Leg	4.000	0.000	199.000	No Ice	2.313	2.375	0.060
(E)			0.000			1/2" Ice	2.517	2.581	0.084
			-1.000			1" Ice	2.728	2.794	0.111
TD-RRH8x20-25	A	From Leg	4.000	0.000	199.000	No Ice	4.045	1.535	0.070
(E)			0.000			1/2" Ice	4.298	1.714	0.097
			-1.000			1" Ice	4.557	1.901	0.128
TD-RRH8x20-25	B	From Leg	4.000	0.000	199.000	No Ice	4.045	1.535	0.070
(E)			0.000			1/2" Ice	4.298	1.714	0.097
			-1.000			1" Ice	4.557	1.901	0.128
TD-RRH8x20-25	C	From Leg	4.000	0.000	199.000	No Ice	4.045	1.535	0.070
(E)			0.000			1/2" Ice	4.298	1.714	0.097
			-1.000			1" Ice	4.557	1.901	0.128

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
(3) 4' x 2" Pipe Mount (E)	A	From Leg	4.000 0.000 -1.000	0.000	199.000	No Ice 0.785 1/2" Ice 1.028 1" Ice 1.281	0.785 1.028 1.281	0.029 0.035 0.044
(3) 4' x 2" Pipe Mount (E)	B	From Leg	4.000 0.000 -1.000	0.000	199.000	No Ice 0.785 1/2" Ice 1.028 1" Ice 1.281	0.785 1.028 1.281	0.029 0.035 0.044
(3) 4' x 2" Pipe Mount (E)	C	From Leg	4.000 0.000 -1.000	0.000	199.000	No Ice 0.785 1/2" Ice 1.028 1" Ice 1.281	0.785 1.028 1.281	0.029 0.035 0.044
Sector Mount [SM 504-3] (14'/TIA)	C	None		0.000	199.000	No Ice 34.250 1/2" Ice 48.980 1" Ice 63.710	34.250 48.980 63.710	1.708 2.286 2.864
*_*_*_**								
7770.00 w/ Mount Pipe (E)	A	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 5.746 1/2" Ice 6.179 1" Ice 6.607	4.254 5.014 5.711	0.055 0.103 0.157
7770.00 w/ Mount Pipe (E)	B	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 5.746 1/2" Ice 6.179 1" Ice 6.607	4.254 5.014 5.711	0.055 0.103 0.157
7770.00 w/ Mount Pipe (E)	C	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 5.746 1/2" Ice 6.179 1" Ice 6.607	4.254 5.014 5.711	0.055 0.103 0.157
(2) SBNHH-1D65A w/ Mount Pipe (E)	A	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 5.954 1/2" Ice 6.390 1" Ice 6.820	5.190 5.961 6.658	0.061 0.114 0.174
(2) SBNHH-1D65A w/ Mount Pipe (E)	B	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 5.954 1/2" Ice 6.390 1" Ice 6.820	5.190 5.961 6.658	0.061 0.114 0.174
(2) SBNHH-1D65A w/ Mount Pipe (E)	C	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 5.954 1/2" Ice 6.390 1" Ice 6.820	5.190 5.961 6.658	0.061 0.114 0.174
(2) DUAL BAND 800/1900 FULL BAND MASTHEAD (E)	A	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 1.328 1/2" Ice 1.473 1" Ice 1.625	0.693 0.808 0.930	0.027 0.038 0.052
(2) DUAL BAND 800/1900 FULL BAND MASTHEAD (E)	B	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 1.328 1/2" Ice 1.473 1" Ice 1.625	0.693 0.808 0.930	0.027 0.038 0.052
(2) DUAL BAND 800/1900 FULL BAND MASTHEAD (E)	C	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 1.328 1/2" Ice 1.473 1" Ice 1.625	0.693 0.808 0.930	0.027 0.038 0.052
7020.00 (E)	A	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 0.102 1/2" Ice 0.147 1" Ice 0.199	0.175 0.239 0.311	0.002 0.005 0.009
7020.00 (E)	B	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 0.102 1/2" Ice 0.147 1" Ice 0.199	0.175 0.239 0.311	0.002 0.005 0.009
7020.00 (E)	C	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 0.102 1/2" Ice 0.147 1" Ice 0.199	0.175 0.239 0.311	0.002 0.005 0.009
RRUS 11 (E)	A	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 2.784 1/2" Ice 2.992 1" Ice 3.207	1.187 1.334 1.490	0.048 0.068 0.092
RRUS 11 (E)	B	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 2.784 1/2" Ice 2.992 1" Ice 3.207	1.187 1.334 1.490	0.048 0.068 0.092
RRUS 11 (E)	C	From Leg	4.000 0.000 1.000	0.000	189.000	No Ice 2.784 1/2" Ice 2.992 1" Ice 3.207	1.187 1.334 1.490	0.048 0.068 0.092

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Project		Date	16:57:31 01/25/18
Client	Crown Castle	Designed by	Yathish

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight	
			Horz	Lateral						
			ft	ft	°	ft	ft ²	ft ²	K	
DC6-48-60-18-8F (Hor.Off./Photo)	A	From Leg	1.000		0.000	189.000	1" Ice	3.207	1.490	0.092
			1.000				No Ice	0.917	0.917	0.019
			0.000				1/2" Ice	1.458	1.458	0.037
DC6-48-60-18-8F (Hor.Off./Photo)	C	From Leg	1.000		0.000	189.000	1" Ice	1.643	1.643	0.057
			1.000				No Ice	0.917	0.917	0.019
			0.000				1/2" Ice	1.458	1.458	0.037
SBNHH-1D65A w/ Mount Pipe (P)	A	From Leg	1.000		0.000	189.000	1" Ice	1.643	1.643	0.057
			1.000				No Ice	5.954	5.190	0.061
			0.000				1/2" Ice	6.390	5.961	0.114
SBNHH-1D65A w/ Mount Pipe (P)	B	From Leg	1.000		0.000	189.000	1" Ice	6.820	6.658	0.174
			1.000				No Ice	5.954	5.190	0.061
			0.000				1/2" Ice	6.390	5.961	0.114
SBNHH-1D65A w/ Mount Pipe (P)	C	From Leg	1.000		0.000	189.000	1" Ice	6.820	6.658	0.174
			1.000				No Ice	5.954	5.190	0.061
			0.000				1/2" Ice	6.390	5.961	0.114
RRUS 32 (P)	A	From Leg	1.000		0.000	189.000	1" Ice	6.820	6.658	0.174
			1.000				No Ice	2.857	1.777	0.055
			0.000				1/2" Ice	3.083	1.968	0.077
RRUS 32 (P)	B	From Leg	1.000		0.000	189.000	1" Ice	3.316	2.166	0.103
			1.000				No Ice	2.857	1.777	0.055
			0.000				1/2" Ice	3.083	1.968	0.077
RRUS 32 (P)	C	From Leg	1.000		0.000	189.000	1" Ice	3.316	2.166	0.103
			1.000				No Ice	2.857	1.777	0.055
			0.000				1/2" Ice	3.083	1.968	0.077
RRUS 32 B66 (P)	A	From Leg	1.000		0.000	189.000	1" Ice	3.316	2.166	0.103
			1.000				No Ice	2.743	1.668	0.053
			0.000				1/2" Ice	2.965	1.855	0.074
RRUS 32 B66 (P)	B	From Leg	1.000		0.000	189.000	1" Ice	3.194	2.049	0.098
			1.000				No Ice	2.743	1.668	0.053
			0.000				1/2" Ice	2.965	1.855	0.074
RRUS 32 B66 (P)	C	From Leg	1.000		0.000	189.000	1" Ice	3.194	2.049	0.098
			1.000				No Ice	2.743	1.668	0.053
			0.000				1/2" Ice	2.965	1.855	0.074
RRUS 32 B2 (P)	A	From Leg	1.000		0.000	189.000	1" Ice	3.194	2.049	0.098
			1.000				No Ice	2.731	1.668	0.053
			0.000				1/2" Ice	2.953	1.855	0.074
RRUS 32 B2 (P)	B	From Leg	1.000		0.000	189.000	1" Ice	3.182	2.049	0.098
			1.000				No Ice	2.731	1.668	0.053
			0.000				1/2" Ice	2.953	1.855	0.074
RRUS 32 B2 (P)	C	From Leg	1.000		0.000	189.000	1" Ice	3.182	2.049	0.098
			1.000				No Ice	2.731	1.668	0.053
			0.000				1/2" Ice	2.953	1.855	0.074
Sector Mount [SM 602-3] (E)	C	None	1.000		0.000	189.000	1" Ice	3.182	2.049	0.098
			1.000				No Ice	33.110	33.110	1.541
			1.000				1/2" Ice	44.900	44.900	2.159
*_*_*_*										
APXV18-206517LS w/ Mount Pipe (Leg Mounted/Photo)	A	From Leg	1.000		0.000	183.000	1" Ice	5.286	4.667	0.053
			1.000				No Ice	5.839	5.822	0.097
			0.000				1" Ice	6.357	6.687	0.149
APXV18-206517LS w/ Mount Pipe (Leg Mounted/Photo)	B	From Leg	1.000		0.000	183.000	1" Ice	5.286	4.667	0.053
			1.000				No Ice	5.839	5.822	0.097
			0.000				1" Ice	6.357	6.687	0.149
APXV18-206517LS w/ Mount Pipe (Leg Mounted/Photo)	C	From Leg	1.000		0.000	183.000	1" Ice	5.286	4.667	0.053
			1.000				No Ice	5.839	5.822	0.097
			0.000				1" Ice	6.357	6.687	0.149
*_*_*_*										

tnxTower B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job	Page		
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	Project	Date	16:57:31 01/25/18	
Client	Crown Castle		Designed by	Yathish

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight
			Horz Lateral	Vert					
ERICSSON AIR 21 B2A B4P w/ Mount Pipe (E)	A	From Leg	2.000	0.000	0.000	139.000	No Ice 6.329	5.642	0.112
			0.000				1/2" Ice 6.775	6.426	0.169
			1.000				1" Ice 7.214	7.131	0.233
ERICSSON AIR 21 B2A B4P w/ Mount Pipe (E)	B	From Leg	2.000	0.000	0.000	139.000	No Ice 6.329	5.642	0.112
			0.000				1/2" Ice 6.775	6.426	0.169
			1.000				1" Ice 7.214	7.131	0.233
ERICSSON AIR 21 B2A B4P w/ Mount Pipe (E)	C	From Leg	2.000	0.000	0.000	139.000	No Ice 6.329	5.642	0.112
			0.000				1/2" Ice 6.775	6.426	0.169
			1.000				1" Ice 7.214	7.131	0.233
ERICSSON AIR 21 B4A B2P w/ Mount Pipe (E)	A	From Leg	2.000	0.000	0.000	139.000	No Ice 6.329	5.642	0.112
			0.000				1/2" Ice 6.775	6.426	0.169
			1.000				1" Ice 7.214	7.131	0.233
ERICSSON AIR 21 B4A B2P w/ Mount Pipe (E)	B	From Leg	2.000	0.000	0.000	139.000	No Ice 6.329	5.642	0.112
			0.000				1/2" Ice 6.775	6.426	0.169
			1.000				1" Ice 7.214	7.131	0.233
ERICSSON AIR 21 B4A B2P w/ Mount Pipe (E)	C	From Leg	2.000	0.000	0.000	139.000	No Ice 6.329	5.642	0.112
			0.000				1/2" Ice 6.775	6.426	0.169
			1.000				1" Ice 7.214	7.131	0.233
KRY 112 144/1 (E)	A	From Leg	2.000	0.000	0.000	139.000	No Ice 0.350	0.175	0.011
			0.000				1/2" Ice 0.426	0.234	0.014
			1.000				1" Ice 0.509	0.301	0.019
KRY 112 144/1 (E)	B	From Leg	2.000	0.000	0.000	139.000	No Ice 0.350	0.175	0.011
			0.000				1/2" Ice 0.426	0.234	0.014
			1.000				1" Ice 0.509	0.301	0.019
KRY 112 144/1 (E)	C	From Leg	2.000	0.000	0.000	139.000	No Ice 0.350	0.175	0.011
			0.000				1/2" Ice 0.426	0.234	0.014
			1.000				1" Ice 0.509	0.301	0.019
Side Arm Mount [SO 104-3] (E)	C	None		0.000		139.000	No Ice 3.300	3.300	0.287
							1/2" Ice 4.130	4.130	0.317
							1" Ice 4.960	4.960	0.347
*_*_*_*_*									
1142-2C (Inactive/TIA)	A	From Leg	6.000	0.000	0.000	128.000	No Ice 2.092	2.092	0.024
			0.000				1/2" Ice 3.374	3.374	0.041
			4.000				1" Ice 4.673	4.673	0.066
Side Arm Mount [SO 308-1] (E)	A	From Leg	3.000	0.000	0.000	128.000	No Ice 0.980	3.030	0.053
			0.000				1/2" Ice 1.700	5.220	0.079
			0.000				1" Ice 2.420	7.410	0.105
*_*_*_*_*									
GPS_A (E)	C	From Leg	2.000	0.000	0.000	51.000	No Ice 0.255	0.255	0.001
			0.000				1/2" Ice 0.320	0.320	0.005
			0.000				1" Ice 0.393	0.393	0.010
Side Arm Mount [SO 701-1] (E)	C	From Leg	1.000	0.000	0.000	51.000	No Ice 0.850	1.670	0.065
			0.000				1/2" Ice 1.140	2.340	0.079
			0.000				1" Ice 1.430	3.010	0.093
*_*_*_*_*									

Dishes

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Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert ft	Azimuth Adjustment °	3 dB Beam Width °	Elevation ft	Outside Diameter ft	Aperture Area ft ²	Weight K	
HPD2-23 (Az.,Face/Prev.SA)	C	Paraboloid w/Shroud (HP)	From Leg	2.000 0.000 4.000	74.000		175.000	2.000	No Ice 1/2" Ice 1" Ice	3.142 3.409 3.676	0.027 0.044 0.062
HPD2-23 (Az.,Face/Prev.SA)	C	Paraboloid w/Shroud (HP)	From Leg	2.000 0.000 4.000	-90.000		175.000	2.000	No Ice 1/2" Ice 1" Ice	3.142 3.409 3.676	0.027 0.044 0.062
*_*_*_*											

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 30 deg - No Ice
5	0.9 Dead+1.6 Wind 30 deg - No Ice
6	1.2 Dead+1.6 Wind 60 deg - No Ice
7	0.9 Dead+1.6 Wind 60 deg - No Ice
8	1.2 Dead+1.6 Wind 90 deg - No Ice
9	0.9 Dead+1.6 Wind 90 deg - No Ice
10	1.2 Dead+1.6 Wind 120 deg - No Ice
11	0.9 Dead+1.6 Wind 120 deg - No Ice
12	1.2 Dead+1.6 Wind 150 deg - No Ice
13	0.9 Dead+1.6 Wind 150 deg - No Ice
14	1.2 Dead+1.6 Wind 180 deg - No Ice
15	0.9 Dead+1.6 Wind 180 deg - No Ice
16	1.2 Dead+1.6 Wind 210 deg - No Ice
17	0.9 Dead+1.6 Wind 210 deg - No Ice
18	1.2 Dead+1.6 Wind 240 deg - No Ice
19	0.9 Dead+1.6 Wind 240 deg - No Ice
20	1.2 Dead+1.6 Wind 270 deg - No Ice
21	0.9 Dead+1.6 Wind 270 deg - No Ice
22	1.2 Dead+1.6 Wind 300 deg - No Ice
23	0.9 Dead+1.6 Wind 300 deg - No Ice
24	1.2 Dead+1.6 Wind 330 deg - No Ice
25	0.9 Dead+1.6 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service

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Comb. No.	Description
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	212.625 - 202.458	Leg	Max Tension	1	0.000	0.000	0.000
			Max. Compression	35	-5.717	0.077	0.001
			Max. Mx	6	-1.018	0.508	0.006
			Max. My	8	-1.636	0.002	-0.529
			Max. Vy	6	1.816	-0.323	0.006
		Diagonal	Max. Vx	24	-1.821	-0.002	0.316
			Max Tension	5	3.424	0.000	0.000
			Max. Compression	4	-3.494	0.000	0.000
			Max. Mx	26	-0.101	0.049	0.000
			Max. Vy	26	-0.029	0.000	0.000
		Horizontal	Max Tension	14	2.534	0.000	0.000
			Max. Compression	2	-2.534	-0.010	-0.003
			Max. Mx	37	0.093	-0.031	-0.002
			Max. My	6	0.042	-0.011	-0.007
			Max. Vy	37	-0.032	-0.031	-0.002
		Top Girt	Max. Vx	6	-0.002	-0.011	-0.007
			Max Tension	15	0.256	-0.005	0.000
			Max. Compression	2	-0.256	0.000	0.000
			Max. Mx	33	-0.040	-0.027	-0.000
			Max. My	18	0.134	-0.006	0.001
		Inner Bracing	Max. Vy	33	0.031	-0.027	-0.000
			Max. Vx	18	-0.000	0.000	0.000
			Max Tension	2	0.004	0.000	0.000
Max. Compression	2		-0.004	0.000	0.000		
Max. Mx	26		-0.000	-0.028	0.000		
T2	202.458 - 182.292	Leg	Max. Vy	26	0.026	0.000	0.000
			Max Tension	15	21.324	0.130	-0.019
			Max. Compression	18	-30.268	0.283	0.000
			Max. Mx	6	1.858	1.381	0.001
			Max. My	12	-3.510	-0.001	1.395
		Diagonal	Max. Vy	6	-1.338	0.129	0.002
			Max. Vx	24	1.362	-0.001	-0.113
			Max Tension	17	10.514	0.000	0.000
			Max. Compression	16	-10.586	0.000	0.000
			Max. Mx	26	-0.090	0.059	0.000
		Horizontal	Max. Vy	26	-0.030	0.000	0.000
			Max Tension	16	5.796	0.000	0.000
			Max. Compression	4	-5.794	-0.012	-0.000
			Max. Mx	37	-0.585	-0.041	-0.003
			Max. My	6	0.331	-0.022	-0.013
		Inner Bracing	Max. Vy	37	0.034	-0.041	-0.003
			Max. Vx	6	0.003	0.000	0.000
			Max Tension	19	0.007	0.000	0.000
			Max. Compression	6	-0.007	0.000	0.000
			Max. Mx	26	-0.000	-0.028	0.000

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T3	182.292 - 162.104	Leg	Max. Vy	26	0.026	0.000	0.000	
			Max Tension	15	62.048	-0.279	0.096	
		Diagonal	Max. Compression	18	-76.131	0.292	0.135	
			Max. Mx	14	46.132	-0.391	0.034	
			Max. My	4	-7.149	-0.043	-0.486	
			Max. Vy	6	-0.970	-0.353	-0.021	
			Max. Vx	24	0.983	-0.031	0.441	
			Max Tension	4	10.750	0.000	0.000	
			Max. Compression	4	-10.843	0.000	0.000	
			Max. Mx	26	-0.142	0.078	0.000	
			Max. Vy	26	-0.036	0.000	0.000	
			Max Tension	16	6.818	0.000	0.000	
		Horizontal	Max. Compression	4	-6.833	-0.014	0.000	
			Max. Mx	37	-0.622	-0.048	-0.003	
			Max. My	10	0.546	-0.000	0.012	
			Max. Vy	37	0.038	-0.048	-0.003	
			Max. Vx	2	-0.003	0.000	0.012	
			Max Tension	3	0.005	0.000	0.000	
			Max. Compression	6	-0.007	0.000	0.000	
			Max. Mx	26	-0.004	-0.038	0.000	
T4	162.104 - 141.896	Leg	Max. Vy	26	0.030	0.000	0.000	
			Max Tension	7	103.750	-1.142	-0.010	
		Diagonal	Max. Compression	2	-128.440	0.336	0.012	
			Max. Mx	22	89.687	-1.515	0.007	
			Max. My	12	-6.651	-0.003	-1.132	
			Max. Vy	14	-0.810	-0.267	0.096	
			Max. Vx	12	0.214	-0.003	-1.132	
			Max Tension	16	10.854	0.000	0.000	
			Max. Compression	16	-10.973	0.000	0.000	
			Max. Mx	26	-0.345	0.100	0.000	
			Max. Vy	26	0.043	0.000	0.000	
			Max Tension	4	7.591	-0.026	0.000	
		Horizontal	Max. Compression	16	-7.578	0.000	0.000	
			Max. Mx	37	0.855	-0.080	-0.004	
			Max. My	10	0.484	-0.001	0.018	
			Max. Vy	37	0.054	-0.080	-0.004	
			Max. Vx	10	-0.003	-0.001	0.018	
			Max Tension	11	0.006	0.000	0.000	
			Max. Compression	14	-0.009	0.000	0.000	
			Max. Mx	26	-0.005	-0.054	0.000	
T5	141.896 - 121.688	Leg	Max. Vy	26	0.036	0.000	0.000	
			Max Tension	7	133.279	-0.935	0.058	
		Diagonal	Max. Compression	10	-160.525	0.679	-0.023	
			Max. Mx	22	102.117	-1.515	0.007	
			Max. My	12	-7.285	-0.003	-1.132	
			Max. Vy	22	-0.430	-1.515	0.007	
			Max. Vx	12	-0.402	-0.003	-1.132	
			Max Tension	16	13.944	0.000	0.000	
			Max. Compression	16	-14.129	0.000	0.000	
			Max. Mx	26	-0.195	0.201	0.000	
			Max. Vy	26	-0.064	0.000	0.000	
			Max Tension	4	8.532	-0.034	-0.000	
		Horizontal	Max. Compression	16	-8.521	0.000	0.000	
			Max. Mx	29	-0.974	-0.104	-0.004	
			Max. My	18	1.094	-0.001	0.017	
			Max. Vy	29	-0.061	-0.104	-0.004	
			Max. Vx	18	-0.003	-0.001	0.017	
			Max Tension	19	0.004	0.000	0.000	
			Inner Bracing	Max. Compression	19	0.004	0.000	0.000
				Max. Mx	19	0.004	0.000	0.000
Max. My	19	0.004		0.000	0.000			
Max. Vy	19	0.004		0.000	0.000			

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T6	121.688 - 101.479	Leg	Max. Compression	37	-0.009	0.000	0.000	
			Max. Mx	26	-0.008	-0.071	0.000	
			Max. Vy	26	-0.041	0.000	0.000	
			Max Tension	7	165.752	-0.690	0.005	
		Diagonal	Max. Compression	10	-196.727	0.997	-0.069	
			Max. Mx	6	161.881	-1.045	0.028	
			Max. My	12	-13.497	-0.043	-1.088	
			Max. Vy	6	0.138	-1.045	0.028	
			Max. Vx	24	-0.161	-0.040	1.087	
			Max Tension	16	13.007	0.000	0.000	
			Horizontal	Max. Compression	16	-13.224	0.000	0.000
				Max. Mx	26	-0.291	0.246	0.000
				Max. Vy	26	0.074	0.000	0.000
				Max Tension	4	8.749	-0.042	-0.000
			Inner Bracing	Max. Compression	16	-8.739	0.000	0.000
				Max. Mx	37	1.145	-0.124	-0.004
		Max. My		18	1.082	-0.014	0.015	
		Max. Vy		37	-0.068	-0.124	-0.004	
		Max. Vx		18	0.002	0.000	0.000	
		Max Tension		19	0.002	0.000	0.000	
Max. Compression	29	-0.011		0.000	0.000			
Max. Mx	26	-0.010		-0.122	0.000			
T7	101.479 - 81.2708	Leg	Max. Vy	26	0.060	0.000	0.000	
			Max Tension	7	194.875	-0.674	0.015	
			Diagonal	Max. Compression	10	-229.984	0.847	-0.051
				Max. Mx	6	176.769	-1.045	0.028
		Max. My		12	-14.551	-0.043	-1.088	
		Max. Vy		6	-0.137	-1.045	0.028	
		Max. Vx		24	0.158	-0.040	1.087	
		Max Tension		16	13.119	0.000	0.000	
		Horizontal		Max. Compression	16	-13.495	0.000	0.000
				Max. Mx	26	-0.435	0.357	0.000
				Max. Vy	26	-0.100	0.000	0.000
				Max Tension	4	9.616	-0.088	-0.000
		Inner Bracing		Max. Compression	16	-9.608	0.000	0.000
				Max. Mx	37	-1.410	-0.215	-0.005
			Max. My	18	0.351	-0.032	0.020	
			Max. Vy	37	0.100	-0.215	-0.005	
			Max. Vx	18	-0.002	-0.032	0.020	
			Max Tension	19	0.001	0.000	0.000	
			Max. Compression	29	-0.014	0.000	0.000	
			Max. Mx	26	-0.013	-0.184	0.000	
T8	81.2708 - 61	Leg	Max. Vy	26	0.078	0.000	0.000	
			Max Tension	7	221.977	-1.397	0.012	
			Max. Compression	10	-261.735	0.797	-0.044	
			Max. Mx	6	203.725	-1.410	0.012	
		Diagonal	Max. My	12	-18.684	-0.051	-1.304	
			Max. Vy	6	0.166	-1.410	0.012	
			Max. Vx	12	0.170	-0.051	-1.304	
			Max Tension	17	12.972	0.000	0.000	
			Max. Compression	16	-13.437	0.000	0.000	
			Max. Mx	26	-0.564	0.425	0.000	
			Horizontal	Max. Vy	26	-0.112	0.000	0.000
				Max Tension	4	10.144	-0.110	-0.000
				Max. Compression	16	-10.137	0.000	0.000
				Max. Mx	37	-1.628	-0.255	-0.005
			Inner Bracing	Max. My	18	0.529	-0.061	0.018
				Max. Vy	37	-0.110	-0.255	-0.005
		Max. Vx		18	0.002	-0.061	0.018	

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T9	61 - 40.6667	Inner Bracing	Max Tension	1	0.000	0.000	0.000	
			Max. Compression	29	-0.017	0.000	0.000	
			Max. Mx	26	-0.016	-0.289	0.000	
			Max. Vy	26	0.108	0.000	0.000	
		Leg	Max Tension	7	248.144	-1.986	0.002	
			Max. Compression	10	-292.818	-2.823	-0.147	
			Max. Mx	2	-292.389	-2.827	0.224	
			Max. My	24	-24.644	-0.644	3.701	
			Max. Vy	10	0.567	2.196	-0.010	
			Max. Vx	24	-0.415	-0.644	3.701	
			Diagonal	Max Tension	17	14.061	0.000	0.000
				Max. Compression	16	-14.582	0.000	0.000
		Max. Mx		26	-0.648	0.490	0.000	
		Max. Vy		26	-0.121	0.000	0.000	
		Horizontal	Max Tension	4	11.502	-0.136	-0.000	
			Max. Compression	16	-11.495	0.000	0.000	
			Max. Mx	37	1.669	-0.305	-0.005	
			Max. My	22	-2.733	-0.155	-0.017	
			Max. Vy	37	-0.120	-0.305	-0.005	
			Max. Vx	22	0.002	0.000	0.000	
Inner Bracing	Max Tension		1	0.000	0.000	0.000		
	Max. Compression		29	-0.017	0.000	0.000		
	Max. Mx	26	-0.016	-0.353	0.000			
	Max. Vy	26	0.118	0.000	0.000			
T10	40.6667 - 20.3333	Leg	Max Tension	7	259.079	1.698	0.064	
			Max. Compression	10	-306.548	-8.317	-0.308	
			Max. Mx	10	-306.155	9.580	0.223	
			Max. My	24	-27.390	-1.361	5.935	
		Diagonal	Max. Vy	10	1.816	9.580	0.223	
			Max. Vx	24	-1.007	-1.361	5.935	
			Max Tension	17	19.519	-0.156	0.105	
			Max. Compression	16	-20.582	0.000	0.000	
			Max. Mx	6	14.847	-0.210	0.080	
			Max. My	4	-19.463	0.007	-0.118	
			Max. Vy	37	-0.081	-0.200	0.009	
			Max. Vx	4	0.010	0.000	0.000	
		Horizontal	Max Tension	4	11.381	-0.203	-0.000	
			Max. Compression	16	-11.375	0.000	0.000	
			Max. Mx	37	-1.998	-0.431	-0.007	
			Max. My	18	0.831	-0.119	0.026	
			Max. Vy	37	0.151	-0.431	-0.007	
			Max. Vx	18	0.002	-0.125	0.026	
			Redund Horz 1 Bracing	Max Tension	10	5.324	0.000	0.000
				Max. Compression	10	-5.324	0.000	0.000
Max. Mx	26	1.010		0.047	0.000			
Max. Vy	26	0.030		0.000	0.000			
Redund Diag 1 Bracing	Max Tension	10	4.918	0.000	0.000			
	Max. Compression	10	-4.918	0.000	0.000			
	Max. Mx	26	1.083	0.095	0.000			
	Max. Vy	26	-0.033	0.000	0.000			
Redund Hip 1 Bracing	Max Tension	5	0.017	0.000	0.000			
	Max. Compression	16	-0.029	0.000	0.000			
	Max. Mx	26	-0.015	0.047	0.000			
	Max. Vy	26	-0.030	0.000	0.000			
Redund Hip Diagonal 1 Bracing	Max Tension	31	0.080	0.000	0.000			
	Max. Compression	29	-0.094	0.000	0.000			

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T11	20.3333 - 0	Inner Bracing	Max. Mx	26	0.071	0.327	0.000
			Max. Vy	26	-0.086	0.000	0.000
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	31	-0.021	0.000	0.000
			Max. Mx	26	-0.018	0.369	0.000
			Max. Vy	26	-0.117	0.000	0.000
		Leg	Max Tension	7	282.458	5.906	0.140
			Max. Compression	10	-335.559	0.000	0.000
			Max. Mx	10	-335.167	8.695	0.207
			Max. My	24	-29.584	-1.363	5.932
			Max. Vy	10	-1.733	8.695	0.207
			Max. Vx	24	0.982	-1.363	5.932
		Diagonal	Max Tension	17	22.676	-0.148	0.104
			Max. Compression	16	-23.698	0.000	0.000
			Max. Mx	4	10.543	-0.209	0.033
			Max. My	4	-23.521	-0.021	-0.118
			Max. Vy	37	-0.082	-0.205	0.009
			Max. Vx	4	0.009	-0.021	-0.118
		Horizontal	Max Tension	4	13.892	-0.244	-0.000
			Max. Compression	16	-13.885	0.000	0.000
			Max. Mx	37	-2.179	-0.447	-0.007
			Max. My	22	3.185	-0.321	-0.026
			Max. Vy	37	0.153	-0.447	-0.007
			Max. Vx	22	-0.002	0.000	0.000
		Redund Horz 1 Bracing	Max Tension	10	5.823	0.000	0.000
			Max. Compression	10	-5.823	0.000	0.000
			Max. Mx	26	1.212	0.053	0.000
		Redund Diag 1 Bracing	Max. Vy	26	-0.030	0.000	0.000
			Max Tension	10	5.030	0.000	0.000
			Max. Compression	10	-5.030	0.000	0.000
		Redund Hip 1 Bracing	Max. Mx	26	1.108	0.103	0.000
			Max. Vy	26	-0.034	0.000	0.000
			Max Tension	5	0.016	0.000	0.000
Redund Hip Diagonal 1 Bracing	Max. Compression	16	-0.027	0.000	0.000		
	Max. Mx	26	-0.012	0.053	0.000		
	Max. Vy	26	0.030	0.000	0.000		
	Max Tension	31	0.076	0.000	0.000		
	Max. Compression	29	-0.086	0.000	0.000		
	Max. Mx	26	0.067	0.351	0.000		
Inner Bracing	Max. Vy	26	0.088	0.000	0.000		
	Max Tension	23	0.000	0.000	0.000		
	Max. Compression	31	-0.018	0.000	0.000		
	Max. Mx	26	-0.015	0.422	0.000		
	Max. Vy	26	-0.121	0.000	0.000		

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg C	Max. Vert	18	368.054	36.869	-21.590
	Max. H _x	18	368.054	36.869	-21.590
	Max. H _z	5	-266.372	-26.657	20.169

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Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg B	Min. Vert	7	-310.834	-33.260	19.492
	Min. H _x	7	-310.834	-33.260	19.492
	Min. H _z	16	323.691	30.359	-22.304
	Max. Vert	10	368.313	-37.275	-20.856
	Max. H _x	23	-309.918	33.641	18.739
	Max. H _z	25	-265.537	27.385	18.831
Leg A	Min. Vert	23	-309.918	33.641	18.739
	Min. H _x	10	368.313	-37.275	-20.856
	Min. H _z	12	324.047	-31.103	-21.002
	Max. Vert	2	368.104	-0.872	42.655
	Max. H _x	20	33.910	6.845	2.481
	Max. H _z	2	368.104	-0.872	42.655
	Min. Vert	15	-309.125	0.901	-38.439
	Min. H _x	9	25.175	-6.827	1.835
Min. H _z	15	-309.125	0.901	-38.439	

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead Only	83.226	0.000	0.000	-9.289	-8.451	0.000
1.2 Dead+1.6 Wind 0 deg - No Ice	99.871	-0.007	-70.722	-8710.791	-9.635	-45.943
0.9 Dead+1.6 Wind 0 deg - No Ice	74.903	-0.007	-70.722	-8708.004	-7.099	-45.943
1.2 Dead+1.6 Wind 30 deg - No Ice	99.871	35.578	-61.561	-7576.155	-4382.393	-33.505
0.9 Dead+1.6 Wind 30 deg - No Ice	74.903	35.578	-61.561	-7573.369	-4379.858	-33.505
1.2 Dead+1.6 Wind 60 deg - No Ice	99.871	61.624	-35.469	-4361.257	-7574.206	-13.452
0.9 Dead+1.6 Wind 60 deg - No Ice	74.903	61.624	-35.469	-4358.470	-7571.671	-13.452
1.2 Dead+1.6 Wind 90 deg - No Ice	99.871	71.212	0.021	-8.188	-8763.428	8.869
0.9 Dead+1.6 Wind 90 deg - No Ice	74.903	71.212	0.021	-5.401	-8760.893	8.869
1.2 Dead+1.6 Wind 120 deg - No Ice	99.871	61.369	35.344	4335.049	-7561.795	30.343
0.9 Dead+1.6 Wind 120 deg - No Ice	74.903	61.369	35.344	4337.836	-7559.260	30.343
1.2 Dead+1.6 Wind 150 deg - No Ice	99.871	35.579	61.490	7540.487	-4381.316	45.170
0.9 Dead+1.6 Wind 150 deg - No Ice	74.903	35.579	61.490	7543.274	-4378.781	45.170
1.2 Dead+1.6 Wind 180 deg - No Ice	99.871	0.015	70.946	8689.251	-12.129	46.523
0.9 Dead+1.6 Wind 180 deg - No Ice	74.903	0.015	70.946	8692.038	-9.594	46.523
1.2 Dead+1.6 Wind 210 deg - No Ice	99.871	-35.594	61.540	7550.207	4365.022	33.281
0.9 Dead+1.6 Wind 210 deg - No Ice	74.903	-35.594	61.540	7552.994	4367.557	33.281
1.2 Dead+1.6 Wind 240 deg - No Ice	99.871	-61.409	35.381	4342.818	7549.527	13.803
0.9 Dead+1.6 Wind 240 deg - No Ice	74.903	-61.409	35.381	4345.605	7552.062	13.803

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Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
No Ice						
1.2 Dead+1.6 Wind 270 deg - No Ice	99.871	-71.198	-0.032	-16.128	8740.690	-8.837
0.9 Dead+1.6 Wind 270 deg - No Ice	74.903	-71.198	-0.032	-13.342	8743.225	-8.837
1.2 Dead+1.6 Wind 300 deg - No Ice	99.871	-61.567	-35.462	-4358.741	7543.086	-30.085
0.9 Dead+1.6 Wind 300 deg - No Ice	74.903	-61.567	-35.462	-4355.954	7545.621	-30.085
1.2 Dead+1.6 Wind 330 deg - No Ice	99.871	-35.572	-61.492	-7563.018	4359.812	-45.171
0.9 Dead+1.6 Wind 330 deg - No Ice	74.903	-35.572	-61.492	-7560.231	4362.347	-45.171
1.2 Dead+1.0 Ice+1.0 Temp	229.076	-0.000	0.000	50.242	282.291	0.000
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	229.076	0.016	-19.689	-2367.746	279.564	-7.563
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	229.076	9.910	-17.076	-2054.775	-940.744	-5.597
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	229.076	17.103	-9.829	-1160.724	-1827.139	-2.160
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	229.076	19.801	-0.014	48.005	-2160.633	1.720
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	229.076	17.125	9.826	1256.064	-1822.760	5.503
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	229.076	9.880	17.045	2149.764	-935.631	7.708
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	229.076	-0.015	19.629	2467.314	284.723	7.671
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	229.076	-9.913	17.072	2154.531	1505.907	5.553
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	229.076	-17.150	9.864	1262.512	2391.769	2.224
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	229.076	-19.798	0.011	52.077	2724.726	-1.714
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	229.076	-17.074	-9.797	-1155.325	2386.732	-5.449
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	229.076	-9.878	-17.045	-2049.327	1499.970	-7.708
Dead+Wind 0 deg - Service	83.226	-0.002	-15.599	-1928.140	-8.339	-10.133
Dead+Wind 30 deg - Service	83.226	7.847	-13.578	-1677.878	-972.823	-7.390
Dead+Wind 60 deg - Service	83.226	13.592	-7.823	-968.778	-1676.831	-2.967
Dead+Wind 90 deg - Service	83.226	15.707	0.005	-8.637	-1939.134	1.956
Dead+Wind 120 deg - Service	83.226	13.536	7.796	949.336	-1674.093	6.693
Dead+Wind 150 deg - Service	83.226	7.847	13.563	1656.349	-972.586	9.963
Dead+Wind 180 deg - Service	83.226	0.003	15.648	1909.728	-8.889	10.261
Dead+Wind 210 deg - Service	83.226	-7.851	13.574	1658.493	956.564	7.341
Dead+Wind 240 deg - Service	83.226	-13.545	7.804	951.050	1658.960	3.045
Dead+Wind 270 deg - Service	83.226	-15.704	-0.007	-10.388	1921.690	-1.949
Dead+Wind 300 deg - Service	83.226	-13.580	-7.822	-968.223	1657.539	-6.636
Dead+Wind 330 deg - Service	83.226	-7.846	-13.563	-1674.980	955.415	-9.963

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.000	-83.226	0.000	0.000	83.226	0.000	0.000%
2	-0.007	-99.871	-70.722	0.007	99.871	70.722	0.000%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
3	-0.007	-74.903	-70.722	0.007	74.903	70.722	0.000%
4	35.578	-99.871	-61.561	-35.578	99.871	61.561	0.000%
5	35.578	-74.903	-61.561	-35.578	74.903	61.561	0.000%
6	61.624	-99.871	-35.469	-61.624	99.871	35.469	0.000%
7	61.624	-74.903	-35.469	-61.624	74.903	35.469	0.000%
8	71.212	-99.871	0.021	-71.212	99.871	-0.021	0.000%
9	71.212	-74.903	0.021	-71.212	74.903	-0.021	0.000%
10	61.369	-99.871	35.344	-61.369	99.871	-35.344	0.000%
11	61.369	-74.903	35.344	-61.369	74.903	-35.344	0.000%
12	35.579	-99.871	61.490	-35.579	99.871	-61.490	0.000%
13	35.579	-74.903	61.490	-35.579	74.903	-61.490	0.000%
14	0.015	-99.871	70.946	-0.015	99.871	-70.946	0.000%
15	0.015	-74.903	70.946	-0.015	74.903	-70.946	0.000%
16	-35.594	-99.871	61.540	35.594	99.871	-61.540	0.000%
17	-35.594	-74.903	61.540	35.594	74.903	-61.540	0.000%
18	-61.409	-99.871	35.381	61.409	99.871	-35.381	0.000%
19	-61.409	-74.903	35.381	61.409	74.903	-35.381	0.000%
20	-71.198	-99.871	-0.032	71.198	99.871	0.032	0.000%
21	-71.198	-74.903	-0.032	71.198	74.903	0.032	0.000%
22	-61.567	-99.871	-35.462	61.567	99.871	35.462	0.000%
23	-61.567	-74.903	-35.462	61.567	74.903	35.462	0.000%
24	-35.572	-99.871	-61.492	35.572	99.871	61.492	0.000%
25	-35.572	-74.903	-61.492	35.572	74.903	61.492	0.000%
26	0.000	-229.076	0.000	0.000	229.076	0.000	0.000%
27	0.016	-229.076	-19.689	-0.016	229.076	19.689	0.000%
28	9.910	-229.076	-17.076	-9.910	229.076	17.076	0.000%
29	17.103	-229.076	-9.829	-17.103	229.076	9.829	0.000%
30	19.801	-229.076	-0.014	-19.801	229.076	0.014	0.000%
31	17.125	-229.076	9.826	-17.125	229.076	-9.826	0.000%
32	9.880	-229.076	17.045	-9.880	229.076	-17.045	0.000%
33	-0.015	-229.076	19.629	0.015	229.076	-19.629	0.000%
34	-9.913	-229.076	17.072	9.913	229.076	-17.072	0.000%
35	-17.150	-229.076	9.864	17.150	229.076	-9.864	0.000%
36	-19.798	-229.076	0.011	19.798	229.076	-0.011	0.000%
37	-17.074	-229.076	-9.797	17.074	229.076	9.797	0.000%
38	-9.878	-229.076	-17.045	9.878	229.076	17.045	0.000%
39	-0.002	-83.226	-15.599	0.002	83.226	15.599	0.000%
40	7.847	-83.226	-13.578	-7.847	83.226	13.578	0.000%
41	13.592	-83.226	-7.823	-13.592	83.226	7.823	0.000%
42	15.707	-83.226	0.005	-15.707	83.226	-0.005	0.000%
43	13.536	-83.226	7.796	-13.536	83.226	-7.796	0.000%
44	7.847	-83.226	13.563	-7.847	83.226	-13.563	0.000%
45	0.003	-83.226	15.648	-0.003	83.226	-15.648	0.000%
46	-7.851	-83.226	13.574	7.851	83.226	-13.574	0.000%
47	-13.545	-83.226	7.804	13.545	83.226	-7.804	0.000%
48	-15.704	-83.226	-0.007	15.704	83.226	0.007	0.000%
49	-13.580	-83.226	-7.822	13.580	83.226	7.822	0.000%
50	-7.846	-83.226	-13.563	7.846	83.226	13.563	0.000%

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	212.625 - 202.458	4.157	41	0.174	0.025
T2	202.458 - 182.292	3.784	41	0.173	0.025

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T3	182.292 - 162.104	3.035	41	0.166	0.023
T4	162.104 - 141.896	2.331	41	0.150	0.018
T5	141.896 - 121.688	1.717	41	0.127	0.014
T6	121.688 - 101.479	1.226	42	0.102	0.012
T7	101.479 - 81.2708	0.828	42	0.081	0.009
T8	81.2708 - 61	0.525	42	0.060	0.008
T9	61 - 40.6667	0.296	42	0.043	0.006
T10	40.6667 - 20.3333	0.137	48	0.025	0.004
T11	20.3333 - 0	0.045	48	0.013	0.002

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
216.625	Flash Beacon Lighting	41	4.157	0.174	0.025	237902
213.625	Lightning Rod 5/8" x 6'	41	4.157	0.174	0.025	237902
212.625	Climb Leg Extension	41	4.157	0.174	0.025	237902
208.000	(2) LPA-80080/6CF w/ Mount Pipe	41	3.988	0.174	0.025	237902
199.000	APXVSP18-C-A20 w/ Mount Pipe	41	3.656	0.173	0.025	224680
189.000	7770.00 w/ Mount Pipe	41	3.282	0.169	0.024	111388
183.000	APXV18-206517LS w/ Mount Pipe	41	3.060	0.166	0.023	63699
179.000	HPD2-23	41	2.915	0.164	0.022	62304
175.000	(4) DB844H90E-XY w/ Mount Pipe	41	2.772	0.161	0.021	62032
167.000	1151-3	41	2.495	0.155	0.019	49020
164.000	1151-3	41	2.394	0.152	0.018	45179
162.000	SD310-HL	41	2.327	0.150	0.018	43517
147.000	1151-3	41	1.861	0.133	0.015	38075
145.000	SD310-HL	41	1.803	0.131	0.015	36512
139.000	ERICSSON AIR 21 B2A B4P w/ Mount Pipe	41	1.639	0.123	0.014	36351
128.000	1142-2C	41	1.367	0.109	0.013	49932
112.000	Side Lighting	42	1.024	0.091	0.011	54067
51.000	GPS_A	42	0.209	0.034	0.005	70561

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	212.625 - 202.458	18.036	6	0.744	0.115
T2	202.458 - 182.292	16.432	6	0.743	0.115
T3	182.292 - 162.104	13.215	6	0.706	0.105
T4	162.104 - 141.896	10.232	8	0.629	0.080
T5	141.896 - 121.688	7.615	8	0.540	0.065
T6	121.688 - 101.479	5.476	8	0.440	0.054
T7	101.479 - 81.2708	3.717	8	0.355	0.042
T8	81.2708 - 61	2.365	8	0.266	0.034
T9	61 - 40.6667	1.341	8	0.190	0.026
T10	40.6667 - 20.3333	0.620	20	0.113	0.017
T11	20.3333 - 0	0.205	20	0.056	0.009

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Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
216.625	Flash Beacon Lighting	6	18.036	0.744	0.115	55845
213.625	Lightning Rod 5/8" x 6'	6	18.036	0.744	0.115	55845
212.625	Climb Leg Extension	6	18.036	0.744	0.115	55845
208.000	(2) LPA-80080/6CF w/ Mount Pipe	6	17.309	0.745	0.115	55845
199.000	APXVSPP18-C-A20 w/ Mount Pipe	6	15.880	0.740	0.114	53391
189.000	7770.00 w/ Mount Pipe	6	14.275	0.724	0.110	29582
183.000	APXV18-206517LS w/ Mount Pipe	6	13.326	0.708	0.106	16184
179.000	HPD2-23	6	12.706	0.695	0.101	14883
175.000	(4) DB844H90E-XY w/ Mount Pipe	6	12.099	0.681	0.096	14907
167.000	1151-3	8	10.923	0.650	0.086	13685
164.000	1151-3	8	10.497	0.637	0.082	13220
162.000	SD310-HL	8	10.218	0.629	0.080	12860
147.000	1151-3	8	8.233	0.564	0.068	9726
145.000	SD310-HL	8	7.987	0.554	0.066	9343
139.000	ERICSSON AIR 21 B2A B4P w/ Mount Pipe	8	7.280	0.525	0.063	9363
128.000	1142-2C	8	6.099	0.470	0.057	12430
112.000	Side Lighting	8	4.585	0.398	0.048	12738
51.000	GPS_A	9	0.947	0.151	0.022	15875

Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T1	212.625	Leg	A325N	0.750	4	0.476	29.821	0.016	✓	1 Bolt Tension
		Diagonal	A325N	0.625	3	1.165	12.425	0.094	✓	1 Bolt Shear
		Horizontal	A325N	0.625	2	1.267	12.425	0.102	✓	1 Bolt Shear
T2	202.458	Leg	A325N	0.875	4	5.331	40.589	0.131	✓	1 Bolt Tension
		Diagonal	A325N	0.625	3	3.529	12.425	0.284	✓	1 Bolt Shear
		Horizontal	A325N	0.625	2	2.898	12.425	0.233	✓	1 Bolt Shear
T3	182.292	Leg	A325N	1.000	4	15.512	53.014	0.293	✓	1 Bolt Tension
		Diagonal	A325N	0.625	3	3.614	12.425	0.291	✓	1 Bolt Shear
		Horizontal	A325N	0.625	2	3.416	12.425	0.275	✓	1 Bolt Shear
T4	162.104	Leg	A325N	1.000	6	17.292	53.014	0.326	✓	1 Bolt Tension
		Diagonal	A325N	0.625	3	3.658	12.425	0.294	✓	1 Bolt Shear
		Horizontal	A325N	0.625	2	3.796	12.425	0.305	✓	1 Bolt Shear
T5	141.896	Leg	A325N	1.000	6	22.213	53.014	0.419	✓	1 Bolt Tension
		Diagonal	A325N	0.625	3	4.710	12.425	0.379	✓	1 Bolt Shear
		Horizontal	A325N	0.625	2	4.266	12.425	0.343	✓	1 Bolt Shear
T6	121.688	Leg	A325N	1.000	6	27.625	53.014	0.521	✓	1 Bolt Tension

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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria	
T7	101.479	Diagonal	A325N	0.625	3	4.408	12.425	0.355	✓	1	Bolt Shear
		Horizontal	A325N	0.625	2	4.374	12.425	0.352	✓	1	Bolt Shear
		Leg	A325N	1.000	8	24.359	53.014	0.459	✓	1	Bolt Tension
T8	81.2708	Diagonal	A325N	0.625	3	4.498	12.425	0.362	✓	1	Bolt Shear
		Horizontal	A325N	0.625	2	4.808	12.425	0.387	✓	1	Bolt Shear
		Leg	A325N	1.000	8	27.747	53.014	0.523	✓	1	Bolt Tension
T9	61	Diagonal	A325N	0.625	3	4.479	12.425	0.360	✓	1	Bolt Shear
		Horizontal	A325N	0.625	2	5.072	12.425	0.408	✓	1	Bolt Shear
		Leg	A325N	1.000	8	31.018	53.014	0.585	✓	1	Bolt Tension
T10	40.6667	Diagonal	A325N	0.625	3	4.861	12.425	0.391	✓	1	Bolt Shear
		Horizontal	A325N	0.625	2	5.751	12.425	0.463	✓	1	Bolt Shear
		Leg	A325N	1.000	8	32.285	53.014	0.609	✓	1	Bolt Tension
T11	20.3333	Diagonal	A325N	0.750	3	6.861	17.892	0.383	✓	1	Bolt Shear
		Horizontal	A325N	0.750	2	5.690	17.892	0.318	✓	1	Bolt Shear
		Diagonal	A325N	0.750	3	7.899	17.892	0.441	✓	1	Bolt Shear
		Horizontal	A325N	0.750	2	6.946	17.892	0.388	✓	1	Bolt Shear

Compression Checks

Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio P _u / φP _n
T1	212.625 - 202.458	ROHN 2.5 STD	10.167	5.083	64.4 K=1.00	1.704	-5.717	56.631	0.101 ¹
T2	202.458 - 182.292	ROHN 3 EH	20.167	6.722	71.0 K=1.00	3.016	-30.268	93.888	0.322 ¹
T3	182.292 - 162.104	ROHN 4 EH	20.223	6.741	54.8 K=1.00	4.407	-76.131	159.259	0.478 ¹
T4	162.104 - 141.896	ROHN 5 EH	20.244	6.748	44.0 K=1.00	6.112	-128.440	238.686	0.538 ¹
T5	141.896 - 121.688	ROHN 6 EHS	20.250	10.125	54.6 K=1.00	6.713	-160.525	242.933	0.661 ¹
T6	121.688 - 101.479	ROHN 6 EH	20.260	10.130	55.4 K=1.00	8.405	-196.727	302.237	0.651 ¹
T7	101.479 - 81.2708	ROHN 6 EH	20.260	10.130	55.4 K=1.00	8.405	-229.984	302.237	0.761 ¹
T8	81.2708 - 61	ROHN 8 EHS	20.328	10.164	41.8 K=1.00	9.719	-261.735	384.981	0.680 ¹
T9	61 - 40.6667	ROHN 8 EHS	20.384	10.192	41.9 K=1.00	9.719	-292.818	384.707	0.761 ¹

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T10	40.6667 - 20.3333	ROHN 8 EH	20.391	10.196	42.5 K=1.00	12.763	-306.548	503.236	0.609 ¹ ✓
T11	20.3333 - 0	ROHN 8 EH	20.373	10.187	42.5 K=1.00	12.763	-335.559	503.352	0.667 ¹ ✓

¹ P_u / φP_n controls

Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T1	212.625 - 202.458	ROHN 2 STD	6.639	6.453	98.4 K=1.00	1.075	-3.494	23.829	0.147 ¹ ✓
T2	202.458 - 182.292	ROHN 2 STD	7.987	7.717	117.6 K=1.00	1.075	-10.586	17.541	0.604 ¹ ✓
T3	182.292 - 162.104	ROHN 2 STD	8.602	8.301	126.5 K=1.00	1.075	-10.843	15.159	0.715 ¹ ✓
T4	162.104 - 141.896	ROHN 2 STD	9.291	8.954	136.5 K=1.00	1.075	-10.858	13.026	0.834 ¹ ✓
T5	141.896 - 121.688	ROHN 2.5 STD	12.600	12.138	153.7 K=1.00	1.704	-14.129	16.287	0.868 ¹ ✓
T6	121.688 - 101.479	ROHN 2.5 STD	13.385	12.964	164.2 K=1.00	1.704	-13.171	14.278	0.922 ¹ ✓
T7	101.479 - 81.2708	ROHN 3 STD	14.235	13.843	142.8 K=1.00	2.228	-13.495	24.700	0.546 ¹ ✓
T8	81.2708 - 61	ROHN 3 STD	15.213	14.731	151.9 K=1.00	2.228	-13.437	21.813	0.616 ¹ ✓
T9	61 - 40.6667	ROHN 3 STD	16.185	15.723	162.2 K=1.00	2.228	-14.582	19.146	0.762 ¹ ✓
T10	40.6667 - 20.3333	ROHN 3 STD	24.652	12.326	127.1 K=1.00	2.228	-20.582	31.156	0.661 ¹ ✓
T11	20.3333 - 0	ROHN 3 STD	25.288	12.644	130.4 K=1.00	2.228	-23.698	29.608	0.800 ¹ ✓

¹ P_u / φP_n controls

Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T1	212.625 - 202.458	ROHN 1.5 STD	8.521	4.141	79.8 K=1.00	0.799	-2.534	22.582	0.112 ¹ ✓
T2	202.458 - 182.292	ROHN 1.5 STD	8.597	4.153	80.0 K=1.00	0.799	-5.794	22.520	0.257 ¹ ✓

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T3	182.292 - 162.104	ROHN 1.5 STD	10.014	4.819	92.9 K=1.00	0.799	-6.833	19.143	0.357 ¹ ✓
T4	162.104 - 141.896	ROHN 2 STD	12.097	5.817	88.7 K=1.00	1.075	-7.578	27.209	0.279 ¹ ✓
T5	141.896 - 121.688	ROHN 2 STD	13.917	6.682	101.9 K=1.00	1.075	-8.521	22.640	0.376 ¹ ✓
T6	121.688 - 101.479	ROHN 2 STD	16.292	7.870	120.0 K=1.00	1.075	-8.739	16.864	0.518 ¹ ✓
T7	101.479 - 81.2708	ROHN 2.5 STD	18.792	9.120	115.5 K=1.00	1.704	-9.608	28.852	0.333 ¹ ✓
T8	81.2708 - 61	ROHN 2.5 STD	21.359	10.320	130.7 K=1.00	1.704	-10.137	22.530	0.450 ¹ ✓
T9	61 - 40.6667	ROHN 2.5 STD	23.927	11.604	147.0 K=1.00	1.704	-11.495	17.820	0.645 ¹ ✓
T10	40.6667 - 20.3333	ROHN 3 STD	25.177	12.229	126.1 K=1.00	2.228	-11.375	31.651	0.359 ¹ ✓
T11	20.3333 - 0	ROHN 3 STD	27.833	13.557	139.8 K=1.00	2.228	-13.885	25.753	0.539 ¹ ✓

¹ P_u / φP_n controls

Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T1	212.625 - 202.458	ROHN 1.5 STD	8.500	4.130	79.6 K=1.00	0.799	-0.256	22.635	0.011 ¹ ✓

¹ P_u / φP_n controls

Redundant Horizontal (1) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T10	40.6667 - 20.3333	ROHN 1.5 STD	6.294	5.935	114.4 K=1.00	0.799	-5.324	13.007	0.409 ¹ ✓
T11	20.3333 - 0	ROHN 1.5 STD	6.958	6.599	127.2 K=1.00	0.799	-5.823	11.053	0.527 ¹ ✓

¹ P_u / φP_n controls

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Redundant Diagonal (1) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T10	40.6667 - 20.3333	ROHN 2 STD	11.628	10.887	166.0 K=1.00	1.075	-4.918	8.811	0.558 ¹ ✓
T11	20.3333 - 0	ROHN 2 STD	12.021	11.347	173.0 K=1.00	1.075	-5.030	8.111	0.620 ¹ ✓

¹ P_u / φP_n controls

Redundant Hip (1) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T10	40.6667 - 20.3333	ROHN 1.5 STD	6.294	6.294	121.3 K=1.00	0.799	-0.029	11.936	0.002 ¹ ✓
T11	20.3333 - 0	ROHN 1.5 STD	6.958	6.958	134.1 K=1.00	0.799	-0.027	10.041	0.003 ¹ ✓

¹ P_u / φP_n controls

Redundant Hip Diagonal (1) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T10	40.6667 - 20.3333	ROHN 2.5 STD	15.204	15.204	192.6 K=1.00	1.704	-0.094	10.381	0.009 ¹ ✓
T11	20.3333 - 0	ROHN 2.5 STD	16.022	16.022	202.9 K=1.00	1.704	-0.086	9.348	0.009 ¹ ✓

¹ P_u / φP_n controls

Inner Bracing Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T1	212.625 - 202.458	L2x2x1/8	4.250	4.250	128.3 K=1.00	0.484	-0.004	6.565	0.001 ¹ ✓
T2	202.458 - 182.292	L2x2x1/8	4.299	4.299	129.8 K=1.00	0.484	-0.007	6.440	0.001 ¹ ✓
T3	182.292 -	L2x2x1/8	5.007	5.007	151.1	0.484	-0.006	4.790	0.001 ¹ ✓

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
	162.104				K=1.00				✓
T4	162.104 - 141.896	L2x2x1/8	6.049	6.049	182.6 K=1.00	0.484	-0.008	3.283	0.002 ¹ ✓
T5	141.896 - 121.688	L2x2x1/8	6.958	6.958	210.0 K=1.00	0.484	-0.009	2.480	0.004 ¹ ✓
T6	121.688 - 101.479	L2 1/2x2 1/2x3/16	8.146	8.146	197.5 K=1.00	0.902	-0.011	5.225	0.002 ¹ ✓
T7	101.479 - 81.2708	L3x3x3/16	9.396	9.396	189.2 K=1.00	1.090	-0.014	6.881	0.002 ¹ ✓
T8	81.2708 - 61	L3 1/2x3 1/2x1/4	10.680	10.680	184.7 K=1.00	1.690	-0.017	11.196	0.002 ¹ ✓
T9	61 - 40.6667	L3 1/2x3 1/2x1/4	11.964	11.964	206.9 K=1.00	1.690	-0.017	8.922	0.002 ¹ ✓
T10	40.6667 - 20.3333	ROHN 3 STD	12.589	12.589	129.8 K=1.00	2.228	-0.021	29.869	0.001 ¹ ✓
T11	20.3333 - 0	ROHN 3 STD	13.917	13.917	143.5 K=1.00	2.228	-0.018	24.440	0.001 ¹ ✓

¹ P_u / φP_n controls

Tension Checks

Leg Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T1	212.625 - 202.458	ROHN 2.5 STD	10.167	5.083	64.4	1.704	0.083	76.682	0.001 ¹ ✓
T2	202.458 - 182.292	ROHN 3 EH	20.167	6.722	71.0	3.016	21.324	135.717	0.157 ¹ ✓
T3	182.292 - 162.104	ROHN 4 EH	20.223	6.741	54.8	4.407	62.048	198.335	0.313 ¹ ✓
T4	162.104 - 141.896	ROHN 5 EH	20.244	6.748	44.0	6.112	103.750	275.039	0.377 ¹ ✓
T5	141.896 - 121.688	ROHN 6 EHS	20.250	10.125	54.6	6.713	133.279	302.097	0.441 ¹ ✓
T6	121.688 - 101.479	ROHN 6 EH	20.260	10.130	55.4	8.405	165.752	378.222	0.438 ¹ ✓
T7	101.479 - 81.2708	ROHN 6 EH	20.260	10.130	55.4	8.405	194.875	378.222	0.515 ¹ ✓
T8	81.2708 - 61	ROHN 8 EHS	20.328	10.164	41.8	9.719	221.977	437.369	0.508 ¹ ✓
T9	61 - 40.6667	ROHN 8 EHS	20.384	10.192	41.9	9.719	248.144	437.369	0.567 ¹ ✓
T10	40.6667 - 20.3333	ROHN 8 EH	20.391	10.196	42.5	12.763	259.079	574.322	0.451 ¹ ✓
T11	20.3333 - 0	ROHN 8 EH	20.373	10.187	42.5	12.763	282.458	574.322	0.492 ¹ ✓

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
									✓

¹ P_u / φP_n controls

Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T1	212.625 - 202.458	ROHN 2 STD	6.639	6.453	98.4	1.075	3.424	48.354	0.071 ¹ ✓
T2	202.458 - 182.292	ROHN 2 STD	7.987	7.717	117.6	1.075	10.514	48.354	0.217 ¹ ✓
T3	182.292 - 162.104	ROHN 2 STD	8.602	8.301	126.5	1.075	10.750	48.354	0.222 ¹ ✓
T4	162.104 - 141.896	ROHN 2 STD	9.055	8.719	132.9	1.075	10.854	48.354	0.224 ¹ ✓
T5	141.896 - 121.688	ROHN 2.5 STD	12.600	12.138	153.7	1.704	13.944	76.682	0.182 ¹ ✓
T6	121.688 - 101.479	ROHN 2.5 STD	12.984	12.563	159.1	1.704	13.007	76.682	0.170 ¹ ✓
T7	101.479 - 81.2708	ROHN 3 STD	14.235	13.843	142.8	2.228	13.119	100.281	0.131 ¹ ✓
T8	81.2708 - 61	ROHN 3 STD	15.213	14.731	151.9	2.228	12.972	100.281	0.129 ¹ ✓
T9	61 - 40.6667	ROHN 3 STD	16.185	15.723	162.2	2.228	14.061	100.281	0.140 ¹ ✓
T10	40.6667 - 20.3333	ROHN 3 STD	24.652	12.326	127.1	2.228	19.519	100.281	0.195 ¹ ✓
T11	20.3333 - 0	ROHN 3 STD	25.288	12.644	130.4	2.228	22.676	100.281	0.226 ¹ ✓

¹ P_u / φP_n controls

Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T1	212.625 - 202.458	ROHN 1.5 STD	8.521	4.141	79.8	0.799	2.534	35.976	0.070 ¹ ✓
T2	202.458 - 182.292	ROHN 1.5 STD	8.597	4.153	80.0	0.799	5.796	35.976	0.161 ¹ ✓
T3	182.292 - 162.104	ROHN 1.5 STD	10.014	4.819	92.9	0.799	6.818	35.976	0.190 ¹ ✓
T4	162.104 - 141.896	ROHN 2 STD	12.097	5.817	88.7	1.075	7.591	48.354	0.157 ¹ ✓

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	Client Crown Castle	Designed by Yathish

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T5	141.896 - 121.688	ROHN 2 STD	13.917	6.682	101.9	1.075	8.532	48.354	0.176 ¹ ✓
T6	121.688 - 101.479	ROHN 2 STD	16.292	7.870	120.0	1.075	8.749	48.354	0.181 ¹ ✓
T7	101.479 - 81.2708	ROHN 2.5 STD	18.792	9.120	115.5	1.704	9.616	76.682	0.125 ¹ ✓
T8	81.2708 - 61	ROHN 2.5 STD	21.359	10.320	130.7	1.704	10.144	76.682	0.132 ¹ ✓
T9	61 - 40.6667	ROHN 2.5 STD	23.927	11.604	147.0	1.704	11.502	76.682	0.150 ¹ ✓
T10	40.6667 - 20.3333	ROHN 3 STD	25.177	12.229	126.1	2.228	11.381	100.281	0.113 ¹ ✓
T11	20.3333 - 0	ROHN 3 STD	27.833	13.557	139.8	2.228	13.892	100.281	0.139 ¹ ✓

¹ P_u / φP_n controls

Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T1	212.625 - 202.458	ROHN 1.5 STD	8.500	4.130	79.6	0.799	0.256	35.976	0.007 ¹ ✓

¹ P_u / φP_n controls

Redundant Horizontal (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T10	40.6667 - 20.3333	ROHN 1.5 STD	6.294	5.935	114.4	0.799	5.324	25.902	0.206 ¹ ✓
T11	20.3333 - 0	ROHN 1.5 STD	6.958	6.599	127.2	0.799	5.823	25.902	0.225 ¹ ✓

¹ P_u / φP_n controls

Redundant Diagonal (1) Design Data (Tension)

tnxTower B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job	100083.002.01 - HRT 105 943201, CT (BU# 806363)	Page	41 of 44
	Project		Date	16:57:31 01/25/18
	Client	Crown Castle	Designed by	Yathish

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T10	40.6667 - 20.3333	ROHN 2 STD	11.628	10.887	166.0	1.075	4.918	34.815	0.141 ¹
T11	20.3333 - 0	ROHN 2 STD	12.021	11.347	173.0	1.075	5.030	34.815	0.144 ¹

¹ P_u / φP_n controls

Redundant Hip (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T10	40.6667 - 20.3333	ROHN 1.5 STD	6.294	6.294	121.3	0.799	0.017	25.902	0.001 ¹
T11	20.3333 - 0	ROHN 1.5 STD	6.958	6.958	134.1	0.799	0.016	25.902	0.001 ¹

¹ P_u / φP_n controls

Redundant Hip Diagonal (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T10	40.6667 - 20.3333	ROHN 2.5 STD	15.204	15.204	192.6	1.704	0.080	55.211	0.001 ¹
T11	20.3333 - 0	ROHN 2.5 STD	16.022	16.022	202.9	1.704	0.076	55.211	0.001 ¹

¹ P_u / φP_n controls

Inner Bracing Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T1	212.625 - 202.458	L2x2x1/8	4.250	4.250	81.4	0.484	0.004	15.694	0.000 ¹
T2	202.458 - 182.292	L2x2x1/8	4.299	4.299	82.4	0.484	0.007	15.694	0.000 ¹
T3	182.292 - 162.104	L2x2x1/8	4.313	4.313	82.6	0.484	0.005	15.694	0.000 ¹
T4	162.104 - 141.896	L2x2x1/8	5.354	5.354	102.6	0.484	0.006	15.694	0.000 ¹

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	Client Crown Castle	Designed by Yathish

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T5	141.896 - 121.688	L2x2x1/8	6.396	6.396	122.6	0.484	0.004	15.694	0.000 ¹ ✓
T6	121.688 - 101.479	L2 1/2x2 1/2x3/16	7.521	7.521	116.0	0.902	0.002	29.225	0.000 ¹ ✓
T7	101.479 - 81.2708	L3x3x3/16	8.771	8.771	112.1	1.090	0.001	35.316	0.000 ¹ ✓
T11	20.3333 - 0	ROHN 3 STD	13.917	13.917	143.5	2.228	0.000	100.281	0.000 ¹ ✓

¹ P_u / φP_n controls

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	φP _{allow} K	% Capacity	Pass Fail
T1	212.625 - 202.458	Leg	ROHN 2.5 STD	1	-5.717	56.631	10.1	Pass
T2	202.458 - 182.292	Leg	ROHN 3 EH	28	-30.268	93.888	32.2	Pass
T3	182.292 - 162.104	Leg	ROHN 4 EH	67	-76.131	159.259	47.8	Pass
T4	162.104 - 141.896	Leg	ROHN 5 EH	108	-128.440	238.686	53.8	Pass
T5	141.896 - 121.688	Leg	ROHN 6 EHS	146	-160.525	242.933	66.1	Pass
T6	121.688 - 101.479	Leg	ROHN 6 EH	173	-196.727	302.237	65.1	Pass
T7	101.479 - 81.2708	Leg	ROHN 6 EH	200	-229.984	302.237	76.1	Pass
T8	81.2708 - 61	Leg	ROHN 8 EHS	227	-261.735	384.981	68.0	Pass
T9	61 - 40.6667	Leg	ROHN 8 EHS	254	-292.818	384.707	76.1	Pass
T10	40.6667 - 20.3333	Leg	ROHN 8 EH	281	-306.548	503.236	60.9	Pass
T11	20.3333 - 0	Leg	ROHN 8 EH	314	-335.559	503.352	66.7	Pass
T1	212.625 - 202.458	Diagonal	ROHN 2 STD	14	-3.494	23.829	14.7	Pass
T2	202.458 - 182.292	Diagonal	ROHN 2 STD	39	-10.586	17.541	60.4	Pass
T3	182.292 - 162.104	Diagonal	ROHN 2 STD	77	-10.843	15.159	71.5	Pass
T4	162.104 - 141.896	Diagonal	ROHN 2 STD	117	-10.858	13.026	83.4	Pass
T5	141.896 - 121.688	Diagonal	ROHN 2.5 STD	156	-14.129	16.287	86.8	Pass
T6	121.688 - 101.479	Diagonal	ROHN 2.5 STD	183	-13.171	14.278	92.2	Pass
T7	101.479 - 81.2708	Diagonal	ROHN 3 STD	210	-13.495	24.700	54.6	Pass
T8	81.2708 - 61	Diagonal	ROHN 3 STD	237	-13.437	21.813	61.6	Pass
T9	61 - 40.6667	Diagonal	ROHN 3 STD	264	-14.582	19.146	76.2	Pass
T10	40.6667 - 20.3333	Diagonal	ROHN 3 STD	303	-20.582	31.156	66.1	Pass
T11	20.3333 - 0	Diagonal	ROHN 3 STD	336	-23.698	29.608	80.0	Pass

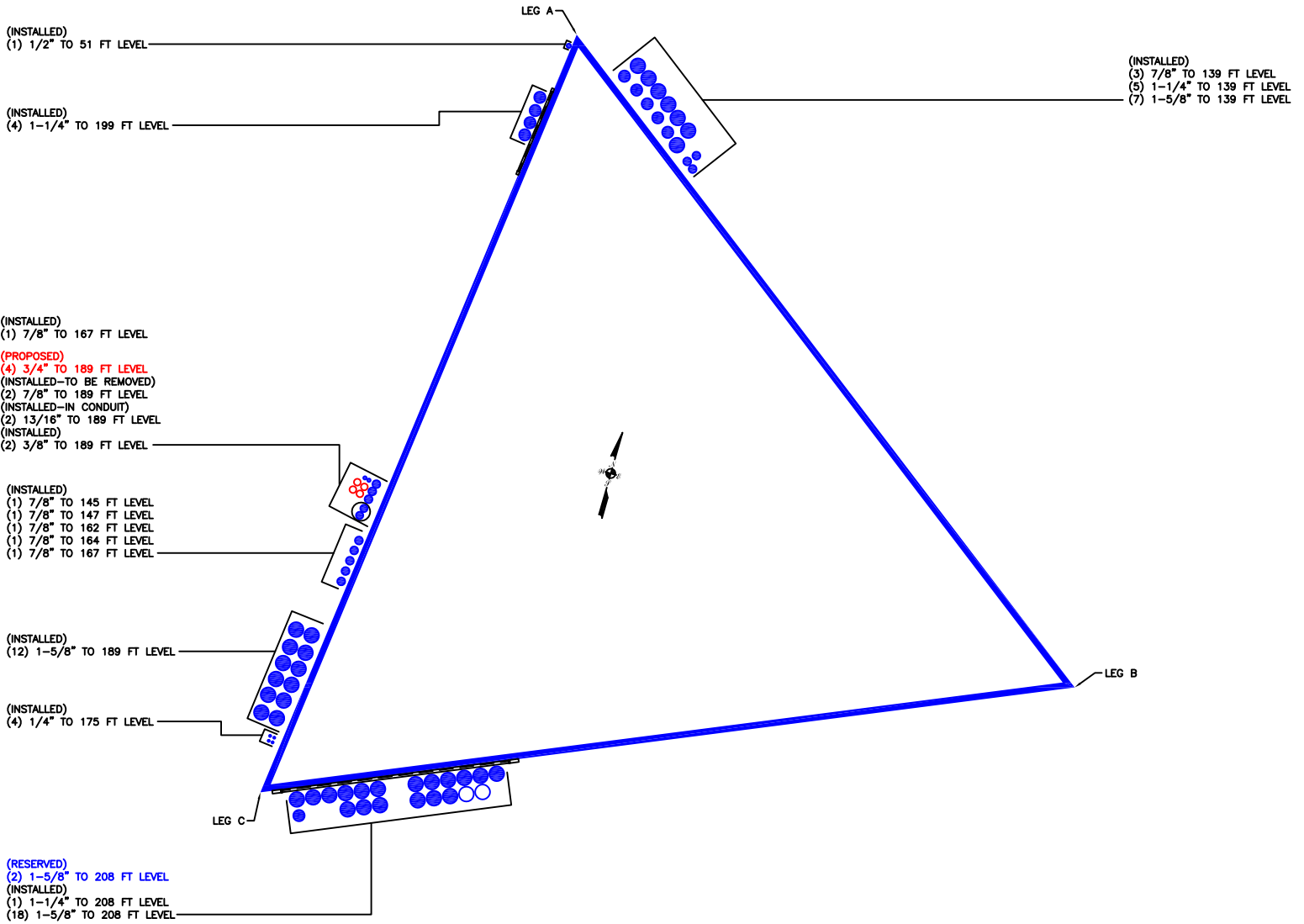
tnxTower B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job 100083.002.01 - HRT 105 943201, CT (BU# 806363)	Page 43 of 44
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	Client Crown Castle	Designed by Yathish

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail	
T1	212.625 - 202.458	Horizontal	ROHN 1.5 STD	13	-2.534	22.582	11.2	Pass	
T2	202.458 - 182.292	Horizontal	ROHN 1.5 STD	37	-5.794	22.520	25.7	Pass	
T3	182.292 - 162.104	Horizontal	ROHN 1.5 STD	76	-6.833	19.143	35.7	Pass	
T4	162.104 - 141.896	Horizontal	ROHN 2 STD	115	-7.578	27.209	27.9	Pass	
T5	141.896 - 121.688	Horizontal	ROHN 2 STD	154	-8.521	22.640	30.5 (b) 37.6	Pass	
T6	121.688 - 101.479	Horizontal	ROHN 2 STD	181	-8.739	16.864	51.8	Pass	
T7	101.479 - 81.2708	Horizontal	ROHN 2.5 STD	208	-9.608	28.852	33.3 38.7 (b)	Pass	
T8	81.2708 - 61	Horizontal	ROHN 2.5 STD	235	-10.137	22.530	45.0	Pass	
T9	61 - 40.6667	Horizontal	ROHN 2.5 STD	262	-11.495	17.820	64.5	Pass	
T10	40.6667 - 20.3333	Horizontal	ROHN 3 STD	299	-11.375	31.651	35.9	Pass	
T11	20.3333 - 0	Horizontal	ROHN 3 STD	332	-13.885	25.753	53.9	Pass	
T1	212.625 - 202.458	Top Girt	ROHN 1.5 STD	5	-0.256	22.635	1.1	Pass	
T10	40.6667 - 20.3333	Redund Horz 1 Bracing	ROHN 1.5 STD	288	-5.324	13.007	40.9	Pass	
T11	20.3333 - 0	Redund Horz 1 Bracing	ROHN 1.5 STD	321	-5.823	11.053	52.7	Pass	
T10	40.6667 - 20.3333	Redund Diag 1 Bracing	ROHN 2 STD	293	-4.918	8.811	55.8	Pass	
T11	20.3333 - 0	Redund Diag 1 Bracing	ROHN 2 STD	322	-5.030	8.111	62.0	Pass	
T10	40.6667 - 20.3333	Redund Hip 1 Bracing	ROHN 1.5 STD	306	-0.029	11.936	0.2	Pass	
T11	20.3333 - 0	Redund Hip 1 Bracing	ROHN 1.5 STD	339	-0.027	10.041	0.3	Pass	
T10	40.6667 - 20.3333	Redund Hip Diagonal 1 Bracing	ROHN 2.5 STD	307	-0.094	10.381	0.9	Pass	
T11	20.3333 - 0	Redund Hip Diagonal 1 Bracing	ROHN 2.5 STD	340	-0.086	9.348	0.9	Pass	
T1	212.625 - 202.458	Inner Bracing	L2x2x1/8	17	-0.004	6.538	0.5	Pass	
T2	202.458 - 182.292	Inner Bracing	L2x2x1/8	41	-0.007	6.440	0.5	Pass	
T3	182.292 - 162.104	Inner Bracing	L2x2x1/8	79	-0.006	4.790	0.6	Pass	
T4	162.104 - 141.896	Inner Bracing	L2x2x1/8	120	-0.007	3.283	0.7	Pass	
T5	141.896 - 121.688	Inner Bracing	L2x2x1/8	159	-0.009	2.480	0.8	Pass	
T6	121.688 - 101.479	Inner Bracing	L2 1/2x2 1/2x3/16	186	-0.011	5.225	0.7	Pass	
T7	101.479 - 81.2708	Inner Bracing	L3x3x3/16	212	-0.014	6.881	0.7	Pass	
T8	81.2708 - 61	Inner Bracing	L3 1/2x3 1/2x1/4	239	-0.017	11.196	0.5	Pass	
T9	61 - 40.6667	Inner Bracing	L3 1/2x3 1/2x1/4	266	-0.017	8.922	0.5	Pass	
T10	40.6667 - 20.3333	Inner Bracing	ROHN 3 STD	311	-0.020	29.869	0.3	Pass	
T11	20.3333 - 0	Inner Bracing	ROHN 3 STD	344	-0.017	24.440	0.2	Pass	
							Summary		
							Leg (T9)	76.1	Pass
							Diagonal (T6)	92.2	Pass
							Horizontal	64.5	Pass

tnxTower B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job 100083.002.01 - HRT 105 943201, CT (BU# 806363)	Page 44 of 44
	Project	Date 16:57:31 01/25/18
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Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail
						(T9)		
						Top Girt (T1)	1.1	Pass
						Redund Horz 1 Bracing (T11)	52.7	Pass
						Redund Diag 1 Bracing (T11)	62.0	Pass
						Redund Hip 1 Bracing (T11)	0.3	Pass
						Redund Hip Diagonal 1 Bracing (T11)	0.9	Pass
						Inner Bracing (T5)	0.8	Pass
						Bolt Checks	60.9	Pass
						RATING =	92.2	Pass

APPENDIX B
BASE LEVEL DRAWING



BUSINESS UNIT: 806363

APPENDIX C
ADDITIONAL CALCULATIONS

Anchor Rod Check for Self Supporting Towers

TIA-222-G, Section 4.9.9

V7.3

Site Data	
BU#:	806363
Site Name:	HRT 105 943201, CT
App #:	421190 Revision # 3

No. of Mods:	0
--------------	---

Anchor Rod Data			
	Existing		
Qty:	10		
Diam:	1		in
Rod Material:	BC (1/4 to 2)		
Strength (Fu):	125		ksi
Yield (Fy):	109		ksi
Net Area (An):	6.06		in ²

Reactions		
Eta Factor, η	0.55	Detail Type
Uplift, Pu:	311	kips
Shear, Vu:	39	kips

l_{ar} :		in
$M_u = 0.65 * l_{ar} * V_u$		ft-kips

Anchor Rod Results:	Existing		
Rod Vu:	3.90		k
Max Rod (Cu+ Vu/η):	38.19		k
Design Axial, Φ*Fu*Anet:	60.6		k
Anchor Rod Stress Ratio:	63.0%		

If Applicable;

$$\left(\frac{V_u}{\phi R_{nv}}\right)^2 + \left[\left(\frac{P_u}{\phi R_{nt}}\right) + \left(\frac{M_u}{\phi R_{nm}}\right)\right]^2 \leq 1$$

Anchor Rod Results with Bending Considered:

When the clear distance from the top of concrete to the bottom of level nut exceeds 1.0 times the diameter of the anchor rod, the following interaction equation shall also be satisfied (see Figure 4-4 of Rev. G):

$$\begin{aligned} \phi R_{nv} &= \phi * 0.45 * F_{ub} * A_b = \text{ } \text{ kips} \\ \phi R_{nt} &= \phi * F_u * A_{net} = \text{ } \text{ kips} \\ \phi R_{nm} &= \phi * F_y * Z = \text{ } \text{ ft-kips} \end{aligned}$$

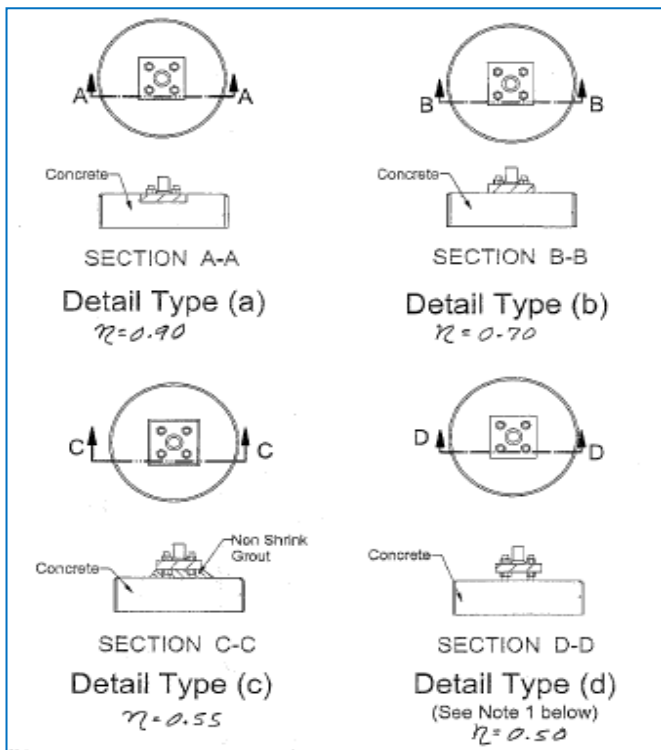


Figure 4-4 of TIA-222-G

Eccentric Load Calculations		
MOI=	246	in ⁴
e=	0	in
Distance to Extreme fibre, y ₀ =	0.0	in
Eccentric load on Existing=	0.000	kips

Maximum Acceptable Ratio: 105 %

Governing Stress Ratio: 63.0% **Pass**

SST Unit Base Foundation



BU # : 806363
 Site Name: HRT 105 943201,
 App. Number: 421190 Revision #

TIA-222 Revision: G

Tower Centroid Offset?:
 Block Foundation?:

Superstructure Analysis Reactions		
Global Moment, M :	8763	ft-kips
Global Axial, P :	100	kips
Global Shear, V :	71	kips
Leg Compression, P_{comp} :	368	kips
Leg Comp. Shear, V_{u, comp} :	43	kips
Leg Uplift, P_{uplift} :	311	kips
Leg Uplift. Shear, V_{u, uplift} :	39	kips
Tower Height, H :	212.625	ft
Base Face Width, BW :	30.041667	ft
BP Dist. Above Fdn, bp_{dist} :	3	in
Anchor Bolt Circle, BC :	12	in

Foundation Analysis Checks				
	Capacity	Demand	Rating	Check
<i>Lateral (Sliding) (kips)</i>	343.72	71.00	20.7%	Pass
<i>Bearing Pressure (ksf)</i>	6.00	1.39	23.1%	Pass
<i>Overturing (kip*ft)</i>	17616.57	9100.25	51.7%	Pass
<i>Pad Flexure (kip*ft)</i>	7259.38	1870.15	25.8%	Pass
<i>Pad Shear - 1-way (kips)</i>	1974.17	215.26	10.9%	Pass
<i>Pad Shear - 2-way (ksi)</i>	0.16	0.04	22.9%	Pass

Soil Rating: 51.7%
 Structural Rating: 25.8%

Pad Properties		
Depth, D :	4.00	ft
Pad Width, W :	40.30	ft
Pad Thickness, T :	4.50	ft
Pad Rebar Size (Bottom), Sp :	7	
Pad Rebar Quantity (Bottom), mp :	55	
Pad Clear Cover, cc_{pad} :	3	in

Material Properties		
Rebar Grade, Fy :	60000	psi
Concrete Compressive Strength, F'c :	3000	psi
Dry Concrete Density, δc :	150	pcf

Soil Properties		
Total Soil Unit Weight, γ :	120	pcf
Ultimate Gross Bearing, Qult :	8.000	ksf
Cohesion, Cu :	0.000	ksf
Friction Angle, φ :	35	degrees
SPT Blow Count, N_{blows} :		
Base Friction, μ :		
Neglected Depth, N :	3.3	ft
Foundation Bearing on Rock?	No	
Groundwater Depth, gw :	3	ft

<-- Toggle between Gross and Net

Exposure Category Determination
BU#806363



- Latitude/Longitude = 41° 17' 20.2", -72° 32' 18.5"
- Tower Height = 212 ft
- Upwind Fetch Radius = Greater of 25 x Tower Height or 3250 ft = 5300 ft
- Minimum Open Patch = 164 ft x 164 ft
- Maximum continuous surface roughness category C arc angle = 15 degrees
- Kmz file saved in folder ... R:\SA Models - Letters\Work Area\Exposure_Topo_KMZ



Exposure Category for this site is **B**.

The determination is based on Crown Castle standard ENG-PRC-10202, Determination of Exposure Category, revision C.

Completed by: Erin Doyle

Approved by: Jason Hedrich

Date: 11/05/2015

Date: 11/06/2015



Unmitigated Percentage (B/C)

Inputs

Tower Height (ft):	180'
Starting Azimuth:	30°
Upwind Fetch Radius (ft):	4500'
20% Unmitigated Limit (ft):	900'
Overlay Size Selected:	30°

Subsector (Degrees)	Total Unmitigated Length (ft)	Percentage of Subsector Unmitigated
15°		0.0%
20°		0.0%
25°	'	0.0%
30°	'	0.0%
35°	295'	6.6%
40°	'	0.0%
45°	'	0.0%
50°	'	0.0%
55°	'	0.0%
60°	'	0.0%
65°		0.0%
70°		0.0%
75°		0.0%
80°		0.0%

THIS SITE IS EXPOSURE:	B
-------------------------------	----------

Length measurements should be taken to the nearest 5' increment.

The determination is based on Crown Castle standard ENG-PRC-10202, Determination of Exposure Category, revision C.

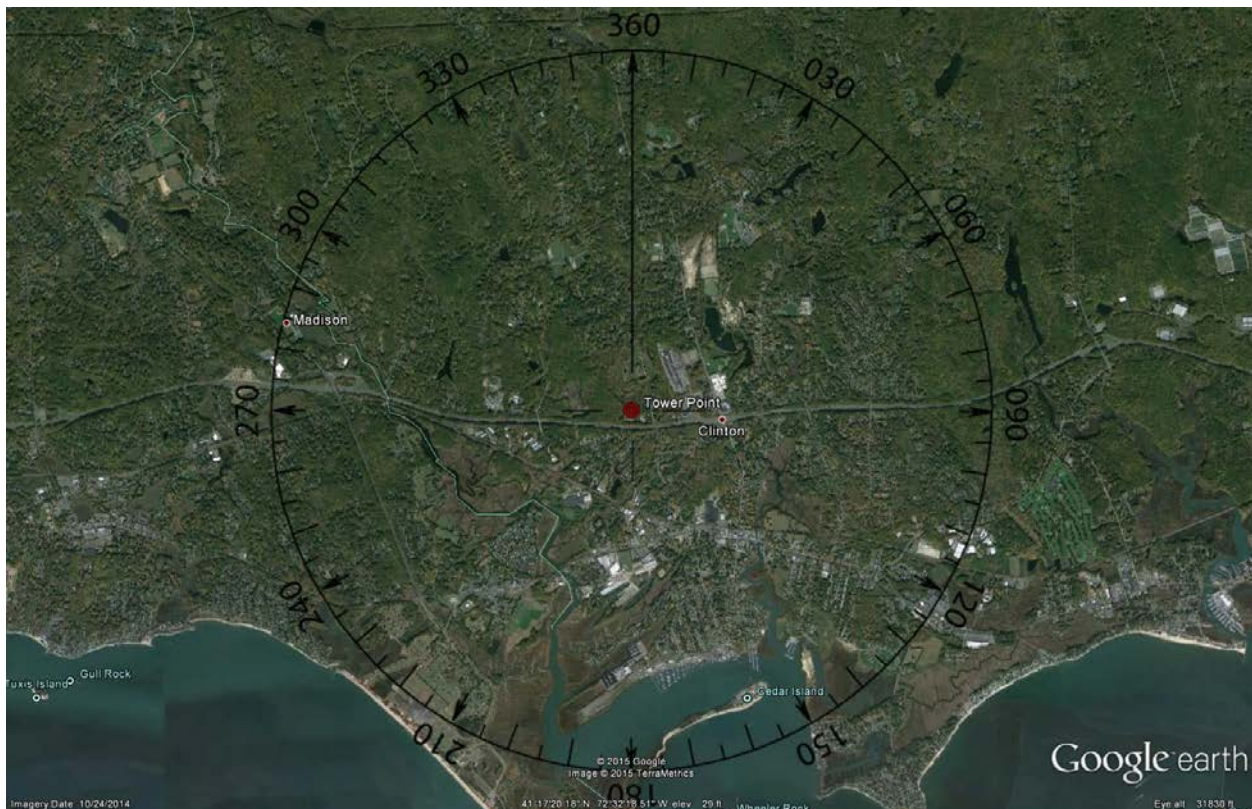
This chart is intended only for use with Exposures B and C and is Not applicable for Exposure D.

LEGEND	
	Considered Subsector
	Bookending Subsector

Topographic Factor Determination BU#806363



- Latitude/Longitude = 41° 17' 20.2", -72° 32' 18.5"
- Tower Height = 212 ft
- Topo Radius = 10,560 ft
- Maximum continuous effective topo arc angle = 0 degrees
- Critical wind azimuth used in topo tool = 0
- Kmz file saved in folder ... R:\SA Models - Letters\Work Area\Exposure_Topo_KMZ



Exposure Category for this site is **B**.
No topo feature.
Topographic Factor (K_{ZF}) at base is 1.0.

The determination is based on Crown Castle standard ENG-PRC-10040, Determination of Topographic Factor, initial release.

Completed by: Erin Doyle

Approved by: Jason Hedrich

Date: 11/05/2015

Date: 11/06/2015



[ASCE 7 Windspeed](#)
[ASCE 7 Ground Snow Load](#)
[Related Resources](#)
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[Contact](#)

Search Results

Query Date: Thu Jan 25 2018

Latitude: 41.2889

Longitude: -72.5385

**ASCE 7-10 Windspeeds
(3-sec peak gust in mph*):**

Risk Category I: 119

Risk Category II: 130

Risk Category III-IV: 140

MRI 10-Year:** 78

MRI 25-Year:** 88

MRI 50-Year:** 97

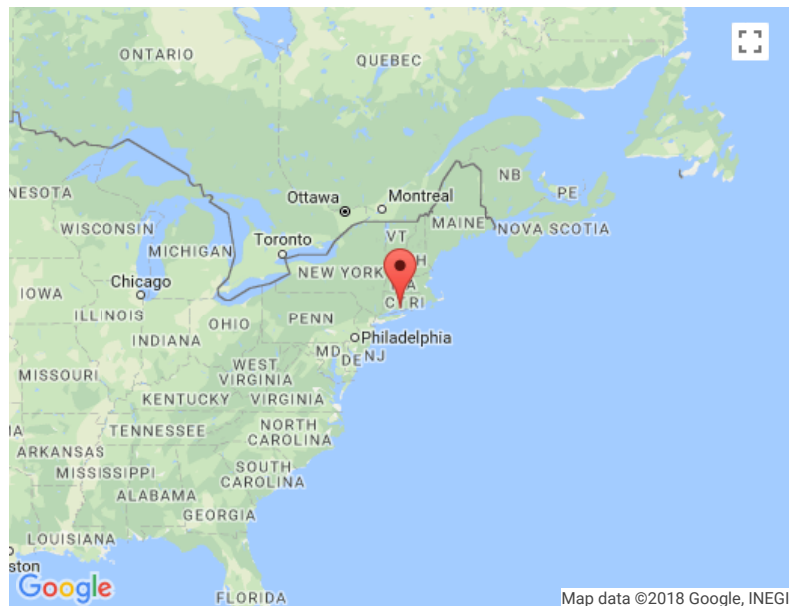
MRI 100-Year:** 106

ASCE 7-05 Windspeed:

115 (3-sec peak gust in mph)

ASCE 7-93 Windspeed:

84 (fastest mile in mph)



*Miles per hour

**Mean Recurrence Interval

Users should consult with local building officials to determine if there are community-specific wind speed requirements that govern.



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49B COW HILL RD

Location 49B COW HILL RD **Mblu** 32/ 6/ 48/ H026570/A
Acct# H0265701 **Owner** HESER RAYMOND
Assessment \$561,600 **Appraisal** \$802,300
PID 106785 **Building Count** 1

Current Value

Appraisal			
Valuation Year	Improvements	Land	Total
2016	\$160,800	\$641,500	\$802,300
Assessment			
Valuation Year	Improvements	Land	Total
2016	\$112,500	\$449,100	\$561,600

Owner of Record

Owner HESER RAYMOND **Sale Price** \$0
Co-Owner CROWN CASTLE ATLANTIC CO LLC **Certificate**
Address 4017 WASHINGTON RD PMB353 **Book & Page** 088/ 061
MCMURRAY, PA 15317 **Sale Date**

Ownership History

Ownership History				
Owner	Sale Price	Certificate	Book & Page	Sale Date
HESER RAYMOND	\$0		088/ 061	
HESER RAYMOND				

Building Information

Building 1 : Section 1

Year Built: 1993
Living Area: 1104
Replacement Cost: \$176,872
Building Percent 87
Good:
Replacement Cost
Less Depreciation: \$153,900

Building Photo

Building Attributes	
Field	Description
STYLE	Telephone Bldg
MODEL	Ind/Comm

Grade	Average
Stories:	1
Occupancy	1
Exterior Wall 1	Brick/Masonry
Exterior Wall 2	
Roof Structure	Flat
Roof Cover	Tar & Gravel
Interior Wall 1	Minim/Masonry
Interior Wall 2	
Interior Floor 1	Concr-Finished
Interior Floor 2	
Heating Fuel	Gas
Heating Type	Hot Air-no Duc
AC Type	Central
Bldg Use	TEL X STA MDL-96
Total Rooms	
Total Bedrms	00
Total Baths	0
1st Floor Use:	4300
Heat/AC	NONE
Frame Type	STEEL
Baths/Plumbing	NONE
Ceiling/Wall	NONE
Rooms/Prtns	AVERAGE
Wall Height	12
% Comn Wall	



(http://images.vgsi.com/photos/ClintonCTPhotos/\00\00\07\11.jpg)

Building Layout



Building Sub-Areas (sq ft)			Legend
Code	Description	Gross Area	Living Area
BAS	First Floor	1104	1104
		1104	1104

Extra Features

Extra Features	Legend
No Data for Extra Features	

Land

Land Use

Use Code	4300
Description	TEL X STA MDL-96
Zone	I-P
Neighborhood	1100
Alt Land Appr Category	No

Land Line Valuation

Size (Acres)	0.18
Frontage	
Depth	
Assessed Value	\$449,100
Appraised Value	\$641,500

Outbuildings

Outbuildings						Legend
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #
FN4	FENCE-8' CHAIN			360 L.F.	\$900	1
PAV2	PAVING-CONC			1296 S.F.	\$2,900	1
SHD5	COMM WOOD			200 S.F.	\$3,100	1

Valuation History

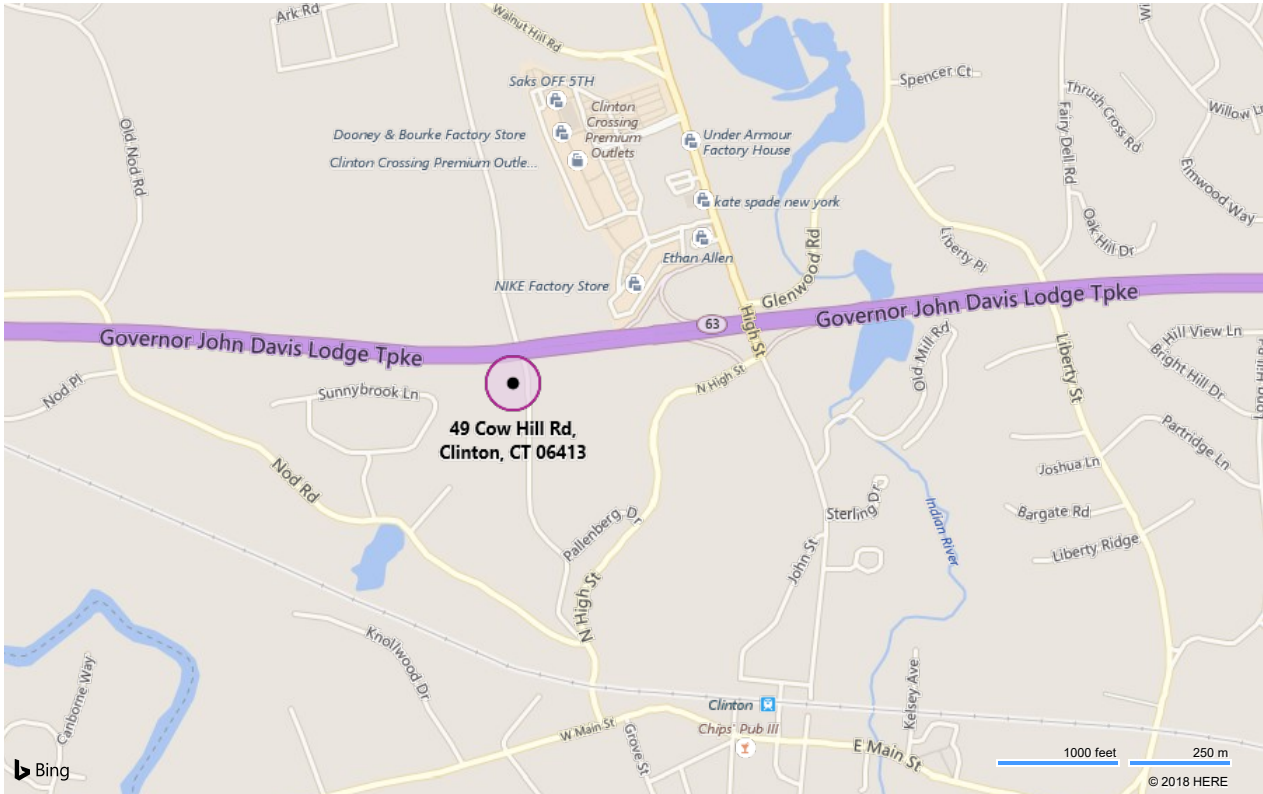
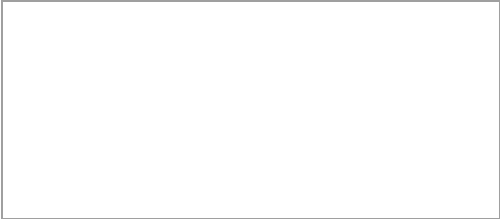
Appraisal			
Valuation Year	Improvements	Land	Total
2010	\$131,500	\$641,500	\$773,000
2009	\$203,500	\$717,300	\$920,800
2005	\$203,500	\$717,300	\$920,800


Assessment			
Valuation Year	Improvements	Land	Total
2010	\$92,200	\$449,100	\$541,300
2009	\$142,600	\$502,100	\$644,700
2005	\$142,600	\$502,100	\$644,700

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49 Cow Hill Rd, Clinton, CT 06413

Location: 41.28688, -72.53608






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usps.com 9405 8036 9930 0600 7117 26 0067 0000 0010 6413
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PRIORITY MAIL 1-DAY™

Expected Delivery Date: 03/05/18

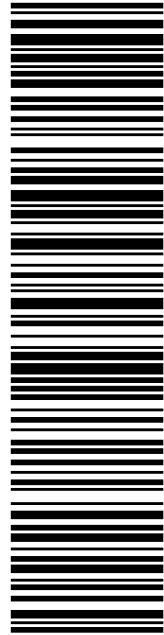
MARK J ROBERTS
 QC DEVELOPMENT
 PO BOX 916
 STORRS CT 06268-0916

0024

C007

SHIP TO: CHRISTINE GOUPIL
 TOWN OF CLINTON
 54 E MAIN ST
 CLINTON CT 06413-2035

USPS TRACKING #



9405 8036 9930 0600 7117 26

Electronic Rate Approved #038555749



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 9405 8036 9930 0600 7117 26**

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Print Date:	03/02/2018	Insurance Fee	\$0.00
Ship Date:	03/03/2018	Total	\$6.70
Expected Delivery Date:	03/05/2018		
Insured Value:	\$50.00		


From: MARK J ROBERTS
 QC DEVELOPMENT
 PO BOX 916
 STORRS CT 06268-0916

To: CHRISTINE GOUPIL
 TOWN OF CLINTON
 54 E MAIN ST
 CLINTON CT 06413-2035

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PRIORITY MAIL 2-DAY™

Expected Delivery Date: 03/05/18

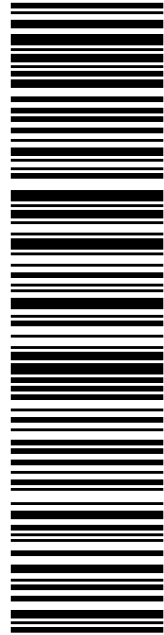
MARK J ROBERTS
 QC DEVELOPMENT
 PO BOX 916
 STORRS CT 06268-0916

0006

R032

SHIP TO: PAUL PEDICONE
 CROWN CASTLE
 3 CORPORATE DR
 STE 101
 HALFMOON NY 12065-8635

USPS TRACKING #



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Click-N-Ship® Label Record

**USPS TRACKING # / Insurance Number:
 9405 8036 9930 0600 7117 33**

Trans. #:	428911728	Priority Mail® Postage:	\$6.70
Print Date:	03/02/2018	Insurance Fee	\$0.00
Ship Date:	03/03/2018	Total	\$6.70
Expected Delivery Date:	03/05/2018		
Insured Value:	\$50.00		

From: MARK J ROBERTS
 QC DEVELOPMENT
 PO BOX 916
 STORRS CT 06268-0916

To: PAUL PEDICONE
 CROWN CASTLE
 3 CORPORATE DR
 STE 101
 HALFMOON NY 12065-8635

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