

# STATE OF CONNECTICUT

## CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: [siting.council@ct.gov](mailto:siting.council@ct.gov)

Internet: [ct.gov/csc](http://ct.gov/csc)

Daniel F. Caruso

Chairman

October 24, 2007

Kenneth C. Baldwin, Esq.  
Robinson & Cole LLP  
280 Trumbull Street  
Hartford, CT 06103-3597

RE: **EM-VER-023-070925** – Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at 540 Cherry Brook Road, Canton, Connecticut.

Dear Attorney Baldwin:

At a public meeting held on October 16, 2007, the Connecticut Siting Council (Council) acknowledged your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies.

The proposed modifications are to be implemented as specified here and in your notice dated September 25, 2007, including the placement of all necessary equipment and shelters within the tower compound. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Any deviation from this format may result in the Council implementing enforcement proceedings pursuant to General Statutes § 16-50u including, without limitation, imposition of expenses resulting from such failure and of civil penalties in an amount not less than one thousand dollars per day for each day of construction or operation in material violation.

Thank you for your attention and cooperation.

Very truly yours,

Daniel F. Caruso  
Chairman

DFC/MP/cm

c: The Honorable Mary B. Tomolonius, First Selectman, Town of Canton  
Neil Pade, Town Planner, Town of Canton  
SBA

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Affirmative Action / Equal Opportunity Employer



STATE OF CONNECTICUT  
CONNECTICUT SITING COUNCIL

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Internet: [ct.gov/csc](http://ct.gov/csc)

Daniel F. Caruso  
Chairman

October 2, 2007

The Honorable Mary B. Tomolonius  
First Selectman  
Town of Canton  
4 Market Street  
P. O. Box 168  
Collinsville, CT 06022-0168

RE: **EM-VER-023-070925** – Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at 540 Cherry Brook Road, Canton, Connecticut.

Dear Ms. Tomolonius:

The Connecticut Siting Council (Council) received this request to modify an existing telecommunications facility, pursuant to Regulations of Connecticut State Agencies Section 16-50j-72.

The Council will consider this item at the next meeting scheduled for October 16, 2007, at 2:00 p.m. in Hearing Room Two, Ten Franklin Square, New Britain, Connecticut.

If you have any questions or comments regarding this proposal, please call me or inform the Council by October 15, 2007

Thank you for your cooperation and consideration.

Very truly yours,

S. Derek Phelps  
Executive Director

SDP/cm

Enclosure: Notice of Intent

c: Neil Pade, Town Planner, Town of Canton

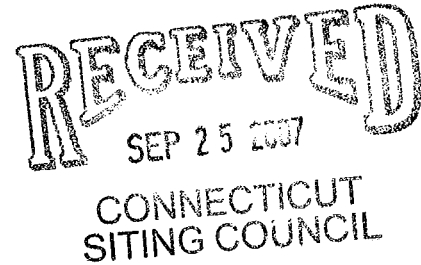
EM-VER-023-070925

280 Trumbull Street  
Hartford, CT 06103-3597  
Main (860) 275-8200  
Fax (860) 275-8299  
kbaldwin@rc.com  
Direct (860) 275-8345

September 25, 2007

*Via Hand Delivery*

S. Derek Phelps  
Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051



Re: **Notice of Exempt Modification**  
**540 Cherry Brook Road**  
**Canton, Connecticut**

Dear Mr. Phelps:

Cellco Partnership d/b/a Verizon Wireless ("Cellco") intends to install antennas on an existing 150-foot self-supporting monopole tower owned by SBA at 540 Cherry Brook Road in Canton, Connecticut. Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Canton First Selectwoman, Mary B. Tomolonius. Pursuant to Council directive, a copy of this letter is also being sent to the North Canton Fire Department, the owner of the land on which the tower is located.

The facility consists of a 150-foot self-supporting monopole tower capable of supporting multiple carriers within a fenced compound at the North Canton Fire Department, 540 Cherry Brook Road in Canton. The tower is currently shared by AT&T with antennas at the 140-foot level and the Town of Canton with whip antennas extending off the top of the tower. The Council approved Cellco's initial request to place twelve cellular antennas at the 150-foot level of the tower on March 15, 2001 (TS-VER-023-010216-1). The antennas approved through TS-VER-023-010216-1 were never installed. Cellco now intends to move forward with development of this site and install twelve (12) panel-type antennas (six cellular and six PCS) at the 150-foot level on the tower. Cellco will also place a 12' x 30' equipment shelter on the ground at the base of the tower within the existing fenced compound. Cello's back-up generator will be fueled by propane. A 1000-gallon propane tank will also be installed inside to fenced compound. Attached behind Tab 1 are Project Plans for the proposed Cellco installation.



*Law Offices*

BOSTON

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NEW YORK CITY

SARASOTA

*www.rc.com*

HART1-1424085-1

S. Derek Phelps  
September 25, 2007  
Page 2


The planned modifications to the Canton facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2):

1. The proposed modification will not increase the overall height of the existing tower. Cellco's antennas will be mounted with their centerline at the 150-foot level on the 150-foot tower.
2. The proposed installation of a 12' x 30' equipment shelter will not require an extension of the fenced compound or lease area.
3. The proposed installation will not increase the noise levels at the facility by six decibels or more.
4. The operation of the antennas will not increase radio frequency ("RF") power density levels at the facility to a level at or above the Federal Communications Commission ("FCC") adopted safety standard. The cumulative worst-case RF power density calculations for the existing antennas and Cellco antennas would be 19.50% of the FCC standard. A copy of the cumulative power density calculations table is attached behind Tab 2.

Also attached, behind Tab 3, is a Structural Analysis confirming that the tower can support the existing and proposed antennas and associated equipment. Please note that the Structural Analysis assumed the previously approved antennas had been installed.

For the foregoing reasons, Cellco respectfully submits that the proposed antenna installation at the facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,



Joey Lee Miranda

Attachments

Copy to:

Mary B. Tomolonius, Canton First Selectwoman  
North Canton Fire Department  
Sandy M. Carter



# Cellco Partnership

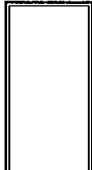
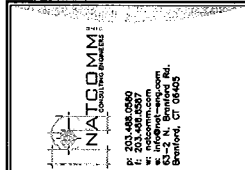
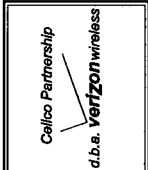
## d.b.a. **verizon** wireless

### WIRELESS COMMUNICATIONS FACILITY

#### NORTH CANTON

#### 540 CHERRY BROOK ROAD NORTH CANTON, CT 06019

REVISIONS		
A	08/24/07	LOG-REVIEW
0	08/24/07	CSG



**NORTH CANTON**  
540 CHERRY BROOK ROAD  
NORTH CANTON, CT 06019

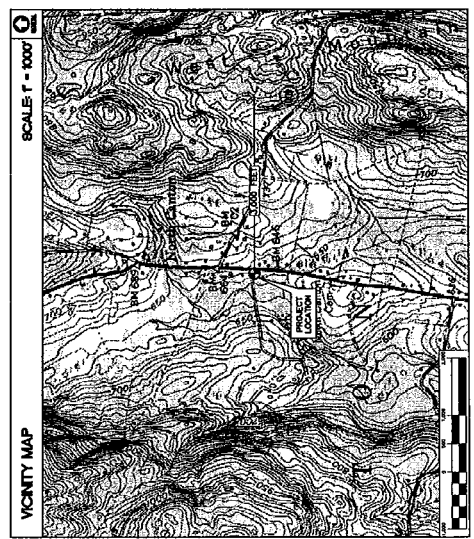
PROJECT NO: 07859  
DRAWN BY: DEB  
CHECKED BY: CTC  
SCALE: AS NOTED  
DATE: 08/28/07

TITLE SHEET

T-1  
DWG. \_1\_ OF 2

PROJECT SUMMARY	
SITE NAME:	NORTH CANTON
SITE ADDRESS:	540 CHERRY BROOK ROAD NORTH CANTON, CT 06019
PROPERTY OWNER:	SBA NETWORK SERVICES, INC. PO BOX 100 ROCKY HAVEN, PA 13427
LESSEE/TENANT:	CELLCO PARTNERSHIP VERIZON WIRELESS 915 EAST BRIDGE WAY EAST HARTFORD, CT 06108
CONTACT PERSON:	CELLCO PARTNERSHIP (860) 602-8219
TOWER COORDINATES:	LATITUDE: 41°53'37.32" LONGITUDE: 72°56'58.92" COORDINATES BASED ON CSC WEB LOG LIST UPDATED 07/28/07

SHEET INDEX	
SHT. NO.	REV. NO.
1-1	0
C-1	0
COMPOUND PLAN AND ELEVATION	

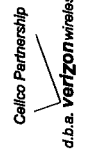



FROM:	TO:	540 CHERRY BROOK ROAD NORTH CANTON, CONNECTICUT
1. START AT 540 CHERRY BROOK ROAD	TO: 540 CHERRY BROOK ROAD	0.0 M
2. TURN LEFT ON E BRIDGE RD.	TO: 540 CHERRY BROOK ROAD	0.3 M
3. TURN RIGHT ON E BRIDGE RD.	TO: 540 CHERRY BROOK ROAD	0.2 M
4. TAKE RIGHT ON E BRIDGE RD.	TO: 540 CHERRY BROOK ROAD	0.2 M
5. TAKE LEFT ON E BRIDGE RD.	TO: 540 CHERRY BROOK ROAD	0.2 M
6. BEAR RIGHT TO FOLLOW W. 540 CHERRY BROOK RD.	TO: 540 CHERRY BROOK ROAD	0.4 M
7. BEAR RIGHT TO FOLLOW W. 540 CHERRY BROOK RD.	TO: 540 CHERRY BROOK ROAD	0.2 M
8. BEAR LEFT ON W. 540 CHERRY BROOK RD.	TO: 540 CHERRY BROOK ROAD	0.2 M
9. BEAR RIGHT ON W. 540 CHERRY BROOK RD.	TO: 540 CHERRY BROOK ROAD	0.2 M
10. TURN LEFT ON W. 540 CHERRY BROOK RD.	TO: 540 CHERRY BROOK ROAD	0.2 M
11. TURN LEFT ON W. 540 CHERRY BROOK RD.	TO: 540 CHERRY BROOK ROAD	0.2 M
12. TURN LEFT ON W. 540 CHERRY BROOK RD.	TO: 540 CHERRY BROOK ROAD	0.2 M
13. TURN LEFT ON W. 540 CHERRY BROOK RD.	TO: 540 CHERRY BROOK ROAD	0.2 M
14. ARRIVE AT 540 CHERRY BROOK ROAD	TO: 540 CHERRY BROOK ROAD	0.1 M

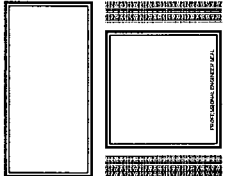
**GENERAL NOTES**  
1. PROPOSED ANTENNA LOCATIONS AND HEIGHTS PROVIDED BY CELLCO PARTNERSHIP.

**PROJECT SCOPE**  
1. THE PROPOSED SCOPE OF WORK GENERALLY INCLUDES THE INSTALLATION OF A 12'x30' PRECAST/CAST-IN-PLACE WIRELESS EQUIPMENT SHELTER ON A CONCRETE FOUNDATION WITHIN THE EXISTING WIRELESS COMMUNICATIONS COMPOUND.  
2. A TOTAL OF TWELVE (12) DIRECTIONAL PANEL ANTENNAS ARE PROPOSED TO BE INSTALLED ON THE EXISTING WIRELESS COMMUNICATIONS TOWER WITH AN ELEVATION OF 165' ABOVE THE EXISTING TOWER BASE PLATE.  
3. ELECTRIC AND TALEO UTILITIES SHALL BE ROUTED UNDERGROUND TO THE PROPOSED EQUIPMENT SHELTER FROM AN EXISTING UTILITY BOARDROOM LOCATED WITHIN THE EXISTING WIRELESS COMMUNICATIONS COMPOUND.  
4. WIRELESS EQUIPMENT SHELTERS ARE PROPOSED TO BE INSTALLED ON A CONCRETE FOUNDATION WITHIN THE EXISTING WIRELESS COMMUNICATIONS COMPOUND FOR REDUNDANT POWER CAPABILITY LOCATED WITHIN THE PROPOSED EQUIPMENT SHELTER.

REVISIONS	
A	08/29/07   CFC REVIEW
B	09/24/07   CFC
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 Verizon wireless  
 d.b.a. Verizon Wireless

  
 NATCOM  
 252-428-8267  
 202-482-2507  
 natcom.com  
 65-2 N. Brentford Rd.  
 Brentford, CT 06408



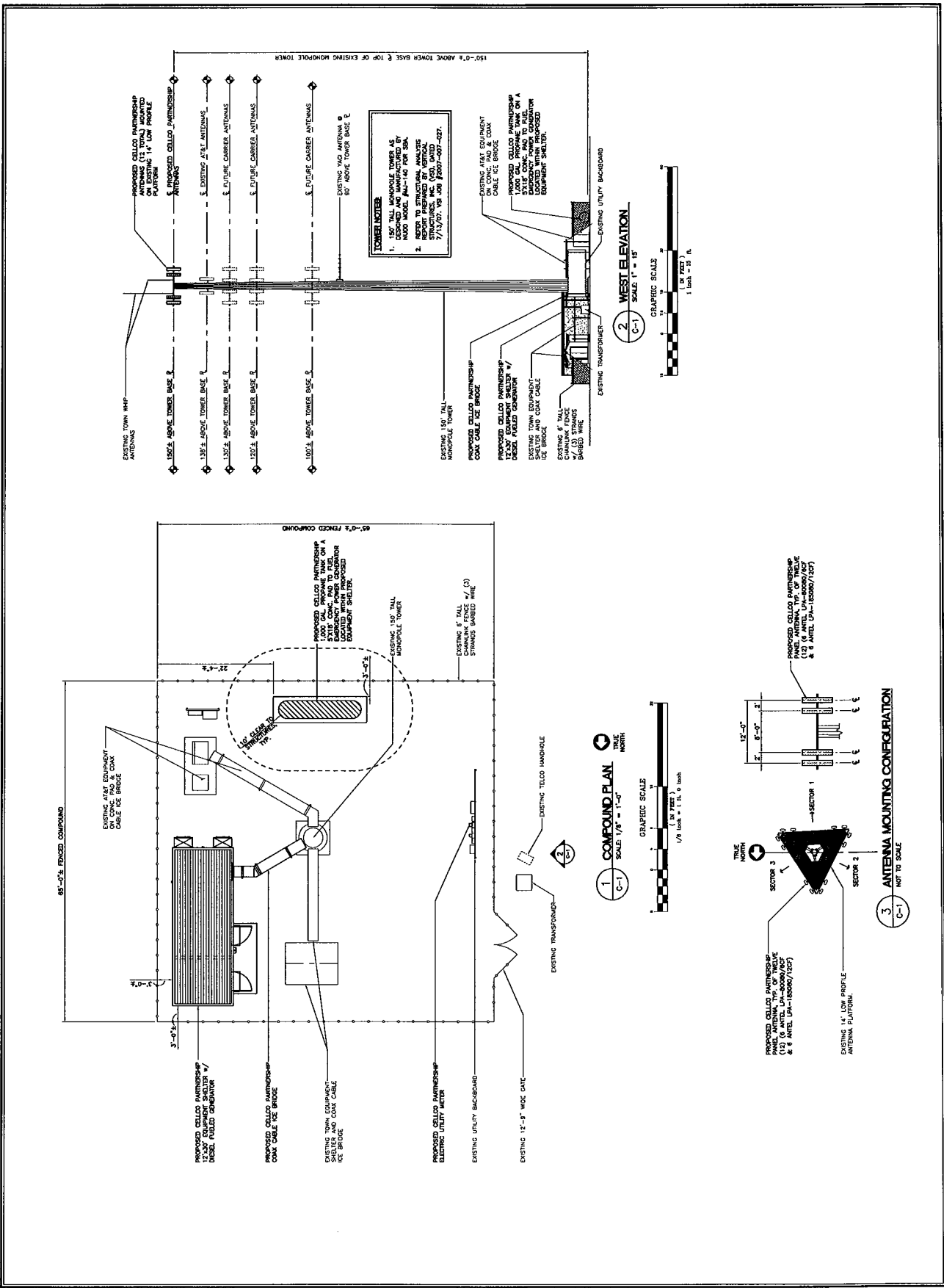
**NORTH CANTON**  
 540 CHERRY BROOK ROAD  
 NORTH CANTON, CT 06459

PROJECT NO: 07469  
 DRAWN BY: DEB  
 CHECKED BY: CFC  
 SCALE: AS NOTED  
 DATE: 08/28/07

COMPOUND  
 PLAN AND  
 ELEVATION

C-1

DWG. 2, OF 2







July 13, 2007

Mr. Mark Luther  
SBA Network Services  
800 South Washington Ave.  
Scranton, PA 18505  
(570) 558-3450

Subject: **Structural Analysis Report  
Verizon Wireless Change-Out  
SBA Site Name: Canton 2, CT  
SBA Site Number: CT01500-S  
150' Nudd MJ-140 Monopole Tower  
Vertical Structures Job Number: 2007-007-027**

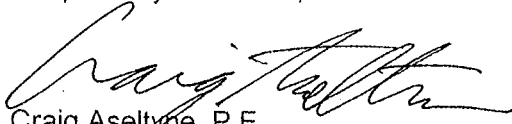
Dear Mr. Luther,

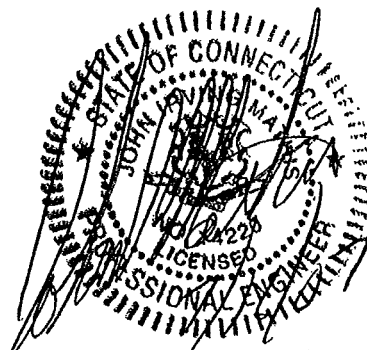
Vertical Structures is pleased to provide you with the results of the structural analysis performed on the 150' tall monopole tower at the Canton 2 site in Canton, Connecticut. The purpose of the analysis was to determine the suitability of the tower upon replacing the twelve (12) existing Decibel DB844H80-XY panel antennas mounted on an existing low-profile platform at 150' with six (6) proposed Antel LPA-80080/6CF panel antennas and six (6) proposed Antel LPA-185080/12CF panel antennas for Verizon Wireless when combined with the existing and reserved equipment on the structure. This analysis has been performed in accordance with the TIA/EIA-222-F standard and local code requirements based upon an 80 MPH basic "fastest mile" wind speed, equivalent to a 95 MPH basic "3-second gust" wind speed per IBC Table 1609.3.1.

Based on our analysis we have determined the tower superstructure and foundation are sufficient for the existing and proposed loading.

Vertical Structures appreciates the opportunity to provide this report and our continuing professional services. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted,

  
Craig Aseltine, P.E.  
Project Engineer





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## INTRODUCTION

The subject tower is located in Canton Center, Connecticut. The 150' tall Nudd MJ-140 monopole tower was designed and manufactured in 2000 for SBA. The tower consists of three (4) 18-sided tapered polygonal sections joined via slip joint connections and one (1) pipe section joined via a bolted flange connection. The tower is founded on a 30' square by 4' thick mat bearing 6' below grade.

## ANALYSIS CRITERIA

The Canton 2 monopole tower was analyzed in accordance with the current EIA-222-F publication, "Structural Standards for Steel Antenna Towers and Antenna Supporting Structures." The existing and proposed antennas, lines and mounts considered in this analysis are listed in Table 1. Applied forces were derived from an 80 MPH basic "fastest mile" wind speed with no ice and a reduced 69 MPH basic "fastest mile" wind speed with a 1/2" of radial ice accumulation. The tower was originally designed for an 80 MPH basic "fastest mile" wind speed with no ice and a reduced 69 MPH basic "fastest mile" wind speed with a 1/2" of radial ice accumulation. The original design loads are listed in Table 2. The EIA minimum basic wind speed for Hartford County, Connecticut is 80 MPH. All coax are assumed to be routed up the interior of the pole unless otherwise noted.

**Table 1 – Proposed, Existing, and Reserved Loads**

Mount Elevation	Carrier Name	Status	Antennas	Mounts	Feedlines
150'	Verizon Wireless	Proposed	(6) Antel LPA-80080/6CF Panels	(1) Nudd 14' L.P. Platform	(12) 1 5/8" Coax
			(6) Antel LPA-185080/12CF Panels		
		Remove	(12) Decibel DB844H80-XY Panels		
	Existing				
NCVFD	Existing	(1) Celwave PD220 Omni	(1) L.P. Platform	(2) 1 5/8" Coax	
		(1) Celwave TD1142 Omni			
138'	AT&T	Existing	(3) Allgon 7250 Panels	(3) 2' Sidearms	(6) 1 5/8" Coax
		Reserved	(3) Allgon 7250 Panels	(1) L.P. Platform	(6) 1 5/8" Coax
			(3) Decibel 978QNB120E-M		(3) 1/2" Coax

**Table 2 – Original Design Loads**

Mount Elevation	Carrier Name	Status	Antennas	Mounts	Feedlines
150'		Design	(12) Decibel DB896 Panels	14' L.P. Platform	(14) 1 5/8" Coax
			(1) Celwave PD220 Omni		
			(1) Celwave TD1142 Omni		
141'		Design	(12) Decibel DB896 Panels	14' L.P. Platform	(12) 1 5/8" Coax
130'		Design	(12) Decibel DB896 Panels	14' L.P. Platform	(12) 1 5/8" Coax
120'		Design	(12) Decibel DB896 Panels	14' L.P. Platform	(12) 1 5/8" Coax
100'		Design	(12) Decibel DB896 Panels	14' L.P. Platform	(12) 1 5/8" Coax
90'		Design	(1) MYA 4505		(1) 1 5/8" Coax

**ANALYSIS PROCEDURE**

**Table 3 – Resources Utilized**

Resource	Remarks
Proposed Loads	SBA E-mail
Existing Loads	SBA E-mail
Tower Drawings	Nudd Drawing No. 00-7221-1
Foundation Drawings	Nudd Drawing No. 00-7221-1
Geotechnical Report	Jaworski Geotech Project No. 99336G

**Analysis Methods**

RISA Tower (Version 5.0), a commercially available software program, was used to create a three-dimensional model of the tower and calculate member stresses for various dead, live, wind, and ice load cases. All loads were computed in accordance with the ANSI/EIA/TIA-222-F or the local building code requirements. Selected output from the analysis is included in Appendix A.

**Assumptions**

1. Tower and structures were built in accordance with the manufacturer's specifications.
2. The tower and structures have been maintained in accordance with manufacturer's specifications.
3. The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Table 1 and any referenced drawings.
4. When applicable, transmission cables are considered to be structural components for calculating wind loads, as allowed by TIA/EIA-222-F.

If any of these assumptions are not valid or have been made in error, this analysis may be affected, and Vertical Structures should be allowed to review any new information to determine its effect on the structural integrity of the tower.

**ANALYSIS RESULTS**

The Canton 2 tower superstructure is found to be adequate for the intended loading at the wind and ice conditions considered. Calculated foundation reactions are within the allowable limits based on the geotechnical information provided. Table 4 summarizes the condition of the tower. Capacities up to 105% are considered acceptable based on the analysis procedures used.

**Table 4 – Tower Component Capacities**

Section Number	Elevation	Percent Capacity Used		
		Pole	Flange Plate	Splice Bolts
1	150' – 139'	20.8	55.7	55.7
2	139' – 89'	53.1	-	-
3	89' – 44'	54.5	-	-
4	44' – 0'	51.2	-	-
Anchor Bolts – Tension		47.7		
Base Plate – Bending		98.0		
Foundation – Moment		29.2		

## APPENDIX A

**DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION
Nudd 14' Low Profile Platform (VSI) (Verizon Wireless)	150	(2) LPA-185080/12CF w/Mount Pipe (Verizon Wireless)	150
(2) LPA-80080/8CF w/Mount Pipe (Verizon Wireless)	150	TD1142 (NCVFD)	150
(2) LPA-80080/8CF w/Mount Pipe (Verizon Wireless)	150	PD220 (NCVFD)	150
(2) LPA-80080/8CF w/Mount Pipe (Verizon Wireless)	150	Pirol 15' Low Profile Platform (VSI) (ATI)	138
(2) LPA-80080/8CF w/Mount Pipe (Verizon Wireless)	150	(2) 7250.00 w/Mount Pipe (ATI)	138
(2) LPA-185080/12CF w/Mount Pipe (Verizon Wireless)	150	(2) 7250.00 w/Mount Pipe (ATI)	138
(2) LPA-185080/12CF w/Mount Pipe (Verizon Wireless)	150	978QNB120E-M w/Mount Pipe (ATI)	138
		978QNB120E-M w/Mount Pipe (ATI)	138
		978QNB120E-M w/Mount Pipe (ATI)	138

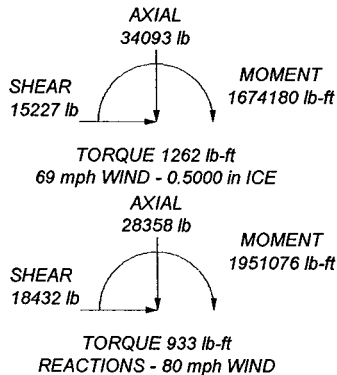
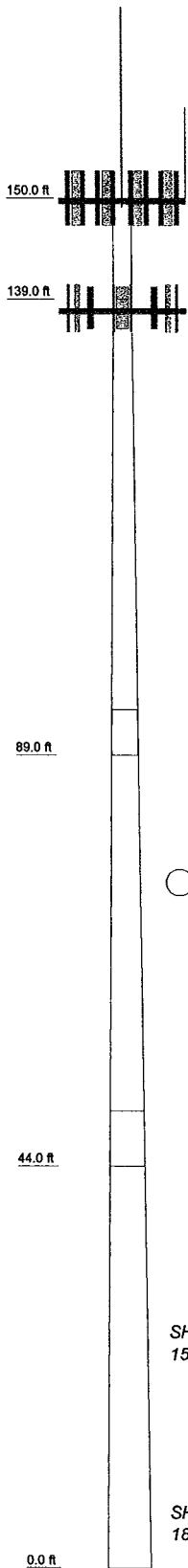
**MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A36	36 ksi	58 ksi	A572-65	65 ksi	80 ksi

**TOWER DESIGN NOTES**

1. Tower is located in Hartford County, Connecticut.
2. Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 69 mph basic wind with 0.50 in ice.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 54.5%

Section	1	2	3	4	
Length (ft)	11'	50'	50'	50'	
Number of Sides	1	18	18	18	
Thickness (in)	0.2810	0.2500	0.3125	0.3750	
Lap Splice (ft)			5'	6'	
Top Dia (in)	24.0000	24.0000	34.1579	43.8872	
Bot Dia (in)	24.0000	35.8421	45.9242	55.5000	
Grade	A36		A572-65		
Weight (lb)	783.4	4005.7	6704.5	9987.4	21481.0



<b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	Job: <b>Canton 2, CT (CT01500-S)</b>
	Project: <b>Vertical Structures Job No. 2007-007-027</b>
Client: <b>SBA</b>	Drawn by: <b>Asel</b>
Code: <b>TIA/EIA-222-F</b>	Date: <b>07/13/07</b>
Path:	Scale: <b>NTS</b>
	Dwg No. <b>E-1</b>

<b>RISATower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, KY 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> Canton 2, CT (CT01500-S)	<b>Page</b> 1 of 7
	<b>Project</b> Vertical Structures Job No. 2007-007-027	<b>Date</b> 18:24:14 07/13/07
	<b>Client</b> SBA	<b>Designed by</b> Asel

## Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

- Tower is located in Hartford County, Connecticut.
- Basic wind speed of 80 mph.
- Nominal ice thickness of 0.5000 in.
- Ice density of 56 pcf.
- A wind speed of 69 mph is used in combination with ice.
- Temperature drop of 50 °F.
- Deflections calculated using a wind speed of 50 mph.
- A non-linear (P-delta) analysis was used.
- Pressures are calculated at each section.
- Stress ratio used in pole design is 1.333.
- Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

## Options

- |  |  |  |
|--|--|--|
| <ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>√ Include Bolts In Member Capacity</li> <li>√ Leg Bolts Are At Top Of Section</li> <li>√ Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>Add IBC .6D+W Combination</li> </ul> | <ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>√ Use Clear Spans For Wind Area</li> <li>√ Use Clear Spans For KL/r</li> <li>√ Retension Guys To Initial Tension</li> <li>√ Bypass Mast Stability Checks</li> <li>Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>√ Autocalc Torque Arm Areas</li> <li>√ SR Members Have Cut Ends</li> <li>Sort Capacity Reports By Component</li> <li>√ Triangulate Diamond Inner Bracing</li> </ul> | <ul style="list-style-type: none"> <li>√ Treat Feedline Bundles As Cylinder</li> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>√ Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>√ SR Leg Bolts Resist Compression</li> <li>√ All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>√ Consider Feedline Torque</li> <li>Include Angle Block Shear Check</li> <li style="background-color: #cccccc;">Poles</li> <li>Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> </ul> |
|--|--|--|

## Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	150'-139'	11'	0'	Round	24.0000	24.0000	0.2810		A36 (36 ksi)
L2	139'-89'	50'	5'	18	24.0000	35.8421	0.2500	1.0000	A572-65 (65 ksi)
L3	89'-44'	50'	6'	18	34.1579	45.9242	0.3125	1.2500	A572-65 (65 ksi)
L4	44'-0'	50'		18	43.8872	55.5000	0.3750	1.5000	A572-65 (65 ksi)

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### Tapered Pole Properties

Section	Tip Dia. in	Area in <sup>2</sup>	I in <sup>4</sup>	r in	C in	I/C in <sup>3</sup>	J in <sup>4</sup>	I/Q in <sup>2</sup>	w in	w/t
L1	24.0000	20.9282	1473.6284	8.3965	12.0000	122.8024	2943.2423	10.4632	0.0000	0
	24.0000	20.9282	1473.6284	8.3965	12.0000	122.8024	2943.2423	10.4632	0.0000	0
L2	24.3702	18.8456	1342.9976	8.4313	12.1920	110.1540	2687.7623	9.4246	3.7840	15.136
	36.3950	28.2423	4520.0706	12.6352	18.2078	248.2493	9046.0889	14.1238	5.8682	23.473
L3	35.8796	33.5704	4858.3966	12.0151	17.3522	279.9872	9723.1860	16.7884	5.4618	17.478
	46.6327	45.2411	11891.1312	16.1922	23.3295	509.7038	23797.9091	22.6249	7.5327	24.105
L4	45.9793	51.7904	12388.2527	15.4468	22.2947	555.6586	24792.8061	25.9001	7.0642	18.838
	56.3562	65.6125	25189.6102	19.5694	28.1940	893.4387	50412.3656	32.8125	9.1080	24.288

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A <sub>f</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft <sup>2</sup>	in					in	in
L1 150'-139'				1	1	1		
L2 139'-89'				1	1	1		
L3 89'-44'				1	1	1		
L4 44'-0'				1	1	1		

### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement	Total Number	C <sub>AA</sub>	Weight
				ft		ft <sup>2</sup> /ft	plf
LDF7-50A (1-5/8 FOAM) (Verizon Wireless)	C	No	Inside Pole	150' - 5'	12	No Ice	0.82
						1/2" Ice	0.82
LDF7-50A (1-5/8 FOAM) (NCVFD) ****	C	No	Inside Pole	150' - 5'	2	No Ice	0.82
						1/2" Ice	0.82
LDF7-50A (1-5/8 FOAM) (AT&T)	C	No	Inside Pole	138' - 5'	12	No Ice	0.82
						1/2" Ice	0.82
LDF4-50A (1/2 FOAM) (AT&T)	C	No	Inside Pole	138' - 5'	3	No Ice	0.15
						1/2" Ice	0.15

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation	Face	A <sub>R</sub>	A <sub>F</sub>	C <sub>AA</sub> In Face	C <sub>AA</sub> Out Face	Weight
	ft		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	lb
L1	150'-139'	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	126.28
L2	139'-89'	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	1078.21
L3	89'-44'	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00



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Tower Section	Tower Elevation ft	Face	$A_R$ ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$C_A A_A$ In Face ft <sup>2</sup>	$C_A A_A$ Out Face ft <sup>2</sup>	Weight lb
L4	44'-0'	C	0.000	0.000	0.000	0.000	979.65
		A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	849.03

### Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	$A_R$ ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$C_A A_A$ In Face ft <sup>2</sup>	$C_A A_A$ Out Face ft <sup>2</sup>	Weight lb
L1	150'-139'	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	126.28
L2	139'-89'	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	1078.21
L3	89'-44'	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	979.65
L4	44'-0'	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	849.03

### Feed Line Center of Pressure

Section	Elevation ft	$CP_x$ in	$CP_z$ in	$CP_x$ Ice in	$CP_z$ Ice in
L1	150'-139'	0.0000	0.0000	0.0000	0.0000
L2	139'-89'	0.0000	0.0000	0.0000	0.0000
L3	89'-44'	0.0000	0.0000	0.0000	0.0000
L4	44'-0'	0.0000	0.0000	0.0000	0.0000

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	$C_A A_A$ Front ft <sup>2</sup>	$C_A A_A$ Side ft <sup>2</sup>	Weight lb
Nudd 14' Low Profile Platform (VSI) (Verizon Wireless)	C	None		0.0000	150'	No Ice	32.00	1350.00
						1/2" Ice	42.00	1750.00
(2) LPA-80080/8CF w/Mount Pipe (Verizon Wireless)	A	From Centroid-Leg	4.00 0'	0.0000	150'	No Ice	6.31	53.20
						1/2" Ice	6.89	134.40
(2) LPA-80080/8CF w/Mount	B	From	4.00	0.0000	150'	No Ice	6.31	53.20

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Horz Lateral	Vert					
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	lb
Pipe (Verizon Wireless)		Centroid-Leg	0'	0'		1/2" Ice	6.89	15.56	134.40
(2) LPA-80080/8CF w/Mount Pipe	C	From	4.00	0'	0.0000	No Ice	6.31	14.07	53.20
Pipe (Verizon Wireless)		Centroid-Leg	0'	0'		1/2" Ice	6.89	15.56	134.40
(2) LPA-185080/12CF w/Mount Pipe	A	From	4.00	0'	0.0000	No Ice	3.55	5.99	32.40
Pipe (Verizon Wireless)		Centroid-Leg	0'	0'		1/2" Ice	3.99	6.94	72.35
(2) LPA-185080/12CF w/Mount Pipe	B	From	4.00	0'	0.0000	No Ice	3.55	5.99	32.40
Pipe (Verizon Wireless)		Centroid-Leg	0'	0'		1/2" Ice	3.99	6.94	72.35
(2) LPA-185080/12CF w/Mount Pipe	C	From	4.00	0'	0.0000	No Ice	3.55	5.99	32.40
Pipe (Verizon Wireless)		Centroid-Leg	0'	0'		1/2" Ice	3.99	6.94	72.35
****									
TD1142 (NCVFD)	B	From	8.00	0'	0.0000	No Ice	2.08	2.08	15.00
		Centroid-Face	0'	5'		1/2" Ice	4.30	4.30	40.00
PD220 (NCVFD)	C	From	8.00	0'	0.0000	No Ice	3.08	3.08	23.00
		Centroid-Face	0'	10'		1/2" Ice	5.30	5.30	48.68
****									
Pirod 15' Low Profile Platform (VSI) (AT&T)	C	None			0.0000	No Ice	17.30	17.30	1500.00
						1/2" Ice	22.10	22.10	2030.00
(2) 7250.00 w/Mount Pipe (AT&T)	A	From	4.00	0'	0.0000	No Ice	4.45	3.54	40.95
		Centroid-Leg	0'	0'		1/2" Ice	5.03	4.72	76.25
(2) 7250.00 w/Mount Pipe (AT&T)	B	From	4.00	0'	0.0000	No Ice	4.45	3.54	40.95
		Centroid-Leg	0'	0'		1/2" Ice	5.03	4.72	76.25
(2) 7250.00 w/Mount Pipe (AT&T)	C	From	4.00	0'	0.0000	No Ice	4.45	3.54	40.95
		Centroid-Leg	0'	0'		1/2" Ice	5.03	4.72	76.25
978QNB120E-M w/Mount Pipe (AT&T)	A	From	4.00	0'	0.0000	No Ice	9.13	5.48	65.55
		Centroid-Leg	0'	0'		1/2" Ice	9.85	6.57	128.40
978QNB120E-M w/Mount Pipe (AT&T)	B	From	4.00	0'	0.0000	No Ice	9.13	5.48	65.55
		Centroid-Leg	0'	0'		1/2" Ice	9.85	6.57	128.40
978QNB120E-M w/Mount Pipe (AT&T)	C	From	4.00	0'	0.0000	No Ice	9.13	5.48	65.55
		Centroid-Leg	0'	0'		1/2" Ice	9.85	6.57	128.40

## Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice

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Comb. No.	Description
4	Dead+Wind 60 deg - No Ice
5	Dead+Wind 90 deg - No Ice
6	Dead+Wind 120 deg - No Ice
7	Dead+Wind 150 deg - No Ice
8	Dead+Wind 180 deg - No Ice
9	Dead+Wind 210 deg - No Ice
10	Dead+Wind 240 deg - No Ice
11	Dead+Wind 270 deg - No Ice
12	Dead+Wind 300 deg - No Ice
13	Dead+Wind 330 deg - No Ice
14	Dead+Ice+Temp
15	Dead+Wind 0 deg+Ice+Temp
16	Dead+Wind 30 deg+Ice+Temp
17	Dead+Wind 60 deg+Ice+Temp
18	Dead+Wind 90 deg+Ice+Temp
19	Dead+Wind 120 deg+Ice+Temp
20	Dead+Wind 150 deg+Ice+Temp
21	Dead+Wind 180 deg+Ice+Temp
22	Dead+Wind 210 deg+Ice+Temp
23	Dead+Wind 240 deg+Ice+Temp
24	Dead+Wind 270 deg+Ice+Temp
25	Dead+Wind 300 deg+Ice+Temp
26	Dead+Wind 330 deg+Ice+Temp
27	Dead+Wind 0 deg - Service
28	Dead+Wind 30 deg - Service
29	Dead+Wind 60 deg - Service
30	Dead+Wind 90 deg - Service
31	Dead+Wind 120 deg - Service
32	Dead+Wind 150 deg - Service
33	Dead+Wind 180 deg - Service
34	Dead+Wind 210 deg - Service
35	Dead+Wind 240 deg - Service
36	Dead+Wind 270 deg - Service
37	Dead+Wind 300 deg - Service
38	Dead+Wind 330 deg - Service

### Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft
L1	150 - 139	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-4153.32	-277.13	-229.45
			Max. Mx	5	-2415.35	-66100.91	-87.20
			Max. My	8	-2415.34	-71.86	-66121.05
			Max. Vy	5	6139.05	-66100.91	-87.20
			Max. Vx	8	6139.05	-71.86	-66121.05
			Max. Torque	22			1265.48
L2	139 - 89	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-12358.05	-277.16	-229.47
			Max. Mx	5	-8763.32	-526810.26	-116.23
			Max. My	8	-8763.30	-97.04	-526830.85
			Max. Vy	5	11802.29	-526810.26	-116.23
			Max. Vx	8	11802.29	-97.04	-526830.85
			Max. Torque	22			1265.40
L3	89 - 44	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-20665.78	-277.20	-229.50
			Max. Mx	5	-16183.69	-	-126.58
						1115938.97	

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft	
L4	44 - 0	Pole	Max. My	8	-16183.69	-106.00	-	
			Max. Vy	5	14969.33	-	1115959.84	
			Max. Vx	8	14969.34	-	1115938.97	
			Max. Torque	16			-106.00	-
			Max Tension	1	0.00	0.00		0.00
			Max. Compression	14	-34093.46	-277.24		-229.54
			Max. Mx	5	-28349.77	-		-127.68
			Max. My	8	-28349.77	-	1950907.79	-
			Max. Vy	5	18444.03	-		1950928.79
			Max. Vx	8	18444.03	-	1950907.79	-
Max. Torque	16					1950928.79		
							-1262.34	

**Compression Checks**

**Pole Design Data**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P/P <sub>a</sub>
L1	150 - 139 (1)	TP24x24x0.281	11'	0'	0.0	21.600	20.9282	-2415.26	452050.00	0.005
L2	139 - 89 (2)	TP35.8421x24x0.25	50'	0'	0.0	39.000	27.3027	-8763.22	1064800.00	0.008
L3	89 - 44 (3)	TP45.9242x34.1579x0.3125	50'	0'	0.0	39.000	43.8406	-16183.70	1709780.00	0.009
L4	44 - 0 (4)	TP55.5x43.8872x0.375	50'	0'	0.0	39.000	65.6125	-28349.80	2558890.00	0.011

**Pole Bending Design Data**

Section No.	Elevation ft	Size	Actual M <sub>x</sub> lb-ft	Actual f <sub>bx</sub> ksi	Allow. F <sub>bx</sub> ksi	Ratio f <sub>bx</sub> /F <sub>bx</sub>	Actual M <sub>y</sub> lb-ft	Actual f <sub>by</sub> ksi	Allow. F <sub>by</sub> ksi	Ratio f <sub>by</sub> /F <sub>by</sub>
L1	150 - 139 (1)	TP24x24x0.281	66159.7	-6.465	23.760	0.272	0.00	0.000	23.760	0.000
L2	139 - 89 (2)	TP35.8421x24x0.25	526896.67	-27.259	39.000	0.699	0.00	0.000	39.000	0.000
L3	89 - 44 (3)	TP45.9242x34.1579x0.3125	1116066.67	-27.987	39.000	0.718	0.00	0.000	39.000	0.000
L4	44 - 0 (4)	TP55.5x43.8872x0.375	1951075.00	-26.205	39.000	0.672	0.00	0.000	39.000	0.000

**Pole Interaction Design Data**

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Section No.	Elevation ft	Size	Ratio	Ratio	Ratio	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
			$\frac{P}{P_a}$	$\frac{f_{bx}}{F_{bx}}$	$\frac{f_{by}}{F_{by}}$			
L1	150 - 139 (1)	TP24x24x0.281	0.005	0.272	0.000	0.277 ✓	1.333	H1-3 ✓
L2	139 - 89 (2)	TP35.8421x24x0.25	0.008	0.699	0.000	0.707 ✓	1.333	H1-3 ✓
L3	89 - 44 (3)	TP45.9242x34.1579x0.3125	0.009	0.718	0.000	0.727 ✓	1.333	H1-3 ✓
L4	44 - 0 (4)	TP55.5x43.8872x0.375	0.011	0.672	0.000	0.683 ✓	1.333	H1-3 ✓

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SF*P <sub>allow</sub> lb	% Capacity	Pass Fail
L1	150 - 139	Pole	TP24x24x0.281	1	-2415.26	602582.62	20.8	Pass
L2	139 - 89	Pole	TP35.8421x24x0.25	2	-8763.22	1419378.34	53.1	Pass
L3	89 - 44	Pole	TP45.9242x34.1579x0.3125	3	-16183.70	2279136.65	54.5	Pass
L4	44 - 0	Pole	TP55.5x43.8872x0.375	4	-28349.80	3411000.23	51.2	Pass
Summary								
Pole (L3)							54.5	Pass
<b>RATING =</b>							<b>54.5</b>	<b>Pass</b>