



June 16, 2016

Melanie A. Bachman
Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Re: Notice of Exempt Modification – Antenna Swap
Property Address: 14 Canton Springs Road, Canton, CT (the “Property”)
Applicant: New Cingular Wireless PCS, LLC (“AT&T”)

Dear Ms. Bachman:

On behalf of AT&T, please accept this application as notification pursuant to R.C.S.A. §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. §16-50j-72(b)(2).

AT&T currently maintains a wireless telecommunications facility consisting of nine (9) wireless telecommunication antennas at an antenna center line height of 130-feet on an existing 140 -foot monopole tower, owned by Cellco Partnership d/b/a Verizon Wireless, and located on the Property (the “Tower”). The Connecticut Siting Council (the “Council”) approved AT&T’s use of the Tower in the following prior decisions; EM-AT&T-023-020503, EM-CING-023-131-04, and EM-CING-023-121102.

AT&T now intends to replace two (2) KMW AM-X-CD-16-65-OOT-RET panel antennas, two (2) Powerwave P65-17-XLH-RR panel antennas and two (2) Andrew SBNH-1D6565C panel antennas with six (6) CCI HPA – 65R-BUU H-8 panel antennas and three (3) Andrew SBNHH-1D65A panel antennas, while retaining three (3) Kathrein 800-10121 panel antennas (for a total of twelve (12) panel antennas), at the 130-foot level. Please refer to Tab 1 for further specifications of the replacement antennas.

In accordance with R.C.S.A. §16-50j-73, a copy of this letter is being sent to Richard Barlow, First Selectman of the Town of Canton, Town Hall, P.O. Box 168, 4 Market Street, Collinsville, CT 06022. A copy of this letter is also being sent to the Canton Fire Dept., 14 Canton Springs Road, Canton, CT 06022; owner of the property where the tower is located; and

33 Boston Post Road West, Marlborough, MA 01752

to the tower owner at Aleksey Tyurin, Verizon Wireless, 99 East River Drive, 9th Floor, East Hartford, CT 06108.

The planned modifications to AT&T's facility fall squarely within those activities explicitly provided for in R.C.S.A. §16-50j-72(b) (2).

1. The proposed modifications will not result in an increase in the height of the existing tower. AT&T's replacement antennas will be installed at the 130-foot level of the 140-foot monopole.
2. The proposed modifications will not involve any changes to ground-mounted equipment and, therefore, will not require an extension of the site boundary.
3. The proposed modifications will not increase the noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the modified facility will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) safety standard. A cumulative worst-case RF emissions calculation for AT&T's modified facility is provided in the RF Emissions Compliance Report, included in Tab 2.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The tower and its foundation can support AT&T's proposed modifications. (See Structural Analysis Report included in Tab 3).

For the foregoing reasons, AT&T respectfully submits that the proposed modifications to the above referenced telecommunications facility constitutes an exempt modification under R.C.S.A. §16-50j-72(b)(2).

Sincerely,

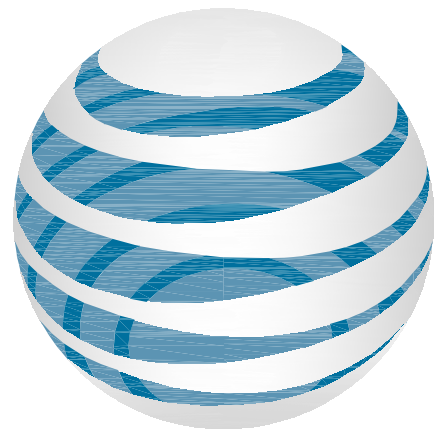
Kristen Smith

Enclosures

CC w/enclosures:

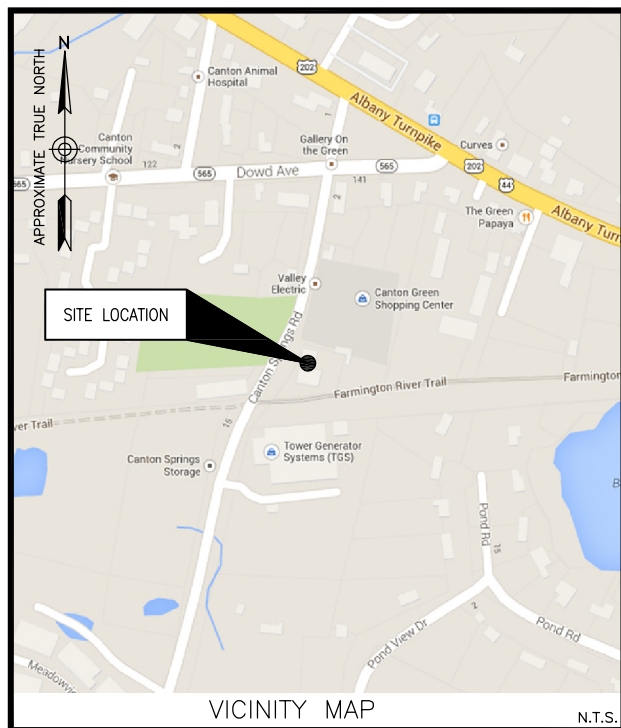
| Richard Barlow, First Selectman Town of Canton
Richard Hutchings, Fire Chief, Canton Fire Dept.

Tab 1



at&t Mobility

SITE NAME: CANTON-FIRE DEPARTMENT
SITE NUMBER: CT1022
14 CANTON SPRINGS ROAD,
CANTON, CT 06019



DIRECTIONS FROM 500 ENTERPRISE DRIVE, ROCKY HILL, CT:
 TAKE 1-91 N. TAKE EXIT 32A-32B ON THE LEFT FOR TRUMBULL ST/1-84W TOWARD WATERBURY. TAKE EXIT 32B TO MERGE ONTO TRUMBULL ST. TURN RIGHT ONTO MAIN ST. CONTINUE ONTO US-44 W. TURN LEFT ONTO DOWD AVE. TAKE THE 1ST LEFT ONTO CANTON SPRINGS RD. DESTINATION WILL BE ON THE LEFT.

SITE COORDINATES:
 LATITUDE: N 41° 49' 22.3"
 LONGITUDE: W 72° 53' 42.6"
 (PER EXISTING AT&T PLANS)

ELEVATION DATA
 GRADE ELEVATION AT TOWER = 339'± A.M.S.L.
 (ELEVATION BASED UPON VELOCOROUTES.ORG)

ANTENNA ELEVATION (TO TOP OF ANTENNA)
 ALPHA SECTOR: 132'-4"± A.G.L.
 BETA SECTOR: 133'-10"± A.G.L.
 GAMMA SECTOR: 133'-10"± A.G.L.

SITE INFORMATION

- REMOVE AND REPLACE ANTENNA MOUNT WITH A NEW MOUNT.
 - REMOVE AND REPLACE (2) PANEL ANTENNAS PER SECTOR WITH (3) PANEL ANTENNAS PER SECTOR FOR A TOTAL OF (9) NEW ANTENNAS.
 - ADD (5) NEW RRUS PER SECTOR ON A NEW MOUNT FOR A TOTAL OF (15) NEW RRUS.
 - ADD (2) NEW SURGE ARRESTORS ON A NEW MOUNT.
 - REMOVE AND REPLACE EXISTING DIPLEXERS WITH (6) NEW DIPLEXERS.
 - ADD (1) NEW 23" EQUIPMENT RACK IN EXISTING SHELTER.
- PROJECT DESCRIPTION**

SITE NAME:
 CANTON-FIRE DEPARTMENT

SITE NUMBER:
 CT1022

LOCATION:
 14 CANTON SPRINGS ROAD,
 CANTON, CT 06019

APPLICANT/LESSEE:
 AT&T MOBILITY
 500 ENTERPRISE DRIVE, SUITE 3A
 ROCKY HILL, CONNECTICUT 06067

PROJECT INFORMATION

THIS DOCUMENT WAS DEVELOPED TO REFLECT A SPECIFIC SITE AND ITS SITE CONDITIONS AND IS NOT TO BE USED FOR ANOTHER SITE OR WHEN OTHER CONDITIONS PERTAIN. REUSE OF THIS DOCUMENT IS AT THE SOLE RISK OF THE USER.

A.D.A. COMPLIANCE:
 FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION.

SHEET NUMBER	DESCRIPTION
T-1	TITLE SHEET
G-1	GENERAL NOTES
C-1	SITE PLAN & EQUIPMENT PLANS
C-2	ANTENNA LAYOUTS & ELEVATIONS
C-3	CONSTRUCTION DETAILS
C-4	ANTENNA SCHEDULE & CONSTRUCTION DETAILS
E-1	GROUNDING NOTES & DETAILS

SHEET INDEX



500 ENTERPRISE DRIVE SUITE 3A
 ROCKY HILL, CT 06067



1997 ANNAPOLIS EXCHANGE PARKWAY
 SUITE 200
 ANNAPOLIS, MD 21401

CT1022
CANTON-FIRE DEPARTMENT

CONSTRUCTION DRAWINGS	
1	05/06/14 ISSUED AS FINAL
0	04/21/14 ISSUED AS FINAL
A	02/07/14 PRELIMINARY SUBMISSION



Dewberry Engineers Inc.
 600 PARSIPPANY ROAD
 SUITE 301
 PARSIPPANY, NJ 07054
 PHONE: 973.739.9400
 FAX: 973.739.9710

ROBERT J. FOLEY, P.E.
 CT LICENSE No. PEN.0029056

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER TO ALTER THIS DOCUMENT.

DRAWN BY: FG

REVIEWED BY: BSH

CHECKED BY: GHN

PROJECT NUMBER: 50063024

JOB NUMBER: 50063025

SITE ADDRESS:

14 CANTON SPRINGS RD.
 CANTON, CT 06019
 HARTFORD COUNTY

SHEET TITLE

TITLE SHEET

SHEET NUMBER

T-1



500 ENTERPRISE DRIVE SUITE 3A
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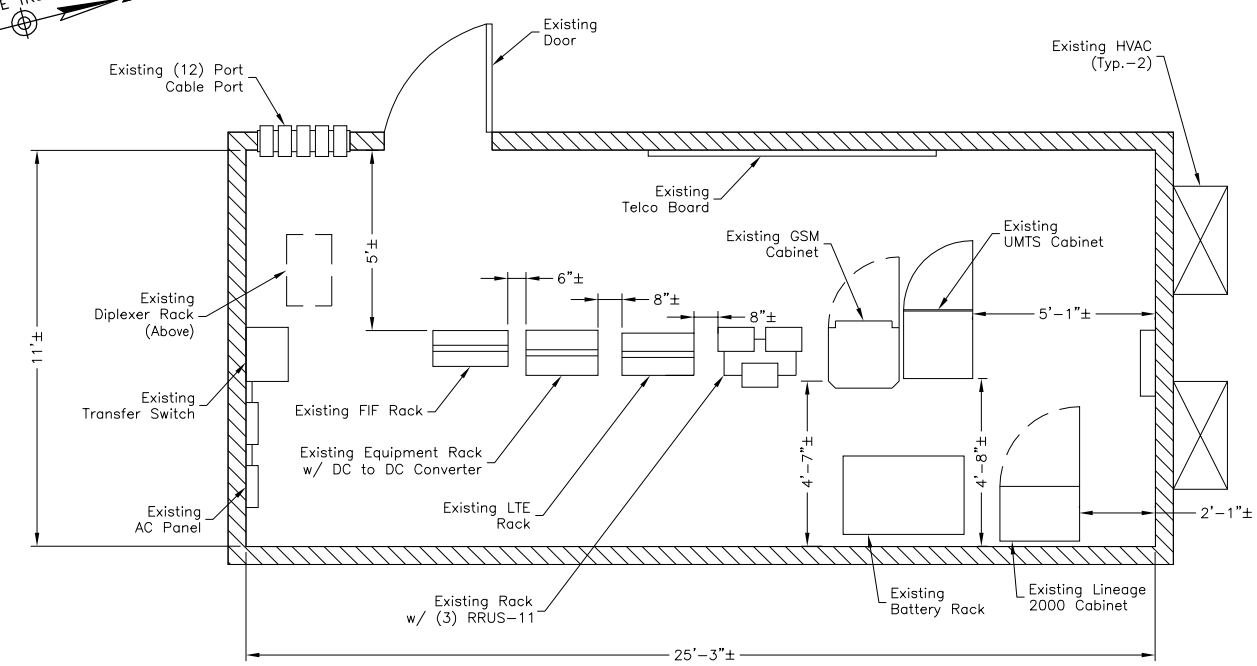
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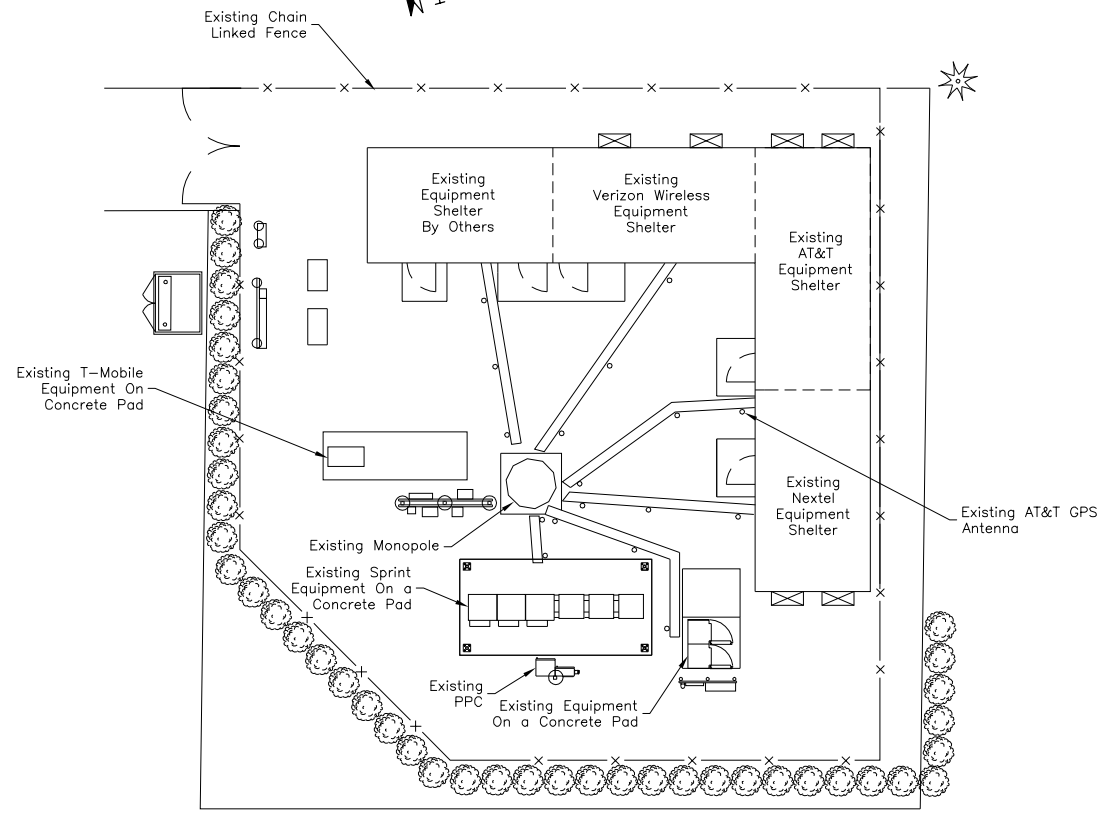
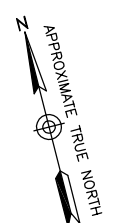
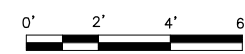
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HARTFORD COUNTY

SHEET TITLE	
SITE PLAN & EQUIPMENT PLANS	
SHEET NUMBER	

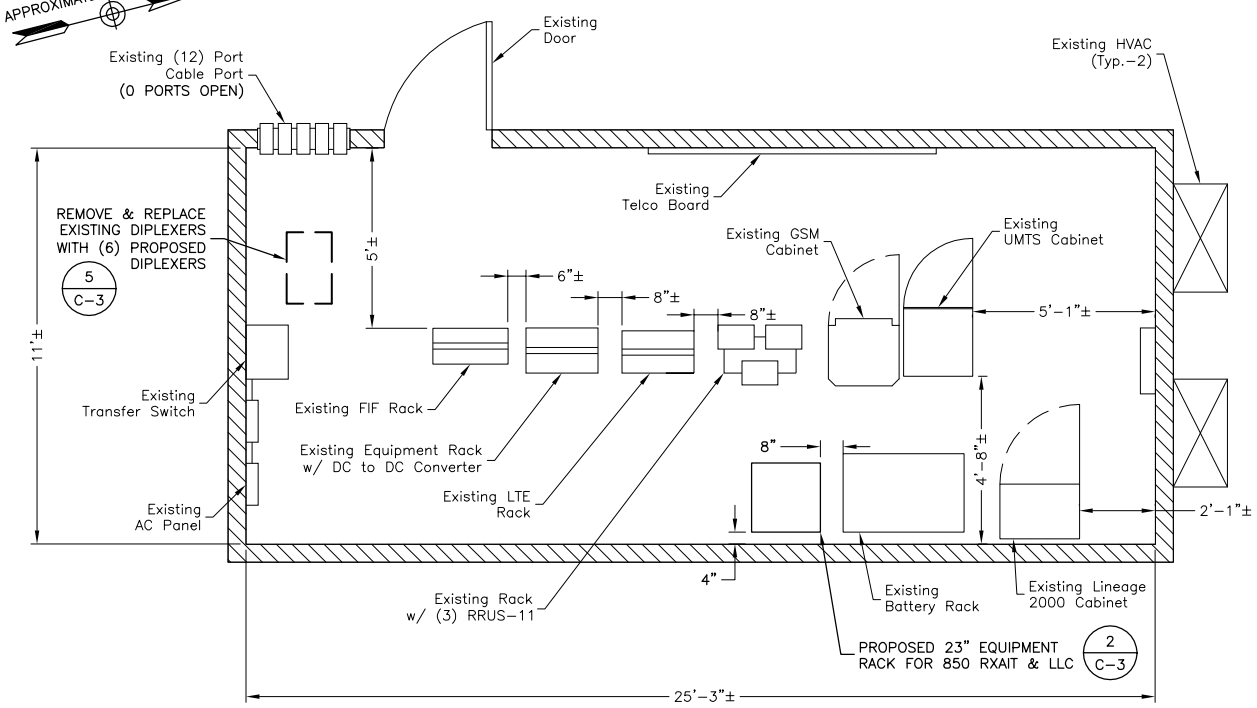
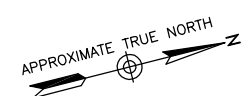
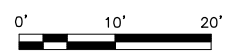
C-1



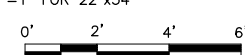
EXISTING EQUIPMENT PLAN 2
SCALE: 3/16"=1' FOR 11"x17"
3/8"=1' FOR 22"x34"



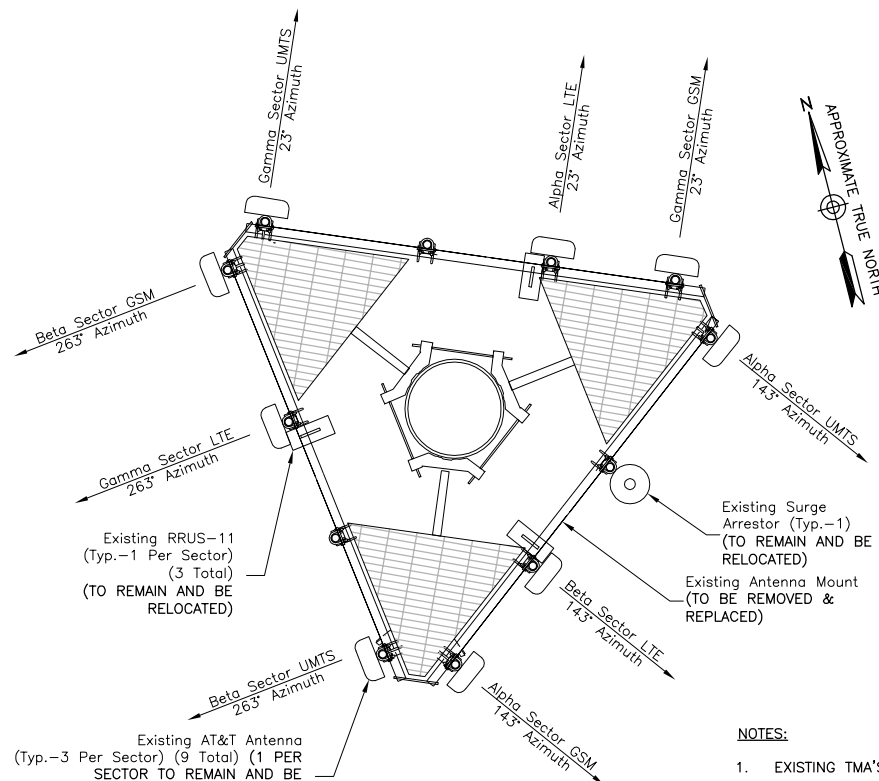
SITE PLAN 1
SCALE: 1"=20' FOR 11"x17"
1"=10' FOR 22"x34"



PROPOSED EQUIPMENT PLAN 3
SCALE: 3/16"=1' FOR 11"x17"
3/8"=1' FOR 22"x34"



- NOTES:**
- NORTH SHOWN AS APPROXIMATE.
 - MOUNT ALL ANTENNAS, COAX, SURGE ARRESTORS, RRU'S ETC. IN ACCORDANCE WITH STRUCTURAL ANALYSIS BY OTHERS.
 - NOT ALL INFORMATION IS SHOWN FOR CLARITY.

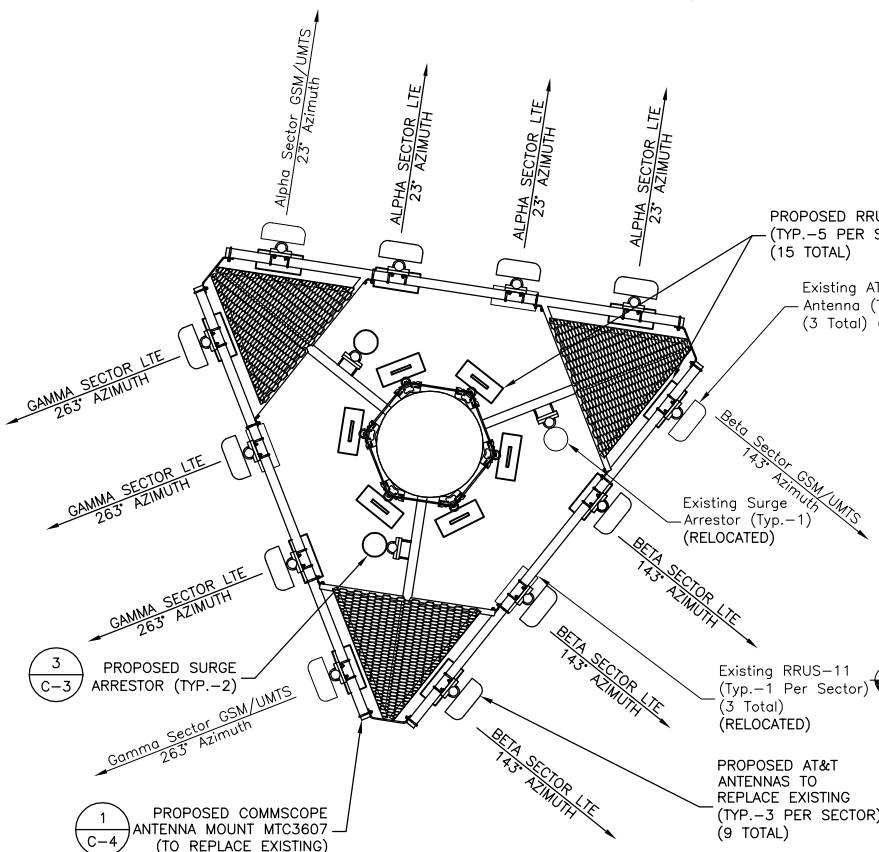


EXISTING ANTENNA LAYOUT

SCALE: N.T.S.

NOTES:

- EXISTING TMA'S NOT SHOWN FOR CLARITY.
- ALL EXISTING TMA'S ARE TO REMAIN.

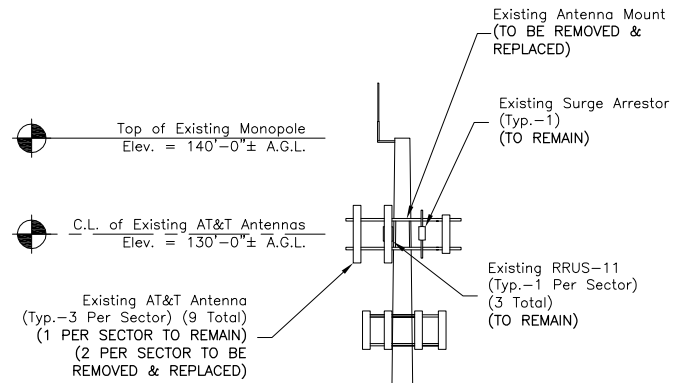


PROPOSED ANTENNA LAYOUT

SCALE: N.T.S.

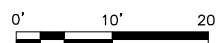
NOTES:

- EXISTING TMA'S NOT SHOWN FOR CLARITY.
- ALL EXISTING TMA'S ARE TO REMAIN AND BE RELOCATED TO NEW MOUNT.



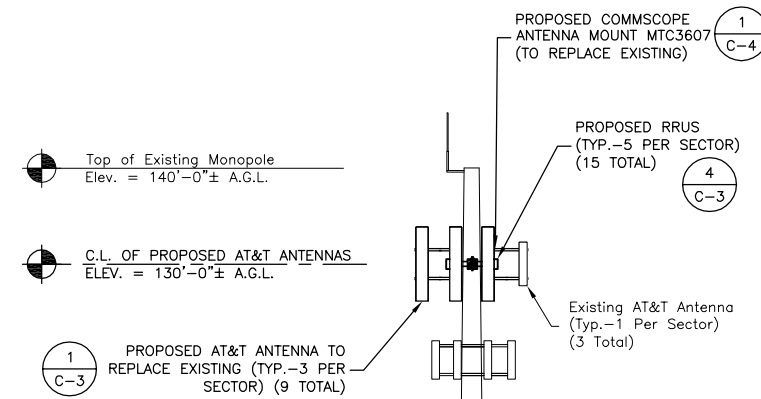
EXISTING WEST ELEVATION

SCALE: 1"=20' FOR 11"x17"
1"=10' FOR 22"x34"



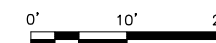
NOTES:

- PRIOR TO START OF ANY WORK, A PASSING STRUCTURAL ANALYSIS SHALL BE PROVIDED BY A CONNECTICUT LICENSED P.E. CONTRACTOR TO OBTAIN A COPY BEFORE STARTING ANY WORK.
- ALL ANTENNAS, COAX, SURGE ARRESTORS, RRUS, ETC. TO BE INSTALLED IN ACCORDANCE WITH STRUCTURAL ANALYSIS PROVIDED BY OTHERS AND FINAL AT&T RF DATA SHEET.



PROPOSED WEST ELEVATION

SCALE: 1"=20' FOR 11"x17"
1"=10' FOR 22"x34"



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**CT1022
CANTON-FIRE
DEPARTMENT**

CONSTRUCTION DRAWINGS

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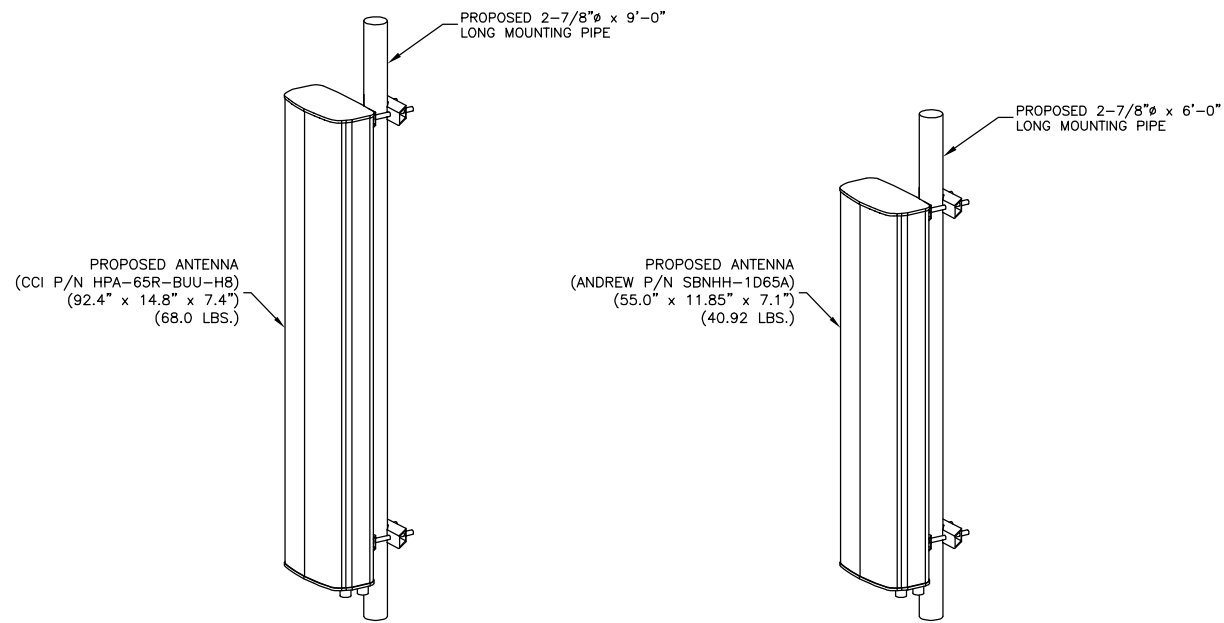
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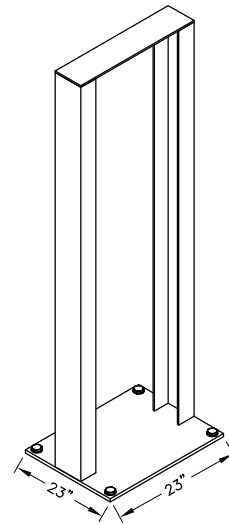
14 CANTON SPRINGS RD.
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HARTFORD COUNTY

SHEET TITLE	ANTENNA LAYOUTS & ELEVATIONS
SHEET NUMBER	



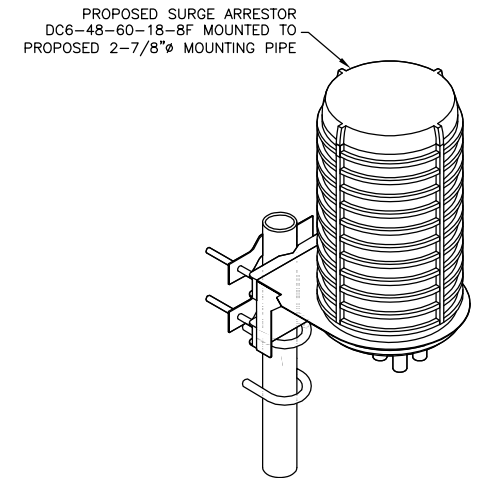
NOTE:
1. PLEASE SEE RFDS FOR SPECIFIC ANTENNA MODEL.

ISOMETRIC ANTENNA DETAILS 1
SCALE: N.T.S.



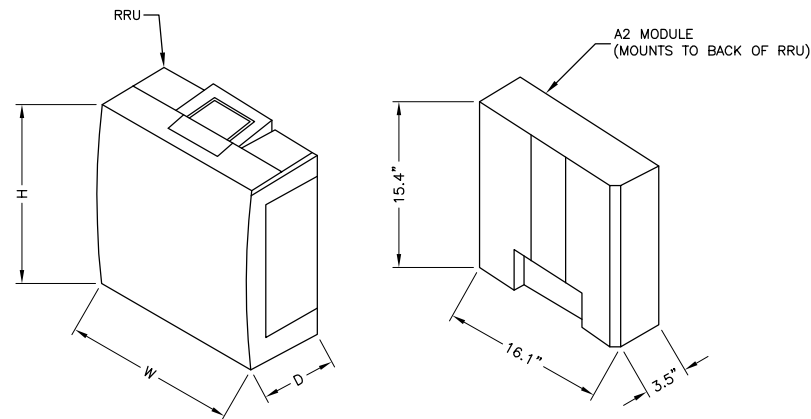
NOTE:
1. CONTRACTOR SHALL SECURE RACK AS PER MANUFACTURER RECOMMENDATIONS.

23" x 23" INDOOR RACK 2
SCALE: N.T.S.



NOTE:
1. ALL ANTENNAS, COAX AND ANTENNA SUPPORT EQUIPMENT TO BE INSTALLED IN ACCORDANCE WITH STRUCTURAL ANALYSIS AND FINAL RF DATA SHEET.

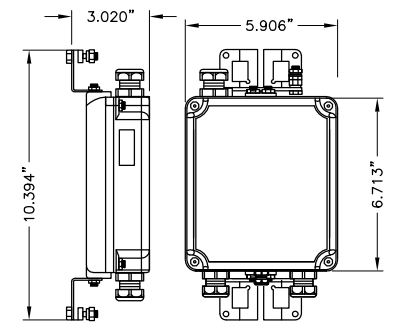
SURGE ARRESTOR MOUNTING DETAIL 3
SCALE: N.T.S.



RRUS MODEL & DIMENSIONS	
ERICSSON MODEL #	DIMENSIONS (HxWxD)
RRUS-11	19.7"x17.0"x7.2"
RRUS-12	20.4"x18.8"x7.5"
RRUS-E2	20.4"x18.8"x7.5"
RRUS-32	29.9"x13.3"x9.5"

RRU NOTES:
1. GROUND EQUIPMENT AND MOUNTS PER MANUFACTURER'S RECOMMENDATIONS AND AT&T STANDARDS.
2. MOUNT EQUIPMENT PER MANUFACTURER'S RECOMMENDATIONS.
3. CONFIRM REQUIRED EQUIPMENT WITH LATEST RFDS.

RRUS & A2 MODULE 4
SCALE: N.T.S.



NOTE:
1. ATTACH DIPLEXER TO RACK IN ACCORDANCE WITH MANUFACTURER RECOMMENDATIONS.

DIPLEXER DETAIL 5
SCALE: N.T.S.

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CANTON, CT 06019
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SHEET TITLE	
CONSTRUCTION DETAILS	
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EXISTING ANTENNA SCHEDULE

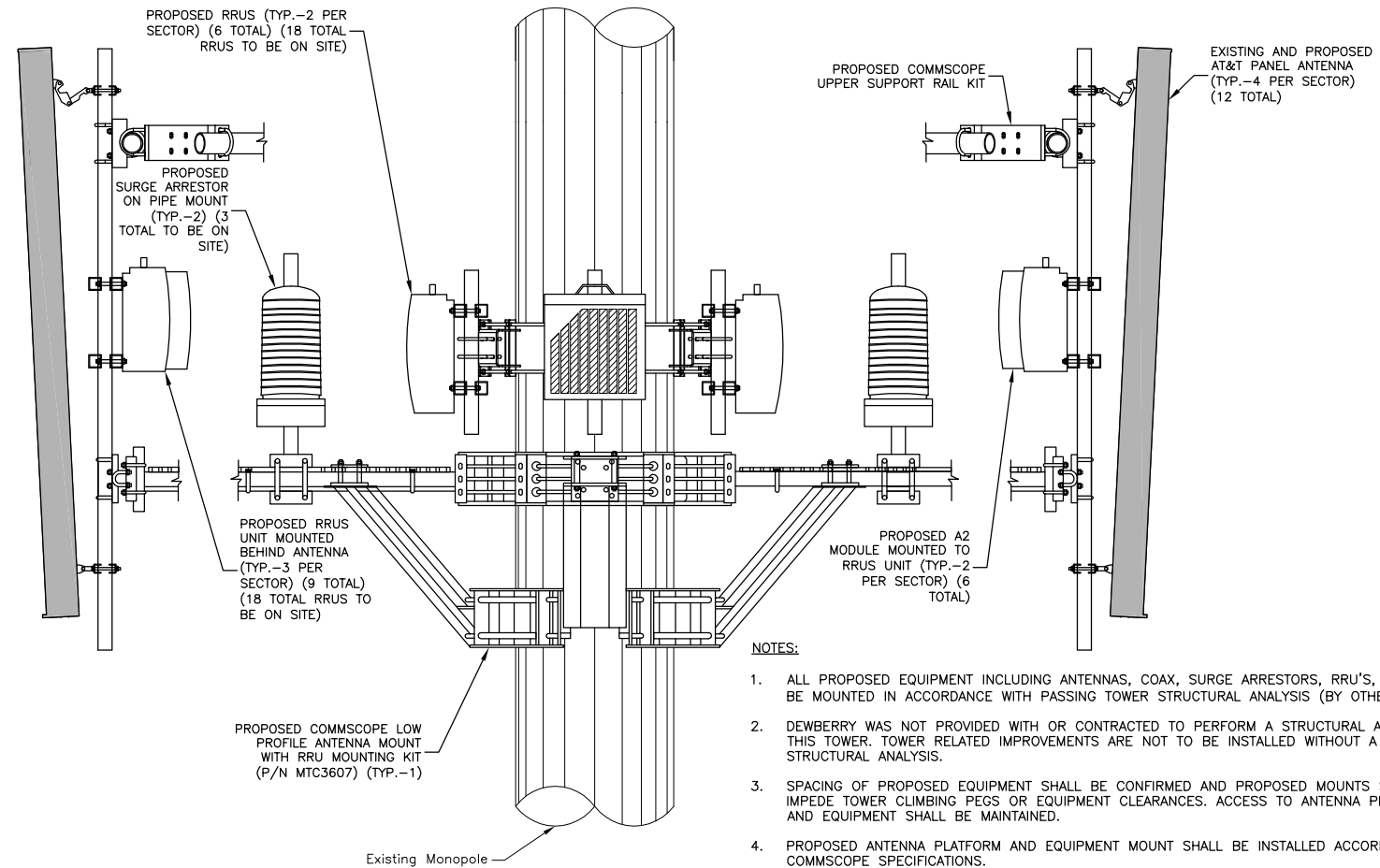
SECTOR	MAKE	MODEL#	SIZE (INCHES)
ALPHA:	KATHREIN	800-10121	54.5x10.3x5.9
	KMW	AM-X-CD-16-65-00T-RET	54.0x12.6x7.87
	KMW	AM-X-CD-16-65-00T-RET	54.0x12.6x7.87
BETA:	KATHREIN	800-10121	54.5x10.3x5.9
	POWERWAVE	P65-17-XLH-RR	96x12x6
	POWERWAVE	P65-17-XLH-RR	96x12x6
GAMMA:	KATHREIN	800-10121	54.5x10.3x5.9
	ANDREW	SBNH-1D6565C	96.4x11.9x7.1
	ANDREW	SBNH-1D6565C	96.4x11.9x7.1

PROPOSED ANTENNA SCHEDULE

SECTOR	MAKE	MODEL#	SIZE (INCHES)
ALPHA:	KATHREIN	800-10121	54.5x10.3x5.9
	ANDREW	SBNHH-1D65A	55.0x11.85x7.1
	ANDREW	SBNHH-1D65A	55.0x11.85x7.1
	ANDREW	SBNHH-1D65A	55.0x11.85x7.1
BETA:	KATHREIN	800-10121	54.5x10.3x5.9
	CCI	HPA-65R-BUU-H8	92.4x14.8x7.4
	CCI	HPA-65R-BUU-H8	92.4x14.8x7.4
	CCI	HPA-65R-BUU-H8	92.4x14.8x7.4
GAMMA:	KATHREIN	800-10121	54.5x10.3x5.9
	CCI	HPA-65R-BUU-H8	92.4x14.8x7.4
	CCI	HPA-65R-BUU-H8	92.4x14.8x7.4
	CCI	HPA-65R-BUU-H8	92.4x14.8x7.4

PROPOSED RRUS SCHEDULE

SECTOR	MAKE	MODEL#	SIZE (INCHES)
ALPHA:	ERICSSON	RRUS-11	19.7x17.0x7.2
	ERICSSON	RRUS-11	19.7x17.0x7.2
	ERICSSON	RRUS-12	20.4x18.5x7.5
	ERICSSON	RRUS-12	20.4x18.5x7.5
	ERICSSON	RRUS-A2	16.4x15.1x3.4
	ERICSSON	RRUS-A2	16.4x15.1x3.4
BETA:	ERICSSON	RRUS-E2	20.4x18.8x7.5
	ERICSSON	RRUS-32	29.9x13.3x9.5
	ERICSSON	RRUS-11	19.7x17.0x7.2
	ERICSSON	RRUS-11	19.7x17.0x7.2
	ERICSSON	RRUS-12	20.4x18.5x7.5
	ERICSSON	RRUS-12	20.4x18.5x7.5
GAMMA:	ERICSSON	RRUS-A2	16.4x15.1x3.4
	ERICSSON	RRUS-A2	16.4x15.1x3.4
	ERICSSON	RRUS-E2	20.4x18.8x7.5
	ERICSSON	RRUS-E2	20.4x18.8x7.5
	ERICSSON	RRUS-32	29.9x13.3x9.5
	ERICSSON	RRUS-32	29.9x13.3x9.5



- NOTES:**
1. ALL PROPOSED EQUIPMENT INCLUDING ANTENNAS, COAX, SURGE ARRESTORS, RRU'S, ETC. SHALL BE MOUNTED IN ACCORDANCE WITH PASSING TOWER STRUCTURAL ANALYSIS (BY OTHERS).
 2. DEWBERRY WAS NOT PROVIDED WITH OR CONTRACTED TO PERFORM A STRUCTURAL ANALYSIS ON THIS TOWER. TOWER RELATED IMPROVEMENTS ARE NOT TO BE INSTALLED WITHOUT A PASSING STRUCTURAL ANALYSIS.
 3. SPACING OF PROPOSED EQUIPMENT SHALL BE CONFIRMED AND PROPOSED MOUNTS SHALL NOT IMPEDE TOWER CLIMBING PEGS OR EQUIPMENT CLEARANCES. ACCESS TO ANTENNA PLATFORM AND EQUIPMENT SHALL BE MAINTAINED.
 4. PROPOSED ANTENNA PLATFORM AND EQUIPMENT MOUNT SHALL BE INSTALLED ACCORDING TO COMMSCOPE SPECIFICATIONS.

PROPOSED ANTENNA MOUNTING DETAIL

SCALE: N.T.S.

1



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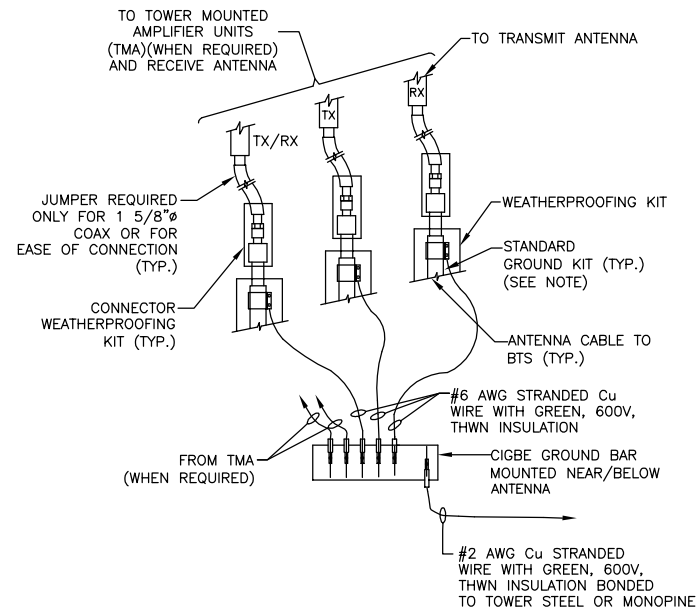
SHEET TITLE

ANTENNA SCHEDULE &
CONSTRUCTION DETAILS

SHEET NUMBER

GROUNDING NOTES:

- THE CONTRACTOR SHALL REVIEW AND INSPECT THE EXISTING FACILITY GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRICT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ), THE SITE-SPECIFIC (UL, LPI, OR NFPA) LIGHTNING PROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND TIA GROUNDING STANDARDS. THE CONTRACTOR SHALL REPORT ANY VIOLATIONS OR ADVERSE FINDINGS TO THE ENGINEER FOR RESOLUTION.
- ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GES'S) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS. ALL AVAILABLE GROUNDING ELECTRODES SHALL BE CONNECTED TOGETHER IN ACCORDANCE WITH THE NEC.
- THE CONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS. USE OF OTHER METHODS MUST BE PRE-APPROVED BY THE ENGINEER IN WRITING.
- THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS ON TOWER SITES AND 10 OHMS OR LESS ON ROOFTOP SITES. WHEN ADDING ELECTRODES, CONTRACTOR SHALL MAINTAIN A MINIMUM DISTANCE BETWEEN THE ADDED ELECTRODE AND ANY OTHER EXISTING ELECTRODE EQUAL TO THE BURIED LENGTH OF THE ROD. IDEALLY, CONTRACTOR SHALL STRIVE TO KEEP THE SEPARATION DISTANCE EQUAL TO TWICE THE BURIED LENGTH OF THE RODS.
- THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT.
- METAL CONDUIT AND TRAY SHALL BE GROUNDING AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH 6 AWG COPPER WIRE AND UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO TRANSMISSION EQUIPMENT.
- CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED. BACK-TO-BACK CONNECTIONS ON OPPOSITE SIDES OF THE GROUND BUS ARE PERMITTED.
- ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
- USE OF 90° BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45° BENDS CAN BE ADEQUATELY SUPPORTED. IN ALL CASES, BENDS SHALL BE MADE WITH A MINIMUM BEND RADIUS OF 8 INCHES.
- EACH INTERIOR TRANSMISSION CABINET FRAME/PLINTH SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH 6 AWG STRANDED, GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRE UNLESS NOTED OTHERWISE IN THE DETAILS. EACH OUTDOOR CABINET FRAME/PLINTH SHALL BE DIRECTLY CONNECTED TO THE BURIED GROUND RING WITH 2 AWG SOLID TIN-PLATED COPPER WIRE UNLESS NOTED OTHERWISE IN THE DETAILS.
- ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING, SHALL BE 2 AWG SOLID TIN-PLATED COPPER UNLESS OTHERWISE INDICATED.
- EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE. CONNECTIONS TO ABOVE GRADE UNITS SHALL BE MADE WITH EXOTHERMIC WELDS WHERE PRACTICAL OR WITH 2-HOLE MECHANICAL TYPE BRASS CONNECTORS WITH STAINLESS STEEL HARDWARE, INCLUDING SET SCREWS. HIGH PRESSURE CRIMP CONNECTORS MAY ONLY BE USED WITH WRITTEN PERMISSION FROM SMARTLINK'S MARKET REPRESENTATIVE.
- EXOTHERMIC WELDS SHALL BE PERMITTED ON TOWERS ONLY WITH THE EXPRESS APPROVAL OF THE TOWER MANUFACTURER OR THE CONTRACTOR'S STRUCTURAL ENGINEER.
- ALL WIRE TO WIRE GROUND CONNECTIONS TO THE INTERIOR GROUND RING SHALL BE FORMED USING HIGH PRESS CRIMPS OR SPLIT BOLT CONNECTORS WHERE INDICATED IN THE DETAILS.
- ON ROOFTOP SITES WHERE EXOTHERMIC WELDS ARE A FIRE HAZARD COPPER COMPRESSION CAP CONNECTORS MAY BE USED FOR WIRE TO WIRE CONNECTIONS. 2-HOLE MECHANICAL TYPE BRASS CONNECTORS WITH STAINLESS STEEL HARDWARE, INCLUDING SET SCREWS SHALL BE USED FOR CONNECTION TO ALL ROOFTOP TRANSMISSION EQUIPMENT AND STRUCTURAL STEEL.
- COAX BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR USING TWO-HOLE MECHANICAL TYPE BRASS CONNECTORS AND STAINLESS STEEL HARDWARE.
- APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.
- MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
- BOND ALL METALLIC OBJECTS WITHIN 6 FT OF THE BURIED GROUND RING WITH 2 AWG SOLID TIN-PLATED COPPER GROUND CONDUCTOR. DURING EXCAVATION FOR NEW GROUND CONDUCTORS, IF EXISTING GROUND CONDUCTORS ARE ENCOUNTERED, BOND EXISTING GROUND CONDUCTORS TO NEW CONDUCTORS.
- GROUND CONDUCTORS USED IN THE FACILITY GROUND AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC PLASTIC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (E.G., NON-METALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT WITH LISTED BONDING FITTINGS.



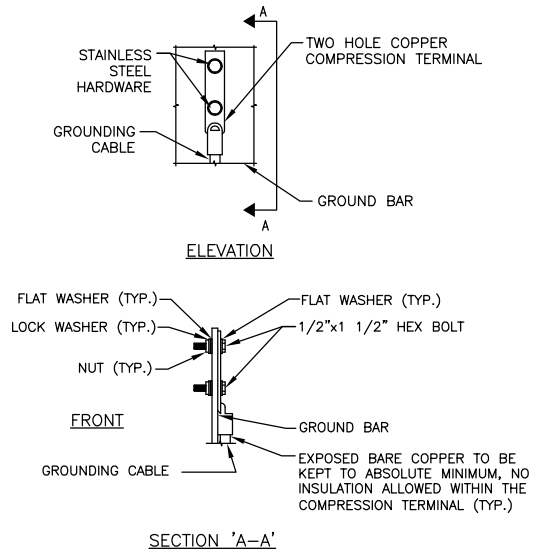
NOTE:

- DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO CIGBE.

CONNECTION OF GROUND WIRES TO GROUNDING BAR (CIGBE)

SCALE: N.T.S.

1



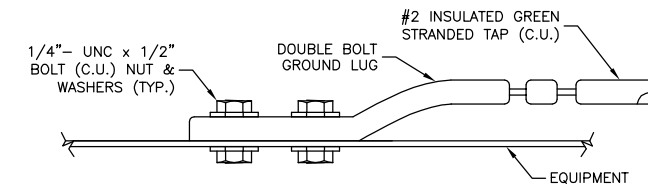
NOTES:

- DOUBLING UP OR STACKING OF CONNECTIONS IS NOT PERMITTED.
- OXIDE INHIBITING COMPOUND TO BE USED AT ALL LOCATIONS.

TYPICAL GROUND BAR MECHANICAL CONNECTION DETAIL

SCALE: N.T.S.

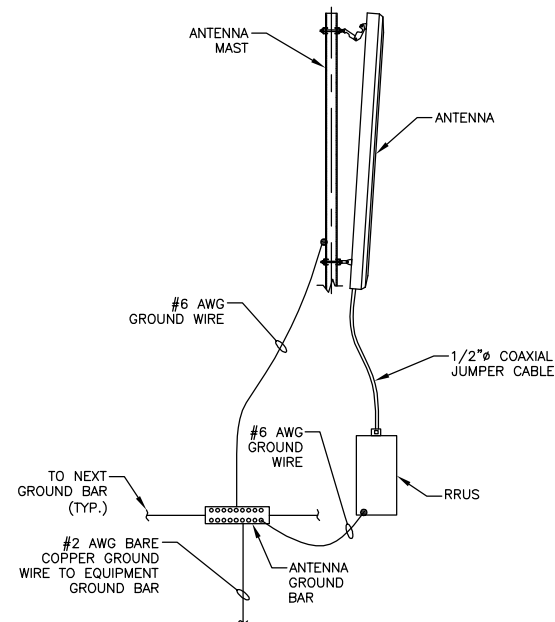
2



CONNECTION TO EQUIPMENT DETAIL

SCALE: N.T.S.

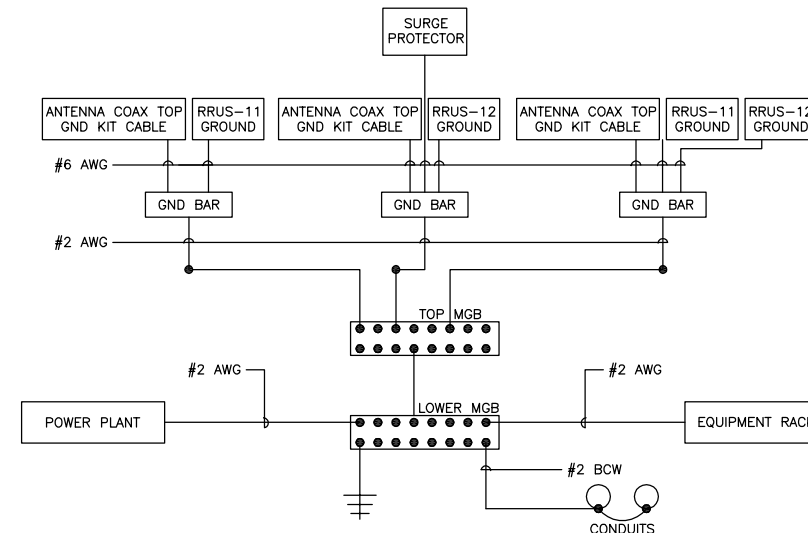
3



TYPICAL ANTENNA GROUNDING DETAIL

SCALE: N.T.S.

4



NOTES:

- BOND ANTENNA GROUNDING KIT CABLE TO TOP CIGBE
- BOND ANTENNA GROUNDING KIT CABLE TO BOTTOM CIGBE.
- SCHEMATIC GROUNDING DIAGRAM IS TYPICAL FOR EACH SECTOR.
- GROUND ALL EQUIPMENT PER MANUFACTURER RECOMMENDATIONS.

SCHEMATIC GROUNDING DIAGRAM

SCALE: N.T.S.

5



500 ENTERPRISE DRIVE SUITE 3A
ROCKY HILL, CT 06067



1997 ANNAPOLIS EXCHANGE PARKWAY
SUITE 200
ANNAPOLIS, MD 21401

**CT1022
CANTON-FIRE
DEPARTMENT**

CONSTRUCTION DRAWINGS

1	05/06/14	ISSUED AS FINAL
0	04/21/14	ISSUED AS FINAL
A	02/07/14	PRELIMINARY SUBMISSION



Dewberry Engineers Inc.
600 PARSIPPANY ROAD
SUITE 301
PARSIPPANY, NJ 07054
PHONE: 973.739.9400
FAX: 973.739.9710

ROBERT J. FOLEY, P.E.
CT LICENSE No. PEN.0029056

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER TO ALTER THIS DOCUMENT.

DRAWN BY:	FG
REVIEWED BY:	BSH
CHECKED BY:	GHN
PROJECT NUMBER:	50063024
JOB NUMBER:	50063025
SITE ADDRESS:	

14 CANTON SPRINGS RD.
CANTON, CT 06019
HARTFORD COUNTY

SHEET TITLE

GROUNDING NOTES
& DETAILS

SHEET NUMBER

Tab 2

Todd Oliver
Smartlink, LLC
Market Manager, NE
33 Boston Post Road, Suite 210
Marlborough, MA 01752

Reference: Smartlink LLC Site, Canton-Fire Department, 14 Canton Springs Road, Canton, CT

Date: 27 May 2014

1. This letter will address the additional RF impact that adding AT&T LTE antennas to the referenced site. Attached are two documents which cover the modeled RF emissions from the site.

2. The first report, "RF Emissions Compliance Report," for the site compiled by Sitesafe, uses the antenna patterns for the antennas at the site to calculate the General Public Maximum Permissible Exposure (MPE) on the ground. The total MPE of all the carriers is 6.934% (based on the General Public MPE) based on this modeling, with AT&T antennas emitting a maximum of 1.491% of the General Public MPE on the ground.

3. The second attachment has the calculations, used by the Connecticut Siting Council, which assumes the maximum antenna gain transmits in a spherical pattern where the worst case results would be at the base of the tower. That calculation, based on the existing antennas, gives a result of 91.6% of the General Public MPE, with the AT&T antennas emitting 26.00% of the General Public MPE on the ground, using the modeling predictions used by Connecticut Siting Council.

4. In either case, the site is compliant with FCC guidelines. If you have any questions regarding this site, the compliance report, please contact me at 719-434-0700 or dcotton@sitesafe.com.

Director, RF Compliance

RF EMISSIONS COMPLIANCE REPORT

Smartlink on behalf of AT&T Mobility, LLC

**AT&T Mobility, LLC Site FA: 10035260
AT&T Mobility, LLC Site ID: CT1022
AT&T Mobility, LLC USID: 140506
AT&T Mobility, LLC Site Name: Canton-Fire Department
14 Canton Springs Road
Canton, CT
5/27/2014**

Report Status:

AT&T Mobility, LLC Is Compliant

Prepared By:

Sitesafe, Inc.

Engineering Statement in Re:
Electromagnetic Energy Analysis
AT&T Mobility, LLC
Canton, CT

My signature on the cover of this document indicates:

That I am registered as a Professional Engineer in the jurisdiction indicated; and

That I have extensive professional experience in the wireless communications engineering industry; and

That I am an employee of Sitesafe, Inc. in Arlington, Virginia; and

That I am thoroughly familiar with the Rules and Regulations of the Federal Communications Commission ("the FCC" and "the FCC Rules") both in general and specifically as they apply to the FCC's Guidelines for Human Exposure to Radiofrequency Electromagnetic Fields; and

That the technical information serving as the basis for this report was supplied by AT&T Mobility, LLC (See attached Site Summary and Carrier documents), and that AT&T Mobility, LLC's installations involve communications equipment, antennas and associated technical equipment at a location referred to as the "Canton-Fire Department" ("the site"); and

That AT&T Mobility, LLC proposes to operate at the site with transmit antennas listed in the carrier summary and with a maximum effective radiated power as specified by AT&T Mobility, LLC and shown on the worksheet, and that worst-case 100% duty cycle have been assumed; and

That this analysis has been performed with the assumption that the ground immediately surrounding the tower is primarily flat or falling; and

That at this time, the FCC requires that certain licensees address specific levels of radio-frequency energy to which workers or members of the public might possibly be exposed (at §1.1307(b) of the FCC Rules); and

That such consideration of possible exposure of humans to radio-frequency radiation must utilize the standards set by the FCC, which is the Federal Agency having jurisdiction over communications facilities; and

That the FCC rules define two tiers of permissible exposure guidelines: 1) "uncontrolled environments," defined as situations in which persons may not be aware of (the "general public"), or may not be able to control their exposure to a transmission facility; and (2) "controlled environments," which defines situations in which persons are aware of their potential for exposure (industry personnel); and

That this statement specifically addresses the uncontrolled environment (which is more conservative than the controlled environment) and the limit set forth in the FCC rules for licensees of AT&T Mobility, LLC's operating frequency as shown on the attached antenna worksheet; and

That when applying the uncontrolled environment standards, the predicted Maximum Power Density at two meters above ground level from the proposed AT&T Mobility, LLC operation is no more than 1.491% of the maximum in any accessible area on the ground and

That it is understood per FCC Guidelines and OET65 Appendix A, that regardless of the existent radio-frequency environment, only those licenses whose contributions exceed five percent of the exposure limit pertinent to their operation(s) bear any responsibility for bringing any non-compliant area(s) into compliance; and

That when applying the uncontrolled environment standards, the cumulative predicted energy density from the proposed operation is no more than 6.934% of the maximum in any accessible area up to two meters above the ground per OET-65; and

That the calculations provided in this report are based on data provided by the client and antenna pattern data supplied by the antenna manufacturer, in accordance with FCC guidelines listed in OET-65. Horizontal and vertical antenna patterns are combined for modeling purposes to accurately reflect the energy two meters above ground level where on-axis energy refers to maximum energy two meters above the ground along the azimuth of the antenna and where area energy refers to the maximum energy anywhere two meters above the ground regardless of the antenna azimuth, accounting for cumulative energy from multiple antennas for the carrier and frequency range indicated; and

That the Occupational Safety and Health Administration has policies in place which address worker safety in and around communications sites, thus individual companies will be responsible for their employees' training regarding Radio Frequency Safety.

In summary, it is stated here that the proposed operation at the site would not result in exposure of the Public to excessive levels of radio-frequency energy as defined in the FCC Rules and Regulations, specifically 47 CFR 1.1307 and that AT&T Mobility, LLC's proposed operation is completely compliant.

Finally, it is stated that access to the tower should be restricted to communication industry professionals, and approved contractor personnel trained in radio-frequency safety; and that the instant analysis addresses exposure levels at two meters above ground level and does not address exposure levels on the tower, or in the immediate proximity of the antennas.

Note: Sitesafe has used data obtained from the “Connecticut Siting Council” to create this report. The manufacturer antenna patterns for AT&T Mobility, LLC were used to determine the RF emissions from the AT&T Mobility, LLC antennas. Generic antennas were used for the other carriers on the tower as this information was not available, or provided at the time the study was conducted. Sitesafe has also referenced the AT&T Mobility, LLC construction diagram for this site.

The following documents below were the primary sources of data used to create this report. The primary document was the “Connecticut Siting Council” document. The AT&T Mobility, LLC construction diagram was referenced when appropriate. Sitesafe has included modeling of the second carrier LTE in this analysis.

Note: Sprint-Nextel has decommissioned the existing iDEN network as of June 30, 2013. The modeling represented in this report follows the “Connecticut Siting Council” data.

Connecticut Siting Council: AlphaExMPowDens 4-16-14

AT&T Mobility, LLC Construction Diagram: 10035260.AE201.140506 (CT1022) Dewberry Rev 1 S&S

^[1] *This Power Density information was taken from the Connecticut Siting Council database dated April 16, 2014.*

^[2] *This Power Density information is based on worse case assumptions from AT&T’s radio frequency engineers.*

**AT&T Mobility, LLC (Proposed)
Canton-Fire Department
Site Summary**

Carrier	Area Maximum Percentage MPE
AT&T Mobility, LLC	0.558 %
AT&T Mobility, LLC	0.304 %
AT&T Mobility, LLC (Proposed/Future)	0 %
AT&T Mobility, LLC (Proposed/Future)	0 %
AT&T Mobility, LLC (Proposed/Future)	0 %
AT&T Mobility, LLC (Proposed/Future)	0 %
AT&T Mobility, LLC (Proposed)	0.239 %
AT&T Mobility, LLC (Proposed)	0.39 %
Canton FD	0.134 %
Nextel (iDEN - Decommissioned)	0.347 %
Pocket (Now MetroPCS)	0.525 %
Sprint-Nextel	0.333 %
Sprint-Nextel	0.329 %
T-Mobile	0.141 %
Verizon Wireless	0.63 %
Verizon Wireless	0.299 %
Verizon Wireless	0.823 %
Verizon Wireless	1.882 %
Composite Site MPE:	6.934 %

Power Density Calculations

Control Number	Site	Carrier	#Channels	ERP/Ch	Ant Ht	Power Density (mW/cm2)	MHz	S	%MPE	Site Total
EM-CING-023-121102	Canton - 14 Canton Springs Road	AT&T UMTS	2	565	130	0.0240	880	0.5867	4.10%	
EM-CING-023-121102	Canton - 14 Canton Springs Road	AT&T UMTS	2	1077	130	0.0458	1900	1.0000	4.58%	
EM-CING-023-121102	Canton - 14 Canton Springs Road	AT&T LTE	1	1615	130	0.0344	734	0.4893	7.02%	
EM-CING-023-121102	Canton - 14 Canton Springs Road	AT&T GSM	4	934	130	0.0795	1900	1.0000	7.95%	
EM-CING-023-121102	Canton - 14 Canton Springs Road	AT&T GSM	1	647	130	0.0138	880	0.5867	2.35%	
EM-VER-023-140221	Canton - 14 Canton Springs Road	Verizon cellular	9	403	120	0.0906	869	0.5793	15.63%	
EM-VER-023-140221	Canton - 14 Canton Springs Road	Verizon PCS	11	438	120	0.1203	1970	1.0000	12.03%	
EM-VER-023-140221	Canton - 14 Canton Springs Road	Verizon AWS	1	1750	120	0.0437	2145	1.0000	4.37%	
EM-VER-023-140221	Canton - 14 Canton Springs Road	Verizon LTE	1	1050	120	0.0262	698	0.4653	5.63%	
EM-Pocket-023-081117B	Canton - 14 Canton Springs Road	Pocket (now MetroPCS)	3	631	83	0.0988	2130	1.0000	9.88%	
EM-SPRINT-023-130405	Canton - 14 Canton Springs Road	Sprint CDMA/LTE	2	693	90	0.0615	1900	1.0000	6.15%	
EM-SPRINT-023-130405	Canton - 14 Canton Springs Road	Sprint CDMA/LTE	1	390	90	0.0173	850	0.5667	3.06%	
TS-SPRINT-023-020415	Canton - 14 Canton Springs Road	Canton FD	1	71	140	0.0013	300	0.2000	0.65%	
TS-SPRINT-023-020415	Canton - 14 Canton Springs Road	Nextel	9	100	110	0.0267	851	0.5673	4.71%	
EM-T-Mobile-023-031007	Canton - 14 Canton Springs Road	T-Mobile	8	121	100	0.0348	1935	1.0000	3.48%	91.60%

Tab 3

Structural Analysis Report

140-ft Existing EEI Monopole

*Proposed AT&T Mobility
Antenna Upgrade*

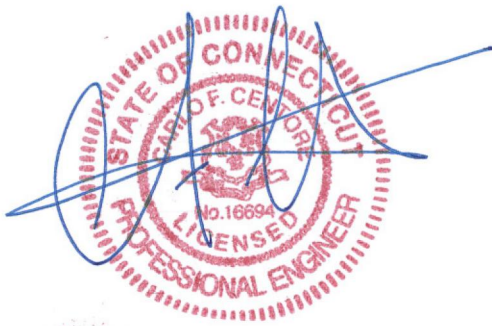
AT&T Site Ref: CT1022

Verizon Site Ref: Canton

*14 Canton Springs Road
Canton, CT*

Centek Project No. 14033.007

Date: May 27, 2014



Prepared for:
AT&T Mobility
500 Enterprise Drive, Suite 3A
Rocky Hill, CT 06067

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Introduction

The purpose of this report is to summarize the results of the non-linear, P- Δ structural analysis of the antenna installation proposed by AT&T Mobility on the existing monopole (tower) owned and operated by Verizon Wireless located in Canton, CT.

The host tower is a 140-ft, four-section, eighteen sided, tapered monopole, originally designed and manufactured by Engineered Endeavors Inc (EEI)—job no: 4960, dated May 13, 1999. The tower geometry, structure member sizes and foundation system information were obtained from a previous structural report prepared by Centek Engineering job no. 13001.107 dated January 6, 2014.

Antenna and appurtenance information were obtained from the aforementioned Centek structural report, a tower mapping report prepared by Eastern Communications Inc. dated April 25, 2014 and a AT&T RF data sheet.

The tower is made up of four (4) tapered vertical sections consisting of A572-65 pole sections. The vertical tower sections are slip joint connected. The diameter of the pole (flat-flat) is 18.00-in at the top and 51.00-in at the base.

AT&T proposes the removal of six (6) existing panel antennas and the installation of nine (9) panel antennas, fifteen (15) remote radio heads, six (6) A2 units and two (2) surge arrestors mounted on a proposed platform. Refer to the Antenna and Appurtenance Summary below for a detailed description of the proposed antenna and appurtenance configuration.

Antenna and Appurtenance Summary

The existing, proposed and future loads considered in this analysis consist of the following:

- TOWN (Existing):
Antennas: One (1) 20-ft x 2" dia. Omni-directional whip antenna mounted on a 4-ft side arm with an elevation of 138-ft above exiting grade.
Coax Cables: Two (2) 7/8" \varnothing coax cables running on the inside of the existing monopole.
- VERIZON (Existing/Reserved):
Antennas: Six (6) Antel LPA-80063-6CF panel antennas, six (6) Antel BXA-70063-6CF panel antennas, six (6) LPA-171063-12CF panel antennas, six (6) RFS FD9R6004/2C-3L Diplexers, six (6) RRH's and two (2) main distribution boxes mounted on a 13-ft platform w/ handrails with a RAD center elevation of 120-ft above grade.
Coax Cables: Twelve (12) 1-5/8" \varnothing coax cables running on the inside of the existing tower and six (6) 1-5/8" \varnothing coax cables and two (2) 1-5/8" \varnothing fiber cable running on the exterior of the existing tower.

- **T-MOBILE (Existing):**
Antennas: Four (4) EMS RR90-17-02DP panel antennas mounted on a 14-ft low profile platform with a RAD center elevation of 100-ft above exiting grade.
Coax Cables: Eight (8) 1-5/8" Ø coax cables running on the inside of the existing monopole.
- **SPRINT (Existing):**
Antennas: Two (2) RFS APXVSP18-C-A20 panel antennas, one (1) KMW ET-X-TU-42-15-37-iR-SP panel antenna, three (3) 1900MHz 4X40W RRH's and three (3) 800MHz 2X50W RRH's mounted on a 14-ft low profile platform with a RAD center elevation of 90-ft above exiting grade.
Coax Cables: Three (3) 1-1/4"Ø Hybriflex cables running on the inside of the existing monopole.
- **METROPCS (Existing):**
Antennas: Three (3) RFS APXV18-206517S panel antennas flush mounted with a RAD center elevation of 83-ft above exiting grade.
Coax Cables: Six (6) 1-5/8" Ø coax cables running on the inside of the existing monopole.
- **AT&T (Existing to Remain/Relocate):**
Antennas: Three (3) Kathrein 800-10121 panel antennas, three (3) Ericsson RRUS-11 remote radio heads, three (3) CCI DTMABP7819VG12A TMA's and one (1) Raycap DC6-48-60-18-8F surge arrester to be relocated to the proposed platform with a RAD center elevation of 130-ft above exiting grade.
Coax Cables: Six (6) 7/8" Ø coax cables running on the inside of the existing monopole. One (1) fiber cable and two (2) dc control cables running within the interior of the existing monopole.
- **AT&T (Existing to Remove):**
Antennas: One (1) KMW AM-X-CD-14-65-00T-RET panel antenna, two (2) KMW AM-X-CD-17-65-00T-RET panel antennas, three (3) Andrew SBNH-1D6565C panel antennas and three (3) CCI DTMABP7819VG12A TMA's mounted on a 13-ft platform w/ handrails with a RAD center elevation of 130-ft above exiting grade.
Coax Cables: Six (6) 7/8" Ø coax cables running on the inside of the existing monopole.
- **AT&T (PROPOSED):**
Antennas: Three (3) Andrew SBNHH-1D65A panel antennas, six (6) CCI HPA-65R-BUU-H8 panel antennas, three (3) Ericsson RRUS-11 remote radio units, six (6) Ericsson RRUS-12 remote radio units, three (3) Ericsson RRUS-32 remote radio units, three (3) Ericsson RRUS-E2 remote radio units, six (6) Ericsson A2 units and two (2) Raycap DC6-48-60-18-8F surge arrestors mounted on steel platform Commscope p/n MTC3607R with a RAD center elevation of 130-ft above existing grade.
Coax Cables: Four (4) dc control cables running on the exterior of the existing tower.

Primary Assumptions Used in the Analysis

- The tower structure's theoretical capacity not including any assessment of the condition of the tower.
- The tower carries the horizontal and vertical loads due to the weight of antennas, ice load and wind.
- Tower is properly installed and maintained.
- Tower is in plumb condition.
- Tower loading for antennas and mounts as listed in this report.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds are fabricated with ER-70S-6 electrodes.
- All members are assumed to be as specified in the original tower design documents or reinforcement drawings.
- All members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
- All member protective coatings are in good condition.
- All tower members were properly designed, detailed, fabricated, installed and have been properly maintained since erection.
- Any deviation from the analyzed antenna loading will require a new analysis for verification of structural adequacy.
- All existing coax cables to be installed as indicated in this report.

A n a l y s i s

The existing tower was analyzed using a comprehensive computer program entitled tnxTower. The program analyzes the tower, considering the worst case loading condition. The tower is considered as loaded by concentric forces along the tower shaft, and the model assumes that the shaft members are subjected to bending, axial, and shear forces.

The existing tower was analyzed for the controlling basic wind speed (fastest mile) with no ice and a 75% reduction of wind force with ½ inch accumulative ice to determine stresses in members as per guidelines of TIA/EIA-222-F-96 entitled “Structural Standards for Steel Antenna Towers and Antenna Supporting Structures”, the American Institute of Steel Construction (AISC) and the Manual of Steel Construction; Allowable Stress Design (ASD).

The controlling wind speed is determined by evaluating the local available wind speed data as provided in Appendix K of the CSBC¹ and the wind speed data available in the TIA/EIA-222-F-96 Standard. The higher of the two wind speeds is utilized in preparation on the tower analysis.

T o w e r L o a d i n g

Tower loading was determined by the basic wind speed as applied to projected surface areas with modification factors per TIA/EIA-222-F, gravity loads of the tower structure and its components, and the application of ½” radial ice on the tower structure and its components.

Basic Wind Speed:	Hartford; v = 80 mph (fastest mile)	[Section 16 of TIA/EIA-222-F-96]
	Canton; v = 95 mph (3 second gust) equivalent to v = 77.5 mph (fastest mile)	[Appendix K of the 2005 CT Building Code Supplement]
	<i>TIA/EIA-222-F wind speed controls.</i>	
Load Cases:	<u>Load Case 1</u> ; 80 mph wind speed w/ no ice plus gravity load – used in calculation of tower stresses and rotation.	[Section 2.3.16 of TIA/EIA-222-F-96]
	<u>Load Case 2</u> ; 69 mph wind speed w/ ½” radial ice plus gravity load – used in calculation of tower stresses. The 69 mph wind speed velocity represents 75% of the wind pressure generated by the 80 mph wind speed.	[Section 2.3.16 of TIA/EIA-222-F-96]
	<u>Load Case 3</u> ; Seismic – not checked	[Section 1614.5 of State Bldg. Code 2005] does not control in the design of this structure type

¹ The 2005 Connecticut State Building Code as amended by the 2009 CT State Supplement. (CSBC)

Tower Capacity

Tower stresses were calculated utilizing the structural analysis software tnxTower. Allowable stresses were determined based on Table 5 of the TIA/EIA code with a 1/3 increase per Section 3.1.1.1 of the same code.

- Calculated stresses were found to be within allowable limits. In Load Case 1, per tnxTower “Section Capacity Table”, this tower was found to be at **82.2%** of its total capacity.

Tower Section	Elevation	Stress Ratio (percentage of capacity)	Result
Pole Shaft (L2)	79.83'-115.36'	82.2%	PASS

Foundation and Anchors

The existing foundation consists of a 6.5-ft square x 4.5-ft long reinforced concrete pier on a 24.0-ft square x 3.0-ft thick reinforced concrete pad. The sub-grade conditions used in the analysis of the existing foundation were obtained from the aforementioned structural report. The base of the tower is connected to the foundation by means of (20) 2.25"Ø, ASTM A615-75 anchor bolts embedded approximately 5-ft into the concrete foundation structure.

- The tower base reactions developed from the governing Load Case 1 were used in the verification of the foundation and its anchors:

Location	Vector	Proposed Reactions
Base	Shear	32 kips
	Compression	40 kips
	Moment	3299 kip-ft

- The foundation was found to be within allowable limits.

Foundation	Design Limit	IBC 2003/2005 CT State Building Code Section 3108.4.2 (FS) ⁽¹⁾	Proposed Loading (FS) ⁽¹⁾	Result
Reinforced Concrete Pad and Pier	OTM ⁽²⁾	2.0	2.15	PASS

Note 1: FS denotes Factor of Safety.

Note 2: OTM denotes Overturning Moment

- The anchor bolts and base plate were found to be within allowable limits.

Tower Component	Design Limit	Stress Ratio (percentage of capacity)	Result
Anchor Bolts	Combined Compression and Bending	68.8%	PASS
Base Plate	Bending	81.3%	PASS

Conclusion

This analysis shows that the subject tower **is adequate** to support the proposed modified antenna configuration.

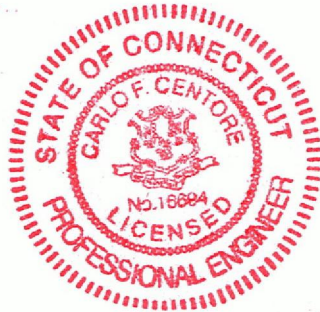
The analysis is based, in part, on the information provided to this office by AT&T Mobility. If the existing conditions are different than the information in this report, Centek Engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

Respectfully Submitted by:



Carlo F. Centore, PE
Principal ~ Structural Engineer



Prepared by:



Timothy J. Lynn, PE
Structural Engineer

*Standard Conditions for Furnishing of
Professional Engineering Services on
Existing Structures*

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of CENTEK engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provide to CENTEK engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the “as new” condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222
- All services performed, results obtained, and recommendations made are in accordance with generally accepted engineering principles and practices. CENTEK engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

General Description of Structural Analysis Program

tnxTower, is an integrated structural analysis and design software package for Designed specifically for the telecommunications industry, tnxTower, formerly ERITower, automates much of the tower analysis and design required by the TIA/EIA 222 Standard.

tnxTower Features:

- tnxTower can analyze and design 3- and 4-sided guyed towers, 3- and 4-sided self-supporting towers and either round or tapered ground mounted poles with or without guys.
- The program analyzes towers using the TIA-222-G (2005) standard or any of the previous TIA/EIA standards back to RS-222 (1959). Steel design is checked using the AISC ASD 9th Edition or the AISC LRFD specifications.
- Linear and non-linear (P-delta) analyses can be used in determining displacements and forces in the structure. Wind pressures and forces are automatically calculated.
- Extensive graphics plots include material take-off, shear-moment, leg compression, displacement, twist, feed line, guy anchor and stress plots.
- tnxTower contains unique features such as True Cable behavior, hog rod take-up, foundation stiffness and much more.

DESIGNED APPURTENANCE LOADING

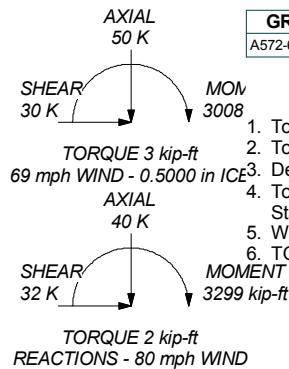
TYPE	ELEVATION	TYPE	ELEVATION
20' x 2" Dia Omni (Town - Existing)	148	LPA-80063/6CF (Verizon - Reserved)	120
4' Side Mount Standoff (Town - Existing)	138	LPA-171063-12CF (Verizon - Reserved)	120
800 10121 (ATI - Existing)	130	BXA-70063/6CF (Verizon - Reserved)	120
800 10121 (ATI - Existing)	130	BXA-70063/6CF (Verizon - Reserved)	120
800 10121 (ATI - Existing)	130	LPA-171063-12CF (Verizon - Reserved)	120
DTMABP7819VG12A TMA (ATI - Existing)	130	LPA-80063/6CF (Verizon - Reserved)	120
DTMABP7819VG12A TMA (ATI - Existing)	130	LPA-80063/6CF (Verizon - Reserved)	120
DTMABP7819VG12A TMA (ATI - Existing)	130	LPA-171063-12CF (Verizon - Reserved)	120
DTMABP7819VG12A TMA (ATI - Existing)	130	BXA-70063/6CF (Verizon - Reserved)	120
RRUS-11 (ATI - Existing)	130	BXA-70063/6CF (Verizon - Reserved)	120
RRUS-11 (ATI - Existing)	130	LPA-171063-12CF (Verizon - Reserved)	120
RRUS-11 (ATI - Existing)	130	LPA-171063-12CF (Verizon - Reserved)	120
DC6-48-60-18-8F Surge Arrestor (ATI - Existing)	130	LPA-80063/6CF (Verizon - Reserved)	120
SBNHH-1D65A (ATI - Proposed)	130	(2) FD9R6004/2C-3L Diplexer (Verizon - Reserved)	120
SBNHH-1D65A (ATI - Proposed)	130	(2) FD9R6004/2C-3L Diplexer (Verizon - Reserved)	120
SBNHH-1D65A (ATI - Proposed)	130	(2) FD9R6004/2C-3L Diplexer (Verizon - Reserved)	120
HPA-65R-BUU-H8 (ATI - Proposed)	130	(2) FD9R6004/2C-3L Diplexer (Verizon - Reserved)	120
HPA-65R-BUU-H8 (ATI - Proposed)	130	RRH2x40-AWS (Verizon - Reserved)	120
HPA-65R-BUU-H8 (ATI - Proposed)	130	RRH2x40-AWS (Verizon - Reserved)	120
HPA-65R-BUU-H8 (ATI - Proposed)	130	RRH2x40-AWS (Verizon - Reserved)	120
HPA-65R-BUU-H8 (ATI - Proposed)	130	RRH2x40-AWS (Verizon - Reserved)	120
HPA-65R-BUU-H8 (ATI - Proposed)	130	RRH2x40-07-U (Verizon - Reserved)	120
HPA-65R-BUU-H8 (ATI - Proposed)	130	RRH2x40-07-U (Verizon - Reserved)	120
RRUS-11 (ATI - Proposed)	130	RRH2x40-07-U (Verizon - Reserved)	120
(2) RRUS-12 (ATI - Proposed)	130	RRH2x40-07-U (Verizon - Reserved)	120
RRUS-32 (ATI - Proposed)	130	13' Platform w/rails (Verizon - Existing)	118
RRUS-E2 (ATI - Proposed)	130	(2) RR90-17-02DP (T-Mobile - Existing)	100
(2) A2 (ATI - Proposed)	130	(2) RR90-17-02DP (T-Mobile - Existing)	100
RRUS-11 (ATI - Proposed)	130	14-ft Low Profile Platform (T-Mobile - Existing)	100
(2) RRUS-12 (ATI - Proposed)	130	14-ft Low Profile Platform (Sprint - Existing)	90
RRUS-32 (ATI - Proposed)	130	FD-RRH 2x50 800 w/ Mount (Sprint - Existing)	90
RRUS-E2 (ATI - Proposed)	130	(2) A2 (ATI - Proposed)	130
(2) A2 (ATI - Proposed)	130	RRUS-11 (ATI - Proposed)	130
RRUS-11 (ATI - Proposed)	130	FD-RRH 4x40 1900 (Sprint - Existing)	90
(2) RRUS-12 (ATI - Proposed)	130	FD-RRH 2x50 800 w/ Mount (Sprint - Existing)	90
RRUS-32 (ATI - Proposed)	130	FD-RRH 4x40 1900 (Sprint - Existing)	90
RRUS-E2 (ATI - Proposed)	130	FD-RRH 4x40 1900 (Sprint - Existing)	90
(2) A2 (ATI - Proposed)	130	APXVSP18-C-A20 w/ Mount (Sprint - Existing)	90
DC6-48-60-18-8F Surge Arrestor (ATI - Proposed)	130	APXVSP18-C-A20 w/ Mount (Sprint - Existing)	90
DC6-48-60-18-8F Surge Arrestor (ATI - Proposed)	130	ET-X-TU-42-15-37-18-IR-SP (Sprint - Existing)	90
Commscope MTC3607R Platform (ATI - Proposed)	130	FD-RRH 2x50 800 w/ Mount (Sprint - Existing)	90
(2) DB-T1-6Z-8AB-0Z (Verizon - Reserved)	120	FD-RRH 4x40 1900 (Sprint - Existing)	90
LPA-80063/6CF (Verizon - Reserved)	120	APXV18-206517S (MetroPCS - Existing)	83
LPA-171063-12CF (Verizon - Reserved)	120	APXV18-206517S (MetroPCS - Existing)	83
BXA-70063/6CF (Verizon - Reserved)	120	Uni-Tri Bracket (MetroPCS - Existing)	83
BXA-70063/6CF (Verizon - Reserved)	120	APXV18-206517S (MetroPCS - Existing)	83
LPA-171063-12CF (Verizon - Reserved)	120		
LPA-80063/6CF (Verizon - Reserved)	120		

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
2. Tower is also designed for a 69 mph basic wind with 0.50 in ice.
3. Deflections are based upon a 50 mph wind.
4. Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
5. Welds are fabricated with ER-70S-6 electrodes.
6. TOWER RATING: 82.2%



141.0 ft
140.0 ft

115.4 ft

79.8 ft

40.3 ft

0.0 ft

Section	1	2	3	4
Length (ft)	24.64	39.06	44.16	45.54
Number of Sides	18	18	18	18
Thickness (in)	0.1875	0.3125	0.4375	0.5000
Socket Length (ft)	3.53	4.59	5.28	39.7436
Top Dia (in)	18.0000	22.9814	30.9366	51.0000
Bot Dia (in)	24.2521	32.7041	41.9334	11.0
Grade			A572-65	
Weight (K)	1.0	3.6	7.5	23.2

Centek Engineering Inc.

63-2 North Branford Rd.

Branford, CT 06405

Phone: (203) 488-0580

FAX: (203) 488-8587

Job: **14033.007 - CT1022**

Project: **140' EEI Monopole - 14 Canton Springs Rd, Canton, CT**

Client: AT&T Mobility

Drawn by: T.JL

App'd:

Code: TIA/EIA-222-F

Date: 05/27/14

Scale: NTS

Path:

Dwg No. E-1

J:\Jobs\140330\W007 - CT102204 - Structural\Draw Documents\Centek\EN Files\140 EEI Monopole Canton.ctb

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	Client AT&T Mobility	Designed by TJL

Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

- Basic wind speed of 80 mph.
- Nominal ice thickness of 0.5000 in.
- Ice density of 56 pcf.
- A wind speed of 69 mph is used in combination with ice.
- Temperature drop of 50 °F.
- Deflections calculated using a wind speed of 50 mph.
- Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards..
- Welds are fabricated with ER-70S-6 electrodes..
- A non-linear (P-delta) analysis was used.
- Pressures are calculated at each section.
- Stress ratio used in pole design is 1.333.
- Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Options

- | | | |
|--|--|---|
| <ul style="list-style-type: none"> Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification √ Use Code Stress Ratios √ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) Add IBC .6D+W Combination | <ul style="list-style-type: none"> Distribute Leg Loads As Uniform Assume Legs Pinned √ Assume Rigid Index Plate Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension √ Bypass Mast Stability Checks Use Azimuth Dish Coefficients √ Project Wind Area of Appurt. Autocalc Torque Arm Areas SR Members Have Cut Ends √ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing | <ul style="list-style-type: none"> Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation √ Consider Feedline Torque Include Angle Block Shear Check <li style="text-align: center;">Poles √ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets |
|--|--|---|

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	140.00-115.36	24.64	3.53	18	18.0000	24.2521	0.1875	0.7500	A572-65 (65 ksi)
L2	115.36-79.83	39.06	4.59	18	22.9814	32.7041	0.3125	1.2500	A572-65 (65 ksi)
L3	79.83-40.26	44.16	5.28	18	30.9366	41.9334	0.4375	1.7500	A572-65 (65 ksi)
L4	40.26-0.00	45.54		18	39.7436	51.0000	0.5000	2.0000	A572-65 (65 ksi)

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	Client AT&T Mobility	Designed by TJL

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	It/Q in ⁵	w in	w/t
L1	18.2777	10.6007	424.9328	6.3234	9.1440	46.4712	850.4248	5.3013	2.8380	15.136
	24.6262	14.3214	1047.8077	8.5429	12.3201	85.0489	2096.9941	7.1621	3.9384	21.005
L2	24.2282	22.4847	1459.7753	8.0475	11.6746	125.0391	2921.4715	11.2445	3.4947	11.183
	33.2086	32.1284	4258.8427	11.4990	16.6137	256.3455	8523.2892	16.0673	5.2059	16.659
L3	32.5745	42.3518	4977.1676	10.8272	15.7158	316.6988	9960.8842	21.1799	4.6748	10.685
	42.5803	57.6222	12535.3942	14.7310	21.3022	588.4563	25087.2828	28.8166	6.6103	15.109
L4	41.6819	62.2795	12117.6844	13.9315	20.1897	600.1905	24251.3135	31.1457	6.1149	12.23
	51.7868	80.1435	25821.9188	17.9275	25.9080	996.6774	51677.8148	40.0794	8.0960	16.192

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft ²	in					in	in
L1 140.00-115.36				1	1	1		
L2 115.36-79.83				1	1	1		
L3 79.83-40.26				1	1	1		
L4 40.26-0.00				1	1	1		

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	C _A A _A ft ² /ft	Weight plf
7/8 (Town - Existing)	A	No	Inside Pole	140.00 - 3.00	2	No Ice 1/2" Ice	0.00 0.54
RG6-Fiber (AT&T - Existing)	C	No	CaAa (Out Of Face)	130.00 - 3.00	1	No Ice 1/2" Ice	0.05 1.61
#8 AWG Copper Wire (AT&T - Existing)	C	No	CaAa (Out Of Face)	130.00 - 3.00	2	No Ice 1/2" Ice	0.01 0.43
1 5/8 (Verizon - Existing)	B	No	Inside Pole	120.00 - 3.00	12	No Ice 1/2" Ice	0.00 1.04
1/2 (Verizon - Existing)	B	No	Inside Pole	120.00 - 3.00	1	No Ice 1/2" Ice	0.00 0.25
1 5/8 (Verizon - Existing)	B	No	CaAa (Out Of Face)	120.00 - 3.00	1	No Ice 1/2" Ice	0.20 2.55
1 5/8 (Verizon - Existing)	B	No	CaAa (Out Of Face)	120.00 - 3.00	5	No Ice 1/2" Ice	0.00 2.55
HYBRIFLEX 1-5/8" (Verizon - Existing)	C	No	CaAa (Out Of Face)	120.00 - 3.00	1	No Ice 1/2" Ice	0.20 3.41
1 5/8 (T-Mobile - Existing)	A	No	Inside Pole	100.00 - 3.00	8	No Ice 1/2" Ice	0.00 1.04
HYBRIFLEX 1-5/8" (Sprint - Existing)	C	No	Inside Pole	90.00 - 3.00	3	No Ice 1/2" Ice	0.00 1.90
1 5/8 (MetroPCS - Existing)	C	No	Inside Pole	83.00 - 3.00	6	No Ice 1/2" Ice	0.00 1.04
HYBRIFLEX 1-5/8" (Verizon - Reserved)	B	No	CaAa (Out Of Face)	120.00 - 3.00	2	No Ice 1/2" Ice	0.20 3.41
#8 AWG Copper Wire (AT&T - Proposed)	C	No	CaAa (Out Of Face)	130.00 - 3.00	4	No Ice 1/2" Ice	0.01 0.43

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Description	Face or Leg	Allow or Shield	Component Type	Placement ft	Total Number	C _{AA}	Weight plf
7/8 (AT&T - Existing)	A	No	Inside Pole	130.00 - 3.00	6	No Ice 1/2" Ice	0.00 0.54

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
L1	140.00-115.36	A	0.000	0.000	0.000	0.000	0.07
		B	0.000	0.000	0.000	2.756	0.11
		C	0.000	0.000	0.000	2.779	0.03
L2	115.36-79.83	A	0.000	0.000	0.000	0.000	0.32
		B	0.000	0.000	0.000	21.105	0.81
		C	0.000	0.000	0.000	11.551	0.19
L3	79.83-40.26	A	0.000	0.000	0.000	0.000	0.50
		B	0.000	0.000	0.000	23.505	0.90
		C	0.000	0.000	0.000	12.864	0.60
L4	40.26-0.00	A	0.000	0.000	0.000	0.000	0.47
		B	0.000	0.000	0.000	22.132	0.85
		C	0.000	0.000	0.000	12.113	0.56

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
L1	140.00-115.36	A	0.500	0.000	0.000	0.000	0.000	0.07
		B		0.000	0.000	0.000	4.148	0.16
		C		0.000	0.000	0.000	13.491	0.08
L2	115.36-79.83	A	0.500	0.000	0.000	0.000	0.000	0.32
		B		0.000	0.000	0.000	31.763	1.24
		C		0.000	0.000	0.000	39.975	0.35
L3	79.83-40.26	A	0.500	0.000	0.000	0.000	0.000	0.50
		B		0.000	0.000	0.000	35.375	1.38
		C		0.000	0.000	0.000	44.520	0.77
L4	40.26-0.00	A	0.500	0.000	0.000	0.000	0.000	0.47
		B		0.000	0.000	0.000	33.310	1.30
		C		0.000	0.000	0.000	41.921	0.73

Feed Line Center of Pressure

Section	Elevation ft	CP _x in	CP _z in	CP _x Ice in	CP _z Ice in
L1	140.00-115.36	0.0040	0.1644	-0.3531	0.3984
L2	115.36-79.83	0.2513	0.4959	-0.1587	0.8004
L3	79.83-40.26	0.2691	0.5311	-0.1785	0.9004
L4	40.26-0.00	0.2625	0.5179	-0.1827	0.9213

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	Client AT&T Mobility	Designed by TJL

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A _A Front	C _A A _A Side	Weight
			Horz	Vert					
			ft	ft	°	ft	ft ²	ft ²	K
20' x 2" Dia Omni (Town - Existing)	C	From Face	4.00	0.0000	148.00	No Ice	4.00	4.00	0.02
			0.00			1/2" Ice	6.03	6.03	0.05
			0.00						
4' Side Mount Standoff (Town - Existing)	C	From Face	2.00	0.0000	138.00	No Ice	2.72	2.72	0.05
			0.00			1/2" Ice	4.91	4.91	0.09
			0.00						
800 10121 (AT&T - Existing)	A	From Face	4.00	0.0000	130.00	No Ice	5.46	3.29	0.05
			-6.00			1/2" Ice	5.88	3.64	0.08
			0.00						
800 10121 (AT&T - Existing)	B	From Face	4.00	0.0000	130.00	No Ice	5.46	3.29	0.05
			-6.00			1/2" Ice	5.88	3.64	0.08
			0.00						
800 10121 (AT&T - Existing)	C	From Face	4.00	0.0000	130.00	No Ice	5.46	3.29	0.05
			-6.00			1/2" Ice	5.88	3.64	0.08
			0.00						
DTMABP7819VG12A TMA (AT&T - Existing)	A	From Face	4.00	0.0000	130.00	No Ice	1.59	0.58	0.02
			0.00			1/2" Ice	1.76	0.70	0.03
			0.00						
DTMABP7819VG12A TMA (AT&T - Existing)	B	From Face	4.00	0.0000	130.00	No Ice	1.59	0.58	0.02
			0.00			1/2" Ice	1.76	0.70	0.03
			0.00						
DTMABP7819VG12A TMA (AT&T - Existing)	C	From Face	4.00	0.0000	130.00	No Ice	1.59	0.58	0.02
			0.00			1/2" Ice	1.76	0.70	0.03
			0.00						
RRUS-11 (AT&T - Existing)	A	From Face	1.00	0.0000	130.00	No Ice	2.99	1.25	0.05
			2.00			1/2" Ice	3.23	1.41	0.07
			0.00						
RRUS-11 (AT&T - Existing)	B	From Face	1.00	0.0000	130.00	No Ice	2.99	1.25	0.05
			2.00			1/2" Ice	3.23	1.41	0.07
			0.00						
RRUS-11 (AT&T - Existing)	C	From Face	1.00	0.0000	130.00	No Ice	2.99	1.25	0.05
			2.00			1/2" Ice	3.23	1.41	0.07
			0.00						
DC6-48-60-18-8F Surge Arrestor (AT&T - Existing)	C	From Face	0.50	0.0000	130.00	No Ice	2.23	2.23	0.02
			0.00			1/2" Ice	2.45	2.45	0.04
			0.00						
LPA-80063/6CF (Verizon - Reserved)	A	From Face	3.00	0.0000	120.00	No Ice	10.31	9.01	0.03
			6.00			1/2" Ice	10.87	9.55	0.10
			0.00						
LPA-171063-12CF (Verizon - Reserved)	A	From Face	3.00	0.0000	120.00	No Ice	5.99	6.05	0.01
			4.00			1/2" Ice	6.46	6.52	0.06
			0.00						
BXA-70063/6CF (Verizon - Reserved)	A	From Face	3.00	0.0000	120.00	No Ice	7.73	4.16	0.02
			1.00			1/2" Ice	8.27	4.60	0.06
			0.00						
BXA-70063/6CF (Verizon - Reserved)	A	From Face	3.00	0.0000	120.00	No Ice	7.73	4.16	0.02
			-1.00			1/2" Ice	8.27	4.60	0.06
			0.00						
LPA-171063-12CF (Verizon - Reserved)	A	From Face	3.00	0.0000	120.00	No Ice	5.99	6.05	0.01
			-4.00			1/2" Ice	6.46	6.52	0.06
			0.00						
LPA-80063/6CF	A	From Face	3.00	0.0000	120.00	No Ice	10.31	9.01	0.03

tnxTower Centek Engineering Inc. 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	Job		14033.007 - CT1022		Page		5 of 23	
	Project		140' EEI Monopole - 14 Canton Springs Rd, Canton, CT		Date		11:31:23 05/27/14	
	Client		AT&T Mobility		Designed by		TJL	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight
			Horz	Vert					
			ft	ft	°	ft	ft ²	ft ²	K
(Verizon - Reserved)			-6.00			1/2" Ice	10.87	9.55	0.10
LPA-80063/6CF	B	From Face	3.00		0.0000	No Ice	10.31	9.01	0.03
(Verizon - Reserved)			6.00			1/2" Ice	10.87	9.55	0.10
LPA-171063-12CF	B	From Face	3.00		0.0000	No Ice	5.99	6.05	0.01
(Verizon - Reserved)			4.00			1/2" Ice	6.46	6.52	0.06
BXA-70063/6CF	B	From Face	3.00		0.0000	No Ice	7.73	4.16	0.02
(Verizon - Reserved)			1.00			1/2" Ice	8.27	4.60	0.06
BXA-70063/6CF	B	From Face	3.00		0.0000	No Ice	7.73	4.16	0.02
(Verizon - Reserved)			-1.00			1/2" Ice	8.27	4.60	0.06
LPA-171063-12CF	B	From Face	3.00		0.0000	No Ice	5.99	6.05	0.01
(Verizon - Reserved)			-4.00			1/2" Ice	6.46	6.52	0.06
LPA-80063/6CF	B	From Face	3.00		0.0000	No Ice	10.31	9.01	0.03
(Verizon - Reserved)			-6.00			1/2" Ice	10.87	9.55	0.10
LPA-80063/6CF	C	From Face	3.00		0.0000	No Ice	10.31	9.01	0.03
(Verizon - Reserved)			6.00			1/2" Ice	10.87	9.55	0.10
LPA-171063-12CF	C	From Face	3.00		0.0000	No Ice	5.99	6.05	0.01
(Verizon - Reserved)			4.00			1/2" Ice	6.46	6.52	0.06
BXA-70063/6CF	C	From Face	3.00		0.0000	No Ice	7.73	4.16	0.02
(Verizon - Reserved)			1.00			1/2" Ice	8.27	4.60	0.06
BXA-70063/6CF	C	From Face	3.00		0.0000	No Ice	7.73	4.16	0.02
(Verizon - Reserved)			-1.00			1/2" Ice	8.27	4.60	0.06
LPA-171063-12CF	C	From Face	3.00		0.0000	No Ice	5.99	6.05	0.01
(Verizon - Reserved)			-4.00			1/2" Ice	6.46	6.52	0.06
LPA-80063/6CF	C	From Face	3.00		0.0000	No Ice	10.31	9.01	0.03
(Verizon - Reserved)			-6.00			1/2" Ice	10.87	9.55	0.10
(2) FD9R6004/2C-3L Diplexer	A	From Face	3.00		0.0000	No Ice	0.37	0.08	0.00
(Verizon - Reserved)			0.00			1/2" Ice	0.45	0.14	0.01
(2) FD9R6004/2C-3L Diplexer	B	From Face	3.00		0.0000	No Ice	0.37	0.08	0.00
(Verizon - Reserved)			0.00			1/2" Ice	0.45	0.14	0.01
(2) FD9R6004/2C-3L Diplexer	C	From Face	3.00		0.0000	No Ice	0.37	0.08	0.00
(Verizon - Reserved)			0.00			1/2" Ice	0.45	0.14	0.01
RRH2x40-AWS	A	From Face	3.00		0.0000	No Ice	2.52	1.59	0.04
(Verizon - Reserved)			0.00			1/2" Ice	2.75	1.80	0.06
RRH2x40-AWS	B	From Face	3.00		0.0000	No Ice	2.52	1.59	0.04
(Verizon - Reserved)			0.00			1/2" Ice	2.75	1.80	0.06
RRH2x40-AWS	C	From Face	3.00		0.0000	No Ice	2.52	1.59	0.04
(Verizon - Reserved)			0.00			1/2" Ice	2.75	1.80	0.06
RRH2x40-07-U	A	From Face	3.00		0.0000	No Ice	2.25	1.23	0.05

tnxTower Centek Engineering Inc. 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	Job		14033.007 - CT1022				Page		6 of 23
	Project		140' EEI Monopole - 14 Canton Springs Rd, Canton, CT				Date		11:31:23 05/27/14
	Client		AT&T Mobility				Designed by		TJL

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
			Horz Lateral ft	Vert ft					
(Verizon - Reserved)				0.00		1/2" Ice	2.45	1.39	0.07
RRH2x40-07-U	B	From Face	3.00	0.0000	120.00	No Ice	2.25	1.23	0.05
(Verizon - Reserved)			0.00			1/2" Ice	2.45	1.39	0.07
RRH2x40-07-U	C	From Face	3.00	0.0000	120.00	No Ice	2.25	1.23	0.05
(Verizon - Reserved)			0.00			1/2" Ice	2.45	1.39	0.07
(2) DB-T1-6Z-8AB-0Z	C	From Face	3.00	0.0000	120.00	No Ice	5.60	2.33	0.04
(Verizon - Reserved)			0.00			1/2" Ice	5.92	2.56	0.08
13' Platform w/rails	C	None		0.0000	118.00	No Ice	31.30	31.30	1.82
(Verizon - Existing)						1/2" Ice	40.20	40.20	2.45
(2) RR90-17-02DP	A	From Face	4.00	0.0000	100.00	No Ice	4.36	1.97	0.02
(T-Mobile - Existing)			0.00			1/2" Ice	4.77	2.31	0.04
(2) RR90-17-02DP	B	From Face	4.00	0.0000	100.00	No Ice	4.36	1.97	0.02
(T-Mobile - Existing)			0.00			1/2" Ice	4.77	2.31	0.04
14-ft Low Profile Platform	C	None		0.0000	100.00	No Ice	16.50	16.50	1.55
(T-Mobile - Existing)						1/2" Ice	20.00	20.00	1.80
APXVSPP18-C-A20 w/ Mount	A	From Face	4.00	0.0000	90.00	No Ice	8.96	8.08	0.12
(Sprint - Existing)			0.00			1/2" Ice	9.66	9.14	0.20
APXVSPP18-C-A20 w/ Mount	B	From Face	4.00	0.0000	90.00	No Ice	8.96	8.08	0.12
(Sprint - Existing)			0.00			1/2" Ice	9.66	9.14	0.20
ET-X-TU-42-15-37-18-iR-SP	C	From Face	4.00	0.0000	90.00	No Ice	8.45	3.31	0.06
(Sprint - Existing)			0.00			1/2" Ice	8.88	3.67	0.11
FD-RRH 2x50 800 w/ Mount	A	From Face	4.00	0.0000	90.00	No Ice	4.92	5.39	0.14
(Sprint - Existing)			2.00			1/2" Ice	5.52	6.08	0.20
FD-RRH 4x40 1900	A	From Face	4.00	0.0000	90.00	No Ice	2.61	2.71	0.06
(Sprint - Existing)			2.00			1/2" Ice	2.84	2.95	0.08
FD-RRH 2x50 800 w/ Mount	B	From Face	4.00	0.0000	90.00	No Ice	4.92	5.39	0.14
(Sprint - Existing)			2.00			1/2" Ice	5.52	6.08	0.20
FD-RRH 4x40 1900	B	From Face	4.00	0.0000	90.00	No Ice	2.61	2.71	0.06
(Sprint - Existing)			2.00			1/2" Ice	2.84	2.95	0.08
FD-RRH 2x50 800 w/ Mount	C	From Face	4.00	0.0000	90.00	No Ice	4.92	5.39	0.14
(Sprint - Existing)			2.00			1/2" Ice	5.52	6.08	0.20
FD-RRH 4x40 1900	C	From Face	4.00	0.0000	90.00	No Ice	2.61	2.71	0.06
(Sprint - Existing)			0.00			1/2" Ice	2.84	2.95	0.08
14-ft Low Profile Platform	C	None		0.0000	90.00	No Ice	16.50	16.50	1.55
(Sprint - Existing)						1/2" Ice	20.00	20.00	1.80
APXV18-206517S	A	From Face	1.50	0.0000	83.00	No Ice	5.17	3.04	0.03
(MetroPCS - Existing)			0.00			1/2" Ice	5.62	3.47	0.05
APXV18-206517S	B	From Face	1.50	0.0000	83.00	No Ice	5.17	3.04	0.03
(MetroPCS - Existing)			0.00			1/2" Ice	5.62	3.47	0.05
APXV18-206517S	C	From Face	1.50	0.0000	83.00	No Ice	5.17	3.04	0.03

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	Project		140' EEI Monopole - 14 Canton Springs Rd, Canton, CT		Date		11:31:23 05/27/14	
	Client		AT&T Mobility		Designed by		TJL	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight	
			Horz	Vert						ft
			Lateral		°	ft	ft ²	ft ²	K	
(MetroPCS - Existing)			0.00			1/2" Ice	5.62	3.47	0.05	
Uni-Tri Bracket	C	None	0.00		0.0000	83.00	No Ice	1.75	1.75	0.00
(MetroPCS - Existing)			0.00				1/2" Ice	1.94	1.94	0.00
SBNHH-1D65A	A	From Face	4.00		0.0000	130.00	No Ice	6.36	3.86	0.04
(AT&T - Proposed)			6.00				1/2" Ice	6.80	4.22	0.08
			0.00							
SBNHH-1D65A	A	From Face	4.00		0.0000	130.00	No Ice	6.36	3.86	0.04
(AT&T - Proposed)			4.00				1/2" Ice	6.80	4.22	0.08
			0.00							
SBNHH-1D65A	A	From Face	4.00		0.0000	130.00	No Ice	6.36	3.86	0.04
(AT&T - Proposed)			-4.00				1/2" Ice	6.80	4.22	0.08
			0.00							
HPA-65R-BUU-H8	B	From Face	4.00		0.0000	130.00	No Ice	13.30	7.52	0.07
(AT&T - Proposed)			6.00				1/2" Ice	13.99	8.09	0.14
			0.00							
HPA-65R-BUU-H8	B	From Face	4.00		0.0000	130.00	No Ice	13.30	7.52	0.07
(AT&T - Proposed)			4.00				1/2" Ice	13.99	8.09	0.14
			0.00							
HPA-65R-BUU-H8	B	From Face	4.00		0.0000	130.00	No Ice	13.30	7.52	0.07
(AT&T - Proposed)			-4.00				1/2" Ice	13.99	8.09	0.14
			0.00							
HPA-65R-BUU-H8	C	From Face	4.00		0.0000	130.00	No Ice	13.30	7.52	0.07
(AT&T - Proposed)			6.00				1/2" Ice	13.99	8.09	0.14
			0.00							
HPA-65R-BUU-H8	C	From Face	4.00		0.0000	130.00	No Ice	13.30	7.52	0.07
(AT&T - Proposed)			4.00				1/2" Ice	13.99	8.09	0.14
			0.00							
HPA-65R-BUU-H8	C	From Face	4.00		0.0000	130.00	No Ice	13.30	7.52	0.07
(AT&T - Proposed)			-4.00				1/2" Ice	13.99	8.09	0.14
			0.00							
RRUS-11	A	From Face	4.00		0.0000	130.00	No Ice	2.99	1.25	0.05
(AT&T - Proposed)			0.00				1/2" Ice	3.23	1.41	0.07
			0.00							
(2) RRUS-12	A	From Face	4.00		0.0000	130.00	No Ice	3.67	1.49	0.06
(AT&T - Proposed)			0.00				1/2" Ice	3.93	1.67	0.08
			0.00							
RRUS-32	A	From Face	4.00		0.0000	130.00	No Ice	3.87	2.76	0.08
(AT&T - Proposed)			0.00				1/2" Ice	4.15	3.02	0.10
			0.00							
RRUS-E2	A	From Face	4.00		0.0000	130.00	No Ice	3.67	1.49	0.06
(AT&T - Proposed)			0.00				1/2" Ice	3.93	1.67	0.08
			0.00							
(2) A2	A	From Face	4.00		0.0000	130.00	No Ice	2.42	0.54	0.02
(AT&T - Proposed)			0.00				1/2" Ice	2.63	0.67	0.03
			0.00							
RRUS-11	B	From Face	4.00		0.0000	130.00	No Ice	2.99	1.25	0.05
(AT&T - Proposed)			0.00				1/2" Ice	3.23	1.41	0.07
			0.00							
(2) RRUS-12	B	From Face	4.00		0.0000	130.00	No Ice	3.67	1.49	0.06
(AT&T - Proposed)			0.00				1/2" Ice	3.93	1.67	0.08
			0.00							
RRUS-32	B	From Face	4.00		0.0000	130.00	No Ice	3.87	2.76	0.08
(AT&T - Proposed)			0.00				1/2" Ice	4.15	3.02	0.10
			0.00							
RRUS-E2	B	From Face	4.00		0.0000	130.00	No Ice	3.67	1.49	0.06
(AT&T - Proposed)			0.00				1/2" Ice	3.93	1.67	0.08

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	Project	140' EEI Monopole - 14 Canton Springs Rd, Canton, CT	Date	11:31:23 05/27/14
	Client	AT&T Mobility	Designed by	TJL

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight	
			Horz Lateral	Vert						
			ft	ft	°	ft	ft ²	ft ²	K	
(2) A2 (AT&T - Proposed)	B	From Face	0.00	4.00	0.0000	130.00	No Ice	2.42	0.54	0.02
			0.00	0.00			1/2" Ice	2.63	0.67	0.03
			0.00	0.00						
RRUS-11 (AT&T - Proposed)	C	From Face	4.00	0.00	0.0000	130.00	No Ice	2.99	1.25	0.05
			0.00	0.00			1/2" Ice	3.23	1.41	0.07
			0.00	0.00						
(2) RRUS-12 (AT&T - Proposed)	C	From Face	4.00	0.00	0.0000	130.00	No Ice	3.67	1.49	0.06
			0.00	0.00			1/2" Ice	3.93	1.67	0.08
			0.00	0.00						
RRUS-32 (AT&T - Proposed)	C	From Face	4.00	0.00	0.0000	130.00	No Ice	3.87	2.76	0.08
			0.00	0.00			1/2" Ice	4.15	3.02	0.10
			0.00	0.00						
RRUS-E2 (AT&T - Proposed)	C	From Face	4.00	0.00	0.0000	130.00	No Ice	3.67	1.49	0.06
			0.00	0.00			1/2" Ice	3.93	1.67	0.08
			0.00	0.00						
(2) A2 (AT&T - Proposed)	C	From Face	4.00	0.00	0.0000	130.00	No Ice	2.42	0.54	0.02
			0.00	0.00			1/2" Ice	2.63	0.67	0.03
			0.00	0.00						
DC6-48-60-18-8F Surge Arrestor (AT&T - Proposed)	A	From Face	0.50	0.00	0.0000	130.00	No Ice	2.23	2.23	0.02
			0.00	0.00			1/2" Ice	2.45	2.45	0.04
			0.00	0.00						
DC6-48-60-18-8F Surge Arrestor (AT&T - Proposed)	B	From Face	0.50	0.00	0.0000	130.00	No Ice	2.23	2.23	0.02
			0.00	0.00			1/2" Ice	2.45	2.45	0.04
			0.00	0.00						
Commscope MTC3607R Platform (AT&T - Proposed)	A	From Face	0.50	0.00	0.0000	130.00	No Ice	53.00	53.00	2.52
			0.00	0.00			1/2" Ice	68.00	68.00	3.20
			0.00	0.00						

Tower Pressures - No Ice

$$G_H = 1.690$$

Section Elevation	z	K _Z	q _z	A _G	F _a	A _F	A _R	A _{leg}	Leg %	C _{AA} In Face	C _{AA} Out Face
ft	ft		psf	ft ²	e	ft ²	ft ²	ft ²		ft ²	ft ²
L1 140.00-115.36	127.07	1.47	24	43.379	A	0.000	43.379	43.379	100.00	0.000	0.000
					B	0.000	43.379	100.00	0.000	2.756	
					C	0.000	43.379	100.00	0.000	2.779	
L2 115.36-79.83	96.90	1.36	22	83.739	A	0.000	83.739	83.739	100.00	0.000	0.000
					B	0.000	83.739	100.00	0.000	21.105	
					C	0.000	83.739	100.00	0.000	11.551	
L3 79.83-40.26	59.64	1.184	19	122.029	A	0.000	122.029	122.029	100.00	0.000	0.000
					B	0.000	122.029	100.00	0.000	23.505	
					C	0.000	122.029	100.00	0.000	12.864	
L4 40.26-0.00	19.40	1	16	154.412	A	0.000	154.412	154.412	100.00	0.000	0.000
					B	0.000	154.412	100.00	0.000	22.132	
					C	0.000	154.412	100.00	0.000	12.113	

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Tower Pressure - With Ice

$G_H = 1.690$

Section Elevation	z	K _Z	q _z	t _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face	C _A A _A Out Face
ft	ft		psf	in	ft ²		ft ²	ft ²	ft ²		ft ²	ft ²
L1 140.00-115.36	127.07	1.47	18	0.5000	45.432	A	0.000	45.432	45.432	100.00	0.000	0.000
						B	0.000	45.432		100.00	0.000	4.148
						C	0.000	45.432		100.00	0.000	13.491
L2 115.36-79.83	96.90	1.36	17	0.5000	86.699	A	0.000	86.699	86.699	100.00	0.000	0.000
						B	0.000	86.699		100.00	0.000	31.763
						C	0.000	86.699		100.00	0.000	39.975
L3 79.83-40.26	59.64	1.184	14	0.5000	125.326	A	0.000	125.326	125.326	100.00	0.000	0.000
						B	0.000	125.326		100.00	0.000	35.375
						C	0.000	125.326		100.00	0.000	44.520
L4 40.26-0.00	19.40	1	12	0.5000	157.767	A	0.000	157.767	157.767	100.00	0.000	0.000
						B	0.000	157.767		100.00	0.000	33.310
						C	0.000	157.767		100.00	0.000	41.921

Tower Pressure - Service

$G_H = 1.690$

Section Elevation	z	K _Z	q _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face	C _A A _A Out Face
ft	ft		psf	ft ²		ft ²	ft ²	ft ²		ft ²	ft ²
L1 140.00-115.36	127.07	1.47	9	43.379	A	0.000	43.379	43.379	100.00	0.000	0.000
					B	0.000	43.379		100.00	0.000	2.756
					C	0.000	43.379		100.00	0.000	2.779
L2 115.36-79.83	96.90	1.36	9	83.739	A	0.000	83.739	83.739	100.00	0.000	0.000
					B	0.000	83.739		100.00	0.000	21.105
					C	0.000	83.739		100.00	0.000	11.551
L3 79.83-40.26	59.64	1.184	8	122.029	A	0.000	122.029	122.029	100.00	0.000	0.000
					B	0.000	122.029		100.00	0.000	23.505
					C	0.000	122.029		100.00	0.000	12.864
L4 40.26-0.00	19.40	1	6	154.412	A	0.000	154.412	154.412	100.00	0.000	0.000
					B	0.000	154.412		100.00	0.000	22.132
					C	0.000	154.412		100.00	0.000	12.113

Tower Forces - No Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 140.00-115.36	0.21	1.04	A	1	0.65	1	1	1	43.379	1.37	55.72	C
			B	1	0.65	1	1	1	43.379			
			C	1	0.65	1	1	1	43.379			
L2 115.36-79.83	1.32	3.63	A	1	0.65	1	1	1	83.739	3.28	92.18	C
			B	1	0.65	1	1	1	83.739			

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Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L3 79.83-40.26	2.00	7.51	C	1	0.65	1	1	1	83.739	3.77	95.38	C
			A	1	0.65	1	1	1	122.029			
			B	1	0.65	1	1	1	122.029			
L4 40.26-0.00	1.88	11.04	C	1	0.65	1	1	1	122.029	3.73	92.58	C
			A	1	0.65	1	1	1	154.412			
			B	1	0.65	1	1	1	154.412			
Sum Weight:	5.41	23.22	C	1	0.65	1	1	154.412	789.23	12.15		
								OTM	kip-ft			

Tower Forces - No Ice - Wind 45 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L1 140.00-115.36	0.21	1.04	A	1	0.65	1	1	1	43.379	1.37	55.72	C
			B	1	0.65	1	1	1	43.379			
			C	1	0.65	1	1	1	43.379			
L2 115.36-79.83	1.32	3.63	A	1	0.65	1	1	1	83.739	3.28	92.18	C
			B	1	0.65	1	1	1	83.739			
			C	1	0.65	1	1	1	83.739			
L3 79.83-40.26	2.00	7.51	A	1	0.65	1	1	1	122.029	3.77	95.38	C
			B	1	0.65	1	1	1	122.029			
			C	1	0.65	1	1	1	122.029			
L4 40.26-0.00	1.88	11.04	A	1	0.65	1	1	1	154.412	3.73	92.58	C
			B	1	0.65	1	1	1	154.412			
			C	1	0.65	1	1	1	154.412			
Sum Weight:	5.41	23.22	C	1	0.65	1	1	154.412	789.23	12.15		
								OTM	kip-ft			

Tower Forces - No Ice - Wind 60 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L1 140.00-115.36	0.21	1.04	A	1	0.65	1	1	1	43.379	1.37	55.72	C
			B	1	0.65	1	1	1	43.379			
			C	1	0.65	1	1	1	43.379			
L2 115.36-79.83	1.32	3.63	A	1	0.65	1	1	1	83.739	3.28	92.18	C
			B	1	0.65	1	1	1	83.739			
			C	1	0.65	1	1	1	83.739			
L3 79.83-40.26	2.00	7.51	A	1	0.65	1	1	1	122.029	3.77	95.38	C
			B	1	0.65	1	1	1	122.029			
			C	1	0.65	1	1	1	122.029			
L4 40.26-0.00	1.88	11.04	A	1	0.65	1	1	1	154.412	3.73	92.58	C
			B	1	0.65	1	1	1	154.412			
			C	1	0.65	1	1	1	154.412			
Sum Weight:	5.41	23.22	C	1	0.65	1	1	154.412	789.23	12.15		
								OTM	kip-ft			

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Tower Forces - No Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 140.00-115.36	0.21	1.04	A	1	0.65	1	1	1	43.379	1.37	55.72	C
			B	1	0.65	1	1	1	43.379			
			C	1	0.65	1	1	1	43.379			
L2 115.36-79.83	1.32	3.63	A	1	0.65	1	1	1	83.739	3.28	92.18	C
			B	1	0.65	1	1	1	83.739			
			C	1	0.65	1	1	1	83.739			
L3 79.83-40.26	2.00	7.51	A	1	0.65	1	1	1	122.029	3.77	95.38	C
			B	1	0.65	1	1	1	122.029			
			C	1	0.65	1	1	1	122.029			
L4 40.26-0.00	1.88	11.04	A	1	0.65	1	1	1	154.412	3.73	92.58	C
			B	1	0.65	1	1	1	154.412			
			C	1	0.65	1	1	1	154.412			
Sum Weight:	5.41	23.22						OTM	789.23 kip-ft	12.15		

Tower Forces - With Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 140.00-115.36	0.31	1.37	A	1	0.65	1	1	1	45.432	1.44	58.44	C
			B	1	0.65	1	1	1	45.432			
			C	1	0.65	1	1	1	45.432			
L2 115.36-79.83	1.91	4.26	A	1	0.65	1	1	1	86.699	3.61	101.69	C
			B	1	0.65	1	1	1	86.699			
			C	1	0.65	1	1	1	86.699			
L3 79.83-40.26	2.65	8.43	A	1	0.65	1	1	1	125.326	3.95	99.77	C
			B	1	0.65	1	1	1	125.326			
			C	1	0.65	1	1	1	125.326			
L4 40.26-0.00	2.50	12.19	A	1	0.65	1	1	1	157.767	3.69	91.70	C
			B	1	0.65	1	1	1	157.767			
			C	1	0.65	1	1	1	157.767			
Sum Weight:	7.37	26.25						OTM	840.17 kip-ft	12.69		

Tower Forces - With Ice - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 140.00-115.36	0.31	1.37	A	1	0.65	1	1	1	45.432	1.44	58.44	C
			B	1	0.65	1	1	1	45.432			
			C	1	0.65	1	1	1	45.432			

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	Client AT&T Mobility	Designed by TJL

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L2 115.36-79.83	1.91	4.26	A	1	0.65	1	1	1	86.699	3.61	101.69	C
			B	1	0.65	1	1	1	86.699			
			C	1	0.65	1	1	1	86.699			
L3 79.83-40.26	2.65	8.43	A	1	0.65	1	1	1	125.326	3.95	99.77	C
			B	1	0.65	1	1	1	125.326			
			C	1	0.65	1	1	1	125.326			
L4 40.26-0.00	2.50	12.19	A	1	0.65	1	1	1	157.767	3.69	91.70	C
			B	1	0.65	1	1	1	157.767			
			C	1	0.65	1	1	1	157.767			
Sum Weight:	7.37	26.25						OTM	840.17 kip-ft	12.69		

Tower Forces - With Ice - Wind 60 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L1 140.00-115.36	0.31	1.37	A	1	0.65	1	1	1	45.432	1.44	58.44	C
			B	1	0.65	1	1	1	45.432			
			C	1	0.65	1	1	1	45.432			
L2 115.36-79.83	1.91	4.26	A	1	0.65	1	1	1	86.699	3.61	101.69	C
			B	1	0.65	1	1	1	86.699			
			C	1	0.65	1	1	1	86.699			
L3 79.83-40.26	2.65	8.43	A	1	0.65	1	1	1	125.326	3.95	99.77	C
			B	1	0.65	1	1	1	125.326			
			C	1	0.65	1	1	1	125.326			
L4 40.26-0.00	2.50	12.19	A	1	0.65	1	1	1	157.767	3.69	91.70	C
			B	1	0.65	1	1	1	157.767			
			C	1	0.65	1	1	1	157.767			
Sum Weight:	7.37	26.25						OTM	840.17 kip-ft	12.69		

Tower Forces - With Ice - Wind 90 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L1 140.00-115.36	0.31	1.37	A	1	0.65	1	1	1	45.432	1.44	58.44	C
			B	1	0.65	1	1	1	45.432			
			C	1	0.65	1	1	1	45.432			
L2 115.36-79.83	1.91	4.26	A	1	0.65	1	1	1	86.699	3.61	101.69	C
			B	1	0.65	1	1	1	86.699			
			C	1	0.65	1	1	1	86.699			
L3 79.83-40.26	2.65	8.43	A	1	0.65	1	1	1	125.326	3.95	99.77	C
			B	1	0.65	1	1	1	125.326			
			C	1	0.65	1	1	1	125.326			
L4 40.26-0.00	2.50	12.19	A	1	0.65	1	1	1	157.767	3.69	91.70	C
			B	1	0.65	1	1	1	157.767			
			C	1	0.65	1	1	1	157.767			

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Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
Sum Weight:	7.37	26.25						OTM	840.17 kip-ft	12.69		

Tower Forces - Service - Wind Normal To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L1 140.00-115.36	0.21	1.04	A	1	0.65	1	1	1	43.379	0.54	21.77	C
			B	1	0.65	1	1	1	43.379			
			C	1	0.65	1	1	1	43.379			
L2 115.36-79.83	1.32	3.63	A	1	0.65	1	1	1	83.739	1.28	36.01	C
			B	1	0.65	1	1	1	83.739			
			C	1	0.65	1	1	1	83.739			
L3 79.83-40.26	2.00	7.51	A	1	0.65	1	1	1	122.029	1.47	37.26	C
			B	1	0.65	1	1	1	122.029			
			C	1	0.65	1	1	1	122.029			
L4 40.26-0.00	1.88	11.04	A	1	0.65	1	1	1	154.412	1.46	36.16	C
			B	1	0.65	1	1	1	154.412			
			C	1	0.65	1	1	1	154.412			
Sum Weight:	5.41	23.22						OTM	308.29 kip-ft	4.75		

Tower Forces - Service - Wind 45 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L1 140.00-115.36	0.21	1.04	A	1	0.65	1	1	1	43.379	0.54	21.77	C
			B	1	0.65	1	1	1	43.379			
			C	1	0.65	1	1	1	43.379			
L2 115.36-79.83	1.32	3.63	A	1	0.65	1	1	1	83.739	1.28	36.01	C
			B	1	0.65	1	1	1	83.739			
			C	1	0.65	1	1	1	83.739			
L3 79.83-40.26	2.00	7.51	A	1	0.65	1	1	1	122.029	1.47	37.26	C
			B	1	0.65	1	1	1	122.029			
			C	1	0.65	1	1	1	122.029			
L4 40.26-0.00	1.88	11.04	A	1	0.65	1	1	1	154.412	1.46	36.16	C
			B	1	0.65	1	1	1	154.412			
			C	1	0.65	1	1	1	154.412			
Sum Weight:	5.41	23.22						OTM	308.29 kip-ft	4.75		

Tower Forces - Service - Wind 60 To Face

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	Client AT&T Mobility	Designed by TJJ

Section Elevation ft	Add Weight K	Self Weight K	Face	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L1 140.00-115.36	0.21	1.04	A	1	0.65	1	1	1	43.379	0.54	21.77	C
			B	1	0.65	1	1	1	43.379			
			C	1	0.65	1	1	1	43.379			
L2 115.36-79.83	1.32	3.63	A	1	0.65	1	1	1	83.739	1.28	36.01	C
			B	1	0.65	1	1	1	83.739			
			C	1	0.65	1	1	1	83.739			
L3 79.83-40.26	2.00	7.51	A	1	0.65	1	1	1	122.029	1.47	37.26	C
			B	1	0.65	1	1	1	122.029			
			C	1	0.65	1	1	1	122.029			
L4 40.26-0.00	1.88	11.04	A	1	0.65	1	1	1	154.412	1.46	36.16	C
			B	1	0.65	1	1	1	154.412			
			C	1	0.65	1	1	1	154.412			
Sum Weight:	5.41	23.22						OTM	308.29 kip-ft	4.75		

Tower Forces - Service - Wind 90 To Face

Section Elevation ft	Add Weight K	Self Weight K	Face	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L1 140.00-115.36	0.21	1.04	A	1	0.65	1	1	1	43.379	0.54	21.77	C
			B	1	0.65	1	1	1	43.379			
			C	1	0.65	1	1	1	43.379			
L2 115.36-79.83	1.32	3.63	A	1	0.65	1	1	1	83.739	1.28	36.01	C
			B	1	0.65	1	1	1	83.739			
			C	1	0.65	1	1	1	83.739			
L3 79.83-40.26	2.00	7.51	A	1	0.65	1	1	1	122.029	1.47	37.26	C
			B	1	0.65	1	1	1	122.029			
			C	1	0.65	1	1	1	122.029			
L4 40.26-0.00	1.88	11.04	A	1	0.65	1	1	1	154.412	1.46	36.16	C
			B	1	0.65	1	1	1	154.412			
			C	1	0.65	1	1	1	154.412			
Sum Weight:	5.41	23.22						OTM	308.29 kip-ft	4.75		

Force Totals

Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, M _x kip-ft	Sum of Overturning Moments, M _z kip-ft	Sum of Torques kip-ft
Leg Weight	23.22					
Bracing Weight	0.00					
Total Member Self-Weight	23.22					
Total Weight	39.95					
Wind 0 deg - No Ice		0.17	-32.40	-3200.41	-21.19	-1.09
Wind 30 deg - No Ice		16.13	-28.15	-2783.00	-1591.49	-0.07
Wind 45 deg - No Ice		22.73	-23.04	-2279.11	-2239.59	0.46
Wind 60 deg - No Ice		27.77	-16.35	-1619.92	-2734.95	0.96
Wind 90 deg - No Ice		31.96	-0.17	-22.81	-3145.18	1.74

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Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, M_x kip-ft	Sum of Overturning Moments, M_z kip-ft	Sum of Torques kip-ft
Wind 120 deg - No Ice		27.60	16.05	1580.38	-2712.26	2.05
Wind 135 deg - No Ice		22.48	22.79	2246.79	-2207.49	2.00
Wind 150 deg - No Ice		15.83	27.98	2760.08	-1552.19	1.81
Wind 180 deg - No Ice		-0.17	32.40	3200.18	24.19	1.09
Wind 210 deg - No Ice		-16.13	28.15	2782.77	1594.49	0.07
Wind 225 deg - No Ice		-22.73	23.04	2278.88	2242.58	-0.46
Wind 240 deg - No Ice		-27.77	16.35	1619.69	2737.95	-0.96
Wind 270 deg - No Ice		-31.96	0.17	22.58	3148.18	-1.74
Wind 300 deg - No Ice		-27.60	-16.05	-1580.61	2715.26	-2.05
Wind 315 deg - No Ice		-22.48	-22.79	-2247.02	2210.49	-2.00
Wind 330 deg - No Ice		-15.83	-27.98	-2760.31	1555.19	-1.81
Member Ice	3.03					
Total Weight Ice	49.76			1.51	0.78	
Wind 0 deg - Ice		0.13	-29.88	-2885.92	-16.48	-1.67
Wind 30 deg - Ice		14.88	-25.94	-2507.71	-1437.00	-0.47
Wind 45 deg - Ice		20.98	-21.22	-2052.42	-2023.62	0.19
Wind 60 deg - Ice		25.64	-15.05	-1457.15	-2472.27	0.85
Wind 90 deg - Ice		29.53	-0.13	-15.75	-2844.89	1.94
Wind 120 deg - Ice		25.51	14.82	1430.28	-2455.02	2.51
Wind 135 deg - Ice		20.79	21.03	2031.03	-1999.21	2.55
Wind 150 deg - Ice		14.65	25.81	2493.47	-1407.11	2.41
Wind 180 deg - Ice		-0.13	29.88	2888.94	18.04	1.67
Wind 210 deg - Ice		-14.88	25.94	2510.73	1438.57	0.47
Wind 225 deg - Ice		-20.98	21.22	2055.43	2025.18	-0.19
Wind 240 deg - Ice		-25.64	15.05	1460.17	2473.84	-0.85
Wind 270 deg - Ice		-29.53	0.13	18.77	2846.46	-1.94
Wind 300 deg - Ice		-25.51	-14.82	-1427.26	2456.58	-2.51
Wind 315 deg - Ice		-20.79	-21.03	-2028.01	2000.78	-2.55
Wind 330 deg - Ice		-14.65	-25.81	-2490.45	1408.67	-2.41
Total Weight	39.95			-0.12	1.50	
Wind 0 deg - Service		0.07	-12.66	-1251.42	-6.32	-0.42
Wind 30 deg - Service		6.30	-11.00	-1088.37	-619.72	-0.03
Wind 45 deg - Service		8.88	-9.00	-891.53	-872.88	0.18
Wind 60 deg - Service		10.85	-6.39	-634.04	-1066.38	0.38
Wind 90 deg - Service		12.49	-0.07	-10.17	-1226.63	0.68
Wind 120 deg - Service		10.78	6.27	616.08	-1057.52	0.80
Wind 135 deg - Service		8.78	8.90	876.40	-860.34	0.78
Wind 150 deg - Service		6.18	10.93	1076.90	-604.36	0.71
Wind 180 deg - Service		-0.07	12.66	1248.81	11.41	0.42
Wind 210 deg - Service		-6.30	11.00	1085.76	624.81	0.03
Wind 225 deg - Service		-8.88	9.00	888.93	877.97	-0.18
Wind 240 deg - Service		-10.85	6.39	631.43	1071.47	-0.38
Wind 270 deg - Service		-12.49	0.07	7.56	1231.72	-0.68
Wind 300 deg - Service		-10.78	-6.27	-618.68	1062.61	-0.80
Wind 315 deg - Service		-8.78	-8.90	-879.00	865.43	-0.78
Wind 330 deg - Service		-6.18	-10.93	-1079.50	609.45	-0.71

Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 45 deg - No Ice
5	Dead+Wind 60 deg - No Ice

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Comb. No.	Description
6	Dead+Wind 90 deg - No Ice
7	Dead+Wind 120 deg - No Ice
8	Dead+Wind 135 deg - No Ice
9	Dead+Wind 150 deg - No Ice
10	Dead+Wind 180 deg - No Ice
11	Dead+Wind 210 deg - No Ice
12	Dead+Wind 225 deg - No Ice
13	Dead+Wind 240 deg - No Ice
14	Dead+Wind 270 deg - No Ice
15	Dead+Wind 300 deg - No Ice
16	Dead+Wind 315 deg - No Ice
17	Dead+Wind 330 deg - No Ice
18	Dead+Ice+Temp
19	Dead+Wind 0 deg+Ice+Temp
20	Dead+Wind 30 deg+Ice+Temp
21	Dead+Wind 45 deg+Ice+Temp
22	Dead+Wind 60 deg+Ice+Temp
23	Dead+Wind 90 deg+Ice+Temp
24	Dead+Wind 120 deg+Ice+Temp
25	Dead+Wind 135 deg+Ice+Temp
26	Dead+Wind 150 deg+Ice+Temp
27	Dead+Wind 180 deg+Ice+Temp
28	Dead+Wind 210 deg+Ice+Temp
29	Dead+Wind 225 deg+Ice+Temp
30	Dead+Wind 240 deg+Ice+Temp
31	Dead+Wind 270 deg+Ice+Temp
32	Dead+Wind 300 deg+Ice+Temp
33	Dead+Wind 315 deg+Ice+Temp
34	Dead+Wind 330 deg+Ice+Temp
35	Dead+Wind 0 deg - Service
36	Dead+Wind 30 deg - Service
37	Dead+Wind 45 deg - Service
38	Dead+Wind 60 deg - Service
39	Dead+Wind 90 deg - Service
40	Dead+Wind 120 deg - Service
41	Dead+Wind 135 deg - Service
42	Dead+Wind 150 deg - Service
43	Dead+Wind 180 deg - Service
44	Dead+Wind 210 deg - Service
45	Dead+Wind 225 deg - Service
46	Dead+Wind 240 deg - Service
47	Dead+Wind 270 deg - Service
48	Dead+Wind 300 deg - Service
49	Dead+Wind 315 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	140 - 115.36	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	18	-9.83	3.17	0.19
			Max. Mx	14	-4.94	127.06	-1.01
			Max. My	2	-4.87	0.56	127.75
			Max. Vy	14	-16.28	127.06	-1.01
			Max. Vx	2	-16.75	0.56	127.75
			Max. Torque	15			3.37
			Max Tension	1	0.00	0.00	0.00
L2	115.36 - 79.83	Pole	Max Tension	1	0.00	0.00	0.00

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L3	79.83 - 40.26	Pole	Max. Compression	18	-22.99	2.61	0.54
			Max. Mx	14	-15.29	821.11	-7.16
			Max. My	2	-15.24	-5.90	836.86
			Max. Vy	14	-24.48	821.11	-7.16
			Max. Vx	2	-24.93	-5.90	836.86
			Max. Torque	15			3.39
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	18	-33.56	1.88	-0.33
			Max. Mx	14	-24.89	1860.71	-14.64
			Max. My	2	-24.86	-13.25	1894.09
L4	40.26 - 0	Pole	Max. Vy	14	-28.48	1860.71	-14.64
			Max. Vx	2	-28.93	-13.25	1894.09
			Max. Torque	33			2.40
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	18	-49.76	0.89	-1.51
			Max. Mx	14	-39.93	3238.13	-23.26
			Max. My	2	-39.93	-21.81	3291.80
			Max. Vy	14	-31.99	3238.13	-23.26
			Max. Vx	2	-32.43	-21.81	3291.80
			Max. Torque	33			2.67

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	27	49.76	0.13	-29.88
	Max. H _x	14	39.95	31.96	-0.17
	Max. H _z	2	39.95	-0.17	32.40
	Max. M _x	2	3291.80	-0.17	32.40
	Max. M _z	6	3234.90	-31.96	0.17
	Max. Torsion	33	2.67	20.79	21.03
	Min. Vert	1	39.95	0.00	0.00
	Min. H _x	6	39.95	-31.96	0.17
	Min. H _z	10	39.95	0.17	-32.40
	Min. M _x	10	-3291.50	0.17	-32.40
	Min. M _z	14	-3238.13	31.96	-0.17
	Min. Torsion	25	-2.67	-20.79	-21.03

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead Only	39.95	0.00	0.00	-0.14	1.57	0.00
Dead+Wind 0 deg - No Ice	39.95	0.17	-32.40	-3291.80	-21.81	-1.32
Dead+Wind 30 deg - No Ice	39.95	16.13	-28.15	-2862.51	-1636.84	-0.33
Dead+Wind 45 deg - No Ice	39.95	22.73	-23.04	-2344.29	-2303.40	0.21
Dead+Wind 60 deg - No Ice	39.95	27.77	-16.35	-1666.31	-2812.92	0.74
Dead+Wind 90 deg - No Ice	39.95	31.96	-0.17	-23.60	-3234.90	1.62
Dead+Wind 120 deg - No Ice	39.95	27.60	16.05	1625.46	-2789.60	2.07
Dead+Wind 135 deg - No Ice	39.95	22.48	22.79	2310.93	-2270.38	2.08
Dead+Wind 150 deg - No Ice	39.95	15.83	27.98	2838.87	-1596.33	1.96

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Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead+Wind 180 deg - No Ice	39.95	-0.17	32.40	3291.50	25.05	1.32
Dead+Wind 210 deg - No Ice	39.95	-16.13	28.15	2862.19	1640.09	0.34
Dead+Wind 225 deg - No Ice	39.95	-22.73	23.04	2343.96	2306.65	-0.21
Dead+Wind 240 deg - No Ice	39.95	-27.77	16.35	1665.98	2816.16	-0.74
Dead+Wind 270 deg - No Ice	39.95	-31.96	0.17	23.26	3238.13	-1.62
Dead+Wind 300 deg - No Ice	39.95	-27.60	-16.05	-1625.79	2792.82	-2.07
Dead+Wind 315 deg - No Ice	39.95	-22.48	-22.79	-2311.25	2273.60	-2.08
Dead+Wind 330 deg - No Ice	39.95	-15.83	-27.98	-2839.18	1599.55	-1.96
Dead+Ice+Temp	49.76	-0.00	0.00	1.51	0.89	0.00
Dead+Wind 0 deg+Ice+Temp	49.76	0.13	-29.88	-3000.13	-17.10	-1.92
Dead+Wind 30 deg+Ice+Temp	49.76	14.88	-25.94	-2607.00	-1493.71	-0.73
Dead+Wind 45 deg+Ice+Temp	49.76	20.98	-21.22	-2133.74	-2103.49	-0.04
Dead+Wind 60 deg+Ice+Temp	49.76	25.64	-15.05	-1514.96	-2569.88	0.65
Dead+Wind 90 deg+Ice+Temp	49.76	29.53	-0.13	-16.54	-2957.23	1.86
Dead+Wind 120 deg+Ice+Temp	49.76	25.51	14.82	1486.77	-2551.91	2.57
Dead+Wind 135 deg+Ice+Temp	49.76	20.79	21.03	2111.31	-2078.04	2.67
Dead+Wind 150 deg+Ice+Temp	49.76	14.65	25.81	2592.06	-1462.50	2.59
Dead+Wind 180 deg+Ice+Temp	49.76	-0.13	29.88	3003.18	18.99	1.92
Dead+Wind 210 deg+Ice+Temp	49.76	-14.88	25.94	2610.05	1495.62	0.73
Dead+Wind 225 deg+Ice+Temp	49.76	-20.98	21.22	2136.78	2105.40	0.04
Dead+Wind 240 deg+Ice+Temp	49.76	-25.64	15.05	1517.99	2571.79	-0.65
Dead+Wind 270 deg+Ice+Temp	49.76	-29.53	0.13	19.56	2959.13	-1.86
Dead+Wind 300 deg+Ice+Temp	49.76	-25.51	-14.82	-1483.74	2553.79	-2.57
Dead+Wind 315 deg+Ice+Temp	49.76	-20.79	-21.03	-2108.28	2079.92	-2.67
Dead+Wind 330 deg+Ice+Temp	49.76	-14.65	-25.81	-2589.02	1464.38	-2.59
Dead+Wind 0 deg - Service	39.95	0.07	-12.66	-1287.34	-7.54	-0.52
Dead+Wind 30 deg - Service	39.95	6.30	-11.00	-1119.47	-639.09	-0.13
Dead+Wind 45 deg - Service	39.95	8.88	-9.00	-916.81	-899.74	0.08
Dead+Wind 60 deg - Service	39.95	10.85	-6.39	-651.69	-1098.97	0.29
Dead+Wind 90 deg - Service	39.95	12.49	-0.07	-9.32	-1263.94	0.64
Dead+Wind 120 deg - Service	39.95	10.78	6.27	635.50	-1089.81	0.82
Dead+Wind 135 deg - Service	39.95	8.78	8.90	903.55	-886.79	0.82
Dead+Wind 150 deg - Service	39.95	6.18	10.93	1110.00	-623.22	0.77
Dead+Wind 180 deg - Service	39.95	-0.07	12.66	1287.03	10.79	0.52
Dead+Wind 210 deg - Service	39.95	-6.30	11.00	1119.16	642.34	0.13
Dead+Wind 225 deg - Service	39.95	-8.88	9.00	916.50	902.99	-0.08
Dead+Wind 240 deg - Service	39.95	-10.85	6.39	651.37	1102.22	-0.29
Dead+Wind 270 deg - Service	39.95	-12.49	0.07	9.01	1267.19	-0.64
Dead+Wind 300 deg - Service	39.95	-10.78	-6.27	-635.82	1093.06	-0.82
Dead+Wind 315 deg - Service	39.95	-8.78	-8.90	-903.86	890.04	-0.82
Dead+Wind 330 deg - Service	39.95	-6.18	-10.93	-1110.31	626.47	-0.77

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-39.95	0.00	0.00	39.95	0.00	0.000%
2	0.17	-39.95	-32.40	-0.17	39.95	32.40	0.000%
3	16.13	-39.95	-28.15	-16.13	39.95	28.15	0.000%
4	22.73	-39.95	-23.04	-22.73	39.95	23.04	0.000%
5	27.77	-39.95	-16.35	-27.77	39.95	16.35	0.000%
6	31.96	-39.95	-0.17	-31.96	39.95	0.17	0.000%
7	27.60	-39.95	16.05	-27.60	39.95	-16.05	0.000%
8	22.48	-39.95	22.79	-22.48	39.95	-22.79	0.000%
9	15.83	-39.95	27.98	-15.83	39.95	-27.98	0.000%
10	-0.17	-39.95	32.40	0.17	39.95	-32.40	0.000%
11	-16.13	-39.95	28.15	16.13	39.95	-28.15	0.000%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
12	-22.73	-39.95	23.04	22.73	39.95	-23.04	0.000%
13	-27.77	-39.95	16.35	27.77	39.95	-16.35	0.000%
14	-31.96	-39.95	0.17	31.96	39.95	-0.17	0.000%
15	-27.60	-39.95	-16.05	27.60	39.95	16.05	0.000%
16	-22.48	-39.95	-22.79	22.48	39.95	22.79	0.000%
17	-15.83	-39.95	-27.98	15.83	39.95	27.98	0.000%
18	0.00	-49.76	0.00	0.00	49.76	0.00	0.000%
19	0.13	-49.76	-29.88	-0.13	49.76	29.88	0.000%
20	14.88	-49.76	-25.94	-14.88	49.76	25.94	0.000%
21	20.98	-49.76	-21.22	-20.98	49.76	21.22	0.000%
22	25.64	-49.76	-15.05	-25.64	49.76	15.05	0.000%
23	29.53	-49.76	-0.13	-29.53	49.76	0.13	0.000%
24	25.51	-49.76	14.82	-25.51	49.76	-14.82	0.000%
25	20.79	-49.76	21.03	-20.79	49.76	-21.03	0.000%
26	14.65	-49.76	25.81	-14.65	49.76	-25.81	0.000%
27	-0.13	-49.76	29.88	0.13	49.76	-29.88	0.000%
28	-14.88	-49.76	25.94	14.88	49.76	-25.94	0.000%
29	-20.98	-49.76	21.22	20.98	49.76	-21.22	0.000%
30	-25.64	-49.76	15.05	25.64	49.76	-15.05	0.000%
31	-29.53	-49.76	0.13	29.53	49.76	-0.13	0.000%
32	-25.51	-49.76	-14.82	25.51	49.76	14.82	0.000%
33	-20.79	-49.76	-21.03	20.79	49.76	21.03	0.000%
34	-14.65	-49.76	-25.81	14.65	49.76	25.81	0.000%
35	0.07	-39.95	-12.66	-0.07	39.95	12.66	0.000%
36	6.30	-39.95	-11.00	-6.30	39.95	11.00	0.000%
37	8.88	-39.95	-9.00	-8.88	39.95	9.00	0.000%
38	10.85	-39.95	-6.39	-10.85	39.95	6.39	0.000%
39	12.49	-39.95	-0.07	-12.49	39.95	0.07	0.000%
40	10.78	-39.95	6.27	-10.78	39.95	-6.27	0.000%
41	8.78	-39.95	8.90	-8.78	39.95	-8.90	0.000%
42	6.18	-39.95	10.93	-6.18	39.95	-10.93	0.000%
43	-0.07	-39.95	12.66	0.07	39.95	-12.66	0.000%
44	-6.30	-39.95	11.00	6.30	39.95	-11.00	0.000%
45	-8.88	-39.95	9.00	8.88	39.95	-9.00	0.000%
46	-10.85	-39.95	6.39	10.85	39.95	-6.39	0.000%
47	-12.49	-39.95	0.07	12.49	39.95	-0.07	0.000%
48	-10.78	-39.95	-6.27	10.78	39.95	6.27	0.000%
49	-8.78	-39.95	-8.90	8.78	39.95	8.90	0.000%
50	-6.18	-39.95	-10.93	6.18	39.95	10.93	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	4	0.00000001	0.00070235
3	Yes	5	0.00000001	0.00036679
4	Yes	5	0.00000001	0.00040267
5	Yes	5	0.00000001	0.00035913
6	Yes	4	0.00000001	0.00094193
7	Yes	5	0.00000001	0.00038439
8	Yes	5	0.00000001	0.00039304
9	Yes	5	0.00000001	0.00033459
10	Yes	5	0.00000001	0.00003858
11	Yes	5	0.00000001	0.00037536
12	Yes	5	0.00000001	0.00040447

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13	Yes	5	0.00000001	0.00038125
14	Yes	5	0.00000001	0.00004608
15	Yes	5	0.00000001	0.00033767
16	Yes	5	0.00000001	0.00039970
17	Yes	5	0.00000001	0.00038928
18	Yes	4	0.00000001	0.00000727
19	Yes	5	0.00000001	0.00017772
20	Yes	5	0.00000001	0.00086109
21	Yes	5	0.00000001	0.00097944
22	Yes	5	0.00000001	0.00085689
23	Yes	5	0.00000001	0.00017805
24	Yes	5	0.00000001	0.00089964
25	Yes	5	0.00000001	0.00095756
26	Yes	5	0.00000001	0.00079987
27	Yes	5	0.00000001	0.00018956
28	Yes	5	0.00000001	0.00089457
29	Yes	5	0.00000001	0.00098860
30	Yes	5	0.00000001	0.00089495
31	Yes	5	0.00000001	0.00019012
32	Yes	5	0.00000001	0.00080949
33	Yes	5	0.00000001	0.00097147
34	Yes	5	0.00000001	0.00091329
35	Yes	4	0.00000001	0.00020050
36	Yes	5	0.00000001	0.00004229
37	Yes	5	0.00000001	0.00004924
38	Yes	5	0.00000001	0.00004086
39	Yes	4	0.00000001	0.00025032
40	Yes	5	0.00000001	0.00004727
41	Yes	5	0.00000001	0.00004781
42	Yes	4	0.00000001	0.00097656
43	Yes	4	0.00000001	0.00024104
44	Yes	5	0.00000001	0.00004466
45	Yes	5	0.00000001	0.00005005
46	Yes	5	0.00000001	0.00004599
47	Yes	4	0.00000001	0.00029130
48	Yes	5	0.00000001	0.00003765
49	Yes	5	0.00000001	0.00004992
50	Yes	5	0.00000001	0.00004888

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	140 - 115.36	33.923	44	2.1502	0.0163
L2	118.89 - 79.83	24.532	44	2.0436	0.0081
L3	84.42 - 40.26	11.826	44	1.3861	0.0021
L4	45.54 - 0	3.300	44	0.6774	0.0007

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
148.00	20' x 2" Dia Omni	44	33.923	2.1502	0.0163	18043
138.00	4' Side Mount Standoff	44	33.014	2.1458	0.0154	18043

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Elevation <i>ft</i>	Appurtenance	Gov. Load Comb.	Deflection <i>in</i>	Tilt °	Twist °	Radius of Curvature <i>ft</i>
130.00	800 10121	44	29.398	2.1221	0.0121	9021
120.00	LPA-80063/6CF	44	25.005	2.0551	0.0084	4570
118.00	13' Platform w/rails	44	24.154	2.0336	0.0078	4278
100.00	(2) RR90-17-02DP	44	17.042	1.7259	0.0039	3307
90.00	APXVSPP18-C-A20 w/ Mount	44	13.583	1.5079	0.0026	2943
83.00	APXV18-206517S	44	11.401	1.3559	0.0020	2783

Maximum Tower Deflections - Design Wind

Section No.	Elevation <i>ft</i>	Horz. Deflection <i>in</i>	Gov. Load Comb.	Tilt °	Twist °
L1	140 - 115.36	86.490	11	5.4821	0.0418
L2	118.89 - 79.83	62.591	11	5.2145	0.0207
L3	84.42 - 40.26	30.205	11	3.5407	0.0062
L4	45.54 - 0	8.433	11	1.7313	0.0022

Critical Deflections and Radius of Curvature - Design Wind

Elevation <i>ft</i>	Appurtenance	Gov. Load Comb.	Deflection <i>in</i>	Tilt °	Twist °	Radius of Curvature <i>ft</i>
148.00	20' x 2" Dia Omni	11	86.490	5.4821	0.0418	7357
138.00	4' Side Mount Standoff	11	84.177	5.4714	0.0396	7357
130.00	800 10121	11	74.979	5.4127	0.0310	3677
120.00	LPA-80063/6CF	11	63.797	5.2437	0.0220	1860
118.00	13' Platform w/rails	11	61.630	5.1892	0.0205	1739
100.00	(2) RR90-17-02DP	11	43.509	4.4068	0.0106	1317
90.00	APXVSPP18-C-A20 w/ Mount	11	34.685	3.8512	0.0075	1165
83.00	APXV18-206517S	11	29.119	3.4638	0.0060	1099

Compression Checks

Pole Design Data

Section No.	Elevation <i>ft</i>	Size	L <i>ft</i>	L _a <i>ft</i>	Kl/r	F _a <i>ksi</i>	A <i>in</i> ²	Actual P K	Allow. P _a K	Ratio P P _a
L1	140 - 115.36 (1)	TP24.2521x18x0.1875	24.64	0.00	0.0	39.000	13.7884	-4.88	537.75	0.009
L2	115.36 - 79.83 (2)	TP32.7041x22.9814x0.3125	39.06	0.00	0.0	39.000	30.9952	-15.23	1208.81	0.013
L3	79.83 - 40.26 (3)	TP41.9334x30.9366x0.4375	44.16	0.00	0.0	39.000	55.7964	-24.86	2176.06	0.011
L4	40.26 - 0 (4)	TP51x39.7436x0.5	45.54	0.00	0.0	39.000	80.1435	-39.93	3125.60	0.013

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Pole Bending Design Data

Section No.	Elevation ft	Size	Actual M_x kip-ft	Actual f_{bx} ksi	Allow. F_{bx} ksi	Ratio $\frac{f_{bx}}{F_{bx}}$	Actual M_y kip-ft	Actual f_{by} ksi	Allow. F_{by} ksi	Ratio $\frac{f_{by}}{F_{by}}$
L1	140 - 115.36 (1)	TP24.2521x18x0.1875	129.03	19.646	39.000	0.504	0.00	0.000	39.000	0.000
L2	115.36 - 79.83 (2)	TP32.7041x22.9814x0.3125	838.92	42.211	39.000	1.082	0.00	0.000	39.000	0.000
L3	79.83 - 40.26 (3)	TP41.9334x30.9366x0.4375	1898.40	41.302	39.000	1.059	0.00	0.000	39.000	0.000
L4	40.26 - 0 (4)	TP51x39.7436x0.5	3298.79	39.718	39.000	1.018	0.00	0.000	39.000	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V K	Actual f_v ksi	Allow. F_v ksi	Ratio $\frac{f_v}{F_v}$	Actual T kip-ft	Actual f_{vt} ksi	Allow. F_{vt} ksi	Ratio $\frac{f_{vt}}{F_{vt}}$
L1	140 - 115.36 (1)	TP24.2521x18x0.1875	16.69	1.211	26.000	0.093	0.78	0.058	26.000	0.002
L2	115.36 - 79.83 (2)	TP32.7041x22.9814x0.3125	24.98	0.806	26.000	0.062	0.67	0.016	26.000	0.001
L3	79.83 - 40.26 (3)	TP41.9334x30.9366x0.4375	28.97	0.519	26.000	0.040	0.52	0.005	26.000	0.000
L4	40.26 - 0 (4)	TP51x39.7436x0.5	32.47	0.405	26.000	0.031	0.34	0.002	26.000	0.000

Pole Interaction Design Data

Section No.	Elevation ft	Ratio $\frac{P}{P_a}$	Ratio $\frac{f_{bx}}{F_{bx}}$	Ratio $\frac{f_{by}}{F_{by}}$	Ratio $\frac{f_v}{F_v}$	Ratio $\frac{f_{vt}}{F_{vt}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	140 - 115.36 (1)	0.009	0.504	0.000	0.093	0.002	0.515	1.333	H1-3+VT ✓
L2	115.36 - 79.83 (2)	0.013	1.082	0.000	0.062	0.001	1.096	1.333	H1-3+VT ✓
L3	79.83 - 40.26 (3)	0.011	1.059	0.000	0.040	0.000	1.071	1.333	H1-3+VT ✓
L4	40.26 - 0 (4)	0.013	1.018	0.000	0.031	0.000	1.031	1.333	H1-3+VT ✓

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF* P_{allow} K	% Capacity	Pass Fail
L1	140 - 115.36	Pole	TP24.2521x18x0.1875	1	-4.88	716.82	38.6	Pass

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Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail	
L2	115.36 - 79.83	Pole	TP32.7041x22.9814x0.3125	2	-15.23	1611.34	82.2	Pass	
L3	79.83 - 40.26	Pole	TP41.9334x30.9366x0.4375	3	-24.86	2900.69	80.3	Pass	
L4	40.26 - 0	Pole	TP51x39.7436x0.5	4	-39.93	4166.42	77.4	Pass	
							Summary		
							Pole (L2)	82.2	Pass
							RATING =	82.2	Pass

Anchor Bolt and Base Plate Analysis:**Input Data:**Tower Reactions:

Overturing Moment =	OM := 3299-ft-kips	(Input From tnxTower)
Shear Force =	Shear := 32-kips	(Input From tnxTower)
Axial Force =	Axial := 40-kips	(Input From tnxTower)

Anchor Bolt Data:

Use ASTM A615 Grade 75

Number of Anchor Bolts =	N := 20	(User Input)
Diameter of Bolt Circle =	D_{bc} := 60-in	(User Input)
Bolt "Column" Distance =	l := 3.0-in	(User Input)
Bolt Ultimate Strength =	F_u := 100-ksi	(User Input)
Bolt Yield Strength =	F_y := 75-ksi	(User Input)
Bolt Modulus =	E := 29000-ksi	(User Input)
Diameter of Anchor Bolts =	D := 2.25-in	(User Input)
Threads per Inch =	n := 4.5	(User Input)

Base Plate Data:

Use ASTM A572 Grade 60

Plate Yield Strength =	$F_{y_{bp}}$:= 60-ksi	(User Input)
Base Plate Thickness =	t_{bp} := 2.25-in	(User Input)
Base Plate Diameter =	D_{bp} := 66.0-in	(User Input)
Outer Pole Diameter =	D_{pole} := 51.0-in	(User Input)

Geometric Layout Data:

Distance from Bolts to Centroid of Pole:

Radius of Bolt Circle =: $R_{bc} := \frac{D_{bc}}{2} = 30\text{-in}$

Distance to Bolts = $i := 1.. N$

$$d_i := \begin{cases} \theta \leftarrow 2\pi \cdot \left(\frac{i}{N}\right) \\ d \leftarrow R_{bc} \cdot \sin(\theta) \end{cases}$$

$d_1 = 9.27\text{-in}$	$d_7 = 24.27\text{-in}$
$d_2 = 17.63\text{-in}$	$d_8 = 17.63\text{-in}$
$d_3 = 24.27\text{-in}$	$d_9 = 9.27\text{-in}$
$d_4 = 28.53\text{-in}$	$d_{10} = 0.00\text{-in}$
$d_5 = 30.00\text{-in}$	$d_{11} = -9.27\text{-in}$
$d_6 = 28.53\text{-in}$	etc.

Critical Distances For Bending in Plate:

Outer Pole Radius = $R_{pole} := \frac{D_{pole}}{2} = 25.5\text{-in}$

Moment Arms of Bolts about Neutral Axis = $MA_i := \text{if}(d_i \geq R_{pole}, d_i - R_{pole}, 0\text{in})$

$MA_1 = 0.00\text{-in}$	$MA_7 = 0.00\text{-in}$
$MA_2 = 0.00\text{-in}$	$MA_8 = 0.00\text{-in}$
$MA_3 = 0.00\text{-in}$	$MA_9 = 0.00\text{-in}$
$MA_4 = 3.03\text{-in}$	$MA_{10} = 0.00\text{-in}$
$MA_5 = 4.50\text{-in}$	$MA_{11} = 0.00\text{-in}$
$MA_6 = 3.03\text{-in}$	etc

Effective Width of Baseplate for Bending = $B_{eff} := .8 \cdot 2 \cdot \sqrt{\left(\frac{D_{bp}}{2}\right)^2 - \left(\frac{D_{pole}}{2}\right)^2} = 33.5\text{-in}$

Anchor Bolt Analysis:

Calculated Anchor Bolt Properties:

Polar Moment of Inertia = $I_p := \sum_i (d_i)^2 = 9 \times 10^3 \cdot \text{in}^2$

Gross Area of Bolt = $A_g := \frac{\pi}{4} \cdot D^2 = 3.976 \cdot \text{in}^2$

Net Area of Bolt = $A_n := \frac{\pi}{4} \cdot \left(D - \frac{0.9743 \cdot \text{in}}{n} \right)^2 = 3.248 \cdot \text{in}^2$

Net Diameter = $D_n := \frac{2 \cdot \sqrt{A_n}}{\sqrt{\pi}} = 2.033 \cdot \text{in}$

Radius of Gyration of Bolt = $r := \frac{D_n}{4} = 0.508 \cdot \text{in}$

Section Modulus of Bolt = $S_x := \frac{\pi \cdot D_n^3}{32} = 0.826 \cdot \text{in}^3$

Check Anchor Bolt Tension Force:

Maximum Tensile Force = $T_{\text{Max}} := \text{OM} \cdot \frac{R_{bc}}{I_p} - \frac{\text{Axial}}{N} = 130 \cdot \text{kips}$

Allowable Tensile Force = $T_{\text{ALL.Gross}} := 1.333 \cdot (0.33 \cdot A_g \cdot F_u) = 174.9 \cdot \text{kips}$ (1.333 increase allowed per TIA/EIA)

$T_{\text{ALL.Net}} := 1.333 \cdot (0.60 \cdot A_n \cdot F_y) = 194.812 \cdot \text{kips}$ (1.333 increase allowed per TIA/EIA)

Bolt Tension % of Capacity = $\frac{T_{\text{Max}}}{T_{\text{ALL.Net}}} \cdot 100 = 67$ Bolts are "upset bolts". Use net area per AISC

Condition1 = $\text{Condition1} := \text{if} \left(\frac{T_{\text{Max}}}{T_{\text{ALL.Net}}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$

Condition1 = "OK"

Check Anchor Bolt Bending Stress:

Maximum Bending Moment = $M_x := \left(\frac{\text{Shear}}{N} \right) \cdot l = 0.4 \cdot \text{ft} \cdot \text{kips}$

Maximum Bending Stress = $f_{bx} := \frac{M_x}{S_x} = 5.8 \cdot \text{ksi}$

Allowable Bending Stress = $F_{bx} := 1.333 \cdot 0.6 \cdot F_y = 60 \cdot \text{ksi}$ (1.333 increase allowed per TIA/EIA)

Check Combined Stress Requirement:

Per ASCE Manual 72: "If the clearance between the base plate and concrete does not exceed two times the bolt diameter a bending stress analysis of the bolts is NOT normally required."

$$l := \begin{cases} l & \text{if } l > 2 \cdot D_n = 0 \text{ in} \\ 0 & \text{otherwise} \end{cases}$$

$$f_{bx} := \begin{cases} f_{bx} & \text{if } l > 2 \cdot D_n = 0 \text{ ksi} \\ 0 & \text{otherwise} \end{cases}$$

Check Anchor Bolt Compression/Combined Stress:

Maximum Compressive Force =

$$C_{Max} := OM \cdot \frac{R_{bc}}{I_p} + \frac{Axial}{N} = 134 \text{ kips}$$

Maximum Compressive Stress =

$$f_a := \frac{C_{Max}}{A_n} = 41.2 \text{ ksi}$$

$$K := 0.65$$

$$C_c := \sqrt{\frac{2 \cdot \pi^2 \cdot E}{F_y}} = 87.364$$

$$F_a := \begin{cases} \frac{\left[1 - \frac{\left(\frac{K \cdot l}{r} \right)^2}{2 \cdot C_c^2} \right] \cdot F_y}{\frac{5}{3} + \frac{3 \cdot \left(\frac{K \cdot l}{r} \right)}{8 \cdot C_c} - \frac{\left(\frac{K \cdot l}{r} \right)^3}{8 \cdot C_c^3}} & \text{if } \frac{K \cdot l}{r} \leq C_c = 45 \text{ ksi} \\ \frac{12 \cdot \pi^2 \cdot E}{23 \cdot \left(\frac{K \cdot l}{r} \right)^2} & \text{if } \frac{K \cdot l}{r} > C_c \end{cases}$$

Allowable Compressive Stress =

$$F_a := 1.333 \cdot F_a = 60 \text{ ksi} \quad (1.333 \text{ increase allowed per TIA/EIA})$$

Combined Stress % of Capacity =

$$\left(\frac{f_a}{F_a} + \frac{f_{bx}}{F_{bx}} \right) \cdot 100 = 68.8$$

Condition 2 =

$$\text{Condition2} := \text{if} \left(\frac{f_a}{F_a} + \frac{f_{bx}}{F_{bx}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

Condition2 = "OK"

Base Plate Analysis:

Force from Bolts = $C_i := \frac{OM \cdot d_i}{I_p} + \frac{Axial}{N}$

$C_1 = 42.8 \cdot \text{kips}$

$C_7 = 108.8 \cdot \text{kips}$

$C_2 = 79.6 \cdot \text{kips}$

$C_8 = 79.6 \cdot \text{kips}$

$C_3 = 108.8 \cdot \text{kips}$

$C_9 = 42.8 \cdot \text{kips}$

$C_4 = 127.5 \cdot \text{kips}$

$C_{10} = 2.0 \cdot \text{kips}$

$C_5 = 134.0 \cdot \text{kips}$

$C_{11} = -38.8 \cdot \text{kips}$

$C_6 = 127.5 \cdot \text{kips}$

etc.

Maximum Bending Stress in Plate = $f_{bp} := \sum_i \frac{6 \cdot C_i \cdot MA_i}{(B_{eff} t_{bp})^2} = 48.7 \cdot \text{ksi}$

Allowable Bending Stress in Plate = $F_{bp} := 1.33 \cdot 0.75 \cdot F_{ybp} = 59.9 \cdot \text{ksi}$

Plate Bending Stress % of Capacity = $\frac{f_{bp}}{F_{bp}} \cdot 100 = 81.3$

Condition3 = $\text{Condition3} := \text{if} \left(\frac{f_{bp}}{F_{bp}} < 1.00, "Ok", "Overstressed" \right)$

Condition3 = "Ok"

Standard Monopole Foundation:

Input Data:

Tower Data

Overturning Moment = OM := 3299-ft-kips (User Input from tnxTower)
 Shear Force = Shear := 32-kip (User Input from tnxTower)
 Axial Force = Axial := 40-kip (User Input from tnxTower)
 Tower Height = H_t := 140-ft (User Input)

Footing Data:

Overall Depth of Footing = D_f := 6.5-ft (User Input)
 Length of Pier = L_p := 4.5-ft (User Input)
 Extension of Pier Above Grade = L_{pag} := 0.5-ft (User Input)
 Diameter of Pier = d_p := 6.5-ft (User Input)
 Thickness of Footing = T_f := 3.0-ft (User Input)
 Width of Footing = W_f := 24.0-ft (User Input)

Anchor Bolt Data:

Length of Anchor Bolts = L_{st} := 72-in (User Input)
 Projection of Anchor Bolts Above Pier = A_{BP} := 12.0-in (User Input)
 Anchor Bolt Diameter = d_{anchor} := 2.25-in (User Input)
 Base Plate Bolt Circle = MP := 60.0-in (User Input)

Material Properties:

Concrete Compressive Strength = f_c := 4000-psi (User Input)
 Steel Reinforcement Yield Strength = f_y := 60000-psi (User Input)
 Anchor Bolt Yield Strength = f_{ya} := 75000-psi (User Input)
 Internal Friction Angle of Soil = Φ_s := 30-deg (User Input)
 Allowable Soil Bearing Capacity = q_s := 4000-psf (User Input)
 Unit Weight of Soil = γ_{soil} := 100-pcf (User Input)
 Unit Weight of Concrete = γ_{conc} := 150-pcf (User Input)
 Foundation Bouyancy = Bouyancy := 0 (User Input) (Yes=1 / No=0)
 Depth to Neglect = n := 0-ft (User Input)
 Cohesion of Clay Type Soil = c := 0-ksf (User Input) (Use 0 for Sandy Soil)
 Seismic Zone Factor = Z := 2 (User Input) (UBC-1997 Fig 23-2)
 Coefficient of Friction Between Concrete = μ := 0.45 (User Input)

Pier Reinforcement:

Bar Size =	$BS_{\text{pier}} := 8$	(User Input)	
Bar Diameter =	$d_{\text{bpier}} := 1.0\text{-in}$	(User Input)	
Number of Bars =	$NB_{\text{pier}} := 44$	(User Input)	
Clear Cover of Reinforcement =	$Cvr_{\text{pier}} := 3\text{-in}$	(User Input)	
Reinforcement Location Factor =	$\alpha_{\text{pier}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Coating Factor =	$\beta_{\text{pier}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Concrete Strength Factor =	$\lambda_{\text{pier}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Reinforcement Size Factor =	$\gamma_{\text{pier}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Diameter of Tie =	$d_{\text{Tie}} := 0.5\text{-in}$	(User Input)	

Pad Reinforcement:

Bar Size =	$BS_{\text{top}} := 8$	(User Input)	(Top of Pad)
Bar Diameter =	$d_{\text{btop}} := 1.0\text{-in}$	(User Input)	(Top of Pad)
Number of Bars =	$NB_{\text{top}} := 28$	(User Input)	(Top of Pad)
Bar Size =	$BS_{\text{bot}} := 8$	(User Input)	(Bottom of Pad)
Bar Diameter =	$d_{\text{bbot}} := 1.0\text{-in}$	(User Input)	(Bottom of Pad)
Number of Bars =	$NB_{\text{bot}} := 40$	(User Input)	(Bottom of Pad)
Clear Cover of Reinforcement =	$Cvr_{\text{pad}} := 3.0\text{-in}$	(User Input)	
Reinforcement Location Factor =	$\alpha_{\text{pad}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Coating Factor =	$\beta_{\text{pad}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Concrete Strength Factor =	$\lambda_{\text{pad}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Reinforcement Size Factor =	$\gamma_{\text{pad}} := 1.0$	(User Input)	(ACI-2008 12.2.4)

Calculated Factors:

Pier Reinforcement Bar Area =	$A_{\text{bpier}} := \frac{\pi \cdot d_{\text{bpier}}^2}{4} = 0.785 \cdot \text{in}^2$	
Pad Top Reinforcement Bar Area =	$A_{\text{btop}} := \frac{\pi \cdot d_{\text{btop}}^2}{4} = 0.785 \cdot \text{in}^2$	
Pad Bottom Reinforcement Bar Area =	$A_{\text{bbot}} := \frac{\pi \cdot d_{\text{bbot}}^2}{4} = 0.785 \cdot \text{in}^2$	
Coefficient of Lateral Soil Pressure =	$K_p := \frac{1 + \sin(\Phi_s)}{1 - \sin(\Phi_s)} = 3$	
Load Factor =	$LF := \begin{cases} 1.333 & \text{if } H_t \leq 700\text{-ft} \\ 1.7 & \text{if } H_t \geq 1200\text{-ft} \\ 1.333 + \left(\frac{H_t - 700\text{ft}}{1200\text{ft} - 700\text{ft}} \right) \cdot 0.4 & \text{otherwise} \end{cases}$	= 1.333

Stability of Footing:

Adjusted Concrete Unit Weight = $\gamma_c := \text{if}(\text{Bouyancy} = 1, \gamma_{\text{conc}} - 62.4\text{pcf}, \gamma_{\text{conc}}) = 150\text{-pcf}$

Adjusted Soil Unit Weight = $\gamma_s := \text{if}(\text{Bouyancy} = 1, \gamma_{\text{soil}} - 62.4\text{pcf}, \gamma_{\text{soil}}) = 100\text{-pcf}$

Passive Pressure = $P_{pn} := K_p \cdot \gamma_s \cdot n + c \cdot 2 \cdot \sqrt{K_p} = 0\text{-ksf}$

$P_{pt} := K_p \cdot \gamma_s \cdot (D_f - T_f) + c \cdot 2 \cdot \sqrt{K_p} = 1.05\text{-ksf}$

$P_{top} := \text{if}[n < (D_f - T_f), P_{pt}, P_{pn}] = 1.05\text{-ksf}$

$P_{bot} := K_p \cdot \gamma_s \cdot D_f + c \cdot 2 \cdot \sqrt{K_p} = 1.95\text{-ksf}$

$P_{ave} := \frac{P_{top} + P_{bot}}{2} = 1.5\text{-ksf}$

$T_p := \text{if}[n < (D_f - T_f), T_f, (D_f - n)] = 3$

$A_p := W_f \cdot T_p = 72$

Ultimate Shear = $S_u := P_{ave} \cdot A_p = 108\text{-kip}$

Weight of Concrete Pad = $WT_c := [(W_f^2 \cdot T_f) + d_p^2 \cdot L_p] \cdot \gamma_c = 287.719\text{-kip}$

Weight of Soil Above Footing = $WT_{s1} := \left[\begin{array}{l} (W_f^2 - d_p^2) \cdot \left[(L_p - L_{pag} - n) \text{ if } (L_p - L_{pag} - n) \geq 0 \right. \\ \left. 0 \text{ if } (L_p - L_{pag} - n) \leq 0 \right] \end{array} \right] \cdot \gamma_s = 213.5\text{-kip}$

Weight of Soil Wedge at Back Face = $WT_{s2} := \left(\frac{D_f^2 \cdot \tan(\phi_s)}{2} \cdot W_f \right) \cdot \gamma_s = 29.272\text{-kip}$

Weight of Soil Wedge at back face Corners = $WT_{s3} := 2 \cdot \left[(D_f)^3 \cdot \frac{\tan(\phi_s)}{3} \right] \cdot \gamma_s = 10.57\text{-kips}$

Total Weight = $WT_{tot} := WT_c + WT_{s1} + \text{Axial} = 541.219\text{-kip}$

Resisting Moment = $M_r := (WT_{tot}) \cdot \frac{W_f}{2} + S_u \cdot \frac{T_f}{3} + [(WT_{s2} + WT_{s3}) \cdot \left(W_f + \frac{D_f \cdot \tan(\phi_s)}{3} \right)] = 7609\text{-kip-ft}$

Overturing Moment = $M_{ot} := OM + \text{Shear} \cdot (L_p + T_f) = 3539\text{-kip-ft}$

Factor of Safety Actual = $FS := \frac{M_r}{M_{ot}} = 2.15$

Factor of Safety Required = $FS_{req} := 2$

OverTurning_Moment_Check := $\text{if}(FS \geq FS_{req}, \text{"Okay"}, \text{"No Good"})$

OverTurning_Moment_Check = "Okay"

Shear Capacity in Pier:

Shear Resistance of Pier =

$$S_p := \frac{\mu \cdot W_{T_{tot}}}{FS_{req}} = 121.774 \cdot \text{kips}$$

$$\text{Shear_Check} := \text{if}(S_p > \text{Shear}, \text{"Okay"}, \text{"No Good"})$$

Shear_Check = "Okay"

Bearing Pressure Caused by Footing:

Area of the Mat =

$$A_{mat} := W_f^2 = 576$$

Section Modulus of Mat =

$$S := \frac{W_f^3}{6} = 2304 \cdot \text{ft}^3$$

Maximum Pressure in Mat =

$$P_{max} := \frac{W_{T_{tot}}}{A_{mat}} + \frac{M_{ot}}{S} = 2.476 \cdot \text{ksf}$$

$$\text{Max_Pressure_Check} := \text{if}(P_{max} < q_s, \text{"Okay"}, \text{"No Good"})$$

Max_Pressure_Check = "Okay"

Minimum Pressure in Mat =

$$P_{min} := \frac{W_{T_{tot}}}{A_{mat}} - \frac{M_{ot}}{S} = -0.596 \cdot \text{ksf}$$

$$\text{Min_Pressure_Check} := \text{if}((P_{min} \geq 0) \cdot (P_{min} < q_s), \text{"Okay"}, \text{"No Good"})$$

Min_Pressure_Check = "No Good"

Distance to Resultant of Pressure Distribution =

$$X_p := \frac{P_{max}}{P_{max} - P_{min}} \cdot \frac{1}{3} = 6.447$$

Distance to Kern =

$$X_k := \frac{W_f}{6} = 4$$

Since Resultant Force is Not in Kern, Area to which Pressure is Applied Must be Reduced.

Eccentricity =

$$e := \frac{M_{ot}}{W_{T_{tot}}} = 6.539$$

Adjusted Soil Pressure =

$$P_a := \frac{2 \cdot W_{T_{tot}}}{3 \cdot W_f \left(\frac{W_f}{2} - e \right)} = 2.753 \cdot \text{ksf}$$

$$q_{adj} := \text{if}(P_{min} < 0, P_a \cdot P_{max}) = 2.753 \cdot \text{ksf}$$

$$\text{Pressure_Check} := \text{if}(q_{adj} < q_s, \text{"Okay"}, \text{"No Good"})$$

Pressure_Check = "Okay"

Concrete Bearing Capacity:

Strength Reduction Factor = $\Phi_c := 0.65$ (ACI-2008 9.3.2.2)

Bearing Strength Between Pier and Pad = $P_b := \Phi_c \cdot 0.85 \cdot f_c \cdot \frac{\pi \cdot d_p^2}{4} = 1.056 \times 10^4 \text{ kips}$ (ACI-2008 10.14)

Bearing_Check := if($P_b > \text{LF} \cdot \text{Axial}$, "Okay", "No Good")

Bearing_Check = "Okay"

Shear Strength of Concrete:

Beam Shear: (Critical section located at a distance d from the face of Pier) (ACI 11.3.1.1)

$\Phi_c := 0.85$ (ACI 9.3.2.5)

$d := T_f - C_{vr_{pad}} - d_{bbot} = 32 \text{ in}$

$d_1 := \frac{W_f}{2} - \frac{d_p}{2}$

$d_2 := d_1 - d$

$L := \left(\frac{W_f}{2} - e \right) \cdot 3$

Slope := if($L > W_f$, $\frac{P_{max} - P_{min}}{W_f}$, $\frac{q_{adj}}{L}$)

$V_{req} := \text{LF} \cdot \left[(q_{adj} - \text{Slope} \cdot d_1) + \left(\frac{\text{Slope} \cdot d_1}{2} \right) \right] \cdot W_f \cdot d_1$

$V_{Avail} := \Phi_c \cdot 2 \cdot \sqrt{f_c \cdot \psi} \cdot W_f \cdot d$ (ACI-2008 11.2.1.1)

Beam_Shear_Check := if($V_{req} < V_{Avail}$, "Okay", "No Good")

Beam_Shear_Check = "Okay"

Punching Shear: (Critical Section Located at a distance of d/2 from the face of pier) (ACI 11.11.1.2)

Critical Perimeter of Punching Shear = $b_o := (d_p + d) \cdot \pi = 28.8$

Area Included Inside Perimeter = $A_{bo} := \frac{\pi \cdot (d_p + d)^2}{4} = 66$

Area Outside of Perimeter = $A_{out} := A_{mat} - A_{bo} = 510$

Guess Value =

$$v_u := 1 \text{ksf}$$

(From "Foundation Analysis and design", By Joseph Bowles, Eq. 8-9)

Given

$$d^2 + d_p \cdot d = \frac{W_{T_{tot}}}{\pi \cdot v_u}$$

$$v_u := \text{Find}(v_u) = 7 \cdot \text{ksf}$$

$$V_u := v_u \cdot d \cdot W_f = 451 \cdot \text{kips}$$

Required Shear Strength =

$$V_{req} := LF \cdot V_u = 601.2 \cdot \text{kips}$$

Available Shear Strength =

$$V_{Avail} := \phi_c \cdot 4 \cdot \sqrt{f_c \cdot \text{psi}} \cdot b_o \cdot d = 2377.9 \cdot \text{kip} \quad (\text{ACI-2008 11.11.2.1})$$

$$\text{Punching_Shear_Check} := \text{if}(V_{req} < V_{Avail}, \text{"Okay"}, \text{"No Good"})$$

$$\text{Punching_Shear_Check} = \text{"Okay"}$$

Steel Reinforcement in Pad:

Required Reinforcement for Bending:

Strength Reduction Factor =

$$\phi_m := .90 \quad (\text{ACI-2008 9.3.2.1})$$

$$q_b := q_{adj} - d_1 \cdot \text{Slope} = 1.283 \cdot \text{ksf}$$

Maximum Bending at Face of Pier =

$$M_u := LF \cdot \left[(q_{adj} - q_b) \cdot \frac{d_1^2}{3} + q_b \cdot \frac{d_1^2}{2} \right] \cdot W_f = 2771.3 \cdot \text{kip-ft}$$

$$\beta := \begin{cases} 0.85 & \text{if } 2500 \cdot \text{psi} \leq f_c \leq 4000 \cdot \text{psi} \\ 0.65 & \text{if } f_c > 8000 \cdot \text{psi} \end{cases} = 0.85$$

$$\left[\left[\left[\left[\frac{f_c}{\text{psi}} - 4000 \right] \right] \cdot 0.5 \right] \right] \text{ otherwise} \quad (\text{ACI-2008 10.2.7.3})$$

$$R_n := \frac{M_u}{\phi_m \cdot W_f \cdot d^2} = 125.3 \cdot \text{psi}$$

$$\rho := \frac{0.85 \cdot f_c}{f_y} \left(1 - \sqrt{1 - \frac{2 \cdot R_n}{0.85 \cdot f_c}} \right) = 0.0021$$

$$\rho_{min} := \rho = 0.00213$$

Required Reinforcement for Temperature and Shrinkage:

$$\rho_{sh} := \begin{cases} .0018 & \text{if } f_y \geq 60000 \text{ psi} \\ .0020 & \text{otherwise} \end{cases} \quad (\text{ACI -2008 7.12.2.1})$$

Check Bottom Bars:

$$A_s := \begin{cases} \rho_{min} \cdot W_f \cdot d & \text{if } \rho_{min} > \frac{\rho_{sh}}{2} = 19.613 \text{ in}^2 \\ \rho_{sh} \cdot W_f \cdot \frac{d}{2} & \text{otherwise} \end{cases}$$

$$A_{s_{prov}} := A_{b_{bot}} \cdot NB_{bot} = 31.4 \text{ in}^2$$

$$Pad_Reinforcement_Bot := \text{if}(A_{s_{prov}} > A_s, \text{"Okay"}, \text{"No Good"})$$

Pad_Reinforcement_Bot = "Okay"

Check top Bars:

$$A_s := \rho_{sh} \cdot \left(W_f \cdot \frac{d}{2} \right) = 8.3 \text{ in}^2$$

$$A_{s_{prov}} := A_{b_{top}} \cdot NB_{top} = 22 \text{ in}^2$$

$$Pad_Reinforcement_Top := \text{if}(A_{s_{prov}} > A_s, \text{"Okay"}, \text{"No Good"})$$

Pad_Reinforcement_Top = "Okay"

Development Length Pad Reinforcement:

Bar Spacing =

$$B_{sPad} := \frac{W_f - 2 \cdot C_{vr_{pad}} - NB_{bot} \cdot d_{b_{bot}}}{NB_{bot} - 1} = 6.21 \text{ in}$$

Spacing or Cover Dimension =

$$c := \text{if} \left(C_{vr_{pad}} < \frac{B_{sPad}}{2}, C_{vr_{pad}}, \frac{B_{sPad}}{2} \right) = 3 \text{ in}$$

Transverse Reinforcement Index =

$$k_{tr} := 0 \quad (\text{ACI-2008 12.2.3})$$

$$L_{dbt} := \frac{3 \cdot f_y \cdot \alpha_{pad} \cdot \beta_{pad} \cdot \gamma_{pad} \cdot \lambda_{pad}}{40 \cdot \sqrt{f_c \cdot \text{psi}} \cdot \frac{c + k_{tr}}{d_{b_{bot}}}} \cdot d_{b_{bot}} = 23.7 \text{ in}$$

Minimum Development Length =

$$L_{dbmin} := 12 \text{ in} \quad (\text{ACI-2008 12.2.1})$$

$$L_{dbtCheck} := \text{if}(L_{dbt} \geq L_{dbmin}, \text{"Use L.dbt"}, \text{"Use L.dbmin"})$$

Available Length in Pad =

$$L_{Pad} := \frac{W_f}{2} - \frac{d_p}{2} - C_{vr_{pad}} = 102 \text{ in}$$

$$L_{pad_Check} := \text{if}(L_{Pad} > L_{dbt}, \text{"Okay"}, \text{"No Good"})$$

Lpad_Check = "Okay"

Steel Reinforcement in Pier:

Area of Pier =

$$A_p := \frac{\pi \cdot d_p^2}{4} = 4778.36 \cdot \text{in}^2$$

$$A_{smin} := 0.01 \cdot 0.5 \cdot A_p = 23.89 \cdot \text{in}^2 \quad (\text{ACI-2008 10.8.4 \& 10.9.1})$$

$$A_{sprov} := N_{B_{pier}} \cdot A_{B_{pier}} = 34.56 \cdot \text{in}^2$$

$$\text{Steel_Area_Check} := \text{if}(A_{sprov} > A_{smin}, \text{"Okay"}, \text{"No Good"})$$

Steel_Area_Check = "Okay"

Bar Spacing In Pier =

$$B_{sPier} := \frac{d_p \cdot \pi}{N_{B_{pier}}} - d_{B_{pier}} = 4.569 \cdot \text{in}$$

Diameter of Reinforcement Cage =

$$\text{Diam}_{cage} := d_p - 2 \cdot C_{vr_{pier}} = 72 \cdot \text{in}$$

Maximum Moment in Pier =

$$M_p := \left[OM + \text{Shear} \cdot \left(L_p + \frac{A_{BP}}{2} \right) \right] \cdot LF = 55330.2 \cdot \text{in} \cdot \text{kips}$$

Pier Check evaluated from outside program and results are listed below;

$$(D \ N \ n \ P_u \ M_{Xu}) := \left(d_p \cdot 12 \ N_{B_{pier}} \ B_{S_{pier}} \ \frac{\text{Axial} \cdot 1.333}{\text{kips}} \ \frac{M_p}{\text{in} \cdot \text{kips}} \right)$$

$$(D \ N \ n \ P_u \ M_{Xu}) = (78 \ 44 \ 8 \ 53.32 \ 5.533 \times 10^4)$$

$$(\phi P_n \ \phi M_{xn} \ f_{sp} \ \rho) := (0 \ 0 \ 0 \ 0)$$

$$(\phi P_n \ \phi M_{xn} \ f_{sp} \ \rho) := \phi P'_n (D, N, n, P_u, M_{Xu})^T$$

$$(\phi P_n \ \phi M_{xn} \ f_{sp} \ \rho) = (58.759 \ 6.097 \times 10^4 \ -60 \ 7.274 \times 10^{-3})$$

$$\text{Axial_Load_Check} := \text{if}(\phi P_n \geq P_u, \text{"Okay"}, \text{"No Good"})$$

Axial_Load_Check = "Okay"

$$\text{Bending_Check} := \text{if}(\phi M_{xn} \geq M_{Xu}, \text{"Okay"}, \text{"No Good"})$$

Bending_Check = "Okay"

Development Length Pier Reinforcement:

Available Length in Foundation:

$$L_{\text{pier}} := L_p - C_{\text{vr}}_{\text{pier}} = 51 \cdot \text{in}$$

$$L_{\text{pad}} := T_f - C_{\text{vr}}_{\text{pad}} = 33 \cdot \text{in}$$

Tension:

(ACI-2008 12.2.3)

Spacing or Cover Dimension =

$$c := \text{if} \left(C_{\text{vr}}_{\text{pier}} < \frac{B_{\text{sPier}}}{2}, C_{\text{vr}}_{\text{pier}}, \frac{B_{\text{sPier}}}{2} \right) = 2.285 \cdot \text{in}$$

Transverse Reinforcement =

$$k_{\text{tr}} := 0 \quad \text{(ACI-2008 12.2.3)}$$

$$L_{\text{dbt}} := \frac{3 \cdot f_y \cdot \alpha_{\text{pier}} \cdot \beta_{\text{pier}} \cdot \gamma_{\text{pier}} \cdot \lambda_{\text{pier}}}{40 \cdot \sqrt{f_c \cdot \text{psi}} \cdot \left(\frac{c + k_{\text{tr}}}{d_{\text{bpier}}} \right)} \cdot d_{\text{bpier}} = 31.14 \cdot \text{in}$$

Minimum Development Length =

$$L_{\text{dh}} := \frac{1200 \cdot d_{\text{bpier}}}{\sqrt{\frac{f_c}{\text{psi}}}} \cdot .7 = 13.282 \cdot \text{in} \quad \text{(ACI 12.2.1)}$$

Pier reinforcement bars are standard 90 degree hooks and therefore development in the pad is computed as follows:

$$L_{\text{db}} := \max(L_{\text{dbt}}, L_{\text{dbmin}})$$

$$L_{\text{tension_Check}} := \text{if}(L_{\text{pier}} + L_{\text{pad}} > L_{\text{db}}, \text{"Okay"}, \text{"No Good"})$$

$$L_{\text{tension_Check}} = \text{"Okay"}$$

Compression:

(ACI-2008 12.3.2)

$$L_{\text{dbc1}} := \frac{.02 \cdot d_{\text{bpier}} \cdot f_y}{\sqrt{f_c \cdot \text{psi}}} = 18.974 \cdot \text{in}$$

$$L_{\text{dbmin}} := 0.0003 \cdot \frac{\text{in}^2}{l_b} \cdot (d_{\text{bpier}} \cdot f_y) = 18 \cdot \text{in}$$

$$L_{\text{dbc}} := \text{if}(L_{\text{dbc1}} \geq L_{\text{dbmin}}, L_{\text{dbc1}}, L_{\text{dbmin}}) = 18.974 \cdot \text{in}$$

$$L_{\text{compression_Check}} := \text{if}(L_{\text{pier}} + L_{\text{pad}} > L_{\text{dbc}}, \text{"Okay"}, \text{"No Good"})$$

$$L_{\text{compression_Check}} = \text{"Okay"}$$

Tie Size and Spacing in Column:

Minimum Tie Size = $Tie_{min} := \text{if}(BS_{pier} \leq 10, 3, 4) = 3$

Used #4 Ties

Seismic Factor = $z := \text{if}(Z \leq 2, 1, 0.5) = 1$ (ACI-2008 21.10.5)

$s_{lim1} := 16 \cdot d_{bpier} \cdot z = 16 \cdot \text{in}$

$s_{lim2} := 48 \cdot d_{Tie} \cdot z = 24 \cdot \text{in}$

$s_{lim3} := D_f \cdot z = 78 \cdot \text{in}$

$s_{lim4} := 18 \cdot \text{in}$

Maximum Spacing = $s_{tie} := \min \left(\begin{matrix} s_{lim1} \\ s_{lim2} \\ s_{lim3} \\ s_{lim4} \end{matrix} \right) = 16 \cdot \text{in}$

Number of Ties Required = $n_{tie} := \frac{L_{pier} - 3 \cdot \text{in}}{s_{tie}} + 1 = 4$

Check Anchor Steel Embedment:

Depth Available = $D_{ab} := L_{st} - A_{BP} = 5 \cdot \text{ft}$

Length of Anchor Bolt = $L_{anchor} := \frac{(0.11 \cdot f_{ya}) \cdot \text{in}}{\sqrt{f_c \cdot \text{psi}}} = 10.87 \cdot \text{ft}$

Depth_Check := $\text{if}(D_{ab} \geq L_{anchor}, \text{"Okay"}, \text{"No Good"})$

Depth_Check = "No Good"

Note: Anchor plate is provided

Section 6 - RBS GENERAL INFORMATION - existing

	GSM 1ST RBS	GSM 2ND RBS	UMTS 1ST RBS	UMTS 2ND RBS	UMTS 3RD RBS	UMTS 4TH RBS	UMTS 5TH RBS	UMTS 6TH RBS	LTE 1ST RBS	LTE 2ND RBS
RBS ID:	130339	130340	208875	428439	250407				413942	
CTS COMMON ID:	032D1022	184D1022	CTV1022	CTU0022	CTU1022				CTL01022	
BTA/TID:	184G	184P	184U	184W	184V				184L	
4-DIGIT SITE ID:	1022	1022	1022	0022	1022				01022	
COW OR TOY?:	No	No	No	No	No				No	
CELL SITE TYPE:	SECTORIZED	SECTORIZED	SECTORIZED	SECTORIZED	SECTORIZED				SECTORIZED	
SITE TYPE:	BTS-CONVENTIONAL	BTS-CONVENTIONAL	MACRO-CONVENTIONAL	BTS-CONVENTIONAL	MACRO-CONVENTIONAL				MACRO-CONVENTIONAL	
BTS LOCATION ID:				INTERNAL					INTERNAL	
ORIGINATING CO:	CINGULAR	CINGULAR	CINGULAR	CINGULAR	CINGULAR				CINGULAR	
CELLULAR NETWORK:	GOLD	GOLD	GOLD	GOLD	GOLD				GOLD	
OPS DISTRICT:	NORTH	CT-SOUTH	CT-NORTH	CT-South	CT-NORTH				CT-SOUTH	
RF DISTRICT:	NORTH	MIDDLETOWN	MIDDLETOWN	NPO Triage	MIDDLETOWN				NPO Triage	
OPS ZONE:	NE_CT_N_LCFD_S_CS	NE_CT_N_HRFR_SW_CS	NE_CT_N_HRFR_SW_CS	NE_CT_N_LCFD_S_CS	NE_CT_N_HRFR_SW_CS				NE_CT_N_LCFD_S_CS	
RF ZONE:	BCT04 - LITCHFIELD	BCT04	BCT10	Hotseat	BCT10				Hotseat	
BASE STATION TYPE:	BASE	BASE	BASE	OVERLAY	OVERLAY				BASE	
EQUIPMENT NAME:	CANTON SE	CANTON FD CANTON SPRINGS RD	CANTON SE	CANTON FD-CANTON SPRINGS RD 2ND NODE	CANTON FD-CANTON SPRINGS RD				CANTON SE LTE	
DISASTER PRIORITY:	0	1	1	1	0				0	

Section 6 - RBS GENERAL INFORMATION - final

	GSM 1ST RBS	GSM 2ND RBS	UMTS 1ST RBS	UMTS 2ND RBS	UMTS 3RD RBS	UMTS 4TH RBS	UMTS 5TH RBS	UMTS 6TH RBS	LTE 1ST RBS	LTE 2ND RBS
RBS ID:	130339	130340	208875	428439	250407				413942	
CTS COMMON ID:	032D1022	184D1022	CTV1022	CTU0022	CTU1022				CTL01022	
BTA/TID:	184G	184P	184U	184W	184V				184L	
4-DIGIT SITE ID:	1022	1022	1022	0022	1022				01022	
COW OR TOY?:	No	No	No	No	No				No	
CELL SITE TYPE:	SECTORIZED	SECTORIZED	SECTORIZED	SECTORIZED	SECTORIZED				SECTORIZED	
SITE TYPE:	BTS-CONVENTIONAL	BTS-CONVENTIONAL	MACRO-CONVENTIONAL	BTS-CONVENTIONAL	MACRO-CONVENTIONAL				MACRO-CONVENTIONAL	
BTS LOCATION ID:				INTERNAL					INTERNAL	
ORIGINATING CO:	CINGULAR	CINGULAR	CINGULAR	CINGULAR	CINGULAR				CINGULAR	
CELLULAR NETWORK:	GOLD	GOLD	GOLD	GOLD	GOLD				GOLD	
OPS DISTRICT:	NORTH	CT-South	CT-North	CT-South	CT-North				CT-South	
RF DISTRICT:	NORTH	Middletown	Middletown	NPO Triage	Middletown				NPO Triage	
OPS ZONE:	NE_CT_N_LCFD_S_CS	NE_CT_N_HRFR_SW_CS	NE_CT_N_HRFR_SW_CS	NE_CT_N_LCFD_S_CS	NE_CT_N_HRFR_SW_CS				NE_CT_N_LCFD_S_CS	
RF ZONE:	BCT04 - LITCHFIELD	BCT04	BCT10	Hotseat	BCT10				Hotseat	
BASE STATION TYPE:	BASE	BASE	BASE	OVERLAY	OVERLAY				BASE	
EQUIPMENT NAME:	CANTON SE	CANTON FD CANTON SPRINGS RD	CANTON SE	CANTON FD-CANTON SPRINGS RD 2ND NODE	CANTON FD-CANTON SPRINGS RD				CANTON SE LTE	
DISASTER PRIORITY:	0	1	1	1	0				0	

Section 7 - RBS SPECIFIC INFORMATION - existing

	GSM 1ST RBS	GSM 2ND RBS	UMTS 1ST RBS	UMTS 2ND RBS	UMTS 3RD RBS	UMTS 4TH RBS	UMTS 5TH RBS	UMTS 6TH RBS	LTE 1ST RBS	LTE 2ND RBS
MSC										
BSC/RNC/MME POOL ID	MDTWCTBSC10	MDTWCTBSC10	MDTWCTNCR0R03	MDTWCTNCR0R03	MDTWCTNCR0R03				FF01	
LAC	05020	05020	05994	05994	05994					
RAC										
EQUIPMENT VENDOR	NOKIA	NOKIA	ERICSSON	ERICSSON	ERICSSON				ERICSSON	
EQUIPMENT TYPE	ULTRASITE	ULTRASITE	3206 INDOOR	6601 MAIN UNIT UMTS	3206 INDOOR				6601 INDOOR MU	
LOCATION										
CABINET LOCATION										
MARKET STATE CODE									CT	
AGPS	Yes	Yes	Yes	Yes	Yes				Yes	
NODE B NUMBER									1022	
PARENT NAME	MIDDLETOWN-GSM MTSO-BSC-10	MIDDLETOWN-GSM MTSO-BSC-10	MIDDLETOWN RNC03	MIDDLETOWN RNC03	MIDDLETOWN RNC03				FF01	

Section 7 - RBS SPECIFIC INFORMATION - final

	GSM 1ST RBS	GSM 2ND RBS	UMTS 1ST RBS	UMTS 2ND RBS	UMTS 3RD RBS	UMTS 4TH RBS	UMTS 5TH RBS	UMTS 6TH RBS	LTE 1ST RBS	LTE 2ND RBS
MSC										
BSC/RNC/MME POOL ID	MDTWCTBSC10	MDTWCTBSC10	MDTWCTNCR0R03	MDTWCTNCR0R03	MDTWCTNCR0R03				FF01	
LAC	05020	05020	05994	05994	05994					
RAC										
EQUIPMENT VENDOR	NOKIA	NOKIA	ERICSSON	ERICSSON	ERICSSON				ERICSSON	
EQUIPMENT TYPE	ULTRASITE	ULTRASITE	3206 INDOOR	6601 MAIN UNIT UMTS	3206 INDOOR				6601 INDOOR MU	
LOCATION										
CABINET LOCATION										
MARKET STATE CODE									CT	
AGPS	Yes	Yes	Yes	Yes	Yes				Yes	
NODE B NUMBER									1022	
PARENT NAME	MIDDLETOWN-GSM MTSO-BSC-10	MIDDLETOWN-GSM MTSO-BSC-10	MIDDLETOWN RNC03	MIDDLETOWN RNC03	MIDDLETOWN RNC03					

Section 8 - RBS INDIVIDUAL INFORMATION - existing

	GSM 1ST 850	GSM 1ST 1900	GSM 2ND 850	GSM 2ND 1900	UMTS 1ST 850	UMTS 1ST 1900	UMTS 2ND 850	UMTS 2ND 1900	UMTS 3RD 850	UMTS 3RD 1900	UMTS 4TH 850	UMTS 4TH 1900	UMTS 5TH 850	UMTS 5TH 1900	UMTS 6TH 850	UMTS 6TH 1900	LTE 1ST 700	LTE 1ST 850	LTE 1ST 1900	LTE 1ST AWS	LTE 2ND 700	LTE 2ND 850	LTE 2ND 1900	LTE 2ND AWS
RBS ID:	130339	130340			208875	250407	428439										413942							
CELL ID/BCF:	032D1022	032D1022			CTV1022	CTV1022	CTU0022										CTL01022							
CTS COMMON ID:	032D1022	184D1022			CTV1022	CTU1022	CTU0022										CTL01022							

Section 8 - RBS INDIVIDUAL INFORMATION - final

	GSM 1ST 850	GSM 1ST 1900	GSM 2ND 850	GSM 2ND 1900	UMTS 1ST 850	UMTS 1ST 1900	UMTS 2ND 850	UMTS 2ND 1900	UMTS 3RD 850	UMTS 3RD 1900	UMTS 4TH 850	UMTS 4TH 1900	UMTS 5TH 850	UMTS 5TH 1900	UMTS 6TH 850	UMTS 6TH 1900	LTE 1ST 700	LTE 1ST 850	LTE 1ST 1900	LTE 1ST AWS	LTE 2ND 700	LTE 2ND 850	LTE 2ND 1900	LTE 2ND AWS
RBS ID:	130339	130340			208875	250407	428439										413942		413942					
CELL ID/BCF:	032D1022	032D1022			CTV1022	CTV1022	CTU0022										CTL01022		CTL01022					
CTS COMMON ID:	032D1022	184D1022			CTV1022	CTU1022	CTU0022										CTL01022		CTL01022					

Section 15A - CURRENT SECTOR/CELL INFORMATION - SECTOR A (OR OMNI)

ANTENNA COMMON FIELDS	ANTENNA POSITION 1	ANTENNA POSITION 2	ANTENNA POSITION 3	ANTENNA POSITION 4	ANTENNA POSITION 5	ANTENNA POSITION 6	ANTENNA POSITION 7
ANTENNA MAKE - MODEL	800 10121 @850		AM-X-CD-14-65-00T-RET	800 10121 @850			
ANTENNA VENDOR	KATHREIN		KMW	KATHREIN			
ANTENNA SIZE (H x W x D)			48X11.8X5.9				
ANTENNA WEIGHT			36.4				
AZIMUTH	143		23	143			
MAGNETIC DECLINATION							
RADIATION CENTER (feet)	132.03		130.03	132.03			
ANTENNA TIP HEIGHT							
MECHANICAL DOWNTILT	0		0	0			
FEEDER AMOUNT							
Antenna RET Motor (QTY/MODEL)							
SURGE ARRESTOR (QTY/MODEL)							
DIPLEXER (QTY/MODEL)							
DUPLEXER (QTY/MODEL)							
Antenna RET CONTROL UNIT (QTY/MODEL)							
DC BLOCK (QTY/MODEL)							
TMA/LNA (QTY/MODEL)	Pwav LGP21401 Single 1900 w/ 850BP (850)			ADC CG1900W850BP Single 1900 w/ 850BP (850)			
CURRENT INJECTORS FOR TMA (QTY/MODEL)							
PDU FOR TMAS (QTY/MODEL)							
FILTER (QTY/MODEL)							
RRH - 700 band (QTY/MODEL)							
RRH - 850 band (QTY/MODEL)							
RRH - 1900 band (QTY/MODEL)							
RRH - AWS band (QTY/MODEL)							
RRH - WCS band (QTY/MODEL)							
Additional RRH #1 - any band (QTY/MODEL)							
Additional RRH #2 - any band (QTY/MODEL)							
Additional Component1 (QTY/MODEL)							
Additional Component2 (QTY/MODEL)							
Additional Component3 (QTY/MODEL)							
Local Market Note1							
Local Market Note2							
Local Market Note3							

PORT SPECIFIC FIELDS	PORT NUMBER	USEID (CSSng)	USEID (Atoll)	ATOLL TXID	TX/RX?	TECHNOLOGY/FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	Feeder Length (feet)	RXAIT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPA/MCPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Cable Number	Cable ID
ANTENNA POSITION 1	PORT 1		25969.A.850.3G.1	CTV10221		UMTS 850	800 10121 @850_Xpol_8dt	13.41		8	0	RFS 7/8 (850)	160.04	0			NO				
	PORT 3		25969.A.1900.3G.1	CTU10227		UMTS 1900	800 10121 @1950_Xpol_0dt	17		0	0	RFS 7/8 (1900)	160.04	0			NO				
	PORT 4		25969.A.1900.3G.1	CTU10227		UMTS 1900	800 10121 @1950_Xpol_0dt	17		0	0	RFS 7/8 (1900)	160.04	0			NO				
ANTENNA POSITION 3	PORT 1		25969.A.700.4G.1	CTL01022_7A_1		LTE 700	AM-X-CD-14-65-00T-RET_725MHz_03DT	14.1		3	-1	FIBER	0.00	0							
ANTENNA POSITION 4	PORT 1		25969.A.850.25G.1	184G10221		GSM 850	800 10121 @850_Xpol_8dt	13.26		8	0	7/8 at 850 MHz	160.04	0			NO	17.78	183.23		
	PORT 3		25969.A.1900.25G.3	184P10224		GSM 1900	800 10121 @1950_Xpol_0dt	17.04		0	0	7/8 at 1900 MHz	160.04	0			NO	17.78	320.62		

Section 15B - CURRENT SECTOR/CELL INFORMATION - SECTOR B

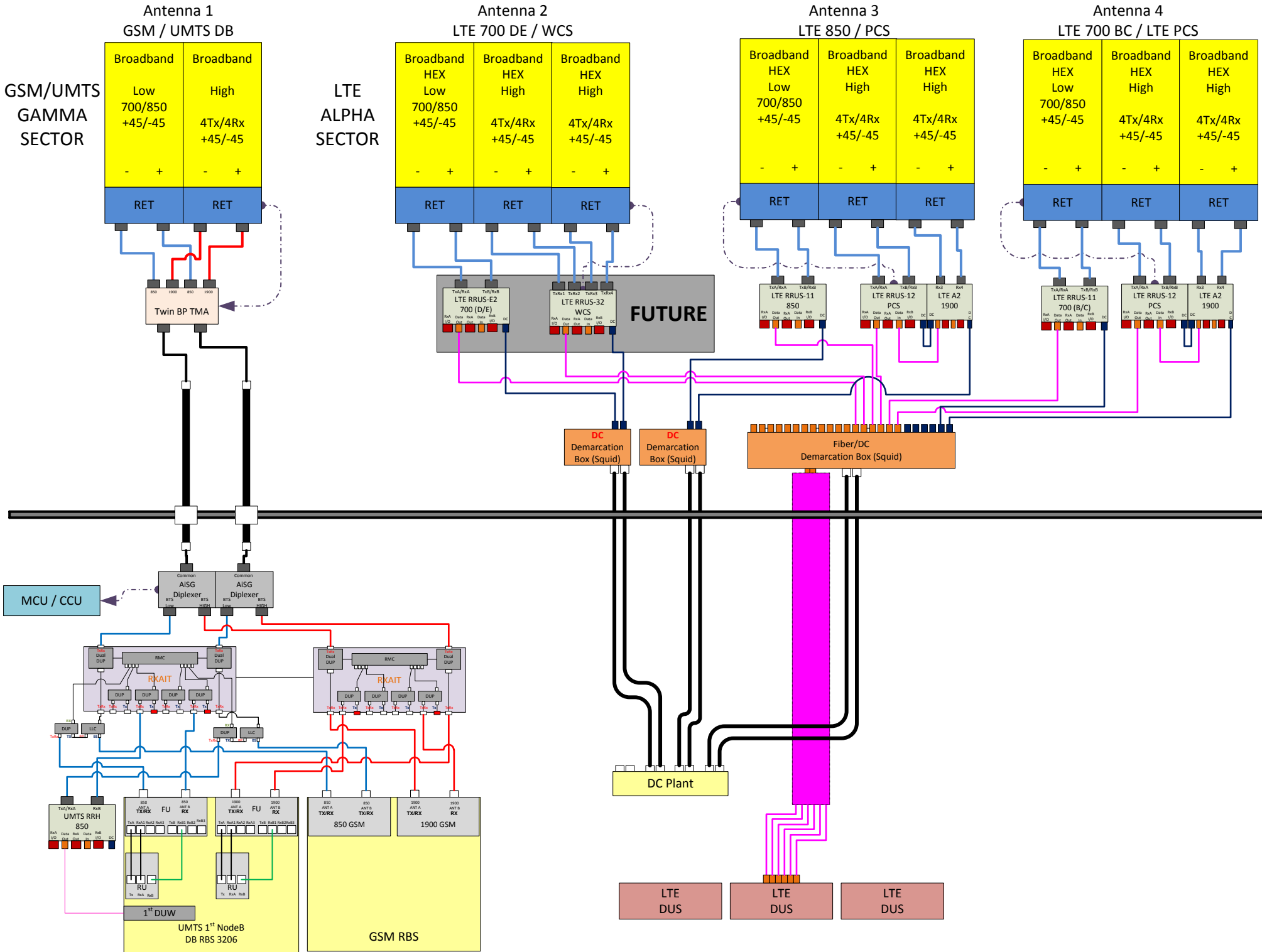
ANTENNA COMMON FIELDS	ANTENNA POSITION 1	ANTENNA POSITION 2	ANTENNA POSITION 3	ANTENNA POSITION 4	ANTENNA POSITION 5	ANTENNA POSITION 6	ANTENNA POSITION 7
ANTENNA MAKE - MODEL	800 10121 @850		P65-17-XLH-RR	800 10121 @850			
ANTENNA VENDOR	KATHREIN		Powerwave	KATHREIN			
ANTENNA SIZE (H x W x D)			96X12X6				
ANTENNA WEIGHT			70				
AZIMUTH	263		143	263			
MAGNETIC DECLINATION							
RADIATION CENTER (feet)	132.03		130.03	132.03			
ANTENNA TIP HEIGHT							
MECHANICAL DOWNTILT	0		0	0			
FEEDER AMOUNT							
Antenna RET Motor (QTY/MODEL)							
SURGE ARRESTOR (QTY/MODEL)							
DIPLEXER (QTY/MODEL)							
DUPLEXER (QTY/MODEL)							
Antenna RET CONTROL UNIT (QTY/MODEL)							
DC BLOCK (QTY/MODEL)							
TMA/LNA (QTY/MODEL)	Pwav LGP21401 Single 1900 w/ 850BP (850)			ADC CG1900W850BP Single 1900 w/ 850BP (850)			
CURRENT INJECTORS FOR TMA (QTY/MODEL)							
PDU FOR TMAS (QTY/MODEL)							
FILTER (QTY/MODEL)							
RRH - 700 band (QTY/MODEL)							
RRH - 850 band (QTY/MODEL)							
RRH - 1900 band (QTY/MODEL)							
RRH - AWS band (QTY/MODEL)							
RRH - WCS band (QTY/MODEL)							
Additional RRH #1 - any band (QTY/MODEL)							
Additional RRH #2 - any band (QTY/MODEL)							
Additional Component1 (QTY/MODEL)							
Additional Component2 (QTY/MODEL)							
Additional Component3 (QTY/MODEL)							
Local Market Note1							
Local Market Note2							
Local Market Note3							

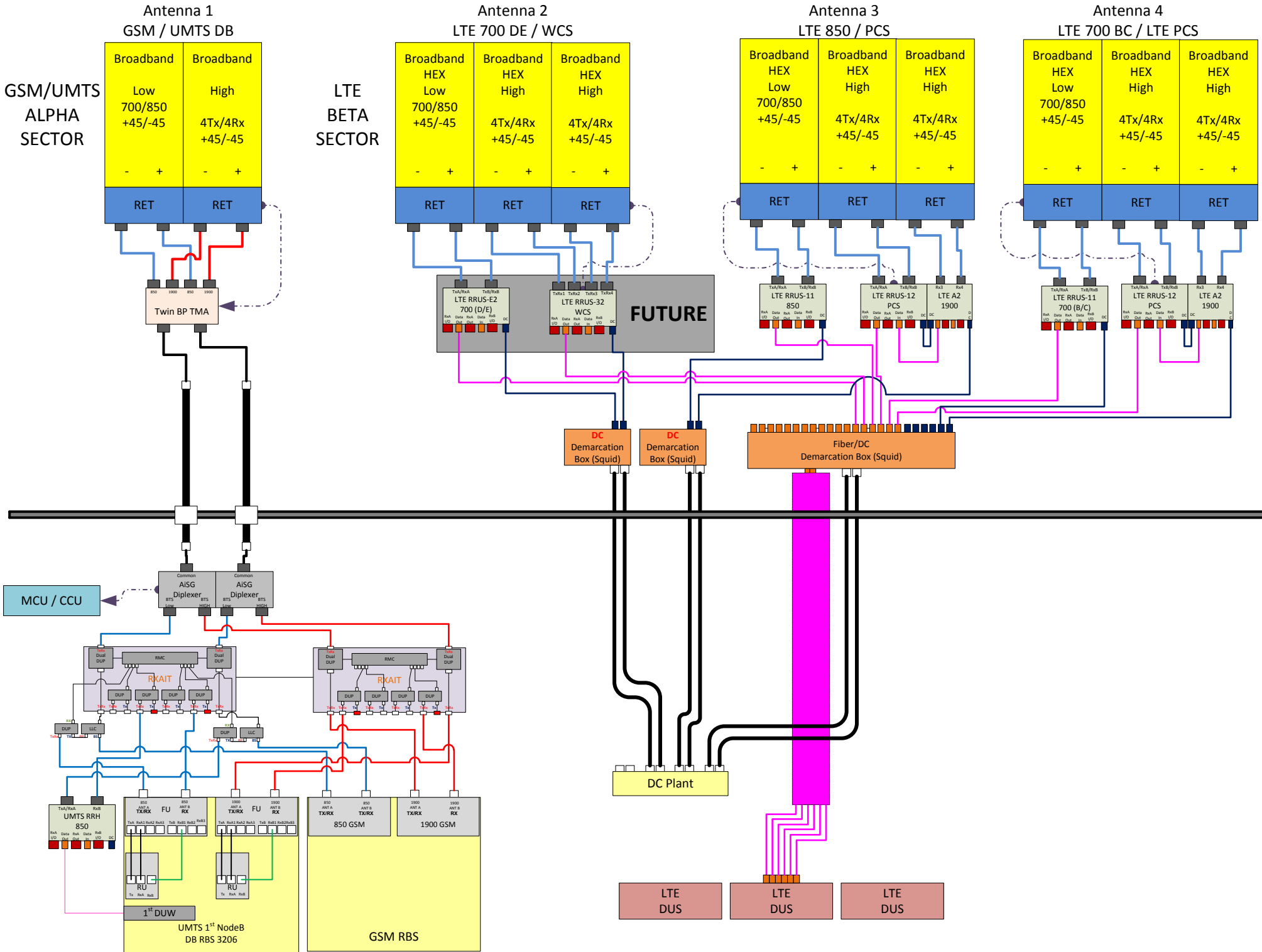
PORT SPECIFIC FIELDS	PORT NUMBER	USEID (CSSng)	USEID (Atoll)	ATOLL TXID	TX/RX?	TECHNOLOGY/FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	Feeder Length (feet)	RXAIT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPA/MCPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Cable Number	Cable ID
ANTENNA POSITION 1	PORT 1	25969.B.850.3G.1	CTV10222			UMTS 850	800 10121 @850_Xpol_8dt	13.41		8	0	RFS 7/8 (850)	160.04	0			NO				
	PORT 3	25969.B.1900.3G.1	CTU10228			UMTS 1900	800 10121 @1950_Xpol_0dt	17		0	0	RFS 7/8 (1900)	160.04	0			NO				
	PORT 4	25969.B.1900.3G.1	CTU10228			UMTS 1900	800 10121 @1950_Xpol_0dt	17		0	0	RFS 7/8 (1900)	160.04	0			NO				
ANTENNA POSITION 3	PORT 1	25969.B.700.4G.1	CTL01022_7B_1			LTE 700	P65-17-XLH-RR_716MHz_00DT	16.39		0	-1	FIBER	0.00	0							
ANTENNA POSITION 4	PORT 1	25969.B.850.25G.1	184G10222			GSM 850	800 10121 @850_Xpol_8dt	13.26		8	0	7/8 at 850 MHz	160.04	0			NO	28.18	290.4		
	PORT 3	25969.B.1900.25G.3	184P10225			GSM 1900	800 10121 @1950_Xpol_0dt	17.04		0	0	7/8 at 1900 MHz	160.04	0			NO	17.78	320.62		

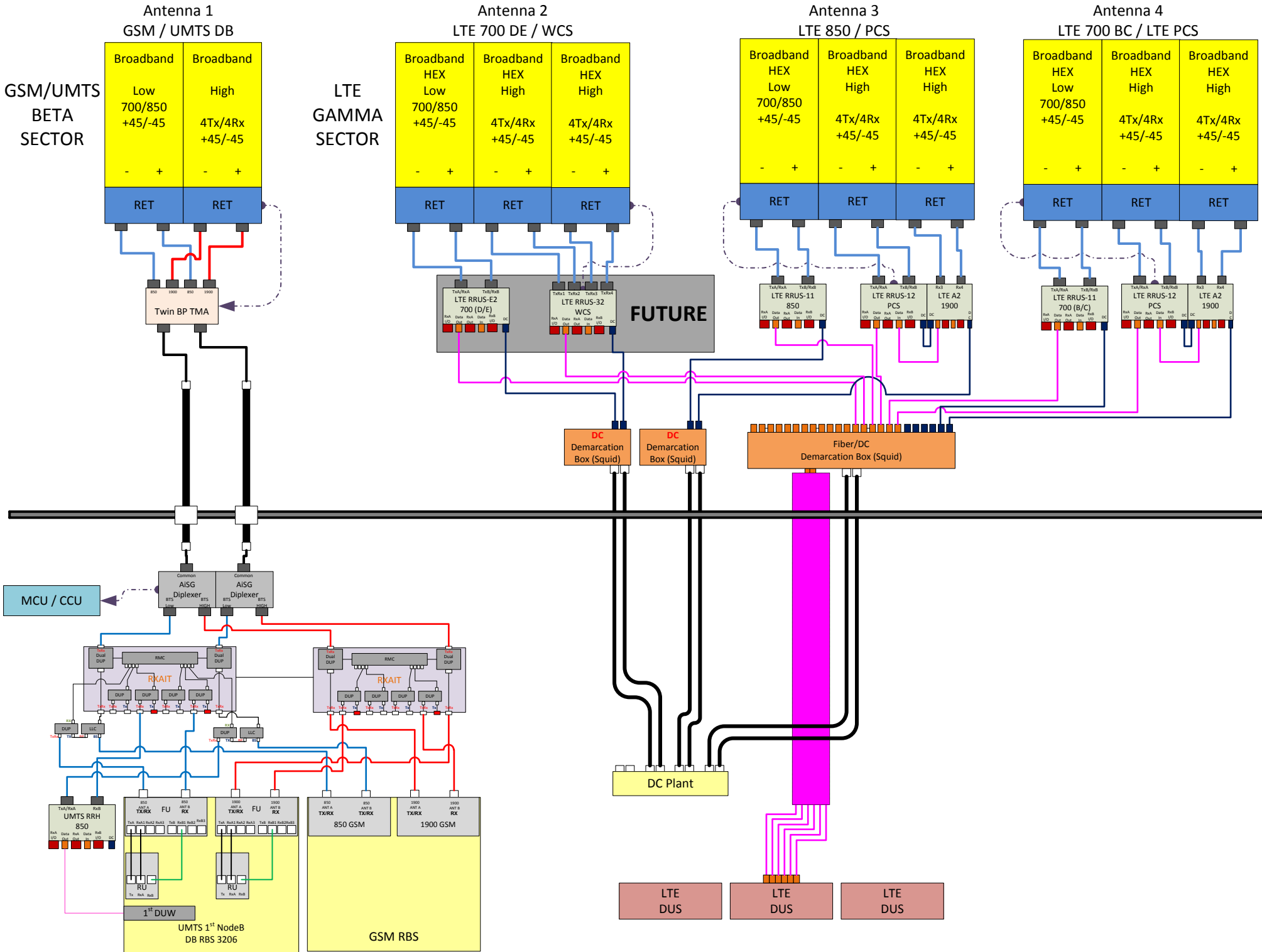
Section 15C - CURRENT SECTOR/CELL INFORMATION - SECTOR C

ANTENNA COMMON FIELDS	ANTENNA POSITION 1	ANTENNA POSITION 2	ANTENNA POSITION 3	ANTENNA POSITION 4	ANTENNA POSITION 5	ANTENNA POSITION 6	ANTENNA POSITION 7
ANTENNA MAKE - MODEL	800 10121 @850		SBNH-1D6565C	800 10121 @850			
ANTENNA VENDOR	KATHREIN		Andrew	KATHREIN			
ANTENNA SIZE (H x W x D)			96.4X11.9X7.1				
ANTENNA WEIGHT			66.1				
AZIMUTH	23		263	23			
MAGNETIC DECLINATION							
RADIATION CENTER (feet)	132.03		130.03	132.03			
ANTENNA TIP HEIGHT							
MECHANICAL DOWNTILT	0		0	0			
FEEDER AMOUNT							
Antenna RET Motor (QTY/MODEL)							
SURGE ARRESTOR (QTY/MODEL)							
DIPLEXER (QTY/MODEL)							
DUPLEXER (QTY/MODEL)							
Antenna RET CONTROL UNIT (QTY/MODEL)							
DC BLOCK (QTY/MODEL)							
TMA/LNA (QTY/MODEL)	Pwav LGP21401 Single 1900 w/ 850BP (850)			ADC CG1900W850BP Single 1900 w/ 850BP (850)			
CURRENT INJECTORS FOR TMA (QTY/MODEL)							
PDU FOR TMAS (QTY/MODEL)							
FILTER (QTY/MODEL)							
RRH - 700 band (QTY/MODEL)							
RRH - 850 band (QTY/MODEL)							
RRH - 1900 band (QTY/MODEL)							
RRH - AWS band (QTY/MODEL)							
RRH - WCS band (QTY/MODEL)							
Additional RRH #1 - any band (QTY/MODEL)							
Additional RRH #2 - any band (QTY/MODEL)							
Additional Component1 (QTY/MODEL)							
Additional Component2 (QTY/MODEL)							
Additional Component3 (QTY/MODEL)							
Local Market Note1							
Local Market Note2							
Local Market Note3							

PORT SPECIFIC FIELDS	PORT NUMBER	USEID (CSSng)	USEID (Atoll)	ATOLL TXID	TX/RX?	TECHNOLOGY/FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	Feeder Length (feet)	RXAIT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPA/MCPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Cable Number	Cable ID
ANTENNA POSITION 1	PORT 1		25969.C.850.3G.1	CTV10223		UMTS 850	800 10121 @850_Xpol_6dt	13.42		6	0	RFS 7/8 (850)	160.04	0			NO				
	PORT 3		25969.C.1900.3G.1	CTU10229		UMTS 1900	800 10121 @1950_Xpol_6dt	16.6		6	0	RFS 7/8 (1900)	160.04	0			NO				
	PORT 4		25969.C.1900.3G.1	CTU10229		UMTS 1900	800 10121 @1950_Xpol_6dt	16.6		6	0	RFS 7/8 (1900)	160.04	0			NO				
ANTENNA POSITION 3	PORT 1		25969.C.700.4G.1	CTL01022_7C_1		LTE 700	SBNH-1D6565C_725MHz_03DT	15.6		3	-1	FIBER	0.00	0							
ANTENNA POSITION 4	PORT 1		25969.C.850.25G.1	184G10223		GSM 850	800 10121 @850_Xpol_6dt	13.26		6	0	7/8 at 850 MHz	160.04	0			NO	28.18	290.4		
	PORT 3		25969.C.1900.25G.3	184P10226		GSM 1900	800 10121 @1950_Xpol_6dt	16.63		6	0	7/8 at 1900 MHz	160.04	0			NO	17.78	291.74		







HexPORT Multi-Band ANTENNA

Model HPA-65R-BUU-H8



The CCI Hexport Multi-Band Antenna Array is an industry first 6-port antenna with full WCS Band Coverage. With four high band ports and two low band ports, our hexport antenna is ready for 4X4 high band MIMO.

Modern networks demand high performance, consequently CCI has incorporated several new and innovative design techniques to provide an antenna with excellent side-lobe performance, sharp elevation beams, and high front to back ratio.

Multiple networks can now be connected to a single antenna, reducing tower loading and leasing expense, while decreasing deployment time and installation cost.

Full band capability for 700 MHz , Cellular 850 MHz, PCS 1900 MHz, AWS 1710/2170 MHz and WCS 2300 MHz coverage in a single enclosure.

Hexport Multi-Band Antenna Array

Benefits

- ◆ Includes WCS Band
- ◆ Reduces tower loading
- ◆ Frees up space for tower mounted E-nodes
- ◆ Single radome with six ports
- ◆ All Band design simplifies radio assignments
- ◆ Sharp elevation beam eases network planning

Features

- ◆ High Band Ports include WCS Band
- ◆ Four High Band ports with two Low Band ports in one antenna
- ◆ Sharp elevation beam
- ◆ Excellent elevation side-lobe performance
- ◆ Excellent MIMO performance due to array spacing
- ◆ Excellent PIM Performance
- ◆ A multi-network solution in one radome

Applications

- ◆ 4x4 MIMO on High Band and 2x2 MIMO on Low Band
- ◆ Adding additional capacity without adding additional antennas
- ◆ Adding WCS Band without increasing antenna count



HexPORT Multi-Band ANTENNA

Model HPA-65R-BUU-H8

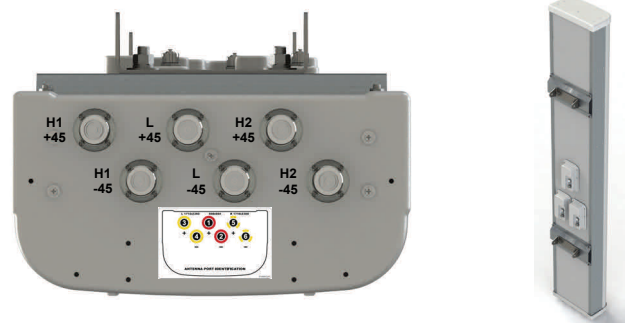
HPA-65R Multi-Band Antenna

Electrical Specifications

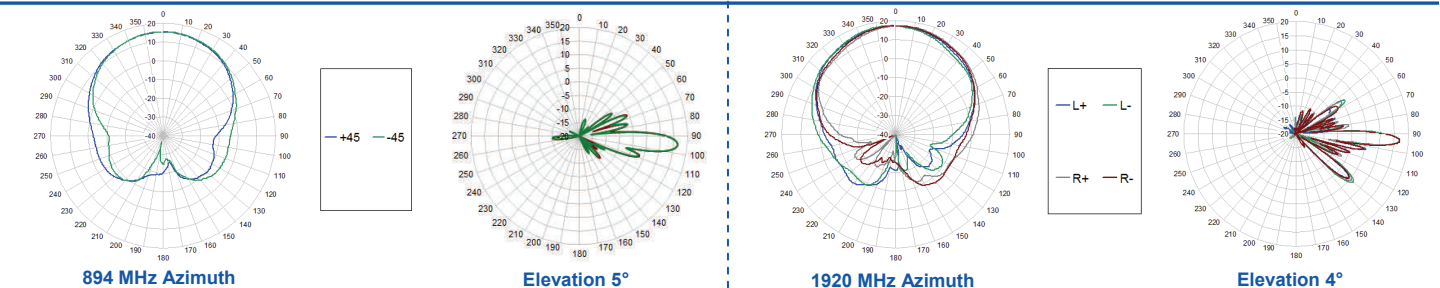
Frequency Range	2 X Low Band Ports which cover the full range from 698-894 MHz		4 X High Band Ports which cover the full range from 1710-2360 MHz			
	698-806 MHz	824-894 MHz	1850-1990 MHz	1710-1755/2110-2170 MHz	2305-2360 MHz	
Gain	15.3 dBi	16.2 dBi	17.1 dBi	16.3 dBi	17.4 dBi	17.7 dBi
Azimuth Beamwidth (-3dB)	65°	61°	62°	68°	64°	60°
Elevation Beamwidth (-3dB)	10.1°	8.4°	5.6°	6.2°	5.0°	4.5°
Electrical Downtilt	2° to 10°	2° to 10°	0° to 8°	0° to 8°	0° to 8°	0° to 8°
Elevation Sidelobes (1st Upper)	< -17 dB	< -17 dB	< -19 dB	< -18 dB	< -18 dB	< -17 dB
Front-to-Back Ratio @180°	> 29 dB	> 28 dB	> 35 dB	> 35 dB	> 35 dB	> 35 dB
Front-to-Back Ratio over ± 20°	> 28 dB	> 27 dB	> 28 dB	> 27 dB	> 28 dB	> 28 dB
Cross-Polar Discrimination (at Peak)	> 24 dB	> 20 dB	> 25 dB	> 25 dB	> 25 dB	> 25 dB
Cross-Polar Discrimination (at ± 60°)	> 16 dB	> 14 dB	> 18 dB	> 18 dB	> 18 dB	> 18 dB
Cross-Polar Port-to-Port Isolation	> 25 dB	> 25 dB	> 25 dB	> 25 dB	> 25 dB	> 25 dB
VSWR	< 1.5:1	< 1.5:1	< 1.5:1	< 1.5:1	< 1.5:1	< 1.5:1
Passive Intermodulation (2x20W)	≤ -150dBc	≤ -150dBc	≤ -150dBc	≤ -150dBc	≤ -150dBc	≤ -150dBc
Input Power	500 Watts CW	500 Watts CW	300 Watts CW	300 Watts CW	300 Watts CW	300 Watts CW
Polarization	Dual Pol 45°	Dual Pol 45°	Dual Pol 45°	Dual Pol 45°	Dual Pol 45°	Dual Pol 45°
Input Impedance	50 Ohms	50 Ohms	50 Ohms	50 Ohms	50 Ohms	50 Ohms
Lightning Protection	DC Ground	DC Ground	DC Ground	DC Ground	DC Ground	DC Ground

Mechanical Specifications

Dimensions (LxWxD)	92.4 x 14.8 x 7.4 inches (2348 x 376 x 189 mm)
Survival Wind Speed	> 150 mph
Front Wind Load	332 lbs (1479 N) @ 100 mph (161 kph)
Side Wind Load	193 lbs (860 N) @ 100 mph (161 kph)
Equivalent Flat Plate Area	13.0 ft ² (1.2 m ²)
Weight (without Mounting)	68 lbs (31 kg)
RET System Weight	5.0 lbs (2.25 kg)
Connector	6; 7-16 DIN female long neck
Mounting Pole	2-5 inches (5-12 cm)



Antenna Patterns*



*Typical antenna patterns. For detail information on antenna pattern, please contact us at info@cciproducts.com. All specifications are subject to change without notice.



SBNHH-1D65A

Andrew® Tri-band Antenna, 1 x 698–896 MHz and 2 x 1710–2360 MHz, 65° horizontal beamwidth, internal RET. Both high bands share the same electrical tilt.

- Interleaved dipole technology providing for attractive, low wind load mechanical package
- The values presented on this datasheet have been calculated based on N-P-BASTA White Paper version 9.6 by the NGMN Alliance

Electrical Specifications

Frequency Band, MHz	698–806	806–896	1710–1880	1850–1990	1920–2180	2300–2360
Gain by all Beam Tilts, average, dBi	13.1	13.1	16.1	16.5	16.7	17.2
Gain by all Beam Tilts Tolerance, dB	±0.5	±0.5	±0.5	±0.3	±0.5	±0.4
Gain by Beam Tilt, average, dBi	0° 13.4 9° 13.1 18° 12.7	0° 13.4 9° 13.1 18° 12.7	0° 16.0 5° 16.2 10° 16.1	0° 16.3 5° 16.5 10° 16.5	0° 16.5 5° 16.8 10° 16.6	0° 17.0 5° 17.3 10° 16.9
Beamwidth, Horizontal, degrees	66	61	70	65	62	61
Beamwidth, Horizontal Tolerance, degrees	±3.1	±5.4	±2.8	±4	±6.6	±4.6
Beamwidth, Vertical, degrees	17.6	15.9	7.1	6.6	6.2	5.5
Beamwidth, Vertical Tolerance, degrees	±1.8	±1.4	±0.3	±0.4	±0.5	±0.3
Beam Tilt, degrees	0–18	0–18	0–10	0–10	0–10	0–10
USLS, dB	15	14	15	15	15	14
Front-to-Back Total Power at 180° ± 30°, dB	22	21	26	26	24	25
CPR at Boresight, dB	22	16	22	25	21	22
CPR at Sector, dB	10	6	12	8	5	4
Isolation, dB	25	25	25	25	25	25
Isolation, Intersystem, dB	30	30	30	30	30	30
VSWR Return Loss, dB	1.5 14.0	1.5 14.0	1.5 14.0	1.5 14.0	1.5 14.0	1.5 14.0
PIM, 3rd Order, 2 x 20 W, dBc	-153	-153	-153	-153	-153	-153
Input Power per Port, maximum, watts	350	350	350	350	350	300
Polarization	±45°	±45°	±45°	±45°	±45°	±45°
Impedance	50 ohm	50 ohm	50 ohm	50 ohm	50 ohm	50 ohm

General Specifications

Antenna Brand	Andrew®
Antenna Type	DualPol® tri-band
Band	Multiband
Brand	DualPol® Teletilt®
Operating Frequency Band	1710 – 2360 MHz 698 – 896 MHz

Mechanical Specifications

Color	Light gray
Lightning Protection	dc Ground
Radiator Material	Aluminum Low loss circuit board
Radome Material	Fiberglass, UV resistant
RF Connector Interface	7-16 DIN Female
RF Connector Location	Bottom
RF Connector Quantity, total	6
Wind Loading, maximum	445.0 N @ 150 km/h

SBNHH-1D65A



Wind Speed, maximum 100.0 lbf @ 150 km/h
241.4 km/h | 150.0 mph

Dimensions

Depth 180.0 mm | 7.1 in
Length 1398.0 mm | 55.0 in
Width 301.0 mm | 11.9 in
Net Weight 15.2 kg | 33.5 lb

Remote Electrical Tilt (RET) Information

Annual Failure Rate, maximum 0.01%
Input Voltage 10–30 Vdc
Power Consumption, idle state, maximum 2.0 W
Power Consumption, normal conditions, maximum 11.0 W
Protocol 3GPP/AISG 2.0 Multi-RET
RET Interface RS-485 Female (daisy chain port ,1) | RS-485 Male (input port, 1)
RET Interface, quantity 1 female | 1 male
RET System Teletilt®

Regulatory Compliance/Certifications

Agency	Classification
RoHS 2011/65/EU	Compliant by Exemption
China RoHS SJ/T 11364-2006	Above Maximum Concentration Value (MCV)
ISO 9001:2008	Designed, manufactured and/or distributed under this quality management system



Included Products

BSAMNT-1 — Wide Profile Antenna Downtilt Mounting Kit for 2.5 - 4.5 in (64 115 mm) OD round members. Kit contains one scissor top bracket set and one bottom bracket set.

RRUS 11

Frequency (AT&T)

- ✓ Band 12 (Lower 700 MHz)
- ✓ Band 4 (AWS, 17/2100 MHz) — 2Q2011

RF Characteristics

- ✓ Output power: 2x30 Watts
- ✓ 2x2 MIMO Capable
- ✓ IBW of 20 MHz
- ✓ Rx Sens.: Better than -105 dBm (5 MHz)

RET/TMA Support

- ✓ AISG 2.0 Compatible
- ✓ Via RET Port and Centre Conductor
- ✓ Cascading
- ✓ 30 VDC Bias

Environmental

- ✓ Self Convection
- ✓ Temperature -40 to 131 F

Power

- ✓ Input voltage: -48 VDC or AC (exemption)
- ✓ Fuse size: 13 – 32 A
 - Recommended: 25 A
- ✓ Power Consumption:
 - Typical 200 Watts
 - Max 310 Watts
 - Excl. RET and TMA load



RRUS 11 Mechanics

Wall and pole mounting brackets

- Reused from RRUW and RRU22
- Vertical Mount Only

Clearing distances:

- Above ≥ 16 in.
- Below ≥ 12 in.
- Side ≥ 0 mm

DC connector

- Bayonet
- Screw terminals in connector plug
- Supported outer cable diameter: 6-18 mm

CPRI connector

- LCD with proprietary cover
- Separate cover available from 1Q2011

Size & Weight

- Band 4: 44 lbs
- Band 12: 50 lbs
- 17.8" x 17.3" x 7.2" incl. sun shield



POWER

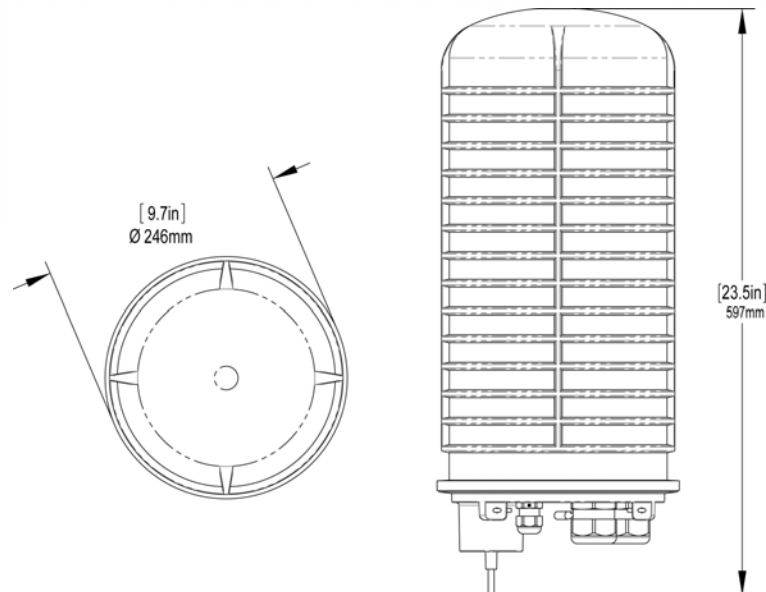
DC6-48-60-18-8F

DC Surge Suppression Solution

The DC6-48-60-18 is a dual chambered, DC surge suppression system for use in multi-circuit, Distributed Antenna Systems. The system will protect up to 6 Remote Radio Heads from voltage surges and lightning, and connect up to 18 fiber pairs. The system is enclosed in a NEMA 4 rated, waterproof enclosure.

FEATURES

- Protects up to 6 Remote Radio Heads, each with its own protection circuit.
- Flexible design allows for installation at the top of a tower for Remote Radio Head protection.
- Includes fiber connections for up to 18 pairs of fiber.
- LED indicators on individual circuits provide visual indication of suppressor status.
- Form 'C' relays allow for remote monitoring of the suppressor status.
- Patented Strikesorb technology provides over 60 kA of surge current capacity per circuit.
- Strikesorb suppression modules are fully recognized to UL 1449-3rd Edition Safety Standard, meeting all intermediate and high current fault requirements to facilitate use in OEM applications.
- Raycap recommends that DC protection system be installed within 2 meters or 6 feet of the radio.
- Dome design is lightweight and aerodynamic providing maximum flexibility for installation on top of towers.





DC6-48-60-18-8F

DC Power Surge Protection

Electrical Specifications	
Model Number	DC6-48-60-18-8F
Nominal Operating Voltage	48 VDC
Nominal Discharge Current (I_n)	20 kA 8/20 μ s
Maximum Discharge Current (I_{max}) per NEMA LS-1	60 kA 8/20 μ s
Maximum Continuous Operating Voltage (U_c)	75 VDC
Voltage Protection Rating	400 V

Mechanical Specifications	
Suppression Connection Method	Compression lug, #2-#14 AWG Copper, #2-#12 Aluminum
Fiber Connection Method	LC-LC Single mode duplex
Environmental Rating	IP 68, 7m 72hrs
Operating Temperature	-40° C to + 80° C
Storage Temperature	-70° C to + 80° C
Cold Temperature Cycling	IEC 61300-2-22e -30° C to + 60° C 200 hrs @ 5 psi
Resistance to Aggressive Materials	CEI IEC 61073-2 including acids and bases
UV Protection	ISO 4892-2 Method A Xenon-Arc 2160 hrs
Weight	20 lbs without Mounting Bracket

STANDARDS

Strikesorb modules are compliant to the following Surge Protection Device (SPD) Standards:

- ANSI/UL 1449 – 3rd Edition
- IEEE C62.41
- NEMA LS-1, IEC 61643-1:2005 2nd Edition: 2005
- IEC 61643-12
- EN 61643-11:2002 (including A11:2007)



G02-00-068 REV 050610

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GS-07F-0435V



Certified to
ISO 9001:2000




TUV Rheinland
of North America

12 PAIR FIBER TRUNK

FTTA fiber trunks are fiber optical cable assemblies connecting base stations and remote radio heads in telecommunication applications. They can be used indoor and outdoor, are UV protected and riser rated. Connectors and fan-out are IP67 protected. This ensures easy handling in an outdoor environment. A pulling sock eases cable hoisting.

Part #	Diameter	Description	QTY
FB-L98B-002-15000 CEQ.32135	10mm	Preconnect Trunk with UL certified cable 12 channels, 24 fibers, single mode LC Duplex to LC Duplex, OD 10.0mm: 15 meter length.	Each
FB-L98B-002-30000 CEQ.32194	10mm	Preconnect Trunk with UL certified cable 12 channels, 24 fibers, single mode LC Duplex to LC Duplex, OD 10.0mm: 30 meter length.	Each
FB-L98B-002-50000 CEQ.32193	10mm	Preconnect Trunk with UL certified cable 12 channels, 24 fibers, single mode LC Duplex to LC Duplex, OD 10.0mm: 50 meter length.	Each
FB-L98B-002-75000 CEQ.32192	10mm	Preconnect Trunk with UL certified cable 12 channels, 24 fibers, single mode LC Duplex to LC Duplex, OD 10.0mm: 75 meter length.	Each
FB-L98B-002-100000 CEQ.32191	10mm	Preconnect Trunk with UL certified cable 12 channels, 24 fibers, single mode LC Duplex to LC Duplex, OD 10.0mm: 100 meter length.	Each
FB-L98B-002-125000 CEQ.32190	10mm	Preconnect Trunk with UL certified cable 12 channels, 24 fibers, single mode LC Duplex to LC Duplex, OD 10.0mm: 125 meter length.	Each



6 CONDUCTOR (3 PAIR) POWER CABLE

600 Volts Power Cable. UL Approved for direct burial or sunlight applications.

Part #	Diameter	Description	QTY
WR-VG86T CEQ.32182	19.2 mm	RSS 8-AWG 6 - Conductor Unshielded 600 Volts Power Cable -# 8 Tinned Copper (three traced red/black pairs) w/ #10 Bare Ground Wire.	Per FT
WR-VG86ST-BRD CEQ.32181	19.7 mm	RSS 8-AWG 6 - Conductor Shielded (Tinned Copper Braid and Aluminum Tape w/ Drain Wire) 600 Volts Power Cable -# 8 Tinned Copper (three traced red/black pairs) w/ #10 Bare Ground Wire.	Per FT

2 CONDUCTOR (SINGLE PAIR) SHIELDED POWER CABLE

600 Volts Power Cable. UL Approved for direct burial or sunlight applications.

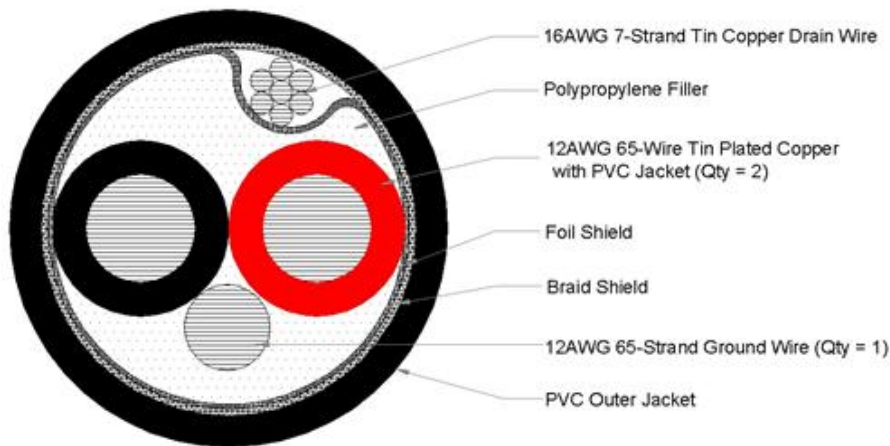
Part #	Diameter	Description	QTY
WR-VG122ST-BRDA CEQ.10224	9.8mm	RSS 12-AWG 2 - Flexible Conductor Shielded (Tinned Copper Braid and Aluminum Tape w/ Drain Wire) 600 Volts Power Cable -# 12 AWG 65 Strands Tinned Copper (red and black) w/ #12 Bare Ground Wire.	Per FT
WR-VG102ST-BRDA CEQ.10225	11.6mm	RSS 10-AWG 2 - Flexible Conductor Shielded (Tinned Copper Braid and Aluminum Tape w/ Drain Wire) 600 Volts Power Cable -# 10 AWG 105 Strands Tinned Copper (red and black) w/ #10 Bare Ground Wire.	Per FT
WR-VG82ST-BRDA CEQ.10226	15.4mm	RSS 8-AWG 2 - Flexible Conductor Shielded (Tinned Copper Braid and Aluminum Tape w/ Drain Wire) 600 Volts Power Cable -# 8 AWG 168 Strands Tinned Copper (red and black) w/ #10 Bare Ground Wire.	Per FT



PWRT-212-S

Remote Radio Head Power Cable, 2 conductor with shield, 12 AWG (3.31 mm²)

Cross Section Drawing



Construction Materials

Construction Type	Non-armored
Conductor Material	Tinned copper
Dielectric Material	PVC
Drain Wire Material	Tinned copper
Filler Material	Polypropylene
Ground Wire Material	Tinned copper
Insulation Material, singles	PVC
Jacket Material	PVC
Outer Shield (Braid) Coverage	65 %
Outer Shield (Braid) Gauge	36 AWG
Outer Shield (Braid) Material	Tinned copper
Outer Shield (Tape) Material	Aluminum/Poly, non-bonded

Dimensions

Cable Weight	0.16 kg/m 0.11 lb/ft
Diameter Over Conductor, singles	2.5654 mm per 65 strand 0.1010 in per 65 strand
Diameter Over Dielectric	3.5814 mm 0.1410 in
Diameter Over Drain Wire	1.5200 mm per 7 strand 0.0598 in per 7 strand
Diameter Over Ground Wire	2.565 mm 0.101 in
Diameter Over Jacket	10.109 mm 0.398 in

PWRT-212-S



Diameter Over Shield (Braid)	7.823 mm 0.308 in
Jacket Thickness	1.143 mm 0.045 in

Electrical Specifications

Conductor dc Resistance	1.68 ohms/kft 5.51 ohms/km
Conductor dc Resistance Note	Maximum value based on a standard condition of 20 °C (68 °F)
Safety Voltage Rating	600 V

Environmental Specifications

Environmental Space	UV resistant for outdoor and/or direct burial installations
Operating Temperature	-40 °C to +90 °C (-40 °F to +194 °F)
Safety Standard	NEC Article 336 (Type TC)

General Specifications

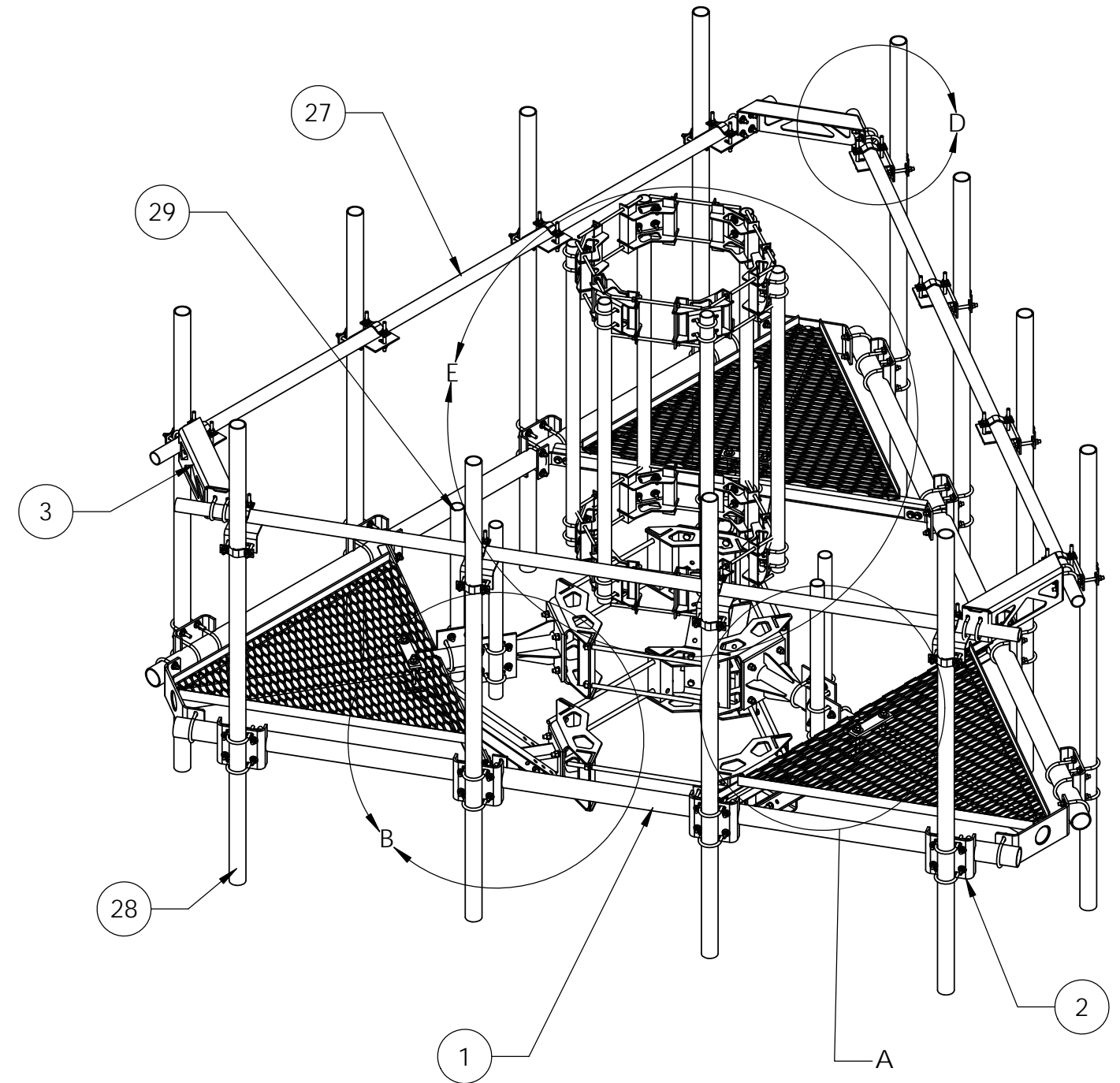
Application	Industrial
Cable Type	Power
Jacket Color	Black
Conductor Gauge, singles	12 AWG
Conductor Type, singles	Stranded
Conductors, quantity	2
Drain Wire Gauge	16 AWG
Ground Wire Gauge	12 AWG
Ground Wire Type	Stranded
Jacket Color, singles	Black Red

Regulatory Compliance/Certifications

Agency	Classification
ISO 9001:2008	Designed, manufactured and/or distributed under this quality management system

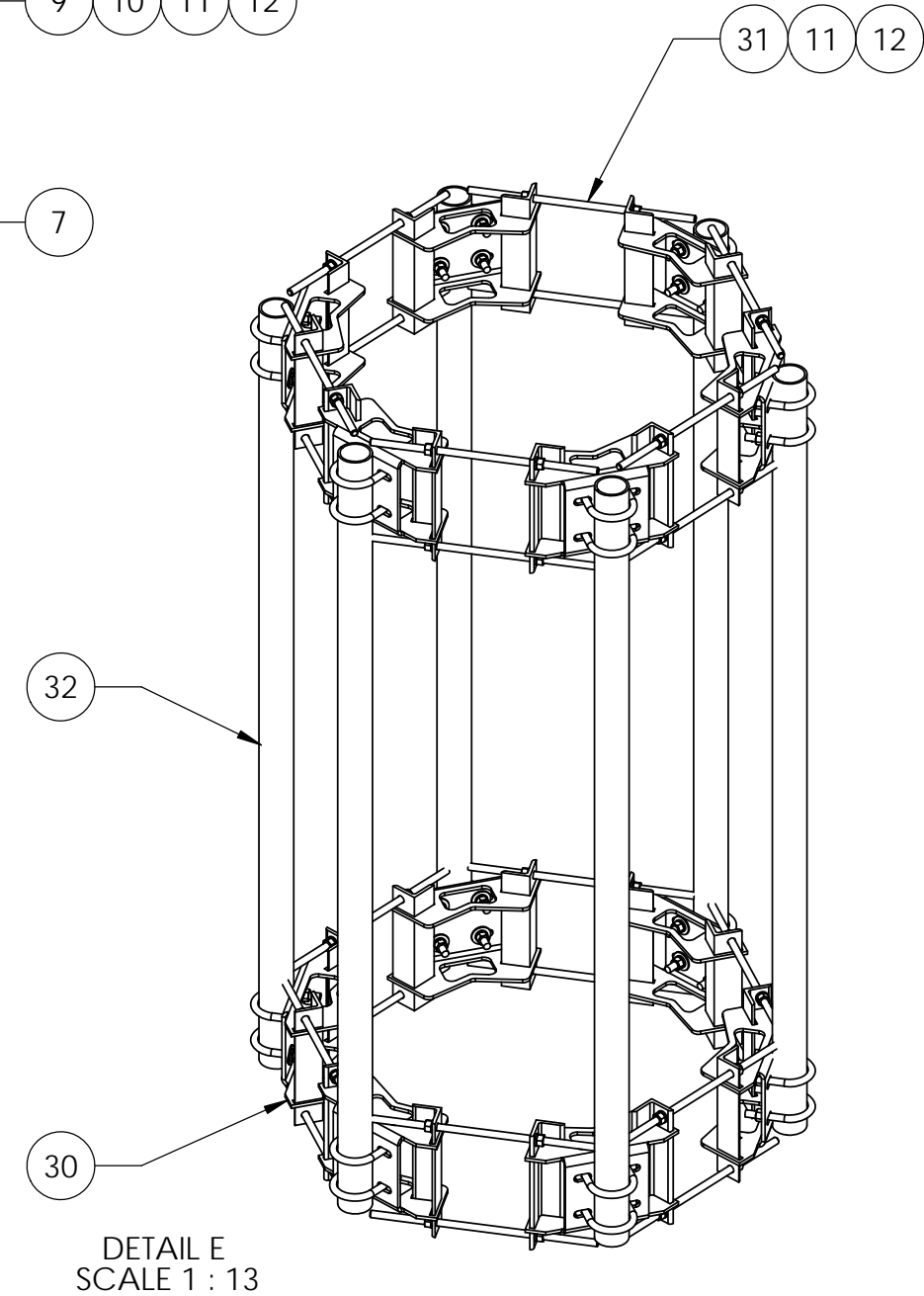
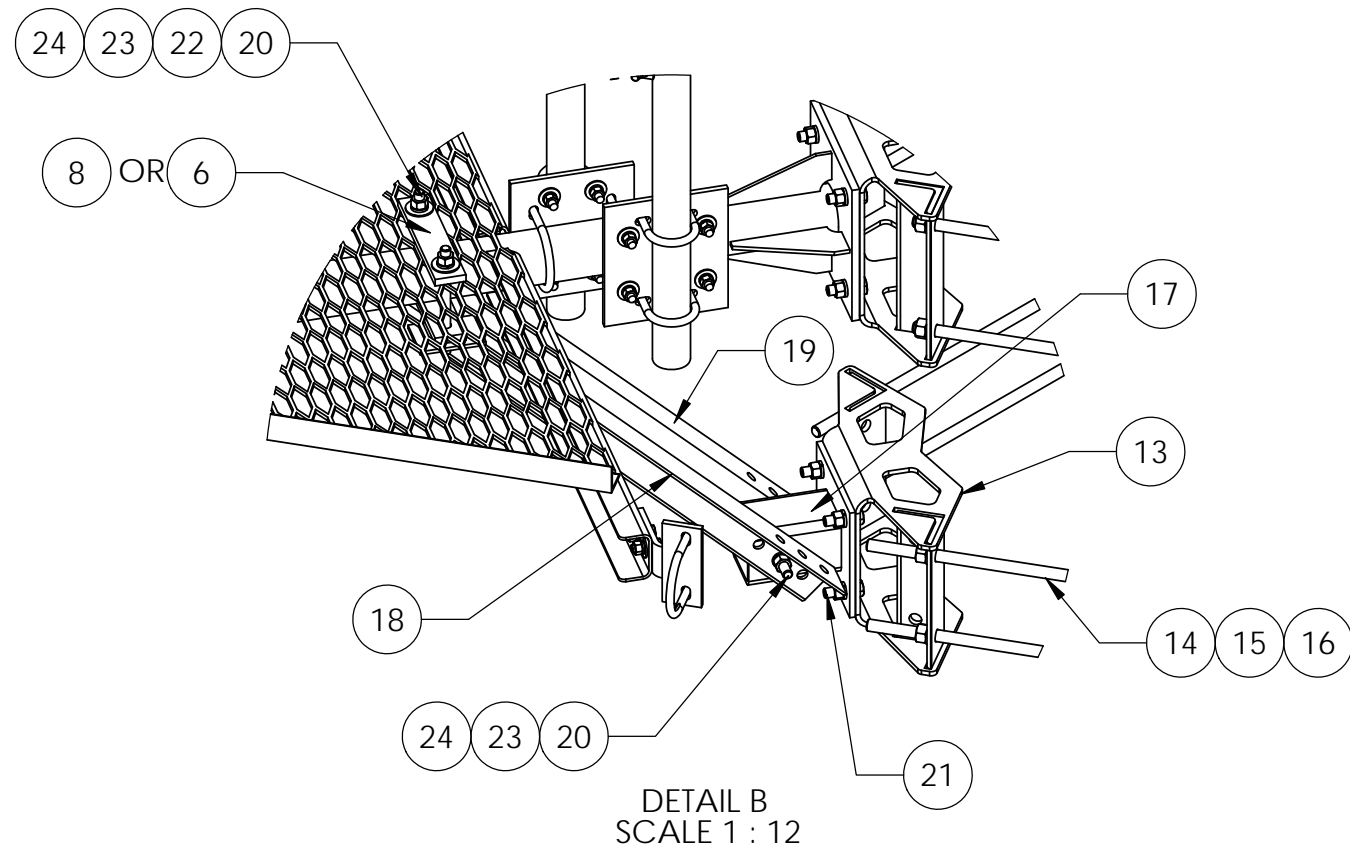
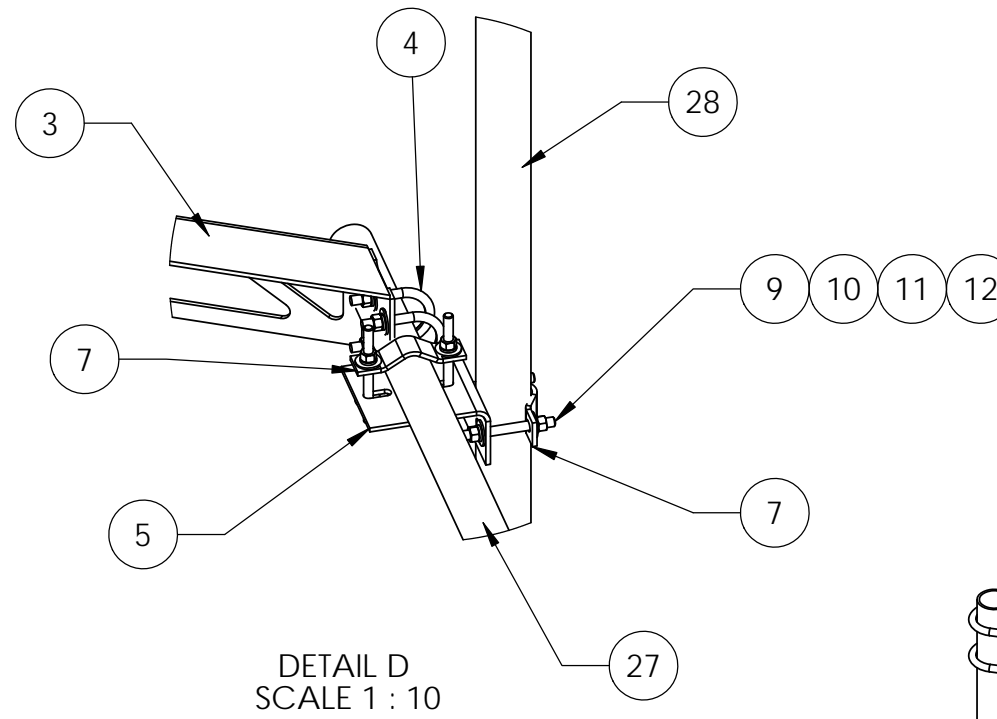
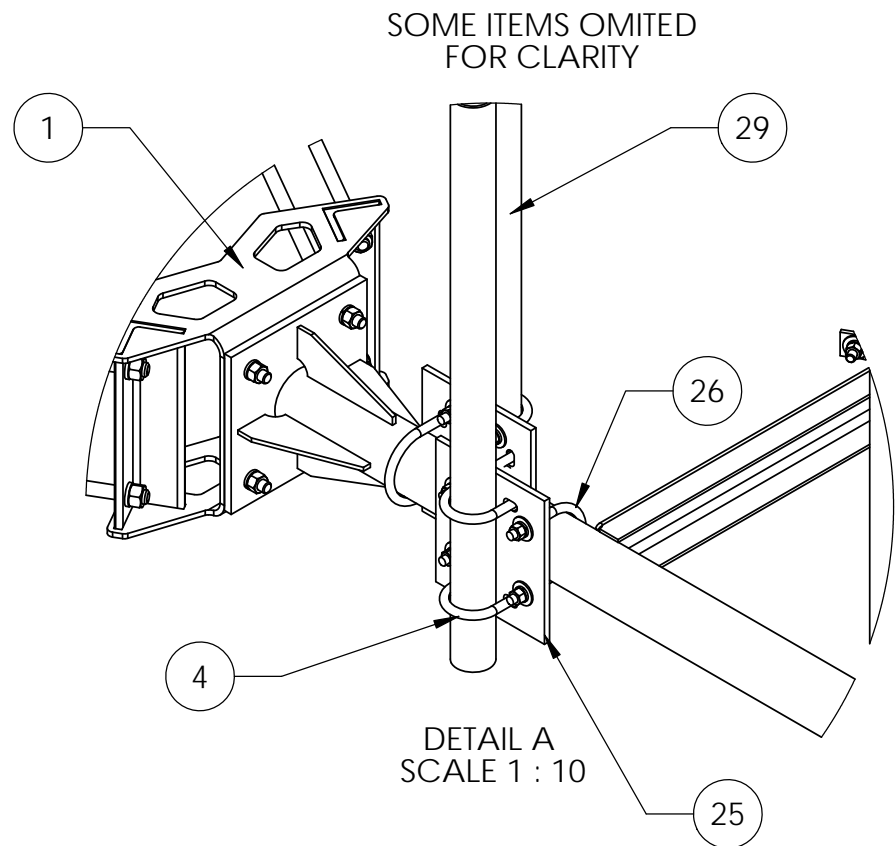
ITEM	PART NO.	DESCRIPTION	QTY.	WEIGHT
1	MC-PK12S-B	LOW PROFILE CO-LOCATION PLATFORM KIT	1	859.05 LBS
2	MT-219M-H	3.5" OD X 2-7/8" OD Clamp Bracket Assembly	12	12.78 LBS
3	MT19525	Corner Weldment	3	14.76 LBS
4	GUB-4240	1/2" X 2-1/2" X 4" GALV U-BOLT	48	0.56 LBS
5	XAU01	Angle BRK	12	3.59 LBS
6	MTC323704	Clamp Bar	6	2.37 LBS
7	ACP10	1.5" - 3.5" O.D. CLAMP HALF	24	0.61 LBS
8	DCP10	SMALL CLAMP HALF	6	2.21 LBS
9	MT-379-6	1/2" X 6" GALV THREADED ROD	48	0.33 LBS
10	GWF-04	1/2" GALV FLAT WASHER	96	0.03 LBS
11	GWL-04	1/2" GALV LOCK WASHER	144	0.01 LBS
12	GN-04	1/2" GALV HEX NUT	144	0.04 LBS
13	MTC306503	CW 1030 Ringmount Weldment	3	28.02 LBS
14	MT38430B7	3/4" X 30" GALV THREADED ROD GRADE B7	6	3.73 LBS
15	GWL-06	3/4" GALV LOCK WASHER	12	0.04 LBS
16	GN-06	3/4" GALV HEX NUT	12	0.14 LBS
17	MTC313802	Kicker Mount Standoff	3	13.08 LBS
18	MTC323701	Left Kicker	3	14.36 LBS
19	MTC323702	Right Kicker	3	14.36 LBS
20	MT-381-8	5/8" X 8" GALV THREADED ROD	9	0.69 LBS
21	GB-0520A	5/8" X 2" GALV BOLT KIT (A325)	12	0.27 LBS
22	GWF-05	5/8" GALV FLAT WASHER	12	0.06 LBS
23	GWL-05	5/8" GALV LOCK WASHER	18	0.03 LBS
24	GN-05	5/8" GALV HEX NUT	18	0.08 LBS
25	XP2030.01	CROSSOVER PLATE ϕ 2-3/8" O.D. TO ϕ 3-1/2" O.D.	6	7.30 LBS
26	GUB-4355	1/2" X 3-5/8" X 5" GALV U-BOLT	12	0.71 LBS
27	MT-651-150	ϕ 2.375" OD x 150" PIPE	3	45.42 LBS
28	MT54696	ϕ 2.875" O.D. X 96 PIPE	12	46.51 LBS
29	MT-649	2 3/8" x 36" Pipe	6	10.90 LBS
30	MTC320001	RRU Ringmount	12	10.52 LBS
31	MT37916	1/2" X 16" GALV THREADED ROD	24	0.88 LBS
32	MT-650-63	ϕ 2.375" OD x 63" PIPE	6	19.08 LBS

REVISIONS				
REV.	ZONE	DESCRIPTION	BY	DATE
A		INITIAL RELEASE	DRR	10/18/13



NOTES:
 1. ALL METRIC DIMENSIONS ARE IN BRACKETS.
 2. FITS MONOPOLES ϕ 10" - ϕ 30".

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	CHECKED BY: TP	SCALE: NTS	DESCRIPTION: Low Profile Co-Location Platform Kit
	DATE: 10/18/13	MATERIAL: A36, A500, A529	DRAWING TYPE: ASSEMBLY DRAWING
	REVISION: A	FINISH: Galv A123	 Hickory, NC 28602 U.S.A.
WEIGHT: 2528.27 LBS			



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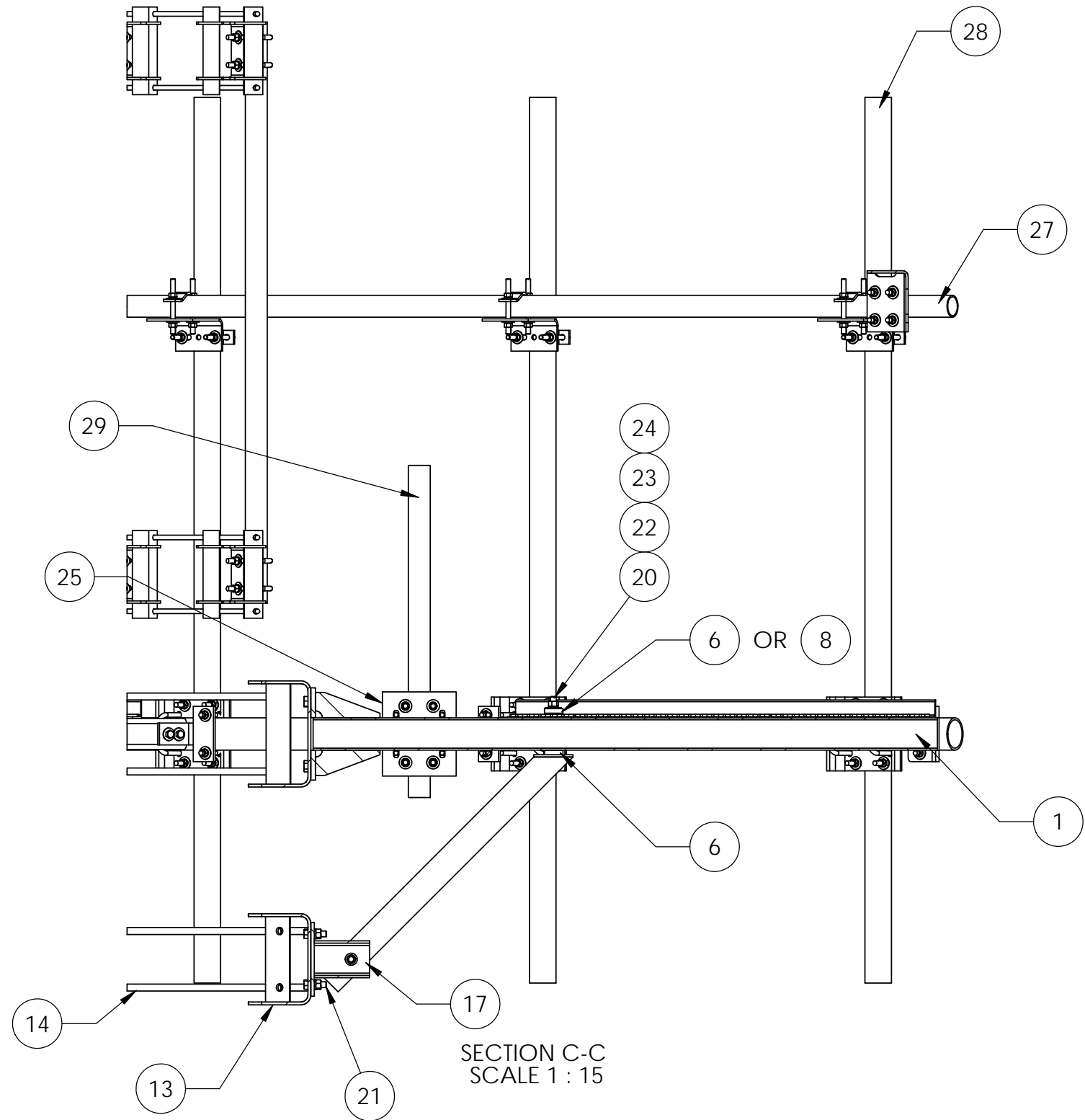
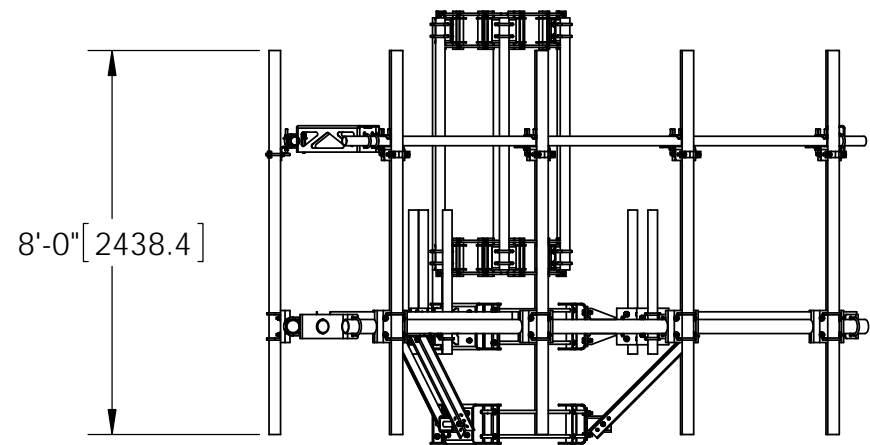
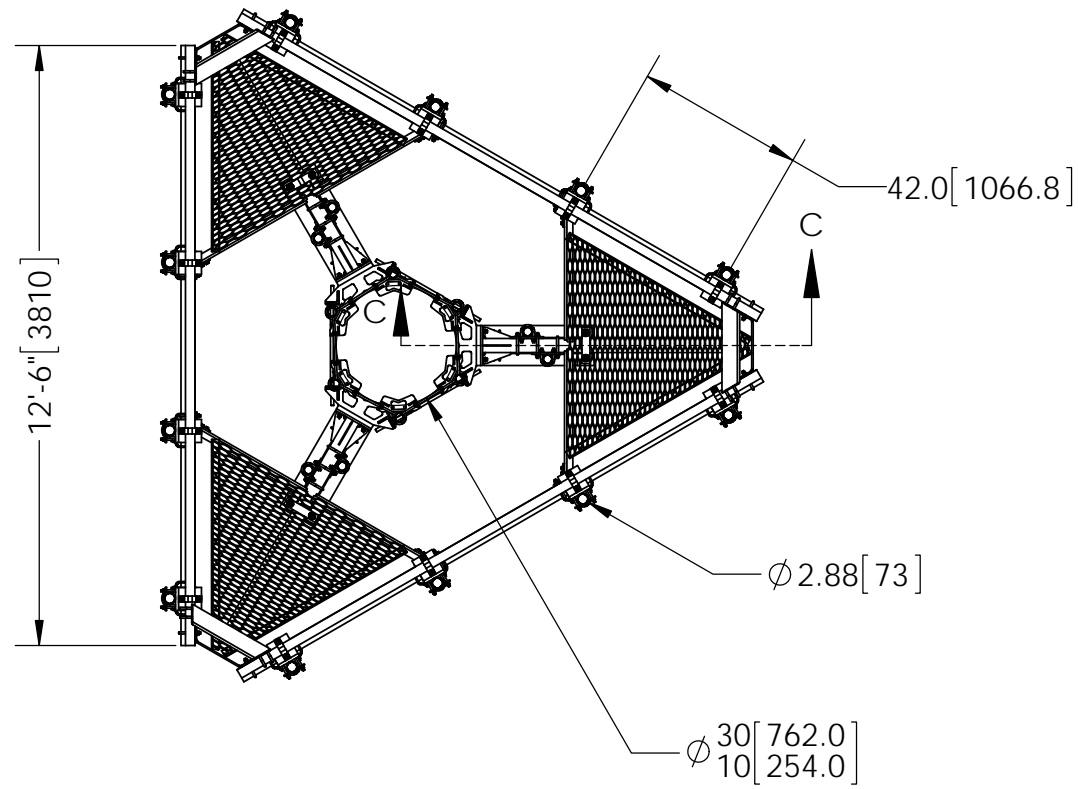
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.XXX = ± .031

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CHECKED BY: TP	SCALE: NTS
DATE: 10/18/13	MATERIAL: A36, A500, A529
REVISION: A	FINISH: Galv A123
	WEIGHT: 2528.27 LBS

PART NUMBER: MTC3607R
DESCRIPTION: Low Profile Co-Location Platform Kit
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COMMSCOPE® Hickory, NC 28602 U.S.A.



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.XXX = ± .031

REMOVE BURRS AND BREAK EDGES .005

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CHECKED BY: TP	SCALE: NTS
DATE: 10/18/13	MATERIAL: A36, A500, A529
REVISION: A	FINISH: Galv A123
	WEIGHT: 2528.27 LBS

PART NUMBER: MTC3607R
DESCRIPTION: Low Profile Co-Location Platform Kit
DRAWING TYPE: ASSEMBLY DRAWING
COMMSCOPE® Hickory, NC 28602 U.S.A.