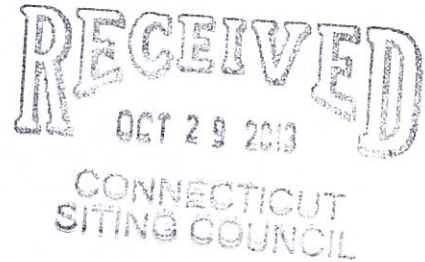




October 25, 2013

VIA OVERNIGHT COURIER

Connecticut Siting Council
10 Franklin Square
New Britain, Connecticut 06051
Attn: Ms. Melanie Bachman, Acting Executive Director



Re: Sprint Spectrum, L.P. – Exempt Modification
761 Federal Road, Brookfield, Connecticut

Dear Ms. Bachman:

This letter and attachments are submitted on behalf of Sprint Spectrum, L.P. ("Sprint"). Sprint is undertaking modifications to certain existing sites in its Connecticut system in order to implement updated technology. Please accept this letter and attachments as notification, pursuant to R.C.S.A. Section 16-50j-73, of construction that constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2). In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter and attachments is being sent to the First Selectman of the Town of Brookfield.

Sprint plans to modify the existing wireless communications facility owned by the Connecticut Light and Power Company and located at 761 Federal Road, Brookfield (coordinates 41°-28'-43.55" N, 73°-24'-29.86" W). Attached are plan and elevation drawings depicting the planned changes, and documentation of the structural sufficiency of the structure to accommodate the revised antenna configuration. Also included is a power density report reflecting the modification to Sprint's operations at the site.

The changes to the facility do not constitute a modification as defined in Connecticut General Statutes ("C.G.S.") Section 16-50i(d) because the general physical characteristics of the facility will not be significantly changed. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in R.C.S.A. Section 16-50j-72(b)(2).

1. Sprint will remove the existing three (3) CMDA antennas and add three (3) dual-band panel LTE antennas flushed mounted to the existing pipe masts at a centerline height of approximately 110.8' AGL. Sprint will also install six (6) RRHs (remote radio heads) and Notch Filters on proposed steel mounts on the existing Concrete Pad. Sprint will also place DC power and fiber runs along the existing coaxial cable run. The

existing coaxial cables will remain. The proposed modifications will not extend the height of the approximately 90' structure, or 180' AGL.

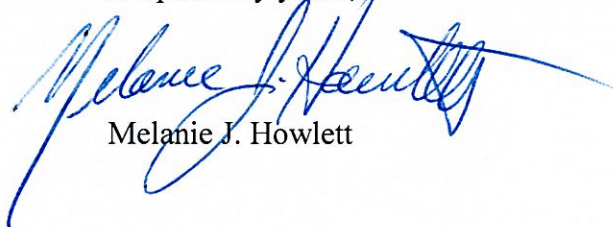
2. Sprint will replace the two (2) existing cabinets with two (2) similar cabinets, add a third cabinet, and add a Fiber/Power Distribution Box on an H-frame, all on the existing Concrete Equipment Pad. The existing GPS antenna will be replaced by another GPS antenna. These changes will have no effect on the site boundaries.

3. The proposed changes will not increase the noise level at the existing facility by six decibels or more. The incremental effect of the proposed changes will be negligible.

4. The changes to the facility will not increase the calculated "worst case" power density for the combined operations at the site to a level at or above the applicable standard for uncontrolled environments as calculated for a mixed frequency site. As indicated on the attached report prepared by EBI Consulting, Sprint's operations at the site will result in a power density of approximately 20.424%; the combined site operations will result in a total power density of approximately 52.584%.

Please contact me by phone at (203) 610-1071 or by e-mail at mjhowlett@optonline.net with questions concerning this matter. Thank you for your consideration.

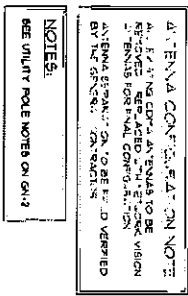
Respectfully yours,



Melanie J. Howlett

Attachments

cc: Honorable William R. Davidson, First Selectman, Town of Brookfield
State of Connecticut (underlying property owner)



SEE UTILITY POLE NOTES ON GN-2

✓-EXISTING ✓✓-INK REUSE

7-11-58
7-12-58

EXISTING 12" SCH. 40X22' LONG
PIPE MUST

· E/5'NS BARNET POWER &
TELCO CABINET

CT54XC713
STP NUMBER
STP NAME
CLIP POLE * 2683 -
BROOKFIELD
STP ADDRESS
161 FEDERAL ROAD
BROOKFIELD, CT 06804

COMPOUND PLAN

STUDENT PROJ. NO.	SECRET NO.
C154XC713	
DATE	
CHECKED BY:	
ADC	

Sprint

1. INTERNATIONAL AND STATE 800
NUMBER, 1-877-855-8800
P: 800-337-7641

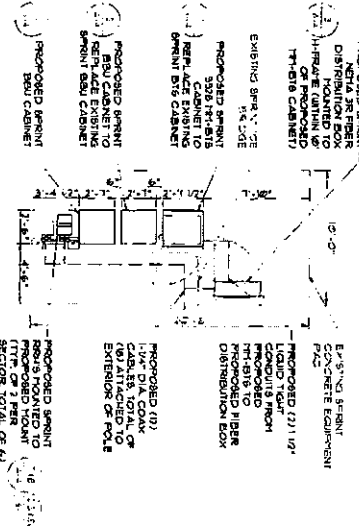
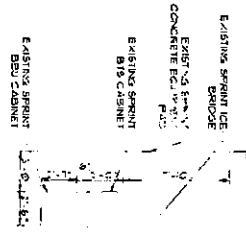
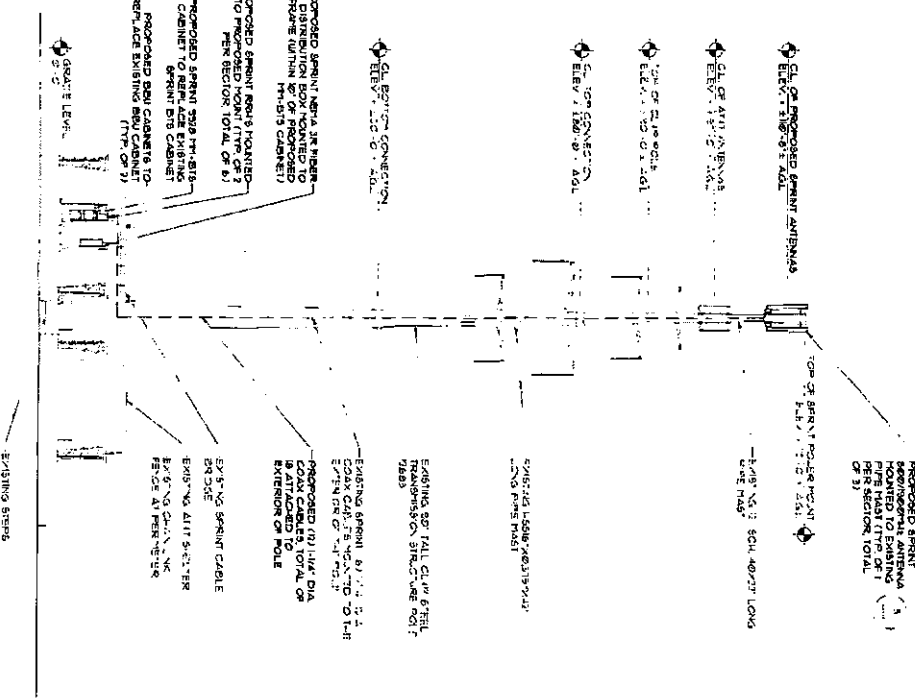
 **Alcatel • Lucent**

100 200 AUSTIN AVE
AUSTIN, TX 78704

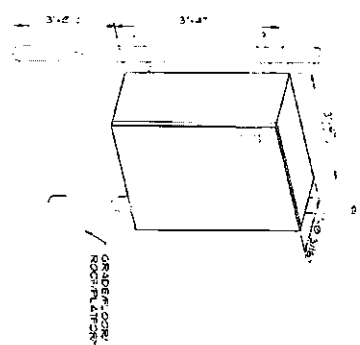
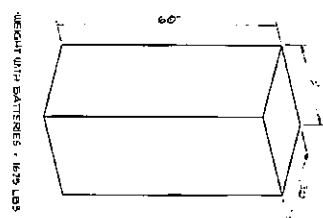
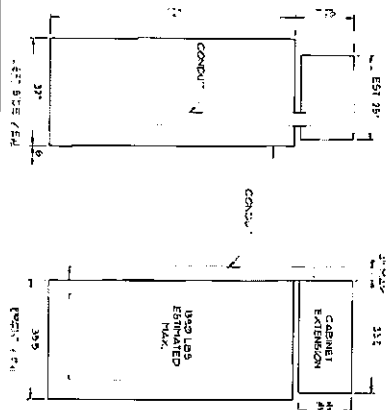
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NOTE:
SEE UTILITY POLE NOTES ON CN-1



DATE	12-1-72
CHECKED BY	ADG



Sprint

1 INTERNATIONAL BLVD, SUITE 800
HAWTHAY, NJ 07495
P: 800.357.7641



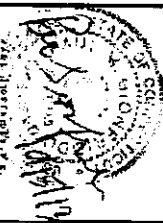
Alcatel-Lucent

900-704-6671/1000 AVE
MURRAY HILL - NJ 07974



Salient
ARCHITECTS, LLC

New Jersey Office
6 East Pennside Avenue
Rm 210 Wood. NJ 07031
P: 201-587-0032 F: 201-587-0506



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SUBMITTALS			
NO	DATE	DESCRIPTION	BY
1	3/07	PRELIMINARY	
2	3/08	RECEIVED FROM CONTRACTOR	AM
3	3/08	STRUCTURAL REVIEW	TS
4	3/08/09	ISSUED AS FINAL	MD/AM

DATE SUBMITTED: CT54XC713

CLIFF POLE • 2683

BROOKFIELD
SITE ADDRESS
T61 FEDERAL ROAD

BROOKFIELD, CT 06804

TIME | TIME

TYPICAL DETAILS

DATE OF BIRTH	DATE OF DEATH
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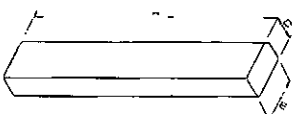
DATE: CT54XC10

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AD

44

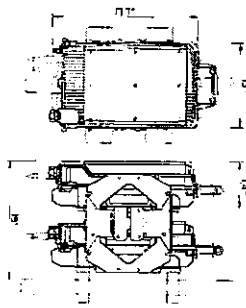
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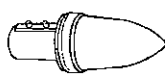
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MODEL NO.	175-175-175
DATE	175-175-175
BY	175-175-175
FOR	175-175-175
AS TO	175-175-175

FEDERAL BUREAU OF INVESTIGATION

DATE = NT5



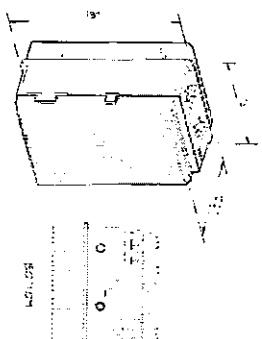
SCALE IN INCHES



- ENVI SPECIFIC CUB	
MODEL NO.	1
CUBIC	1
WEIGHT	1
GP: 1000, 1000	1

*ALL FOUNDATIONS ON PILES OF 1'-1.145' MAX DIA

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
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SECRET

60-1117-115.

1971
SHEL - NIT

4-A



**Northeast
Utilities System**

107 Selden Street, Berlin, CT 06037

Northeast Utilities Service Company
P.O. Box 270
Hartford, CT 06141-0270
(203) 665-5000

October 4, 2013

Ms. Jennifer Gaudet
HPC Development

Sprint,
1 International Blvd.
Suite 300
Mahwah NJ
07495

RE: Sprint Antenna Site, CT-03XC329, 63-69 Myrtle Ave., Stamford CT, structure 1217

Dear Ms. Gaudet:

Based on the structural review letter dated August 27, 2013 provided by Centek Engineering stating that the proposed structural loading has not increased when compared to the original structural modification report dated July 10, 2013; we have reviewed for acceptance this modification.

Since there are no outstanding structural or site related issues to resolve at this time, construction at these locations may begin as soon as scheduling allows. You may contact Mr. O'Brien (860-665-6987); once the lease issues are secured you may then contact Mr. John Landry directly (860-665-5425) to begin the construction arrangements

Sincerely,

Robert Gray

Transmission Line Engineering

REF: 12047.CO2 - CT03XC329-Stamford.pdf
12047.002 CT03XC329 - Antenna Upgrade Compliance Letter.pdf
NV_CT03XC329_Issued as Final CDs.pdf

Structural Analysis of PCS
Mast and CL&P Pole

Sprint Site Ref: CT54XC713

CL&P Structure No. 2683
90' Electric Transmission Pole

761 Federal Road
Brookfield, CT

CEN TEK Project No. 12047.CO13

Date: April 18, 2013



Prepared for:
Sprint Nextel
8 Airline Drive, Suite 105
Albany, NY 12205

Table of Contents

SECTION 1 - REPORT

- INTRODUCTION
- PRIMARY ASSUMPTIONS USED IN THE ANALYSIS
- ANALYSIS
- DESIGN BASIS
- RESULTS
- CONCLUSION

SECTION 2 - CONDITIONS & SOFTWARE

- STANDARD ENGINEERING CONDITIONS
- GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAMS
 - RISA 3-D
 - PLS POLE

SECTION 3 - DESIGN CRITERIA

- CRITERIA FOR DESIGN OF PCS FACILITIES ON OR EXTENDING ABOVE METAL ELECTRIC TRANSMISSION TOWERS
- NU DESIGN CRITERIA TABLE
- PCS SHAPE FACTOR CRITERIA
- WIRE LOADS SHEET

SECTION 4 - DRAWINGS

- EL-1 POLE AND MAST ELEVATION

SECTION 5 - EIA/TIA-222-F LOAD CALCULATIONS FOR MAST ANALYSIS

- MAST WIND & ICE LOAD

SECTION 6 - MAST ANALYSIS PER EIA/TIA-222F

- LOAD CASES AND COMBINATIONS (TIA/EIA LOADING)
- RISA 3-D ANALYSIS REPORT
- MAST CONNECTION TO CL&P TOWER ANALYSIS

**SECTION 7 - NECS/NU LOAD CALCULATIONS FOR OBTAINING
MAST REACTIONS APPLIED TO UTILITY STRUCTURE**

- MAST WIND LOAD

**SECTION 8 - MAST ANALYSIS PER NESC/NU FOR OBTAINING
REACTIONS APPLIED TO UTILITY STRUCTURE**

- LOAD CASES AND COMBINATIONS (NESC/NU LOADING)
- RISA 3-D ANALYSIS REPORT

**SECTION 9 - PLS POLE RESULTS FROM MAST REACTIONS
CALCULATED IN RISA WITH NESC/NU CRITERIA**

- COAX CABLE LOAD ON CL&P TOWER CALCULATION
- PLS REPORT
- ANCHOR BOLT ANALYSIS.

SECTION 10 - REFERENCE MATERIAL

- RFDS SHEET
- EQUIPMENT CUT SHEETS

Introduction

The purpose of this report is to analyze the existing mast and 90' CL&P pole located at 761 Federal Road in Brookfield, CT for the proposed antenna and equipment upgrade by Sprint.

The existing/proposed loads consist of the following:

- **AT&T (Existing to Remain):**
Antennas: Three (3) Powerwave 7770 panel antennas, three (3) Powerwave P65-16-XLH-RR panel antennas, three (3) Powerwave TTAW-07BP111-001 TMAs and six (6) Powerwave LGP-21401 TMAs mounted on dual standoff mounts to the existing mast with a RAD center elevation of 97-ft above grade level.
Coax Cables: Fifteen (15) 1-1/4" Ø coax cables running on the exterior of the CL&P pole.
- **SPRINT (Existing to Remain):**
Coax Cables: Six (6) 1-1/4" Ø coax cables running on the exterior of the CL&P pole and pcs mast.
Mast: 12" Sch. 40 x 22' long pipe flange connected to a HSS16"x0.375" x 42' long pipe.
- **SPRINT (Existing to Remove):**
Antennas: Three (3) EMS RR90-17-00DP panel antennas flush mounted on the existing mast with a RAD center elevation of 110-ft-8-in above grade level.
- **SPRINT (Proposed):**
Antennas: Three (3) RFS APXVSP18-C panel antennas panel antennas flush mounted on the existing mast with a RAD center elevation of 110-ft-8-in above grade level.
Coax Cables: Twelve (12) 1-1/4" Ø coax cables running on the exterior of the CL&P pole and pcs mast.

Primary assumptions used in the analysis

- Allowable steel stresses are defined by AISC-ASD 9th edition for design of the PCS Mast and antenna supporting elements.
- ASCE Manual No. 72, "Design of Steel Transmission Pole Structures Second Edition", defines allowable steel stresses for evaluation of the CL&P utility pole.
- All utility pole members are adequately protected to prevent corrosion of steel members.
- All proposed antenna mounts are modeled as listed above.
- Pipe mast will be properly installed and maintained.
- No residual stresses exist due to incorrect pole erection.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds conform to the requirements of AWS D1.1.
- Pipe mast and utility pole will be in plumb condition.
- Utility pole was properly installed and maintained and all members were properly designed, detailed, fabricated, and installed and have been properly maintained since erection.
- Any deviation from the analyzed loading will require a new analysis for verification of structural adequacy.

A n a l y s i s

Structural analysis of the existing *Mast* was independently completed using the current version of RISA-3D computer program licensed to CEN TEK Engineering, Inc.

The existing mast consisting of a 12" Sch. 40 x 22' long pipe conforming to ASTM A53 Grade B (Fy = 35ksi) flange connected to a HSS16"x0.375" x 42' long pipe conforming to ASTM A500 Grade B (Fy = 42ksi) connected at two points to the existing pole was analyzed for its ability to resist loads prescribed by the TIA/EIA standard. Section 5 of this report details these gravity and lateral wind loads. NESC prescribed loads were also applied to the mast in order to obtain reactions needed for analyzing the CL&P pole structure. These loads are developed in Section 7 of this report. Load cases and combinations used in RISA-3D for TIA/EIA loading and for NESC/NU loading are listed in report Sections 6 and 8, respectively.

An envelope solution was first made to determine maximum and minimum forces, stresses, and deflections to confirm the selected section as adequate. Additional analyses were then made to determine the NESC forces to be applied to the CL&P pole structure.

The RISA-3D program contains a library of all AISC shapes and corresponding section properties are computed and applied directly within the program. The program's Steel Code Check option was also utilized. The forces calculated in RISA-3D using NESC guidelines were then applied to the CL&P pole using PLS-Pole. Maximum usage for the pole was calculated considering the additional forces from the mast and associated appurtenances.

D e s i g n B a s i s

Our analysis was performed in accordance with TIA/EIA-222-F-1996, ASCE Manual No. 72 – "Design of Steel Transmission Pole Structures Second Edition", NESC C2-2007 and Northeast Utilities Design Criteria.

The CL&P pole structure, considering existing and future conductor and shield wire loading, with the pcs antenna mast was analyzed under two conditions:

▪ UTILITY POLE ANALYSIS

The purpose of this analysis is to determine the adequacy of the existing utility pole to support the proposed antenna loads. The loading and design requirements were analyzed in accordance with the NU Design Criteria Table, NESC C2-2007 ~ Construction Grade B, and ASCE Manual No. 72.

Load cases considered:

Load Case 1: NESC Heavy

Wind Pressure.....	4.0 psf
Radial Ice Thickness.....	0.5"
Vertical Overload Capacity Factor.....	1.50
Wind Overload Capacity Factor.....	2.50
Wire Tension Overload Capacity Factor.....	1.65

Load Case 2: NESC Extreme

Wind Speed.....	100 mph ⁽¹⁾
Radial Ice Thickness.....	0"

Note 1: NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading, 1.25 x Gust Response Factor (wind speed: 3-second gust)

MAST ASSEMBLY ANALYSIS

Mast, appurtenances and connections to the utility tower were analyzed and designed in accordance with the NU Design Criteria Table, TIA/EIA-222-F, and AISC-ASD standards.

Load cases considered:

Load Case 1:

Wind Speed..... 85 mph ⁽²⁾
 Radial Ice Thickness..... 0"

Load Case 2:

Wind Pressure..... 75% of 85 mph wind pressure
 Radial Ice Thickness..... 0.5"

| Note 2: Per NU Mast Design Criteria Exception 1.

Results

MAST ASSEMBLY

The existing mast was determined to be structurally **adequate**.

Member	Stress Ratio (% of capacity)	Result
12" Sch. 40 pipe x 22-ft long	67.7%	PASS
HSS 16"x0.375" pipe x 42-ft long	79.4%	PASS
5/8" Ø ASTM A325 Bolt	42.6% ⁽¹⁾	PASS

Note 1 – 1/3 increase in allowable stress not used per OTRM 059.

UTILITY POLE

This analysis finds that the subject utility pole is adequate to support the proposed antenna mast and related appurtenances. The pole stresses meet the requirements set forth by the ASCE Manual No. 72, "Design of Steel Transmission Pole Structures Second Edition", for the applied NESC Heavy and Hi-Wind load cases. The detailed analysis results are provided in Section 9 of this report. The analysis results are summarized as follows:

A maximum usage of **70.57%** occurs in the utility pole under the **NESC Heavy** loading condition.

POLE SECTION:

The utility pole was found to be within allowable limits.

Tower Section	Elevation	Stress Ratio (% of capacity)	Result
Tube Number 3	0'-20.67' (AGL)	70.57%	PASS

BASE PLATE:

The base plate was found to be within allowable limits from the PLS output based on 10 bend lines.

Tower Component	Design Limit	Stress Ratio (percentage of capacity)	Result
Base Plate	Bending	58.15%	PASS

▪ **FOUNDATION AND ANCHORS**

The existing foundation consists of a 10-ft square x 9-ft long reinforced concrete pier with (16) rock anchors. The base of the tower is connected to the foundation by means of (12) 2.25"Ø, ASTM A615-75 anchor bolts embedded approximately 8-ft into the concrete foundation structure. Foundation information was obtained from NUSCO drawing # 01039-60001.

Review of the foundation and anchor design consisted of verification of applied loads obtained from the tower design calculations and code checks of allowable stresses:

BASE REACTIONS:

From PLS-Pole analysis of CL&P pole based on NESC/NU prescribed loads.

Load Case	Shear	Axial	Moment
NESC Heavy Wind	24.81 kips	50.88 kips	1892.67 ft-kips
NESC Extreme Wind	26.69 kips	24.93 kips	1911.26 ft-kips

Note 1 – 10% increase applied to tower base reactions per OTRM 051

ANCHOR BOLTS:

The anchor bolts were found to be within allowable limits.

Tower Component	Design Limit	Stress Ratio (% of capacity)	Result
Anchor Bolts	Tension	35.27%	PASS

FOUNDATION:

The foundation was found to be within allowable limits.

Foundation	Design Limit	Allowable Load ⁽¹⁾	Proposed Loading ⁽²⁾	Result
Reinf. Conc. Pier w/ Rock Anchors	OTM ⁽³⁾	2973.5 ft-kips	2102.4 ft-kips	PASS

Note 1: Allowable Load taken from NUSCO drawing no. 01039-50002.

Note 2: 10% increase to PLS base reactions used in foundation analysis per OTRM 051.

Note 3: OTM denotes Overturning Moment

Conclusions and Recommendations

This analysis shows that the subject utility pole is adequate to support the proposed Sprint equipment upgrade.

The analysis is based, in part on the information provided to this office by Northeast Utilities and Sprint. If the existing conditions are different than the information in this report, CENTEK engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

Respectfully Submitted by:

Carlo F. Centore, PE
 Principal ~ Structural Engineer

REPORT

Prepared by:

Timothy J. Lynn, EIT
 Structural Engineer

SECTION 1-4

CEN TEK Engineering, Inc.
Structural Analysis – 90-ft CL&P Pole # 2683
Sprint Antenna Upgrade – CT54XC713
Brookfield, CT
April 18, 2013

STANDARD CONDITIONS FOR FURNISHING OF
PROFESSIONAL ENGINEERING SERVICES ON
EXISTING STRUCTURES

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of CENTEK engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provided to CENTEK engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the "as new" condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222.
- All services are performed, results obtained, and recommendations made in accordance with generally accepted engineering principles and practices. CENTEK engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM ~ RISA - 3 D

RISA-3D Structural Analysis Program is an integrated structural analysis and design software package for buildings, bridges, tower structures, etc.

Modeling Features:

- Comprehensive CAD-like graphic drawing/editing capabilities that let you draw, modify and load elements as well as snap, move, rotate, copy, mirror, scale, split, merge, mesh, delete, apply, etc.
- Versatile drawing grids (orthogonal, radial, skewed)
- Universal snaps and object snaps allow drawing without grids
- Versatile general truss generator
- Powerful graphic select/unselect tools including box, line, polygon, invert, criteria, spreadsheet selection, with locking
- Saved selections to quickly recall desired selections
- Modification tools that modify single items or entire selections
- Real spreadsheets with cut, paste, fill, math, sort, find, etc.
- Dynamic synchronization between spreadsheets and views so you can edit or view any data in the plotted views or in the spreadsheets
- Simultaneous view of multiple spreadsheets
- Constant in-stream error checking and data validation
- Unlimited undo/redo capability
- Generation templates for grids, disks, cylinders, cones, arcs, trusses, tanks, hydrostatic loads, etc.
- Support for all units systems & conversions at any time
- Automatic interaction with RISASection libraries
- Import DXF, RISA-2D, STAAD and ProSteel 3D files
- Export DXF, SDNF and ProSteel 3D files

Analysis Features:

- Static analysis and P-Delta effects
- Multiple simultaneous dynamic and response spectra analysis using Gupta, CQC or SRSS mode combinations
- Automatic inclusion of mass offset (5% or user defined) for dynamic analysis
- Physical member modeling that does not require members to be broken up at intermediate joints
- State of the art 3 or 4 node plate/shell elements
- High-end automatic mesh generation — draw a polygon with any number of sides to create a mesh of well-formed quadrilateral (NOT triangular) elements.
- Accurate analysis of tapered wide flanges - web, top and bottom flanges may all taper independently
- Automatic rigid diaphragm modeling
- Area loads with one-way or two-way distributions
- Multiple simultaneous moving loads with standard AASHTO loads and custom moving loads for bridges, cranes, etc.
- Torsional warping calculations for stiffness, stress and design
- Automatic Top of Member offset modeling
- Member end releases & rigid end offsets
- Joint master-slave assignments
- Joints detachable from diaphragms
- Enforced joint displacements
- 1-Way members, for tension only bracing, slipping, etc.

- 1-Way springs, for modeling soils and other effects
- Euler members that take compression up to their buckling load, then turn off.
- Stress calculations on any arbitrary shape
- Inactive members, plates, and diaphragms allows you to quickly remove parts of structures from consideration
- Story drift calculations provide relative drift and ratio to height
- Automatic self-weight calculations for members and plates
- Automatic subgrade soil spring generator

Graphics Features:

- Unlimited simultaneous model view windows
- Extraordinary "true to scale" rendering, even when drawing
- High-speed redraw algorithm for instant refreshing
- Dynamic scrolling stops right where you want
- Plot & print virtually everything with color coding & labeling
- Rotate, zoom, pan, scroll and snap views
- Saved views to quickly restore frequent or desired views
- Full render or wire-frame animations of deflected model and dynamic mode shapes with frame and speed control
- Animation of moving loads with speed control
- High quality customizable graphics printing

Design Features:

- Designs concrete, hot rolled steel, cold formed steel and wood
- ACI 1999/2002, BS 8110-97, CSA A23.3-94, IS456:2000, EC 2-1992 with consistent bar sizes through adjacent spans
- Exact integration of concrete stress distributions using parabolic or rectangular stress blocks
- Concrete beam detailing (Rectangular, T and L)
- Concrete column interaction diagrams
- Steel Design Codes: AISC ASD 9th, LRFD 2nd & 3rd, HSS Specification, CAN/CSA-S16.1-1994 & 2004, BS 5950-1-2000, IS 800-1984, Euro 3-1993 including local shape databases
- AISI 1999 cold formed steel design
- NDS 1991/1997/2001 wood design, including Structural Composite Lumber, multi-ply, full sawn
- Automatic spectra generation for UBC 1997, IBC 2000/2003
- Generation of load combinations: ASCE, UBC, IBC, BOCA, SBC, ACI
- Unbraced lengths for physical members that recognize connecting elements and full lengths of members
- Automatic approximation of K factors
- Tapered wide flange design with either ASD or LRFD codes
- Optimization of member sizes for all materials and all design codes, controlled by standard or user-defined lists of available sizes and criteria such as maximum depths
- Automatic calculation of custom shape properties
- Steel Shapes: AISC, HSS, CAN, ARBED, British, Euro, Indian, Chilean
- Light Gage Shapes: AISI, SSMA, Dale / Incor, Dietrich, MarinoWARE
- Wood Shapes: Complete NDS species/grade database
- Full seamless integration with RISAFoot (Ver 2 or better) for advanced footing design and detailing
- Plate force summation tool

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Structural Analysis – 90-ft CL&P Pole # 2683
Sprint Antenna Upgrade – CT54XC713
Brookfield, CT
April 18, 2013

Results Features:

- Graphic presentation of color-coded results and plotted designs
- Color contours of plate stresses and forces with quadratic smoothing, the contours may also be animated
- Spreadsheet results with sorting and filtering of: reactions, member & joint deflections, beam & plate forces/stresses, optimized sizes, code designs, concrete reinforcing, material takeoffs, frequencies and mode shapes
- Standard and user-defined reports
- Graphic member detail reports with force/stress/deflection diagrams and detailed design calculations and expanded diagrams that display magnitudes at any dialed location
- Saved solutions quickly restore analysis and design results.

GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM ~ PLS-TOWER

PLS-TOWER is a Microsoft Windows program for the analysis and design of steel latticed towers used in electric power lines or communication facilities. Both self-supporting and guyed towers can be modeled. The program performs design checks of structures under user specified loads. For electric power structures it can also calculate maximum allowable wind and weight spans and interaction diagrams between different ratios of allowable wind and weight spans.

Modeling Features:

- Powerful graphics module (stress usages shown in different colors)
- Graphical selection of joints and members allows graphical editing and checking
- Towers can be shown as lines, wire frames or can be rendered as 3-d polygon surfaces
- Can extract geometry and connectivity information from a DXF CAD drawing
- CAD design drawings, title blocks, drawing borders or photos can be tied to structure model
- XML based post processor interface
- Steel Detailing Neutral File (SDNF) export to link with detailing packages
- Can link directly to line design program PLS-CADD
- Automatic generation of structure files for PLS-CADD
- Databases of steel angles, rounds, bolts, guys, etc.
- Automatic generation of joints and members by symmetries and interpolations
- Automated mast generation (quickly builds model for towers that have regular repeating sections) via graphical copy/paste
- Steel angles and rounds modeled either as truss, beam or tension-only elements
- Guys are easily handled (can be modeled as exact cable elements)

Analysis Features:

- Automatic handling of tension-only members
- Automatic distribution of loads in 2-part suspension insulators (v-strings, horizontal vees, etc.)
- Automatic calculation of tower dead, ice, and wind loads as well as drag coefficients according to:
 - ASCE 74-1991
 - NESC 2002
 - NESC 2007
 - IEC 60826:2003
 - EN50341-1:2001 (CENELEC)
 - EN50341-3-9:2001 (UK NNA)
 - EN50341-3-17:2001 (Portugal NNA)
 - ESAA C(b)1-2003 (Australia)
 - TPNZ (New Zealand)
 - REE (Spain)
 - EIA/TIA 222-F
 - ANSI/TIA 222-G
 - CSA S37-01
- Automated microwave antenna loading as per EIA/TIA 222-F and ANSI/TIA 222-G
- Minimization of problems caused by unstable joints and mechanisms
- Automatic bandwidth minimization and ability to solve large problems
- Design checks according to (other standards can be added easily):
 - ASCE Standard 10-90

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- AS 3995 (Australian Standard 3995)
- BS 8100 (British Standard 8100)
- EN50341-1 (CENELEC, both empirical and analytical methods are available)
- ECCS 1985
- NGT-ECCS
- PN-90/B-03200
- EIA/TIA 222-F
- ANSI/TIA 222-G
- CSA S37-01
- EDF/RTE Resal
- IS 802 (India Standard 802)

Results Features:

- Design summaries printed for each group of members
 - Easy to interpret text, spreadsheet and graphics design summaries
 - Automatic determination of allowable wind and weight spans
 - Automatic determination of interaction diagrams between allowable wind and weight spans
 - Capability to batch run multiple tower configurations and consolidate the results
 - Automated optimum angle member size selection and bolt quantity determination
- Tool for interactive angle member sizing and bolt quantity determination.

Criteria for Design of PCS Facilities On or
Extending Above Metal Electric Transmission
Towers & Analysis of Transmission Towers
Supporting PCS Masts ⁽¹⁾

Introduction

This criteria is the result from an evaluation of the methods and loadings specified by the separate standards, which are used in designing telecommunications towers and electric transmission towers. That evaluation is detailed elsewhere, but in summary; the methods and loadings are significantly different. This criteria specifies the manner in which the appropriate standard is used to design PCS facilities including masts and brackets (hereafter referred to as "masts"), and to evaluate the electric transmission towers to support PCS masts. The intent is to achieve an equivalent level of safety and security under the extreme design conditions expected in Connecticut and Massachusetts.

ANSI Standard TIA/EIA-222 covering the design of telecommunications structures specifies a working strength/allowable stress design approach. This approach applies the loads from extreme weather loading conditions, and designs the structure so that it does not exceed some defined percentage of failure strength (allowable stress).

ANSI Standard C2-2007 (National Electrical Safety Code) covering the design of electric transmission metal structures is based upon an ultimate strength/yield stress design approach. This approach applies a multiplier (overload capacity factor) to the loads possible from extreme weather loading conditions, and designs the structure so that it does not exceed its ultimate strength (yield stress).

Each standard defines the details of how loads are to be calculated differently. Most of the NU effort in "unifying" both codes was to establish what level of strength each approach would provide, and then increasing the appropriate elements of each to achieve a similar level of security under extreme weather loadings.

Two extreme weather conditions are considered. The first is an extreme wind condition (hurricane) based upon a 50-year recurrence (2% annual probability). The second is a winter condition combining wind and ice loadings.

The following sections describe the design criteria for any PCS mast extending above the top of an electric transmission tower, and the analysis criteria for evaluating the loads on the transmission tower from such a mast from the lower portions of such a mast, and loads on the pre-existing electric lower portions of such a mast, and loads on the pre-existing electric transmission tower and the conductors it supports.

| Note 1: Prepared from documentation provide from Northeast Utilities.

P C S M a s t

The PCS facility (mast, external cable/trays, including the initial and any planned future support platforms, antennas, etc. extending the full height above the top level of the electric transmission structure) shall be designed in accordance with the provisions of TIA/EIA Standard 222 with two exceptions:

1. An 85 mph extreme wind speed shall be used for locations in all counties throughout the NU system.
2. The stress increase of TIA Section 3.1.1.1 is disallowed. The combined wind and ice condition shall consider ½" radial ice in combination with the wind load (0.75 Wi) as specified in TIA section 2.3.16.

E L E C T R I C T R A N S M I S S I O N T O W E R

The electric transmission tower shall be analyzed using yield stress theory in accordance with the attached table titled "NU Design Criteria". This specifies uniform loadings (different from the TIA loadings) on each of the following components of the installed facility:

- PCS mast for its total height above ground level, including the initial and planned future support platforms, antennas, etc. above the top of an electric transmission structure.
- Conductors and related devices and hardware.
- Electric transmission structure. The loads from the PCS facility and from the electric conductors shall be applied to the structure at conductor and PCS mast attachment points, where those loads transfer to the tower.

The uniform loadings and factors specified for the above components in the table are based upon the National Electrical Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to TIA and its loads and factors with the exceptions noted above. (Note that the NESC does not require the projected wind surfaces of structures and equipment to be increased by the ice covering.)

In the event that the electric transmission tower is not sufficient to support the additional loadings of the PCS mast, reinforcement will be necessary to upgrade the strength of the overstressed members.



Northeast Utilities Overhead Transmission Standards



Attachment A

NU Design Criteria

			Basic Wind Speed V (MPH)	Pressure Q (PSF)	Height Factor Kz	Gust Factor Gh	Load or Stress Factor	Force Coef - Shape Factor
Ice Condition	TIA/EIA	Antenna Mount	TIA	TIA (.75Wi)	TIA	TIA	TIA, Section 3.1.1.1 disallowed for connection design	TIA
	NESC Heavy	Tower/Pole Analysis with antennas extending above top of Tower/Pole (Yield Stress)	---	4	1.00	1.00	2.50	1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with Antennas below top of Tower/Pole (on two faces)	---	4	1.00	1.00	2.50	1.6 Flat Surfaces 1.3 Round Surfaces
		Conductors:	Conductor loads provided by NU					
High Wind Condition	TIA/EIA	Antenna Mount	85	TIA	TIA	TIA	TIA, Section 3.1.1.1 disallowed for connection design	TIA
	NESC Extreme Wind	Tower/Pole Analysis with antennas extending above top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading 1.25 x Gust Response Factor Height above ground level based on top of Mast/Antenna					1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with Antennas below top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading Height above ground level based on top of Tower/Pole					1.6 Flat Surfaces 1.3 Round Surfaces
		Conductors:	Conductor loads provided by NU					
NESC Extreme Ice with Wind Condition*		Tower/Pole Analysis with antennas extending above top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250D: Extreme Ice with Wind Loading 4PSF Wind Load 1.25 x Gust Response Factor Height above ground level based on top of Mast/Antenna					1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with Antennas below top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250D: Extreme Ice with Wind Loading 4PSF Wind Load Height above ground level based on top of Tower/Pole					1.6 Flat Surfaces 1.3 Round Surfaces
		Conductors:	Conductor loads provided by NU					

* Only for Structures Installed after 2007

Communication Antennas on Transmission Structures (CL&P & WMECo Only)

Northeast Utilities
Approved by: KMS (NU)

Design
NU Confidential Information

OTRM 059
Page 7 of 9

Rev.1
03/17/2011



Northeast Utilities Overhead Transmission Standards



Shape Factor Criteria shall be per TIA Shape Factors.

- 2) STEP 2 - The electric transmission structure analysis and evaluation shall be performed in accordance with NESC requirements and shall include the mast and antenna loads determined from NESC applied loading conditions (not TIA/EIA Loads) on the structure and mount as specified below, and shall include the wireless communication mast and antenna loads per NESC criteria)

The structure shall be analyzed using yield stress theory in accordance with Attachment A, "NU Design Criteria." This specifies uniform loadings (different from the TIA loadings) on each of the following components of the installed facility:

- a) Wireless communication mast for its total height above ground level, including the initial and any planned future equipment (Support Platforms, Antennas, TMA's etc.) above the top of an electric transmission structure.
- b) Conductors and related devices and hardware (wire loads will be provided by NU).
- c) Electric Transmission Structure
 - i) The loads from the wireless communication equipment components based on NESC and NU Criteria in Attachment A, and from the electric conductors shall be applied to the structure at conductor and wireless communication mast attachment points, where those loads transfer to the tower.
 - ii) Shape Factor Multiplier:

NESC Structure Shape	Cd
Polyround (for polygonal steel poles)	1.3
Flat	1.6
Open Lattice	3.2

- iii) When Coaxial Cables are mounted along side the pole structure, the shape multiplier shall be:

Mount Type	Cable Cd	Pole Cd
Coaxial Cables on outside periphery (One layer)	1.45	1.45
Coaxial Cables mounted on stand offs	1.6	1.3

- d) The uniform loadings and factors specified for the above components in Attachment A, "NU Design Criteria" are based upon the National Electric Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to the TIA and its loads and factors with the exceptions noted above.

Note: The NESC does not require ice load be included in the supporting structure. (Ice on conductors and shield wire only, and NU will provide these loads).

- e) Mast reaction loads shall be evaluated for local effects on the transmission structure members at the attachment points.

Communication Antennas on Transmission Structures (CL&P & WMECo Only)

Northeast Utilities Approved by: KMS (NU)	Design NU Confidential Information	OTRM 059	Rev.1 03/17/2011
		Page 3 of 9	

Wire Ld

TITLE _____
STRUCT CL&P #2683

06/05/2000

CONDUCTOR

	AHEAD	BACK
	LINNET	LINNET
	336	336
	26/7 ACSR	26/7 ACSR
DIAM =	0.720	0.720
WEIGHT =	0.462	0.462
TENSION (LBS)	AHEAD 4,000	BACK 4,000

LOADCASE	NESC HEAVY
WIND (PSF)	4
ICE (IN)	0.50
OLF ANG	1.65
OLF WIND	2.50
OLF WT	1.50

STR	ANGLE	WIND SPAN	WGT SPAN	NESC HEAVY		
				H	L	V
BACK	7	243	345	1152	-6551	632
AHEAD	7	243	345	1153	6551	632
TOTALS	14.0	486	690	2305	0	1263

Wire Ld

TITLE _____
STRUCT CL&P #2683

06/05/2000

CONDUCTOR

AHEAD		BACK	
LINNET	▼	LINNET	▼
336		336	
26/7 ACSR		26/7 ACSR	
DIAM =	0.720		0.720
WEIGHT =	0.462		0.462
TENSION (LBS)	AHEAD 2,904		BACK 2,904

LOADCASE	HI WIND	▼
WIND (PSF)	20	
ICE (IN)	0.00	
OLF ANG	1.15	
OLF WIND	1.15	
OLF WT	1.15	

STR	ANGLE	WIND SPAN	WGT SPAN	HI WIND		
				H	L	V
BACK	7	243	345	742	-3315	183
AHEAD	7	243	345	742	3315	183
TOTALS	14.0	486	690	1484	0	367

Wire Ld

TITLE _____
STRUCT CL&P #2683

06/05/2000

CONDUCTOR SHIELD WIRE

	AHEAD	BACK
	3/8 AW	3/8 AW
	0.000	0.000
	7 #8 Al Weld	7 #8 Al Weld
DIAM =	0.385	0.385
WEIGHT =	0.262	0.262
TENSION (LBS)	AHEAD 2,749	BACK 2,749

LOADCASE	HI WIND
WIND (PSF)	20
ICE (IN)	0.00
OLF ANG	1.15
OLF WIND	1.15
OLF WT	1.15

STR	ANGLE	WIND SPAN	WGT SPAN	HI WIND		
				H	L	V
BACK	7	243	345	564	-3138	104
AHEAD	7	243	345	565	3138	104
TOTALS	14.0	486	690	1129	0	208

Wire Ld

TITLE _____
STRUCT CL&P #2683

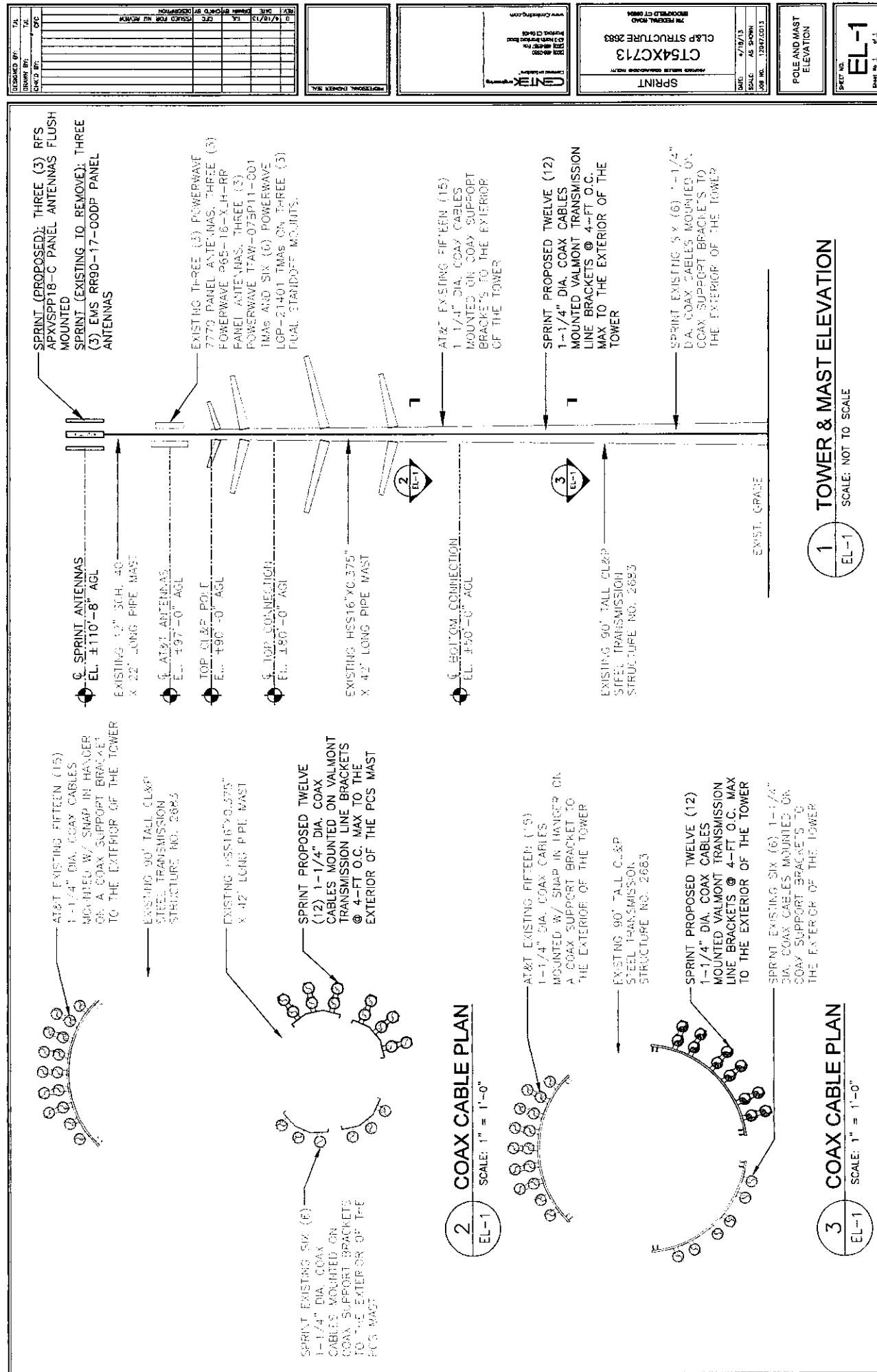
06/05/2000

CONDUCTOR SHIELD WIRE

	AHEAD	BACK
	3/8 AW	3/8 AW
	0.000	0.000
	7 #8 Al Weld	7 #8 Al Weld
DIAM =	0.385	0.385
WEIGHT =	0.262	0.262
TENSION (LBS)	AHEAD 4,200	BACK 4,200

LOADCASE	NESC HEAVY
WIND (PSF)	4
ICE (IN)	0.50
OLF ANG	1.65
OLF WIND	2.50
OLF WT	1.50

STR	ANGLE	WIND SPAN	WGT SPAN	NESC HEAVY		
				H	L	V
BACK	7	243	345	1125	-6878	420
AHEAD	7	243	345	1125	6878	420
TOTALS	14.0	486	690	2250	0	840



Subject:

Load Analysis of Pipe Mast on CL&P
 Structure #2683

Location:

Brookfield, CT

Rev. 0: 4/17/13

Prepared by: T.J.L. Checked by: C.F.C.
 Job No. 12047.CO13

**Development of Design Heights, Exposure Coefficients,
 and Velocity Pressures Per TIA/EIA**

Wind Speeds

Basic Wind Speed	$V := 85$	mph	(User Input per NU Mast Design Criteria Exception 1)
Basic Wind Speed with Ice	$V_i := 74$	mph	(User Input per TIA/EIA-222-F Section 2.3.16)

Heights above ground level, z

Mast 1	$z_{mast1} := 103$	ft	(User Input)
Mast 2	$z_{mast2} := 71$	ft	(User Input)
Sprint	$z_{Sprint} := 110.67$	ft	(User Input)
AT&T	$z_{AT\&T} := 97$	ft	(User Input)
Coax Cable	$z_{coax} := 82$	ft	(User Input)

Exposure Coefficients, k_z

(per TIA/EIA-222-F Section 2.3.3)

Mast 1	$Kz_{mast1} := \left(\frac{z_{mast1}}{33} \right)^{\frac{2}{7}} = 1.384$
Mast 2	$Kz_{mast2} := \left(\frac{z_{mast2}}{33} \right)^{\frac{2}{7}} = 1.245$
Sprint	$Kz_{Sprint} := \left(\frac{z_{Sprint}}{33} \right)^{\frac{2}{7}} = 1.413$
AT&T	$Kz_{AT\&T} := \left(\frac{z_{AT\&T}}{33} \right)^{\frac{2}{7}} = 1.361$
Coax Cable	$Kz_{coax} := \left(\frac{z_{coax}}{33} \right)^{\frac{2}{7}} = 1.297$

Subject:

Load Analysis of Pipe Mast on CL&P
Structure #2683

Location:

Brookfield, CT

Rev. 0: 4/17/13

Prepared by: T.J.L. Checked by: C.F.C.
Job No. 12047.CO13

Velocity Pressure without ice, q_z

(per TIA/EIA-222-F Section 2.3.3)

Mast 1	$q_{z_{mast1}} := 0.00256 \cdot K_{z_{mast1}} \cdot V^2 = 25.604$
Mast 2	$q_{z_{mast2}} := 0.00256 \cdot K_{z_{mast2}} \cdot V^2 = 23.022$
Sprint	$q_{z_{Sprint}} := 0.00256 \cdot K_{z_{Sprint}} \cdot V^2 = 26.135$
AT&T	$q_{z_{AT\&T}} := 0.00256 \cdot K_{z_{AT\&T}} \cdot V^2 = 25.169$
Coax Cable	$q_{z_{coax}} := 0.00256 \cdot K_{z_{coax}} \cdot V^2 = 23.989$

Velocity Pressure with ice, q_{zICE}

(per TIA/EIA-222-F Section 2.3.3)

Mast 1	$q_{zICE_{mast1}} := 0.00256 \cdot K_{z_{mast1}} \cdot V_i^2 = 19.406$
Mast 2	$q_{zICE_{mast2}} := 0.00256 \cdot K_{z_{mast2}} \cdot V_i^2 = 17.449$
Sprint	$q_{zICE_{Sprint}} := 0.00256 \cdot K_{z_{Sprint}} \cdot V_i^2 = 19.808$
AT&T	$q_{zICE_{AT\&T}} := 0.00256 \cdot K_{z_{AT\&T}} \cdot V_i^2 = 19.076$
Coax Cable	$q_{zICE_{coax}} := 0.00256 \cdot K_{z_{coax}} \cdot V_i^2 = 18.182$

TIA/EIA Common Factors:

Gust Response Factor =	$G_H := 1.69$	(User Input per TIA/EIA-222-F Section 2.3.4)
Gust Response Factor Multiplier =	$m := 1.25$	(User Input per TIA/EIA-222-F Section 2.3.4.4)
Radial Ice Thickness =	$I_r := 0.50$ in	(User Input per TIA/EIA-222-F Section 2.3.1)
Radial Ice Density =	$I_d := 56.00$ pcf	(User Input)

Development of Wind & Ice Load on PCS Mast

(per TIA/EIA-222-F-1996 Criteria)

Existing Upper Mast Data

Mast Shape =

(Pipe 12 STD)

(User Input)

Mast Diameter =

Round

(User Input)

$$D_{\text{mast}} := 12.8 \text{ in} \quad (\text{User Input})$$

Mast Length =

$$L_{\text{mast}} := 22.417 \text{ ft} \quad (\text{User Input})$$

Mast Thickness =

$$t_{\text{mast}} := 0.375 \text{ in} \quad (\text{User Input})$$

Mast Aspect Ratio =

$$A_{r_{\text{mast}}} := \frac{12L_{\text{mast}}}{D_{\text{mast}}} = 21.0$$

Mast Force Coefficient =

$$C_{a_{\text{mast}}} = 1.11 \quad (\text{per TIA/EIA-222-F Table 3})$$

Wind Load (without ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

Mast Projected Surface Area =

$$A_{\text{mast}} := \frac{D_{\text{mast}}}{12} = 1.067 \text{ sf/ft}$$

Total Mast Wind Force =

$$qz_{\text{mast}1} \cdot G_H \cdot C_{a_{\text{mast}}} \cdot A_{\text{mast}} = 51 \text{ plf} \quad \text{BLC 5}$$

Wind Load (with ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

Mast Projected Surface Area w/ Ice =

$$A_{\text{ICE}_{\text{mast}}} := \frac{(D_{\text{mast}} + 2 \cdot I_r)}{12} = 1.15 \text{ sf/ft}$$

Total Mast Wind Force w/ Ice =

$$qz_{\text{ICE}_{\text{mast}1}} \cdot G_H \cdot C_{a_{\text{mast}}} \cdot A_{\text{ICE}_{\text{mast}}} = 42 \text{ plf} \quad \text{BLC 4}$$

Gravity Loads (without ice)

Weight of the mast =

$$\text{Self Weight} \quad (\text{Computed internally by Risa-3D}) \quad \text{plf} \quad \text{BLC 1}$$

Gravity Loads (ice only)

Ice Area per Linear Foot =

$$A_{i_{\text{mast}}} := \frac{\pi}{4} \left[(D_{\text{mast}} + I_r \cdot 2)^2 - D_{\text{mast}}^2 \right] = 20.9 \text{ sq in}$$

Weight of Ice on Mast =

$$W_{\text{ICE}_{\text{mast}}} := I_d \cdot \frac{A_{i_{\text{mast}}}}{144} = 8 \text{ plf} \quad \text{BLC 3}$$

Development of Wind & Ice Load on PCS Mast

(per TIA/EIA-222-F-1996 Criteria)

Existing Lower Mast Data

Mast Shape =	Round	(User Input)
Mast Diameter =	$D_{\text{mast}} := 16.0$ in	(User Input)
Mast Length =	$L_{\text{mast}} := 41.583$ ft	(User Input)
Mast Thickness =	$t_{\text{mast}} := 0.375$ in	(User Input)
Mast Aspect Ratio =	$Ar_{\text{mast}} := \frac{12L_{\text{mast}}}{D_{\text{mast}}} = 31.2$	
Mast Force Coefficient =	$Ca_{\text{mast}} = 1.2$	(per TIA/EIA-222-F Table 3)

Wind Load (without ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

Mast Projected Surface Area =	$A_{\text{mast}} := \frac{D_{\text{mast}}}{12} = 1.333$	sf/ft
-------------------------------	---	-------

Total Mast Wind Force =

$qz_{\text{mast}}^2 \cdot G_H \cdot Ca_{\text{mast}} \cdot A_{\text{mast}} = 62$	plf	BLC 5
--	-----	--------------

Wind Load (with ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

Mast Projected Surface Area w/ Ice =	$A_{\text{ICE}_{\text{mast}}} := \frac{(D_{\text{mast}} + 2 \cdot Ir)}{12} = 1.417$	sf/ft
--------------------------------------	---	-------

Total Mast Wind Force w/ Ice =

$qz_{\text{ICE}_{\text{mast}}}^2 \cdot G_H \cdot Ca_{\text{mast}} \cdot A_{\text{ICE}_{\text{mast}}} = 50$	plf	BLC 4
--	-----	--------------

Gravity Loads (without ice)

Weight of the mast =

Self Weight	(Computed internally by Risa-3D)	plf	BLC 1
-------------	----------------------------------	-----	--------------

Gravity Loads (ice only)

Ice Area per Linear Foot =

$A_{i_{\text{mast}}} := \frac{\pi}{4} \left[(D_{\text{mast}} + Ir \cdot 2)^2 - D_{\text{mast}}^2 \right] = 25.9$	sq in
---	-------

Weight of Ice on Mast =

$W_{\text{ICE}_{\text{mast}}} := Id \cdot \frac{A_{i_{\text{mast}}}}{144} = 10$	plf	BLC 3
---	-----	--------------

Development of Wind & Ice Load on Antennas

Existing Antenna Data:

Antenna Model =
 Antenna Shape =
 Antenna Height =
 Antenna Width =
 Antenna Thickness =
 Antenna Weight =
 Number of Antennas =
 Antenna Aspect Ratio =
 Antenna Force Coefficient =

(per TIA/EIA-222-F-1996 Criteria)

(Sprint)

RFS APX VSPP18-C

Flat (User Input)

$L_{ant} := 72$ in (User Input)

$W_{ant} := 11.8$ in (User Input)

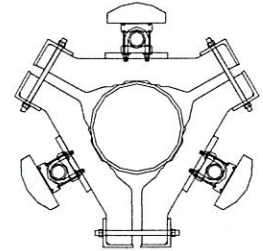
$T_{ant} := 7$ in (User Input)

$WT_{ant} := 57$ lbs (User Input)

$N_{ant} := 3$ (User Input)

$Ar_{ant} := \frac{L_{ant}}{W_{ant}} = 6.1$

$Ca_{ant} = 1.4$ (per TIA/EIA-222-F-1996 Table 3)



Wind Load (without ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

*Assumes Maximum Possible Wind Pressure
 Applied to all Antennas Simultaneously*

Surface Area for One Antenna =

$SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 5.9$ sf

Antenna Projected Surface Area =

$A_{ant} := SA_{ant} \cdot N_{ant} = 17.7$ sf

Total Antenna Wind Force =

$F_{ant} := qz_{Sprint} \cdot G_H \cdot Ca_{ant} \cdot A_{ant} = 1094$ lbs **BLC 5**

Wind Load (with ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

*Assumes Maximum Possible Wind Pressure
 Applied to all Antennas Simultaneously*

Surface Area for One Antenna w/ Ice =

$SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 6.5$ sf

Antenna Projected Surface Area w/ Ice =

$A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 19.5$ sf

Total Antenna Wind Force w/ Ice =

$F_{ICEant} := qz_{ICE} \cdot G_H \cdot Ca_{ant} \cdot A_{ICEant} = 912$ lbs **BLC 4**

Gravity Load (without ice)

Weight of All Antennas =

$WT_{ant} \cdot N_{ant} = 171$ lbs **BLC 2**

Gravity Loads (ice only)

Volume of Each Antenna =

$V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 5947$ cu in

Volume of Ice on Each Antenna =

$V_{ice} := (L_{ant} + 1) \cdot (W_{ant} + 1) \cdot (T_{ant} + 1) - V_{ant} = 1528$ cu in

Weight of Ice on Each Antenna =

$W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 50$ lbs

Weight of Ice on All Antennas =

$W_{ICEant} \cdot N_{ant} = 149$ lbs **BLC 3**

Development of Wind & Ice Load on Antenna Mounts

Mount Data:

Mount Type:

(per TIA/EIA-222-F-1996 Criteria)

(Sprint)

Microflect Universal Tri-Bracket

Mount Shape =

Flat (User Input)

Pipe Mount Length =

$L_{mnt} := 72$ in (User Input)

2 inch Pipe Mount Linear Weight =

$W_{mnt} := 3.66$ plf (User Input)

Pipe Mount Outside Diameter =

$D_{mnt} := 2.375$ in (User Input)

Number of Mounting Pipes =

$N_{mnt} := 3$ (User Input)

Tri Bracket Weight =

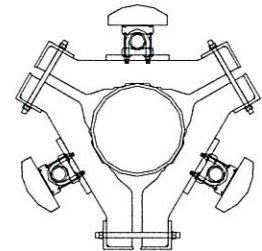
$W_{tb.mnt} := 197$ lbs (User Input)

Mount Aspect Ratio =

$Ar_{mnt} := \frac{L_{mnt}}{D_{mnt}} = 30$

Mount Force Coefficient =

$Ca_{mnt} = 2$ (per TIA/EIA-222-F Table 3)



Wind Load (without ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

Assumes Mount is Shielded by Antenna

Mount Projected Surface Area =

$A_{mnt} := 0.0$ sf

Total Mount Wind Force =

$F_{mnt} := qz_{Sprint} \cdot G_H \cdot Ca_{mnt} \cdot A_{mnt} = 0$ lbs **BLC 5**

Wind Load (with ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

Assumes Mount is Shielded by Antenna

Mount Projected Surface Area w/ Ice =

$A_{ICEmnt} := 0.0$ sf

Total Mount Wind Force =

$F_{mnt} := qz_{ICE} \cdot G_H \cdot Ca_{mnt} \cdot A_{ICEmnt} = 0$ lbs **BLC 4**

Gravity Loads (without ice)

(per TIA/EIA-222-F-1996)

Weight Each Pipe Mount =

$WT_{mnt} := W_{mnt} \cdot \frac{L_{mnt}}{12} = 22$ lbs

Weight of All Mounts =

$WT_{mnt} \cdot N_{mnt} + W_{tb.mnt} = 263$ lbs **BLC 2**

Gravity Loads (ice only)

(per TIA/EIA-222-F-1996)

Volume of Each Pipe =

$V_{mnt} := \frac{\pi}{4} \cdot D_{mnt}^2 \cdot L_{mnt} = 319$ cu in

Volume of Ice on Each Pipe =

$V_{ice} := \left[\frac{\pi}{4} \cdot \left[(D_{mnt} + 1)^2 \right] \cdot (L_{mnt} + 1) \right] - V_{mnt} = 334$ cu in

Weight of Ice each mount (incl. hardware) =

$W_{ICEmnt} := \frac{V_{ice}}{1728} \cdot Id = 11$ lbs

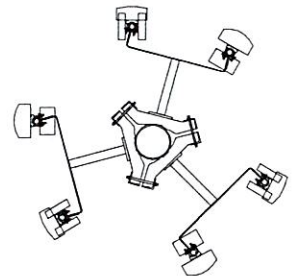
Weight of Ice on All Mounts =

$W_{ICEmnt} \cdot N_{mnt} + 5 = 37$ lbs **BLC 3**

Development of Wind & Ice Load on Antennas

Existing Antenna Data:

Antenna Model =	Powerwave 7770.00	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 55$	in (User Input)
Antenna Width =	$W_{ant} := 11$	in (User Input)
Antenna Thickness =	$T_{ant} := 5$	in (User Input)
Antenna Weight =	$WT_{ant} := 39$	lbs (User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)
Antenna Aspect Ratio =	$Ar_{ant} := \frac{L_{ant}}{W_{ant}} = 5.0$	
Antenna Force Coefficient =	$Ca_{ant} = 1.4$	(per TIA/EIA-222-F-1996 Table 3)



Wind Load (without ice)

*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna =	$SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 4.2$	sf
Antenna Projected Surface Area =	$A_{ant} := SA_{ant} \cdot N_{ant} = 12.6$	sf

Total Antenna Wind Force =

$$F_{ant} := qz_{AT\&T} \cdot G_H \cdot Ca_{ant} \cdot A_{ant} = 751 \quad \text{lbs} \quad \text{BLC 5}$$

Wind Load (with ice)

*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna w/ Ice =	$SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 4.7$	sf
Antenna Projected Surface Area w/ Ice =	$A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 14$	sf

Total Antenna Wind Force w/ Ice =

$$F_{ant} := qz_{ICEAT\&T} \cdot G_H \cdot Ca_{ant} \cdot A_{ICEant} = 632 \quad \text{lbs} \quad \text{BLC 4}$$

Gravity Load (without ice)

Weight of All Antennas =

$$WT_{ant} \cdot N_{ant} = 117 \quad \text{lbs} \quad \text{BLC 2}$$

Gravity Loads (ice only)

Volume of Each Antenna =	$V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 3025$	cu in
Volume of Ice on Each Antenna =	$V_{ice} := (L_{ant} + 1) \cdot (W_{ant} + 1) \cdot (T_{ant} + 1) - V_{ant} = 1007$	cu in
Weight of Ice on Each Antenna =	$W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 33$	lbs
Weight of Ice on All Antennas =	$W_{ICEant} \cdot N_{ant} = 98$	lbs BLC 3

Development of Wind & Ice Load on Antennas

Proposed Antenna Data:

Antenna Model =
Antenna Shape =
Antenna Height =
Antenna Width =
Antenna Thickness =
Antenna Weight =
Number of Antennas =
Antenna Aspect Ratio =
Antenna Force Coefficient =

(per TIA/EIA-222-F-1996 Criteria)

(AT&T)

Powerwave P65-16-XLH-RR

Flat (User Input)

$L_{ant} := 72$ in (User Input)

$W_{ant} := 12$ in (User Input)

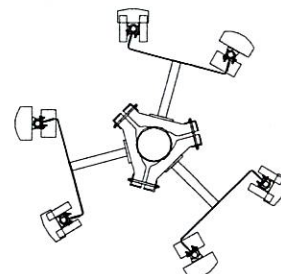
$T_{ant} := 6$ in (User Input)

$WT_{ant} := 64$ lbs (User Input)

$N_{ant} := 3$ (User Input)

$Ar_{ant} := \frac{L_{ant}}{W_{ant}} = 6.0$

$Ca_{ant} = 1.4$ (per TIA/EIA-222-F-1996 Table 3)



Wind Load (without ice)

*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna =

$SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 6$ sf

Antenna Projected Surface Area =

$A_{ant} := SA_{ant} \cdot N_{ant} = 18$ sf

Total Antenna Wind Force =

$F_{ant} := qz_{AT\&T} \cdot G_H \cdot Ca_{ant} \cdot A_{ant} = 1072$ lbs **BLC 5**

Wind Load (with ice)

*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna w/ Ice =

$SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 6.6$ sf

Antenna Projected Surface Area w/ Ice =

$A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 19.8$ sf

Total Antenna Wind Force w/ Ice =

$F_{ant} := qz_{ICE} \cdot G_H \cdot Ca_{ant} \cdot A_{ICEant} = 892$ lbs **BLC 4**

Gravity Load (without ice)

Weight of All Antennas =

$WT_{ant} \cdot N_{ant} = 192$ lbs **BLC 3**

Gravity Loads (ice only)

Volume of Each Antenna =

$V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 5184$ cu in

Volume of Ice on Each Antenna =

$V_{ice} := (L_{ant} + 1) \cdot (W_{ant} + 1) \cdot (T_{ant} + 1) - V_{ant} = 1459$ cu in

Weight of Ice on Each Antenna =

$W_{ICEant} := \frac{V_{ice}}{1728} \cdot \rho_d = 47$ lbs

Weight of Ice on All Antennas =

$W_{ICEant} \cdot N_{ant} = 142$ lbs **BLC 3**

Development of Wind & Ice Load on TMA's

Existing TMA Data:

TMA Model =
TMA Shape =
TMA Height =
TMA Width =
TMA Thickness =
TMA Weight =
Number of TMA's =
TMA Aspect Ratio =
TMA Force Coefficient =

(per TIA/EIA-222-F-1996 Criteria)

(AT&T)

Powerwave LGP 21401

Flat (User Input)

$L_{TMA} := 14.4$ in (User Input)

$W_{TMA} := 9.2$ in (User Input)

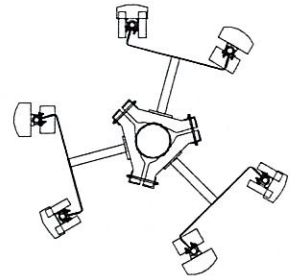
$T_{TMA} := 2.6$ in (User Input)

$WT_{TMA} := 14.1$ lbs (User Input)

$N_{TMA} := 6$ (User Input)

$Ar_{TMA} := \frac{L_{TMA}}{W_{TMA}} = 1.6$

$Ca_{TMA} = 1.4$ (per TIA/EIA-222-F Table 3)



Wind Load (without ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

*Assumes Maximum Possible Wind Pressure
Applied to All TMA's Simultaneously*

Surface Area for One TMA =

$SA_{TMA} := \frac{L_{TMA} \cdot W_{TMA}}{144} = 0.9$ sf

TMA Projected Surface Area =

$A_{TMA} := SA_{TMA} \cdot N_{TMA} = 5.5$ sf

Total TMA Wind Force =

$F_{TMA} := qz_{AT\&T} \cdot G_H \cdot Ca_{TMA} \cdot A_{TMA} = 329$ lbs **BLC 5**

Wind Load (with ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

*Assumes Maximum Possible Wind Pressure
Applied to All TMA's Simultaneously*

Surface Area for One TMA w/ Ice =

$SA_{ICETMA} := \frac{(L_{TMA} + 1) \cdot (W_{TMA} + 1)}{144} = 1.1$ sf

TMA Projected Surface Area w/ Ice =

$A_{ICETMA} := SA_{ICETMA} \cdot N_{TMA} = 6.5$ sf

Total TMA Wind Force w/ Ice =

$F_{ITMA} := qz_{ICE} \cdot G_H \cdot Ca_{TMA} \cdot A_{ICETMA} = 295$ lbs **BLC 4**

Gravity Load (without ice)

Weight of All TMA's =

$WT_{TMA} \cdot N_{TMA} = 85$ lbs **BLC 2**

Gravity Load (ice only)

Volume of Each TMA =

$V_{TMA} := L_{TMA} \cdot W_{TMA} \cdot T_{TMA} = 344$ cu in

Volume of Ice on Each TMA =

$V_{ice} := (L_{TMA} + 1) \cdot (W_{TMA} + 1) \cdot (T_{TMA} + 1) - V_{TMA} = 221$ cu in

Weight of Ice on Each TMA =

$W_{ICETMA} := \frac{V_{ice}}{1728} \cdot Id = 7$ lbs

Weight of Ice on All TMA's

$W_{ICETMA} \cdot N_{TMA} = 43$ lbs **BLC 3**

Development of Wind & Ice Load on TMA's

Proposed TMA Data:

TMA Model =
TMA Shape =
TMA Height =
TMA Width =
TMA Thickness =
TMA Weight =
Number of TMA's =
TMA Aspect Ratio =
TMA Force Coefficient =

(per TIA/EIA-222-F-1996 Criteria)

(AT&T)

Powerwave TTAW-07BP111-001

Flat (User Input)

$L_{TMA} := 9.9$ in (User Input)

$W_{TMA} := 6.7$ in (User Input)

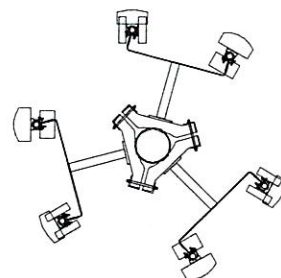
$T_{TMA} := 5.4$ in (User Input)

$WT_{TMA} := 18$ lbs (User Input)

$N_{TMA} := 3$ (User Input)

$Ar_{TMA} := \frac{L_{TMA}}{W_{TMA}} = 1.5$

$Ca_{TMA} = 1.4$ (per TIA/EIA-222-F Table 3)



Wind Load (without ice)

*Assumes Maximum Possible Wind Pressure
Applied to All TMA's Simultaneously*

Surface Area for One TMA =

$SA_{TMA} := \frac{L_{TMA} \cdot W_{TMA}}{144} = 0.5$ sf

TMA Projected Surface Area =

$A_{TMA} := SA_{TMA} \cdot N_{TMA} = 1.4$ sf

Total TMA Wind Force =

$F_{TMA} := qz_{AT\&T} \cdot G_H \cdot Ca_{TMA} \cdot A_{TMA} = 82$ lbs **BLC 5**

Wind Load (with ice)

*Assumes Maximum Possible Wind Pressure
Applied to All TMA's Simultaneously*

Surface Area for One TMA w/ Ice =

$SA_{ICETMA} := \frac{(L_{TMA} + 1) \cdot (W_{TMA} + 1)}{144} = 0.6$ sf

TMA Projected Surface Area w/ Ice =

$A_{ICETMA} := SA_{ICETMA} \cdot N_{TMA} = 1.7$ sf

Total TMA Wind Force w/ Ice =

$F_{iTMA} := qz_{ICE} \cdot G_H \cdot Ca_{TMA} \cdot A_{ICETMA} = 79$ lbs **BLC 4**

Gravity Load (without ice)

Weight of All TMA's =

$WT_{TMA} \cdot N_{TMA} = 54$ lbs **BLC 2**

Gravity Load (ice only)

Volume of Each TMA =

$V_{TMA} := L_{TMA} \cdot W_{TMA} \cdot T_{TMA} = 358$ cu in

Volume of Ice on Each TMA =

$V_{ice} := (L_{TMA} + 1) \cdot (W_{TMA} + 1) \cdot (T_{TMA} + 1) - V_{TMA} = 179$ cu in

Weight of Ice on Each TMA =

$W_{ICETMA} := \frac{V_{ice}}{1728} \cdot Id = 6$ lbs

Weight of Ice on All TMA's

$W_{ICETMA} \cdot N_{TMA} = 17$ lbs **BLC 3**

Subject:

Load Analysis of Pipe Mast on CL&P
Structure #2683

Location:

Brookfield, CT

Rev. 0: 4/17/13

Prepared by: T.J.L. Checked by: C.F.C.
Job No. 12047.CO13

Development of Wind & Ice Load on Antenna Mounts

(per TIA/EIA-222-F-1996 Criteria)

Mount Data:

(AT&T)

Mount Type:

(3) Valmont Dual Standoff
Mounts w/ Universal Tr-B Racket

Mount Shape =

Flat (User Input)

Mount Area =

$A_{mnt} := 4.0$ sq ft (User Input)

Mount Area w/ Ice =

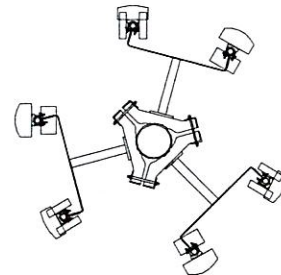
$A_{ICE.mnt} := 4.6$ sq ft (User Input)

Mount Weight =

$WT_{mnt} := 610$ lbs (User Input)

Mount Weight w/ Ice =

$WT_{ICE.mnt} := 700$ lbs (User Input)



Wind Load (without ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

Total Mount Wind Force =

$F_{mnt} := qz_{AT\&T} \cdot G_H \cdot C_{a_{mnt}} \cdot A_{mnt} = 238$ lbs **BLC 5**

Wind Load (with ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

Total Mount Wind Force w/ Ice =

$F_{mnt} := qz_{ICE} \cdot qz_{AT\&T} \cdot G_H \cdot C_{a_{mnt}} \cdot A_{ICE.mnt} = 208$ lbs **BLC 4**

Gravity Load (without ice)

Weight of Mount =

$WT_{mnt} = 610$ lbs **BLC 2**

Gravity Loads (ice only)

Weight of Ice on Mount =

$WT_{ICE.mnt} - WT_{mnt} = 90$ lbs **BLC 3**

Development of Wind & Ice Load on Coax Cables

Existing Coax Cable Data:

Coax Type =
Shape =
Coax Outside Diameter =
Coax Cable Length =
Weight of Coax per foot =
Total Number of Coax =
No. of Coax Projecting Outside Face of PCS Mast =

Coax aspect ratio,

Coax Cable Force Factor Coefficient =

per TIA/EIA-222-F-96 Criteria

(Sprint)

HELIAX 1-1/4"

Round (User Input)

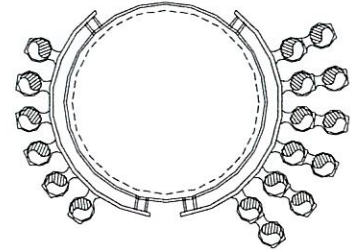
$D_{coax} := 1.55$ in (User Input)

$L_{coax} := 64$ ft (User Input)

$Wt_{coax} := 0.66$ plf (User Input)

$N_{coax} := 18$ (User Input)

$NP_{coax} := 3$ (User Input)



Note: AT&T Existing/Proposed
cables attached to CL&P Pole

$$Ar_{coax} := \frac{(L_{coax} \cdot 12)}{D_{coax}} = 495.5$$

$Ca_{coax} = 1.2$ TIA/EIA-222-F-96 Table 3

Wind Load (without ice)

Coax projected surface area =

per TIA/EIA-222-F-96 Section 2.3.2

$$A_{coax} := \frac{(NP_{coax} \cdot D_{coax})}{12} = 0.4 \text{ ft}$$

Total Coax Wind Force =

$$F_{coax} := Ca_{coax} \cdot qz_{coax} \cdot G_H \cdot A_{coax} = 19 \text{ plf} \quad \text{BLC 5}$$

Wind Load (with ice)

Coax projected surface area w/ Ice =

per TIA/EIA-222-F-96 Section 2.3.2

$$AICE_{coax} := \frac{(NP_{coax} \cdot D_{coax} + 2 \cdot Ir)}{12} = 0.5 \text{ ft}$$

Total Coax Wind Force w/ Ice =

$$F_{i_{coax}} := Ca_{coax} \cdot qz_{ICE_{coax}} \cdot G_H \cdot AICE_{coax} = 17 \text{ plf} \quad \text{BLC 4}$$

Gravity Loads (without ice)

Weight of all cables w/o ice

$$WT_{coax} := Wt_{coax} \cdot N_{coax} = 12 \text{ plf} \quad \text{BLC 2}$$

Gravity Loads (ice only)

Ice Area per Linear Foot =

$$Ai_{coax} := \frac{\pi}{4} [(D_{coax} + 2 \cdot Ir)^2 - D_{coax}^2] = 3.2 \text{ sq in}$$

Ice Weight All Coax per foot =

$$WT_{i_{coax}} := N_{coax} \cdot Id \cdot \frac{Ai_{coax}}{144} = 23 \text{ plf} \quad \text{BLC 3}$$

Development of Wind & Ice Load on Brace Member

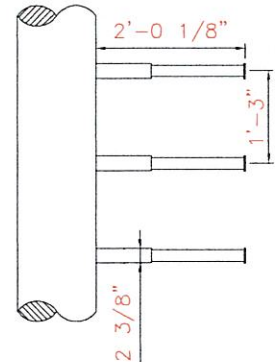
Member Data:

Shape =
Diameter =
Length =
Spacing =
Weight =

(per TIA/EIA-222-F-1996 Criteria)

Step Bolts

Round (User Input)
 $D_{mem} := 2.38$ in (User Input)
 $L_{mem} := 2$ ft (User Input)
 $Sp_{mem} := 1.25$ ft (User Input)
 $Wt_{mem} := 3.66$ plf



Member Aspect Ratio =

$$Ar_{mem} := \frac{D_{mem}}{L_{mem} \cdot 12} = 0.1$$

Member Force Coefficient =

$Ca_{mem} = 1.4$ (per TIA/EIA-222-F-1996 Table 3)

Wind Load (without ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

Member Projected Surface Area =

$$A_{mem} := \frac{D_{mem} \cdot L_{mem}}{12 \cdot Sp_{mem}} = 0.3 \text{ sf/ft}$$

Total Member Wind Force =

$$F_{mem} := qz_{mast1} \cdot G_H \cdot Ca_{mem} \cdot A_{mem} = 19 \text{ lbs BLC 5}$$

Wind Load (with ice)

(per TIA/EIA-222-F-1996 Section 2.3.2)

Member Projected Surface Area w/ Ice =

$$A_{ICEmem} := \frac{(D_{mem} + 2 \cdot Ir) \cdot (L_{mem} + 2 \cdot Ir)}{12 \cdot Sp_{mem}} = 0.7 \text{ plf}$$

Total Member Wind Force w/ Ice =

$$F_{mem} := qz_{ICE mast1} \cdot G_H \cdot Ca_{mem} \cdot A_{ICEmem} = 31 \text{ lbs BLC 4}$$

Gravity Load (without ice)

Weight of Member =

$$\frac{Wt_{mem} \cdot L_{mem}}{Sp_{mem}} = 6 \text{ lbs BLC 2}$$

Gravity Loads (ice only)

Ice Area per Linear foot =

$$Ai_{mem} := \frac{1}{4} \cdot \pi \cdot [(D_{mem} + 2 \cdot Ir)^2 - D_{mem}^2] = 4.5 \text{ sq in}$$

Weight of Ice on Member =

$$W_{ICEmem} := Id \cdot \frac{Ai_{mem}}{144} \cdot \frac{L_{mem}}{Sp_{mem}} = 3 \text{ lbs BLC 3}$$

CEN TEK engineering, INC. Consulting Engineers 63-2 North Branford Road Branford, CT 06405 Ph. 203-488-0580 / Fax. 203-488-8587	Subject: Analysis of TIA/EIA Wind and Ice Loads for Analysis of Mast Only Tabulated Load Cases Location: Brookfield, CT Date: 4/17/13	Prepared by: T.J.L. Checked by: C.F.C. Job No. 12047.CO13
Load Case	Description	
1	Self Weight (Mast)	
2	Weight of Appurtenances	
3	Weight of Ice Only on PCS Structure ⁽¹⁾	
4	TIA/EIA Wind with Ice on PCS Structure ⁽¹⁾	
5	TIA/EIA Wind on PCS Structure ⁽¹⁾	
Footnotes: (1) PCS Structure includes: Mast and Appurtenances		

CEN TEK engineering, INC.
Consulting Engineers

63-2 North Branford Road
 Branford, CT 06405

Ph. 203-488-0580 / Fax. 203-488-8587

Subject: Analysis of TIA/EIA Wind and Ice Loads for Analysis of Mast Only
Load Combinations Table

Location: Brookfield, CT

Date: 4/17/13

Prepared by: T.J.L.

Checked by: C.F.C.

Job No. 12047.CO13

Load Combination	Description	Envelope Wind									
		Southern	Factor	P-Delta	BLC	Factor	BLC	Factor	BLC	Factor	BLC
1	TIA/EIA Wind + Ice on PCS Structure	1		1	1	2	1	3	1	4	1
2	TIA/EIA Wind on PCS Structure	1		1	1	2	1	5	1		
Footnotes: (1) BLC = Basic Load Case (2) PCS Structure includes: Mast and Appurtenances											

Global

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation	Yes
Include Warping	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Vertical Axis	Y
Global Member Orientation Plane	XZ

Hot Rolled Steel Code	AISC 9th: ASD
Cold Formed Steel Code	AISI 1999: ASD
Wood Code	AF&PA NDS-97: ASD
Wood Temperature	< 100F
Concrete Code	ACI 318-02
Masonry Code	ACI 530-05/08: ASD
Aluminum Code	AA ADM1-05: ASD

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	PCA Load Contour
Parame Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections	Yes
Bad Framing Warnings	No
Unused Force Warnings	Yes

Seismic Code	UBC 1997
Seismic Base Elevation (ft)	Not Entered
Ct X	.035
Ct Z	.035
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	8.5
R Z	8.5
Ct Exp. X	.75
Ct Exp. Z	.75
Ca	.36
Cv	.54
Nv	1
SD1	1
SDS	1
S1	1
Occupancy Code	4
Seismic Zone	3
Use Group	I
Use Gravity Self Wt in Diaphragm Mass	Yes
Use Deck Self Wt in Diaphragm Mass	Yes
Use Lateral Self Wt in Diaphragm Mass	Yes
Seismic Detailing Code	None
Om X	1
Om Z	1
Rho X	1
Rho Z	1

Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (1/E...	Density[k/ft...	Yield[ksi]	Ry	Fu[ksi]	Rt
1	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
2	A572 Gr.50	29000	11154	.3	.65	.49	50	1.1	58	1.2
3	A992	29000	11154	.3	.65	.49	50	1.1	58	1.2
4	A500 Gr.42	29000	11154	.3	.65	.49	42	1.3	58	1.1
5	A500 Gr.46	29000	11154	.3	.65	.49	46	1.2	58	1.1
6	A53 Gr. B	29000	11154	.3	.65	.49	35	1.5	58	1.2

Hot Rolled Steel Design Parameters

	Label	Shape	Length	Lbyy[ft]	Lbzz[ft]	Lcomp top[ft]	Lcomp bot[ft]	Kyy	Kzz	Cm-...	Cm-...	Cb	y sw...	z sw...	Function
1	M1	Existing ..	41.583	30		30									Lateral
2	M2	Existing ..	22.417	30		30									Lateral

Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design Rul...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	Existing Upper Mast	PIPE 12.0	Beam	Pipe	A53 Gr. B	Typical	13.6	262	262	523
2	Existing Lower Mast	HSS16X0.375	Beam	Pipe	A500 Gr.42	Typical	17.2	526	526	1050

Member Primary Data

	Label	I Joint	J Joint	K Joint	Rotate(deg)	Section/Shape	Type	Design List	Material	Design Rules
1	M1	BOT_CO...	SPLICE			Existing Lower...	Beam	Pipe	A500 Gr.42	Typical
2	M2	SPLICE	TOP_MA...			Existing Upper...	Beam	Pipe	A53 Gr. B	Typical

Joint Coordinates and Temperatures

	Label	X [ft]	Y [ft]	Z [ft]	Temp [F]	Detach From Diap...
1	BOT CONN	0	0	0	0	
2	TOP CONN	0	30	0	0	
3	SPLICE	0	41.583	0	0	
4	TOP MAST	0	64	0	0	
5	CL SPRINT	0	60.667	0	0	
6	CL ATT	0	47	0	0	

Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]	Footing
1	BOT CONN	Reaction	Reaction	Reaction		Reaction		
2	TOP CONN	Reaction	Reaction	Reaction		Reaction		

Joint Loads and Enforced Displacements (BLC 2 : Weight of Appurtenances)

	Joint Label	L,D,M	Direction	Magnitude[(k,k-ft), (in,rad), (k*s^2/f...
1	CL SPRINT	L	Y	-.171
2	CL SPRINT	L	Y	-.263
3	CL ATT	L	Y	-.117
4	CL ATT	L	Y	-.192
5	CL ATT	L	Y	-.085

Joint Loads and Enforced Displacements (BLC 2 : Weight of Appurtenances) (Continued)

	Joint Label	L,D,M	Direction	Magnitude[(k,k-ft), (in,rad), (k*s^2/f...
6	CL ATT	L	Y	-.054
7	CL ATT	L	Y	-.61

Joint Loads and Enforced Displacements (BLC 3 : Weight of Ice Only on PCS Struct)

	Joint Label	L,D,M	Direction	Magnitude[(k,k-ft), (in,rad), (k*s^2/f...
1	CL SPRINT	L	Y	-.149
2	CL SPRINT	L	Y	-.037
3	CL ATT	L	Y	-.098
4	CL ATT	L	Y	-.142
5	CL ATT	L	Y	-.043
6	CL ATT	L	Y	-.017
7	CL ATT	L	Y	-.09

Joint Loads and Enforced Displacements (BLC 4 : TIA/EIA Wind with Ice on PCS Str)

	Joint Label	L,D,M	Direction	Magnitude[(k,k-ft), (in,rad), (k*s^2/f...
1	CL SPRINT	L	X	.912
2	CL ATT	L	X	.632
3	CL ATT	L	X	.892
4	CL ATT	L	X	.295
5	CL ATT	L	X	.079
6	CL ATT	L	X	.208

Joint Loads and Enforced Displacements (BLC 5 : TIA/EIA Wind on PCS Structure)

	Joint Label	L,D,M	Direction	Magnitude[(k,k-ft), (in,rad), (k*s^2/f...
1	CL SPRINT	L	X	1.094
2	CL ATT	L	X	.751
3	CL ATT	L	X	1.072
4	CL ATT	L	X	.329
5	CL ATT	L	X	.082
6	CL ATT	L	X	.238

Member Point Loads

Member Label	Direction	Magnitude[k,k-ft]	Location[ft, %]
No Data to Print ...			

Member Distributed Loads (BLC 2 : Weight of Appurtenances)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/f...	Start Location[ft, %]	End Location[ft, %]
1	M1	Y	-.012	-.012	0	0
2	M2	Y	-.012	-.012	0	0
3	M1	Y	-.006	-.006	36	0
4	M2	Y	-.006	-.006	0	10

Member Distributed Loads (BLC 3 : Weight of Ice Only on PCS Struct)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/f...	Start Location[ft, %]	End Location[ft, %]
1	M1	Y	-.01	-.01	0	0
2	M2	Y	-.008	-.008	0	0
3	M1	Y	-.023	-.023	0	0
4	M2	Y	-.023	-.023	0	0

Member Distributed Loads (BLC 3 : Weight of Ice Only on PCS Struct) (Continued)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/f...	Start Location[ft, %]	End Location[ft, %]
5	M1	Y	-.003	-.003	36	0
6	M2	Y	-.003	-.003	0	10

Member Distributed Loads (BLC 4 : TIA/EIA Wind with Ice on PCS Str)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/f...	Start Location[ft, %]	End Location[ft, %]
1	M1	X	.05	.05	0	0
2	M2	X	.042	.042	0	0
3	M1	X	.017	.017	0	0
4	M2	X	.017	.017	0	0
5	M1	X	.031	.031	36	0
6	M2	X	.031	.031	0	10

Member Distributed Loads (BLC 5 : TIA/EIA Wind on PCS Structure)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/f...	Start Location[ft, %]	End Location[ft, %]
1	M1	X	.062	.062	0	0
2	M2	X	.051	.051	0	0
3	M1	X	.019	.019	0	0
4	M2	X	.019	.019	0	0
5	M1	X	.019	.019	36	0
6	M2	X	.019	.019	0	10

Basic Load Cases

	BLC Description	Category	X Gr...	Y Gr...	Z Grav...	Joint	Point	Distri...	Area(...Surfa...
1	Self Weight (PCS Mast)	None		-1					
2	Weight of Appurtenances	None				7		4	
3	Weight of Ice Only on PCS Struct	None				7		6	
4	TIA/EIA Wind with Ice on PCS Str	None				6		6	
5	TIA/EIA Wind on PCS Structure	None				6		6	

Load Combinations

	Description	So...	PDelta	SRSS	BLCFac...	BLCFac...	BLCFac...	BLCFac...	BLCFac...	BLCFac...	BLCFac...	BLCFac...
1	TIA/EIA Wind + Ice on PCS ...	Yes			1	1	2	1	3	1	4	1
2	TIA/EIA Wind on PCS Struct...	Yes			1	1	2	1	5	1		
3	Self Weight				1	1						

Envelope Member Section Forces

	Member	Sec	Axial[k]	LC	y Shear[k]	LC	z Shear[k]	LC	Torque[k-ft]	LC	y-y Momen...	LC	z-z Momen...	LC
1	M1	1	max	1	1.553	1	-2.497	1	0	1	0	1	0	1
2			min	2	1.058	2	-2.813	2	0	1	0	1	0	1
3		2	max	1	.477	1	-3.194	1	0	1	0	1	33.626	2
4			min	2	.325	2	-3.656	2	0	1	0	1	29.583	1
5		3	max	2	-.408	2	-3.89	1	0	1	0	1	76.005	2
6			min	1	-.6	1	-4.498	2	0	1	0	1	66.407	1
7		4	max	1	5.286	1	6.273	2	0	1	0	1	113.35	2
8			min	2	3.625	2	5.52	1	0	1	0	1	98.47	1
9		5	max	1	4.159	1	5.325	2	0	1	0	1	52.807	2
10			min	2	2.858	2	4.651	1	0	1	0	1	45.187	1

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[k]	LC	y Shear[k]	LC	z Shear[k]	LC	Torque[k-ft]	LC	y-y Momen...	LC	z-z Momen...	LC
11	M2	1	max	4.159	1	5.325	2	0	1	0	1	0	1	52.807	2
12			min	2.858	2	4.651	1	0	1	0	1	0	1	45.187	1
13		2	max	2.161	1	2.354	2	0	1	0	1	0	1	24.824	2
14			min	1.44	2	2.04	1	0	1	0	1	0	1	20.932	1
15		3	max	1.621	1	1.879	2	0	1	0	1	0	1	13.013	2
16			min	1.087	2	1.573	1	0	1	0	1	0	1	10.889	1
17		4	max	1.12	1	1.486	2	0	1	0	1	0	1	3.584	2
18			min	.761	2	1.243	1	0	1	0	1	0	1	2.998	1
19		5	max	0	1	0	1	0	1	0	1	0	1	0	1
20			min	0	1	0	1	0	1	0	1	0	1	0	1

Envelope Member Section Stresses

Member	Sec		Axial[ksi]	LC	y Shear[...]	LC	z Shear[...]	LC	y-Top[ksi]	LC	y-Bot[ksi]	LC	z-Top[ksi]	LC	z-Bot[ksi]	LC
1	M1	1	max	.09	1	-.29	1	0	1	0	1	0	1	0	1	0
2			min	.062	2	-.327	2	0	1	0	1	0	1	0	1	0
3		2	max	.028	1	-.371	1	0	1	-5.399	1	6.137	2	0	1	0
4			min	.019	2	-.425	2	0	1	-6.137	2	5.399	1	0	1	0
5		3	max	-.024	2	-.452	1	0	1	-12.12	1	13.872	2	0	1	0
6			min	-.035	1	-.523	2	0	1	-13.872	2	12.12	1	0	1	0
7		4	max	.307	1	.729	2	0	1	-17.972	1	20.687	2	0	1	0
8			min	.211	2	.642	1	0	1	-20.687	2	17.972	1	0	1	0
9		5	max	.242	1	.619	2	0	1	-8.247	1	9.638	2	0	1	0
10			min	.166	2	.541	1	0	1	-9.638	2	8.247	1	0	1	0
11	M2	1	max	.306	1	.783	2	0	1	-13.194	1	15.419	2	0	1	0
12			min	.21	2	.684	1	0	1	-15.419	2	13.194	1	0	1	0
13		2	max	.159	1	.346	2	0	1	-6.112	1	7.248	2	0	1	0
14			min	.106	2	.3	1	0	1	-7.248	2	6.112	1	0	1	0
15		3	max	.119	1	.276	2	0	1	-3.179	1	3.8	2	0	1	0
16			min	.08	2	.231	1	0	1	-3.8	2	3.179	1	0	1	0
17		4	max	.082	1	.219	2	0	1	-.875	1	1.046	2	0	1	0
18			min	.056	2	.183	1	0	1	-1.046	2	.875	1	0	1	0
19		5	max	0	1	0	1	0	1	0	1	0	1	0	1	0
20			min	0	1	0	1	0	1	0	1	0	1	0	1	0

Envelope Joint Reactions

Joint		X [k]	LC	Y [k]	LC	Z [k]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	BOT_CONN	max	2.813	2	1.553	1	0	1	0	1	0	1	0
2		min	2.497	1	1.058	2	0	1	0	1	0	1	0
3	TOP_CONN	max	-10.107	1	6.962	1	0	1	0	1	0	1	0
4		min	-11.613	2	4.767	2	0	1	0	1	0	1	0
5	Totals:	max	-7.61	1	8.515	1	0	1					
6		min	-8.799	2	5.825	2	0	1					

Envelope Joint Displacements

Joint		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation...	LC	Y Rotation...	LC	Z Rotation...	LC
1	BOT_CONN	max	0	1	0	2	0	1	0	1	0	1	4.805e-3
2		min	0	2	0	1	0	1	0	1	0	1	4.214e-3
3	TOP_CONN	max	0	2	0	2	0	1	0	1	0	1	-9.242e-3

Company : CENTEK Engineering, INC.
 Designer : tj, cfc
 Job Number : 12047.CO13 /Sprint CT54XC713 CL&P # 2683 - Mast

Apr 18, 2013
 8:30 AM
 Checked By: _____

Envelope Joint Displacements (Continued)

	Joint		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation...	LC	Y Rotation...	LC	Z Rotation...	LC
4		min	0	1	0	1	0	1	0	1	0	1	-1.059e-2	2
5	SPLICE	max	2.218	2	0	2	0	1	0	1	0	1	-1.736e-2	1
6		min	1.932	1	-.001	1	0	1	0	1	0	1	-1.997e-2	2
7	TOP MAST	max	9.013	2	-.002	2	0	1	0	1	0	1	-2.329e-2	1
8		min	7.806	1	-.003	1	0	1	0	1	0	1	-2.699e-2	2
9	CL SPRINT	max	7.934	2	-.002	2	0	1	0	1	0	1	-2.328e-2	1
10		min	6.874	1	-.003	1	0	1	0	1	0	1	-2.698e-2	2
11	CL ATT	max	3.665	2	-.001	2	0	1	0	1	0	1	-2.075e-2	1
12		min	3.187	1	-.002	1	0	1	0	1	0	1	-2.396e-2	2

Envelope AISC ASD Steel Code Checks

Member	Shape	Code Check	Loc[ft]	LC	Sh...	Loc[ft]	Fa...	Ft [ksi]	Fb y-y [ksi]	Fb.....	AS...
1	M1	HSS16...	.794	29.8...	2	.04430.321	215...	25.2	27.72	27...	1.6...H2...
2	M2	PIPE_1...	.677	0	2	.056	0	214...	21	23.1	23.1...1.6...H1...

Company : CENTEK Engineering, INC.
Designer : tjf, cfc
Job Number : 12047.CO13 /Sprint CT54XC713 CL&P # 2683 - Mast

Apr 18, 2013

8:33 AM

Checked By: _____

Joint Reactions

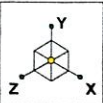
	LC	Joint Label	X [k]	Y [k]	Z [k]	MX [k-ft]	MY [k-ft]	MZ [k-ft]
1	1	BOT CONN	2.497	1.553	0	0	0	0
2	1	TOP CONN	-10.107	6.962	0	0	0	0
3	1	Totals:	-7.61	8.515	0			
4	1	COG (ft):	X: 0	Y: 36.053	Z: 0			

Company : CENTEK Engineering, INC.
Designer : tjf, cfc
Job Number : 12047.CO13 /Sprint CT54XC713 CL&P # 2683 - Mast

Apr 18, 2013
8:33 AM
Checked By: _____

Joint Reactions

LC		Joint Label	X [k]	Y [k]	Z [k]	MX [k-ft]	MY [k-ft]	MZ [k-ft]
1	2	BOT CONN	2.813	1.058	0	0	0	0
2	2	TOP CONN	-11.613	4.767	0	0	0	0
3	2	Totals:	-8.799	5.825	0			
4	2	COG (ft):	X: 0	Y: 36.07	Z: 0			

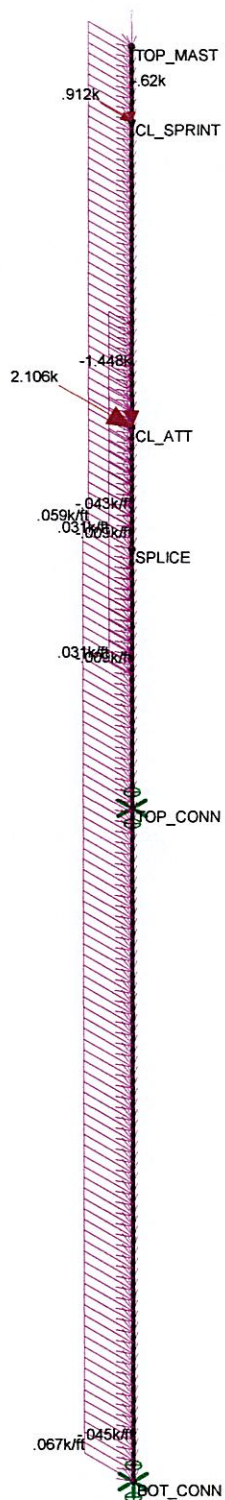


Code Check	
No Calc	
> 1.0	
90-1.0	
75-90	
50-75	
0-50	



Solution: Envelope

CEN TEK Engineering, INC.	CL&P # 2683 - Mast Unity Check	
tjl, cfc		Apr 18, 2013 at 8:31 AM
12047.CO13 /Sprint CT54...		EIA-TIA Loads.r3d



Loads: LC 1, TIA/EIA Wind + Ice on PCS Structure

CEN TEK Engineering, INC.

tjl, cfc

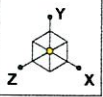
12047.CO13 /Sprint CT54...

CL&P # 2683 - Mast

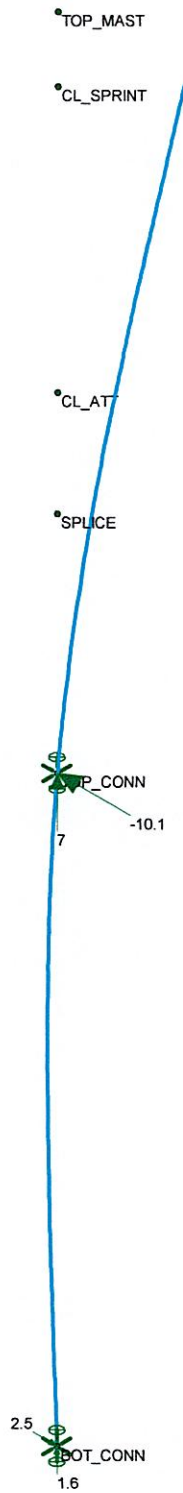
LC #1 Loads

Apr 18, 2013 at 8:31 AM

EIA-TIA Loads.r3d



Code Check	
No Calc	
> 1.0	
90-1.0	
75-90	
50-75	
0 - 50	



Results for LC 1, TIA/EIA Wind + Ice on PCS Structure
Z-moment Reaction units are k and k-ft

CEN TEK Engineering, INC.

tjl, cfc

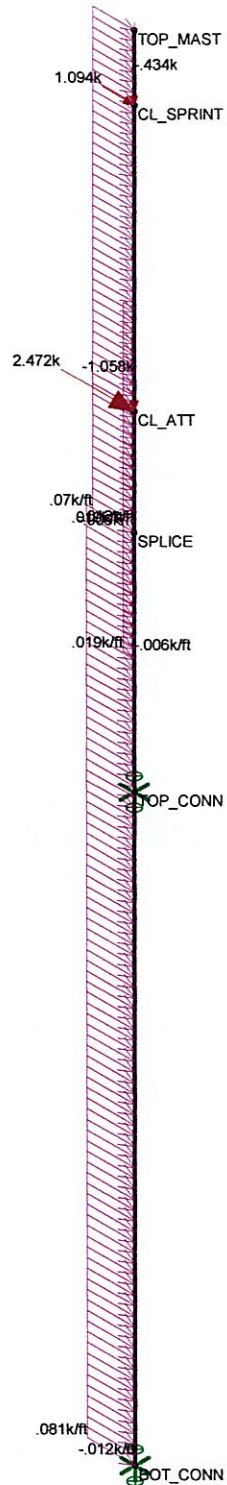
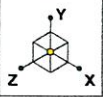
12047.CO13 /Sprint CT54...

CL&P # 2683 - Mast

LC #1 Reactions and Deflected Shape

Apr 18, 2013 at 8:32 AM

EIA-TIA Loads.r3d

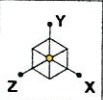


Loads: LC 2, TIA/EIA Wind on PCS Structure

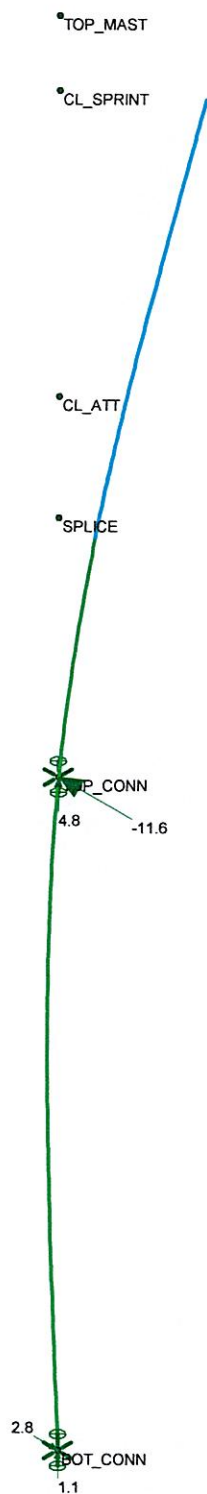
CEN TEK Engineering, INC.
tjl, cfc
12047.CO13 /Sprint CT54...

CL&P # 2683 - Mast
LC #2 Loads

Apr 18, 2013 at 8:32 AM
EIA-TIA Loads.r3d



Code Check	
No Calc	
> 1.0	
90-1.0	
75-90	
50-75	
0-50	



Results for LC 2, TIA/EIA Wind on PCS Structure
Z-moment Reaction units are k and k-ft

CEN TEK Engineering, INC.

tjl, cfc

12047.CO13 /Sprint CT54...

CL&P # 2683 - Mast

LC #2 Reactions and Deflected Shape

Apr 18, 2013 at 8:33 AM

EIA-TIA Loads.r3d

Mast Connection to CL&P Tower:

Reactions:

Moment =	Moment := 0-kips	(Input From Risa-3D)
Vertical =	Vertical := 4.8-kips	(Input From Risa-3D)
Horizontal x-dir =	Horizontal _x := 11.6-kips	(Input From Risa-3D)
Horizontal z-dir =	Horizontal _z := 11.6-kips	(Input From Risa-3D)

Bolt Data:

Bolt Type =	ASTMA325	(User Input)
Bolt Diameter =	D := 0.625-in	(User Input)
Number of Bolts =	N _b := 4	(User Input)
Allowable Tensile Strength =	F _t := 13.8-kips	(User Input)
Allowable Shear Strength =	F _v := 7.36-kips	(User Input)

$$\text{Shear Force} = f_v := \frac{\sqrt{\text{Horizontal}_z^2 + \text{Vertical}^2}}{N_b} = 3.1\text{-kips}$$

$$\text{Bolt Shear \% of Capacity} = \frac{f_v}{F_v} = 42.64\%$$

$$\text{Check Bolt Shear} = \text{Bolt_Shear} := \text{if} \left(\frac{f_v}{F_v} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

Bolt_Shear = "OK"

$$\text{Tension Force} = f_t := \frac{\text{Horizontal}_x}{N_b} = 2.9\text{-kips}$$

$$\text{Bolt Tension \% of Capacity} = \frac{f_t}{F_t} = 21.01\%$$

$$\text{Check Bolt Tension} = \text{Bolt_Tension} := \text{if} \left(\frac{f_t}{F_t} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

Bolt_Tension = "OK"

Subject:

Load Analysis of Pipe Mast on CL&P
 Structure #2683

Location:

Brookfield, CT

Rev. 0: 4/17/13

Prepared by: T.J.L. Checked by: C.F.C.
 Job No. 12047.CO13

Basic Components

Heavy Wind Pressure = $p := 4.00$ psf (User Input NESC 2007 Figure 250-1 & Table 250-1)
 Basic Windspeed = $V := 100$ mph (User Input NESC 2007 Figure 250-2(e))
 Radial Ice Thickness = $Ir := 0.50$ in (User Input)
 Radial Ice Density = $Id := 56.0$ pcf (User Input)

Factors for Extreme Wind Calculation

Elevation of Top of PCS Mast Above Grade = $TME := 114$ ft (User Input)
 Multiplier Gust Response Factor = $m := 1.25$ (User Input - Only for NESC Extreme wind case)
 NESC Factor = $kv := 1.43$ (User Input from NESC 2007 Table 250-3 equation)
 Importance Factor = $I := 1.0$ (User Input from NESC 2007 Section 250.C.2)
 Velocity Pressure Coefficient = $Kz := 2.01 \cdot \left(\frac{TME}{900} \right)^{\frac{2}{9.5}} = 1.301$ (NESC 2007 Table 250-2)
 Exposure Factor = $Es := 0.346 \left[\frac{33}{(0.67 \cdot TME)} \right]^{\frac{1}{7}} = 0.307$ (NESC 2007 Table 250-3)
 Response Term = $Bs := \frac{1}{\left(1 + 0.375 \cdot \frac{TME}{220} \right)} = 0.837$ (NESC 2007 Table 250-3)
 Gust Response Factor = $Grf := \frac{\left[1 + \left(2.7 \cdot Es \cdot Bs^{\frac{1}{2}} \right) \right]}{kv^2} = 0.86$ (NESC 2007 Table 250-3)
 Wind Pressure = $qz := 0.00256 \cdot Kz \cdot V^2 \cdot Grf \cdot I = 28.6$ psf (NESC 2007 Section 250.C.2)

Shape Factors

Shape Factor for Round Members = $CdR := 1.3$ (User Input)
 Shape Factor for Flat Members = $CdF := 1.6$ (User Input)
 Shape Factor for Coax Cables Attached to Outside of P de = $Cd_{coax} := 1.45$ (User Input)

NUS Design Criteria Issued April 12, 2007

Overload Factors

NU Design Criteria Table

Overload Factors for Wind Loads:

NESC Heavy Loading = 2.5 (User Input) Apply in Risa-3D Analysis
 NESC Extreme Loading = 1.0 (User Input) Apply in Risa-3D Analysis

Overload Factors for Vertical Loads:

NESC Heavy Loading = 1.5 (User Input) Apply in Risa-3D Analysis
 NESC Extreme Loading = 1.0 (User Input) Apply in Risa-3D Analysis

Development of Wind & Ice Load on PCS Mast

Existing Upper PCS Mast Data:

(Pipe 12 STD)

Mast Shape = Round (User Input)
Mast Diameter = $D_{mast} := 12.8$ in (User Input)
Mast Length = $L_{mast} := 22.417$ ft (User Input)
Mast Thickness = $t_{mast} := 0.375$ in (User Input)

Wind Load (NESC Extreme)

Mast Projected Surface Area =

$$A_{mast} := \frac{D_{mast}}{12} = 1.067 \quad \text{sf/ft}$$

Total Mast Wind Force (Above NU Structure) =

$$qz \cdot C_d R \cdot A_{mast} m = 50 \quad \text{plf} \quad \text{BLC 5}$$

Wind Load (NESE Heavy)

Mast Projected Surface Area w/ Ice =

$$A_{ICE_{mast}} := \frac{(D_{mast} + 2 \cdot Ir)}{12} = 1.15 \quad \text{sf/ft}$$

Total Mast Wind Force w/ Ice =

$$p \cdot C_d R \cdot A_{ICE_{mast}} = 6 \quad \text{plf} \quad \text{BLC 4}$$

Gravity Loads (without ice)

Weight of the mast =

Self Weight (Computed internally by Risa-3D) plf **BLC 1**

Gravity Loads (ice only)

Ice Area per Linear Foot =

$$A_{i_{mast}} := \frac{\pi}{4} \left[(D_{mast} + Ir \cdot 2)^2 - D_{mast}^2 \right] = 20.9 \quad \text{sq in}$$

Weight of Ice on Mast =

$$W_{ICE_{mast}} := Id \cdot \frac{A_{i_{mast}}}{144} = 8 \quad \text{plf} \quad \text{BLC 3}$$

Development of Wind & Ice Load on PCS Mast

Existing Lower PCS Mast Data:

HSS 16.0x0.375

Mast Shape = Round (User Input)

Mast Diameter = $D_{\text{mast}} := 16.0$ in (User Input)

Mast Length = $L_{\text{mast}} := 41.583$ ft (User Input)

Mast Thickness = $t_{\text{mast}} := 0.375$ in (User Input)

Wind Load (NESC Extreme)

Mast Projected Surface Area =

$$A_{\text{mast}} := \frac{D_{\text{mast}}}{12} = 1.333 \quad \text{sf/ft}$$

Total Mast Wind Force (Below NU Structure) =

$$qz \cdot C_d R \cdot A_{\text{mast}} = 50 \quad \text{plf} \quad \text{BLC 5}$$

Wind Load (NESE Heavy)

Mast Projected Surface Area w/ Ice =

$$A_{\text{ICE mast}} := \frac{(D_{\text{mast}} + 2 \cdot I_r)}{12} = 1.417 \quad \text{sf/ft}$$

Total Mast Wind Force w/ Ice =

$$p \cdot C_d R \cdot A_{\text{ICE mast}} = 7 \quad \text{plf} \quad \text{BLC 4}$$

Gravity Loads (without ice)

Weight of the mast =

$$\text{Self Weight} \quad (\text{Computed internally by Risa-3D}) \quad \text{plf} \quad \text{BLC 1}$$

Gravity Loads (ice only)

Ice Area per Linear Foot =

$$A_{\text{mast}} := \frac{\pi}{4} \left[(D_{\text{mast}} + I_r \cdot 2)^2 - D_{\text{mast}}^2 \right] = 25.9 \quad \text{sq in}$$

Weight of Ice on Mast =

$$W_{\text{ICE mast}} := I_d \cdot \frac{A_{\text{mast}}}{144} = 10 \quad \text{plf} \quad \text{BLC 3}$$

Subject:

Load Analysis of Pipe Mast on CL&P
 Structure #2683

Location:

Brookfield, CT

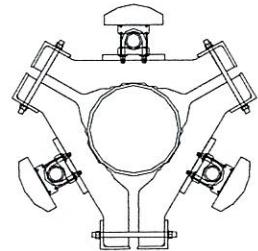
Rev. 0: 4/17/13

Prepared by: T.J.L. Checked by: C.F.C.
 Job No. 12047.CO13

Development of Wind & Ice Load on Antennas

Existing Antenna Data:

Antenna Model =	RFS APX VSPP18-C	(Sprint)
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 72$ in	(User Input)
Antenna Width =	$W_{ant} := 11.8$ in	(User Input)
Antenna Thickness =	$T_{ant} := 7$ in	(User Input)
Antenna Weight =	$WT_{ant} := 57$ lbs	(User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)



Wind Load (NESC Extreme)

*Assumes Maximum Possible Wind Pressure
 Applied to all Antennas Simultaneously*

Surface Area for One Antenna =

$$SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 5.9 \quad sf$$

Antenna Projected Surface Area =

$$A_{ant} := SA_{ant} \cdot N_{ant} = 17.7 \quad sf$$

Total Antenna Wind Force =

$$F_{ant} := qz \cdot C_d \cdot F \cdot A_{ant} \cdot m = 1014 \quad lbs \quad \text{BLC 5}$$

Wind Load (NESC Heavy)

*Assumes Maximum Possible Wind Pressure
 Applied to all Antennas Simultaneously*

Surface Area for One Antenna w/ Ice =

$$SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 6.5 \quad sf$$

Antenna Projected Surface Area w/ Ice =

$$A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 19.5 \quad sf$$

Total Antenna Wind Force w/ Ice =

$$F_{i_{ant}} := p \cdot C_d \cdot F \cdot A_{ICEant} = 125 \quad lbs \quad \text{BLC 4}$$

Gravity Load (without ice)

Weight of All Antennas =

$$WT_{ant} \cdot N_{ant} = 171 \quad lbs \quad \text{BLC 2}$$

Gravity Load (ice only)

Volume of Each Antenna =

$$V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 5947 \quad cu \text{ in}$$

Volume of Ice on Each Antenna =

$$V_{ice} := (L_{ant} + 1) \cdot (W_{ant} + 1) \cdot (T_{ant} + 1) - V_{ant} = 1528 \quad cu \text{ in}$$

Weight of Ice on Each Antenna =

$$W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 50 \quad lbs$$

Weight of Ice on All Antennas =

$$W_{ICEant} \cdot N_{ant} = 149 \quad lbs \quad \text{BLC 3}$$

Development of Wind & Ice Load on Antenna Mounts

Mount Data:

Mount Type:

(Sprint)

Microflex Universal Tri-Bracket

Mount Shape =

Flat

(User Input)

Pipe Mount Length =

$L_{mnt} = 72$ in (User Input)

2 inch Pipe Mount Linear Weight =

$W_{mnt} = 3.66$ plf (User Input)

Pipe Mount Outside Diameter =

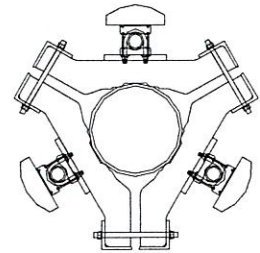
$D_{mnt} = 2.375$ in (User Input)

Number of Mounting Pipes =

$N_{mnt} = 3$ (User Input)

Tri-Bracket Weight =

$W_{tb.mnt} = 197$ lbs (User Input)



Wind Load (NESC Extreme)

Assumes Mount is Shielded by Antenna

Mount Projected Surface Area =

$A_{mnt} = 0.0$

sf

Total Mount Wind Force =

$F_{mnt} = qz \cdot C_d \cdot A_{mnt} \cdot m = 0$

lbs

BLC 5

Wind Load (NESC Heavy)

Assumes Mount is Shielded by Antenna

Mount Projected Surface Area w/ Ice =

$A_{ICEmnt} = 0.0$

sf

Total Mount Wind Force =

$F_{mnt} = p \cdot C_d \cdot A_{ICEmnt} = 0$

lbs

BLC 4

Gravity Loads (without ice)

Weight Each Pipe Mount =

$WT_{mnt} = W_{mnt} \cdot \frac{L_{mnt}}{12} = 22$

lbs

Weight of All Mounts =

$WT_{mnt} \cdot N_{mnt} + W_{tb.mnt} = 263$

lbs

BLC 2

Gravity Load (ice only)

(per TIA/EIA-222-F-1996)

Volume of Each Pipe =

$V_{mnt} = \frac{\pi}{4} \cdot D_{mnt}^2 \cdot L_{mnt} = 319$

cu in

Volume of Ice on Each Pipe =

$V_{ice} = \left[\frac{\pi}{4} \cdot (D_{mnt} + 1)^2 \cdot (L_{mnt} + 1) \right] - V_{mnt} = 334$

cu in

Weight of Ice each mount (incl. hardware) =

$W_{ICEmnt} = \frac{V_{ice}}{1728} \cdot \rho_{ice} = 11$

lbs

Weight of Ice on All Mounts =

$W_{ICEmnt} \cdot N_{mnt} + 5 = 37$

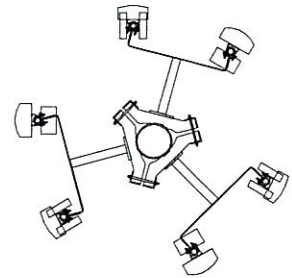
lbs

BLC 3

Development of Wind & Ice Load on Antennas

Existing Antenna Data:

Antenna Model =	Powerwave 7770.00	(AT&T)
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 55$ in	(User Input)
Antenna Width =	$W_{ant} := 11.0$ in	(User Input)
Antenna Thickness =	$T_{ant} := 5$ in	(User Input)
Antenna Weight =	$WT_{ant} := 39$ lbs	(User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)



Wind Load (NESC Extreme)

*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna =

$$SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 4.2 \quad sf$$

Antenna Projected Surface Area =

$$A_{ant} := SA_{ant} \cdot N_{ant} = 12.6 \quad sf$$

Total Antenna Wind Force =

$$F_{ant} := qz \cdot C_d \cdot F \cdot A_{ant} \cdot m = 722 \quad lbs \quad \text{BLC 5}$$

Wind Load (NESC Heavy)

*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna w/ Ice =

$$SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 4.7 \quad sf$$

Antenna Projected Surface Area w/ Ice =

$$A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 14 \quad sf$$

Total Antenna Wind Force w/ Ice =

$$F_{ant} := p \cdot C_d \cdot F \cdot A_{ICEant} = 90 \quad lbs \quad \text{BLC 4}$$

Gravity Load (without ice)

Weight of All Antennas =

$$WT_{ant} \cdot N_{ant} = 117 \quad lbs \quad \text{BLC 2}$$

Gravity Load (ice only)

Volume of Each Antenna =

$$V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 3025 \quad cu \text{ in}$$

Volume of Ice on Each Antenna =

$$V_{ice} := (L_{ant} + 1) \cdot (W_{ant} + 1) \cdot (T_{ant} + 1) - V_{ant} = 1007 \quad cu \text{ in}$$

Weight of Ice on Each Antenna =

$$W_{ICEant} := \frac{V_{ice}}{1728} \cdot \rho_d = 33 \quad lbs$$

Weight of Ice on All Antennas =

$$W_{ICEant} \cdot N_{ant} = 98 \quad lbs \quad \text{BLC 3}$$

Development of Wind & Ice Load on Antennas

Proposed Antenna Data:

Antenna Model =
 Antenna Shape =
 Antenna Height =
 Antenna Width =
 Antenna Thickness =
 Antenna Weight =
 Number of Antennas =

(AT&T)

Powerwave P65-16-XLH-RR

Flat (User Input)

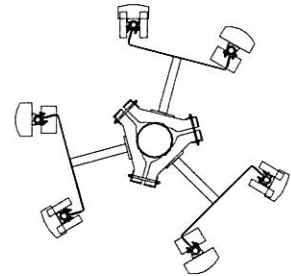
$L_{ant} := 72$ in (User Input)

$W_{ant} := 12$ in (User Input)

$T_{ant} := 6$ in (User Input)

$WT_{ant} := 64$ lbs (User Input)

$N_{ant} := 3$ (User Input)



Wind Load (NESC Extreme)

*Assumes Maximum Possible Wind Pressure
 Applied to all Antennas Simultaneously*

Surface Area for One Antenna =

$$SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 6 \quad sf$$

Antenna Projected Surface Area =

$$A_{ant} := SA_{ant} \cdot N_{ant} = 18 \quad sf$$

Total Antenna Wind Force =

$$F_{ant} := qz \cdot C_d \cdot F \cdot A_{ant} = 1031 \quad lbs \quad \text{BLC 5}$$

Wind Load (NESC Heavy)

*Assumes Maximum Possible Wind Pressure
 Applied to all Antennas Simultaneously*

Surface Area for One Antenna w/ Ice =

$$SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 6.6 \quad sf$$

Antenna Projected Surface Area w/ Ice =

$$A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 19.8 \quad sf$$

Total Antenna Wind Force w/ Ice =

$$F_{ant} := p \cdot C_d \cdot F \cdot A_{ICEant} = 127 \quad lbs \quad \text{BLC 4}$$

Gravity Load (without ice)

Weight of All Antennas =

$$WT_{ant} \cdot N_{ant} = 192 \quad lbs \quad \text{BLC 2}$$

Gravity Load (ice only)

Volume of Each Antenna =

$$V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 5184 \quad cu \text{ in}$$

Volume of Ice on Each Antenna =

$$V_{ice} := (L_{ant} + 1) \cdot (W_{ant} + 1) \cdot (T_{ant} + 1) - V_{ant} = 1459 \quad cu \text{ in}$$

Weight of Ice on Each Antenna =

$$W_{ICEant} := \frac{V_{ice}}{1728} \cdot \rho_d = 47 \quad lbs$$

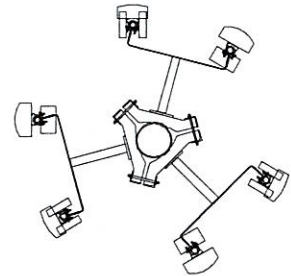
Weight of Ice on All Antennas =

$$W_{ICEant} \cdot N_{ant} = 142 \quad lbs \quad \text{BLC 3}$$

Development of Wind & Ice Load on TMA's

Existing TMA Data:

TMA Model =	Powerwave LGP 21401	(AT&T)
TMA Shape =	Flat	(User Input)
TMA Height =	$L_{TMA} := 14.4$ in	(User Input)
TMA Width =	$W_{TMA} := 9.2$ in	(User Input)
TMA Thickness =	$T_{TMA} := 2.6$ in	(User Input)
TMA Weight =	$WT_{TMA} := 14.1$ lbs	(User Input)
Number of TMA's =	$N_{TMA} := 6$	(User Input)



Wind Load (NESC Extreme)

*Assumes Maximum Possible Wind Pressure
 Applied to All TMA's Simultaneously*

Surface Area for One TMA =

$$SA_{TMA} := \frac{L_{TMA} \cdot W_{TMA}}{144} = 0.9 \quad sf$$

TMA Projected Surface Area =

$$A_{TMA} := SA_{TMA} \cdot N_{TMA} = 5.5 \quad sf$$

Total TMA Wind Force =

$$F_{TMA} := qz \cdot C_d \cdot A_{TMA} \cdot m = 316 \quad lbs \quad \text{BLC 5}$$

Wind Load (NESC Heavy)

*Assumes Maximum Possible Wind Pressure
 Applied to All TMA's Simultaneously*

Surface Area for One TMA w/ Ice =

$$SA_{ICETMA} := \frac{(L_{TMA} + 1) \cdot (W_{TMA} + 1)}{144} = 1.1 \quad sf$$

TMA Projected Surface Area w/ Ice =

$$A_{ICETMA} := SA_{ICETMA} \cdot N_{TMA} = 6.5 \quad sf$$

Total TMA Wind Force w/ Ice =

$$F_{ITMA} := p \cdot C_d \cdot A_{ICETMA} = 42 \quad lbs \quad \text{BLC 4}$$

Gravity Load (without ice)

Weight of All TMA's =

$$WT_{TMA} \cdot N_{TMA} = 85 \quad lbs \quad \text{BLC 2}$$

Gravity Load (ice only)

Volume of Each TMA =

$$V_{TMA} := L_{TMA} \cdot W_{TMA} \cdot T_{TMA} = 344 \quad cu \text{ in}$$

Volume of Ice on Each TMA =

$$V_{ice} := (L_{TMA} + 1)(W_{TMA} + 1)(T_{TMA} + 1) - V_{TMA} = 221 \quad cu \text{ in}$$

Weight of Ice on Each TMA =

$$W_{ICETMA} := \frac{V_{ice}}{1728} \cdot \rho_d = 7 \quad lbs$$

Weight of Ice on All TMA's

$$W_{ICETMA} \cdot N_{TMA} = 43 \quad lbs \quad \text{BLC 3}$$

Development of Wind & Ice Load on TMA's

Proposed TMA Data:

TMA Model =
 TMA Shape =
 TMA Height =
 TMA Width =
 TMA Thickness =
 TMA Weight =
 Number of TMA's =

(AT&T)

Powerwave TTAW-07BP111-001

Flat (User Input)

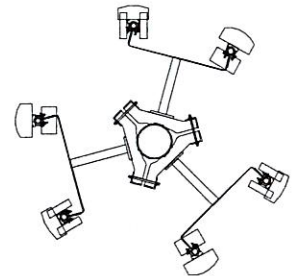
$L_{TMA} := 9.9$ in (User Input)

$W_{TMA} := 6.7$ in (User Input)

$T_{TMA} := 5.4$ in (User Input)

$WT_{TMA} := 18$ lbs (User Input)

$N_{TMA} := 3$ (User Input)



Wind Load (NESC Extreme)

*Assumes Maximum Possible Wind Pressure
 Applied to All TMA's Simultaneously*

Surface Area for One TMA =

$$SA_{TMA} := \frac{L_{TMA} \cdot W_{TMA}}{144} = 0.5 \quad \text{sf}$$

TMA Projected Surface Area =

$$A_{TMA} := SA_{TMA} \cdot N_{TMA} = 1.4 \quad \text{sf}$$

Total TMA Wind Force =

$$F_{TMA} := qz \cdot C_d \cdot A_{TMA} \cdot m = 79 \quad \text{lbs} \quad \text{BLC 5}$$

Wind Load (NESC Heavy)

*Assumes Maximum Possible Wind Pressure
 Applied to All TMA's Simultaneously*

Surface Area for One TMA w/ Ice =

$$SA_{ICETMA} := \frac{(L_{TMA} + 1) \cdot (W_{TMA} + 1)}{144} = 0.6 \quad \text{sf}$$

TMA Projected Surface Area w/ Ice =

$$A_{ICETMA} := SA_{ICETMA} \cdot N_{TMA} = 1.7 \quad \text{sf}$$

Total TMA Wind Force w/ Ice =

$$F_{ITMA} := p \cdot C_d \cdot A_{ICETMA} = 11 \quad \text{lbs} \quad \text{BLC 4}$$

Gravity Load (without ice)

Weight of All TMA's =

$$WT_{TMA} \cdot N_{TMA} = 54 \quad \text{lbs} \quad \text{BLC 2}$$

Gravity Load (ice only)

Volume of Each TMA =

$$V_{TMA} := L_{TMA} \cdot W_{TMA} \cdot T_{TMA} = 358 \quad \text{cu in}$$

Volume of Ice on Each TMA =

$$V_{ice} := (L_{TMA} + 1)(W_{TMA} + 1)(T_{TMA} + 1) - V_{TMA} = 179 \quad \text{cu in}$$

Weight of Ice on Each TMA =

$$W_{ICETMA} := \frac{V_{ice}}{1728} \cdot \rho_{ice} = 6 \quad \text{lbs}$$

Weight of Ice on All TMA's

$$W_{ICETMA} \cdot N_{TMA} = 17 \quad \text{lbs} \quad \text{BLC 3}$$

Subject:

Load Analysis of Pipe Mast on CL&P
 Structure #2683

Location:

Brookfield, CT

Rev. 0: 4/17/13

Prepared by: T.J.L. Checked by: C.F.C.
 Job No. 12047.CO13

Development of Wind & Ice Load on Mounts

Mount Data:

(AT&T)

Mount Type =

(3) Valmont Dual Standoff Mounts
 w/ Universal Tri-B-Racket

Mount Shape =

Flat (User Input)

Mount Area =

$A_{mnt} := 4.0$ sq ft (User Input)

Mount Area w/ Ice =

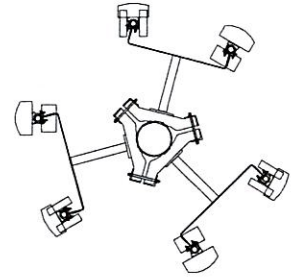
$A_{ICEmnt} := 4.6$ sq ft (User Input)

Mount Weight =

$WT_{mnt} := 610$ lbs (User Input)

Mount Weight w/ Ice =

$WT_{ICEmnt} := 700$ lbs (User Input)



Wind Load (NESC Extreme)

Total Mount Wind Force =

$$F_{mnt} := qz \cdot C_d \cdot A_{mnt} \cdot m = 229$$

lbs **BLC 5**

Wind Load (NESC Heavy)

Total Mount Wind Force w/ Ice =

$$F_{i,mnt} := p \cdot C_d \cdot A_{ICEmnt} = 29$$

lbs **BLC 4**

Gravity Load (without ice)

Weight of Mount =

$$WT_{mnt} = 610$$

lbs **BLC 2**

Gravity Load (ice only)

Weight of Ice on Mount =

$$WT_{ICEmnt} - WT_{mnt} = 90$$

lbs **BLC 3**

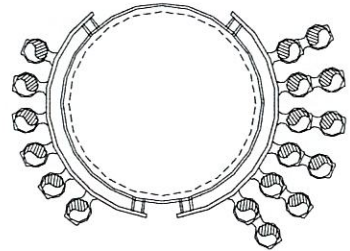
Development of Wind & Ice Load on Coax Cables

Existing Coax Cable Data:

Coax Type =
 Shape =
 Coax Outside Diameter =
 Coax Cable Length =
 Weight of Coax per foot =
 Total Number of Coax =
 No. of Coax Projecting Outside Face of PCS Mast =

(Sprint)

HELIAX 1-1/4"
 Round (User Input)
 $D_{\text{coax}} := 1.55$ in (User Input)
 $L_{\text{coax}} := 64$ ft (User Input)
 $Wt_{\text{coax}} := 0.66$ plf (User Input)
 $N_{\text{coax}} := 18$ (User Input)
 $NP_{\text{coax}} := 3$ (User Input)



Note: AT&T Existing/Proposed
 cables attached to CL&P Pole

Wind Load (NESC Extreme)

Coax projected surface area =

$$A_{\text{coax}} := \frac{(NP_{\text{coax}} D_{\text{coax}})}{12} = 0.4 \quad \text{sf/ft}$$

Total Coax Wind Force (Above NU Structure) =

$$F_{\text{coax}} := qz \cdot C_d \cdot A_{\text{coax}} \cdot m = 20 \quad \text{plf} \quad \text{BLC 5}$$

Total Coax Wind Force (Below NU Structure) =

$$F_{\text{coax}} := qz \cdot C_d \cdot A_{\text{coax}} \cdot m = 16 \quad \text{plf} \quad \text{BLC 5}$$

Wind Load (NESC Heavy)

Coax projected surface area w/ Ice =

$$A_{\text{ICE}_{\text{coax}}} := \frac{(NP_{\text{coax}} D_{\text{coax}} + 2 \cdot Ir)}{12} = 0.5 \quad \text{sf/ft}$$

Total Coax Wind Force w/ Ice =

$$F_{\text{ICE}_{\text{coax}}} := p \cdot C_d \cdot A_{\text{ICE}_{\text{coax}}} = 3 \quad \text{plf} \quad \text{BLC 4}$$

Gravity Loads (without ice)

Weight of all cables w/o ice

$$WT_{\text{coax}} := Wt_{\text{coax}} \cdot N_{\text{coax}} = 12 \quad \text{plf} \quad \text{BLC 2}$$

Gravity Load (ice only)

Ice Area per Linear Foot =

$$A_{\text{ICE}_{\text{coax}}} := \frac{\pi}{4} \left[(D_{\text{coax}} + 2 \cdot Ir)^2 - D_{\text{coax}}^2 \right] = 3.2 \quad \text{sq in}$$

Ice Weight All Coax per foot =

$$WT_{\text{ICE}_{\text{coax}}} := N_{\text{coax}} \cdot Id \cdot \frac{A_{\text{ICE}_{\text{coax}}}}{144} = 23 \quad \text{plf} \quad \text{BLC 3}$$

Subject:

Load Analysis of Pipe Mast on CL&P
 Structure #2683

Location:

Brookfield, CT

Rev. 0: 4/17/13

Prepared by: T.J.L. Checked by: C.F.C.
 Job No. 12047.CO13

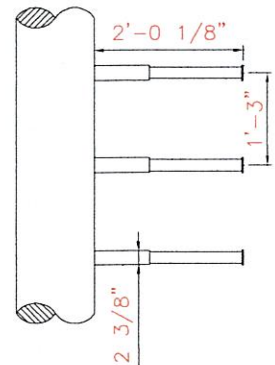
Development of Wind & Ice Load on Brace Member

Member Data:

Shape =
 Diameter =
 Length =
 Spacing =
 Weight =

Step Bolts

Round (User Input)
 $D_{mem} := 2.38$ in (User Input)
 $L_{mem} := 2$ ft (User Input)
 $Sp_{mem} := 1.25$ ft (User Input)
 $Wt_{mem} := 3.66$ plf (User Input)



Wind Load (NESC Extreme)

Member Projected Surface Area =

$$A_{mem} := \frac{D_{mem} \cdot L_{mem}}{12 \cdot Sp_{mem}} = 0.317 \quad \text{sf/ft}$$

Total Member Wind Force =

$$qz \cdot C_d R \cdot A_{mem} \cdot m = 15 \quad \text{plf} \quad \text{BLC 5}$$

Wind Load (NESE Heavy)

Member Projected Surface Area w/ Ice =

$$A_{ICE_{mem}} := \frac{(D_{mem} + 2 \cdot Ir) \cdot (L_{mem} + 2 \cdot Ir)}{12 \cdot Sp_{mem}} = 0.676 \quad \text{sf/ft}$$

Total Member Wind Force w/ Ice =

$$p \cdot C_d R \cdot A_{ICE_{mem}} = 4 \quad \text{plf} \quad \text{BLC 4}$$

Gravity Loads (without ice)

Weight of the Member =

$$\frac{Wt_{mem} \cdot L_{mem}}{Sp_{mem}} = 6 \quad \text{plf} \quad \text{BLC 2}$$

Gravity Loads (ice only)

Ice Area per Linear Foot =

$$A_{i_{mem}} := \frac{1}{4} \cdot \pi \cdot [(D_{mem} + 2 \cdot Ir)^2 - D_{mem}^2] = 4.5 \quad \text{sq in}$$

Weight of Ice on Member =

$$W_{ICE_{mem}} := Id \cdot \frac{A_{i_{mem}}}{144} \cdot \frac{L_{mem}}{Sp_{mem}} = 3 \quad \text{plf} \quad \text{BLC 3}$$

CENTEK engineering, INC. Consulting Engineers 63-2 North Branford Road Branford, CT 06405 Ph. 203-488-0580 / Fax. 203-488-8587	Subject: Analysis of NESC Heavy Wind and NESC Extreme Wind for Obtaining PCS Structure Reactions Applied to CL&P Pole Tabulated Load Cases Location: Brookfield, CT Date: 4/17/13	Prepared by: T.J.L.	Checked by: C.F.C.	Job No. 12047.CO13
Load Case	Description			
1	Self Weight (Mast)			
2	Weight of Appurtenances			
3	Weight of Ice Only on PCS Structure ⁽¹⁾			
4	NESC Heavy Wind on PCS Structure ⁽¹⁾			
5	NESC Extreme Wind on PCS Structure ⁽¹⁾			
Footnotes: (1) PCS Structure includes: Mast and Appurtenances				

CEN TEK engineering, INC. Consulting Engineers 63-2 North Branford Road Branford, CT 06405 Ph. 203-488-0580 / Fax. 203-488-8587		Subject: Analysis of NESC Heavy Wind and NESC Extreme Wind for Obtaining PCS Structure Reactions Applied to CL&P Pole Load Combinations Table Location: Brookfield, CT Date: 4/17/13 Prepared by: T.J.L. Checked by: C.F.C. Job No. 12047.CO13									
Load Combination	Description	Envelope Wind									
		Solution	Factor	P-Delta	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor
1	NESC Heavy Wind on PCS Structure	1			1	1.5	2	1.5	3	1.5	4 2.5
2	NESC Extreme Wind on PCS Structure	1			1	1	2	1	5	1	
Footnotes: (1) BLC = Basic Load Case (2) PCS Structure includes: Mast and Appurtenances											

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Global

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation	Yes
Include Warping	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Vertical Axis	Y
Global Member Orientation Plane	XZ

Hot Rolled Steel Code	AISC 9th: ASD
Cold Formed Steel Code	AISI 1999: ASD
Wood Code	AF&PA NDS-97: ASD
Wood Temperature	< 100F
Concrete Code	ACI 318-02
Masonry Code	ACI 530-05/08: ASD
Aluminum Code	AA ADM1-05: ASD

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	PCA Load Contour
Parame Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections	Yes
Bad Framing Warnings	No
Unused Force Warnings	Yes

Seismic Code	UBC 1997
Seismic Base Elevation (ft)	Not Entered
Ct X	.035
Ct Z	.035
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	8.5
R Z	8.5
Ct Exp. X	.75
Ct Exp. Z	.75
Ca	.36
Cv	.54
Nv	1
SD1	1
SDS	1
S1	1
Occupancy Code	4
Seismic Zone	3
Use Group	I
Use Gravity Self Wt in Diaphragm Mass	Yes
Use Deck Self Wt in Diaphragm Mass	Yes
Use Lateral Self Wt in Diaphragm Mass	Yes
Seismic Detailing Code	None
Om X	1
Om Z	1
Rho X	1
Rho Z	1

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Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (1E..	Density[k/f...	Yield[ksi]	Ry	Fu[ksi]	Rt
1	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
2	A572 Gr.50	29000	11154	.3	.65	.49	50	1.1	58	1.2
3	A992	29000	11154	.3	.65	.49	50	1.1	58	1.2
4	A500 Gr.42	29000	11154	.3	.65	.49	42	1.3	58	1.1
5	A500 Gr.46	29000	11154	.3	.65	.49	46	1.2	58	1.1
6	A53 Gr. B	29000	11154	.3	.65	.49	35	1.5	58	1.2

Hot Rolled Steel Design Parameters

	Label	Shape	Length...	Lbyy[ft]	Lbzz[ft]	Lcomp top[ft]	Lcomp bot[ft]	Kyy	Kzz	Cm-...	Cm-...	Cb	y sw...	z sw...	Function
1	M1	Existing ...	41.583	30		30									Lateral
2	M2	Existing ...	22.417	30		30									Lateral

Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design Rul...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	Existing Upper Mast	PIPE 12.0	Beam	Pipe	A53 Gr. B	Typical	13.6	262	262	523
2	Existing Lower Mast	HSS16X0.375	Beam	Pipe	A500 Gr.42	Typical	17.2	526	526	1050

Member Primary Data

	Label	I Joint	J Joint	K Joint	Rotate(deg)	Section/Shape	Type	Design List	Material	Design Rules
1	M1	BOT_CO...	SPLICE			Existing Lower...	Beam	Pipe	A500 Gr.42	Typical
2	M2	SPLICE	TOP_MA...			Existing Upper...	Beam	Pipe	A53 Gr. B	Typical

Joint Coordinates and Temperatures

	Label	X [ft]	Y [ft]	Z [ft]	Temp [F]	Detach From Diap...
1	BOT CONN	0	0	0	0	
2	TOP CONN	0	30	0	0	
3	SPLICE	0	41.583	0	0	
4	TOP MAST	0	64	0	0	
5	CL SPRINT	0	60.667	0	0	
6	CL ATT	0	47	0	0	

Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]	Footing
1	BOT CONN	Reaction	Reaction	Reaction		Reaction		
2	TOP CONN	Reaction	Reaction	Reaction		Reaction		

Joint Loads and Enforced Displacements (BLC 2 : Weight of Appurtenances)

	Joint Label	L,D,M	Direction	Magnitude[(k,k-ft), (in,rad), (k*s^2/f...
1	CL SPRINT	L	Y	-171
2	CL SPRINT	L	Y	-263
3	CL ATT	L	Y	-117
4	CL ATT	L	Y	-192
5	CL ATT	L	Y	-.085

Joint Loads and Enforced Displacements (BLC 2 : Weight of Appurtenances) (Continued)

	Joint Label	L,D,M	Direction	Magnitude[(k,k-ft), (in,rad), (k*s^2/f...
6	CL ATT	L	Y	-.054
7	CL ATT	L	Y	-.61

Joint Loads and Enforced Displacements (BLC 3 : Weight of Ice Only on PCS Struct)

	Joint Label	L,D,M	Direction	Magnitude[(k,k-ft), (in,rad), (k*s^2/f...
1	CL SPRINT	L	Y	-.149
2	CL SPRINT	L	Y	-.037
3	CL ATT	L	Y	-.098
4	CL ATT	L	Y	-.142
5	CL ATT	L	Y	-.043
6	CL ATT	L	Y	-.017
7	CL ATT	L	Y	-.09

Joint Loads and Enforced Displacements (BLC 4 : NESC Heavy Wind on PCS Structure)

	Joint Label	L,D,M	Direction	Magnitude[(k,k-ft), (in,rad), (k*s^2/f...
1	CL SPRINT	L	X	.125
2	CL ATT	L	X	.09
3	CL ATT	L	X	.127
4	CL ATT	L	X	.042
5	CL ATT	L	X	.011
6	CL ATT	L	X	.029

Joint Loads and Enforced Displacements (BLC 5 : NESC Extreme Wind on PCS Structu)

	Joint Label	L,D,M	Direction	Magnitude[(k,k-ft), (in,rad), (k*s^2/f...
1	CL SPRINT	L	X	1.014
2	CL ATT	L	X	.722
3	CL ATT	L	X	1.031
4	CL ATT	L	X	.316
5	CL ATT	L	X	.079
6	CL ATT	L	X	.229

Member Point Loads

Member Label	Direction	Magnitude[k,k-ft]	Location[ft, %]
No Data to Print ...			

Member Distributed Loads (BLC 2 : Weight of Appurtenances)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/f...	Start Location[ft, %]	End Location[ft, %]
1	M1	Y	-.012	-.012	0	0
2	M2	Y	-.012	-.012	0	0
3	M1	Y	-.006	-.006	36	0
4	M2	Y	-.006	-.006	0	10

Member Distributed Loads (BLC 3 : Weight of Ice Only on PCS Struct)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/f...	Start Location[ft, %]	End Location[ft, %]
1	M1	Y	-.01	-.01	0	0
2	M2	Y	-.008	-.008	0	0
3	M1	Y	-.023	-.023	0	0
4	M2	Y	-.023	-.023	0	0

Member Distributed Loads (BLC 3 : Weight of Ice Only on PCS Struct) (Continued)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/f...	Start Location[ft,%]	End Location[ft,%]
5	M1	Y	-.003	-.003	36	0
6	M2	Y	-.003	-.003	0	10

Member Distributed Loads (BLC 4 : NESC Heavy Wind on PCS Structure)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/f...	Start Location[ft,%]	End Location[ft,%]
1	M1	X	.007	.007	0	0
2	M2	X	.006	.006	0	0
3	M1	X	.003	.003	0	0
4	M2	X	.003	.003	0	0
5	M1	X	.004	.004	36	0
6	M2	X	.004	.004	0	10

Member Distributed Loads (BLC 5 : NESC Extreme Wind on PCS Structu)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/f...	Start Location[ft,%]	End Location[ft,%]
1	M1	X	.05	.05	0	0
2	M2	X	.05	.05	0	0
3	M1	X	.016	.016	0	0
4	M2	X	.02	.02	0	0
5	M1	X	.015	.015	36	0
6	M2	X	.015	.015	0	10

Basic Load Cases

	BLC Description	Category	X Gr...	Y Gr...	Z Grav...	Joint	Point	Distri...	Area(...Surfa...
1	Self Weight (PCS Mast)	None		-1					
2	Weight of Appurtenances	None				7		4	
3	Weight of Ice Only on PCS Struct	None				7		6	
4	NESC Heavy Wind on PCS Structure	None				6		6	
5	NESC Extreme Wind on PCS Structu	None				6		6	

Load Combinations

	Description	So...	PDelta	SRSS	BLCFac...	BLCFac...	BLCFac...	BLCFac...	BLCFac...	BLCFac...	BLCFac...	BLCFac...	BLCFac...
1	NESC Heavy Wind on PCS ...	Yes			1	1.5	2	1.5	3	1.5	4	2.5	
2	NESC Extreme Wind on PCS...	Yes			1	1	2	1	5	1			
3	Self Weight				1	1							

Envelope Member Section Forces

	Member	Sec	Axial[k]	LC	y Shear[k]	LC	z Shear[k]	LC	Torque[k-ft]	LC	y-y Momen...	LC	z-z Momen...	LC
1	M1	1	max	2.329	1	-.879	1	0	1	0	1	0	1	0
2			min	1.058	2	-2.841	2	0	1	0	1	0	1	0
3		2	max	.715	1	-1.139	1	0	1	0	1	0	1	33.098
4			min	.325	2	-3.527	2	0	1	0	1	0	1	10.486
5		3	max	-.408	2	-1.399	1	0	1	0	1	0	1	73.328
6			min	-.899	1	-4.213	2	0	1	0	1	0	1	23.675
7		4	max	7.929	1	5.88	2	0	1	0	1	0	1	107.893
8			min	3.625	2	1.98	1	0	1	0	1	0	1	35.244
9		5	max	6.239	1	5.11	2	0	1	0	1	0	1	50.566
10			min	2.858	2	1.664	1	0	1	0	1	0	1	16.166

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[k]	LC	y Shear[k]	LC	z Shear[k]	LC	Torque[k-ft]	LC	y-y Momen...	LC	z-z Momen...	LC
11	M2	1	max	6.239	1	5.11	2	0	1	0	1	0	1	50.566	2
12			min	2.858	2	1.664	1	0	1	0	1	0	1	16.166	1
13		2	max	3.241	1	2.257	2	0	1	0	1	0	1	23.707	2
14			min	1.44	2	.735	1	0	1	0	1	0	1	7.489	1
15		3	max	2.431	1	1.799	2	0	1	0	1	0	1	12.383	2
16			min	1.087	2	.565	1	0	1	0	1	0	1	3.874	1
17		4	max	1.681	1	1.406	2	0	1	0	1	0	1	3.402	2
18			min	.761	2	.439	1	0	1	0	1	0	1	1.063	1
19		5	max	0	1	0	1	0	1	0	1	0	1	0	1
20			min	0	1	0	1	0	1	0	1	0	1	0	1

Envelope Member Section Stresses

Member	Sec		Axial[ksi]	LC	y Shear[...]	LC	z Shear[...]	LC	y-Top[ksi]	LC	y-Bot[ksi]	LC	z-Top[ksi]	LC	z-Bot[ksi]	LC
1	M1	1	max	.135	1	-.102	1	0	1	0	1	0	1	0	1	0
2			min	.062	2	-.33	2	0	1	0	1	0	1	0	1	0
3		2	max	.042	1	-.132	1	0	1	-1.914	1	6.041	2	0	1	0
4			min	.019	2	-.41	2	0	1	-6.041	2	1.914	1	0	1	0
5		3	max	-.024	2	-.163	1	0	1	-4.321	1	13.383	2	0	1	0
6			min	-.052	1	-.49	2	0	1	-13.383	2	4.321	1	0	1	0
7		4	max	.461	1	.684	2	0	1	-6.432	1	19.692	2	0	1	0
8			min	.211	2	.23	1	0	1	-19.692	2	6.432	1	0	1	0
9		5	max	.363	1	.594	2	0	1	-2.951	1	9.229	2	0	1	0
10			min	.166	2	.194	1	0	1	-9.229	2	2.951	1	0	1	0
11	M2	1	max	.459	1	.751	2	0	1	-4.72	1	14.764	2	0	1	0
12			min	.21	2	.245	1	0	1	-14.764	2	4.72	1	0	1	0
13		2	max	.238	1	.332	2	0	1	-2.187	1	6.922	2	0	1	0
14			min	.106	2	.108	1	0	1	-6.922	2	2.187	1	0	1	0
15		3	max	.179	1	.264	2	0	1	-1.131	1	3.616	2	0	1	0
16			min	.08	2	.083	1	0	1	-3.616	2	1.131	1	0	1	0
17		4	max	.124	1	.207	2	0	1	-.31	1	.993	2	0	1	0
18			min	.056	2	.064	1	0	1	-.993	2	.31	1	0	1	0
19		5	max	0	1	0	1	0	1	0	1	0	1	0	1	0
20			min	0	1	0	1	0	1	0	1	0	1	0	1	0

Envelope Joint Reactions

	Joint		X [k]	LC	Y [k]	LC	Z [k]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	BOT_CONN	max	2.841	2	2.329	1	0	1	0	1	0	1	0	1
2		min	.879	1	1.058	2	0	1	0	1	0	1	0	1
3	TOP_CONN	max	-3.639	1	10.442	1	0	1	0	1	0	1	0	1
4		min	-10.779	2	4.767	2	0	1	0	1	0	1	0	1
5	Totals:	max	-2.76	1	12.772	1	0	1						
6		min	-7.938	2	5.825	2	0	1						

Envelope Joint Displacements

Joint			X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation...	LC	Y Rotation...	LC	Z Rotation...	LC
1	BOT CONN	max	0	1	0	2	0	1	0	1	0	1	4.686e-3	2
2		min	0	2	0	1	0	1	0	1	0	1	1.498e-3	1
3	TOP CONN	max	0	2	0	2	0	1	0	1	0	1	-3.298e-3	1

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Envelope Joint Displacements (Continued)

	Joint		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation...	LC	Y Rotation...	LC	Z Rotation...	LC
4		min	0	1	0	1	0	1	0	1	0	1	-1.019e-2	2
5	SPLICE	max	2.127	2	0	2	0	1	0	1	0	1	-6.201e-3	1
6		min	.69	1	-.002	1	0	1	0	1	0	1	-1.914e-2	2
7	TOP MAST	max	8.636	2	-.002	2	0	1	0	1	0	1	-8.32e-3	1
8		min	2.789	1	-.004	1	0	1	0	1	0	1	-2.584e-2	2
9	CL SPRINT	max	7.603	2	-.002	2	0	1	0	1	0	1	-8.317e-3	1
10		min	2.456	1	-.004	1	0	1	0	1	0	1	-2.583e-2	2
11	CL ATT	max	3.514	2	-.001	2	0	1	0	1	0	1	-7.415e-3	1
12		min	1.138	1	-.003	1	0	1	0	1	0	1	-2.296e-2	2

Envelope AISC ASD Steel Code Checks

Member	Shape	Code Check	Loc[ft]	LC	Sh...	Loc[ft]	Fa...	Ft [ksi]	Fb y-y [ksi]	Fb.....	AS...
1	M1	HSS16...	.756	29.8...	2	.04130.321	215...	25.2	27.72	27...	1.6...H2...
2	M2	PIPE_1...	.649	0	2	.054	0	214...	21	23.1	1.6...H1...

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8:39 AM
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Joint Reactions

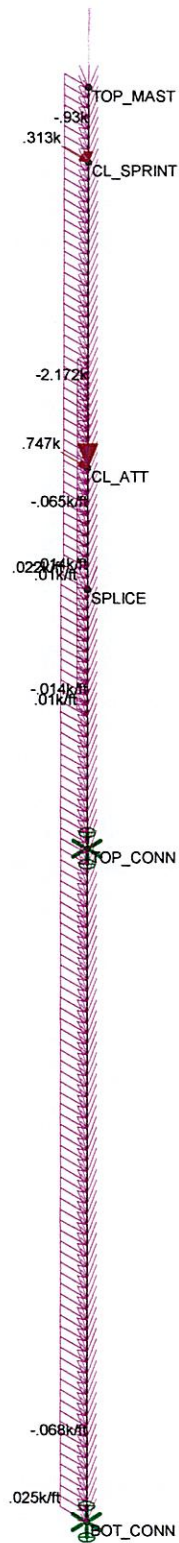
	LC	Joint Label	X [k]	Y [k]	Z [k]	MX [k-ft]	MY [k-ft]	MZ [k-ft]
1	1	BOT CONN	.879	2.329	0	0	0	0
2	1	TOP CONN	-3.639	10.442	0	0	0	0
3	1	Totals:	-2.76	12.772	0			
4	1	COG (ft):	X: 0	Y: 36.053	Z: 0			

Company : CENTEK Engineering, INC.
Designer : tjf, cfc
Job Number : 12047.CO13 /Sprint CT54XC713CL&P # 2683 - Mast

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8:40 AM
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Joint Reactions

LC		Joint Label	X [k]	Y [k]	Z [k]	MX [k-ft]	MY [k-ft]	MZ [k-ft]
1	2	BOT CONN	2.841	1.058	0	0	0	0
2	2	TOP CONN	-10.779	4.767	0	0	0	0
3	2	Totals:	-7.938	5.825	0			
4	2	COG (ft):	X: 0	Y: 36.07	Z: 0			

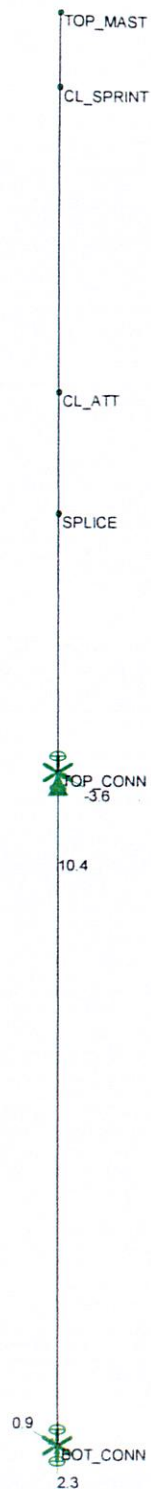


Loads: LC 1, NESC Heavy Wind on PCS Structure

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CL&P # 2683 - Mast
LC #1 Loads

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NESC Loads.r3d



Results for LC 1, NESC Heavy Wind on PCS Structure
Z-moment Reaction units are k and k-ft

CEN TEK Engineering, INC.

tjl, cfc

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CL&P # 2683 - Mast

LC #1 Reactions

Apr 18, 2013 at 8:39 AM

NESC Loads.r3d

Coax Cable on CL&P Pole

(Below 50-ft AGL)

Distance Between Coax Cable Attach Points =

Coaxial Cable Span =

$$\text{CoaxSpan} := \begin{pmatrix} 10 \\ 10 \\ 10 \\ 10 \\ 10 \end{pmatrix} \cdot \text{ft} \quad (\text{User Input})$$

Diameter of Coax Cable =

$$D_{\text{coax1}} := 1.55 \cdot \text{in} \quad (\text{User Input})$$

Weight of Coax Cable =

$$W_{\text{coax1}} := 0.66 \cdot \text{plf} \quad (\text{User Input})$$

Number of Coax Cables =

$$N_{\text{coax1}} := 33 \quad (\text{User Input}) \quad \begin{matrix} 15 \text{ AT\&T Cables and} \\ 18 \text{ Sprint Cables} \end{matrix}$$

Number of Projected Coax Cables =

$$NP_{\text{coax1}} := 4 \quad (\text{User Input})$$

Extreme Wind Pressure =

$$q_z := 28.6 \cdot \text{psf} \quad (\text{User Input})$$

Heavy Wind Pressure =

$$p := 4 \cdot \text{psf} \quad (\text{User Input})$$

Radial Ice Thickness =

$$I_r := 0.5 \cdot \text{in} \quad (\text{User Input})$$

Radial Ice Density =

$$I_d := 56 \cdot \text{pcf} \quad (\text{User Input})$$

Shape Factor =

$$C_{d_{\text{coax}}} := 1.45 \quad (\text{User Input})$$

Overload Factor for NESC Heavy Wind Load =

$$OF_{HW} := 2.5 \quad (\text{User Input})$$

Overload Factor for NESC Extreme Wind Load =

$$OF_{EW} := 1.0 \quad (\text{User Input})$$

Overload Factor for NESC Heavy Vertical Load =

$$OF_{HV} := 1.5 \quad (\text{User Input})$$

Overload Factor for NESC Extreme Vertical Load =

$$OF_{EV} := 1.0 \quad (\text{User Input})$$

Wind Area with Ice =

$$A_{\text{ice}} := (NP_{\text{coax1}} \cdot D_{\text{coax1}} + 2 \cdot I_r) = 7.2 \cdot \text{in}$$

Wind Area without Ice =

$$A := (NP_{\text{coax1}} \cdot D_{\text{coax1}}) = 6.2 \cdot \text{in}$$

Ice Area per Lner Ft =

$$A_{i_{\text{coax1}}} := \frac{\pi}{4} \cdot \left[(D_{\text{coax1}} + 2 \cdot I_r)^2 - D_{\text{coax1}}^2 \right] = 0.022 \cdot \text{ft}^2$$

Weight of Ice on All Coax Cables =

$$W_{\text{ice}} := A_{i_{\text{coax1}}} \cdot I_d \cdot N_{\text{coax1}} = 41 \cdot \text{plf}$$

Heavy Vertical Load =

$$\text{Heavy}_{\text{Vert}} := \overrightarrow{[(N_{\text{coax1}} \cdot W_{\text{coax1}} + W_{\text{ice}}) \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{HV}}]}$$

Heavy Transverse Load =

$$\text{Heavy}_{\text{Trans}} := \overrightarrow{(p \cdot A_{\text{ice}} \cdot C_{\text{d}_{\text{coax}}} \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{HW}})}$$

$$\text{Heavy}_{\text{Vert}} = \begin{pmatrix} 947 \\ 947 \\ 947 \\ 947 \\ 947 \end{pmatrix} \text{ lb}$$

$$\text{Heavy}_{\text{Trans}} = \begin{pmatrix} 87 \\ 87 \\ 87 \\ 87 \\ 87 \end{pmatrix} \text{ lb}$$

Extreme Vertical Load =

$$\text{Extreme}_{\text{Vert}} := \overrightarrow{[(N_{\text{coax1}} \cdot W_{\text{coax1}}) \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{EV}}]}$$

Extreme Transverse Load =

$$\text{Extreme}_{\text{Trans}} := \overrightarrow{[(qz \cdot A \cdot C_{\text{d}_{\text{coax}}}) \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{EW}}]}$$

$$\text{Extreme}_{\text{Vert}} = \begin{pmatrix} 218 \\ 218 \\ 218 \\ 218 \\ 218 \end{pmatrix} \text{ lb}$$

$$\text{Extreme}_{\text{Trans}} = \begin{pmatrix} 214 \\ 214 \\ 214 \\ 214 \\ 214 \end{pmatrix} \text{ lb}$$

Coax Cable on CL&P Pole

(Above 50-ft AGL)

Distance Between Coax Cable Attach Points =

Coaxial Cable Span =

$$CoaxSpan := \begin{pmatrix} 10 \\ 10 \\ 10 \\ 10 \end{pmatrix} \cdot ft \quad (User Input)$$

Diameter of Coax Cable =

$$D_{coax1} := 1.55 \cdot in \quad (User Input)$$

Weight of Coax Cable =

$$W_{coax1} := 0.66 \cdot plf \quad (User Input)$$

Number of Coax Cables =

$$N_{coax1} := 15 \quad (User Input)$$

15 AT&T Cables

Number of Projected Coax Cables =

$$NP_{coax1} := 2 \quad (User Input)$$

Sprint Cables on PCS Mast

Extreme Wind Pressure =

$$qz := 28.6 \cdot psf \quad (User Input)$$

Heavy Wind Pressure =

$$p := 4 \cdot psf \quad (User Input)$$

Radial Ice Thickness =

$$Ir := 0.5 \cdot in \quad (User Input)$$

Radial Ice Density =

$$Id := 56 \cdot pcf \quad (User Input)$$

Shape Factor =

$$Cd_{coax} := 1.45 \quad (User Input)$$

Overload Factor for NESC Heavy Wind Load =

$$OF_{HW} := 2.5 \quad (User Input)$$

Overload Factor for NESC Extreme Wind Load =

$$OF_{EW} := 1.0 \quad (User Input)$$

Overload Factor for NESC Heavy Vertical Load =

$$OF_{HV} := 1.5 \quad (User Input)$$

Overload Factor for NESC Extreme Vertical Load =

$$OF_{EV} := 1.0 \quad (User Input)$$

Wind Area with Ice =

$$A_{ice} := (NP_{coax1} \cdot D_{coax1} + 2 \cdot Ir) = 4.1 \cdot in$$

Wind Area without Ice =

$$A := (NP_{coax1} \cdot D_{coax1}) = 3.1 \cdot in$$

Ice Area per Liner Ft =

$$A_{i_{coax1}} := \frac{\pi}{4} \cdot \left[(D_{coax1} + 2 \cdot Ir)^2 - D_{coax1}^2 \right] = 0.022 \cdot ft^2$$

Weight of Ice on All Coax Cables =

$$W_{ice} := A_{i_{coax1}} \cdot Id \cdot N_{coax1} = 19 \cdot plf$$

Heavy Vertical Load =

$$\text{HeavyVert} := \overrightarrow{[(N_{\text{coax1}} \cdot W_{\text{coax1}} + W_{\text{ice}}) \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{HV}}]}$$

Heavy Transverse Load =

$$\text{HeavyTrans} := \overrightarrow{(p \cdot A_{\text{ice}} \cdot C_{\text{dcoax}} \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{HW}})}$$

$$\text{HeavyVert} = \begin{pmatrix} 430 \\ 430 \\ 430 \\ 430 \end{pmatrix} \text{ lb}$$

$$\text{HeavyTrans} = \begin{pmatrix} 50 \\ 50 \\ 50 \\ 50 \end{pmatrix} \text{ lb}$$

Extreme Vertical Load =

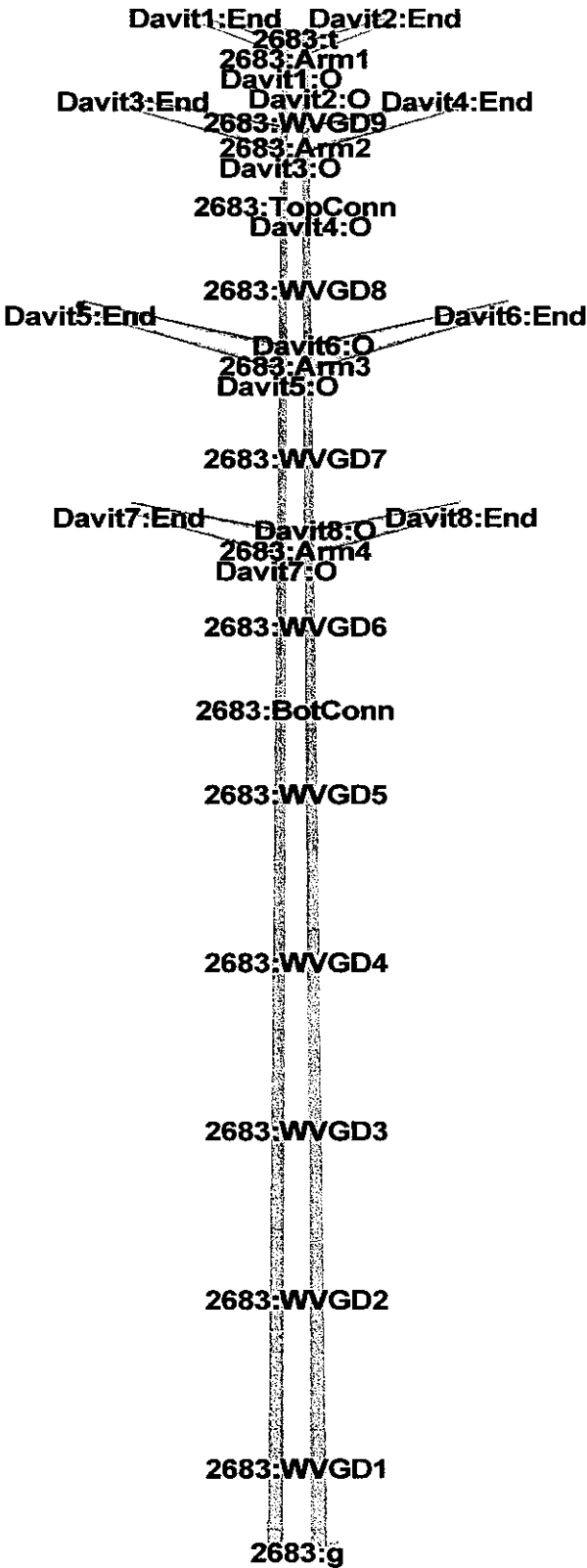
$$\text{ExtremeVert} := \overrightarrow{[(N_{\text{coax1}} \cdot W_{\text{coax1}}) \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{EV}}]}$$

Extreme Transverse Load =

$$\text{ExtremeTrans} := \overrightarrow{[(qz \cdot A \cdot C_{\text{dcoax}}) \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{EW}}]}$$

$$\text{ExtremeVert} = \begin{pmatrix} 99 \\ 99 \\ 99 \\ 99 \end{pmatrix} \text{ lb}$$

$$\text{ExtremeTrans} = \begin{pmatrix} 107 \\ 107 \\ 107 \\ 107 \end{pmatrix} \text{ lb}$$



Project Name : 12047.CO13 - Brookfield, CT
 Project Notes: CL&P # 2683/ Sprint - CT54XC713
 Project File : J:\Jobs\1204700.WI\CO13 - CT54XC713\Calcs\PLS Pole\cl&p structure # 2683.pol
 Date run : 8:50:46 AM Thursday, April 18, 2013
 by : PLS-POLE Version 11.11
 Licensed to : Centek Engineering Inc

Successfully performed nonlinear analysis

The model has 0 warnings.

Loads from file: j:\jobs\1204700.wi\co13 - ct54xc713\cales\pls pole\cl&p #2683.lca

*** Analysis Results:

Maximum element usage is 70.57% for Steel Pole "2683" in load case "NESC Heavy"
 Maximum insulator usage is 14.73% for Clamp "Clamp21" in load case "NESC Extreme"

Summary of Joint Support Reactions For All Load Cases:

Load Case	Joint Label	Long. Force (kips)	Tran. Force (kips)	Vert. Force (kips)	Shear Force (kips)	Tran. Moment (ft-k)	Long. Moment (ft-k)	Vert. Moment (ft-k)	Bending Found. %
NESC Heavy	2683:g	-0.09	-24.81	50.88	24.81	1892.67	-3.74	-0.01	1892.67 0.00
NESC Extreme	2683:g	-0.03	-26.69	24.93	26.69	1911.26	-1.17	-0.00	1911.26 0.00

Summary of Tip Deflections For All Load Cases:

Note: positive tip load results in positive rotation

Load Case	Joint Label	Long. Defl. (in)	Tran. Defl. (in)	Vert. Resultant Defl. (in)	Long. Rot. (deg)	Tran. Rot. (deg)	Twist (deg)
NESC Heavy	2683:t	0.05	36.67	-0.84	36.68	0.00	-3.37 0.00
NESC Extreme	2683:t	0.02	36.64	-0.82	36.65	0.00	-3.32 0.00

Tubes Summary:

Pole Label Num.	Tube Weight (lbs)	Load Case Usage %	Maximum Resultant Moment (ft-k)
2683 1	3397	NESC Heavy 47.89	552.18
2683 2	5750	NESC Heavy 61.15	1394.02
2683 3	3427	NESC Heavy 70.57	1892.67

*** Overall summary for all load cases - Usage = Maximum Stress / Allowable Stress

Summary of Steel Pole Usages:

Steel Pole Label	Pole Maximum Usage %	Load Case Segment Number	Weight (lbs)
2683	70.57	NESC Heavy 25	13920.9

Summary of Tubular Davit Usages:

Centek Engineering Inc - cl&p structure # 2683

Tubular Davit Maximum		Load Case Segment Weight	
Label	Usage %	Number	(lbs)
Davit1	1.36	NESC Extreme	1 77.9
Davit2	8.43	NESC Heavy	1 77.9
Davit3	2.27	NESC Heavy	1 321.0
Davit4	4.97	NESC Heavy	1 321.0
Davit5	3.02	NESC Heavy	1 434.7
Davit6	7.07	NESC Heavy	1 434.7
Davit7	2.35	NESC Heavy	1 321.0
Davit8	5.03	NESC Heavy	1 321.0

*** Maximum Stress Summary for Each Load Case

Summary of Maximum Usages by Load Case:

Load Case Maximum Element		Element	
Usage %	Label	Type	
NESC Heavy	70.57	2683 Steel Pole	
NESC Extreme	70.33	2683 Steel Pole	

Summary of Steel Pole Usages by Load Case:

Load Case Maximum Steel Pole Segment	
Usage %	Label Number
NESC Heavy	70.57 2683 25
NESC Extreme	70.33 2683 25

Summary of Base Plate Usages by Load Case:

Load Case Pole Bend Length Vertical		X		Y Bending		Bolt		# Bolts		Max Bolt		Minimum		Usage	
Label Line	#	(in)	(kips)	Moment	(ft-k)	(ksi)	(ft-k)	Moment	(ft-k)	Sum	Bend Line	Acting On	Plate	Thickness	
NESC Heavy	2683	6 30.544	50.878	1892.667	-3.740	31.983	122.110	6	85.898	2.288	58.15				
NESC Extreme	2683	6 30.544	24.935	1911.258	-1.173	31.473	120.164	6	84.304	2.269	57.22				

Summary of Tubular Davit Usages by Load Case:

Load Case Maximum Tubular Davit Segment	
Usage %	Label Number
NESC Heavy	8.43 Davit2 1
NESC Extreme	3.19 Davit2 1

Summary of Insulator Usages:

Insulator Insulator Maximum		Load Case Weight	
Label	Type Usage %	(lbs)	
Clamp1	Clamp 3.00	NESC Heavy	0.0
Clamp2	Clamp 3.00	NESC Heavy	0.0
Clamp3	Clamp 3.29	NESC Heavy	0.0

Clamp4	Clamp	3.29	NESC Heavy	0.0
Clamp5	Clamp	3.29	NESC Heavy	0.0
Clamp6	Clamp	3.29	NESC Heavy	0.0
Clamp7	Clamp	3.29	NESC Heavy	0.0
Clamp8	Clamp	3.29	NESC Heavy	0.0
Clamp9	Clamp	1.19	NESC Heavy	0.0
Clamp10	Clamp	1.19	NESC Heavy	0.0
Clamp11	Clamp	1.19	NESC Heavy	0.0
Clamp12	Clamp	1.19	NESC Heavy	0.0
Clamp13	Clamp	1.19	NESC Heavy	0.0
Clamp14	Clamp	0.54	NESC Heavy	0.0
Clamp15	Clamp	0.54	NESC Heavy	0.0
Clamp16	Clamp	0.54	NESC Heavy	0.0
Clamp17	Clamp	0.54	NESC Heavy	0.0
Clamp21	Clamp	14.73	NESC Extreme	0.0
Clamp23	Clamp	3.79	NESC Extreme	0.0

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*** Weight of structure (lbs):
Weight of Tubular Davit Arms:      2309.3
Weight of Steel Poles:             13920.9
Total:                             16230.2

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*** End of Report

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*
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* PLE AND FRAME ANALYSIS AND DESIGN
* Copyright Power Line Systems, Inc. 1999-2011
*

Project Name : 12047.CO13 - Brookfield, CT
Project Notes: CL&P # 2683/ Sprint - CT54XC713
Project File : J:\Jobs\1204700.WI\CO13 - CT54XC713\Calcs\PLS Pole\cl&p structure # 2683.pol
Date run : 8:50:45 AM Thursday, April 18, 2013
by : PLS-POLE Version 11.11
Licensed to : Centek Engineering Inc

Successfully performed nonlinear analysis

The model has 0 warnings.

Modeling options:

Offset Arms from Pole/Mast: Yes
Offset Braces from Pole/Mast: Yes
Offset Guys from Pole/Mast: Yes
Offset Posts from Pole/Mast: Yes
Offset Strains from Pole/Mast: Yes
Use Alternate Convergence Process: No
Steel poles checked with ASCE/SEI 48-05

Default Modulus of Elasticity for Steel = 29000.00 (ksi)
Default Weight Density for Steel = 490.00 (lbs/ft^3)

Steel Pole Properties:

Steel Pole Ultimate Property Number	Stock Length	Default	Base	Shape	Tip	Base	Taper Default	Tubes	Modulus of Elasticity	Weight Density	Shape	Strength Distance
		Embedded	Plate		Diameter	Diameter		Drag			At	Check From

Trans. Label	Long. Length	Coef.	Override	Base	Type	Tip
Load	(ft)	(in)	(in/ft)	(ksi) (lbs/ft^3)		(ft)
(kips)	(kips)					
CL&P2683	2683	8F	19.63	43.21	0	0
0.0000	90.00	0	Yes	1.3	3 tubes	Calculated
0.0000	0.0000	0	0	0	0	0.000

Steel Tubes Properties:

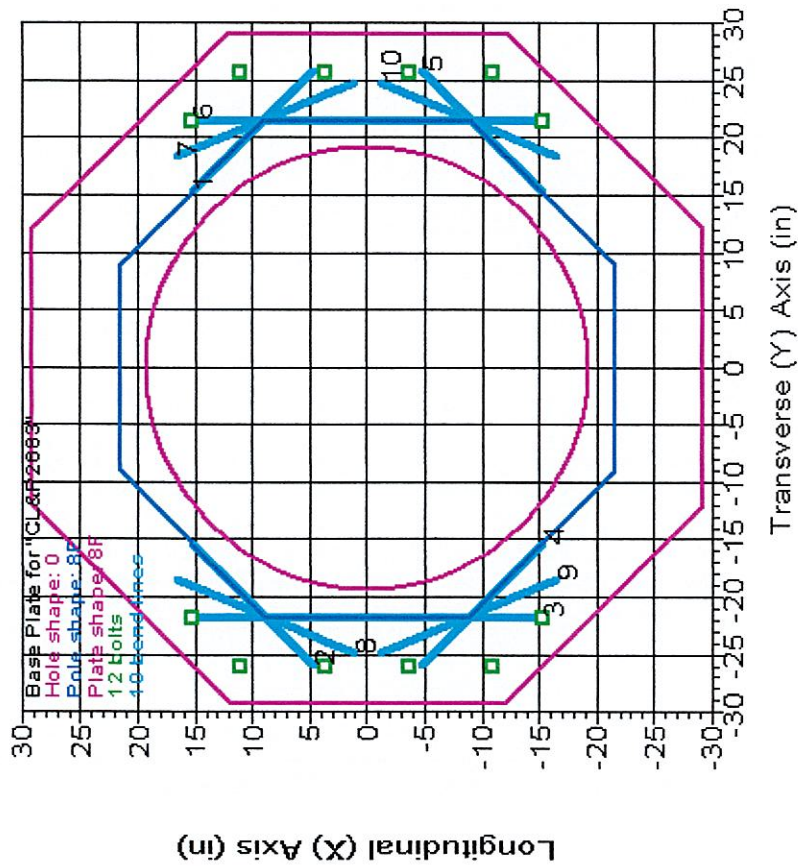
Pole Tube Property No.	Length (ft)	Thickness (in)	Lap Length (ft)	Lap Gap Stress (in)	Yield Moment Cap. (ft-k)	Tube Weight (lbs)	Center of Gravity (ft)	Calculated Tube Top Diameter (in)	Tube Bot. Diameter (in)	Lap Length (ft)
CL&P2683 1	39	0.3125	4.167	0.000	65.000	0.000	3397	0.27728	19.63	30.44
CL&P2683 2	40.1667	0.375	5.250	0.000	65.000	0.000	5750	0.27728	28.66	39.80
CL&P2683 3	20.25	0.375	0.000	0.000	65.000	0.000	3427	0.27728	37.59	43.20

Base Plate Properties:

Pole Plate Property	Plate Diam.	Plate Shape	Plate Thick.	Plate Weight (lbs)	Plate Bend Length (in)	Hole Diam.	Hole Shape	Hole Size	Steel Density (lbs/ft^3)	Steel Yield Stress (ksi)	Bolt Diam.	Bolt Pattern	Bolt Num.	Cage X	Cage Y	Bolt Inertia (in^4)
CL&P2683 58.250	8F	3.000	1347	0.000	38.750	0	490.00	55.000	2.250	51.750	12	28733.01	5839.56			

Base Plate Bolt Coordinates for Property "CL&P2683":

Bolt X Coord.	Bolt Y Coord.	Bolt Angle (deg)
0.5894	0.8357	0
0.4251	1	0
0.1425	1	0



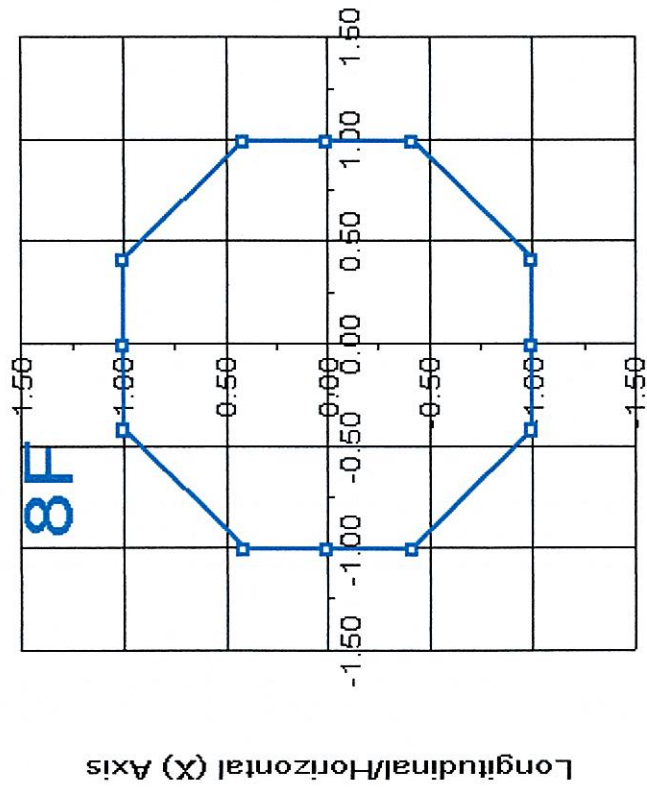
Steel Pole Connectivity:

Pole Label	Tip Joint	Base Joint	X of Base (ft)	Y of Base (ft)	Z of Base (ft)	Inclin. About X (deg)	Inclin. About Y (deg)	Property Set	Attach Labels	Base Connect	Embed & Override (ft)
2683			0	0	0	0	0	CL&P2683	15 labels	Fixed	0.00

Relative Attachment Labels for Steel Pole "2683":

Joint Label	Distance From Origin/Top Joint (ft)	Global Z of Attach (ft)
2683:WVGD1	0.00	5.00
2683:WVGD2	0.00	15.00
2683:WVGD3	0.00	25.00
2683:WVGD4	0.00	35.00
2683:WVGD5	0.00	45.00

2683:WVGDD6	0.00	55.00
2683:WVGDD7	0.00	65.00
2683:WVGDD8	0.00	75.00
2683:TopConn	0.00	80.00
2683:BotConn	0.00	50.00
2683:WVGDD9	0.00	85.00
2683:Arm1	0.00	88.92
2683:Arm2	0.00	83.50
2683:Arm3	0.00	70.50
2683:Arm4	0.00	59.50



Transverse/Vertical (Y) Axis

Pole Steel Properties:

Warning: Capacities and usages printed in splices are listed for the inner tube except at the splice top which uses the outer tube. ??

Element Label	Joint Label	Joint Position	Rel. Outer Dist. (ft)	Area (in^2)	T-Moment Inertia (in^4)	D/t Max.	Fy (ksi)	Fa Min. (ksi)	ASCE Cap Trans. (ft-k)	ASCE Cap Long. (ft-k)
2683	2683:t	2683:t Ori	0.00	19.63	20.00	985.95	0.00	21.9	65.00	544.26
2683	2683:Arm1	2683:Arm1 End	1.08	19.92	20.31	1032.52	0.00	22.3	65.00	561.40
2683	2683:Arm1	2683:Arm1 Ori	1.08	19.92	20.31	1032.52	0.00	22.3	65.00	561.40
2683	2683:WVGDD9	2683:WVGDD9 End	5.00	21.01	21.43	1213.85	0.00	23.7	65.00	625.85
2683	2683:WVGDD9	2683:WVGDD9 Ori	5.00	21.01	21.43	1213.85	0.00	23.7	65.00	625.85

Davit Steel Property Shape Label At End	Stock Number	Steel Shape	Base Diameter	Tip Diameter	Taper	Drill	Modulus of Elasticity (ksi)	Geometry	Strength	Vertical Capacity	Tension Capacity	Compressive Capacity	Long. Capacity	Yield Stress	Weight Density
			(in)	(in)	(in/ft)	Coef.			Check	(lbs)	(lbs)	(lbs)	(lbs)	(ksi)	(lbs/ft^3)
2683	2683:Arm2	2683:Arm2 End	6.50	21.43	21.87	1288.49	1288.49	0.00 24.3	65.00 65.00	651.44	651.44	651.44	651.44		
2683	2683:Arm2	2683:Arm2 Ori	6.50	21.43	21.87	1288.49	1288.49	0.00 24.3	65.00 65.00	651.44	651.44	651.44	651.44		
2683	2683:TopConn	2683:TopConn End	10.00	22.40	22.87	1474.42	1474.42	0.00 25.5	65.00 65.00	713.14	713.14	713.14	713.14		
2683	2683:TopConn	2683:TopConn Ori	10.00	22.40	22.87	1474.42	1474.42	0.00 25.5	65.00 65.00	713.15	713.15	713.15	713.15		
2683	2683:WVG8	2683:WVG8 End	15.00	23.78	24.31	1769.84	1769.84	0.00 27.4	65.00 65.00	806.14	806.14	806.14	806.14		
2683	2683:WVG8	2683:WVG8 Ori	15.00	23.78	24.31	1769.84	1769.84	0.00 27.4	65.00 65.00	806.14	806.14	806.14	806.14		
2683	2683:Arm3	2683:Arm3 End	19.50	25.03	25.60	2067.33	2067.33	0.00 29.0	65.00 65.00	894.70	894.70	894.70	894.70		
2683	2683:Arm3	2683:Arm3 Ori	19.50	25.03	25.60	2067.33	2067.33	0.00 29.0	65.00 65.00	894.70	894.70	894.70	894.70		
2683	2683:0	Tube 1 End	22.25	25.79	26.39	2264.58	2264.58	0.00 30.0	65.00 65.00	951.09	951.09	951.09	951.09		
2683	2683:0	Tube 1 Ori	22.25	25.79	26.39	2264.58	2264.58	0.00 30.0	65.00 65.00	951.09	951.09	951.09	951.09		
2683	2683:WVG7	2683:WVG7 End	25.00	26.56	27.18	2474.00	2474.00	0.00 31.1	65.00 65.00	1009.21	1009.21	1009.21	1009.21		
2683	2683:WVG7	2683:WVG7 Ori	25.00	26.56	27.18	2474.00	2474.00	0.00 31.1	65.00 65.00	1009.21	1009.21	1009.21	1009.21		
2683	2683:1	Tube 1 End	27.75	27.32	27.97	2695.95	2695.95	0.00 32.1	65.00 65.00	1069.06	1069.06	1069.06	1069.06		
2683	2683:1	Tube 1 Ori	27.75	27.32	27.97	2695.95	2695.95	0.00 32.1	65.00 65.00	1069.06	1069.06	1069.06	1069.06		
2683	2683:Arm4	2683:Arm4 End	30.50	28.08	28.76	2930.78	2930.78	0.00 33.1	65.00 64.24	1117.35	1117.35	1117.35	1117.35		
2683	2683:Arm4	2683:Arm4 Ori	30.50	28.08	28.76	2930.78	2930.78	0.00 33.1	65.00 64.24	1117.35	1117.35	1117.35	1117.35		
2683	2683:2	SpliceT End	34.83	29.28	30.00	3327.87	3327.87	0.00 34.7	65.00 62.89	1191.09	1191.09	1191.09	1191.09		
2683	2683:2	SpliceT Ori	34.83	29.28	30.00	3327.87	3327.87	0.00 34.7	65.00 62.89	1191.10	1191.10	1191.10	1191.10		
2683	2683:WVG6	2683:WVG6 End	35.00	28.70	35.20	3734.31	3734.31	0.00 27.6	65.00 65.00	1409.35	1409.35	1409.35	1409.35		
2683	2683:WVG6	2683:WVG6 Ori	35.00	28.70	35.20	3734.31	3734.31	0.00 27.6	65.00 65.00	1409.35	1409.35	1409.35	1409.35		
2683	2683:3	SpliceB End	39.00	29.81	36.58	4190.24	4190.24	0.00 28.8	65.00 65.00	1522.59	1522.59	1522.59	1522.59		
2683	2683:3	SpliceB Ori	39.00	29.81	36.58	4190.24	4190.24	0.00 28.8	65.00 65.00	1522.59	1522.59	1522.59	1522.59		
2683	2683:BotConn	2683:BotConn End	40.00	30.09	36.93	4309.75	4309.75	0.00 29.1	65.00 65.00	1551.58	1551.58	1551.58	1551.58		
2683	2683:BotConn	2683:BotConn Ori	40.00	30.09	36.93	4309.75	4309.75	0.00 29.1	65.00 65.00	1551.58	1551.58	1551.58	1551.58		
2683	2683:WVG5	2683:WVG5 End	45.00	31.48	38.65	4941.47	4941.47	0.00 30.6	65.00 65.00	1700.66	1700.66	1700.66	1700.66		
2683	2683:WVG5	2683:WVG5 Ori	45.00	31.48	38.65	4941.47	4941.47	0.00 30.6	65.00 65.00	1700.66	1700.66	1700.66	1700.66		
2683	2683:4	Tube 2 End	50.00	32.86	40.37	5632.09	5632.09	0.00 32.2	65.00 65.00	1856.57	1856.57	1856.57	1856.57		
2683	2683:4	Tube 2 Ori	50.00	32.86	40.37	5632.09	5632.09	0.00 32.2	65.00 65.00	1856.57	1856.57	1856.57	1856.57		
2683	2683:WVG4	2683:WVG4 End	55.00	34.25	42.09	6384.24	6384.24	0.00 33.7	65.00 63.72	1979.56	1979.56	1979.56	1979.56		
2683	2683:WVG4	2683:WVG4 Ori	55.00	34.25	42.09	6384.24	6384.24	0.00 33.7	65.00 63.72	1979.56	1979.56	1979.56	1979.56		
2683	2683:5	Tube 2 End	60.00	35.64	43.82	7200.54	7200.54	0.00 35.2	65.00 62.42	2102.06	2102.06	2102.06	2102.06		
2683	2683:5	Tube 2 Ori	60.00	35.64	43.82	7200.54	7200.54	0.00 35.2	65.00 62.42	2102.06	2102.06	2102.06	2102.06		
2683	2683:WVG3	2683:WVG3 End	65.00	37.02	45.54	8083.61	8083.61	0.00 36.8	65.00 61.12	2224.22	2224.22	2224.22	2224.22		
2683	2683:WVG3	2683:WVG3 Ori	65.00	37.02	45.54	8083.61	8083.61	0.00 36.8	65.00 61.12	2224.22	2224.22	2224.22	2224.22		
2683	2683:6	SpliceT End	69.75	38.34	47.18	8986.78	8986.78	0.00 38.2	65.00 59.89	2339.57	2339.57	2339.57	2339.57		
2683	2683:6	SpliceT Ori	69.75	38.34	47.18	8986.78	8986.78	0.00 38.2	65.00 59.89	2339.57	2339.57	2339.57	2339.57		
2683	2683:7	Tube 2 End	72.38	38.32	47.15	8971.06	8971.06	0.00 38.2	65.00 59.91	2337.64	2337.64	2337.64	2337.64		
2683	2683:7	Tube 2 Ori	72.38	38.32	47.15	8971.06	8971.06	0.00 38.2	65.00 59.91	2337.64	2337.64	2337.64	2337.64		
2683	2683:WVG2	2683:WVG2 End	75.00	39.05	48.05	9497.27	9497.27	0.00 39.0	65.00 59.23	2400.97	2400.97	2400.97	2400.97		
2683	2683:WVG2	2683:WVG2 Ori	75.00	39.05	48.05	9497.27	9497.27	0.00 39.0	65.00 59.23	2400.97	2400.97	2400.97	2400.97		
2683	2683:8	Tube 3 End	80.00	40.43	49.78	10555.72	10555.72	0.00 40.5	65.00 57.93	2520.52	2520.52	2520.52	2520.52		
2683	2683:8	Tube 3 Ori	80.00	40.43	49.78	10555.72	10555.72	0.00 40.5	65.00 57.93	2520.52	2520.52	2520.52	2520.52		
2683	2683:WVG1	2683:WVG1 End	85.00	41.82	51.50	11690.03	11690.03	0.00 42.0	65.00 56.63	2638.31	2638.31	2638.31	2638.31		
2683	2683:WVG1	2683:WVG1 Ori	85.00	41.82	51.50	11690.03	11690.03	0.00 42.0	65.00 56.63	2638.31	2638.31	2638.31	2638.31		
2683	2683:g	2683:g End	90.00	43.20	53.22	12902.82	12902.82	0.00 43.6	65.00 55.27	2751.00	2751.00	2751.00	2751.00		

Tubular Davit Properties:

Davit Steel Property Shape Label At End	Stock Number	Steel Shape	Base Diameter	Tip Diameter	Taper	Drill	Modulus of Elasticity (ksi)	Geometry	Strength	Vertical Capacity	Tension Capacity	Compressive Capacity	Long. Capacity	Yield Stress	Weight Density
			(in)	(in)	(in/ft)	Coef.			Check	(lbs)	(lbs)	(lbs)	(lbs)	(ksi)	(lbs/ft^3)
ARM A	0	0.1875	10.76	6	0	1.3	29000	1 point	Calculated	0	0	0	0	65	0

ARM B 0 0.25 18.5 9 0 1.3 29000 1 point Calculated 0 0 65 0
ARM C 0 0.25 18.5 9 0 1.3 29000 1 point Calculated 0 0 65 0

Intermediate Joints for Davit Property "ARM A":

Joint Horz. Vert.
Label Offset Offset
(ft) (ft)

End 4.5 -1.5

Intermediate Joints for Davit Property "ARM B":

Joint Horz. Vert.
Label Offset Offset
(ft) (ft)

End 8.67 -2

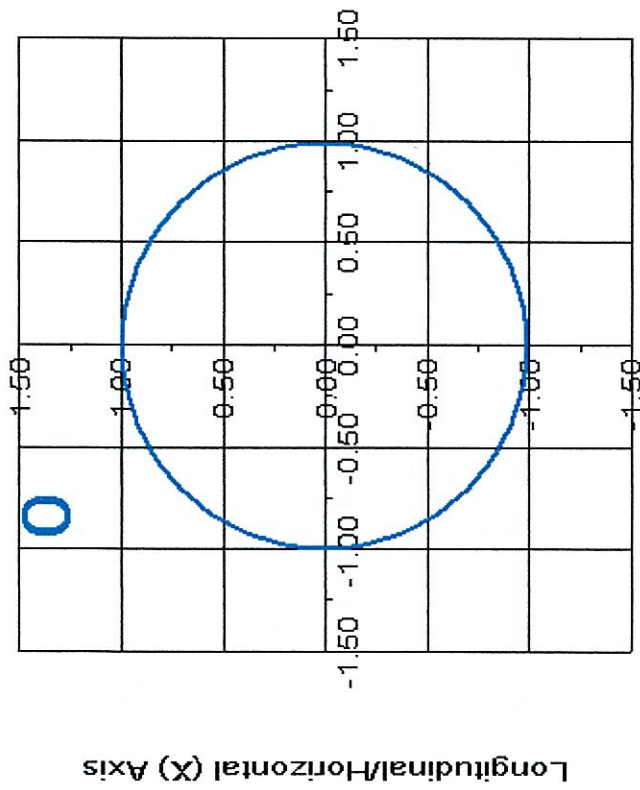
Intermediate Joints for Davit Property "ARM C":

Joint Horz. Vert.
Label Offset Offset
(ft) (ft)

End 11.67 -3

Tubular Davit Arm Connectivity:

Davit Label	Attach Label	Davit Property Set	Davit Azimuth (deg)
Davit1	2683:Arm1	ARM A	180
Davit2	2683:Arm1	ARM A	0
Davit3	2683:Arm2	ARM B	180
Davit4	2683:Arm2	ARM B	0
Davit5	2683:Arm3	ARM C	180
Davit6	2683:Arm3	ARM C	0
Davit7	2683:Arm4	ARM B	180
Davit8	2683:Arm4	ARM B	0



Transverse/Vertical (Y) Axis

Tubular Davit Arm Steel Properties:

Element Label	Joint Label	Joint Position	Rel. Dist. (ft)	Outer Diam. (in)	Area (in ²)	V-Moment Inertia (in ⁴)	H-Moment Inertia (in ⁴)	D/t	W/t Max.	Fy (ksi)	Fa Min. (ksi)	ASCE V-Mom. (ft-k)	ASCE H-Mom. (ft-k)
Davit1	Davit1:0	Origin	0.00	10.76	6.23	87.04	87.04	57.39	0.0	65.00	65.00	87.64	87.64
Davit1	Davit1:End	End	4.74	6.00	3.42	14.47	14.47	32.00	0.0	65.00	65.00	26.13	26.13
Davit2	Davit2:0	Origin	0.00	10.76	6.23	87.04	87.04	57.39	0.0	65.00	65.00	87.64	87.64
Davit2	Davit2:End	End	4.74	6.00	3.42	14.47	14.47	32.00	0.0	65.00	65.00	26.13	26.13
Davit3	Davit3:0	Origin	0.00	18.50	14.33	596.86	596.86	74.00	0.0	65.00	65.00	349.51	349.51
Davit3	#Davit3:0	End	4.45	13.75	10.60	241.63	241.63	55.00	0.0	65.00	65.00	190.37	190.37
Davit3	#Davit3:0	Origin	4.45	13.75	10.60	241.63	241.63	55.00	0.0	65.00	65.00	190.37	190.37
Davit3	Davit3:End	End	8.90	9.00	6.87	65.82	65.82	36.00	0.0	65.00	65.00	79.23	79.23
Davit4	Davit4:0	Origin	0.00	18.50	14.33	596.86	596.86	74.00	0.0	65.00	65.00	349.51	349.51
Davit4	#Davit4:0	End	4.45	13.75	10.60	241.63	241.63	55.00	0.0	65.00	65.00	190.37	190.37
Davit4	#Davit4:0	Origin	4.45	13.75	10.60	241.63	241.63	55.00	0.0	65.00	65.00	190.37	190.37
Davit4	Davit4:End	End	8.90	9.00	6.87	65.82	65.82	36.00	0.0	65.00	65.00	79.23	79.23
Davit5	Davit5:0	Origin	0.00	18.50	14.33	596.86	596.86	74.00	0.0	65.00	65.00	349.51	349.51

Davit5	#Davit5:0	End	5.00	14.56	11.24	287.65	287.65	58.23	0.0	65.00	65.00	214.05	214.05
Davit5	#Davit5:0	Origin	5.00	14.56	11.24	287.65	287.65	58.23	0.0	65.00	65.00	214.05	214.05
Davit5	#Davit5:1	End	8.52	11.78	9.05	150.51	150.51	47.12	0.0	65.00	65.00	138.43	138.43
Davit5	#Davit5:1	Origin	8.52	11.78	9.05	150.51	150.51	47.12	0.0	65.00	65.00	138.43	138.43
Davit5	Davit5:End	End	12.05	9.00	6.87	65.82	65.82	36.00	0.0	65.00	65.00	79.23	79.23
Davit6	Davit6:0	Origin	0.00	18.50	14.33	596.86	596.86	74.00	0.0	65.00	65.00	349.51	349.51
Davit6	#Davit6:0	End	5.00	14.56	11.24	287.65	287.65	58.23	0.0	65.00	65.00	214.05	214.05
Davit6	#Davit6:0	Origin	5.00	14.56	11.24	287.65	287.65	58.23	0.0	65.00	65.00	214.05	214.05
Davit6	#Davit6:1	End	8.52	11.78	9.05	150.51	150.51	47.12	0.0	65.00	65.00	138.43	138.43
Davit6	#Davit6:1	Origin	8.52	11.78	9.05	150.51	150.51	47.12	0.0	65.00	65.00	138.43	138.43
Davit6	Davit6:End	End	12.05	9.00	6.87	65.82	65.82	36.00	0.0	65.00	65.00	79.23	79.23
Davit7	Davit7:0	Origin	0.00	18.50	14.33	596.86	596.86	74.00	0.0	65.00	65.00	349.51	349.51
Davit7	#Davit7:0	End	4.45	13.75	10.60	241.63	241.63	55.00	0.0	65.00	65.00	190.37	190.37
Davit7	#Davit7:0	Origin	4.45	13.75	10.60	241.63	241.63	55.00	0.0	65.00	65.00	190.37	190.37
Davit7	Davit7:End	End	8.90	9.00	6.87	65.82	65.82	36.00	0.0	65.00	65.00	79.23	79.23
Davit8	Davit8:0	Origin	0.00	18.50	14.33	596.86	596.86	74.00	0.0	65.00	65.00	349.51	349.51
Davit8	#Davit8:0	End	4.45	13.75	10.60	241.63	241.63	55.00	0.0	65.00	65.00	190.37	190.37
Davit8	#Davit8:0	Origin	4.45	13.75	10.60	241.63	241.63	55.00	0.0	65.00	65.00	190.37	190.37
Davit8	Davit8:End	End	8.90	9.00	6.87	65.82	65.82	36.00	0.0	65.00	65.00	79.23	79.23

*** Insulator Data

Clamp Properties:

Label	Stock	Holding	Number	Capacity
				(lbs)
clamp	clamp1	8e+004		

Clamp Insulator Connectivity:

Clamp Label	Structure And Tip Attach	Property Min. Set	Required Vertical Load (uplift)
Clamp1	Davit1:End	Clamp	No Limit
Clamp2	Davit2:End	Clamp	No Limit
Clamp3	Davit3:End	Clamp	No Limit
Clamp4	Davit4:End	Clamp	No Limit
Clamp5	Davit5:End	Clamp	No Limit
Clamp6	Davit6:End	Clamp	No Limit
Clamp7	Davit7:End	Clamp	No Limit
Clamp8	Davit8:End	Clamp	No Limit
Clamp9	2683:WVGD1	Clamp	No Limit
Clamp10	2683:WVGD2	Clamp	No Limit
Clamp11	2683:WVGD3	Clamp	No Limit
Clamp12	2683:WVGD4	Clamp	No Limit
Clamp13	2683:WVGD5	Clamp	No Limit
Clamp14	2683:WVGD6	Clamp	No Limit
Clamp15	2683:WVGD7	Clamp	No Limit
Clamp16	2683:WVGD8	Clamp	No Limit
Clamp17	2683:WVGD9	Clamp	No Limit
Clamp21	2683:TopConn	Clamp	No Limit
Clamp23	2683:BotConn	Clamp	No Limit

Loads from file: j:\jobs\1204700.wi\col3 - ct54xc713\calcs\pls pole\cl&p #2683.lca

Loading Method Parameters:

Structure Height Summary (used for calculating wind/ice adjust with height):

structure height summary (used for calculating Z of ground for height adjust 0.00 (ft) and structure Z coordinate that will be put on the centerline ground profile in PLS-CADD.

2 of ground for wind height adjust 0.00 (ft)
Ground elevation shift 0.00 (ft)

Ground elevation shift	0.00 (ft)
2 of ground with shift	0.00 (ft)

Point	Location	Depth (ft)	Temperature (°C)	Temperature (°F)
1	Surface	0.00	10.00	50.00
2	10 m	10.00	10.00	50.00
3	20 m	20.00	10.00	50.00
4	30 m	30.00	10.00	50.00
5	40 m	40.00	10.00	50.00
6	50 m	50.00	10.00	50.00
7	60 m	60.00	10.00	50.00
8	70 m	70.00	10.00	50.00
9	80 m	80.00	10.00	50.00
10	90 m	90.00	10.00	50.00
11	100 m	100.00	10.00	50.00
12	110 m	110.00	10.00	50.00
13	120 m	120.00	10.00	50.00
14	130 m	130.00	10.00	50.00
15	140 m	140.00	10.00	50.00
16	150 m	150.00	10.00	50.00
17	160 m	160.00	10.00	50.00
18	170 m	170.00	10.00	50.00
19	180 m	180.00	10.00	50.00
20	190 m	190.00	10.00	50.00
21	200 m	200.00	10.00	50.00
22	210 m	210.00	10.00	50.00
23	220 m	220.00	10.00	50.00
24	230 m	230.00	10.00	50.00
25	240 m	240.00	10.00	50.00
26	250 m	250.00	10.00	50.00
27	260 m	260.00	10.00	50.00
28	270 m	270.00	10.00	50.00
29	280 m	280.00	10.00	50.00
30	290 m	290.00	10.00	50.00
31	300 m	300.00	10.00	50.00
32	310 m	310.00	10.00	50.00
33	320 m	320.00	10.00	50.00
34	330 m	330.00	10.00	50.00
35	340 m	340.00	10.00	50.00
36	350 m	350.00	10.00	50.00
37	360 m	360.00	10.00	50.00
38	370 m	370.00	10.00	50.00
39	380 m	380.00	10.00	50.00
40	390 m	390.00	10.00	50.00
41	400 m	400.00	10.00	50.00
42	410 m	410.00	10.00	50.00
43	420 m	420.00	10.00	50.00
44	430 m	430.00	10.00	50.00
45	440 m	440.00	10.00	50.00
46	450 m	450.00	10.00	50.00
47	460 m	460.00	10.00	50.00
48	470 m	470.00	10.00	50.00
49	480 m	480.00	10.00	50.00
50	490 m	490.00	10.00	50.00
51	500 m	500.00	10.00	50.00
52	510 m	510.00	10.00	50.00
53	520 m	520.00	10.00	50.00
54	530 m	530.00	10.00	50.00
55	540 m	540.00	10.00	50.00
56	550 m	550.00	10.00	50.00
57	560 m	560.00	10.00	50.00
58	570 m	570.00	10.00	50.00
59	580 m	580.00	10.00	50.00
60	590 m	590.00	10.00	50.00
61	600 m	600.00	10.00	50.00
62	610 m	610.00	10.00	50.00
63	620 m	620.00	10.00	50.00
64	630 m	630.00	10.00	50.00
65	640 m	640.00	10.00	50.00
66	650 m	650.00	10.00	50.00
67	660 m	660.00	10.00	50.00
68	670 m	670.00	10.00	50.00
69	680 m	680.00	10.00	50.0

Structure	Top of structure (highest joint)	Structure height
1	90.42 (ft)	90.42 (ft)

structure height	90.42 (ft)
structure height above ground	90.42 (ft)

[illegible]

	(in) (lbs/ft ³)	(deg F)	% or (ft)
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[illegible]

Point Loads for Load Case "NESC Heavy":

Joint Label	Vertical		Transverse		Longitudinal		Load Comment
	Load (lbs)		Load (lbs)		Load (lbs)		
Davit1:End	840		2250		0		
Davit2:End	840		2250		0		
Davit3:End	1263		2305		0		
Davit4:End	1263		2305		0		
Davit5:End	1263		2305		0		
Davit6:End	1263		2305		0		
Davit7:End	1263		2305		0		
Davit8:End	1263		2305		0		
2683:TopConn	10442		3639		0		
2683:BotConn	2329		-879		0		
2683:WVG01	947		87		0		
2683:WVG02	947		87		0		
2683:WVG03	947		87		0		
2683:WVG04	947		87		0		
2683:WVG05	947		87		0		
2683:WVG06	430		50		0		

2683:WVGD7 430 50 0
 2683:WVGD8 430 50 0
 2683:WVGD9 430 50 0

Point Loads for Load Case "NESC Extreme":

Joint Label	Vertical Load (lbs)	Transverse Load (lbs)	Longitudinal Load (lbs)	Comment
Davit1:End	208	1129	0	
Davit2:End	208	1129	0	
Davit3:End	367	1484	0	
Davit4:End	367	1484	0	
Davit5:End	367	1484	0	
Davit6:End	367	1484	0	
Davit7:End	367	1484	0	
Davit8:End	367	1484	0	
2683:TopConn	4767	10779	0	
2683:BotConn	1058	-2841	0	
2683:WVGD1	218	214	0	
2683:WVGD2	218	214	0	
2683:WVGD3	218	214	0	
2683:WVGD4	218	214	0	
2683:WVGD5	218	214	0	
2683:WVGD6	99	107	0	
2683:WVGD7	99	107	0	
2683:WVGD8	99	107	0	
2683:WVGD9	99	107	0	

Detailed Pole Loading Data for Load Case "NESC Extreme":

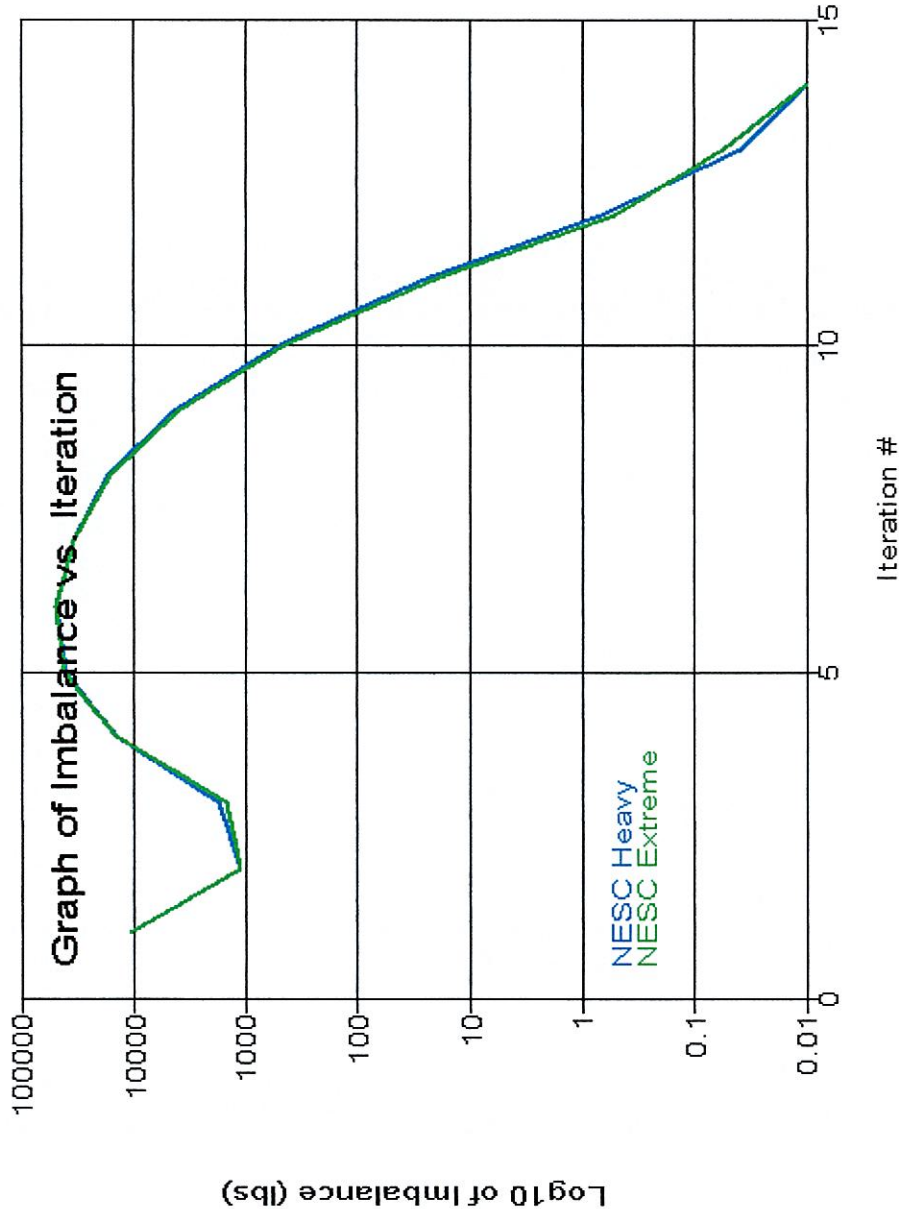
Notes: Does not include loads from equipment, arms, guys, braces, etc. or user input loads.
 Wind load is calculated for the undeformed shape of a pole.

Pole Label	Top Joint	Bottom Joint	Section Top Z (ft)	Section Bottom Z (ft)	Section Average Diameter (in)	Outer Diameter (in)	Reynolds Number	Drag Coef.	Adjusted Wind Pressure (psf)	Adjusted Ice Thickness (in)	Pole Vert. Load (lbs)	Pole Wind Load (lbs)	Pole Ice Load (lbs)	Pole Ice Wind Load (lbs)	Long. Wind Load (lbs)
2683	2683:t	2683:Arm1	90.00	88.92	89.46	19.775	1.5e+006	1.000	25.60	0.00	74.07	45.56	0.00	0.00	45.56
2683	2683:Arm1	2683:WVGD9	88.92	85.00	86.96	20.468	1.55e+006	1.000	25.60	0.00	278.40	171.18	0.00	0.00	171.18
2683	2683:WVGD9	2683:Arm2	85.00	83.50	84.25	21.219	1.61e+006	1.000	25.60	0.00	110.50	67.91	0.00	0.00	67.91
2683	2683:Arm2	2683:TopConn	83.50	80.00	81.75	21.913	1.66e+006	1.000	25.60	0.00	266.39	163.63	0.00	0.00	163.63
2683	2683:TopConn	2683:WVGD8	80.00	75.00	77.50	23.091	1.75e+006	1.000	25.60	0.00	401.32	246.32	0.00	0.00	246.32
2683	2683:WVGD8	2683:Arm3	75.00	70.50	72.75	24.408	1.85e+006	1.000	25.60	0.00	382.07	234.34	0.00	0.00	234.34
2683	2683:Arm3	2683:WVGD7	70.50	67.75	69.13	25.413	1.93e+006	1.000	25.60	0.00	243.23	149.10	0.00	0.00	149.10
2683	2683:WVGD7	2683:Arm4	67.75	65.00	66.38	26.176	1.98e+006	1.000	25.60	0.00	250.62	153.58	0.00	0.00	153.58
2683	2683:Arm4	2683:WVGD6	65.00	62.25	63.63	26.938	2.04e+006	1.000	25.60	0.00	258.01	158.05	0.00	0.00	158.05
2683	2683:WVGD6	2683:BotConn	62.25	59.50	60.88	27.701	2.1e+006	1.000	25.60	0.00	265.40	162.52	0.00	0.00	162.52
2683	2683:BotConn	2683:WVGD5	59.50	55.17	57.33	28.683	2.17e+006	1.000	25.60	0.00	433.19	265.18	0.00	0.00	265.18
2683	2683:WVGD5	2683:Arm5	55.17	51.00	53.08	28.994	2.2e+006	1.000	25.60	0.00	36.98	10.31	0.00	0.00	10.31
2683	2683:Arm5	2683:WVGD4	51.00	50.00	50.50	29.259	2.22e+006	1.000	25.60	0.00	905.35	249.70	0.00	0.00	249.70
2683	2683:WVGD4	2683:BotConn	50.00	45.00	47.50	30.784	2.33e+006	1.000	25.60	0.00	642.92	328.39	0.00	0.00	328.39
2683	2683:BotConn	2683:WVGD3	45.00	40.00	42.50	32.171	2.44e+006	1.000	25.60	0.00	672.23	343.18	0.00	0.00	343.18
2683	2683:WVGD3	2683:Arm6	40.00	35.00	37.50	33.557	2.54e+006	1.000	25.60	0.00	701.54	357.97	0.00	0.00	357.97
2683	2683:Arm6	2683:WVGD2	35.00	30.00	32.50	34.943	2.65e+006	1.000	25.60	0.00	730.85	372.76	0.00	0.00	372.76
2683	2683:WVGD2	2683:WVGD1	30.00	25.00	27.50	36.330	2.75e+006	1.000	25.60	0.00	760.16	387.55	0.00	0.00	387.55

2683	2683:WVGD3	25.00	20.25	22.63	37.682	2.85e+006	1.000	25.60	0.00	749.30	381.87	0.00	381.87	0.00
2683		20.25	17.63	18.94	38.329	2.9e+006	1.000	25.60	0.00	842.55	214.66	0.00	214.66	0.00
2683	2683:WVGD2	17.63	15.00	16.31	38.682	2.93e+006	1.000	25.60	0.00	858.71	216.64	0.00	216.64	0.00
2683	2683:WVGD2	15.00	10.00	12.50	39.739	3.01e+006	1.000	25.60	0.00	832.34	423.92	0.00	423.92	0.00
2683	2683:WVGD1	10.00	5.00	7.50	41.125	3.12e+006	1.000	25.60	0.00	861.55	438.71	0.00	438.71	0.00
2683	2683:g	5.00	0.00	2.50	42.512	3.22e+006	1.000	25.60	0.00	890.86	453.50	0.00	453.50	0.00

*** Analysis Results:

Maximum element usage is 70.57% for Steel Pole "2683" in load case "NESC Heavy"
Maximum insulator usage is 14.73% for Clamp "Clamp21" in load case "NESC Extreme"



*** Analysis Results for Load Case No. 1 "NESC Heavy" - Number of iterations in SAPS 14

Equilibrium Joint Positions and Rotations for Load Case "NESC Heavy":

Joint Label	X-Displ (ft)	Y-Displ (ft)	Z-Displ (ft)	X-Rot (deg)	Y-Rot (deg)	Z-Rot (deg)	X-Pos (ft)	Y-Pos (ft)	Z-Pos (ft)
2683:g	0	0	0	0.0000	0.0000	0.0000	0	0	0
2683:t	0.004452	3.056	-0.07008	-3.3725	0.0044	0.0001	0.004452	3.056	89.93
2683:Arm1	0.00437	2.992	-0.06821	-3.3725	0.0044	0.0001	0.00437	2.992	88.85

2683:WVG9	0.004072	2.762	-0.06143	-3.3565	0.0044	0.0001	0.004072	2.762	84.94
2683:Arm2	0.003958	2.674	-0.05886	-3.3466	0.0044	0.0001	0.003958	2.674	83.44
2683:TopConn	0.003693	2.471	-0.05292	-3.3039	0.0043	0.0001	0.003693	2.471	79.95
2683:WVG8	0.003317	2.186	-0.04469	-3.2062	0.0043	0.0001	0.003317	2.186	74.96
2683:Arm3	0.002986	1.939	-0.03776	-3.0869	0.0042	0.0001	0.002986	1.939	70.46
2683:WVG7	0.002592	1.651	-0.03007	-2.8955	0.0040	0.0001	0.002592	1.651	64.97
2683:Arm4	0.002215	1.383	-0.02339	-2.6730	0.0038	0.0000	0.002215	1.383	59.48
2683:WVG6	0.001923	1.181	-0.01869	-2.4687	0.0036	0.0000	0.001923	1.181	54.98
2683:BotConn	0.001616	0.9743	-0.01429	-2.2504	0.0034	0.0000	0.001616	0.9743	49.99
2683:WVG5	0.001331	0.7874	-0.01064	-2.0335	0.0031	0.0000	0.001331	0.7874	44.99
2683:WVG4	0.0008304	0.4735	-0.005388	-1.5618	0.0026	0.0000	0.0008304	0.4735	34.99
2683:WVG3	0.0004367	0.2401	-0.002338	-1.1055	0.0019	0.0000	0.0004367	0.2401	25
2683:WVG2	0.0001616	0.08559	-0.0008085	-0.6548	0.0012	0.0000	0.0001616	0.08559	15
2683:WVG1	1.889e-005	0.009557	-0.0001743	-0.2118	0.0004	0.0000	1.889e-005	0.009557	5
Davit1:O	0.004375	2.993	-0.01938	-3.3725	0.0044	0.0001	0.004375	2.163	88.9
Davit1:End	0.004517	3.09	0.2427	-3.3728	0.0044	0.0001	0.004517	-2.241	90.66
Davit2:O	0.004365	2.991	-0.1171	-3.3725	0.0044	0.0001	0.004365	3.821	88.8
Davit2:End	0.004452	3.072	-0.3894	-3.4640	0.0044	0.0001	0.004452	8.403	90.03
Davit3:O	0.003963	2.676	-0.006742	-3.3466	0.0044	0.0001	0.003963	1.783	83.49
Davit3:End	0.004167	2.806	0.4936	-3.3198	0.0044	0.0001	0.004167	-6.756	85.99
Davit4:O	0.003952	2.672	-0.111	-3.3466	0.0044	0.0001	0.003952	3.565	83.39
Davit4:End	0.004053	2.776	-0.6266	-3.4147	0.0044	0.0001	0.004053	12.34	84.87
Davit5:O	0.002991	1.94	0.0184	-3.0869	0.0042	0.0001	0.002991	0.8971	70.52
Davit5:End	0.003273	2.117	0.6369	-3.0396	0.0042	0.0001	0.003273	-10.6	74.14
Davit6:O	0.002981	1.937	-0.09393	-3.0869	0.0042	0.0001	0.002981	2.98	70.41
Davit6:End	0.003141	2.085	-0.7421	-3.2187	0.0042	0.0001	0.003141	14.8	72.76
Davit7:O	0.00222	1.384	0.03118	-2.6730	0.0038	0.0000	0.00222	0.2143	59.53
Davit7:End	0.002388	1.486	0.4308	-2.6450	0.0038	0.0001	0.002388	-8.354	61.93
Davit8:O	0.002211	1.382	-0.07795	-2.6730	0.0038	0.0000	0.002211	2.552	59.42
Davit8:End	0.00231	1.467	-0.4906	-2.7420	0.0038	0.0000	0.00231	11.31	61.01

Joint Support Reactions for Load Case "NESC Heavy":

Joint Label	X (kips)	Y (kips)	Z Comp. Usage %	Uplift Result. Force Usage %	Result. Force Usage %	Trans. (Local Mx) (ft-k)	Long. Mom. (Local Mx) (ft-k)	Tors. Mom. (ft-k)	Axial Force (kips)	Y-M. Moment Usage % (ft-k)	Z-M. Moment Usage % (ft-k)	Long. Shear (kips)	P/A (ksi)	M/S. (ksi)	V/Q. (ksi)	T/R. (ksi)	Res. (ksi)	Max. Usage %
2683:g	-0.09	0.0	-24.81	0.0	50.88	0.0	0.0	56.61	0.0	1892.67	0.0	-3.7	0.0	-0.01	0.0	0.0	0.0	0.0

Detailed Steel Pole Usages for Load Case "NESC Heavy":

Element Label	Joint Label	Joint Position	Rel. Dist. (ft)	Trans. Defl. (in)	Long. Defl. (in)	Vert. Defl. (in)	Trans. (Local Mx) (ft-k)	Long. Mom. (Local Mx) (ft-k)	Tors. Mom. (ft-k)	Axial Force (kips)	Y-M. Moment Usage % (ft-k)	Z-M. Moment Usage % (ft-k)	Long. Shear (kips)	P/A (ksi)	M/S. (ksi)	V/Q. (ksi)	T/R. (ksi)	Res. (ksi)	Max. Usage %
2683	2683:t	Origin	0.00	36.67	0.05	-0.84	-0.00	-0.00	0.0	-0.06	0.01	-0.00	-0.00	0.00	0.00	0.00	0.00	0.00	0.0
2683	2683:Arm1	End	1.08	35.90	0.05	-0.82	0.02	-0.00	0.0	-0.06	0.01	-0.00	-0.00	0.00	0.00	0.00	0.00	0.00	0.0
2683	2683:Arm1	Origin	1.08	35.90	0.05	-0.82	6.93	-0.00	0.0	-1.96	4.69	-0.00	-0.10	0.80	0.18	0.00	0.95	1.5	2
2683	2683:WVG9	End	5.00	33.14	0.05	-0.74	25.33	-0.01	0.0	-1.96	4.69	-0.00	-0.09	2.63	0.17	0.00	2.74	4.2	2
2683	2683:WVG9	Origin	5.00	33.14	0.05	-0.74	25.33	-0.01	0.0	-2.68	4.84	-0.00	-0.13	2.63	0.18	0.00	2.77	4.3	2
2683	2683:Arm2	End	6.50	32.09	0.05	-0.71	32.59	-0.02	0.0	-2.68	4.84	-0.00	-0.12	3.25	0.18	0.00	3.39	5.2	2
2683	2683:Arm2	Origin	6.50	32.09	0.05	-0.71	42.19	-0.02	0.0	-6.17	9.73	-0.01	-0.28	4.21	0.35	0.00	4.53	7.0	2
2683	2683:TopConn	End	10.00	29.65	0.04	-0.64	76.24	-0.05	0.0	-6.17	9.73	-0.01	-0.27	6.95	0.34	0.00	7.24	11.1	2
2683	2683:TopConn	Origin	10.00	29.65	0.04	-0.64	76.24	-0.05	0.0	-16.90	14.08	-0.01	-0.74	6.95	0.49	0.00	7.74	11.9	2
2683	2683:WVG8	End	15.00	26.23	0.04	-0.54	146.63	-0.10	0.0	-16.90	14.08	-0.01	-0.70	11.83	0.46	0.00	12.55	19.3	2
2683	2683:WVG8	Origin	15.00	26.23	0.04	-0.54	146.63	-0.10	0.0	-17.94	14.27	-0.01	-0.74	11.83	0.47	0.00	12.59	19.4	2
2683	2683:Arm3	End	19.50	23.26	0.04	-0.45	210.86	-0.17	0.0	-17.94	14.27	-0.01	-0.70	15.32	0.44	0.00	16.04	24.7	2
2683	2683:Arm3	Origin	19.50	23.26	0.04	-0.45	225.28	-0.17	0.0	-22.01	19.18	-0.02	-0.86	16.37	0.59	0.00	17.26	26.6	2

2683	Tube 1	End	22.25	21.51	0.03	-0.41	278.01	-0.22	0.0	-22.01	19.18	-0.02	-0.83	19.01	0.58	0.00	19.87	30.6	2
2683	Tube 1	Origin	22.25	21.51	0.03	-0.41	278.01	-0.22	0.0	-22.41	19.23	-0.02	-0.85	19.01	0.58	0.00	19.88	30.6	2
2683	2683:WVGD7	End	25.00	19.81	0.03	-0.36	330.91	-0.28	0.0	-22.41	19.23	-0.02	-0.82	21.32	0.56	0.00	22.17	34.1	2
2683	2683:WVGD7	Origin	25.00	19.81	0.03	-0.36	330.91	-0.28	0.0	-23.25	19.36	-0.02	-0.86	21.32	0.56	0.00	22.20	34.1	2
2683	Tube 1	End	27.75	18.17	0.03	-0.32	384.15	-0.35	0.0	-23.25	19.36	-0.02	-0.83	23.37	0.55	0.00	24.22	37.3	2
2683	Tube 1	Origin	27.75	18.17	0.03	-0.32	384.15	-0.35	0.0	-23.68	19.42	-0.03	-0.85	23.37	0.55	0.00	24.23	37.3	2
2683	2683:Arm4	End	30.50	16.60	0.03	-0.28	437.55	-0.42	0.0	-23.68	19.42	-0.03	-0.82	25.16	0.54	0.00	26.00	40.5	2
2683	2683:Arm4	Origin	30.50	16.60	0.03	-0.28	437.55	-0.42	0.0	-27.53	24.25	-0.03	-0.96	25.71	0.67	0.00	26.70	41.6	2
2683	SpliceF	End	34.83	14.25	0.02	-0.23	552.18	-0.55	0.0	-27.53	24.25	-0.03	-0.92	29.17	0.64	0.00	30.10	47.9	2
2683	SpliceF	Origin	34.83	14.25	0.02	-0.23	552.18	-0.55	0.0	-27.93	24.29	-0.03	-0.93	29.17	0.64	0.00	30.12	47.9	2
2683	2683:WVGD6	End	35.00	14.17	0.02	-0.22	556.22	-0.56	0.0	-27.93	24.29	-0.03	-0.79	25.66	0.55	0.00	26.47	40.7	2
2683	2683:WVGD6	Origin	35.00	14.17	0.02	-0.22	556.22	-0.56	0.0	-29.10	24.41	-0.03	-0.83	25.66	0.55	0.00	26.51	40.8	2
2683	SplicesB	End	39.00	12.17	0.02	-0.18	653.84	-0.69	0.0	-29.10	24.41	-0.03	-0.80	27.93	0.53	0.00	28.74	44.2	2
2683	SplicesB	Origin	39.00	12.17	0.02	-0.18	653.84	-0.69	0.0	-29.92	24.46	-0.04	-0.82	27.93	0.53	0.00	28.76	44.2	2
2683	2683:BotConn	End	40.00	11.69	0.02	-0.17	678.30	-0.73	0.0	-29.92	24.46	-0.04	-0.81	28.43	0.53	0.00	29.25	45.0	2
2683	2683:BotConn	Origin	40.00	11.69	0.02	-0.17	678.30	-0.73	0.0	-32.91	23.72	-0.04	-0.89	28.43	0.51	0.00	29.33	45.1	2
2683	2683:WVGD5	End	45.00	9.45	0.02	-0.13	796.90	-0.92	0.0	-32.91	23.72	-0.04	-0.85	30.47	0.49	0.00	31.34	48.2	2
2683	2683:WVGD5	Origin	45.00	9.45	0.02	-0.13	796.90	-0.92	0.0	-34.94	23.91	-0.04	-0.90	30.47	0.49	0.00	31.39	48.3	2
2683	Tube 2	End	50.00	7.44	0.01	-0.09	916.44	-1.14	0.0	-34.94	23.91	-0.04	-0.87	32.10	0.47	0.00	32.98	50.7	2
2683	Tube 2	Origin	50.00	7.44	0.01	-0.09	916.44	-1.14	0.0	-36.06	23.98	-0.05	-0.89	32.10	0.47	0.00	33.01	50.8	2
2683	2683:WVGD4	End	55.00	5.68	0.01	-0.06	1036.32	-1.38	0.0	-36.06	23.98	-0.05	-0.86	33.38	0.45	0.00	34.24	53.7	2
2683	2683:WVGD4	Origin	55.00	5.68	0.01	-0.06	1036.32	-1.37	0.0	-38.18	24.15	-0.05	-0.91	33.38	0.45	0.00	34.29	53.8	2
2683	Tube 2	End	60.00	4.16	0.01	-0.04	1157.09	-1.64	0.0	-38.18	24.15	-0.05	-0.87	34.38	0.44	0.00	35.26	56.5	2
2683	Tube 2	Origin	60.00	4.16	0.01	-0.04	1157.09	-1.64	0.0	-39.39	24.22	-0.06	-0.90	34.38	0.44	0.00	35.29	56.5	2
2683	2683:WVGD3	End	65.00	2.88	0.01	-0.03	1278.18	-1.92	0.0	-39.39	24.22	-0.06	-0.87	35.15	0.42	0.00	36.02	58.9	2
2683	2683:WVGD3	Origin	65.00	2.88	0.01	-0.03	1278.18	-1.92	0.0	-41.56	24.39	-0.06	-0.91	35.15	0.42	0.00	36.07	59.0	2
2683	SpliceF	End	69.75	1.88	0.00	-0.02	1394.02	-2.22	0.0	-41.56	24.39	-0.06	-0.88	35.71	0.41	0.00	36.60	61.1	2
2683	SpliceF	Origin	69.75	1.88	0.00	-0.02	1394.02	-2.22	0.0	-42.83	24.43	-0.07	-0.91	35.71	0.41	0.00	36.62	61.2	2
2683	Tube 2	End	72.38	1.42	0.00	-0.01	1458.16	-2.39	0.0	-42.83	24.43	-0.07	-0.91	37.39	0.41	0.00	38.31	63.9	2
2683	Tube 2	Origin	72.38	1.42	0.00	-0.01	1458.16	-2.39	0.0	-44.15	24.47	-0.07	-0.94	37.39	0.41	0.00	38.34	64.0	2
2683	2683:WVGD2	End	75.00	1.03	0.00	-0.01	1522.39	-2.57	0.0	-44.15	24.47	-0.07	-0.92	37.58	0.40	0.00	38.51	65.0	2
2683	2683:WVGD2	Origin	75.00	1.03	0.00	-0.01	1522.39	-2.57	0.0	-46.44	24.61	-0.07	-0.97	37.58	0.41	0.00	38.55	65.1	2
2683	Tube 3	End	80.00	0.45	0.00	-0.01	1645.43	-2.93	0.0	-46.44	24.61	-0.07	-0.93	37.84	0.39	0.00	38.78	67.0	2
2683	Tube 3	Origin	80.00	0.45	0.00	-0.01	1645.43	-2.93	0.0	-47.81	24.66	-0.08	-0.96	37.84	0.39	0.00	38.81	67.0	2
2683	2683:WVGD1	End	85.00	0.11	0.00	-0.00	1768.70	-3.32	0.0	-47.81	24.66	-0.08	-0.93	37.99	0.38	0.00	38.93	68.7	2
2683	2683:WVGD1	Origin	85.00	0.11	0.00	-0.00	1768.70	-3.32	0.0	-50.16	24.79	-0.08	-0.97	37.99	0.38	0.00	38.97	68.8	2
2683	2683:g	End	90.00	0.00	0.00	0.00	1892.67	-3.74	0.0	-50.16	24.79	-0.08	-0.94	38.06	0.37	0.00	39.00	70.6	2

Detailed Tubular Davit Arm Usages for Load Case "NESC Heavy":

Element Label	Joint Label	Joint Position	Rel. Dist. (ft)	Trans. Defl. (in)	Long. Defl. (in)	Vert. Defl. (in)	Vert. Mom. (ft-k)	Horz. Mom. (ft-k)	Tors. Mom. (ft-k)	Axial Force (kips)	Vert. Shear (kips)	Horz. Shear (kips)	P/A (ksi)	M/S. (ksi)	V/Q. (ksi)	T/R. (ksi)	Res. (ksi)	Max. At Usage Pt. %
Davit1	Davit1:O	Origin	0.00	35.92	0.05	-0.23	0.02	0.00	0.0	-2.42	-0.00	-0.00	-0.39	0.02	0.00	0.00	0.41	0.6
Davit1	Davit1:End	End	4.74	37.07	0.05	2.91	-0.00	0.00	0.0	-2.42	-0.00	-0.00	-0.71	0.00	0.00	0.00	0.71	1.1
Davit2	Davit2:O	Origin	0.00	35.89	0.05	-1.40	-6.89	-0.00	0.0	1.94	1.45	0.00	0.31	5.11	0.47	0.00	5.48	8.4
Davit2	Davit2:End	End	4.74	36.87	0.05	-4.67	0.00	0.00	0.0	1.94	1.45	0.00	0.57	0.00	0.85	0.00	1.57	2.4
Davit3	Davit3:O	Origin	0.00	32.11	0.05	-0.08	-6.85	0.00	0.0	-2.66	0.88	-0.00	-0.19	1.27	0.12	0.00	1.48	2.3
Davit3	#Davit3:O	End	4.45	32.89	0.05	2.93	-2.93	0.00	0.0	-2.66	0.88	-0.00	-0.25	1.00	0.17	0.00	1.28	2.0
Davit3	#Davit3:O	Origin	4.45	32.89	0.05	2.93	-2.93	0.00	0.0	-2.59	0.66	-0.00	-0.24	1.00	0.12	0.00	1.26	1.9
Davit3	Davit3:End	End	8.90	33.68	0.05	5.92	-0.00	0.00	0.0	-2.59	0.66	-0.00	-0.38	0.00	0.19	0.00	0.50	0.8
Davit4	Davit4:O	Origin	0.00	32.07	0.05	-1.33	-16.44	-0.00	-0.0	2.00	1.97	0.00	0.14	3.06	0.27	0.00	3.23	5.0
Davit4	#Davit4:O	End	4.45	32.69	0.05	-4.41	-7.69	-0.00	-0.0	2.00	1.97	0.00	0.19	2.62	0.37	0.00	2.89	4.4
Davit4	#Davit4:O	Origin	4.45	32.69	0.05	-4.41	-7.69	-0.00	0.0	2.05	1.73	0.00	0.19	2.62	0.33	0.00	2.87	4.4

Davit4	Davit4:End	End	8.90	33.31	0.05	-7.52	0.00	0.00	0.0	2.05	1.73	0.00	0.30	0.00	0.50	0.00	0.92	1.4	1
Davit5	Davit5:0	Origin	0.00	23.28	0.04	0.22	-9.45	0.01	0.0	-2.72	0.97	-0.00	-0.19	1.76	0.13	0.00	1.96	3.0	1
Davit5	#Davit5:0	End	5.00	24.17	0.04	3.32	-4.62	0.00	0.0	-2.72	0.97	-0.00	-0.24	1.40	0.17	0.00	1.67	2.6	1
Davit5	#Davit5:0	Origin	5.00	24.17	0.04	3.32	-4.62	0.00	0.0	-2.65	0.73	-0.00	-0.24	1.40	0.13	0.00	1.65	2.5	1
Davit5	#Davit5:1	End	8.52	24.79	0.04	5.48	-2.04	0.00	0.0	-2.65	0.73	-0.00	-0.29	0.96	0.16	0.00	1.28	2.0	1
Davit5	#Davit5:1	Origin	8.52	24.79	0.04	5.48	-2.04	0.00	0.0	-2.60	0.58	-0.00	-0.29	0.96	0.13	0.00	1.27	1.9	1
Davit5	Davit5:End	End	12.05	25.40	0.04	7.64	-0.00	0.00	0.0	-2.60	0.58	-0.00	-0.38	0.00	0.17	0.00	0.48	0.7	1
Davit6	Davit6:0	Origin	0.00	23.25	0.04	-1.13	-23.84	-0.01	-0.0	1.92	2.17	0.00	0.13	4.43	0.30	0.00	4.60	7.1	1
Davit6	#Davit6:0	End	5.00	23.97	0.04	-4.31	-12.97	-0.00	-0.0	1.92	2.17	0.00	0.17	3.94	0.39	0.00	4.16	6.4	1
Davit6	#Davit6:0	Origin	5.00	23.97	0.04	-4.31	-12.97	-0.00	-0.0	1.97	1.92	0.00	0.18	3.94	0.34	0.00	4.15	6.4	1
Davit6	#Davit6:1	End	8.52	24.49	0.04	-6.59	-6.20	-0.00	-0.0	1.97	1.92	0.00	0.22	2.91	0.42	0.00	3.21	4.9	1
Davit6	#Davit6:1	Origin	8.52	24.49	0.04	-6.59	-6.20	-0.00	0.0	2.00	1.76	0.00	0.22	2.91	0.39	0.00	3.20	4.9	1
Davit6	Davit6:End	End	12.05	25.02	0.04	-8.91	0.00	0.00	0.0	2.00	1.76	0.00	0.29	0.00	0.51	0.00	0.93	1.4	1
Davit7	Davit7:0	Origin	0.00	16.61	0.03	0.37	-7.13	0.00	0.0	-2.65	0.91	-0.00	-0.19	1.33	0.13	0.00	1.53	2.4	1
Davit7	#Davit7:0	End	4.45	17.23	0.03	2.78	-3.06	0.00	0.0	-2.65	0.91	-0.00	-0.25	1.05	0.17	0.00	1.33	2.0	1
Davit7	#Davit7:0	Origin	4.45	17.23	0.03	2.78	-3.06	0.00	0.0	-2.59	0.69	-0.00	-0.24	1.05	0.13	0.00	1.31	2.0	1
Davit7	Davit7:End	End	8.90	17.84	0.03	5.17	-0.00	0.00	0.0	-2.59	0.69	-0.00	-0.38	0.00	0.20	0.00	0.51	0.8	1
Davit8	Davit8:0	Origin	0.00	16.58	0.03	-0.94	-16.66	-0.00	-0.0	1.98	1.99	0.00	0.14	3.10	0.28	0.00	3.27	5.0	1
Davit8	#Davit8:0	End	4.45	17.09	0.03	-3.39	-7.79	-0.00	-0.0	1.98	1.99	0.00	0.19	2.66	0.38	0.00	2.92	4.5	1
Davit8	#Davit8:0	Origin	4.45	17.09	0.03	-3.39	-7.79	-0.00	0.0	2.03	1.75	0.00	0.19	2.66	0.33	0.00	2.91	4.5	1
Davit8	Davit8:End	End	8.90	17.60	0.03	-5.89	0.00	0.00	0.0	2.03	1.75	0.00	0.29	0.00	0.51	0.00	0.93	1.4	1

Summary of Clamp Capacities and Usages for Load Case "NESC Heavy":

Clamp Label	Force (kips)	Input Capacity (kips)	Factored Holding Capacity (kips)	Usage %
Clamp1	2.402	80.00	80.00	3.00
Clamp2	2.402	80.00	80.00	3.00
Clamp3	2.628	80.00	80.00	3.29
Clamp4	2.628	80.00	80.00	3.29
Clamp5	2.628	80.00	80.00	3.29
Clamp6	2.628	80.00	80.00	3.29
Clamp7	2.628	80.00	80.00	3.29
Clamp8	2.628	80.00	80.00	3.29
Clamp9	0.951	80.00	80.00	1.19
Clamp10	0.951	80.00	80.00	1.19
Clamp11	0.951	80.00	80.00	1.19
Clamp12	0.951	80.00	80.00	1.19
Clamp13	0.951	80.00	80.00	1.19
Clamp14	0.433	80.00	80.00	0.54
Clamp15	0.433	80.00	80.00	0.54
Clamp16	0.433	80.00	80.00	0.54
Clamp17	0.433	80.00	80.00	0.54
Clamp21	11.058	80.00	80.00	13.82
Clamp23	2.489	80.00	80.00	3.11

Equilibrium Joint Positions and Rotations for Load Case "NESC Extreme":

Joint Label	X-Displ (ft)	Y-Displ (ft)	Z-Displ (ft)	X-Rot (deg)	Y-Rot (deg)	Z-Rot (deg)	X-Pos (ft)	Y-Pos (ft)	Z-Pos (ft)
2683:g	0	0	0	0.0000	0.0000	0.0000	0	0	0
2683:t	0.001396	3.053	-0.06851	-3.3235	0.0014	0.0000	0.001396	3.053	89.93
2683:Arm1	0.00137	2.991	-0.0667	-3.3235	0.0014	0.0000	0.00137	2.991	88.85
2683:WVGD9	0.001277	2.764	-0.06011	-3.3152	0.0014	0.0000	0.001277	2.764	84.94
2683:Arm2	0.001241	2.677	-0.0576	-3.3101	0.0014	0.0000	0.001241	2.677	83.44
2683:TopConn	0.001158	2.475	-0.05178	-3.2859	0.0014	0.0000	0.001158	2.475	79.95
2683:WVGD8	0.00104	2.191	-0.04365	-3.2099	0.0013	0.0000	0.00104	2.191	74.96
2683:Arm3	0.0009365	1.943	-0.03676	-3.0974	0.0013	0.0000	0.0009365	1.943	70.46
2683:WVGD7	0.000813	1.654	-0.0291	-2.9084	0.0013	0.0000	0.000813	1.654	64.97
2683:Arm4	0.0006949	1.385	-0.02245	-2.6826	0.0012	0.0000	0.0006949	1.385	59.48
2683:WVGD6	0.0006031	1.182	-0.0178	-2.4749	0.0011	0.0000	0.0006031	1.182	54.98
2683:BotConn	0.000507	0.9753	-0.01346	-2.2529	0.0011	0.0000	0.000507	0.9753	49.99
2683:WVGD5	0.0004173	0.7883	-0.009896	-2.0235	0.0010	0.0000	0.0004173	0.7883	44.99
2683:WVGD4	0.0002604	0.4746	-0.004816	-1.5614	0.0008	0.0000	0.0002604	0.4746	35
2683:WVGD3	0.000137	0.241	-0.001928	-1.1070	0.0006	0.0000	0.000137	0.241	25
2683:WVGD2	5.07e-005	0.08612	-0.0005587	-0.6575	0.0004	0.0000	5.07e-005	0.08612	15
2683:WVGD1	5.926e-006	0.009651	-8.978e-005	-0.2134	0.0001	0.0000	5.926e-006	0.009651	5
Davit1:O	0.001372	2.992	-0.01857	-3.3235	0.0014	0.0000	0.001372	2.992	88.9
Davit1:End	0.001416	3.087	-0.2404	-3.3359	0.0014	0.0000	0.001416	-2.243	90.66
Davit2:O	0.001369	2.989	-0.1148	-3.3235	0.0014	0.0000	0.001369	3.819	88.81
Davit2:End	0.001396	3.069	-0.3801	-3.3573	0.0014	0.0000	0.001396	8.399	90.04
Davit3:O	0.001243	2.678	-0.006052	-3.3101	0.0014	0.0000	0.001243	1.786	83.49
Davit3:End	0.001307	2.808	-0.4911	-3.3094	0.0014	0.0000	0.001307	-6.755	85.99
Davit4:O	0.001239	2.675	-0.1092	-3.3101	0.0014	0.0000	0.001239	3.568	83.39
Davit4:End	0.001271	2.777	-0.6155	-3.3372	0.0014	0.0000	0.001271	12.34	84.88
Davit5:O	0.0009382	1.945	-0.0196	-3.0974	0.0013	0.0000	0.0009382	0.9015	70.52
Davit5:End	0.001027	2.124	-0.6456	-3.0976	0.0013	0.0000	0.001027	-10.59	74.15
Davit6:O	0.0009348	1.941	-0.09312	-3.0974	0.0013	0.0000	0.0009348	2.984	70.41
Davit6:End	0.0009847	2.088	-0.7345	-3.1516	0.0013	0.0000	0.0009847	14.8	72.77
Davit7:O	0.0006964	1.386	0.03232	-2.6826	0.0012	0.0000	0.0006964	0.2163	59.53
Davit7:End	0.0007496	1.49	0.4357	-2.6812	0.0012	0.0000	0.0007496	-8.351	61.94
Davit8:O	0.0006934	1.384	-0.07721	-2.6826	0.0012	0.0000	0.0006934	2.554	59.42
Davit8:End	0.0007244	1.468	-0.4877	-2.7103	0.0012	0.0000	0.0007244	11.31	61.01

Joint Support Reactions for Load Case "NESC Extreme":

Joint Label	X Force Usage (kips)	X Force %	Y Force Usage (kips)	Y Force %	Z Comp. Force Usage (kips)	Uplift Force Usage %	Result. Force (kips)	Result. Usage %	X Moment Usage (ft-k)	Y Moment Usage (ft-k)	Z Moment Usage (ft-k)	Z-M. Usage %	Max.			
2683:g	-0.03	0.0	-26.69	0.0	24.93	0.0	0.0	36.52	0.0	1911.26	0.0	-1.2	0.0	-0.00	0.0	0.0

Detailed Steel Pole Usages for Load Case "NESC Extreme":

Element Label	Joint Label	Joint Position	Rel. Trans. Dist. (ft)	Trans. Defl. (in)	Long. Defl. (in)	Vert. Defl. (in)	Trans. Defl. (in)	Trans. Mom. (Local Mx) (ft-k)	Long. Mom. (Local My) (ft-k)	Tors. Mom. (ft-k)	Axial Force (kips)	Shear Force (kips)	Tran. Shear (kips)	Long. Shear (kips)	P/A M/S. (ksi) (ksi)	V/Q. (ksi) (ksi)	T/R. (ksi) (ksi)	Res. Usage Pt. %	Max. At Usage Pt. %
2683	2683:t	Origin	0.00	36.64	0.02	-0.82	-0.00	-0.00	0.00	0.0	-0.04	-0.00	-0.00	0.00	0.00	0.00	0.00	0.0	4

Davit2	Davit2:0	Origin	0.00	35.87	0.02	-1.38	-2.54	-0.00	0.0	1.03	0.54	0.00	0.16	1.89	0.17	0.00	2.07	3.2	1
Davit2	Davit2:End	End	4.74	36.83	0.02	-4.56	0.00	0.00	0.0	1.03	0.54	0.00	0.30	0.00	0.31	0.00	0.62	1.0	1
Davit3	Davit3:0	Origin	0.00	32.14	0.01	-0.07	-0.57	0.00	0.0	-1.59	0.13	-0.00	-0.11	0.11	0.02	0.00	0.22	0.3	1
Davit3	#Davit3:0	End	4.45	32.92	0.02	2.91	0.03	0.00	0.0	-1.59	0.13	-0.00	-0.15	0.01	0.03	0.00	0.17	0.3	1
Davit3	#Davit3:0	Origin	4.45	32.92	0.02	2.91	0.03	0.00	0.0	-1.55	-0.01	-0.00	-0.15	0.01	0.00	0.00	0.16	0.2	1
Davit3	Davit3:End	End	8.90	33.70	0.02	5.89	-0.00	0.00	0.0	-1.55	-0.01	-0.00	-0.22	0.00	0.00	0.00	0.22	0.3	1
Davit4	Davit4:0	Origin	0.00	32.10	0.01	-1.31	-6.76	-0.00	-0.0	1.36	0.84	0.00	0.10	1.26	0.12	0.00	1.37	2.1	1
Davit4	#Davit4:0	End	4.45	32.71	0.02	-4.34	-3.02	-0.00	-0.0	1.36	0.84	0.00	0.13	1.03	0.16	0.00	1.19	1.8	1
Davit4	#Davit4:0	Origin	4.45	32.71	0.02	-4.34	-3.02	-0.00	0.0	1.39	0.68	0.00	0.13	1.03	0.13	0.00	1.18	1.8	1
Davit4	Davit4:End	End	8.90	33.32	0.02	-7.39	0.00	0.00	0.0	1.39	0.68	0.00	0.20	0.00	0.20	0.00	0.40	0.6	1
Davit5	Davit5:0	Origin	0.00	23.33	0.01	0.24	-0.85	0.00	0.0	-1.62	0.18	-0.00	-0.11	0.16	0.03	0.00	0.28	0.4	1
Davit5	#Davit5:0	End	5.00	24.23	0.01	3.35	0.06	0.00	0.0	-1.62	0.18	-0.00	-0.14	0.02	0.03	0.00	0.17	0.3	1
Davit5	#Davit5:0	Origin	5.00	24.23	0.01	3.35	0.06	0.00	0.0	-1.57	0.04	-0.00	-0.14	0.02	0.01	0.00	0.16	0.2	1
Davit5	#Davit5:1	End	8.52	24.86	0.01	5.55	0.20	0.00	0.0	-1.57	0.04	-0.00	-0.17	0.09	0.01	0.00	0.27	0.4	1
Davit5	#Davit5:1	Origin	8.52	24.86	0.01	5.55	0.20	0.00	0.0	-1.54	-0.06	-0.00	-0.17	0.09	0.01	0.00	0.26	0.4	1
Davit5	Davit5:End	End	12.05	25.48	0.01	7.75	-0.00	0.00	0.0	-1.54	-0.06	-0.00	-0.22	0.00	0.02	0.00	0.23	0.3	1
Davit6	Davit6:0	Origin	0.00	23.30	0.01	-1.12	-10.24	-0.00	-0.0	1.32	0.98	0.00	0.09	1.90	0.14	0.00	2.01	3.1	1
Davit6	#Davit6:0	End	5.00	24.02	0.01	-4.29	-5.32	-0.00	-0.0	1.32	0.98	0.00	0.12	1.61	0.18	0.00	1.76	2.7	1
Davit6	#Davit6:0	Origin	5.00	24.02	0.01	-4.29	-5.32	-0.00	-0.0	1.35	0.81	0.00	0.12	1.61	0.14	0.00	1.75	2.7	1
Davit6	#Davit6:1	End	8.52	24.54	0.01	-6.55	-2.46	-0.00	-0.0	1.35	0.81	0.00	0.15	1.16	0.18	0.00	1.34	2.1	1
Davit6	#Davit6:1	Origin	8.52	24.54	0.01	-6.55	-2.46	-0.00	0.0	1.37	0.70	0.00	0.15	1.16	0.15	0.00	1.34	2.1	1
Davit6	Davit6:End	End	12.05	25.05	0.01	-8.81	0.00	0.00	0.0	1.37	0.70	0.00	0.20	0.00	0.20	0.00	0.41	0.6	1
Davit7	Davit7:0	Origin	0.00	16.64	0.01	0.39	-0.73	0.00	0.0	-1.59	0.15	-0.00	-0.11	0.14	0.02	0.00	0.25	0.4	1
Davit7	#Davit7:0	End	4.45	17.26	0.01	2.81	-0.05	0.00	0.0	-1.59	0.15	-0.00	-0.15	0.02	0.03	0.00	0.17	0.3	1
Davit7	#Davit7:0	Origin	4.45	17.26	0.01	2.81	-0.05	0.00	0.0	-1.55	0.01	-0.00	-0.15	0.02	0.00	0.00	0.16	0.3	1
Davit7	Davit7:End	End	8.90	17.87	0.01	5.23	-0.00	0.00	0.0	-1.55	0.01	-0.00	-0.22	0.00	0.00	0.00	0.23	0.3	1
Davit8	Davit8:0	Origin	0.00	16.61	0.01	-0.93	-6.90	-0.00	-0.0	1.35	0.86	0.00	0.09	1.28	0.12	0.00	1.39	2.1	1
Davit8	#Davit8:0	End	4.45	17.11	0.01	-3.38	-3.08	-0.00	-0.0	1.35	0.86	0.00	0.13	1.05	0.16	0.00	1.21	1.9	1
Davit8	#Davit8:0	Origin	4.45	17.11	0.01	-3.38	-3.08	-0.00	0.0	1.38	0.69	0.00	0.13	1.05	0.13	0.00	1.21	1.9	1
Davit8	Davit8:End	End	8.90	17.62	0.01	-5.85	0.00	0.00	0.0	1.38	0.69	0.00	0.20	0.00	0.20	0.00	0.40	0.6	1

Summary of Clamp Capacities and Usages for Load Case "NESC Extreme":

Clamp Label	Force (kips)	Input Holding Capacity (kips)	Factored Holding Capacity (kips)	Usage %
Clamp1	1.148	80.00	80.00	1.44
Clamp2	1.148	80.00	80.00	1.44
Clamp3	1.529	80.00	80.00	1.91
Clamp4	1.529	80.00	80.00	1.91
Clamp5	1.529	80.00	80.00	1.91
Clamp6	1.529	80.00	80.00	1.91
Clamp7	1.529	80.00	80.00	1.91
Clamp8	1.529	80.00	80.00	1.91
Clamp9	0.305	80.00	80.00	0.38
Clamp10	0.305	80.00	80.00	0.38
Clamp11	0.305	80.00	80.00	0.38
Clamp12	0.305	80.00	80.00	0.38
Clamp13	0.305	80.00	80.00	0.38

Clamp14	0.146	80.00	80.00	0.18
Clamp15	0.146	80.00	80.00	0.18
Clamp16	0.146	80.00	80.00	0.18
Clamp17	0.146	80.00	80.00	0.18
Clamp21	11.786	80.00	80.00	14.73
Clamp23	3.032	80.00	80.00	3.79

*** Overall summary for all load cases - Usage = Maximum Stress / Allowable Stress

Summary of Steel Pole Usages:

Steel Pole Maximum Label Usage %	Load Case Segment Number	Weight (lbs)
2683	70.57 NESC Heavy	25 13920.9

Base Plate Results by Bend Line:

Pole Label	Load Case	Bend Line #	Start X (ft)	Start Y (ft)	End X (ft)	End Y (ft)	End Length	Bending Stress (ksi)	Bolt # Acting	Bolts Mom. Sum (ft-k)	Max Load (kips)	Min Plate Thickness (in)	Actual Thickness (in)	Usage %
2683	NESC Heavy	1	1.271	1.275	0.390	2.156	14.956	31.288	2	58.491	85.226	2.263	3.000	56.89
2683	NESC Heavy	2	0.390	-2.156	1.271	-1.275	14.956	28.227	2	52.769	-77.419	2.149	3.000	51.32
2683	NESC Heavy	3	-1.273	-1.800	1.273	-1.800	30.544	28.812	6	110.004	-77.419	2.171	3.000	52.39
2683	NESC Heavy	4	-1.271	-1.275	-0.390	-2.156	14.956	27.907	2	52.171	-76.746	2.137	3.000	50.74
2683	NESC Heavy	5	-0.390	2.156	-1.271	1.275	14.956	31.607	2	59.088	85.898	2.274	3.000	57.47
2683	NESC Heavy	6	1.273	1.800	-1.273	1.800	30.544	31.983	6	122.110	85.898	2.288	3.000	58.15
2683	NESC Heavy	7	1.381	1.537	0.097	2.069	16.676	29.704	3	61.918	85.449	2.205	3.000	54.01
2683	NESC Heavy	8	0.097	-2.069	1.381	-1.537	16.676	26.855	3	55.979	-77.419	2.096	3.000	48.83
2683	NESC Heavy	9	-1.381	-1.537	-0.097	-2.069	16.676	26.620	3	55.489	-76.970	2.087	3.000	48.40
2683	NESC Heavy	10	-0.097	2.069	-1.381	1.537	16.676	29.939	3	62.408	85.898	2.213	3.000	54.44
2683	NESC Extreme	1	1.271	1.275	0.390	2.156	14.956	30.828	2	57.631	84.094	2.246	3.000	56.05
2683	NESC Extreme	2	0.390	-2.156	1.271	-1.275	14.956	29.271	2	54.721	-80.149	2.189	3.000	53.22
2683	NESC Extreme	3	-1.273	-1.800	1.273	-1.800	30.544	29.919	6	114.230	-80.149	2.213	3.000	54.40
2683	NESC Extreme	4	-1.271	-1.275	-0.390	-2.156	14.956	29.171	2	54.534	-79.938	2.185	3.000	53.04
2683	NESC Extreme	5	-0.390	2.156	-1.271	1.275	14.956	30.928	2	57.819	84.304	2.250	3.000	56.23
2683	NESC Extreme	6	1.273	1.800	-1.273	1.800	30.544	31.473	6	120.164	84.304	2.269	3.000	57.22
2683	NESC Extreme	7	1.381	1.537	0.097	2.069	16.676	29.276	3	61.026	84.164	2.189	3.000	53.23
2683	NESC Extreme	8	0.097	-2.069	1.381	-1.537	16.676	27.839	3	58.029	-80.149	2.134	3.000	50.62
2683	NESC Extreme	9	-1.381	-1.537	-0.097	-2.069	16.676	27.765	3	57.875	-80.008	2.132	3.000	50.48
2683	NESC Extreme	10	-0.097	2.069	-1.381	1.537	16.676	29.350	3	61.180	84.304	2.192	3.000	53.36

Summary of Tubular Davit Usages:

Tubular Davit Maximum Label Usage %	Load Case Segment Number	Weight (lbs)
Davit1	1.36 NESC Extreme	1 77.9
Davit2	8.43 NESC Heavy	1 77.9
Davit3	2.27 NESC Heavy	1 321.0
Davit4	4.97 NESC Heavy	1 321.0
Davit5	3.02 NESC Heavy	1 434.7
Davit6	7.07 NESC Heavy	1 434.7
Davit7	2.35 NESC Heavy	1 321.0
Davit8	5.03 NESC Heavy	1 321.0

*** Maximum Stress Summary for Each Load Case

Summary of Maximum Usages by Load Case:

Load Case Maximum Element	Element
---------------------------	---------

Usage %	Label	Type
NESC Heavy	70.57	2683 Steel Pole
NESC Extreme	70.33	2683 Steel Pole

Summary of Steel Pole Usages by Load Case:

Load Case Maximum Steel Pole Segment		
Usage %	Label	Number
NESC Heavy	70.57	2683 25
NESC Extreme	70.33	2683 25

Summary of Base Plate Usages by Load Case:

Load Case	Pole Label	Bend Line #	Length (in)	Vertical Load (kips)	X Moment (ft-k)	Y Bending Moment (ft-k)	Stress (ksi)	Bolt Sum Bend Line Moment (ft-k)	# Bolts Acting On Bend Line	Max Bolt Load For Bend Line (kips)	Minimum Plate Thickness (in)	Usage %
NESC Heavy	2683	6	30.544	50.878	1892.667	-3.740	31.983	122.110	6	85.898	2.288	58.15
NESC Extreme	2683	6	30.544	24.935	1911.258	-1.173	31.473	120.164	6	84.304	2.269	57.22

Summary of Tubular Davit Usages by Load Case:

Load Case Maximum Tubular Davit Segment		
Usage %	Label	Number
NESC Heavy	8.43	Davit2 1
NESC Extreme	3.19	Davit2 1

Summary of Insulator Usages:

Insulator Label	Insulator Type	Insulator Maximum Usage %	Load Case Weight (lbs)
Clamp1	Clamp	3.00	NESC Heavy 0.0
Clamp2	Clamp	3.00	NESC Heavy 0.0
Clamp3	Clamp	3.29	NESC Heavy 0.0
Clamp4	Clamp	3.29	NESC Heavy 0.0
Clamp5	Clamp	3.29	NESC Heavy 0.0
Clamp6	Clamp	3.29	NESC Heavy 0.0
Clamp7	Clamp	3.29	NESC Heavy 0.0
Clamp8	Clamp	3.29	NESC Heavy 0.0
Clamp9	Clamp	1.19	NESC Heavy 0.0
Clamp10	Clamp	1.19	NESC Heavy 0.0
Clamp11	Clamp	1.19	NESC Heavy 0.0
Clamp12	Clamp	1.19	NESC Heavy 0.0
Clamp13	Clamp	1.19	NESC Heavy 0.0
Clamp14	Clamp	0.54	NESC Heavy 0.0
Clamp15	Clamp	0.54	NESC Heavy 0.0
Clamp16	Clamp	0.54	NESC Heavy 0.0
Clamp17	Clamp	0.54	NESC Heavy 0.0
Clamp21	Clamp	14.73	NESC Extreme 0.0
Clamp23	Clamp	3.79	NESC Extreme 0.0

Loads At Insulator Attachments For All Load Cases:

Load Case	Insulator Label	Insulator Type	Structure Attach Label	Structure Attach Load X (kips)	Structure Attach Load Y (kips)	Structure Attach Load Z (kips)	Structure Attach Load Res. (kips)
NESC Heavy	Clamp1	Clamp	Davit1:End	0.000	2.250	0.840	2.402
NESC Heavy	Clamp2	Clamp	Davit2:End	0.000	2.250	0.840	2.402
NESC Heavy	Clamp3	Clamp	Davit3:End	0.000	2.305	1.263	2.628
NESC Heavy	Clamp4	Clamp	Davit4:End	0.000	2.305	1.263	2.628
NESC Heavy	Clamp5	Clamp	Davit5:End	0.000	2.305	1.263	2.628
NESC Heavy	Clamp6	Clamp	Davit6:End	0.000	2.305	1.263	2.628
NESC Heavy	Clamp7	Clamp	Davit7:End	0.000	2.305	1.263	2.628
NESC Heavy	Clamp8	Clamp	Davit8:End	0.000	2.305	1.263	2.628
NESC Heavy	Clamp9	Clamp	2683:WVGD1	0.000	0.087	0.947	0.951
NESC Heavy	Clamp10	Clamp	2683:WVGD2	0.000	0.087	0.947	0.951
NESC Heavy	Clamp11	Clamp	2683:WVGD3	0.000	0.087	0.947	0.951
NESC Heavy	Clamp12	Clamp	2683:WVGD4	0.000	0.087	0.947	0.951
NESC Heavy	Clamp13	Clamp	2683:WVGD5	0.000	0.087	0.947	0.951
NESC Heavy	Clamp14	Clamp	2683:WVGD6	0.000	0.050	0.430	0.433
NESC Heavy	Clamp15	Clamp	2683:WVGD7	0.000	0.050	0.430	0.433
NESC Heavy	Clamp16	Clamp	2683:WVGD8	0.000	0.050	0.430	0.433
NESC Heavy	Clamp17	Clamp	2683:WVGD9	0.000	0.050	0.430	0.433
NESC Heavy	Clamp21	Clamp	2683:TopConn	0.000	3.639	10.442	11.058
NESC Heavy	Clamp23	Clamp	2683:BotConn	0.000	-0.879	2.329	2.489
NESC Extreme	Clamp1	Clamp	Davit1:End	0.000	1.129	0.208	1.148
NESC Extreme	Clamp2	Clamp	Davit2:End	0.000	1.129	0.208	1.148
NESC Extreme	Clamp3	Clamp	Davit3:End	0.000	1.484	0.367	1.529
NESC Extreme	Clamp4	Clamp	Davit4:End	0.000	1.484	0.367	1.529
NESC Extreme	Clamp5	Clamp	Davit5:End	0.000	1.484	0.367	1.529
NESC Extreme	Clamp6	Clamp	Davit6:End	0.000	1.484	0.367	1.529
NESC Extreme	Clamp7	Clamp	Davit7:End	0.000	1.484	0.367	1.529
NESC Extreme	Clamp8	Clamp	Davit8:End	0.000	1.484	0.367	1.529
NESC Extreme	Clamp9	Clamp	2683:WVGD1	0.000	0.214	0.218	0.305
NESC Extreme	Clamp10	Clamp	2683:WVGD2	0.000	0.214	0.218	0.305
NESC Extreme	Clamp11	Clamp	2683:WVGD3	0.000	0.214	0.218	0.305
NESC Extreme	Clamp12	Clamp	2683:WVGD4	0.000	0.214	0.218	0.305
NESC Extreme	Clamp13	Clamp	2683:WVGD5	0.000	0.214	0.218	0.305
NESC Extreme	Clamp14	Clamp	2683:WVGD6	0.000	0.107	0.099	0.146
NESC Extreme	Clamp15	Clamp	2683:WVGD7	0.000	0.107	0.099	0.146
NESC Extreme	Clamp16	Clamp	2683:WVGD8	0.000	0.107	0.099	0.146
NESC Extreme	Clamp17	Clamp	2683:WVGD9	0.000	0.107	0.099	0.146
NESC Extreme	Clamp21	Clamp	2683:TopConn	0.000	10.779	4.767	11.786
NESC Extreme	Clamp23	Clamp	2683:BotConn	0.000	-2.841	1.058	3.032

Overturning Moments For User Input Concentrated Loads:

Moments are static equivalents based on central axis of 0,0 (i.e. a single pole).

Load Case	Total Tran. Load (kips)	Total Long. Load (kips)	Total Vert. Load (kips)	Total Transverse Overturning Moment (ft-k)	Total Longitudinal Overturning Moment (ft-k)
NESC Heavy	21.725	0.000	28.484	1695.440	-0.000
NESC Extreme	20.598	0.000	9.929	1635.592	-0.000

*** Weight of structure (lbs):
Weight of Tubular Davit Arms: 2309.3

Weight of Steel Poles: 13920.9
Total: 16230.2

*** End of Report

Subject:

Anchor Bolt Analysis
CL&P Pole #2683

Location:

Brookfield, CT

Rev. 0: 4/18/13

Prepared by: T.J.L. Checked by: C.F.C.
Job No. 12047.CO13**Anchor Bolt Analysis:****Input Data:**Bolt Force:Maximum Tensile Force = $T_{Max} := 85.9\text{-kips}$ (User Input from PLS-Pole)Anchor Bolt Data:

Use ASTM A615 Grade 75

Number of Anchor Bolts = $N := 12$ (User Input)Bolt "Column" Distance = $I := 3.0\text{-in}$ (User Input)Bolt Ultimate Strength = $F_u := 100\text{-ksi}$ (User Input)Bolt Yield Strength = $F_y := 75\text{-ksi}$ (User Input)Bolt Modulus = $E := 29000\text{-ksi}$ (User Input)Diameter of Anchor Bolts = $D := 2.25\text{-in}$ (User Input)Threads per Inch = $n := 4.5$ (User Input)**Anchor Bolt Analysis:**Calculated Anchor Bolt Properties:

Net Area of Bolt =

$$A_n := \frac{\pi}{4} \cdot \left(D - \frac{0.9743\text{-in}}{n} \right)^2 = 3.248\text{-in}^2$$

Bolt Tension Check:

Allowable Tensile Force (Net Area) =

$$T_{ALL.Net} := 1.0 \cdot (A_n \cdot F_y) = 243.576\text{-kips}$$

Bolt Tension % of Capacity =

$$\frac{T_{Max}}{T_{ALL.Net}} = 35.27\%$$

Condition1 =

$$\text{Condition1} := \text{if} \left(\frac{T_{Max}}{T_{ALL.Net}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

Condition1 = "OK"

Product Data Sheet APXVSP18-C



Triple Band Dual Polarized Antenna, 806-1995, 65deg, 16-18dBi, 1.8m, VET, 0-10deg, 0.5m AISG Cable

Product Description

This antenna is an ideal choice for dual band site upgrade for high traffic areas. It features 4 ports in 1900 MHz and 2 ports in 800 MHz.

Features/Benefits

- Variable electrical downtilt – provides enhanced precision in controlling intercell interference. The tilt is infield adjustable 0-10 deg.
- High suppression of all upper sidelobes (Typically < 18 dB)
- Independent control of electrical downtilt for 800 and PCS bands
- Remote tilt – AISG compatible
- Low profile for low visual impact
- Quick and easy to adjust
- High front-to-back ratio

Technical Specifications

Electrical Specifications

Frequency Range, MHz	806-869	1850-1995	1850-1995
Horizontal Beamwidth, deg	65	65	65
Vertical Beamwidth, deg	11.5	5.5	5.5
Electrical Downtilt, deg		0-10	
Gain, dBi (dBd)	15.5 (13.4)	18.0 (15.9)	18.0 (15.9)
1st Upper Sidelobe Suppression, dB, typ. @ T0° & T8°		>18	
Front-To-Back Ratio, dB, @ 180° ± 15°	>30	>27	>27
Polarization		Dual pol +/-45°	
Return Loss, dB		> 14	
Isolation between Ports, dB		>28	
3rd Order IMP @ 2 x 43 dBm, @ 2 min. duration		>110	
Cross Polar Discrimination (XPD) 0°, dB	>15	>20	>20
Cross Polar Discrimination (XPD) ± 60°, dB	>9.5	>11	>11
HBW Squint across same band ports, °		±5	
Impedance, Ohms		50	
Maximum Power Input, W		250	
Lightning Protection		Direct Ground	
Connector Type		(6) 7-16 DIN Female	

Mechanical Specifications

Dimensions - HxWxD, mm (in)	1829 x 302 x 178 (72.0 x 11.8 x 7)
Weight w/o Mtg Hardware, kg (lb)	25.8 (57)
Radome Material	ASA
Radome Color	Light Grey RAL7035
Mounting Hardware Material	Diecasted Aluminum and Galvanized Steel

Ordering Information

Mounting Hardware	APM40-2 Downtilt Kit
AISG System Cable	0.5 m, included
Mounting Pipe Diameter, mm (in)	60-120 (2.4-4.7)
Mounting Hardware Weight, kg (lb)	3.4 (7.5)

**RADIO FREQUENCY EMISSIONS ANALYSIS REPORT
EVALUATION OF HUMAN EXPOSURE POTENTIAL
TO NON-IONIZING EMISSIONS**

Sprint Existing Facility

Site ID: CT54XC713

**NU Pole # 2683 Brookfield
761 Federal Street
Brookfield, CT 06804**

May 29, 2013

EBI Project Number: 62136525

May 29, 2013

Sprint
Attn: RF Engineering Manager
1 International Boulevard, Suite 800
Mahwah, NJ 07495

Re: Emissions Values for Site: **CT54XC713 – NU Pole # 2683 Brookfield**

EBI Consulting was directed to analyze the proposed upgrades to the existing Sprint facility located at 761 Federal Street, Brookfield, CT, for the purpose of determining whether the emissions from the proposed Sprint equipment upgrades on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limit for the cellular band is approximately $567 \mu\text{W}/\text{cm}^2$, and the general population exposure limit for the PCS band is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed upgrades to the existing Sprint Wireless antenna facility located at 761 Federal Street, Brookfield, CT, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. All calculations were performed assuming the main lobe of the antenna was focused at the base of the tower to present a worst case scenario. Actual values seen from this site will be dramatically less than those shown in this report. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all emissions were calculated using the following assumptions:

- 1) 2 CDMA Carriers (1900 MHz) were considered for each sector of the proposed installation.
- 2) 1 CDMA Carrier (850 MHz) was considered for each sector of the proposed installation
- 3) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 4) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The actual gain in this direction was used per the manufactures supplied specifications.
- 5) The antenna used in this modeling is the APXVSP18-C-A20. This is based on feedback from the carrier with regards to anticipated antenna selection. This antenna has a 15.9 dBd gain value at its main lobe at 1900 MHz and 13.4 dBd at its main lobe for 850 MHz. All calculations were performed assuming the main lobe of the antenna was focused at the base of the tower to present a worst case scenario.

- 6) The antenna mounting height centerline of the proposed antennas is **110.67 feet** above ground level (AGL)
- 7) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculation were done with respect to uncontrolled / general public threshold limits

Site ID CT54XC713 - NU Pole # 2683 Brookfield Site Address 761 Federal Street, Brookfield, CT, 06804 Site Type Utility Transmission Pole																	
Sector 1																	
Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain in direction of sample point (dbd)	Antenna Height (ft)	analysis height	Cable Size	Cable Loss (dB)	Additional Loss	ERP	Power Density Value	Power Density Percentage
1a	RFS	APXVSP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	2	40	15.9	110.67	104.67	1/2 "	0.5	0	1386.9474	45.51154	4.55115%
1a	RFS	APXVSP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	13.4	110.67	104.67	1/2 "	0.5	0	389.96892	12.79651	2.25688%
Sector total Power Density Value: 6.808%																	
Sector 2																	
Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain in direction of sample point (dbd)	Antenna Height (ft)	analysis height	Cable Size	Cable Loss (dB)	Additional Loss	ERP	Power Density Value	Power Density Percentage
2a	RFS	APXVSP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	2	40	15.9	110.67	104.67	1/2 "	0.5	0	1386.9474	45.51154	4.55115%
2a	RFS	APXVSP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	13.4	110.67	104.67	1/2 "	0.5	0	389.96892	12.79651	2.25688%
Sector total Power Density Value: 6.808%																	
Sector 3																	
Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain in direction of sample point (dbd)	Antenna Height (ft)	analysis height	Cable Size	Cable Loss (dB)	Additional Loss	ERP	Power Density Value	Power Density Percentage
3a	RFS	APXVSP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	2	40	15.9	110.67	104.67	1/2 "	0.5	0	1386.9474	45.51154	4.55115%
3a	RFS	APXVSP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	13.4	110.67	104.67	1/2 "	0.5	0	389.96892	12.79651	2.25688%
Sector total Power Density Value: 6.808%																	

Site Composite MPE %	
Carrier	MPE %
Sprint	20.424%
AT&T	32.160%
Total Site MPE %	52.584%

Summary

All calculations performed for this analysis yielded results that were well within the allowable limits for general public exposure to RF Emissions.

The anticipated Maximum Composite contributions from the Sprint facility are **20.424% (6.808% from each sector)** of the allowable FCC established general public limit considering all three sectors simultaneously sampled at the ground level.

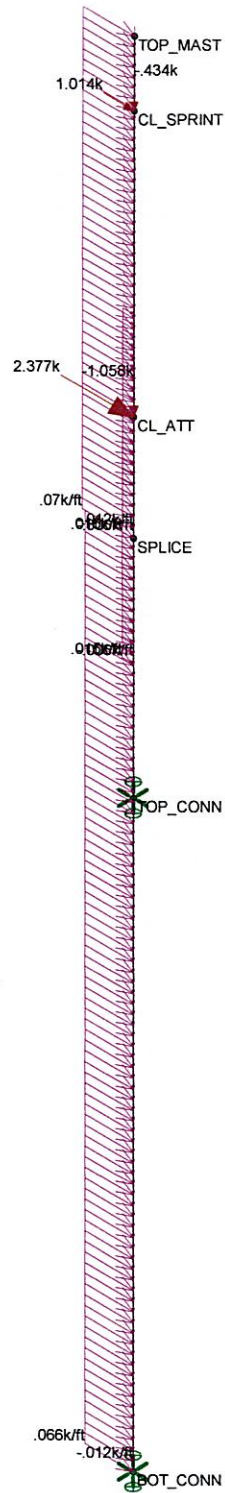
The anticipated composite MPE value for this site assuming all carriers present is **52.584%** of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.



Scott Heffernan
RF Engineering Director

EBI Consulting
21 B Street
Burlington, MA 01803



Loads: LC 2, NESC Extreme Wind on PCS Structure

CEN TEK Engineering, INC.

tjl, cfc

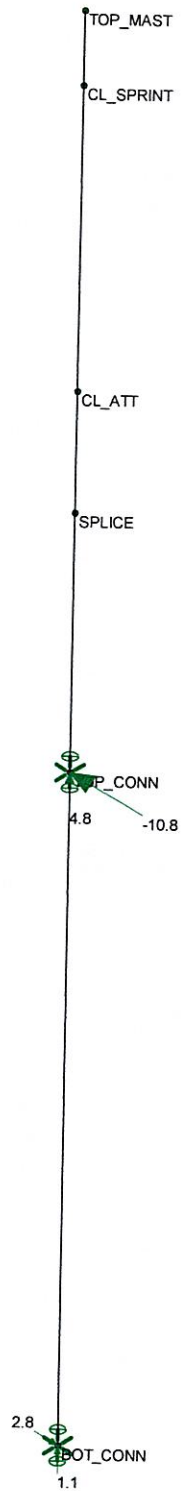
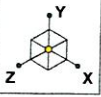
12047.CO13 /Sprint CT54...

CL&P # 2683 - Mast

LC #2 Loads

Apr 18, 2013 at 8:38 AM

NESC Loads.r3d



Results for LC 2, NESC Extreme Wind on PCS Structure
Z-moment Reaction units are k and k-ft

CEN TEK Engineering, INC.

tjl, cfc

12047.CO13 /Sprint CT54...

CL&P # 2683 - Mast
LC #2 Reactions

Apr 18, 2013 at 8:40 AM

NESC Loads.r3d