

56 Prospect Street, P.O. Box 270 Hartford, CT 06103

Kathleen M. Shanley Manager – Transmission Siting

Tel: (860) 728-4527

March 3, 2022

Melanie A. Bachman Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

RE: Notice of Exempt Modification

Eversource Site # 6581

790 Willis Street, Bristol, CT 06010

Latitude: 41-38-56 N / Longitude: 72-56-50 W

Dear Ms. Bachman:

The Connecticut Light and Power Company doing business as Eversource Energy ("Eversource") currently maintains multiple antennas and microwave dishes at various mounting heights on an existing 130-foot self-support tower located at 790 Willis Street in Bristol. See Attachment A, Parcel Map and Property Card. The tower and property are owned by Eversource. Eversource is seeking the Connecticut Siting Council's authorization for the installation of one 20-foot dipole antenna to be mounted at 130 feet above ground level ("AGL") on a heavy-duty mounting kit, and the removal of one 24-foot 3 ½-inch omni directional antenna and associated mount at 130 feet. There will be no changes to the area of the fenced compound, the tower or other existing antennas and equipment mounted on the tower. The tower and existing and proposed equipment on the tower are depicted on Attachment B, Construction Drawings, dated January 19, 2022 and Attachment C, Structural Analysis, dated January 13, 2022. The Connecticut Siting Council approved the self-support tower at this location in Petition No. 800 in January 2007.

The modification is required to eliminate transmitter induced noise issues from two antennas previously installed as part of Eversource's program to update its obsolete analog voice radio communications system to a modern digital voice communications system (refer to EM-EVER-017-200423, dated May 11, 2020). The transmitter issue manifests as passive intermodulation, or PIM, noise located on the receive frequencies, which limits the system level coverage capability of the site.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies ("R.C.S.A.") §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this notice is being delivered to Jeffrey Caggiano, Mayor for the City of Bristol and Robert M. Flanagan, AICP, City Planner for the City of Bristol via the United States Postal Service or private carrier. Proof of delivery is attached. See <u>Attachment D</u>, Proof of Delivery of Notice.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2):

- 1. There will be no change to the height of the existing tower.
- 2. The proposed modifications will not require the extension of the site boundary.
- 3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
- 4. The operation of the new antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard as shown in the attached Radio Frequency Emissions Report, dated February 7, 2022 (Attachment E Power Density Report)¹.
- 5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
- 6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, Eversource respectfully submits that the proposed modifications to the above referenced telecommunications facility constitute an exempt modification under R.C.S.A. § 16-50j-72(b)(2). One original and two copies of this notice are enclosed.

Communications regarding this Notice of Exempt Modification should be directed to Kathleen Shanley at (860) 728-4527.

By:

Kathleen M. Shanley

Manager – Transmission Siting

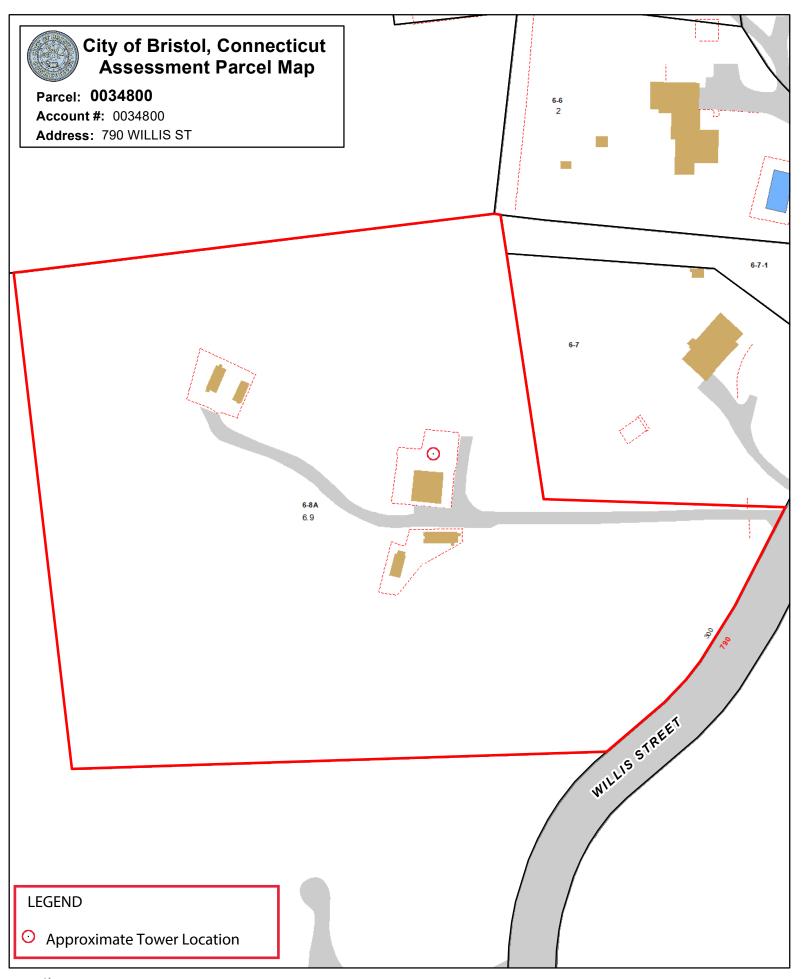
cc: Honorable Jeffrey Caggiano, Mayor, City of Bristol Robert M. Flanagan, AICP, City Planner, City of Bristol

Attachments

- A. Parcel Map and Property Card
- B. Construction Drawings
- C. Structural Analysis
- D. Proof of Delivery of Notice
- E. Power Density Report

¹ It should be noted that the Power Density Report denotes each channel as a transmitter. The depiction of antennas in the Structural Analysis and Construction Drawings accurately reflects the number of antennas. Also, the "Antenna Height" column on Table 1 in the Power Density Report reflects the Transmit or "TX" antenna centerline.







790 WILLIS ST

Location 790 WILLIS ST

Mblu 06/ / 8A/ /

Acct# 0034800

Owner CONN LIGHT + POWER CO

Assessment \$449,190

Appraisal \$641,700

PID 5681

Building Count 1

Current Value

Appraisal				
Valuation Year Improvements Land Tot				
2017	\$392,100	\$249,600	\$641,700	
	Assessment			
Valuation Year	Improvements	Land	Total	
2017	\$274,470	\$174,720	\$449,190	

Owner of Record

Owner

CONN LIGHT + POWER CO

Co-Owner

Address

107 SELDEN ST BERLIN, CT 06037 Sale Price \$0

Certificate 1

Book & Page 0277/0293

Sale Date 01/25/1952

Ownership History

Ownership History				
Owner	Sale Price	Certificate	Book & Page	Sale Date
CONN LIGHT + POWER CO	\$0	1	0277/0293	01/25/1952

Building Information

Building 1 : Section 1

Year Built:

1950

Living Area:

900

Replacement Cost:

\$40,248

Building Percent

65

Good:

Replacement Cost

Less Depreciation:

\$26,200

Building Attributes		
Field	Description	

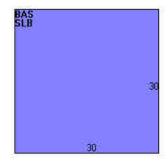
STYLE	Warehouse
MODEL	Ind/Comm
Stories:	1
Occupancy	1.00
Exterior Wall 1	Concr/Cinder
Exterior Wall 2	
Roof Structure	Gable
Roof Cover	Asphalt Shingl
Interior Wall 1	Minim/Masonry
Interior Wall 2	
Interior Floor 1	Concr-Finished
Interior Floor 2	
Heating Fuel	Electric
Heating Type	Hot Air-no Duc
AC Type	Unit/AC
Struct Class	
Bldg Use	Public Utility
Bedrooms	
Full Baths	
Half Baths	
Usrfld 218	
Usrfld 219	
1st Floor Use:	
Heat/AC	Heat/AC Pkgs
Frame Type	Masonry
Baths/Plumbing	Light
Ceiling/Wall	None
Rooms/Prtns	Light
Wall Height	8.00
% Comn Wall	

Building Photo



(http://images.vgsi.com/photos2/BristolCTPhotos//\00\05\61\14

Building Layout



(http://images.vgsi.com/photos2/BristolCTPhotos//Sketches/568

	Building Sub-Areas (sq ft)		
Code	Description	Gross Area	Living Area
BAS	First Floor	900	900
SLB	Slab	900	0
		1,800	900

Extra Features

Extra Features	<u>Legend</u>
No Data for Extra Features	

Land

Land Use		Land Line Valua	ation
Use Code	436	Size (Acres)	6.9

Description Public Utility

Zone R-25

Neighborhood 50 Alt Land Appr No

Category

Frontage 300

Depth

Assessed Value \$174,720 **Appraised Value** \$249,600

Outbuildings

	Outbuildings			<u>Legend</u>		
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #
CELL	Cell Tower/Site			2.00 UNITS	\$210,000	1
СВ3	PreCastConcCel			300.00 S.F.	\$54,000	1
СВ3	PreCastConcCel			300.00 S.F.	\$54,000	1
FCP	Carport			900.00 S.F.	\$5,600	1
GAR1	Garage	FR	Frame	420.00 S.F.	\$6,300	1
CB3	PreCastConcCel			200.00 S.F.	\$36,000	1

Valuation History

Appraisal				
Valuation Year Improvements Land			Total	
19	\$392,100	\$249,600	\$641,700	
2018	\$392,100	\$249,600	\$641,700	
2017	\$392,100	\$249,600	\$641,700	

Assessment				
Valuation Year Improvements Land To				
19	\$274,470	\$174,720	\$449,190	
2018	\$274,470	\$174,720	\$449,190	
2017	\$274,470	\$174,720	\$449,190	

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SOUTH MTN RADIO 790 WILLIS ST BRISTOL, CT 06010

LOCATION MAP

PROJECT SUMMARY

THE GENERAL SCOPE OF WORK CONSISTS OF THE FOLLOWING:

- INSTALL (1) NEW DIPOLE ANTENNA AT ELEVATION 150'-0"± AGL INSTEAD OF (1) NEW OMNI/WHIP ANTENNA AT ELEVATION 152'-0"± AGL
- 2. INSTALL (1) NEW MICROWAVE DISH AT ELEVATION 90'-0"± AGL
- 3. INSTALL (1) NEW RACK WITH DMR EQUIPMENT IN EXISTING TELECOM ROOM

GOVERNING CODES

2018 CONNECTICUT STATE BUILDING CODE (2015 IBC BASIS) 2017 NATIONAL ELECTRIC CODE

GENERAL NOTES

THE FACILITY IS LINMANNED AND NOT FOR HUMAN HABITATION A TECHNICIAN WILL VISIT THE SITE AS REQUIRED FOR ROUTINE MAINTENANCE. THE PROJECT WILL NOT RESULT IN ANY SIGNIFICANT DISTURBANCE OR EFFECT ON DRAINAGE; NO SANITARY SEWER SERVICE, POTABLE WATER, OR TRASH DISPOSAL IS REQUIRED AND NO COMMERCIAL SIGNAGE IS PROPOSED.

SITE INFORMATION

SITE NAME: SITE ID NUMBER:

SOUTH MTN RADIO

BRISTOL, CT 06010

BLOCK: LOT: ZONE:

R-25

LATITUDE: 41° 38' 56.0" N 72° 56′ 50.0″ W

ELEVATION: 1047'± AMSL

FEMA/FIRM DESIGNATION:

CONTACT INFORMATION

APPLICANTS: EVERSOURCE ENERGY 107 SELDEN STREET

BERLIN, CT 06037

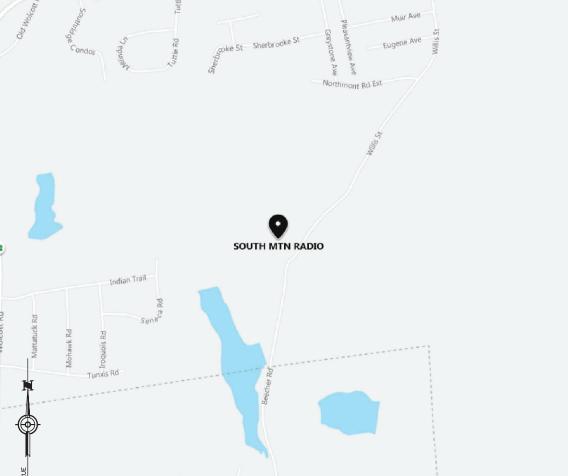
PROPERTY OWNER: EVERSOURCE ENERGY 107 SELDEN STREET BERLIN, CT 06037

EVERSOURCE ENERGY PROJECT MANAGER: NIKOLL PRECI (860) 655-3079

(800) 286-2000

TELCO PROVIDER: FRONTIER (800) 921-8102

CALL BEFORE YOU DIG:



NO SCALE

DESIGN TYPE

SITE UPGRADE SELF-SUPPORT TOWER

	DRAWING INDEX
SHEET NO:	SHEET TITLE
T-1	TITLE SHEET
C-1	SITE PLAN
C-2	TOWER ELEVATION
C-3	TOWER ELEVATION
C-4	ANTENNA EQUIPMENT
G-1	GROUNDING DETAILS
N-1	NOTES & SPECIFICATIONS
N-2	NOTES & SPECIFICATIONS
N-3	NOTES & SPECIFICATIONS
<u> </u>	

DO NOT SCALE DRAWINGS

SUBCONTRACTOR SHALL VERIFY ALL PLANS & EXISTING DIMENSIONS & CONDITIONS ON THE JOB SITE & SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME



SERVICE ALERT

UTILITIES PROTECTION CENTER, INC.

48 HOURS BEFORE YOU DIG

EVERSURCE

107 SELDEN STREET BERLIN, CT 06037 PHONE: (800) 286-2000



6800 W 115TH ST, SUITE 2292 OVERLAND PARK, KS 66211 PHONE: (913) 458-3595

PROJECT NO 403093 DRAWN BY TYW CHECKED BY:

1	01/19/22	ISSUED FOR FILING
0	03/09/20	ISSUED FOR FILING
REV	DATE	DESCRIPTION
-		



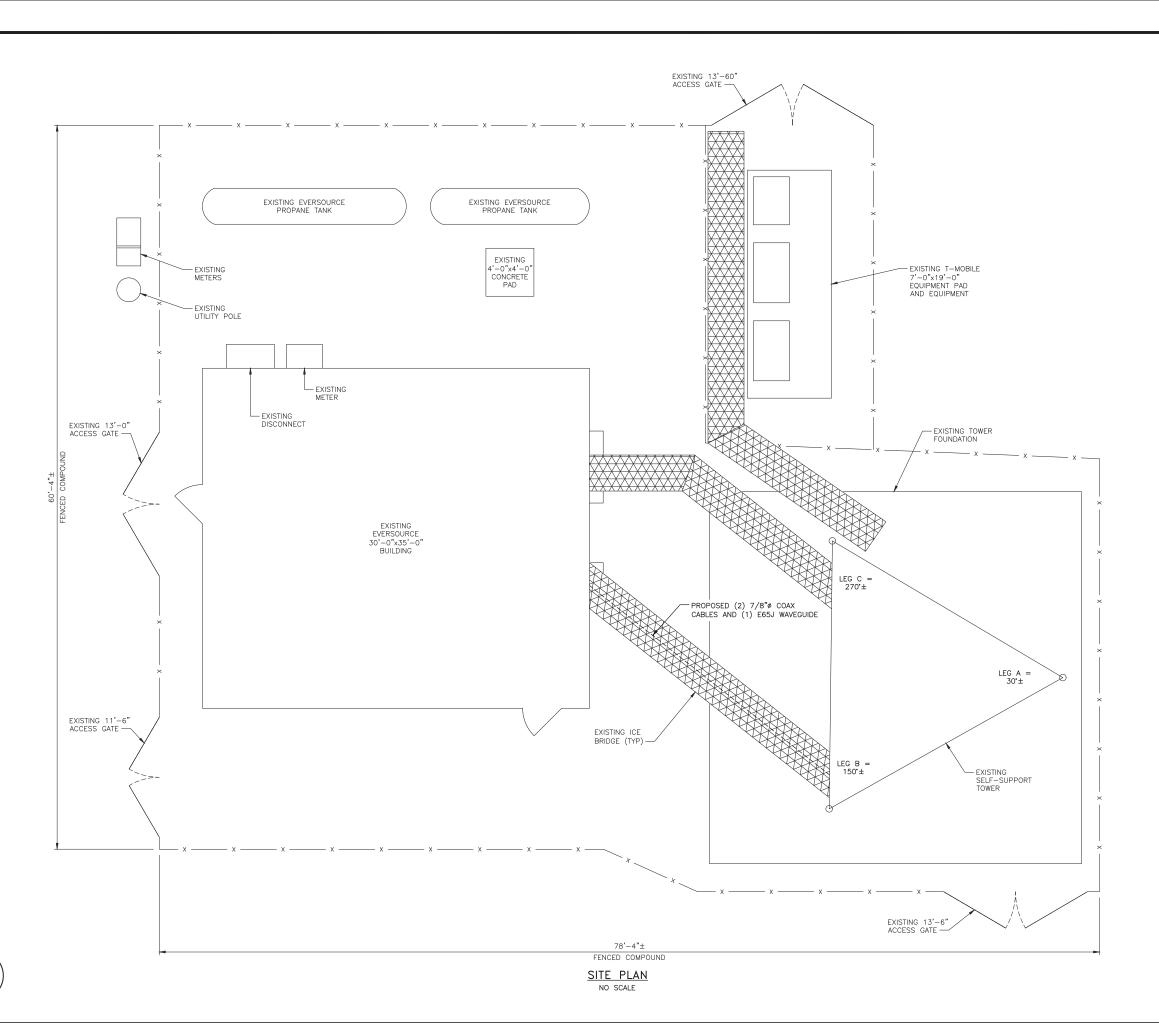
IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

SOUTH MTN RADIO 790 WILLIS ST BRISTOL, CT 06010

SHEET TITLE

TITLE SHEET

T-1



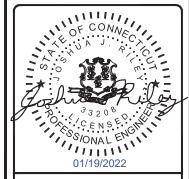




6800 W 115TH ST, SUITE 2292 OVERLAND PARK, KS 66211 PHONE: (913) 458-3595

PROJECT NO:	403093
DRAWN BY:	TYW
CHECKED BY:	JR

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	1	01/19/22	ISSUED FOR FILING
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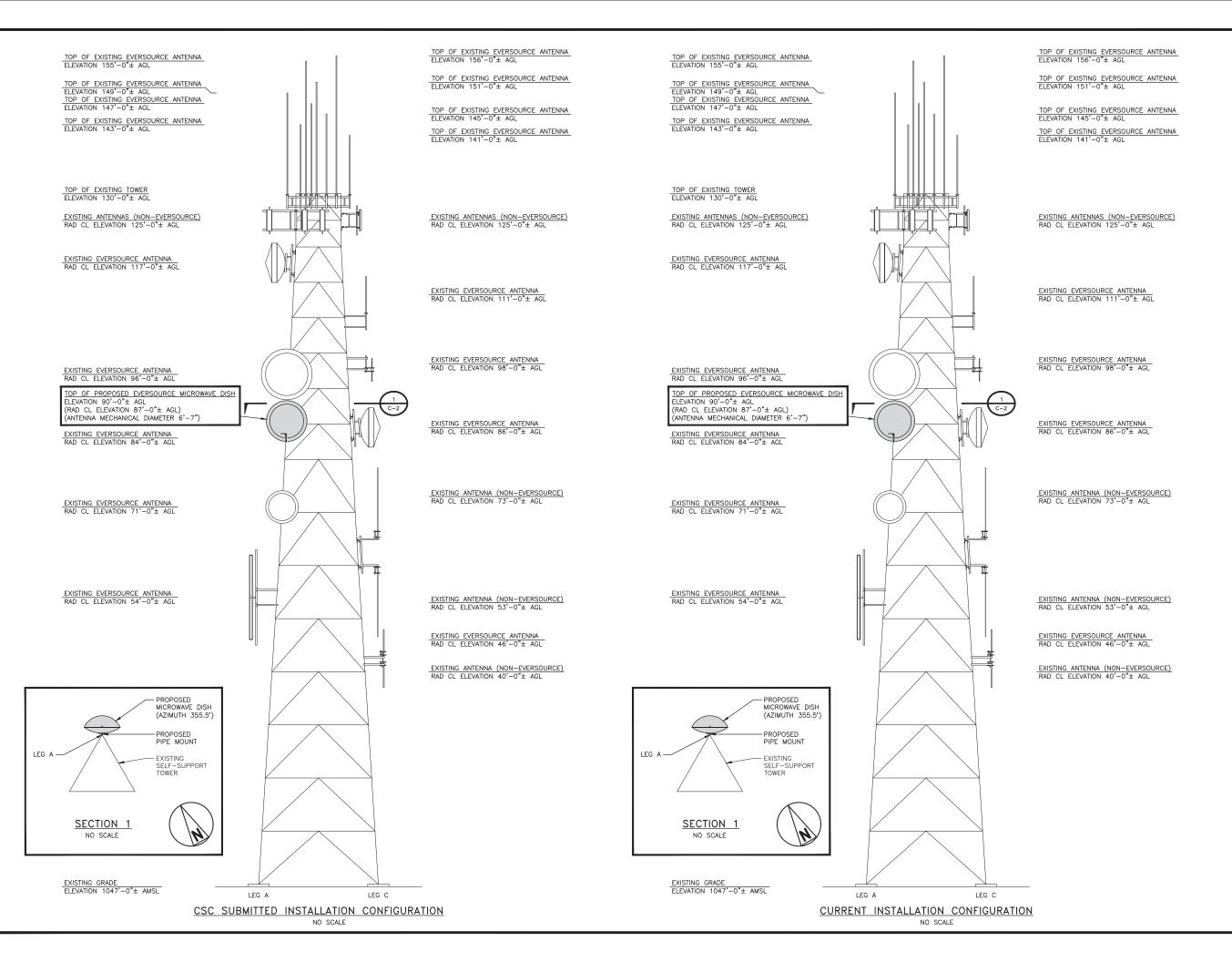
SOUTH MTN RADIO 790 WILLIS ST BRISTOL, CT 06010

SHEET TITLE

SITE PLAN

SHEET NUMBER





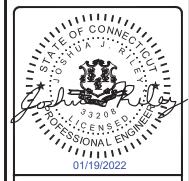




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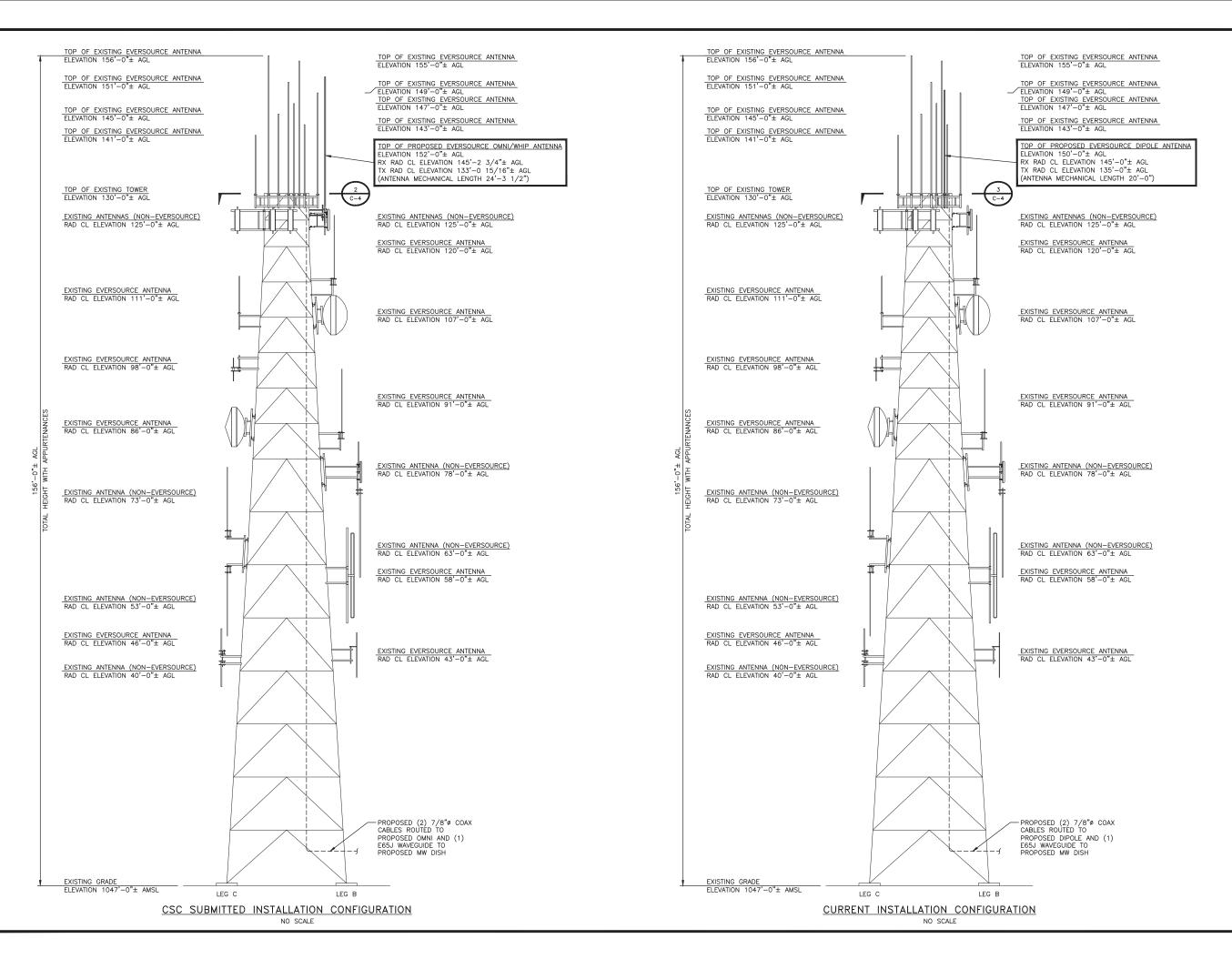
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> SOUTH MTN RADIO 790 WILLIS ST BRISTOL, CT 06010

> > SHEET TITLE

TOWER ELEVATION

SHEET NUMBER







6800 W 115TH ST, SUITE 2292 OVERLAND PARK, KS 66211 PHONE: (913) 458-3595

ı	PROJECT NO:	403093
	DRAWN BY:	TYW
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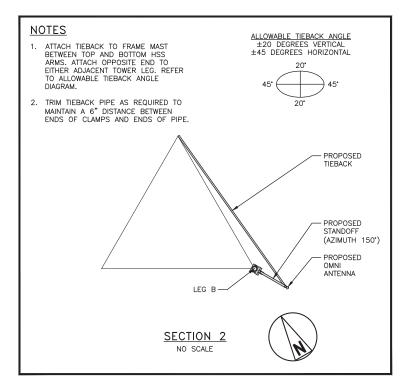
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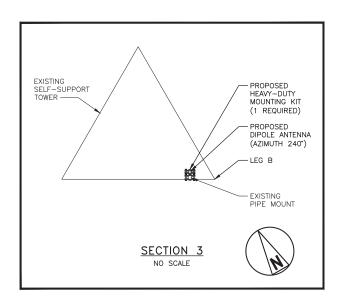
> SOUTH MTN RADIO 790 WILLIS ST BRISTOL, CT 06010

> > SHEET TITLE

TOWER ELEVATION

SHEET NUMBER









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	DRAWN BY:	TYW
П	CHECKED BY:	JR

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	REV	DATE	DESCRIPTION



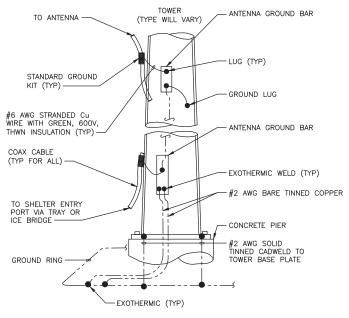
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> SOUTH MTN RADIO 790 WILLIS ST BRISTOL, CT 06010

> > SHEET TITLE

ANTENNA EQUIPMENT

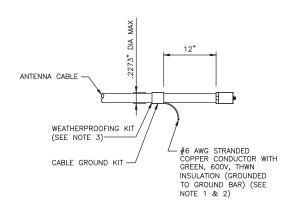
SHEET NUMBER



<u>NOTE</u>

1. NUMBER OF GROUND BARS MAY VARY DEPENDING ON THE TYPE OF TOWER, ANTENNA LOCATION AND CONNECTION ORIENTATION. PROVIDE AS REQUIRED.

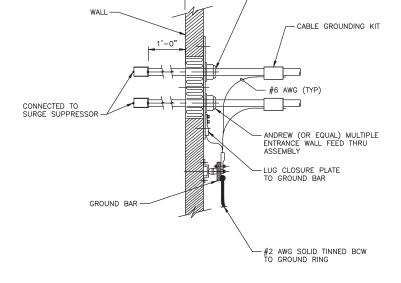
ANTENNA CABLE GROUNDING NO SCALE



NOTES

- DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.
- 2. GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.
- 3. WEATHER PROOFING SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.

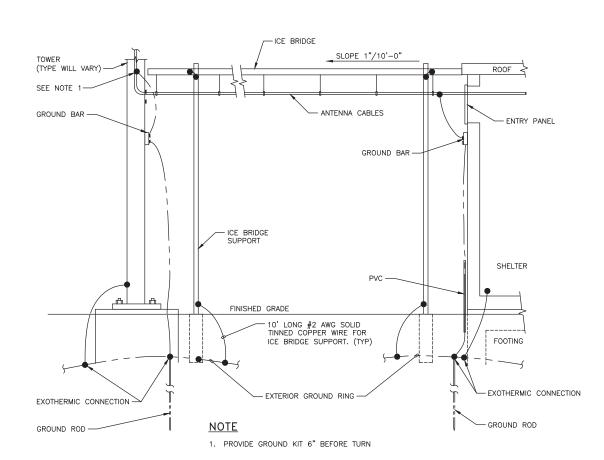
CONNECTION OF CABLE GROUND KIT TO ANTENNA CABLE NO SCALE



INDOOR | OUTDOOR

- ANTENNA CABLES (TYP)

CABLE INSTALLATION WITH WALL FEED THRU ASSEMBLY NO SCALE



ICE BRIDGE AND ANTENNA CABLE DETAIL NO SCALE



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01/19/22

SOUTH MTN RADIO 790 WILLIS ST BRISTOL, CT 06010

SHEET TITLE

GROUNDING DETAILS

SHEET NUMBER

G-1

EVERSURCE

107 SELDEN STREET BERLIN, CT 06037

PHONE: (800) 286-2000



6800 W 115TH ST, SUITE 2292 OVERLAND PARK, KS 66211 PHONE: (913) 458-3595

ı	PROJECT NO:	403093
ı	DRAWN BY:	TYW
ı	CHECKED BY:	JR
ı		

1	01/19/22	ISSUED FOR FILING
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DESIGN BASIS

1. GOVERNING CODE: 2018 CONNECTICUT STATE BUILDING CODE (2015 IBC BASIS).

GENERAL CONDITIONS

- IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO COMPLY WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL BUILDING CODES, PERMIT CONDITIONS AND SAFETY CODES DURING CONSTRUCTION.
- THE ENGINEER IS NOT: A GUARANTOR OF THE INSTALLING CONTRACTOR'S WORK; RESPONSIBLE FOR SAFETY IN, ON OR ABOUT THE WORK SITE; IN CONTROL OF THE SAFETY OR ADEQUACY OF ANY BUILDING COMPONENT, SCAFFOLDING OR SUPERINTENDING THE WORK.
- THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL PERMITS, INSPECTIONS, TESTING AND CERTIFICATES NEEDED FOR LEGAL OCCUPANCY OF THE FINISHED PROJECT.
- 4. THE CONTRACTOR IS RESPONSIBLE TO REVIEW THIS COMPLETE PLAN SET AND VERIFY THE EXISTING CONDITIONS SHOWN IN THESE PLANS AS THEY RELATE TO THE WORK PRIOR TO SUBMITTING PRICE. SIGNIFICANT DEVIATIONS FROM WHAT IS SHOWN AFFECTING THE WORK SHALL BE REPORTED IMMEDIATELY TO THE CONSTRUCTION MANAGER.
- 5. DETAILS INCLUDED IN THIS PLAN SET ARE TYPICAL AND APPLY TO SIMILAR CONDITIONS.
- 5. EXISTING ELECTRICAL AND MECHANICAL FIXTURES, PIPING, WIRING, AND EQUIPMENT OBSTRUCTING THE WORK SHALL BE REMOVED AND/OR RELOCATED AS DIRECTED BY THE CONSTRUCTION MANAGER. TEMPORARY SERVICE INTERRUPTIONS MUST BE COORDINATED WITH OWNER.
- THE CONTRACTOR SHALL DILIGENTLY PROTECT THE EXISTING BUILDING/SITE CONDITIONS AND THOSE
 OF ANY ADJOINING BUILDING/SITES AND RESTORE ANY DAMAGE CAUSED BY HIS ACTIVITIES TO THE
 PRE-CONSTRUCTION CONDITION.
- 8. THE CONTRACTOR SHALL SAFEGUARD AGAINST: CREATING A FIRE HAZARD, AFFECTING TENANT EGRESS OR COMPROMISING BUILDING SITE SECURITY MEASURES.
- THE CONTRACTOR SHALL REMOVE ALL DEBRIS AND CONSTRUCTION WASTE FROM THE SITE EACH DAY. WORK AREAS SHALL BE SWEPT AND MADE CLEAN AT THE END OF EACH WORK DAY.
- THE CONTRACTOR'S HOURS OF WORK SHALL BE IN ACCORDANCE WITH LOCAL CODES AND ORDINANCES AND BE APPROVED BY OWNER.
- 11. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE CONSTRUCTION MANAGER IF ASBESTOS IS ENCOUNTERED DURING THE EXECUTION OF HIS WORK. THE CONTRACTOR SHALL CEASE ALL ACTIVITIES WHERE THE ASBESTOS MATERIAL IS FOUND UNTIL NOTIFIED BY THE CONSTRUCTION MANAGER TO RESUME OPERATIONS.

THERMAL & MOISTURE PROTECTION

- FIRE-STOP ALL PENETRATIONS FOR ELECTRICAL CONDUITS OR WAVEGUIDE CABLING THROUGH BUILDING WALLS, FLOORS, AND CEILINGS SHALL BE FIRESTOPPED WITH ACCEPTED MATERIALS TO MAINTAIN THE FIRE RATING OF THE EXISTING ASSEMBLY. ALL FILL MATERIAL SHALL BE SHAPED, FITTED, AND PERMANENTLY SECURED IN PLACE. FIRESTOPPING SHALL BE INSTALLED IN ACCORDANCE WITH ASTM E814.
- HILTI CP620 FIRE FOAM OR 3M FIRE BARRIER FILL, VOID OR CAVITY MATERIAL OR ACCEPTED EQUAL SHALL BE APPLIED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND ASSOCIATED UNDERWRITERS LABORATORIES (UL) SYSTEM NUMBER.
- FIRESTOPPING SHALL BE APPLIED AS SOON AS PRACTICABLE AFTER PENETRATIONS ARE MADE AND EQUIPMENT INSTALLED.
- 4. FIRESTOPPED PENETRATIONS SHALL BE LEFT EXPOSED AND MADE AVAILABLE FOR INSPECTION BEFORE CONCEALING SUCH PENETRATIONS. FIRESTOPPING MATERIAL CERTIFICATES SHALL BE MADE AVAILABLE AT THE TIME OF INSPECTION.
- 5. ANY BUILDING ROOF PENETRATION AND/OR RESTORATION SHALL BE PERFORMED SO THAT THE ROOF WARRANTY IN PLACE IS NOT COMPROMISED. CONTRACTOR SHALL ARRANGE FOR OWNER'S ROOFING CONTRACTOR TO PERFORM ANY AND ALL ROOFING WORK IF SO REQUIRED BY EXISTING ROOF WARRANTY. OTHERWISE, ROOF SHALL BE MADE WATERTIGHT WITH LIKE CONSTRUCTION AS SOON AS PRACTICABLE AND AT COMPLETION OF CONSTRUCTION.
- ALL PENETRATIONS INTO AND/OR THROUGH BUILDING EXTERIOR WALLS SHALL BE SEALED WITH SILICONE SEALER.
- WHERE CONDUIT AND CABLES PENETRATES FIRE RATED WALLS AND FLOORS, FIRE GROUT ALL
 PENETRATIONS IN ORDER TO MAINTAIN THE FIRE RATING USING A LISTED FIRE SEALING DEVICE OR
 CROUT
- CONTRACTOR TO REMOVE AND RE-INSTALL ALL FIRE PROOFING AS REQUIRED DURING CONSTRUCTION.

SUBMITTALS

- 1. CONTRACTOR TO SUBMIT SHOP DRAWINGS TO ENGINEER FOR REVIEW PRIOR TO FABRICATION.
- 2. CONTRACTOR TO NOTIFY ENGINEER FOR INSPECTION PRIOR TO CLOSING PENETRATIONS.
- 3. CONTRACTORS SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO FABRICATION AND ERECTION OF ANY MATERIAL. THE ENGINEER SHALL BE NOTIFIED OF ANY CONDITIONS WHICH PRECLUDE COMPLETION OF THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- 4. ALL STEEL MATERIAL EXPOSED TO WEATHER SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT-DIPPED GALVANIZED) COATINGS" ON IRON AND STEEL PRODUCTS.
- 5. THE ENGINEER SHALL BE NOTIFIED OF ANY INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NONCONFORMING MATERIALS OR CONDITIONS FOR REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER REVIEW.

STEEL

MATERIAL:

WIDE FLANGE: ASTM A572, GR 50
TUBING: ASTM A500, GR C
PIPE: ASTM A53, GR B
BOLTS: ASTM A325
GRATING: TYPE GW-2 (1"x3/"

GRATING: TYPE GW-2 (1"x3/16" BARS)

ALL STEEL SHAPES SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 WITH A

- DAMAGED GALVANIZED SURFACES SHALL BE CLEANED WITH A WIRE BRUSH AND PAINTED WITH TWO
 COATS OF COLD ZINC, "CALVANOX", "DRY GALV", "ZINC IT", OR APPROVED EQUIVALENT, IN
 ACCORDANCE WITH MANUFACTURER'S GUIDELINES. TOUCH UP DAMAGED NON GALVANIZED STEEL WITH
 SAME PAINT IN SHOP OR FIELD.
- DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AISC "MANUAL OF STEEL CONSTRUCTION" 13TH EDITION.
- 4. THE STEEL STRUCTURE IS DESIGNED TO BE SELF—SUPPORTING AND STABLE AFTER COMPLETION. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION.
- 5. ALL STEEL ELEMENTS SHALL BE INSTALLED PLUMB AND LEVEL.
- 6. TOWER MANUFACTURER'S DESIGNS SHALL PREVAIL FOR TOWER.

SITE GENERAL

- CONTRACTOR SHALL FOLLOW CONDITIONS OF ALL APPLICABLE PERMITS AND WORK IN ACCORDANCE WITH OSHA REGULATIONS.
- 2. THESE PLANS DEPICT KNOWN UNDERGROUND STRUCTURES, CONDUITS, AND/OR PIPELINES. THE LOCATIONS FOR THESE ELEMENTS ARE BASED UPON THE VARIOUS RECORD DRAWINGS AVAILABLE. THE CONTRACTOR IS HEREBY ADVISED THAT THESE DRAWINGS MAY NOT ACCURATELY DEPICT AS—BUILT LOCATIONS AND OTHER UNKNOWN STRUCTURES. THE CONTRACTOR SHALL THEREFORE DETERMINE THE EXACT LOCATION OF EXISTING UNDERGROUND ELEMENTS AND EXCAVATE WITH CARE AFTER CALLING MARKOUT SERVICE AT 1-800-272-4480 48 HOURS BEFORE DIGGING, DRILLING OR BLASTING.
- 3. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, FIBER OPTIC, AND OTHER UTILITIES WHERE ENCOUNTERED, SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION, SHALL BE RELOCATED AS DIRECTED BY ENGINEER. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR PIER DRILLING AROUND OR NEAR UTILITIES. CONTRACTOR SHALL HAND DIG UTILITIES AS NEEDED. CONTRACTOR SHALL PROVIDE, BUT IS NOT LIMITED TO, APPROPRIATE A) FALL PROTECTION, B) CONFINED SPACE ENTRY, C) ELECTRICAL SAFETY, AND D) TRENCHING AND EXCAVATION.
- 4. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES, AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
- 5. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC, FIBER OPTIC, OR OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED, AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT THE POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF THE CONSTRUCTION MANAGER.
- 6. CONTRACTOR IS RESPONSIBLE FOR REPAIRING OR REPLACING STRUCTURES OR UTILITIES DAMAGED DURING CONSTRUCTION.
- CONTRACTOR SHALL PROTECT EXISTING PAVED AND GRAVEL SURFACES, CURBS, LANDSCAPE AND STRUCTURES AND RESTORE SITE OR PRE—CONSTRUCTION CONDITION WITH AS GOOD, OR BETTER, MATERIALS. NEW MATERIALS SHALL MATCH EXISTING THICKNESS AND TYPE.
- THE CONTRACTOR SHALL SHORE ALL TRENCH EXCAVATIONS GREATER THAN 5 FEET IN DEPTH OR LESS WHERE SOIL CONDITIONS ARE DEEMED UNSTABLE. ALL SHEETING AND/OR SHORING METHODS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER.
- THE CONTRACTOR IS RESPONSIBLE FOR MANAGING GROUNDWATER LEVELS IN THE VICINITY OF EXCAVATIONS TO PROTECT ADJACENT PROPERTIES AND NEW WORK. GROUNDWATER SHALL BE DRAINED IN ACCORDANCE WITH LOCAL SEDIMENTATION AND EROSION CONTROL GUIDELINES.



107 SELDEN STREET BERLIN, CT 06037 PHONE: (800) 286-2000



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PROJECT NO:	403093
DRAWN BY:	TYW
CHECKED BY:	JR

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SOUTH MTN RADIO 790 WILLIS ST BRISTOL, CT 06010

SHEET TITLE

NOTES & SPECIFICATIONS

SHEET NUMBER

N-1

ELECTRICAL

- CONTRACTOR SHALL VERIFY EXISTING ELECTRIC SERVICE TYPE AND CAPACITY AND ORDER NEW ELECTRIC SERVICE FROM LOCAL ELECTRIC UTILITY, WHERE APPLICABLE.
- ALL ELECTRICAL WORK SHALL BE IN ACCORDANCE WITH ALL APPLICABLE CODES, AND SHALL BE
 ACCEPTABLE TO ALL AUTHORITIES HAVING JURISDICTION. WHERE A CONFLICT EXISTS BETWEEN CODES,
 PLAN AND SPECIFICATIONS, OR AUTHORITIES HAVING JURISDICTION, THE MORE STRINGENT
 AUTHORITIES SHALL APPLY.
- CONTRACTOR SHALL PROVIDE ALL LABOR, MATERIALS, INSURANCE, EQUIPMENT, INSTALLATION, CONSTRUCTION TOOLS, TRANSPORTATION, ETC, FOR A COMPLETE AND PROPERLY OPERATIVE SYSTEM ENERGIZED THROUGHOUT AND AS INDICATED ON THE DRAWINGS AND AS SPECIFIED HEREIN AND/OR OTHERWISE REQUIRED.
- 4. ALL ELECTRICAL CONDUCTORS SHALL BE 100% COPPER AND SHALL HAVE TYPE THHN INSULATION UNLESS INDICATED OTHERWISE.
- CONDUIT SHALL BE THREADED RIGID GALVANIZED STEEL OR EMT WITH ONLY COMPRESSION TYPE COUPLINGS AND CONNECTORS. ALL MADE UP WRENCH TIGHT.
- ALL BURIED CONDUIT SHALL BE MINIMUM SCH 40 PVC UNLESS NOTED OTHERWISE, OR AS PER LOCAL CODE REQUIREMENTS.
- PROVIDE FLEXIBLE STEEL CONDUIT OR LIQUID TIGHT FLEXIBLE STEEL CONDUIT TO ALL VIBRATING EQUIPMENT, INCLUDING HVAC UNITS, TRANSFORMERS, MOTORS, ETC, OR WHERE EQUIPMENT IS PLACED UPON A SLAB ON GRADE.
- 8. ALL BRANCH CIRCUITS AND FEEDERS SHALL HAVE A SEPARATE GREEN INSULATED EQUIPMENT GROUNDING CONDUCTOR BONDED TO ALL ENCLOSURES, PULLBOXES, ETC.
- 9. CONDUIT AND CABLE WITHIN CORRIDORS SHALL BE CONCEALED AND EXPOSED ELSEWHERE, UNLESS NOTED OTHERWISE
- 10. ELECTRICAL MATERIALS INSTALLED ON ROOFTOP SHALL BE LISTED FOR NEMA 3R USE. —AND ALL WIRING WITHIN A VENTILATION DUCT SHALL BE LISTED FOR SUCH USE. IN GENERAL WIRING METHODS WITHIN A DUCT SHALL BE AN MC CABLE WITH SMOOTH OR CORRUGATED METAL JACKET AND HAVE NO OUTER COVERING OVER THE METAL JACKET. INTERLOCKED ARMOR TYPE OF MC CABLE IS NOT ACCEPTABLE FOR THIS APPLICATION. CONTRACTOR CAN ALSO USE TYPE MI CABLE IN THE VENTILATION DUCT PROVIDED IT DOES NOT HAVE ANY OUTER COVERINGS OVER THE METAL EXTERIOR.
- 11. WIRING DEVICES SHALL BE SPECIFICATION GRADE, AND WIRING DEVICE COVER PLATES SHALL BE PLASTIC WITH ENGRAVING AS SPECIFIED.
- 12. GROUNDING SYSTEM RESISTANCE SHALL BE MEASURED, RECORDED, AND DATED USING MEGGER DET14 OR SIMILAR INSTRUMENT. GROUND RESISTANCE SHALL NOT EXCEED 5 OHMS. IF THE RESISTANCE VALUE IS EXCEEDED, NOTIFY CONSTRUCTION MANAGER FOR FURTHER INSTRUCTION.
- 13. COORDINATE WITH BUILDING MANAGEMENT BEFORE PERFORMING ANY WORK INVOLVING EXISTING SYSTEMS OR EQUIPMENT IN ORDER TO DETERMINE THE EFFECT, IF ANY, ON OTHER TENANTS WITHIN THE BUILDING, AND TO DETERMINE THE APPROPRIATE TIME FOR PERFORMING THIS WORK.
- 14. THE CONTRACTOR SHALL BE REQUIRED TO VISIT THE SITE PRIOR TO SUBMITTING BID IN ORDER TO DETERMINE THE EXTENT OF THE EXISTING CONDITIONS.
- 15. ALL CONDUCTOR ENDS SHALL BE TAGGED AND ELECTRICAL EQUIPMENT LABELED WITH ENGRAVED IDENTIFICATION PLATES.
- 16. CONTRACTOR IS RESPONSIBLE FOR ALL CONTROL WIRING AND ALARM TIE-INS.

GROUNDING

- #6 THWN SHALL BE STRANDED #6 COPPER WITH GREEN THWN INSULATION SUITABLE FOR WET INSTALLATIONS.
- 2. #2 THWN SHALL BE STRANDED #2 COPPER WITH THWN INSULATION SUITABLE FOR WET
- ALL LUGS SHALL BE 2-HOLE, LONG BARREL, TINNED SOLID COPPER UNLESS OTHERWISE SPECIFIED, LUGS SHALL BE THOMAS AND BETTS SERIES 548##BE OR EQUIVALENT (IE #2 THWN - 54856BE, #2 SOLID - 54856BE, AND #6 THWN - 54852BE).
- 4. ALL HARDWARE, BOLTS, NUTS, AND WASHERS SHALL BE 18-8 STAINLESS STEEL. EVERY CONNECTION SHALL BE BOLT-FLAT WASHER-BUSS-LUG-FLAT WASHER-BELLEVILLE WASHER-NUT IN THAT EXACT ORDER. BACK-TO-BACK LUGGING, BOLT-FLAT WASHER-LUG-BUSS-LUG-FLAT WASHER-BELLEVILLE WASHER-NUT, IN THAT EXACT ORDER, IS ACCEPTED WHERE NECESSARY TO CONNECT MANY LUGS TO A BUSS BAR. STACKING OF LUGS, BUSS-LUG-LUG, IS NOT ACCEPTABLE.
- WHERE CONNECTIONS ARE MADE TO STEEL OR DISSIMILAR METALS, A THOMAS AND BETTS DRAGON TOOTH WASHER MODEL DTWXXX SHALL BE USED BETWEEN THE LUG AND THE STEEL, BOLT-FLAT WASHER-STEEL-DRAGON TOOTH WASHER-LUG-FLAT WASHER-BELEVILE WASHER-NUT
- 6. ALL CONNECTIONS, INTERIOR AND EXTERIOR, SHALL BE MADE WITH THOMAS AND BETTS KPOR-SHIELD. COAT ALL WIRES BEFORE LUGGING AND COAT ALL SURFACES BEFORE CONNECTING.
- 7. THE MINIMUM BEND RADIUS SHALL BE 8 INCHES FOR #6 WIRE AND SMALLER AND 12 INCHES FOR WIRE LARGER THAN #6.
- 8. BOND THE FENCE TO THE GROUND RING AT EACH CORNER, AND AT EACH GATE POST WITH #2 SOLID TINNED WIRE. EXOTHERMIC WELD BOTH ENDS.
- 9. GROUND KITS SHALL BE SOLID COPPER STRAP WITH #6 WIRE 2—HOLE COMPRESSION CRIMPED LUGS AND SHALL BE SEALED ACCORDING TO MANUFACTURER INSTRUCTIONS.
- 10. FERROUS METAL CLIPS WHICH COMPLETELY SURROUND THE GROUNDING CONDUCTOR SHALL BE USED.
- 11. GROUND BARS SHALL BE FURNISHED AND INSTALLED WITH PRE-DRILLED HOLE DIAMETERS AND SPACINGS. GROUND BARS SHALL NEITHER BE FIELD FABRICATED NOR NEW HOLES DRILLED. GROUND LUGS SHALL MATCH THE SPACING ON THE BAR. HARDWARE DIAMETER SHALL BE MINIMUM 3.8 INCH.

ANTENNA & CABLE NOTES

- 1. THE CONTRACTOR SHALL FURNISH AND INSTALL ALL TRANSMISSION CABLES, JUMPERS, CONNECTORS, GROUNDING STRAPS, ANTENNAS, MOUNTS AND HARDWARE. ALL MATERIALS SHALL BE INSPECTED BY THE CONTRACTOR FOR DAMAGE UPON DELIVERY. JUMPERS SHALL BE SUPPLIED AT ANTENNAS AND EQUIPMENT INSIDE SHELTER COORDINATE LENGTH OF JUMP CABLES WITH EVERSOURCE. COORDINATE AND VERIEY ALL OF THE MATERIALS TO BE PROVIDED WITH EVERSOURCE PRIOR TO SUBMITTING BID AND ORDERING MATERIALS.
- AFTER INSTALLATION, THE TRANSMISSION LINE SYSTEM SHALL BE PIM/SWEEP TESTED FOR PROPER INSTALLATION AND DAMAGE WITH ANTENNAS CONNECTED. CONTRACTOR TO OBTAIN LATEST TESTING PROCEDURES FROM EVERSOURCE PRIOR TO BIDDING.
- 3. ANTENNA CABLES SHALL BE COLOR CODED AT THE FOLLOWING LOCATIONS:
 - AT THE ANTENNAS.
 - AT THE WAVEGUIDE ENTRY PLATE ON BOTH SIDES OF THE EQUIPMENT SHELTER WALL.
- JUMPER CABLES AT THE EQUIPMENT ENTER.
- 4. SYSTEM INSTALLATION:
- THE CONTRACTOR SHALL INSTALL ALL CABLES AND ANTENNAS TO THE MANUFACTURER'S SPECIFICATIONS. THE CONTRACTOR IS RESPONSIBLE FOR THE PROCUREMENT AND INSTALLATION OF THE FOLLOWING:
- ALL CONNECTORS, ASSOCIATED CABLE MOUNTING, AND GROUNDING HARDWARE.
- WALL MOUNTS, STANDOFFS, AND ASSOCIATED HARDWARE.
- 1/2 INCH HELIAX ANTENNA JUMPERS OF APPROPRIATE LENGTHS.
- 5. MINIMUM BENDING RADIUS FOR COAXIAL CABLES:
- 7/8 INCH, RMIN = 15 INCHES
- 1 5/8 INCH, RMIN = 25 INCHES
- 6. CABLE SHALL BE INSTALLED WITH A MINIMUM NUMBER OF BENDS WHERE POSSIBLE. CABLE SHALL NOT BE LEFT UNTERMINATED AND SHALL BE SEALED IMMEDIATELY AFTER BEING INSTALLED.
- 7. ALL CABLE CONNECTIONS OUTSIDE SHALL BE COVERED WITH WATERPROOF SPLICING KIT.
- 8. CONTRACTOR SHALL VERIFY EXACT LENGTH AND DIRECTION OF TRAVEL IN FIELD PRIOR TO CONSTRUCTION.
- 9. CABLE SHALL BE FURNISHED WITHOUT SPLICES AND WITH CONNECTORS AT EACH END.



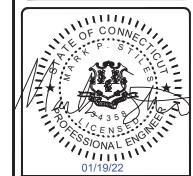
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1	CHECKED BY:	JR

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SOUTH MTN RADIO 790 WILLIS ST BRISTOL, CT 06010

SHEET TITLE

NOTES & SPECIFICATIONS

SHEET NUMBER

N-2

SYMBOLS

EXOTHERMIC CONNECTION COMPRESSION CONNECTION **□** 5/8"øx10-'0" COPPER CLAD STEEL GROUND ROD. TEST GROUND ROD WITH INSPECTION SLEEVE GROUNDING CONDUCTOR \bigcirc A KEY NOTES CHAINLINK FENCE WOOD FENCE LEASE AREA ICE BRIDGE CABLE TRAY GAS LINE UNDERGROUND - E/T ------ E/T ------ E/T ------ELECTRICAL/TELCO UNDERGROUND — E/C — E/C — ELECTRICAL/CONTROL UNDERGROUND ELECTRICAL UNDERGROUND PROPERTY LINE (PL)

ABBREVIATIONS

AC ALTERNATING CURRENT MGB MASTER GROUNDING BAR AIC AMPERAGE INTERRUPTION CAPACITY AUXILIARY NETWORK INTERFACE MW MICROWAVE ASYNCHRONOUS TRANSFER MODE MANUAL TRANSFER SWITCH ATS AUTOMATIC TRANSFER SWITCH NEC NATIONAL ELECTRICAL CODE AWG AMERICAN WIRE GAUGE oc ON CENTER AWS ADVANCED WIRELESS SERVICES PP POLARIZING PRESERVING BATT BATTERY PRIMARY CONTROL UNIT PCU BASEBAND UNIT PDU PROTOCOL DATA UNIT BTC BARE TINNED COPPER CONDUCTOR PWR POWER BASE TRANSCEIVER STATION CLIMATE CONTROL UNIT RET REMOTE ELECTRICAL TILT CCU CDMA CODE DIVISION MULTIPLE ACCESS RMC RIGID METALLIC CONDUIT CHG CHARGING RADIO FREQUENCY CLU CLIMATE UNIT RUC RACK USER COMMISSIONING COMM COMMON RRH REMOTE RADIO HEAD DC DIRECT CURRENT RRU REMOTE RADIO UNIT DIA DIAMETER RWY RACEWAY SMALL FORM-FACTOR PLUGGABLE DWG DRAWING ELECTRICAL CONDUCTOR SMART INTEGRATED ACCESS DEVICE EMT ELECTRICAL METALLIC TUBING SSC SITE SOLUTIONS CABINET FIF FACILITY INTERFACE FRAME 1544KBPS DIGITAL LINE GEN GENERATOR TDMA TIME-DIVISION MULTIPLE ACCESS GLOBAL POSITIONING SYSTEM TOWER MOUNT AMPLIFIER GSM GLOBAL SYSTEM FOR MOBILE TVSS TRANSIENT VOLTAGE SUPPRESSION SYSTEM HVAC HEAT/VENTILATION/AIR CONDITIONING INTERCONNECTION FRAME UMTS UNIVERSAL MOBILE TELECOMMUNICATION SYSTEM UPS UNINTERRUPTIBLE POWER SUPPLY IGR INTERIOR GROUNDING RING (HALO) (DC POWER PLANT) LTE LONG TERM EVOLUTION

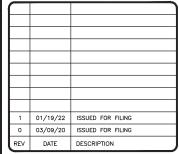


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SOUTH MTN RADIO 790 WILLIS ST BRISTOL, CT 06010

SHEET TITLE

NOTES & SPECIFICATIONS

SHEET NUMBER

N-3

REFERENCE CUTSHEETS

BASE STATION ANTENNAS





876F-70-2HSMP40DF1/2

The 876F-70-2HSMP40DF1/2 Dual Exposed Dipole is well suited for multicoupled RF system. It has an extremely rugged design for use in severe environmental conditions. It has internal cabling and a fix dipole-to-mast spacing. This antenna is a special version of the 876F-70 with increased spacing between the two antennas, giving an isolation of 40 dB. It's heavy duty and Low PIM deign. This antenna can be black anodized, please contact technical support for more information.

The ½ wave pattern spacing version offer bidirectional pattern with more than 5 dBd Gain at 220 MHz.

Electrical Specifications	876F-70-2HSMP40DF1/2
Frequency Range, MHz	215-225
Nominal Gain, dBd	5.0 (5.2 @ 220MHz)
Isolation , dB	40
Bandwidth 1.5:1 VSWR, MHz	1.5:1 (10)
Polarization	Vertical
Pattern	Bidirectional
Power Rating, Watts	300
PIM. (2x20W, 3rd ord.), dBc	150
Nominal Impedance, Ohms	50
Lightning Protection	DC Ground
Termination	Dual Foods Torminating in 7/16 DIN F
Terminadori	Dual Feeds Terminating in 7/16 DIN F
Mechanical Specifications	876F-70-HDWSM-40
	-
Mechanical Specifications	876F-70-HDWSM-40
Mechanical Specifications Length, in (mm)	876F-70-HDWSM-40 240 (6096)
Mechanical Specifications Length, in (mm) Width (1/2 Wave Spacing), in (mm)	876F-70-HDWSM-40 240 (6096) 43 (1092)
Mechanical Specifications Length, in (mm) Width (1/2 Wave Spacing), in (mm) Weight, lbs. (kg)	876F-70-HDWSM-40 240 (6096) 43 (1092) 130 (59)
Mechanical Specifications Length, in (mm) Width (1/2 Wave Spacing), in (mm) Weight, lbs. (kg) Rated Wind Velocity, No Ice, mph (km/h)	876F-70-HDWSM-40 240 (6096) 43 (1092) 130 (59) 140 (225)
Mechanical Specifications Length, in (mm) Width (1/2 Wave Spacing), in (mm) Weight, lbs. (kg) Rated Wind Velocity, No Ice, mph (km/h) Rated Wind Velocity, 1/2" ice, mph (km/h)	876F-70-HDWSM-40 240 (6096) 43 (1092) 130 (59) 140 (225) 105 (169)
Mechanical Specifications Length, in (mm) Width (1/2 Wave Spacing), in (mm) Weight, lbs. (kg) Rated Wind Velocity, No Ice, mph (km/h) Rated Wind Velocity, 1/2" ice, mph (km/h) Lateral Thrust @ 100 mph, wind, lbs. (N)	876F-70-HDWSM-40 240 (6096) 43 (1092) 130 (59) 140 (225) 105 (169) 222 (988)



876F-70-2HSMP40DF1/2



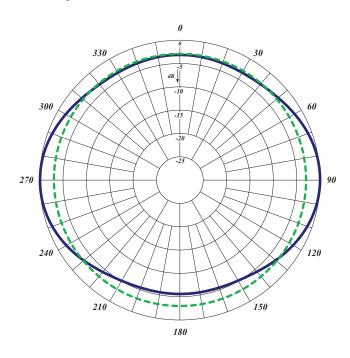
Tel: US 1.877.825.2007 / CAN 1.800.603.1454

215-225 MHz

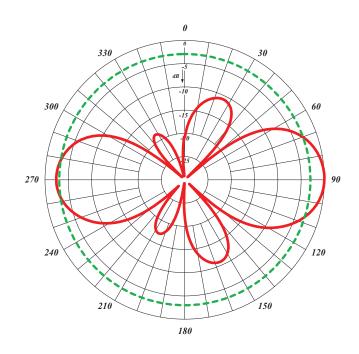
870 SERIES DUAL EXPOSED DIPOLE

876F-70-2HSMP40DF1/2





876F-70-2HSMP40DF1/2: Horizontal Radiation pattern



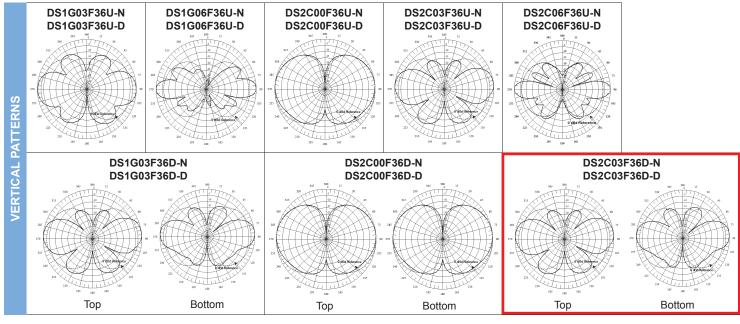
876F-70-2HSMP40DF1/2: Vertical Radiation pattern





VHF Omni Antennas (160-222 MHz)

		160-174 MHz							2	17-22	2 MH	z					
	Model Number	DS1G03F36U-N	DS1G03F36U-D	DS1G06F36U-N	DS1G06F36U-D	DS1G03F36D-N	DS1G03F36D-D	DS2C00F36U-N	DS2C00F36U-D	DS2C03F36U-N	DS2C03F36U-D	DS2C06F36U-N	DS2C06F36U-D	DS2C00F36D-N	DS2C00F36D-D	DS2C03F36D-N	DS2C03F36D-D
	Input Connector	N(F)	7/16 DIN	N(F)	7/16 DIN	N(F)	7/16 DIN	N(F)	7/16 DIN	N(F)	7/16 DIN	N(F)	7/16 DIN	N(F)	7/16 DIN	N(F)	7/16 DIN
	Туре	Sin	gle	Sin	gle	Dι	ıal	Sin	gle	Sin	gle	Sin	gle	Du	ıal	Dυ	
	Bandwidth, MHz	1	4	1	4	1	4		5	į	5	į	5	5	5	5	5
ب	Power, Watts	50	00	50	00	35	50	50	00	50	00	50	00	35	50	35	50
CTRICAL	Gain, dBd	3		(6	3	3	()	3		(6	0		3	
	Horizontal Beamwidth, degrees	360		360		36	60	36	60	36	60	36	60	36	60	36	0
	Vertical Beamwidth, degrees	30		16		30		6	0	30		16		60		3	0
	Beam Tilt, degrees	0		0		()	()	()	()	()	C)
	Isolation (minimum), dB	N.	/A	N.	/A	3	0	N	/A	N	/A	N	/A	3	0	3	0
	Number of Connectors	1	1		1	2	2		1		1		1	2	2	2	2
CAL	Flat Plate Area, ft2(m2)	2.53 ((0.24)	4.38 ((0.41)	4.5 (0.42)	1.9 (0.18)	1.9 (0.18)	2.58	(0.24)	2.4 (0.22)	4.1 (0	0.38)
AND	Lateral Windload Thrust, lbf(N)	95 (4	423)	164 ((730)	169 ((752)	53 (236)	69 (307)	108	(480)	90 (4	400)	169 (752)
MECHANI	Survival Wind Speed without ice, mph(kph) with 0.5" radial ice, mph(kph)	110 (93 (75 (60 (121) (97)	75 (65 ((357) (311)		(277) (241)		(177) 154)		(209) (185)	75 (<i>*</i> 65 (<i>*</i>	
	Mounting Hardware included	DSH	3V3R	DSH:	3V3N	DSH:	3V3N	DSH:	2V3R	DSH	2V3R	DSH:	3V3N	DSH	3V3R	DSH3	3V3N
8	Length, ft(m)	12.7	(3.9)	21.9	(6.7)	22.3	(6.8)	7.7	(2.3)	9.9	(3)	18.1	(5.5)	13.6	(4.1)	24.3	(7.4)
SNOIS	Radome O.D., in(cm)	3 (7	7.6)	3 (7	7.6)	3 (7	7.6)	3 (7.6)	3 (7	7.6)	3 (7	7.6)	3 (7	7.6)	3 (7	'.6)
ENSI	Mast O.D., in(cm)	2.5 ((6.4)	2.5	(6.4)	2.5	(6.4)	2.5	(6.4)	2.5	(6.4)	2.5	(6.4)	2.5 ((6.4)	2.5 (6.4)
	Net Weight w/o bracket, lb(kg)	37 (1	16.8)	60 (2	27.2)	63 (2	28.6)	19 (8.6)	26 (11.8)	47 (2	21.3)	40 (1	18.1)	70 (3	31.8)
	Shipping Weight, lb(kg)	67 (3	30.4)	90 (4	10.8)	93 (4	12.2)	39 (17.7)	56 (2	25.4)	77 (3	34.9)	70 (3	31.8)	100 (45.4)
	DS1G03F36U-N DS1G	06F36	II N		Desc	00F36	II NI		S2C03	E2611	N	D	22006	F3611-	NI		



MOUNTING KIT TO CONNECT NEW DIPOLE TO EXISTING PIPE MOUNT

Antenna Mounting Hardware





DSH1V3R



DSH2H3R



REGULAR MOUNTING

Mount aluminum base station antennas to round or angled tower legs. Center section of each clamp is welded to provide mechanical stability and all parts are hot-dipped galvanized steel.

MODELS	DSH1V3R	DSH2V3R	DSH2H3R	DSH3V3R
	DSH1V4R	DSH2V4R	DSH2H4R	DSH3V4R
Antenna Length, ft(m)	0 (0) to 3.5 (1)	3.5 (1) to 10 (3.1)	3.5 (1) to 10 (3.1)	10 (3.1) to 14 (4.3)
# of Clamps	1	2	2	3
Mounting	Vertical	Vertical	Horizontal	Vertical
Pipe Mount, in (mm): 3R	1.3 (32) to 3.5 (89)			
Pipe Mount, in (mm): 4R	1.3 (32) to 4 (102)			
Weight, lb (kg)	4 (1.8)	7 (3.2)	10 (4.5)	9(4.1)
Shipping Weight, lb (kg)	6 (2.7)	8 (3.6)	12(5.5)	10(4.5)

HEAVY-DUTY "NO-TORSION" MOUNTING

Utilizes three clamps on a galvanized steel tube to mount antennas to round tower members.

MODEL	DSH3V3N	DSH3V4N	
Antenna Length, ft(m)	14 (4.3) and greater	14 (4.3) and greater	
# of Clamps	3	3	
Mounting	Vertical	Vertical	
Pipe Mount, in (mm)	3 (76.2) MAX.	4 (101.6) MAX.	
Weight, lb (kg)	28 (12.7)	28 (12.7)	
Shipping Weight, lb (kg)	30 (13.6)	30 (13.6)	
Shipping Dimensions (W x H x D), in(mm)	11 x 33 x 4 (279 x 838 x 102)	11 x 33 x 4 (279 x 838 x 102)	

TOP SWAY BRACE - OUTRIGGER MOUNTING

Limit tip deflection on 3-inch diameter fiberglass antennas in high wind conditions. Attaches to the tower legs using supplied DSH2H3R hardware kit (above). Recommended on top-mounted antennas >16 feet long.

MODEL	DSH2H3S		
# of Clamps	2		
Mounting	Horizontal		
Flange Inner Diameter, in (mm)	3.38 (85.7)		
Tube Diameter, in (mm)	2 (50.8)		
Length to Center of Flange ft (m)	12 feet (3.6)		
Weight, lb (kg)	10 (4.5)		
Shipping Weight, lb (kg)	20 (9.1)		
Shipping Dimensions	11 x 33 x 4		
(W x H x D), in (mm)	(279 x 838 x 102)		

DIRECTIONAL ANTENNA MOUNTING HARDWARE Model **DB380**



Antenna Length	N/A
# of Clamps	2
Mounting	Antenna-to-Pipe
Pipe Mount, in(mm)	3.5 (76.2)
Weight, lb (kg)	10 (4.5)
Shipping Weight, lb (kg)	20 (9.1)



TrunkLine Antenna, Standard (FCC 101, Cat A), Single Polarized, 6 ft

RFS Microwave Antennas are designed for microwave systems in all common frequency ranges from 4 GHz to 24 GHz. This allows the use of antennas in areas where extreme wind conditions are normal. The antennas utilise a conventional feed system and are available in three performance classes offering complete flexibility when designing a network. Standard Performance antennas are economical solutions for systems where side lobe suppression is of less importance. These antennas are required for use in networks where there is a low interference potential. Antennas are available in 2 ft (0.6m) to 12 ft (3.7m) diameters. Antennas from 4ft up to 12 ft (3.7m) can be equipped with a moulded radome to reduce wind load and to protect the feed against the accumulation of ice and snow.



Antenna

FEATURES / BENEFITS

Field-proven reliability and long life

Withstanding winds up to 200 km/h (125 mph), an optional sway bar is available for added assurance in case mistakes are made during installation

A single-piece configuration and compact packaging to reduce transportation costs

Frequencies ranging from 4 GHz to 15 GHz with support for two wideband frequency ranges (5.725-6.875 and 7.125-8.5 GHz) to reduce antenna requirements and simplify logistics

Technical Features

GENERAL SPECIFICATIONS		
Product Type		Point to point antennas
Profile		TrunkLine
Performance		Improved Performance
Polarization		Single
Antenna Input		CPR137G
Reflector		1-part
Radome		Optional
Antenna color		White RAL 9010
Swaybar		1: (2.0 m x Ø60 mm)
ELECTRICAL SPECIFICATIONS		
Frequency	GHz	5.925 - 6.875
3dB beamwidth	degrees	1.7
Low Band Gain	dBi	38.4
Mid Band Gain	dBi	39.1
High Band Gain	dBi	39.7
F/B Ratio	dB	55.0
XPD	dB	30.0
Max VSWR / R L	VSWR / dB	1.08 (28.3)
Regulatory Compliance		FCC Category A
MECHANICAL SPECIFICATIONS		
Diameter	ft (m)	6 (1.8)
Elevation Adjustment	degrees	± 5
Azimuth Adjustment	degrees	± 5
Polarization Adjustment	degrees	± 5
Mounting Pipe Diameter minimum	mm (in)	114 (4.5)
Mounting Pipe Diameter maximum	mm (in)	114 (4.5)
Approximate Weight	kg (lb)	65 (141)
Survival Windspeed	km/h (mph)	200 (125)
Operational Windspeed	km/h (mph)	190 (118)
STRUCTURE		
Radome Material		Fiberglass
FURTHER ACCESSORIES		
optional Swaybar		1: SMA-SK-60-2000A (2.0 m x Ø60mm)
Further Accessories		SMA-SKO-UNIVERSAL-L : Universal sway bar fixation kit
		,

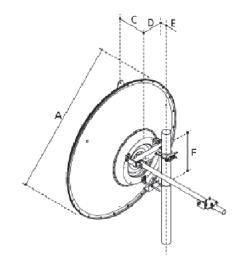
PAD6-W59BC REV: A REV DATE: 05. Apr 12 www.rfsworld.com



TrunkLine Antenna, Standard (FCC 101, Cat A), Single Polarized, 6 ft

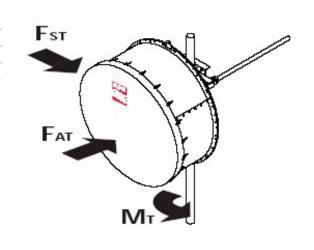
Mount Outline

Dimension A	mm (in)	2000 (79)
Dimension C	mm (in)	364 (14.3)
Dimension D for 114mm (4.5in) Pipe	mm (in)	175 (6.9)
Dimension E	mm (in)	283 (11.1)
Dimension F	mm (in)	590 (23.2)



Wind Load

FST Side force max. @ survival wind speed	N (lb)	2910 (651)
FAT Axial force max. @ survival wind speed	N (lb)	9900 (2217)
MT Torque maximum @ survival wind speed	Nm (lb ft)	3055 (2270)



External Document Links

Complete Antenna installation

RPE (IQ-Link format)

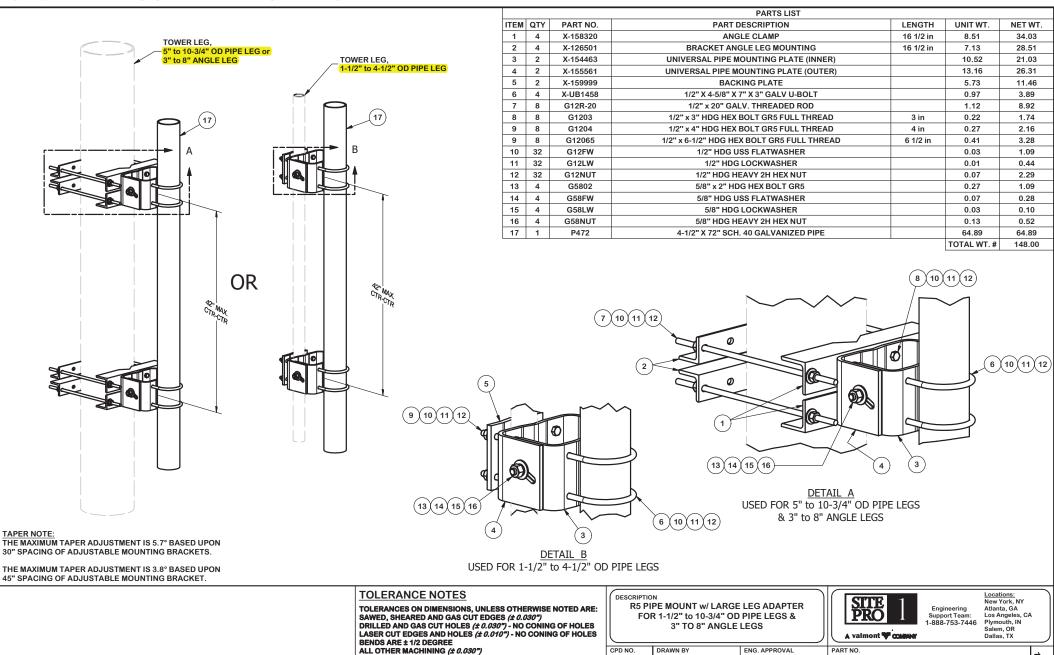
RPE (PDF format)

RPE (Pathloss format)

Only available in North America

Notes

PAD6-W59BC REV: A REV DATE: 05. Apr 12 www.rfsworld.com



4718

CLASS SUB

81 01

RH18 3/30/2010

CUSTOMER

CHECKED BY

4/21/2010

BMC

DWG. NO.

DRAWING USAGE

R5-LL

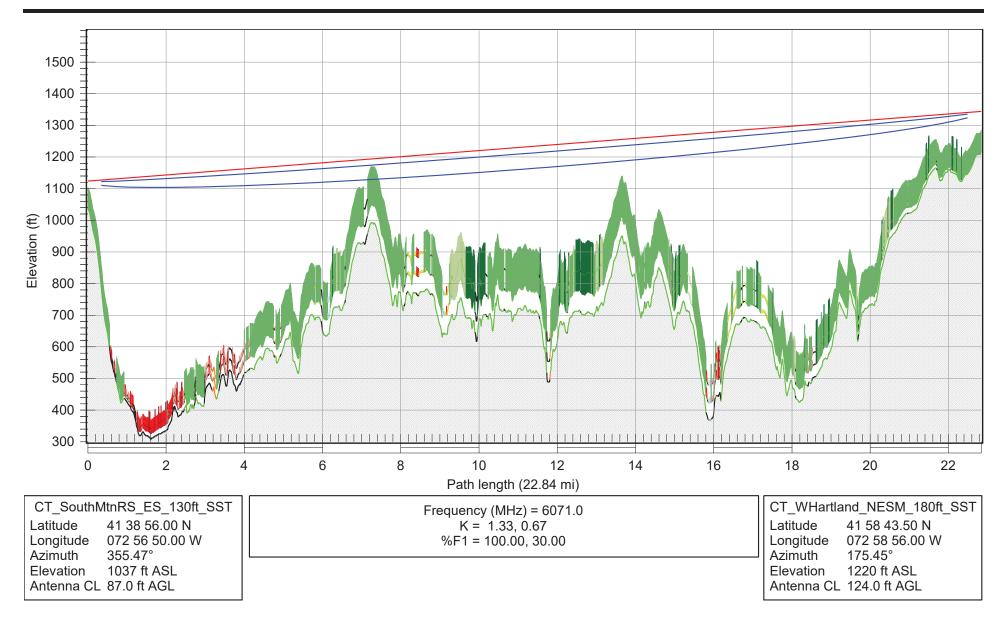
R5-LL

OF OF

ALL OTHER ASSEMBLY (± 0.060")

PROPRIETARY NOTE:
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Microwave Path Data Sheet COMSEARCH

19700 Janelia Farm Boulevard, Ashburn, VA, 20147

(703)636-5234 www.comsearch.com

PCN Date: 09/18/2019 New Path

Job Number: 190918COMSDS04 RCN Number: 19091852

Administrative Information	SOUTH MTN CT	W HARTLAND CT
City/County	Bristol/Hartford	/Hartford
Status / License Basis	Engineering Proposal / PRIMARY OPERATION	Engineering Proposal / PRIMARY OPERATION
Call Sign	KVG93	
Licensee Code	S68716	S68716
Licensee Name	Eversource Energy Service Company	Eversource Energy Service Company
Radio Service / Station Class	MG Microwave Industrial/Business Pool	FXO Fixed
Site Information		
Latitude (NAD 83)	41 ° 38' 56.0" N	41 ° 58' 43.5" N
Longitude (NAD 83)	72 ° 56' 50.0" W	72 ° 58' 56.0" W
Ground Elevation (m/ft-AMSL)	310.60 / 1019.0	371.71 / 1219.5
Antenna Structure Registration #		0 , . =
Path Azimuth (°)	355.473	175.450
Path Length (km / miles)	36.753 / 22.837	110.400
Faul Lengul (Kill / Illiles)	30.133 / 22.031	
Transmit Antenna	44008C	44008C
Manufacturer	RFS	RFS
Model	PAD6-59B	PAD6-59B
Gain(dBi) / Beamwidth(°) / Tilt(°)		38.7 / 1.80 / -0.24
` ,	26.52 / 87.0	37.80 / 124.0
Centerline (m / ft - AGL)	20.32 / 07.0	31.00 / 124.0

Same As Transmit

Receive Antenna

Manufacturer

Model

Gain (dBi) / Beamwidth (°) Centerline (m / ft - AGL)

Diversity Receive Antenna

Manufacturer

Model

Gain (dBi) / Beamwidth (°) Centerline (m / ft - AGL)

Radio Information	TEEV61		TEEV61			
Manufacturer	Aviat Networks, Inc.		Aviat Networks, Inc.			
Model	I600V4EHPL6-30M 256Q 179	9	I600V4EHPL6-30M 256Q 179			
Model Description	ECLIPSE IRU 600 RAC 60-6X MAX	TP	ECLIPSE IRU 600 RAC 60-6X MAX TP			
Emission Designator / Modulation	30M0D7W 256 QAM		30M0D7W 256 QAM			
_oading	1 CH DIG 179000.000		1 CH DIG 179000.000			
Stability (%)	0.0005		0.0005			
	Nominal Coordinated Ma	ıximum	Nominal Coordinated Maximum			
ower (dBm)	37.0		37.0			
Received Level (dBm)	-31.9		-31.9			
EIRP (dBm)	72.6		72.1			
Fixed Loss: Tx / Common (dB)	0.0 / 3.1		0.0 / 3.6			
Free Space Loss (dB)		139.6				
Fransmit Frequencies (MHz)	5945.2000V(11T)		6197.2400V(21T)			



Date: January 13, 2022



Subject: Structural Analysis Report

Eversource Designation: Number: ES-004

Site Name: SouthMtnsRS

Engineering Firm Designation: Black & Veatch Corp Project Number: 405025

Site Data: 790 Willis Street, Bristol, Hartford County, CT

Latitude 41° 38' 56.0", Longitude -72° 56' 50.0'

130 Foot - Self Support Tower

Black & Veatch Corp is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC1: Proposed Equipment Configuration

Sufficient Capacity - 41.8%

This analysis utilizes an ultimate 3-second gust wind of 130 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: Sanyukta R. Arvikar

Respectfully submitted by:

Joshua J Riley, P.E. Professional Engineer



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tnxTower Output

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Additional Calculations

1) INTRODUCTION

This tower is a 130 ft Self Support tower manufactured by Radian in December of 2006.

2) ANALYSIS CRITERIA

TIA-222 Revision: TIA-222-H

Risk Category:

Wind Speed: 130 mph

Exposure Category:
Topographic Factor:
Ice Thickness:
Wind Speed with Ice:
Seismic Ss:
Seismic S1:
O.064
Service Wind Speed:
60 mph

Seismic loading does not control per engineering judgement.

Table 1 - Proposed Equipment Configuration

Mounting Level (ft)	4-	Number of Antennas	Antenna Manufacturer		Number of Feed Lines	Feed Line Size (in)	Note		
130.0	140.0	1	comprod	876F-70-2	2	7/8	1		
87.0	87.0		1 rfs		rfs	PAD6-W59BC	1	E65J	
07.0	07.0	1	site pro 1	R5-LL [PM 602-1]	1	⊏000	-		

Note:

¹⁾ Antenna to be installed on existing pipe mount

Table 2 - Other Considered Equipment

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
	141.5	1	unknown	25' Omni			
	141.0	1	unknown	24' Omni			
	139.0	1 unknown 21' Omni					
	138.0	1	unknown	18' Omni			
127.0	137.0	1	unknown	16' Omni	10	7/8	1
127.0	136.0	1	unknown	16' Omni	10	110	'
	135.0	1	unknown	12' Omni			
	134.0	1	unknown	10' Omni			
	129.0	1	unknown	10"x8"x3" TMA			
	127.0	1	tower mounts	Sector Mount [SM 501-3]			
		3	rfs celwave	APXVAALL24_43 w/ Mount Pipe			
		3	ericsson	AIR6449 w/ Mount Pipe			
125.0	125.0	3	ericsson	AIR32 w/ Mount Pipe	3	6x24 fiber	1
		3	ericsson	4449			
		3	ericsson	4415			
			tower mounts	Sector Mount [SM 502-3]			
447.0	447.0	1	tower mounts	6' x 3" Mount Pipe		F00	4
117.0	117.0	1	unknown PA6-59		- 1	E60	1
440.0	120.0	1	1 celwave PD1142-1		1	1/2	_
113.0	113.0	1	tower mounts	Side Arm Mount [SO 306-1]	1	7/8	1
407.0	407.0	1	tower mounts	6' x 3" Mount Pipe	4	E05	4
107.0	107.0	1	unknown	6 FT Dish	1	E65	1
404.0	111.0	1	celwave	PD1142-1		7/0	4
104.0	104.0	1	tower mounts	Side Arm Mount [SO 306-1]	1	7/8	1
00.0	00.0	1	antennae	DB205-A		7/0	
98.0	98.0	1	tower mounts	Side Arm Mount [SO 306-1]	1	7/8	1
00.0	00.0	1	tower mounts	6' x 3" Mount Pipe		F00	
96.0	96.0	1	unknown	8 FT Dish	1	E60	1
20.0	00.0	1	tower mounts	6' x 3" Mount Pipe		F00	
86.0	86.0	1	unknown	PAD8-59AW	1	E60	1
04.0	91.0	1	celwave	PD1142-1		4/0	
84.0	84.0	1	tower mounts	Side Arm Mount [SO 306-1]	1	1/2	1
0.1.0		1	antennae	2' Yagi		= 10	
84.0	84.0	1	tower mounts	4'x2" Pipe Mount	1	7/8	1
78.0		1	andrew panel antennas	SBNH-1D6565A w/ Mount Pipe			
77.0	77.0	1	tower mounts	Sector Mount [SM 402-1]	1	1/2	1
	77.0	1	miscl	TMA	4	1 5/8	
	67.0	2	antennae	3" Dia 20' Omni			
-		1	tower mounts	6' x 3" Mount Pipe			
71.0 71.0		1	unknown	4 FT Dish	1	E65	1

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Model o		Number of Feed Lines	Feed Line Size (in)	Note
	73.0	1	antennae	3" Dia 20' Omni			
	73.0	1	unknown	Diamond X-500A	1	7/8	
63.0	62.0	1	tower mounts	Sector Mount [SM 402-1]	1	1/2	1
	63.0		miscl	TMA	3	1 5/8	
	53.0 2		antennae	3" Dia 20' Omni			
58.0	50.0		tower mounts	ower mounts Side Arm Mount [SO 306-1]		1/2	1
36.0	58.0	1	decibel	DB212-1	1	1/2	'
F4.0	F4.0	1	tower mounts	Side Arm Mount [SO 306-1]	4	1/0	1
54.0	54.0	1	decibel	DB212-1	l	1/2	
	46.0	1	antennae	3" Dia. 6' Omni	_	0.40	
43.0	43.0	1	tower mounts	Side Arm Mount [SO 306-1]	1	3/8 7/8	1
40.0		1	antennae	3" Dia. 6' Omni	'	170	
42.0	43.0		tower mounts	Side Arm Mount [SO 306-1]	1	2/0	1
43.0	43.0	1	decibel	DB230-2B		3/8	'

Note:

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Remarks	Reference	Source
TOWER STRUCTURAL ANALYSIS REPORTS	Centek Engineering, Inc., dated 09/16/2013	Tower geometry and geotechnical data	Eversource
TOWER STRUCTURAL ANALYSIS REPORTS	Centek Engineering, Inc., dated 06/14/2019	Tower geometry, tower loading and geotechnical data	Eversource
TOWER STRUCTURAL ANALYSIS REPORTS	Centek Engineering, Inc., dated 03/02/2021	Tower loading	Connecticut Siting Council

3.1) Analysis Method

tnxTower (version 8.1.1.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built and maintained in accordance with the manufacturer's specifications.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 3) This analysis was performed under the assumption that all information provided to Black & Veatch is current and correct. This is to include site data, appurtenance loading, tower/foundation details, and geotechnical data.
- 4) Tower loading is based on 2018 drone mapping photos and previous tower analyses.
- 5) The existing base plate grout was considered in this analysis. Grout must be maintained and inspected periodically and must be replaced if damaged or cracked

¹⁾ Existing Equipment

This analysis may be affected if any assumptions are not valid or have been made in error. Black & Veatch Corp should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	130 - 120	Leg	ROHN 2.5 STD	1	-12.20	60.05	20.3	Pass
T2	120 - 100	Leg	ROHN 3 STD	29	-23.60	74.43	31.7	Pass
Т3	100 - 80	Leg	ROHN 4 STD	69	-42.56	122.04	34.9	Pass
T4	80 - 60	Leg	ROHN 5 STD	107	-62.09	150.53	41.2	Pass
T5	60 - 40	Leg	ROHN 5 EH	134	-85.40	211.17	40.4	Pass
T6	40 - 20	Leg	ROHN 6 EHS	161	-107.07	256.16	41.8	Pass
T7	20 - 0	Leg	ROHN 6 EH	189	-128.00	318.80	40.2	Pass
T1	130 - 120	Diagonal	ROHN 2 STD	9	-3.39	25.36	13.4	Pass
T2	120 - 100	Diagonal	ROHN 2.5 STD	36	-4.71	35.92	13.1	Pass
T3	100 - 80	Diagonal	ROHN 2.5 STD	74	-6.27	31.52	19.9	Pass
T4	80 - 60	Diagonal	ROHN 2.5 X-STR	113	-8.46	21.63	39.1	Pass
T5	60 - 40	Diagonal	ROHN 3 STD	140	-7.88	29.61	26.6	Pass
T6	40 - 20	Diagonal	ROHN 3 STD	165	-8.05	26.21	30.7	Pass
T7	20 - 0	Diagonal	ROHN 3 STD	192	-8.22	22.99	35.7	Pass
T1	130 - 120	Horizontal	ROHN 1.5 STD	7	-2.43	23.71	10.2	Pass
T2	120 - 100	Horizontal	ROHN 2 STD	34	-2.87	34.21	8.4 10.0 (b)	Pass
T3	100 - 80	Horizontal	ROHN 2 STD	73	-4.24	28.55	14.9	Pass
T4	80 - 60	Horizontal	ROHN 2 STD	112	-4.95	23.75	20.8	Pass
T5	60 - 40	Horizontal	ROHN 2 STD	139	-5.04	17.60	28.6	Pass
T6	40 - 20	Horizontal	ROHN 2.5 STD	163	-5.46	30.30	18.0 19.3 (b)	Pass
T7	20 - 0	Horizontal	ROHN 2.5 STD	190	-5.85	23.43	25.0	Pass
T1	130 - 120	Top Girt	ROHN 1.5 STD	4	-0.48	23.77	2.0	Pass
T1	130 - 120	Inner Bracing	L2x2x1/8	16	-0.00	8.80	0.8	Pass
T2	120 - 100	Inner Bracing	L2x2x1/8	42	-0.01	6.48	0.9	Pass
T3	100 - 80	Inner Bracing	L2x2x1/8	79	-0.01	4.43	1.1	Pass
T4	80 - 60	Inner Bracing	L2x2x1/8	120	-0.01	3.34	1.2	Pass
T5	60 - 40	Inner Bracing	L2 1/2x2 1/2x3/16	147	-0.02	6.99	0.9	Pass
T6	40 - 20	Inner Bracing	L 3x3x3/16	174	-0.02	9.16	0.9	Pass
T7	20 - 0	Inner Bracing	L3 1/2x3 /12x1/4	201	-0.02	14.24	0.8	Pass
							Summary	
						Leg (T6)	41.8	Pass
						Diagonal (T4)	39.1	Pass
						Horizontal (T5)	28.6	Pass
						Top Girt (T1)	2.0	Pass
						Inner Bracing (T4)	1.2	Pass
						Bolt Checks	26.7	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
						Rating =	41.8	Pass

Table 5 - Tower Component Stresses vs. Capacity - LC1

Notes	Notes Component Elevation (ft)		% Capacity	Pass / Fail
1	Anchor Rods	0	27.3	Pass
1	Base Foundation	0	27.8	Pass
1	Base Foundation Soil Interaction	0	33.6	Pass

Structure Rating (max from all components) = 41.8%
--

Note:

4.1) Recommendation

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

¹⁾ See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity. Rating per TIA-222-H Section 15.5

Maximum Tower Deflections - Service Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist	Check*
No.		Deflection	Load			
	ft	in	Comb.	0	0	
T1	130 - 120	1.009	44	0.066	0.007	OK
T2	120 - 100	0.869	44	0.065	0.007	OK
T3	100 - 80	0.606	44	0.057	0.007	OK
T4	80 - 60	0.385	44	0.045	0.006	OK
T5	60 - 40	0.219	44	0.032	0.005	OK
T6	40 - 20	0.1	44	0.021	0.004	OK

^{*}Limit State Deformation (TIA-222-H Section 2.8.2)

Critical Deflections of Tower at the MW Dish Elevations - Service Wind

Elevation (ft)	MW Dish	Tilt (°)	Twist (°)	Diameter, D (ft)	Frequency, α (GHz)	Decibel Points	Deformation Limit $(\theta)^*$	Deformation Limit Exceeded?
117	PA6-59	0.064	0.007	6	10	10 dB	0.885	Not Exceeded
107	6 FT Dish	0.06	0.007	6	10	10 dB	0.885	Not Exceeded
96	8 FT Dish	0.054	0.007	8	10	10 dB	0.664	Not Exceeded
87	PAD6- W59BC	0.049	0.006	6.58333	10	10 dB	0.807	Not Exceeded
86	PAD8-59AW	0.048	0.006	8	10	10 dB	0.664	Not Exceeded
71	4 FT Dish	0.039	0.006	4	10	10 dB	1.328	Not Exceeded

^{*}Limit per TIA-222-H Annex D

¹⁾ Maximum Rotation = 4 Degrees

²⁾ Maximum Deflection = 0.03 * Tower Height = 47 in.

Maximum Tower Deflections - Design Wind

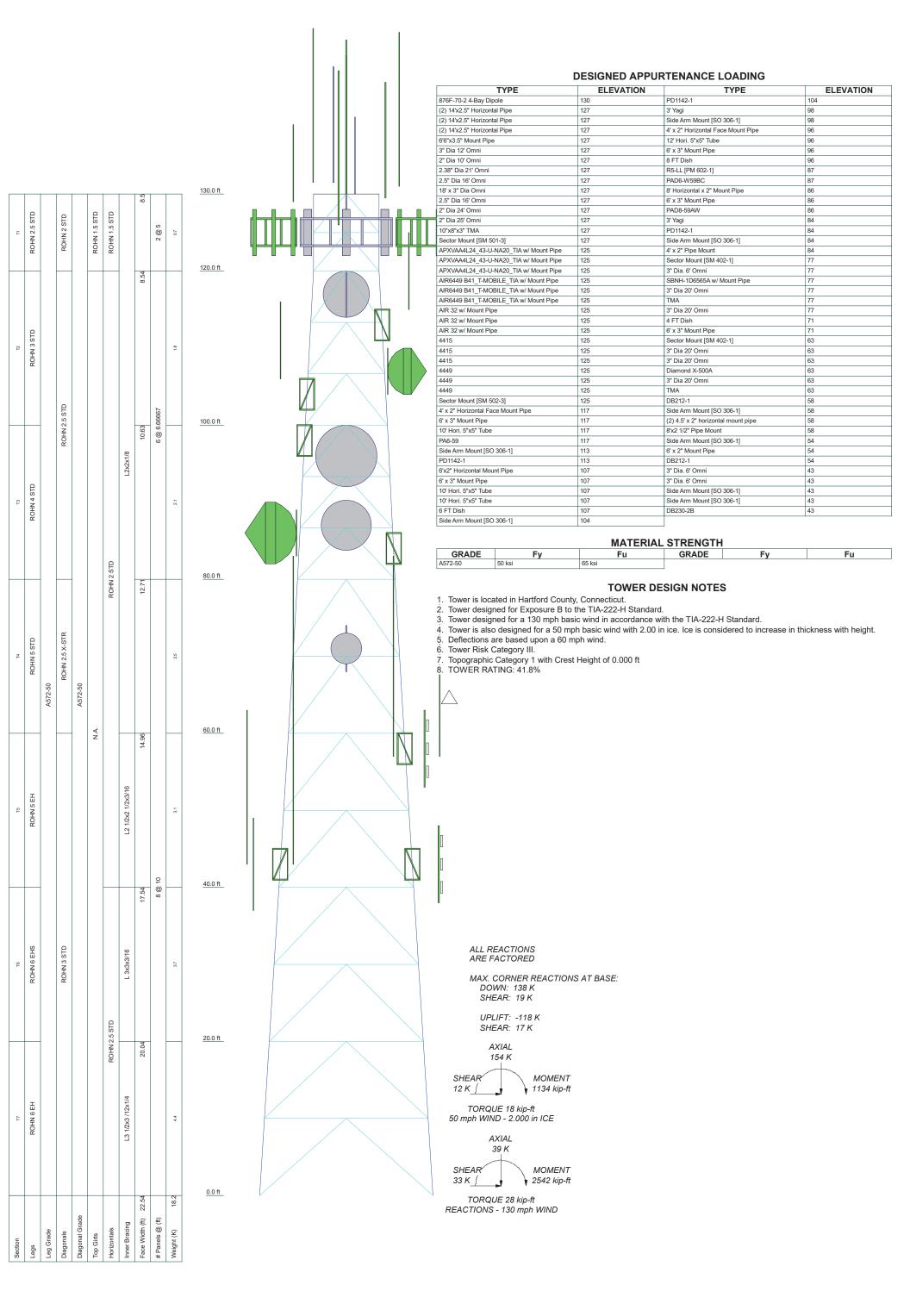
Section	Elevation	Horz.	Gov.	Tilt	Twist	Combined	Check*
No.		Deflection	Load			Max	
	ft	in	Comb.	0	0		
T1	130 - 120	2.81	44	0.183	0.021	0.184	OK
T2	120 - 100	2.418	44	0.18	0.021	0.181	OK
T3	100 - 80	1.685	44	0.157	0.019	0.158	OK
T4	80 - 60	1.069	44	0.123	0.018	0.124	OK
T5	60 - 40	0.609	44	0.088	0.015	0.089	OK
T6	40 - 20	0.28	44	0.057	0.01	0.058	OK

^{*}Up to 0.5 degree is considered acceptable per SUB090 Section 7

Critical Deflections of Tower at the MW Dish Elevations - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Tilt Twist	
		Load				ft
ft		Comb.	in	0	0	
117	PA6-59	44	2.303	0.178	0.021	59845.000
107	6 FT Dish	44	1.932	0.167	0.02	47266.000
96	8 FT Dish	44	1.551	0.151	0.019	36745.000
87	PAD6-W59BC	44	1.268	0.136	0.018	31007.000
86	PAD8-59AW	44	1.238	0.134	0.018	30438.000
71	4 FT Dish	44	0.844	0.107	0.017	32092.000

APPENDIX A TNXTOWER OUTPUT



FAX: (913) 458-8136

Tower Input Data

The main tower is a 3x free standing tower with an overall height of 130.000 ft above the ground line.

The base of the tower is set at an elevation of 0.000 ft above the ground line.

The face width of the tower is 8.500 ft at the top and 22.540 ft at the base.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

- Tower is located in Hartford County, Connecticut.
- Tower base elevation above sea level: 1047.000 ft.
- Basic wind speed of 130 mph.
- Risk Category III.
- Exposure Category B.
- Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- Topographic Category: 1.
- Crest Height: 0.000 ft.
- Nominal ice thickness of 2.000 in.
- Ice thickness is considered to increase with height.
- Ice density of 56 pcf.
- A wind speed of 50 mph is used in combination with ice.
- Temperature drop of 50 °F.
- Deflections calculated using a wind speed of 60 mph.
- Pressures are calculated at each section.
- Stress ratio used in tower member design is 1.05.
- Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification Use Code Stress Ratios Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile

√ Include Bolts In Member Capacity

Leg Bolts Are At Top Of Section

Secondary Horizontal Braces Leg
Use Diamond Inner Bracing (4 Sided)
SR Members Have Cut Ends
SR Members Are Concentric

Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area
- √ Use Clear Spans For KL/r
 Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt.

Autocalc Torque Arm Areas

Add IBC .6D+W Combination
Sort Capacity Reports By Component

 Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs Use ASCE 10 X-Brace Ly Rules
√ Calculate Redundant Bracing Forces

Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation

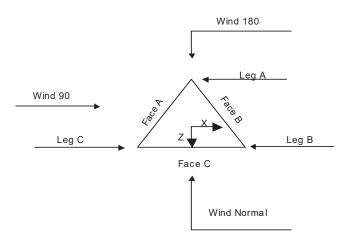
√ Consider Feed Line Torque

✓ Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice

Exemption

Poles

Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known



Triangular Tower

Te	ower (Section	Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	ft			ft		ft
T1	130.000-			8.500	1	10.000
	120.000					
T2	120.000-			8.540	1	20.000
	100.000					
T3	100.000-80.000			10.630	1	20.000
T4	80.000-60.000			12.710	1	20.000
T5	60.000-40.000			14.960	1	20.000
Т6	40.000-20.000			17.540	1	20.000
T7	20.000-0.000			20.040	1	20.000

Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft		Panels		in	in
T1	130.000-	5.000	K Brace Down	No	Yes	0.000	0.000
	120.000						
T2	120.000-	6.667	K Brace Down	No	Yes	0.000	0.000
	100.000						
T3	100.000-80.000	6.667	K Brace Down	No	Yes	0.000	0.000
T4	80.000-60.000	10.000	K Brace Down	No	Yes	0.000	0.000
T5	60.000-40.000	10.000	K Brace Down	No	Yes	0.000	0.000
T6	40.000-20.000	10.000	K Brace Down	No	Yes	0.000	0.000
T7	20.000-0.000	10.000	K Brace Down	No	Yes	0.000	0.000

Tower Section Geometry (cont'd)

Tower	Leg	Leg	Leg	Diagonal	Diagonal	Diagonal
Elevation	Type	Size	Grade	Туре	Size	Grade
ft						
T1 130.000-	Pipe	ROHN 2.5 STD	A572-50	Pipe	ROHN 2 STD	A572-50
120.000			(50 ksi)			(50 ksi)
T2 120.000-	Pipe	ROHN 3 STD	A572-50	Pipe	ROHN 2.5 STD	A572-50
100.000			(50 ksi)			(50 ksi)
T3 100.000-	Pipe	ROHN 4 STD	A572-50	Pipe	ROHN 2.5 STD	A572-50
80.000			(50 ksi)			(50 ksi)
T4 80.000-	Pipe	ROHN 5 STD	A572-50	Pipe	ROHN 2.5 X-STR	A572-50
60.000			(50 ksi)			(50 ksi)
T5 60.000-	Pipe	ROHN 5 EH	A572-50	Pipe	ROHN 3 STD	A572-50
40.000			(50 ksi)			(50 ksi)
T6 40.000-	Pipe	ROHN 6 EHS	A572-50	Pipe	ROHN 3 STD	A572-50
20.000			(50 ksi)			(50 ksi)
T7 20.000-	Pipe	ROHN 6 EH	A572-50	Pipe	ROHN 3 STD	A572-50
0.000			(50 ksi)			(50 ksi)

Tower Section Geometry (cont'd)											
Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizonta Grade				
Γ1 130.000- 120.000	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 1.5 STD	A572-50 (50 ksi)				
T2 120.000- 100.000	None	Flat Bar		`A36 ´ (36 ksi)	Pipe	ROHN 2 STD	À572-50 (50 ksi)				
T3 100.000- 80.000	None	Flat Bar		`A36 [′] (36 ksi)	Pipe	ROHN 2 STD	À572-50 (50 ksi)				
T4 80.000- 60.000	None	Flat Bar		`A36 ´ (36 ksi)	Pipe	ROHN 2 STD	À572-50 (50 ksi)				
T5 60.000- 40.000	None	Flat Bar		`A36 [′] (36 ksi)	Pipe	ROHN 2 STD	À572-50 (50 ksi)				
T6 40.000- 20.000	None	Flat Bar		`A36 [′] (36 ksi)	Pipe	ROHN 2.5 STD	À572-50 (50 ksi)				
T7 20.000- 0.000	None	Flat Bar		`A36 [′] (36 ksi)	Pipe	ROHN 2.5 STD	À572-50 (50 ksi)				

		Tower Se	ection G	eometry	(cont'd)	
Tower Elevation ft	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
T1 130.000- 120.000	Solid Round		A572-50 (50 ksi)	Single Angle	L2x2x1/8	A36 (36 ksi)
T2 120.000- 100.000	Solid Round		A572-50 (50 ksi)	Single Angle	L2x2x1/8	A36 (36 ksi)
T3 100.000- 80.000	Solid Round		A572-50 (50 ksi)	Single Angle	L2x2x1/8	A36 (36 ksi)
T4 80.000- 60.000	Solid Round		À572-50 (50 ksi)	Single Angle	L2x2x1/8	A36 (36 ksi)
T5 60.000- 40.000	Solid Round		À572-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x3/16	`A36 ´ (36 ksi)
T6 40.000- 20.000	Solid Round		À572-50 (50 ksi)	Single Angle	L 3x3x3/16	`A36 [′] (36 ksi)
T7 20.000- 0.000	Solid Round		A572-50 (50 ksi)	Single Angle	L3 1/2x3 /12x1/4	A36 (36 ksi)

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft ²	in					in	in	in
T1 130.000- 120.000	0.000	0.375	A36 (36 ksi)	1	1.05	1.05	36.000	36.000	36.000
T2 120.000- 100.000	0.000	0.375	`A36 ´ (36 ksi)	1	1.05	1.05	36.000	36.000	36.000
T3 100.000- 80.000	0.000	0.375	A36 (36 ksi)	1	1.05	1.05	36.000	36.000	36.000
T4 80.000- 60.000	0.000	0.375	A36 (36 ksi)	1	1.05	1.05	36.000	36.000	36.000
T5 60.000- 40.000	0.000	0.375	A36 (36 ksi)	1	1.05	1.05	36.000	36.000	36.000
T6 40.000- 20.000	0.000	0.375	`A36 ´ (36 ksi)	1	1.05	1.05	36.000	36.000	36.000
T7 20.000- 0.000	0.000	0.375	`A36 ´ (36 ksi)	1	1.05	1.05	36.000	36.000	36.000

Tower Section Geometry (cont'd)

						K Fac	ctors1			
Tower Elevation	Calc K Single	Calc K Solid	Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace
_	Angles	Rounds		X	X	X	X	X	X	X
ft				Y	Y	Y	Y	Y	Y	Y
T1 130.000-	Yes	Yes	1	1	1	1	1	1	1	1
120.000				1	1	1	1	1	1	1
T2 120.000-	Yes	Yes	1	1	1	1	1	1	1	1
100.000				1	1	1	1	1	1	1
T3 100.000-	Yes	Yes	1	1	1	1	1	1	1	1
80.000				1	1	1	1	1	1	1
T4 80.000-	Yes	Yes	1	1	1	1	1	1	1	1
60.000				1	1	1	1	1	1	1
T5 60.000-	Yes	Yes	1	1	1	1	1	1	1	1
40.000				1	1	1	1	1	1	1
T6 40.000-	Yes	Yes	1	1	1	1	1	1	1	1
20.000				1	1	1	1	1	1	1
T7 20.000-	Yes	Yes	1	1	1	1	1	1	1	1
0.000				1	1	1	1	1	1	1

¹Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg								Diago	nal	Тор G	irt	Botton	n Girt	Mid	Girt	Long Ho	rizontal	Short Ho	rizontal
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U						
T1 130.000- 120.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75						
T2 120.000- 100.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75						
T3 100.000- 80.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75						

Tower Elevation ft	Leg		Diago	nal	Top G	irt	Botton	n Girt	Mid	Girt	Long Ho	rizontal	Short Ho	rizontal
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T4 80.000- 60.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T5 60.000- 40.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T6 40.000- 20.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T7 20.000- 0.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75

Tower Elevation ft	Redundant Horizontal		Redun Diago		Redundan Diagoi		Redunda Horizo		Redur Vert		Redund	ant Hip	Redunda Diago	,
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 130.000- 120.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T2 120.000- 100.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T3 100.000- 80.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T4 80.000- 60.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T5 60.000- 40.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T6 40.000- 20.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T7 20.000- 0.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75

Tower Section Geometry (cont'd)

Tower	Leg	Leg		Diagor	nal	Top G	irt	Bottom	Girt	Mid G	irt	Long Hori	zontal	l	-
Elevation	Connection													Horizor	ntal
ft	Type	5 " 6"		- " O'		5 " 0"		D # 01		5 // 6/	.,	D # 01		5 " 6"	
		Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.						
		in		in		in		in		in		in		in	
T1 130.000-	Flange	0.750	4	0.625	3	0.625	0	0.625	0	0.625	0	0.625	2	0.625	0
120.000		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T2 120.000-	Flange	0.875	4	0.625	3	0.625	0	0.625	0	0.625	0	0.625	2	0.625	0
100.000	_	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T3 100.000-	Flange	1.000	4	0.625	3	0.625	0	0.625	0	0.625	0	0.625	2	0.625	0
80.000	_	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T4 80.000-	Flange	1.000	4	0.625	3	0.625	0	0.625	0	0.625	0	0.625	2	0.625	0
60.000	•	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T5 60.000-	Flange	1.000	6	0.625	3	0.625	0	0.625	0	0.625	0	0.625	2	0.625	0
40.000	· ·	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T6 40.000-	Flange	1.000	6	0.625	3	0.625	0	0.625	0	0.625	0	0.625	2	0.625	0
20.000	· ·	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T7 20.000-	Flange	1.000	0	0.625	3	0.625	0	0.625	0	0.625	0	0.625	2	0.625	0
0.000	Ü	A325N		A325N		A325N		A325N		A325N		A325N		A325N	

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or	Allow Shield	Exclude From	Componen t	Placement	Face Offset	Lateral Offset	#	# Per	Clear Spacin	Width or Diameter	Perimete r	Weight
	Leg		Torque Calculation	Type	ft	in	(Frac FW)		Row	g in	in	in	plf
Climbing Ladder (Af)	С	No	No	Af (CaAa)	130.000 - 0.000	10.000	0.4	1	1	3.000	3.000		8.40
Safety Line 3/8	С	No	No	Ar (CaAa)	130.000 - 0.000	10.000	0.4	1	1	0.375	0.375		0.22
LDF5- 50A(7/8)	С	No	No	Ar (CaAa)	130.000 - 7.000	0.000	-0.44	10	10	1.000	1.030		0.33
Feedline Ladder (Af)	С	No	No	Af (CaAa)	130.000 - 0.000	0.000	-0.4	1	1	3.000	3.000		8.40
E60	С	No	No	Ar (CaAa)	117.000 - 107.000	0.000	-0.375	1	1	1.000	2.200		1.10
E65+E60	С	No	No	Ar (CaAa)	107.000 - 96.000	0.000	-0.375	2	2	1.000	2.200		1.10
E60+E65+E6 0	С	No	No	Ar (CaAa)	96.000 - 86.000	0.000	-0.375	3	3	1.000	2.200		1.10
E60+E60+E6 5+E60	С	No	No	Ar (CaAa)	86.000 - 71.000	0.000	-0.375	4	4	1.000	2.200		1.10
E65+E60+E6 0+E65+E60	С	No	No	Ar (CaAa)	71.000 - 7.000	0.000	-0.375	5	5	1.000	2.200		1.10
HYBRIFLEX 1-5/8"	Α	No	No	Ar (CaAa)	125.000 - 7.000	0.000	-0.42	3	3	1.000	1.980		0.82
Feedline Ladder (Af)	Α	No	No	Af (CaAa)	130.000 - 0.000	0.000	-0.42	1	1	3.000	3.000		8.40
LDF5- 50A(7/8)	С	No	No	Ar (CaAa)	113.000 - 7.000	3.000	-0.47	1	1	1.000	1.030		0.33
LDF4- 75A(1/2)	С	No	No	Ar (CaAa)	113.000 - 7.000	2.000	-0.454	1	1	0.500	0.630		0.16
LDF5- 50A(7/8)	С	No	No	Ar (CaAa)	104.000 - 7.000	1.500	-0.47	1	1	1.000	1.030		0.33
LDF5- (50A(7/8)	С	No	No	Ar (CaAa)	98.000 - 7.000	4.500	-0.47	1	1	1.000	1.030		0.33
LDF5- (50A(7/8)	С	No	No	Ar (CaAa)	84.000 - 7.000	2.000	-0.46	1	1	1.000	1.030		0.33
LDF4- 50A(1/2)	С	No	No	Ar (CaAa)	84.000 - 7.000	2.000	-0.445	1	1	0.500	0.625		0.15
Feedline Ladder (Af)	С	No	No	Af (CaAa)	80.000 - 0.000	0.000	0.42	1	1	3.000	3.000		8.40
LDF4- 50A(1/2)	С	No	No	Ar (CaAa)	77.000 - 7.000	0.000	0.405	1	1	0.500	0.625		0.15
LDF7-50A(1- 5/8)	С	No	No	Ar (CaAa)	77.000 - 7.000	0.000	0.37	4	4	1.000	1.980		0.82
LDF5- 50A(7/8)	С	No	No	Ar (CaAa)	63.000 - 7.000	0.000	0.44	1	1	1.000	1.030		0.33
LDF4- 50A(1/2)	С	No	No	Ar (CaAa)	63.000 - 7.000	0.000	0.415	1	1	0.500	0.625		0.15
LDF7-50A(1- 5/8)	С	No	No	Ar (CaAa)	63.000 - 7.000	0.000	0.47	3	3	1.000	1.980		0.82
LDF4- 75A(1/2)	С	No	No	Ar (CaAa)	58.000 - 7.000	2.000	-0.438	1	1	0.500	0.630		0.16
LDF4- 75A(1/2)	С	No	No	Ar (CaAa)	54.000 - 7.000	2.000	-0.43	1	1	0.500	0.630		0.16
LDF5- 50A(7/8)	С	No	No	Ar (CaAa)	43.000 - 7.000	0.000	0.43	1	1	1.000	1.030		0.33
LDF2- 50A(3/8)	С	No	No	Ar (CaAa)	43.000 - 7.000	2.000	-0.418	1	1	0.440	0.440		80.0
LDF2- 50A(3/8) ****Proposed ***	С	No	No	Ar (CaAa)	43.000 - 7.000	2.000	-0.424	1	1	0.440	0.440		0.08
LDF5- 50A(7/8)	С	No	No	Ar (CaAa)	127.000 - 0.000	1.500	-0.412	2	1	0.500	1.030		0.33
E65	С	No	No	Ar (CaAa)	87.000 - 0.000	4.500	-0.412	1	1	0.500	2.200		1.10
**													

	Feed Line/Linear Appurtenances - Entered As Area										
Description	Face Allow	Exclude	Componen	Placement	Total	$C_{A}A_{A}$	Weight				
Description	or Shiel		t	riacement	Number	$\mathcal{C}_{A}\mathcal{A}_{A}$	vveignt				
	Leg	Torque Calculation	Type	ft		ft²/ft	plf				
**											

Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	A_R	A_F	C _A A _A	C_AA_A	Weight
Sectio	Elevation				In Face	Out Face	J
n	ft		ft ²	ft ²	ft ²	ft ²	K
T1	130.000-120.000	Α	0.000	0.000	7.970	0.000	0.10
		В	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	22.117	0.000	0.21
T2	120.000-100.000	Α	0.000	0.000	21.880	0.000	0.22
		В	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	53.320	0.000	0.45
T3	100.000-80.000	Α	0.000	0.000	21.880	0.000	0.22
		В	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	68.546	0.000	0.52
T4	80.000-60.000	Α	0.000	0.000	21.880	0.000	0.22
		В	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	107.445	0.000	0.81
T5	60.000-40.000	Α	0.000	0.000	21.880	0.000	0.22
		В	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	127.489	0.000	0.89
T6	40.000-20.000	Α	0.000	0.000	21.880	0.000	0.22
		В	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	131.240	0.000	0.90
T7	20.000-0.000	Α	0.000	0.000	17.722	0.000	0.20
		В	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	99.051	0.000	0.77

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Sectio	Tower Elevation	Face or	Ice Thickness	A_R	A_F	C _A A _A In Face	C _A A _A Out Face	Weight
n	ft	Leg	in	ft²	ft ²	ft ²	ft ²	K
T1	130.000-120.000	A	2.628	0.000	0.000	19.661	0.000	0.47
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	66.600	0.000	1.45
T2	120.000-100.000	Α	2.594	0.000	0.000	57.773	0.000	1.23
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	175.603	0.000	3.68
T3	100.000-80.000	Α	2.543	0.000	0.000	57.217	0.000	1.21
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	234.878	0.000	4.67
T4	80.000-60.000	Α	2.480	0.000	0.000	56.537	0.000	1.17
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	346.446	0.000	6.85
T5	60.000-40.000	Α	2.398	0.000	0.000	55.653	0.000	1.13
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	423.737	0.000	7.97
T6	40.000-20.000	Α	2.278	0.000	0.000	54.369	0.000	1.07
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	442.797	0.000	7.97
T7	20.000-0.000	Α	2.041	0.000	0.000	40.044	0.000	0.80
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	305.012	0.000	5.27

Feed	l ine	Center	of	Pressure
I CCU		OCITION	VI.	ııcəsuic

Section	Elevation	CP _X	CPz	CP_X	CPz
				Ice	Ice
	ft	in	in	in	in
T1	130.000-120.000	3.384	8.185	3.532	9.223
T2	120.000-100.000	4.494	10.970	7.168	14.311
T3	100.000-80.000	9.740	13.955	14.807	19.312
T4	80.000-60.000	5.496	20.381	10.628	27.371
T5	60.000-40.000	1.358	24.024	6.546	33.538
T6	40.000-20.000	1.534	26.372	8.501	38.194
T7	20.000-0.000	-0.374	24.870	5.731	36.017

Shielding Factor Ka

Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.	•	Segment	No Ice	Ice
		011 11 11 (16)	Elev.		0.5450
T1	1	Climbing Ladder (Af)	120.00 -	0.6000	0.5472
T1	2	Safety Line 3/8	130.00 120.00 -	0.6000	0.5472
' '		Salety Line 3/6	130.00	0.0000	0.5472
T1	3	LDF5-50A(7/8)	120.00 -	0.6000	0.5472
			130.00		3.3
T1	4	Feedline Ladder (Af)	120.00 -	0.6000	0.5472
			130.00		
T1	10	HYBRIFLEX 1-5/8"	120.00 -	0.6000	0.5472
T4	40	Farallina Laddan (A6)	125.00	0.0000	0.5470
T1	13	Feedline Ladder (Af)	120.00 -	0.6000	0.5472
T1	32	LDF5-50A(7/8)	130.00 120.00 -	0.6000	0.5472
'''	52	EDI 3-30A(110)	127.00	0.0000	0.5472
T2	1	Climbing Ladder (Af)	100.00 -	0.6000	0.6000
		- 3 ()	120.00		
T2	2	Safety Line 3/8	100.00 -	0.6000	0.6000
		-	120.00		
T2	3	LDF5-50A(7/8)	100.00 -	0.6000	0.6000
то.	_	- III	120.00	0.0000	0.0000
T2	4	Feedline Ladder (Af)	100.00 -	0.6000	0.6000
T2	5	E60	120.00 107.00 -	0.6000	0.6000
12	3	Loo	117.00	0.0000	0.0000
T2	6	E65+E60	100.00 -	0.6000	0.6000
		200 200	107.00	0.000	0.000
T2	10	HYBRIFLEX 1-5/8"	100.00 -	0.6000	0.6000
			120.00		
T2	13	Feedline Ladder (Af)	100.00 -	0.6000	0.6000
			120.00		
T2	14	LDF5-50A(7/8)	100.00 -	0.6000	0.6000
T2	15	LDE4 75 A (4/2)	113.00 100.00 -	0.6000	0.6000
12	15	LDF4-75A(1/2)	113.00	0.6000	0.6000
T2	16	LDF5-50A(7/8)	100.00 -	0.6000	0.6000
12	10	251 0 001 (170)	104.00	0.0000	0.0000
T2	32	LDF5-50A(7/8)	100.00 -	0.6000	0.6000
		` ,	120.00		
Т3	1	Climbing Ladder (Af)	80.00 -	0.6000	0.6000
	_		100.00		
Т3	2	Safety Line 3/8	80.00 -	0.6000	0.6000
Т3	3	LDF5-50A(7/8)	100.00 80.00 -	0.6000	0.6000
13	٥	LDF3-30A(7/6)	100.00		0.0000
		l	100.00		

Tower	Feed Line	Description	Feed Line	Ka	K _a
Section	Record No.	·	Segment Elev.	No lce	Ice
ТЗ	4	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
Т3	6	E65+E60	96.00 - 100.00	0.6000	0.6000
Т3	7	E60+E65+E60	86.00 -	0.6000	0.6000
Т3	8	E60+E60+E65+E60	96.00 80.00 - 86.00	0.6000	0.6000
Т3	10	HYBRIFLEX 1-5/8"	80.00 - 100.00	0.6000	0.6000
Т3	13	Feedline Ladder (Af)	80.00 -	0.6000	0.6000
T3	14	LDF5-50A(7/8)	100.00 80.00 - 100.00	0.6000	0.6000
T3	15	LDF4-75A(1/2)	80.00 -	0.6000	0.6000
T3	16	LDF5-50A(7/8)	100.00 80.00 - 100.00	0.6000	0.6000
ТЗ	17	LDF5-50A(7/8)	80.00 - 98.00	0.6000	0.6000
Т3	18	LDF5-50A(7/8)	80.00 - 84.00	0.6000	0.6000
Т3	19	LDF4-50A(1/2)	80.00 - 84.00	0.6000	0.6000
Т3	32	LDF5-50A(7/8)	80.00 - 100.00	0.6000	0.6000
Т3	33	E65	80.00 - 87.00	0.6000	0.6000
T4	1	Climbing Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T4	2	Safety Line 3/8	60.00 - 80.00	0.6000	0.6000
T4	3	LDF5-50A(7/8)	60.00 - 80.00	0.6000	0.6000
T4	4	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T4	8	E60+E60+E65+E60	71.00 - 80.00	0.6000	0.6000
T4	9	E65+E60+E60+E65+E60	60.00 - 71.00	0.6000	0.6000
T4	10	HYBRIFLEX 1-5/8"	60.00 - 80.00	0.6000	0.6000
T4	13	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T4	14	LDF5-50A(7/8)	60.00 - 80.00	0.6000	0.6000
T4	15	LDF4-75A(1/2)	60.00 - 80.00	0.6000	0.6000
T4	16	LDF5-50A(7/8)	60.00 - 80.00	0.6000	0.6000
T4	17	LDF5-50A(7/8)	60.00 - 80.00	0.6000	0.6000
T4	18	LDF5-50A(7/8)	60.00 - 80.00	0.6000	0.6000
T4	19	LDF4-50A(1/2)	60.00 - 80.00	0.6000	0.6000
T4	20	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T4	21	LDF4-50A(1/2)	60.00 - 77.00	0.6000	0.6000
T4	22	LDF7-50A(1-5/8)	60.00 - 77.00	0.6000	0.6000
T4	23	LDF5-50A(7/8)	60.00 - 63.00	0.6000	0.6000
T4	24	LDF4-50A(1/2)	60.00 - 63.00	0.6000	0.6000
T4	25	LDF7-50A(1-5/8)	60.00 - 63.00	0.6000	0.6000
T4	32	LDF5-50A(7/8)	60.00 -	0.6000	0.6000

Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.	Восопраст	Segment	No Ice	Ice
			<i>Elev.</i> 80.00		
T4	33	E65	60.00 -	0.6000	0.6000
T5	1	Climbing Ladder (Af)	80.00 40.00 - 60.00	0.6000	0.6000
T5	2	Safety Line 3/8	40.00 - 60.00	0.6000	0.6000
T5	3	LDF5-50A(7/8)	40.00 - 60.00	0.6000	0.6000
Т5	4	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T5	9	E65+E60+E60+E65+E60	40.00 - 60.00	0.6000	0.6000
T5	10	HYBRIFLEX 1-5/8"	40.00 - 60.00	0.6000	0.6000
T5	13	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
Т5	14	LDF5-50A(7/8)	40.00 - 60.00	0.6000	0.6000
T5	15	LDF4-75A(1/2)	40.00 - 60.00	0.6000	0.6000
T5	16	LDF5-50A(7/8)	40.00 - 60.00	0.6000	0.6000
T5	17	LDF5-50A(7/8)	40.00 - 60.00	0.6000	0.6000
T5	18	LDF5-50A(7/8)	40.00 - 60.00	0.6000	0.6000
T5	19	LDF4-50A(1/2)	40.00 - 60.00	0.6000	0.6000
T5	20	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T5	21	LDF4-50A(1/2)	40.00 - 60.00	0.6000	0.6000
T5	22	LDF7-50A(1-5/8)	40.00 - 60.00	0.6000	0.6000
T5	23	LDF5-50A(7/8)	40.00 - 60.00	0.6000	0.6000
T5	24	LDF4-50A(1/2)	40.00 - 60.00	0.6000	0.6000
T5	25	LDF7-50A(1-5/8)	40.00 - 60.00	0.6000	0.6000
T5	26	LDF4-75A(1/2)	40.00 - 58.00	0.6000	0.6000
T5	27	LDF4-75A(1/2)	40.00 - 54.00	0.6000	0.6000
T5	28	LDF5-50A(7/8)	40.00 - 43.00	0.6000	0.6000
T5	29	LDF2-50A(3/8)	40.00 - 43.00	0.6000	0.6000
T5	30	LDF2-50A(3/8)	40.00 - 43.00	0.6000	0.6000
T5	32	LDF5-50A(7/8)	40.00 - 60.00	0.6000	0.6000
T5	33	E65	40.00 - 60.00	0.6000	0.6000
Т6	1	Climbing Ladder (Af)	20.00 - 40.00	0.6000	0.6000
Т6	2	Safety Line 3/8	20.00 - 40.00	0.6000	0.6000
Т6	3	LDF5-50A(7/8)	20.00 - 40.00	0.6000	0.6000
Т6	4	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
Т6	9	E65+E60+E60+E65+E60	20.00 - 40.00	0.6000	0.6000
Т6	10	HYBRIFLEX 1-5/8"	20.00 - 40.00	0.6000	0.6000
Т6	13	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000

Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.		Segment Elev.	No Ice	Ice
Т6	14	LDF5-50A(7/8)	20.00 - 40.00	0.6000	0.6000
Т6	15	LDF4-75A(1/2)	20.00 - 40.00	0.6000	0.6000
Т6	16	LDF5-50A(7/8)	20.00 - 40.00	0.6000	0.6000
Т6	17	LDF5-50A(7/8)	20.00 - 40.00	0.6000	0.6000
Т6	18	LDF5-50A(7/8)	20.00 - 40.00	0.6000	0.6000
Т6	19	LDF4-50A(1/2)	20.00 - 40.00	0.6000	0.6000
Т6	20	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
Т6	21	LDF4-50A(1/2)	20.00 - 40.00	0.6000	0.6000
Т6	22	LDF7-50A(1-5/8)	20.00 - 40.00	0.6000	0.6000
Т6	23	LDF5-50A(7/8)	20.00 - 40.00	0.6000	0.6000
Т6	24	LDF4-50A(1/2)	20.00 - 40.00	0.6000	0.6000
Т6	25	LDF7-50A(1-5/8)	20.00 - 40.00	0.6000	0.6000
Т6	26	LDF4-75A(1/2)	20.00 - 40.00	0.6000	0.6000
Т6	27	LDF4-75A(1/2)	20.00 - 40.00	0.6000	0.6000
Т6	28	LDF5-50A(7/8)	20.00 - 40.00	0.6000	0.6000
Т6	29	LDF2-50A(3/8)	20.00 - 40.00	0.6000	0.6000
Т6	30	LDF2-50A(3/8)	20.00 - 40.00	0.6000	0.6000
Т6	32	LDF5-50A(7/8)	20.00 - 40.00	0.6000	0.6000
Т6	33	E65	20.00 - 40.00	0.6000	0.6000
T7	1	Climbing Ladder (Af)	0.00 - 20.00	0.6000	0.6000
<u>T7</u>	2	Safety Line 3/8	0.00 - 20.00	0.6000	0.6000
T7	3	LDF5-50A(7/8)	7.00 - 20.00	0.6000	0.6000
<u>T7</u>	4	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T7	9	E65+E60+E60+E65+E60	7.00 - 20.00	0.6000	0.6000
T7	10	HYBRIFLEX 1-5/8"	7.00 - 20.00	0.6000	0.6000
T7	13	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T7	14	LDF5-50A(7/8)	7.00 - 20.00	0.6000	0.6000
T7	15	LDF4-75A(1/2)	7.00 - 20.00	0.6000	0.6000
T7	16	LDF5-50A(7/8)	7.00 - 20.00	0.6000	0.6000
T7	17	LDF5-50A(7/8)	7.00 - 20.00	0.6000	0.6000
T7	18	LDF5-50A(7/8)	7.00 - 20.00	0.6000	0.6000
T7	19	LDF4-50A(1/2)	7.00 - 20.00	0.6000	0.6000
T7	20	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T7	21	LDF4-50A(1/2)	7.00 - 20.00	0.6000	0.6000
T7	22	LDF7-50A(1-5/8)	7.00 - 20.00	0.6000	0.6000
T7	23	LDF5-50A(7/8)	7.00 - 20.00	0.6000	0.6000
T7	24	LDF4-50A(1/2)	7.00 - 20.00	0.6000	0.6000
T7	25	LDF7-50A(1-5/8)	7.00 - 20.00	0.6000	0.6000
T7	26	LDF4-75A(1/2)	7.00 - 20.00	0.6000	0.6000
T7	27	LDF4-75A(1/2)	7.00 - 20.00	0.6000	0.6000
T7	28	LDF5-50A(7/8)	7.00 - 20.00	0.6000	0.6000
T7	29	LDF2-50A(3/8)	7.00 - 20.00	0.6000	0.6000
T7	30	LDF2-50A(3/8)	7.00 - 20.00	0.6000	0.6000
T7	32	LDF5-50A(7/8)	0.00 - 20.00	0.6000	0.6000
T7	33	E65	0.00 - 20.00	0.6000	0.6000

Discrete Tower Loads											
Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight		
			ft ft ft	۰	ft		ft²	ft²	К		
Sector Mount [SM 501-3]	Α	From Face	0.000	0.000	127.000	No Ice	20.430	20.430	0.90		
			0.000			1/2"	30.050	30.050	1.28		
			0.000			Ice 1" Ice	40.280 64.650	40.280	1.80		
						2" Ice	04.000	64.650	3.23		
2) 14'x2.5" Horizontal Pipe	Α	From Face	0.000	0.000	127.000	No Ice	4.169	0.024	0.08		
			-4.000	0.000		1/2"	5.651	0.097	0.11		
			0.000			Ice	7.150	0.171	0.15		
						1" Ice	10.198	0.318	0.26		
	_					2" Ice					
2) 14'x2.5" Horizontal Pipe	В	From Face	0.000	0.000	127.000	No Ice	4.169	0.024	0.08		
			-4.000			1/2"	5.651	0.097	0.11		
			0.000			lce 1" lce	7.150 10.198	0.171 0.318	0.15 0.26		
						2" Ice	10.196	0.316	0.20		
2) 14'x2.5" Horizontal Pipe	С	From Face	0.000	0.000	127.000	No Ice	4.169	0.024	0.08		
_,			-4.000	0.000		1/2"	5.651	0.097	0.11		
			0.000			Ice	7.150	0.171	0.15		
						1" Ice	10.198	0.318	0.26		
	_					2" Ice					
6'6"x3.5" Mount Pipe	С	From Face	0.000	0.000	127.000	No Ice	2.161	2.161	0.02		
			0.000			1/2"	2.554	2.554	0.03		
			0.000			Ice 1" Ice	2.957 3.790	2.957 3.790	0.06 0.11		
						2" Ice	3.790	3.790	0.11		
3" Dia 12' Omni	В	From Face	0.000	0.000	127.000	No Ice	3.600	3.600	0.02		
			-2.000			1/2"	4.833	4.833	0.05		
			8.000			Ice	6.083	6.083	0.08		
						1" Ice	8.017	8.017	0.17		
211 Dia 401 Omai	0	Г.,	2 000	0.000	407.000	2" Ice	0.000	2.000	0.04		
2" Dia 10' Omni	С	From Leg	3.000 0.000	0.000	127.000	No Ice 1/2"	2.000 3.030	2.000 3.030	0.01 0.03		
			7.000			Ice	4.060	4.060	0.03		
			7.000			1" Ice	6.120	6.120	0.04		
						2" lce	0.120	0.120	0.01		
2.38" Dia 21' Omni	Α	From Face	0.000	0.000	127.000	No Ice	4.998	4.998	0.01		
			1.000			1/2"	7.126	7.126	0.05		
			12.000			Ice	9.271	9.271	0.10		
						1" Ice	13.611	13.611	0.24		
2.5" Dia 16' Omni	0	Г Г	0.000	0.000	407.000	2" Ice	4.000	4.000	0.00		
2.5 Dia 16 Omni	С	From Face	0.000 1.000	0.000	127.000	No Ice 1/2"	4.000 5.629	4.000 5.629	0.03 0.06		
			9.000			Ice	7.275	7.275	0.00		
			0.000			1" Ice	10.617	10.617	0.21		
						2" Ice					
18' x 3" Dia Omni	Α	From Leg	3.000	0.000	127.000	No Ice	5.400	5.400	0.14		
			0.000			1/2"	7.233	7.233	0.18		
			11.000			Ice	9.083	9.083	0.23		
						1" Ice	12.833	12.833	0.36		
2 5" Dia 16! Omni	D	Erom Loc	1 000	0.000	127.000	2" Ice	4.000	4.000	0.00		
2.5" Dia 16' Omni	В	From Leg	1.000 0.000	0.000	121.000	No Ice 1/2"	4.000 5.629	4.000 5.629	0.03 0.06		
			10.000			lce	5.629 7.275	7.275	0.06		
			10.000			1" Ice	10.617	10.617	0.10		
						2" Ice			J. <u>~</u> .		
2" Dia 24' Omni	В	From Leg	3.000	0.000	127.000	No Ice	4.800	4.800	0.05		
		-	0.000			1/2"	7.225	7.225	0.08		
			14.000			Ice	9.667	9.667	0.13		
						1" Ice	14.600	14.600	0.28		
						2" Ice					

2" Ice

No Ice

127.000

5.000

5.000

0.05

С

From Leg

0.000

0.000

2" Dia 25' Omni

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			Vert ft ft ft	۰	ft		ft²	ft²	К
			0.000			1/2"	7.525	7.525	0.09
			14.500			Ice 1" Ice 2" Ice	10.067 15.200	10.067 15.200	0.14 0.30
10"x8"x3" TMA	С	From Leg	2.000	0.000	127.000	No Ice	1.000	0.410	0.01
			0.000 2.000			1/2" Ice	1.131 1.270	0.510 0.618	0.02 0.03
			2.000			1" Ice 2" Ice	1.570	0.853	0.05
*** Sector Mount [SM 502-3]	Α	None		0.000	125.000	No Ice	29.820	29.820	1.67
00000:00 [0 002 0]				0.000	0.000	1/2"	42.210	42.210	2.27
						Ice 1" Ice 2" Ice	54.430 78.490	54.430 78.490	3.05 5.18
APXVAA4L24_43-U-	Α	From Face	3.000	0.000	125.000	No Ice	20.480	10.869	0.20
NA20_TIA w/ Mount Pipe			0.000 0.000			1/2" Ice	21.231 21.990	12.393 13.942	0.34 0.48
			0.000			1" Ice	23.444	16.291	0.40
. =	_					2" Ice			
APXVAA4L24_43-U- NA20_TIA w/ Mount Pipe	В	From Face	3.000 0.000	0.000	125.000	No Ice 1/2"	20.480 21.231	10.869 12.393	0.20 0.34
NAZO_NA W/ Wodner ipc			0.000			Ice	21.990	13.942	0.48
					405.000	1" Ice 2" Ice	23.444	16.291	0.81
APXVAA4L24_43-U- NA20_TIA w/ Mount Pipe	С	From Face	3.000 0.000	0.000	125.000	No Ice 1/2"	20.480 21.231	10.869 12.393	0.20 0.34
NAZO_TIA W/ WOUNT TIPO			0.000			Ice	21.990	13.942	0.48
						1" Ice 2" Ice	23.444	16.291	0.81
AIR6449 B41 T-	Α	From Face	3.000	0.000	125.000	No Ice	5.870	3.270	0.13
MOBILE_TIA w/ Mount			0.000			1/2"	6.233	3.728	0.18
Pipe			0.000			Ice 1" Ice 2" Ice	6.606 7.382	4.203 5.200	0.23 0.36
AIR6449 B41_T-	В	From Face	3.000	0.000	125.000	No Ice	5.870	3.270	0.13
MOBILE_TIA w/ Mount Pipe			0.000 0.000			1/2" Ice	6.233 6.606	3.728 4.203	0.18 0.23
Fipe			0.000			1" Ice	7.382	5.200	0.23
AIR6449 B41_T-	С	From Face	3.000	0.000	125.000	2" Ice No Ice	5.870	3.270	0.13
MOBILE TIA w/ Mount	C	I IOIII I ace	0.000	0.000	123.000	1/2"	6.233	3.728	0.13
Pipe			0.000			Ice	6.606	4.203	0.23
						1" Ice 2" Ice	7.382	5.200	0.36
AIR 32 w/ Mount Pipe	Α	From Face	3.000	0.000	125.000	No Ice	3.760	3.150	0.12
			0.000 0.000			1/2"	4.120 4.480	3.490 3.840	0.18 0.25
			0.000			Ice 1" Ice 2" Ice	5.240	4.580	0.25
AIR 32 w/ Mount Pipe	В	From Face	3.000	0.000	125.000	No Ice	3.760	3.150	0.12
			0.000 0.000			1/2" Ice	4.120 4.480	3.490 3.840	0.18 0.25
			0.000			1" Ice 2" Ice	5.240	4.580	0.41
AIR 32 w/ Mount Pipe	С	From Face	3.000	0.000	125.000	No Ice	3.760	3.150	0.12
			0.000 0.000			1/2" Ice	4.120 4.480	3.490 3.840	0.18 0.25
			0.000			1" Ice 1" Ice 2" Ice	5.240	4.580	0.23
4415	Α	From Face	3.000	0.000	125.000	No Ice	1.856	0.683	0.04
			0.000 0.000			1/2" Ice	2.027 2.204	0.801 0.925	0.06 0.07
			0.000			1" Ice	2.582	1.196	0.07
						2" Ice			

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			Vert ft ft ft	۰	ft		ft²	ft²	K
4415	В	From Face	3.000	0.000	125.000	No Ice	1.856	0.683	0.04
1110		1101111 400	0.000	0.000	120.000	1/2"	2.027	0.801	0.06
			0.000			Ice	2.204	0.925	0.07
						1" Ice 2" Ice	2.582	1.196	0.11
4415	С	From Face	3.000	0.000	125.000	No Ice	1.856	0.683	0.04
			0.000			1/2"	2.027	0.801	0.06
			0.000			Ice 1" Ice	2.204 2.582	0.925 1.196	0.07 0.11
4440	^	Г Г	2 000	0.000	105.000	2" Ice	4.000	4 400	0.07
4449	Α	From Face	3.000	0.000	125.000	No Ice 1/2"	1.969	1.402	0.07
			0.000			lce	2.145 2.329	1.558 1.720	0.09 0.11
			0.000			1" Ice 2" Ice	2.719	2.068	0.16
4449	В	From Face	3.000	0.000	125.000	No Ice	1.969	1.402	0.07
1110		1101111 400	0.000	0.000	120.000	1/2"	2.145	1.558	0.09
			0.000			Ice	2.329	1.720	0.11
						1" Ice	2.719	2.068	0.16
						2" Ice			
4449	С	From Face	3.000	0.000	125.000	No Ice	1.969	1.402	0.07
			0.000			1/2"	2.145	1.558	0.09
			0.000			Ice	2.329	1.720	0.11
						1" Ice 2" Ice	2.719	2.068	0.16
10' Hori. 5"x5" Tube	В	From Face	0.000	0.000	117.000	No Ice	5.000	0.208	0.16
			0.000			1/2"	5.712	0.268	0.20
			0.000			Ice	6.423	0.334	0.25
						1" Ice 2" Ice	7.847	0.490	0.33
4' x 2" Horizontal Face	Α	From Leg	0.500	0.000	117.000	No Ice	0.870	0.010	0.01
Mount Pipe	^	i ioni Leg	0.000	0.000	117.000	1/2"	1.110	0.050	0.01
Would Tipe			0.000			Ice	1.370	0.100	0.02
			0.000			1" Ice 2" Ice	1.900	0.240	0.06
6' x 3" Mount Pipe	Α	From Leg	0.500	0.000	117.000	No Ice	1.767	1.767	0.03
·		Ü	0.000			1/2"	2.129	2.129	0.04
			0.000			Ice	2.501	2.501	0.06
						1" Ice	3.272	3.272	0.11
						2" Ice			
Side Arm Mount [SO 306-	В	From Leg	0.000	0.000	113.000	No Ice	0.410	2.260	0.04
1]			0.000			1/2"	0.810	3.830	0.06
			0.000			Ice	1.230	5.480	0.09
						1" Ice 2" Ice	2.080	9.370	0.19
PD1142-1	В	From Leg	4.000	0.000	113.000	No Ice	1.316	1.316	0.01
		3	0.000			1/2"	3.210	3.210	0.02
			7.000			Ice	5.121	5.121	0.05
						1" Ice 2" Ice	8.993	8.993	0.14
10' Hori. 5"x5" Tube	С	From Face	0.000	0.000	107.000	No Ice	5.000	0.208	0.16
			0.000			1/2"	5.712	0.268	0.20
			0.000			Ice	6.423	0.334	0.25
						1" Ice 2" Ice	7.847	0.490	0.33
10' Hori. 5"x5" Tube	В	From Face	0.000	0.000	107.000	No Ice	5.000	0.208	0.16
			0.000			1/2"	5.712	0.268	0.20
			0.000			Ice	6.423	0.334	0.25
						1" Ice 2" Ice	7.847	0.490	0.33
6'x2" Horizontal Mount	В	From Face	0.500	0.000	107.000	No Ice	1.425	0.047	0.03
Pipe			0.000			1/2"	1.842	0.077	0.04
			0.000			Ice	2.266	0.115	0.06
						1" Ice	3.137	0.212	0.11
						2" Ice			

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			Vert ft ft ft	۰	ft		ft²	ft²	K
6' x 3" Mount Pipe	В	From Leg	0.500	0.000	107.000	No Ice	1.767	1.767	0.03
o no mount ipo	_		0.000	0.000		1/2"	2.129	2.129	0.04
			0.000			Ice	2.501	2.501	0.06
						1" Ice 2" Ice	3.272	3.272	0.11
Side Arm Mount [SO 306-	С	From Leg	0.000	0.000	104.000	No Ice	0.410	2.260	0.04
1]			0.000			1/2"	0.810	3.830	0.06
			0.000			Ice 1" Ice	1.230 2.080	5.480 9.370	0.09 0.19
DD4449.4	0	From Log	4.000	0.000	104.000	2" Ice	1 216	1 216	0.01
PD1142-1	С	From Leg	4.000	0.000	104.000	No Ice 1/2"	1.316	1.316	0.01
			0.000 7.000			lce	3.210 5.121	3.210 5.121	0.02 0.05
			7.000			1" Ice 2" Ice	8.993	8.993	0.14
Side Arm Mount [SO 306-	С	From Leg	0.000	0.000	98.000	No Ice	0.410	2.260	0.04
1]	Ŭ	r rom Log	0.000	0.000	00.000	1/2"	0.810	3.830	0.06
-1			0.000			Ice	1.230	5.480	0.09
						1" Ice	2.080	9.370	0.19
						2" Ice			
3' Yagi	С	From Leg	4.000	0.000	98.000	No Ice	2.083	2.083	0.03
3		3	0.000			1/2"	3.787	3.787	0.05
			0.000			Ice	5.517	5.517	0.09
						1" Ice 2" Ice	9.055	9.055	0.18
12' Hori. 5"x5" Tube	Α	From Face	0.000	0.000	96.000	No Ice	6.000	0.208	0.19
			0.000			1/2"	6.854	0.268	0.24
			0.000			Ice	7.708	0.334	0.30
						1" Ice 2" Ice	9.416	0.490	0.40
4' x 2" Horizontal Face	Α	From Leg	0.500	0.000	96.000	No Ice	0.870	0.010	0.01
Mount Pipe			0.000			1/2"	1.110	0.050	0.02
			0.000			Ice	1.370	0.100	0.03
						1" Ice 2" Ice	1.900	0.240	0.06
6' x 3" Mount Pipe	Α	From Leg	0.500	0.000	96.000	No Ice	1.767	1.767	0.03
			0.000			1/2"	2.129	2.129	0.04
			0.000			Ice	2.501	2.501	0.06
						1" Ice	3.272	3.272	0.11
8' Horizontal x 2" Mount	С	From Face	0.500	0.000	86.000	2" Ice	1.900	0.047	0.02
	C	FIOIII Face	0.000	0.000	00.000	No Ice 1/2"	2.450	0.047 0.077	0.03 0.05
Pipe			0.000			Ice	3.008	0.077	0.03
			0.000			1" Ice	4.145	0.212	0.14
						2" Ice	4.140	0.212	0.14
6' x 3" Mount Pipe	С	From Leg	0.500	0.000	86.000	No Ice	1.767	1.767	0.03
		3	0.000			1/2"	2.129	2.129	0.04
			0.000			Ice	2.501	2.501	0.06
						1" Ice 2" Ice	3.272	3.272	0.11
PD1142-1	В	From Leg	4.000	0.000	84.000	No Ice	1.316	1.316	0.01
		_	0.000			1/2"	3.210	3.210	0.02
			7.000			Ice	5.121	5.121	0.05
						1" Ice 2" Ice	8.993	8.993	0.14
Side Arm Mount [SO 306-	В	From Leg	0.000	0.000	84.000	No Ice	0.410	2.260	0.04
1]			0.000			1/2"	0.810	3.830	0.06
			0.000			Ice	1.230	5.480	0.09
						1" Ice 2" Ice	2.080	9.370	0.19
3' Yagi	Α	From Leg	0.500	0.000	84.000	No Ice	2.083	2.083	0.03
			0.000			1/2"	3.787	3.787	0.05
			0.000			Ice	5.517	5.517	0.09
						1" Ice 2" Ice	9.055	9.055	0.18

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			Vert ft ft ft	0	ft		ft²	ft²	K
4' x 2" Pipe Mount	Α	From Leg	0.500 0.000 0.000	0.000	84.000	No Ice 1/2" Ice 1" Ice	0.785 1.028 1.281 1.814	0.785 1.028 1.281 1.814	0.03 0.04 0.04 0.07
Sector Mount [SM 402-1]	В	From Leg	0.000 0.000 0.000	0.000	77.000	2" Ice No Ice 1/2" Ice 1" Ice	9.720 13.660 17.550 25.280	7.050 9.870 12.660 18.130	0.28 0.40 0.57 1.01
SBNH-1D6565A w/ Mount Pipe	В	From Leg	3.000 -6.000 1.000	0.000	77.000	2" Ice No Ice 1/2" Ice 1" Ice 2" Ice	5.599 6.007 6.417 7.263	4.774 5.446 6.095 7.443	0.06 0.11 0.16 0.30
3" Dia 20' Omni	В	From Leg	3.000 -6.000 -10.000	0.000	77.000	No Ice 1/2" Ice 1" Ice 2" Ice	4.000 6.000 8.000 12.000	4.000 6.000 8.000 12.000	0.06 0.10 0.14 0.23
3" Dia. 6' Omni	В	From Leg	3.000 6.000 3.000	0.000	77.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.767 2.129 2.501 3.272	1.767 2.129 2.501 3.272	0.05 0.06 0.08 0.12
3" Dia 20' Omni	В	From Leg	3.000 6.000 -10.000	0.000	77.000	No Ice 1/2" Ice 1" Ice 2" Ice	4.000 6.000 8.000 12.000	4.000 6.000 8.000 12.000	0.06 0.10 0.14 0.23
ТМА	В	From Leg	3.000 0.000 0.000	0.000	77.000	No Ice 1/2" Ice 1" Ice	0.600 0.704 0.815 1.059	0.407 0.497 0.593 0.815	0.01 0.02 0.02 0.04
6' x 3" Mount Pipe	Α	From Leg	0.500 0.000 0.000	0.000	71.000	2" Ice No Ice 1/2" Ice 1" Ice	1.767 2.129 2.501 3.272	1.767 2.129 2.501 3.272	0.03 0.04 0.06 0.11
Sector Mount [SM 402-1]	С	From Leg	0.000 0.000 0.000	0.000	63.000	2" Ice No Ice 1/2" Ice 1" Ice	9.720 13.660 17.550 25.280	7.050 9.870 12.660 18.130	0.28 0.40 0.57 1.01
3" Dia 20' Omni	С	From Leg	3.000 -6.000 10.000	0.000	63.000	2" Ice No Ice 1/2" Ice 1" Ice	4.000 6.000 8.000 12.000	4.000 6.000 8.000 12.000	0.06 0.10 0.14 0.23
3" Dia 20' Omni	С	From Leg	3.000 -6.000 -10.000	0.000	63.000	2" Ice No Ice 1/2" Ice 1" Ice	4.000 6.000 8.000 12.000	4.000 6.000 8.000 12.000	0.06 0.10 0.14 0.23
Diamond X-500A	С	From Leg	3.000 6.000 10.000	0.000	63.000	2" Ice No Ice 1/2" Ice 1" Ice	4.998 7.126 9.271 13.611	4.998 7.126 9.271 13.611	0.01 0.05 0.10 0.24
3" Dia 20' Omni	С	From Leg	3.000 6.000 -10.000	0.000	63.000	2" Ice No Ice 1/2" Ice 1" Ice 2" Ice	4.000 6.000 8.000 12.000	4.000 6.000 8.000 12.000	0.06 0.10 0.14 0.23

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		$C_A A_A$ Front	C _A A _A Side	Weight
			Vert ft ft ft	۰	ft		ft²	ft²	К
TMA	С	From Leg	3.000 0.000 0.000	0.000	63.000	No Ice 1/2" Ice 1" Ice 2" Ice	0.600 0.704 0.815 1.059	0.407 0.497 0.593 0.815	0.01 0.02 0.02 0.04
(2) 4.5' x 2" horizontal mount pipe	В	From Leg	1.000 0.000 0.000	0.000	58.000	No Ice 1/2" Ice 1" Ice 2" Ice	0.860 1.180 1.460 2.050	0.010 0.040 0.090 0.210	0.01 0.02 0.03 0.06
8'x2 1/2" Pipe Mount	В	From Leg	1.000 0.000 0.000	0.000	58.000	No Ice 1/2" Ice 1" Ice 2" Ice	2.300 3.132 3.620 4.620	2.300 3.132 3.620 4.620	0.04 0.06 0.08 0.14
DB212-1	В	From Leg	3.000 0.000 0.000	0.000	58.000	No Ice 1/2" Ice 1" Ice 2" Ice	4.400 8.418 12.452 20.570	4.400 8.418 12.452 20.570	0.03 0.07 0.13 0.34
Side Arm Mount [SO 306- 1]	В	From Leg	0.000 0.000 0.000	0.000	58.000	No Ice 1/2" Ice 1" Ice 2" Ice	0.410 0.810 1.230 2.080	2.260 3.830 5.480 9.370	0.04 0.06 0.09 0.19
Side Arm Mount [SO 306- 1]	Α	From Leg	0.000 0.000 0.000	0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	0.410 0.810 1.230 2.080	2.260 3.830 5.480 9.370	0.04 0.06 0.09 0.19
6' x 2" Mount Pipe	Α	From Leg	0.000 0.000 0.000	0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.425 1.925 2.294 3.060	1.425 1.925 2.294 3.060	0.02 0.03 0.05 0.09
DB212-1	Α	From Leg	4.000 0.000 0.000	0.000	54.000	No Ice 1/2" Ice 1" Ice 2" Ice	4.400 8.418 12.452 20.570	4.400 8.418 12.452 20.570	0.03 0.07 0.13 0.34
Side Arm Mount [SO 306- 1]	С	From Leg	0.000 0.000 0.000	0.000	43.000	No Ice 1/2" Ice 1" Ice 2" Ice	0.410 0.810 1.230 2.080	2.260 3.830 5.480 9.370	0.04 0.06 0.09 0.19
3" Dia. 6' Omni	С	From Leg	4.000 0.000 3.000	0.000	43.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.767 2.129 2.501 3.272	1.767 2.129 2.501 3.272	0.05 0.06 0.08 0.12
3" Dia. 6' Omni	С	From Leg	4.000 0.000 -3.000	0.000	43.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.767 2.129 2.501 3.272	1.767 2.129 2.501 3.272	0.05 0.06 0.08 0.12
Side Arm Mount [SO 306- 1]	В	From Leg	0.000 0.000 0.000	0.000	43.000	No Ice 1/2" Ice 1" Ice	0.410 0.810 1.230 2.080	2.260 3.830 5.480 9.370	0.04 0.06 0.09 0.19
DB230-2B	В	From Leg	4.000 0.000	0.000	43.000	2" Ice No Ice 1/2"	2.100 3.780	2.100 3.780	0.10 0.14

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		$C_A A_A$ Front	C _A A _A Side	Weight
			Vert ft ft ft	۰	ft		ft²	ft²	K
			0.000			Ice	5.460	5.460	0.17
						1" Ice 2" Ice	8.820	8.820	0.23
****Proposed***									
876F-70-2 4-Bay Dipole	С	From Face	0.000	0.000	130.000	No Ice	10.680	10.680	0.13
			0.000			1/2"	17.160	17.160	0.17
			10.000			Ice	23.640	23.640	0.21
						1" Ice 2" Ice	36.600	36.600	0.29
R5-LL [PM 602-1]	Α	From Leg	0.500	0.000	87.000	No Ice	5.250	1.580	0.09
		Ü	0.000			1/2"	6.500	1.950	0.12
			0.000			Ice	7.750	2.320	0.14
						1" Ice 2" Ice	10.250	3.060	0.19
***						Z 10 0			
**									

					Dishe	es					
Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter		Aperture Area	Weight
				ft	۰	۰	ft	ft		ft ²	K
PA6-59	A	Paraboloid w/Radome	From Leg	0.500 0.000 0.000	0.000		117.000	6.000	No Ice 1/2" Ice 1" Ice 2" Ice	28.300 29.050 29.801 31.302	0.19 0.33 0.48 0.78
6 FT Dish	В	Paraboloid w/Radome	From Leg	0.500 0.000 0.000	0.000		107.000	6.000	No Ice 1/2" Ice 1" Ice 2" Ice	28.300 29.050 29.801 31.302	0.19 0.33 0.48 0.78
8 FT Dish	Α	Paraboloid w/Radome	From Leg	0.500 0.000 0.000	0.000		96.000	8.000	No Ice 1/2" Ice 1" Ice 2" Ice	50.300 51.292 52.284 54.268	0.76 0.04 0.30 0.57 1.09
PAD8-59AW	С	Paraboloid w/Radome	From Leg	0.500 0.000 0.000	0.000		86.000	8.000	No Ice 1/2" Ice 1" Ice 2" Ice	50.300 51.292 52.284 54.268	0.04 0.30 0.57 1.09
4 FT Dish	Α	Paraboloid w/Radome	From Leg	0.500 0.000 0.000	0.000		71.000	4.000	No Ice 1/2" Ice 1" Ice 2" Ice	12.570 13.100 13.620 14.680	0.08 0.15 0.21 0.35
***** PAD6-W59BC	Α	Paraboloid w/Radome	From Leg	1.000 0.000 0.000	-34.500		87.000	6.583	No Ice 1/2" Ice 1" Ice 2" Ice	34.040 34.910 35.770 37.510	0.14 0.29 0.47 0.83

Load Combinations

Comb.	Description
No.	
- 1	Dood Only

1

Dead Only 1.2 Dead+1.0 Wind 0 deg - No Ice

Comb.	Description
No.	
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 300 deg+1.0 ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
42	Dead+Wind 120 deg - Service
43 44	Dead+Wind 150 deg - Service Dead+Wind 150 deg - Service
44 45	Dead+Wind 180 deg - Service Dead+Wind 180 deg - Service
45 46	
	Dead+Wind 210 deg - Service
47 48	Dead+Wind 240 deg - Service
	Dead+Wind 270 deg - Service
49 50	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Sectio n No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	130 - 120	Leg	Max Tension	23	2.43	-0.37	0.03
			Max. Compression	35	-12.20	-0.02	0.00
			Max. Mx	14	1.18	0.70	-0.03
			Max. My	8	-0.97	0.01	-0.76
			Max. Vy	22	-1.09	-0.37	0.03
			Max. Vx	25	1.10	0.00	0.39
		Diagonal	Max Tension	9	3.32	0.00	0.00
		· ·	Max. Compression	8	-3.39	0.00	0.00
			Max. Mx	26	-0.16	0.07	0.00
			Max. Vy	26	0.04	0.00	0.00
		Horizontal	Max Tension	22	2.48	-0.01	0.00
			Max. Compression	11	-2.43	0.00	0.00
			Max. Mx	37	-0.20	-0.05	-0.00

Sectio n No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
IVO.			Max. My	22	-1.10	-0.01	-0.01
			Max. Vy	37	0.05	-0.01	-0.01
			Max. Vx	22	0.00	0.00	0.00
		Top Girt	Max Tension	23	0.48	-0.00	0.00
			Max. Compression	10	-0.48	0.00	0.00
			Max. Mx	37	-0.15	-0.04	-0.00
			Max. My	18	0.25	-0.01	0.00
			Max. Vy	37	0.05	-0.04	-0.00
			Max. Vx	18	-0.00	-0.01	0.00
		Inner Bracing	Max Tension	3	0.00	0.00	0.00
			Max. Compression	22	-0.00	0.00	0.00
			Max. Mx	26	-0.00	-0.04	0.00
TO	100 100	1.00	Max. Vy	26 15	0.04	0.00	0.00
T2	120 - 100	Leg	Max Tension	15 31	15.89 -23.60	-0.22 0.06	-0.03
			Max. Compression Max. Mx	14	4.20	0.40	-0.02 0.02
			Max. My	19	1.35	-0.14	0.02
			Max. Vy	22	0.31	-0.08	0.08
			Max. Vx	18	-0.40	-0.04	0.13
		Diagonal	Max Tension	25	4.59	0.00	0.00
		9	Max. Compression	24	-4.71	0.00	0.00
			Max. Mx	26	-0.14	0.14	0.00
			Max. Vy	26	-0.07	0.00	0.00
		Horizontal	Max Tension	24	2.89	0.00	0.00
			Max. Compression	25	-2.87	0.00	0.00
			Max. Mx	33	-0.09	-0.09	-0.00
			Max. My	14	-0.32	-0.03	-0.01
			Max. Vy	33	0.07	-0.09	-0.00
			Max. Vx	2	0.00	0.00	0.00
		Inner Bracing	Max Tension	3	0.00	0.00	0.00
			Max. Compression	37	-0.01	0.00	0.00
			Max. Mx	26	-0.01	-0.06	0.00
Т3	100 - 80	Leg	Max. Vy Max Tension	26 15	0.05 34.00	0.00 -0.12	0.00 -0.06
13	100 - 60	Leg	Max. Compression	2	-42.56	0.40	-0.06 -0.16
			Max. Mx	14	20.49	0.49	0.04
			Max. My	5	-3.44	-0.03	-0.61
			Max. Vy	6	-0.43	-0.19	-0.02
			Max. Vx	11	-0.50	-0.09	-0.19
		Diagonal	Max Tension	13	6.12	0.00	0.00
		•	Max. Compression	12	-6.27	0.00	0.00
			Max. Mx	26	-0.21	0.18	0.00
			Max. Vy	26	0.08	0.00	0.00
		Horizontal	Max Tension	12	4.26	-0.02	-0.00
			Max. Compression	13	-4.24	-0.02	-0.00
			Max. Mx	33	-0.00	-0.11	-0.00
			Max. My	14	-0.36	-0.04	-0.01
			Max. Vy	33 14	-0.08	-0.11	-0.00
		Innor Bracing	Max. Vx Max Tension	11	-0.00	-0.04	-0.01
		Inner Bracing	Max. Compression	33	0.00 -0.01	0.00 0.00	0.00 0.00
			Max. Mx	26	-0.01	-0.08	0.00
			Max. Vy	26	0.06	0.00	0.00
T4	80 - 60	Leg	Max Tension	15	51.78	-0.45	-0.18
		3	Max. Compression	10	-62.09	0.44	-0.25
			Max. Mx	22	36.85	-0.53	0.17
			Max. My	12	-7.46	-0.06	-0.68
			Max. Vý	29	0.26	-0.42	-0.04
			Max. Vx	4	-0.41	-0.04	-0.61
		Diagonal	Max Tension	13	8.26	0.00	0.00
			Max. Compression	12	-8.46	0.00	0.00
			Max. Mx	26	-0.00	0.30	0.00
		11. 1	Max. Vy	26	-0.10	0.00	0.00
		Horizontal	Max Tension	12	5.00	-0.03	-0.00
			Max. Compression	13	-4.95	-0.02	-0.00
			Max. Mx	33	-0.32	-0.14	-0.00
			Max. My Max. Vy	14 33	-0.63 0.09	-0.05 -0.14	-0.01 -0.00
			Max. Vx	33 14	0.09	-0.14 -0.05	-0.00 -0.01
			IVIGA. VA	17	0.00	-0.00	-0.01

Sectio n	Elevation ft	Component Type	Condition	Gov. Load	Axial	Major Axis Moment	Minor Axis Moment
No.				Comb.	K	kip-ft	kip-ft
		Inner Bracing	Max Tension	3	0.00	0.00	0.00
			Max. Compression	37	-0.01	0.00	0.00
			Max. Mx	26	-0.01	-0.11	0.00
T C	00 40	1	Max. Vy	26	-0.06	0.00	0.00
T5	60 - 40	Leg	Max Tension	15	72.46	-0.35	0.08
			Max. Compression	10	-85.40	0.34	-0.16
			Max. Mx	6	54.95	-0.52	-0.06
			Max. My	12	-7.92 0.40	-0.06	-0.68
			Max. Vy	37	-0.19	-0.26	0.12
		Diagonal	Max. Vx Max Tension	4 13	-0.24 7.63	0.00 0.00	-0.41 0.00
		Diagonal		12	-7.88		0.00
			Max. Compression Max. Mx	26	0.03	0.00 0.39	0.00
			Max. Vy	26	0.03	0.00	0.00
		Horizontal	Max Tension	12	5.11	-0.04	-0.00
		Honzontal	Max. Compression	13	- 5.04	-0.04	-0.00
			Max. Mx	33	-0.24	-0.03 -0.17	-0.00
			Max. My	2	0.70	-0.17	0.01
			Max. Vy	33	0.70	-0.02	-0.00
			Max. Vx	2	0.10	0.00	0.00
		Inner Presing	Max Tension	1	0.00	0.00	0.00
		Inner Bracing	Max. Compression	37			
			'		-0.02	0.00	0.00
			Max. Mx Max. Vy	26 26	-0.02 -0.09	-0.18 0.00	0.00 0.00
T6	40 - 20	Log	Max Tension	15		-0.42	0.00
10	40 - 20	Leg	Max. Compression	10	91.77 -107.07	0.28	-0.07
			Max. Mx	14		-0.42	0.07
				12	80.09 -9.92	-0.42	-0.53
			Max. My	6			
			Max. Vy	22	-0.09	-0.41	-0.07
		Diagonal	Max. Vx		0.12	0.13	0.50
		Diagonal	Max Tension	9 8	7.70 -8.05	0.00 0.00	0.00 0.00
			Max. Compression Max. Mx	26	-0.03	0.46	0.00
			Max. Vy	26	-0.02 -0.13	0.46	0.00
		Horizontal	Max Tension	8	5.61	0.00	0.00
		Honzontal	Max. Compression	9	-5.46	0.00	0.00
			Max. Mx	33	-0.10	-0.27	-0.00
			Max. My	14	-0.17	-0.10	-0.01
			Max. Vy	33	0.13	-0.10	-0.00
			Max. Vx	14	0.00	0.00	0.00
		Inner Bracing	Max Tension	1	0.00	0.00	0.00
		minor Brasing	Max. Compression	37	-0.02	0.00	0.00
			Max. Mx	26	-0.02	-0.25	0.00
			Max. Vy	26	0.11	0.00	0.00
T7	20 - 0	Leg	Max Tension	15	109.93	-0.54	-0.00
		3	Max. Compression	2	-128.00	0.00	-0.00
			Max. Mx	27	-76.15	0.69	-0.00
			Max. My	12	-12.52	-0.04	-0.55
			Max. Vý	6	-0.13	-0.53	-0.07
			Max. Vx	10	-0.14	-0.29	-0.49
		Diagonal	Max Tension	9	7.78	0.00	0.00
			Max. Compression	8	-8.22	0.00	0.00
			Max. Mx	26	-0.08	0.50	0.00
			Max. Vy	26	-0.13	0.00	0.00
		Horizontal	Max Tension	8	6.04	0.00	0.00
			Max. Compression	11	-5.85	0.00	0.00
			Max. Mx	33	0.29	-0.27	-0.00
			Max. My	14	-0.12	-0.12	-0.01
			Max. Vy	33	0.13	-0.27	-0.00
			Max. Vx	14	-0.00	0.00	0.00
		Inner Bracing	Max Tension	1	0.00	0.00	0.00
		IIIIIGI DIAGIIIU					
		miler bracing		37	-0.02	0.00	0.00
		milei bracing	Max. Compression Max. Mx	37 26		0.00 -0.35	0.00 0.00

Mavimum	Reactions
IVIAXIIIIUIII	Reactions

Location	Condition	Gov. Load	Vertical K	Horizontal, X K	Horizontal, Z K
		Comb.	105.00	40.00	
Leg C	Max. Vert	18	135.68	16.32	-8.62
	Max. H _x	18	135.68	16.32	-8.62
	Max. H _z	5	- 95.84	-11.97	8.13
	Min. Vert	7	-106.97	-14.11	7.37
	Min. H _x	7	-106.97	-14.11	7.37
	Min. H _z	16	123.88	14.12	-9.32
Leg B	Max. Vert	10	137.22	-16.64	-8.77
Ü	Max. H _x	23	-108.24	14.20	7.38
	Max. H₂	25	-98.16	12.09	8.42
	Min. Vert	23	-108.24	14.20	7.38
	Min. H _x	10	137.22	-16.64	-8.77
	Min. H _z	12	125.37	-14.34	-9.68
Leg A	Max. Vert	2	137.76	0.06	19.28
Ü	Max. H _x	21	6.51	2.16	0.64
	Max. H _z	2	137.76	0.06	19.28
	Min. Vert	15	-118.35	-0.11	-17.31
	Min. H _x	8	9.59	-2.26	0.94
	Min. H _z	15	-118.35	-0.11	-17.31

Tower Mast Reaction Summary

Load Combination	Vertical	Shear _x	Shearz	Overturning Moment, M _x	Overturning Moment, M _z	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Dead Only	32.79	0.00	0.00	25.61	9.16	0.00
1.2 Dead+1.0 Wind 0 deg - No Ice	39.35	0.08	-32.03	-2433.00	4.75	2.81
0.9 Dead+1.0 Wind 0 deg -	29.51	0.08	-32.03	-2440.68	2.01	2.81
No Ice						
1.2 Dead+1.0 Wind 30 deg -	39.35	15.62	-27.30	-2064.53	-1182.90	21.96
No Ice	00.54	45.00	07.00	0070.00	4405.05	04.00
0.9 Dead+1.0 Wind 30 deg - No Ice	29.51	15.62	-27.30	-2072.22	-1185.65	21.96
1.2 Dead+1.0 Wind 60 deg - No Ice	39.35	25.84	-14.77	-1109.99	-1984.87	26.22
0.9 Dead+1.0 Wind 60 deg -	29.51	25.84	-14.77	-1117.67	-1987.61	26.22
No Ice						
1.2 Dead+1.0 Wind 90 deg - No Ice	39.35	28.48	0.40	68.75	-2232.56	22.94
0.9 Dead+1.0 Wind 90 deg -	29.51	28.48	0.40	61.07	-2235.31	22.94
No Ice						
1.2 Dead+1.0 Wind 120 deg	39.35	26.80	15.69	1256.37	-2071.96	27.69
- No Ice	00.54	00.00	45.00	40.40.00	0074.74	07.00
0.9 Dead+1.0 Wind 120 deg - No Ice	29.51	26.80	15.69	1248.69	-2074.71	27.69
1.2 Dead+1.0 Wind 150 deg - No Ice	39.35	16.43	28.32	2211.97	-1253.07	16.10
0.9 Dead+1.0 Wind 150 deg	29.51	16.43	28.32	2204.29	-1255.82	16.10
- No Ice						
1.2 Dead+1.0 Wind 180 deg - No Ice	39.35	0.02	32.19	2509.85	11.85	-3.76
0.9 Dead+1.0 Wind 180 deg	29.51	0.02	32.19	2502.17	9.10	-3.76
- No Ice 1.2 Dead+1.0 Wind 210 deg	39.35	-15.95	27.77	2172.86	1242.01	-23.28
- No Ice						
0.9 Dead+1.0 Wind 210 deg - No Ice	29.51	-15.95	27.77	2165.18	1239.27	-23.28
1.2 Dead+1.0 Wind 240 deg	39.35	-26.13	15.54	1246.90	2042.63	-27.50
- No Ice 0.9 Dead+1.0 Wind 240 deg	29.51	-26.13	15.54	1239.21	2039.88	-27.50
- No Ice	29.31	-20.13				
1.2 Dead+1.0 Wind 270 deg	39.35	-28.04	0.42	72.73	2221.16	-24.37

Load Combination	Vertical	Shear _x	Shearz	Overturning Moment, M _x	Overturning Moment, Mz	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
- No Ice 0.9 Dead+1.0 Wind 270 deg	29.51	-28.04	0.42	65.05	2218.41	-24.37
- No Ice 1.2 Dead+1.0 Wind 300 deg - No Ice	39.35	-25.88	-14.94	-1122.84	2011.58	-28.42
0.9 Dead+1.0 Wind 300 deg - No Ice	29.51	-25.88	-14.94	-1130.53	2008.83	-28.42
1.2 Dead+1.0 Wind 330 deg - No Ice	39.35	-15.75	-27.92	-2110.74	1213.99	-16.77
0.9 Dead+1.0 Wind 330 deg - No Ice	29.51	-15.75	-27.92	-2118.43	1211.24	-16.77
1.2 Dead+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 0	153.69 153.69	0.00 0.03	0.00 -12.19	201.03 -729.42	-21.09 -23.08	-0.00 5.32
deg+1.0 lce+1.0 Temp 1.2 Dead+1.0 Wind 30	153.69	5.86	-10.24	-584.59	-468.70	14.30
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	153.69	9.65	-5.59	-232.09	-766.72	17.20
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	153.69	11.01	0.05	206.26	-877.86	17.46
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	153.69	9.88	5.77	649.82	-785.97	15.34
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	153.69	5.99	10.44	1002.60	-480.06	7.03
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	153.69	-0.01	12.22	1134.09	-19.90	-5.48
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	153.69	-5.91	10.32	994.44	432.70	-14.51
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	153.69	-9.69	5.72	646.57	730.50	-17.41
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	153.69	-10.94	0.08	208.94	830.35	-17.69
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 330	153.69 153.69	-9.73 -5.88	-5.65 -10.37	-235.94 -593.99	730.41 427.97	-15.46 -7.14
deg+1.0 lce+1.0 Temp Dead+Wind 0 deg - Service	32.79	-5.00	-10.37 -6.92	-503.03	7.83	0.60
Dead+Wind 30 deg - Service	32.79	3.38	-5.90	-424.03	-247.07	4.68
Dead+Wind 60 deg - Service	32.79	5.59	-3.20	-219.30	-419.30	5.59
Dead+Wind 90 deg - Service	32.79	6.16	0.08	33.71	-472.58	4.89
Dead+Wind 120 deg - Service	32.79	5.79	3.39	288.60	-437.86	5.90
Dead+Wind 150 deg - Service	32.79	3.55	6.12	493.56	-262.02	3.43
Dead+Wind 180 deg - Service	32.79	0.00	6.95	557.52	9.34	-0.80
Dead+Wind 210 deg - Service	32.79	-3.44	6.00	485.23	273.30	-4.96
Dead+Wind 240 deg - Service	32.79	-5.65	3.36	286.58	445.24	-5.86
Dead+Wind 270 deg - Service	32.79	-6.07	0.09	34.56	483.78	-5.19
Dead+Wind 300 deg - Service	32.79	-5.60	-3.23	-222.03	438.62	-6.05
Dead+Wind 330 deg - Service	32.79	-3.40	-6.03	-433.87	267.33	-3.57

Solution Summary

	Sur	n of Applied Force	es				
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
1	0.00	-32.79	0.00	0.00	32.79	0.00	0.000%
2	0.08	-39.35	-32.03	-0.08	39.35	32.03	0.000%
3	0.08	-29.51	-32.03	-0.08	29.51	32.03	0.000%
4	15.62	-39.35	-27.30	-15.62	39.35	27.30	0.000%

	Sur	n of Applied Force	es		Sum of Reaction	ns	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
5	15.62	-29.51	-27.30	-15.62	29.51	27.30	0.000%
6	25.84	-39.35	-14.77	-25.84	39.35	14.77	0.000%
7	25.84	-29.51	-14.77	-25.84	29.51	14.77	0.000%
8	28.48	-39.35	0.40	-28.48	39.35	-0.40	0.000%
9	28.48	-29.51	0.40	-28.48	29.51	-0.40	0.000%
10	26.80	-39.35	15.69	-26.80	39.35	-15.69	0.000%
11	26.80	-29.51	15.69	-26.80	29.51	-15.69	0.000%
12	16.43	-39.35	28.32	-16.43	39.35	-28.32	0.000%
13	16.43	-29.51	28.32	-16.43	29.51	-28.32	0.000%
14	0.02	-39.35	32.19	-0.02	39.35	-32.19	0.000%
15	0.02	-29.51	32.19	-0.02	29.51	-32.19	0.000%
16	-15.95	-39.35	27.77	15.95	39.35	-27.77	0.000%
17	-15.95	-29.51	27.77	15.95	29.51	-27.77	0.000%
18	-26.13	-39.35	15.54	26.13	39.35	-15.54	0.000%
19	-26.13	-29.51	15.54	26.13	29.51	-15.54	0.000%
20	-28.04	-39.35	0.42	28.04	39.35	-0.42	0.000%
21	-28.04	-29.51	0.42	28.04	29.51	-0.42	0.000%
22	-25.88	-39.35	-14.94	25.88	39.35	14.94	0.000%
23	-25.88	-29.51	-14.94	25.88	29.51	14.94	0.000%
24	-15.75	-39.35	-27.92	15.75	39.35	27.92	0.000%
25	-15.75	-29.51	-27.92	15.75	29.51	27.92	0.000%
26	0.00	-153.69	0.00	0.00	153.69	0.00	0.000%
27	0.03	-153.69	-12.19	-0.03	153.69	12.19	0.000%
28	5.86	-153.69	-10.24	-5.86	153.69	10.24	0.000%
29	9.65	-153.69	-5.59	-9.65	153.69	5.59	0.000%
30	11.01	-153.69	0.05	-11.01	153.69	-0.05	0.000%
31	9.88	-153.69	5.77	-9.88	153.69	-5.77	0.000%
32	5.99	-153.69	10.44	-5.99	153.69	-10.44	0.000%
33	-0.01	-153.69	12.22	0.01	153.69	-12.22	0.000%
34	- 5.91	-153.69	10.32	5.91	153.69	-10.32	0.000%
35	-9.69	-153.69	5.72	9.69	153.69	-5.72	0.000%
36	-10.94	-153.69	0.08	10.94	153.69	-0.08	0.000%
37	-9.73	-153.69	-5.65	9.73	153.69	5.65	0.000%
38	-5.88	-153.69	-10.37	5.88	153.69	10.37	0.000%
39	0.02	-32.79	-6.92	-0.02	32.79	6.92	0.000%
40	3.38	-32.79	-5.90	-3.38	32.79	5.90	0.000%
41	5.59	-32.79	-3.20	-5.59	32.79	3.20	0.000%
42	6.16	-32.79	0.08	-6.16	32.79	-0.08	0.000%
43	5.79	-32.79	3.39	-5.79	32.79	-3.39	0.000%
44	3.55	-32.79	6.12	-3.55	32.79	-6.12	0.000%
45	0.00	-32.79	6.95	-0.00	32.79	-6.95	0.000%
46	-3.44	-32.79	6.00	3.44	32.79	-6.00	0.000%
47	-5.65	-32.79	3.36	5.65	32.79	-3.36	0.000%
48	-6.07	-32.79	0.09	6.07	32.79	-0.09	0.000%
49	-5.60	-32.79	-3.23	5.60	32.79	3.23	0.000%
50	-3.40	-32.79	-6.03	3.40	32.79	6.03	0.000%

Maximum Tower Deflections - Service Wind

Section No.	Elevation	Horz. Deflection	Gov. Load	Tilt	Twist
	ft	in	Comb.	•	0
T1	130 - 120	1.009	44	0.066	0.007
T2	120 - 100	0.869	44	0.065	0.007
T3	100 - 80	0.606	44	0.057	0.007
T4	80 - 60	0.385	44	0.045	0.006
T5	60 - 40	0.219	44	0.032	0.005
T6	40 - 20	0.100	44	0.021	0.004
T7	20 - 0	0.030	44	0.010	0.002

Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	٥	٥	ft
130.000	876F-70-2 4-Bay Dipole	44	1.009	0.066	0.007	315602
127.000	Sector Mount [SM 501-3]	44	0.967	0.066	0.007	315602
125.000	Sector Mount [SM 502-3]	44	0.939	0.066	0.008	315602
117.000	PA6-59	44	0.828	0.064	0.007	150659
113.000	Side Arm Mount [SO 306-1]	44	0.774	0.063	0.007	140781
107.000	6 FT Dish	44	0.695	0.060	0.007	128868
104.000	Side Arm Mount [SO 306-1]	44	0.656	0.059	0.007	121816
98.000	Side Arm Mount [SO 306-1]	44	0.582	0.055	0.007	106652
96.000	8 FT Dish	44	0.558	0.054	0.007	102394
87.000	PAD6-W59BC	44	0.456	0.049	0.006	86955
86.000	PAD8-59AW	44	0.446	0.048	0.006	85388
84.000	PD1142-1	44	0.425	0.047	0.006	82387
77.000	Sector Mount [SM 402-1]	44	0.356	0.043	0.006	80791
71.000	4 FT Dish	44	0.303	0.039	0.006	89281
63.000	Sector Mount [SM 402-1]	44	0.240	0.034	0.005	103901
58.000	(2) 4.5' x 2" horizontal mount pipe	44	0.205	0.031	0.005	108626
54.000	Side Arm Mount [SO 306-1]	44	0.178	0.028	0.005	106270
43.000	Side Arm Mount [SO 306-1]	44	0.115	0.022	0.004	98935

Maximum Tower Deflections - Design Wind

Section No.	Elevation	Horz. Deflection	Gov. Load	Tilt	Twist
	ft	in	Comb.	0	۰
T1	130 - 120	4.631	12	0.302	0.035
T2	120 - 100	3.984	12	0.297	0.035
T3	100 - 80	2.774	12	0.259	0.032
T4	80 - 60	1.759	12	0.203	0.029
T5	60 - 40	1.002	12	0.145	0.024
T6	40 - 20	0.460	12	0.094	0.017
T7	20 - 0	0.138	13	0.043	0.009

Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	۰	ft
130.000	876F-70-2 4-Bay Dipole	12	4.631	0.302	0.035	80042
127.000	Sector Mount [SM 501-3]	12	4.436	0.301	0.035	80042
125.000	Sector Mount [SM 502-3]	12	4.306	0.300	0.035	80042
117.000	PA6-59	12	3.794	0.294	0.035	36828
113.000	Side Arm Mount [SO 306-1]	12	3.545	0.288	0.034	33049
107.000	6 FT Dish	12	3.181	0.276	0.033	28641
104.000	Side Arm Mount [SO 306-1]	12	3.004	0.269	0.033	26597
98.000	Side Arm Mount [SO 306-1]	12	2.663	0.254	0.032	23167
96.000	8 FT Dish	12	2.553	0.249	0.032	22224
87.000	PAD6-W59BC	12	2.086	0.224	0.030	18734
86.000	PAD8-59AW	12	2.038	0.221	0.030	18390
84.000	PD1142-1	12	1.942	0.215	0.030	17743
77.000	Sector Mount [SM 402-1]	12	1.629	0.194	0.029	17449
71.000	4 FT Dish	12	1.388	0.176	0.028	19414
63.000	Sector Mount [SM 402-1]	12	1.101	0.153	0.025	22882
58.000	(2) 4.5' x 2" horizontal mount	12	0.938	0.139	0.024	24079
	pipe					
54.000	Side Arm Mount [SO 306-1]	12	0.817	0.129	0.023	23511
43.000	Side Arm Mount [SO 306-1]	12	0.528	0.102	0.018	21780

Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of	Maximum Load	Allowable Load	Ratio Load	Allowable Ratio	Criteria
	ft			in	Bolts	per Bolt K	per Bolt K	Allowable	•	
T1	130	Leg	A325N	0.750	4	1.02	30.10	0.034	1.05	Bolt Tension
		Diagonal	A325N	0.625	3	1.13	13.81	0.082	1.05	Bolt Shear
		Horizontal	A325N	0.625	2	1.24	13.81	0.090	1.05	Bolt Shear
T2	120	Leg	A325N	0.875	4	3.97	41.56	0.096	1.05	Bolt Tension
		Diagonal	A325N	0.625	3	1.57	13.81	0.114	1.05	Bolt Shear
		Horizontal	A325N	0.625	2	1.45	13.81	0.105	1.05	Bolt Shear
T3	100	Leg	A325N	1.000	4	8.50	54.52	0.156	1.05	Bolt Tension
		Diagonal	A325N	0.625	3	2.09	13.81	0.151	1.05	Bolt Shear
		Horizontal	A325N	0.625	2	2.13	13.81	0.154	1.05	Bolt Shear
T4	80	Leg	A325N	1.000	4	12.94	54.52	0.237	1.05	Bolt Tension
		Diagonal	A325N	0.625	3	2.82	13.81	0.204	1.05	Bolt Shear
		Horizontal	A325N	0.625	2	2.50	13.81	0.181	1.05	Bolt Shear
T5	60	Leg	A325N	1.000	6	12.08	54.52	0.222	1.05	Bolt Tension
		Diagonal	A325N	0.625	3	2.63	13.81	0.190	1.05	Bolt Shear
		Horizontal	A325N	0.625	2	2.56	13.81	0.185	1.05	Bolt Shear
T6	40	Leg	A325N	1.000	6	15.29	54.52	0.281	1.05	Bolt Tension
		Diagonal	A325N	0.625	3	2.68	13.81	0.194	1.05	Bolt Shear
		Horizontal	A325N	0.625	2	2.80	13.81	0.203	1.05	Bolt Shear
T7	20	Diagonal	A325N	0.625	3	2.74	13.81	0.198	1.05	Bolt Shear
		Horizontal	A325N	0.625	2	3.02	13.81	0.219	1.05	Bolt Shear

Compression Checks

Leg Design	Data ((Compression)
33		(

Section No.	Elevation	Size	L	Lu	KI/r	Α	P_u	ϕP_n	Ratio P _u
	ft		ft	ft		in²	K	K	φ P _n
T1	130 - 120	ROHN 2.5 STD	10.000	5.000	63.3 K=1.00	1.704	-12.20	57.19	0.213 1
T2	120 - 100	ROHN 3 STD	20.036	6.679	68.9 K=1.00	2.228	-23.60	70.89	0.333 1
Т3	100 - 80	ROHN 4 STD	20.036	6.679	53.1 K=1.00	3.174	-42.56	116.23	0.366 ¹
T4	80 - 60	ROHN 5 STD	20.042	10.021	64.0 K=1.00	4.300	-62.09	143.37	0.433 1
T5	60 - 40	ROHN 5 EH	20.055	10.028	65.4 K=1.00	6.112	-85.40	201.11	0.425 1
T6	40 - 20	ROHN 6 EHS	20.052	10.026	54.1 K=1.00	6.713	-107.07	243.97	0.439 1
T7	20 - 0	ROHN 6 EH	20.052	10.026	54.8 K=1.00	8.405	-128.00	303.62	0.422 1

¹ P_u / ϕP_n controls

Diagonal Design Data (Compression)
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Section No.	Elevation	Size	L	Lu	KI/r	Α	Pu	φP _n	Ratio P.,
	ft		ft	ft		in ²	K	K	ΦP_n
T1	130 - 120	ROHN 2 STD	6 575	6.391	97.4	1.075	-3 39	24 16	0.140 1

Section No.	Elevation	Size	L	L_u	KI/r	Α	P_u	ϕP_n	Ratio P _u
	ft		ft	ft		in²	K	K	ϕP_n
					K=1.00				
T2	120 - 100	ROHN 2.5 STD	8.528	8.295	105.1 K=1.00	1.704	-4.71	34.21	0.138 ¹
Т3	100 - 80	ROHN 2.5 STD	9.213	8.941	113.2 K=1.00	1.704	-6.27	30.02	0.209 ¹
Т4	80 - 60	ROHN 2.5 X-STR	12.492	12.106	157.2 K=1.00	2.254	-8.46	20.60	0.411 ¹
T5	60 - 40	ROHN 3 STD	13.306	12.955	133.6 K=1.00	2.228	-7.88	28.20	0.279 ¹
Т6	40 - 20	ROHN 3 STD	14.161	13.771	142.0 K=1.00	2.228	-8.05	24.96	0.322 1
Т7	20 - 0	ROHN 3 STD	15.071	14.702	151.6 K=1.00	2.228	-8.22	21.90	0.375 ¹

¹ P_u / ϕP_n controls

	Horizontal Design Data (Compression)										
Section No.	Elevation	Size	L	Lu	KI/r	Α	P_u	ϕP_n	Ratio Pu		
	ft		ft	ft		in²	K	K	ϕP_n		
T1	130 - 120	ROHN 1.5 STD	8.520	4.140	79.8 K=1.00	0.799	-2.43	22.58	0.107 1		
T2	120 - 100	ROHN 2 STD	9.933	4.821	73.5 K=1.00	1.075	-2.87	32.58	0.088 1		
Т3	100 - 80	ROHN 2 STD	12.017	5.821	88.7 K=1.00	1.075	-4.24	27.19	0.156 ¹		
T4	80 - 60	ROHN 2 STD	13.835	6.686	101.9 K=1.00	1.075	-4.95	22.62	0.219 ¹		
T5	60 - 40	ROHN 2 STD	16.250	7.893	120.3 K=1.00	1.075	-5.04	16.76	0.301 1		
T6	40 - 20	ROHN 2.5 STD	18.790	9.119	115.5 K=1.00	1.704	-5.46	28.86	0.189 ¹		
T7	20 - 0	ROHN 2.5 STD	21.290	10.369	131.3 K=1.00	1.704	-5.85	22.32	0.262 1		

¹ P_u / ϕP_n controls

		Top Girt	Desig	n Dat	a (Cor	npres	sion)		
Section No.	Elevation	Size	L	Lu	KI/r	Α	P_u	φ P _n	Ratio P _u
	ft		ft	ft		in²	K	K	$\frac{u}{\phi P_n}$
T1	130 - 120	ROHN 1.5 STD	8.500	4.130	79.6 K=1.00	0.799	-0.48	22.63	0.021 1

¹ P_u / ϕP_n controls

		Inner Brad	cing De	sign l	Data (Comp	ression	1)	
Section No.	Elevation	Size	L	Lu	KI/r	Α	Pu	φP _n	Ratio P _u
	ft		ft	ft		in ²	K	K	${\Phi P_n}$
T1	130 - 120	L2x2x1/8	4.260	4.260	128.6 K=1.00	0.484	-0.00	8.38	0.000 1

Section No.	Elevation	Size	L	Lu	KI/r	Α	P_u	ϕP_n	Ratio P _u
	ft		ft	ft		in²	K	K	ΦP_n
T2	120 - 100	L2x2x1/8	4.967	4.967	149.9 K=1.00	0.484	-0.01	6.17	0.001 1
Т3	100 - 80	L2x2x1/8	6.008	6.008	181.4 K=1.00	0.484	-0.01	4.21	0.002 1
T4	80 - 60	L2x2x1/8	6.918	6.918	208.8 K=1.00	0.484	-0.01	3.18	0.004 1
T5	60 - 40	L2 1/2x2 1/2x3/16	8.125	8.125	197.0 K=1.00	0.902	-0.02	6.65	0.002 1
T6	40 - 20	L 3x3x3/16	9.395	9.395	189.1 K=1.00	1.090	-0.02	8.73	0.002 1
T7	20 - 0	L3 1/2x3 /12x1/4	10.645	10.645	184.2 K=1.00	1.688	-0.02	14.24	0.001*1

^{*} DL controls

Tension Checks

	Leg Design Data (Tension)									
Section No.	Elevation	Size	L	Lu	KI/r	Α	Pu	φP _n	Ratio P _u	
	ft		ft	ft		in ²	K	K	ϕP_n	
T1	130 - 120	ROHN 2.5 STD	10.000	5.000	63.3	1.704	1.82	76.68	0.024 1	
T2	120 - 100	ROHN 3 STD	20.036	6.679	68.9	2.228	15.89	100.28	0.158 ¹	
T3	100 - 80	ROHN 4 STD	20.036	6.679	53.1	3.174	34.00	142.83	0.238 1	
T4	80 - 60	ROHN 5 STD	20.042	10.021	64.0	4.300	51.78	193.49	0.268 1	
T5	60 - 40	ROHN 5 EH	20.055	10.028	65.4	6.112	72.46	275.04	0.263 ¹	
T6	40 - 20	ROHN 6 EHS	20.052	10.026	54.1	6.713	91.77	302.10	0.304 1	
T7	20 - 0	ROHN 6 EH	20.052	10.026	54.8	8.405	109.93	378.22	0.291 1	

¹ P_u / ϕP_n controls

Diagonal Design Data (Tension)

Section No.	Elevation	Size	L	Lu	KI/r	Α	P_u	ϕP_n	Ratio P _u
	ft		ft	ft		in²	K	K	ΦP_n
T1	130 - 120	ROHN 2 STD	6.575	6.391	97.4	1.075	3.32	48.35	0.069 1
T2	120 - 100	ROHN 2.5 STD	8.528	8.295	105.1	1.704	4.59	76.68	0.060 ¹
T3	100 - 80	ROHN 2.5 STD	9.213	8.941	113.2	1.704	6.12	76.68	0.080 1
T4	80 - 60	ROHN 2.5 X-STR	12.492	12.106	157.2	2.254	8.26	101.41	0.081 ¹
T5	60 - 40	ROHN 3 STD	13.306	12.955	133.6	2.228	7.63	100.28	0.076 ¹
T6	40 - 20	ROHN 3 STD	14.161	13.771	142.0	2.228	7.70	100.28	0.077 1
T7	20 - 0	ROHN 3 STD	15.071	14.702	151.6	2.228	7.78	100.28	0.078 1

¹ P_u / ϕP_n controls

Horizontal Design Data (Tension)

¹ P_u / ϕP_n controls

Section	Elevation	Size	L	Lu	KI/r	Α	P_u	ϕP_n	Ratio
No.									P_u
	ft		ft	ft		in²	K	K	ϕP_n
T1	130 - 120	ROHN 1.5 STD	8.520	4.140	79.8	0.799	2.48	35.98	0.069 ¹
T2	120 - 100	ROHN 2 STD	9.933	4.821	73.5	1.075	2.89	48.35	0.060 ¹
T3	100 - 80	ROHN 2 STD	12.017	5.821	88.7	1.075	4.26	48.35	0.088 1
T4	80 - 60	ROHN 2 STD	13.835	6.686	101.9	1.075	5.00	48.35	0.103 ¹
T5	60 - 40	ROHN 2 STD	16.250	7.893	120.3	1.075	5.11	48.35	0.106 ¹
T6	40 - 20	ROHN 2.5 STD	18.790	9.119	115.5	1.704	5.61	76.68	0.073 1
T7	20 - 0	ROHN 2.5 STD	21.290	10.369	131.3	1.704	6.04	76.68	0.079 ¹

¹ P_u / ϕP_n controls

Top Girt Design Data (Tension)									
Section No.	Elevation	Size	L	Lu	Kl/r	А	Pu	φ P _n	Ratio P.,
710.	ft		ft	ft		in ²	K	K	$\frac{-}{\phi P_n}$
T1	130 - 120	ROHN 1.5 STD	8.500	4.130	79.6	0.799	0.48	35.98	0.013 1

 $^{^{1}}$ P $_{u}$ / ϕP_{n} controls

Inner Bracing Design Data (Tension)									
Section No.	Elevation	Size	L	Lu	KI/r	Α	P_u	ϕP_n	Ratio Pu
	ft		ft	ft		in ²	K	K	ϕP_n
T1	130 - 120	L2x2x1/8	4.260	4.260	81.6	0.484	0.00	15.69	0.000 1
T2	120 - 100	L2x2x1/8	4.270	4.270	81.8	0.484	0.00	15.69	0.000 1
T3	100 - 80	L2x2x1/8	6.008	6.008	115.1	0.484	0.00	15.69	0.000 1
T4	80 - 60	L2x2x1/8	6.355	6.355	121.8	0.484	0.00	15.69	0.000

 $^{^{1}}$ P $_{u}$ / ϕP_{n} controls

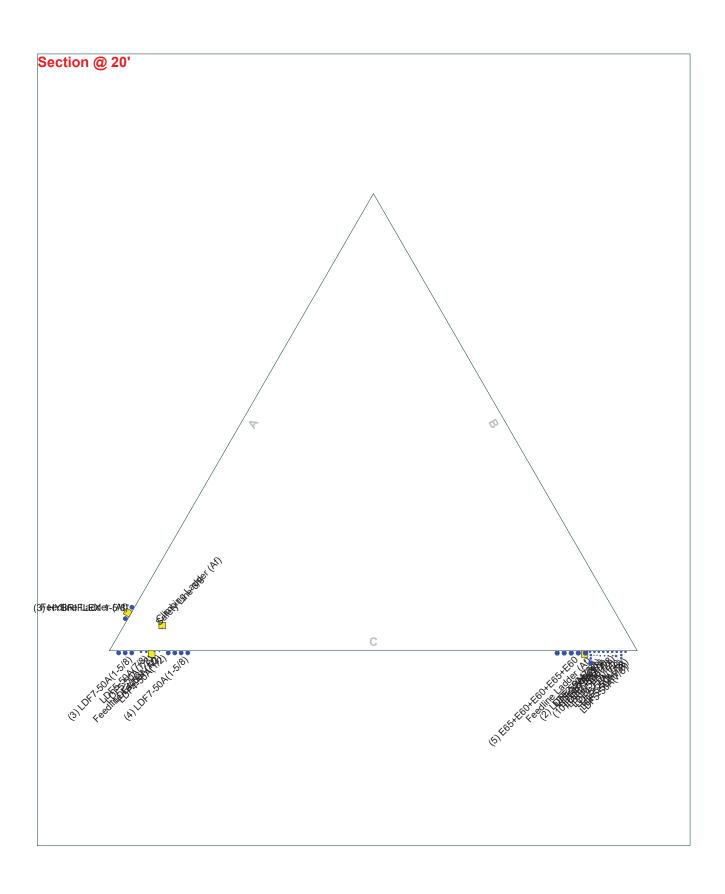
Section Capacity Table

Section	Elevation	Component	Size	Critical	Р	øP _{allow}	%	Pass
No.	ft	Type		Element	K	K	Capacity	Fail
T1	130 - 120	Leg	ROHN 2.5 STD	1	-12.20	60.05	20.3	Pass
T2	120 - 100	Leg	ROHN 3 STD	29	-23.60	74.43	31.7	Pass
T3	100 - 80	Leg	ROHN 4 STD	69	- 42.56	122.04	34.9	Pass
T4	80 - 60	Leg	ROHN 5 STD	107	-62.09	150.53	41.2	Pass
T5	60 - 40	Leg	ROHN 5 EH	134	-85.40	211.17	40.4	Pass
T6	40 - 20	Leg	ROHN 6 EHS	161	-107.07	256.16	41.8	Pass
T7	20 - 0	Leg	ROHN 6 EH	189	-128.00	318.80	40.2	Pass
T1	130 - 120	Diagonal	ROHN 2 STD	9	-3.39	25.36	13.4	Pass
T2	120 - 100	Diagonal	ROHN 2.5 STD	36	-4.71	35.92	13.1	Pass
Т3	100 - 80	Diagonal	ROHN 2.5 STD	74	-6.27	31.52	19.9	Pass
T4	80 - 60	Diagonal	ROHN 2.5 X-STR	113	-8.46	21.63	39.1	Pass
T5	60 - 40	Diagonal	ROHN 3 STD	140	-7.88	29.61	26.6	Pass
T6	40 - 20	Diagonal	ROHN 3 STD	165	-8.05	26.21	30.7	Pass
T7	20 - 0	Diagonal	ROHN 3 STD	192	-8.22	22.99	35.7	Pass
T1	130 - 120	Horizontal	ROHN 1.5 STD	7	-2.43	23.71	10.2	Pass
T2	120 - 100	Horizontal	ROHN 2 STD	34	-2.87	34.21	8.4	Pass
							10.0 (b)	
T3	100 - 80	Horizontal	ROHN 2 STD	73	-4.24	28.55	14.9 ´	Pass
T4	80 - 60	Horizontal	ROHN 2 STD	112	-4.95	23.75	20.8	Pass
T5	60 - 40	Horizontal	ROHN 2 STD	139	-5.04	17.60	28.6	Pass

Section	Elevation ft	Component	Size	Critical Element	P K	øP _{allow} K	% Canacity	Pass Fail
<u>No.</u>		Туре	DOLINIO 5 07D				Capacity	
Т6	40 - 20	Horizontal	ROHN 2.5 STD	163	-5.46	30.30	18.0	Pass
T-7	00 0	11	DOUNIO F OTD	400	F 0F	00.40	19.3 (b)	D
T7	20 - 0	Horizontal	ROHN 2.5 STD	190	-5.85	23.43	25.0	Pass
T1	130 - 120	Top Girt	ROHN 1.5 STD	4	-0.48	23.77	2.0	Pass
T1	130 - 120	Inner Bracing	L2x2x1/8	16	-0.00	8.80	8.0	Pass
T2	120 - 100	Inner Bracing	L2x2x1/8	42	-0.01	6.48	0.9	Pass
Т3	100 - 80	Inner Bracing	L2x2x1/8	79	-0.01	4.43	1.1	Pass
T4	80 - 60	Inner Bracing	L2x2x1/8	120	-0.01	3.34	1.2	Pass
T5	60 - 40	Inner Bracing	L2 1/2x2 1/2x3/16	147	-0.02	6.99	0.9	Pass
T6	40 - 20	Inner Bracing	L 3x3x3/16	174	-0.02	9.16	0.9	Pass
T7	20 - 0	Inner Bracing	L3 1/2x3 /12x1/4	201	-0.02	14.24	8.0	Pass
		· ·					Summary	
						Leg (T6)	41.8	Pass
						Diagonal	39.1	Pass
						(Ť4)		
						Horizontal	28.6	Pass
						(T5)		
						Top Girt	2.0	Pass
						(T1)		
						Inner	1.2	Pass
						Bracing	1.2	1 455
						(T4)		
						Bolt	26.7	Pass
						Checks	20.1	1 055
						RATING =	41.8	Pass
						KATING -	41.0	F455

APPENDIX B BASE LEVEL DRAWING

__ App In Face App Out Face Round ___ __ Flat ___



Black & Veatch Corp. Overland Park, KS 66211 Phone: (913) 458-6909

FAX: (913) 458-8136

Job:	ES-004 SouthMtnRS							
	ject: 405025							
	ent: Eversource	Drawn by: Josh Riley	App'd:					
Cod	de: TIA-222-H	Date: 01/13/22	Scale: NTS					
Pat		ateri South Mrs BS South Mrs BS TSA Update CSC Benutzmitral Antenna Sussi ES-004 South Mrs BS Structural An	Dwg No. E-7					

APPENDIX C ADDITIONAL CALCULATIONS

Eversource #: Site Name: SouthMtnRS Designed By SA Checked By:JR

Date: 1/13/2022



▶ References

ANCHOR ROD ANALYSIS

Project TIAR evision: Information

Site Name: ES-004 SouthMtnsRS

TIA-222-G 105% Allowable?

No Yes

Rev-G

Rev-H

Max Leg Reactions

Apply TIA-222-H Section 15.5?

No Yes

Compression

Uplift Axial C := 138·kip Axial U := 18·kip

Shear C := 19·kip

Shear U := 17·kip

Anchor Rod Data

Diameter of Anchor Rod:

D := 1 · in

Anchor Rod Grade:

Number of Anchor Rods:

Length from top of concrete to

bottom of anchor rod leveling nut:

 $lar := 2.5 \cdot in$

N := 8

Threads in Shear Plane?:

Thread Series:

Coarse Fine 8-Thread

No

Grout Factor η:

0.90 0.70 0.55 0.50

Consider Base Plate Grout? Yes

Threads per Inch: n = 8 (Thread selection invalid if n = 0)

Rod Ultimate Strength: $Fu = 125 \cdot ksi$

Νo

Rod Yield Strength: $Fy = 105 \cdot ksi$

Anchor Rod Plastic

 $Z := \frac{1}{6} \cdot \left(D - \frac{0.9743 \text{ in}}{n} \right)^3 = 0.113 \cdot \text{in}^3$ Section Modulus:

(based on tension root

diameter)

 $r := \left(\frac{1}{4}\right) \cdot \left(D - \frac{0.9743 \text{ in}}{n}\right) = 0.22 \cdot \text{in}$ Radius of Gyration:

Net Area of Anchor Rod:

An := $\frac{\pi}{4} \cdot \left(D - \frac{0.9743 \text{in}}{n} \right)^2 = 0.606 \cdot \text{in}^2$

Nominal Unthreaded

Ab := $\frac{\pi}{4} \cdot (D)^2 = 0.785 \cdot in^2$ Area of Anchor Rod:

F1554-105 A687 A354-BC A354-BD A449 A572-42 A572-50 A572-55 A572-60 A572-65 A588-42 A588-46 A588-50 A36M-42 A36M-45 A36M-50 A36M-55 A500-50 A514-GR100 A53-B-35 A53-B-42 A607-60 A607-65 S-128 S-22

TIA-222-G/H Section 4.9.6.1

Designed By SA Checked By:JR

Date: 1/13/2022



TIA-222-G/H Section 4.9.6.1

Anchor Rod Design Capacities

Design Tension Strength:

 $Rnt := Fu \cdot An = 75.718 \cdot kip$

 $\phi t = 0.75$

 $\phi Rnt := \phi t \cdot Rnt = 56.788 \cdot kip$

Design Compression Strength:

 $Rnc := Fy \cdot An = 63.603 \cdot kip$

 $\phi c = 1$

 $\phi Rnc := \phi c \cdot Rnc = 63.603 \cdot kip$

Design Buckling Strength:

 $K_0 := 1.2$ TIA-222-H Section 4.5.4.2

Fcr = 102.033 · ksi

 $Fe = 1.533 \times 10^3 \cdot ksi$

 $Rnb := Fcr \cdot An = 61.806 \cdot kip$

 $\phi c = 1$

 Φ Rnb := Φ c·Rnb = 61.806·kip

Design Shear Strength: TIA-222-G/H Section 4.9.6.3

Rnv := 0.55·Fu·Ab if Thread_Type = "No" \[\lambda TIA = "Rev-G" \]
0.45·Fu·Ab if Thread_Type = "Yes" \[\lambda TIA = "Rev-G" \]
0.625·Fu·Ab if Thread_Type = "No" \[\lambda TIA = "Rev-H" \]
0.5·Fu·Ab if Thread Type = "Yes" \[\lambda TIA = "Rev-H" \]

 $Rnv = 49.087 \cdot kip$

Rnvc := $0.6 \cdot \text{Fy} \cdot 0.5 \cdot \text{An} = 19.081 \cdot \text{kip}$

TIA-222-H Section 4.9.9

 $\phi v = 0.75$ $\phi c = 1$

 ϕ Rnv := ϕ v·Rnv = 36.816·kip ϕ Rnvc := ϕ c·Rnvc = 19.081·kip

Design Flexural Strength: TIA-222-G/H Section 4.7.1

Rmn := $Fy \cdot Z = 11.853 \cdot kip \cdot in$

 $\phi f = 0.9$

 $\phi Rmn := \phi f \cdot Rmn = 10.668 \cdot kip \cdot in$

Date: 1/13/2022



Anchor Rod Loading Demands

Tension Demand:

Put :=
$$\frac{Axial_U}{N} = 2.25 \cdot kip$$

Compression Demand:

$$Puc := \frac{Axial_C}{N} = 17.25 \cdot kip$$

Shear Demand:

$$Vut := \frac{Shear_U}{N} = 2.125 \cdot kip$$

$$Vuc := \frac{Shear_C}{N} = 2.375 \cdot kip$$

SR g = 0.108

Moment Demand:

$$Mut := 0.65 \cdot lar \cdot Vut = 3.453 \cdot kip \cdot in$$

$$Muc := 0.65 \cdot lar \cdot Vuc = 3.859 \cdot kip \cdot in$$

Anchor Rod Interaction Check

TIA-222-G Section 4.9.9

$$SR_g := \begin{array}{|c|c|} \hline Put + \dfrac{Vut}{\eta} & \text{if } \eta > 0.50 \\ \hline \dfrac{Put + \dfrac{Vut}{\eta}}{\varphi Rnt} & \text{if } \eta = 0.50 \land lar \leq D \land Put > Puc \\ \hline \dfrac{Puc + \dfrac{Vuc}{\eta}}{\varphi Rnt} & \text{if } \eta = 0.50 \land lar \leq D \land Put < Puc \\ \hline \dfrac{\left(\dfrac{Vut}{\varphi Rnv}\right)^2 + \left(\dfrac{Put}{\varphi Rnt} + \dfrac{Mut}{\varphi Rmn}\right)^2}{\varphi Rnt} & \text{if } \eta = 0.5 \land lar > D \land Put > Puc \\ \hline \left(\dfrac{Vuc}{\varphi Rnv}\right)^2 + \left(\dfrac{Puc}{\varphi Rnt} + \dfrac{Muc}{\varphi Rmn}\right)^2 & \text{if } \eta = 0.5 \land lar > D \land Put < Puc \\ \hline \end{array}$$

Date: 1/13/2022



Anchor Rod Interaction Check

TIA-222-H Section 4.9.9

$$SR_{Pt} := \left[\left(\frac{Put}{\varphi Rnt} \right)^{2} + \left(\frac{Vut}{\varphi Rnv} \right)^{2} \text{ if } lar \leq D \right]$$

$$\left(\frac{Put}{\varphi Rnt} \right)^{2} + \left(\frac{Vut}{\varphi Rnv} \right)^{2} \text{ if } D < lar \leq 3 \cdot in \land Grout = "Yes"}$$

$$\left(\frac{Put}{\varphi Rnt} + \frac{Mut}{\varphi Rmn} \right)^{2} + \left(\frac{Vut}{\varphi Rnv} \right)^{2} \text{ if } 3 \cdot in < lar \land Grout = "Yes"}$$

$$\left(\frac{Put}{\varphi Rnt} + \frac{Mut}{\varphi Rmn} \right)^{2} + \left(\frac{Vut}{\varphi Rnv} \right)^{2} \text{ if } D < lar \land Grout = "No"}$$

$$SR_{Pt} = 4.901 \times 10^{-3}$$

$$\begin{split} SR_Pc := & \left(\frac{Puc}{\varphi Rnc} \right) + \left(\frac{Vuc}{\varphi Rnvc} \right)^2 \quad \text{if } lar \leq D \\ & \left(\frac{Puc}{\varphi Rnc} \right) + \left(\frac{Vuc}{\varphi Rnvc} \right)^2 \quad \text{if } D < lar \leq 3 \cdot \text{in} \wedge \text{Grout} = "Yes" \\ & \left(\frac{Puc}{\varphi Rnc} + \frac{Muc}{\varphi Rmn} \right) + \left(\frac{Vuc}{\varphi Rnvc} \right)^2 \quad \text{if } 3 \cdot \text{in} < lar \wedge \text{Grout} = "Yes" \\ & \left(\frac{Puc}{\varphi Rnc} + \frac{Muc}{\varphi Rmn} \right) + \left(\frac{Vuc}{\varphi Rnvc} \right)^2 \quad \text{if } D < lar \leq 4 \cdot D \wedge \text{Grout} = "No" \\ & \left(\frac{Puc}{\varphi Rnb} + \frac{Muc}{\varphi Rmn} \right) + \left(\frac{Vuc}{\varphi Rnvc} \right)^2 \quad \text{if } lar > 4 \cdot D \wedge \text{Grout} = "No" \end{split}$$

$$SR Pc = 0.287$$

$$SR := \begin{cases} SR_g & \text{if TIA} = "Rev-G" \\ max(SR_Pt, SR_Pc) & \text{if TIA} = "Rev-H" \land S15 = "No" \\ \hline \frac{max(SR_Pt, SR_Pc)}{1.05} & \text{if TIA} = "Rev-H" \land S15 = "Yes" \end{cases}$$

Eversource #: Site Name: SouthMtnRS Designed By SA Checked By:JR

Date: 1/13/2022



Anchor Rod Results

Axial Tension Demand: Put = $2.25 \cdot \text{kip}$

Axial Tension Capacity: $\phi Rnt = 56.788 \cdot kip$

Axial Compression Demand: $Puc = 17.25 \cdot kip$

Axial Compression Capacity: $\phi Rnc = 63.603 \cdot kip$

Shear Tension Demand: $Vut = 2.125 \cdot kip$

Tension Shear Capacity: $\phi Rnv = 36.816 \cdot kip$

Shear Compression Demand: $Vuc = 2.375 \cdot kip$

Compresison Shear Capacity: $\phi R_{nvc} = 19.081 \cdot kip$

Moment Tension Demand: $M_{ut} = 3.453 \cdot kip \cdot in$

Moment Compression Demand: $M_{uc} = 3.859 \cdot kip \cdot in$

Moment Capacity: $\phi R_{mn} = 10.668 \cdot kip \cdot in$

Governing Stress Ratio

 $SR = 27.305 \cdot \%$

Check_{SR} = "Passing"

SST Unit Base Foundation

ES-004 SouthMtnsRS

TIA-222 Revision:

Top & Bot. Pad Rein. Different?:	
Tower Centroid Offset?:	
Block Foundation?:	~

Superstructure Analysis Reactions					
Global Moment, M :	2542	ft-kips			
Global Axial, P :	39	kips			
Global Shear, V :	33	kips			
Leg Compression, P _{comp} :	138	kips			
Leg Comp. Shear, V_{u_comp} :	19	kips			
Leg Uplift, P_{uplift} :	118	kips			
Leg Uplift. Shear, V u_uplift:	17	kips			
Tower Height, H :	130	ft			
Base Face Width, BW :	22.5417	ft			
BP Dist. Above Fdn, bp _{dist} :	2	in			
Anchor Bolt Circle, BC :	10	in			

Foundation Analysis Checks					
	Capacity	Demand	Rating*	Check	
Lateral (Sliding) (kips)	206.03	33.00	15.3%	Pass	
Bearing Pressure (ksf)	9.00	1.14	12.1%	Pass	
Overturning (kip*ft)	7965.51	2679.50	33.6%	Pass	
Pad Flexure (kip*ft)	1877.46	547.49	27.8%	Pass	
Pad Shear - 1-way (kips)	1541.78	75.55	4.7%	Pass	
Pad Shear - Comp 2-way (ksi)	0.190	0.018	9.2%	Pass	
Flexural 2-way (Comp) (kip*ft)	978.81	0.00	0.0%	Pass	
Pad Shear - Tension 2-way (ksi)	0.190	0.016	7.9%	Pass	
Flexural 2-way (Tension) (kip*ft)	978.81	0.00	0.0%	Pass	

*Rating per TIA-222-H Section 15.5

Soil Rating*:	33.6%
Structural Rating*:	27.8%

Pad Properties					
Depth, D :	4.00	ft			
Pad Width, W :	31.00	ft			
Pad Thickness, T:	4.00	ft			
Pad Rebar Size (Bottom), Sp:	7				
Pad Rebar Quantity (Bottom), mp:	16				
Pad Clear Cover, cc _{pad} :	3	in			

Material Properties					
Rebar Grade, Fy :	60	ksi			
Concrete Compressive Strength, F'c:	4	ksi			
Dry Concrete Density, δ c :	150	pcf			

Soil Properties						
Total Soil Unit Weight, γ :	100	pcf				
Ultimate Gross Bearing, Qult:	12.000	ksf				
Cohesion, Cu :	0.000	ksf				
Friction Angle, $oldsymbol{arphi}$:	34	degrees				
SPT Blow Count, N _{blows} :	10					
Base Friction, μ :	0.45					
Neglected Depth, N:	3.3	ft				
Foundation Bearing on Rock?	No					
Groundwater Depth, gw :	N/A	ft				

<-- Toggle between Gross and Net



Date: 02Mar22 Wgt: 2.00 LBS

SHIPPING: SPECIAL: HANDLING: 100.00 TOTAL:

0.00 0.00 0.00 0.00

Dep:

Date: 02Mar22 Wgt: 5.00 LBS

RIGIN ID:EFBA (203) 562-9885 J.IPPING IOSEPH MERRITT CO. , HAMILTON STREET

HAVEN, CT 065115920 NITED STATES US

SHIPPING: SPECIAL: HANDLING: 200.00 TOTAL:

0.0 0.0 0.0 0.0

Svcs: PRIORITY OVERNIGHT TRCK: 6437 3911 1991

ORIGIN ID:EFBA (203) 562-9885 SHIPPING JOSEPH MERRITT CO. OW HAMILTON STREET

.!EW HAVEN, CT 065115920 UNITED STATES US

SHIP DATE: 02MAR22 ACTWGT: 2.00 LB MAN CAD: 0517347/CAFE3509

BILL THIRD PARTY

ROBERT M. FLANAGAN CITY OF BRISTOL NORTH MAIN STREET

BRISTOL CT 06010

PO: S0424674



FedEx Express 1211828121181

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Date: 02Mar22 Wgt: 2.00 LBS

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SHIP DATE: 02MAR22 ACTWGT: 5.00 LB MAN CAD: 0517347/CAFE3509

BILL THIRD PARTY

CONNECTICUT SITING COUNCIL FRANKLIN SQUARE

Sves: PRIORITY OVERNIGHT TRCK: 6437 3911 2005

NEW BRITAIN CT 06051

PO: S0424674



FedE:

10:30 03 PRIORITY OVERNIGH

BDL

06051 CT-US BDL

Part # 156148-434 RITZ 01/1/4 ...



CAGGIANO, 3911 1980 100.00 STREET JEFFREY Svcs: PRIORITY TRCK: 6437 562-9885 NORTH MAIN NEW HAVEN, CT 065115920 UNITED STATES US 2 (203) ORIGIN ID: EFBA (20 HIPPING JOSEPH MERRITT CO. 60 HAMILTON STREET HONORABLE

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PO: S0424674

06010

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RISTOL







ATTACHMENT E - POWER DENSITY REPORT



Calculated Radio Frequency Emissions Report



ES-004

790 Willis Street

Bristol, CT 06010

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1. Introduction

The purpose of this report is to investigate compliance with applicable FCC regulations for the Eversource installation on the existing self-support tower located at 790 Willis Street in Bristol, CT.

Eversource has recently installed one omnidirectional antenna (Comprod 876F-70-2HSMP40DF1/2) for both transmit and receive purposes as part of its 220 MHz communications system and one microwave dish for backhaul communications. The original proposal also consisted of one omnidirectional antenna and one microwave dish, however the model of the original omni-directional antenna (db Spectra DS20C03F36D-D) has changed. This report provides an updated analysis based on the current installation as reflected in the updated plans ¹.

This report considers the existing antenna configuration as detailed by Eversource along with power density information of the other existing antennas to calculate the cumulative % MPE (Maximum Permissible Exposure) of the facility at ground level.

2. FCC Guidelines for Evaluating RF Radiation Exposure Limits

In 1985, the FCC established rules to regulate radio frequency (RF) exposure from FCC licensed antenna facilities. In 1996, the FCC updated these rules, which were further amended in August 1997 by OET Bulletin 65 Edition 97-01. These new rules include Maximum Permissible Exposure (MPE) limits for transmitters operating between 300 kHz and 100 GHz. The FCC MPE limits are based upon those recommended by the National Council on Radiation Protection and Measurements (NCRP), developed by the Institute of Electrical and Electronics Engineers, Inc., (IEEE) and adopted by the American National Standards Institute (ANSI).

The FCC general population/uncontrolled limits set the maximum exposure to which most people may be subjected. General population/uncontrolled exposures apply in situations in which the general public may be exposed, or in which persons that are exposed as a consequence of their employment may not be fully aware of the potential for exposure or cannot exercise control over their exposure.

Public exposure to radio frequencies is regulated and enforced in units of milliwatts per square centimeter (mW/cm²). The general population exposure limits for the various frequency ranges are defined in the attached "FCC Limits for Maximum Permissible Exposure (MPE)" in Attachment B of this report.

Higher exposure limits are permitted under the occupational/controlled exposure category, but only for persons who are exposed as a consequence of their employment and who have been made fully aware of the potential for exposure, and they must be able to exercise control over their exposure. General population/uncontrolled limits are five times more stringent than the levels that are acceptable for occupational, or radio frequency trained individuals. Attachment B contains excerpts from OET Bulletin 65 and defines the Maximum Exposure Limit.

Finally, it should be noted that the MPE limits adopted by the FCC for both general population/uncontrolled exposure and for occupational/controlled exposure incorporate a substantial margin of safety and have been established to be well below levels generally accepted as having the potential to cause adverse health effects.

¹ Stamped Black & Veatch site drawings dated 1/19/2022 (Rev. 1).



3. Power Density Calculation Methods

The power density calculation results were generated using the following formula as outlined in FCC bulletin OET 65, and Connecticut Siting Council recommendations:

Power Density =
$$\left(\frac{1.6^2 \times 1.64 \times ERP}{4\pi \times R^2}\right)$$
 X Off Beam Loss

Where:

EIRP = Effective Isotropic Radiated Power = 1.64 x ERP

R = Radial Distance = $\sqrt{(H^2 + V^2)}$

H = Horizontal Distance from antenna

V = Vertical Distance from radiation center of antenna

Ground reflection factor of 1.6

Off Beam Loss is determined by the selected antenna pattern

These calculations assume that the antennas are operating at 100 percent capacity and full power, and that all antenna channels are transmitting simultaneously. Obstructions (trees, buildings, etc.) that would normally attenuate the signal are not taken into account. The calculations assume even terrain in the area of study and do not consider actual terrain elevations which could attenuate the signal. As a result, the calculated power density and corresponding % MPE levels reported below are much higher than the actual levels will be from the final installation.

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4. Calculated % MPE Results

Table 1 below outlines the power density information for the site. The Eversource omnidirectional and microwave antennas have narrow vertical beamwidths of 40° and 1.7°, respectively; therefore, the majority of the RF power is focused out towards the horizon. Please refer to Attachment C, for the vertical patterns of the recently installed Eversource antennas. Likewise, the other transmit antennas exhibit similar directionality of varying vertical beamwidths. As a result, there will be less RF power directed below the antennas relative to the horizon, and consequently lower power density levels around the base of the facility. The calculated results in Table 1 include a nominal 10 dB off-beam pattern loss for the omnidirectional and panel antennas, and 30 dB off-beam pattern loss for the highly directional microwave dish to account for the lower relative gain below the antennas. Any inactive or receive-only antennas are not listed in the table unless specified otherwise, as they are irrelevant in terms of the % MPE calculations.

	Antenna	Operating	Number	ERP Per	Power		
Carrier	Height	Frequency	of Trans.	Transmitter	Density	Limit	% MPE
	(Feet)	(MHz)	or rrais.	(Watts)	(mw/cm ²)		
Eversource	141.5	154.46375	1	250	0.0005	0.2000	0.24%
Eversource	141	153.695	1	250	0.0005	0.2000	0.25%
Eversource	139	145.14	1	250	0.0005	0.2000	0.25%
Eversource	138	224.22	1	250	0.0005	0.2000	0.26%
Eversource	137	451.1	1	250	0.0005	0.3007	0.17%
Eversource	135	939.4375	1	250	0.0005	0.6263	0.09%
Eversource	134	939.95	1	250	0.0005	0.6266	0.09%
Eversource	117	6034.15	1	7000	0.0002	1.0000	0.02%
Eversource	120	47.76	1	250	0.0007	0.2000	0.35%
Eversource	107	6735	1	7000	0.0002	1.0000	0.02%
Eversource	111	37.76	1	250	0.0008	0.2000	0.41%
Eversource	98	174	1	250	0.0011	0.2000	0.53%
Eversource	96	6805	1	7000	0.0003	1.0000	0.03%
Eversource	86	6004.5	1	7000	0.0004	1.0000	0.04%
Eversource	91	37.46	1	250	0.0012	0.2000	0.62%
Eversource	84	900	1	250	0.0015	0.6000	0.25%
Eversource	71	6805	1	7000	0.0006	1.0000	0.06%
Eversource	73	146.52	1	250	0.0020	0.2000	1.00%
Eversource	73	448.375	1	250	0.0020	0.2989	0.67%
Eversource	58	48.34	1	250	0.0033	0.2000	1.66%
Eversource	54	48.4	1	250	0.0039	0.2000	1.95%
Eversource	46	173.25	1	250	0.0056	0.2000	2.81%
Eversource	43	37.6	1	250	0.0066	0.2000	3.28%
CSP	78	851	1	315	0.0022	0.5673	0.39%
CSP	67	775	1	250	0.0024	0.5167	0.47%
CSP	53	775	1	199	0.0032	0.5167	0.63%
CSP	40	851.0125	1	158	0.0049	0.5673	0.87%
T-Mobile	125	2500	1	3590	0.0091	1.0000	0.91%
T-Mobile	125	2500	1	3590	0.0091	1.0000	0.91%
T-Mobile	125	700	1	2256	0.0057	0.4667	1.23%
T-Mobile	125	600	1	1128	0.0029	0.4000	0.72%
T-Mobile	125	600	1	1128	0.0029	0.4000	0.72%
T-Mobile	125	1900	1	3166	0.0080	1.0000	0.80%
T-Mobile	125	2100	1	4308	0.0109	1.0000	1.09%
T-Mobile	125	1900	1	2034	0.0052	1.0000	0.52%
T-Mobile	125	1900	1	4034	0.0102	1.0000	1.02%
Eversource	87	5945.2	1	11147	0.0006	1.0000	0.06%
Eversource	135	217	4	124	0.0011	0.2000	0.54%
						Total	25.92%

Table 1: Proposed Facility % MPE ^{2 3}

² The operating parameters for the Eversource and CSP (CT State Police) were provided directly by each respective operator. The T-Mobile information was sourced from the CSC Power Density Database dated 07/16/2021. For reference, this data is included as Attachment D. Please note that % MPE values listed are rounded to two decimal points and the total % MPE listed is a summation of each unrounded contribution. Therefore, summing each rounded value may not identically match the total value reflected in the table.

³ Antenna heights listed for Eversource are in reference to the antenna centerline and based upon information provided by Eversource with respect to the Black & Veatch Structural Analysis Report dated 01/13/2022. The available data for the CSP antennas do not have a one-toone match with the structural analysis, so a worst-case antenna height was applied for those antennas.



5. Conclusion

The above analysis concludes that RF exposure at ground level with the new Eversource 220 MHz and microwave antenna installation is below the maximum power density limits as outlined by the FCC in the OET Bulletin 65 Ed. 97-01. Using the conservative calculation methods discussed herein, the highest expected percent of Maximum Permissible Exposure at ground level with the proposed installation is 25.92% of the FCC General Population/Uncontrolled limit.

As noted previously, the calculated % MPE levels are more conservative (higher) than the actual levels will be from the finished installation.

6. Statement of Certification

I certify to the best of my knowledge that the statements in this report are true and accurate. The calculations follow guidelines set forth in FCC OET Bulletin 65 Edition 97-01, IEEE Std. C95.1, and IEEE Std. C95.3.

Schail Umani

Report Prepared By: Sohail Usmani

Senior RF Engineer

C Squared Systems, LLC

Keith Willante

Reviewed/Approved By: Keith Vellante

Director – RF Services C Squared Systems, LLC February 7, 2022

Date

February 8, 2022

Date



Attachment A: References

OET Bulletin 65 - Edition 97-01 - August 1997 Federal Communications Commission Office of Engineering & Technology

<u>IEEE C95.1-2005, IEEE Standard Safety Levels With Respect to Human Exposure to Radio Frequency Electromagnetic Fields, 3 kHz to 300 GHz</u> IEEE-SA Standards Board

IEEE C95.3-2002 (R2008), IEEE Recommended Practice for Measurements and Computations of Radio Frequency Electromagnetic Fields With Respect to Human Exposure to Such Fields, 100 kHz-300 GHz IEEE-SA Standards Board

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Attachment B: FCC Limits for Maximum Permissible Exposure (MPE)

(A) Limits for Occupational/Controlled Exposure⁴

Frequency Range (MHz)	Electric Field Strength (E) (V/m)	Magnetic Field Strength (E) (A/m)	Power Density (S) (mW/cm ²)	Averaging Time E ² , H ² or S (minutes)
0.3-3.0	614	1.63	(100)*	6
3.0-30	1842/f	4.89/f	$(900/f^2)*$	6
30-300	61.4	0.163	1.0	6
300-1500	-	-	f/300	6
1500-100,000	-	-	5	6

(B) Limits for General Population/Uncontrolled Exposure⁵

Frequency Range (MHz)	Electric Field Strength (E) (V/m)	Magnetic Field Strength (E) (A/m)	Power Density (S) (mW/cm ²)	Averaging Time $ E ^2$, $ H ^2$ or S (minutes)
0.3-1.34	614	1.63	(100)*	30
1.34-30	824/f	2.19/f	$(180/f^2)*$	30
30-300	27.5	0.073	0.2	30
300-1500	-	-	f/1500	30
1500-100,000	-	-	1.0	30

f = frequency in MHz * Plane-wave equivalent power density

Table 2: FCC Limits for Maximum Permissible Exposure (MPE)

4

⁴ Occupational/controlled limits apply in situations in which persons are exposed as a consequence of their employment provided those persons are fully aware of the potential for exposure and can exercise control over their exposure. Limits for occupational/controlled exposure also apply in situations when an individual is transient through a location where occupational/controlled limits apply provided he or she is made aware of the potential for exposure

⁵ General population/uncontrolled exposures apply in situations in which the general public may be exposed, or in which persons that are exposed as a consequence of their employment may not be fully aware of the potential for exposure or cannot exercise control over their exposure



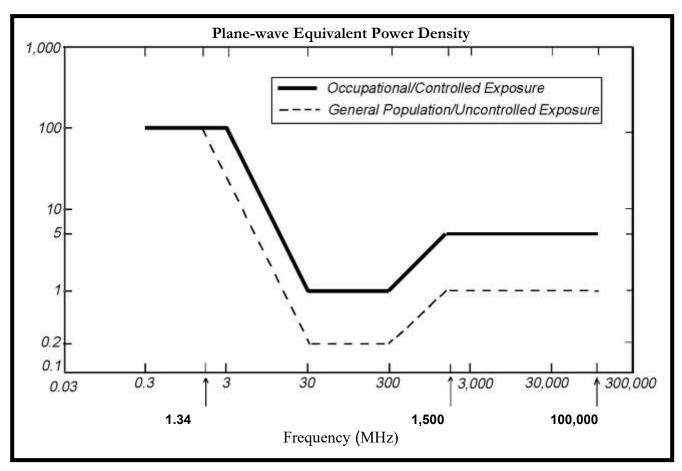


Figure 1: Graph of FCC Limits for Maximum Permissible Exposure (MPE)



Attachment C: Eversource Antenna Data Sheets and Electrical Patterns

217 MHz

Manufacturer: COMPROD

876F-70-2HSMP40DF1/2 Model #:

Frequency Band: 215-225 MHz

Gain: 5.0 dBd

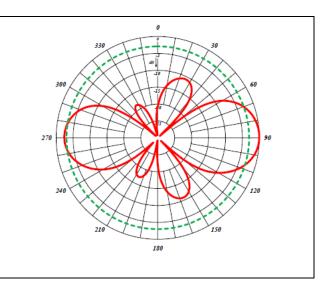
Vertical ~ 40° Beamwidth:

Horizontal

360° Beamwidth:

Polarization: Vertical

> 20.0 Length:



5945.2 MHz

Manufacturer: RFS

Model #: PAD6-59

Frequency Band: 5.925-6.875 MHz

Gain: 38.4 dBi

Vertical

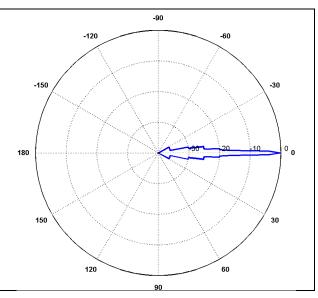
1.7° Beamwidth:

Horizontal 1.7°

Beamwidth:

Polarization: Single

Diameter: 6'





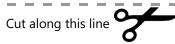
Attachment D: Current CSC Power Density Data for the Subject Facility (07/16/2021)

Carrier	Antenna Height (Feet)	Operating Frequency (MHz)	Number of Trans.	ERP Per Transmitt er (Watts)	Power Density (mw/cm²)	Limit	% MPE
Amateur Radio	126	448.325	1	650	0.00162	0.2989	0.54%
Amateur Radio	126	224.22	1	100	0.00025	0.2000	0.12%
CL&P	127	153.695	1	5	0.00001	0.2000	0.01%
CL&P	127	451.1	1	189	0.00046	0.3007	0.15%
CL&P	127	154.46375	1	990	0.00243	0.2000	1.22%
CL&P	122	952.55625	1	71	0.00019	0.6350	0.03%
CL&P	125	937	3	200	0.00152	0.6247	0.24%
CL&P	115	48.34	1	100	0.00030	0.2000	0.15%
CL&P	109	6765	1	5250	0.01780	1.0000	1.78%
CL&P	102	6835	1	309	0.00121	1.0000	0.12%
CL&P	102	6735	1	1738	0.00678	1.0000	0.68%
CTSP	108	800	5	100	0.00173	0.5333	0.32%
CL&P	92	6805	1	1660	0.00807	1.0000	0.81%
CL&P	81	6865	1	288	0.00184	1.0000	0.18%
CL&P	81	37.76	1	100	0.00064	0.2000	0.32%
CL&P	58	48.4	1	120	0.00160	0.2000	0.80%
CL&P	53	53.05	1	100	0.00163	0.2000	0.81%
CL&P	52	37.46	1	115	0.00195	0.2000	0.98%
CL&P	38	37.6	1	446	0.01566	0.2000	7.83%
CL&P	37	173.25	1	204	0.00763	0.2000	3.82%
CL&P	37	928.55625	1	32	0.00120	0.6190	0.19%
T-Mobile	125	2500	1	3590	0.0091	1.0000	0.91%
T-Mobile	125	2500	1	3590	0.0091	1.0000	0.91%
T-Mobile	125	700	1	2256	0.0057	0.4667	1.23%
T-Mobile	125	600	1	1128	0.0029	0.4000	0.72%
T-Mobile	125	600	1	1128	0.0029	0.4000	0.72%
T-Mobile	125	1900	1	3166	0.0080	1.0000	0.80%
T-Mobile	125	2100	1	4308	0.0109	1.0000	1.09%
T-Mobile	125	1900	1	2034	0.0052	1.0000	0.52%
T-Mobile	125	1900	1	4034	0.0102	1.0000	1.02%
						Total	29.04%

■"21E0952022 #15tt01tt72# ■"6tEE00"



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